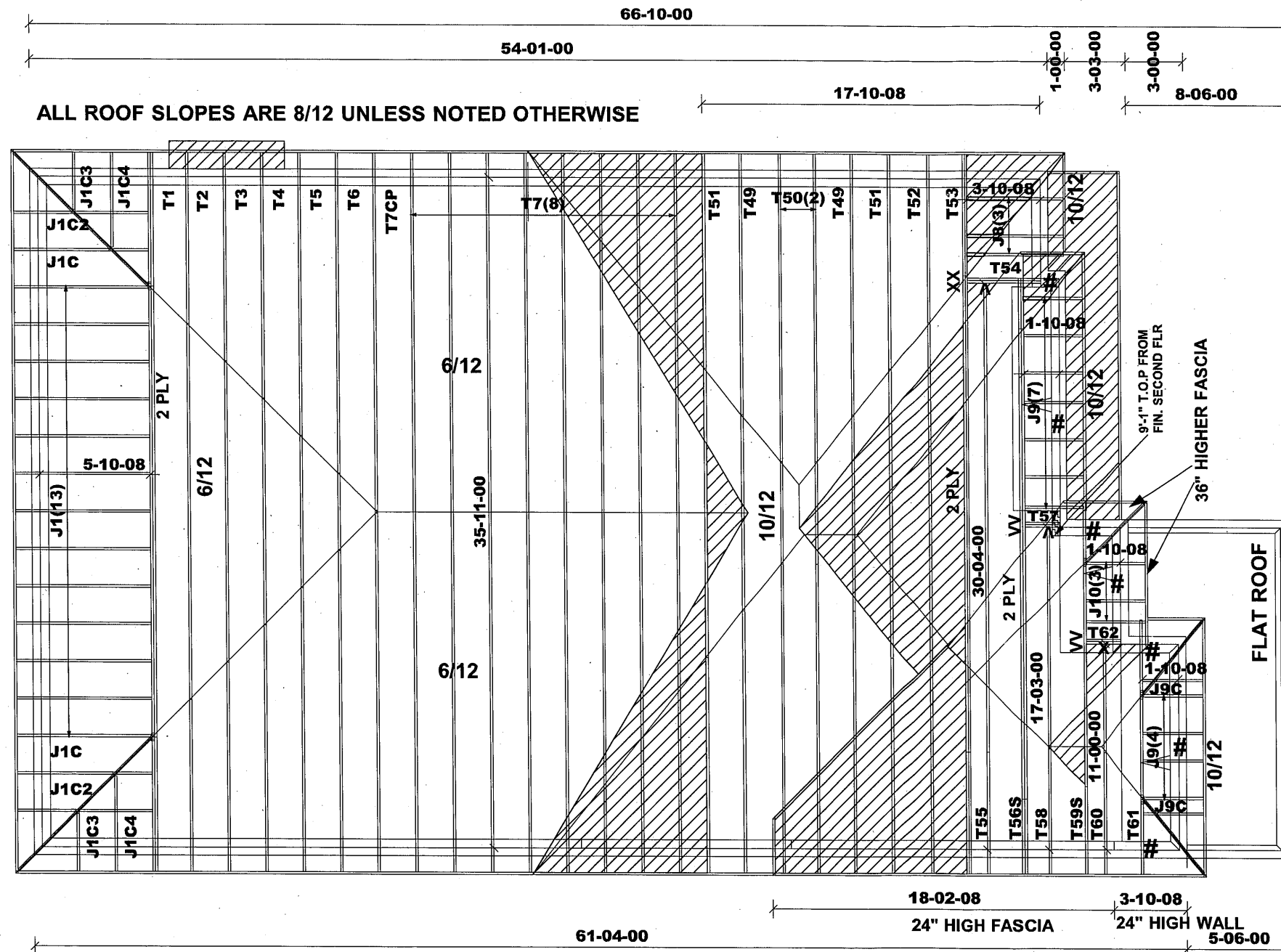




ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
HGUS26 - (X)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)

DENOTES:
#: 24" RAISED CEILING AND T.O.P

DENOTES:
CONVENTIONAL FRAMING



Structural component only
DWG# T-2307567

TRUSS PLACEMENT PLAN.

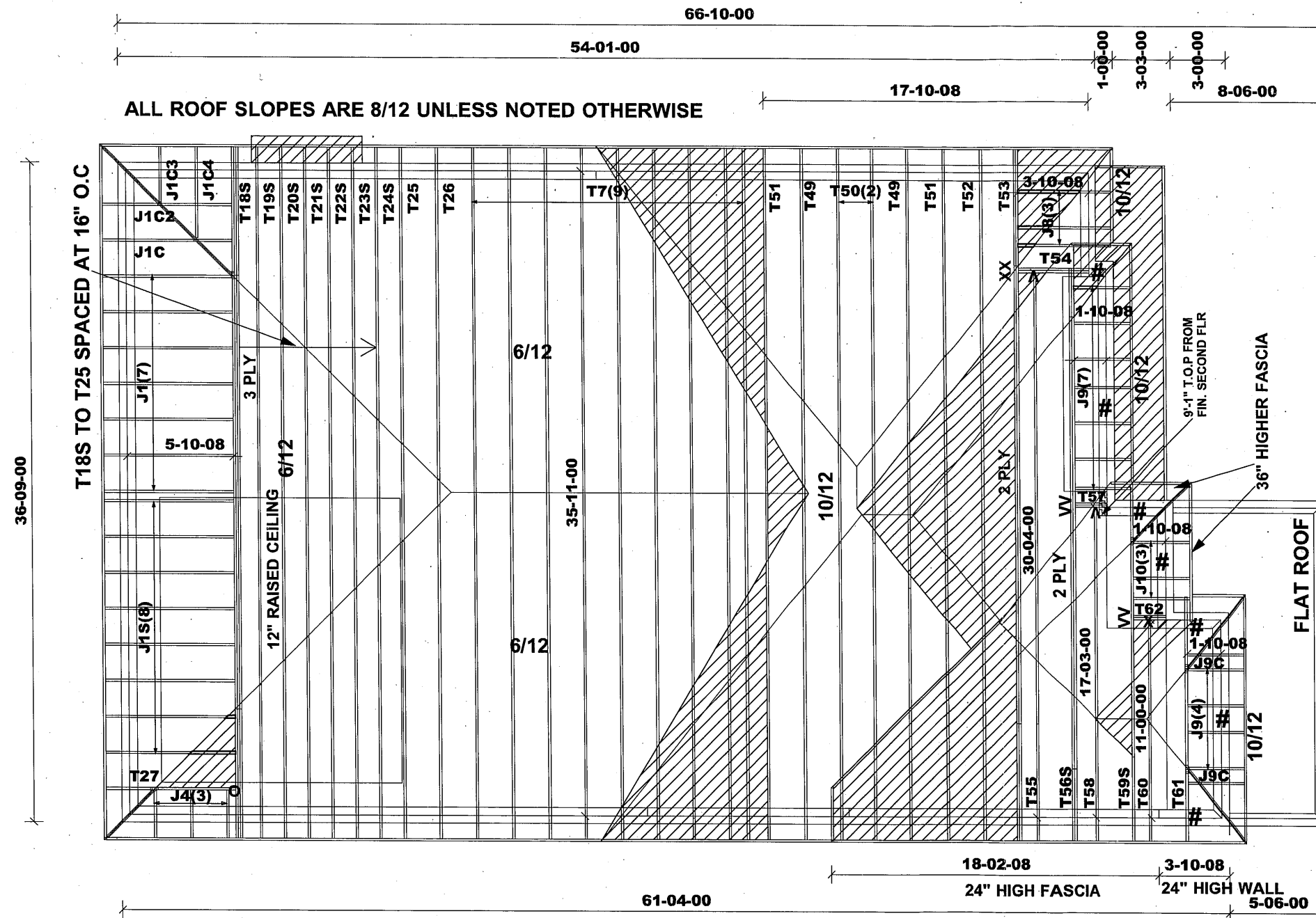
This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.

DWG# T-2126452 to T-2126456,
T-2126467, T-2126481 to T-2126492,
T-2126520 to T-2126537, T-1800218



Job Track: 52647	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT - 4505 / C-STD (4 OR 5 BED)
Layout ID: 433590	Project: FORESTSIDE ESTATES	Millen ver 8.5.3.233
Plan Log: 206922	Date: 2023-06-07	Designer: ND/JC
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.		



ALL ROOF SLOPES ARE 8/12 UNLESS NOTED OTHERWISE

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26 - (X)
LUS26-2 - (VV)
HGUS26-2 - (XX)

DENOTES:
#: 24" RAISED CEILING AND T.O.P

 DENOTES:
CONVENTIONAL
FRAMING



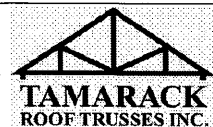
Structural component only
DWG# T-2307568

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.

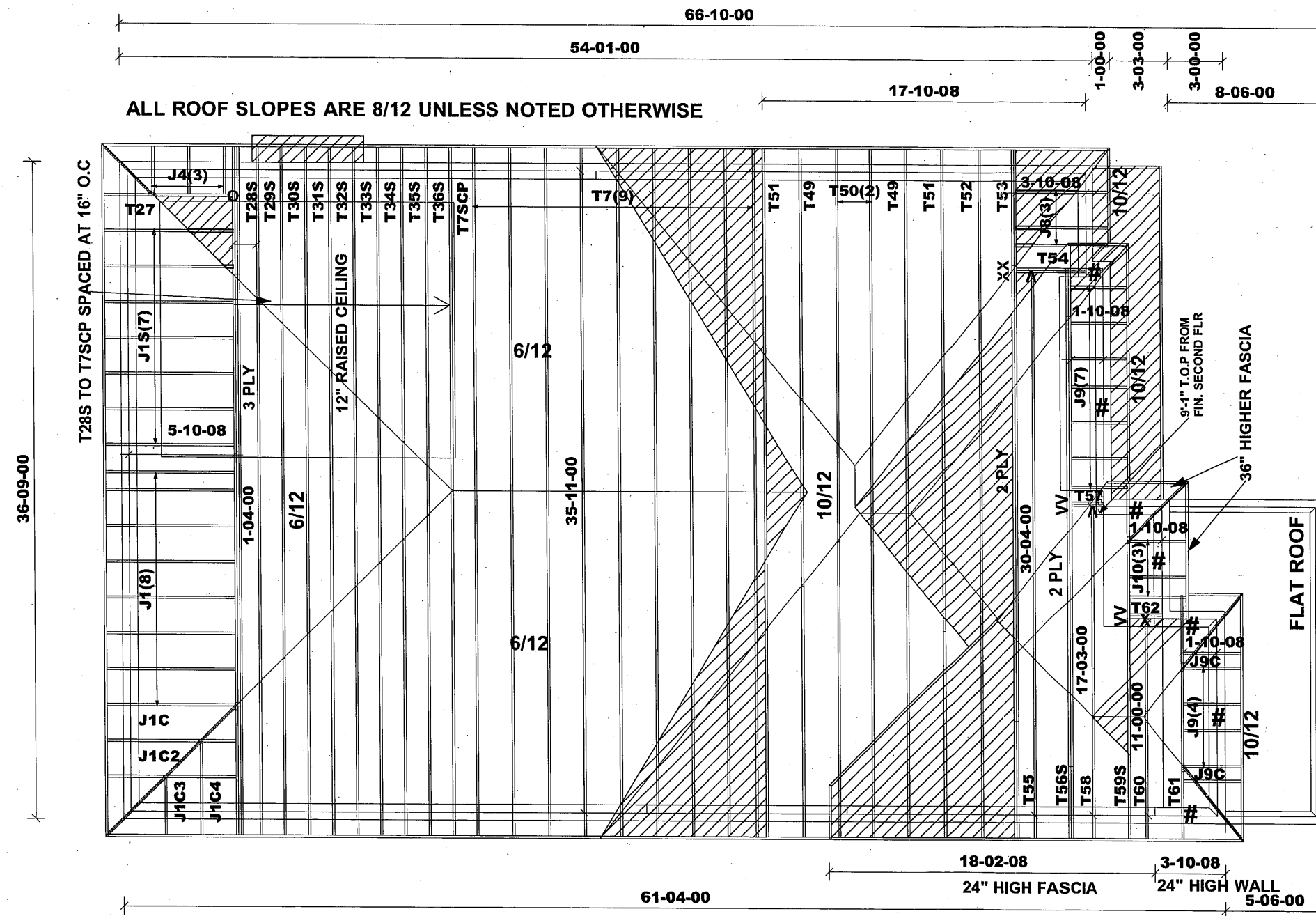
DWG # T-2126452 to T-2126456,
T-2126461 to T-2126468, T-2126520
to T-2126537, T-1800218



Job Track: **52647**
Layout ID: **433591**
Plan Log: **206922**

Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **FORESTSIDE ESTATES**
Date: 2023-06-07 Designer: **ND/JC**

Model / Elevation: **UNIT - 4505 / C- OPT. COFF (4 BED)**
Mitek ver 8.5.3.233
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

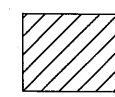
All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

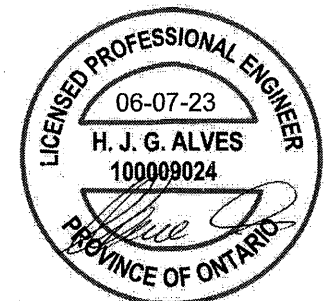
DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26 - (X)
LUS26-2 - (VV)
HGUS26-2 - (XX)

DENOTES:
#: 24" RAISED CEILING AND T.O.P

 DENOTES:
CONVENTIONAL
FRAMING



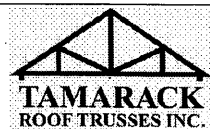
Structural component only
DWG# T-2307569

DWG# T-2126452 to T-2126456,
T-2126467, T-2126481 to T-2126482,
T-2126492 to T-2126502, T-2126520
to T-2126537, T-1800218

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.

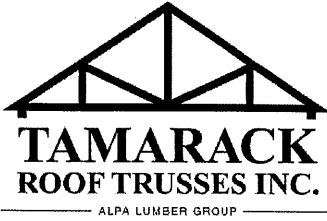


Job Track: **52647**
Layout ID: **433592**
Plan Log: **206922**
Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **FORESTSIDE ESTATES**
Date: 2023-06-07
Designer: **ND/JC**

Model / Elevation: **UNIT - 4505 / C- OPT. COFF (5 BED)**
Mitek ver 8.5.3.233

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST



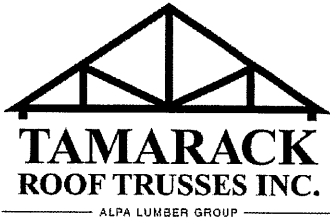
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT-4505
 Lot #:
 Elevation: C- STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 206922
 Layout ID: 433590
 Ref #: 14094
 Page: 1 of 3
 Date: 06-07-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	35-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	333.31 207.33		
	1	T2 Hip	6 /12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.28 90.50		
	1	T3 Hip	6 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	148.12 93.00		
	1	T4 Hip	6 /12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	146.64 90.50		
	1	T5 Hip	6 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	163.39 103.33		
	1	T6 Hip	6 /12	35-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	158.74 100.33		
	8	T7 Common	6 /12	35-11-00	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1209.3 741.33		
	1	T7CP Hip	6 /12	35-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	150.98 92.67		
	2	T49 Hip	8 /12	35-11-00	9-10-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	343.98 216.67		
	2	T50 Hip	8 /12	35-11-00	11-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	359.47 222.67		
	2	T51 Hip	8 /12	35-11-00	8-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	322.55 199.33		
	1	T52 Hip	8 /12	35-11-00	6-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	147.86 90.83		
	1 2-ply	T53 Hip Girder	8 /12	35-11-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	343.42 210.00		
	1 2-ply	T54 Flat Girder	0 /12	3-10-08	2-00-00	2 x 4 2 x 6		2-00-00 2-00-00	36.93 24.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT-4505
 Lot #:
 Elevation: C- STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 206922
 Layout ID: 433590
 Ref #: 14094
 Page: 2 of 3
 Date: 06-07-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T55 Hip	8 /12	30-04-00	6-08-07	2 x 4	1-03-08 1-03-08	3-02-13 3-04-13	132.8 82.83		
	1 2-ply	T56S Hip Girder	8 /12	30-07-00	5-02-07	2 x 4 2 x 6	1-03-08	3-04-13 1-04-13	311.48 199.33		
	1 2-ply	T57 Jack-Closed Girder	10 /12	1-10-08	5-02-07	2 x 4 2 x 6		3-07-11 5-02-07	35.18 24.00		
	1	T58 Hip	8 /12	17-03-00	5-11-13	2 x 4	1-03-08 1-03-08	2-04-13 3-04-13	83.12 52.83		
	1	T59S Hip Girder	8 /12	17-03-00	5-07-13	2 x 4 2 x 6	1-03-08 1-03-08	3-04-13 2-04-13	101.7 63.33		
	1	T60 Hip	8 /12	11-00-00	6-10-07	2 x 4	1-03-08	3-04-13 1-04-13	63.11 41.50		
	1	T61 Hip Girder	8 /12	11-00-00	3-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	49 30.83		
	1 2-ply	T62 Jack-Closed Girder	8 /12	1-10-08	3-07-13	2 x 4 2 x 6		2-04-13 3-07-13	26.45 17.67		
	13	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	234.21 151.67		
	2	J1C Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	30.71 19.33		
	2	J1C2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	25.61 16.67		
	2	J1C3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	16.85 11.33		
	2	J1C4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	21.95 14.00		
	3	J8 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.64 29.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT-4505
 Lot #:
 Elevation: C- STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 206922
 Layout ID: 433590
 Ref #: 14094
 Page: 3 of 3
 Date: 06-07-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	11	J9 Jack-Open	10 /12	1-10-08	3-02-07	2 x 4	1-03-08	1-07-11 3-02-07	111.32 77.00		
	2	J9C Jack-Open	10 /12	1-01-09	2-07-00	2 x 4	1-03-08 8-15	1-07-11 2-07-00	18.04 12.67		
	3	J10 Jack-Open	8 /12	1-10-08	3-07-13	2 x 4	1-03-08	2-04-13 3-07-13	32.92 20.50		

TOTAL # TRUSS= 78

TOTAL BFT OF ALL TRUSSES= 3347.65 BFT.

TOTAL WEIGHT OF ALL TRSSES 5348.05 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26	
2	Hardware	LJS26DS	
2	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 6

DELIVERY SHIPLIST



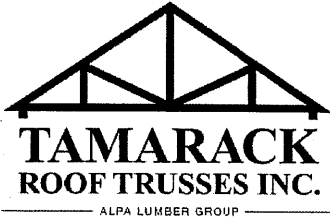
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT-4505
 Lot #:
 Elevation: C- OPT. COFF (4 BED)

Job Track: 52647
 PlanLog: 206922
 Layout ID: 433591
 Ref #: 14094
 Page: 1 of 3
 Date: 06-07-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	T7 Common	6 /12	35-11-00	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1360.46 834.00		
	1 3-ply	T18S Hip Girder	6 /12	35-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	513.74 329.00		
	1	T19S Hip	6 /12	35-11-00	4-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	147.17 94.67		
	1	T20S Hip	6 /12	35-11-00	5-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	152.3 95.67		
	1	T21S Hip	6 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	159.46 100.67		
	1	T22S Hip	6 /12	35-11-00	6-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	154.36 99.00		
	1	T23S Hip	6 /12	35-11-00	7-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	157.09 99.17		
	1	T24S Hip	6 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	160.27 102.00		
	1	T25 Hip	6 /12	35-11-00	8-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	145.69 90.67		
	1	T26 Hip	6 /12	35-11-00	9-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	163.77 102.67		
	1	T27 Half Hip Girder	6 /12	5-10-08	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	23.14 16.50		
	2	T49 Hip	8 /12	35-11-00	9-10-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	343.98 216.67		
	2	T50 Hip	8 /12	35-11-00	11-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	353.78 219.33		
	2	T51 Hip	8 /12	35-11-00	8-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	322.55 199.33		

DELIVERY SHIPLIST



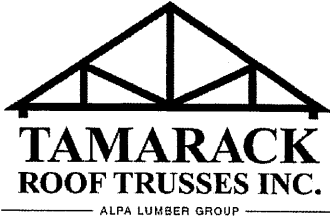
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT-4505
 Lot #:
 Elevation: C- OPT. COFF (4 BED)

Job Track: 52647
 PlanLog: 206922
 Layout ID: 433591
 Ref #: 14094
 Page: 2 of 3
 Date: 06-07-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T52 Hip	8 /12	35-11-00	6-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	147.86 90.83		
	1 2-ply	T53 Hip Girder	8 /12	35-11-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	341.05 210.67		
	1 2-ply	T54 Flat Girder	0 /12	3-10-08	2-00-00	2 x 4 2 x 6		2-00-00 2-00-00	36.93 24.67		
	1	T55 Hip	8 /12	30-04-00	6-08-07	2 x 4	1-03-08 1-03-08	3-02-13 3-04-13	132.8 82.83		
	1 2-ply	T56S Hip Girder	8 /12	30-07-00	5-02-07	2 x 4 2 x 6	1-03-08	3-04-13 1-04-13	311.48 199.33		
	1 2-ply	T57 Jack-Closed Girder	10 /12	1-10-08	5-02-07	2 x 4		3-07-11 5-02-07	33.75 22.67		
	1	T58 Hip	8 /12	17-03-00	5-11-13	2 x 4	1-03-08 1-03-08	2-04-13 3-04-13	83.12 52.83		
	1	T59S Hip Girder	8 /12	17-03-00	5-07-13	2 x 4 2 x 6	1-03-08 1-03-08	3-04-13 2-04-13	100.28 62.50		
	1	T60 Hip	8 /12	11-00-00	6-10-07	2 x 4	1-03-08	3-04-13 1-04-13	63.11 41.50		
	1	T61 Hip Girder	8 /12	11-00-00	3-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	49 30.83		
	1 2-ply	T62 Jack-Closed Girder	8 /12	1-10-08	3-07-13	2 x 4		2-04-13 3-07-13	24.99 17.67		
	7	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	126.11 81.67		
	1	J1C Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	15.35 9.67		
	1	J1C2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	12.8 8.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT-4505
 Lot #:
 Elevation: C- OPT. COFF (4 BED)

Job Track: 52647
 PlanLog: 206922
 Layout ID: 433591
 Ref #: 14094
 Page: 3 of 3
 Date: 06-07-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J1C3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	8.43 5.67		
	1	J1C4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	10.98 7.00		
	8	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	162.46 114.67		
	3	J4 Jack-Open	6 /12	1-04-08	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	20.92 17.00		
	3	J8 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.64 29.00		
	11	J9 Jack-Open	10 /12	1-10-08	3-02-07	2 x 4	1-03-08	1-07-11 3-02-07	111.35 77.00		
	2	J9C Jack-Open	10 /12	1-01-09	2-07-00	2 x 4	1-03-08 8-15	1-07-11 2-07-00	18.04 12.67		
	3	J10 Jack-Open	8 /12	1-10-08	3-07-13	2 x 4	1-03-08	2-04-13 3-07-13	32.92 20.50		

TOTAL # TRUSS= 84

TOTAL BFT OF ALL TRUSSES= 3818.86

BFT.

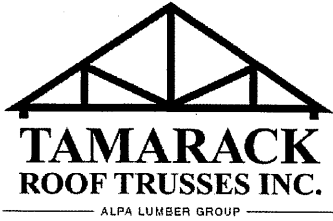
TOTAL WEIGHT OF ALL TRSSES 6047.14 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LUS24	
2	Hardware	LJS26DS	
2	Hardware	LUS26-2	
1	Hardware	HGUS26	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 7

DELIVERY SHIPLIST



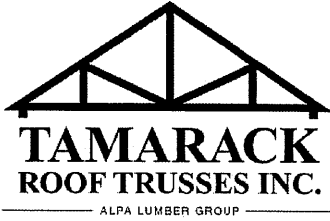
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT-4505
 Lot #:
 Elevation: C- OPT. COFF (5 BED)

Job Track: 52647
 PlanLog: 206922
 Layout ID: 433592
 Ref #: 14094
 Page: 1 of 3
 Date: 06-07-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	T7 Common	6 /12	35-11-00	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1360.46 834.00		
	1	T7SCP Hip	6 /12	35-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	168.94 106.00		
	1	T27 Half Hip Girder	6 /12	5-10-08	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	23.14 16.50		
	1 3-ply	T28S Hip Girder	6 /12	35-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	508.39 326.00		
	1	T29S Hip	6 /12	35-11-00	4-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	148.03 95.83		
	1	T30S Hip	6 /12	35-11-00	5-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	153.56 97.83		
	1	T31S Hip	6 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	154.07 97.83		
	1	T32S Hip	6 /12	35-11-00	6-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	159.96 103.67		
	1	T33S Hip	6 /12	35-11-00	7-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	164.97 104.33		
	1	T34S Hip	6 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	162.18 102.67		
	1	T35S Hip	6 /12	35-11-00	8-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	164.55 105.50		
	1	T36S Hip	6 /12	35-11-00	9-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	167.6 106.83		
	2	T49 Hip	8 /12	35-11-00	9-10-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	343.98 216.67		
	2	T50 Hip	8 /12	35-11-00	11-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	353.78 219.33		

DELIVERY SHIPLIST

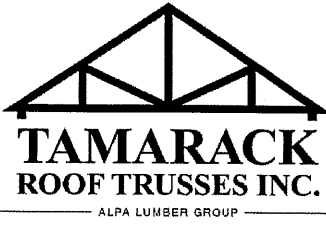


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT-4505
 Lot #:
 Elevation: C- OPT. COFF (5 BED)

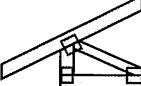
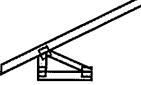
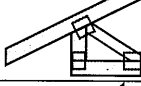
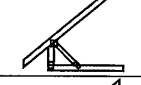
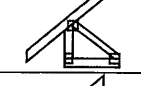
Job Track: 52647
 PlanLog: 206922
 Layout ID: 433592
 Ref #: 14094
 Page: 2 of 3
 Date: 06-07-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T51 Hip	8 /12	35-11-00	8-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	322.55 199.33		
	1	T52 Hip	8 /12	35-11-00	6-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	147.86 90.83		
	1 2-ply	T53 Hip Girder	8 /12	35-11-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	341.05 210.67		
	1 2-ply	T54 Flat Girder	0 /12	3-10-08	2-00-00	2 x 4 2 x 6		2-00-00 2-00-00	36.93 24.67		
	1	T55 Hip	8 /12	30-04-00	6-08-07	2 x 4	1-03-08 1-03-08	3-02-13 3-04-13	132.8 82.83		
	1 2-ply	T56S Hip Girder	8 /12	30-07-00	5-02-07	2 x 4 2 x 6	1-03-08	3-04-13 1-04-13	311.48 199.33		
	1 2-ply	T57 Jack-Closed Girder	10 /12	1-10-08	5-02-07	2 x 4		3-07-11 5-02-07	33.75 22.67		
	1	T58 Hip	8 /12	17-03-00	5-11-13	2 x 4	1-03-08 1-03-08	2-04-13 3-04-13	83.12 52.83		
	1	T59S Hip Girder	8 /12	17-03-00	5-07-13	2 x 4 2 x 6	1-03-08 1-03-08	3-04-13 2-04-13	100.28 62.50		
	1	T60 Hip	8 /12	11-00-00	6-10-07	2 x 4	1-03-08	3-04-13 1-04-13	63.11 41.50		
	1	T61 Hip Girder	8 /12	11-00-00	3-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	49 30.83		
	1 2-ply	T62 Jack-Closed Girder	8 /12	1-10-08	3-07-13	2 x 4		2-04-13 3-07-13	24.99 17.67		
	8	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	144.13 93.33		
	1	J1C Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	15.35 9.67		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP		DELIVERY SHIPLIST			
		Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
		Builder:	ROYAL PINE HOMES	PlanLog:	206922
		Project:	FORESTSIDE ESTATES	Layout ID:	433592
		Location:	BRAMPTON	Ref #	14094
		Model:	UNIT-4505	Page:	3 of 3
		Lot #:		Date:	06-07-2023
		Elevation:	C- OPT. COFF (5 BED)	Designer:	
				Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J1C2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	12.8 8.33		
	1	J1C3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	8.43 5.67		
	1	J1C4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	10.98 7.00		
	7	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	142.15 100.33		
	3	J4 Jack-Open	6 /12	1-04-08	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	20.92 17.00		
	3	J8 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.64 29.00		
	11	J9 Jack-Open	10 /12	1-10-08	3-02-07	2 x 4	1-03-08	1-07-11 3-02-07	111.35 77.00		
	2	J9C Jack-Open	10 /12	1-01-09	2-07-00	2 x 4	1-03-08 8-15	1-07-11 2-07-00	18.04 12.67		
	3	J10 Jack-Open	8 /12	1-10-08	3-07-13	2 x 4	1-03-08	2-04-13 3-07-13	32.92 20.50		

TOTAL # TRUSS= 85 TOTAL BFT OF ALL TRUSSES= 3949.15 BFT. TOTAL WEIGHT OF ALL TRSSES 6243.25 LBS

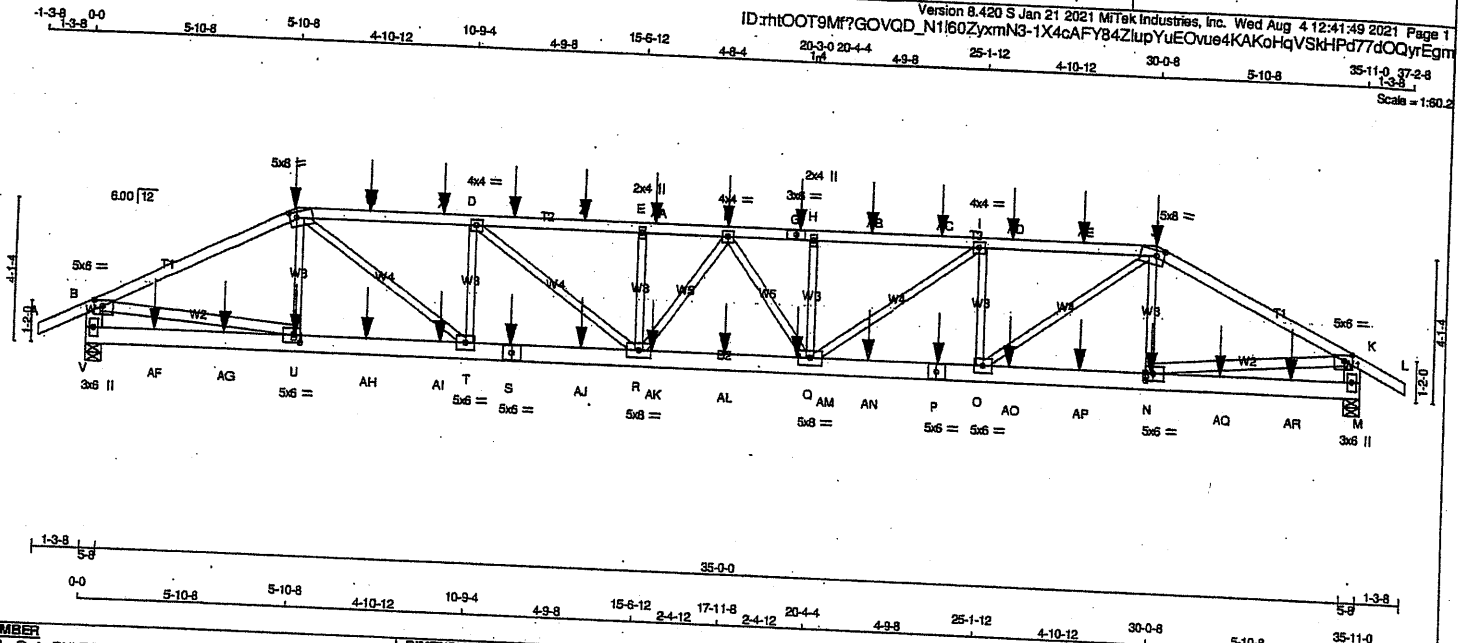
HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LUS24	
2	Hardware	LJS26DS	
2	Hardware	LUS26-2	
1	Hardware	HGUS26	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 7

JOB NAME 420375	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 12:41:49 2021 Page 1
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LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
B - U	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-G	12	SIDE(61.0)
G-J	12	SIDE(61.0)
J-L	12	SIDE(61.0)
V-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-S	12	SIDE(183.1)
S-P	12	SIDE(0.0)
P-M	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
U-C	6	SIDE(19.2)
N-J	6	SIDE(19.2)
2x3	6	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
V	3722	0	0	3722	0	0	5-8	5-8	5-8
M	3721	0	0	3721	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SO
V	2640	1689 / 0	0 / 0	0 / 0	0 / 0	950 / 0	0 /
M	2639	1689 / 0	0 / 0	0 / 0	0 / 0	950 / 0	0 /
READING							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CS (LC)	MAX. CS (LC)	
FR-TO		FROM	TO		FR-TO				
A-B	0 / 26	-84.9	-84.9	0.06 (1)	10.00	U-C	-448 / 13	0.06 (1)	
B-C	-5801 / 0	-84.9	-84.9	0.55 (1)	3.84	C-T	0 / 3299	0.41 (1)	
C-W	-7799 / 0	-84.9	-84.9	0.65 (1)	3.00	T-D	-1836 / 0	0.23 (1)	
W-X	-7799 / 0	-84.9	-84.9	0.65 (1)	3.00	D-R	0 / 1494	0.18 (1)	
X-D	-7799 / 0	-84.9	-84.9	0.65 (1)	3.00	R-E	-609 / 0	0.08 (1)	
D-Y	-8979 / 0	-84.9	-84.9	0.74 (1)	2.70	Q-H	-603 / 0	0.08 (1)	
Y-Z	-8979 / 0	-84.9	-84.9	0.74 (1)	2.70	Q-I	0 / 1494	0.18 (1)	
Z-E	-8979 / 0	-84.9	-84.9	0.74 (1)	2.70	C-I	-1834 / 0	0.23 (1)	
E-AA	-8979 / 0	-84.9	-84.9	0.41 (1)	2.99	O-J	0 / 3299	0.41 (1)	
AA-F	-8979 / 0	-84.9	-84.9	0.41 (1)	2.99	N-J	-448 / 13	0.06 (1)	
F-G	-8975 / 0	-84.9	-84.9	0.42 (1)	2.98	B-U	0 / 5229	0.46 (1)	
G-H	-8975 / 0	-84.9	-84.9	0.42 (1)	2.98	N-K	0 / 5227	0.46 (1)	
H-AB	-8975 / 0	-84.9	-84.9	0.74 (1)	2.70	R-F	-169 / 0	0.03 (1)	
AB-AC	-8975 / 0	-84.9	-84.9	0.74 (1)	2.70	F-Q	-175 / 0	0.03 (1)	
AC-I	-8975 / 0	-84.9	-84.9	0.74 (1)	2.70				
I-AD	-7796 / 0	-84.9	-84.9	0.64 (1)	3.00				
AD-AE	-7796 / 0	-84.9	-84.9	0.64 (1)	3.00				
AE-J	-7796 / 0	-84.9	-84.9	0.64 (1)	3.00				
J-K	-5799 / 0	-84.9	-84.9	0.55 (1)	3.64				
K-L	0 / 26	-84.9	-84.9	0.06 (1)	10.00				
V-B	-3637 / 0	0.0	0.0	0.13 (1)	7.37				
M-K	-3635 / 0	0.0	0.0	0.13 (1)	7.37				
V-AF	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
AF-AG	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
AG-U	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
U-AH	0 / 5173	-18.5	-18.5	0.38 (1)	10.00				
AH-AI	0 / 5173	-18.5	-18.5	0.38 (1)	10.00				
AI-T	0 / 5173	-18.5	-18.5	0.38 (1)	10.00				
T-S	0 / 7800	-18.5	-18.5	0.57 (1)	10.00				
S-AJ	0 / 7800	-18.5	-18.5	0.57 (1)	10.00				
AJ-R	0 / 7800	-18.5	-18.5	0.57 (1)	10.00				
R-AK	0 / 9070	-18.5	-18.5	0.66 (1)	10.00				
AK-AL	0 / 9070	-18.5	-18.5	0.66 (1)	10.00				
AL-AM	0 / 9070	-18.5	-18.5	0.66 (1)	10.00				
AM-Q	0 / 9070	-18.5	-18.5	0.66 (1)	10.00				
Q-AN	0 / 7796	-18.5	-18.5	0.57 (1)	10.00				
AN-P	0 / 7796	-18.5	-18.5	0.57 (1)	10.00				
P-O	0 / 7796	-18.5	-18.5	0.57 (1)	10.00				
O-AO	0 / 5171	-18.5	-18.5	0.38 (1)	10.00				
AO-AP	0 / 5171	-18.5	-18.5	0.38 (1)	10.00				
AP-N	0 / 5171	-18.5	-18.5	0.38 (1)	10.00				
N-AQ	0 / 0	-18.5	-18.5	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/809 (0.53")
CS: TC=0.74/1.00 (D-E:1), BC=0.66/1.00 (Q-R:1), WB=0.46/1.00 (B-U:1), SS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

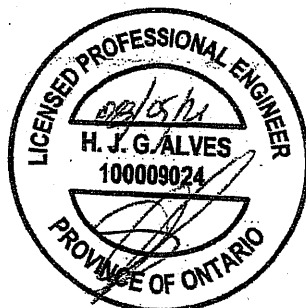
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (U) (INPUT = 0.90)
JSI METAL = 0.73 (S) (INPUT = 1.00)



Structural component only
DWG# T-2126461

JOB NAME 420375 Tamarack Roof Truss, Burlington	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed Aug 4 12:41:49 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.00	2.75
C	TTWW-m	MT20	5.0	8.0	2.00	2.50
D, F, I						
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-w	MT20	2.0	4.0		
J	TTWW-m	MT20	5.0	8.0	2.00	2.50
K	TMVW-p	MT20	5.0	8.0	2.00	2.75
M	BMV1-p	MT20	3.0	6.0		
N	BMVW-t	MT20	5.0	6.0	2.50	2.50
O	BMVW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW-w	MT20	5.0	8.0		
R	BMVW-w	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMVW-t	MT20	5.0	6.0		
U	BMVW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1-p	MT20	3.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

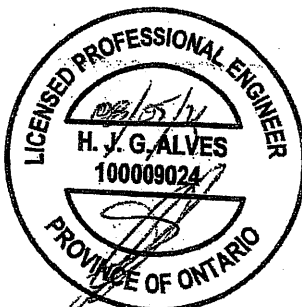
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC. LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO							
AQ-AR	0/0	-18.5	-18.5 0.07 (4)	10.00			
AR-M	0/0	-18.5	-18.5 0.07 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-374	-374		FRONT	VERT	TOTAL		C1
F	17-11-8	-113	-113		FRONT	VERT	TOTAL		C1
G	19-11-12	-113	-113		FRONT	VERT	TOTAL		C1
J	30-8-8	-374	-374		FRONT	VERT	TOTAL		C1
N	29-11-12	-29	-29		FRONT	VERT	TOTAL		C1
P	23-11-12	-29	-29		FRONT	VERT	TOTAL		C1
S	11-11-4	-29	-29		FRONT	VERT	TOTAL		C1
U	5-11-4	-29	-29		FRONT	VERT	TOTAL		C1
W	7-11-4	-113	-113		FRONT	VERT	TOTAL		C1
X	9-11-4	-113	-113		FRONT	VERT	TOTAL		C1
Y	11-11-4	-113	-113		FRONT	VERT	TOTAL		C1
Z	13-11-4	-113	-113		FRONT	VERT	TOTAL		C1
AA	15-11-4	-113	-113		FRONT	VERT	TOTAL		C1
AB	21-11-12	-113	-113		FRONT	VERT	TOTAL		C1
AC	23-11-12	-113	-113		FRONT	VERT	TOTAL		C1
AD	25-11-12	-113	-113		FRONT	VERT	TOTAL		C1
AE	27-11-12	-113	-113		FRONT	VERT	TOTAL		C1
AF	1-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AG	3-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AH	7-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AI	9-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AJ	13-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AK	15-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AL	17-11-8	-29	-29		FRONT	VERT	TOTAL		C1
AM	19-11-12	-29	-29		FRONT	VERT	TOTAL		C1
AN	21-11-12	-29	-29		FRONT	VERT	TOTAL		C1
AO	25-11-12	-29	-29		FRONT	VERT	TOTAL		C1
AP	27-11-12	-29	-29		FRONT	VERT	TOTAL		C1
AQ	31-11-12	-29	-29		FRONT	VERT	TOTAL		C1
AR	33-11-12	-29	-29		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

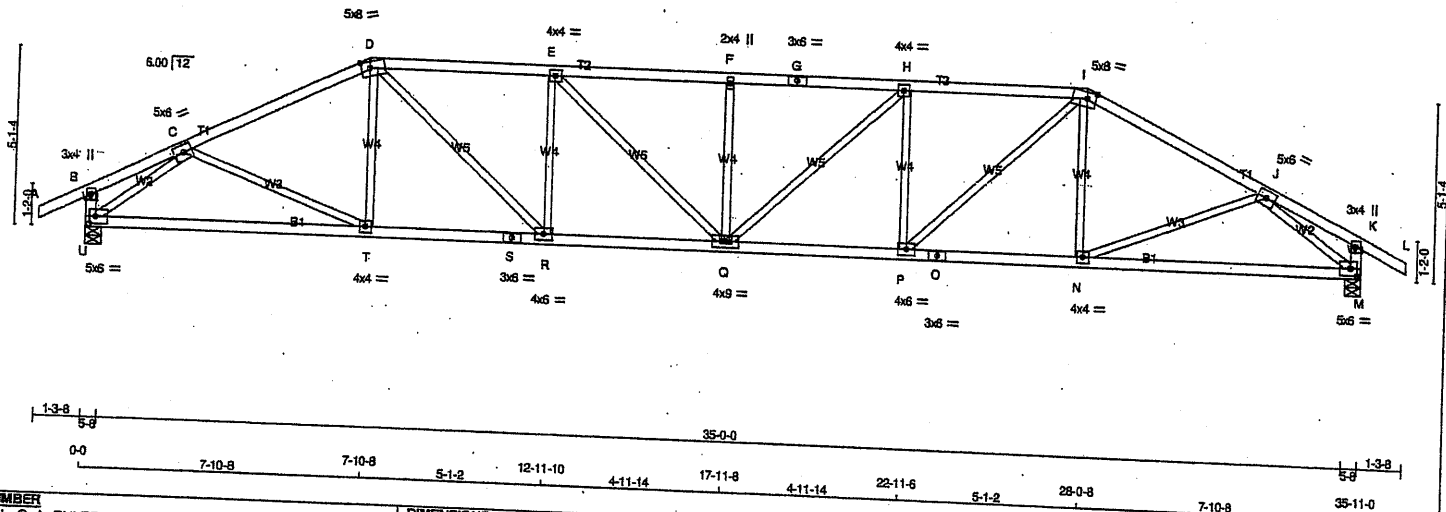


Structural component only
DWG# T-2126461

JOB NAME 420375	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 12:41:50 2021 Page 1
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1-3-8 0-0 4-0-8 4-0-8 3-10-0 7-10-8 5-1-2 12-11-10 4-11-14 17-11-8 4-11-14 22-11-6 5-1-2 28-0-8 3-10-0 31-10-8 4-0-8 35-11-0 37-3-8 1-3-8
Scale = 1:50.2



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - G	2x4 DRY	No.2	SPF		
G - I	2x4 DRY	No.2	SPF		
I - L	2x4 DRY	No.2	SPF		
U - B	2x4 DRY	No.2	SPF		
M - K	2x4 DRY	No.2	SPF		
U - S	2x4 DRY	No.2	SPF		
S - O	2x4 DRY	No.2	SPF		
O - M	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.00
E	TMVW-t	MT20	4.0	4.0		
F	TMV+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	2.25	3.00
J	TMVW-t	MT20	5.0	6.0		
K	TMV+p	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	6.0	2.50	2.25
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	6.0		
Q	BMVW-t	MT20	4.0	9.0		
R	BMVW-t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	6.0		
T	BMVW-t	MT20	4.0	4.0		
U	BMVW-t	MT20	5.0	6.0	2.50	2.25

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
U	1971	0	1971	0	0	5-8
M	1971	0	1971	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS		PERM.LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	LIVE	WIND				
U	1397	900/0	0/0	0/0	0/0	0/0	498/0	0/0
M	1397	900/0	0/0	0/0	0/0	0/0	498/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX. UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
		FROM	TO				
FR-TO							
A-B	0/28	-84.9	-84.9	0.11 (1)	C-T	0/275	0.06 (1)
B-C	0/28	-84.9	-84.9	0.24 (1)	10.00	T-D	0/103
C-D	-2696/0	-84.9	-84.9	0.44 (1)	3.85	D-R	0/103
D-E	-3243/0	-84.9	-84.9	0.51 (1)	3.46	R-E	0/1162
E-F	-3497/0	-84.9	-84.9	0.54 (1)	3.32	E-Q	-727/0
F-G	-3497/0	-84.9	-84.9	0.54 (1)	3.32	Q-F	0/353
G-H	-3497/0	-84.9	-84.9	0.54 (1)	3.32	F-H	-390/0
H-I	-3243/0	-84.9	-84.9	0.51 (1)	3.46	H-P	0/353
I-J	-2696/0	-84.9	-84.9	0.44 (1)	3.85	P-I	-727/0
J-K	0/28	-84.9	-84.9	0.24 (1)	10.00	N-I	0/1162
K-L	0/28	-84.9	-84.9	0.11 (1)	10.00	N-J	0/103
U-B	-170/0	0.0	0.0	0.02 (1)	7.81	U-C	-2761/0
M-K	-170/0	0.0	0.0	0.02 (1)	7.81	J-M	-2761/0
U-T	0/2144	-18.5	-18.5	0.48 (1)	10.00		
T-S	0/2397	-18.5	-18.5	0.52 (1)	10.00		
S-R	0/2397	-18.5	-18.5	0.52 (1)	10.00		
R-Q	0/3243	-18.5	-18.5	0.57 (1)	10.00		
Q-P	0/3243	-18.5	-18.5	0.57 (1)	10.00		
P-O	0/2397	-18.5	-18.5	0.52 (1)	10.00		
O-N	0/2397	-18.5	-18.5	0.52 (1)	10.00		
N-M	0/2144	-18.5	-18.5	0.48 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 36.7 PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 27.2 P.S.F. 'G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TC=0.54/1.00 (E-F:1), BC=0.57/1.00 (P-Q:1), WB=0.63/1.00 (J-M:1), SS=0.20/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MAX MIN MAX MIN MAX MIN		
MT20	650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL = 0.250 inches

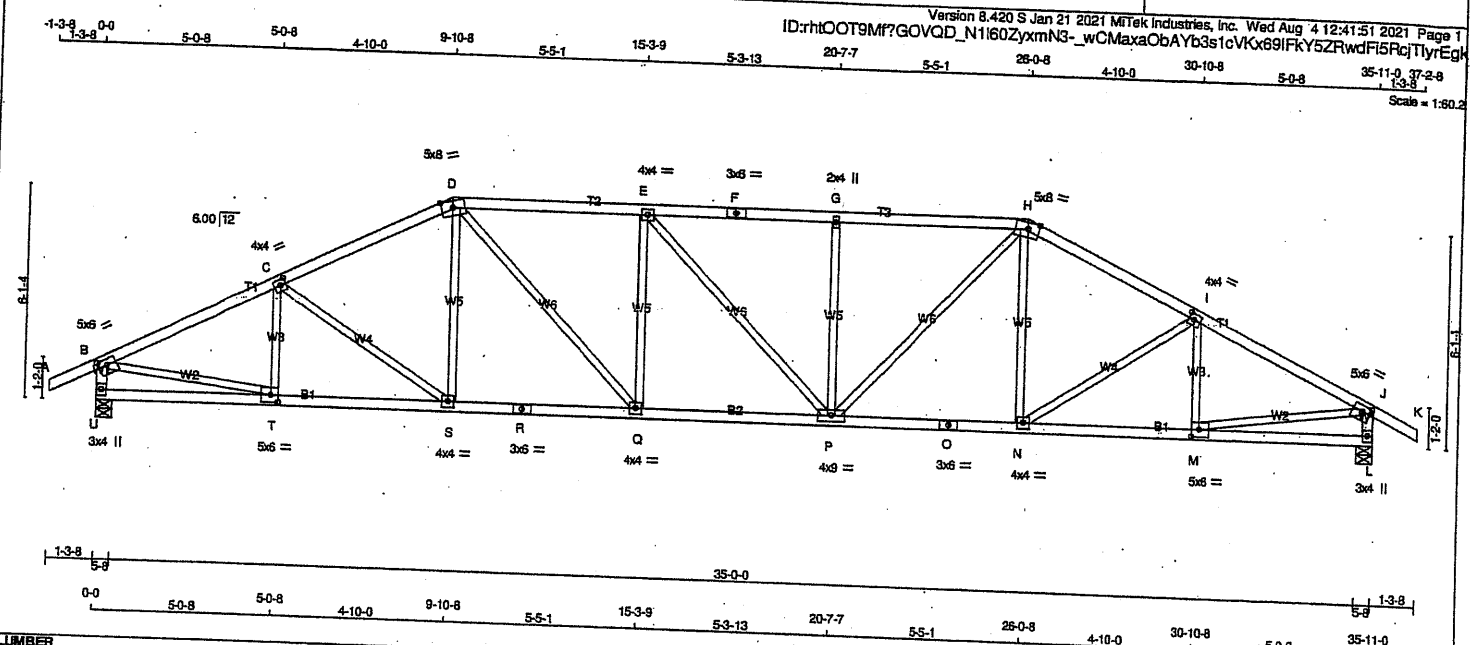
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (M) (INPUT = 0.90)
JSI METAL = 0.70 (S) (INPUT = 1.00)



Structural component only
DWG# T-2126462

JOB NAME 420375 Tamarack Roof Truss, Burlington	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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LUMBER

N. L. & A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.75
I	TMVW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	5.0	6.0	2.00	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	6.0	2.50	2.50
N, Q, S						
O	BMVW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
R	BMVW-w	MT20	4.0	9.0		
T	BS-t	MT20	3.0	6.0		
U	BMVW-t	MT20	5.0	8.0	2.50	2.50
V	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	1971	0	5-8
U	1971	0	5-8
L	1971	0	5-8

UNFACTORED REACTIONS

1ST LOOSE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	1397	900 / 0	0 / 0	0 / 0	498 / 0
U	1397	900 / 0	0 / 0	0 / 0	498 / 0
L	1397	900 / 0	0 / 0	0 / 0	498 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED TORSION (LBS)
FR-TO	A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	T-C	-327 / 0	0.07 (1)
	B-C	-2723 / 0	-84.9	-84.9	0.36 (1)	3.91	C-S	-194 / 0	0.12 (1)
	C-D	-2588 / 0	-84.9	-84.9	0.34 (1)	4.01	S-D	0 / 211	0.05 (4)
	D-E	-2851 / 0	-84.9	-84.9	0.40 (1)	3.78	D-Q	0 / 810	0.18 (1)
	E-F	-2849 / 0	-84.9	-84.9	0.37 (1)	3.82	Q-E	-494 / 0	0.29 (1)
	F-G	-2849 / 0	-84.9	-84.9	0.40 (1)	3.78	E-P	-2 / 0	0.00 (1)
	G-H	-2849 / 0	-84.9	-84.9	0.37 (1)	3.82	P-G	-493 / 0	0.29 (1)
	H-I	-2588 / 0	-84.9	-84.9	0.36 (1)	3.91	I-H	0 / 807	0.18 (1)
	I-J	-2723 / 0	-84.9	-84.9	0.34 (1)	4.01	H-N	0 / 212	0.05 (4)
	J-K	0 / 26	-84.9	-84.9	0.11 (1)	10.00	N-I	-193 / 0	0.12 (1)
	U-B	-1928 / 0	0.0	0.0	0.19 (1)	6.06	M-I	-327 / 0	0.07 (1)
	L-J	-1928 / 0	0.0	0.0	0.19 (1)	6.06	M-J	0 / 2489	0.56 (1)
	U-T	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
	T-S	0 / 2454	-18.5	-18.5	0.45 (1)	10.00			
	S-R	0 / 2298	-18.5	-18.5	0.43 (1)	10.00			
	R-O	0 / 2298	-18.5	-18.5	0.43 (1)	10.00			
	Q-P	0 / 2851	-18.5	-18.5	0.51 (1)	10.00			
	P-O	0 / 2298	-18.5	-18.5	0.43 (1)	10.00			
	O-N	0 / 2298	-18.5	-18.5	0.43 (1)	10.00			
	N-M	0 / 2454	-18.5	-18.5	0.45 (1)	10.00			
	M-L	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015.

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2019, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.20")

CALCULATED VERT. DEFL. (LL) = L/999 (0.16")

ALLOWABLE DEFL. (TL) = L/360 (1.20")

CALCULATED VERT. DEFL. (TL) = L/999 (0.32")

CSI: TC=0.40/1.00 (G-H:1), BC=0.51/1.00 (P-Q:1), WB=0.56/1.00 (B-T:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (T) (INPUT = 0.90)

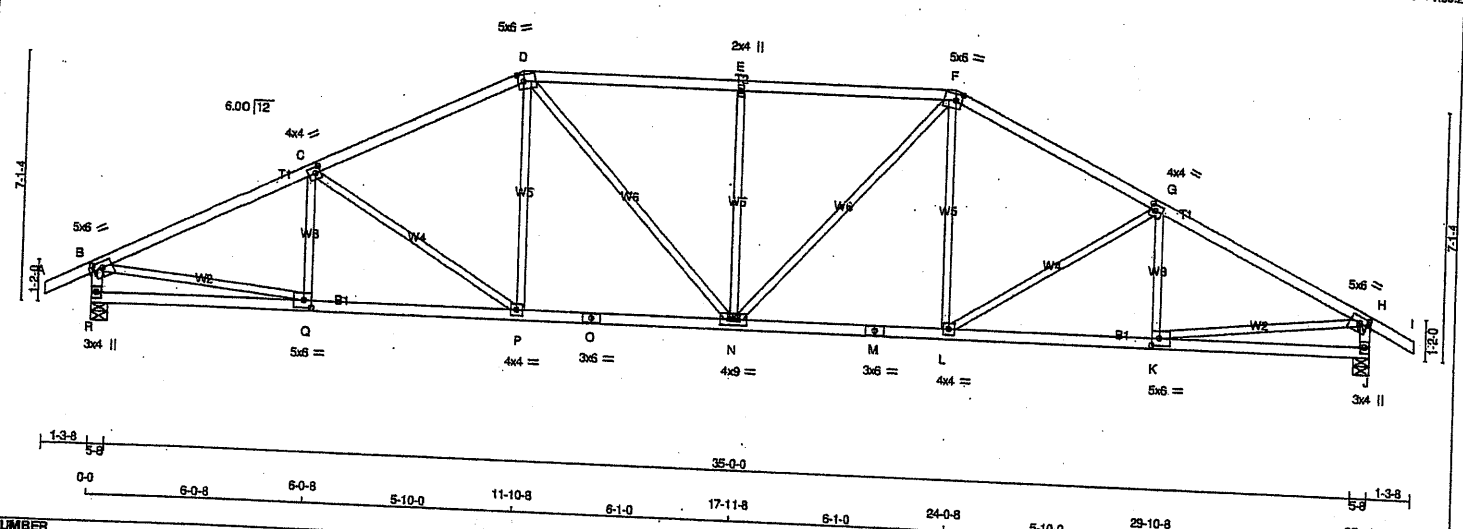
JSI METAL = 0.70 (O) (INPUT = 1.00)



Structural component only
DWG# T-2126463

JOB NAME 420375	TRUSS NAME T4	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed Aug 4 12:41:52 2021 Page 1
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Scale = 1:50.2



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMWW-t	MT20	5.0	6.0	2.00	2.75
I	BMV1-p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.50
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1-p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	JT	GROSS REACTION	JT	BRG	JT	BRG
R	VERT	R	DOWN	R	IN-SX	R	IN-SX
J	HORIZ	J	HORIZ	J	UP-LIFT	J	UP-LIFT
J	1971	J	1971	J	5-8	J	5-8
J	0	J	0	J	5-8	J	5-8
J	0	J	0	J	5-8	J	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	JT	MAX. MIN. COMPONENT REACTIONS	JT	MAX. MIN. COMPONENT REACTIONS	JT	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	JT	SNOW	JT	LIVE	JT	PERM. LIVE
R	1397	R	900 / 0	R	0 / 0	R	0 / 0
J	1397	J	900 / 0	J	0 / 0	J	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)
FR-TO				FR-TO			
A-B	0 / 26	-84.9	-84.9	Q-C	-240 / 8	0.06	0.06
B-C	-2775 / 0	-84.9	-84.9	C-P	-398 / 0	0.38	0.38
C-D	-2457 / 0	-84.9	-84.9	P-D	0 / 332	0.07	0.07
D-E	-2508 / 0	-84.9	-84.9	D-N	0 / 496	0.11	0.11
E-F	-2508 / 0	-84.9	-84.9	N-E	-633 / 0	0.55	0.55
F-G	-2457 / 0	-84.9	-84.9	N-F	0 / 496	0.11	0.11
G-H	-2775 / 0	-84.9	-84.9	L-F	0 / 332	0.07	0.07
H-I	0 / 26	-84.9	-84.9	L-G	-398 / 0	0.38	0.38
J-B	-1922 / 0	0.0	0.0	K-G	-240 / 8	0.06	0.06
J-H	-1922 / 0	0.0	0.0	B-Q	0 / 2530	0.57	0.57
R-Q	0 / 0	-18.5	-18.5	K-H	0 / 2530	0.57	0.57
Q-P	0 / 2505	-18.5	-18.5				
P-O	0 / 2177	-18.5	-18.5				
O-N	0 / 2177	-18.5	-18.5				
N-M	0 / 2177	-18.5	-18.5				
M-L	0 / 2177	-18.5	-18.5				
L-K	0 / 2505	-18.5	-18.5				
K-J	0 / 0	-18.5	-18.5				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (1.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.28")

CSI: TC=0.54/1.00 (G-H:1), BC=0.48/1.00 (K-L:1), WB=0.57/1.00 (H-K:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

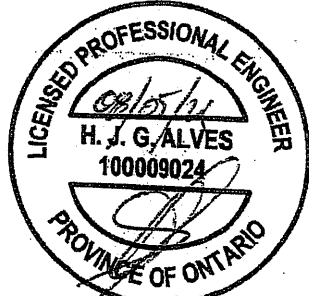
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES:
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

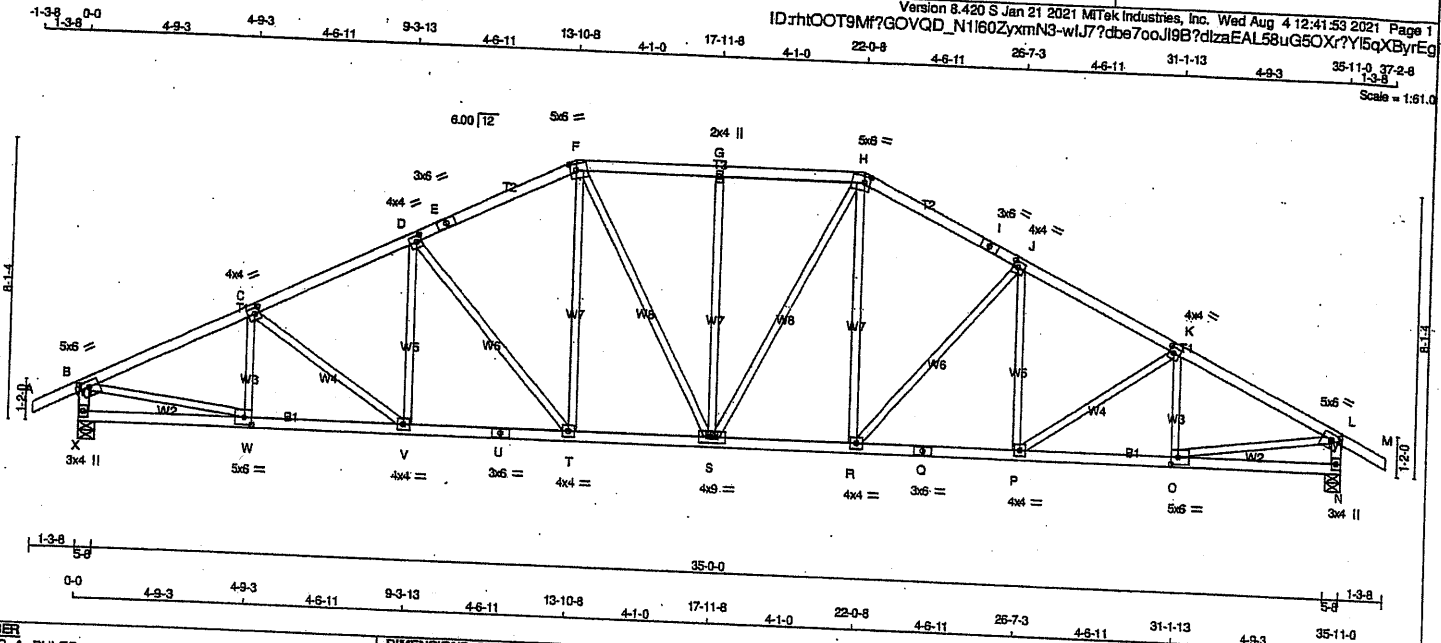
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.67 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126464

JOB NAME 420375	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Wed Aug 4 12:41:53 2021 Page 1	



LUMBER		N. L. G. A. RULES	
CHORDS	SIZE	DRY	LUMBER
A - E	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - M	2x4	DRY	No.2
N - L	2x4	DRY	No.2
X - B	2x4	DRY	No.2
U - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	6.0	2.00	2.75
C, D, J, K					
C TMVW-t	MT20	4.0	4.0	2.00	1.75
E TS-t	MT20	3.0	6.0		
F TTWW-m	MT20	5.0	6.0	2.25	2.00
G TMW-w	MT20	2.0	4.0		
H TTWW-m	MT20	5.0	6.0	2.25	2.00
I TS-t	MT20	3.0	6.0		
L TMVW-t	MT20	5.0	6.0	2.00	2.75
N BMV1-p	MT20	3.0	4.0		
O BMVW-t	MT20	5.0	6.0	2.50	2.50
P, R, T, V					
P BMVW-t	MT20	4.0	4.0		
Q BS-t	MT20	3.0	6.0		
S BMVW-w	MT20	4.0	9.0		
U BS-t	MT20	3.0	6.0		
W BMVW-t	MT20	5.0	6.0	2.50	2.50
X BMV1-p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	IN-SX	BRG	IN-SX	BRG
X	1971	0	0	1971	0	5-8	5-8	5-8	5-8
N	1971	0	0	1971	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
X	1397	900 / 0	0 / 0	0 / 0	0 / 0	0 / 0	498 / 0	498 / 0	0 / 0
N	1397	900 / 0	0 / 0	0 / 0	0 / 0	0 / 0	498 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	LC1 (FLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	W-C	-349 / 0	0.07 (1)
B-C	-2692 / 0	-84.9	-84.9	0.33 (1)	3.96	C-V	-80 / 0	0.04 (1)
C-D	-2634 / 0	-84.9	-84.9	0.28 (1)	4.05	V-D	0 / 133	0.04 (4)
D-E	-2280 / 0	-84.9	-84.9	0.27 (1)	4.31	D-T	-516 / 0	0.52 (1)
E-F	-2280 / 0	-84.9	-84.9	0.27 (1)	4.45	T-F	0 / 478	0.11 (1)
F-G	-2156 / 0	-84.9	-84.9	0.22 (1)	4.45	F-S	0 / 274	0.06 (1)
G-H	-2156 / 0	-84.9	-84.9	0.22 (1)	4.45	S-G	-418 / 0	0.53 (1)
H-I	-2280 / 0	-84.9	-84.9	0.27 (1)	4.31	I-H	0 / 274	0.06 (1)
I-J	-2280 / 0	-84.9	-84.9	0.27 (1)	4.31	R-H	0 / 478	0.11 (1)
J-K	-2634 / 0	-84.9	-84.9	0.28 (1)	4.05	R-J	-516 / 0	0.52 (1)
K-L	-2692 / 0	-84.9	-84.9	0.39 (1)	3.96	P-J	0 / 133	0.04 (4)
L-M	0 / 26	-84.9	-84.9	0.11 (1)	10.00	P-K	-80 / 0	0.04 (1)
X-B	-1930 / 0	0.0	0.0	0.20 (1)	6.06	B-W	-349 / 0	0.07 (1)
N-L	-1930 / 0	0.0	0.0	0.20 (1)	6.06	C-K	0 / 2460	0.55 (1)
						O-L	0 / 2460	0.55 (1)
X-W	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
W-V	0 / 2421	-18.5	-18.5	0.43 (1)	10.00			
V-U	0 / 2356	-18.5	-18.5	0.42 (1)	10.00			
U-T	0 / 2356	-18.5	-18.5	0.42 (1)	10.00			
T-S	0 / 2029	-18.5	-18.5	0.36 (1)	10.00			
S-R	0 / 2029	-18.5	-18.5	0.36 (1)	10.00			
R-Q	0 / 2356	-18.5	-18.5	0.42 (1)	10.00			
Q-P	0 / 2356	-18.5	-18.5	0.42 (1)	10.00			
P-O	0 / 2421	-18.5	-18.5	0.43 (1)	10.00			
O-N	0 / 0	-18.5	-18.5	0.43 (1)	10.00			
		-18.5	-18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (1.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.24")

CSI: TC=0.33/1.00 (K-L:1), BC=0.43/1.00 (O-P:1), WB=0.55/1.00 (L-O:1), SS=0.17/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

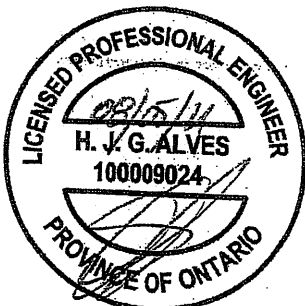
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

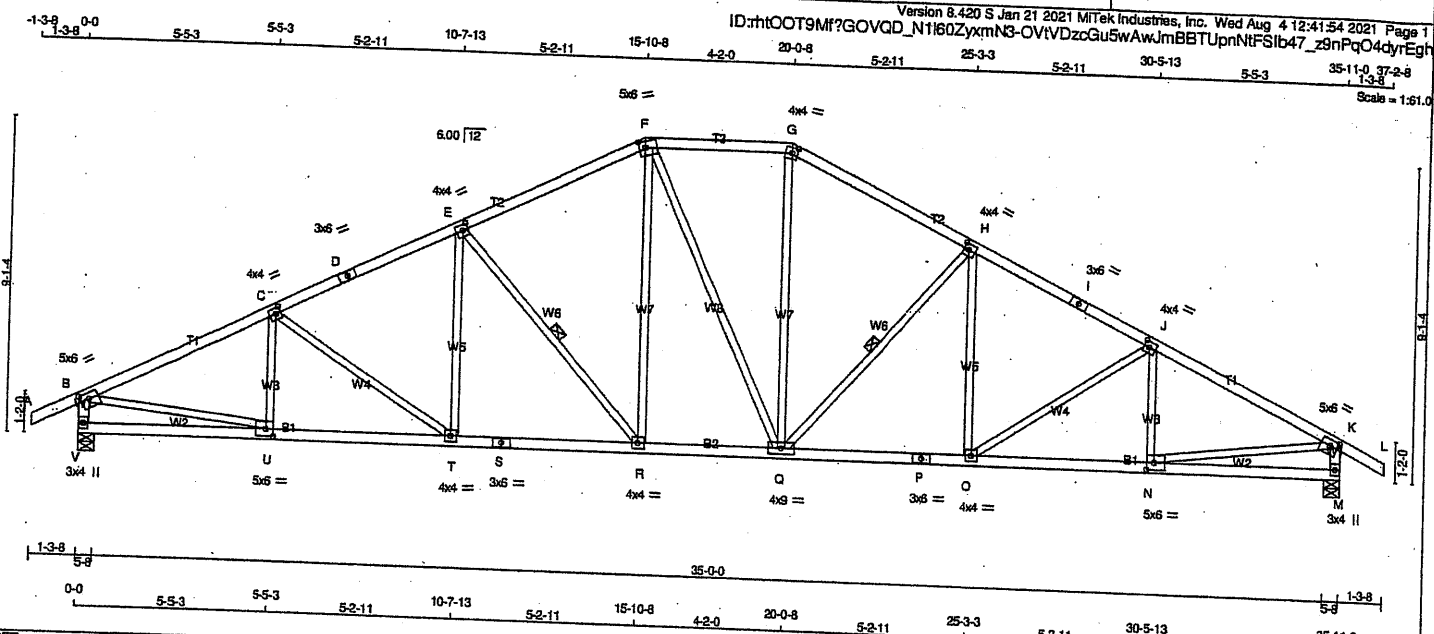
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.89 (O) (INPUT=0.90)
JSI METAL=0.72 (U) (INPUT=1.00)



Structural component only
DWG# T-2126465

JOB NAME 420375	TRUSS NAME T6	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - G	2x4	DRY No.2
G - I	2x4	DRY No.2
I - L	2x4	DRY No.2
L - M	2x4	DRY No.2
M - K	2x4	DRY No.2
K - P	2x4	DRY No.2
P - M	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

LATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00
C	E, H, J				
D	TMVW-t	MT20	4.0	4.0	2.00
E	TS-t	MT20	3.0	6.0	
F	TTWW-m	MT20	5.0	6.0	2.25
G	TTW-m	MT20	4.0	4.0	2.00
I	TS-t	MT20	3.0	6.0	
K	TMVW-t	MT20	5.0	6.0	2.00
M	BMV1+p	MT20	3.0	4.0	
N	BMWW-t	MT20	5.0	6.0	2.50
O	R, T				
P	BMWW-t	MT20	4.0	4.0	
Q	BS-t	MT20	3.0	6.0	
S	BMWWW-t	MT20	4.0	9.0	
U	BS-t	MT20	3.0	6.0	
V	BMWW-t	MT20	5.0	6.0	2.50
X	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
V	1971	0	1971	0	5-8	5-8
M	1971	0	1971	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
V	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO	FR-TO		
A-B	0 / 26	-84.9	-84.9 0.11 (1)	U-C	-287 / 0	0.07 (1)
B-C	-2736 / 0	-84.9	-84.9 0.42 (1)	C-T	-210 / 0	0.15 (1)
C-D	-2561 / 0	-84.9	-84.9 0.35 (1)	T-E	0 / 219	0.05 (4)
D-E	-2561 / 0	-84.9	-84.9 0.35 (1)	E-R	-653 / 0	0.29 (1)
E-F	-2105 / 0	-84.9	-84.9 0.34 (1)	F-Q	0 / 576	0.13 (1)
F-G	-1873 / 0	-84.9	-84.9 0.23 (1)	Q-F	0 / 3	0.00 (1)
G-H	-2106 / 0	-84.9	-84.9 0.34 (1)	H-Q	0 / 580	0.13 (1)
H-I	-2561 / 0	-84.9	-84.9 0.35 (1)	I-J	-651 / 0	0.29 (1)
I-J	-2561 / 0	-84.9	-84.9 0.35 (1)	J-K	0 / 216	0.05 (4)
J-K	-2737 / 0	-84.9	-84.9 0.42 (1)	K-L	-210 / 0	0.15 (1)
K-L	0 / 26	-84.9	-84.9 0.11 (1)	L-U	-287 / 0	0.07 (1)
L-U	-1926 / 0	0.0	0.0 0.19 (1)	U-N	0 / 2494	0.56 (1)
U-N	-1926 / 0	0.0	0.0 0.19 (1)	N-K	0 / 2495	0.56 (1)
N-K	0 / 0	-18.5	-18.5 0.11 (4)			
V-U	0 / 2484	-18.5	-18.5 0.45 (1)			
U-T	0 / 2291	-18.5	-18.5 0.42 (1)			
T-S	0 / 2291	-18.5	-18.5 0.42 (1)			
S-R	0 / 1871	-18.5	-18.5 0.35 (1)			
R-Q	0 / 2290	-18.5	-18.5 0.42 (1)			
Q-P	0 / 2290	-18.5	-18.5 0.42 (1)			
P-O	0 / 2484	-18.5	-18.5 0.44 (1)			
O-N	0 / 0	-18.5	-18.5 0.11 (4)			
N-M						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.42/1.00 (J-K:1), BC=0.45/1.00 (T-U:1),
WB=0.58/1.00 (K-N:1), SS=0.19/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

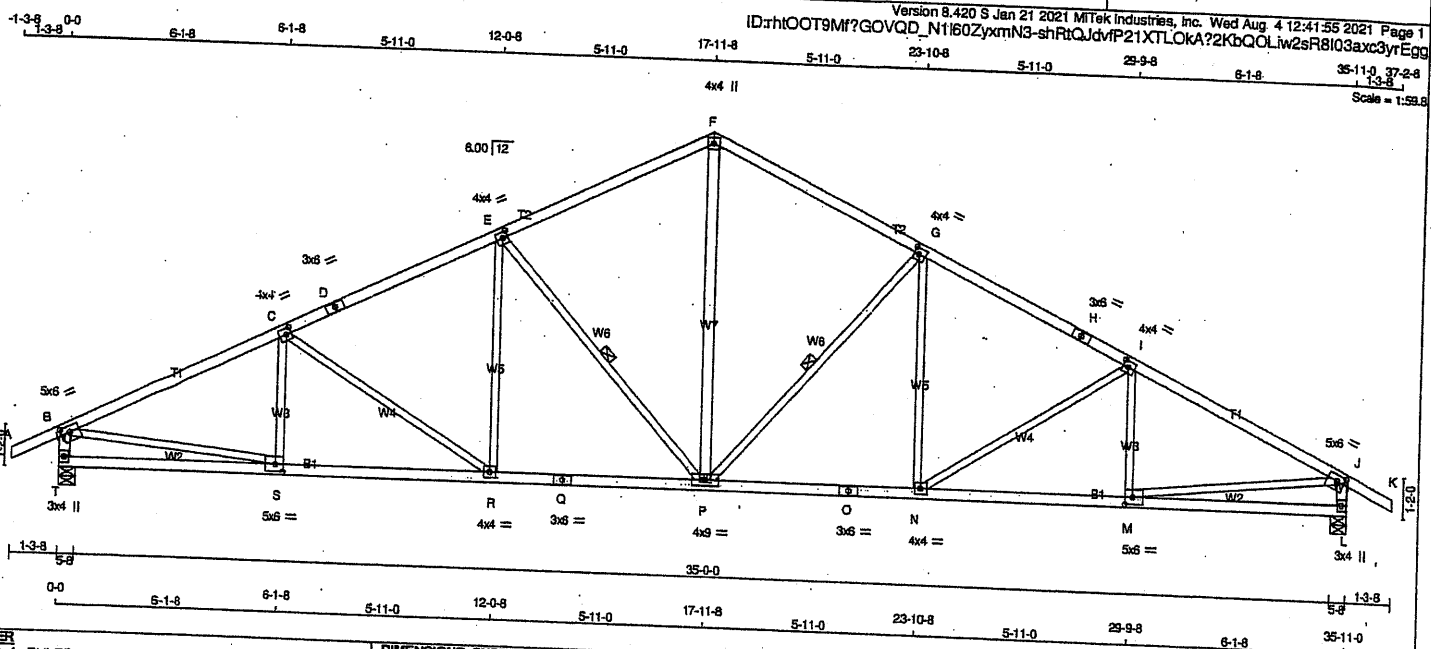
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (N) (INPUT = 0.90)
JSI METAL = 0.68 (S) (INPUT = 1.00)



Structural component only
DWG# T-2126466

JOB NAME 420375 Tamarack Roof Truss, Burlington	TRUSS NAME T7	QUANTITY 9	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - Q	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT P - F	2x3	DRY	No.2

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	5.0	6.0	2.00 2.75
C, E, G, I					
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TS-t	MT20	3.0	6.0	
F	TTW-p	MT20	4.0	4.0	
H	TS-t	MT20	3.0	6.0	
J	TMVW-t	MT20	5.0	6.0	2.00 2.75
L	BMV1-p	MT20	3.0	4.0	
M	BMVW-t	MT20	5.0	6.0	2.50 2.50
N	BMVW-t	MT20	4.0	4.0	
O	BS-t	MT20	3.0	6.0	
P	BS-t	MT20	4.0	9.0	
Q	BS-t	MT20	3.0	6.0	
R	BMVW-t	MT20	4.0	4.0	
S	BMVW-t	MT20	5.0	6.0	2.50 2.50
T	BMV1-p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	UPLIFT		
T	1971	0	1971	0	5-8	5-8
L	1971	0	1971	0	5-8	5-8

UNFACTORED REACTIONS						
JT	1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND PERM. LIVE	DEAD SOIL
	SNOW	LIVE	SNOW	LIVE		
T	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0
L	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P, E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 26	-84.9	-84.9 0.11 (1)	P-F	0 / 1295
B-C	-2764 / 0	-84.9	-84.9 0.54 (1)	P-G	-778 / 0
C-D	-2469 / 0	-84.9	-84.9 0.44 (1)	N-G	0 / 293
D-E	-2469 / 0	-84.9	-84.9 0.44 (1)	N-I	-340 / 0
E-F	-1923 / 0	-84.9	-84.9 0.42 (1)	M-I	-229 / 13
F-G	-1923 / 0	-84.9	-84.9 0.42 (1)	E-P	-778 / 0
G-H	-2469 / 0	-84.9	-84.9 0.44 (1)	R-E	0 / 293
H-I	-2469 / 0	-84.9	-84.9 0.44 (1)	C-R	-340 / 0
I-J	-2764 / 0	-84.9	-84.9 0.54 (1)	S-C	-229 / 13
J-K	0 / 26	-84.9	-84.9 0.11 (1)	B-S	0 / 2515
T-B	-1922 / 0	0.0	0.0 0.19 (1)	M-J	0 / 2515
L-J	-1922 / 0	0.0	0.0 0.19 (1)		
T-S	0 / 0	-18.5	-18.5 0.15 (4)		
S-R	0 / 2491	-18.5	-18.5 0.46 (1)		
R-Q	0 / 2208	-18.5	-18.5 0.44 (1)		
Q-P	0 / 2208	-18.5	-18.5 0.44 (1)		
P-O	0 / 2208	-18.5	-18.5 0.44 (1)		
O-N	0 / 2208	-18.5	-18.5 0.44 (1)		
N-M	0 / 2491	-18.5	-18.5 0.46 (1)		
M-L	0 / 0	-18.5	-18.5 0.15 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.54/1.00 (I-J:1), BC=0.48/1.00 (R-S:1), WB=0.57/1.00 (B-S:1), SS=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

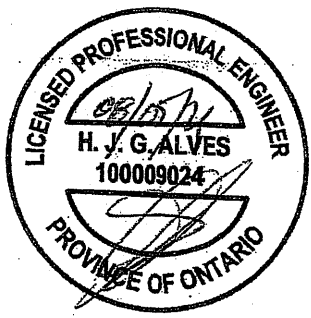
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

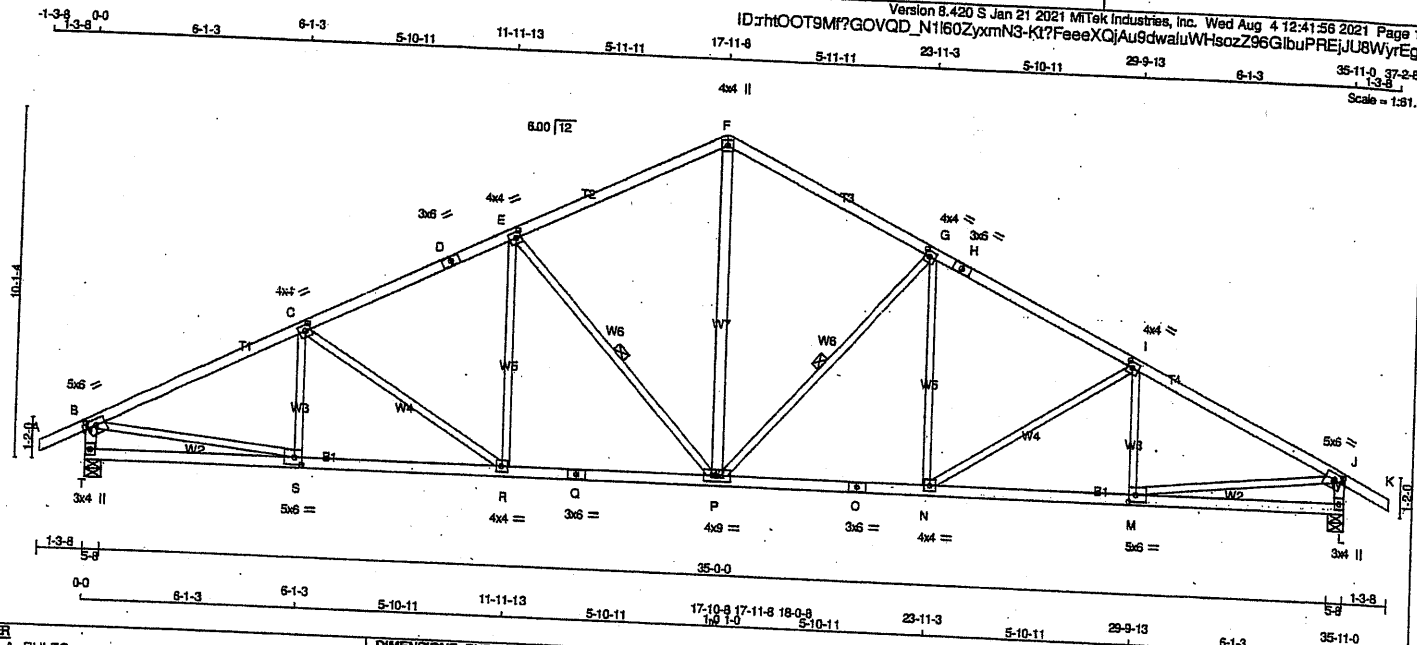
JSI GRIP = 0.89 (J) (INPUT = 0.90)
JSI METAL = 0.68 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2126467

JOB NAME 420375 Tamarack Roof Truss, Burlington	TRUSS NAME T7CP	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 12:41:56 2021 Page 1
ID: JtH00T9M?GOVQD_N1160ZymN3-K1?FeeeXQjAu9dwaluWHsozZ96GibuPREjU8WyrEgf
Scale = 1:51.0



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
P - F	2x4	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW-t	MT20	4.0	6.0	2.00	2.75
C, E, G, I					
C TMW-t	MT20	4.0	6.0	2.00	2.75
D TS-t	MT20	3.0	6.0		
F TTW+p	MT20	4.0	6.0		
H TS-t	MT20	3.0	6.0		
J TMW-t	MT20	5.0	6.0	2.00	2.75
L BMV1+p	MT20	3.0	6.0		
M BMW-t	MT20	5.0	6.0	2.50	2.50
N BMW-t	MT20	4.0	6.0		
O BS-t	MT20	3.0	6.0		
P BMW-t	MT20	4.0	6.0		
Q BS-t	MT20	3.0	6.0		
R BMW-t	MT20	4.0	6.0		
S BMW-t	MT20	5.0	6.0	2.50	2.50
T BMV1+p	MT20	3.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		DOWN	HORIZ	BRG		BRG	
JT	VERT	1971	0	IN-SX		IN-SX	
T	VERT	1971	0	5-8		5-8	
L	VERT	1971	0	5-8		5-8	

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM	LIVE	DEAD		SOIL	
T	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0	0 / 0	0 / 0
L	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	MAX (PL)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX (PL)
FR-TO					FR-TO		
A-B	0 / 26	-84.9	-84.9 0.11 (1)	10.00	S-C	-232 / 11	0.06 (1)
B-C	-2763 / 0	-84.9	-84.9 0.54 (1)	3.72	C-R	-333 / 0	0.32 (1)
C-D	-2473 / 0	-84.9	-84.9 0.44 (1)	4.02	R-E	0 / 290	0.07 (1)
D-E	-2473 / 0	-84.9	-84.9 0.44 (1)	4.02	N-G	0 / 290	0.07 (1)
E-F	-1923 / 0	-84.9	-84.9 0.43 (1)	4.44	N-I	-333 / 0	0.32 (1)
F-G	-1923 / 0	-84.9	-84.9 0.43 (1)	4.44	M-I	-232 / 11	0.06 (1)
G-H	-2473 / 0	-84.9	-84.9 0.44 (1)	4.02	B-S	0 / 2514	0.57 (1)
H-I	-2473 / 0	-84.9	-84.9 0.44 (1)	4.02	M-J	0 / 2514	0.57 (1)
I-J	-2763 / 0	-84.9	-84.9 0.54 (1)	3.72	P-F	0 / 1291	0.21 (1)
J-K	0 / 26	-84.9	-84.9 0.11 (1)	10.00	E-P	-779 / 0	0.44 (1)
T-B	-1922 / 0	0.0	0.0 0.19 (1)	6.07	P-G	-779 / 0	0.44 (1)
L-J	-1922 / 0	0.0	0.0 0.19 (1)	6.07			
T-S	0 / 0	-18.5	-18.5 0.15 (4)	10.00			
S-R	0 / 2490	-18.5	-18.5 0.46 (1)	10.00			
R-Q	0 / 2212	-18.5	-18.5 0.44 (1)	10.00			
Q-P	0 / 2212	-18.5	-18.5 0.44 (1)	10.00			
P-O	0 / 2212	-18.5	-18.5 0.44 (1)	10.00			
O-N	0 / 2212	-18.5	-18.5 0.44 (1)	10.00			
N-M	0 / 2490	-18.5	-18.5 0.46 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.15 (4)	10.00			

TOTAL WEIGHT = 154 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.14")
ALLOWABLE DEFL.(TL) = $L/360$ (1.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.27")

CSI: TC=0.54/1.00 (I-J:1), BC=0.46/1.00 (M-N:1), WB=0.57/1.00 (J-M:1), SSI=0.21/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

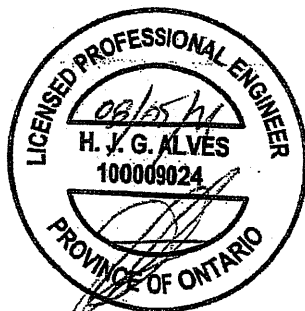
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL = 0.250 inches

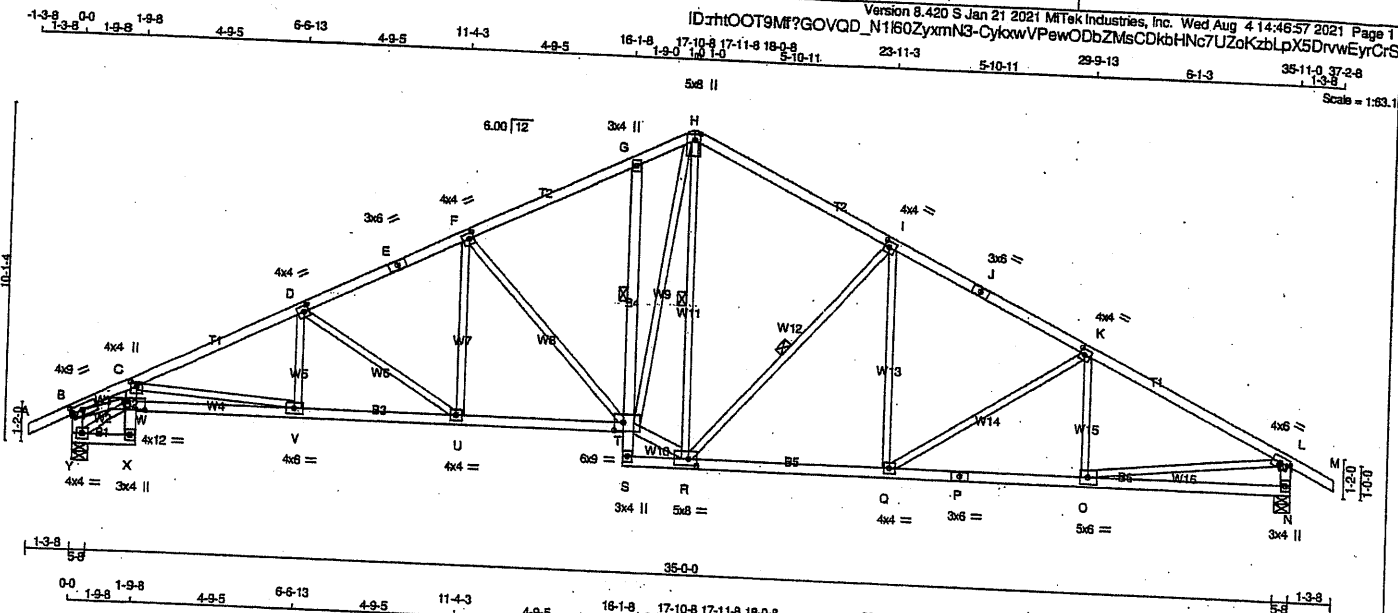
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.68 (C) (INPUT = 1.00)



Structural component only
DWG# T-2126468

JOB NAME 420377	TRUSS NAME T7SCP	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS			
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - M	2x4	DRY	No.2
Y - B	2x4	DRY	No.2
N - L	2x4	DRY	No.2
Y - X	2x4	DRY	No.2
X - C	2x4	DRY	No.2
W - T	2x4	DRY	No.2
S - G	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
T - R	2x4	DRY	No.2

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	9.0	2.00	4.00
C TMVW-p	MT20	4.0	4.0	1.50	2.00
D, F, I, K					
D TMVW-t	MT20	4.0	4.0	2.00	1.75
E TS-t	MT20	3.0	6.0		
G TMV-p	MT20	3.0	4.0		
H TTWW-p	MT20	5.0	8.0	2.25	2.00
J TS-t	MT20	3.0	6.0		
L TMVW-t	MT20	4.0	6.0		
N BMV-p	MT20	3.0	4.0		
O BMVW-t	MT20	3.0	4.0		
P BS-t	MT20	5.0	6.0		
Q BMVW-t	MT20	4.0	4.0		
R BMVW-t	MT20	5.0	8.0	2.25	3.00
S BMV-p	MT20	3.0	4.0		
T BVWVW-t	MT20	6.0	9.0	3.00	3.25
U BMVW-t	MT20	4.0	4.0		
V BMVW-t	MT20	4.0	6.0		
W BVWVW-t	MT20	4.0	12.0	2.75	6.75
X BMV-p	MT20	3.0	4.0		
Y BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	DOWN	UPLIFT	INPUT BRG	RECORD BRG	
N	1313	0	0	1313	0	0	5-8	5-8	
Y	1316	0	0	1316	0	0	5-8	5-8	
UNFACTORED REACTIONS									
JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS					
N	930	599	0	0/0	PERM	LIVE	WIND	DEAD	SOIL
Y	932	601	0	0/0	0/0	0/0	0/0	332/0	0/0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, Y									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T, I-R, H-R.									
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LO)	MAX UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO					FR-TO			FR-TO	
A-B	0/17				C-V	-957/0	0.40 (1)		
B-C	-2993/0	-56.6	0.07 (1)	10.00	D-U	0/128	0.03 (4)		
C-D	-2384/0	-56.6	0.21 (1)	3.89	E-F	-479/0	0.27 (1)		
D-E	-1984/0	-56.6	0.32 (1)	4.19	F-T	0/329	0.07 (1)		
E-F	-1984/0	-56.6	0.22 (1)	4.84	G-H	-558/0	0.50 (1)		
F-G	-1550/0	-56.6	0.08 (1)	5.22	H-I	-515/0	0.30 (1)		
G-H	-1552/0	-56.6	0.16 (1)	5.16	I-J	0/1269	0.30 (1)		
H-I	-1284/0	-56.6	0.08 (1)	5.22	J-K	-223/0	0.21 (1)		
I-J	-1645/0	-56.6	0.27 (1)	5.41	K-L	-153/8	0.04 (1)		
J-K	-1645/0	-56.6	0.26 (1)	4.94	L-M	0/1674	0.38 (1)		
K-L	-1840/0	-56.6	0.32 (1)	4.67	M-N	-152/0	0.11 (1)		
L-M	0/17	-56.6	0.32 (1)	4.67	N-L	-291/0	0.04 (1)		
M-N	-1144/0	-56.6	0.07 (1)	10.00					
N-L	-1280/0	0.0	0.12 (1)	7.43					
		0.0	0.13 (1)	7.13					
Y-X	0/242	-12.3	-0.04 (1)	10.00					
X-W	0/11	0.0	0.06 (1)	10.00					
W-C	0/334	0.0	0.04 (1)	10.00					
W-V	0/3102	-12.3	-0.34 (1)	10.00					
V-U	0/2153	-12.3	-0.38 (1)	10.00					
U-T	0/1753	-12.3	-0.32 (1)	10.00					
S-T	-24/0	0.0	0.04 (1)	7.81					
T-G	-205/0	0.0	0.05 (1)	6.25					
S-R	0/38	-12.3	-0.09 (4)	10.00					
R-Q	0/1472	-12.3	-0.32 (1)	10.00					
Q-P	0/1658	-12.3	-0.31 (1)	10.00					
P-O	0/1658	-12.3	-0.31 (1)	10.00					
O-N	0/0	-12.3	-0.10 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 16.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 088-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
 ALLOWABLE DEFL.(TL) = L/360 (1.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.32/1.00 (C-D:1), BC=0.54/1.00 (V-W:1), WB=0.60/1.00 (B-W:1), SS=0.36/1.00 (C-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

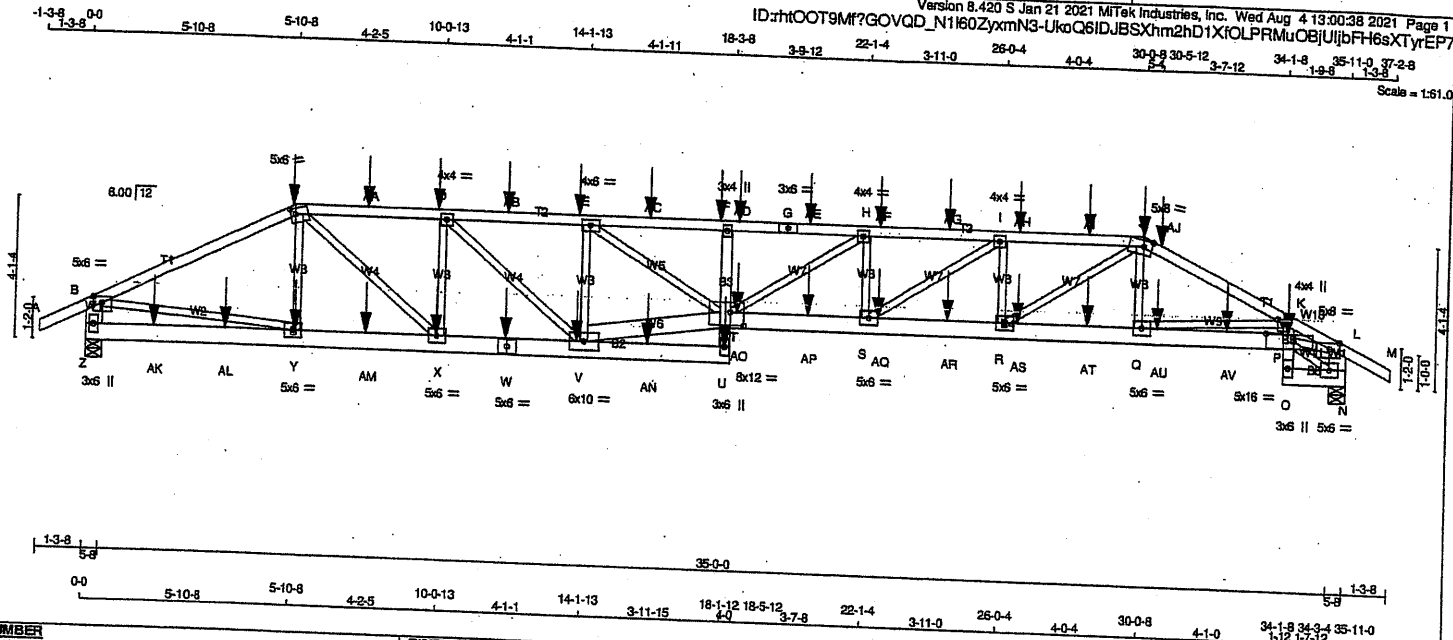
JSI GRIP = 0.86 (R) (INPUT = 0.90)
 JSI METAL = 0.65 (W) (INPUT = 1.00)



Structural component only
 DWG# T-2126493

JOB NAME 420376	TRUSS NAME T18S	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

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ID:ht00T9Mf?GOVQD_N1160ZyxmN3-UkoQ6IDJBSXhm2hD1XfOLPRMuOBjUljbFH6sXTyrEP7
Scale = 1:51.0



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Z - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Z - W	2x6	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
U - F	2x4	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT V - T	2x3	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(91.5)
C - G	12	SIDE(91.5)
G - J	12	SIDE(91.5)
J - M	12	SIDE(91.5)
Z - B	12	TOP
N - L	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Z - W	12	SIDE(274.8)
W - U	12	SIDE(274.8)
T - P	12	SIDE(91.5)
O - N	12	TOP
U - F	12	TOP
K - O	12	SIDE(50.2)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x6	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	VERT	GROSS REACTION	DOWN		
JT	3781	0	3781	0	5-8	5-8
Z	3781	0	3781	0	5-8	5-8
N	4180	0	4180	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS		LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW					
JT	2682	1712 / 0	0 / 0	0 / 0	0 / 0	970 / 0	0 / 0
Z	2682	1712 / 0	0 / 0	0 / 0	0 / 0	1019 / 0	0 / 0
N	2957	1938 / 0	0 / 0	0 / 0	0 / 0	1019 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM	TO			FR-TO			
A-B	0 / 26	-84.9	-84.9	0.04 (1)	10.00	Y-C	-481 / 0	0.04 (1)	
B-C	-5892 / 0	-84.9	-84.9	0.27 (1)	4.60	V-E	-3247 / 0	0.26 (1)	
C-AA	-7627 / 0	-84.9	-84.9	0.21 (1)	4.17	V-T	0 / 8989	0.35 (1)	
AA-D	-7627 / 0	-84.9	-84.9	0.21 (1)	4.17	E-T	0 / 4518	0.34 (1)	
D-AB	-8984 / 0	-84.9	-84.9	0.24 (1)	3.88	Q-J	0 / 7771	0.06 (1)	
AB-E	-8984 / 0	-84.9	-84.9	0.24 (1)	3.88	Q-K	-2909 / 0	0.31 (1)	
E-AC	-12783 / 0	-84.9	-84.9	0.35 (1)	3.22	B-Y	0 / 5311	0.40 (1)	
AC-F	-12783 / 0	-84.9	-84.9	0.35 (1)	3.22	P-N	-1013 / 0	0.05 (1)	
F-AD	-12908 / 0	-84.9	-84.9	0.34 (1)	3.21	P-L	0 / 8778	0.66 (1)	
AD-G	-12908 / 0	-84.9	-84.9	0.34 (1)	3.21	R-J	0 / 3629	0.27 (1)	
G-AE	-12908 / 0	-84.9	-84.9	0.34 (1)	3.21	T-H	0 / 993	0.07 (1)	
AE-H	-12908 / 0	-84.9	-84.9	0.34 (1)	3.21	R-I	-1837 / 0	0.11 (1)	
H-AF	-12909 / 0	-84.9	-84.9	0.30 (1)	3.34	S-H	-1094 / 0	0.08 (1)	
AF-AG	-12099 / 0	-84.9	-84.9	0.30 (1)	3.34	S-I	0 / 2235	0.17 (1)	
AG-I	-12099 / 0	-84.9	-84.9	0.30 (1)	3.34	D-V	0 / 1853	0.14 (1)	
I-AH	-10265 / 0	-84.9	-84.9	0.27 (1)	3.63	C-X	0 / 3175	0.24 (1)	
AH-AI	-10265 / 0	-84.9	-84.9	0.27 (1)	3.63	X-D	-1995 / 0	0.16 (1)	
AI-J	-10265 / 0	-84.9	-84.9	0.27 (1)	3.63				
J-AJ	-8036 / 0	-84.9	-84.9	0.30 (1)	3.99				
AJ-K	-8036 / 0	-84.9	-84.9	0.30 (1)	3.99				
K-L	-9837 / 0	-84.9	-84.9	0.30 (1)	3.99				
L-M	0 / 26	-84.9	-84.9	0.25 (1)	3.86				
Z-B	-3688 / 0	0.0	0.0	0.08 (1)	7.81				
N-L	-3644 / 0	0.0	0.0	0.08 (1)	7.81				
Z-AK	0 / 0	-18.5	-18.5	0.05 (4)	10.00				
AK-AL	0 / 0	-18.5	-18.5	0.05 (4)	10.00				
AL-Y	0 / 0	-18.5	-18.5	0.05 (4)	10.00				
Y-AM	0 / 5253	-18.5	-18.5	0.23 (1)	10.00				
AM-X	0 / 5253	-18.5	-18.5	0.23 (1)	10.00				
X-W	0 / 7628	-18.5	-18.5	0.33 (1)	10.00				
W-V	0 / 7628	-18.5	-18.5	0.33 (1)	10.00				
V-AN	0 / 343	-18.5	-18.5	0.03 (1)	10.00				
AN-U	0 / 343	-18.5	-18.5	0.03 (1)	10.00				
U-T	0 / 105	0.0	0.0	0.16 (1)	10.00				
T-F	-657 / 0	0.0	0.0	0.16 (1)	7.81				
T-AO	0 / 12099	-18.5	-18.5	0.53 (1)	10.00				
AO-AP	0 / 12099	-18.5	-18.5	0.53 (1)	10.00				
AP-S	0 / 12099	-18.5	-18.5	0.53 (1)	10.00				
S-AQ	0 / 10265	-18.5	-18.5	0.47 (1)	10.00				
AQ-AR	0 / 10265	-18.5	-18.5	0.47 (1)	10.00				
AR-R	0 / 10265	-18.5	-18.5	0.47 (1)	10.00				
R-AS	0 / 7259	-18.5	-18.5	0.32 (1)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC0 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.31")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/732 (0.59")

CSI: TC=0.35/1.00 (E-F:1), BC=0.53/1.00 (S-T:1),
WB=0.68/1.00 (L-P:1), SS=0.37/1.00 (K-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION-LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF

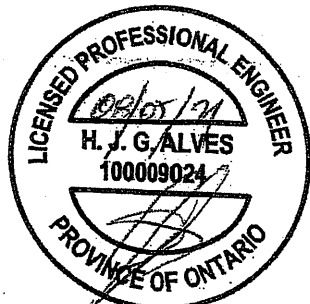
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

CONTINUED ON PAGE 2



Structural component only
DWG# T-2126483

JOB NAME 420376	TRUSS NAME T18S	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

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ID: rhtOOT9Mf?GOVQD N160ZymN3-UkoQ8IDJBSXhm2hD1XfOLPRMuOBiUlibF6sXTvrEP7

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	2.00	3.00
C	FTWW-m	MT20	5.0	8.0	2.00	1.75
D, H, I						
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	8.0		
F	TMV-p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	8.0		
J	TTWW-m	MT20	5.0	8.0	2.25	3.00
K	TMVW-p	MT20	4.0	4.0	1.50	2.00
L	TMVW-p	MT20	5.0	8.0	2.00	3.50
N	BMVW-t	MT20	5.0	6.0		
O	BMV-p	MT20	3.0	6.0		
P	BMVW-t	MT20	5.0	16.0	2.25	9.50
Q, R, S, X, Y						
Q	BMVW-t	MT20	5.0	6.0		
T	BMVW-t	MT20	8.0	12.0	4.50	4.50
U	BMV-p	MT20	3.0	6.0		
V	BMVW-t	MT20	6.0	10.0		
W	BS-t	MT20	5.0	6.0		
Z	BMV1-p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)
FR-TO	0 / 7259	-18.5	-18.5 0.32 (1)	UNBRAC	10.00
AS-AT	0 / 7259	-18.5	-18.5 0.32 (1)	LENGTH FR-TO	10.00
AT-Q	0 / 7259	-18.5	-18.5 0.32 (1)		10.00
Q-AU	0 / 10112	-18.5	-18.5 0.42 (1)		10.00
AU-AV	0 / 10112	-18.5	-18.5 0.42 (1)		10.00
AV-P	0 / 10112	-18.5	-18.5 0.42 (1)		10.00
O-P	0 / 17	0.0	0.0 0.39 (1)		10.00
P-K	0 / 1164	0.0	0.0 0.45 (1)		10.00
C-N	0 / 889	-18.5	-18.5 0.04 (1)		10.00

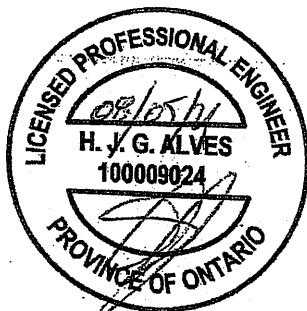
SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-374	-374		FRONT	VERT	TOTAL		C1
D	9-11-4	-113	-113		FRONT	VERT	TOTAL		C1
E	13-11-4	-113	-113		FRONT	VERT	TOTAL		C1
F	17-11-8	-113	-113		FRONT	VERT	TOTAL		C1
J	30-0-8	-30	-30		FRONT	VERT	DEAD		C1
J	30-0-8	-134	-134		FRONT	VERT	SNOW		C1
P	34-3-4	-345	-345		FRONT	VERT	TOTAL		C1
U	18-1-12	-35	-35		FRONT	VERT	TOTAL		C1
V	13-11-4	-29	-29		FRONT	VERT	TOTAL		C1
W	11-11-4	-29	-29		FRONT	VERT	TOTAL		C1
X	9-11-4	-29	-29		FRONT	VERT	TOTAL		C1
Y	5-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AA	7-11-4	-113	-113		FRONT	VERT	TOTAL		C1
AB	11-11-4	-113	-113		FRONT	VERT	TOTAL		C1
AC	15-11-4	-113	-113		FRONT	VERT	TOTAL		C1
AD	18-5-12	-69	-69		FRONT	VERT	TOTAL		C1
AE	20-5-12	-69	-69		FRONT	VERT	TOTAL		C1
AF	22-5-12	-69	-69		FRONT	VERT	TOTAL		C1
AG	24-5-12	-69	-69		FRONT	VERT	TOTAL		C1
AH	26-5-12	-69	-69		FRONT	VERT	TOTAL		C1
AI	28-5-12	-69	-69		FRONT	VERT	TOTAL		C1
AJ	30-5-12	-66	-66		FRONT	VERT	TOTAL		C1
AK	1-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AL	3-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AM	7-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AN	15-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AO	18-5-12	-69	-69		FRONT	VERT	TOTAL		C1
AP	20-5-12	-64	-64		FRONT	VERT	TOTAL		C1
AQ	22-5-12	-64	-64		FRONT	VERT	TOTAL		C1
AR	24-5-12	-64	-64		FRONT	VERT	TOTAL		C1
AS	26-5-12	-64	-64		FRONT	VERT	TOTAL		C1
AT	28-5-12	-64	-64		FRONT	VERT	TOTAL		C1
AU	30-5-12	-64	-64		FRONT	VERT	TOTAL		C1
AV	32-5-12	-64	-64		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP = 0.90 (L) (INPUT = 0.90)
JSI METAL = 0.57 (P) (INPUT = 1.00)



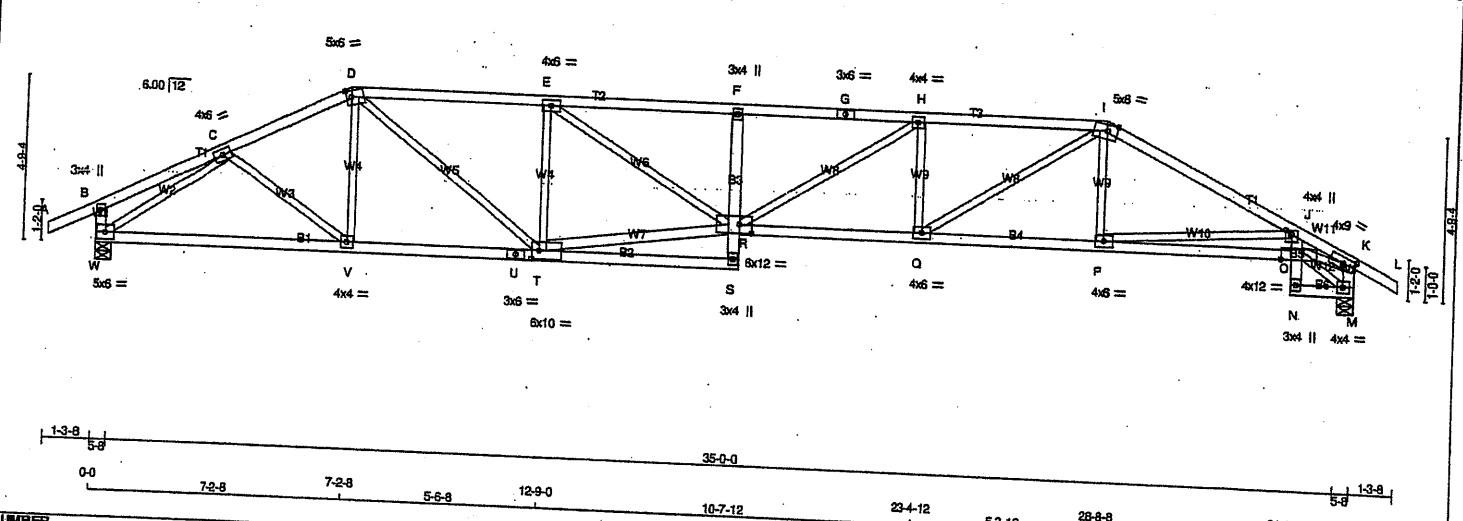
Structural component only
DWG# T-2126483

JOB NAME: 420376 TRUSS NAME: T19S QUANTITY: 1 PLY: 1 JOB DESC.: ROYAL PINE HOMES TRUSS DESC.: Tamarack Roof Truss, Burlington

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ID: rhtOOT9Mf?GOVQD_N1160ZymN3-6JZnAV0VVhj9j5WILmz1qxUONGIASKS5RcB0syr_Dw

Scale = 1:81.0



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. & A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
M - N	2x4	DRY	No.2	SPF
N - O	2x4	DRY	No.2	SPF
O - P	2x4	DRY	No.2	SPF
P - Q	2x4	DRY	No.2	SPF
Q - R	2x4	DRY	No.2	SPF
R - S	2x4	DRY	No.2	SPF
S - T	2x4	DRY	No.2	SPF
T - U	2x4	DRY	No.2	SPF
U - V	2x4	DRY	No.2	SPF
V - W	2x4	DRY	No.2	SPF
W - X	2x4	DRY	No.2	SPF
X - Y	2x4	DRY	No.2	SPF
Y - Z	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
T - R	2x4	DRY	No.2	SPF

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
JT	TMV+P	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMVW-t	MT20	4.0	6.0		
E	TMV+P	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	5.0	6.0	2.25	3.50
I	TMVW+P	MT20	4.0	4.0	1.50	2.00
J	TMVW-t	MT20	4.0	9.0	2.00	4.00
K	BMVW-t	MT20	4.0	4.0		
L	BMV+P	MT20	3.0	4.0		
M	BVMWV-t	MT20	4.0	12.0	3.25	7.00
N	BMVW-t	MT20	4.0	6.0		
O	BMVW-t	MT20	4.0	6.0		
P	BVMWVW-t	MT20	6.0	12.0	3.00	4.25
Q	BMV+P	MT20	3.0	4.0		
R	BMVWVW-t	MT20	6.0	10.0	3.00	2.50
S	BS-t	MT20	3.0	6.0		
T	BMVW-t	MT20	4.0	4.0		
U	BMVW-t	MT20	4.0	4.0		
V	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
VERT	DOWN	IN-SX	IN-SX
1313	0	5-8	5-8
1316	0	5-8	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
W	930	599 / 0 0 / 0 0 / 0 332 / 0 0 / 0
M	932	601 / 0 0 / 0 0 / 0 332 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS	MEMB.	MAX. FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	MAX. FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)
FR-TO													
A-B	0 / 17	-56.6	-56.6	0.07 (1)	10.00	0.03 (4)		C-V	0 / 115	0.03 (4)			
B-C	0 / 8	-56.6	-56.6	0.09 (1)	10.00	0.02 (4)		V-D	0 / 56	0.02 (4)			
C-D	-1804 / 0	-56.6	-56.6	0.13 (1)	4.87	0.20 (1)		D-T	0 / 901	0.20 (1)			
D-E	-2302 / 0	-56.6	-56.6	0.37 (1)	4.19	0.30 (1)		T-E	-910 / 0	0.30 (1)			
E-F	-3182 / 0	-56.6	-56.6	0.46 (1)	3.58	0.38 (1)		T-R	0 / 2254	0.38 (1)			
F-G	-3211 / 0	-56.6	-56.6	0.42 (1)	3.61	0.23 (1)		E-R	0 / 1027	0.23 (1)			
G-H	-3211 / 0	-56.6	-56.6	0.39 (1)	3.61	0.04 (4)		P-I	0 / 189	0.04 (4)			
H-I	-2937 / 0	-56.6	-56.6	0.45 (1)	4.11	0.70 (1)		P-J	-1233 / 0	0.70 (1)			
I-J	-2294 / 0	-56.6	-56.6	0.17 (1)	7.81	0.62 (1)		W-C	-1877 / 0	0.62 (1)			
J-K	-3134 / 0	-56.6	-56.6	0.07 (1)	10.00	0.05 (1)		O-M	-306 / 0	0.05 (1)			
K-L	0 / 17	-56.6	-56.6	0.07 (1)	10.00	0.63 (1)		O-K	0 / 2821	0.63 (1)			
W-B	-181 / 0	0.0	0.0	0.02 (1)	7.81	0.24 (1)		Q-I	0 / 1053	0.24 (1)			
M-K	-1135 / 0	0.0	0.0	0.11 (1)	7.45	0.07 (1)		R-H	0 / 329	0.07 (1)			
W-V	0 / 1511	-12.3	-12.3	0.32 (1)	10.00	0.12 (1)		Q-H	-517 / 0	0.12 (1)			
V-U	0 / 1605	-12.3	-12.3	0.33 (1)	10.00								
U-T	0 / 1605	-12.3	-12.3	0.33 (1)	10.00								
T-S	0 / 101	-12.3	-12.3	0.09 (4)	10.00								
S-R	0 / 32	0.0	0.0	0.14 (1)	10.00								
R-F	-273 / 0	0.0	0.0	0.14 (1)	7.81								
Q-P	0 / 2937	-12.3	-12.3	0.26 (1)	10.00								
P-O	0 / 2056	-12.3	-12.3	0.19 (1)	10.00								
O-P	0 / 3275	-12.3	-12.3	0.28 (1)	10.00								
N-O	0 / 11	0.0	0.0	0.38 (1)	10.00								
O-J	0 / 343	0.0	0.0	0.44 (1)	10.00								
N-M	0 / 254	-12.3	-12.3	0.05 (1)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 16.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (1.20")

CALCULATED VERT. DEFL (LL) = L/969 (0.23")

ALLOWABLE DEFL (TL) = L/360 (1.20")

CALCULATED VERT. DEFL (TL) = L/966 (0.45")

CS1: TC=0.46/1.00 (E-F:1), BC=0.44/1.00 (J-O:1), WB=0.70/1.00 (J-P:1), SS=0.38/1.00 (J-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)

JSI METAL= 0.69 (O) (INPUT = 1.00)



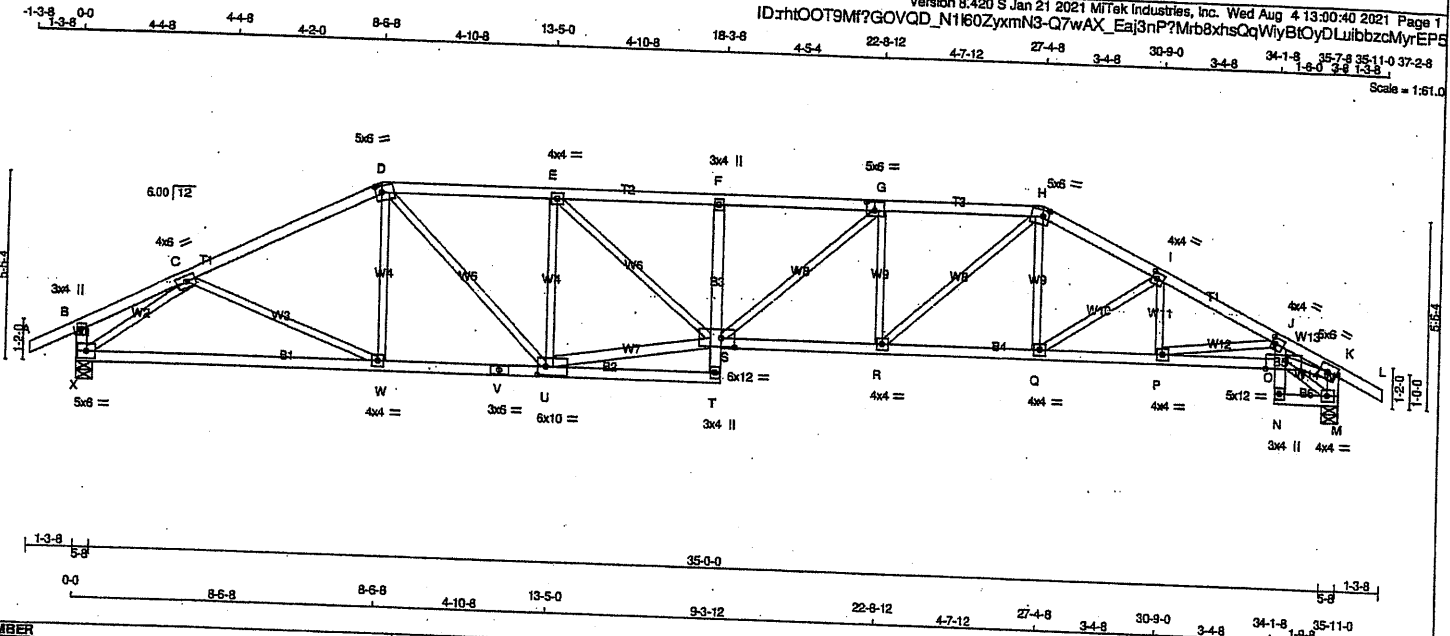
Structural component only

DWG# T-2126484

JOB NAME 420376	TRUSS NAME T20S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 13:00:40 2021 Page 1
ID: rhtOOT9Mf7GOVQD_N160ZyxmN3-Q7wAX_Eaj3nP7Mr8xhsQqWiyBtOyDLuibbzcmrEPs

Scale = 1:61.0



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - L	2x4	DRY	No.2
X - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
X - V	2x4	DRY	No.2
V - T	2x4	DRY	No.2
T - F	2x4	DRY	No.2
S - O	2x4	DRY	No.2
N - J	2x4	DRY	No.2
N - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
U - S	2x4	DRY	No.2

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-t	MT20	4.0	6.0		
D TTWW-m	MT20	5.0	6.0	2.25	2.00
E TMVW-t	MT20	4.0	4.0		
F TMV+p	MT20	3.0	4.0		
G TSWW-t	MT20	5.0	6.0	3.00	2.75
H TTWW-m	MT20	5.0	6.0	2.25	2.00
I TMVW-t	MT20	4.0	4.0	2.00	1.75
J TMVW-t	MT20	4.0	4.0	2.00	1.25
K BMVW-t	MT20	4.0	4.0		
M BMV+p	MT20	3.0	4.0		
N BVWVW-t	MT20	5.0	12.0	Edge	6.75
P, Q, R, W					
S BMVW-t	MT20	4.0	4.0		
T BVWVWVW-t	MT20	6.0	12.0	3.00	4.50
U BMV+p	MT20	3.0	4.0		
V BMVWVW-t	MT20	6.0	10.0	3.00	2.75
V BS-t	MT20	3.0	6.0		
X BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	IN-SX	IN-SX
X	1313	0	1313	0	5-8	5-8	5-8
M	1316	0	1316	0	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS	
JT	1ST LCASE	JT	1ST LCASE
X	930	599 / 0	0 / 0
M	932	601 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MEMB.		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)
A-B	0 / 17	A-B	0 / 17	A-B	0 / 17
B-C	0 / 19	B-C	0 / 19	B-C	0 / 19
C-D	-1779 / 0	C-D	-1779 / 0	C-D	-1779 / 0
D-E	-2038 / 0	D-E	-2038 / 0	D-E	-2038 / 0
E-F	-2681 / 0	E-F	-2681 / 0	E-F	-2681 / 0
F-G	-2697 / 0	F-G	-2697 / 0	F-G	-2697 / 0
G-H	-2486 / 0	G-H	-2486 / 0	G-H	-2486 / 0
H-I	-2180 / 0	H-I	-2180 / 0	H-I	-2180 / 0
I-J	-2498 / 0	I-J	-2498 / 0	I-J	-2498 / 0
J-K	-2930 / 0	J-K	-2930 / 0	J-K	-2930 / 0
K-L	0 / 17	K-L	0 / 17	K-L	0 / 17
L-M	-121 / 0	L-M	-121 / 0	L-M	-121 / 0
M-K	-1147 / 0	M-K	-1147 / 0	M-K	-1147 / 0
X-W	0 / 1476	X-W	0 / 1476	X-W	0 / 1476
W-V	0 / 1581	W-V	0 / 1581	W-V	0 / 1581
V-U	0 / 1581	V-U	0 / 1581	V-U	0 / 1581
U-T	0 / 68	U-T	0 / 68	U-T	0 / 68
T-S	0 / 30	T-S	0 / 30	T-S	0 / 30
S-F	-279 / 0	S-F	-279 / 0	S-F	-279 / 0
F-R	0 / 2466	F-R	0 / 2466	F-R	0 / 2466
R-O	0 / 1943	R-O	0 / 1943	R-O	0 / 1943
O-P	0 / 2262	O-P	0 / 2262	O-P	0 / 2262
P-O	0 / 3025	P-O	0 / 3025	P-O	0 / 3025
N-O	0 / 11	N-O	0 / 11	N-O	0 / 11
O-J	0 / 348	O-J	0 / 348	O-J	0 / 348
N-M	0 / 236	N-M	0 / 236	N-M	0 / 236

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 16.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.36")

CSI: TC=0.32/1.00 (G-H:1), BC=0.52/1.00 (O-P:1), WB=0.59/1.00 (K-O:1), SS=0.35/1.00 (J-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (K) (INPUT = 0.90)
JSI METAL = 0.88 (K) (INPUT = 1.00)

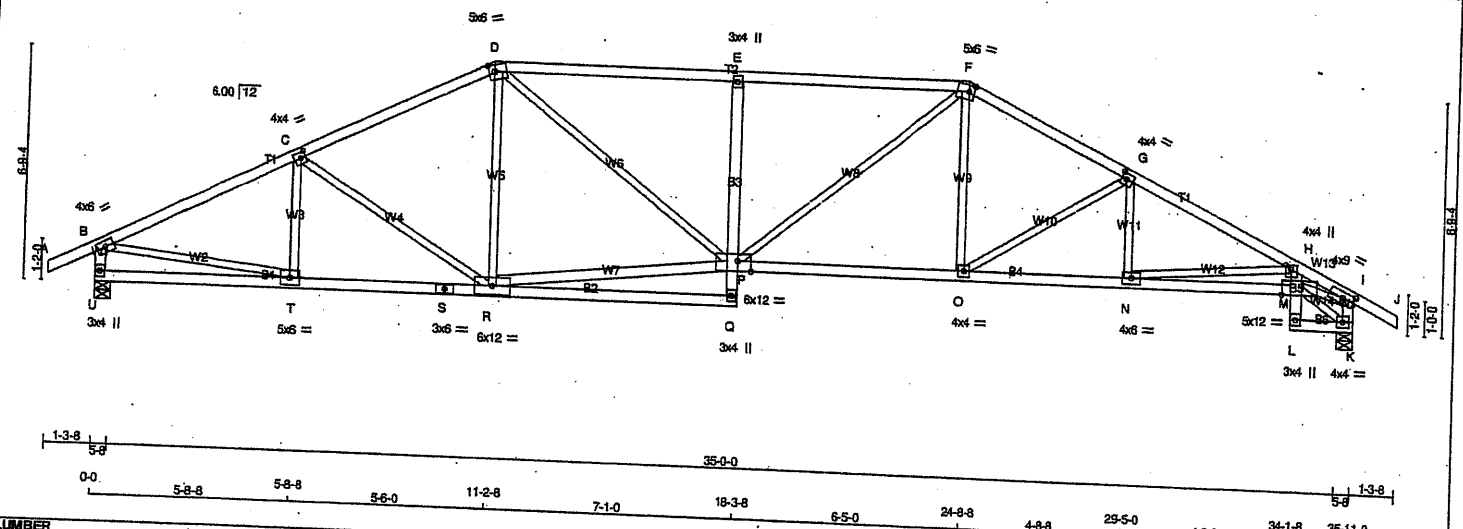


Structural component only
DWG# T-2126485

JOB NAME 420376	TRUSS NAME T22S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 13:00:42 2021 Page 1
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Scale = 1:51.0



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - J	2x4	DRY	No.2	SPF	
U - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
U - S	2x4	DRY	No.2	SPF	
S - Q	2x4	DRY	No.2	SPF	
Q - E	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
L - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
R - P	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY									
BUILDING DESIGNER									
BEARINGS									
FACTORED		MAXIMUM FACTORED		INPUT		REQD			
GROSS REACTION		GROSS REACTION		BRG		BRG			
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX		IN-SX	
U	1313	0	1313	0	0	5-8		5-8	
K	1316	0	1316	0	0	5-8		5-8	

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
K	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 23.3 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 36.7 PSF
SPACING = 16.0 IN. C/C	

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")

CALCULATED VERT. DEFL.(LL) = L/999 (0.15")

ALLOWABLE DEFL.(TL) = L/360 (1.20")

CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.43/1.00 (D-E:1), BC=0.54/1.00 (M-N:1), WB=0.60/1.00 (H-M:1), SS=0.36/1.00 (H-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
650	371	1747
788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (D) (INPUT = 0.90)

JSI METAL = 0.52 (M) (INPUT = 1.00)

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TTVW-m	MT20	5.0	6.0	2.25 2.00
E	TMV+p	MT20	3.0	4.0	
F	TTVW-m	MT20	5.0	6.0	2.25 2.00
G	TMVW-t	MT20	4.0	4.0	2.00 1.75
H	TMVW+p	MT20	4.0	4.0	1.50 2.00
I	TMVW-t	MT20	4.0	9.0	2.00 4.00
K	BMVW-t	MT20	4.0	4.0	
L	BMV+p	MT20	3.0	4.0	
M	BMVW-t	MT20	5.0	12.0	Edge 6.75
N	BMVW-t	MT20	4.0	6.0	
O	BMVW-t	MT20	4.0	4.0	
P	BMVW-t	MT20	6.0	12.0	Edge 4.50
Q	BMV+p	MT20	3.0	4.0	
R	BMVW-t	MT20	6.0	12.0	
S	BS-t	MT20	3.0	6.0	
T	BMVW-t	MT20	5.0	6.0	
U	BMV+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

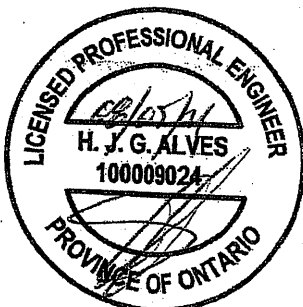
BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.

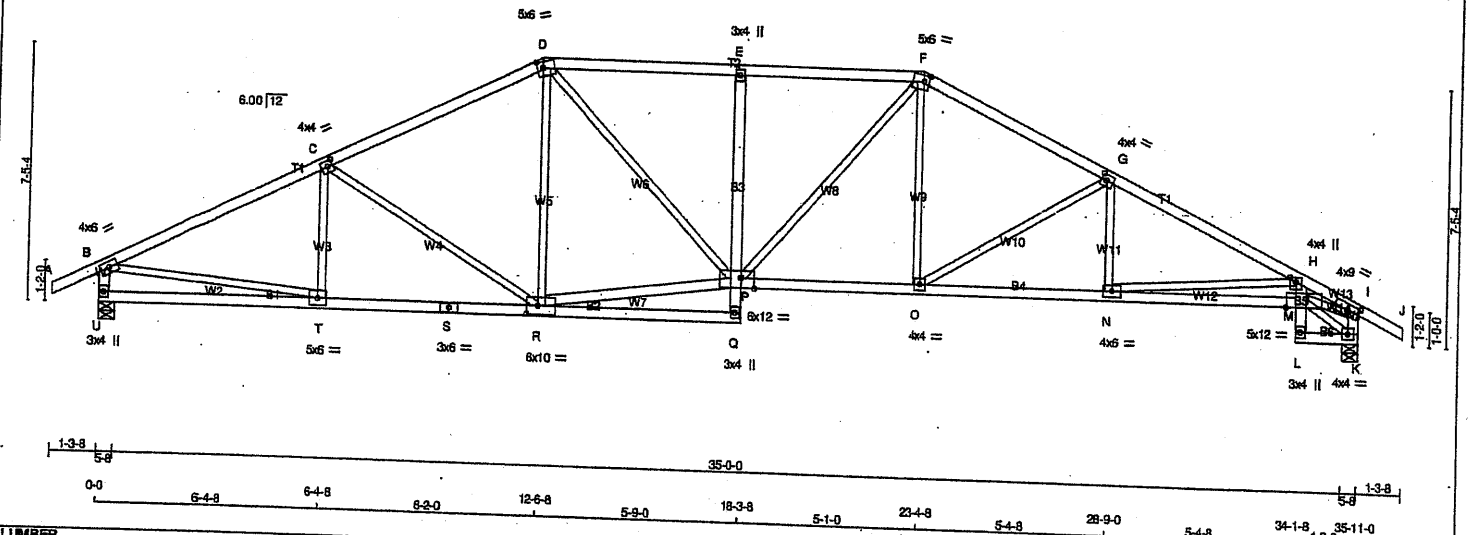
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO					FR-TO				
A-B	0 / 17	-56.6	-56.6	0.07 (1)	10.00	T-C	-185 / 0	0.05 (1)	
B-C	-1837 / 0	-56.6	-56.6	0.28 (1)	4.70	C-R	-213 / 0	0.18 (1)	
C-D	-1673 / 0	-56.6	-56.6	0.27 (1)	4.89	R-D	0 / 81	0.02 (4)	
D-E	-2070 / 0	-56.6	-56.6	0.43 (1)	4.32	D-P	0 / 1447	0.23 (1)	
E-F	-2079 / 0	-56.6	-56.6	0.41 (1)	4.32	P-F	0 / 751	0.17 (1)	
F-G	-1963 / 0	-56.6	-56.6	0.24 (1)	4.61	F-P	0 / 493	0.10 (1)	
G-H	-2354 / 0	-56.6	-56.6	0.21 (1)	4.19	O-F	0 / 359	0.08 (1)	
H-I	-2978 / 0	-56.6	-56.6	0.21 (1)	3.88	O-G	-506 / 0	0.28 (1)	
I-J	0 / 17	-56.6	-56.6	0.07 (1)	10.00	N-G	0 / 117	0.03 (4)	
U-B	-1280 / 0	0.0	0.0	0.13 (1)	7.13	N-H	-925 / 0	0.37 (1)	
K-I	-1144 / 0	0.0	0.0	0.12 (1)	7.43	B-T	0 / 1676	0.39 (1)	
						M-K	-289 / 0	0.04 (1)	
						M-I	0 / 2652	0.60 (1)	
U-T	0 / 0	-12.3	-12.3	0.09 (4)	10.00				
T-S	0 / 1658	-12.3	-12.3	0.34 (1)	10.00				
S-R	0 / 1658	-12.3	-12.3	0.34 (1)	10.00				
R-Q	0 / 48	-12.3	-12.3	0.13 (4)	10.00				
Q-P	0 / 39	-12.3	-12.3	0.13 (4)	10.00				
P-E	-468 / 0	0.0	0.0	0.07 (1)	10.00				
P-O	0 / 1747	0.0	0.0	0.17 (1)	7.81				
O-N	0 / 2165	-12.3	-12.3	0.34 (1)	10.00				
N-M	0 / 3083	-12.3	-12.3	0.41 (1)	10.00				
L-M	0 / 11	-12.3	-12.3	0.54 (1)	10.00				
M-H	0 / 334	0.0	0.0	0.36 (1)	10.00				
L-K	0 / 240	-12.3	-12.3	0.04 (1)	10.00				



Structural component only
DWG# T-2126487

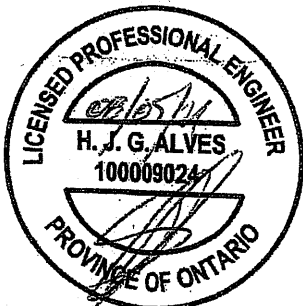


LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
U - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
U - S	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - E	2x4	DRY	No.2
P - M	2x4	DRY	No.2
L - H	2x4	DRY	No.2
L - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
R - P	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
J	TYPE	PLATES	W	LEN	Y	X
B	TMWV-i	MT20	4.0	6.0		
C	TMWV-i	MT20	4.0	6.0	2.00	1.75
D	TTWV-m	MT20	5.0	6.0	2.25	2.00
E	TMV+p	MT20	3.0	4.0		
F	TTWV-m	MT20	5.0	6.0	2.25	2.00
G	TMWV-i	MT20	4.0	4.0	2.00	1.75
H	TMWV+p	MT20	4.0	4.0	1.50	2.00
I	TMWV-i	MT20	4.0	9.0	2.00	4.00
K	BMWV-i	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BMWV-W-I	MT20	5.0	12.0	Edge	6.75
N	BMWV-i	MT20	4.0	6.0		
O	BMWV-i	MT20	4.0	4.0		
P	BMVWV-W-I	MT20	6.0	12.0	Edge	4.50
Q	BMV+p	MT20	3.0	4.0		
R	BMWVW-i	MT20	6.0	10.0	3.00	4.00
S	BS-i	MT20	3.0	6.0		
T	BMWV-i	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only.
DWG# T-2126488

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQD
	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
U	1313	0	1313	0	0	5-8	5-8
K	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
COMBINED		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
U	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U. K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				FACTORED			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)			
FR-TO		FROM	TO		FR-TO					
A-B	0 / 17	-56.6	-56.6	0.07 (1)	10.00	T-C	-145 / 13	0.04 (1)		
B-C	-1853 / 0	-56.6	-56.6	0.35 (1)	4.60	C-R	-306 / 0	0.34 (1)		
C-D	-1806 / 0	-56.6	-56.6	0.33 (1)	4.90	R-D	0 / 55	0.02 (4)		
D-E	-1830 / 0	-56.6	-56.6	0.26 (1)	4.72	R-P	0 / 1407	0.23 (1)		
E-F	-1835 / 0	-56.6	-56.6	0.25 (1)	4.72	D-P	0 / 610	0.14 (1)		
F-G	-1853 / 0	-56.6	-56.6	0.29 (1)	4.67	P-F	0 / 291	0.07 (1)		
G-H	-2347 / 0	-56.6	-56.6	0.35 (1)	4.18	O-F	0 / 376	0.08 (1)		
H-I	-3007 / 0	-56.6	-56.6	0.19 (1)	3.89	C-G	-569 / 0	0.43 (1)		
I-J	0 / 17	-56.6	-56.6	0.07 (1)	10.00	N-G	0 / 135	0.04 (4)		
U-B	-1278 / 0	0.0	0.0	0.13 (1)	7.13	H-N	-1004 / 0	0.53 (1)		
K-I	-1143 / 0	0.0	0.0	0.12 (1)	7.43	B-T	0 / 1689	0.38 (1)		
						M-K	-292 / 0	0.60 (1)		
						M-I	0 / 2684	0.04 (1)		
U-T	0 / 0	-12.3	-12.3	0.10 (4)	10.00					
T-S	0 / 1674	-12.3	-12.3	0.33 (1)	10.00					
S-R	0 / 1674	-12.3	-12.3	0.33 (1)	10.00					
R-Q	0 / 34	-12.3	-12.3	0.10 (4)	10.00					
Q-P	0 / 30	0.0	0.0	0.05 (1)	10.00					
P-E	-373 / 0	0.0	0.0	0.17 (1)	7.81					
P-O	0 / 1646	-12.3	-12.3	0.31 (1)	10.00					
O-N	0 / 2122	-12.3	-12.3	0.39 (1)	10.00					
N-M	0 / 3119	-12.3	-12.3	0.55 (1)	10.00					
L-M	0 / 11	0.0	0.0	0.37 (1)	10.00					
M-H	0 / 331	0.0	0.0	0.42 (1)	10.00					
L-K	0 / 243	-12.3	-12.3	0.04 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

$$\text{ALLOWABLE DEFL (LL)} = L/360 \text{ (1.20")}$$

CALCULATED VERT. DEF_L(LL) = $L/999$ (0.14")

ALLOWABLE DEFL(TL)= L/360 (1.20")
CALCULATED VERT DEFL(TL) = 1.222 IN

ASSOCIATED VERT. DEFL (TL) = 1/999 (0.28")

CSI: TC=0.36/1.00 (B-C:1), BC=0.55/1.00 (M-N:1).

W.B=0.60/1.00 (I-M:1) , SSI=0.36/1.00 (H-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

INTERCOMMITTEE

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

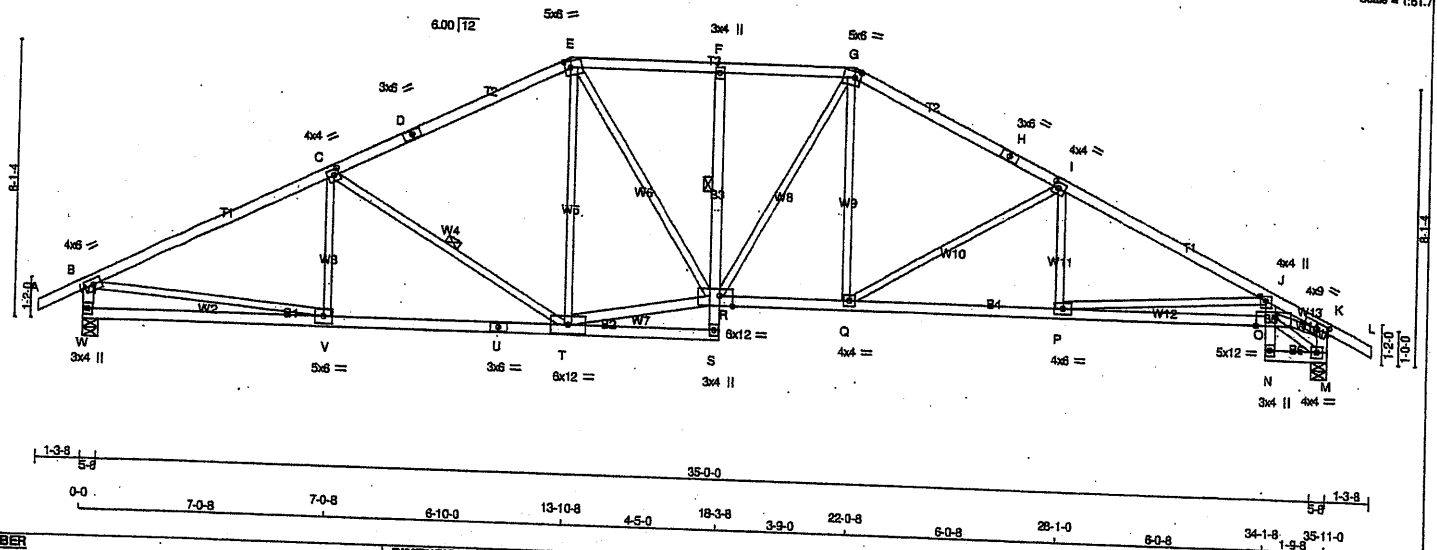
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)

USI METAL= 0.52 (M) (INPUT = 1.00)

JOB NAME: 420376 TRUSS NAME: T24S QUANTITY: 1 PLY: 1 JOB DESC: ROYAL PINE HOMES TRUSS DESC: Tamarack Roof Truss, Burlington



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	8.0		
C TMVW-t	MT20	4.0	4.0	2.00	1.75
D TS-t	MT20	3.0	6.0		
E TTWW-m	MT20	5.0	6.0	2.25	2.00
F TMV-p	MT20	3.0	4.0		
G TTWW-m	MT20	5.0	6.0	2.25	2.00
H TS-t	MT20	3.0	6.0		
I TMVW-t	MT20	4.0	4.0	2.00	1.75
J TMVW-p	MT20	4.0	4.0	1.50	2.00
K TMVW-t	MT20	4.0	9.0	2.00	4.00
M BMVW-t	MT20	4.0	4.0		
N BMV-p	MT20	3.0	4.0		
O BVWVW-t	MT20	5.0	12.0	Edge	8.75
P BMVW-t	MT20	4.0	4.0		
Q BVWVW-t	MT20	6.0	12.0	Edge	4.50
R BMV-p	MT20	3.0	4.0		
T BMVWVW-t	MT20	6.0	12.0		
U BS-t	MT20	3.0	6.0		
V BMVW-t	MT20	5.0	6.0		
W BMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT	1313	0	1313	0	5-8	5-8
W	1316	0	1316	0	5-8	5-8
M	1316	0	1316	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
W	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
M	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-R, C-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CS1 (LC)	MAX. UNBRACED LENGTH	W E B S	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
			FROM	TO						
A-B		0 / 17	-56.6	-56.6	0.07 (1)	10.00	V-C	-112 / 35	0.04 (1)	
B-C		-1861 / 0	-56.6	-56.6	0.46 (1)	4.49	C-T	-392 / 0	0.18 (1)	
C-D		-1534 / 0	-56.6	-56.6	0.41 (1)	4.88	T-E	-23 / 42	0.03 (1)	
D-E		-1534 / 0	-56.6	-56.6	0.41 (1)	4.88	T-R	0 / 1365	0.22 (1)	
E-F		-1639 / 0	-56.6	-56.6	0.14 (1)	5.06	E-R	0 / 539	0.12 (1)	
F-G		-1642 / 0	-56.6	-56.6	0.14 (1)	5.06	E-R	0 / 539	0.12 (1)	
G-H		-1746 / 0	-56.6	-56.6	0.35 (1)	4.71	Q-G	0 / 397	0.09 (1)	
H-I		-1746 / 0	-56.6	-56.6	0.35 (1)	4.71	Q-G	0 / 397	0.09 (1)	
I-J		-2300 / 0	-56.6	-56.6	0.42 (1)	4.16	P-I	0 / 150	0.04 (4)	
J-K		-3039 / 0	-56.6	-56.6	0.17 (1)	3.89	P-J	-1065 / 0	0.76 (1)	
K-L		0 / 17	-56.6	-56.6	0.07 (1)	7.13	O-M	-296 / 0	0.04 (1)	
W-B		-1276 / 0	0.0	0.0	0.13 (1)	7.44	O-K	0 / 2718	0.61 (1)	
M-K		-1141 / 0	0.0	0.0	0.12 (1)	7.44	O-K	0 / 2718	0.61 (1)	
W-V		0 / 0	-12.3	-12.3	0.14 (4)	10.00				
V-U		0 / 1683	-12.3	-12.3	0.33 (1)	10.00				
U-T		0 / 1683	-12.3	-12.3	0.33 (1)	10.00				
T-S		0 / 24	-12.3	-12.3	0.09 (4)	10.00				
S-R		0 / 20	0.0	0.0	0.04 (1)	10.00				
R-F		-276 / 0	0.0	0.0	0.03 (1)	6.25				
Q-P		0 / 1549	-12.3	-12.3	0.29 (1)	10.00				
P-O		0 / 2079	-12.3	-12.3	0.39 (1)	10.00				
N-O		0 / 3158	-12.3	-12.3	0.56 (1)	10.00				
O-J		0 / 11	0.0	0.0	0.37 (1)	10.00				
C-J		0 / 331	0.0	0.0	0.42 (1)	10.00				
N-M		0 / 246	-12.3	-12.3	0.05 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	DL =	23.3	PSF
BOT CH.	DL =	6.0	PSF
	LL =	0.0	PSF
	LL =	7.4	PSF
TOTAL LOAD		36.7	PSF

SPACING = 16.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")

CALCULATED VERT. DEFL.(LL) = L/999 (0.14")

ALLOWABLE DEFL.(TL) = L/360 (1.20")

CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.46/1.00 (B-C:1), BC=0.58/1.00 (O-P:1), WB=0.76/1.00 (J-P:1), SS=0.37/1.00 (J-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR SECTION (PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788

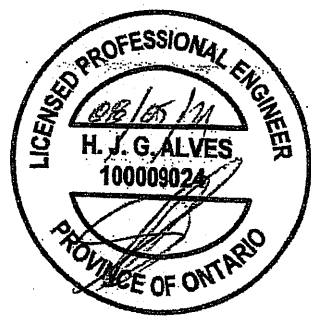
1787

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT= 0.90)

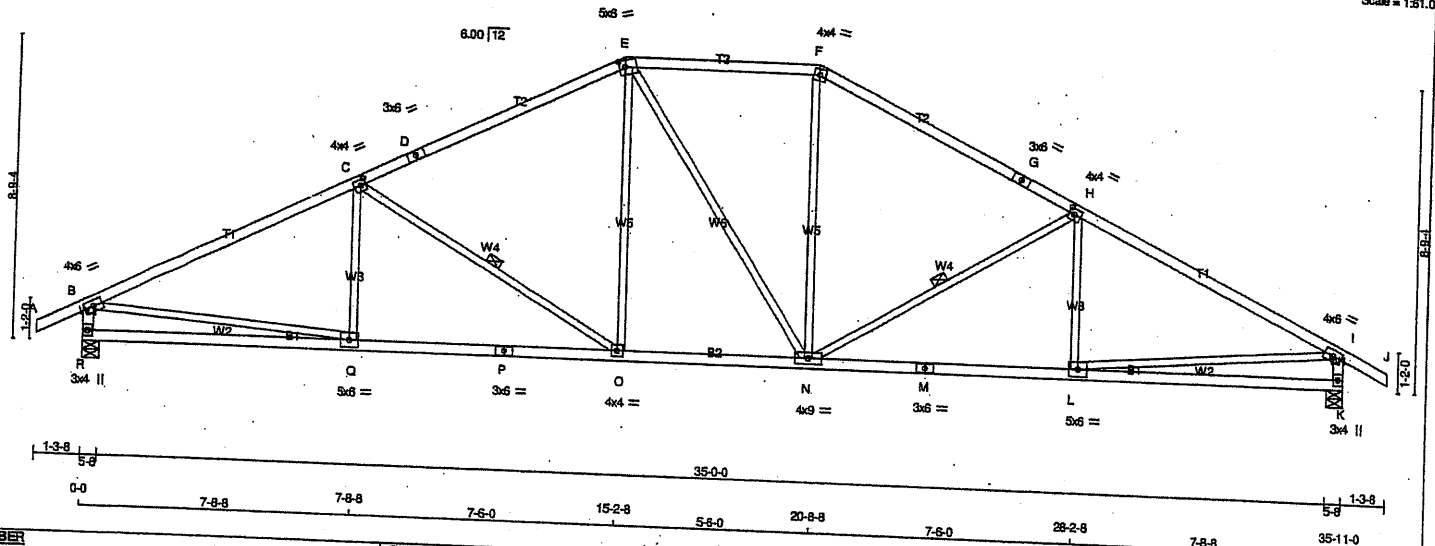
JSI METAL= 0.53 (O) (INPUT= 1.00)



Structural component only
DWG# T-2126489

JOB NAME 420376	TRUSS NAME T25	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

Version 8.420 9 Jan 21 2021 MTEK Industries, Inc. Wed Aug 4 13:00:46 2021 Page 1
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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
R - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TS-t	MT20	3.0	6.0	
E	TTW-m	MT20	5.0	6.0	2.25 2.00
F	TS-t	MT20	4.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	4.0	2.00 1.75
I	TMVW-t	MT20	4.0	6.0	
K	BMV1-p	MT20	3.0	4.0	
L	BMVW-t	MT20	5.0	6.0	
M	BS-t	MT20	3.0	6.0	
N	BMVW-t	MT20	4.0	9.0	
O	BMVW-t	MT20	4.0	4.0	
P	BS-t	MT20	3.0	6.0	
Q	BMVW-t	MT20	5.0	6.0	
R	BMV1-p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
R	1314	0	1314	0	0	5-8
K	1314	0	1314	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
R	931	600 / 0	0 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
K	931	600 / 0	0 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 17			
B-C	-1864 / 0	-56.6	-56.6	0.07 (1)
C-D	-1457 / 0	-56.6	-56.6	0.57 (1)
D-E	-1457 / 0	-56.6	-56.6	0.50 (1)
E-F	-1286 / 0	-56.6	-56.6	0.50 (1)
F-G	-1458 / 0	-56.6	-56.6	0.24 (1)
G-H	-1458 / 0	-56.6	-56.6	0.50 (1)
H-I	-1863 / 0	-56.6	-56.6	0.57 (1)
I-J	0 / 17	-56.6	-56.6	0.07 (1)
J-K	-1275 / 0	0.0	0.0	0.13 (1)
K-L	-1275 / 0	0.0	0.0	0.13 (1)
R-Q	0 / 0	-12.3	-12.3	0.18 (4)
Q-P	0 / 1687	-12.3	-12.3	0.36 (1)
P-O	0 / 1687	-12.3	-12.3	0.36 (1)
O-N	0 / 1286	-12.3	-12.3	0.26 (1)
N-M	0 / 1687	-12.3	-12.3	0.36 (1)
M-L	0 / 1687	-12.3	-12.3	0.36 (1)
L-K	0 / 0	-12.3	-12.3	0.18 (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.57/1.00 (B-C:1), BC=0.36/1.00 (C-Q:1), WB=0.38/1.00 (B-Q:1), SS=0.19/1.00 (H-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

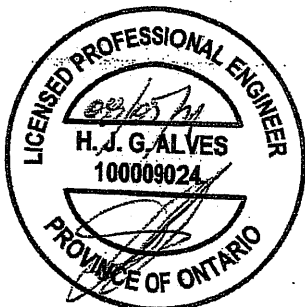
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

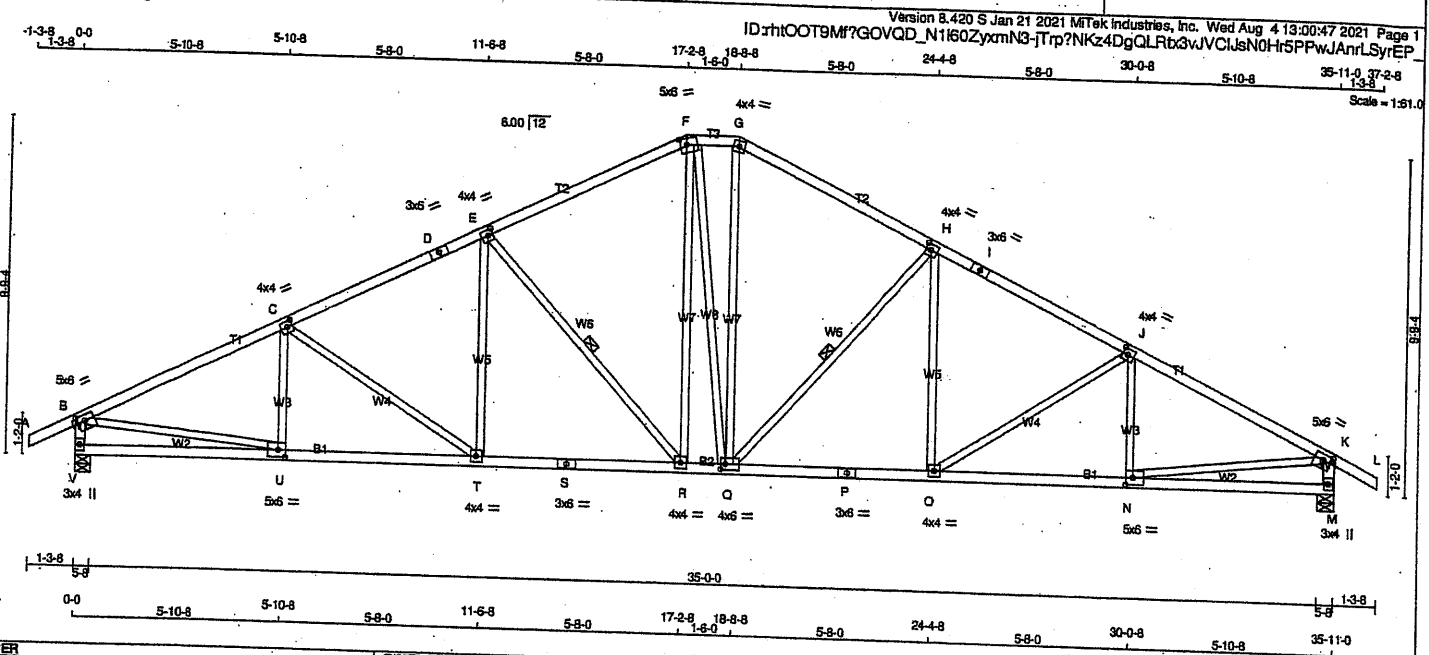
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)
JSI METAL = 0.57 (P) (INPUT = 1.00)



Structural component only
DWG# T-2126490



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	6.0	2.00	2.75
C, E, H, J					
D TMVW-t	MT20	4.0	4.0	2.00	1.75
F TS-t	MT20	3.0	6.0		
G TTW-m	MT20	5.0	6.0	2.25	2.25
I TS-t	MT20	4.0	4.0		
K TMVW-t	MT20	3.0	6.0		
M BMV1+p	MT20	5.0	6.0	2.00	2.75
N BMVW-t	MT20	3.0	4.0		
O, R, T					
P BS-t	MT20	4.0	4.0		
Q BMVW-t	MT20	4.0	6.0	2.00	1.50
S BS-t	MT20	3.0	6.0		
U BMVW-t	MT20	5.0	6.0	2.50	2.50
V BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
V	1971	0	1971	0	5-8	IN-SX
M	1971	0	1971	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
V	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	U-C	-251 / 0	0.06 (1)
B-C	-2755 / 0	-84.9	-84.9	0.50 (1)	3.77	C-T	-291 / 0	0.25 (1)
C-D	-2505 / 0	-84.9	-84.9	0.40 (1)	4.03	T-E	0 / 275	0.06 (4)
D-E	-2505 / 0	-84.9	-84.9	0.40 (1)	4.03	E-R	-743 / 0	0.39 (1)
E-F	-1981 / 0	-84.9	-84.9	0.39 (1)	4.44	R-F	0 / 588	0.13 (1)
F-G	-1764 / 0	-84.9	-84.9	0.06 (1)	5.00	F-Q	0 / 33	0.01 (1)
G-H	-1986 / 0	-84.9	-84.9	0.39 (1)	4.43	Q-G	0 / 823	0.14 (1)
H-I	-2503 / 0	-84.9	-84.9	0.40 (1)	4.03	Q-H	-734 / 0	0.39 (1)
I-J	-2503 / 0	-84.9	-84.9	0.40 (1)	3.77	O-H	0 / 265	0.06 (4)
J-K	-2756 / 0	-84.9	-84.9	0.50 (1)	10.00	N-J	-294 / 0	0.26 (1)
K-L	0 / 26	-84.9	-84.9	0.11 (1)	6.07	B-U	0 / 2509	0.56 (1)
V-B	-1923 / 0	0.0	0.0	0.19 (1)	6.06	N-K	0 / 2509	0.56 (1)
M-K	-1924 / 0	0.0	0.0	0.19 (1)				
V-U	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
U-T	0 / 2482	-18.5	-18.5	0.46 (1)	10.00			
T-S	0 / 2240	-18.5	-18.5	0.43 (1)	10.00			
S-R	0 / 2240	-18.5	-18.5	0.43 (1)	10.00			
R-Q	0 / 1759	-18.5	-18.5	0.37 (1)	10.00			
Q-P	0 / 2239	-18.5	-18.5	0.45 (1)	10.00			
P-O	0 / 2239	-18.5	-18.5	0.45 (1)	10.00			
O-N	0 / 2483	-18.5	-18.5	0.45 (1)	10.00			
N-M	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.50/1.00 (J-K:1), BC=0.48/1.00 (T-U:1), WB=0.58/1.00 (K-N:1), SS=0.21/1.00 (J-K:1).

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

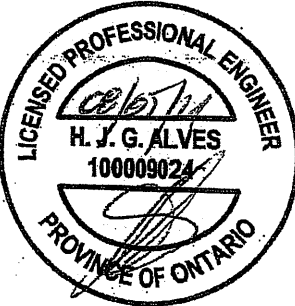
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

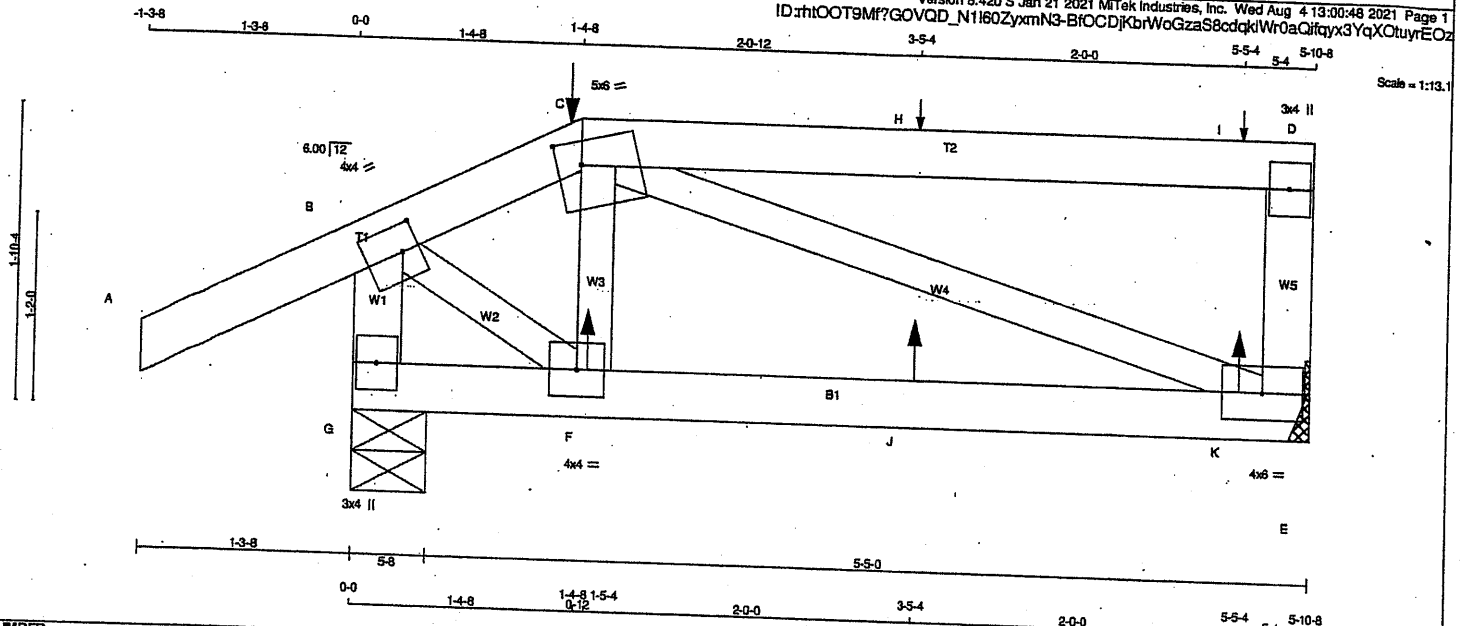
JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.73 (S) (INPUT = 1.00)



Structural component only
DWG# T-2126491

JOB NAME 420376	TRUSS NAME T27	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 13:00:48 2021 Page 1
ID:rh100T9Mf?GOVQD_N1160ZyxmN3-BROCDJKrWoGzaS8cdqkIW0aQlftqy3YqXOtuyrEOz



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-t	MT20	4.0	4.0	2.00 1.25
C TTWW-m	MT20	5.0	6.0	1.75 1.75
D TMV-p	MT20	3.0	4.0	
E BMVW1-t	MT20	4.0	6.0	
F BMVW-t	MT20	4.0	4.0	
G BMV1-p	MT20	3.0	4.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD.	
GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
JT	483	483	0	0	0	5-8	5-8
E	483	483	0	0	0	5-8	5-8
G	561	561	0	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	338	243 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
G	460	344 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	MAX. (LBS)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 17	-56.6	-56.6	0.08 (1)	10.00	F-C	-249 / 0
B-C	-529 / 0	-56.6	-56.6	0.08 (1)	6.25	C-E	-466 / 0
C-H	0 / 0	-140.6	-140.6	0.53 (1)	10.00	B-F	0 / 528
H-I	0 / 0	-140.6	-140.6	0.53 (1)	10.00		
I-D	0 / 0	-140.6	-140.6	0.53 (1)	10.00		
E-D	-316 / 0	0.0	0.0	0.04 (1)	7.81		
G-B	-650 / 0	0.0	0.0	0.07 (1)	7.81		
G-F	0 / 0	-12.3	-12.3	0.05 (4)	10.00		
F-J	0 / 440	-12.3	-12.3	0.11 (1)	10.00		
J-K	0 / 440	-12.3	-12.3	0.11 (1)	10.00		
K-E	0 / 440	-12.3	-12.3	0.11 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-4-8	-30	-30		FRONT	VERT	DEAD		C1
C	1-4-8	-52	-52	53	FRONT	VERT	TOTAL		C1
F	1-5-4	-134	-134		FRONT	VERT	SNOW		C1
H	3-5-4	7	1	9	FRONT	VERT	TOTAL		C1
I	5-5-4	1	1	53	FRONT	VERT	TOTAL		C1
J	3-5-4	7	1	45	FRONT	VERT	TOTAL		C1
K	5-5-4	6	1	8	FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 16.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSK: TC=0.53/1.00 (C-D-1), BC=0.11/1.00 (E-F-1),
WB=0.16/1.00 (C-E-1), SSI=0.27/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

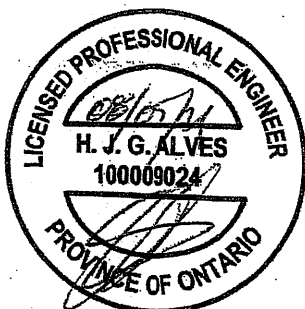
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

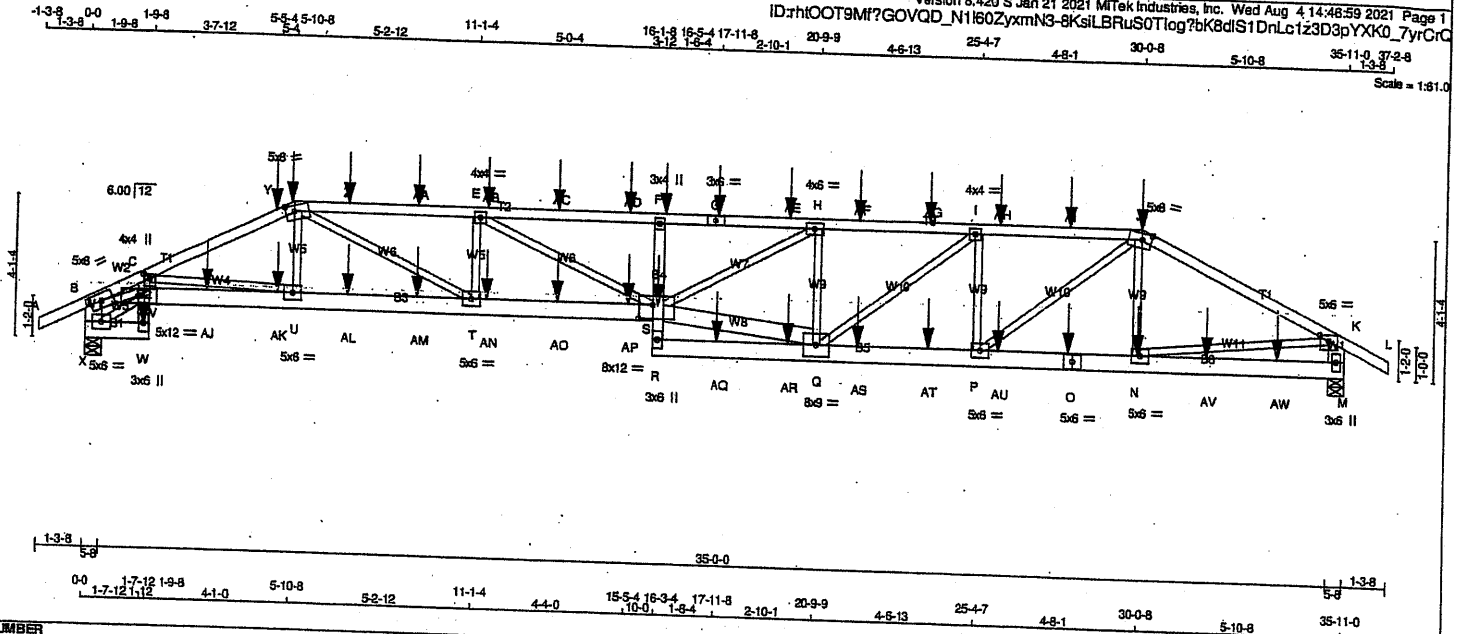
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126492

JOB NAME 420377	TRUSS NAME T28S	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 14:46:59 2021 Page 1 ID:rhOOT9M?GOVQD_N160ZyxmN3-8KsILBRuSOTlog?bK8dIS1DnLc1z3D3pYXK0_7yrCrc					
Scale = 1:51.0					



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
X - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
X - W	2x6	DRY	No.2
W - C	2x4	DRY	No.2
V - S	2x6	DRY	No.2
R - F	2x4	DRY	No.2
R - O	2x6	DRY	No.2
O - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - Q	2x6	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	12	SIDE(91.5)
D-G	12	SIDE(91.5)
G-J	12	SIDE(91.5)
J-L	12	SIDE(91.5)
X-B	2	TOP
M-K	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X-W	2	SIDE(183.1)
V-S	2	SIDE(274.6)
R-O	2	SIDE(10.5)
O-M	2	SIDE(274.6)
W-C	1	TOP
F-R	1	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	8
2x6	2	6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
M	3783	0	0	3783	0	0	5-8	5-8	5-8
X	4260	0	0	4260	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
M	2684	1715 / 0	0 / 0
X	3013	1976 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	CS1 (LC)	MEMB.	FORCE (LBS)
FR-TO	0 / 26	FROM TO	CS1 (LC)	FR-TO	0 / 1729
A-B	-10146 / 0	-84.9 -84.9 0.04 (1)	10.00	C-U	-3212 / 0
B-C	-9118 / 0	-84.9 -84.9 0.27 (1)	3.58	U-D	0 / 831
C-Y	-9118 / 0	-84.9 -84.9 0.31 (1)	3.96	S-Q	0 / 9098
Y-D	-11078 / 0	-84.9 -84.9 0.31 (1)	3.96	S-H	0 / 4045
D-Z	-11078 / 0	-84.9 -84.9 0.41 (1)	3.41	Q-H	-2857 / 0
Z-AA	-11078 / 0	-84.9 -84.9 0.41 (1)	3.41	Q-I	0 / 1729
AA-E	-12853 / 0	-84.9 -84.9 0.50 (1)	3.12	P-J	-1939 / 0
E-AB	-12853 / 0	-84.9 -84.9 0.50 (1)	3.12	P-K	0 / 3322
AB-AC	-12853 / 0	-84.9 -84.9 0.50 (1)	3.12	N-J	-469 / 5
AC-AD	-12853 / 0	-84.9 -84.9 0.50 (1)	3.12	N-K	0 / 5319
AD-F	-12720 / 0	-84.9 -84.9 0.50 (1)	3.12	X-V	-1060 / 0
F-G	-12720 / 0	-84.9 -84.9 0.50 (1)	3.12	B-V	0 / 9058
G-AE	-12720 / 0	-84.9 -84.9 0.42 (1)	3.18	E-S	0 / 2008
A-E-H	-12720 / 0	-84.9 -84.9 0.42 (1)	3.18	D-T	0 / 4214
H-AF	-9189 / 0	-84.9 -84.9 0.31 (1)	3.77	T-E	-1738 / 0
AF-AG	-9189 / 0	-84.9 -84.9 0.31 (1)	3.77		
AG-I	-9189 / 0	-84.9 -84.9 0.31 (1)	3.77		
I-AH	-7860 / 0	-84.9 -84.9 0.27 (1)	4.06		
AH-AI	-7860 / 0	-84.9 -84.9 0.27 (1)	4.06		
AI-J	-7860 / 0	-84.9 -84.9 0.27 (1)	4.06		
J-K	-5901 / 0	-84.9 -84.9 0.28 (1)	4.60		
K-L	0 / 26	-84.9 -84.9 0.08 (1)	7.81		
X-B	-3695 / 0	0.0 0.0 0.08 (1)	7.81		
M-K	-3693 / 0	0.0 0.0 0.08 (1)	7.81		

X-W	0 / 906	-18.5 -18.5 0.04 (1)	10.00
W-V	0 / 592	0.0 0.0 0.44 (1)	10.00
V-C	0 / 1327	0.0 0.0 0.48 (1)	10.00
V-AJ	0 / 10490	-18.5 -18.5 0.45 (1)	10.00
AJ-AK	0 / 10490	-18.5 -18.5 0.45 (1)	10.00
AK-U	0 / 10490	-18.5 -18.5 0.45 (1)	10.00
U-AL	0 / 7339	-18.5 -18.5 0.34 (1)	10.00
AL-AM	0 / 7339	-18.5 -18.5 0.34 (1)	10.00
AM-T	0 / 7339	-18.5 -18.5 0.34 (1)	10.00
T-AN	0 / 11078	-18.5 -18.5 0.50 (1)	10.00
AN-AO	0 / 11078	-18.5 -18.5 0.50 (1)	10.00
AO-AP	0 / 11078	-18.5 -18.5 0.50 (1)	10.00
AP-S	0 / 11078	-18.5 -18.5 0.50 (1)	10.00
R-S	0 / 75	0.0 0.0 0.17 (1)	10.00
S-F	-787 / 0	0.0 0.0 0.17 (1)	7.81
R-AQ	0 / 365	-18.5 -18.5 0.04 (4)	10.00
AQ-AR	0 / 365	-18.5 -18.5 0.04 (4)	10.00
AR-Q	0 / 365	-18.5 -18.5 0.04 (4)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 23.3	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 38.7	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.31")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/725 (0.59")

CSI: TC=0.50/1.00 (E-F:1), BC=0.50/1.00 (S-T:1),
WB=0.88/1.00 (B-V:1), SSI=0.39/1.00 (C-V:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

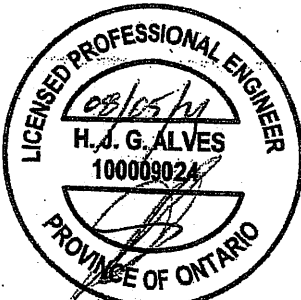
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



Structural component only
DWG# T-2126494

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TTWW-m	MT20	5.0	8.0	2.00	3.00
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	4.0	4.0		
J	TTWW-m	MT20	5.0	8.0	2.25	3.25
K	TMVW-p	MT20	5.0	6.0	2.00	3.00
M	BMV1+p	MT20	3.0	6.0		
N, P, T, U						
N	BMVW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMVWW-t	MT20	8.0	9.0		
R	BMV+p	MT20	3.0	6.0		
S	BMVWWW-t	MT20	8.0	12.0	5.00	4.75
V	BMVWW-t	MT20	5.0	12.0		
W	BMV+p	MT20	3.0	6.0		
X	BMVW1-t	MT20	5.0	6.0		

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO		
Q-AS	0 / 7880	-18.5	-18.5 0.34 (1)	10.00	
AS-AT	0 / 7880	-18.5	-18.5 0.34 (1)	10.00	
AT-P	0 / 7880	-18.5	-18.5 0.34 (1)	10.00	
P-AU	0 / 5282	-18.5	-18.5 0.24 (1)	10.00	
AL-O	0 / 5282	-18.5	-18.5 0.24 (1)	10.00	
O-N	0 / 5282	-18.5	-18.5 0.24 (1)	10.00	
N-AV	0 / 0	-18.5	-18.5 0.05 (4)	10.00	
AV-AW	0 / 0	-18.5	-18.5 0.05 (4)	10.00	
AW-M	0 / 0	-18.5	-18.5 0.05 (4)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-30	-30		FRONT	VERT	DEAD		C1
D	5-10-8	-133	-133		FRONT	VERT	SNOW		C1
F	18-5-4	-113	-113		FRONT	VERT	TOTAL		C1
G	17-11-8	-113	-113		FRONT	VERT	TOTAL		C1
J	30-0-8	-374	-374		FRONT	VERT	TOTAL		C1
N	29-11-12	-29	-29		FRONT	VERT	TOTAL		C1
O	27-11-12	-29	-29		FRONT	VERT	TOTAL		C1
S	18-3-4	-35	-35		FRONT	VERT	TOTAL		C1
W	1-7-12	-402	-402		FRONT	VERT	TOTAL		C1
Y	5-5-4	-90	-90		FRONT	VERT	TOTAL		C1
Z	7-5-4	-73	-73		FRONT	VERT	TOTAL		C1
AA	9-5-4	-73	-73		FRONT	VERT	TOTAL		C1
AB	11-5-4	-73	-73		FRONT	VERT	TOTAL		C1
AC	13-5-4	-73	-73		FRONT	VERT	TOTAL		C1
AD	15-5-4	-73	-73		FRONT	VERT	TOTAL		C1
AE	19-11-12	-113	-113		FRONT	VERT	TOTAL		C1
AF	21-11-12	-113	-113		FRONT	VERT	TOTAL		C1
AG	23-11-12	-113	-113		FRONT	VERT	TOTAL		C1
AH	25-11-12	-113	-113		FRONT	VERT	TOTAL		C1
AI	27-11-12	-113	-113		FRONT	VERT	TOTAL		C1
AJ	3-5-4	-60	-60		FRONT	VERT	TOTAL		C1
AK	5-5-4	-60	-60		FRONT	VERT	TOTAL		C1
AL	7-5-4	-60	-60		FRONT	VERT	TOTAL		C1
AM	9-5-4	-60	-60		FRONT	VERT	TOTAL		C1
AN	11-5-4	-60	-60		FRONT	VERT	TOTAL		C1
AO	13-5-4	-60	-60		FRONT	VERT	TOTAL		C1
AP	15-5-4	-61	-61		FRONT	VERT	TOTAL		C1
AQ	17-11-8	-29	-29		FRONT	VERT	TOTAL		C1
AR	19-11-12	-29	-29		FRONT	VERT	TOTAL		C1
AS	21-11-12	-29	-29		FRONT	VERT	TOTAL		C1
AT	23-11-12	-29	-29		FRONT	VERT	TOTAL		C1
AU	25-11-12	-29	-29		FRONT	VERT	TOTAL		C1
AV	31-11-12	-29	-29		FRONT	VERT	TOTAL		C1
AW	33-11-12	-29	-29		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

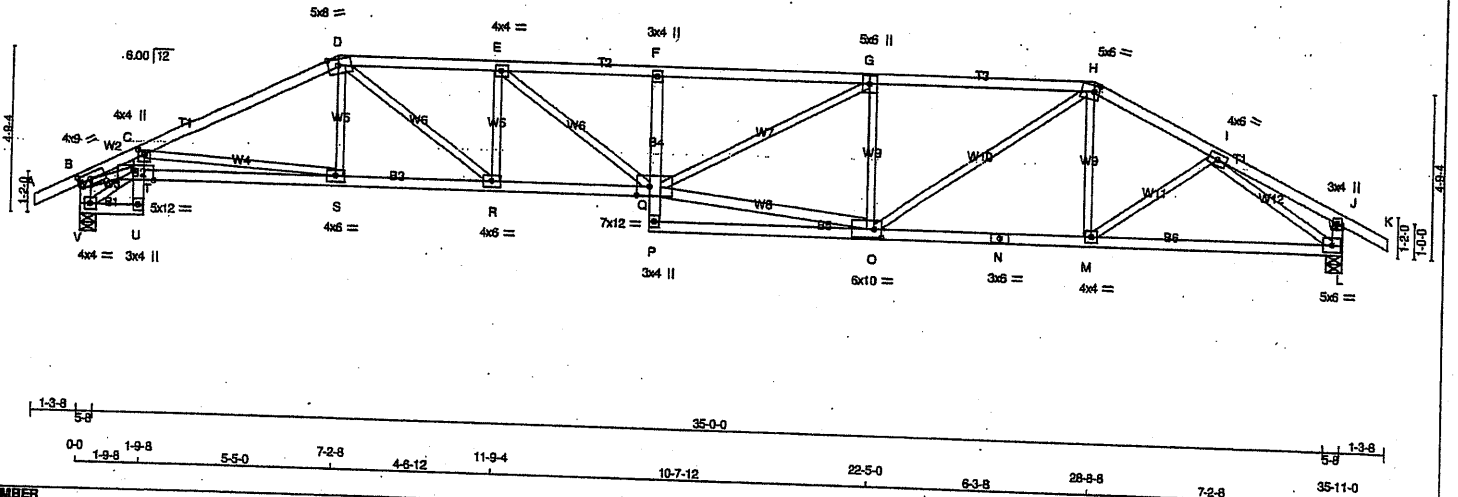
JSI GRIP = 0.86 (D) (INPUT = 0.90)
 JSI METAL = 0.82 (D) (INPUT = 1.00)



Structural component only
 DWG# T-2126494

JOB NAME 420377	TRUSS NAME T29S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Thu Aug 5 07:30:54 2021 Page 1
ID:htOOT9Mf?GOVQD_N1160ZymN3_Z_0MdpC1DSiVLHT4nSENTUki9DjduZP1ueJyr_8F
Scale = 1:51.0



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
V - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
V - U	2x4	DRY	No.2
U - C	2x4	DRY	No.2
T - Q	2x4	DRY	2100F 1.8E
P - F	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2

ALL WEBS

EXCEPT	2x3	DRY	No.2
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Q - O

2x4	DRY	No.2
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DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	9.0	2.00	4.00
C TMVW-p	MT20	4.0	4.0	1.50	2.00
D TTWW-m	MT20	5.0	8.0	2.25	3.50
E TMVW-t	MT20	4.0	4.0		
F TMV-p	MT20	4.0	4.0		
G TSWW-H	MT20	5.0	6.0		
H TTWW-m	MT20	5.0	6.0	2.25	1.75
I TMVW-t	MT20	4.0	4.0		
J TMV-p	MT20	3.0	4.0		
L BMVW-t	MT20	5.0	6.0		
M BMVW-t	MT20	4.0	4.0		
N BS-t	MT20	3.0	6.0		
O BMVW-t	MT20	6.0	10.0	3.00	2.50
P BMV-p	MT20	3.0	4.0		
Q BVVWVW-I	MT20	7.0	12.0	3.25	4.50
R BMVW-t	MT20	4.0	6.0		
S BMVW-t	MT20	4.0	6.0		
T BVVWVW-I	MT20	5.0	12.0	Edge	6.75
U BMV-p	MT20	3.0	4.0		
V BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
L	1313	0	1313	0	5-8	5-8
V	1316	0	1316	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SC
JT	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
L							
V	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						
A-B	0 / 17	-56.6	-56.6 0.07 (1)	10.00	C-S	-1234 / 0
B-C	-3134 / 0	-56.6	-56.6 0.17 (1)	3.83	S-D	0 / 186
C-D	-2293 / 0	-56.6	-56.6 0.45 (1)	4.11	Q-O	0 / 2270
D-E	-2829 / 0	-56.6	-56.6 0.28 (1)	3.93	Q-G	0 / 946
E-F	-3204 / 0	-56.6	-56.6 0.40 (1)	3.59	O-G	-795 / 0
F-G	-3172 / 0	-56.6	-56.6 0.51 (1)	3.57	O-H	0 / 878
G-H	-2317 / 0	-56.6	-56.6 0.59 (1)	3.94	M-H	0 / 58
H-I	-1804 / 0	-56.6	-56.6 0.13 (1)	4.87	M-I	0 / 118
I-J	0 / 8	-56.6	-56.6 0.09 (1)	10.00	I-L	-1877 / 0
J-K	0 / 17	-56.6	-56.6 0.07 (1)	10.00	V-T	-308 / 0
K-L	-1135 / 0	0.0	0.0 0.11 (1)	7.45	B-T	0 / 2821
L-J	-161 / 0	0.0	0.0 0.02 (1)	7.81	E-O	0 / 474
V-U	0 / 254	-12.3	-12.3 0.05 (1)	10.00	D-R	0 / 973
U-T	0 / 11	0.0	0.0 0.38 (1)	10.00	R-E	-543 / 0
T-C	0 / 344	0.0	0.0 0.44 (1)	10.00		
T-S	0 / 3275	-12.3	-12.3 0.28 (1)	10.00		
S-R	0 / 2055	-12.3	-12.3 0.19 (1)	10.00		
R-Q	0 / 2829	-12.3	-12.3 0.24 (1)	10.00		
Q-P	0 / 36	0.0	0.0 0.16 (1)	10.00		
P-O	-342 / 0	0.0	0.0 0.16 (1)	7.81		
O-N	0 / 110	-12.3	-12.3 0.11 (1)	4.00		
N-M	0 / 1605	-12.3	-12.3 0.33 (1)	10.00		
M-L	0 / 1511	-12.3	-12.3 0.32 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF: PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.20")

CALCULATED VERT. DEFL. (LL) = $L/999$ (0.23")

ALLOWABLE DEFL. (TL) = $L/360$ (1.20")

CALCULATED VERT. DEFL. (TL) = $L/982$ (0.44")

CSI: TC=0.59/1.00 (G-H:1), BC=0.44/1.00 (C-T:1), WB=0.70/1.00 (C-S:1), SS=0.38/1.00 (C-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 Inches

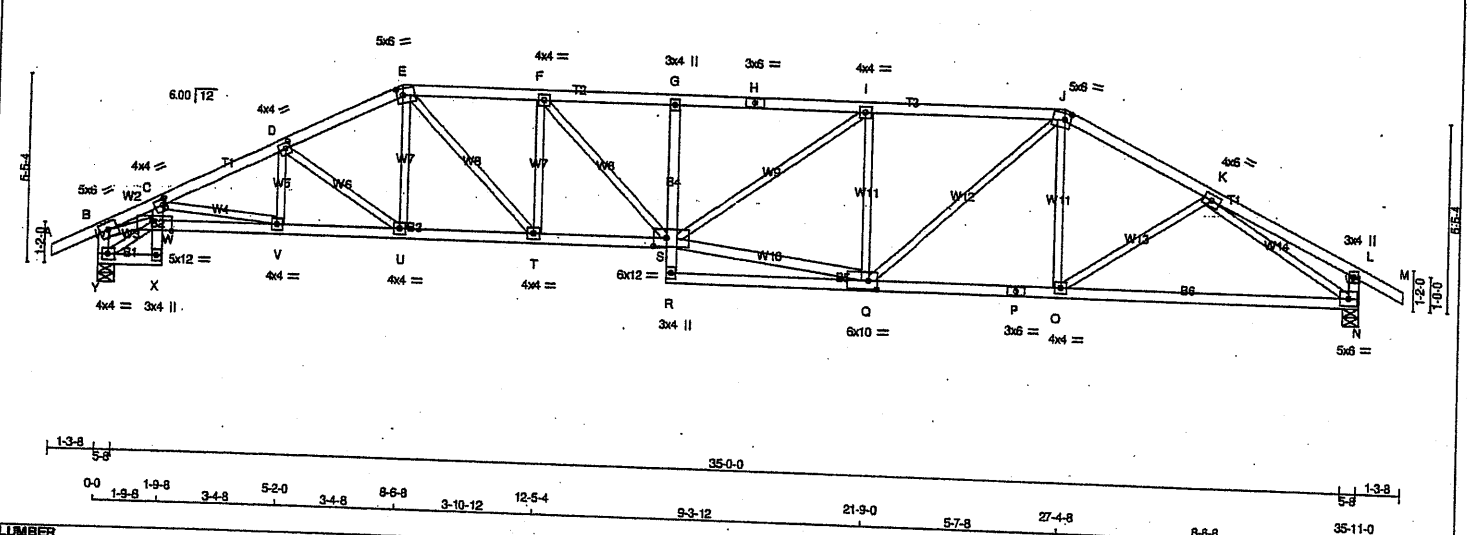
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)

JSI METAL= 0.55 (T) (INPUT = 1.00)



Structural component only
DWG# T-2126495



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - M	2x4	DRY	No.2
Y - B	2x4	DRY	No.2
N - L	2x4	DRY	No.2
Y - X	2x4	DRY	No.2
X - C	2x4	DRY	No.2
W - S	2x4	DRY	No.2
R - G	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - Q	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-t	MT20	4.0	4.0		
G	TMV-p	MT20	3.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	4.0		
J	TTWW-m	MT20	5.0	6.0	2.25	2.00
K	TMVW-t	MT20	4.0	4.0		
L	TMV-p	MT20	3.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		
O, T, U, V						
Q	BMVW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	6.0	10.0	3.00	2.75
Q	BMV-p	MT20	3.0	4.0		
S	BMVW-t	MT20	6.0	12.0	3.00	4.50
W	BMVW-t	MT20	5.0	12.0	Edge	6.75
X	BMV-p	MT20	3.0	4.0		
Y	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
N	1313	0	1313	0
Y	1316	0	1316	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	930	599/0	0/0	0/0	0/0	332/0	0/0
Y	932	601/0	0/0	0/0	0/0	332/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, Y

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO				FR-TO			
A-B	0/17	-56.6	0.07 (1)	U-E	0/284	0.06 (1)	
B-C	-2930/0	-56.6	0.29 (1)	S-Q	0/2063	0.33 (1)	
C-D	-2498/0	-56.6	0.30 (1)	S-Q	0/884	0.15 (1)	
D-E	-2179/0	-56.6	0.20 (1)	Q-I	-778/0	0.34 (1)	
E-F	-2437/0	-56.6	0.19 (1)	Q-J	0/897	0.16 (1)	
F-G	-2688/0	-56.6	0.20 (1)	O-J	0/94	0.03 (4)	
G-H	-2650/0	-56.6	0.42 (1)	O-K	0/53	0.02 (4)	
H-I	-2650/0	-56.6	0.42 (1)	K-N	-1908/0	0.86 (1)	
I-J	-2093/0	-56.6	0.37 (1)	Y-W	-284/0	0.04 (1)	
J-K	-1778/0	-56.6	0.20 (1)	B-W	0/2601	0.59 (1)	
K-L	0/11	-56.6	0.14 (1)	F-S	0/340	0.08 (1)	
L-M	0/17	-56.6	0.07 (1)	E-T	0/722	0.16 (1)	
Y-B	-1147/0	0.0	0.12 (1)	T-F	-479/0	0.14 (1)	
N-L	-174/0	0.0	0.02 (1)	D-U	-400/0	0.12 (1)	
				V-D	0/93	0.02 (4)	
				C-V	-775/0	0.18 (1)	
Y-X	0/236	-12.3	-12.3	0.04 (1)			
X-W	0/11	0.0	0.0	0.36 (1)			
W-C	0/347	0.0	0.0	0.41 (1)			
W-V	0/3025	-12.3	-12.3	0.52 (1)			
V-U	0/2263	-12.3	-12.3	0.39 (1)			
U-T	0/1942	-12.3	-12.3	0.33 (1)			
T-S	0/2437	-12.3	-12.3	0.42 (1)			
R-S	0/33	0.0	0.0	0.11 (1)			
S-G	-245/0	0.0	0.0	0.11 (1)			
R-Q	0/74	-12.3	-12.3	0.09 (4)			
Q-P	0/1581	-12.3	-12.3	0.36 (1)			
P-O	0/1581	-12.3	-12.3	0.36 (1)			
O-N	0/1583	-12.3	-12.3	0.36 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.36")

CSI: TC=0.42/1.00 (G-t:1), BC=0.52/1.00 (V-W:1), WB=0.88/1.00 (K-N:1), SS=0.35/1.00 (C-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

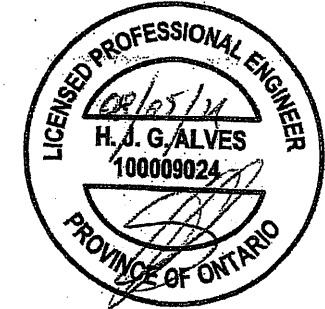
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

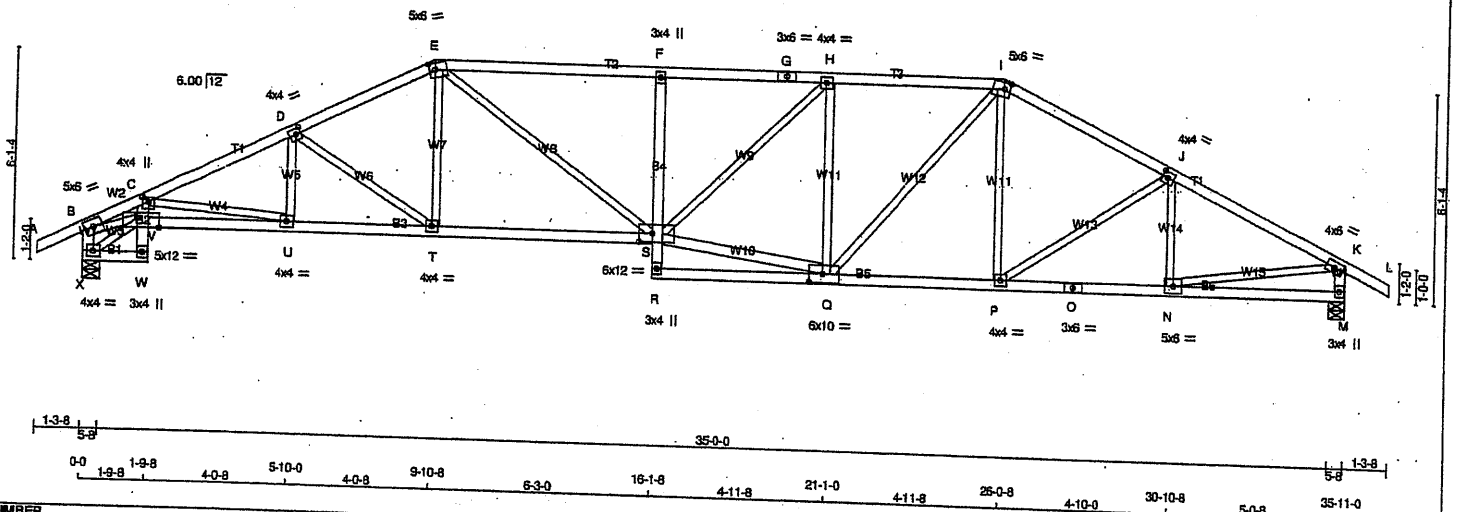
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (B) (INPUT = 0.90)
JSI METAL = 0.88 (B) (INPUT = 1.00)



JOB NAME 420377	TRUSS NAME T31S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 14:42:02 2021 Page 1					
ID:htOOT9M?GOVQD_N1160ZyxmN3-ZvYq_CThxtf8kA0HBS4frkOp1HGbcGEVZgbSyrCrn					
Scale = 1:51.0					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - W	2x4	DRY	No.2
W - C	2x4	DRY	No.2
V - S	2x4	DRY	No.2
R - F	2x4	DRY	No.2
O - M	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS EXCEPT S - Q	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	5.0	6.0	
C	TMVW+p	MT20	4.0	4.0	1.50 2.00
D	TMVW-t	MT20	4.0	4.0	2.00 1.75
E	TTVW-m	MT20	5.0	6.0	2.25 2.00
F	TMV+p	MT20	3.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	4.0	
I	TTVW-m	MT20	5.0	6.0	2.25 2.00
J	TMVW-t	MT20	4.0	4.0	2.00 1.75
K	TMVW-t	MT20	4.0	6.0	
M	BMV1+p	MT20	3.0	4.0	
N	BMVW-t	MT20	5.0	6.0	
O	BS-t	MT20	3.0	6.0	
P, T, U					
Q	BMVW-t	MT20	4.0	4.0	
P	BMVW-t	MT20	6.0	10.0	3.00 4.50
R	BMV+p	MT20	3.0	4.0	
S	BVMWVW-t	MT20	6.0	12.0	3.00 4.50
V	BVMWVW-t	MT20	5.0	12.0	Edge 7.25
W	BMV+p	MT20	3.0	4.0	
X	BMVW-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
M	1313	0	1313	0	5-8	5-8	5-8	
X	1316	0	1316	0	5-8	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
	SNOW	LIVE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	SNOW	LIVE	PERM. LIVE	WIND
M	930	599 / 0	0 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
X	932	601 / 0	0 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO				FR-TO			
A-B	0 / 17	-56.6	-56.6 0.07 (1)	10.00	T-E	0 / 331	0.07 (1)
B-C	-2952 / 0	-56.6	-56.6 0.23 (1)	3.88	E-S	0 / 596	0.13 (1)
C-D	-2444 / 0	-56.6	-56.6 0.28 (1)	4.16	S-Q	0 / 1863	0.30 (1)
D-E	-2073 / 0	-56.6	-56.6 0.20 (1)	4.55	S-H	0 / 814	0.14 (1)
E-F	-2323 / 0	-56.6	-56.6 0.38 (1)	4.17	Q-H	-713 / 0	0.42 (1)
F-G	-2312 / 0	-56.6	-56.6 0.28 (1)	4.27	Q-I	0 / 529	0.12 (1)
G-H	-2312 / 0	-56.6	-56.6 0.28 (1)	4.27	P-I	0 / 136	0.03 (4)
H-I	-1869 / 0	-56.6	-56.6 0.21 (1)	4.74	P-J	-131 / 0	0.08 (1)
I-J	-1722 / 0	-56.6	-56.6 0.21 (1)	4.90	N-K	-216 / 0	0.05 (1)
J-K	-1814 / 0	-56.6	-56.6 0.22 (1)	4.79	N-K	0 / 1658	0.37 (1)
K-L	0 / 17	-56.6	-56.6 0.07 (1)	10.00	X-V	-287 / 0	0.04 (1)
X-B	-1146 / 0	0.0	0.0 0.12 (1)	7.43	B-V	0 / 2625	0.59 (1)
M-K	-1284 / 0	0.0	0.0 0.13 (1)	7.11	D-T	-448 / 0	0.18 (1)
X-W	0 / 238	-12.3	-12.3 0.04 (1)	10.00	U-D	0 / 101	0.03 (4)
W-V	0 / 11	0.0	0.0 0.38 (1)	10.00	C-U	-851 / 0	0.26 (1)
V-C	0 / 339	0.0	0.0 0.41 (1)	10.00			
V-U	0 / 3052	-12.3	-12.3 0.53 (1)	10.00			
U-T	0 / 2211	-12.3	-12.3 0.41 (1)	10.00			
T-S	0 / 1846	-12.3	-12.3 0.35 (1)	10.00			
R-S	0 / 27	0.0	0.0 0.08 (1)	10.00			
S-F	-361 / 0	0.0	0.0 0.10 (1)	7.81			
R-Q	0 / 53	-12.3	-12.3 0.07 (4)	10.00			
Q-P	0 / 1529	-12.3	-12.3 0.28 (1)	10.00			
P-O	0 / 1634	-12.3	-12.3 0.29 (1)	10.00			
O-N	0 / 1634	-12.3	-12.3 0.29 (1)	10.00			
N-M	0 / 0	-12.3	-12.3 0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.38/1.00 (E-F-1), BC=0.53/1.00 (U-V-1), WB=0.59/1.00 (B-V-1), SS=0.35/1.00 (C-V-1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY)- SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788, 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

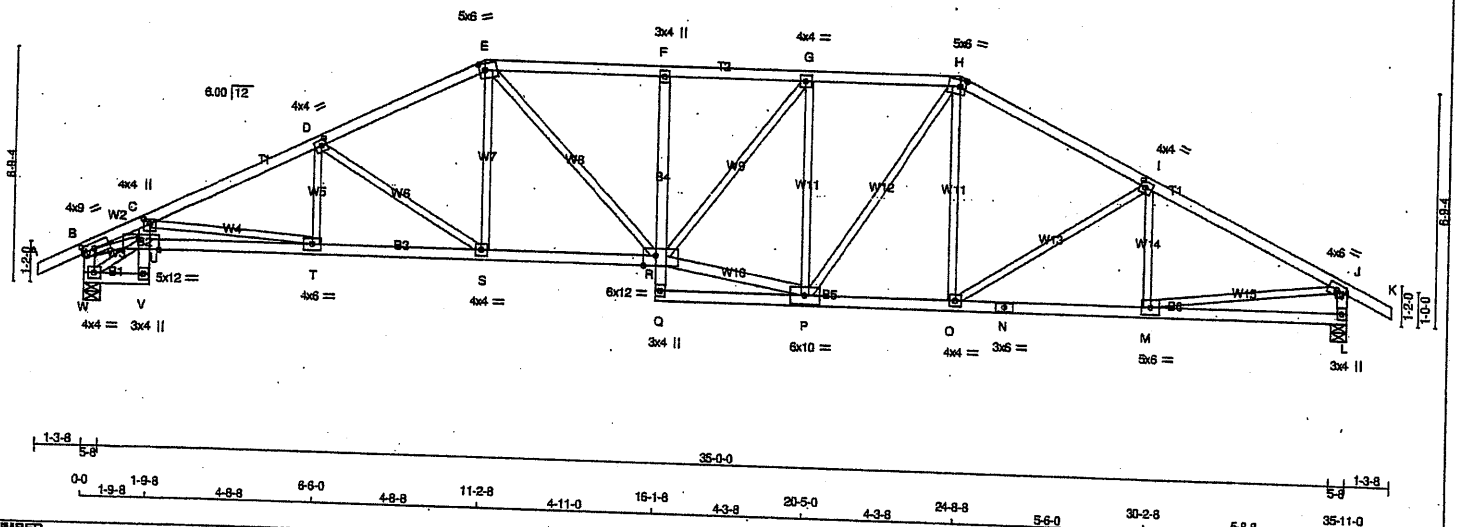
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.68 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126497

JOB NAME 420377	TRUSS NAME T32S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 14:47:03 2021 Page 1					
ID: rhtOOT9Mf?GOVQD_N160ZymN3-165CBYUPWE_kHJMZ_ihtNWGDNO?2MPT9ID7uyrCrM					
Scale = 1:81.0					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
W - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
W - V	2x4	DRY	No.2
V - O	2x4	DRY	No.2
O - F	2x4	DRY	No.2
F - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
R - P	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	TTVW-m	MT20	5.0	6.0	2.25	2.00
F	TMV-p	MT20	3.0	4.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTVW-m	MT20	5.0	6.0	2.25	2.00
I	TMVW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	4.0	4.0		
L	BMV1-p	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	6.0	10.0		
Q	BMV-p	MT20	3.0	4.0		
R	BMVW-t	MT20	6.0	12.0	Edge	4.25
S	BMVW-t	MT20	4.0	4.0		
T	BMVW-t	MT20	4.0	6.0		
U	BMVW-t	MT20	5.0	12.0	Edge	6.75
V	BMV-p	MT20	3.0	4.0		
W	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	VERT	GROSS REACTION	DOWN		
JT	1313	0	1313	0	5-8	5-8
L	1313	0	1313	0	5-8	5-8
W	1316	0	1316	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0
L	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0
W	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 17	-56.6	-56.6	0.07 (1)	10.00	C-T	-925 / 0	0.37 (1)
B-C	-2978 / 0	-56.6	-56.6	0.21 (1)	3.88	T-D	0 / 122	0.03 (4)
C-D	-2395 / 0	-56.6	-56.6	0.31 (1)	4.18	D-S	-511 / 0	0.29 (1)
D-E	-1960 / 0	-56.6	-56.6	0.24 (1)	4.81	S-E	0 / 347	0.08 (1)
E-F	-2026 / 0	-56.6	-56.6	0.22 (1)	4.57	E-R	0 / 417	0.09 (1)
F-G	-2019 / 0	-56.6	-56.6	0.17 (1)	4.63	R-P	0 / 1708	0.27 (1)
G-H	-1693 / 0	-56.6	-56.6	0.15 (1)	4.99	R-G	0 / 530	0.12 (1)
H-I	-1665 / 0	-56.6	-56.6	0.26 (1)	4.90	P-Q	-671 / 0	0.51 (1)
I-J	-1840 / 0	-56.6	-56.6	0.28 (1)	4.70	H-P	0 / 396	0.09 (1)
J-K	0 / 17	-56.6	-56.6	0.07 (1)	10.00	O-H	0 / 185	0.04 (1)
K-L	-1144 / 0	0.0	0.0	0.12 (1)	7.43	C-I	-224 / 0	0.19 (1)
L-J	-1282 / 0	0.0	0.0	0.13 (1)	7.12	M-I	-175 / 0	0.04 (1)
						M-J	0 / 1678	0.38 (1)
W-V	0 / 240	-12.3	-12.3	0.04 (1)	10.00	W-U	-289 / 0	0.04 (1)
V-U	0 / 11	0.0	0.0	0.36 (1)	10.00	B-U	0 / 2652	0.60 (1)
U-T	0 / 333	0.0	0.0	0.41 (1)	10.00			
T-S	0 / 3083	-12.3	-12.3	0.54 (1)	10.00			
S-R	0 / 2166	-12.3	-12.3	0.39 (1)	10.00			
R-Q	0 / 1744	-12.3	-12.3	0.32 (1)	10.00			
Q-P	0 / 24	0.0	0.0	0.06 (1)	10.00			
P-O	-288 / 0	0.0	0.0	0.09 (1)	7.81			
O-N	0 / 39	-12.3	-12.3	0.05 (4)	10.00			
N-M	0 / 1476	-12.3	-12.3	0.27 (1)	10.00			
M-L	0 / 1660	-12.3	-12.3	0.30 (1)	10.00			
	0 / 1660	-12.3	-12.3	0.30 (1)	10.00			
	0 / 0	-12.3	-12.3	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 16.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF ORC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.15")
ALLOWABLE DEFL. (TL) = $L/360$ (1.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.29")

CSI: TC=0.31/1.00 (C-D:1), BC=0.54/1.00 (T-U:1), WB=0.60/1.00 (B-U:1), SS=0.36/1.00 (C-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

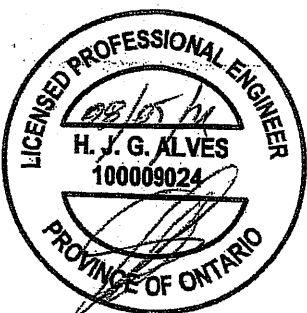
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

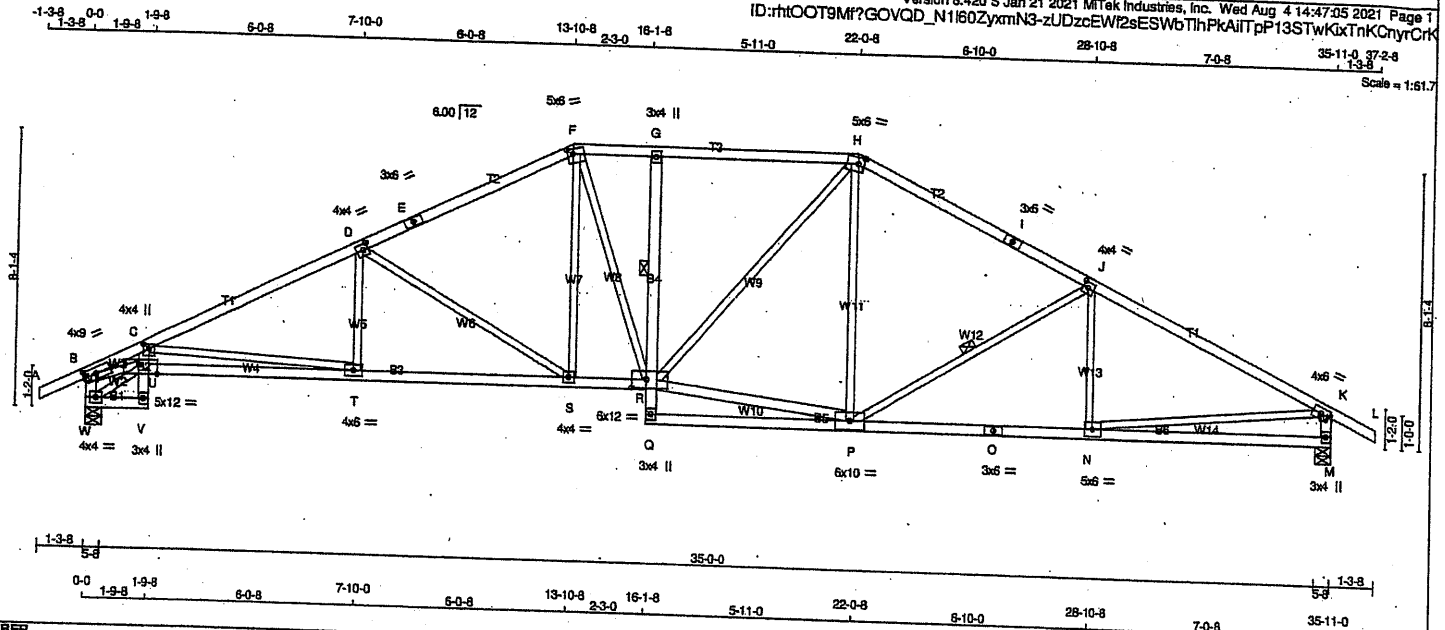
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.52 (U) (INPUT = 1.00)



Structural component only
DWG# T-2126498

JOB NAME 420377	TRUSS NAME T34S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 14:47:05 2021 Page 1 ID:rh00T9Mf?GOVQD_N1160ZymN3-zUDzcEW2sESWbTihPKAiTpP13STwKxTnKcnyrCrk					
Scale = 1:51.7					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - L	2x4	DRY	No.2
M - K	2x4	DRY	No.2
N - V	2x4	DRY	No.2
W - C	2x4	DRY	No.2
U - R	2x4	DRY	No.2
Q - G	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
R - P	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	9.0	2.00 4.00
C	TMVW-p	MT20	4.0	4.0	1.50 2.00
D	TMVW-t	MT20	4.0	4.0	2.00 1.75
E	TS-t	MT20	3.0	6.0	
F	TTWW-m	MT20	5.0	6.0	1.75 2.00
G	TMV-p	MT20	3.0	4.0	
H	TTWW-m	MT20	5.0	6.0	2.25 2.00
I	TS-t	MT20	3.0	6.0	
J	TMVW-t	MT20	4.0	4.0	2.00 1.75
K	TMVW-t	MT20	4.0	6.0	
M	BMV1-p	MT20	3.0	4.0	
N	BMVW-t	MT20	5.0	6.0	
O	BS-t	MT20	3.0	6.0	
P	BMVWW-t	MT20	6.0	10.0	
Q	BMV-p	MT20	3.0	4.0	
R	BMVWWW-t	MT20	6.0	12.0	3.00 5.00
S	BMVW-t	MT20	4.0	4.0	
T	BMVW-t	MT20	4.0	6.0	
U	BMVWW-t	MT20	5.0	12.0	Edge 6.75
V	BMV-p	MT20	3.0	4.0	
W	BMVW-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
JT	1313	0	1313	0	5-8	5-8	
M	1316	0	1316	0	5-8	5-8	
W	1316	0	1316	0	5-8	5-8	

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE		WIND		DEAD		SOIL	
M	930	599 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0	0 / 0	0 / 0
W	932	601 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-R, J-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 17	-56.6	-56.6 0.07 (1)	10.00	C-T	-1085 / 0	0.76 (1)
B-C	-3039 / 0	-56.6	-56.6 0.17 (1)	3.89	T-D	0 / 151	0.04 (4)
C-D	-2300 / 0	-56.6	-56.6 0.42 (1)	4.16	D-S	-831 / 0	0.64 (1)
D-E	-1745 / 0	-56.6	-56.6 0.35 (1)	4.72	S-F	0 / 391	0.09 (1)
E-F	-1745 / 0	-56.6	-56.6 0.35 (1)	4.72	F-R	0 / 266	0.06 (1)
F-G	-1636 / 0	-56.6	-56.6 0.20 (1)	4.99	R-P	0 / 1345	0.22 (1)
G-H	-1632 / 0	-56.6	-56.6 0.28 (1)	4.94	P-H	0 / 431	0.10 (1)
H-I	-1535 / 0	-56.6	-56.6 0.41 (1)	4.88	P-H	0 / 82	0.03 (4)
I-J	-1535 / 0	-56.6	-56.6 0.41 (1)	4.88	P-J	-390 / 0	0.18 (1)
J-K	-1860 / 0	-56.6	-56.6 0.46 (1)	4.49	N-J	-113 / 33	0.04 (1)
K-L	0 / 17	-56.6	-56.6 0.07 (1)	10.00	N-K	0 / 1695	0.38 (1)
W-B	-1141 / 0	0.0	0.0 0.12 (1)	7.44	W-U	-296 / 0	0.04 (1)
M-K	-1276 / 0	0.0	0.0 0.13 (1)	7.13	B-U	0 / 2718	0.61 (1)
W-V	0 / 246	-12.3	-12.3 0.05 (1)	10.00			
V-U	0 / 11	0.0	0.0 0.37 (1)	10.00			
U-C	0 / 331	0.0	0.0 0.42 (1)	10.00			
U-T	0 / 3158	-12.3	-12.3 0.56 (1)	10.00			
T-S	0 / 2079	-12.3	-12.3 0.39 (1)	10.00			
S-R	0 / 1548	-12.3	-12.3 0.28 (1)	10.00			
Q-R	0 / 31	0.0	0.0 0.04 (1)	10.00			
R-G	-319 / 0	0.0	0.0 0.04 (1)	6.25			
Q-P	0 / 28	-12.3	-12.3 0.11 (4)	10.00			
P-O	0 / 1682	-12.3	-12.3 0.34 (1)	10.00			
O-N	0 / 1682	-12.3	-12.3 0.34 (1)	10.00			
N-M	0 / 0	-12.3	-12.3 0.13 (4)	10.00			

TOTAL WEIGHT = 182 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.28")

CSI: TC=0.46/1.00 (J-K:1), BC=0.56/1.00 (T-U:1), WB=0.76/1.00 (C-T:1), SS=0.37/1.00 (C-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

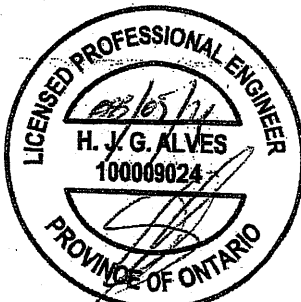
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (K) (INPUT = 0.90)
JSI METAL = 0.53 (O) (INPUT = 1.00)



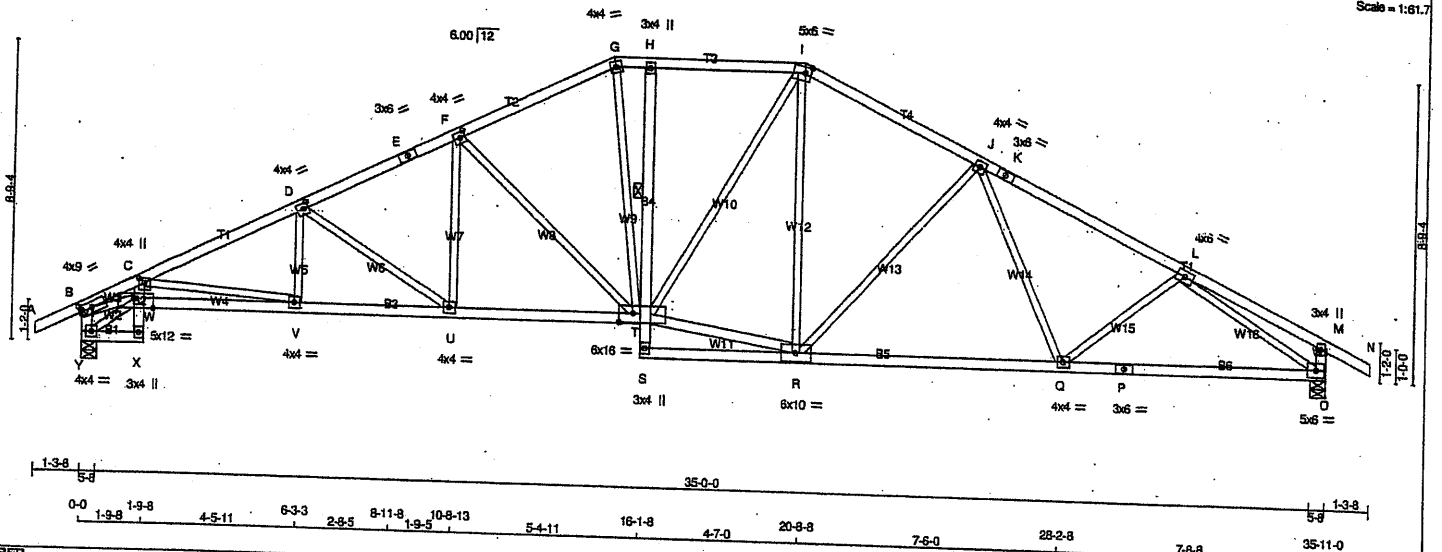
Structural component only
DWG# T-2126500

JOB NAME 420377	TRUSS NAME T35S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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1-3-8 0-0 1-9-8 4-5-11 6-3-3 4-5-11 10-8-13 4-5-11 15-2-8 16-1-8 47-0 20-8-8 50-0 25-8-8 50-0 30-8-8 52-8 35-11-0 37-2-8 1-3-8

Scale = 1:61.7



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - E	2x4 DRY	No.2	SPF
G - I	2x4 DRY	No.2	SPF
I - K	2x4 DRY	No.2	SPF
K - N	2x4 DRY	No.2	SPF
Y - B	2x4 DRY	No.2	SPF
O - M	2x4 DRY	No.2	SPF
Y - X	2x4 DRY	No.2	SPF
X - C	2x4 DRY	No.2	SPF
W - T	2x4 DRY	No.2	SPF
S - H	2x4 DRY	No.2	SPF
S - P	2x4 DRY	No.2	SPF
P - O	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT T - R	2x3 DRY	No.2	SPF
	2x4 DRY	No.2	SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.75
G	TTW-m	MT20	4.0	4.0		
H	TMV-p	MT20	3.0	4.0		
I	TTWV-m	MT20	5.0	6.0	2.25	2.00
J	TMVW-t	MT20	4.0	4.0		
K	TS-t	MT20	3.0	6.0		
L	TMVW-t	MT20	4.0	4.0		
M	TMV-p	MT20	3.0	4.0		
O	BMVW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q, U, V						
Q	BMVW-t	MT20	4.0	4.0		
R	BMVW-t	MT20	6.0	10.0		
S	BMV-p	MT20	3.0	4.0		
T	BMVW-t	MT20	6.0	18.0	3.25	5.00
W	BMVW-t	MT20	5.0	12.0	Edge	6.75
X	BMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
O	1313	0	1313	0
Y	1316	0	1316	0

UNFACTORED REACTIONS

	1ST LOSE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
Y	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, Y

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 17	-56.6	-56.6 0.07 (1)	10.00	T-R	0 / 1295	0.21 (1)
B-C	-2980 / 0	-56.6	-56.6 0.21 (1)	3.88	T-I	0 / 378	0.09 (1)
C-D	-2407 / 0	-56.6	-56.6 0.30 (1)	4.18	R-I	0 / 97	0.03 (4)
D-E	-2015 / 0	-56.6	-56.6 0.20 (1)	4.61	R-J	-400 / 0	0.52 (1)
E-F	-2015 / 0	-56.6	-56.6 0.20 (1)	4.61	J-Q	0 / 100	0.03 (4)
F-G	-1568 / 0	-56.6	-56.6 0.17 (1)	5.13	Q-L	0 / 54	0.02 (4)
G-H	-1478 / 0	-56.6	-56.6 0.10 (1)	5.31	L-O	-1922 / 0	0.80 (1)
H-I	-1476 / 0	-56.6	-56.6 0.16 (1)	5.24	G-T	0 / 836	0.14 (1)
I-J	-1450 / 0	-56.6	-56.6 0.22 (1)	5.21	Y-W	-290 / 0	0.04 (1)
J-K	-1766 / 0	-56.6	-56.6 0.26 (1)	4.82	B-W	0 / 2654	0.60 (1)
K-L	-1766 / 0	-56.6	-56.6 0.26 (1)	4.82	C-V	-920 / 0	0.33 (1)
L-M	0 / 13	-56.6	-56.6 0.17 (1)	10.00	V-D	0 / 119	0.03 (4)
M-N	0 / 17	-56.6	-56.6 0.07 (1)	10.00	D-U	-451 / 0	0.22 (1)
Y-B	-1144 / 0	0.0	0.0 0.12 (1)	7.43	U-F	0 / 321	0.07 (1)
O-M	-164 / 0	0.0	0.0 0.02 (1)	7.81	F-T	-560 / 0	0.61 (1)
Y-X	0 / 240	-12.3	-12.3 0.04 (1)	10.00			
X-W	0 / 11	0.0	0.0 0.26 (1)	10.00			
W-C	0 / 336	0.0	0.0 0.41 (1)	10.00			
W-V	0 / 3085	-12.3	-12.3 0.54 (1)	10.00			
V-U	0 / 2174	-12.3	-12.3 0.39 (1)	10.00			
U-T	0 / 1799	-12.3	-12.3 0.34 (1)	10.00			
S-T	0 / 19	0.0	0.0 0.03 (1)	10.00			
T-H	-238 / 0	0.0	0.0 0.03 (1)	8.25			
S-R	0 / 19	-12.3	-12.3 0.10 (4)	10.00			
R-Q	0 / 1545	-12.3	-12.3 0.33 (1)	10.00			
Q-P	0 / 1569	-12.3	-12.3 0.34 (1)	10.00			
P-O	0 / 1569	-12.3	-12.3 0.34 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 36.7 PSF	

SPACING = 16.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD.

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.30/1.00 (C-D:1), BC=0.54/1.00 (V-W:1),
WB=0.80/1.00 (L-O:1), SS=0.36/1.00 (C-W:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

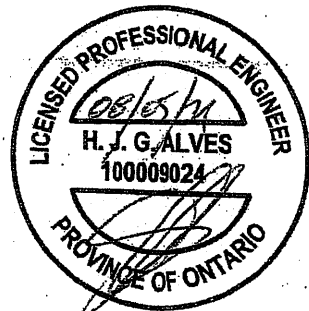
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI)	(PLI)	(PLI)
MAX MIN MAX MIN MAX MIN		
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (V) (INPUT = 0.90)
JSI METAL=0.52 (W) (INPUT = 1.00)

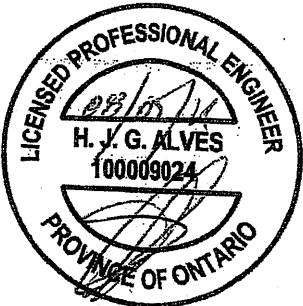


Structural component only
DWG# T-2126501

JOB NAME 420377	TRUSS NAME T35S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

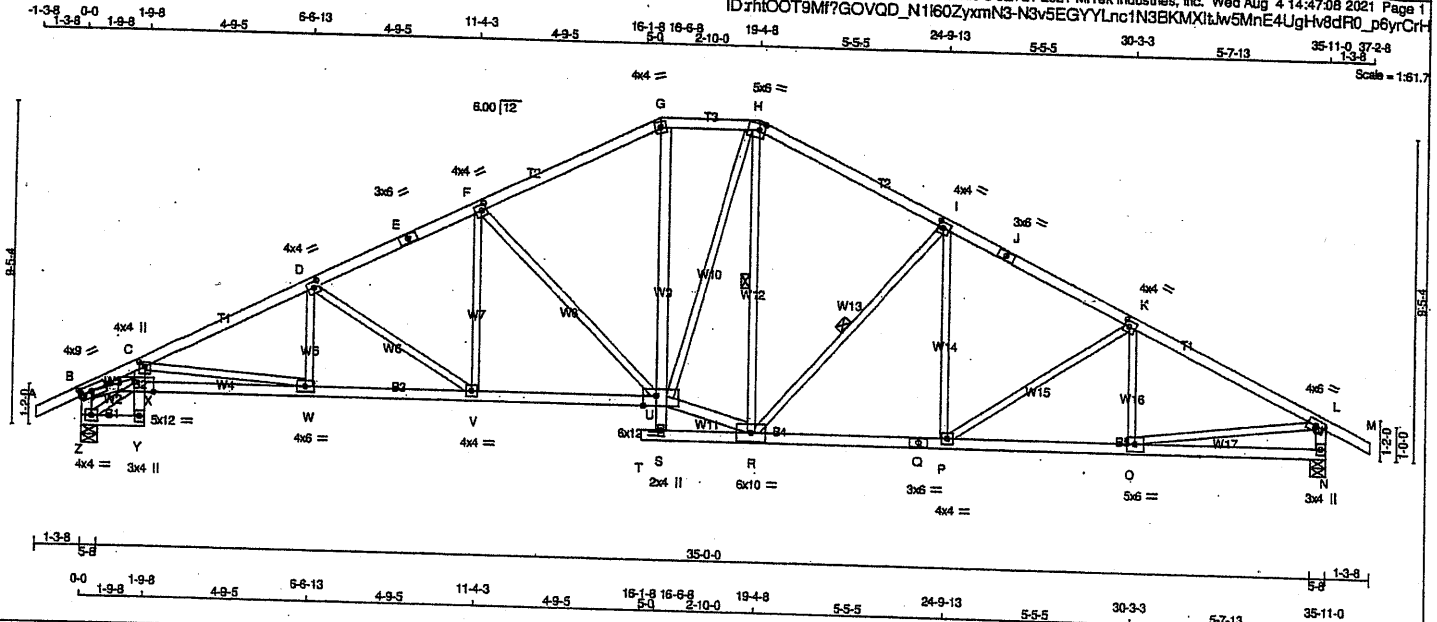
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 14:47:07 2021 Page 2
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PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
Y BMVW1-t MT20 4.0 4.0
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2126501 *W*

JOB NAME 420377	TRUSS NAME T36S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 Mitak Industries, Inc. Wed Aug 4 14:47:08 2021 Page 1	



LUMBER			
N.L.G.A. RULES	SIZE	DRY	DESCR.
CHORDS			
A - E	2x4	DRY	No.2
F - G	2x4	DRY	No.2
H - J	2x4	DRY	No.2
K - M	2x4	DRY	No.2
N - P	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
S - Y	2x4	DRY	No.2
Z - C	2x4	DRY	No.2
D - U	2x4	DRY	No.2
V - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
S - G	2x4	DRY	No.2
U - R	2x4	DRY	No.2

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMVW-t	MT20	4.0	9.0 2.00 4.00
C TMVW+p	MT20	4.0	4.0 1.50 2.00
D, F, I, K			
E TMVW-t	MT20	4.0	4.0 2.00 1.75
F TS-t	MT20	3.0	6.0
G TTW-m	MT20	4.0	4.0
H TTWV-m	MT20	5.0	6.0 2.25 2.00
I TS-t	MT20	3.0	6.0
J TMVW-t	MT20	4.0	6.0
N BMV-t	MT20	3.0	4.0
O BMVW-t	MT20	5.0	6.0
P BMVW-t	MT20	4.0	4.0
Q BS-t	MT20	3.0	6.0
R BMVWV-t	MT20	6.0	10.0
S BMVW-w	MT20	2.0	4.0
U BMVWVW-t	MT20	8.0	12.0 Edge 4.50
V BMVW-t	MT20	4.0	4.0
W BMVW-t	MT20	4.0	6.0
X BMVWV-t	MT20	5.0	12.0 Edge 6.75
Y BMV+p	MT20	3.0	4.0
Z BMVW-t	MT20	4.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	DOWN	UP	BRG	IN-SX	REQD
N	1316	0	0	1316	0	0	5-8	5-8	5-8
Z	1320	0	0	1320	0	0	5-8	5-8	5-8
UNFACTORED REACTIONS									
JT	1ST CASE	MAX/MIN	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD
N	933	599 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	334 / 0	335 / 0
Z	936	601 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	334 / 0	335 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, Z

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-R, I-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)
FR-TO									
A-B	0 / 17	-58.6	-58.6	0.07 (1)	10.00	0.00	0.00	0.00	0.00
B-C	-3006 / 0	-58.6	-58.6	0.20 (1)	3.88	0.00	0.00	0.00	0.00
C-D	-2391 / 0	-58.6	-58.6	0.32 (1)	4.17	0.00	0.00	0.00	0.00
D-E	-1976 / 0	-58.6	-58.6	0.21 (1)	4.63	0.00	0.00	0.00	0.00
E-F	-1976 / 0	-58.6	-58.6	0.21 (1)	4.63	0.00	0.00	0.00	0.00
F-G	-1524 / 0	-58.6	-58.6	0.08 (1)	5.51	0.00	0.00	0.00	0.00
G-H	-1360 / 0	-58.6	-58.6	0.21 (1)	5.13	0.00	0.00	0.00	0.00
H-I	-1372 / 0	-58.6	-58.6	0.22 (1)	4.92	0.00	0.00	0.00	0.00
I-J	-1892 / 0	-58.6	-58.6	0.22 (1)	5.33	0.00	0.00	0.00	0.00
J-K	-1692 / 0	-58.6	-58.6	0.22 (1)	4.92	0.00	0.00	0.00	0.00
K-L	-1895 / 0	-58.6	-58.6	0.22 (1)	4.92	0.00	0.00	0.00	0.00
L-M	0 / 17	-58.6	-58.6	0.27 (1)	4.72	0.00	0.00	0.00	0.00
Z-B	-1147 / 0	0.0	0.0	0.12 (1)	7.43	0.00	0.00	0.00	0.00
N-L	-1285 / 0	0.0	0.0	0.13 (1)	7.11	0.00	0.00	0.00	0.00
FR-TO									
A-B	0 / 243	-12.3	-12.3	0.04 (1)	10.00	0.00	0.00	0.00	0.00
Y-X	0 / 11	0.0	0.0	0.37 (1)	10.00	0.00	0.00	0.00	0.00
X-C	0 / 335	0.0	0.0	0.42 (1)	10.00	0.00	0.00	0.00	0.00
X-W	0 / 3116	-12.3	-12.3	0.54 (1)	10.00	0.00	0.00	0.00	0.00
W-V	0 / 2159	-12.3	-12.3	0.38 (1)	10.00	0.00	0.00	0.00	0.00
V-U	0 / 1767	-12.3	-12.3	0.33 (1)	10.00	0.00	0.00	0.00	0.00
T-S	0 / 0	-12.3	-12.3	0.00 (4)	10.00	0.00	0.00	0.00	0.00
S-R	0 / 15	-12.3	-12.3	0.08 (4)	10.00	0.00	0.00	0.00	0.00
R-Q	0 / 1514	-12.3	-12.3	0.29 (1)	10.00	0.00	0.00	0.00	0.00
Q-P	0 / 1514	-12.3	-12.3	0.29 (1)	10.00	0.00	0.00	0.00	0.00
P-O	0 / 1652	-12.3	-12.3	0.30 (1)	10.00	0.00	0.00	0.00	0.00
O-N	0 / 0	-12.3	-12.3	0.08 (4)	10.00	0.00	0.00	0.00	0.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 16.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")

CALCULATED VERT. DEFL.(LL) = L/999 (0.14")

ALLOWABLE DEFL.(TL) = L/360 (1.20")

CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.32/1.00 (C-D:1), BC=0.54/1.00 (W-X:1), WB=0.70/1.00 (F-U:1), SS=0.36/1.00 (C-X:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)

MAX MIN MAX MIN MAX MIN

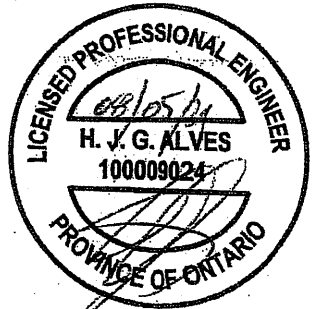
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

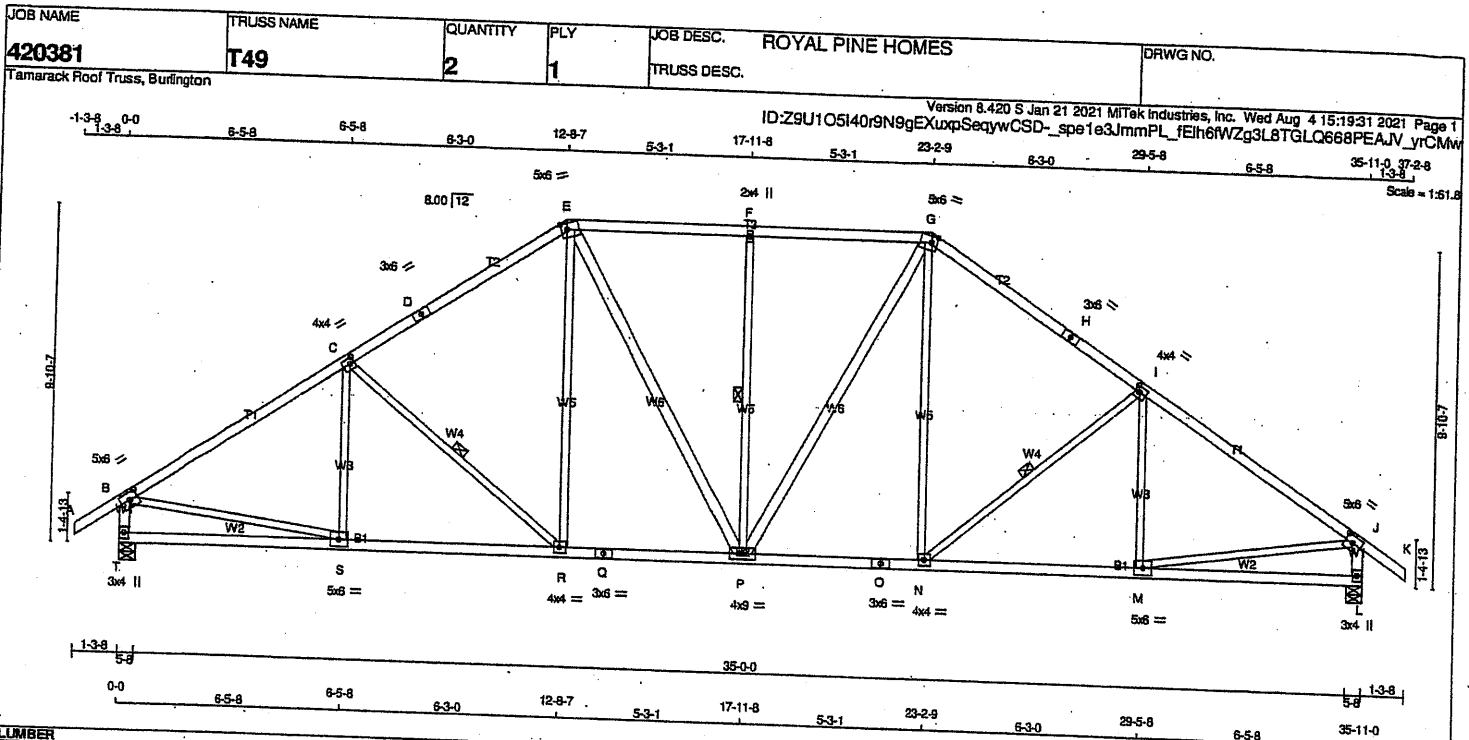
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (L) (INPUT = 0.90)

JSI METAL = 0.52 (X) (INPUT = 1.00)



Structural component only
DWG# T-2126502



LUMBER

N. L. G. A. RULES	
CHORDS	SIZE
A - D	2x4
D - E	2x4
E - G	2x4
G - H	2x4
H - K	2x4
T - B	2x4
L - J	2x4
T - Q	2x4
Q - O	2x4
O - L	2x4

ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	6.0	2.50	2.75
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	6.0		
E TTWW-m	MT20	5.0	6.0	2.25	2.00
F TMVW-t	MT20	2.0	4.0		
G TTWW-m	MT20	5.0	6.0	2.25	2.00
H TS-t	MT20	3.0	6.0		
I TMVW-t	MT20	4.0	4.0	2.00	1.50
J TMVW-t	MT20	5.0	6.0	2.50	2.75
K BMV1-p	MT20	3.0	4.0		
L BMVW-t	MT20	5.0	6.0		
M BMVW-t	MT20	4.0	4.0		
N BS-t	MT20	3.0	6.0		
O BMVW-t	MT20	4.0	4.0		
P BS-t	MT20	3.0	6.0		
Q BMVW-t	MT20	4.0	4.0		
R BMVW-t	MT20	5.0	6.0		
S BMV1-p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
T	1973	0	0	5-8
L	1973	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED							
T	1398	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1398	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	MAX. FACTORED	W E B S
MEMB.	FORCE (LBS)	FORCE (LBS)	FROM	TO	CSI (LC)	LENGTH	FR-TO	FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	S-C	-189 / 35	0.09 (1)	
B-C	-2286 / 0	-84.9	-84.9	0.80 (1)	3.97	C-R	-420 / 0	0.20 (1)	
C-D	-1956 / 0	-84.9	-84.9	0.54 (1)	4.26	R-E	0 / 378	0.09 (1)	
D-E	-1956 / 0	-84.9	-84.9	0.54 (1)	4.26	E-P	0 / 361	0.06 (1)	
E-F	-1773 / 0	-84.9	-84.9	0.34 (1)	4.67	P-F	-543 / 0	0.38 (1)	
F-G	-1773 / 0	-84.9	-84.9	0.34 (1)	4.67	G-H	0 / 361	0.08 (1)	
G-H	-1956 / 0	-84.9	-84.9	0.54 (1)	4.26	H-I	-420 / 0	0.09 (1)	
H-I	-1956 / 0	-84.9	-84.9	0.54 (1)	4.26	I-J	-420 / 0	0.20 (1)	
I-J	-2286 / 0	-84.9	-84.9	0.60 (1)	3.97	J-K	-189 / 35	0.09 (1)	
J-K	0 / 32	-84.9	-84.9	0.11 (1)	10.00	K-L	0 / 1942	0.44 (1)	
K-L	-1923 / 0	0.0	0.0	0.20 (1)	6.07	L-M	0 / 1942	0.44 (1)	
L-M	-1923 / 0	0.0	0.0	0.20 (1)	6.07				
T-S	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
S-R	0 / 1915	-18.5	-18.5	0.40 (1)	10.00				
R-Q	0 / 1599	-18.5	-18.5	0.32 (1)	10.00				
Q-P	0 / 1599	-18.5	-18.5	0.32 (1)	10.00				
P-O	0 / 1599	-18.5	-18.5	0.32 (1)	10.00				
O-N	0 / 1599	-18.5	-18.5	0.32 (1)	10.00				
N-M	0 / 1915	-18.5	-18.5	0.40 (1)	10.00				
M-L	0 / 0	-18.5	-18.5	0.18 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.09")
ALLOWABLE DEFL. (TL) = $L/360$ (1.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.17")

CSI: TC=0.60/1.00 (J-I:1), BC=0.40/1.00 (M-N:1),
WB=0.44/1.00 (J-M:1), SSI=0.22/1.00 (J-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

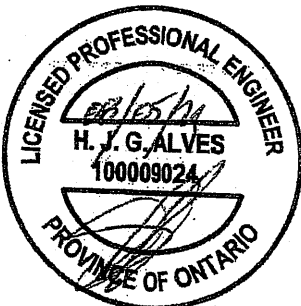
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

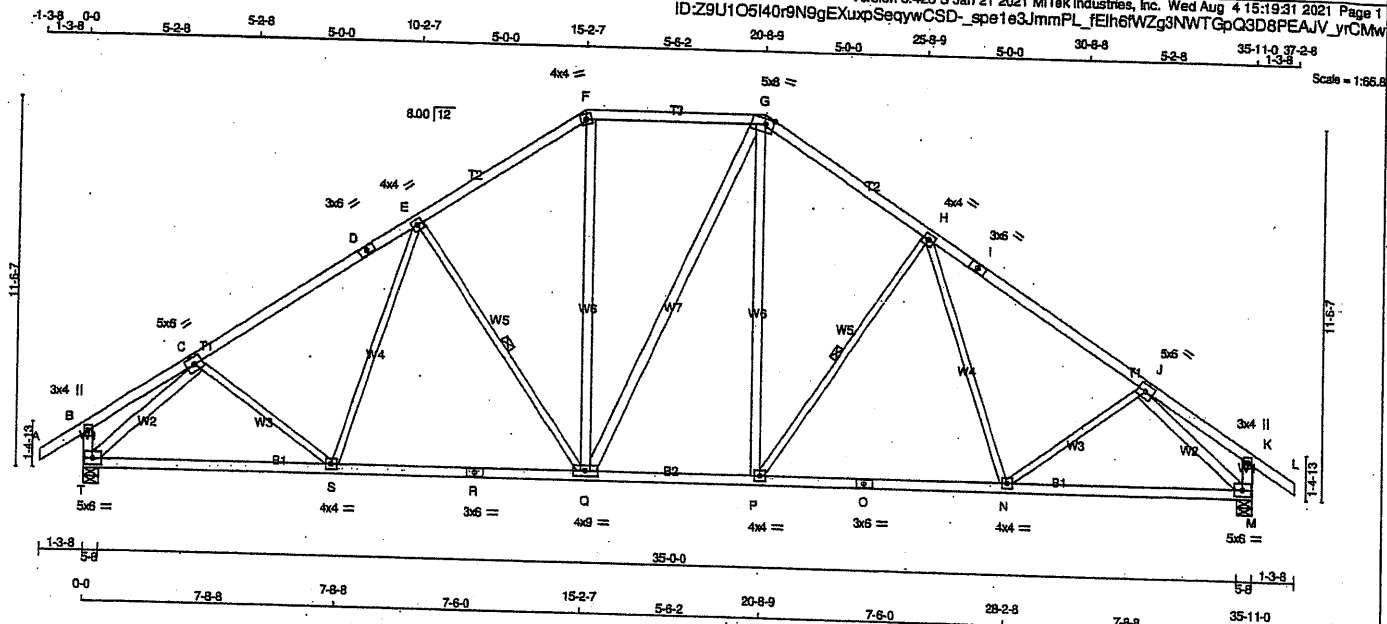
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.56 (J) (INPUT = 1.00)



Structural component only
DWG# T-2126524

JOB NAME 420381	TRUSS NAME T50	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed Aug 4 15:19:31 2021 Page 1 ID-Z9U1O5140r9N9gEXuxpSeqywCSD- spe1e3JmmPL_feihsWZg3NWTGpQ3D8PEAJV_yrCMW			
		TRUSS DESC.			



LUMBER	N.L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
L - S	2x4	DRY	No.2	SPF	
S - R	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
C - S	2x3	DRY	No.2	SPF	
S - E	2x3	DRY	No.2	SPF	
E - Q	2x3	DRY	No.2	SPF	
Q - H	2x3	DRY	No.2	SPF	
H - N	2x3	DRY	No.2	SPF	
N - J	2x3	DRY	No.2	SPF	

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMWW-t	MT20	5.0	6.0			
D	TS-t	MT20	3.0	6.0			
E	TMWW-t	MT20	4.0	4.0			
F	TTW-m	MT20	4.0	4.0			
G	TTWW-m	MT20	5.0	8.0	2.00	3.00	
H	TMWW-t	MT20	4.0	4.0			
I	TS-t	MT20	3.0	6.0			
J	TMWW-t	MT20	5.0	6.0			
K	TMV+p	MT20	3.0	4.0			
M	BMVW-t	MT20	5.0	6.0			
N, P, S							
N	BMWW-t	MT20	4.0	4.0			
O	BS-t	MT20	3.0	6.0			
Q	BMWWW-t	MT20	4.0	9.0			
R	BS-t	MT20	3.0	6.0			
T	BMVW-t	MT20	5.0	6.0			

NOTES (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
T	VERT	HORZ	UPLIFT	IN-SX
M	1973	0	1973	0
	0	1973	0	5-8
	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
T	1398
M	1398

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	W EBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
A-B	0/32	-84.9	-84.9	0.11 (1)	10.00	C-S	0/87	0.03 (4)
B-C	0/40	-84.9	-84.9	0.33 (1)	10.00	S-E	0/137	0.05 (4)
C-D	-2156/0	-84.9	-84.9	0.51 (1)	4.21	E-Q	-564/0	0.35 (1)
D-E	-2156/0	-84.9	-84.9	0.51 (1)	4.21	Q-F	0/589	0.09 (1)
E-F	-1777/0	-84.9	-84.9	0.39 (1)	4.61	F-G	0/1	0.00 (1)
F-G	-1455/0	-84.9	-84.9	0.36 (1)	5.02	G-H	0/581	0.09 (1)
G-H	-1777/0	-84.9	-84.9	0.39 (1)	4.61	H-I	-565/0	0.35 (1)
H-I	-2156/0	-84.9	-84.9	0.51 (1)	4.21	I-J	0/138	0.05 (4)
I-J	-2156/0	-84.9	-84.9	0.51 (1)	4.21	J-K	0/88	0.03 (4)
J-K	0/40	-84.9	-84.9	0.33 (1)	10.00	K-L	-2458/0	0.62 (1)
K-L	0/32	-84.9	-84.9	0.11 (1)	10.00	L-M	-2458/0	0.62 (1)
L-M	-186/0	0.0	0.0	0.02 (1)	7.81			
M-K	-186/0	0.0	0.0	0.02 (1)	7.81			
T-S	0/1746	-18.5	-18.5	0.43 (1)	10.00			
S-R	0/1761	-18.5	-18.5	0.43 (4)	10.00			
R-Q	0/1761	-18.5	-18.5	0.43 (4)	10.00			
Q-P	0/1455	-18.5	-18.5	0.32 (1)	10.00			
P-O	0/1761	-18.5	-18.5	0.43 (1)	10.00			
O-N	0/1761	-18.5	-18.5	0.43 (1)	10.00			
N-M	0/1746	-18.5	-18.5	0.43 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.51/1.00 (H-J:1), BC=0.43/1.00 (N-P:1),
WB=0.62/1.00 (J-M:1), SSI=0.19/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

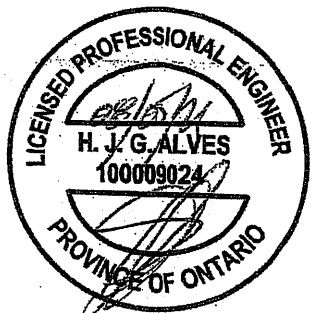
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

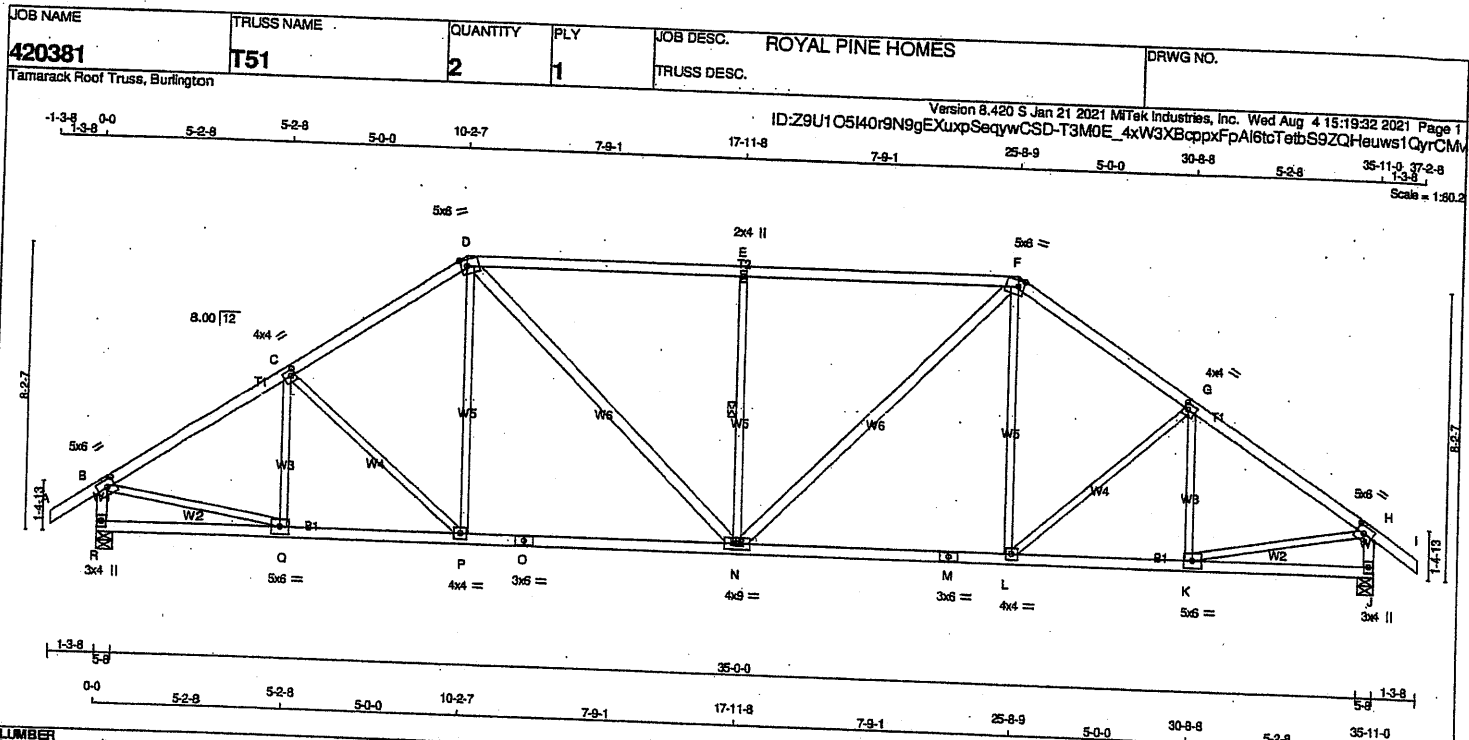
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (J) (INPUT = 0.90)
JSI METAL = 0.64 (O) (INPUT = 1.00)



Structural component only
DWG# T-2126525



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - I	2x4	DRY No.2
I - B	2x4	DRY No.2
J - H	2x4	DRY No.2
R - O	2x4	DRY No.2
O - M	2x4	DRY No.2
M - J	2x4	DRY No.2
ALL WEBS EXCEPT	2x3	DRY No.2
D - N	2x4	DRY No.2
N - F	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
PLATE	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-t	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	BMV1+p	MT20	3.0	6.0	2.50	2.75
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	1973	0	1973	0	5-8	5-8
J	1973	0	1973	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS					
JT		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1398	900/0	0/0	0/0	0/0	498/0	0/0
J	1398	900/0	0/0	0/0	0/0	498/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0/32	-84.9	-84.9	0.11 (1)	10.00	Q-C	-298 / 0 0.10 (1)
B-C	-2230 / 0	-84.9	-84.9	0.37 (1)	4.25	C-P	-208 / 0 0.18 (1)
C-D	-2102 / 0	-84.9	-84.9	0.35 (1)	4.37	P-D	0 / 261 0.06 (4)
D-E	-2197 / 0	-84.9	-84.9	0.81 (1)	3.56	D-N	0 / 672 0.11 (1)
E-F	-2197 / 0	-84.9	-84.9	0.81 (1)	3.56	N-E	-811 / 0 0.35 (1)
F-G	-2102 / 0	-84.9	-84.9	0.35 (1)	4.37	N-F	0 / 672 0.11 (1)
G-H	-2230 / 0	-84.9	-84.9	0.37 (1)	4.25	L-F	0 / 261 0.06 (4)
H-I	0/32	-84.9	-84.9	0.11 (1)	10.00	L-G	-208 / 0 0.18 (1)
R-B	-1929 / 0	0.0	0.0	0.20 (1)	6.06	K-G	-298 / 0 0.10 (1)
J-H	-1929 / 0	0.0	0.0	0.20 (1)	6.06	B-Q	0 / 1918 0.43 (1)
						K-H	0 / 1918 0.43 (1)
R-Q	0/0	-18.5	-18.5	0.11 (4)	10.00		
Q-P	0/1878	-18.5	-18.5	0.39 (1)	10.00		
P-O	0/1726	-18.5	-18.5	0.40 (1)	10.00		
C-N	0/1726	-18.5	-18.5	0.40 (1)	10.00		
N-M	0/1726	-18.5	-18.5	0.40 (1)	10.00		
M-L	0/1726	-18.5	-18.5	0.40 (1)	10.00		
L-K	0/1878	-18.5	-18.5	0.39 (1)	10.00		
K-J	0/0	-18.5	-18.5	0.11 (4)	10.00		

TOTAL WEIGHT = 2 X 161 = 323 lb [MIF]

DESIGN CRITERIA

SPECIFIED LOADS:
TOR CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.81/1.00 (D-E:1), BC=0.40/1.00 (N-P:1),
WB=0.43/1.00 (H-K:1), SS=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

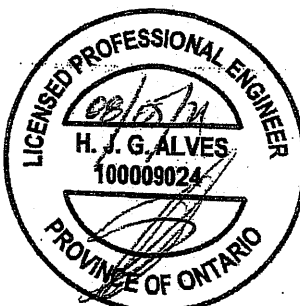
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.55 (H) (INPUT = 1.00)



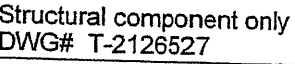
Structural component only
DWG# T-2126526

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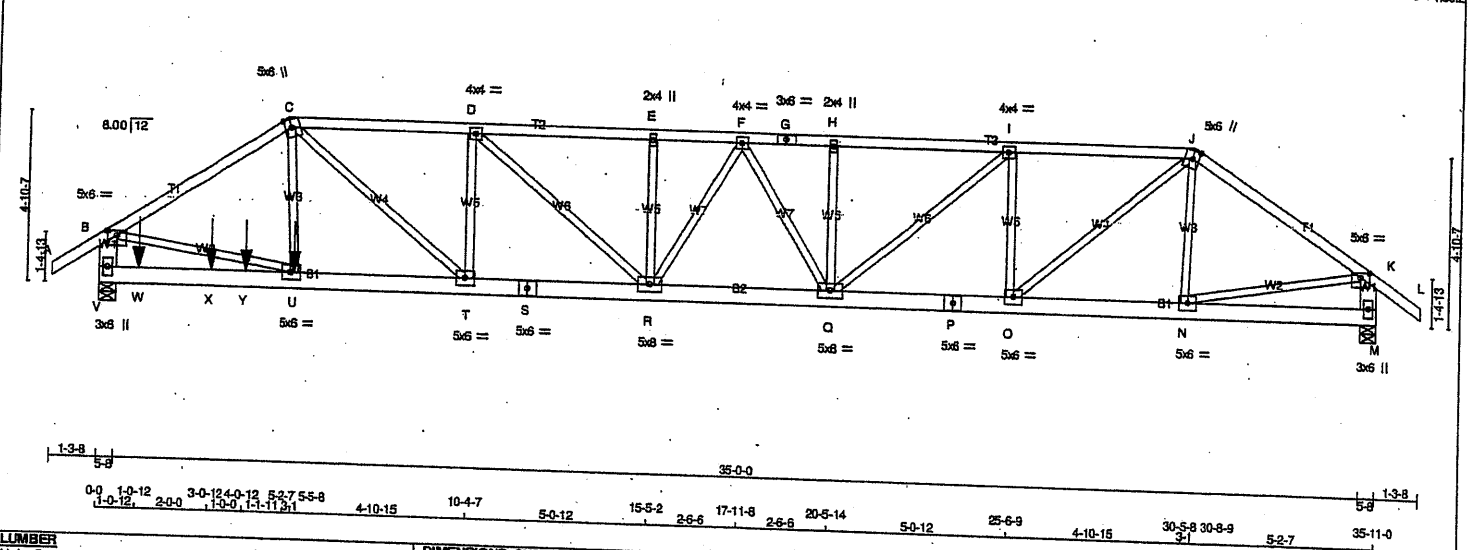
NOTES: (1)
) Lateral braces to be a minimum of 2X4 SPF #2.

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
R-TO		FROM TO			FR-TO		
B	0/32	-84.9	-84.9 0.11 (1)	10.00	C-R	0/179	0.04 (4)
D	0/30	-84.9	-84.9 0.22 (1)	10.00	R-D	0/112	0.04 (4)
C		-84.9	-84.9 0.35 (1)	4.28	D-P	0/1112	0.25 (1)
E	-2208/0	-84.9	-84.9 0.85 (1)	3.43	P-E	-629/0	0.44 (1)
F	-2644/0	-84.9	-84.9 0.83 (1)	3.43	E-O	-2/0	0.00 (1)
G	-2643/0	-84.9	-84.9 0.83 (1)	3.43	O-G	-628/0	0.44 (1)
H	-2643/0	-84.9	-84.9 0.84 (1)	3.44	C-H	0/1110	0.25 (1)
I	-2209/0	-84.9	-84.9 0.35 (1)	4.28	M-H	0/112	0.04 (4)
J	0/30	-84.9	-84.9 0.22 (1)	10.00	M-I	0/179	0.04 (4)
K	0/32	-84.9	-84.9 0.11 (1)	10.00	S-C	-2400/0	0.70 (1)
B	-187/0	0.0	0.0 0.02 (1)	7.81	I-L	-2401/0	0.70 (1)
J	-187/0	0.0	0.0 0.02 (1)	7.81			
R	0/1657	-18.5	-18.5 0.41 (1)	10.00			
Q	0/1821	-18.5	-18.5 0.43 (1)	10.00			
P	0/1821	-18.5	-18.5 0.43 (1)	10.00			
O	0/2645	-18.5	-18.5 0.50 (1)	10.00			
N	0/1821	-18.5	-18.5 0.44 (1)	10.00			
M	0/1821	-18.5	-18.5 0.44 (1)	10.00			
L	0/1668	-18.5	-18.5 0.41 (1)	10.00			

100



JOB NAME 420381	TRUSS NAME T53	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						
Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 15:19:35 2021 Page 1 ID:Z9U10540r9N9gEXuxpSeqyWCD-te28t06qp_vnTGXWwXjSkWE5e4e?MvXjK8WelyrCms						



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS

CHORDS : (0.122"x3") SPIRAL NAILS

WEBS : (0.122"x3") SPIRAL NAILS

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
V	3606	0	3606	0	0	5-8	5-8		
M	2283	0	2283	0	0	5-8	5-8		
UNFACTORED REACTIONS									
1ST LOASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
V	2556	1648 / 0	0 / 0	0 / 0	0 / 0	910 / 0	0 / 0		
M	1604	1034 / 0	0 / 0	0 / 0	0 / 0	570 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS		MAX. FACTORED		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD (PLF)	LC1	MAX.	MAX.	MEMB.	FORCE (LBS)	MAX.
FR-TO		FROM	TO	CSI (LC)	UNBRAC	LENGTH	FR-TO		CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.05 (1)	10.00		C-T	0 / 1235	0.15 (1)
B-C	-4518 / 0	-84.9	-84.9	0.37 (1)	4.21	T-D	-700 / 0	0.12 (1)	
C-D	-4747 / 0	-84.9	-84.9	0.30 (1)	4.18	D-R	0 / 298	0.04 (1)	
D-E	-4970 / 0	-84.9	-84.9	0.31 (1)	4.10	R-E	-329 / 0	0.06 (1)	
E-F	-4970 / 0	-84.9	-84.9	0.15 (1)	4.22	Q-H	-329 / 0	0.06 (1)	
F-G	-4643 / 0	-84.9	-84.9	0.15 (1)	4.35	Q-I	0 / 1177	0.15 (1)	
G-H	-4643 / 0	-84.9	-84.9	0.15 (1)	4.35	O-I	-1285 / 0	0.22 (1)	
H-I	-4643 / 0	-84.9	-84.9	0.30 (1)	4.22	O-J	0 / 2079	0.26 (1)	
I-J	-3763 / 0	-84.9	-84.9	0.28 (1)	4.64	N-K	0 / 2251	0.28 (1)	
J-K	-2661 / 0	-84.9	-84.9	0.28 (1)	5.23	R-F	0 / 248	0.03 (1)	
K-L	0 / 32	-84.9	-84.9	0.06 (1)	10.00	F-Q	-418 / 0	0.09 (1)	
V-B	-3532 / 0	0.0	0.0	0.13 (1)	7.45	C-U	0 / 1297	0.16 (1)	
M-K	-2220 / 0	0.0	0.0	0.08 (1)	7.81	B-U	0 / 3822	0.47 (1)	
V-W	0 / 0	-18.5	-18.5	0.05 (4)	10.00	N-J	-291 / 0	0.05 (1)	
W-X	0 / 0	-18.5	-18.5	0.05 (4)	10.00				
X-Y	0 / 0	-18.5	-18.5	0.05 (4)	10.00				
Y-U	0 / 0	-18.5	-18.5	0.05 (4)	10.00				
U-T	0 / 3819	-18.5	-18.5	0.28 (1)	10.00				
T-S	0 / 4747	-18.5	-18.5	0.34 (1)	10.00				
S-R	0 / 4747	-18.5	-18.5	0.34 (1)	10.00				
R-Q	0 / 4848	-18.5	-18.5	0.35 (1)	10.00				
Q-P	0 / 3769	-18.5	-18.5	0.28 (1)	10.00				
P-O	0 / 3763	-18.5	-18.5	0.28 (1)	10.00				
O-N	0 / 2201	-18.5	-18.5	0.16 (1)	10.00				
N-M	0 / 0	-18.5	-18.5	0.03 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	5-5-8	-1343	-1343		BACK	VERT	TOTAL		C1
W	1-0-12	-7	-7		BACK	VERT	TOTAL		C1
X	3-0-12	-7	-7		BACK	VERT	TOTAL		C1
Y	4-0-12	-7	-7		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 172 = 343 lb

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 23.3 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 36.7 PSF
SPACING	= 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")

CALCULATED VERT. DEFL.(LL) = L/999 (0.13")

ALLOWABLE DEFL.(TL) = L/360 (1.20")

CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.37/1.0 (B-C:1), BC=d.35/1.0 (Q-R:1), WB=0.47/1.0 (B-U:1), SSI=0.11/1.0 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

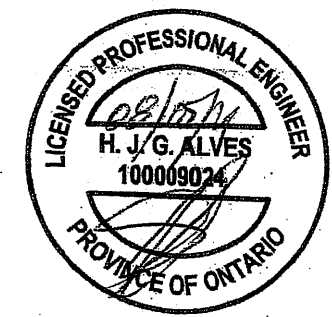
NAIL VALUES					
PLATE GRIP(DRY)	SHEAR	SECTION			
(PSI)	(PL)	(FL)			
MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (N) (INPUT = 0.90)

JSI METAL = 0.43 (N) (INPUT = 1.00)



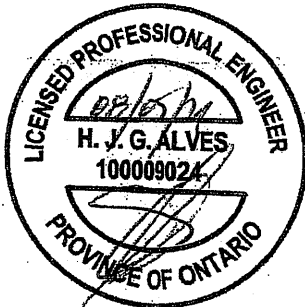
Structural component only

DWG# T-2126528

JOB NAME 420381	TRUSS NAME T53	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 15:18:35 2021 Page 2 ID:Z9U1O5I40r9N9dEXuxpSeqvwCSD-te28t06qp vmTGXWwxjSkWE5e4e?MvXjKr8WefyrCMs		

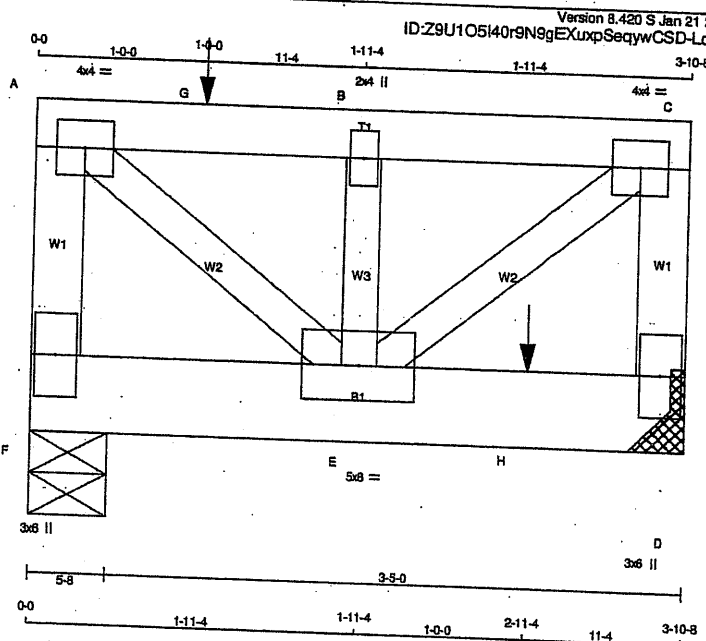
PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	1.50	3.00
C	TTWV+m	MT20	5.0	6.0	2.75	2.25
D, F, I						
D	TMWV-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
J	TTWV+m	MT20	5.0	6.0	2.75	2.25
K	TMWV-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N, O, T, U						
N	BMWV-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWVW-t	MT20	5.0	6.0		
R	BMWVW-t	MT20	5.0	3.0		
S	BS-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2126528 *mn*

JOB NAME 420381	TRUSS NAME T54	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. & A. RULES	CHORDS	SIZE	LUMBER
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PILES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMVW-w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1-p	MT20	3.0	6.0		
E	BMVWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG	BRG	BRG
F	1904	0	1904	0	0	5-8	5-8
D	1916	0	1916	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	1351	861 / 0	0 / 0	0 / 0	490 / 0	0 / 0
D	1357	877 / 0	0 / 0	0 / 0	480 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	LC1 MAX	CS1 (LC)
FR-TO		FROM TO		FR-TO			
F-A	-1878 / 0	0.0	0.0 0.11 (1)	A-E	0 / 1694	0.21 (1)	
A-G	-1298 / 0	-84.9	-84.9 0.57 (1)	E-B	-1352 / 0	0.11 (1)	
G-B	-1298 / 0	-84.9	-84.9 0.57 (1)	B-C	0 / 1694	0.21 (1)	
B-C	-1298 / 0	-84.9	-84.9 0.21 (1)				
D-C	-1030 / 0	0.0	0.0 0.06 (1)				
F-E	0 / 0	-18.5	-18.5 0.01 (1)				
E-H	0 / 0	-18.5	-18.5 0.30 (1)				
H-D	0 / 0	-18.5	-18.5 0.30 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-0-0	-1245	-1245	---	TOP	VERT	TOTAL	---	C1
H	2-11-4	-1179	-1179	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 18 = 37 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TFC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS1: TC=0.57/1.00 (A-B:1), BC=0.30/1.00 (D-E:1), WB=0.21/1.00 (A-E:1), SS=0.48/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.82 (A) (INPUT = 0.90)
JSI METAL=0.23 (A) (INPUT = 1.00)



Structural component only
DWG# T-2126529

JOB NAME 420381	TRUSS NAME T54	QUANTITY 1	PLY 2	JOB DESC. . ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 15:19:38 2021 Page 2 ID:Z9U1O5I40r9N9qEXuxpSeqywcSD-LqcX4L7Sa1d4Q6jUfEhGimCMU? 5QutZVu4AByrCMr		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

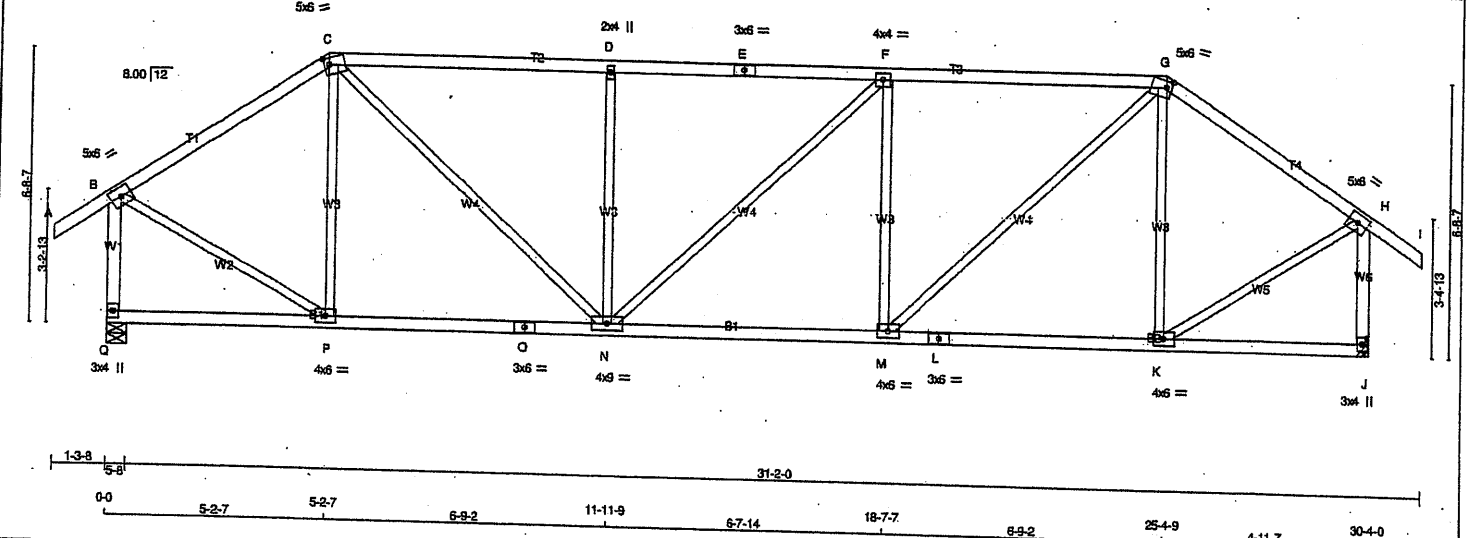


Structural component only
 DWG# T-2126529 *TH*

JOB NAME 420381	TRUSS NAME T55	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 Mtek Industries, Inc. Wed Aug 4 15:19:37 2021 Page 1
ID:Z9U105140r9N9gEXwpSeqywCSD-p0Avih74Lc9Uiahv1MlwpJLMuK2qp10o9ddleyrCMo

Scale = 1:51.5



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-t	MT20	5.0	8.0		
B	TTWW-m	MT20	5.0	6.0	2.00	1.50
C	TMW-w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TMW-t	MT20	5.0	6.0		
H	BMV1-p	MT20	5.0	8.0		
I	BMV1-p	MT20	3.0	4.0		
K	BMW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
Q	BMV1-p	MT20	3.0	4.0		

NOTES:

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	OUT-SX	IN-SX	OUT-SX
Q	1684	0	1684	0	0	5-8	5-8	5-8
J	1684	0	1684	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
Q	COMBINED	SNOW	LIVE			
Q	1193	770 / 0	0 / 0	0 / 0	423 / 0	0 / 0
J	1193	770 / 0	0 / 0	0 / 0	423 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. (PLF)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	CS (LC)	FR-TO	
A-B	0 / 32	-84.9 -84.9	0.11 (1)	P-C	-465 / 0
B-C	-1294 / 0	-84.9 -84.9	0.46 (1)	C-N	0 / 1060
C-D	-1838 / 0	-84.9 -84.9	0.67 (1)	N-D	-620 / 0
D-E	-1838 / 0	-84.9 -84.9	0.67 (1)	E-F	0 / 17
E-F	-1838 / 0	-84.9 -84.9	0.67 (1)	F-G	-633 / 0
F-G	-1826 / 0	-84.9 -84.9	0.88 (1)	G-H	0 / 1100
G-H	-1246 / 0	-84.9 -84.9	0.41 (1)	H-I	-509 / 0
H-I	0 / 32	-84.9 -84.9	0.11 (1)	I-J	0 / 1227
I-J	-1648 / 0	0.0 0.0	0.30 (1)	J-K	0 / 1212
J-K	-1650 / 0	0.0 0.0	0.32 (1)	K-L	0.27 (1)
Q-P	0 / 0	-18.5 -18.5	0.16 (4)		
P-O	0 / 1068	-18.5 -18.5	0.26 (1)		
O-N	0 / 1068	-18.5 -18.5	0.26 (1)		
N-M	0 / 1826	-18.5 -18.5	0.38 (1)		
M-L	0 / 1027	-18.5 -18.5	0.25 (1)		
L-K	0 / 1027	-18.5 -18.5	0.25 (1)		
K-J	0 / 0	-18.5 -18.5	0.15 (4)		

TOTAL WEIGHT = 133 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 36.7	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.68/1.00 (F-G:1), BC=0.38/1.00 (M-N:1),
WS=0.47/1.00 (F-M:1), SS=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (G) (INPUT = 0.90)
JSI METAL = 0.37 (O) (INPUT = 1.00)

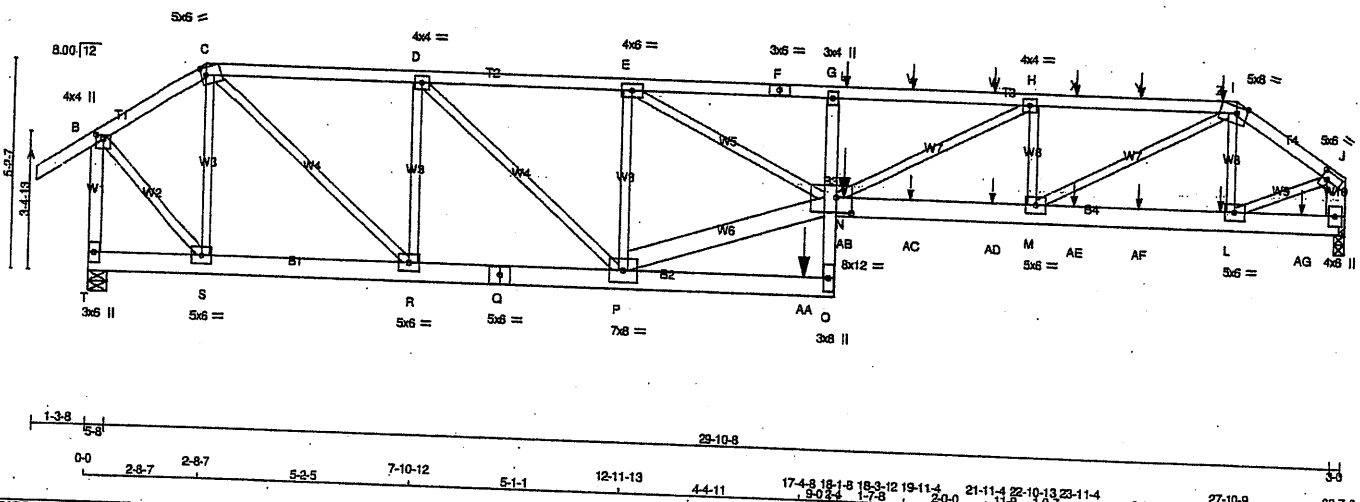


Structural component only
DWG# T-2126530

JOB NAME 420381	TRUSS NAME T56S	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 15:19:38 2021 Page 1
ID:Z9U1O5I40r9N9gEXuPSeqwCSD-HDKHV18i6vHLKkG5b4H9M8rcziHYZIVAOpNBE4yrCMp

1-3-8 0-0 2-8-7 2-8-7 5-2-5 7-10-12 5-1-1 12-11-13 5-3-15 18-3-12 19-11-4 21-11-4 22-10-13 23-11-4 25-8-4 27-8-4 27-10-9 30-7-0
Scale = 1:52.2



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - F	2x4	DRY	No.2	SPF		
F - I	2x4	DRY	No.2	SPF		
I - J	2x4	DRY	No.2	SPF		
J - K	2x4	DRY	No.2	SPF		
K - Q	2x6	DRY	No.2	SPF		
Q - O	2x6	DRY	No.2	SPF		
O - G	2x4	DRY	No.2	SPF		
N - K	2x6	DRY	No.2	SPF		
ALL WEBS EXCEPT P - N	2x3	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)

TOP CHORDS: (0.122"x3") SPIRAL NAILS

A-C	1	12	TOP
C-F	1	12	TOP
F-I	1	12	TOP
I-J	1	12	TOP
J-K	1	12	TOP
K-Q	2	12	TOP

BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS

T-Q	2	12	TOP
Q-O	2	12	TOP
N-K	2	12	TOP
O-G	1	12	TOP

WEBS: (0.122"x3") SPIRAL NAILS

2x3	1	6	TOP
2x6	2	6	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
JT	1892	0	1892	0	5-8	5-8
T	1892	0	1892	0	3-0	3-0
K	1838	0	1838	0	5-8	5-8

	UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	1ST CASE	2ND CASE	SNOW	LIVE			
JT	1340	869 / 0	0 / 0	0 / 0	0 / 0	471 / 0	0 / 0
T	1303	834 / 0	0 / 0	0 / 0	0 / 0	469 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB. MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD (PLF)

MAX. UNBRACED LENGTH (LC)

WEBS

MEMB. MAX. FACTORED FORCE (LBS)

MAX. UNBRACED LENGTH (LC)

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 886-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.02")

CALCULATED VERT. DEFL.(LL) = L/999 (0.14")

ALLOWABLE DEFL.(TL) = L/360 (1.02")

CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.30/1.00 (G+H:1), BC=0.30/1.00 (M+N:1), WB=0.33/1.00 (E-P:1), SS=0.14/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.25 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)

JSI METAL= 0.28 (E) (INPUT = 1.00)



Structural component only
DWG# T-2126531

SPECIFIED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	U									
U	27-6-4	1	1	70	70	FRONT	VERT	TOTAL	—	C1
V	19-11-4	1	1	70	70	FRONT	VERT	TOTAL	—	C1
W	21-11-4	1	1	70	70	FRONT	VERT	TOTAL	—	C1
X	23-11-4	1	1	70	70	FRONT	VERT	TOTAL	—	C1

JOB NAME 420381	TRUSS NAME T56S	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:Z9U1O5I40r9N9qEXuxpSeqvCSD-HDKtV18I6vHLKkG5b4H9M8rczlhYZIVA0pNBE4vrCMp

PLATES (table is in inches)

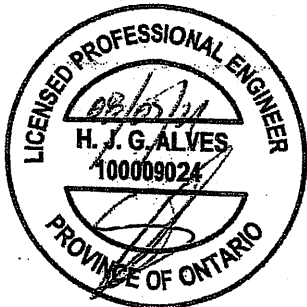
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TFWW-m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TFWW-m	MT20	5.0	8.0	2.00	3.00
J	TMVW-t	MT20	5.0	6.0	2.50	1.75
K	BMV1+p	MT20	4.0	6.0		
L, M, R, S						
L	BMWW-t	MT20	5.0	6.0		
N	BVMWWW-t	MT20	8.0	12.0	4.50	4.50
O	BMV+p	MT20	3.0	8.0		
P	BMWWW-t	MT20	7.0	8.0		
Q	BS-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

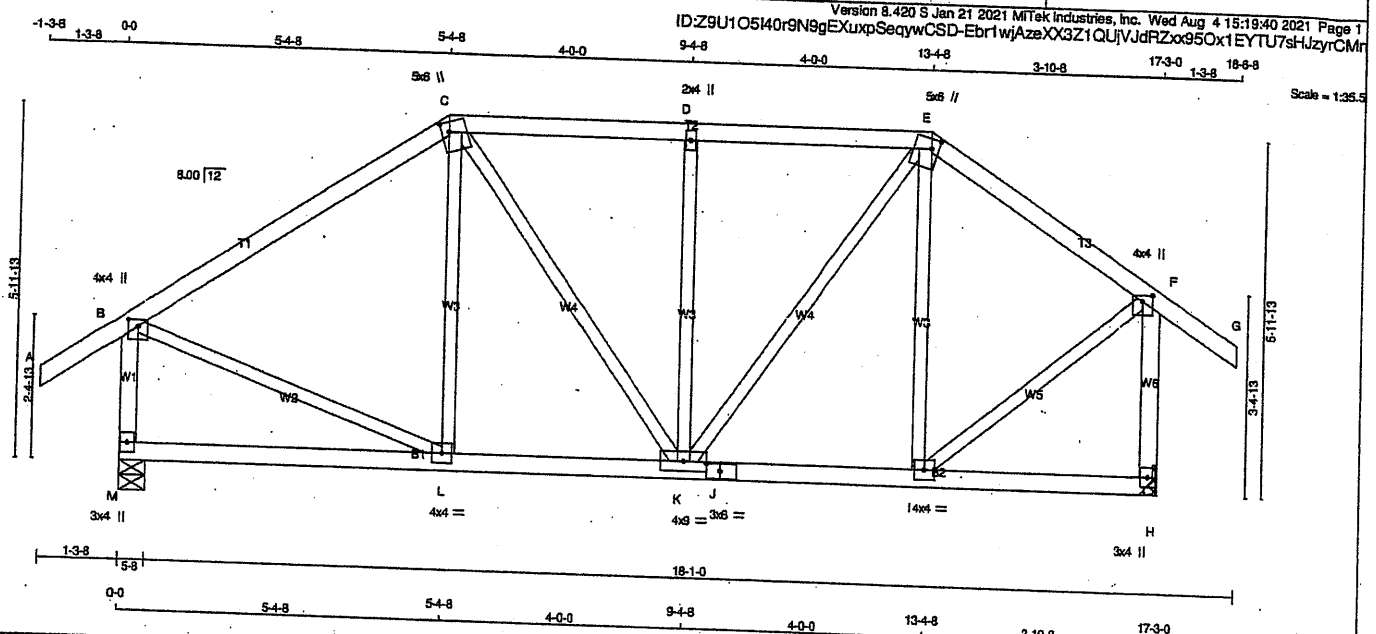
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	25-6-4	1	1	70	FRONT	VERT	TOTAL	—	C1
Z	27-6-4	1	1	55	FRONT	VERT	TOTAL	—	C1
AA	17-4-8	-318	-318	—	FRONT	VERT	TOTAL	—	C1
AB	18-3-12	-3	-3	—	FRONT	VERT	TOTAL	—	C1
AC	19-11-4	1	1	—	FRONT	VERT	TOTAL	—	C1
AD	21-11-4	1	1	—	FRONT	VERT	TOTAL	—	C1
AE	23-11-4	1	1	—	FRONT	VERT	TOTAL	—	C1
AF	25-6-4	1	1	—	FRONT	VERT	TOTAL	—	C1
AG	29-6-4	1	1	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2126531 *M*

JOB NAME 420381	TRUSS NAME T58	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
C	TTWW+m	MT20	5.0	8.0	2.00 1.50
D	TMVW+p	MT20	2.0	4.0	
E	TTWW+m	MT20	5.0	8.0	2.00 1.50
F	TMVW+p	MT20	4.0	4.0	1.25 2.00
H	BMV1+p	MT20	3.0	4.0	
I	BMVW-t	MT20	4.0	4.0	
J	BS-t	MT20	3.0	6.0	1.50 2.75
K	BMVW-t	MT20	4.0	4.0	
L	BMVW-t	MT20	4.0	4.0	
M	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
M	1008	0	1008	0	0	5-8	5-8	5-8	5-8
H	1008	0	1008	0	0	MECHANICAL	MECHANICAL	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	713	468 / 0	0 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
H	713	468 / 0	0 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

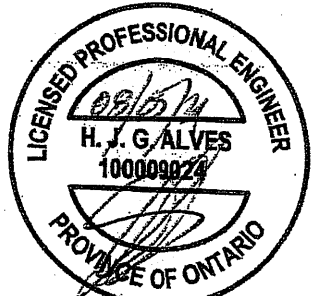
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	UNBRAC
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	L-C	-118 / 35	0.07 (1)	
B-C	-713 / 0	-84.9	-84.9	0.32 (1)	6.25	C-K	0 / 188	0.04 (1)	
C-D	-899 / 0	-84.9	-84.9	0.17 (1)	6.25	K-D	-413 / 0	0.23 (1)	
D-E	-899 / 0	-84.9	-84.9	0.17 (1)	6.25	E-F	0 / 383	0.09 (1)	
E-F	-582 / 0	-84.9	-84.9	0.11 (1)	6.25	F-G	-286 / 0	0.18 (1)	
F-G	0 / 32	-84.9	-84.9	0.11 (1)	10.00	G-L	0 / 834	0.14 (1)	
M-B	-968 / 0	0.0	0.0	0.12 (1)	7.81	H-F	0 / 812	0.14 (1)	
H-F	-979 / 0	0.0	0.0	0.19 (1)	7.81				
M-L	0 / 0	-18.5	-18.5	0.12 (4)	10.00				
L-K	0 / 591	-18.5	-18.5	0.17 (4)	10.00				
K-J	0 / 478	-18.5	-18.5	0.12 (1)	10.00				
J-I	0 / 478	-18.5	-18.5	0.12 (1)	10.00				
I-H	0 / 0	-18.5	-18.5	0.07 (4)	10.00				



Structural component only
DWG# T-2126533

TOTAL WEIGHT = 83 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.32/1.00 (B-C:1), BC=0.17/1.00 (K-L:4), WB=0.23/1.00 (D-K:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

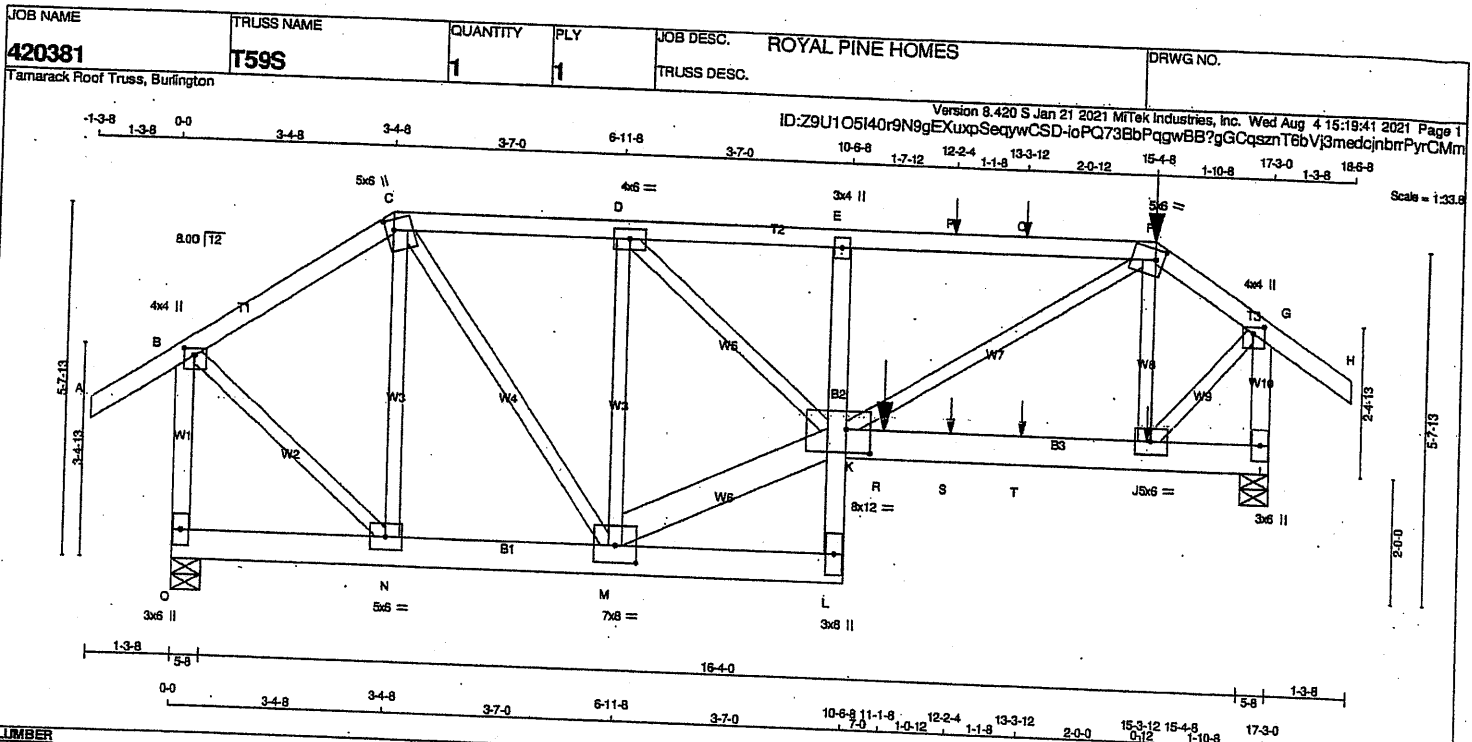
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.71 (L) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x6	DRY	No.2
L - E	2x4	DRY	No.2
K - I	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
M - K 2x6 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A - B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C - D	TTWW+m	MT20	5.0	6.0	2.00	1.50
D - E	TMVW-t	MT20	4.0	6.0		
E - F	TMV+p	MT20	3.0	4.0		
F - G	TTWW-m	MT20	5.0	6.0	2.00	1.50
G - H	TMVW+p	MT20	4.0	4.0	1.25	2.00
I - J	BMV1+p	MT20	3.0	6.0		
J - K	BMVW-t	MT20	5.0	6.0		
K - L	BMVW-t	MT20	8.0	12.0	4.50	4.50
L - M	BMV+p	MT20	3.0	8.0		
M - N	BMVW-t	MT20	7.0	8.0	3.25	4.00
N - O	BMVW-t	MT20	5.0	6.0		
O - P	BMV1+p	MT20	3.0	6.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
O	1131	0	1131	0	5-8	5-8
I	1290	0	1290	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
O	800	525 / 0	0 / 0	0 / 0	0 / 0	275 / 0	0 / 0
I	911	805 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED	FORCE	VERT. LOAD	CS1 (LC)			MAX. FACTORED	FORCE
FR-TO		(LBS)		(PLF)				
A-B	0 / 32		FROM TO		10.00	N-C	-399 / 0	0.19 (1)
B-C	-668 / 0		-84.9 -84.9	0.13 (1)	6.25	C-M	0 / 594	0.15 (1)
C-D	-875 / 0		-84.9 -84.9	0.17 (5)	6.25	M-D	-930 / 0	0.43 (1)
D-E	-1558 / 0		-84.9 -84.9	0.29 (1)	4.92	M-K	0 / 1003	0.13 (1)
E-P	-1558 / 0		-84.9 -84.9	0.34 (1)	4.85	D-K	0 / 925	0.23 (1)
P-Q	-1558 / 0		-84.9 -84.9	0.34 (1)	4.85	K-F	0 / 1108	0.27 (1)
Q-F	-1558 / 0		-84.9 -84.9	0.34 (1)	4.85	J-F	-452 / 0	0.10 (1)
F-G	-811 / 0		-84.9 -84.9	0.13 (1)	6.25	B-N	0 / 730	0.18 (1)
G-H	0 / 32		-84.9 -84.9	0.13 (1)	10.00	J-G	0 / 927	0.23 (1)
O-B	-1107 / 0		0.0 0.0	0.21 (1)	7.49			
I-G	-1337 / 0		0.0 0.0	0.17 (1)	6.97			
O-N	0 / 0		-18.5 -18.5	0.03 (4)	10.00			
N-M	0 / 545		-18.5 -18.5	0.09 (1)	10.00			
M-L	0 / 15		-18.5 -18.5	0.03 (4)	10.00			
L-K	0 / 30		0.0 0.0	0.05 (1)	10.00			
K-E	-410 / 0		0.0 0.0	0.06 (1)	7.81			
K-R	0 / 638		-18.5 -18.5	0.24 (1)	10.00			
R-S	0 / 638		-18.5 -18.5	0.24 (1)	10.00			
S-T	0 / 638		-18.5 -18.5	0.24 (1)	10.00			
T-J	0 / 638		-18.5 -18.5	0.24 (1)	10.00			
J-I	0 / 0		-18.5 -18.5	0.10 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.
F	15-4-8	-45	-45	70	FRONT	VERT	TOTAL	C1
J	15-3-12	1	1	70	FRONT	VERT	TOTAL	C1
P	12-2-4	1	1	70	FRONT	VERT	TOTAL	C1
Q	13-3-12	1	1	70	FRONT	VERT	TOTAL	C1
R	11-1-8	-240	-240	70	FRONT	VERT	TOTAL	C1
S	12-2-4	1	1	70	FRONT	VERT	TOTAL	C1
T	13-3-12	1	1	70	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 102 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.58")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.04")
ALLOWABLE DEFL. (TL) = $L/360$ (0.58")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.08")

CS1: TC=0.34/1.00 (E-F:1), BC=0.24/1.00 (J-K:1),
WB=0.43/1.00 (D-M:1), SS=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

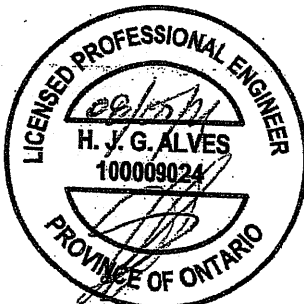
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

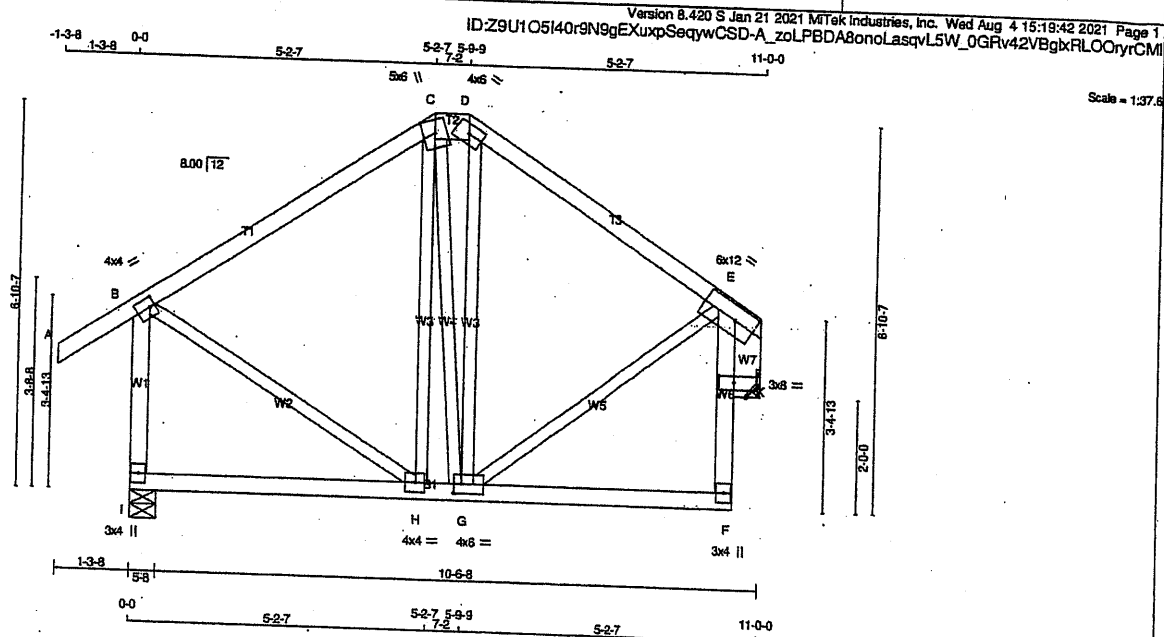
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.89 (G) (INPUT = 0.90)
JSI METAL=0.27 (G) (INPUT = 1.00)



Structural component only
DWG# T-2126534

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420381	T60	1	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						



<u>LUMBER</u>			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - C	2x4	DRY	No.2
C - D	2x6	DRY	No.2
D - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)						
J	TYPE	PLATES	W	LEN	Y	X
B	TMWV-t	MT20	4.0	4.0	2.00	1.00
C	TTWW+m	MT20	5.0	6.0	2.75	2.50
D	TMW-h	MT20	4.0	6.0		
E	TMWVW-t	MT20	6.0	12.0	3.75	4.50
F	BMV+p	MT20	3.0	4.0		
G	BMWVW-t	MT20	4.0	6.0	2.00	1.50
H	BMWV-t	MT20	4.0	4.0		
I	BMV-t+p	MT20	3.0	4.0		
J	KP-p	MT20	3.0	8.0	1.50	3.25

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

[illegible]

UNFACTORED REACTIONS

1ST CASE							
MAX/MIN COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K(E)	376	238 / 0	0 / 0				
I	478	316 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0
				0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 32	-84.9	-84.9 0.11 (1)	10.00	H-C	-65 / 25	0.05 (1)
B-C	-286 / 0	-84.9	-84.9 0.40 (1)	8.25	G-D	-71 / 15	0.06 (1)
C-D	-237 / 0	-84.9	-84.9 0.00 (1)	8.25	B-H	0 / 279	0.06 (1)
D-E	-283 / 0	-84.9	-84.9 0.33 (1)	8.25	C-G	-24 / 0	0.02 (1)
I-B	-639 / 0	0.0	0.0 0.12 (1)	7.81	E-K	0 / 232	0.05 (1)
F-J	0 / 38	0.0	0.0 0.13 (1)	10.00	G-E	-540 / 0	0.04 (1)
J-E	0 / 38	0.0	0.0 0.13 (1)	10.00	J-K	0 / 108	0.00 (1)
I-H	0 / 0	-18.5	-18.5 0.11 (4)	10.00			
H-G	0 / 240	-18.5	-18.5 0.13 (4)	10.00			
G-F	0 / 49	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD		=	36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC.2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (0.35")
CALCULATED VERT. DEFL(LL) = L/999 (0.00")
ALLOWABLE DEFL(TL)= L/360 (0.35")
CALCULATED VERT. DEFL(TL) = L/999 (0.02")

CSI: TC=0.40/1.00 (B-C:1), BC=0.13/1.00 (G-H:4),
WB=0.06/1.00 (B-H:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

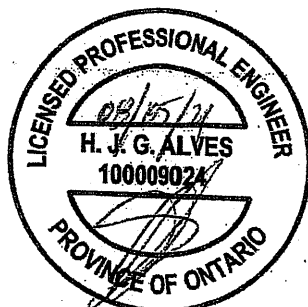
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		SHEAR		SECTION	
PLATE	GRIP(DRY) (PSI)		(PLI)		(PLI)
	MAX MIN	MAX MIN		MAX MIN	
MT20	650 371	1747 788		1987 1873	

PLATE PLACEMENT TOL = 0.250 inches

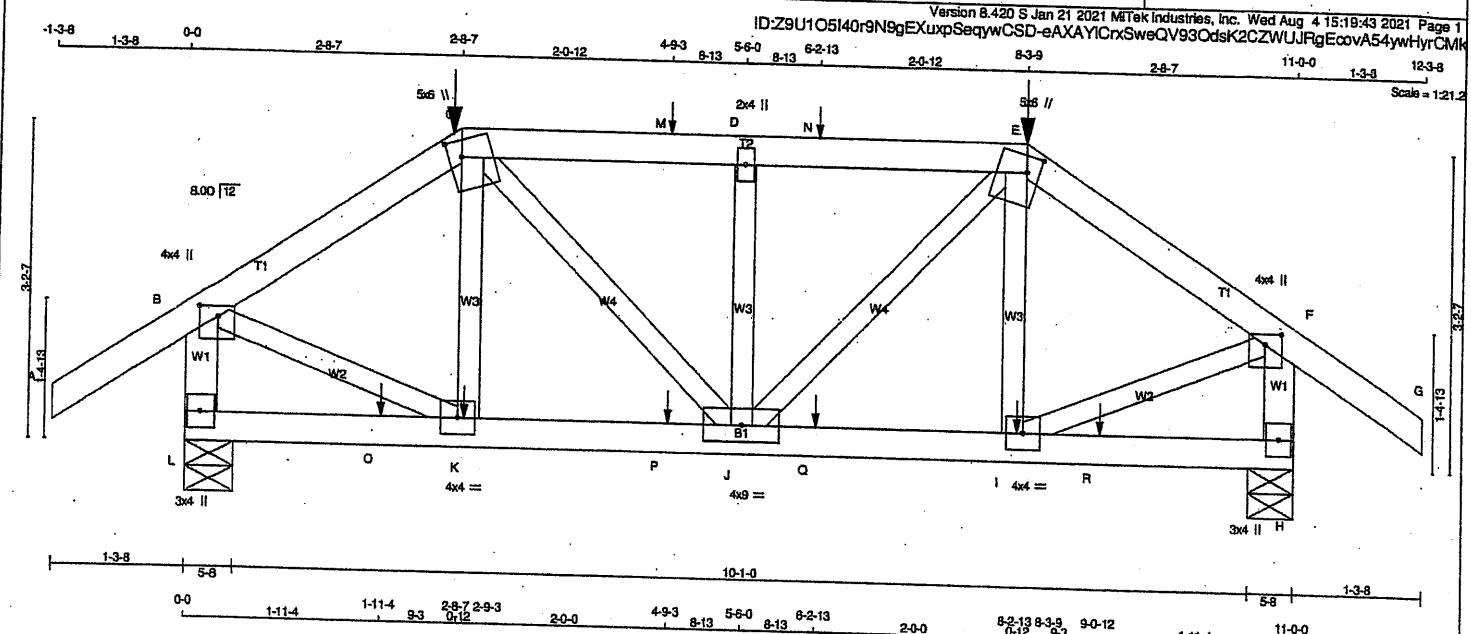
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126535

JOB NAME 420381	TRUSS NAME T61	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Wed Aug 4 15:18:43 2021 Page 1
ID:Z9U1O5140r9N9gEXuPSeqyWCSd-eAXAYICrXSwQV93OdsK2CZWUJRgEccvA54ywhrCMk
Scale = 1:21.2

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 49 lb			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD	DESIGN CRITERIA	
A - C	2x4	DRY	No.2	SPF		GROSS REACTION	GROSS REACTION	BRG	BRG	SPECIFIED LOADS:	
C - E	2x4	DRY	No.2	SPF		VERT	DOWN	HORZ	UPLIFT	TOP CH. LL = 23.3 PSF	
E - G	2x4	DRY	No.2	SPF		H	790	0	0	DL = 6.0 PSF	
L - F	2x4	DRY	No.2	SPF			761	0	0	BOT CH. LL = 0.0 PSF	
H - F	2x4	DRY	No.2	SPF				5-8	5-8	DL = 7.4 PSF	
L - H	2x4	DRY	No.2	SPF				5-8	5-8	TOTAL LOAD = 36.7 PSF	
ALL WEBS	2x3	DRY	No.2	SPF		UNFACTORED REACTIONS				SPACING = 24.0 IN/C	
EXCEPT						1ST LOASE					
DRY: SEASONED LUMBER.						MAX/MIN. COMPONENT REACTIONS					

1ST LOASE	MAX/MIN. COMPONENT REACTIONS	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H
JT	COMBINED	
L	554	390 / 0
H	535	371 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING			
TOTAL LOAD CASES: (7)			
CHORDS	MAX. FACTORED	FACTORED	WEBS
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	MAX. FACTORED
FR-TO	FROM TO	CS1 (LC)	UNBRACED LENGTH

BMWW-t	MT20	4.0	4.0
BMV1+p	MT20	3.0	4.0

MEMBERS				WEBS			
MEMB.		FORCE (LBS)	MAX. FACTORED	MEMB.		FORCE (LBS)	MAX. FACTORED
FR-TO	FROM TO	CS1 (LC)	UNBRACED LENGTH	FR-TO	FROM TO	CS1 (LC)	UNBRACED LENGTH
A-B	0 / 32	-84.9	-84.9 0.13 (1)	10.00	K-C	-138 / 7	0.03 (1)
B-C	-612 / 0	-84.9	-84.9 0.12 (1)	6.25	C-J	0 / 204	0.05 (1)
C-M	-645 / 0	-84.9	-84.9 0.11 (1)	6.25	J-D	-281 / 0	0.06 (1)
M-D	-645 / 0	-84.9	-84.9 0.11 (1)	6.25	J-E	0 / 242	0.06 (1)
D-N	-645 / 0	-84.9	-84.9 0.11 (1)	6.25	E-F	-125 / 7	0.03 (1)
N-E	-645 / 0	-84.9	-84.9 0.11 (1)	6.25	B-K	0 / 544	0.13 (1)
E-F	-580 / 0	-84.9	-84.9 0.12 (1)	6.25	F-H	0 / 516	0.13 (1)
F-G	0 / 32	-84.9	-84.9 0.13 (1)	10.00			
L-B	-768 / 0	0.0	0.0 0.09 (1)	7.81			
H-F	-740 / 0	0.0	0.0 0.08 (1)	7.81			

L-O	0 / 0	-18.5	-18.5 0.04 (4)	10.00
O-K	0 / 0	-18.5	-18.5 0.04 (4)	10.00
K-P	0 / 503	-18.5	-18.5 0.11 (1)	10.00
P-J	0 / 503	-18.5	-18.5 0.11 (1)	10.00
J-Q	0 / 477	-18.5	-18.5 0.10 (1)	10.00
Q-I	0 / 477	-18.5	-18.5 0.10 (1)	10.00
I-R	0 / 0	-18.5	-18.5 0.04 (4)	10.00
R-H	0 / 0	-18.5	-18.5 0.04 (4)	10.00

(58 PSF OF 27.2 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.11/1.00 (J-K:1) ,
WB=0.13/1.00 (B-K:1) , SS=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

ALLOWABLE DEFL.(LL) = L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")
CSI: TC=0.13/1.00 (A-B-1), BC=0.11/1.00 (J-K-1),
WB=0.13/1.00 (B-K-1), SS=0.12/1.00 (C-D-1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 1.00

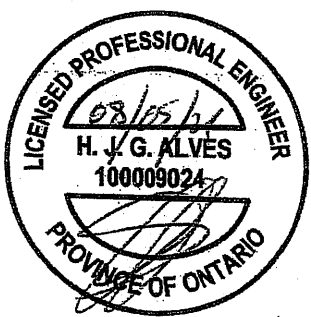
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

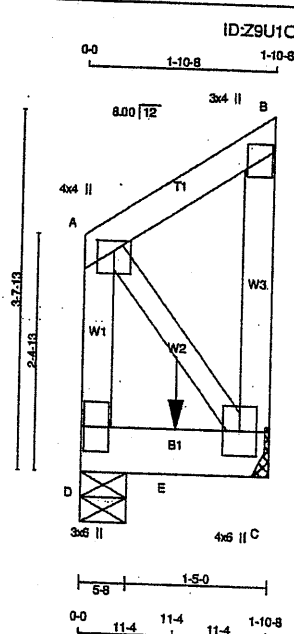
PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.
JSI GRIP = 0.60 (K) (INPUT = 0.90)
JSI METAL = 0.17 (K) (INPUT = 1.00)

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2126536



JOB NAME 420381 Tamarack Roof Truss, Burlington	TRUSS NAME T62	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
D - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - C	2x8	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				No.2	SPF
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D - A 1	12	TOP
A - B 1	12	TOP
B - C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D - C 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX
D	353	0	353	0	0	5-8	5-8		
C	353	0	353	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	DEAD	SOIL
D	250	163/0	0/0	0/0
C	250	163/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
D-A	-80/0	0.0	0.01 (1)	A-C	0/0
A-B	0/0	-84.9	0.03 (1)		0.00 (1)
C-B	-80/0	0.0	0.01 (1)		
D-E	0/0	-18.5	0.09 (1)		
E-C	0/0	-18.5	0.09 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	11-4	-362	-362		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 13 = 26 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.03/1.00 (A-B:1), BC=0.09/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SS=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

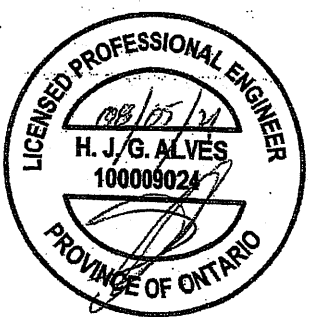
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

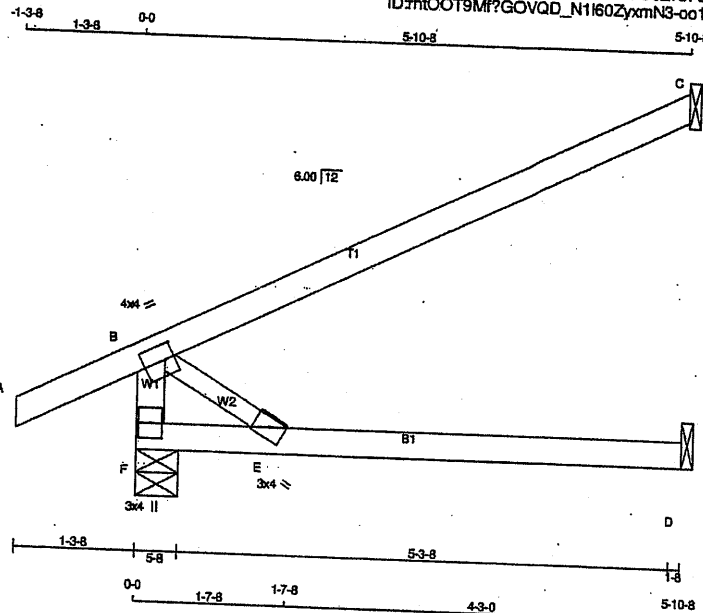
JSI GRIP= 0.03 (A) (INPUT = 0.90)
JSI METAL= 0.01 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126537

JOB NAME 420375	TRUSS NAME J1	QUANTITY 13	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:htOOT9Mf?GOVQD_N1160ZymN3-oo1CHARUBo99E9VMXEXDQlrfuqmG5ZEReaRyrEgv



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	4.0	4.0	2.00 1.25
E	BMVW-w	MT20	3.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	419	0	419	0	5-8	5-8
C	249	0	249	0	1-8	1-8
D	54	0	61	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
F	255	200/0	0/0	0/0	95/0	0/0	0/0
C	172	137/0	0/0	0/0	35/0	0/0	0/0
D	43	0/0	0/0	0/0	43/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO			FR-TO		
F-B	-364/0	0.0	0.0	0.04 (1)	7.81	0/0	0.00 (1)
A-B	0/26	-84.9	-84.9	0.11 (1)	10.00		
B-C	0/0	-84.9	-84.9	0.50 (1)	10.00		
F-E	0/0	-18.5	-18.5	0.14 (4)	10.00		
E-D	0/0	-18.5	-18.5	0.19 (4)	10.00		

TOTAL WEIGHT = 13 X 18 = 234 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CS1: TC=0.50/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

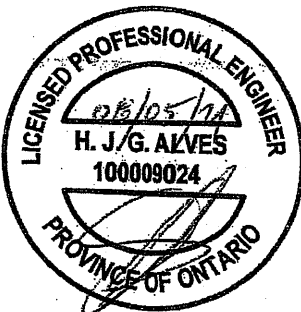
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

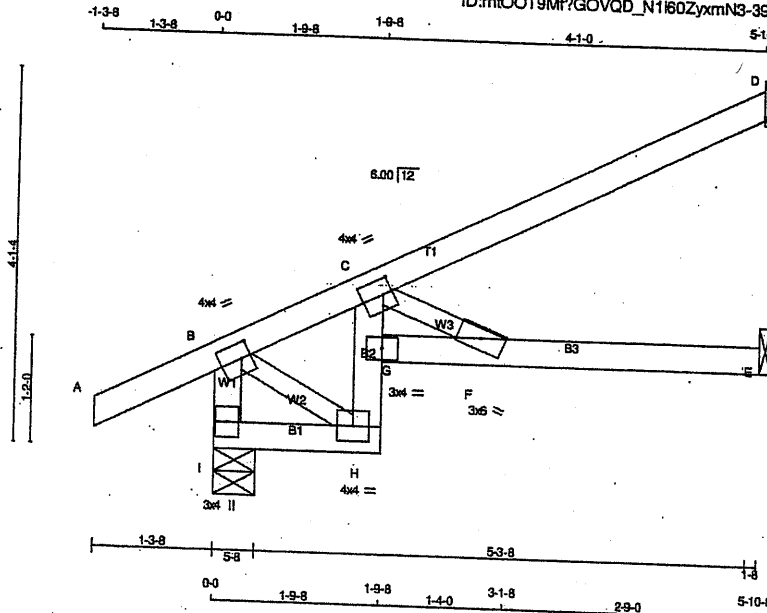
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126452

JOB NAME 420376	TRUSS NAME J1S	QUANTITY 8	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 13:00:35 2021 Page 1 ID: rhtOOT9Mf?GOVQD_N160ZyxmN3-396HUGARuX97vbeMO5hjmpsyABCH499YJtCw9yrEPA					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
I - B	2x4	DRY No.2
A - D	2x4	DRY No.2
I - H	2x4	DRY No.2
H - C	2x4	DRY No.2
G - E	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.

SPF	SPF	SPF	SPF	SPF
SPF	SPF	SPF	SPF	SPF
SPF	SPF	SPF	SPF	SPF
SPF	SPF	SPF	SPF	SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
I	432	0	432	0	5-8	5-8
D	184	0	184	0	1-8	1-8
E	107	0	107	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT ID E	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	304	207 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
	128	96 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
	79	33 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
I-B	-417/0	0.0	0.0	0.04 (1)	7.81	B-H	0/192	0.04 (1)	
A-B	0/28	-84.9	-84.9	0.11 (1)	10.00	C-F	-481/0	0.07 (1)	
B-C	-216/0	-84.9	-84.9	0.11 (1)	6.25				
C-D	0/4	-84.9	-84.9	0.27 (1)	10.00				
I-H	0/0	-18.5	-18.5	0.01 (4)	10.00				
H-G	-73/0	0.0	0.0	0.23 (1)	7.81				
G-C	0/134	0.0	0.0	0.25 (1)	10.00				
G-F	0/419	-18.5	-18.5	0.39 (1)	10.00				
F-E	0/0	-18.5	-18.5	0.32 (1)	10.00				

TOTAL WEIGHT = 8 X 21 = 168 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/587$ (0.12")

CSI: TC=0.27/1.00 (C-D:1), BC=0.39/1.00 (F-G:1), WB=0.07/1.00 (C-F:1), SSI=0.19/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
		1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

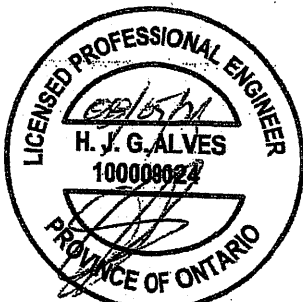
JSI GRIP = 0.36 (C) (INPUT = 0.90)
JSI METAL = 0.15 (G) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMVW-w	MT20	3.0	6.0		
G	BMVW-t	MT20	3.0	4.0		
H	BMVW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

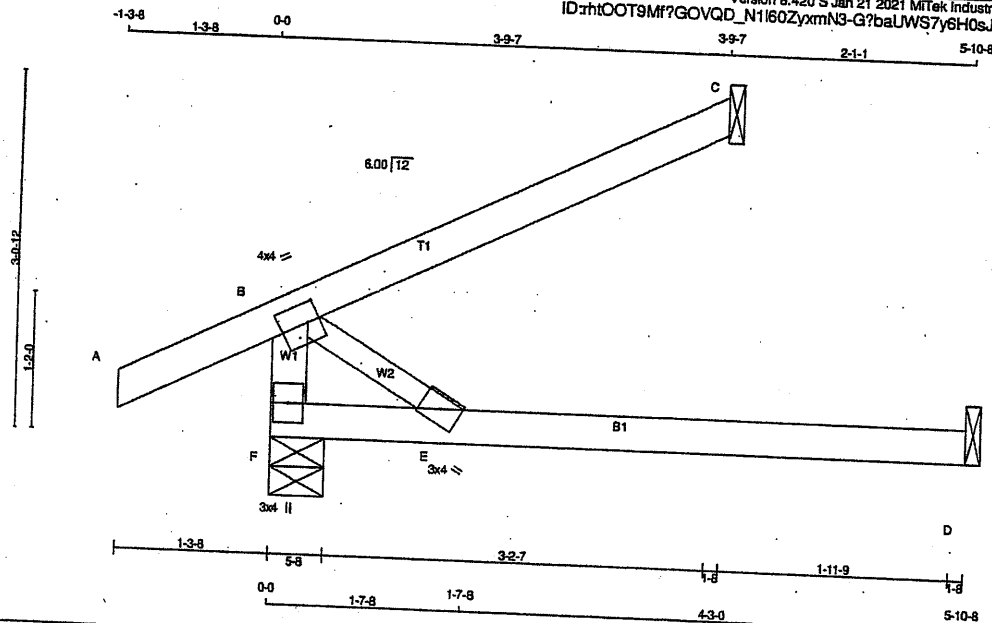
NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2126481

JOB NAME 420375	TRUSS NAME J1C	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						



TOTAL WEIGHT = 2 X 15 = 31 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	No.2	SPF
F - B	2x4	DRY	SPF
A - C	2x4	DRY	SPF
F - D	2x4	DRY	SPF
ALL WEBS	2x3	DRY	SPF
DRY: SEASONED LUMBER.		No.2	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW-t	MT20	4.0	4.0	2.00
E	BMV-w	MT20	3.0	4.0	1.25
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG	BRG	BRG
F	330	330	0	0	0	0	0
C	181	181	0	0	0	0	0
D	54	54	0	0	0	0	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	MAX. MIN. COMPONENT REACTIONS	MAX. MIN. COMPONENT REACTIONS	MAX. MIN. COMPONENT REACTIONS	MAX. MIN. COMPONENT REACTIONS	MAX. MIN. COMPONENT REACTIONS
F	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	234	151 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	111	88 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	MAX. CS1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
F-B	-276 / 0	0.0	0.0	0.03 (1)	7.81	0 / 0	0.00 (1)
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00		
B-C	0 / 0	-84.9	-84.9	0.21 (1)	10.00		
F-E	0 / 0	-18.5	-18.5	0.14 (4)	10.00		
E-D	0 / 0	-18.5	-18.5	0.19 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CS1: TC=0.21/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

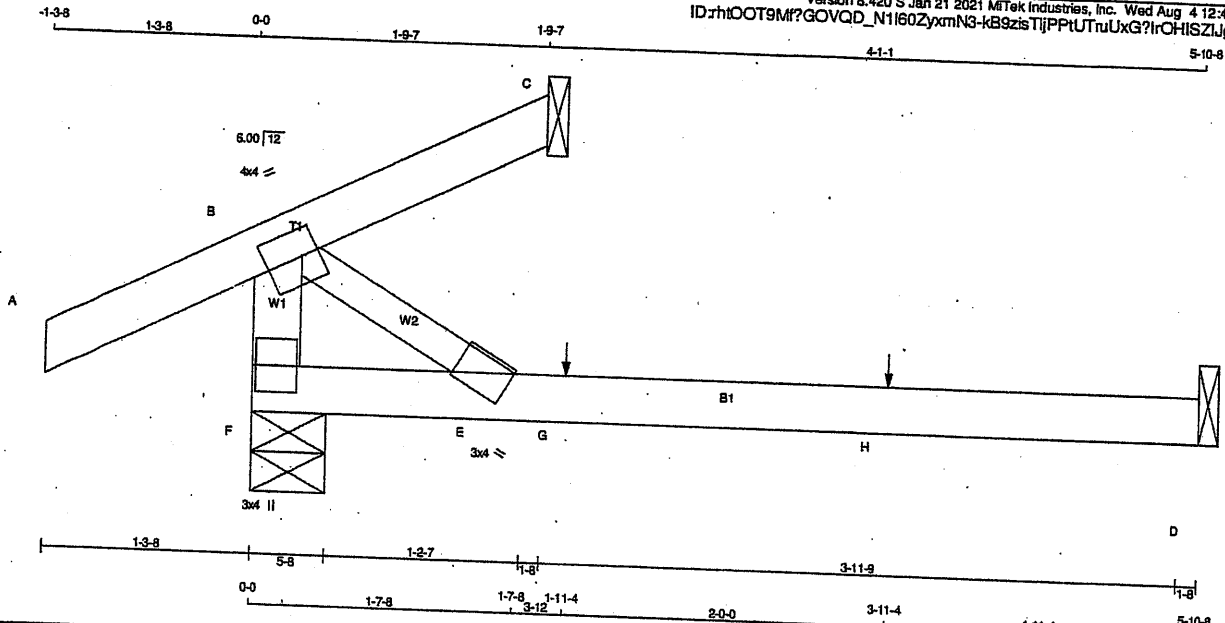
JSI GRIP = 0.16 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126453

JOB NAME 420375	TRUSS NAME J1C2	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
TM	TMVW-t	MT20	4.0	4.0 2.00 1.25
BM	BMV-w	MT20	3.0	4.0
BM	BMV-p	MT20	3.0	4.0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	289	0	289	0	5-8	5-8
C	32	0	32	0	1-8	1-8
D	54	0	61	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

JT	UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	1ST LOASE COMBINED	SNOW	LIVE	PERM.LIVE			
F	205	129/0	0/0	0/0	0/0	77/0	0/0
C	22	18/0	0/0	0/0	0/0	5/0	0/0
D	43	0/0	0/0	0/0	0/0	43/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC LENGTH			
FR-TO							
F-B	-235/0	0.0	0.0	0.02 (1)	7.81	0/0	0.00 (1)
A-B	0/26	-84.9	-84.9	0.12 (1)	10.00		
B-C	-20/0	-84.9	-84.9	0.11 (1)	6.25		
F-E	0/0	-18.5	-18.5	0.14 (4)	10.00		
E-G	0/0	-18.5	-18.5	0.19 (4)	10.00		
G-H	0/0	-18.5	-18.5	0.19 (4)	10.00		
H-D	0/0	-18.5	-18.5	0.19 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX.	MAX+	FACE DIR.
G	1-11-4	1	1	—	FRONT VERT
H	3-11-4	1	1	—	FRONT VERT

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSK TC=0.12/1.00 (A-B:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=0.96 NAIL=0.96 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

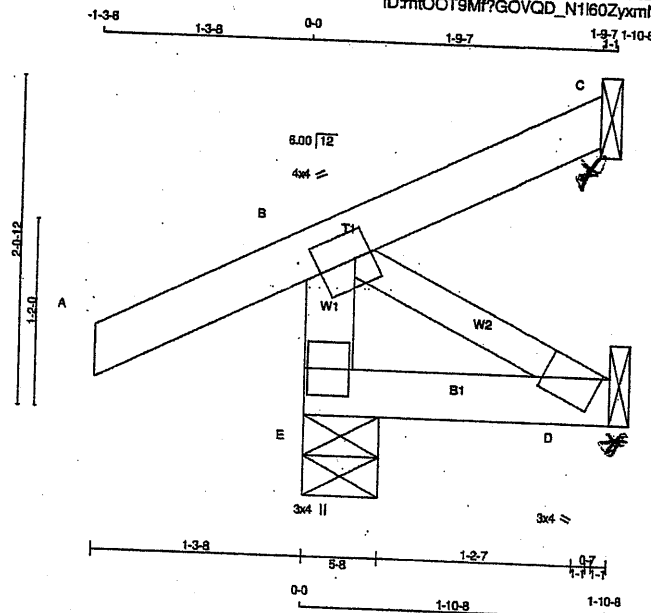
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126454

JOB NAME 420375	TRUSS NAME J1C3	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 12:41:43 2021 Page 1	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMW1-t	MT20	4.0	4.0	2.00	1.25
D BMW1+w	MT20	3.0	4.0	2.00	1.75
E BMW1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	250	0	250	0	5-8	5-8
C	32	0	32	0	1-8	1-8
D	15	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	174	129 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
C	22	18 / -24	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (PLF)	LC1 MAX (CSI (LC))	MEMB. MAX. FORCE (LBS)	MAX. FORCE (LBS)	MAX. FORCE (LBS)	MAX. FORCE (LBS)
FR-TO								
E-B	-235 / 0	0.0	0.0	0.02 (1)	7.81	B-D	0 / 0	0.00 (1)
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00			
B-C	-20 / 0	-84.9	-84.9	0.11 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 8 = 17 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.11/1.00 (A-B:1), BC=0.01/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

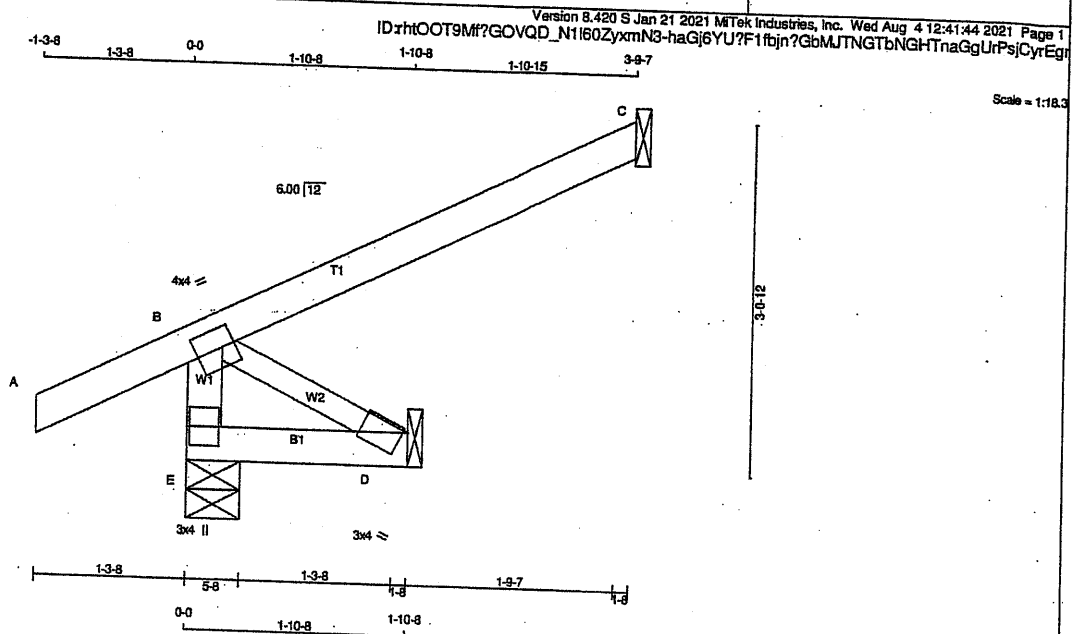
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126455

JOB NAME 420375	TRUSS NAME J1C4	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.	SPF
SPF	
SPF	
SPF	

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	BMV1+w	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES-

- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	291	0	291	0	5-8	5-8
C	161	0	161	0	1-8	1-8
D	15	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	203	151/0	0/0	0/0	0/0	51/0	0/0
C	111	88/0	0/0	0/0	0/0	23/0	0/0
D	12	0/0	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED				WEBS			
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CS1 (LC)	
FR-TO		FROM	TO			FR-TO			
E-B	-276/0	0.0	0.0	0.03 (1)	7.81	B-D	0/0	0.00 (1)	
A-B	0/26	-84.9	-84.9	0.12 (5)	10.00				
B-C	0/0	-84.9	-84.9	0.21 (1)	10.00				
E-D	0/0	-18.5	-18.5	0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 11 = 22 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS1: TC=0.21/1.00 (B-C:1), BC=0.01/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

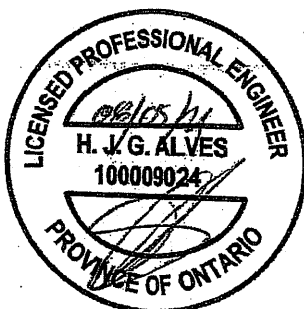
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126456

Scale = 1:12.2

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
JT	243	0	243	0	0	5-8
E	15	0	15	0	-33	1-8
C	0	0	11	0	-7	1-8
D						1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS					
JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	169	128 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
C	10	8 / -22	0 / 0	0 / 0	0 / 0	3 / 0	0 / 0
D	0	0 / -9	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE
	(LBS)	PL		CSI (L/C)	UNBRAC		(LBS)
FR-TO		FROM TO			LENGTH	FR-TO	MAX CSI (L/C)
E-B	-217 / 0	0.0	0.0	0.03 (5)	7.81		
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00		
B-C	-20 / 0	-84.9	-84.9	0.08 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.03 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = $3 \times 6 = 18 \text{ lb}$

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA-086-14
- TPIC 2014

DESIGN ASSUMPTIONS

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL) = $L/360$ (0.19")
 CALCULATED VERT. DEFL(LL) = $L/999$ (0.00")
 ALLOWABLE DEFL(TL) = $L/360$ (0.19")
 CALCULATED VERT. DEFL(TL) = $L/999$ (0.00")

CSI: TC=0.11/1.00 (A-B:1), BC=0.03/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

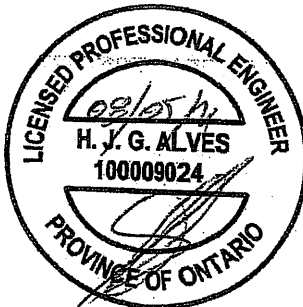
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		SHEAR		SECTION	
PLATE	GRIP(DRY) (PSI)	(PLI)		(PLI)	
	MAX MIN	MAX MIN		MAX MIN	
MT20	650 371	1747 788		1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

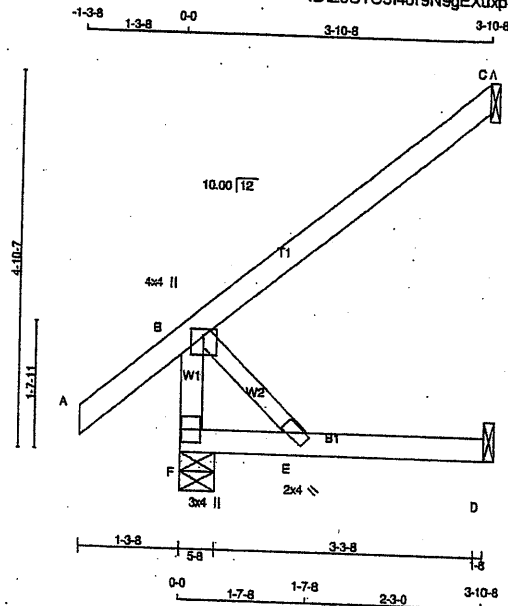
PLATE ROTATION TOL = 5.0 Deg.

USI GRIP= 0.09 (E) (INPUT = 0.90)
USI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126482

JOB NAME 420381	TRUSS NAME J8	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						



Scale = 1/2" = 1'-0"

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS		No.2	SPF
F - B	2x4	DRY	SPF
A - C	2x4	DRY	SPF
F - D	2x4	DRY	SPF
ALL WEBS	2x3	DRY	SPF
DRY: SEASONED LUMBER.		No.2	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
F	367	0	367	0	5-8	5-8
G(C)	123	0	123	0	1-8	1-8
D	27	0	30	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	258	177 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
G(C)	85	88 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	22	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, G(C)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)
FR-TO						
F-B	-344 / 0	0.0	0.0	0.08 (4)	7.81	0.01 (4)
A-B	0 / 38	-84.9	-84.9	0.14 (5)	10.00	
B-C	-26 / 0	-84.9	-84.9	0.22 (1)	6.25	
F-E	-32 / 0	-18.5	-18.5	0.03 (4)	6.25	
E-D	0 / 0	-18.5	-18.5	0.04 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 15 = 45 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.22/1.00 (B-C:1), BC=0.04/1.00 (D-E:4), WB=0.01/1.00 (B-E:4), SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)

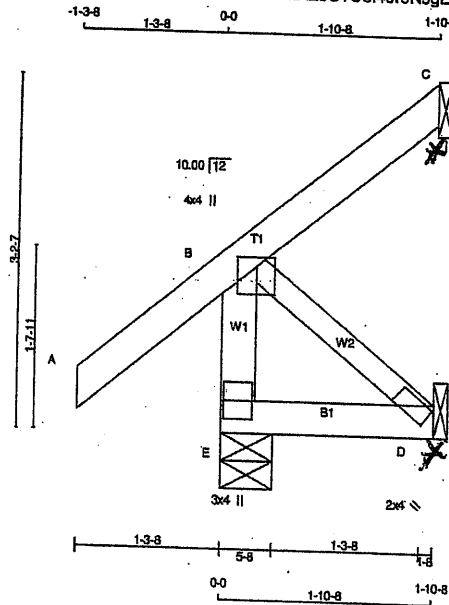


Structural component only
DWG# T-2126520

JOB NAME 420381	TRUSS NAME J9	QUANTITY 11	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 15:19:28 2021 Page 1
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Scale = 1:19.1



LUMBER	N. L. G. A. RULES
CHORDS	SIZE
E - B	2x4 DRY No.2
A - C	2x4 DRY No.2
E - D	2x4 DRY No.2
ALL WEBS	2x3 DRY No.2
DRY: SEASONED LUMBER.	

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+w	MT20	2.0	4.0	3.25	
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
E	256	0	256	0	5-8
C	36	0	36	0	5-8
D	16	0	16	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	179	132 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	25	20 / -24	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1	MAX. CSI (LC)	UNBRAC LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO							FR-TO			
E-B	-241 / 0	0.0	0.0	0.03 (1)	7.81		B-D	0 / 0	0.00 (1)	
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00					
B-C	-28 / 0	-84.9	-84.9	0.11 (1)	6.25					
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00					

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 11 X 10 = 111.1 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (B-D:1), SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

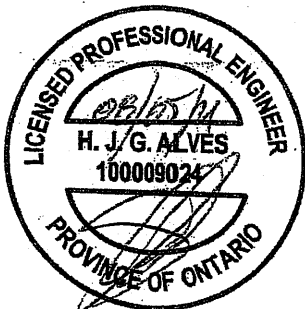
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

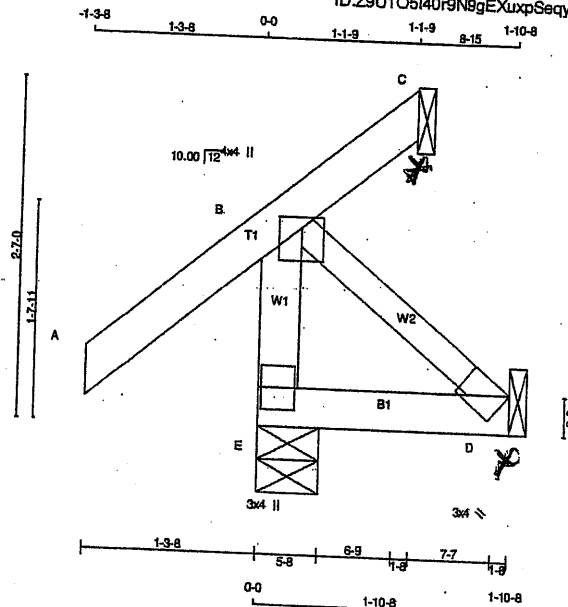
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126521



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+w	MT20	3.0	4.0	3.00	Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	253	0	253	0	5-8	5-8
C	-24	0	0	-66	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D
 PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	177	130/0	0/0	0/0	0/0	48/0	0/0
C	-16	0/-39	0/0	0/0	0/0	0/-3	0/0
D	13	0/0	0/0	0/0	0/0	13/0	0/0

BEARING MATERIAL TO BE SPREAD OVER

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM	TO	UNBRAC	FR-TO				
E-B	-237 / 0	0.0	0.0	0.03 (1)	B-D	0 / 0	0.00 (1)		
A-B	0 / 38	-84.9	-84.9	0.12 (1)					
B-C	-46 / 0	-84.9	-84.9	0.11 (1)					
E-D	0 / 0	-18.5	-18.5	0.02 (4)					
				10.00					
				8.25					

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 9 = 18 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 5.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SS=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.18 (B) (INPUT = 0.90)
 JSI METAL = 0.05 (B) (INPUT = 1.00)



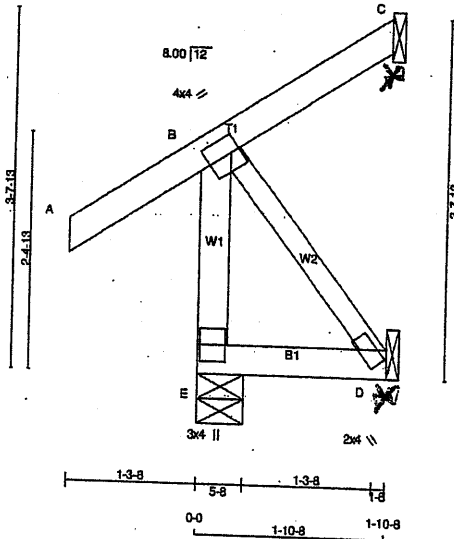
Structural component only
 DWG# T-2126522

JOB NAME 420381	TRUSS NAME J10	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed Aug 4 15:19:30 2021 Page 1
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-1-3-8 1-3-8 0-0 1-10-8 1-10-8

Scale = 1:21.4



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.00
D BMW1-w	MT20	2.0	4.0	2.75	0.25
E BMV1-p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
E	255	0	255	0
C	37	0	37	0
D	18	0	18	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
E	178	131/0	0/0	0/0	0/0	47/0	0/0
C	25	20/-23	0/0	0/0	0/0	5/0	0/0
D	13	0/0	0/0	0/0	0/0	13/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO			
E-B	-239/0	0.0	0.0	0.03 (1)	7.81	B-D	0/0	0.00 (1)	
A-B	0/32	-84.9	-84.9	0.11 (1)	10.00				
B-C	-24/0	-84.9	-84.9	0.11 (1)	6.25				
E-D	0/0	-18.5	-18.5	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 11 = 33 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")

CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.11/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.15 (B) (INPUT = 0.90)

JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126523



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering and General Safety notes.

T-1900218

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	D. (in.)	L. (in.)	LUMBER		BEARING PLATE	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER					
MAXIMUM NUMBER OF TOE-NAILS					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

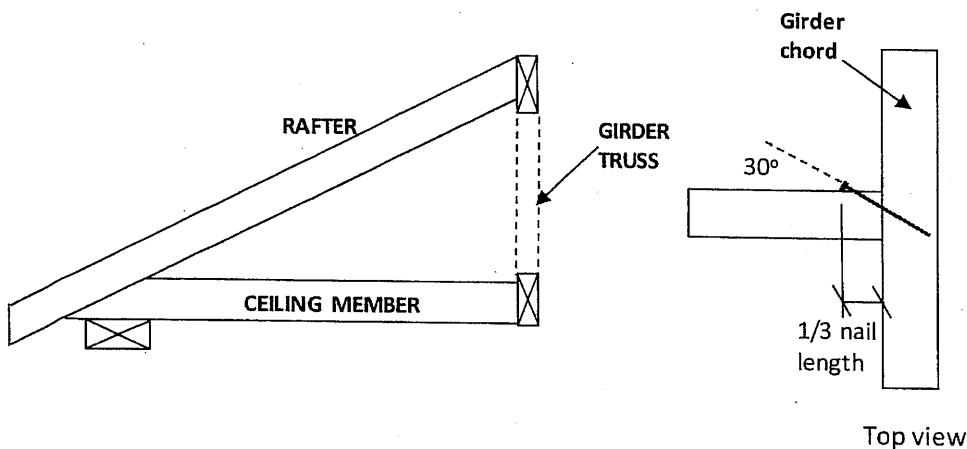


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

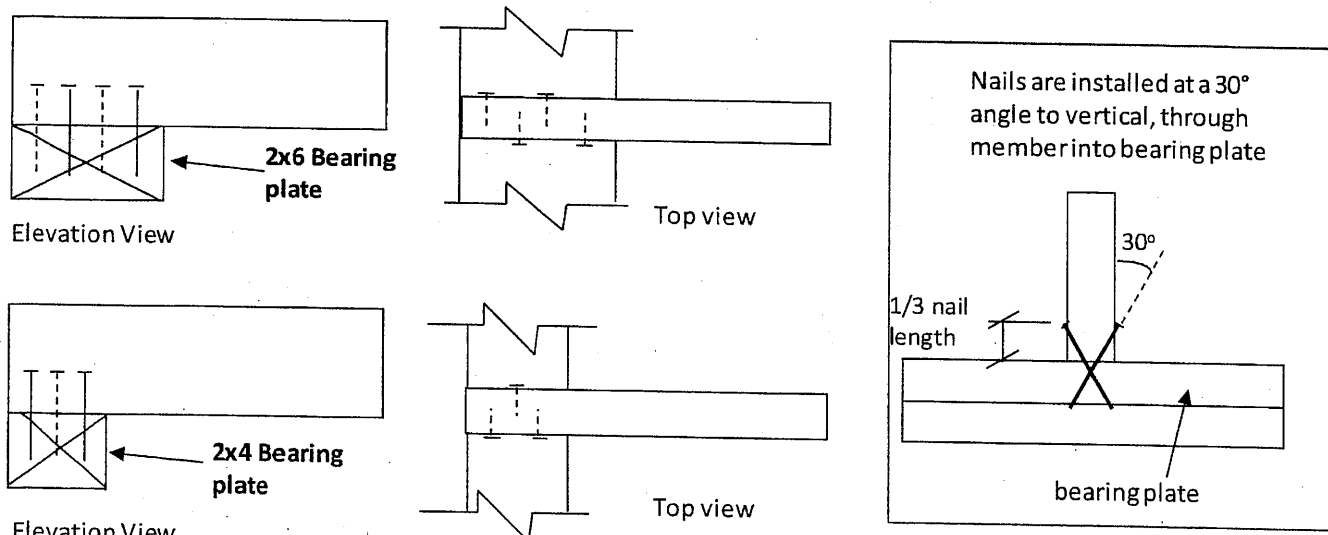
PEO
Certificate No. 10889485



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS**SIMPSON**
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

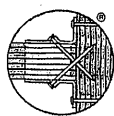
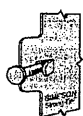
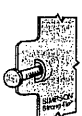
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

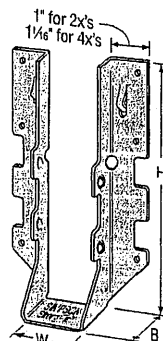
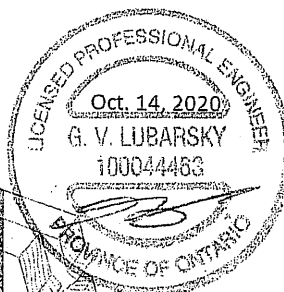
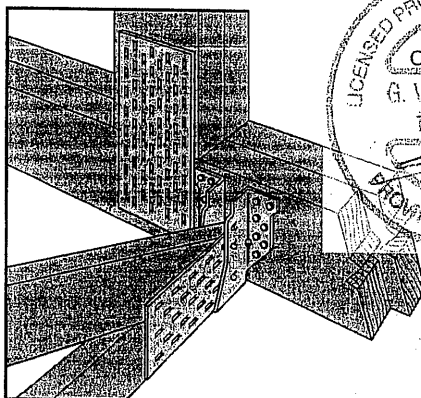
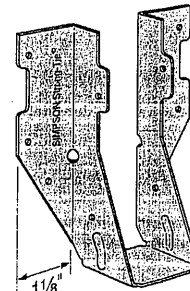
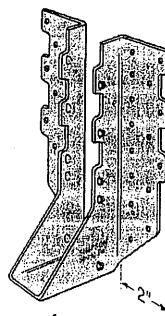
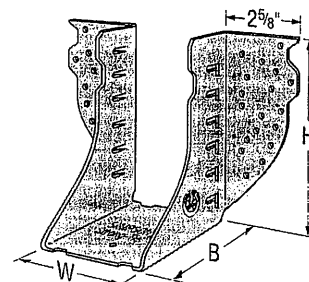
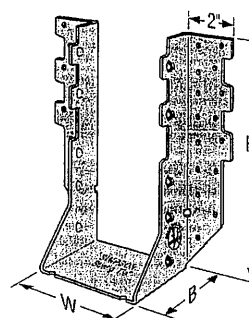
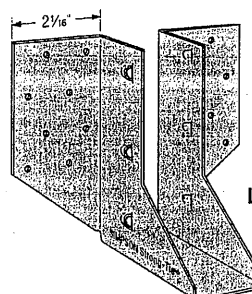
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.

Double-Shear
Nailing
Top ViewDouble-Shear
Nailing
Side View;
Do not
bend tabDome Double-Shear
Nailing
Side View
(available on
some models)

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)

 **LUS28****LU26L** **HUS210**
(HUS26, HUS28,
and HHUS similar) **HGUS28-2** **HHUS210-2****LJS26DS**

TECHNICAL BULLETIN

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

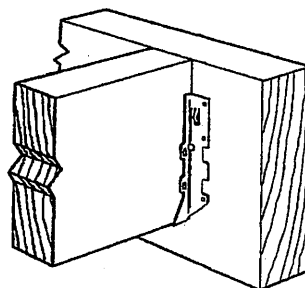
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

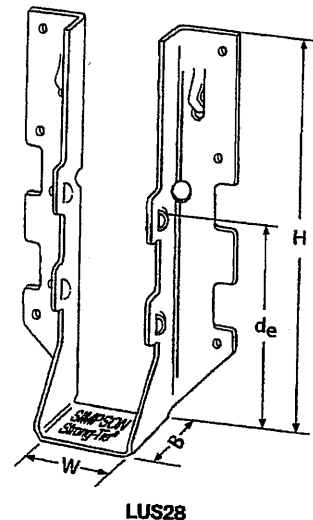
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

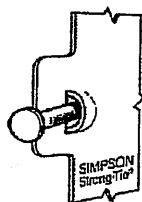


Typical LUS
Installation



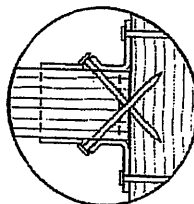
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1½	3½	1½	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¾	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6½	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¾	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 15/16	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

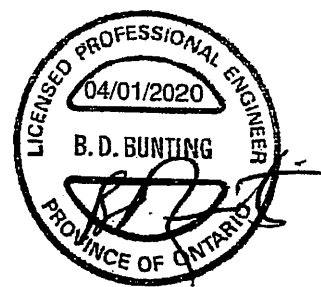


Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

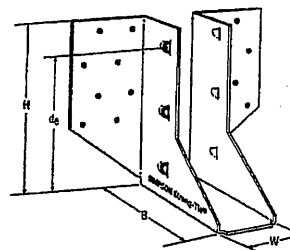
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

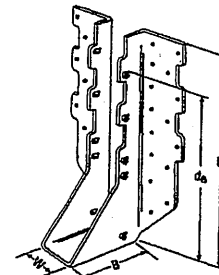
- Use all specified fasteners
- Nails: 16d = 0.182" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

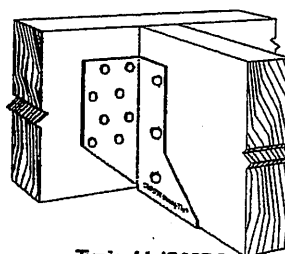
- See current catalogue for options



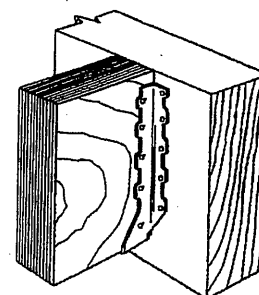
LJS26DS



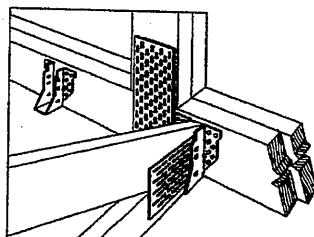
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



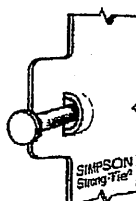
Typical HUS
Installation



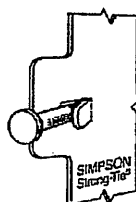
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 7/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

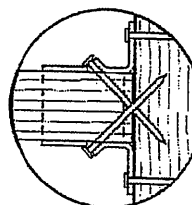
1. d_e is the distance from the seat of the hanger to the highest joist nail.



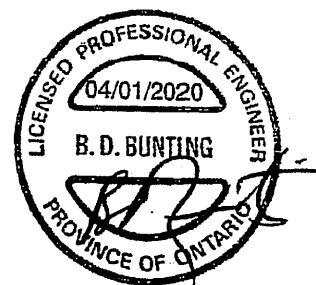
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

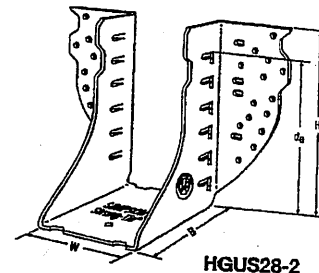
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

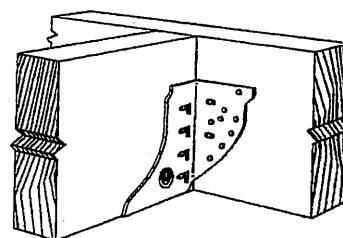
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

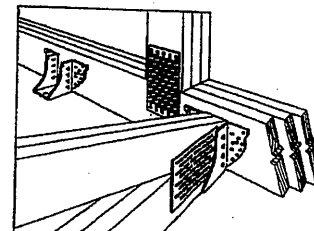
- See current catalogue for options



HGUS28-2



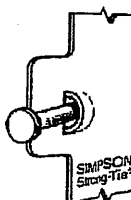
Typical HGUS
Installation



Typical HGUS Installation
(Truss Designer to
provide fastener quantity
for connecting multiple
members together)

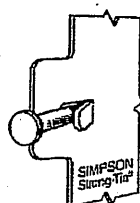
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _u =1.15)	(K _u =1.00)	(K _u =1.15)	(K _u =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_s is the distance from the seat of the hanger to the highest joist nail.

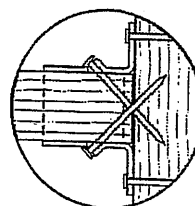


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

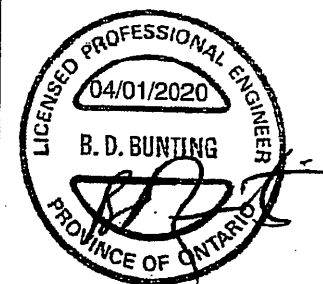
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

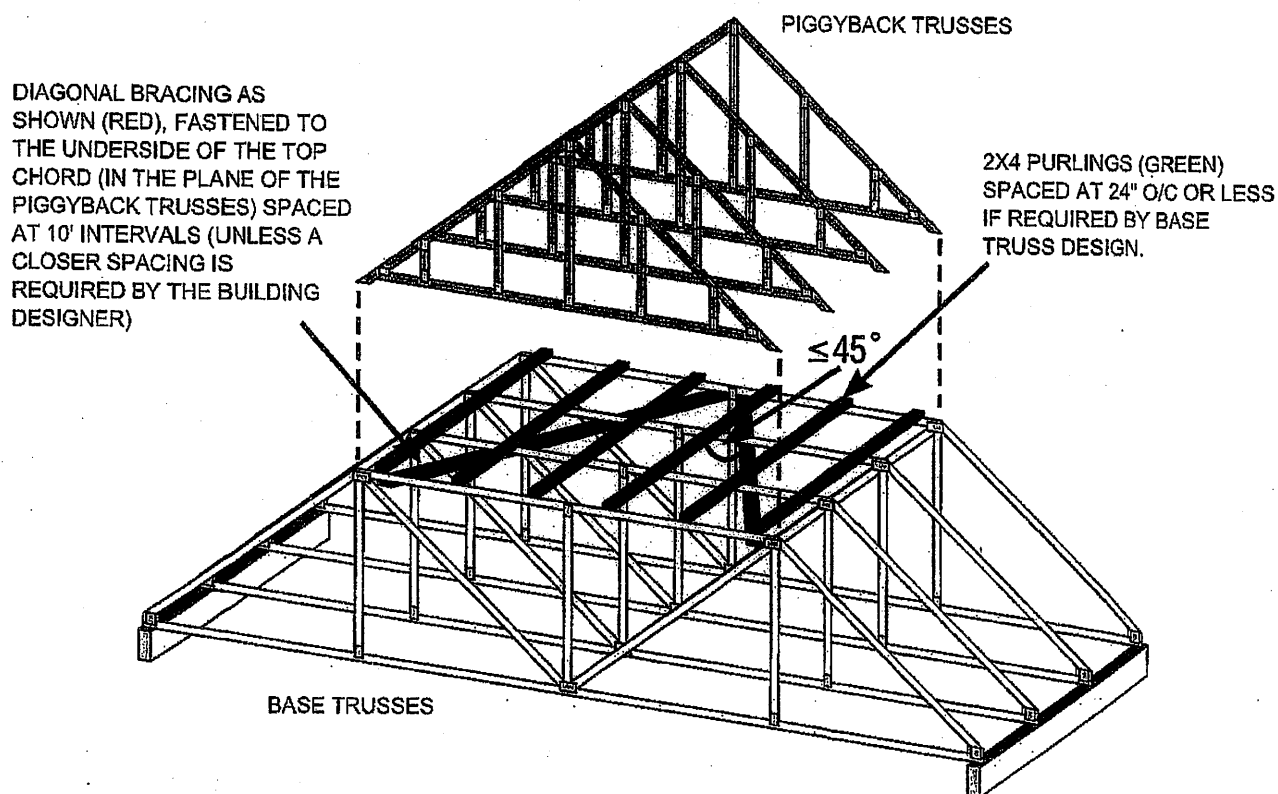
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

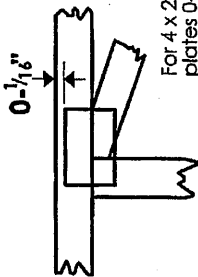
Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x-y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in Mitek software or upon request.

PLATE SIZE

4 x 4
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

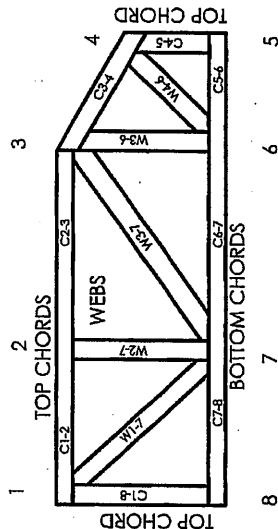
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek
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Mitek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
- Design assumes trusses will be suitably protected from the environment in accord with TPIC.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with TPIC Quality Criteria.