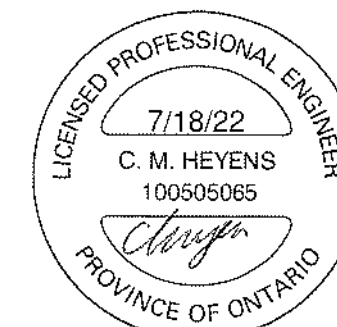


Products				
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	20
J1 DJ	18-00-00	11 7/8" NI-40x	2	10
J2	16-00-00	11 7/8" NI-40x	1	26
J3	14-00-00	11 7/8" NI-40x	1	7
J4	12-00-00	11 7/8" NI-40x	1	1
J5	8-00-00	11 7/8" NI-40x	1	9
J6	6-00-00	11 7/8" NI-40x	1	6
J7	4-00-00	11 7/8" NI-40x	1	6
J8	2-00-00	11 7/8" NI-40x	1	4
B1	18-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B4	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B3	10-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B2	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1

Connector Summary		
Qty	Manuf	Product
16	H1	IUS2.56/11.88
12	H1	IUS2.56/11.88
4	H1	IUS2.56/11.88
2	H4	HUS1.81/10

DWG# TF22071107 TO TF22071110



STRUCTURAL COMPONENTS ONLY
DWG# TF22071123

THIS IS A FLOOR COMPONENT PLACEMENT PLAN ONLY.

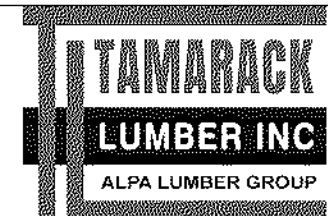
The wood beams and joists outlined on this plan are designed as individual building components to be incorporated into the design of the building at the specification of the building designer. Please see the individual beam reports, joist reports, and/or joist span tables for each component identified on this placement plan.

The supporting structure is to be specified by the building designer prior to the installation of joist(s) and/or beam(s). The building designer is responsible for the bracing of the floor system and its integration into the bracing of the overall structure. All components labelled "by others" or "as per plan", and all steel beams, are not within the scope of work of this seal.

The building designer must review and approve this plan to ascertain conformity to the overall structural plan of the building. All dimensions to be verified on site.

DATE: 7/15/22

1st FLOOR FRAMING
STANDARD



FROM PLAN DATED: 2022/01/11

BUILDER: ROYAL PINE HOMES

SITE: FORESTSIDE ESTATES

MODEL: 3804

ELEVATION: A

LOT:

CITY: BRAMPTON

SALESMAN: Rick DiCiano

DESIGNER: PL

REVISION:

REFER TO THE NORDIC INSTALLATION GUIDE
FOR PROPER STORAGE AND INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 SPF #2 REQ'D
UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER
CONCENTRATED LOADS. SEE FIGURE 1.

CANTILEVERED JOISTS INCLUDING CANT' OVER
BRICK REQ. 1-JOIST BLOCKING ALONG BEARING
AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES
4/5 FOR REINFORCEMENT REQUIREMENTS.

FOR HOLES INCLUDING DUCT CHASE AND FIELD
CUT OPENINGS SEE FIGURE 6 AND TABLES 6.1/6.2.
CERAMIC TILE APPLICATION AS PER OBC 9.30.6.

ALL CONNECTORS MUST BE INSTALLED AS PER THE
MANUFACTURER'S SPECIFICATIONS USING THE
MANUFACTURER SPECIFIED FASTENERS.

ALL BEAM HANGER FASTENERS INSTALLED INTO
THE SUPPORTING MEMBER MUST BE A MINIMUM
OF 3.5" IN LENGTH UNLESS OTHERWISE SPECIFIED
BY THE SUPPORTING MEMBER ENGINEER OF RECORD.

LOADING:

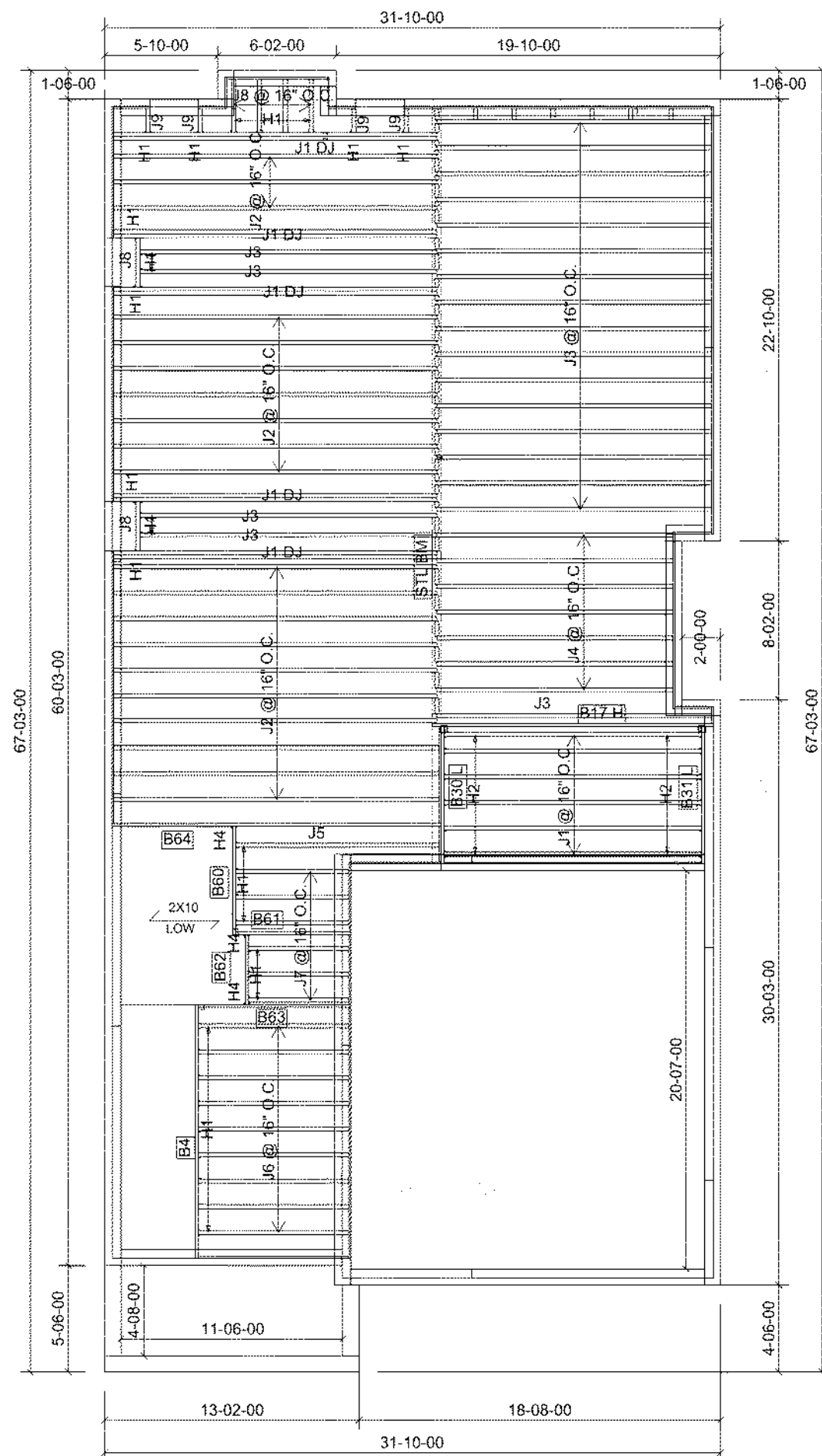
LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 lb/ft²

TILE LOAD: +5.0 lb/ft²

JOIST LL DEFLECTION LIMIT: L/480

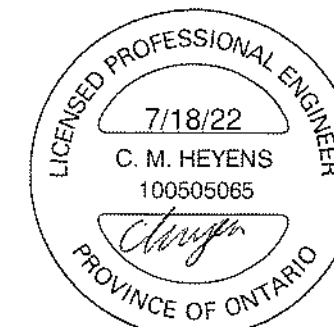
SUBFLOOR: 3/4" GLUED AND NAILED



Products				
PlotID	Length	Product	Plies	Net Qty
J1	14-00-00	9 1/2" NI-40x	1	6
J2	18-00-00	11 7/8" NI-40x	1	20
J1 DJ	18-00-00	11 7/8" NI-40x	2	10
J3	16-00-00	11 7/8" NI-40x	1	21
J4	14-00-00	11 7/8" NI-40x	1	7
J5	12-00-00	11 7/8" NI-40x	1	1
J6	8-00-00	11 7/8" NI-40x	1	9
J7	6-00-00	11 7/8" NI-40x	1	6
J8	4-00-00	11 7/8" NI-40x	1	6
J9	2-00-00	11 7/8" NI-40x	1	4
B30 L	8-00-00	1 3/4" x 9 1/2" (2.0E 3100) WestFraser LVL	1	1
B31 L	8-00-00	1 3/4" x 9 1/2" (2.0E 3100) WestFraser LVL	1	1
B64	18-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B17 H	16-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B4	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B61	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B63	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B60	6-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B62	4-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1

Connector Summary		
Qty	Manuf	Product
16	H1	IUS2.56/11.88
12	H1	IUS2.56/11.88
4	H1	IUS2.56/11.88
12	H2	IUS2.56/9.5
3	H4	HUS1.81/10

DWG# TF22071110 TO TF22071118



STRUCTURAL COMPONENTS ONLY
DWG# TF22071124

THIS IS A FLOOR COMPONENT PLACEMENT PLAN ONLY.

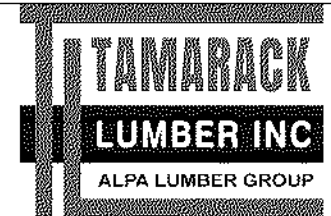
The wood beams and joists outlined on this plan are designed as individual building components to be incorporated into the design of the building at the specification of the building designer. Please see the individual beam reports, joist reports, and/or joist span tables for each component identified on this placement plan.

The supporting structure is to be specified by the building designer prior to the installation of joist(s) and/or beam(s). The building designer is responsible for the bracing of the floor system and its integration into the bracing of the overall structure. All components labelled "by others" or "as per plan", and all steel beams, are not within the scope of work of this seal.

The building designer must review and approve this plan to ascertain conformity to the overall structural plan of the building. All dimensions to be verified on site.

DATE: 7/15/22

**1st FLOOR FRAMING
SUNKEN**



FROM PLAN DATED: 2022/01/11

BUILDER: ROYAL PINE HOMES

SITE: FORESTSIDE ESTATES

MODEL: 3804

ELEVATION: A

LOT:

CITY: BRAMPTON

SALESMAN: Rick DiCiano

DESIGNER: PL

REVISION:

REFER TO THE NORDIC INSTALLATION GUIDE
FOR PROPER STORAGE AND INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 SPF #2 REQ'D
UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER
CONCENTRATED LOADS. SEE FIGURE 1.

CANTILEVERED JOISTS INCLUDING **CANT' OVER**
BRICK REQ. 1-JOIST BLOCKING ALONG BEARING
AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES
4/5 FOR REINFORCEMENT REQUIREMENTS.

FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD**
CUT OPENINGS SEE FIGURE 6 AND TABLES 6.1/6.2.
CERAMIC TILE APPLICATION AS PER OBC 9.30.6.

ALL **CONNECTORS** MUST BE INSTALLED AS PER THE
MANUFACTURER'S SPECIFICATIONS USING THE
MANUFACTURER SPECIFIED FASTENERS.

ALL **BEAM HANGER FASTENERS** INSTALLED INTO
THE **SUPPORTING MEMBER** MUST BE A MINIMUM
OF **3.5"** IN LENGTH UNLESS OTHERWISE SPECIFIED
BY THE SUPPORTING MEMBER ENGINEER OF RECORD.

LOADING:

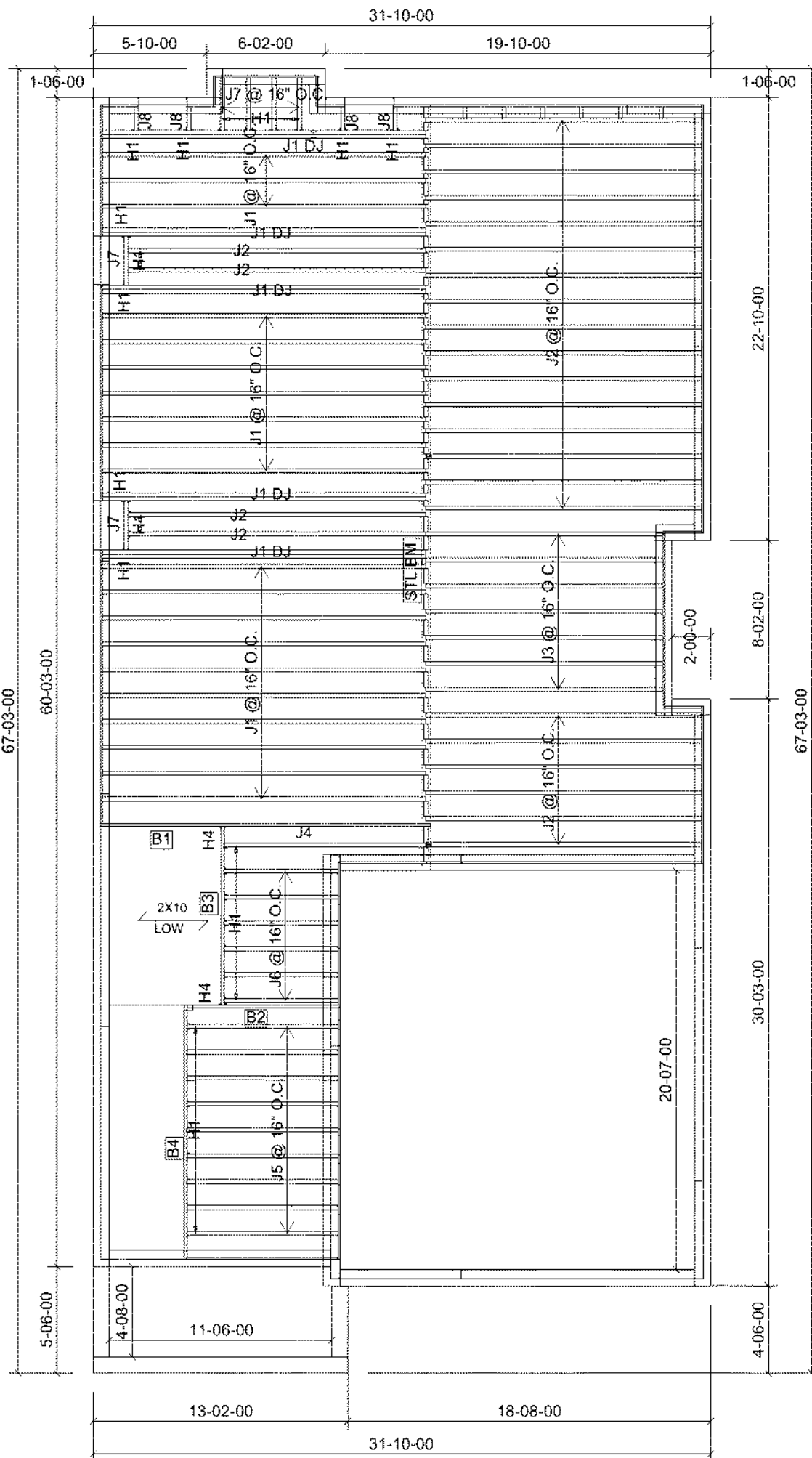
LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 lb/ft²

TILE LOAD: +5.0 lb/ft²

JOIST LL DEFLECTION LIMIT: L/480

SUBFLOOR: 3/4" GLUED AND NAILED



Products				
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	20
J1 DJ	18-00-00	11 7/8" NI-40x	2	10
J2	16-00-00	11 7/8" NI-40x	1	26
J3	14-00-00	11 7/8" NI-40x	1	7
J4	12-00-00	11 7/8" NI-40x	1	1
J5	8-00-00	11 7/8" NI-40x	1	9
J6	6-00-00	11 7/8" NI-40x	1	6
J7	4-00-00	11 7/8" NI-40x	1	6
J8	2-00-00	11 7/8" NI-40x	1	4
B1	18-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B4	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B3	10-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B2	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1

Connector Summary		
Qty	Manuf	Product
16	H1	IUS2.56/11.88
12	H1	IUS2.56/11.88
4	H1	IUS2.56/11.88
2	H4	HUS1.81/10

DWG# TF22071107 TO TF22071110



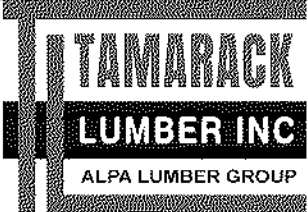
STRUCTURAL COMPONENTS ONLY
DWG# TF22071125

**THIS IS A FLOOR COMPONENT
PLACEMENT PLAN ONLY.**

The wood beams and joists outlined on this plan are designed as individual building components to be incorporated into the design of the building at the specification of the building designer. Please see the individual beam reports, joist reports, and/or joist span tables for each component identified on this placement plan. The supporting structure is to be specified by the building designer prior to the installation of joist(s) and/or beam(s). The building designer is responsible for the bracing of the floor system and its integration into the bracing of the overall structure. All components labelled "by others" or "as per plan", and all steel beams, are not within the scope of work of this seal. The building designer must review and approve this plan to ascertain conformity to the overall structural plan of the building. All dimensions to be verified on site.

DATE: 7/15/22

**1st FLOOR FRAMING
STANDARD**



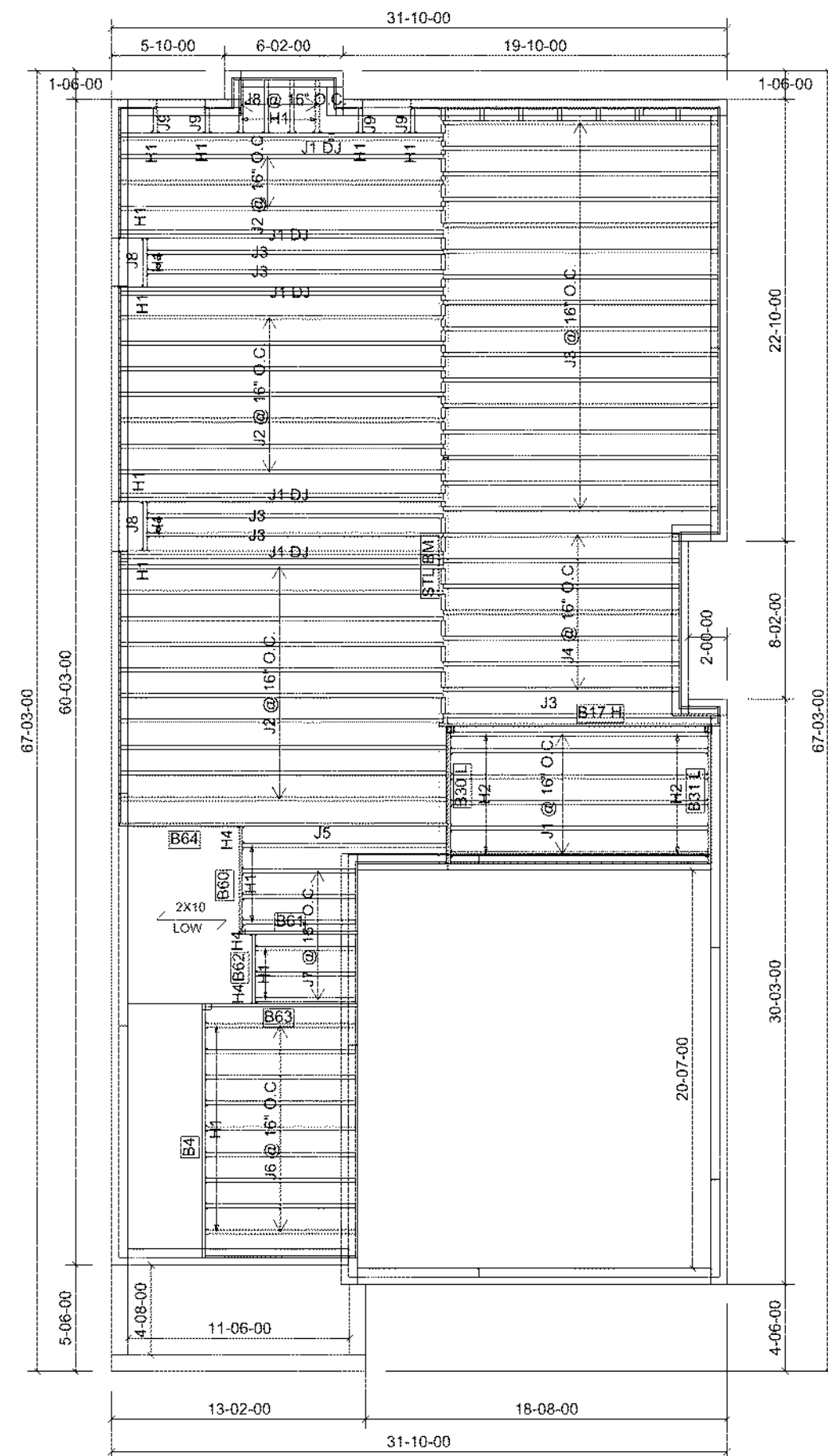
FROM PLAN DATED: 2022/01/11
BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
ELEVATION: B
LOT:
CITY: BRAMPTON
SALESMAN: Rick DiCiano
DESIGNER: PL
REVISION:

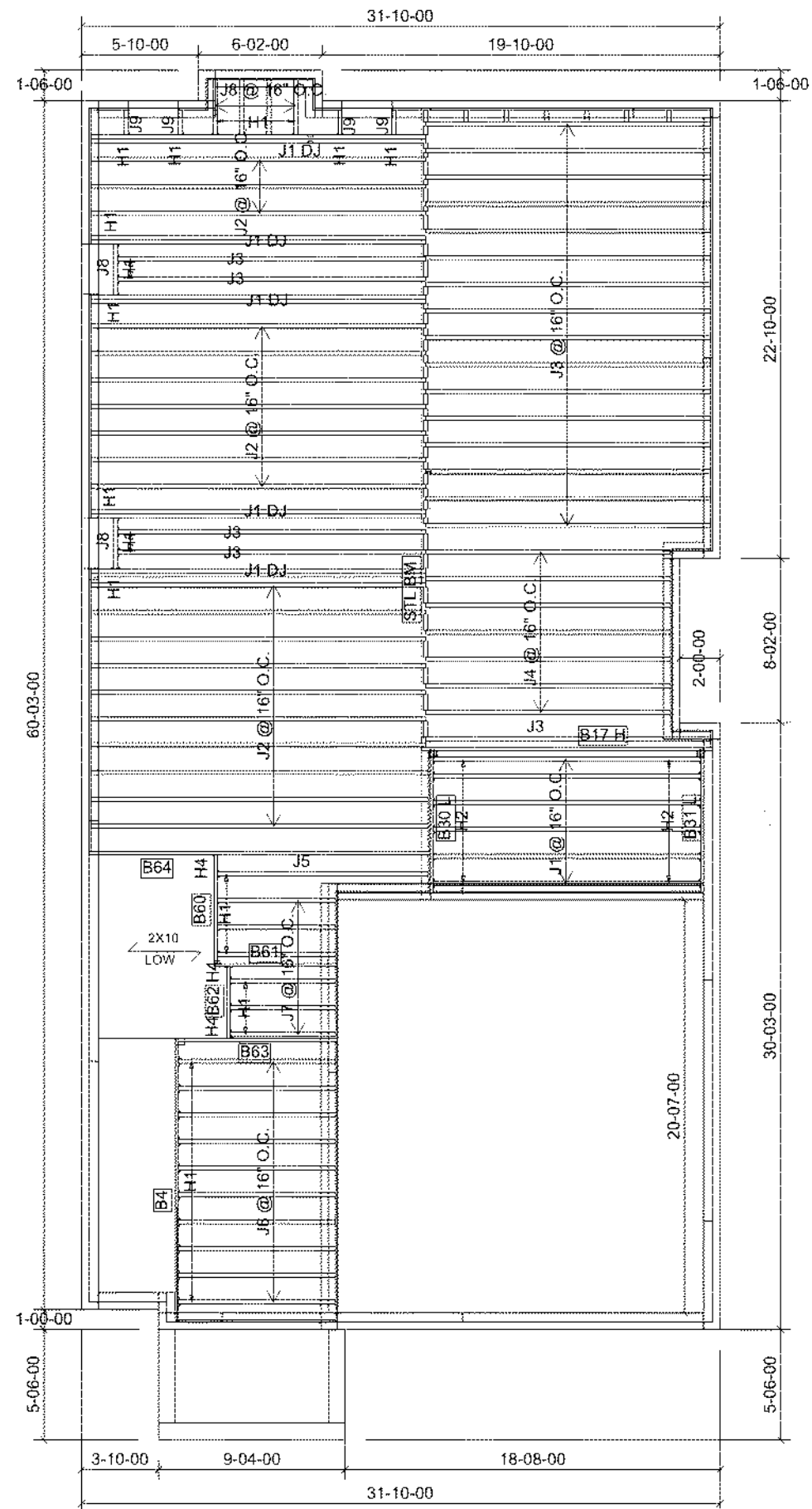
REFER TO THE **NORDIC INSTALLATION GUIDE** FOR PROPER STORAGE AND INSTALLATION.
SQUASH BLOCKS OF 2x4, 2x6, 2x8 SPF #2 REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. **MULTIPLE SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1.
CANTILEVERED JOISTS INCLUDING **CANT' OVER BRICK** REQ. 1-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4/5 FOR REINFORCEMENT REQUIREMENTS.
FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD CUT OPENINGS** SEE FIGURE 6 AND TABLES 6.1/6.2.
CERAMIC TILE APPLICATION AS PER OBC 9.30.6.

ALL **CONNECTORS** MUST BE INSTALLED AS PER THE **MANUFACTURER'S SPECIFICATIONS** USING THE **MANUFACTURER SPECIFIED FASTENERS**.
ALL **BEAM HANGER FASTENERS** INSTALLED INTO THE **SUPPORTING MEMBER** MUST BE A MINIMUM OF **3.5"** IN LENGTH UNLESS OTHERWISE SPECIFIED BY THE **SUPPORTING MEMBER ENGINEER OF RECORD**.

LOADING:
LIVE LOAD: 40.0 lb/ft²
DEAD LOAD: 15.0 lb/ft²
TILE LOAD: +5.0 lb/ft²

JOIST LL DEFLECTION LIMIT: L/480
SUBFLOOR: 3/4" GLUED AND NAILED





Products				
PlotID	Length	Product	Plies	Net Qty
J1	14-00-00	9 1/2" NI-40x	1	6
J2	18-00-00	11 7/8" NI-40x	1	20
J1 DJ	18-00-00	11 7/8" NI-40x	2	10
J3	16-00-00	11 7/8" NI-40x	1	21
J4	14-00-00	11 7/8" NI-40x	1	7
J5	12-00-00	11 7/8" NI-40x	1	1
J6	8-00-00	11 7/8" NI-40x	1	10
J7	6-00-00	11 7/8" NI-40x	1	6
J8	4-00-00	11 7/8" NI-40x	1	6
J9	2-00-00	11 7/8" NI-40x	1	4
B30 L	8-00-00	1 3/4" x 9 1/2" (2.0E 3100) WestFraser LVL	1	1
B31 L	8-00-00	1 3/4" x 9 1/2" (2.0E 3100) WestFraser LVL	1	1
B64	18-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B17 H	16-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B4	16-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B61	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B63	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B60	6-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B62	4-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1

Connector Summary		
Qty	Manuf	Product
17	H1	IUS2.56/11.88
12	H1	IUS2.56/11.88
4	H1	IUS2.56/11.88
12	H2	IUS2.56/9.5
3	H4	HUS1.81/10

DWG# TF22071110 TO TF22071118



STRUCTURAL COMPONENTS ONLY
DWG# TF22071128

THIS IS A FLOOR COMPONENT PLACEMENT PLAN ONLY.

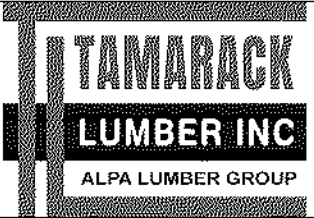
The wood beams and joists outlined on this plan are designed as individual building components to be incorporated into the design of the building at the specification of the building designer. Please see the individual beam reports, joist reports, and/or joist span tables for each component identified on this placement plan.

The supporting structure is to be specified by the building designer prior to the installation of joist(s) and/or beam(s). The building designer is responsible for the bracing of the floor system and its integration into the bracing of the overall structure. All components labelled "by others" or "as per plan", and all steel beams, are not within the scope of work of this seal.

The building designer must review and approve this plan to ascertain conformity to the overall structural plan of the building. All dimensions to be verified on site.

DATE: 7/15/22

1st FLOOR FRAMING
SUNKEN



FROM PLAN DATED: 2022/01/11

BUILDER: ROYAL PINE HOMES

SITE: FORESTSIDE ESTATES

MODEL: 3804

ELEVATION: C

LOT:

CITY: BRAMPTON

SALESMAN: Rick DiCiano

DESIGNER: PL

REVISION:

REFER TO THE NORDIC INSTALLATION GUIDE
FOR PROPER STORAGE AND INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 SPF #2 REQ'D
UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER
CONCENTRATED LOADS. SEE FIGURE 1.

CANTILEVERED JOISTS INCLUDING CANT' OVER
BRICK REQ. 1-JOIST BLOCKING ALONG BEARING
AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES
4/5 FOR REINFORCEMENT REQUIREMENTS.

FOR HOLES INCLUDING DUCT CHASE AND FIELD
CUT OPENINGS SEE FIGURE 6 AND TABLES 6.1/6.2.
CERAMIC TILE APPLICATION AS PER OBC 9.30.6.

ALL CONNECTORS MUST BE INSTALLED AS PER THE
MANUFACTURER'S SPECIFICATIONS USING THE
MANUFACTURER SPECIFIED FASTENERS.

ALL BEAM HANGER FASTENERS INSTALLED INTO
THE SUPPORTING MEMBER MUST BE A MINIMUM
OF 3.5" IN LENGTH UNLESS OTHERWISE SPECIFIED
BY THE SUPPORTING MEMBER ENGINEER OF RECORD.

LOADING:

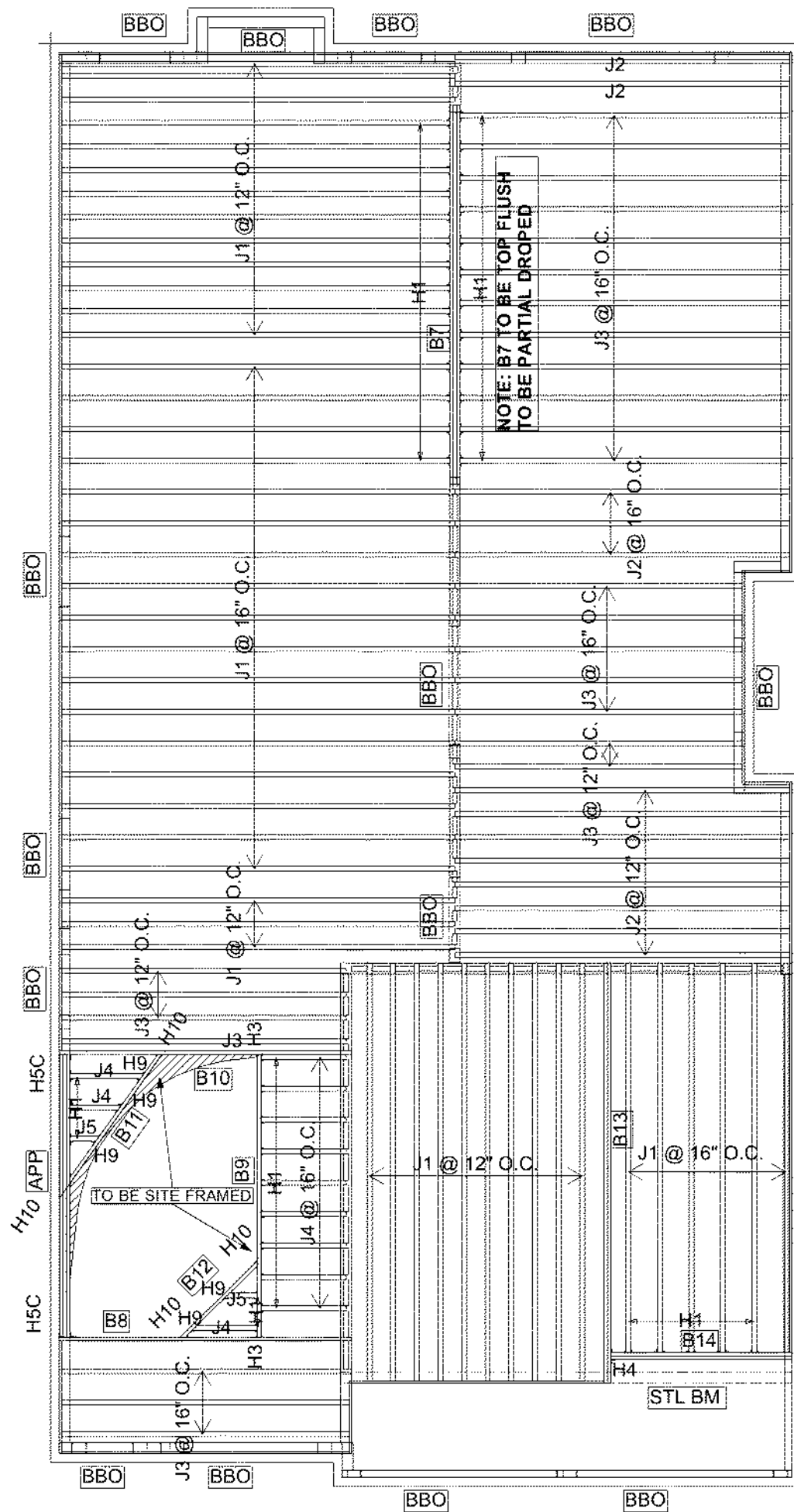
LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 lb/ft²

TILE LOAD: +5.0 lb/ft²

JOIST LL DEFLECTION LIMIT: L/480

SUBFLOOR: 3/4" GLUED AND NAILED



Products				
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	49
J2	16-00-00	11 7/8" NI-40x	1	13
J3	14-00-00	11 7/8" NI-40x	1	26
J4	4-00-00	11 7/8" NI-40x	1	12
J5	2-00-00	11 7/8" NI-40x	1	2
B13	18-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B10	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B8	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B9	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
APP	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	3	3
B11	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B14	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B12	6-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B7	17-00-00	1 3/4" x 14" (2.0E 3100) WestFraser LVL	3	3

Connector Summary		
Qty	Manuf	Product
11	H1	IUS2.56/11.88
5	H1	IUS2.56/11.88
3	H1	IUS2.56/11.88
26	H1	IUS2.56/11.88
1	H3	HGUS410
1	H3	HUS1.81/10
1	H4	HGUS410
2	H5C	HUC610
3	H9	LSSR2.56Z
2	H9	LSSR2.56Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z

DWG# TF22071099 TO TF22071106



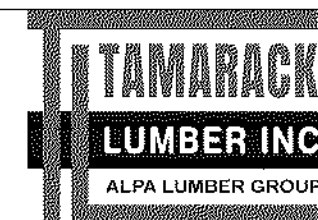
STRUCTURAL COMPONENTS ONLY
DWG# TF22071129

THIS IS A FLOOR COMPONENT PLACEMENT PLAN ONLY.

The wood beams and joists outlined on this plan are designed as individual building components to be incorporated into the design of the building at the specification of the building designer. Please see the individual beam reports, joist reports, and/or joist span tables for each component identified on this placement plan.
The supporting structure is to be specified by the building designer prior to the installation of joist(s) and/or beam(s). The building designer is responsible for the bracing of the floor system and its integration into the bracing of the overall structure. All components labelled "by others" or "as per plan", and all steel beams, are not within the scope of work of this seal.
The building designer must review and approve this plan to ascertain conformity to the overall structural plan of the building. All dimensions to be verified on site.

DATE: 7/15/22

**2nd FLOOR FRAMING
STANDARD**



FROM PLAN DATED: 2022/01/11

BUILDER: ROYAL PINE HOMES

SITE: FORESTSIDE ESTATES

MODEL: 3804

ELEVATION: A

LOT:

CITY: BRAMPTON

SALESMAN: Rick DiCiano

DESIGNER: PL

REVISION:

REFER TO THE **NORDIC INSTALLATION GUIDE**
FOR PROPER STORAGE AND INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 SPF #2 REQ'D
UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER
CONCENTRATED LOADS. SEE FIGURE 1.

CANTILEVERED JOISTS INCLUDING **CANT' OVER**
BRICK REQ. 1-JOIST BLOCKING ALONG BEARING
AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES
4/5 FOR REINFORCEMENT REQUIREMENTS.

FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD**
CUT OPENINGS SEE FIGURE 6 AND TABLES 6.1/6.2.
CERAMIC TILE APPLICATION AS PER OBC 9.30.6.

ALL **CONNECTORS** MUST BE INSTALLED AS PER THE
MANUFACTURER'S SPECIFICATIONS USING THE
MANUFACTURER SPECIFIED FASTENERS.

ALL **BEAM HANGER FASTENERS** INSTALLED INTO
THE **SUPPORTING MEMBER** MUST BE A MINIMUM
OF **3.5"** IN LENGTH UNLESS OTHERWISE SPECIFIED
BY THE SUPPORTING MEMBER ENGINEER OF RECORD.

LOADING:

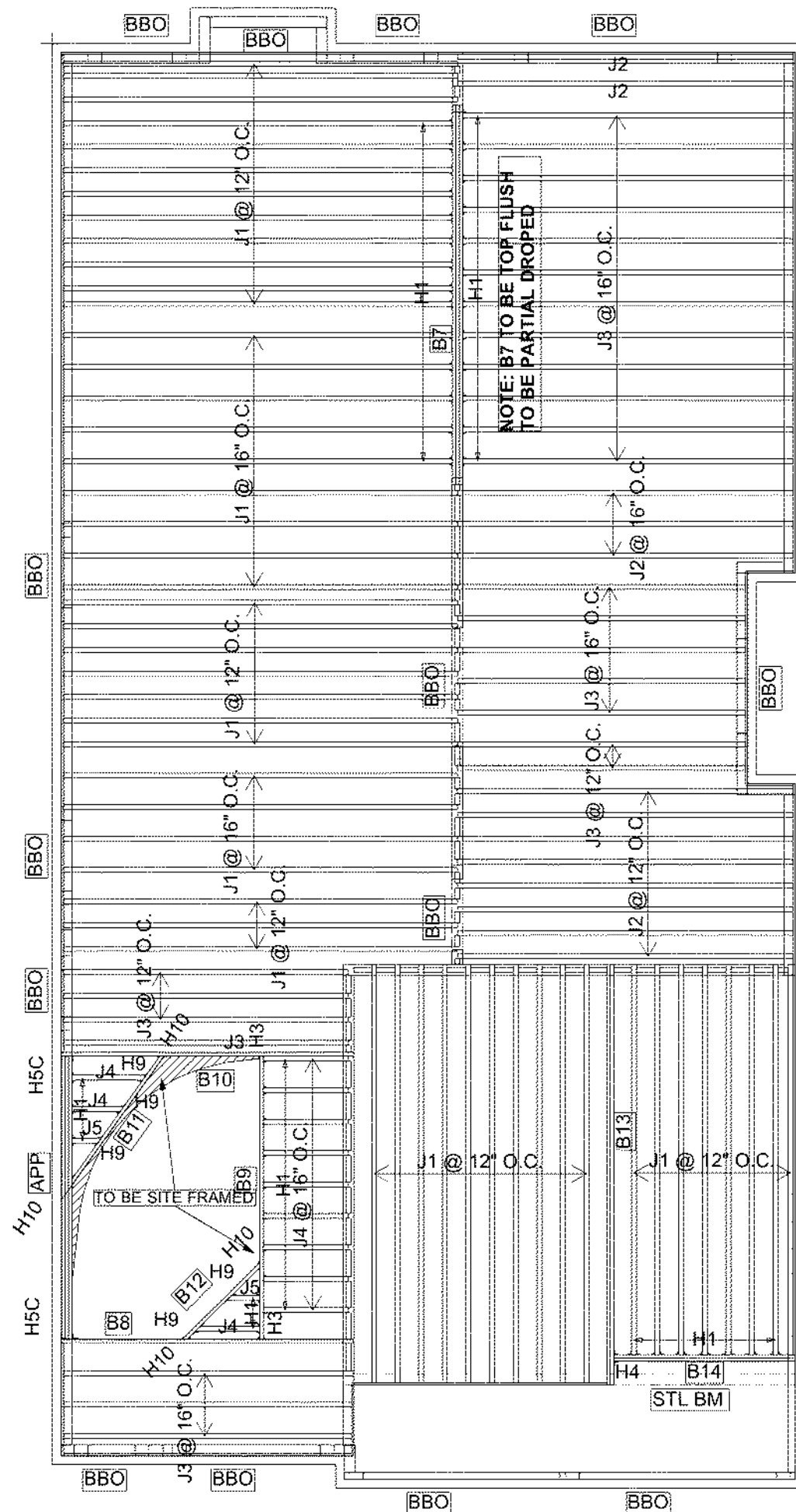
LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 lb/ft²

TILE LOAD: +5.0 lb/ft²

JOIST LL DEFLECTION LIMIT: L/480

SUBFLOOR: 5/8" GLUED AND NAILED



Products				
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	53
J2	16-00-00	11 7/8" NI-40x	1	13
J3	14-00-00	11 7/8" NI-40x	1	26
J4	4-00-00	11 7/8" NI-40x	1	12
J5	2-00-00	11 7/8" NI-40x	1	2
B13	18-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B10	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B8	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B9	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
APP	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	3	3
B11	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B14	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B12	6-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B7	18-00-00	1 3/4" x 14" (2.0E 3100) WestFraser LVL	3	3

Connector Summary		
Qty	Manuf	Product
11	H1	IUS2.56/11.88
7	H1	IUS2.56/11.88
3	H1	IUS2.56/11.88
26	H1	IUS2.56/11.88
1	H3	HGUS410
1	H3	HUS1.81/10
1	H4	HGUS410
2	H3C	HUC610
3	H9	LSSR2.56Z
2	H9	LSSR2.56Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z

DWG# TF22071099 TO TF22071106



STRUCTURAL COMPONENTS ONLY
DWG# TF22071130

THIS IS A FLOOR COMPONENT PLACEMENT PLAN ONLY.

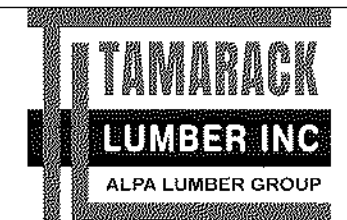
The wood beams and joists outlined on this plan are designed as individual building components to be incorporated into the design of the building at the specification of the building designer. Please see the individual beam reports, joist reports, and/or joist span tables for each component identified on this placement plan.

The supporting structure is to be specified by the building designer prior to the installation of joist(s) and/or beam(s). The building designer is responsible for the bracing of the floor system and its integration into the bracing of the overall structure. All components labelled "by others" or "as per plan", and all steel beams, are not within the scope of work of this seal.

The building designer must review and approve this plan to ascertain conformity to the overall structural plan of the building. All dimensions to be verified on site.

DATE: 7/15/22

**2nd FLOOR FRAMING
OPT 5 BED**



FROM PLAN DATED: 2022/01/11

BUILDER: ROYAL PINE HOMES

SITE: FORESTSIDE ESTATES

MODEL: 3804

ELEVATION: A

LOT:

CITY: BRAMPTON

SALESMAN: Rick DiCiano

DESIGNER: PL

REVISION:

REFER TO THE **NORDIC INSTALLATION GUIDE**
FOR PROPER STORAGE AND INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 SPF #2 REQ'D
UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER
CONCENTRATED LOADS. SEE FIGURE 1.

CANTILEVERED JOISTS INCLUDING **CANT' OVER**
BRICK REQ. 1-JOIST BLOCKING ALONG BEARING
AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES
4/5 FOR REINFORCEMENT REQUIREMENTS.

FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD**
CUT OPENINGS SEE FIGURE 6 AND TABLES 6.1/6.2.
CERAMIC TILE APPLICATION AS PER OBC 9.30.6.

ALL **CONNECTORS** MUST BE INSTALLED AS PER THE
MANUFACTURER'S SPECIFICATIONS USING THE
MANUFACTURER SPECIFIED FASTENERS.

ALL **BEAM HANGER FASTENERS** INSTALLED INTO
THE **SUPPORTING MEMBER** MUST BE A MINIMUM
OF **3.5"** IN LENGTH UNLESS OTHERWISE SPECIFIED
BY THE SUPPORTING MEMBER ENGINEER OF RECORD.

LOADING:

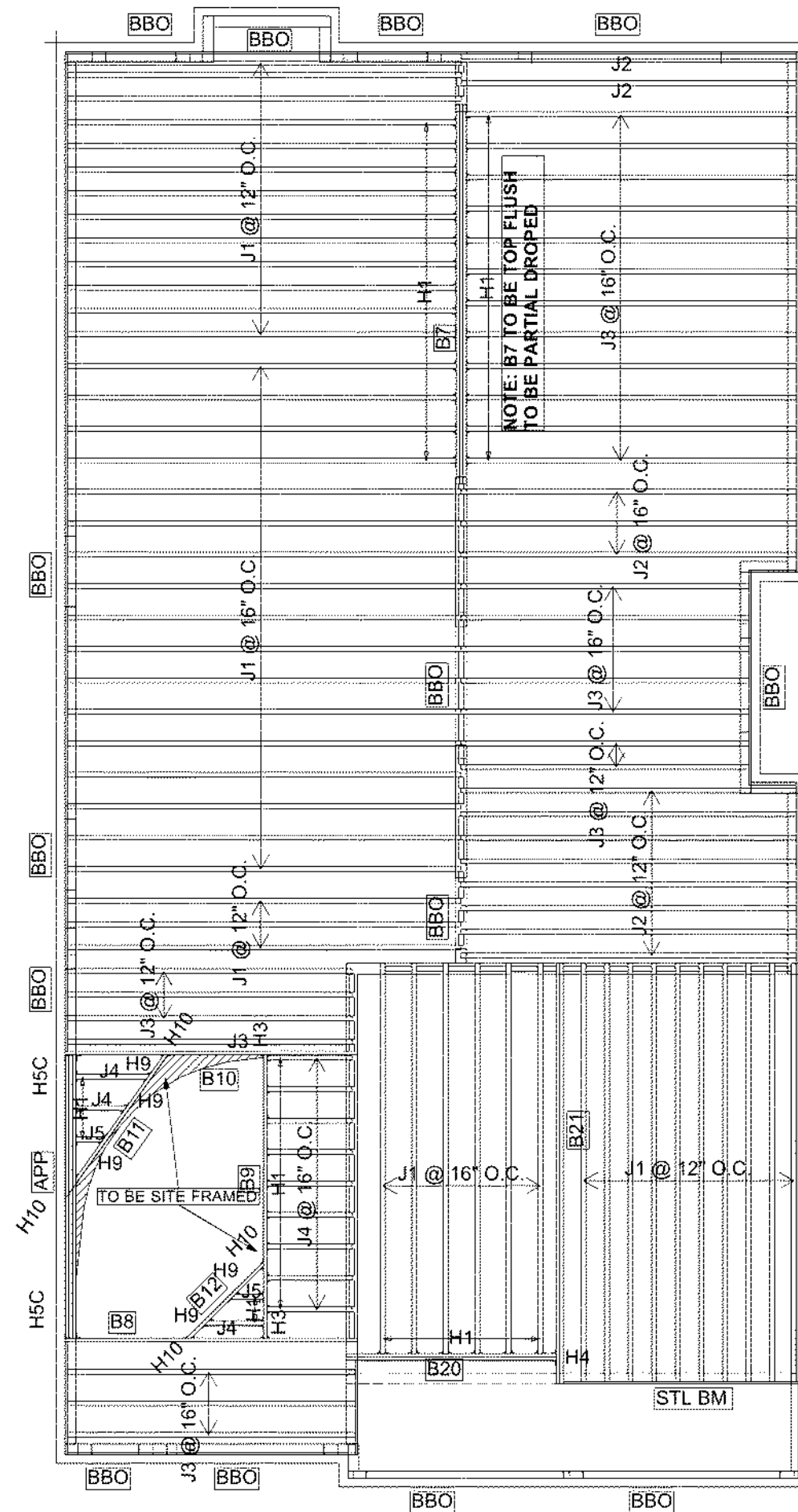
LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 lb/ft²

TILE LOAD: +5.0 lb/ft²

JOIST LL DEFLECTION LIMIT: L/480

SUBFLOOR: 5/8" GLUED AND NAILED



Products				
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	49
J2	16-00-00	11 7/8" NI-40x	1	13
J3	14-00-00	11 7/8" NI-40x	1	26
J4	4-00-00	11 7/8" NI-40x	1	12
J5	2-00-00	11 7/8" NI-40x	1	2
B21	18-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B10	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B8	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B9	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
APP	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	3	3
B20	10-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B11	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B12	6-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B7	17-00-00	1 3/4" x 14" (2.0E 3100) WestFraser LVL	3	3

Connector Summary		
Qty	Manuf	Product
11	H1	IUS2.56/11.88
6	H1	IUS2.56/11.88
3	H1	IUS2.56/11.88
26	H1	IUS2.56/11.88
1	H3	HGUS410
1	H3	HUS1.81/10
1	H4	HGUS410
2	H5C	HUC610
3	H9	LSSR2.56Z
2	H9	LSSR2.56Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z

DWG# TF22071099 TO TF22071104
DWG# TF22071119 TO TF22071120



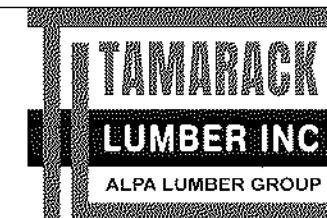
STRUCTURAL COMPONENTS ONLY
DWG# TF22071131

THIS IS A FLOOR COMPONENT PLACEMENT PLAN ONLY.

The wood beams and joists outlined on this plan are designed as individual building components to be incorporated into the design of the building at the specification of the building designer. Please see the individual beam reports, joist reports, and/or joist span tables for each component identified on this placement plan. The supporting structure is to be specified by the building designer prior to the installation of joist(s) and/or beam(s). The building designer is responsible for the bracing of the floor system and its integration into the bracing of the overall structure. All components labelled "by others" or "as per plan", and all steel beams, are not within the scope of work of this seal. The building designer must review and approve this plan to ascertain conformity to the overall structural plan of the building. All dimensions to be verified on site.

DATE: 7/15/22

**2nd FLOOR FRAMING
STANDARD**



FROM PLAN DATED: 2022/01/11

BUILDER: ROYAL PINE HOMES

SITE: FORESTSIDE ESTATES

MODEL: 3804

ELEVATION: B

LOT:

CITY: BRAMPTON

SALESMAN: Rick DiCiano

DESIGNER: PL

REVISION:

REFER TO THE **NORDIC INSTALLATION GUIDE**
FOR PROPER STORAGE AND INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 SPF #2 REQ'D
UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER
CONCENTRATED LOADS. SEE FIGURE 1.

CANTILEVERED JOISTS INCLUDING **CANT' OVER**
BRICK REQ. 1-JOIST BLOCKING ALONG BEARING
AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES
4/5 FOR REINFORCEMENT REQUIREMENTS.

FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD**
CUT OPENINGS SEE FIGURE 6 AND TABLES 6.1/6.2.
CERAMIC TILE APPLICATION AS PER OBC 9.30.6.

ALL **CONNECTORS** MUST BE INSTALLED AS PER THE
MANUFACTURER'S SPECIFICATIONS USING THE
MANUFACTURER SPECIFIED FASTENERS.

ALL **BEAM HANGER FASTENERS** INSTALLED INTO
THE **SUPPORTING MEMBER** MUST BE A MINIMUM
OF **3.5"** IN LENGTH UNLESS OTHERWISE SPECIFIED
BY THE SUPPORTING MEMBER ENGINEER OF RECORD.

LOADING:

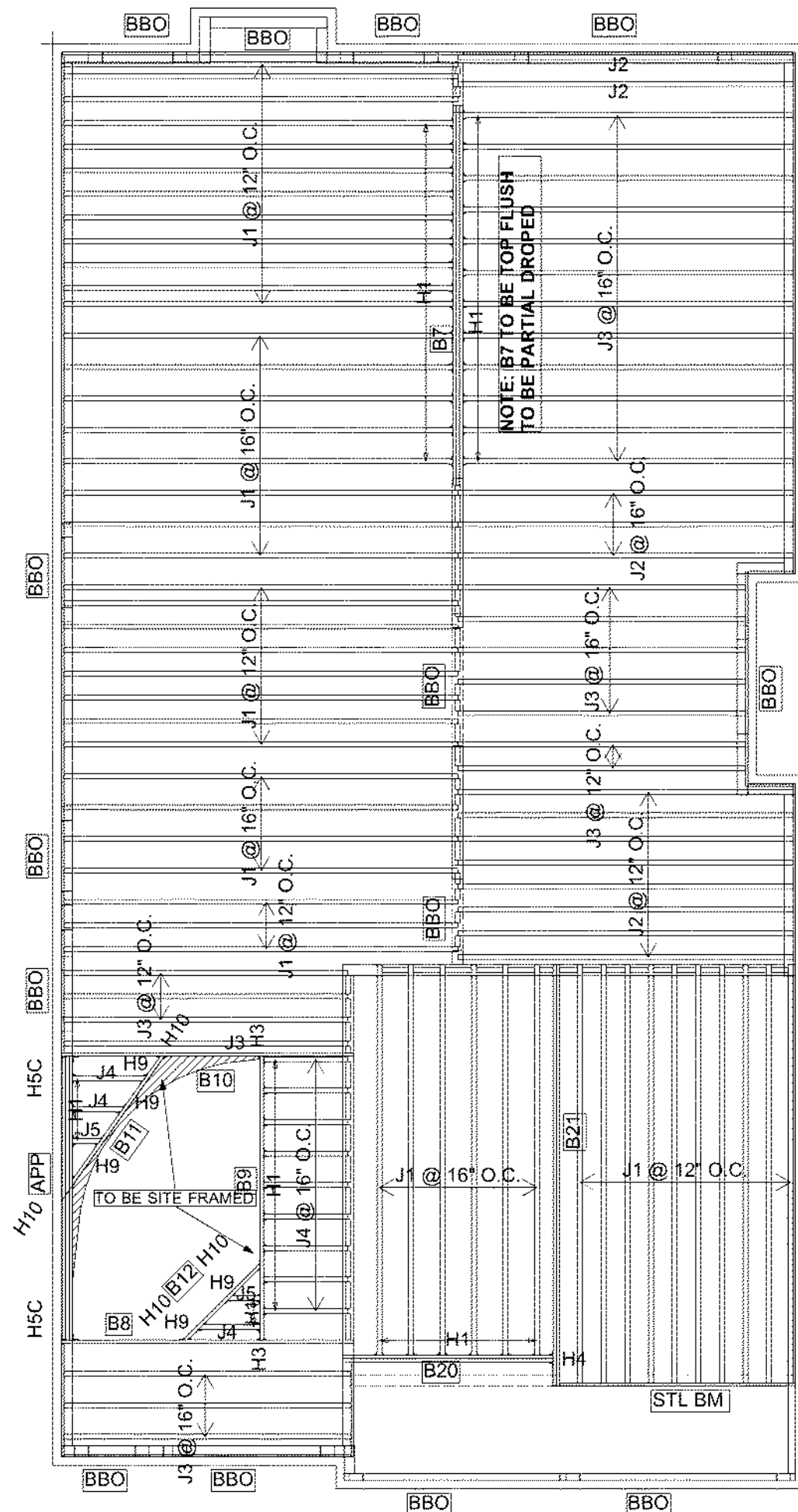
LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 lb/ft²

TILE LOAD: +5.0 lb/ft²

JOIST LL DEFLECTION LIMIT: L/480

SUBFLOOR: 5/8" GLUED AND NAILED



Products				
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	51
J2	16-00-00	11 7/8" NI-40x	1	13
J3	14-00-00	11 7/8" NI-40x	1	26
J4	4-00-00	11 7/8" NI-40x	1	12
J5	2-00-00	11 7/8" NI-40x	1	2
B21	18-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B10	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B8	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B9	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
APP	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	3	3
B20	10-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B11	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B12	6-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B7	17-00-00	1 3/4" x 14" (2.0E 3100) WestFraser LVL	3	3

Connector Summary		
Qty	Manuf	Product
11	H1	IUS2.56/11.88
6	H1	IUS2.56/11.88
3	H1	IUS2.56/11.88
26	H1	IUS2.56/11.88
1	H3	HGUS1.81/10
1	H3	HGUS410
1	H4	HGUS410
2	H5C	HUC610
3	H9	LSSR2.56Z
2	H9	LSSR2.56Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z

DWG# TF22071099 TO TF22071104
DWG# TF22071119 TO TF22071120



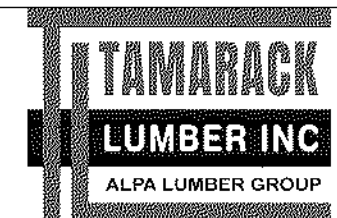
STRUCTURAL COMPONENTS ONLY
DWG# TF22071132

THIS IS A FLOOR COMPONENT PLACEMENT PLAN ONLY.

The wood beams and joists outlined on this plan are designed as individual building components to be incorporated into the design of the building at the specification of the building designer. Please see the individual beam reports, joist reports, and/or joist span tables for each component identified on this placement plan.
The supporting structure is to be specified by the building designer prior to the installation of joist(s) and/or beam(s). The building designer is responsible for the bracing of the floor system and its integration into the bracing of the overall structure. All components labelled "by others" or "as per plan", and all steel beams, are not within the scope of work of this seal.
The building designer must review and approve this plan to ascertain conformity to the overall structural plan of the building. All dimensions to be verified on site.

DATE: 7/15/22

**2nd FLOOR FRAMING
OPT 5 BED**



FROM PLAN DATED: 2022/01/11

BUILDER: ROYAL PINE HOMES

SITE: FORESTSIDE ESTATES

MODEL: 3804

ELEVATION: B

LOT:

CITY: BRAMPTON

SALESMAN: Rick DiCiano

DESIGNER: PL

REVISION:

REFER TO THE **NORDIC INSTALLATION GUIDE**
FOR PROPER STORAGE AND INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 SPF #2 REQ'D
UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER
CONCENTRATED LOADS. SEE FIGURE 1.

CANTILEVERED JOISTS INCLUDING **CANT' OVER**
BRICK REQ. 1-JOIST BLOCKING ALONG BEARING
AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES
4/5 FOR REINFORCEMENT REQUIREMENTS.

FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD**
CUT OPENINGS SEE FIGURE 6 AND TABLES 6.1/6.2.
CERAMIC TILE APPLICATION AS PER OBC 9.30.6.

ALL **CONNECTORS** MUST BE INSTALLED AS PER THE
MANUFACTURER'S SPECIFICATIONS USING THE
MANUFACTURER SPECIFIED FASTENERS.

ALL **BEAM HANGER FASTENERS** INSTALLED INTO
THE **SUPPORTING MEMBER** MUST BE A MINIMUM
OF **3.5"** IN LENGTH UNLESS OTHERWISE SPECIFIED
BY THE SUPPORTING MEMBER ENGINEER OF RECORD.

LOADING:

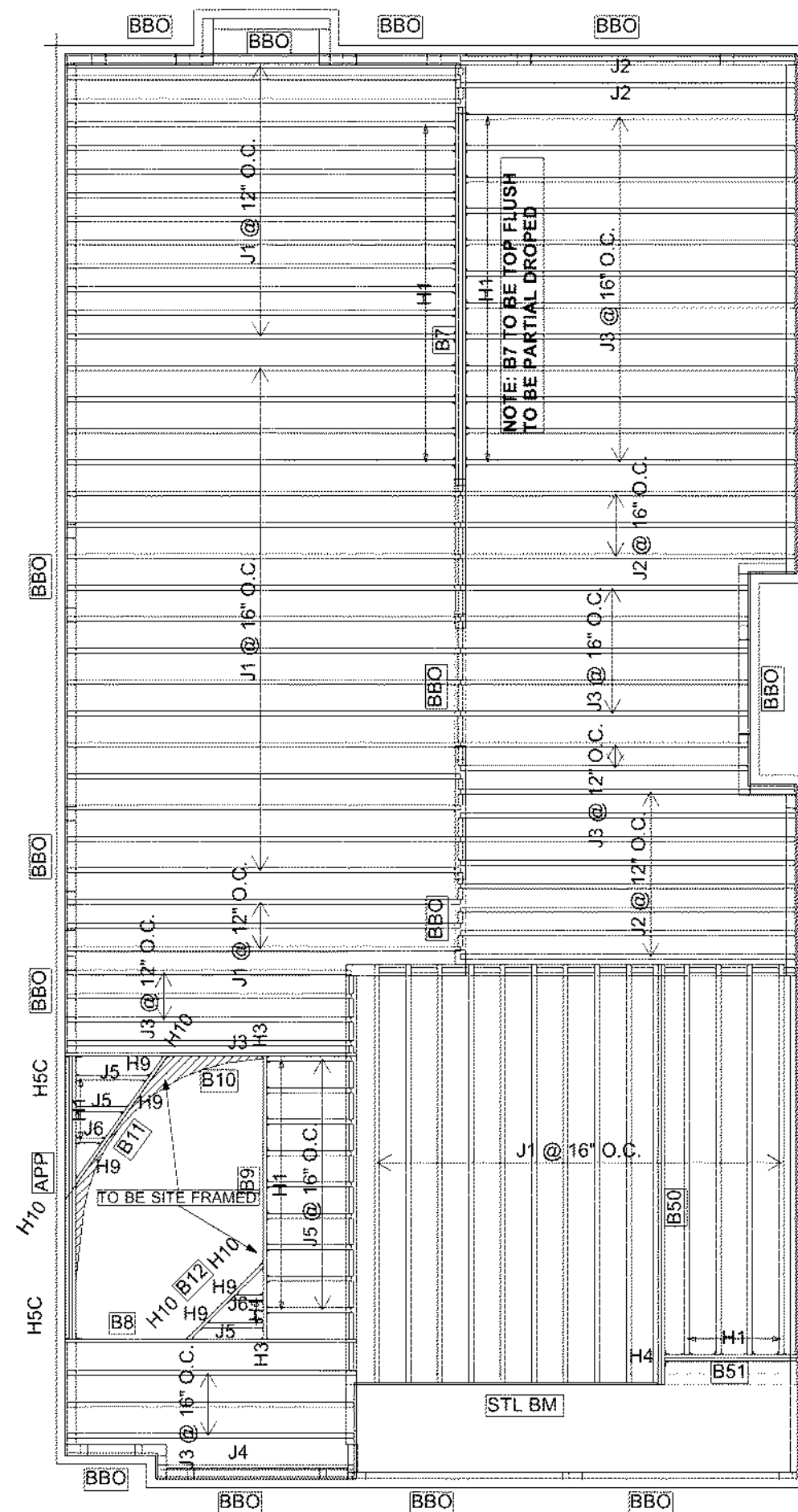
LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 lb/ft²

TILE LOAD: +5.0 lb/ft²

JOIST LL DEFLECTION LIMIT: L/480

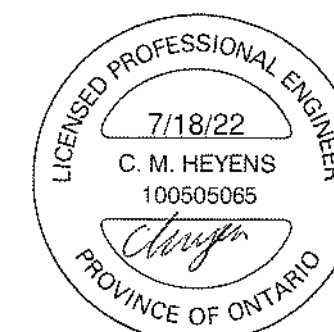
SUBFLOOR: 5/8" GLUED AND NAILED



Products				
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	47
J2	16-00-00	11 7/8" NI-40x	1	13
J3	14-00-00	11 7/8" NI-40x	1	26
J4	10-00-00	11 7/8" NI-40x	1	1
J5	4-00-00	11 7/8" NI-40x	1	12
J6	2-00-00	11 7/8" NI-40x	1	2
B50	18-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B10	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B8	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B9	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
APP	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	3	3
B11	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B12	6-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B51	6-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B7	17-00-00	1 3/4" x 14" (2.0E 3100) WestFraser LVL	3	3

Connector Summary		
Qty	Manuf	Product
11	H1	IUS2.56/11.88
4	H1	IUS2.56/11.88
3	H1	IUS2.56/11.88
26	H1	IUS2.56/11.88
1	H3	HGUS410
1	H3	HUS1.81/10
1	H4	HGUS410
2	H5C	HUC610
3	H9	LSSR2.56Z
2	H9	LSSR2.56Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z

DWG# TF22071099 TO TF22071104
DWG# TF22071121 TO TF22071122



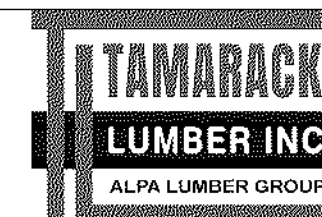
STRUCTURAL COMPONENTS ONLY
DWG# TF22071133

THIS IS A FLOOR COMPONENT PLACEMENT PLAN ONLY.

The wood beams and joists outlined on this plan are designed as individual building components to be incorporated into the design of the building at the specification of the building designer. Please see the individual beam reports, joist reports, and/or joist span tables for each component identified on this placement plan.
The supporting structure is to be specified by the building designer prior to the installation of joist(s) and/or beam(s). The building designer is responsible for the bracing of the floor system and its integration into the bracing of the overall structure. All components labelled "by others" or "as per plan", and all steel beams, are not within the scope of work of this seal.
The building designer must review and approve this plan to ascertain conformity to the overall structural plan of the building. All dimensions to be verified on site.

DATE: 7/15/22

**2nd FLOOR FRAMING
STANDARD**



FROM PLAN DATED: 2022/01/11

BUILDER: ROYAL PINE HOMES

SITE: FORESTSIDE ESTATES

MODEL: 3804

ELEVATION: C

LOT:

CITY: BRAMPTON

SALESMAN: Rick DiCiano

DESIGNER: PL

REVISION:

REFER TO THE NORDIC INSTALLATION GUIDE
FOR PROPER STORAGE AND INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 SPF #2 REQ'D
UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER
CONCENTRATED LOADS. SEE FIGURE 1.

CANTILEVERED JOISTS INCLUDING **CANT' OVER**
BRICK REQ. 1-JOIST BLOCKING ALONG BEARING
AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES
4/5 FOR REINFORCEMENT REQUIREMENTS.

FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD**
CUT OPENINGS SEE FIGURE 6 AND TABLES 6.1/6.2.
CERAMIC TILE APPLICATION AS PER OBC 9.30.6.

ALL **CONNECTORS** MUST BE INSTALLED AS PER THE
MANUFACTURER'S SPECIFICATIONS USING THE
MANUFACTURER SPECIFIED FASTENERS.

ALL **BEAM HANGER FASTENERS** INSTALLED INTO
THE **SUPPORTING MEMBER** MUST BE A MINIMUM
OF **3.5"** IN LENGTH UNLESS OTHERWISE SPECIFIED
BY THE SUPPORTING MEMBER ENGINEER OF RECORD.

LOADING:

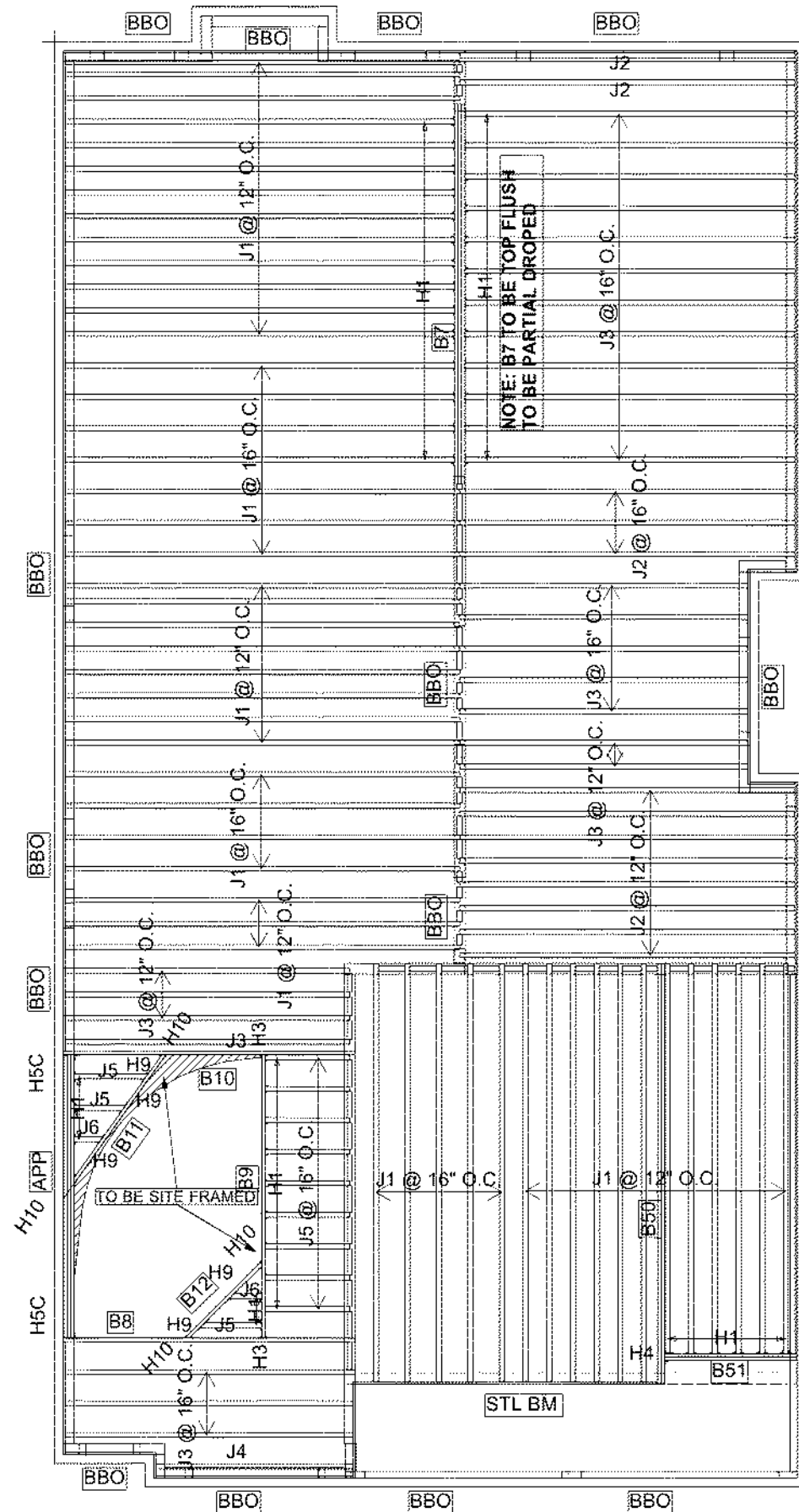
LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 lb/ft²

TILE LOAD: +5.0 lb/ft²

JOIST LL DEFLECTION LIMIT: L/480

SUBFLOOR: 5/8" GLUED AND NAILED



Products				
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	52
J2	16-00-00	11 7/8" NI-40x	1	13
J3	14-00-00	11 7/8" NI-40x	1	26
J4	10-00-00	11 7/8" NI-40x	1	1
J5	4-00-00	11 7/8" NI-40x	1	12
J6	2-00-00	11 7/8" NI-40x	1	2
B50	18-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B10	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B8	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B9	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
APP	14-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	3	3
B11	8-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B12	6-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	1	1
B51	6-00-00	1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	2	2
B7	18-00-00	1 3/4" x 14" (2.0E 3100) WestFraser LVL	3	3

Connector Summary		
Qty	Manuf	Product
11	H1	IUS2.56/11.88
6	H1	IUS2.56/11.88
3	H1	IUS2.56/11.88
26	H1	IUS2.56/11.88
1	H3	HGUS410
1	H3	HUS1.81/10
1	H3	HGUS410
2	H5C	HUC610
3	H9	LSSR2.56Z
2	H9	LSSR2.56Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z
1	H10	LSSR1.81Z

DWG# TF22071099 TO TF22071104
DWG# TF22071121 TO TF22071122



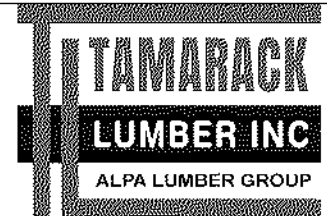
STRUCTURAL COMPONENTS ONLY
DWG# TF22071134

THIS IS A FLOOR COMPONENT PLACEMENT PLAN ONLY.

The wood beams and joists outlined on this plan are designed as individual building components to be incorporated into the design of the building at the specification of the building designer. Please see the individual beam reports, joist reports, and/or joist span tables for each component identified on this placement plan.
The supporting structure is to be specified by the building designer prior to the installation of joist(s) and/or beam(s). The building designer is responsible for the bracing of the floor system and its integration into the bracing of the overall structure. All components labelled "by others" or "as per plan", and all steel beams, are not within the scope of work of this seal.
The building designer must review and approve this plan to ascertain conformity to the overall structural plan of the building. All dimensions to be verified on site.

DATE: 7/15/22

**2nd FLOOR FRAMING
OPT 5 BED**



FROM PLAN DATED: 2022/01/11

BUILDER: ROYAL PINE HOMES

SITE: FORESTSIDE ESTATES

MODEL: 3804

ELEVATION: C

LOT:

CITY: BRAMPTON

SALESMAN: Rick DiCiano

DESIGNER: PL

REVISION:

REFER TO THE **NORDIC INSTALLATION GUIDE**
FOR PROPER STORAGE AND INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 SPF #2 REQ'D
UNDER INTERIOR UNIFORM LOAD BEARING WALLS.

MULTIPLE SQUASH BLOCKS REQ'D UNDER
CONCENTRATED LOADS. SEE FIGURE 1.

CANTILEVERED JOISTS INCLUDING **CANT' OVER**
BRICK REQ. 1-JOIST BLOCKING ALONG BEARING
AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES
4/5 FOR REINFORCEMENT REQUIREMENTS.

FOR **HOLES** INCLUDING **DUCT CHASE** AND **FIELD**
CUT OPENINGS SEE FIGURE 6 AND TABLES 6.1/6.2.
CERAMIC TILE APPLICATION AS PER OBC 9.30.6.

ALL **CONNECTORS** MUST BE INSTALLED AS PER THE
MANUFACTURER'S SPECIFICATIONS USING THE
MANUFACTURER SPECIFIED FASTENERS.

ALL **BEAM HANGER FASTENERS** INSTALLED INTO
THE **SUPPORTING MEMBER** MUST BE A MINIMUM
OF **3.5"** IN LENGTH UNLESS OTHERWISE SPECIFIED
BY THE SUPPORTING MEMBER ENGINEER OF RECORD.

LOADING:

LIVE LOAD: 40.0 lb/ft²

DEAD LOAD: 15.0 lb/ft²

TILE LOAD: +5.0 lb/ft²

JOIST LL DEFLECTION LIMIT: L/480

SUBFLOOR: 5/8" GLUED AND NAILED

NORDIC

INSTALLATION GUIDE NORDIC JOIST

NS-G133 
ENGLISH
VERSION
2020-10-01

Engineered Wood Products

BASIC INSTALLATION GUIDE FOR RESIDENTIAL FLOORS

 **NORDIC
JOIST**

**NORDIC
STRUCTURES**

nordic.ca

INSTALLING NORDIC I-JOISTS

1. Installation of Nordic I-joists shall be as shown in details 1.
2. Except for cutting to length, I-joist flanges should never be cut, drilled or notched.
3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
4. Concentrated loads should only be applied to the top surface of the top flange. Concentrated loads should not be suspended from the bottom flange with the exception of light loads, such as ceiling fans or light fixtures.
5. I-joists must be protected from the weather prior to installation.
6. I-joists must not be used in applications where they will be permanently exposed to weather, or will reach a moisture content of 15 percent or greater, such as in swimming pool or hot tub areas. They must not be installed where they will remain in direct contact with concrete or masonry.
7. End bearing length must be at least 1-3/4 inch. For multiple-span joists, intermediate bearing length must be at least 3-1/2 inches.
8. Ends of floor joists shall be restrained to prevent rollover. Use rim board or I-joist blocking panels.
9. I-joists installed beneath bearing walls perpendicular to the joists shall have full-depth blocking panels, rim board, or squash blocks (cripple blocks) to transfer gravity loads from above the floor system to the wall or foundation below.
10. For I-joists installed directly beneath bearing walls parallel to the joists or used as rim board or blocking panels, the maximum vertical load using a single I-joist is 3,300 plf, and 6,000 plf if double I-joists are used.
11. Continuous lateral support of the I-joist's compression flange is required to prevent rotation and buckling. In simple span uses, lateral support of the top flange is normally supplied by the floor sheathing. In multiple-span or cantilever applications, bracing of the I-joist's bottom flange is also required at interior supports of multiple-span joists, and at the end support next to the cantilever extension. The ends of all cantilever extensions must be laterally braced as shown in details 3, 4, or 5.
12. Nails installed in flange face or edge shall be spaced in accordance with the applicable building code requirements or approved building plans, but should not be closer than those specified on page 3.3 of the Nordic Joist Technical Guide (NS-GT3).
13. Details 1 show only I-joist-specific fastener requirements. For other fastener requirements, see the applicable building code.
14. For proper temporary bracing of wood I-joists and placement of temporary construction loads, see [APA Technical Note: Temporary Construction Loads over I-Joist Roofs and Floors, Form J735](#).

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity.

SAFETY AND CONSTRUCTION PRECAUTIONS

I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

Avoid Accidents by Following these Important Guidelines:

1. Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
 2. When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
 - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2-inch nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two I-joists.
 - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
 4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
 5. Never install a damaged I-joist.
- Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.



Do not walk on I-joists until fully fastened and braced, or serious injuries can result.



Never stack building materials over unsheathed I-joists. Once sheathed, do not overstress I-joist with concentrated loads from building materials.

NORDIC I-JOIST SERIES

RESIDENTIAL SERIES

NI-20
2x3 S-P-F No. 2
3/8 in. web
Depths
9-1/2 and 11-7/8 in.
33 pieces per unit

NI-40x
2x3 1950F MSR
3/8 in. web
Depths
9-1/2, 11-7/8 and 14 in.
33 pieces per unit

NI-60
2x3 2100F MSR
3/8 in. web
Depths
9-1/2, 11-7/8, 14 and 16 in.
33 pieces per unit

NI-80
2x4 2100F MSR
3/8 in. web
Depths
9-1/2, 11-7/8, 14 and 16 in.
23 pieces per unit

NI-90
2x4 2400F MSR
3/8 in. web
Depths
11-7/8, 14 and 16 in.
23 pieces per unit

RIM BOARDS
Width Length
1-1/8 in. 16 ft
Depths
9-1/2 to 16 in.
APA Rim Board Plus

WEB STIFFENERS

2 Concentrated Load (Load Stiffener)

Tight joint, no gap

End Bearing (Bearing Stiffener)

Gap

Tight joint, no gap

Flange width 2-1/2" or 3-1/2"

Approx. 2"

1/8"-1/4" Gap

Four 2-1/2" nails, 3" nails required for I-joists with 3-1/2" flange width

No gap

Stiffener Size Requirements

Flange width (in.)	Web stiffener size each side of web (in.)
2-1/2	1 x 2-5/16 Minimum width
3-1/2	1-1/2 x 2-5/16 Minimum width

NAIL SPACING

Nailing into flange face

Nailing into flange edge

Nailed to Only One Flange Edge (Top View)

Closest nail spacing

Nailed to Both Flange Edges (Top View)

Closest nail spacing

1/2 offset spacing ⁽¹⁾

Recommended Closest Nail Spacing for Fastening Sheathing to I-joist Flanges to Minimize Splitting

Fastener size (diameter x length)	Flange face nailing ⁽²⁾			Flange edge nailing ⁽³⁾		
	End distance (in.)	Nail spacing (in.)	End distance (in.)	Nail spacing (in.)	Nail spacing (in.)	
0.128" or smaller in diameter, and 3-1/4" or shorter in length	2	2	2	2	4	
					4	
Greater than 0.128" up to 0.148" in diameter, and 3-1/4" or shorter in length	2	3	2	3	6	
					6	

⁽¹⁾ If more than one row is required, offset rows a minimum of 1/2 inch and stagger.

⁽²⁾ Closest nail spacing measured from one flange edge. Nails on opposite flange edge must be offset one-half the minimum spacing.

⁽³⁾ Backer blocks must be long enough to permit required nailing without splitting.

1a Nordic I-joist blocking panel

2-1/2" nails at 6" o.c. to top plate (when used for lateral shear transfer, nail to bearing plate with same nailing as required for floor sheathing)

Attach I-joist to top plate per detail 1b

1b Rim board

One 2-1/2" nail at top and bottom flange

Attach rim board to top plate using 2-1/2" toe-nails at 6" o.c.

One 2-1/2" face nail at each side at bearing

Note:
1. To avoid splitting flange, start nails at least 1-1/2 inch from end of I-joist. Nails may be driven at an angle to avoid splitting of bearing plate.

1g Load-bearing wall above shall align vertically with the wall below. Other conditions, such as offset bearing walls, are not covered by this detail.

Blocking panel required over all interior supports under load-bearing walls or when floor joists are not continuous over support. The NBC requires blocking at load-bearing and non-load-bearing walls constructed with required braced wall panels (shearwalls).

Joist attachment per detail 1b

2-1/2" nails at 6" o.c. to top plate

Nordic I-joist blocking panel per detail 1a

Notes:
1. An occasional blocking panel (one per line of blocking) may be left out for the passage of plumbing or ventilation ducts. For other applications, contact Nordic Structures.
2. For other options, see details 1g-1 to 1g-5.

1h Use backer block if hanger load exceeds 360 lbf. Before installing a backer block to a double I-joist, drive three additional 3" nails through the webs and filler block where the backer block will fit. Clinch. Install backer block tight to top flange. Use twelve 3" nails, clinched when possible. Maximum resistance for hanger for this detail = 1,620 lbf.

Double I-joist header

Filler block per detail 1p

Top- or face-mount hanger

Backer block required:
- Only on the loaded side for top-mount hangers
- On both sides for face-mount hangers

Flange width (in.)	Material thickness required (in.) ⁽¹⁾	Minimum depth (in.) ⁽²⁾
2-1/2	1	5-1/2
3-1/2	1-1/2	7-1/4

⁽¹⁾ Minimum grade for backer block material shall be S-P-F No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-Q325 Standard.

⁽²⁾ For face-mount hangers use net joist depth minus 3-1/4 inches for joists with 1-1/2-inch-thick flanges.

Notes:
1. Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.
2. For hanger resistance, see manufacturer's recommendations.
3. Verify double I-joist resistance to support concentrated loads.
4. Backer blocks must be long enough to permit required nailing without splitting.

1j Top- or face-mount hanger installed per manufacturer's recommendations

1k Top-mount hanger installed per manufacturer's recommendations

2x plate flush with inside face of wall or beam. 1/8" overhang allowed past inside face of wall or beam.

1l Multiple I-joist header with full depth filler block shown. Nordic Lam or SCL headers may also be used. Verify header resistance to support concentrated loads.

Backer block per detail 1h

1m Install hanger per manufacturer's recommendations

Filler block per detail 1p

1n Do not bevel-cut I-joist beyond inside face of wall

Attach I-joist per detail 1b

Notes:
1. Blocking required at bearing for lateral support, not shown for clarity.

1p Filler block

Offset nails from opposite face by 6"

1/8" to 1/4" gap between top flange and filler block

Notes:
1. Support back of I-joist web during nailing to prevent damage to web/flange connection.
2. Leave a 1/8-inch to 1/4-inch gap between top of filler block and bottom of top I-joist flange.
3. Filler block is required between joists for full length of span.
4. For flange width of 2-1/2 inches, nail joists together with two rows of 3-inch nails at 12 inches o.c. (clinched when possible) on each side of the double I-joist (total of four nails per foot). For flange width of 3-1/2 inches, use two rows of 3-inch nails at 6 inches o.c. on each side of the double I-joist (total of eight nails per foot).
5. The maximum factored load may be applied to one side of the double I-joist using this detail is 860 dBf.

Filler Block Requirements for Double I-joist Construction

Flange width (in.)	Net depth (in.)	Filler block size (in.)	Example
2-1/2	9-1/2	2-1/8 to 2-1/4 x 6	2x8 x 5/8" or 3/4" sheathing
	11-7/8	2-1/8 to 2-1/4 x 8	2x8 x 5/8" or 3/4" sheathing
	14	2-1/8 to 2-1/4 x 10	2x10 x 5/8" or 3/4" sheathing
3-1/2	9-1/2	3 x 6	2 x 2x6
	11-7/8	3 x 8	2 x 2x6
	14	3 x 10	2 x 2x10

Notes:
1. The height of the filler block may be different from that specified in the table, as long as it allows nailing and respects the required gap.

1s-1 See note 2

1/8" gap minimum

One 2-1/2" nail at top and bottom flange

Rim board

2-1/2" nails at 6" o.c.

One 2-1/2" nail, one side only

2x4 minimum

Blocking panel (note 1)

Notes:
1. In some local codes, blocking panels are prescriptively required in the first joist space (or first and second joist spaces) next to the starter joist. Where required, see local code requirements for spacing of the blocking panels. As a minimum, it is recommended to use blocking panels spaced at 4 feet on centre.
2. Details shown are for minimum blocking attachment. Transfer of lateral loads may require additional fasteners. In such cases, nail size, spacing and specific design detailing shall be provided by the building designer.
3. Where blocking panels are required between adjacent joists, the blocking panels can be staggered by approximately 3 inches, and end-nailed as shown.
4. Nails attaching lumber piece to I-joist web should be driven from the web side and clinched on the lumber side.

This document supersedes all previous versions. For the latest version, consult nordic.ca or contact Nordic Structures.

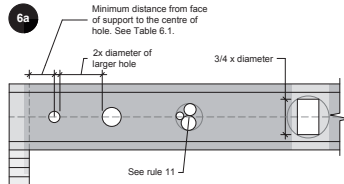
FOR ALL construction details →DC3

WEB HOLES AND OPENINGS

WEB HOLES IN I-JOISTS

Rules for Cutting Holes in I-Joists

1. The distance between the inside edge of the support and the centreline of any hole shall be in compliance with the requirements of Table 6.1.
2. I-joist top and bottom flanges must never be cut, notched or otherwise modified.
3. Whenever possible, field-cut holes should be centred on the middle of the web.
4. The maximum size hole that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the opening and the adjacent I-joist flange.
5. The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
6. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole - or twice the length of the longest side of the longest rectangular hole - and each hole must be sized and located in compliance with the requirements of Table 6.1.
7. Holes measuring 1-1/2 inch or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.
8. A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
9. All holes shall be cut in accordance with the restrictions listed above and as illustrated in detail 6a.
10. Limit three maximum-size holes per span.
11. A group of round holes at approximately the same location shall be permitted if it meets the requirements for a single round hole circumscribed around them.

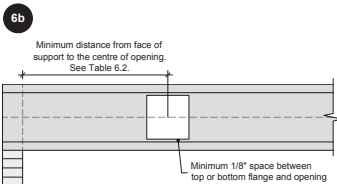


- Notes:**
1. Never drill, cut or notch the flange, or over-cut the web.
2. Holes in web should be cut with a sharp saw.
3. For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch-diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the I-joist.

DUCT CHASE OPENINGS

Rules for Cutting Duct Chase Openings in I-joists

1. The distance between the inside edge of the support and the centreline of a duct chase opening shall be in compliance with the requirements of Table 6.2.
2. I-joist top and bottom flanges must never be cut, notched or otherwise modified.
3. The maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the opening and the adjacent I-joist flange.
4. All openings shall be cut in accordance with the restrictions listed above and as illustrated in detail 6b.
5. Limit one maximum-size duct chase opening per span.

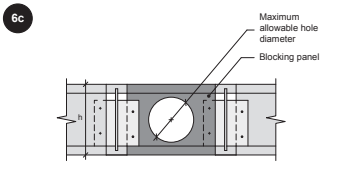


- Notes:**
1. Never drill, cut or notch the flange, or over-cut the web.
2. Holes in web should be cut with a sharp saw.
3. Avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch-diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the I-joist.

HOLES IN BLOCKING PANELS

Maximum Allowable Hole Size in Lateral-restraint-only Blocking Panels

1. The maximum allowable hole size for a lateral-restraint-only blocking panel is 2/3 of the lesser dimension of the blocking's depth or length. Assuming the blocking panel is longer than its height (or depth), the table aside applies. For other applications, contact Nordic Structures.
2. Holes cut into the blocking panels are subject to the following limitations:
 - The top and bottom flanges of an I-joist blocking panel must never be cut, notched or otherwise modified.
 - Field-cut holes must be centred in the blocking horizontally.
 - While round holes are preferred, rectangle holes may be used provided the corners are not over cut. Slightly rounding corners or pre-drilling corners with a 1-inch-diameter bit is recommended.
 - All holes must be cut in a workman-like manner in accordance with the limitations listed above.



I-joist or rim board blocking depth (in.)	Maximum allowable hole diameter or (in.) ⁽¹⁾
9-1/2	6-1/4
11-7/8	7-3/4
14	9-3/4
16	10-1/2

⁽¹⁾ Maximum allowable hole diameter in blocking panel, where the blocking panel is longer than its height.

TABLE 6.1 - LOCATION OF WEB HOLES

Simple or multiple span		Minimum distance from inside face of any support to centre of hole (ft.-in.)															
Joist depth	Joist series	2	3	4	5	6	6-1/4	7	8	8-5/8	9	10	10-3/4	11	12	12-3/4	
9-1/2"	NI-20	0'-7"	1'-6"	2'-10"	4'-3"	5'-8"	6'-0"	-	-	-	-	-	-	-	-	-	-
	NI-40x	0'-7"	1'-6"	3'-0"	4'-4"	6'-0"	6'-4"	-	-	-	-	-	-	-	-	-	-
	NI-60	1'-3"	2'-6"	4'-0"	5'-4"	7'-0"	7'-5"	-	-	-	-	-	-	-	-	-	-
	NI-80	2'-3"	3'-6"	5'-0"	6'-6"	8'-2"	8'-8"	-	-	-	-	-	-	-	-	-	-
11-7/8"	NI-20	0'-7"	0'-8"	1'-0"	2'-4"	3'-8"	4'-0"	5'-0"	6'-8"	7'-9"	-	-	-	-	-	-	-
	NI-40x	0'-7"	0'-8"	1'-3"	2'-8"	4'-0"	4'-4"	5'-5"	7'-2"	8'-4"	-	-	-	-	-	-	-
	NI-60	0'-7"	1'-8"	3'-0"	4'-3"	5'-9"	6'-0"	7'-3"	6'-10"	10'-0"	-	-	-	-	-	-	-
	NI-80	1'-6"	2'-10"	4'-2"	5'-6"	7'-0"	7'-5"	8'-6"	10'-3"	11'-4"	-	-	-	-	-	-	-
14"	NI-20	0'-7"	0'-8"	1'-5"	3'-2"	4'-10"	5'-4"	6'-9"	8'-9"	10'-2"	-	-	-	-	-	-	-
	NI-40x	0'-7"	0'-8"	1'-0"	2'-4"	2'-9"	3'-9"	5'-2"	6'-0"	6'-6"	8'-3"	10'-2"	-	-	-	-	-
	NI-60	0'-7"	0'-8"	1'-8"	3'-0"	4'-3"	4'-8"	5'-8"	7'-2"	8'-0"	8'-8"	10'-4"	11'-9"	-	-	-	-
	NI-80	0'-10"	2'-0"	3'-4"	4'-9"	6'-2"	6'-5"	7'-6"	9'-0"	10'-0"	10'-8"	12'-4"	13'-9"	-	-	-	-
16"	NI-20	0'-7"	0'-8"	0'-10"	2'-5"	4'-0"	4'-5"	5'-9"	7'-5"	8'-8"	9'-4"	11'-4"	12'-11"	-	-	-	-
	NI-40x	0'-7"	0'-8"	0'-8"	1'-6"	2'-10"	3'-2"	4'-2"	5'-8"	6'-4"	7'-0"	8'-5"	9'-8"	10'-2"	12'-2"	13'-9"	-
	NI-60	0'-7"	1'-3"	2'-6"	3'-10"	5'-3"	5'-6"	6'-6"	8'-0"	9'-0"	9'-5"	11'-0"	12'-3"	12'-9"	14'-5"	16'-0"	-
	NI-80	0'-7"	0'-8"	0'-8"	1'-9"	3'-3"	3'-8"	4'-9"	6'-5"	7'-5"	8'-0"	9'-10"	11'-3"	11'-9"	13'-6"	15'-4"	-

- Notes:**
1. Tabulated values are applicable to residential floor construction meeting the above design criteria.
2. The above table is based on the I-joists being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

Design Criteria	
Joist spacing	Up to 24 inches
Loads	Live load = 40 psf and dead load = 15 psf
Deflection limits	L/480 under live load and L/240 under total load

TABLE 6.2 - LOCATION OF DUCT CHASE OPENINGS

Simple span		Minimum distance from inside face of any support to centre of opening (ft.-in.)															
Joist depth	Joist series	Duct chase length (in.)															
		8	10	12	14	16	18	20	22	24							
9-1/2"	NI-20	4'-1"	4'-5"	4'-10"	-	-	-	-	-	-	-	-	-	-	-	-	-
	NI-40x	5'-3"	5'-8"	6'-0"	6'-5"	6'-10"	7'-3"	7'-8"	-	-	-	-	-	-	-	-	-
	NI-60	5'-4"	5'-9"	6'-2"	6'-7"	7'-1"	7'-5"	8'-0"	-	-	-	-	-	-	-	-	-
	NI-80	5'-3"	5'-8"	6'-0"	6'-5"	6'-10"	7'-3"	7'-8"	8'-2"	8'-6"	-	-	-	-	-	-	-
11-7/8"	NI-20	5'-9"	6'-2"	6'-6"	-	-	-	-	-	-	-	-	-	-	-	-	-
	NI-40x	6'-8"	7'-2"	7'-6"	8'-1"	8'-6"	9'-1"	9'-6"	-	-	-	-	-	-	-	-	-
	NI-60	6'-8"	7'-2"	7'-6"	8'-0"	8'-6"	9'-0"	9'-3"	9'-9"	-	-	-	-	-	-	-	-
	NI-80	7'-2"	7'-7"	8'-0"	8'-5"	8'-10"	9'-3"	9'-8"	10'-2"	10'-8"	-	-	-	-	-	-	-
14"	NI-90	7'-6"	7'-11"	8'-4"	8'-9"	9'-2"	9'-7"	10'-1"	10'-7"	10'-11"	-	-	-	-	-	-	-
	NI-40x	8'-1"	8'-7"	9'-0"	9'-6"	10'-1"	10'-7"	11'-2"	-	-	-	-	-	-	-	-	-
	NI-60	8'-9"	9'-3"	9'-8"	10'-11"	10'-6"	11'-1"	11'-6"	-	-	-	-	-	-	-	-	-
	NI-80	9'-0"	9'-3"	9'-9"	10'-1"	10'-7"	11'-1"	11'-6"	12'-1"	12'-6"	-	-	-	-	-	-	-
16"	NI-90	9'-2"	9'-8"	10'-0"	10'-6"	11'-1"	11'-5"	11'-9"	12'-4"	12'-11"	-	-	-	-	-	-	-
	NI-60	10'-3"	10'-8"	11'-2"	11'-6"	12'-1"	12'-6"	13'-2"	-	-	-	-	-	-	-	-	-
	NI-80	10'-4"	10'-9"	11'-3"	11'-9"	12'-1"	12'-7"	13'-1"	13'-8"	14'-1"	-	-	-	-	-	-	-
	NI-90	10'-9"	11'-2"	11'-8"	12'-0"	12'-6"	13'-0"	13'-6"	14'-2"	14'-10"	-	-	-	-	-	-	-



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 2ND FLOOR
Label: B7 - i7418
Type: Beam

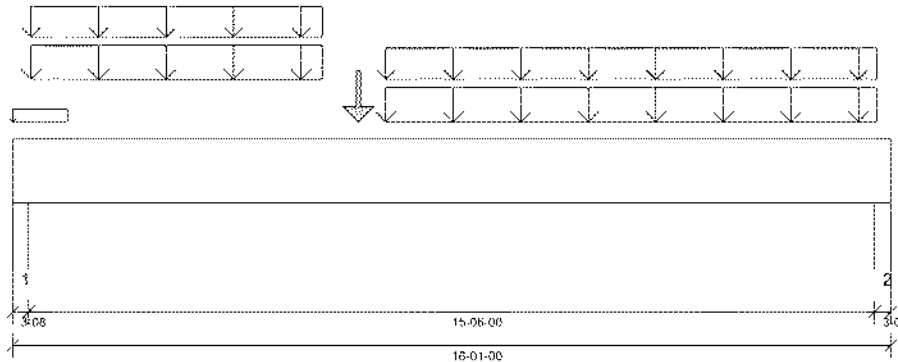
3 Ply Member
1 3/4" x 14" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 2 1/2"
- 615 psi Wall @ 15'- 10 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	7'- 8"	1.25D + 1.5L	1.00	40969 lb ft	72216 lb ft	Passed - 57%
Factored Shear:	14'- 7 1/2"	1.25D + 1.5L	1.00	10666 lb	24431 lb	Passed - 44%
Live Load (LL) Pos. Defl.:	8'- 1/2"	L		0.358"	L/360	Passed - L/519
Total Load (TL) Pos. Defl.:	8'- 1/2"	D + L		0.550"	L/240	Passed - L/338

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Upward Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3'-08"	1.25D + 1.5L	1.00	10278 lb		19143 lb	11324 lb	Passed - 91%
2	3'-08"	1.25D + 1.5L	1.00	10704 lb		19110 lb	11304 lb	Passed - 95%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	16'- 1"	Self Weight	Top	21 lb/ft	-	-	-
Uniform	0'	1'	FC2 Floor Decking (Plan View Fill)	Top	0 lb/ft	0 lb/ft	-	-
Uniform	0'- 4"	5'- 8"	Smoothed Load	Back	167 lb/ft	333 lb/ft	-	-
Uniform	0'- 4"	5'- 8"	Smoothed Load	Front	141 lb/ft	282 lb/ft	-	-
Uniform	6'- 10"	15'- 10"	Smoothed Load	Back	167 lb/ft	333 lb/ft	-	-
Uniform	6'- 10"	15'- 10"	Smoothed Load	Front	146 lb/ft	292 lb/ft	-	-
Point	6'- 4"	6'- 4"	J3(7655)	Front	188 lb	376 lb	-	-
Point	6'- 4"	6'- 4"	J1(7693)	Back	194 lb	389 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3 1/2"	6(i1508)	2535 lb	4732 lb	-	-
2	15'- 9 1/2"	16'- 1"	7(i1509)	2642 lb	4942 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of all plies. (Consult with manufacturer for guideline pertaining to this design option.)
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

PLY TO PLY CONNECTION:
4 ROWS OF 3.25" PNEUMATIC GUN
NAILS (0.120"x3.25") @ 8" O/C
NAIL FROM BOTH FACES (STAGGER 1/2 SPACE)

PLY TO PLY CONNECTION ASSUMES ANY
SUPPORTED BEAM HANGERS ARE FASTENED
TO THIS BEAM WITH MIN. 3.5" FASTENERS.



STRUCTURAL COMPONENT ONLY
DWG # TF22071099



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 2ND FLOOR
Label: B8 - i7953
Type: Beam

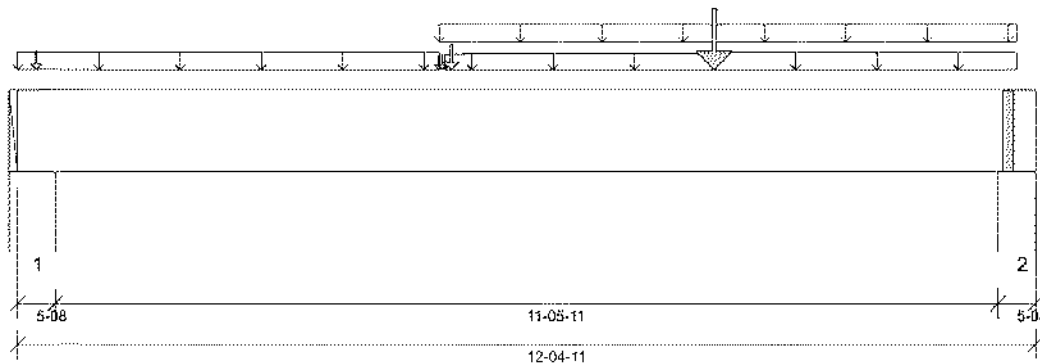
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 4'- 8 1/16"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Wall @ 12'- 3/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	8'- 5 3/4"	1.25D + 1.5L	1.00	5720 lb ft	17672 lb ft	Passed - 32%
Factored Shear:	10'- 11 5/16"	1.25D + 1.5L	1.00	1702 lb	6908 lb	Passed - 25%
Live Load (LL) Pos. Defl.:	6'- 6 3/16"	L		0.120"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	6'- 6 1/16"	D + L		0.190"	L/240	Passed - L/724

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08	1.25D + 1.5L + S	1.00	1253 lb		10010 lb	5921 lb	Passed - 21%
2	5'-08	1.25D + 1.5L	1.00	1851 lb		10010 lb	5921 lb	Passed - 31%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	12'- 4 11/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	5'- 1 9/16"	FC2 Floor Decking (Plan View File)	Top	14 lb/ft	29 lb/ft	-	-
Uniform	5'- 1 9/16"	12'- 1 15/16"	FC2 Floor Decking (Plan View File)	Top	14 lb/ft	27 lb/ft	-	-
Uniform	5'- 6 3/8"	8'- 5 3/4"	FC2 Floor Decking (Plan View File)	Top	5 lb/ft	9 lb/ft	-	-
Uniform	8'- 5 3/4"	12'- 1 15/16"	FC2 Floor Decking (Plan View File)	Top	13 lb/ft	26 lb/ft	-	-
Tapered	5'- 2 7/16"	5'- 6 3/8"	FC2 Floor Decking (Plan View File)	Top	1 To 2 lb/ft	2 To 5 lb/ft	-	-
Point	5'- 3 3/8"	5'- 3 3/8"	B12(i7971)	Back	93 lb	160 lb	-	-
Point	8'- 5 3/4"	8'- 5 3/4"	B9(i7970)	Back	403 lb	716 lb	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	E59(i1750)	Top	38 lb	-	44 lb	-
Point	5'- 1 9/16"	5'- 1 9/16"	FC2 Floor Decking (Plan View File)	Top	-	0 lb	-	-
Point	5'- 2"	5'- 2"	FC2 Floor Decking (Plan View File)	Top	-	0 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	E17(i1487)	355 lb	515 lb	45 lb	-
2	11'- 11 3/16"	12'- 4 11/16"	2(i1503)	484 lb	825 lb	-1 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071100



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 2ND FLOOR
Label: B9 - i7970
Type: Beam

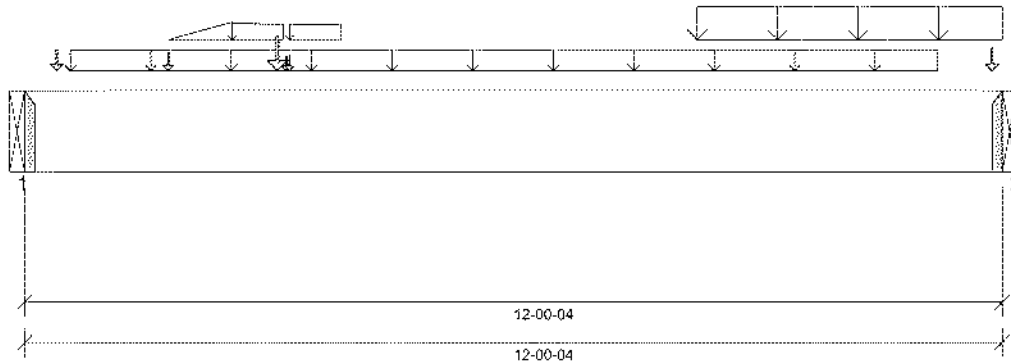
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'
- 615 psi Beam @ 12'- 1/4"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	7'- 5 1/16"	1.25D + 1.5L	1.00	5458 lb ft	17672 lb ft	Passed - 31%
Factored Shear:	11'- 3/8"	1.25D + 1.5L	1.00	2072 lb	6908 lb	Passed - 30%
Live Load (LL) Pos. Defl.:	6'- 2 1/2"	L		0.141"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	6'- 2 3/8"	D + L		0.219"	L/240	Passed - L/659

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	1584 lb		2730 lb	-	Passed - 58%
2	1-08	1.25D + 1.5L	1.00	2706 lb		2730 lb	-	Passed - 99%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories		
			Top	Face	Member			
1	HUS1.81/10		-	-	-	Connector manually specified by the user.		
2	HGUS410		-	-	-	Connector manually specified by the user.		

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	12'- 1/4"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	3'- 3 1/16"	3'- 10 3/4"	FC2 Floor Decking (Plan View File)	Top	2 lb/ft	3 lb/ft	-	-
Uniform	8'- 3 1/4"	12'- 1/4"	User Load	Back	120 lb/ft	240 lb/ft	-	-
Tapered	0'- 6 3/4"	11'- 2 3/4"	Smoothed Load	Front	36 To 38 lb/ft	72 To 76 lb/ft	-	-
Tapered	1'- 9 1/4"	2'- 6 5/8"	FC2 Floor Decking (Plan View File)	Top	0 To 16 lb/ft	0 To 31 lb/ft	-	-
Tapered	2'- 6 5/8"	3'- 2 3/16"	FC2 Floor Decking (Plan View File)	Top	8 To 2 lb/ft	16 To 3 lb/ft	-	-
Point	11'- 10 3/4"	11'- 10 3/4"	J4(i7946)	Front	29 lb	57 lb	-	-
Point	0'- 4 3/4"	0'- 4 3/4"	J4(i7772)	Back	23 lb	45 lb	-	-
Point	1'- 9 1/4"	1'- 9 1/4"	J5(i7777)	Back	13 lb	26 lb	-	-
Point	3'- 1 5/16"	3'- 1 5/16"	B12(i7971)	Back	79 lb	132 lb	-	-
Point	3'- 2 5/8"	3'- 2 5/8"	FC2 Floor Decking (Plan View File)	Top	-	0 lb	-	-
Point	3'- 3 1/16"	3'- 3 1/16"	FC2 Floor Decking (Plan View File)	Top	-	0 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B8(i7953)	403 lb	716 lb	-	-
2	12'- 1/4"	12'- 1/4"	B10(i7969)	666 lb	1252 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071101



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 2ND FLOOR
Label: B10 - i7969
Type: Beam

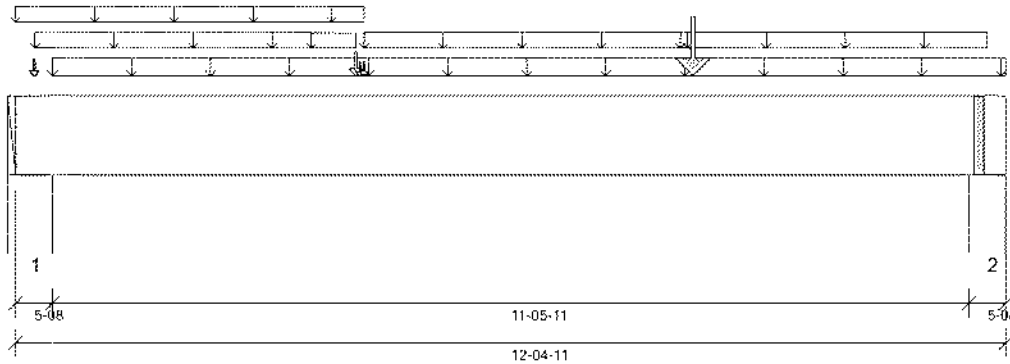
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:
Top: 0' Bottom: 4'- 9/16"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Wall @ 12'- 3/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	8'- 5 3/4"	1.25D + 1.5L	1.00	8940 lb ft	17593 lb ft	Passed - 51%
Factored Shear:	10'- 11 5/16"	1.25D + 1.5L	1.00	2607 lb	6877 lb	Passed - 38%
Live Load (LL) Pos. Defl.:	6'- 6 1/2"	L		0.160"	L/360	Passed - L/862
Total Load (TL) Pos. Defl.:	6'- 5 11/16"	D + L		0.302"	L/240	Passed - L/455
Permanent Deflection:	6'- 4 3/4"			-	L/360	Passed - L/995

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08"	1.25D + 1.5L + S	1.00	2060 lb		10001 lb	5916 lb	Passed - 35%
2	5'-08"	1.25D + 1.5L	1.00	2761 lb		9965 lb	5895 lb	Passed - 47%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	12'- 4 11/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	4'- 4 5/16"	FC2 Floor Decking (Plan View Fill)	Top	5 lb/ft	9 lb/ft	-	-
Uniform	0'- 2 7/8"	3'- 6 3/8"	FC2 Floor Decking (Plan View Fill)	Top	10 lb/ft	20 lb/ft	-	-
Uniform	0'- 5 1/2"	12'- 4 11/16"	User Load	Top	60 lb/ft	-	-	-
Uniform	4'- 4 5/16"	8'- 4 7/8"	FC2 Floor Decking (Plan View Fill)	Top	5 lb/ft	11 lb/ft	-	-
Uniform	8'- 4 7/8"	12'- 1 15/16"	FC2 Floor Decking (Plan View Fill)	Top	7 lb/ft	13 lb/ft	-	-
Tapered	3'- 8 3/8"	4'- 3 1/16"	FC2 Floor Decking (Plan View Fill)	Top	3 To 1 lb/ft	7 To 2 lb/ft	-	-
Point	0'- 2 7/8"	0'- 2 7/8"	APP(i7320)	Front	0 lb	0 lb	-	-
Point	4'- 3 1/8"	4'- 3 1/8"	B11(i7331)	Front	147 lb	257 lb	-	-
Point	8'- 5 3/4"	8'- 5 3/4"	B9(i7970)	Front	666 lb	1252 lb	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	E83(i1782)	Top	46 lb	-	57 lb	-
Point	4'- 3 11/16"	4'- 3 11/16"	FC2 Floor Decking (Plan View Fill)	Top	-	0 lb	-	-
Point	4'- 4 5/16"	4'- 4 5/16"	FC2 Floor Decking (Plan View Fill)	Top	-	0 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	E17(i1487)	794 lb	682 lb	59 lb	-
2	11'- 11 3/16"	12'- 4 11/16"	2(i1503)	957 lb	1034 lb	-2 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071102



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 2ND FLOOR
Label: B11 - i7331
Type: Beam

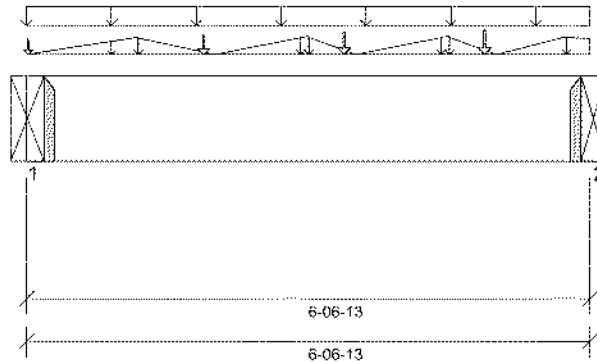
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0'- 1 7/8" Bottom: 1'- 9 1/4"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'
- 615 psi Beam @ 6'- 6 13/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 8 7/16"	1.25D + 1.5L	1.00	926 lb ft	17672 lb ft	Passed - 5%
Factored Shear:	5'- 6 15/16"	1.25D + 1.5L	1.00	449 lb	6908 lb	Passed - 7%
Total Load (TL) Pos. Defl.:	3'- 3 15/16"	D + L		0.011"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Upward Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	487 lb		2730 lb	-	Passed - 18%
2	1-08	1.25D + 1.5L	1.00	565 lb		2730 lb	-	Passed - 21%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LSSR1.81Z		-	-	-	Connector manually specified by the user.
2	LSSR1.81Z		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	-0'	6'- 6 13/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	-0'	6'- 6 13/16"	User Load	Top	20 lb/ft	40 lb/ft	-	-
Tapered	-0'	1'- 3 5/8"	FC2 Floor Decking (Plan View Fill)	Top	0 To 8 lb/ft	0 To 17 lb/ft	-	-
Tapered	1'- 3 5/8"	2'- 2 7/16"	FC2 Floor Decking (Plan View Fill)	Top	4 To 0 lb/ft	9 To 0 lb/ft	-	-
Tapered	2'- 2 7/16"	3'- 3 1/2"	FC2 Floor Decking (Plan View Fill)	Top	0 To 15 lb/ft	0 To 31 lb/ft	-	-
Tapered	3'- 3 1/2"	3'- 10 1/8"	FC2 Floor Decking (Plan View Fill)	Top	8 To 0 lb/ft	15 To 0 lb/ft	-	-
Tapered	3'- 10 1/8"	4'- 11 1/8"	FC2 Floor Decking (Plan View Fill)	Top	0 To 15 lb/ft	0 To 31 lb/ft	-	-
Tapered	4'- 11 1/8"	5'- 5 3/4"	FC2 Floor Decking (Plan View Fill)	Top	8 To 0 lb/ft	15 To 0 lb/ft	-	-
Tapered	5'- 5 3/4"	6'- 3 9/16"	FC2 Floor Decking (Plan View Fill)	Top	0 To 12 lb/ft	0 To 23 lb/ft	-	-
Tapered	6'- 3 9/16"	6'- 6 13/16"	FC2 Floor Decking (Plan View Fill)	Top	6 To 1 lb/ft	12 To 2 lb/ft	-	-
Point	2'- 3/4"	2'- 3/4"	J5(i7907)	Back	14 lb	27 lb	-	-
Point	3'- 8 7/16"	3'- 8 7/16"	J4(i7803)	Back	24 lb	48 lb	-	-
Point	5'- 4 1/16"	5'- 4 1/16"	J4(i7939)	Back	30 lb	61 lb	-	-
Point	0'- 1/4"	0'- 1/4"	FC2 Floor Decking (Plan View Fill)	Top	0 lb	1 lb	-	-


UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	-0'	0'- 2 7/16"	APP(i7320)	130 lb	219 lb	-	-
2	6'- 5 9/16"	6'- 6 13/16"	B10(i7969)	147 lb	257 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



	BUILDER: ROYAL PINE HOMES SITE: FORESTSIDE ESTATES MODEL: 3804 CITY: BRAMPTON	Job Name: 3804 -ELEV A STD Level: 2ND FLOOR Label: B11 - i7331 Type: Beam	1 Ply Member 1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	Status: Design Passed
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DESIGN NOTES

- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.





BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 2ND FLOOR
Label: B12 - i7971
Type: Beam

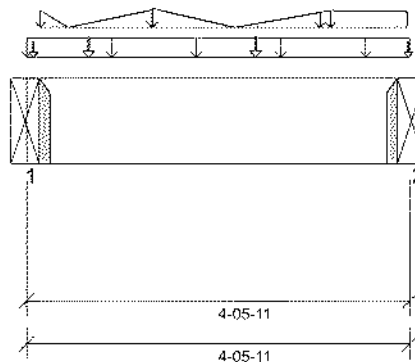
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0'- 1 3/4" Bottom: 1'- 7 13/16"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'
- 615 psi Beam @ 4'- 5 11/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 3 3/16"	1.25D + 1.5L	1.00	384 lb ft	17672 lb ft	Passed - 2%
Factored Shear:	3'- 5 13/16"	1.25D + 1.5L	1.00	199 lb	6908 lb	Passed - 3%

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	349 lb		2730 lb	-	Passed - 13%
2	1-08	1.25D + 1.5L	1.00	305 lb		2730 lb	-	Passed - 11%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement/Accessories		
			Top	Face	Member			
1	LSSR1.81Z	-	-	-	-	Connector manually specified by the user.		
2	LSSR1.81Z	-	-	-	-	Connector manually specified by the user.		

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	4'- 5 11/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	4'- 5 11/16"	User Load	Top	20 lb/ft	40 lb/ft	-	-
Tapered	0'- 1 7/8"	0'- 5 7/8"	FC2 Floor Decking (Plan View Fill)	Top	7 To 0 lb/ft	13 To 0 lb/ft	-	-
Tapered	0'- 5 7/8"	1'- 5 5/8"	FC2 Floor Decking (Plan View Fill)	Top	0 To 10 lb/ft	0 To 20 lb/ft	-	-
Tapered	1'- 5 5/8"	2'- 5 1/4"	FC2 Floor Decking (Plan View Fill)	Top	20 To 0 lb/ft	39 To 0 lb/ft	-	-
Tapered	2'- 5 1/4"	3'- 6 9/16"	FC2 Floor Decking (Plan View Fill)	Top	0 To 6 lb/ft	0 To 11 lb/ft	-	-
Tapered	3'- 6 9/16"	4'- 5 5/16"	FC2 Floor Decking (Plan View Fill)	Top	6 To 1 lb/ft	11 To 2 lb/ft	-	-
Point	0'- 8 11/16"	0'- 8 11/16"	J4(i7772)	Front	17 lb	35 lb	-	-
Point	2'- 8 1/16"	2'- 8 1/16"	J5(i7777)	Front	17 lb	35 lb	-	-
Point	0'- 15/16"	0'- 15/16"	FC2 Floor Decking (Plan View Fill)	Top	0 lb	1 lb	-	-
Point	4'- 5 1/2"	4'- 5 1/2"	FC2 Floor Decking (Plan View Fill)	Top	-	0 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 1 3/4"	B8(i7953)	93 lb	160 lb	-	-
2	4'- 3 15/16"	4'- 5 11/16"	B8(i7970)	79 lb	132 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071104



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 2ND FLOOR
Label: B13 - i7388
Type: Beam

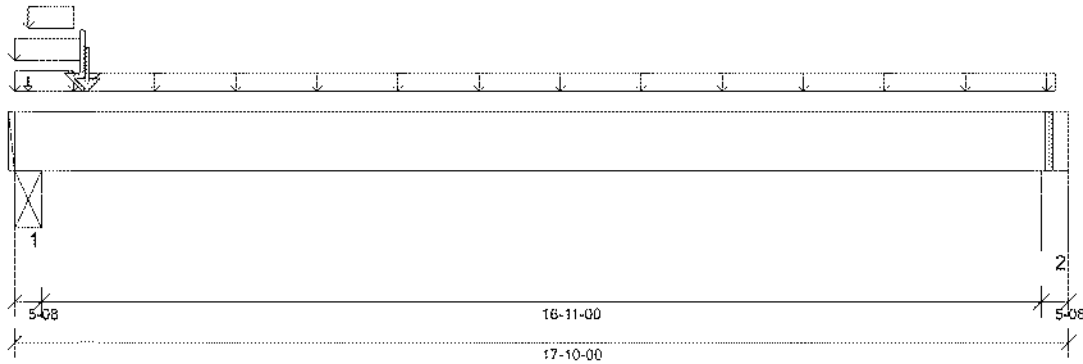
2 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 16'- 1"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'- 4 1/2"
- 615 psi Wall @ 17'- 5 1/2"

PLY TO PLY CONNECTION:
4 ROWS OF 3.25" PNEUMATIC GUN
NAILS (0.120"x3.25") @ 12" O/C

PLY TO PLY CONNECTION ASSUMES ANY
SUPPORTED BEAM HANGERS ARE FASTENED
TO THIS BEAM WITH MIN. 3.5" FASTENERS.



STRUCTURAL COMPONENT ONLY
DWG # TF22071105

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 9 13/16"	1.25D + 1.5L + S	1.00	6647 lb ft	35345 lb ft	Passed - 19%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	0.94	4866 lb	13050 lb	Passed - 37%
Live Load (LL) Pos. Defl.:	8'- 4 7/16"	L + 0.5S		0.141"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	8'- 4 1/4"	D + L + 0.5S		0.259"	L/240	Passed - L/783

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08"	1.25D + 1.5S + L	1.00	7482 lb		20020 lb	11839 lb	Passed - 63%
2	5'-08"	1.25D + 1.5L + S	1.00	1160 lb		20020 lb	11843 lb	Passed - 10%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	17'- 10"	Self Weight	Top	12 lb/ft	-	-	-
Uniform	0'	1'- 1 3/4"	User Load	Top	30 lb/ft	33 lb/ft	72 lb/ft	-
Uniform	-0'	1'	E55(i1748)	Top	100 lb/ft	-	-	-
Uniform	0'- 2 3/4"	1'	E55(i1748)	Top	30 lb/ft	33 lb/ft	72 lb/ft	-
Uniform	1'	17'- 7 1/4"	FC2 Floor Decking (Plan View Fill)	Top	20 lb/ft	39 lb/ft	-	-
Point	1'- 1 3/4"	1'- 1 3/4"	B14(i7857)	Front	1313 lb	1270 lb	647 lb	-
Point	0'- 2 3/4"	0'- 2 3/4"	E55(i1748)	Top	7 lb	8 lb	16 lb	-
Point	1'- 2 3/4"	1'- 2 3/4"	E69(i1768)	Top	694 lb	106 lb	1200 lb	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	STL BM (i1532)	2359 lb	1710 lb	1932 lb	-
2	17'- 4 1/2"	17'- 10"	5(i1504)	349 lb	389 lb	69 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of all plies. (Consult with manufacturer for guideline pertaining to this design option.)
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=1.500", W=3.500". LDF=1.00, Pf=2855 lb, Qr=5460 lb, Result=52.29%.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 2ND FLOOR
Label: B14 - i7857
Type: Beam

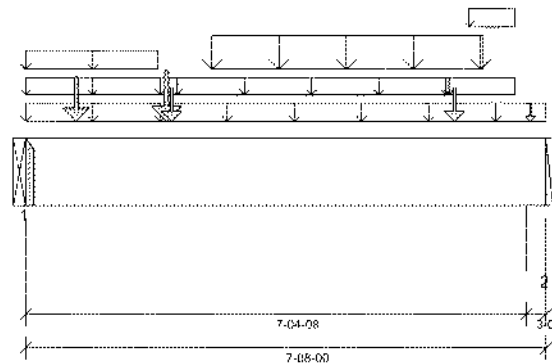
2 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'
- 615 psi Wall @ 7'- 5 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 4 15/16"	1.25D + 1.5L + S	1.00	7921 lb ft	35345 lb ft	Passed - 22%
Factored Shear:	0'- 11 7/8"	1.25D + 1.5L + S	1.00	3753 lb	13815 lb	Passed - 27%
Live Load (LL) Pos. Defl.:	3'- 8 9/16"	L + 0.5S		0.032"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 8 9/16"	D + L + 0.5S		0.058"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L + S	1.00	4209 lb		5460 lb	-	Passed - 77%
2	3-08	1.25D + 1.5L + S	1.00	4023 lb		12740 lb	7536 lb	Passed - 53%

CONNECTOR INFORMATION

ID	Part No	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories		
			Top	Face	Member			
1	HGUS410		-	-	-	Connector manually specified by the user.		

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	7'- 8"	Self Weight	Top	12 lb/ft	-	-	-
Uniform	0'	7'- 8"	User Load	Top	30 lb/ft	33 lb/ft	72 lb/ft	-
Uniform	-0'	2'- 2 13/16"	E69(i1768)	Top	100 lb/ft	-	-	-
Uniform	-0'	1'- 11 5/16"	E69(i1768)	Top	48 lb/ft	-	101 lb/ft	-
Uniform	2'- 2 13/16"	6'- 2 13/16"	E68(i1767)	Top	100 lb/ft	-	-	-
Uniform	2'- 8 15/16"	6'- 8 15/16"	Smoothed Load	Back	169 lb/ft	338 lb/ft	-	-
Uniform	6'- 2 13/16"	7'- 2 1/2"	E54(i1747)	Top	100 lb/ft	-	-	-
Uniform	6'- 6 5/16"	7'- 2 1/2"	E54(i1747)	Top	48 lb/ft	-	101 lb/ft	-
Point	0'- 8 15/16"	0'- 8 15/16"	J1(i7834)	Back	188 lb	376 lb	-	-
Point	2'- 15/16"	2'- 15/16"	J1(i7913)	Back	225 lb	451 lb	-	-
Point	2'- 1 13/16"	2'- 1 13/16"	E69(i1768)	Top	145 lb	-	233 lb	-
Point	6'- 3 13/16"	6'- 3 13/16"	E54(i1747)	Top	144 lb	-	230 lb	-
Point	7'- 5 1/4"	7'- 5 1/4"	E53(i1753)	Top	49 lb	10 lb	44 lb	-

UNFACTORED REACTIONS


ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B13(i7388)	1313 lb	1270 lb	647 lb	-
2	7'- 4 1/2"	7'- 8"	E41(i1740)	1282 lb	1173 lb	677 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of all plies. (Consult with manufacturer for guideline pertaining to this design option.)
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071106 PG 1/2

	BUILDER: ROYAL PINE HOMES SITE: FORESTSIDE ESTATES MODEL: 3804 CITY: BRAMPTON	Job Name: 3804 -ELEV A STD Level: 2ND FLOOR Label: B14 - I7857 Type: Beam	2 Ply Member 1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	Status: Design Passed
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PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.





BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 1ST FLOOR
Label: B1 - i7967
Type: Beam

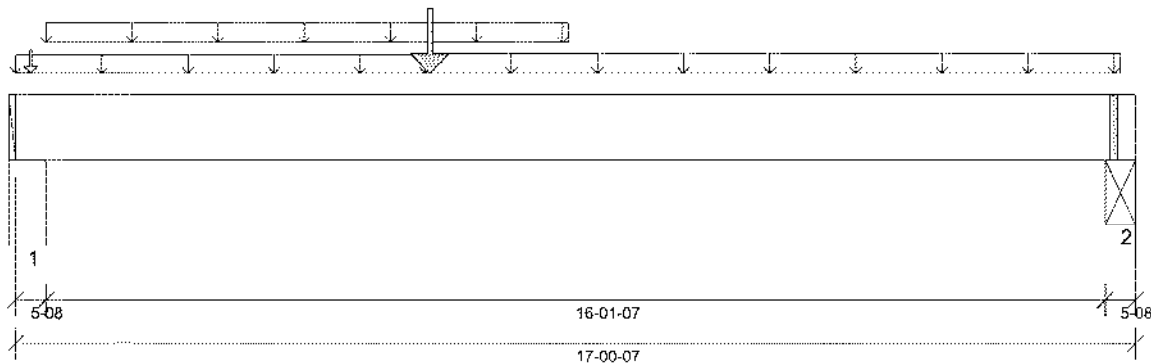
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 10'- 2 7/16"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Beam @ 16'- 7 15/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 3 5/8"	1.25D + 1.5L	0.98	11404 lb ft	17265 lb ft	Passed - 66%
Factored Neg. Moment:	0'- 4 1/2"	1.25D + 1.5L + S	0.98	42 lb ft	4399 lb ft	Passed - 1%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	0.98	2198 lb	6748 lb	Passed - 33%
Live Load (LL) Pos. Defl.:	8'- 2 3/16"	L		0.359"	L/360	Passed - L/539
Total Load (TL) Pos. Defl.:	8'- 1 3/8"	D + L		0.735"	L/240	Passed - L/263
Permanent Deflection:	8'- 5/8"			-	L/360	Passed - L/530

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L + S	0.98	2627 lb		9821 lb	5809 lb	Passed - 45%
2	5-08	1.25D + 1.5L	0.98	1697 lb		9779 lb	5783 lb	Passed - 29%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	17'- 7/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	6'- 2 3/4"	FC1 Floor Decking (Plan View Fill)	Top	14 lb/ft	28 lb/ft	-	-
Uniform	0'- 5 1/2"	8'- 5"	User Load	Top	60 lb/ft	-	-	-
Uniform	8'- 2 3/4"	16'- 9 11/16"	FC1 Floor Decking (Plan View Fill)	Top	24 lb/ft	48 lb/ft	-	-
Point	6'- 3 5/8"	6'- 3 5/8"	B3(i7960)	Front	634 lb	739 lb	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	E47(i1746)	Top	90 lb	58 lb	57 lb	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	W30(i35)	1061 lb	839 lb	58 lb	-
2	16'- 6 15/16"	17'- 7/16"	STL BM (i53)	581 lb	637 lb	-1 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071107



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 1ST FLOOR
Label: B2 - i7950
Type: Beam

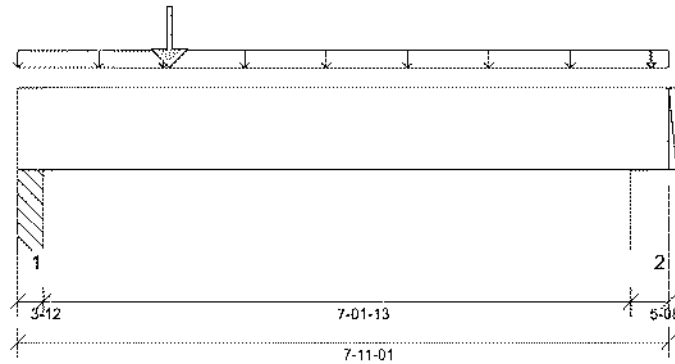
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 5'- 6 1/2"

Factored Resistance of Support Material:

- 615 psi Column @ 0'- 2 3/4"
- 615 psi Wall @ 7'- 6 9/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	1'- 10 3/16"	1.25D + 1.5L	1.00	3779 lb ft	17672 lb ft	Passed - 21%
Factored Shear:	1'- 3 5/8"	1.25D + 1.5L	1.00	2323 lb	6908 lb	Passed - 34%
Live Load (LL) Pos. Defl.:	3'- 5 7/8"	L		0.026"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 5 7/8"	D + L		0.043"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3'-12	1.25D + 1.5L	1.00	2401 lb		6826 lb	4036 lb	Passed - 59%
2	5'-08	1.25D + 1.5L	1.00	917 lb		10010 lb	5921 lb	Passed - 15%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	7'- 11 1/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	1'- 9 5/16"	FC1 Floor Decking (Plan View Fill)	Top	11 lb/ft	22 lb/ft	-	-
Uniform	1'- 9 5/16"	7'- 11 1/16"	FC1 Floor Decking (Plan View Fill)	Top	13 lb/ft	27 lb/ft	-	-
Point	1'- 10 3/16"	1'- 10 3/16"	B3(i7960)	Back	746 lb	1227 lb	-	-
Point	7'- 8 7/16"	7'- 8 7/16"	2(i1503)	Top	19 lb	14 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3 3/4"	PB01(i68)	663 lb	1068 lb	-	-
2	7'- 5 9/16"	7'- 11 1/16"	W35(i62)	254 lb	380 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071108



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 1ST FLOOR
Label: B3 - i7960
Type: Beam

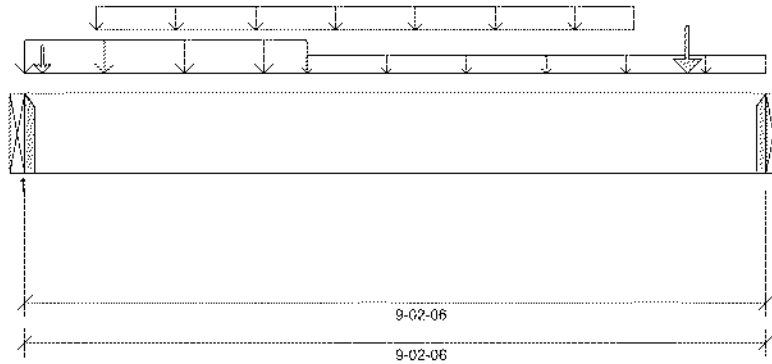
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'
- 615 psi Beam @ 9'- 2 3/8"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 2 5/8"	1.25D + 1.5L	1.00	5044 lb ft	17672 lb ft	Passed - 29%
Factored Shear:	0'- 11 7/8"	1.25D + 1.5L	1.00	2054 lb	6908 lb	Passed - 30%
Live Load (LL) Pos. Defl.:	4'- 5"	L		0.067"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 5 11/16"	D + L		0.117"	L/240	Passed - L/946

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	2770 lb		2770 lb	-	Passed - 100%
2	1-08	1.25D + 1.5L	1.00	1906 lb		2730 lb	-	Passed - 70%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories		
			Top	Face	Member			
1	HUS1.81/10		-	-	-	Connector manually specified by the user.		
2	HUS1.81/10		-	-	-	Connector manually specified by the user.		

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 2 3/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	3'- 6"	User Load	Back	120 lb/ft	240 lb/ft	-	-
Uniform	0'- 10 5/8"	7'- 6 5/8"	Smoothed Load	Front	59 lb/ft	118 lb/ft	-	-
Uniform	3'- 6"	9'- 2 3/8"	User Load	Top	60 lb/ft	-	-	-
Point	0'- 2 5/8"	0'- 2 5/8"	J6(7949)	Front	48 lb	96 lb	-	-
Point	8'- 2 5/8"	8'- 2 5/8"	J4(7497)	Front	123 lb	245 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B2(7950)	748 lb	1227 lb	-	-
2	9'- 2 3/8"	9'- 2 3/8"	B1(7967)	634 lb	739 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071109



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A STD
Level: 1ST FLOOR
Label: B4 - i7958
Type: Beam

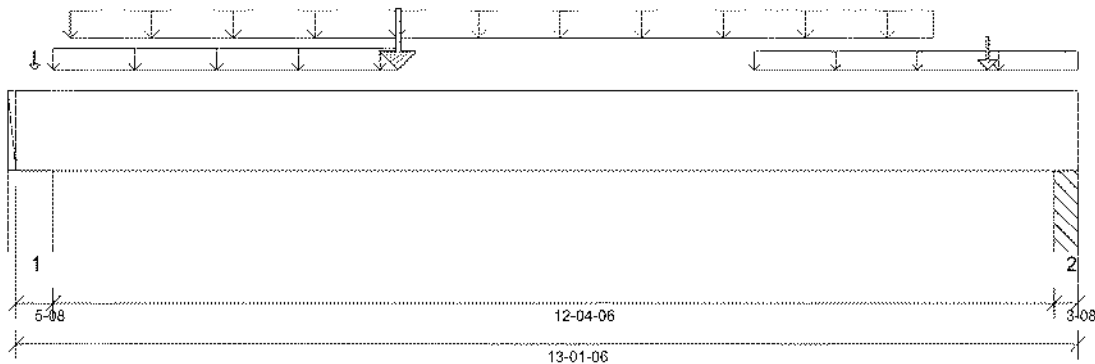
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:36



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Column @ 12'- 10 7/8"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 4"	1.25D + 1.5L	1.00	10282 lb ft	17672 lb ft	Passed - 58%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L + S	1.00	3161 lb	6908 lb	Passed - 46%
Live Load (LL) Pos. Defl.:	6'- 5 3/8"	L		0.271"	L/360	Passed - L/547
Total Load (TL) Pos. Defl.:	6'- 5 13/16"	D + L		0.427"	L/240	Passed - L/347
Permanent Deflection:	6'- 6 1/2"			-	L/360	Passed - L/979

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08	1.25D + 1.5L + S	1.00	3405 lb		10010 lb	5921 lb	Passed - 58%
2	3'-08	1.25D + 1.5L	1.00	2749 lb		6370 lb	3767 lb	Passed - 73%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	13'- 1 3/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'- 5 1/2"	4'- 8 5/8"	User Load	Top	40 lb/ft	80 lb/ft	-	-
Uniform	0'- 8"	11'- 4"	Smoothed Load	Front	78 lb/ft	157 lb/ft	-	-
Uniform	9'- 1 3/8"	13'- 1 3/8"	User Load	Top	60 lb/ft	-	-	-
Point	12'	12'	J5(i7955)	Front	94 lb	188 lb	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	E31(i1523)	Top	35 lb	7 lb	11 lb	-
Point	4'- 8 5/8"	4'- 8 5/8"	User Load	Top	240 lb	480 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	W31(i46)	874 lb	1540 lb	11 lb	-
2	12'- 9 7/8"	13'- 1 3/8"	PBO1(i68)	815 lb	1148 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071110



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A OPT SUNKEN
Level: 1ST FLOOR
Label: B64 - i8102
Type: Beam

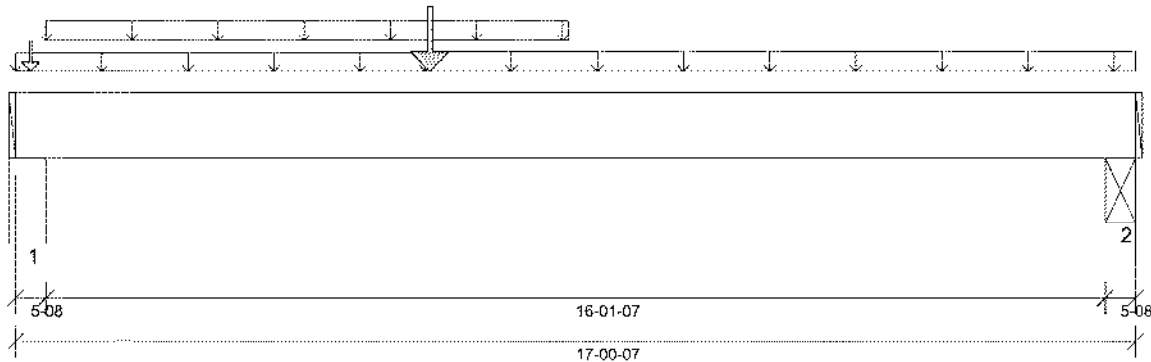
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:40



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 10'- 2 7/16"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Beam @ 16'- 7 15/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 3 5/8"	1.25D + 1.5L	0.95	7872 lb ft	16831 lb ft	Passed - 47%
Factored Neg. Moment:	0'- 4 1/2"	1.25D + 1.5L + S	0.96	42 lb ft	4399 lb ft	Passed - 1%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	0.95	1603 lb	6579 lb	Passed - 24%
Live Load (LL) Pos. Defl.:	8'- 3 7/8"	L		0.243"	L/360	Passed - L/796
Total Load (TL) Pos. Defl.:	8'- 2 3/8"	D + L		0.535"	L/240	Passed - L/361
Permanent Deflection:	8'- 1 3/16"			-	L/360	Passed - L/682

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5S + L	0.82	1767 lb		8241 lb	4875 lb	Passed - 36%
2	5-08	1.25D + 1.5L	0.95	1380 lb		9533 lb	5637 lb	Passed - 24%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	17'- 7/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	6'- 2 3/4"	FC1 Floor Decking (Plan View Fill)	Top	14 lb/ft	28 lb/ft	-	-
Uniform	0'- 5 1/2"	8'- 5"	User Load	Top	60 lb/ft	-	-	-
Uniform	8'- 2 3/4"	17'- 7/16"	FC1 Floor Decking (Plan View Fill)	Top	24 lb/ft	48 lb/ft	-	-
Point	6'- 3 5/8"	6'- 3 5/8"	B60(i6152)	Front	353 lb	349 lb	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	E47(i1746)	Top	90 lb	58 lb	57 lb	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	W30(i35)	880 lb	588 lb	58 lb	-
2	16'- 6 15/16"	17'- 7/16"	STL BM (i53)	486 lb	510 lb	-1 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071111



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A OPT SUNKEN
Level: 1ST FLOOR
Label: B63 - i8090
Type: Beam

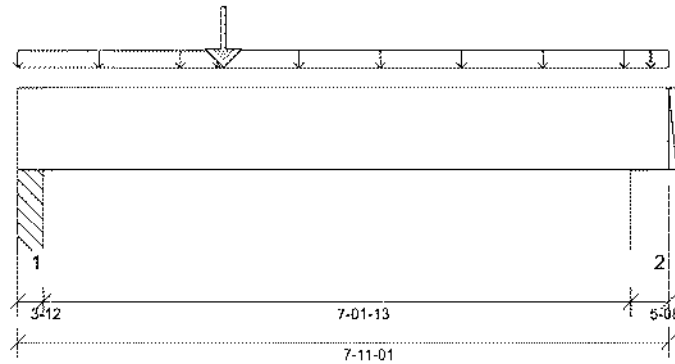
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:40



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 4'- 10 5/8"

Factored Resistance of Support Material:

- 615 psi Column @ 0'- 2 3/4"
- 615 psi Wall @ 7'- 6 9/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 6 1/16"	1.25D + 1.5L	1.00	2431 lb ft	17672 lb ft	Passed - 14%
Factored Shear:	1'- 3 5/8"	1.25D + 1.5L	1.00	1073 lb	6908 lb	Passed - 16%
Live Load (LL) Pos. Defl.:	3'- 7 3/16"	L		0.019"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 7 1/4"	D + L		0.029"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3'-12	1.25D + 1.5L	1.00	1151 lb		6826 lb	4036 lb	Passed - 29%
2	5'-08	1.25D + 1.5L	1.00	716 lb		10010 lb	5921 lb	Passed - 12%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	7'- 11 1/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	2'- 5 3/16"	FC1 Floor Decking (Plan View Fill)	Top	11 lb/ft	22 lb/ft	-	-
Uniform	2'- 5 3/16"	7'- 11 1/16"	FC1 Floor Decking (Plan View Fill)	Top	13 lb/ft	27 lb/ft	-	-
Point	2'- 6 1/16"	2'- 6 1/16"	B62(i8070)	Back	321 lb	619 lb	-	-
Point	7'- 8 7/16"	7'- 8 7/16"	2(i1503)	Top	19 lb	15 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3 3/4"	PBO1(i68)	296 lb	530 lb	-	-
2	7'- 5 9/16"	7'- 11 1/16"	W35(i52)	193 lb	308 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071112



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A OPT SUNKEN
Level: 1ST FLOOR
Label: B60 - i8152
Type: Beam

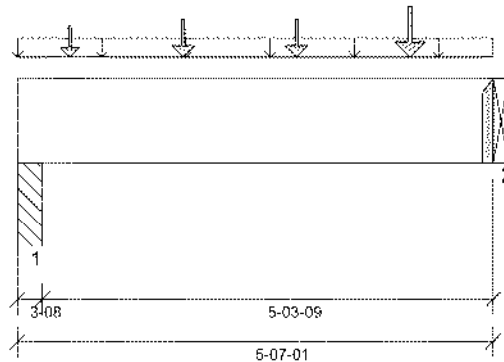
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:40



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Column @ 0'- 2 1/2"
- 615 psi Beam @ 5'- 7 1/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 3 3/8"	1.25D + 1.5L	0.99	1314 lb ft	17480 lb ft	Passed - 8%
Factored Shear:	1'- 3 3/8"	1.25D + 1.5L	0.99	580 lb	6832 lb	Passed - 8%
Total Load (TL) Pos. Defl.:	2'- 11 1/4"	D + L	0.010"	L/240		Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3'-08	1.25D + 1.5L	0.99	923 lb		6301 lb	3726 lb	Passed - 25%
2	1'-08	1.25D + 1.5L	0.99	965 lb		2700 lb	-	Passed - 36%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements	Other Information or Requirement for Reinforcement Accessories
2	HUS1.81/10	-	Top Face Member	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 7 1/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	5'- 7 1/16"	User Load	Top	60 lb/ft	-	-	-
Point	0'- 7 3/8"	0'- 7 3/8"	J7(i7970)	Front	56 lb	112 lb	-	-
Point	1'- 11 3/8"	1'- 11 3/8"	J7(i8075)	Front	79 lb	157 lb	-	-
Point	3'- 3 3/8"	3'- 3 3/8"	J7(i8071)	Front	79 lb	157 lb	-	-
Point	4'- 7 3/8"	4'- 7 3/8"	J5(i8153)	Front	123 lb	245 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3 1/2"	PBO2(i7663)	353 lb	322 lb	-	-
2	5'- 7 1/16"	5'- 7 1/16"	B64(i8102)	353 lb	349 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071113



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A OPT SUNKEN
Level: 1ST FLOOR
Label: B17 H - i7892
Type: Beam

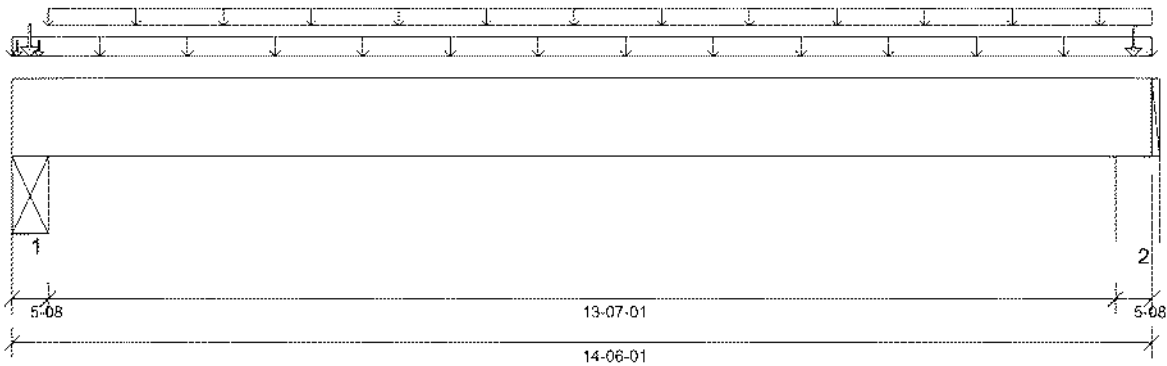
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:40



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 13'- 7 1/16"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'- 4 1/2"
- 615 psi Wall @ 14'- 1 9/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	7'- 3 3/16"	1.25D + 1.5L	0.68	2489 lb ft	11966 lb ft	Passed - 21%
Factored Neg. Moment:	0'- 4 1/2"	1.25D + 1.5L	0.68	40 lb ft	3304 lb ft	Passed - 1%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	0.68	620 lb	4677 lb	Passed - 13%
Live Load (LL) Pos. Defl.:	7'- 3 3/8"	L		0.019"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	7'- 3 1/16"	D + L		0.139"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	0.68	996 lb		6778 lb	4008 lb	Passed - 25%
2	5-08	1.25D + 1.5S + L	0.65	893 lb		6506 lb	3849 lb	Passed - 23%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	14'- 6 1/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	14'- 6 1/16"	User Load	Top	60 lb/ft	-	-	-
Uniform	0'- 5 1/2"	14'- 8 1/16"	FC1 Floor Decking (Plan View File)	Top	6 lb/ft	11 lb/ft	-	-
Point	0'- 3/4"	0'- 3/4"	FC1 Floor Decking (Plan View File)	Top	0 lb	1 lb	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	S(i1507)	Top	62 lb	100 lb	-	-
Point	0'- 4 1/8"	0'- 4 1/8"	FC1 Floor Decking (Plan View File)	Top	1 lb	2 lb	-	-
Point	14'- 3 5/16"	14'- 3 5/16"	E10(i1491)	Top	61 lb	-	57 lb	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	STL BM (i53)	582 lb	183 lb	-	-
2	14'- 9/16"	14'- 8 1/16"	W25(i31)	578 lb	79 lb	57 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071114



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A OPT SUNKEN
Level: 1ST FLOOR
Label: B61 - i7887
Type: Beam

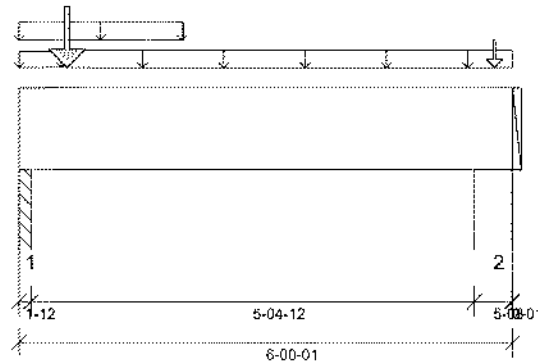
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:40



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0'- 1/16" Bottom: 4'- 10 5/8"

Factored Resistance of Support Material:

- 615 psi Column @ 0'- 3/4"
- 615 psi Wall @ 5'- 7 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	1'- 3 1/2"	1.25D + 1.5L	1.00	755 lb ft	17672 lb ft	Passed - 4%
Factored Neg. Moment:	5'- 7 1/2"	1.25D + 1.5L	1.00	57 lb ft	9192 lb ft	Passed - 1%
Factored Shear:	1'- 1 5/8"	1.25D + 1.5L	1.00	1272 lb	6908 lb	Passed - 18%

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1'-12"	1.25D + 1.5L	1.00	1418 lb		3185 lb	1883 lb	Passed - 75%
2	5'-08"	1.25D + 1.5L	1.00	685 lb		10010 lb	5921 lb	Passed - 12%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	6'- 1/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	2'	User Load	Top	60 lb/ft	-	-	-
Uniform	0'	0'- 6 1/8"	FC1 Floor Decking (Plan View F1b)	Top	6 lb/ft	12 lb/ft	-	-
Uniform	0'- 6 1/8"	6'	FC1 Floor Decking (Plan View F1b)	Top	13 lb/ft	27 lb/ft	-	-
Point	0'- 7"	0'- 7"	B62(48070)	Front	302 lb	582 lb	-	-
Point	5'- 9 7/16"	5'- 9 7/16"	2(i1503)	Top	120 lb	120 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 1 3/4"	PBO2(i7663)	425 lb	599 lb	-	-
2	5'- 6 1/2"	6'	W35(i52)	230 lb	258 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071115



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A OPT SUNKEN
Level: 1ST FLOOR
Label: B62 - i8070
Type: Beam

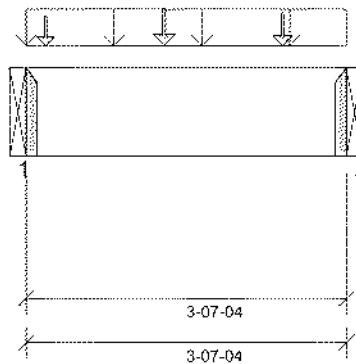
1 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:40



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'
- 615 psi Beam @ 3'- 7 1/4"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	1'- 7 1/2"	1.25D + 1.5L	1.00	1185 lb ft	17672 lb ft	Passed - 7%
Factored Shear:	0'- 11 7/8"	1.25D + 1.5L	1.00	626 lb	6908 lb	Passed - 9%

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	1319 lb		2730 lb	-	Passed - 48%
2	1-08	1.25D + 1.5L	1.00	1261 lb		2730 lb	-	Passed - 46%

CONNECTOR INFORMATION

ID	Part No	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement/Accessories		
			Top	Face	Member			
1	HUS1.81/10		-	-	-	Connector manually specified by the user.		
2	HUS1.81/10		-	-	-	Connector manually specified by the user.		

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	3'- 7 1/4"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	-0'	3'- 7 1/4"	User Load	Back	120 lb/ft	240 lb/ft	-	-
Point	0'- 2 5/8"	0'- 2 5/8"	J7(7967)	Front	43 lb	85 lb	-	-
Point	1'- 6 5/8"	1'- 6 5/8"	J7(8109)	Front	70 lb	140 lb	-	-
Point	2'- 10 5/8"	2'- 10 5/8"	J7(8072)	Front	56 lb	111 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B63(i8090)	321 lb	619 lb	-	-
2	3'- 7 1/4"	3'- 7 1/4"	B61(i7887)	302 lb	562 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071116



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A OPT SUNKEN
Level: 1ST FLOOR
Label: B30 L - i7732
Type: Beam

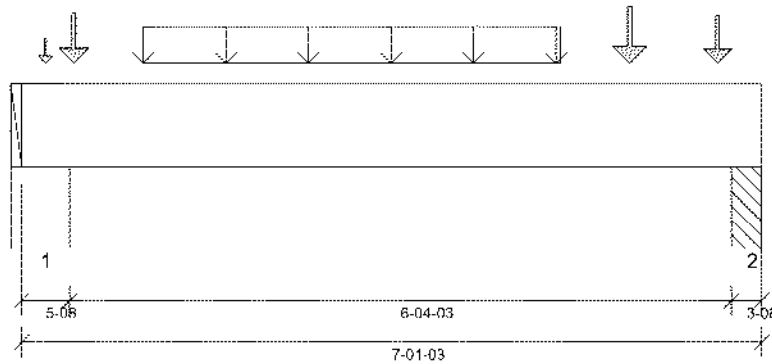
1 Ply Member
1 3/4" x 9 1/2" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:40



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Column @ 6'- 10 11/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 2"	1.25D + 1.5L	1.00	3043 lb ft	11650 lb ft	Passed - 26%
Factored Shear:	6'- 3/16"	1.25D + 1.5L	1.00	2008 lb	5526 lb	Passed - 36%
Live Load (LL) Pos. Defl.:	3'- 7 5/8"	L		0.044"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 7 5/8"	D + L		0.067"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-0"	1.25D + 1.5L	1.00	2116 lb		10010 lb	5921 lb	Passed - 36%
2	3'-0"	1.25D + 1.5L	1.00	2014 lb		6370 lb	3767 lb	Passed - 53%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	7'- 1 3/16"	Self Weight	Top	5 lb/ft	-	-	-
Uniform	1'- 2"	5'- 2"	Smoothed Load	Front	136 lb/ft	272 lb/ft	-	-
Point	0'- 6"	0'- 6"	J1(i7777)	Front	125 lb	249 lb	-	-
Point	5'- 10"	5'- 10"	J1(i7781)	Front	148 lb	296 lb	-	-
Point	6'- 8 1/8"	6'- 8 1/8"	J1(i7782)	Front	115 lb	229 lb	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	1(i1504)	Top	42 lb	55 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	W34(i50)	521 lb	976 lb	-	-
2	6'- 9 11/16"	7'- 1 3/16"	PBO4(i7735)	486 lb	939 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071117



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV A OPT SUNKEN
Level: 1ST FLOOR
Label: B31 L - i7733
Type: Beam

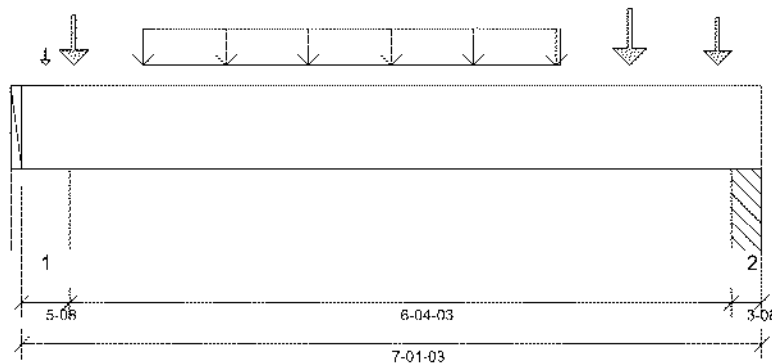
1 Ply Member
1 3/4" x 9 1/2" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:41



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Column @ 6'- 10 11/16"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 2"	1.25D + 1.5L	1.00	3052 lb ft	11650 lb ft	Passed - 26%
Factored Shear:	6'- 3/16"	1.25D + 1.5L	1.00	2011 lb	5526 lb	Passed - 36%
Live Load (LL) Pos. Defl.:	3'- 7 5/8"	L		0.044"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 7 5/8"	D + L		0.067"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08	1.25D + 1.5L	1.00	2002 lb		10010 lb	5921 lb	Passed - 34%
2	3'-08	1.25D + 1.5L	1.00	2017 lb		6370 lb	3767 lb	Passed - 54%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	7'- 1 3/16"	Self Weight	Top	5 lb/ft	-	-	-
Uniform	1'- 2"	5'- 2"	Smoothed Load	Back	136 lb/ft	272 lb/ft	-	-
Point	0'- 6"	0'- 6"	J1(i7777)	Back	125 lb	249 lb	-	-
Point	5'- 10"	5'- 10"	J1(i7781)	Back	148 lb	296 lb	-	-
Point	6'- 8 1/8"	6'- 8 1/8"	J1(i7782)	Back	115 lb	229 lb	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	1(i1504)	Top	16 lb	2 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	W34(i50)	494 lb	920 lb	-	-
2	6'- 9 11/16"	7'- 1 3/16"	PBO3(i7734)	487 lb	942 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
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- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



STRUCTURAL COMPONENT ONLY
DWG # TF22071118



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV B STD
Level: 2ND FLOOR
Label: B21 - i7690
Type: Beam

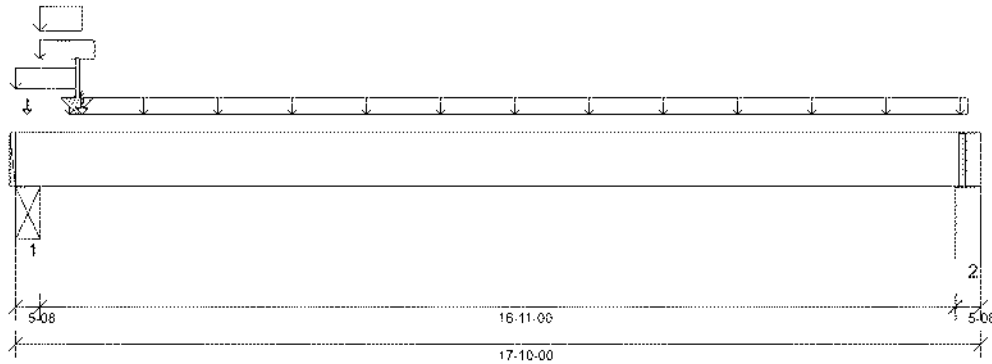
2 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:51



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 16'- 1"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'- 4 1/2"
- 615 psi Wall @ 17'- 5 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 7/16"	1.25D + 1.5L + S	1.00	6009 lb ft	35345 lb ft	Passed - 17%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	0.96	4795 lb	13243 lb	Passed - 36%
Live Load (LL) Pos. Defl.:	8'- 4 5/8"	L + 0.5S		0.128"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	8'- 4 7/16"	D + L + 0.5S		0.238"	L/240	Passed - L/853

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Upward Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08"	1.25D + 1.5L + S	1.00	6771 lb		20020 lb	11839 lb	Passed - 57%
2	5'-08"	1.25D + 1.5L + S	1.00	1069 lb		20020 lb	11843 lb	Passed - 9%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	17'- 10"	Self Weight	Top	12 lb/ft	-	-	-
Uniform	0'	1'- 1 3/4"	User Load	Top	30 lb/ft	33 lb/ft	72 lb/ft	-
Uniform	0'- 5 5/16"	1'- 5 1/2"	E93(i6999)	Top	100 lb/ft	-	-	-
Uniform	0'- 5 5/16"	1'- 2 3/4"	E93(i6999)	Top	77 lb/ft	-	151 lb/ft	-
Uniform	1'	17'- 7 1/4"	FC2 Floor Decking (Plan View Fill)	Top	18 lb/ft	36 lb/ft	-	-
Point	1'- 1 3/4"	1'- 1 3/4"	B20(i7609)	Back	1502 lb	1452 lb	713 lb	-
Point	0'- 2 5/8"	0'- 2 5/8"	E92(i7004)	Top	73 lb	10 lb	90 lb	-
Point	1'- 2 3/4"	1'- 2 3/4"	E93(i6999)	Top	262 lb	-	496 lb	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	STL BM (i1532)	2217 lb	1738 lb	1455 lb	-
2	17'- 4 1/2"	17'- 10"	5(i1504)	330 lb	367 lb	45 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of all plies. (Consult with manufacturer for guideline pertaining to this design option.)
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=3.500", W=3.500". LDF=1.00, Pf=4769 lb, Qr=10920 lb, Result=43.67%.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



STRUCTURAL COMPONENT ONLY
DWG # TF22071119



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV B STD
Level: 2ND FLOOR
Label: B20 - i7609
Type: Beam

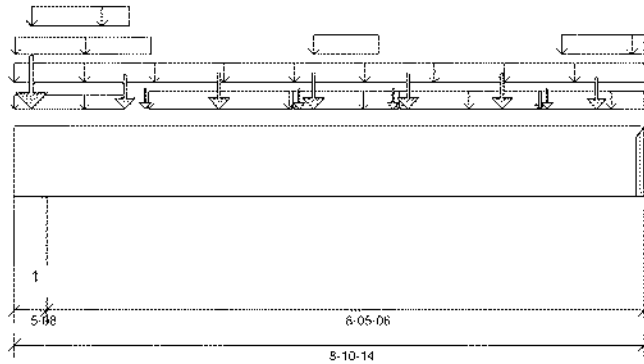
2 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:51



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Beam @ 8'- 10 7/8"

PLY TO PLY CONNECTION:
4 ROWS OF 3.25" PNEUMATIC GUN
NAILS (0.120"x3.25") @ 8" O/C

PLY TO PLY CONNECTION ASSUMES ANY
SUPPORTED BEAM HANGERS ARE FASTENED
TO THIS BEAM WITH MIN. 3.5" FASTENERS.



STRUCTURAL COMPONENT ONLY
DWG # TF22071120 PG 1/2

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 3 5/8"	1.25D + 1.5L + S	1.00	10647 lb ft	35345 lb ft	Passed - 30%
Factored Neg. Moment:	0'- 4 1/2"	1.25D + 1.5S + L	1.00	218 lb ft	35345 lb ft	Passed - 1%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L + S	1.00	4186 lb	13815 lb	Passed - 30%
Live Load (LL) Pos. Defl.:	4'- 7 3/4"	L + 0.5S		0.056"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 7 3/4"	D + L + 0.5S		0.101"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L + S	1.00	6265 lb		20020 lb	11842 lb	Passed - 53%
2	1-08	1.25D + 1.5L + S	1.00	4854 lb		5460 lb	-	Passed - 89%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements	Other Information or Requirement for Reinforcement Accessories
2	HGUS410		Top Face Member	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 10 7/8"	Self Weight	Top	12 lb/ft	-	-	-
Uniform	0'	8'- 10 7/8"	User Load	Top	30 lb/ft	33 lb/ft	72 lb/ft	-
Uniform	-0'	1'- 6 3/4"	FC2 Floor Decking (Plan View File)	Top	3 lb/ft	6 lb/ft	-	-
Uniform	0'- 3/16"	1'- 11 3/16"	E98(i6998)	Top	100 lb/ft	-	-	-
Uniform	0'- 2 15/16"	1'- 7 1/2"	E98(i6998)	Top	48 lb/ft	-	101 lb/ft	-
Uniform	1'- 10 13/16"	3'- 11 5/16"	E97(i7009)	Top	100 lb/ft	-	-	-
Uniform	3'- 11 5/16"	5'- 5 1/4"	E96(i7001)	Top	100 lb/ft	-	-	-
Uniform	4'- 2 13/16"	5'- 1 3/4"	E96(i7001)	Top	48 lb/ft	-	101 lb/ft	-
Uniform	5'- 5 1/4"	7'- 5 5/16"	E95(i7002)	Top	100 lb/ft	-	-	-
Uniform	7'- 5 5/16"	8'- 10 7/8"	E94(i7003)	Top	100 lb/ft	-	-	-
Uniform	7'- 8 13/16"	8'- 10 7/8"	E94(i7003)	Top	48 lb/ft	-	101 lb/ft	-
Point	1'- 6 3/4"	1'- 6 3/4"	J1(i7599)	Back	212 lb	425 lb	-	-
Point	2'- 10 3/4"	2'- 10 3/4"	J1(i7733)	Back	225 lb	451 lb	-	-
Point	4'- 2 3/4"	4'- 2 3/4"	J1(i7613)	Back	225 lb	451 lb	-	-
Point	5'- 6 3/4"	5'- 6 3/4"	J1(i7752)	Back	225 lb	451 lb	-	-
Point	6'- 10 3/4"	6'- 10 3/4"	J1(i7709)	Back	225 lb	451 lb	-	-
Point	8'- 2 3/4"	8'- 2 3/4"	J1(i7639)	Back	182 lb	364 lb	-	-
Point	0'- 2 15/16"	0'- 2 15/16"	E98(i6998)	Top	251 lb	406 lb	513 lb	-
Point	1'- 10 3/16"	1'- 10 3/16"	E98(i6998)	Top	84 lb	-	136 lb	-
Point	4'- 5/16"	4'- 5/16"	E96(i7001)	Top	80 lb	-	128 lb	-
Point	5'- 4 1/4"	5'- 4 1/4"	E96(i7001)	Top	83 lb	-	134 lb	-
Point	7'- 6 5/16"	7'- 6 5/16"	E94(i7003)	Top	80 lb	-	128 lb	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	2(i1503)	1807 lb	1850 lb	1317 lb	-
2	8'- 10 7/8"	8'- 10 7/8"	B21(i7690)	1502 lb	1452 lb	713 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of all plies. (Consult with manufacturer for guideline pertaining to this design option.)



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV B STD
Level: 2ND FLOOR
Label: B20 - i7609
Type: Beam

2 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

DESIGN NOTES

- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.





BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV C STD
Level: 2ND FLOOR
Label: B50 - i9821
Type: Beam

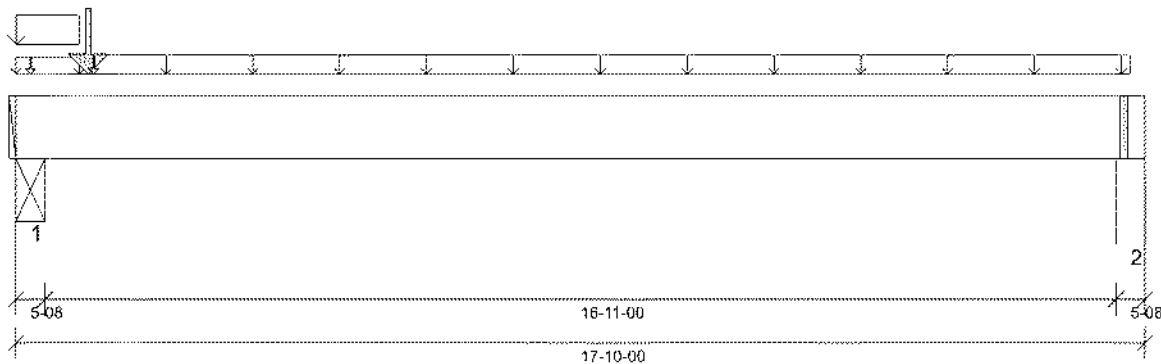
2 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:58



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 16'- 1"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'- 4 1/2"
- 615 psi Wall @ 17'- 5 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 11 7/16"	1.25D + 1.5L + S	1.00	3940 lb ft	35238 lb ft	Passed - 11%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	0.95	2922 lb	13103 lb	Passed - 22%
Live Load (LL) Pos. Defl.:	8'- 5 15/16"	L + 0.5S		0.082"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	8'- 5 7/8"	D + L + 0.5S		0.160"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L + S	1.00	3845 lb		19959 lb	11803 lb	Passed - 33%
2	5-08	1.25D + 1.5L	0.95	739 lb		18988 lb	11232 lb	Passed - 7%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	17'- 10"	Self Weight	Top	12 lb/ft	-	-	-
Uniform	0'	1'	FC2 Floor Decking (Plan View Fill)	Top	4 lb/ft	8 lb/ft	-	-
Uniform	0'- 1/16"	1'	E98(i8478)	Top	130 lb/ft	33 lb/ft	72 lb/ft	-
Uniform	1'	17'- 7 1/4"	FC2 Floor Decking (Plan View Fill)	Top	13 lb/ft	27 lb/ft	-	-
Point	1'- 1 3/4"	1'- 1 3/4"	B51(i9844)	Front	1005 lb	794 lb	510 lb	-
Point	0'- 2 13/16"	0'- 2 13/16"	E98(i8478)	Top	11 lb	-	23 lb	-
Point	1'- 2 3/4"	1'- 2 3/4"	E97(i8476)	Top	48 lb	6 lb	39 lb	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	STL BM (i1532)	1371 lb	1026 lb	624 lb	-
2	17'- 4 1/2"	17'- 10"	1(i1504)	259 lb	259 lb	20 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of all plies. (Consult with manufacturer for guideline pertaining to this design option.)
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=3.500", W=3.500". LDF=1.00, Pf=2968 lb, Qr=10920 lb, Result=27.18%.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

PLY TO PLY CONNECTION:

4 ROWS OF 3.25" PNEUMATIC GUN
NAILS (0.120"x3.25") @ 12" O/C

PLY TO PLY CONNECTION ASSUMES ANY
SUPPORTED BEAM HANGERS ARE FASTENED
TO THIS BEAM WITH MIN. 3.5" FASTENERS.



STRUCTURAL COMPONENT ONLY
DWG # TF22071121



BUILDER: ROYAL PINE HOMES
SITE: FORESTSIDE ESTATES
MODEL: 3804
CITY: BRAMPTON

Job Name: 3804 -ELEV C STD
Level: 2ND FLOOR
Label: B51 - i9844
Type: Beam

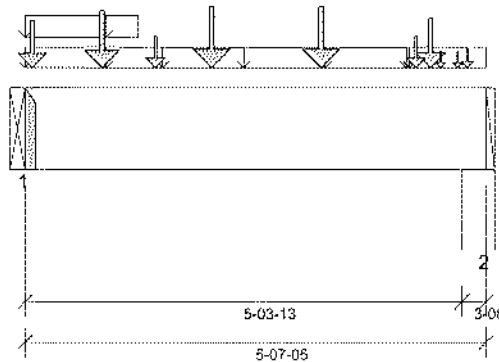
2 Ply Member
1 3/4" x 11 7/8" (2.0E 3100)
WestFraser LVL

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.5.3.233.Update5.15

Report Version: 2021.03.26 07/15/2022 10:58



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,
ABC 2019, OBC 2012 (2019
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240.

Lateral Restraint Requirements:

Both ends of the member and the outer supports
must be laterally restrained. Top and bottom edges
of the member must be fully restrained or have the
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Beam @ 0'
- 615 psi Wall @ 5'- 4 13/16"

PLY TO PLY CONNECTION:
4 ROWS OF 3.25" PNEUMATIC GUN
NAILS (0.120"x3.25") @ 6" O/C

PLY TO PLY CONNECTION ASSUMES ANY
SUPPORTED BEAM HANGERS ARE FASTENED
TO THIS BEAM WITH MIN. 3.5" FASTENERS.

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 3 1/4"	1.25D + 1.5L + S	1.00	3514 lb ft	35345 lb ft	Passed - 10%
Factored Shear:	4'- 3 15/16"	1.25D + 1.5L + S	1.00	2305 lb	13815 lb	Passed - 17%
Total Load (TL) Pos. Defl.:	2'- 8 3/16"	D + L + 0.5S		0.014"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L + S	1.00	2916 lb		5460 lb	-	Passed - 53%
2	3-08	1.25D + 1.5L + S	1.00	2723 lb		12740 lb	7536 lb	Passed - 36%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements	Other Information or Requirement for Reinforcement Accessories
1	HGUS410		Top Face Member	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 7 5/16"	Self Weight	Top	12 lb/ft	-	-	-
Uniform	0'	1'- 8 1/16"	E97(i8476)	Top	100 lb/ft	-	-	-
Uniform	0'	1'- 4 9/16"	E97(i8476)	Top	42 lb/ft	-	78 lb/ft	-
Uniform	1'- 8 1/16"	4'- 8 1/8"	E96(i8477)	Top	100 lb/ft	-	-	-
Uniform	4'- 8 1/8"	5'- 7 5/16"	E95(i8486)	Top	100 lb/ft	-	-	-
Point	0'- 11 1/4"	0'- 11 1/4"	J1(i9845)	Back	204 lb	408 lb	-	-
Point	2'- 3 1/4"	2'- 3 1/4"	J1(i9613)	Back	225 lb	451 lb	-	-
Point	3'- 7 1/4"	3'- 7 1/4"	J1(i9613)	Back	225 lb	451 lb	-	-
Point	4'- 11 1/4"	4'- 11 1/4"	J1(i9648)	Back	169 lb	339 lb	-	-
Point	0'- 1 1/16"	0'- 1 1/16"	E97(i8476)	Top	171 lb	-	300 lb	-
Point	1'- 7 1/16"	1'- 7 1/16"	E97(i8476)	Top	103 lb	-	141 lb	-
Point	4'- 9 1/8"	4'- 9 1/8"	E95(i8486)	Top	101 lb	-	138 lb	-
Point	5'- 11/16"	5'- 11/16"	E95(i8486)	Top	8 lb	-	14 lb	-
Point	5'- 3 3/16"	5'- 3 3/16"	E95(i8486)	Top	5 lb	-	9 lb	-
Point	5'- 4 9/16"	5'- 4 9/16"	E53(i1753)	Top	42 lb	-	32 lb	-


UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B50(i9821)	1005 lb	794 lb	510 lb	-
2	5'- 3 13/16"	5'- 7 5/16"	E41(i1740)	934 lb	855 lb	232 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of all plies. (Consult with manufacturer for guideline pertaining to this design option.)
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



	BUILDER: ROYAL PINE HOMES SITE: FORESTSIDE ESTATES MODEL: 3804 CITY: BRAMPTON	Job Name: 3804 -ELEV C STD Level: 2ND FLOOR Label: B51 - I9844 Type: Beam	2 Ply Member 1 3/4" x 11 7/8" (2.0E 3100) WestFraser LVL	Status: Design Passed
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PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



Maximum Floor Spans – S2.1

Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 15 psf
Deflection limits:	L/480 under live load and L/240 under total load
Sheathing:	5/8 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

Joist depth	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-1"	14'-3"	13'-10"	-	15'-7"	14'-9"	14'-3"	-
	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-
	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	-
11-7/8"	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	-
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	-
	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	-
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	-
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	-
14"	NI-40x	20'-1"	18'-8"	17'-10"	-	20'-10"	19'-4"	18'-6"	-
	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	-
	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	-
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	-
16"	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	-
	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	-
	NI-90	24'-5"	22'-6"	21'-6"	-	25'-1"	23'-2"	22'-2"	-

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-8"	15'-3"	14'-5"	-	16'-8"	15'-3"	14'-5"	-
	NI-40x	17'-11"	17'-0"	16'-1"	-	18'-5"	17'-1"	16'-1"	-
	NI-60	18'-2"	17'-1"	16'-4"	-	18'-8"	17'-4"	16'-4"	-
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	-
11-7/8"	NI-20	19'-7"	18'-2"	17'-3"	-	19'-11"	18'-3"	17'-3"	-
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-2"	-
	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-
	NI-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-
14"	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	21'-8"	-
	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-
	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	-
16"	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-
	NI-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-
	NI-90	28'-8"	26'-6"	25'-3"	-	29'-3"	27'-2"	25'-11"	-

Notes:

1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans – S4.1

Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 15 psf
Deflection limits:	L/480 under live load and L/240 under total load
Sheathing:	3/4 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

Joist depth	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	15'-2"
	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
11-7/8"	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-2"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
14"	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
16"	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10"
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
11-7/8"	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"
	NI-60	22'-1"	20'-7"	19'-8"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-0"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-7"
14"	NI-40x	24'-5"	22'-9"	21'-9"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
16"	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	24'-9"	23'-1"
	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

Notes:

1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans – S6.1

Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 15 psf
Deflection limits:	L/480 under live load and L/240 under total load
Sheathing:	5/8 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

Joist depth	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	14'-11"	14'-1"	13'-7"	-	15'-4"	14'-6"	14'-1"	-
	NI-40x	15'-11"	15'-0"	14'-6"	-	16'-4"	15'-5"	14'-11"	-
	NI-60	16'-1"	15'-2"	14'-8"	-	16'-6"	15'-7"	15'-1"	-
	NI-80	17'-1"	16'-1"	15'-6"	-	17'-5"	16'-5"	15'-10"	-
11-7/8"	NI-20	16'-9"	15'-10"	15'-4"	-	17'-4"	16'-4"	15'-10"	-
	NI-40x	17'-10"	16'-10"	16'-3"	-	18'-6"	17'-4"	16'-9"	-
	NI-60	18'-1"	17'-0"	16'-5"	-	18'-9"	17'-6"	16'-11"	-
	NI-80	19'-6"	18'-0"	17'-4"	-	20'-1"	18'-7"	17'-9"	-
	NI-90	19'-11"	18'-4"	17'-8"	-	20'-5"	18'-11"	18'-1"	-
14"	NI-40x	19'-10"	18'-4"	17'-8"	-	20'-6"	19'-1"	18'-3"	-
	NI-60	20'-2"	18'-8"	17'-11"	-	20'-10"	19'-4"	18'-6"	-
	NI-80	21'-8"	20'-0"	19'-1"	-	22'-4"	20'-8"	19'-9"	-
	NI-90	22'-1"	20'-5"	19'-6"	-	22'-9"	21'-0"	20'-1"	-
16"	NI-60	22'-0"	20'-4"	19'-6"	-	22'-9"	21'-1"	20'-2"	-
	NI-80	23'-7"	21'-10"	20'-10"	-	24'-4"	22'-6"	21'-6"	-
	NI-90	24'-1"	22'-2"	21'-2"	-	24'-9"	22'-11"	21'-10"	-

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-6"	15'-1"	14'-3"	-	16'-6"	15'-1"	14'-3"	-
	NI-40x	17'-9"	16'-10"	15'-11"	-	18'-2"	16'-11"	15'-11"	-
	NI-60	17'-11"	16'-11"	16'-2"	-	18'-5"	17'-2"	16'-2"	-
	NI-80	19'-3"	17'-10"	17'-3"	-	19'-8"	18'-3"	17'-7"	-
11-7/8"	NI-20	19'-4"	18'-0"	17'-1"	-	19'-9"	18'-1"	17'-1"	-
	NI-40x	20'-10"	19'-4"	18'-6"	-	21'-5"	19'-11"	19'-0"	-
	NI-60	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-3"	-
	NI-80	22'-6"	20'-10"	19'-11"	-	23'-1"	21'-5"	20'-5"	-
	NI-90	23'-0"	21'-3"	20'-4"	-	23'-6"	21'-10"	20'-10"	-
14"	NI-40x	23'-5"	21'-8"	20'-9"	-	24'-0"	22'-5"	21'-5"	-
	NI-60	23'-9"	22'-0"	21'-0"	-	24'-5"	22'-8"	21'-8"	-
	NI-80	25'-4"	23'-6"	22'-5"	-	25'-11"	24'-1"	23'-0"	-
	NI-90	25'-10"	23'-11"	22'-9"	-	26'-5"	24'-6"	23'-4"	-
16"	NI-60	26'-2"	24'-3"	23'-2"	-	26'-11"	25'-0"	23'-11"	-
	NI-80	27'-11"	25'-10"	24'-7"	-	28'-7"	26'-6"	25'-3"	-
	NI-90	28'-5"	26'-3"	25'-0"	-	29'-0"	26'-11"	25'-8"	-

Notes:

1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans – S7.1

Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 15 psf
Deflection limits:	L/480 under live load and L/240 under total load
Sheathing:	3/4 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

Joist depth	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	16'-11"	15'-11"	15'-4"	14'-9"	17'-4"	16'-4"	15'-9"	15'-1"
	NI-60	17'-1"	16'-1"	15'-6"	14'-10"	17'-6"	16'-6"	15'-11"	15'-3"
	NI-80	18'-1"	17'-0"	16'-4"	15'-8"	18'-7"	17'-4"	16'-8"	16'-0"
11-7/8"	NI-20	17'-10"	16'-10"	16'-2"	15'-7"	18'-5"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-3"	17'-10"	17'-2"	16'-6"	19'-10"	18'-5"	17'-8"	16'-11"
	NI-60	19'-6"	18'-1"	17'-4"	16'-8"	20'-1"	18'-8"	17'-10"	17'-1"
	NI-80	20'-11"	19'-4"	18'-5"	17'-7"	21'-5"	19'-10"	18'-11"	17'-11"
	NI-90	21'-4"	19'-9"	18'-9"	17'-10"	21'-10"	20'-3"	19'-3"	18'-3"
14"	NI-40x	21'-4"	19'-9"	18'-10"	17'-11"	22'-0"	20'-5"	19'-6"	18'-6"
	NI-60	21'-8"	20'-1"	19'-2"	18'-2"	22'-4"	20'-9"	19'-9"	18'-9"
	NI-80	23'-3"	21'-6"	20'-5"	19'-4"	23'-10"	22'-1"	21'-0"	19'-11"
	NI-90	23'-9"	21'-11"	20'-10"	19'-8"	24'-3"	22'-6"	21'-5"	20'-3"
16"	NI-60	23'-7"	21'-10"	20'-10"	19'-9"	24'-4"	22'-7"	21'-7"	20'-5"
	NI-80	25'-4"	23'-5"	22'-3"	21'-1"	26'-0"	24'-1"	22'-11"	21'-8"
	NI-90	25'-10"	23'-10"	22'-8"	21'-5"	26'-5"	24'-6"	23'-4"	22'-0"

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-7"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
	NI-60	18'-10"	17'-6"	16'-6"	15'-5"	19'-1"	17'-6"	16'-6"	15'-5"
	NI-80	20'-2"	18'-9"	17'-11"	16'-10"	20'-7"	19'-2"	18'-2"	16'-10"
11-7/8"	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-9"	20'-3"	19'-4"	17'-8"	22'-4"	20'-5"	19'-4"	17'-8"
	NI-60	22'-0"	20'-6"	19'-7"	18'-4"	22'-7"	20'-10"	19'-8"	18'-4"
	NI-80	23'-6"	21'-10"	20'-10"	19'-9"	24'-0"	22'-5"	21'-4"	20'-0"
	NI-90	24'-0"	22'-4"	21'-3"	20'-1"	24'-6"	22'-10"	21'-9"	20'-7"
14"	NI-40x	24'-4"	22'-8"	21'-8"	19'-5"	25'-0"	23'-2"	21'-9"	19'-5"
	NI-60	24'-9"	23'-0"	22'-0"	20'-9"	25'-5"	23'-8"	22'-4"	20'-10"
	NI-80	26'-5"	24'-6"	23'-4"	22'-1"	27'-0"	25'-2"	24'-0"	22'-8"
	NI-90	26'-11"	25'-0"	23'-10"	22'-6"	27'-5"	25'-7"	24'-5"	23'-1"
16"	NI-60	27'-2"	25'-4"	24'-2"	22'-10"	27'-11"	26'-1"	24'-9"	23'-1"
	NI-80	29'-0"	26'-11"	25'-8"	24'-3"	29'-7"	27'-7"	26'-4"	24'-11"
	NI-90	29'-6"	27'-5"	26'-1"	24'-8"	30'-1"	28'-1"	26'-9"	25'-4"

Notes:

1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans – M2.1

Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 20 psf
Deflection limits:	L/480 under live load and L/240 under total load
Sheathing:	5/8 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

Joist depth	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-1"	14'-3"	13'-10"	-	15'-7"	14'-9"	14'-3"	-
	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-
	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	-
11-7/8"	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	-
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	-
	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	-
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	-
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	-
14"	NI-40x	20'-1"	18'-8"	17'-10"	-	20'-10"	19'-4"	18'-6"	-
	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	-
	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	-
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	-
16"	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	-
	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	-
	NI-90	24'-5"	22'-6"	21'-6"	-	25'-1"	23'-2"	22'-2"	-

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-8"	15'-3"	14'-5"	-	16'-8"	15'-3"	14'-5"	-
	NI-40x	17'-11"	17'-0"	16'-1"	-	18'-5"	17'-1"	16'-1"	-
	NI-60	18'-2"	17'-1"	16'-4"	-	18'-8"	17'-4"	16'-4"	-
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	-
11-7/8"	NI-20	19'-7"	18'-2"	17'-3"	-	19'-11"	18'-3"	17'-3"	-
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-0"	-
	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-
	NI-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-
14"	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	20'-11"	-
	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-
	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	-
16"	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-
	NI-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-
	NI-90	28'-8"	26'-6"	25'-3"	-	29'-3"	27'-2"	25'-11"	-

Notes:

1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans – M4.1

Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 20 psf
Deflection limits:	L/480 under live load and L/240 under total load
Sheathing:	3/4 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

Joist depth	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	14'-11"
	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
11-7/8"	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
14"	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
16"	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10"
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	14'-11"	18'-10"	17'-2"	16'-3"	14'-11"
	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
11-7/8"	NI-20	20'-1"	18'-5"	17'-5"	16'-1"	20'-1"	18'-5"	17'-5"	16'-1"
	NI-40x	21'-10"	20'-4"	19'-0"	17'-0"	22'-5"	20'-6"	19'-0"	17'-0"
	NI-60	22'-1"	20'-7"	19'-8"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-0"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-7"
14"	NI-40x	24'-5"	22'-9"	20'-11"	18'-8"	25'-1"	22'-11"	20'-11"	18'-8"
	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
16"	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	24'-9"	23'-1"
	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

Notes:

1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans – M6.1

Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 20 psf
Deflection limits:	L/480 under live load and L/240 under total load
Sheathing:	5/8 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

Joist depth	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	14'-11"	14'-1"	13'-7"	-	15'-4"	14'-6"	14'-1"	-
	NI-40x	15'-11"	15'-0"	14'-6"	-	16'-4"	15'-5"	14'-11"	-
	NI-60	16'-1"	15'-2"	14'-8"	-	16'-6"	15'-7"	15'-1"	-
	NI-80	17'-1"	16'-1"	15'-6"	-	17'-5"	16'-5"	15'-10"	-
11-7/8"	NI-20	16'-9"	15'-10"	15'-4"	-	17'-4"	16'-4"	15'-10"	-
	NI-40x	17'-10"	16'-10"	16'-3"	-	18'-6"	17'-4"	16'-9"	-
	NI-60	18'-1"	17'-0"	16'-5"	-	18'-9"	17'-6"	16'-11"	-
	NI-80	19'-6"	18'-0"	17'-4"	-	20'-1"	18'-7"	17'-9"	-
	NI-90	19'-11"	18'-4"	17'-8"	-	20'-5"	18'-11"	18'-1"	-
14"	NI-40x	19'-10"	18'-4"	17'-8"	-	20'-6"	19'-1"	18'-3"	-
	NI-60	20'-2"	18'-8"	17'-11"	-	20'-10"	19'-4"	18'-6"	-
	NI-80	21'-8"	20'-0"	19'-1"	-	22'-4"	20'-8"	19'-9"	-
	NI-90	22'-1"	20'-5"	19'-6"	-	22'-9"	21'-0"	20'-1"	-
16"	NI-60	22'-0"	20'-4"	19'-6"	-	22'-9"	21'-1"	20'-2"	-
	NI-80	23'-7"	21'-10"	20'-10"	-	24'-4"	22'-6"	21'-6"	-
	NI-90	24'-1"	22'-2"	21'-2"	-	24'-9"	22'-11"	21'-10"	-

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-6"	15'-1"	14'-3"	-	16'-6"	15'-1"	14'-3"	-
	NI-40x	17'-9"	16'-10"	15'-11"	-	18'-2"	16'-11"	15'-11"	-
	NI-60	17'-11"	16'-11"	16'-2"	-	18'-5"	17'-2"	16'-2"	-
	NI-80	19'-3"	17'-10"	17'-3"	-	19'-8"	18'-3"	17'-7"	-
11-7/8"	NI-20	19'-4"	18'-0"	17'-1"	-	19'-9"	18'-1"	17'-1"	-
	NI-40x	20'-10"	19'-4"	18'-6"	-	21'-5"	19'-11"	19'-0"	-
	NI-60	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-3"	-
	NI-80	22'-6"	20'-10"	19'-11"	-	23'-1"	21'-5"	20'-5"	-
	NI-90	23'-0"	21'-3"	20'-4"	-	23'-6"	21'-10"	20'-10"	-
14"	NI-40x	23'-5"	21'-8"	20'-9"	-	24'-0"	22'-5"	20'-11"	-
	NI-60	23'-9"	22'-0"	21'-0"	-	24'-5"	22'-8"	21'-8"	-
	NI-80	25'-4"	23'-6"	22'-5"	-	25'-11"	24'-1"	23'-0"	-
	NI-90	25'-10"	23'-11"	22'-9"	-	26'-5"	24'-6"	23'-4"	-
16"	NI-60	26'-2"	24'-3"	23'-2"	-	26'-11"	25'-0"	23'-11"	-
	NI-80	27'-11"	25'-10"	24'-7"	-	28'-7"	26'-6"	25'-3"	-
	NI-90	28'-5"	26'-3"	25'-0"	-	29'-0"	26'-11"	25'-8"	-

Notes:

1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans – M7.1

Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 20 psf
Deflection limits:	L/480 under live load and L/240 under total load
Sheathing:	3/4 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

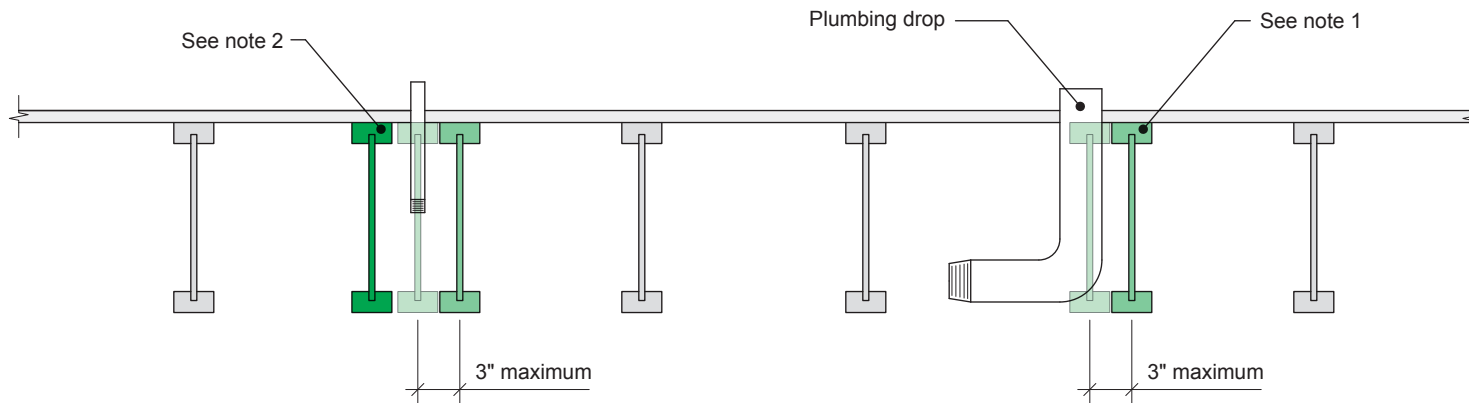
Joist depth	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	16'-11"	15'-11"	15'-4"	14'-9"	17'-4"	16'-4"	15'-9"	14'-11"
	NI-60	17'-1"	16'-1"	15'-6"	14'-10"	17'-6"	16'-6"	15'-11"	15'-3"
	NI-80	18'-1"	17'-0"	16'-4"	15'-8"	18'-7"	17'-4"	16'-8"	16'-0"
11-7/8"	NI-20	17'-10"	16'-10"	16'-2"	15'-7"	18'-5"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-3"	17'-10"	17'-2"	16'-6"	19'-10"	18'-5"	17'-8"	16'-11"
	NI-60	19'-6"	18'-1"	17'-4"	16'-8"	20'-1"	18'-8"	17'-10"	17'-1"
	NI-80	20'-11"	19'-4"	18'-5"	17'-7"	21'-5"	19'-10"	18'-11"	17'-11"
	NI-90	21'-4"	19'-9"	18'-9"	17'-10"	21'-10"	20'-3"	19'-3"	18'-3"
14"	NI-40x	21'-4"	19'-9"	18'-10"	17'-11"	22'-0"	20'-5"	19'-6"	18'-6"
	NI-60	21'-8"	20'-1"	19'-2"	18'-2"	22'-4"	20'-9"	19'-9"	18'-9"
	NI-80	23'-3"	21'-6"	20'-5"	19'-4"	23'-10"	22'-1"	21'-0"	19'-11"
	NI-90	23'-9"	21'-11"	20'-10"	19'-8"	24'-3"	22'-6"	21'-5"	20'-3"
16"	NI-60	23'-7"	21'-10"	20'-10"	19'-9"	24'-4"	22'-7"	21'-7"	20'-5"
	NI-80	25'-4"	23'-5"	22'-3"	21'-1"	26'-0"	24'-1"	22'-11"	21'-8"
	NI-90	25'-10"	23'-10"	22'-8"	21'-5"	26'-5"	24'-6"	23'-4"	22'-0"

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-7"	17'-2"	16'-3"	14'-11"	18'-10"	17'-2"	16'-3"	14'-11"
	NI-60	18'-10"	17'-6"	16'-6"	15'-5"	19'-1"	17'-6"	16'-6"	15'-5"
	NI-80	20'-2"	18'-9"	17'-11"	16'-10"	20'-7"	19'-2"	18'-2"	16'-10"
11-7/8"	NI-20	20'-1"	18'-5"	17'-5"	16'-1"	20'-1"	18'-5"	17'-5"	16'-1"
	NI-40x	21'-9"	20'-3"	19'-0"	17'-0"	22'-4"	20'-5"	19'-0"	17'-0"
	NI-60	22'-0"	20'-6"	19'-7"	18'-4"	22'-7"	20'-10"	19'-8"	18'-4"
	NI-80	23'-6"	21'-10"	20'-10"	19'-9"	24'-0"	22'-5"	21'-4"	20'-0"
	NI-90	24'-0"	22'-4"	21'-3"	20'-1"	24'-6"	22'-10"	21'-9"	20'-7"
14"	NI-40x	24'-4"	22'-8"	20'-11"	18'-8"	25'-0"	22'-11"	20'-11"	18'-8"
	NI-60	24'-9"	23'-0"	22'-0"	20'-9"	25'-5"	23'-8"	22'-4"	20'-10"
	NI-80	26'-5"	24'-6"	23'-4"	22'-1"	27'-0"	25'-2"	24'-0"	22'-8"
	NI-90	26'-11"	25'-0"	23'-10"	22'-6"	27'-5"	25'-7"	24'-5"	23'-1"
16"	NI-60	27'-2"	25'-4"	24'-2"	22'-10"	27'-11"	26'-1"	24'-9"	23'-1"
	NI-80	29'-0"	26'-11"	25'-8"	24'-3"	29'-7"	27'-7"	26'-4"	24'-11"
	NI-90	29'-6"	27'-5"	26'-1"	24'-8"	30'-1"	28'-1"	26'-9"	25'-4"

Notes:

1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

7c

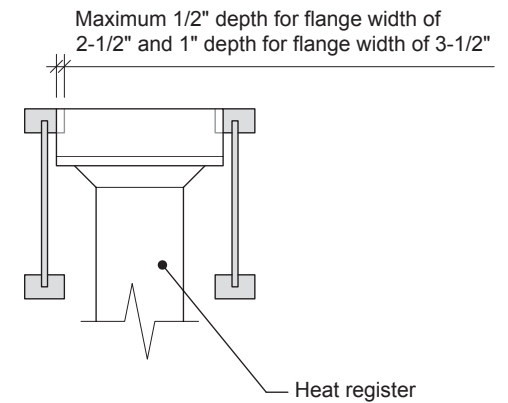
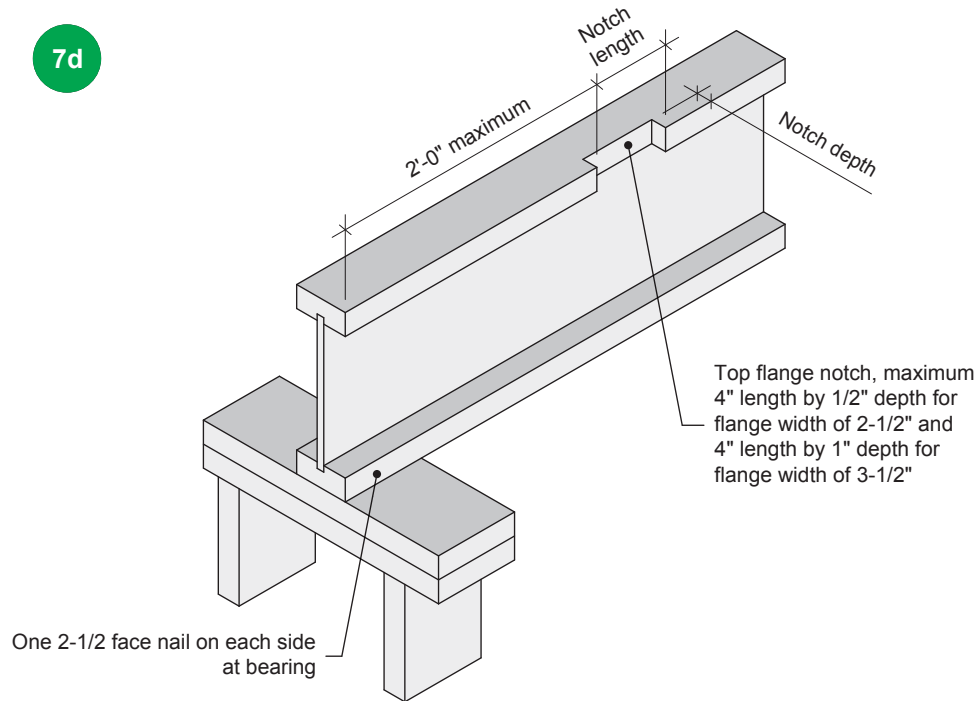


Notes:

1. To prevent interference with plumbing, a joist may be shifted up to 3 inches if the edge of the floor panel is supported and the span rating is not exceeded.
2. In all other cases, an additional joist is required.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity.

7d



Notes:

1. Blocking required at bearing for lateral support, not shown for clarity.
2. The maximum dimensions for a notch on the side of the top flange are 4-inch length by 1/2-inch depth for flange width of 2-1/2 inches, and 4-inch length by 1-inch depth for flange width of 3-1/2 inches.
3. This detail applies to simple-span joists and multiple-span joists where the notch is located at the end half-span.
4. For other applications, contact Nordic Structures.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity.