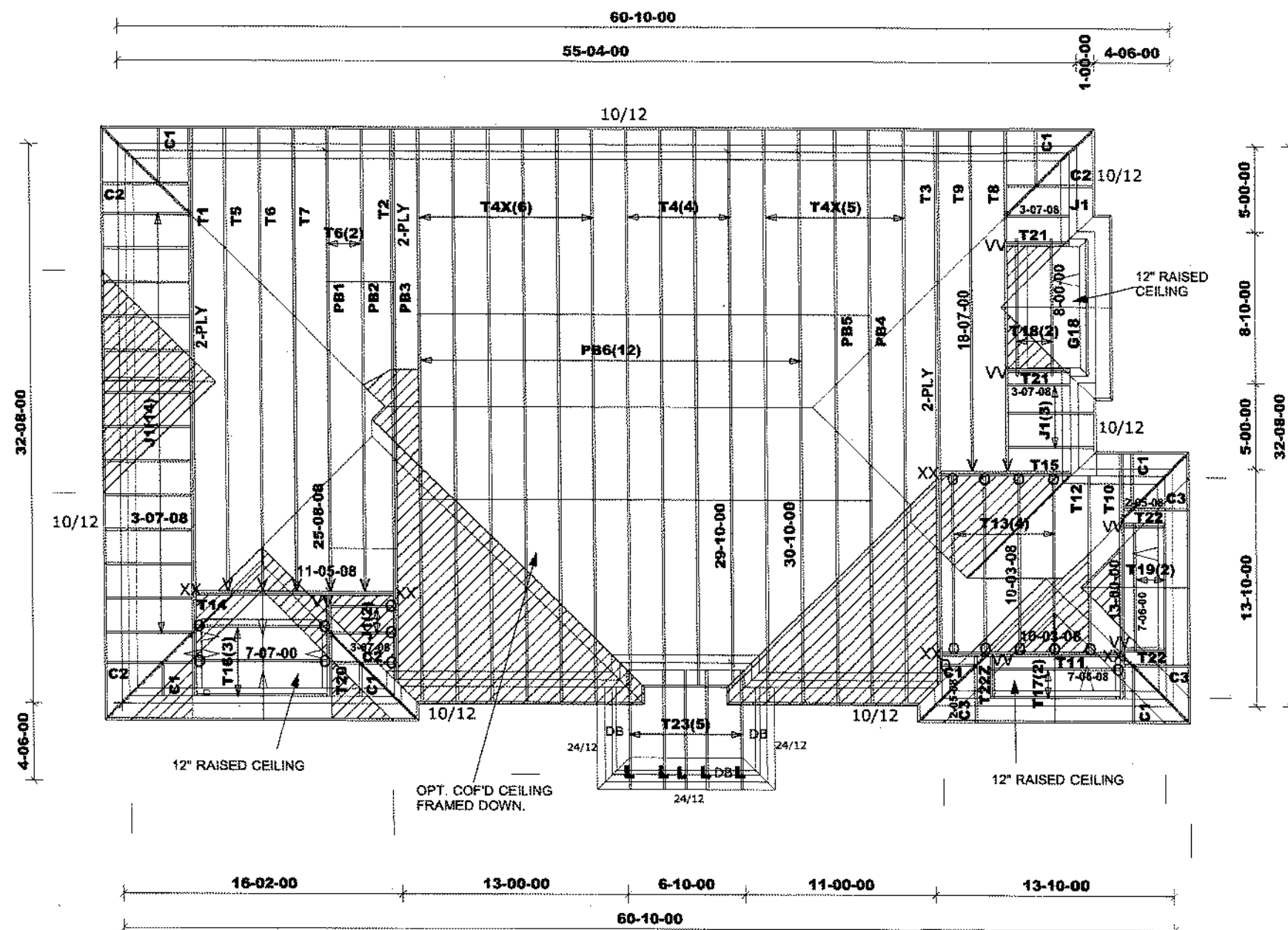




ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

10/12 ROOF PITCHES UNLESS
OTHERWISE NOTED

All conventional framing to conform with
Part 9 of O.B.C. 2012. Roof rafters that
cross over or meet trusses to be min. 2x4
SPF #2 @ 24" o/c with a vertical post to the
truss at each cross point. Vertical posts
longer than 6' to have lateral bracing so that
the distance between the post end points
and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC
2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

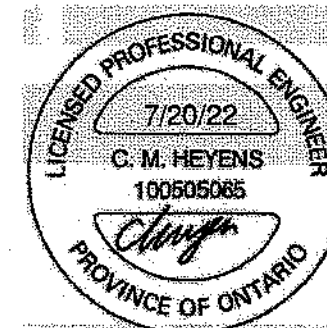
HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (L)

DENOTES:
CONVENTIONAL
FRAMING

2-PLY - T10, T11, T14, T20, T21,
T22, T22Z.

DB - DROPPED BEAMS BY OTHERS

DWG# TR22070549 TO TR22070584



STRUCTURAL COMPONENTS ONLY
DWG# TR22070694

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

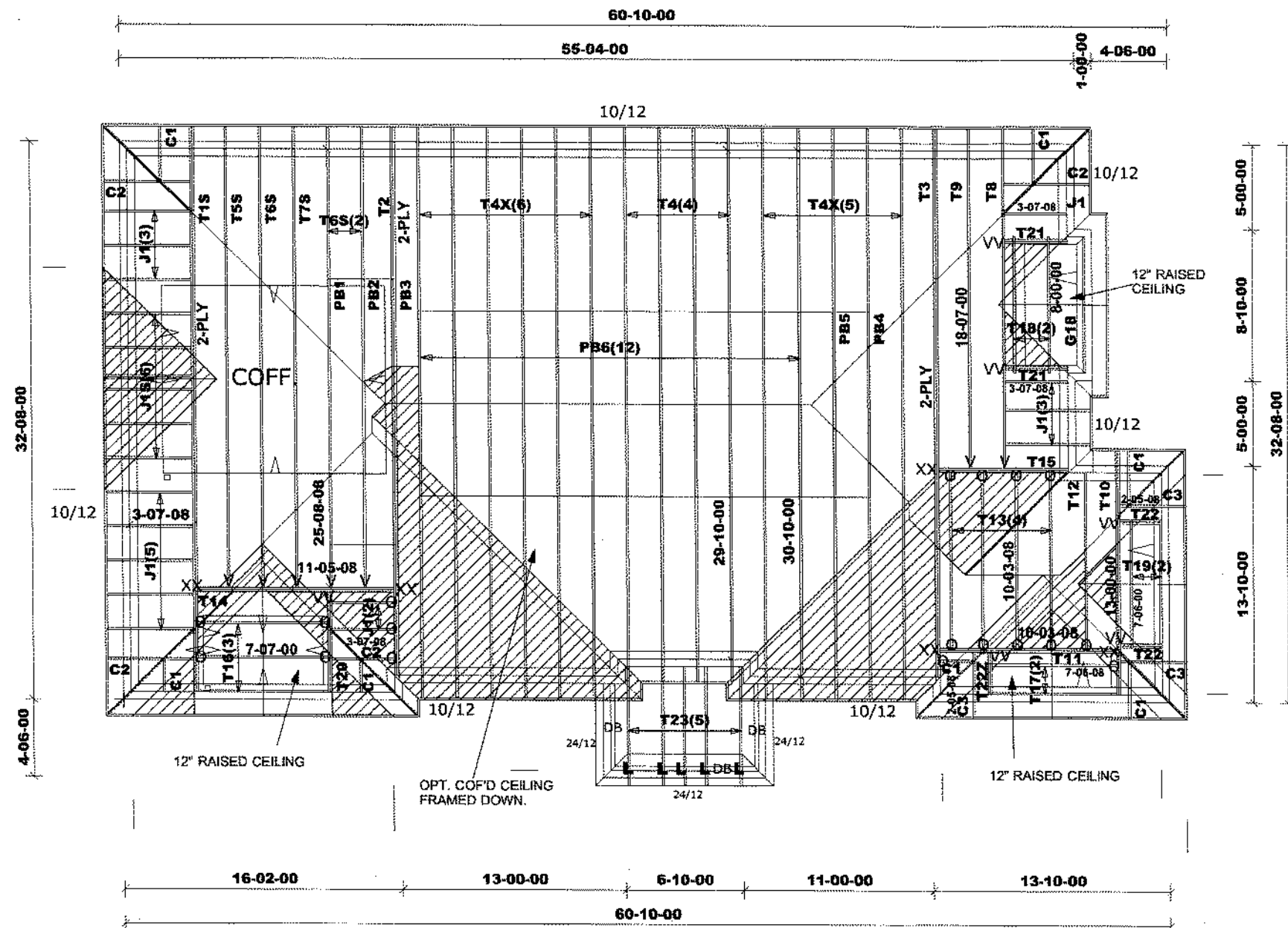
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



Job Track: **52647**
Layout ID: **425136**
Plan Log: **205911**

Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **FORESTSIDE ESTATES**
Date: 7/20/22 Designer:

Model / Elevation: **UNIT 4502 - COR / A STD (4 OR 5 BED)**
Mitek ver 6.5.3.233
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

10/12 ROOF PITCHES UNLESS
OTHERWISE NOTED

All conventional framing to conform with
Part 9 of O.B.C. 2012. Roof rafters that
cross over or meet trusses to be min. 2x4
SPF #2 @ 24" o/c with a vertical post to the
truss at each cross point. Vertical posts
longer than 6' to have lateral bracing so that
the distance between the post end points
and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC
2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

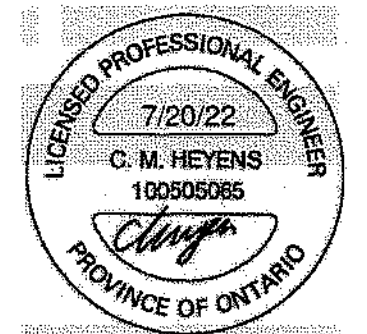
HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (L)

DENOTES:
CONVENTIONAL
FRAMING

2-PLY - T10, T11, T14, T20, T21,
T22, T22Z.

DB - DROPPED BEAMS BY OTHERS

DWG# TR22070553 TO TR22070589



STRUCTURAL COMPONENTS ONLY
DWG# TR22070695

TRUSS PLACEMENT PLAN.

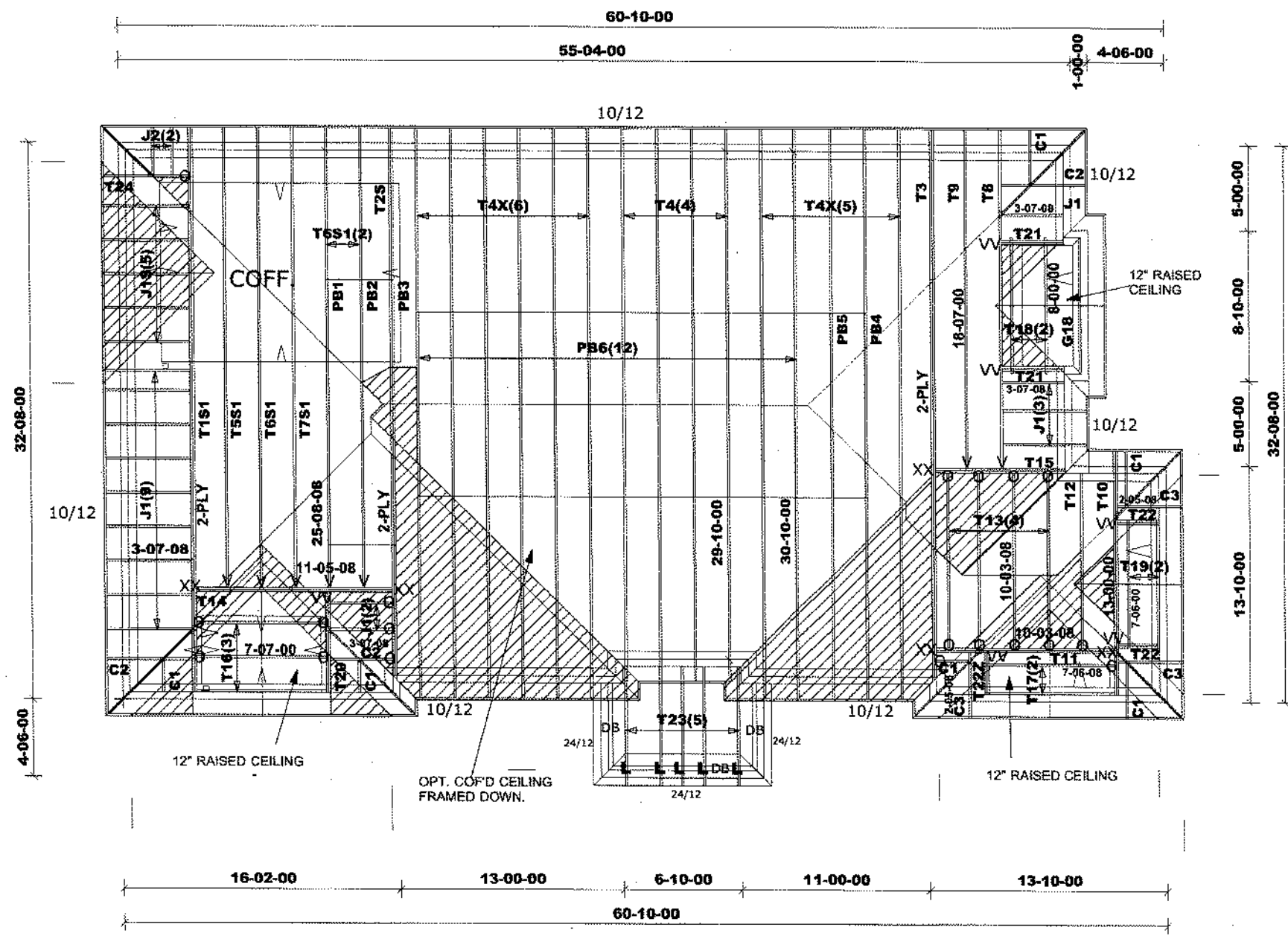
This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



Job Track: **52647**
Layout ID: **425137**
Plan Log: **205911**
Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **FORESTSIDE ESTATES**
Date: 7/20/22 Designer:

Model / Elevation: **UNIT 4502 - COR / A 4 BED OPT RAISED CEILING**
Mitek ver 8.5.3.233
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE



ASPHALT SHINGLES
 FINISHED OVERHANG: 12"
 2x6 EXTERIOR WALLS
 2x6 FASCIA BOARD
 HEEL: R.T.M.C.

10/12 ROOF PITCHES UNLESS
 OTHERWISE NOTED

All conventional framing to conform with
 Part 9 of O.B.C. 2012. Roof rafters that
 cross over or meet trusses to be min. 2x4
 SPF #2 @ 24" o/c with a vertical post to the
 truss at each cross point. Vertical posts
 longer than 6' to have lateral bracing so that
 the distance between the post end points
 and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC
 2012(2019 AMENDMENT)
 OCCUPANCY: RESIDENTIAL | PART: 9
 Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
 TCSL = 23.3 psf
 TCDL = 6.0 psf
 BCLL = 0.0 psf
 BCDL = 7.4 psf

HARDWARE:
 LUS24 - (O)
 LJS26DS - (V)
 LUS26-2 - (VV)
 HGUS26-2 - (XX)
 H2.5T - (L)

DENOTES:
 CONVENTIONAL
 FRAMING

2-PLY - T10, T11, T14, T20, T21,
 T22, T22Z.

DB - DROPPED BEAMS BY OTHERS

DWG# TR22070554 TO TR22070585
 DWG# TR22070590 TO TR22070596

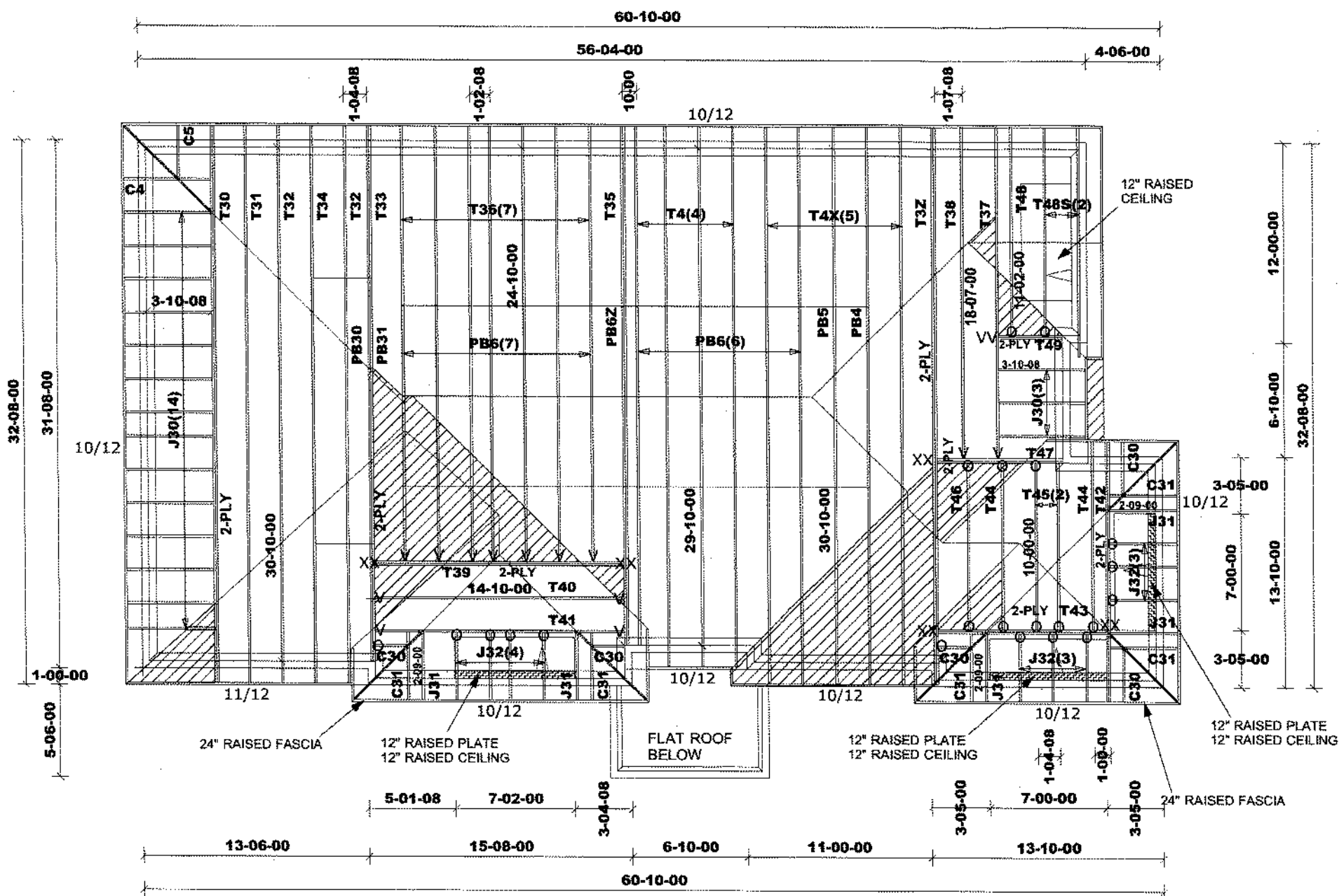
STRUCTURAL COMPONENTS ONLY
 DWG# TR22070696

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.

	Job Track: 52647	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4502 - COR / A 5 BED OPT RAISED CEILING
	Layout ID: 425138	Project: FORESTSIDE ESTATES	Mitek ver 8.5.3.293
	Plan Log: 205911	Date: 7/20/22	Designer:
			THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

10/12 ROOF PITCHES UNLESS
OTHERWISE NOTED

All conventional framing to conform with
Part 9 of O.B.C. 2012. Roof rafters that
cross over or meet trusses to be min. 2x4
SPF #2 @ 24" o/c with a vertical post to the
truss at each cross point. Vertical posts
longer than 6' to have lateral bracing so that
the distance between the post end points
and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC
2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

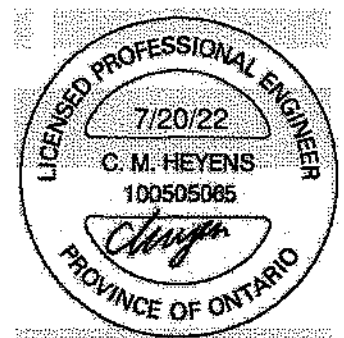
DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (L)

DENOTES:
CONVENTIONAL
FRAMING

DB - DROPPED BEAMS BY OTHERS

DWG# TR22070554 TO TR22070558
DWG# TR22070597 TO TR22070628



STRUCTURAL COMPONENTS ONLY
DWG# TR22070697

TRUSS PLACEMENT PLAN.

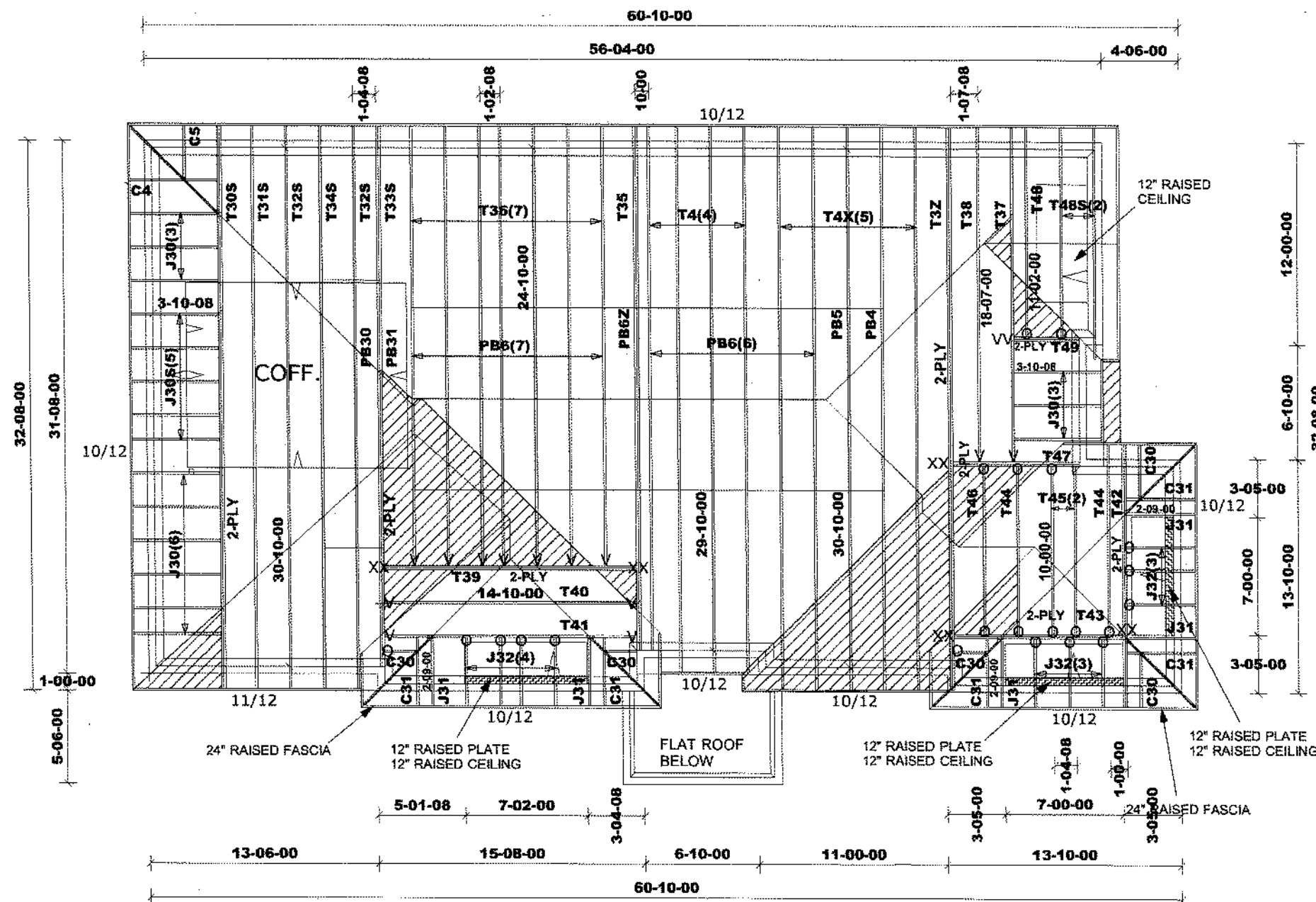
This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



Job Track: 52647	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4502 - COR / B STD (4 OR 5 BED)
Layout ID: 425139	Project: FORESTSIDE ESTATES	These drawings constitute the property of TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
Plan Log: 205911	Date: 7/20/22	Designer:

Mitek ver 6.5.3.233



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

10/12 ROOF PITCHES UNLESS
OTHERWISE NOTED

All conventional framing to conform with
Part 9 of O.B.C. 2012. Roof rafters that
cross over or meet trusses to be min. 2x4
SPF #2 @ 24" o/c with a vertical post to the
truss at each cross point. Vertical posts
longer than 6' to have lateral bracing so that
the distance between the post end points
and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC
2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

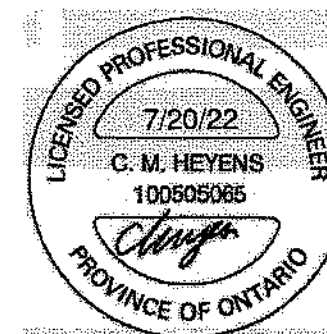
DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (L)

 **DENOTES:**
CONVENTIONAL
FRAMING

DB - DROPPED BEAMS BY OTHERS

DWG# TR22070554 TO TR22070558
DWG# TR22070602 TO TR22070634



STRUCTURAL COMPONENTS ONLY
DWG# TR22070698

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



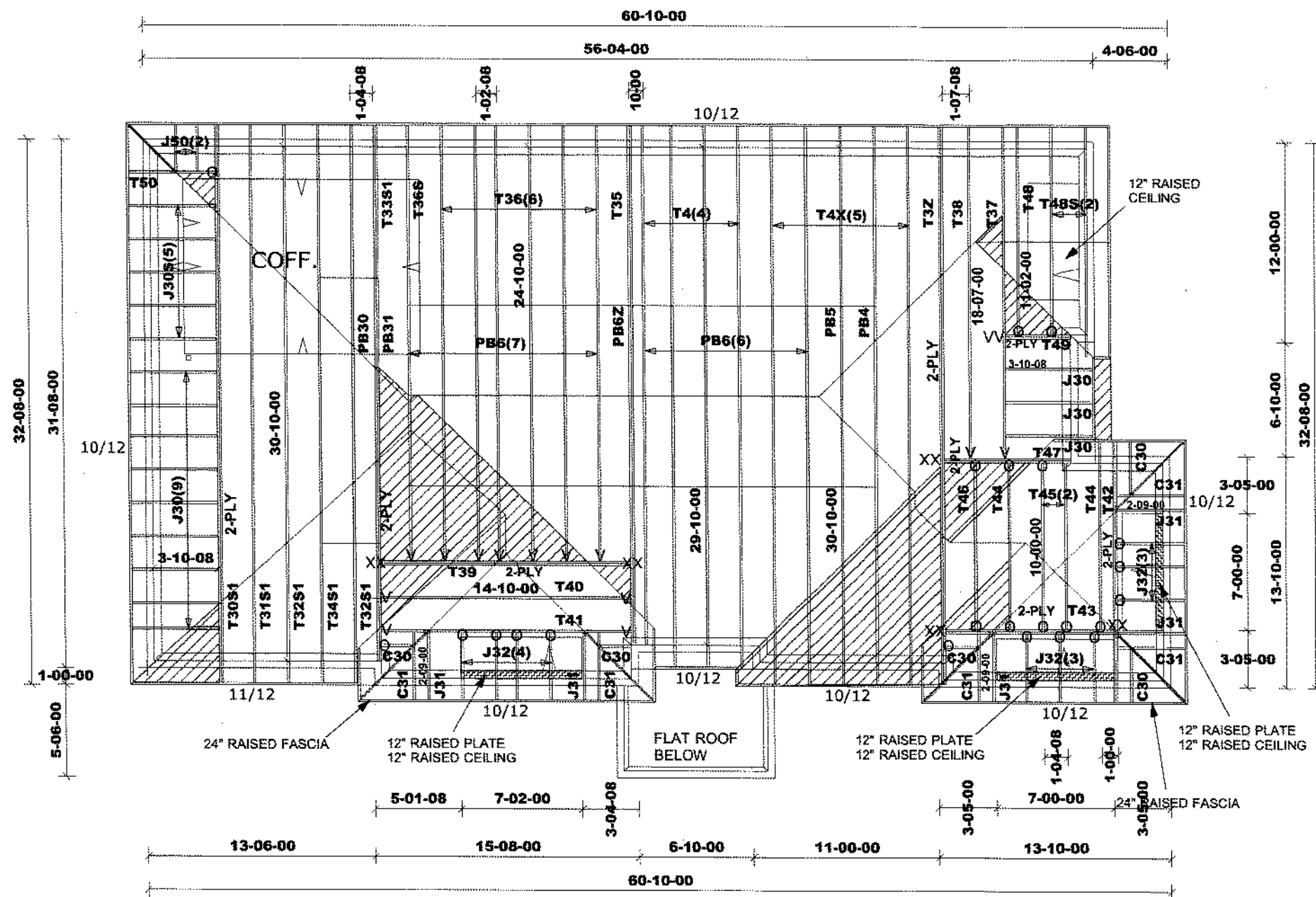
Job Track: **52647**
Layout ID: **425140**
Plan Log: **205911**

Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **FORESTSIDE ESTATES**

Date: 7/20/22

Designer:

Model / Elevation: **UNIT 4502 - COR / B 4 BED OPT RAISED CEILING**
Mitek ver 6.5.3.233
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

10/12 ROOF PITCHES UNLESS
OTHERWISE NOTED

All conventional framing to conform with
Part 9 of O.B.C. 2012. Roof rafters that
cross over or meet trusses to be min. 2x4
SPF #2 @ 24" o/c with a vertical post to the
truss at each cross point. Vertical posts
longer than 6' to have lateral bracing so that
the distance between the post end points
and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC
2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

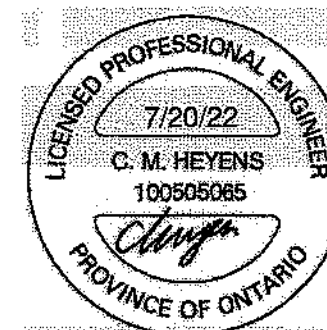
DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (L)

 DENOTES:
CONVENTIONAL
FRAMING

DB - DROPPED BEAMS BY OTHERS

DWG# TR22070554 TO TR22070558
DWG# TR22070602 TO TR22070629
DWG# TR22070635 TO TR22070642



STRUCTURAL COMPONENTS ONLY
DWG# TR22070699

TRUSS PLACEMENT PLAN.

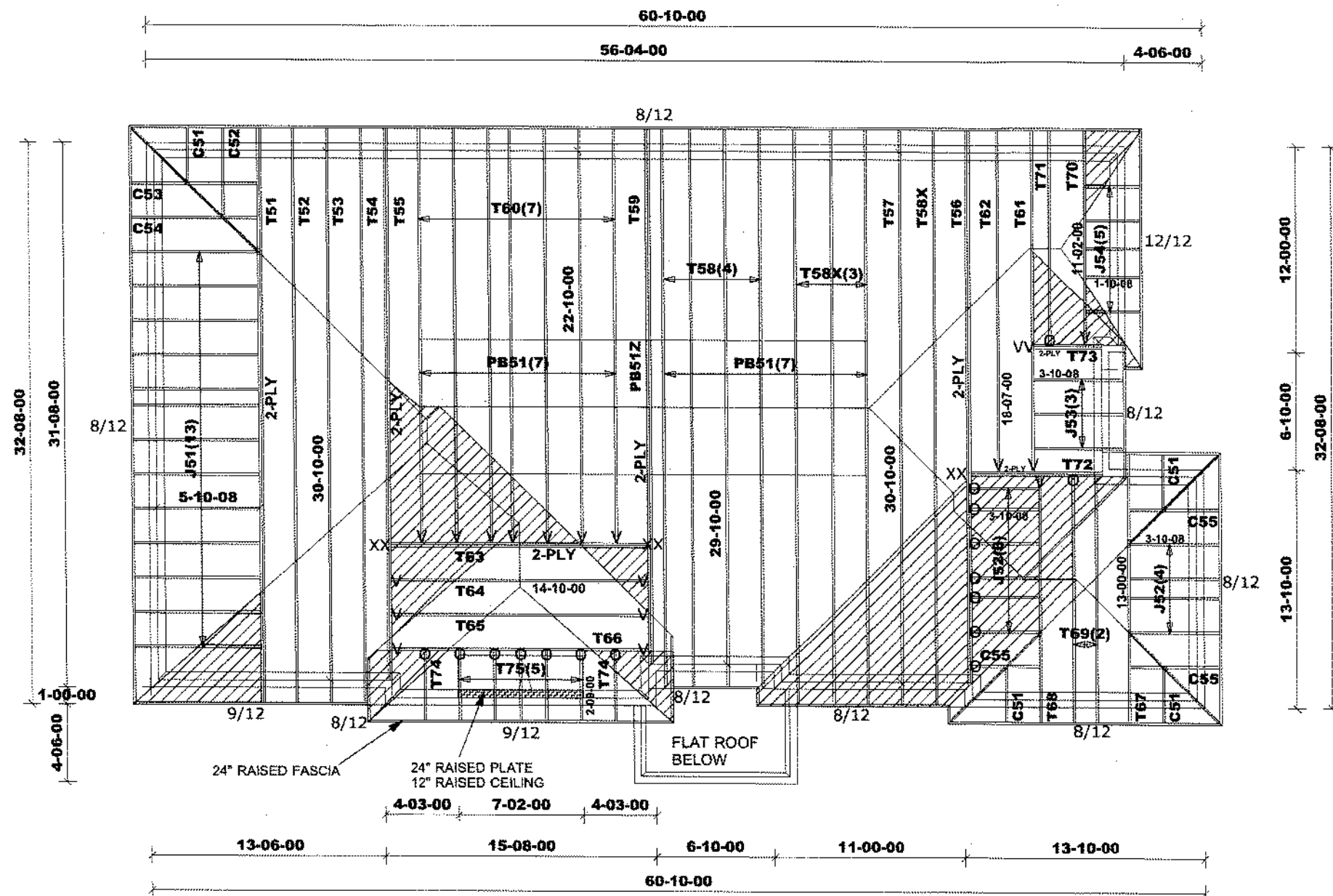
This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



Job Track: **52647**
Layout ID: **425141**
Plan Log: **205911**
Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **FORESTSIDE ESTATES**
Date: 7/20/22
Designer:

Model / Elevation: **UNIT 4502 - COR / B 5 BED OPT RAISED CEILING**
Mitek ver 6.5.3.233
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
 FINISHED OVERHANG: 12"
 2x6 EXTERIOR WALLS
 2x6 FASCIA BOARD
 HEEL: R.T.M.C.

8/12 ROOF PITCHES UNLESS
 OTHERWISE NOTED

All conventional framing to conform with
 Part 9 of O.B.C. 2012. Roof rafters that
 cross over or meet trusses to be min. 2x4
 SPF #2 @ 24" o/c with a vertical post to the
 truss at each cross point. Vertical posts
 longer than 6' to have lateral bracing so that
 the distance between the post end points
 and lateral bracing does not exceed 6'.

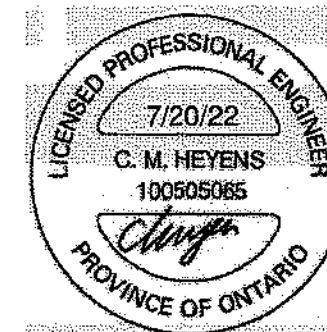
DESIGN CONFORMS WITH OBC
 2012(2019 AMENDMENT)
 OCCUPANCY: RESIDENTIAL | PART: 9
 Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
 TCSL = 23.3 psf
 TCDL = 6.0 psf
 BCLL = 0.0 psf
 BCDL = 7.4 psf

HARDWARE:
 LUS24 - (O)
 LJS26DS - (V)
 LUS26-2 - (VV)
 HGUS26-2 - (XX)
 H2.5T - (L)

 DENOTES:
 CONVENTIONAL
 FRAMING

DWG# TR22070643 TO TR22070679



STRUCTURAL COMPONENTS ONLY
 DWG# TR22070700

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

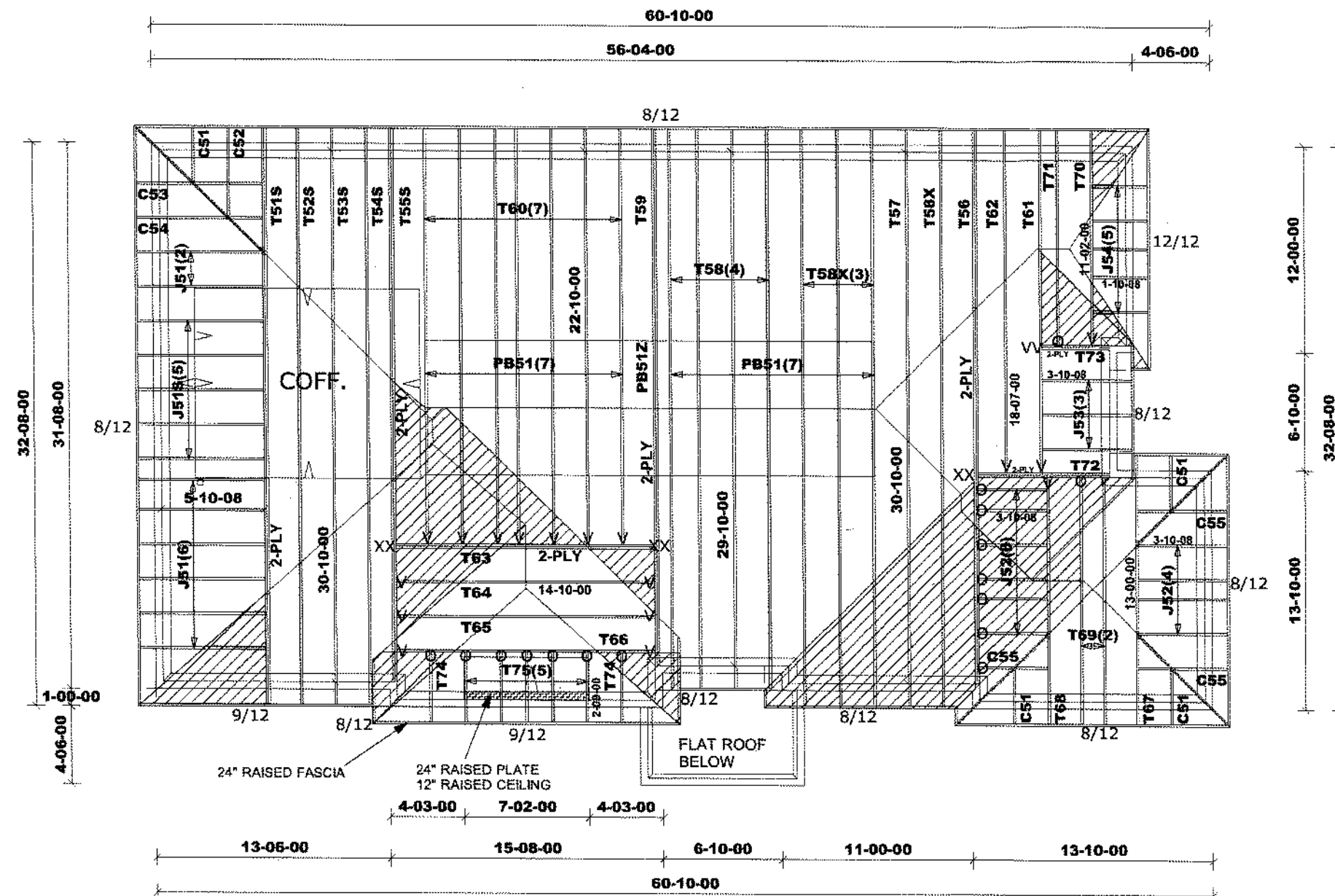
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



Job Track: **52647**
 Layout ID: **425142**
 Plan Log: **205911**
 Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
 Project: **FORESTSIDE ESTATES**
 Date: 7/20/22 Designer:

Model / Elevation: **UNIT 4502 - COR / C STD (4 OR 5 BED)**
 THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 6.5.3.233



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

8/12 ROOF PITCHES UNLESS
OTHERWISE NOTED

All conventional framing to conform with
Part 9 of O.B.C. 2012. Roof rafters that
cross over or meet trusses to be min. 2x4
SPF #2 @ 24" o/c with a vertical post to the
truss at each cross point. Vertical posts
longer than 6' to have lateral bracing so that
the distance between the post end points
and lateral bracing does not exceed 6'.

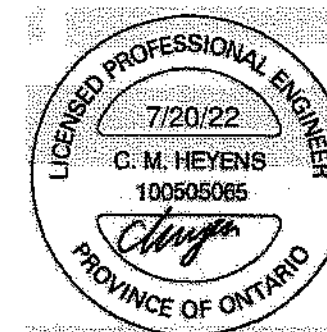
DESIGN CONFORMS WITH OBC
2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LUS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (L)

 DENOTES:
CONVENTIONAL
FRAMING

DWG# TR22070648 TO TR22070685



STRUCTURAL COMPONENTS ONLY
DWG# TR22070701

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

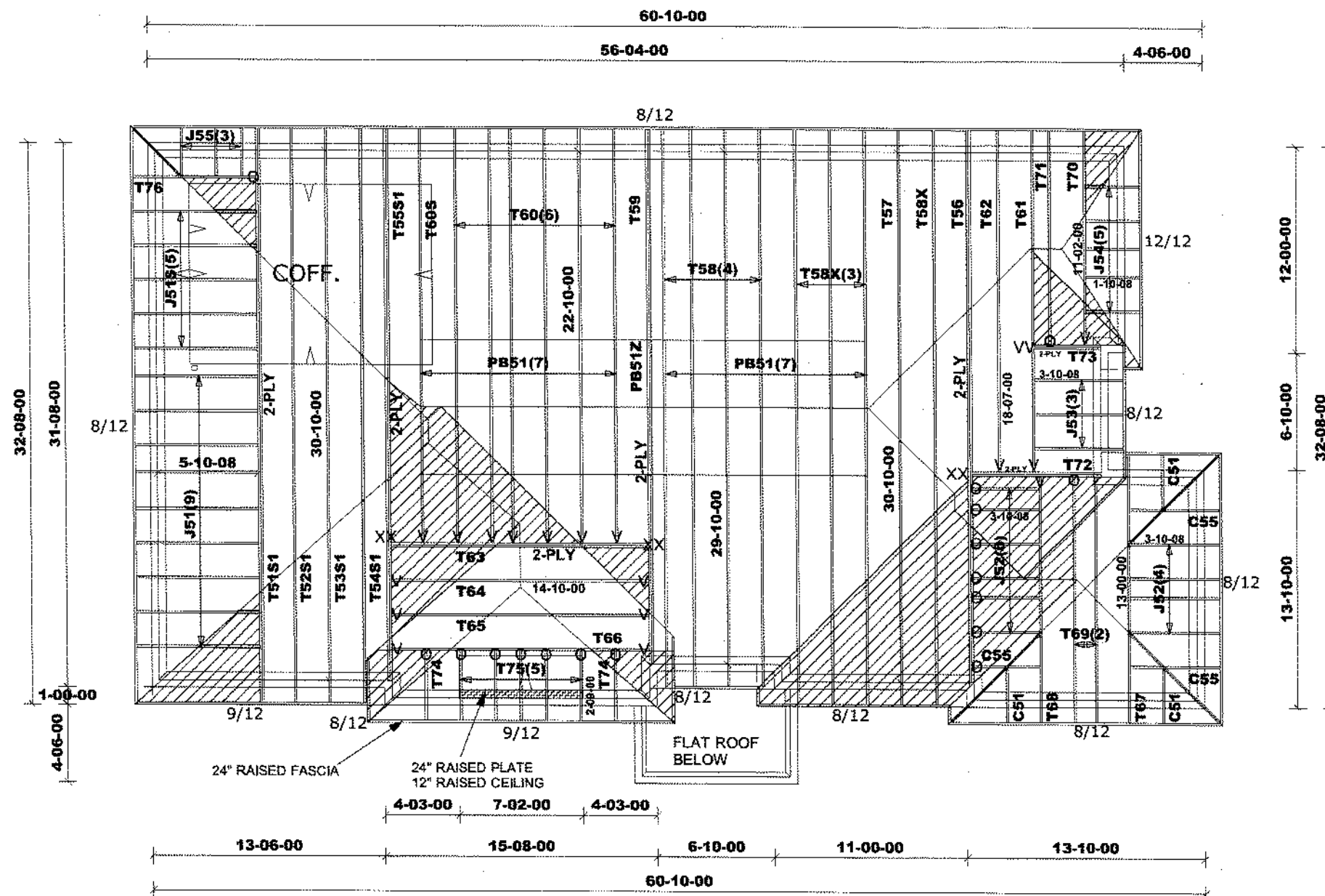
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



Job Track: **52647**
Layout ID: **425143**
Plan Log: **205911**

Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **FORESTSIDE ESTATES**
Date: 7/20/22 Designer:

Model / Elevation: **UNIT 4502 - COR / C 4 BED OPT RAISED CEILING**
Mitek ver 8.5.3.233
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

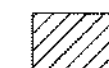
8/12 ROOF PITCHES UNLESS
OTHERWISE NOTED

All conventional framing to conform with
Part 9 of O.B.C. 2012. Roof rafters that
cross over or meet trusses to be min. 2x4
SPF #2 @ 24" o/c with a vertical post to the
truss at each cross point. Vertical posts
longer than 6' to have lateral bracing so that
the distance between the post end points
and lateral bracing does not exceed 6'.

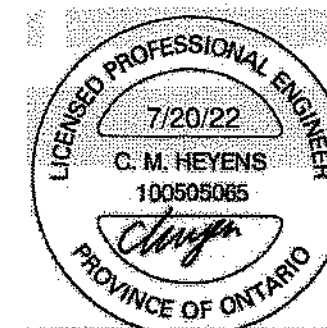
DESIGN CONFORMS WITH OBC
2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (L)

 DENOTES:
CONVENTIONAL
FRAMING

DWG# TR22070648 TO TR22070680
DWG# TR22070686 TO TR22070693



STRUCTURAL COMPONENTS ONLY
DWG# TR22070702

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



Job Track: **52647**
Layout ID: **425144**
Plan Log: **205911**

Builder / Location: **ROYAL PINE HOMES / BRAMPTON**
Project: **FORESTSIDE ESTATES**
Date: 7/20/22 Designer:

Model / Elevation: **UNIT 4502 - COR / C 5 BED OPT RAISED CEILING**
Mitek ver 6.5.3.233
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: A STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425136
 Ref #
 Page: 1 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10 / 12	31-10-00	4-08-05	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	399.15 238.67		
	1 2-ply	T2 Piggyback Base Girder	10 / 12	30-10-00	7-11-15	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	404.12 249.33		
	1 2-ply	T3 Hip Girder	10 / 12	30-10-00	7-10-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	392.11 244.00		
	4	T4 Piggyback Base	10 / 12	29-10-00	9-06-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	616.65 384.67		
	11	T4X Hip	10 / 12	30-10-00	10-02-08	2 x 4	1-03-08 1-03-08	1-07-11 9-11	1718.17 1063.33		
	1	T5 Half Hip	10 / 12	25-08-08	6-03-15	2 x 4	1-03-08	1-07-11 6-03-15	110.18 68.83		
	3	T6 Hip	10 / 12	25-08-08	7-11-15	2 x 4	1-03-08	1-07-11 5-10-15	441.47 274.00		
	1	T7 Hip	10 / 12	25-08-08	9-07-15	2 x 4	1-03-08	1-07-11 5-10-15	143.07 89.50		
	1	T8 Half Hip Girder	10 / 12	18-07-00	4-07-15	2 x 4 2 x 6	1-03-08	1-07-11 4-07-15	91.73 56.83		
	1	T9 Half Hip	10 / 12	18-07-00	6-03-15	2 x 4	1-03-08	1-07-11 6-03-15	81.94 51.33		
	1 2-ply	T10 Hip Girder	10 / 12	13-00-00	3-08-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	130.84 86.33		
	1 2-ply	T11 Half Hip Girder	10 / 12	10-03-08	3-08-04	2 x 4 2 x 6		1-07-11 3-08-04	100.23 65.67		
	1	T12 Hip	10 / 12	10-03-08	5-04-04	2 x 4		1-07-11 3-10-12	50.8 34.33		
	4	T13 Common	10 / 12	10-03-08	6-07-11	2 x 4		1-07-11 3-00-12	181.27 116.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: A STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425136
 Ref #
 Page: 2 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T14 Hip Girder	10 / 12	11-05-08	6-06-07	2 x 4 2 x 6		1-07-11 4-10-07	148.25 94.00		
	1 2-ply	T15 Monopitch Girder	10 / 12	7-06-00	7-10-11	2 x 4 2 x 6		1-07-11 7-10-11	88.95 59.00		
	3	T16 Roof Special	10 / 12	7-07-00	6-09-06	2 x 4		2-07-07 2-07-07	115.44 78.00		
	2	T17 Roof Special	10 / 12	7-06-08	5-09-11	2 x 4		1-08-03 1-07-12	67.07 46.33		
	2	T18 Common	10 / 12	8-00-00	3-11-11	2 x 4 2 x 6		7-11 7-11	60.23 38.67		
	1	G18 GABLE	10 / 12	8-00-00	3-11-11	2 x 4 2 x 6		7-11 7-11	31.85 20.00		
	2	T19 Common	10 / 12	7-06-00	4-09-11	2 x 4		1-08-03 1-08-03	64.6 45.00		
	1 2-ply	T20 Half Hip Girder	10 / 12	5-10-08	4-07-15	2 x 4 2 x 6		1-07-11 4-07-15	67.85 43.67		
	2 2-ply	T21 Flat Girder	0 / 12	3-07-08	1-00-00	2 x 4		1-00-00 1-00-00	46.85 34.67		
	2 2-ply	T22 Monopitch Girder	0 / 12	2-05-08	1-00-00	2 x 4		1-00-00 1-00-00	32.45 25.33		
	1 2-ply	T22Z Monopitch Girder	0 / 12	2-05-08	1-00-00	2 x 4		1-00-00 1-00-00	16.22 12.67		
	5	T23 Half Hip	24 / 12	6-01-00	3-03-00	2 x 4	10-08	1-03-08 3-03-00	174.27 120.00		
	1	PB1 Piggyback	10 / 12	15-07-00	3-04-00	2 x 4			48.32 31.33		
	1	PB2 Piggyback	10 / 12	15-07-00	5-00-00	2 x 4			49.52 31.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: A STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425136
 Ref #
 Page: 3 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	PB3 Piggyback	10 /12	15-07-00	4-03-04	2 x 4			96.66 61.67		
	1	PB4 Piggyback	10 /12	10-10-00	1-08-00	2 x 4			31.62 20.33		
	1	PB5 Piggyback	10 /12	10-10-00	3-04-00	2 x 4			33.16 21.67		
	12	PB6 Piggyback	10 /12	10-10-00	4-06-03	2 x 4			359.76 232.00		
	20	J1 Jack-Open	10 /12	3-07-08	4-07-15	2 x 4	1-03-08	1-07-11 4-07-15	289.65 193.33		
	7	C1 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	68.31 49.00		
	4	C2 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-10-01	1-07-11 3-01-09	47.02 33.33		
	3	C3 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-05-00 8-01	1-07-11 3-01-09	31.83 23.00		

TOTAL # TRUSS= 120

TOTAL BFT OF ALL TRUSSES= 4337.82

BFT.

TOTAL WEIGHT OF ALL TRSSES 6831.6 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
14	Hardware	LUS24	
7	Hardware	LJS26DS	
6	Hardware	LUS26-2	
5	Hardware	HGUS26-2	
5	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= 37

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: A OPT (4 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425137
 Ref #
 Page: 1 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	10 / 12	31-10-00	4-08-05	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	421.81 260.67		
	1 2-ply	T2 Piggyback Base Girder	10 / 12	30-10-00	7-11-15	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	404.12 249.33		
	1 2-ply	T3 Hip Girder	10 / 12	30-10-00	7-10-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	406.8 253.33		
	4	T4 Piggyback Base	10 / 12	29-10-00	9-06-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	616.65 384.67		
	11	T4X Hip	10 / 12	30-10-00	10-02-08	2 x 4	1-03-08 1-03-08	1-07-11 9-11	1718.17 1063.33		
	1	T5S Half Hip	10 / 12	25-08-08	6-03-15	2 x 4	1-03-08	1-07-11 6-03-15	126.66 81.17		
	3	T6S Hip	10 / 12	25-08-08	7-11-15	2 x 4	1-03-08	1-07-11 5-10-15	468.51 298.00		
	1	T7S Hip	10 / 12	25-08-08	9-07-15	2 x 4	1-03-08	1-07-11 5-10-15	167.48 106.50		
	1	T8 Half Hip Girder	10 / 12	18-07-00	4-07-15	2 x 4 2 x 6	1-03-08	1-07-11 4-07-15	91.73 56.83		
	1	T9 Half Hip	10 / 12	18-07-00	6-03-15	2 x 4	1-03-08	1-07-11 6-03-15	81.94 51.33		
	1 2-ply	T10 Hip Girder	10 / 12	13-00-00	3-08-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	130.84 86.33		
	1 2-ply	T11 Half Hip Girder	10 / 12	10-03-08	3-08-04	2 x 4 2 x 6		1-07-11 3-08-04	100.23 65.67		
	1	T12 Hip	10 / 12	10-03-08	5-04-04	2 x 4		1-07-11 3-10-12	50.8 34.33		
	4	T13 Common	10 / 12	10-03-08	6-07-11	2 x 4		1-07-11 3-00-12	181.27 116.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: A OPT (4 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425137
 Ref #
 Page: 2 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T14 Hip Girder	10 / 12	11-05-08	6-06-07	2 x 4 2 x 6		1-07-11 4-10-07	148.25 94.00		
	1 2-ply	T15 Monopitch Girder	10 / 12	7-06-00	7-10-11	2 x 4 2 x 6		1-07-11 7-10-11	88.95 59.00		
	3	T16 Roof Special	10 / 12	7-07-00	6-09-06	2 x 4		2-07-07 2-07-07	115.44 78.00		
	2	T17 Roof Special	10 / 12	7-06-08	5-09-11	2 x 4		1-08-03 1-07-12	67.07 46.33		
	2	T18 Common	10 / 12	8-00-00	3-11-11	2 x 4 2 x 6		7-11 7-11	60.23 38.67		
	1	G18 GABLE	10 / 12	8-00-00	3-11-11	2 x 4 2 x 6		7-11 7-11	31.85 20.00		
	2	T19 Common	10 / 12	7-06-00	4-09-11	2 x 4		1-08-03 1-08-03	64.6 45.00		
	1 2-ply	T20 Half Hip Girder	10 / 12	5-10-08	4-07-15	2 x 4 2 x 6		1-07-11 4-07-15	67.85 43.67		
	2 2-ply	T21 Flat Girder	0 / 12	3-07-08	1-00-00	2 x 4		1-00-00 1-00-00	46.85 34.87		
	2 2-ply	T22 Monopitch Girder	0 / 12	2-05-08	1-00-00	2 x 4		1-00-00 1-00-00	32.45 25.33		
	1 2-ply	T22Z Monopitch Girder	0 / 12	2-05-08	1-00-00	2 x 4		1-00-00 1-00-00	16.22 12.67		
	5	T23 Half Hip	24 / 12	6-01-00	3-03-00	2 x 4	10-08	1-03-08 3-03-00	174.27 120.00		
	1	PB1 Piggyback	10 / 12	15-07-00	3-04-00	2 x 4			48.32 31.33		
	1	PB2 Piggyback	10 / 12	15-07-00	5-00-00	2 x 4			49.52 31.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: A OPT (4 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425137
 Ref #
 Page: 3 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	PB3 Piggyback	10 / 12	15-07-00	4-03-04	2 x 4			96.66 61.67		
	1	PB4 Piggyback	10 / 12	10-10-00	1-08-00	2 x 4			31.62 20.33		
	1	PB5 Piggyback	10 / 12	10-10-00	3-04-00	2 x 4			33.16 21.67		
	12	PB6 Piggyback	10 / 12	10-10-00	4-06-03	2 x 4			359.76 232.00		
	14	J1 Jack-Open	10 / 12	3-07-08	4-07-15	2 x 4	1-03-08	1-07-11 4-07-15	202.76 135.33		
	6	J1S Jack-Open	10 / 12	3-07-08	4-07-15	2 x 4	1-03-08	1-07-11 3-07-15	115.14 79.00		
	7	C1 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	68.31 49.00		
	4	C2 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 1-10-01	1-07-11 3-01-09	47.02 33.33		
	3	C3 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-05-00 8-01	1-07-11 3-01-09	31.83 23.00		

TOTAL # TRUSS= 120

TOTAL BFT OF ALL TRUSSES= 4443.49

BFT.

TOTAL WEIGHT OF ALL TRSSES 6965.14 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
14	Hardware	LUS24	
7	Hardware	LJS26DS	
6	Hardware	LUS26-2	
5	Hardware	HGUS26-2	
5	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= 37

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: A OPT (5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425138
 Ref #
 Page: 1 of 4
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S1 Hip Girder	10 / 12	31-10-00	4-08-05	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	417.93 255.33		
	1 2-ply	T2S Piggyback Base Girder	10 / 12	30-10-00	7-11-15	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	421.96 266.67		
	1 2-ply	T3 Hip Girder	10 / 12	30-10-00	7-10-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	392.11 244.00		
	4	T4 Piggyback Base	10 / 12	29-10-00	9-06-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	616.65 384.67		
	11	T4X Hip	10 / 12	30-10-00	10-02-08	2 x 4	1-03-08 1-03-08	1-07-11 9-11	1718.17 1063.33		
	1	T5S1 Half Hip	10 / 12	25-08-08	6-03-15	2 x 4	1-03-08	1-07-11 6-03-15	122.62 77.83		
	3	T6S1 Hip	10 / 12	25-08-08	7-11-15	2 x 4	1-03-08	1-07-11 5-10-15	441.61 281.00		
	1	T7S1 Hip	10 / 12	25-08-08	9-07-15	2 x 4	1-03-08	1-07-11 5-10-15	160.61 103.17		
	1	T8 Half Hip Girder	10 / 12	18-07-00	4-07-15	2 x 4 2 x 6	1-03-08	1-07-11 4-07-15	91.73 56.83		
	1	T9 Half Hip	10 / 12	18-07-00	6-03-15	2 x 4	1-03-08	1-07-11 6-03-15	81.94 51.33		
	1 2-ply	T10 Hip Girder	10 / 12	13-00-00	3-08-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	130.84 86.33		
	1 2-ply	T11 Half Hip Girder	10 / 12	10-03-08	3-08-04	2 x 4 2 x 6		1-07-11 3-08-04	100.23 65.67		
	1	T12 Hip	10 / 12	10-03-08	5-04-04	2 x 4		1-07-11 3-10-12	50.8 34.33		
	4	T13 Common	10 / 12	10-03-08	6-07-11	2 x 4		1-07-11 3-00-12	181.27 116.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: A OPT (5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425138
 Ref #
 Page: 2 of 4
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T14 Hip Girder	10 /12	11-05-08	6-06-07	2 x 4 2 x 6		1-07-11 4-10-07	148.25 94.00		
	1 2-ply	T15 Monopitch Girder	10 /12	7-06-00	7-10-11	2 x 4 2 x 6		1-07-11 7-10-11	88.95 59.00		
	3	T16 Roof Special	10 /12	7-07-00	6-09-06	2 x 4		2-07-07 2-07-07	115.44 78.00		
	2	T17 Roof Special	10 /12	7-06-08	5-09-11	2 x 4		1-08-03 1-07-12	67.07 48.33		
	2	T18 Common	10 /12	8-00-00	3-11-11	2 x 4 2 x 6		7-11 7-11	60.23 38.67		
	1	G18 GABLE	10 /12	8-00-00	3-11-11	2 x 4 2 x 6		7-11 7-11	31.85 20.00		
	2	T19 Common	10 /12	7-06-00	4-09-11	2 x 4		1-08-03 1-08-03	64.6 45.00		
	1 2-ply	T20 Half Hip Girder	10 /12	5-10-08	4-07-15	2 x 4 2 x 6		1-07-11 4-07-15	67.85 43.67		
	2 2-ply	T21 Flat Girder	0 /12	3-07-08	1-00-00	2 x 4		1-00-00 1-00-00	46.85 34.67		
	2 2-ply	T22 Monopitch Girder	0 /12	2-05-08	1-00-00	2 x 4		1-00-00 1-00-00	32.45 25.33		
	1 2-ply	T22Z Monopitch Girder	0 /12	2-05-08	1-00-00	2 x 4		1-00-00 1-00-00	16.22 12.67		
	5	T23 Half Hip	24 /12	6-01-00	3-03-00	2 x 4	10-08	1-03-08 3-03-00	174.27 120.00		
	1	T24 Half Hip Girder	10 /12	3-07-08	2-10-11	2 x 4	1-03-08	1-07-11 2-10-11	20.56 14.67		
	1	PB1 Piggyback	10 /12	15-07-00	3-04-00	2 x 4			48.32 31.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: A OPT (5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425138
 Ref #:
 Page: 3 of 4
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	PB2 Piggyback	10 /12	15-07-00	5-00-00	2 x 4			49.52 31.33		
	1 2-ply	PB3 Piggyback	10 /12	15-07-00	4-03-04	2 x 4			96.66 61.67		
	1	PB4 Piggyback	10 /12	10-10-00	1-08-00	2 x 4			31.62 20.33		
	1	PB5 Piggyback	10 /12	10-10-00	3-04-00	2 x 4			33.16 21.67		
	12	PB6 Piggyback	10 /12	10-10-00	4-06-03	2 x 4			359.76 232.00		
	15	J1 Jack-Open	10 /12	3-07-08	4-07-15	2 x 4	1-03-08	1-07-11 4-07-15	217.24 145.00		
	5	J1S Jack-Open	10 /12	3-07-08	4-07-15	2 x 4	1-03-08	1-07-11 3-07-15	95.95 65.83		
	2	J2 Jack-Open	10 /12	1-06-00	2-10-11	2 x 4	1-03-08	1-07-11 2-10-11	17.81 12.67		
	6	C1 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	58.55 42.00		
	3	C2 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-10-01	1-07-11 3-01-09	35.26 25.00		
	3	C3 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-05-00 8-01	1-07-11 3-01-09	31.83 23.00		

TOTAL # TRUSS= 121

TOTAL BFT OF ALL TRUSSES= 4431

BFT.

TOTAL WEIGHT OF ALL TRSSES 6938.76 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
14	Hardware	LUS24	
7	Hardware	LJS26DS	
6	Hardware	LUS26-2	
5	Hardware	HGUS26-2	
5	Hardware	H2.5T	

DELIVERY SHIPLIST		
	Lumber Yard:	TAMARACK LUMBER
	Builder:	ROYAL PINE HOMES
	Project:	FORESTSIDE ESTATES
	Location:	BRAMPTON
	Model:	UNIT 4502 - COR
	Lot #:	
	Elevation:	A OPT (5 BED)
	Job Track:	52647
	PlanLog:	205911
	Layout ID:	425138
	Ref #	
	Page:	4 of 4
	Date:	07-21-2022
	Designer:	
	Sales Rep:	Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
-----	------	-------	--------

TOTAL NUMBER OF ITEMS= 37

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: B STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425139
 Ref #
 Page: 1 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T3Z Hip Girder	10 / 12	30-10-00	7-10-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	406.25 253.33		
	4	T4 Piggyback Base	10 / 12	29-10-00	9-06-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	616.65 384.67		
	5	T4X Hip	10 / 12	30-10-00	10-02-08	2 x 4	1-03-08 1-03-08	1-07-11 9-11	787.17 485.83		
	1 2-ply	T30 Roof Special Girder	10 / 12	30-10-00	4-11-00	2 x 6	1-03-08 1-03-08	1-07-11 1-09-02	374.42 220.67		
	1	T31 Roof Special	10 / 12	30-10-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-09-02	131.25 81.50		
	2	T32 Roof Special	10 / 12	30-10-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-09-02	304.52 191.33		
	1 2-ply	T33 Piggyback Base Girder	10 / 12	30-10-00	8-03-00	2 x 6	1-03-08 1-03-08	1-07-11 1-09-02	482.66 294.00		
	1	T34 Roof Special	10 / 12	30-10-00	9-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-09-02	159.67 100.17		
	1 2-ply	T35 Piggyback Base Girder	10 / 12	29-10-00	9-06-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	385.06 233.33		
	7	T36 Piggyback Base	10 / 12	24-10-00	9-06-11	2 x 4	1-03-08	1-07-11 5-09-11	951.7 592.67		
	1	T37 Half Hip Girder	10 / 12	18-07-00	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 4-10-07	92.1 58.33		
	1	T38 Half Hip	10 / 12	18-07-00	6-06-07	2 x 4	1-03-08	1-07-11 6-06-07	82.89 51.33		
	1 2-ply	T39 Hip Girder	10 / 12	14-10-00	9-03-03	2 x 6		3-07-11 3-07-11	275.93 172.00		
	1	T40 Hip	10 / 12	14-10-00	7-07-03	2 x 4		3-07-11 3-07-11	77.47 50.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: B STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425139
 Ref #
 Page: 2 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T41 Hip Girder	10 / 12	14-10-00	5-11-03	2 x 4 2 x 6		3-07-11 3-07-11	82.95 53.33		
	1 2-ply	T42 Hip Girder	10 / 12	13-00-00	5-11-03	2 x 4 2 x 6	1-03-08 1-03-08	3-07-11 3-07-11	161.03 105.33		
	1 2-ply	T43 Half Hip Girder	10 / 12	10-00-00	5-11-03	2 x 4 2 x 6		3-07-11 5-11-03	123.08 79.33		
	2	T44 Hip	10 / 12	10-00-00	6-09-03	2 x 4		3-07-11 6-01-11	111.09 73.33		
	2	T45 Hip	10 / 12	10-00-00	8-05-03	2 x 4		3-07-11 6-01-11	131.9 83.33		
	1	T46 Common	10 / 12	10-00-00	7-07-11	2 x 4		3-07-11 3-03-11	48.42 30.83		
	1 2-ply	T47 Monopitch Girder	10 / 12	7-06-00	7-10-11	2 x 4 2 x 6		1-07-11 7-10-11	90.83 60.33		
	1	T48 Common	10 / 12	11-02-00	6-03-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	49.72 33.00		
	2	T48S Roof Special	10 / 12	11-02-00	6-03-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	118.41 79.67		
	1 2-ply	T49 Monopitch Girder	10 / 12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1	PB4 Piggyback	10 / 12	10-10-00	1-08-00	2 x 4			31.62 20.33		
	1	PB5 Piggyback	10 / 12	10-10-00	3-04-00	2 x 4			33.16 21.67		
	13	PB6 Piggyback	10 / 12	10-10-00	4-06-03	2 x 4			389.74 251.33		
	1 2-ply	PB6Z Piggyback	10 / 12	10-10-00	4-06-03	2 x 4			59.96 38.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: B STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425139
 Ref #
 Page: 3 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	PB30 Piggyback	10 / 12	15-11-02	3-04-00	2 x 4			51.73 32.83		
	1 2-ply	PB31 Piggyback	10 / 12	15-11-02	4-05-12	2 x 4			100.27 61.67		
	17	J30 Jack-Open	10 / 12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	257.36 164.33		
	5	J31 Jack-Open	10 / 12	2-09-00	5-11-03	2 x 4	1-03-08	3-07-11 5-11-03	79.51 53.33		
	10	J32 Half Hip	10 / 12	2-09-00	5-11-03	2 x 4	1-03-08	2-07-11 4-11-03	204.56 140.00		
	1	C4 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.04 8.33		
	1	C5 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.76 7.00		
	5	C30 Jack-Open	10 / 12	1-09-07	5-01-09	2 x 4	1-03-08 1-01	3-07-11 5-01-09	87.37 48.67		
	5	C31 Jack-Open	10 / 12	1-09-07	5-01-09	2 x 4	1-03-08 11-09	3-07-11 5-01-09	72.36 50.00		

TOTAL # TRUSS= 114

TOTAL BFT OF ALL TRUSSES= 4693.8

BFT.

TOTAL WEIGHT OF ALL TRSSES 7457.55 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
22	Hardware	LUS24	
13	Hardware	LJS26DS	
1	Hardware	LUS26-2	
5	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 41

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: B OPT (4 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425140
 Ref #
 Page: 1 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T3Z Hip Girder	10/12	30-10-00	7-10-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	406.26 253.33		
	4	T4 Piggyback Base	10/12	29-10-00	9-06-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	616.65 384.67		
	5	T4X Hip	10/12	30-10-00	10-02-08	2 x 4	1-03-08 1-03-08	1-07-11 9-11	780.99 483.33		
	1 2-ply	T30S Hip Girder	10/12	30-10-00	4-11-00	2 x 6	1-03-08 1-03-08	1-07-11 1-09-02	407.42 249.33		
	1	T31S Hip	10/12	30-10-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-09-02	149.81 94.17		
	2	T32S Hip	10/12	30-10-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-09-02	326.6 205.00		
	1 2-ply	T33S Piggyback Base Girder	10/12	30-10-00	8-03-00	2 x 6	1-03-08 1-03-08	1-07-11 1-09-02	501.37 315.33		
	1	T34S Hip	10/12	30-10-00	9-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-09-02	184.6 117.83		
	1 2-ply	T35 Piggyback Base Girder	10/12	29-10-00	9-06-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	385.06 233.33		
	7	T36 Piggyback Base	10/12	24-10-00	9-06-11	2 x 4	1-03-08	1-07-11 5-09-11	951.7 592.67		
	1	T37 Half Hip Girder	10/12	18-07-00	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 4-10-07	92.1 58.33		
	1	T38 Half Hip	10/12	18-07-00	6-06-07	2 x 4	1-03-08	1-07-11 6-06-07	82.89 51.33		
	1 2-ply	T39 Hip Girder	10/12	14-10-00	9-03-03	2 x 6		3-07-11 3-07-11	275.93 172.00		
	1	T40 Hip	10/12	14-10-00	7-07-03	2 x 4		3-07-11 3-07-11	77.47 50.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: B OPT (4 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425140
 Ref #
 Page: 2 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T41 Hip Girder	10 /12	14-10-00	5-11-03	2 x 4 2 x 6		3-07-11 3-07-11	82.95 53.33		
	1 2-ply	T42 Hip Girder	10 /12	13-00-00	5-11-03	2 x 4 2 x 6	1-03-08 1-03-08	3-07-11 3-07-11	161.03 105.33		
	1 2-ply	T43 Half Hip Girder	10 /12	10-00-00	5-11-03	2 x 4 2 x 6		3-07-11 5-11-03	123.08 79.33		
	2	T44 Hip	10 /12	10-00-00	6-09-03	2 x 4		3-07-11 6-01-11	111.09 73.33		
	2	T45 Hip	10 /12	10-00-00	8-05-03	2 x 4		3-07-11 6-01-11	131.9 83.33		
	1	T46 Common	10 /12	10-00-00	7-07-11	2 x 4		3-07-11 3-03-11	48.42 30.83		
	1 2-ply	T47 Monopitch Girder	10 /12	7-06-00	7-10-11	2 x 4 2 x 6		1-07-11 7-10-11	90.63 60.33		
	1	T48 Common	10 /12	11-02-00	6-03-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	49.72 33.00		
	2	T48S Roof Special	10 /12	11-02-00	6-03-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	118.41 79.67		
	1 2-ply	T49 Monopitch Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1	PB4 Piggyback	10 /12	10-10-00	1-08-00	2 x 4			31.62 20.33		
	1	PB5 Piggyback	10 /12	10-10-00	3-04-00	2 x 4			33.16 21.67		
	13	PB6 Piggyback	10 /12	10-10-00	4-06-03	2 x 4			389.74 251.33		
	1 2-ply	PB6Z Piggyback	10 /12	10-10-00	4-06-03	2 x 4			59.96 38.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: B OPT (4 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425140
 Ref #
 Page: 3 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	PB30 Piggyback	10 / 12	15-11-02	3-04-00	2 x 4			51.73 32.83		
	1 2-ply	PB31 Piggyback	10 / 12	15-11-02	4-05-12	2 x 4			100.27 61.67		
	12	J30 Jack-Open	10 / 12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	181.67 116.00		
	5	J30S Jack-Open	10 / 12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 3-10-07	99.23 65.83		
	5	J31 Jack-Open	10 / 12	2-09-00	5-11-03	2 x 4	1-03-08	3-07-11 5-11-03	79.51 53.33		
	10	J32 Half Hip	10 / 12	2-09-00	5-11-03	2 x 4	1-03-08	2-07-11 4-11-03	204.56 140.00		
	1	C4 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.04 8.33		
	1	C5 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.76 7.00		
	5	C30 Jack-Open	10 / 12	1-09-07	5-01-09	2 x 4	1-03-08 1-01	3-07-11 5-01-09	67.37 46.87		
	5	C31 Jack-Open	10 / 12	1-09-07	5-01-09	2 x 4	1-03-08 11-09	3-07-11 5-01-09	72.36 50.00		

TOTAL # TRUSS= 114

TOTAL BFT OF ALL TRUSSES= 4802.79

BFT.

TOTAL WEIGHT OF ALL TRSSES 7592.19 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
22	Hardware	LUS24	
13	Hardware	LJS26DS	
1	Hardware	LUS26-2	
5	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 41

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: B OPT (5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425141
 Ref #
 Page: 1 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T3Z Hip Girder	10 / 12	30-10-00	7-10-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	406.8 253.33		
	4	T4 Piggyback Base	10 / 12	29-10-00	9-06-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	616.65 384.67		
	5	T4X Hip	10 / 12	30-10-00	10-02-08	2 x 4	1-03-08 1-03-08	1-07-11 9-11	780.99 483.33		
	1 2-ply	T30S1 Hip Girder	10 / 12	30-10-00	4-11-00	2 x 6	1-03-08 1-03-08	1-07-11 1-09-02	407.26 249.33		
	1	T31S1 Hip	10 / 12	30-10-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-09-02	143.71 91.00		
	2	T32S1 Hip	10 / 12	30-10-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-09-02	325.03 206.67		
	1 2-ply	T33S1 Piggyback Base Girder	10 / 12	30-10-00	8-03-00	2 x 6	1-03-08 1-03-08	1-07-11 1-09-02	506.07 318.00		
	1	T34S1 Hip	10 / 12	30-10-00	9-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-09-02	181.19 116.17		
	1 2-ply	T35 Piggyback Base Girder	10 / 12	29-10-00	9-06-11	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	385.06 233.33		
	6	T36 Piggyback Base	10 / 12	24-10-00	9-06-11	2 x 4	1-03-08	1-07-11 5-09-11	815.75 508.00		
	1	T36S Piggyback Base	10 / 12	24-10-00	9-06-11	2 x 4	1-03-08	1-07-11 5-09-11	153.04 96.50		
	1	T37 Half Hip Girder	10 / 12	18-07-00	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 4-10-07	92.1 58.33		
	1	T38 Half Hip	10 / 12	18-07-00	6-06-07	2 x 4	1-03-08	1-07-11 6-06-07	82.89 51.33		
	1 2-ply	T39 Hip Girder	10 / 12	14-10-00	9-03-03	2 x 6		3-07-11 3-07-11	275.93 172.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: B OPT (5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425141
 Ref #
 Page: 2 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T40 Hip	10 / 12	14-10-00	7-07-03	2 x 4		3-07-11 3-07-11	77.47 50.67		
	1	T41 Hip Girder	10 / 12	14-10-00	5-11-03	2 x 4 2 x 6		3-07-11 3-07-11	82.95 53.33		
	1 2-ply	T42 Hip Girder	10 / 12	13-00-00	5-11-03	2 x 4 2 x 6	1-03-08 1-03-08	3-07-11 3-07-11	161.03 105.33		
	1 2-ply	T43 Half Hip Girder	10 / 12	10-00-00	5-11-03	2 x 4 2 x 6		3-07-11 5-11-03	123.08 79.33		
	2	T44 Hip	10 / 12	10-00-00	6-09-03	2 x 4		3-07-11 6-01-11	111.09 73.33		
	2	T45 Hip	10 / 12	10-00-00	8-05-03	2 x 4		3-07-11 6-01-11	131.9 83.33		
	1	T46 Common	10 / 12	10-00-00	7-07-11	2 x 4		3-07-11 3-03-11	48.42 30.83		
	1 2-ply	T47 Monopitch Girder	10 / 12	7-06-00	7-10-11	2 x 4 2 x 6		1-07-11 7-10-11	88.95 59.00		
	1	T48 Common	10 / 12	11-02-00	6-03-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	49.72 33.00		
	2	T48S Roof Special	10 / 12	11-02-00	6-03-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	118.41 79.67		
	1 2-ply	T49 Monopitch Girder	10 / 12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1	T50 Half Hip Girder	10 / 12	3-10-08	2-10-11	2 x 4	1-03-08	1-07-11 2-10-11	21.26 14.67		
	1	PB4 Piggyback	10 / 12	10-10-00	1-08-00	2 x 4			31.62 20.33		
	1	PB5 Piggyback	10 / 12	10-10-00	3-04-00	2 x 4			33.16 21.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: B OPT (5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425141
 Ref #
 Page: 3 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	13	PB6 Piggyback	10 /12	10-10-00	4-06-03	2 x 4			389.74 251.33		
	1 2-ply	PB6Z Piggyback	10 /12	10-10-00	4-06-03	2 x 4			59.96 38.67		
	1	PB30 Piggyback	10 /12	15-11-02	3-04-00	2 x 4			51.73 32.83		
	1 2-ply	PB31 Piggyback	10 /12	15-11-02	4-05-12	2 x 4			100.27 61.67		
	12	J30 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	181.67 116.00		
	5	J30S Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 3-10-07	99.23 65.83		
	5	J31 Jack-Open	10 /12	2-09-00	5-11-03	2 x 4	1-03-08	3-07-11 5-11-03	79.51 53.33		
	10	J32 Half Hip	10 /12	2-09-00	5-11-03	2 x 4	1-03-08	2-07-11 4-11-03	204.56 140.00		
	2	J50 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	19.52 14.00		
	5	C30 Jack-Open	10 /12	1-09-07	5-01-09	2 x 4	1-03-08 1-01	3-07-11 5-01-09	67.37 46.67		
	5	C31 Jack-Open	10 /12	1-09-07	5-01-09	2 x 4	1-03-08 11-09	3-07-11 5-01-09	72.36 50.00		

TOTAL # TRUSS= 115

TOTAL BFT OF ALL TRUSSES= 4826.14

BFT.

TOTAL WEIGHT OF ALL TRSSES 7620.57 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
22	Hardware	LUS24	
13	Hardware	LJS26DS	
1	Hardware	LUS26-2	
5	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 41

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: C STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425142
 Ref #
 Page: 1 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T51 Roof Special Girder	8 / 12	30-10-00	5-04-02	2 x 6	1-03-08 1-03-08	1-04-14 1-06-04	380.51 231.33		
	1	T52 Roof Special	8 / 12	30-10-00	6-07-14	2 x 4	1-03-08 1-03-08	1-04-14 1-06-04	138.87 88.17		
	1	T53 Roof Special	8 / 12	30-10-00	7-11-14	2 x 4	1-03-08 1-03-08	1-04-14 1-06-04	139.04 87.83		
	1	T54 Roof Special	8 / 12	30-10-00	9-03-14	2 x 4	1-03-08 1-03-08	1-04-14 1-06-04	147.97 92.50		
	1 2-ply	T55 Roof Special Girder	8 / 12	30-10-00	10-03-02	2 x 6	1-03-08 1-03-08	1-04-14 1-06-04	489.68 295.33		
	1 2-ply	T56 Hip Girder	8 / 12	30-10-00	7-04-15	2 x 6	1-03-08 1-03-08	1-04-14 1-04-14	436.58 272.00		
	1	T57 Hip	8 / 12	30-10-00	10-06-09	2 x 4	1-03-08 1-03-08	1-04-14 8-14	143.57 89.50		
	4	T58 Piggyback Base	8 / 12	29-10-00	8-08-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	538.45 330.67		
	4	T58X Hip	8 / 12	30-10-00	9-02-09	2 x 4	1-03-08 1-03-08	1-04-14 8-14	554.63 340.00		
	1 2-ply	T59 Piggyback Base Girder	8 / 12	29-10-00	8-08-15	2 x 6	1-03-08 1-03-08	1-04-14 1-04-14	441.85 268.00		
	7	T60 Piggyback Base	8 / 12	22-10-00	8-08-14	2 x 4	1-03-08	1-04-14 6-00-14	796.43 484.17		
	1	T61 Half Hip Girder	8 / 12	18-07-00	4-11-14	2 x 4 2 x 6	1-03-08	1-04-14 4-11-14	92.41 57.83		
	1	T62 Half Hip	8 / 12	18-07-00	6-03-14	2 x 4	1-03-08	1-04-14 6-03-14	83.56 51.67		
	1 2-ply	T63 Common Girder	8 / 12	14-10-00	8-04-03	2 x 4 2 x 6		3-04-14 3-04-14	204.61 129.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: C STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425142
 Ref #
 Page: 2 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T64 Common	8 / 12	14-10-00	8-04-03	2 x 4		3-04-14 3-04-14	71.28 45.33		
	1	T65 Hip	8 / 12	14-10-00	7-01-00	2 x 4		3-04-14 3-04-14	69.9 44.00		
	1	T66 Hip Girder	8 / 12	14-10-00	5-07-00	2 x 4 2 x 6		3-04-14 3-04-14	80.32 52.17		
	1	T67 Hip Girder	8 / 12	13-00-00	3-11-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	55.99 36.33		
	1	T68 Half Hip Girder	8 / 12	13-00-00	3-11-14	2 x 4	1-03-08	1-04-14 3-11-14	54.9 35.33		
	2	T69 Hip	8 / 12	13-00-00	5-03-03	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	118.21 73.67		
	1	T70 Hip Girder	8 / 12	11-02-00	3-09-00	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	49.87 32.67		
	1	T71 Common	8 / 12	11-02-00	5-01-09	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	46.45 29.83		
	1 2-ply	T72 Monopitch Girder	8 / 12	7-06-00	7-04-14	2 x 4 2 x 6		2-04-14 7-04-14	88.71 56.33		
	1 2-ply	T73 Monopitch Girder	8 / 12	3-10-08	4-11-14	2 x 4 2 x 6		2-04-14 4-11-14	44.68 28.00		
	2	T74 Half Hip	9 / 12	2-09-00	4-07-14	2 x 4	1-03-08	3-06-04 4-07-14	52.73 38.00		
	5	T75 Half Hip	9 / 12	2-09-00	5-07-00	2 x 4	1-03-08	1-06-04 3-07-00	89.86 64.17		
	14	PB51 Piggyback	8 / 12	7-10-00	2-07-05	2 x 4			250.73 158.67		
	1 2-ply	PB51Z Piggyback	8 / 12	7-10-00	2-07-05	2 x 4			35.82 22.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: C STD (4 OR 5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425142
 Ref #
 Page: 3 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	13	J51 Jack-Open	8 / 12	5-10-08	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	248.16 151.67		
	10	J52 Jack-Open	8 / 12	3-10-08	3-11-14	2 x 4	1-03-08	1-04-14 3-11-14	140.68 90.00		
	3	J53 Jack-Open	8 / 12	3-10-08	4-11-14	2 x 4	1-03-08	2-04-14 4-11-14	47.5 30.50		
	5	J54 Jack-Open	12 / 12	1-10-08	3-09-00	2 x 4	1-03-08	1-10-08 3-09-00	54 35.00		
	4	C51 Jack-Open	8 / 12	1-09-07	2-07-03	2 x 4	1-03-08 1-01	1-04-14 2-07-03	35.7 22.67		
	1	C52 Jack-Open	8 / 12	1-10-08	3-11-03	2 x 4	1-03-08 1-10-15	1-04-14 2-07-14	11.67 7.67		
	1	C53 Jack-Open	8 / 12	1-09-07	2-07-03	2 x 4	1-03-08 4-01-01	1-04-14 2-07-03	13.49 8.33		
	1	C54 Jack-Open	8 / 12	3-09-07	3-11-03	2 x 4	1-03-08 2-01-01	1-04-14 3-11-03	16.23 10.33		
	3	C55 Jack-Open	8 / 12	1-09-07	2-07-03	2 x 4	1-03-08 2-01-01	1-04-14 2-07-03	33.62 21.00		

TOTAL # TRUSS= 108

TOTAL BFT OF ALL TRUSSES= 3912.67

BFT.

TOTAL WEIGHT OF ALL TRSSES 6288.62 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
16	Hardware	LUS24	
17	Hardware	LJS26DS	
1	Hardware	LUS26-2	
3	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 37



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: C OPT (4 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425143
 Ref #
 Page: 1 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T51S Hip Girder	8 /12	30-10-00	5-04-02	2 x 6	1-03-08 1-03-08	1-04-14 1-06-04	402.49 251.33		
	1	T52S Hip	8 /12	30-10-00	6-07-14	2 x 4	1-03-08 1-03-08	1-04-14 1-06-04	147.69 94.83		
	1	T53S Hip	8 /12	30-10-00	7-11-14	2 x 4	1-03-08 1-03-08	1-04-14 1-06-04	164.77 104.67		
	1	T54S Hip	8 /12	30-10-00	9-03-14	2 x 4	1-03-08 1-03-08	1-04-14 1-06-04	173.67 110.33		
	1 2-ply	T55S Hip Girder	8 /12	30-10-00	10-03-02	2 x 6	1-03-08 1-03-08	1-04-14 1-06-04	530.95 328.00		
	1 2-ply	T56 Hip Girder	8 /12	30-10-00	7-04-15	2 x 6	1-03-08 1-03-08	1-04-14 1-04-14	436.58 272.00		
	1	T57 Hip	8 /12	30-10-00	10-06-09	2 x 4	1-03-08 1-03-08	1-04-14 8-14	143.57 89.50		
	4	T58 Piggyback Base	8 /12	29-10-00	8-08-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	538.45 330.67		
	4	T58X Hip	8 /12	30-10-00	9-02-09	2 x 4	1-03-08 1-03-08	1-04-14 8-14	554.63 340.00		
	1 2-ply	T59 Piggyback Base Girder	8 /12	29-10-00	8-08-15	2 x 6	1-03-08 1-03-08	1-04-14 1-04-14	441.85 268.00		
	7	T60 Piggyback Base	8 /12	22-10-00	8-08-14	2 x 4	1-03-08	1-04-14 6-00-14	786.43 484.17		
	1	T61 Half Hip Girder	8 /12	18-07-00	4-11-14	2 x 4 2 x 6	1-03-08	1-04-14 4-11-14	92.41 57.83		
	1	T62 Half Hip	8 /12	18-07-00	6-03-14	2 x 4	1-03-08	1-04-14 6-03-14	83.56 51.67		
	1 2-ply	T63 Common Girder	8 /12	14-10-00	8-04-03	2 x 4 2 x 6		3-04-14 3-04-14	204.61 128.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: C OPT (4 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425143
 Ref #
 Page: 2 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T64 Common	8 / 12	14-10-00	8-04-03	2 x 4		3-04-14 3-04-14	71.28 45.33		
	1	T65 Hip	8 / 12	14-10-00	7-01-00	2 x 4		3-04-14 3-04-14	69.9 44.00		
	1	T66 Hip Girder	8 / 12	14-10-00	5-07-00	2 x 4 2 x 6		3-04-14 3-04-14	80.32 52.17		
	1	T67 Hip Girder	8 / 12	13-00-00	3-11-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	55.99 36.33		
	1	T68 Half Hip Girder	8 / 12	13-00-00	3-11-14	2 x 4	1-03-08	1-04-14 3-11-14	54.9 35.33		
	2	T69 Hip	8 / 12	13-00-00	5-03-03	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	118.21 73.67		
	1	T70 Hip Girder	8 / 12	11-02-00	3-09-00	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	49.87 32.67		
	1	T71 Common	8 / 12	11-02-00	5-01-09	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	46.45 29.83		
	1 2-ply	T72 Monopitch Girder	8 / 12	7-06-00	7-04-14	2 x 4 2 x 6		2-04-14 7-04-14	88.71 56.33		
	1 2-ply	T73 Monopitch Girder	8 / 12	3-10-08	4-11-14	2 x 4 2 x 6		2-04-14 4-11-14	44.68 28.00		
	2	T74 Half Hip	9 / 12	2-09-00	4-07-14	2 x 4	1-03-08	3-06-04 4-07-14	52.73 38.00		
	5	T75 Half Hip	9 / 12	2-09-00	5-07-00	2 x 4	1-03-08	1-06-04 3-07-00	89.86 64.17		
	14	PB51 Piggyback	8 / 12	7-10-00	2-07-05	2 x 4			250.73 158.67		
	1 2-ply	PB51Z Piggyback	8 / 12	7-10-00	2-07-05	2 x 4			35.82 22.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: C OPT (4 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425143
 Ref #
 Page: 3 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	8	J51 Jack-Open	8 / 12	5-10-08	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	152.71 93.33		
	5	J51S Jack-Open	8 / 12	5-10-08	5-03-14	2 x 4	1-03-08	1-04-14 4-03-14	114.36 73.33		
	10	J52 Jack-Open	8 / 12	3-10-08	3-11-14	2 x 4	1-03-08	1-04-14 3-11-14	140.68 90.00		
	3	J53 Jack-Open	8 / 12	3-10-08	4-11-14	2 x 4	1-03-08	2-04-14 4-11-14	47.5 30.50		
	5	J54 Jack-Open	12 / 12	1-10-08	3-09-00	2 x 4	1-03-08	1-10-08 3-09-00	54 35.00		
	4	C51 Jack-Open	8 / 12	1-09-07	2-07-03	2 x 4	1-03-08 1-01	1-04-14 2-07-03	35.7 22.67		
	1	C52 Jack-Open	8 / 12	1-10-08	3-11-03	2 x 4	1-03-08 1-10-15	1-04-14 2-07-14	11.67 7.67		
	1	C53 Jack-Open	8 / 12	1-09-07	2-07-03	2 x 4	1-03-08 4-01-01	1-04-14 2-07-03	13.49 8.33		
	1	C54 Jack-Open	8 / 12	3-09-07	3-11-03	2 x 4	1-03-08 2-01-01	1-04-14 3-11-03	16.23 10.33		
	3	C55 Jack-Open	8 / 12	1-09-07	2-07-03	2 x 4	1-03-08 2-01-01	1-04-14 2-07-03	33.62 21.00		

TOTAL # TRUSS= 108

TOTAL BFT OF ALL TRUSSES= 4021.66

BFT.

TOTAL WEIGHT OF ALL TRSSES 6431.04 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
16	Hardware	LUS24	
17	Hardware	LJS26DS	
1	Hardware	LUS26-2	
3	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 37

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: C OPT (5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425144
 Ref #
 Page: 1 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T51S1 Hip Girder	8 /12	30-10-00	5-04-02	2 x 6	1-03-08 1-03-08	1-04-14 1-06-04	389.15 243.33		
	1	T52S1 Hip	8 /12	30-10-00	6-07-14	2 x 4	1-03-08 1-03-08	1-04-14 1-06-04	146.29 95.33		
	1	T53S1 Hip	8 /12	30-10-00	7-11-14	2 x 4	1-03-08 1-03-08	1-04-14 1-06-04	157.81 100.83		
	1	T54S1 Hip	8 /12	30-10-00	9-03-14	2 x 4	1-03-08 1-03-08	1-04-14 1-06-04	162.49 102.83		
	1 2-ply	T55S1 Hip Girder	8 /12	30-10-00	10-03-02	2 x 6	1-03-08 1-03-08	1-04-14 1-06-04	489.17 298.00		
	1 2-ply	T56 Hip Girder	8 /12	30-10-00	7-04-15	2 x 6	1-03-08 1-03-08	1-04-14 1-04-14	436.58 272.00		
	1	T57 Hip	8 /12	30-10-00	10-06-09	2 x 4	1-03-08 1-03-08	1-04-14 8-14	143.57 89.50		
	4	T58 Piggyback Base	8 /12	29-10-00	8-08-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	538.45 330.67		
	4	T58X Hip	8 /12	30-10-00	9-02-09	2 x 4	1-03-08 1-03-08	1-04-14 8-14	554.63 340.00		
	1 2-ply	T59 Piggyback Base Girder	8 /12	29-10-00	8-08-15	2 x 6	1-03-08 1-03-08	1-04-14 1-04-14	441.85 268.00		
	6	T60 Piggyback Base	8 /12	22-10-00	8-08-14	2 x 4	1-03-08	1-04-14 6-00-14	674.08 415.00		
	1	T60S Piggyback Base	8 /12	22-10-00	8-08-14	2 x 4	1-03-08	1-04-14 6-00-14	130.7 83.50		
	1	T61 Half Hip Girder	8 /12	18-07-00	4-11-14	2 x 4 2 x 6	1-03-08	1-04-14 4-11-14	92.41 57.83		
	1	T62 Half Hip	8 /12	18-07-00	6-03-14	2 x 4	1-03-08	1-04-14 6-03-14	83.56 51.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: C OPT (5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425144
 Ref #
 Page: 2 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T63 Common Girder	8 /12	14-10-00	8-04-03	2 x 4 2 x 6		3-04-14 3-04-14	204.61 129.33		
	1	T64 Common	8 /12	14-10-00	8-04-03	2 x 4		3-04-14 3-04-14	71.28 45.33		
	1	T65 Hip	8 /12	14-10-00	7-01-00	2 x 4		3-04-14 3-04-14	69.9 44.00		
	1	T66 Hip Girder	8 /12	14-10-00	5-07-00	2 x 4 2 x 6		3-04-14 3-04-14	80.32 52.17		
	1	T67 Hip Girder	8 /12	13-00-00	3-11-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	55.99 36.33		
	1	T68 Half Hip Girder	8 /12	13-00-00	3-11-14	2 x 4	1-03-08	1-04-14 3-11-14	54.9 35.33		
	2	T69 Hip	8 /12	13-00-00	5-03-03	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	118.21 73.67		
	1	T70 Hip Girder	8 /12	11-02-00	3-09-00	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	49.87 32.67		
	1	T71 Common	8 /12	11-02-00	5-01-09	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	46.45 29.83		
	1 2-ply	T72 Monopitch Girder	8 /12	7-06-00	7-04-14	2 x 4 2 x 6		2-04-14 7-04-14	88.71 56.33		
	1 2-ply	T73 Monopitch Girder	8 /12	3-10-08	4-11-14	2 x 4 2 x 6		2-04-14 4-11-14	44.68 28.00		
	2	T74 Half Hip	9 /12	2-09-00	4-07-14	2 x 4	1-03-08	3-06-04 4-07-14	52.73 38.00		
	5	T75 Half Hip	9 /12	2-09-00	5-07-00	2 x 4	1-03-08	1-06-04 3-07-00	89.86 64.17		
	1	T76 Half Hip Girder	8 /12	5-10-08	2-04-14	2 x 4	1-03-08	1-04-14 2-04-14	25.14 16.50		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE ESTATES
 Location: BRAMPTON
 Model: UNIT 4502 - COR
 Lot #:
 Elevation: C OPT (5 BED)

Job Track: 52647
 PlanLog: 205911
 Layout ID: 425144
 Ref #
 Page: 3 of 3
 Date: 07-21-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	14	PB51 Piggyback	8 /12	7-10-00	2-07-05	2 x 4			250.73 158.67		
	1 2-ply	PB51Z Piggyback	8 /12	7-10-00	2-07-05	2 x 4			35.82 22.67		
	9	J51 Jack-Open	8 /12	5-10-08	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	171.8 105.00		
	5	J51S Jack-Open	8 /12	5-10-08	5-03-14	2 x 4	1-03-08	1-04-14 4-03-14	114.36 73.33		
	10	J52 Jack-Open	8 /12	3-10-08	3-11-14	2 x 4	1-03-08	1-04-14 3-11-14	140.68 90.00		
	3	J53 Jack-Open	8 /12	3-10-08	4-11-14	2 x 4	1-03-08	2-04-14 4-11-14	47.5 30.50		
	5	J54 Jack-Open	12 /12	1-10-08	3-09-00	2 x 4	1-03-08	1-10-08 3-09-00	54 35.00		
	3	J55 Jack-Open	8 /12	1-09-07	2-07-03	2 x 4	1-03-08 1-01	1-04-14 2-07-03	26.77 17.00		
	3	C51 Jack-Open	8 /12	1-09-07	2-07-03	2 x 4	1-03-08 1-01	1-04-14 2-07-03	26.77 17.00		
	3	C55 Jack-Open	8 /12	1-09-07	2-07-03	2 x 4	1-03-08 2-01-01	1-04-14 2-07-03	33.62 21.00		

TOTAL # TRUSS= 109

TOTAL BFT OF ALL TRUSSES= 4000.32

BFT.

TOTAL WEIGHT OF ALL TRSSES 6395.43 LBS

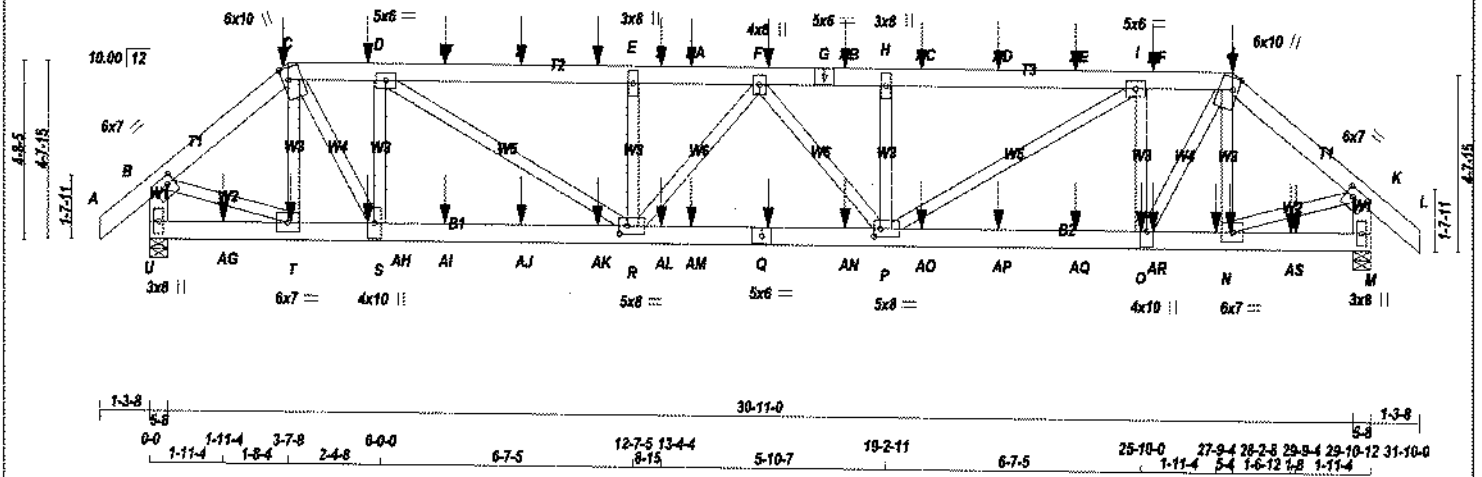
HARDWARE

QTY	TYPE	MODEL	LENGTH
17	Hardware	LUS24	
17	Hardware	LJS26DS	
1	Hardware	LUS26-2	
3	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 38

JOB NAME 425136	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 M/Tek Industries, Inc. Wed Jul 20 09:58:08 2022 Page 1
 ID:00V9YTQ5oz7QIFCPkyhhuza06Z-qkoFY0Yn0AG063 bK9DAQWvdv0FumE Ye.WdHywDUD
 1-3-8 0-0 3-7-8 3-7-8 5-8-4 6-0-0 12-7-5 13-4-4 15-11-0 19-2-11 6-7-5 25-10-0 28-2-8 3-7-8 31-10-0 33-1-8
 1-3-8 1-3-8 3-7-8 2-9-12 3-12 6-7-5 8-15 2-6-12 3-3-11 6-7-5 2-4-8 28-2-8 3-7-8 31-10-0 33-1-8
 Scale = 1:58.2



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x6	DRY	No.2
C - G	2x6	DRY	No.2
G - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
Q - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-G	2	12
G-J	2	12
J-L	2	12
U-B	2	12
M-K	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U-Q	2	12
Q-M	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6
D-S	1	2
I-O	1	2

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
 GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
 TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
U	3356	0	3356	0	5-8
M	6516	0	6516	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
U	2399	1553 / 0	0 / 0
M	4613	2999 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.01 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS		MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (PLF)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 39	-84.9	-84.9	0.03 (1)	10.00	T-C	-716 / 0
B-C	-3356 / 0	-84.9	-84.9	0.07 (1)	5.90	C-S	0 / 2949
C-V	-4139 / 0	-84.9	-84.9	0.15 (1)	5.47	S-D	-2427 / 0
V-D	-4139 / 0	-84.9	-84.9	0.15 (1)	5.47	O-I	-879 / 0
D-W	-6989 / 0	-84.9	-84.9	0.24 (1)	4.37	O-J	0 / 5339
W-X	-6989 / 0	-84.9	-84.9	0.24 (1)	4.37	N-J	-679 / 0
X-Y	-6989 / 0	-84.9	-84.9	0.24 (1)	4.37	B-T	0 / 2822
Y-E	-6989 / 0	-84.9	-84.9	0.24 (1)	4.37	N-K	0 / 5581
E-Z	-6989 / 0	-84.9	-84.9	0.18 (1)	4.41	P-I	0 / 386
Z-AA	-6989 / 0	-84.9	-84.9	0.18 (1)	4.41	D-R	0 / 3375
AA-F	-6989 / 0	-84.9	-84.9	0.18 (1)	4.41	P-H	-711 / 0
F-G	-8298 / 0	-84.9	-84.9	0.23 (1)	4.06	R-E	-715 / 0
G-AA	-8298 / 0	-84.9	-84.9	0.23 (1)	4.06	R-F	-1194 / 0
AB-H	-8298 / 0	-84.9	-84.9	0.23 (1)	4.06	F-P	0 / 923
H-AC	-8298 / 0	-84.9	-84.9	0.30 (1)	4.01		
AC-AD	-8298 / 0	-84.9	-84.9	0.30 (1)	4.01		
AD-AE	-8298 / 0	-84.9	-84.9	0.30 (1)	4.01		
AE-I	-8298 / 0	-84.9	-84.9	0.30 (1)	4.01		
I-AF	-7971 / 0	-84.9	-84.9	0.17 (1)	4.19		
AF-J	-7971 / 0	-84.9	-84.9	0.17 (1)	4.19		
J-K	-7000 / 0	-84.9	-84.9	0.12 (1)	4.46		
K-L	0 / 39	-84.9	-84.9	0.03 (1)	10.00		
U-B	-3356 / 0	0.0	0.0	0.12 (1)	7.60		
M-K	-6368 / 0	0.0	0.0	0.23 (1)	5.88		
U-AG	0 / 0	-18.5	-18.5	0.02 (4)	10.00		
AG-T	0 / 0	-18.5	-18.5	0.02 (4)	10.00		
T-AH	0 / 2681	-18.5	-18.5	0.20 (1)	10.00		
AH-S	0 / 2681	-18.5	-18.5	0.20 (1)	10.00		
S-AI	0 / 4141	-18.5	-18.5	0.31 (1)	10.00		
AI-AJ	0 / 4141	-18.5	-18.5	0.31 (1)	10.00		
AJ-AK	0 / 4141	-18.5	-18.5	0.31 (1)	10.00		
AK-R	0 / 4141	-18.5	-18.5	0.31 (1)	10.00		
R-AL	0 / 7727	-18.5	-18.5	0.55 (1)	10.00		
AL-AM	0 / 7727	-18.5	-18.5	0.55 (1)	10.00		
AM-Q	0 / 7727	-18.5	-18.5	0.55 (1)	10.00		
Q-AN	0 / 7727	-18.5	-18.5	0.55 (1)	10.00		
AN-P	0 / 7727	-18.5	-18.5	0.55 (1)	10.00		
P-AO	0 / 7872	-18.5	-18.5	0.80 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2015, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.26")
 CS1: TC=0.30/1.00 (H-I), BC=0.60/1.00 (O-P), WS=0.49/1.00 (K-N), SSI=0.14/1.00 (H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

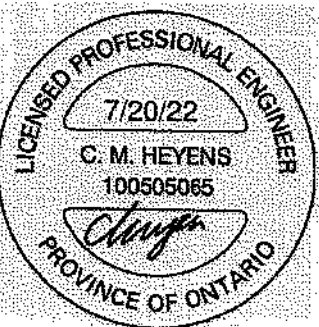
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
 JSI METAL= 0.71 (Q) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
 DWG # TR22070549 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 6.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 09:58:08 2022 Page 2
ID: c0V9YTq5oz7QIFCPkyhtuzaQ6Z-gkciFY0Yn0AG063_bK9DAcWYdv0FumE_YeJWdHwOUD

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	6.0	7.0	2.50	2.25
C	TTWW+m	MT20	6.0	10.0	3.75	1.75
D	TMVW-t	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	8.0		
F	TMVW-t	MT20	4.0	6.0		
G	TS-I	MT20	5.0	6.0		
H	TMVW-w	MT20	3.0	8.0		
I	TMVW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	6.0	10.0	3.75	1.75
K	TMVW-I	MT20	6.0	7.0	2.50	2.25
M	BMV1+p	MT20	3.0	8.0		
N	BMVW-t	MT20	6.0	7.0		
O	BMVW-t	MT20	4.0	10.0		
P	BMVWVW-t	MT20	5.0	8.0	2.50	2.00
Q	BS-I	MT20	5.0	6.0		
R	BMVWVW-t	MT20	5.0	8.0	2.50	2.50
S	BMVW-t	MT20	4.0	10.0		
T	BMVW-t	MT20	6.0	7.0		
U	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

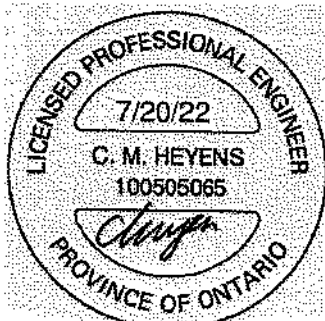
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX (LBS)	MAX. UNBRACED LENGTH (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX (LBS)
FR-TO		FROM	TO		FR-TO		
AC-AP	0 / 7972	-18.5	-18.5	0.60 (1)	10.00		
AP-AQ	0 / 7972	-18.5	-18.5	0.60 (1)	10.00		
AQ-O	0 / 7972	-18.5	-18.5	0.60 (1)	10.00		
O-AR	0 / 5329	-18.5	-18.5	0.43 (1)	10.00		
AR-N	0 / 5329	-18.5	-18.5	0.43 (1)	10.00		
N-AS	0 / 0	-18.5	-18.5	0.10 (1)	10.00		
AS-M	0 / 0	-18.5	-18.5	0.10 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-7-8	-170	-170		FRONT	VERT	TOTAL		C1
F	16-1-12	-48	-48		FRONT	VERT	TOTAL		C1
J	28-2-8	-170	-170		FRONT	VERT	TOTAL		C1
N	27-9-4	-263	-263		BACK	VERT	TOTAL		C1
N	28-1-12	-12	-12		FRONT	VERT	TOTAL		C1
O	25-10-0	-2876	-2876		BACK	VERT	TOTAL		C1
Q	16-1-12	-12	-12		FRONT	VERT	TOTAL		C1
T	3-8-4	-12	-12		FRONT	VERT	TOTAL		C1
V	8-8-4	-48	-48		FRONT	VERT	TOTAL		C1
W	7-8-4	-48	-48		FRONT	VERT	TOTAL		C1
X	8-8-4	-48	-48		FRONT	VERT	TOTAL		C1
Y	11-8-4	-48	-48		FRONT	VERT	TOTAL		C1
Z	13-4-4	-48	-48		FRONT	VERT	TOTAL		C1
AA	14-1-12	-48	-48		FRONT	VERT	TOTAL		C1
AB	18-1-12	-48	-48		FRONT	VERT	TOTAL		C1
AC	20-1-12	-48	-48		FRONT	VERT	TOTAL		C1
AD	22-1-12	-48	-48		FRONT	VERT	TOTAL		C1
AE	24-1-12	-48	-48		FRONT	VERT	TOTAL		C1
AF	26-1-12	-48	-48		FRONT	VERT	TOTAL		C1
AG	1-11-4	-12	-12		FRONT	VERT	TOTAL		C1
AH	5-8-4	-12	-12		FRONT	VERT	TOTAL		C1
AI	7-8-4	-12	-12		FRONT	VERT	TOTAL		C1
AJ	9-8-4	-12	-12		FRONT	VERT	TOTAL		C1
AK	11-8-4	-12	-12		FRONT	VERT	TOTAL		C1
AL	13-4-4	-12	-12		FRONT	VERT	TOTAL		C1
AM	14-1-12	-12	-12		FRONT	VERT	TOTAL		C1
AN	18-1-12	-12	-12		FRONT	VERT	TOTAL		C1
AO	20-1-12	-12	-12		FRONT	VERT	TOTAL		C1
AP	22-1-12	-12	-12		FRONT	VERT	TOTAL		C1
AQ	24-1-12	-12	-12		FRONT	VERT	TOTAL		C1
AR	26-1-12	-12	-12		FRONT	VERT	TOTAL		C1
AS	29-9-4	-263	-263		BACK	VERT	TOTAL		C1
AS	29-10-12	-12	-12		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22070549 PG 2/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425137	T1S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:04:37 2022 Page 2
ID:0V9YTq5oz7QIFCPkynhtuza06Z-xacpuZYAXA3AultXeSRs7MujkSXIGF7f1bnEjwDO8

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	8.0	9.0	Edge	2.75
D	TMVW-t	MT20	5.0	6.0		
E	TMVW+w	MT20	3.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW+p	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	6.0	10.0	3.75	1.75
J	TMVW-t	MT20	6.0	7.0	2.50	2.25
L	BMV1+p	MT20	3.0	8.0		
M	BMVW-t	MT20	6.0	7.0		
N	BMVWW-t	MT20	6.0	10.0	2.50	3.50
O	BMV+p	MT20	3.0	8.0		
P	BMVWW-t	MT20	10.0	12.0	5.50	7.25
Q	BMVWW-t	MT20	5.0	8.0	2.50	2.50
R	BMVWW-t	MT20	10.0	12.0	5.50	7.50
S	BMV+p	MT20	3.0	8.0		
T	BMVWW-t	MT20	6.0	10.0		
U	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

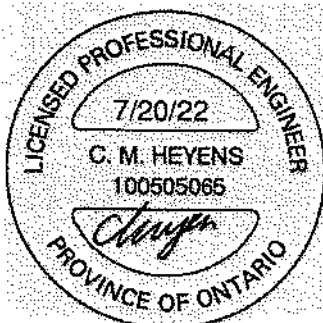
CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MEMB.	FORCE (LBS)	CS1 (LC)	CS1 (LC)
FR-TO		FROM	TO	FR-TO		LENGTH	
AA-AN	0 / 273	-18.5	-18.5 0.09 (1)	10.00			
AN-AO	0 / 273	-18.5	-18.5 0.09 (1)	10.00			
AO-N	0 / 273	-18.5	-18.5 0.09 (1)	10.00			
N-AP	0 / 5285	-18.5	-18.5 0.42 (1)	10.00			
AP-M	0 / 5285	-18.5	-18.5 0.42 (1)	10.00			
MAQ	0 / 0	-18.5	-18.5 0.10 (1)	10.00			
AO-L	0 / 0	-18.5	-18.5 0.10 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-7-8	-170	-170	---	FRONT	VERT	TOTAL	---	C1
D	7-8-4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
E	13-4-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
I	28-2-8	-170	-170	---	FRONT	VERT	TOTAL	---	C1
M	27-9-4	-263	-263	---	BACK	VERT	TOTAL	---	C1
M	28-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
N	25-10-0	-2876	-2876	---	BACK	VERT	TOTAL	---	C1
Q	13-4-4	-48	-48	---	TOP	VERT	TOTAL	---	C1
S	7-10-12	-18	-18	---	FRONT	VERT	TOTAL	---	C1
T	3-8-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
V	5-8-4	-48	-48	---	FRONT	VERT	TOTAL	---	C1
W	9-8-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
X	11-8-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
Y	14-1-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
Z	18-1-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
AA	18-1-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
AB	20-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AC	22-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AD	24-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AE	26-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AF	1-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AG	5-8-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AH	9-8-4	-48	-48	---	TOP	VERT	TOTAL	---	C1
AI	11-8-4	-48	-48	---	TOP	VERT	TOTAL	---	C1
AJ	14-1-12	-48	-48	---	TOP	VERT	TOTAL	---	C1
AK	16-1-12	-48	-48	---	TOP	VERT	TOTAL	---	C1
AL	18-1-12	-48	-48	---	TOP	VERT	TOTAL	---	C1
AM	20-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AN	22-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AO	24-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AP	26-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AQ	28-9-4	-263	-263	---	BACK	VERT	TOTAL	---	C1
AQ	29-10-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

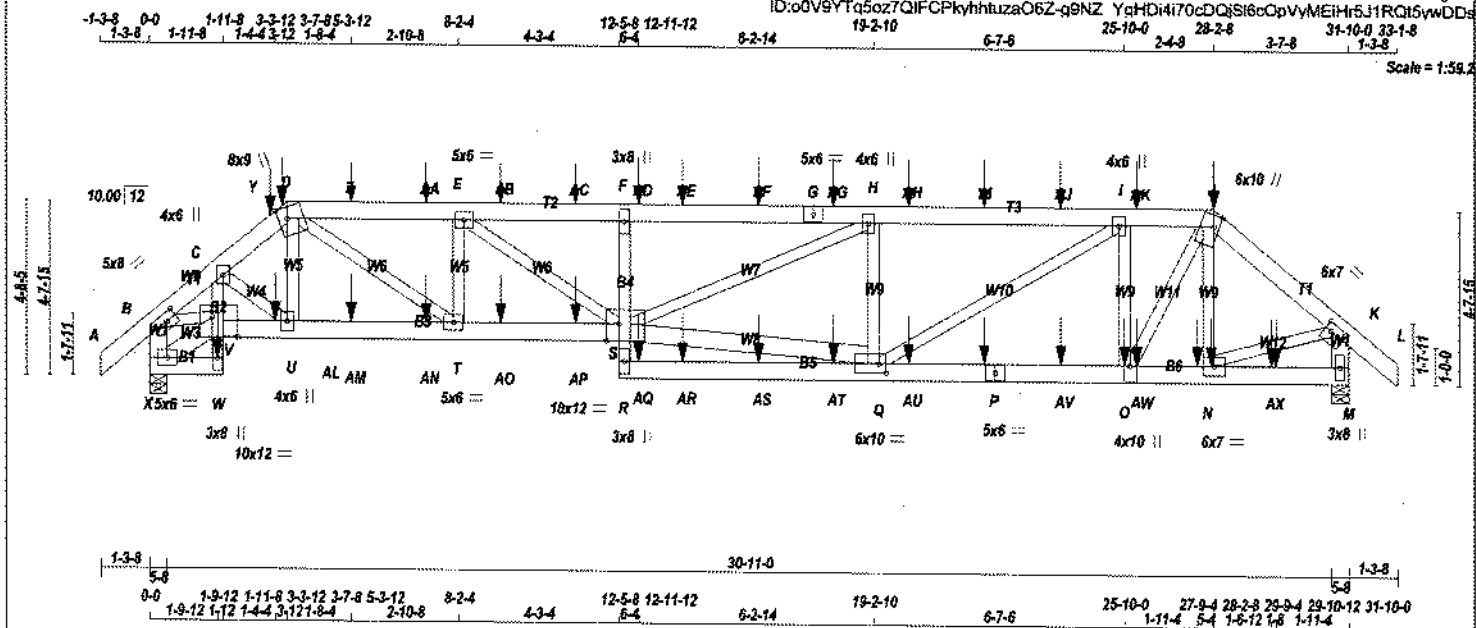


STRUCTURAL COMPONENT ONLY
DWG # TR22070586 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425138	T1S1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:15:35 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhtuza06Z-g9NZ YgHDI4i70cDQjSi6cOpVvMEIH5J1RQI5vwDDs



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF	
D - G	2x6	DRY	No.2	SPF	
G - J	2x6	DRY	No.2	SPF	
J - L	2x6	DRY	No.2	SPF	
X - B	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
X - W	2x6	DRY	No.2	SPF	
W - C	2x4	DRY	No.2	SPF	
V - S	2x4	DRY	No.2	SPF	
R - F	2x4	DRY	No.2	SPF	
P - P	2x6	DRY	No.2	SPF	
P - M	2x6	DRY	No.2	SPF	

ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
S - Q	2x6	DRY	No.2	SPF
X - V	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	2	12
D-G	2	12
G-J	2	12
J-L	2	12
X-B	2	12
M-K	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X-W	2	12
V-S	2	12
R-P	2	12
P-M	2	12
W-C	1	12
F-R	1	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6
1x6	1	2
2x6	2	6

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
M	6499	0	0	5-8
X	3469	0	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
M	4601	2990 / 0
X	2456	1598 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, X

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MAX. FACTORED		FACTORED	MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)	
FR-TO		FROM TO	FR-TO		
A-B	0 / 39	-84.9	C-U	-315 / 0	
B-C	-4931 / 0	-84.9	U-D	0 / 199	
C-Y	-4537 / 0	-84.9	D-T	0 / 3860	
Y-D	-4537 / 0	-84.9	T-E	-2014 / 0	
D-Z	-6703 / 0	-84.9	E-S	0 / 2674	
Z-AA	-6703 / 0	-84.9	S-Q	0 / 8174	
AA-E	-6703 / 0	-84.9	Q-H	0 / 888	
E-AB	-9030 / 0	-84.9	H-I	-1207 / 0	
AB-AC	-9030 / 0	-84.9	I-J	0 / 434	
AC-F	-9030 / 0	-84.9	J-I	-642 / 0	
F-AD	-8952 / 0	-84.9	O-J	0 / 5283	
AD-AE	-8952 / 0	-84.9	N-K	-889 / 0	
AE-AF	-8952 / 0	-84.9	K-J	0 / 5666	
AF-G	-8952 / 0	-84.9	X-V	-105 / 0	
G-AG	-8952 / 0	-84.9	B-V	0 / 3698	
AG-H	-8952 / 0	-84.9			
H-AH	-8294 / 0	-84.9			
AH-AI	-8294 / 0	-84.9			
AI-AJ	-8294 / 0	-84.9			
AJ-I	-8294 / 0	-84.9			
IAK	-7929 / 0	-84.9			
AK-J	-7929 / 0	-84.9			
J-K	-6981 / 0	-84.9			
K-L	0 / 39	-84.9			
X-B	-3368 / 0	0.0			
M-K	-6351 / 0	0.0			
X-W	0 / 80	-18.5			
W-V	0 / 188	0.0			
V-C	0 / 441	0.0			
U-L	0 / 3769	-18.5			
AL-U	0 / 3769	-18.5			
U-AM	0 / 3533	-18.5			
AM-AN	0 / 3533	-18.5			
AN-T	0 / 3533	-18.5			
T-AO	0 / 6703	-18.5			
AO-AP	0 / 6703	-18.5			
AP-S	0 / 6703	-18.5			
R-S	0 / 97	0.0			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEFL. (LL) = L/999 (0.17")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEFL. (TL) = L/999 (0.32")

CSF: TC=0.35/1.00 (F-H-1), BC=0.80/1.00 (D-Q-1)
WB=0.53/1.00 (Q-S-1), SS=0.15/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

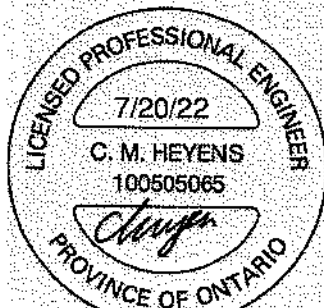
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION-
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070591 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425138	T1S1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:15:35 2022 Page 2
ID:00V9YTg5oz7QIFCPkyhttuzaC6Z-q9NZ_YgHDI4i70cDQISi6cOpVyMEHr5J1RQI5wvDDd

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	8.0	2.50	3.25
C	TMVW+p	MT20	4.0	6.0		
D	TTWW+m	MT20	8.0	9.0	Edge	2.75
E	TMVW-1	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	8.0		
G	TS-1	MT20	5.0	6.0		
H	TMVW+1	MT20	4.0	6.0		
I	TMVW+1	MT20	4.0	6.0		
J	TTWW+m	MT20	6.0	10.0	3.75	1.75
K	TMVW-1	MT20	6.0	7.0	2.50	2.25
M	BMV1+p	MT20	3.0	8.0		
N	BMVW-1	MT20	6.0	7.0		
O	BMVW+1	MT20	4.0	10.0		
P	BS-1	MT20	5.0	6.0		
Q	BMVW+1	MT20	6.0	10.0	3.00	2.25
R	BMV+p	MT20	3.0	8.0		
S	BMVW+1	MT20	10.0	12.0	Edge	4.00
T	BMVW-1	MT20	5.0	6.0		
U	BMVW+1	MT20	4.0	6.0		
V	BMVW-1	MT20	10.0	12.0	6.50	8.00
W	BMV+p	MT20	3.0	8.0		
X	BMVW-1	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
S-F	-589 / 0	0.0	0.0	0.19 (1)		7.81	
R-AQ	0 / 249	-18.5	-18.5	0.08 (4)		10.00	
AD-AR	0 / 249	-18.5	-18.5	0.08 (4)		10.00	
AR-AS	0 / 249	-18.5	-18.5	0.08 (4)		10.00	
AS-AT	0 / 249	-18.5	-18.5	0.08 (4)		10.00	
AT-AQ	0 / 249	-18.5	-18.5	0.08 (4)		10.00	
Q-AU	0 / 7930	-18.5	-18.5	0.60 (1)		10.00	
AU-P	0 / 7930	-18.5	-18.5	0.60 (1)		10.00	
P-AV	0 / 7930	-18.5	-18.5	0.60 (1)		10.00	
AV-O	0 / 7930	-18.5	-18.5	0.60 (1)		10.00	
O-AW	0 / 5315	-18.5	-18.5	0.43 (1)		10.00	
AW-N	0 / 5315	-18.5	-18.5	0.43 (1)		10.00	
N-AX	0 / 0	-18.5	-18.5	0.10 (1)		10.00	
AX-M	0 / 0	-18.5	-18.5	0.10 (1)		10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	3-7-8	-20	-20	---	FRONT	VERT	DEAD	---	C1
D	3-7-8	-74	-74	---	FRONT	VERT	SNOW	---	C1
J	28-2-8	-170	-170	---	FRONT	VERT	TOTAL	---	C1
N	27-9-4	-263	-263	---	BACK	VERT	TOTAL	---	C1
N	28-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
O	25-10-0	-2876	-2876	---	BACK	VERT	TOTAL	---	C1
P	22-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
W	1-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
Y	3-3-12	-16	-16	---	FRONT	VERT	TOTAL	---	C1
Z	5-3-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
AA	7-3-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
AB	9-3-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
AC	11-3-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
AD	12-11-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AE	14-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AF	16-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AG	18-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AH	20-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AI	22-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AJ	24-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AK	26-1-12	-48	-48	---	FRONT	VERT	TOTAL	---	C1
AL	3-3-12	-48	-48	---	TOP	VERT	TOTAL	---	C1
AM	5-3-12	-48	-48	---	TOP	VERT	TOTAL	---	C1
AN	7-3-12	-48	-48	---	TOP	VERT	TOTAL	---	C1
AO	9-3-12	-48	-48	---	TOP	VERT	TOTAL	---	C1
AP	11-3-12	-48	-48	---	TOP	VERT	TOTAL	---	C1
AQ	12-11-12	-17	-17	---	FRONT	VERT	TOTAL	---	C1
AR	14-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AS	16-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AT	18-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AU	20-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AV	24-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AW	26-1-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1
AX	29-9-4	-263	-263	---	BACK	VERT	TOTAL	---	C1
AX	29-10-12	-12	-12	---	FRONT	VERT	TOTAL	---	C1

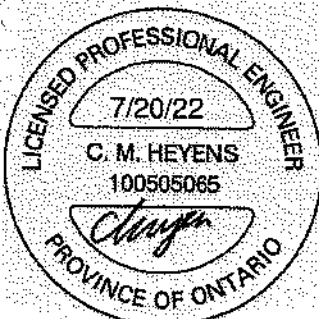
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

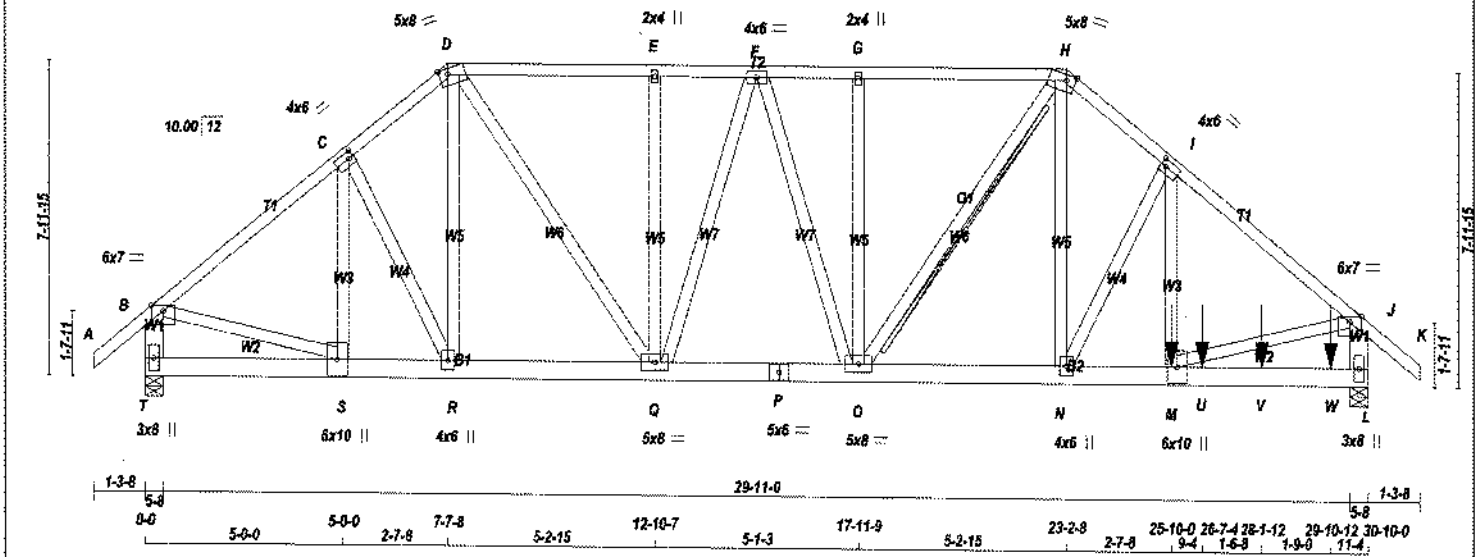
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (K) (INPUT = 0.90)
JSI METAL = 0.75 (P) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070591 PG 2/2



TOTAL WEIGHT = 2 X 210 = 419 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
A - O	2x4	DRY	No.2	SPF	FACTORED	GROSS REACTION	MAXIMUM FACTORED
D - H	2x4	DRY	No.2	SPF	VERT	DOWN	DOWN
H - K	2x4	DRY	No.2	SPF	HORZ	HORZ	HORZ
T - B	2x6	DRY	No.2	SPF	UPLIFT	UPLIFT	UPLIFT
L - J	2x6	DRY	No.2	SPF	IN-SX	IN-SX	IN-SX
T - P	2x6	DRY	No.2	SPF	IN-SX	IN-SX	IN-SX
P - L	2x6	DRY	No.2	SPF	IN-SX	IN-SX	IN-SX

ALL WEBS EXCEPT				UNFACTORED REACTIONS			
2x4	DRY	No.2	SPF	1ST CASE	MAX. MIN. COMPONENT REACTIONS		
				COMBINED	SNOW	LIVE	PERM. LIVE
				1786	1160 / 0	0 / 0	0 / 0
				4443	2933 / 0	0 / 0	0 / 0

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-D	12	TOP
D-H	12	TOP
H-K	12	TOP
T-B	12	TOP
L-J	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
T-P	12	TOP
P-L	12	TOP

WEBS: (0.122"x3") SPIRAL NAILS
I-M 1 3
2x4 1 3
R-C 1 6
R-D 1 6
N-H 1 6
N-I 1 6
S-B 1 6
M-J 1 6
O-H 1 6
Q-D 1 6
Q-E 1 6
Q-F 1 6
F-O 1 6

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF
SPACING = 24.0 IN. C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03')
CALCULATED VERT. DEFL. (LL) = L/999 (0.07')
ALLOWABLE DEFL. (TL) = L/360 (1.03')
CALCULATED VERT. DEFL. (TL) = L/999 (0.12')
CSI: TC=0.46/1.00 (L-J), BC=0.35/1.00 (M-N), WB=0.77/1.00 (I-N), SSI=0.16/1.00 (L-M)
DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

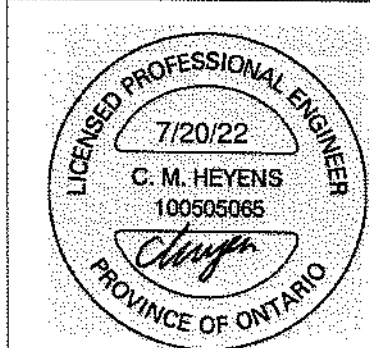
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM TO		FR-TO			

A-B	0/38	-84.9	-84.9	0.08 (1)	10.00	S-C	-455/0	0.08 (1)
B-C	-2607/0	-84.9	-84.9	0.19 (1)	5.45	C-R	0/8	0.00 (4)
C-D	-2690/0	-84.9	-84.9	0.17 (1)	5.38	R-D	0/104	0.01 (4)
D-E	-2986/0	-84.9	-84.9	0.21 (1)	5.15	N-H	0/3053	0.27 (1)
E-F	-2986/0	-84.9	-84.9	0.19 (1)	5.15	N-I	-3331/0	0.77 (1)
F-G	-3512/0	-84.9	-84.9	0.19 (1)	4.81	M-I	0/3802	0.34 (1)
G-H	-3512/0	-84.9	-84.9	0.22 (1)	4.81	S-S	0/2080	0.18 (1)
H-I	-4777/0	-84.9	-84.9	0.18 (1)	4.28	M-J	0/5305	0.47 (1)
I-J	-6692/0	-84.9	-84.9	0.46 (1)	3.53	O-H	-312/0	0.11 (1)
J-K	0/38	-84.9	-84.9	0.06 (1)	10.00	D-Q	0/1656	0.15 (1)
T-B	-2471/0	0.0	0.0	0.09 (1)	7.81	O-G	-445/0	0.18 (1)
L-J	-5858/0	0.0	0.0	0.22 (1)	6.10	Q-E	-451/0	0.19 (1)
						Q-F	-877/0	0.42 (1)
						F-O	0/843	0.07 (1)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM TO		FR-TO			

T-S	0/0	-18.5	-18.5	0.03 (4)	10.00		
S-R	0/2021	-18.5	-18.5	0.15 (1)	10.00		
R-Q	0/2026	-18.5	-18.5	0.15 (1)	10.00		
Q-P	0/3244	-18.5	-18.5	0.23 (1)	10.00		
P-O	0/3244	-18.5	-18.5	0.23 (1)	10.00		
O-N	0/3889	-18.5	-18.5	0.26 (1)	10.00		
N-M	0/5154	-18.5	-18.5	0.35 (1)	10.00		
M-U	0/0	-18.5	-18.5	0.24 (1)	10.00		
U-V	0/0	-18.5	-18.5	0.24 (1)	10.00		
V-W	0/0	-18.5	-18.5	0.24 (1)	10.00		
W-L	0/0	-18.5	-18.5	0.24 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX. MAX+	FACE	DIR.	TYPE	HEEL
M	25-10-0	-3228	-3228	FRONT	VERT	TOTAL	C1
U	26-7-4	-199	-199	TOP	VERT	TOTAL	C1
V	28-1-12	-199	-199	TOP	VERT	TOTAL	C1



STRUCTURAL COMPONENT ONLY
DWG # TR22070553 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T2	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 09:58:40 2022 Page 2
ID:c0V9YTg5oz7QIFCPkyhhtuza06Z-uuYszUOcFrCj1W8T2kTZrKza?bOBvive3CIQvwDTT

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	6.0	7.0	Edge	
C	TMWV-t	MT20	4.0	6.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	1.75	2.75
E	TMWV-w	MT20	2.0	4.0		
F	TMWV-t	MT20	4.0	6.0		
G	TMWV-w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	1.75	2.75
I	TMWV-t	MT20	4.0	6.0	2.00	1.50
J	TMWV-p	MT20	6.0	7.0	Edge	
L	BMV1-p	MT20	3.0	8.0		
M	BMWV-t	MT20	6.0	10.0		
N	BMWV-t	MT20	4.0	6.0		
O	BMWV-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWV-t	MT20	5.0	8.0		
R	BMWV-t	MT20	4.0	6.0		
S	BMWV-t	MT20	6.0	10.0		
T	BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

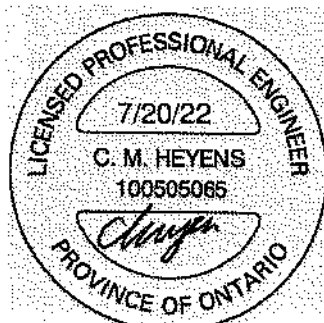
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	29-10-12	-177	-177	--	TOP	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22070553 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425138	T2S	1	2	TRUSS DESC.		

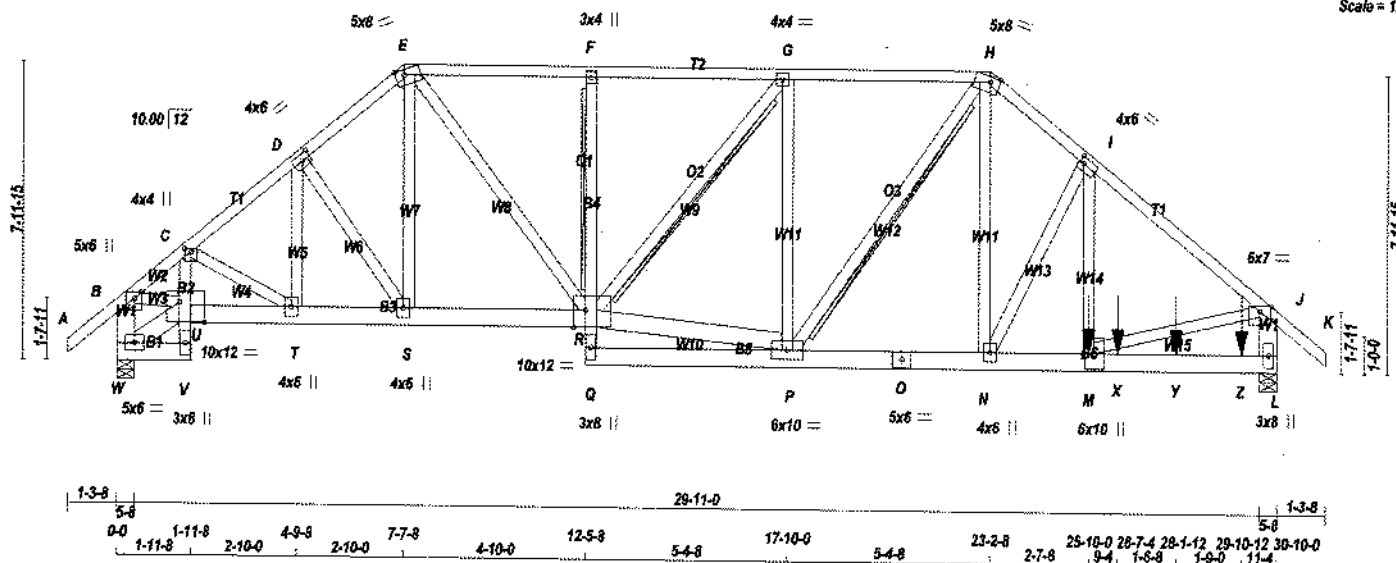
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:15:36 2022 Page 1

ID:00V9YTq5oz7QifCPkyhhtuza06Z-9MoxCuhw 0CZkABP Qz fqwvaMILRhPFxhAZPXyWDD

1-3-8 0-0 1-11-8 2-10-0 4-9-8 2-10-0 7-7-8 4-10-0 12-5-8 5-4-8 17-10-0 5-4-8 23-2-8 2-7-8 25-10-0 5-0-0 30-10-0 32-1-8 1-3-8

Scale = 1:59.3



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
I - K	2x4	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
M - N	2x6	DRY	No.2	SPF	
O - P	2x6	DRY	No.2	SPF	
Q - R	2x4	DRY	No.2	SPF	
S - T	2x4	DRY	No.2	SPF	
U - V	2x4	DRY	No.2	SPF	
W - X	2x6	DRY	No.2	SPF	
Y - Z	2x6	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
R - P	2x6	DRY	No.2	SPF	
W - U	2x6	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
SPACING (IN)		
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-E	12	TOP
F-H	12	TOP
I-K	12	TOP
L-J	12	TOP
M-N	12	TOP
O-P	12	TOP
Q-R	12	TOP
S-T	12	TOP
U-V	12	TOP
W-X	12	TOP
Y-Z	12	TOP
ALL WEBS : (0.122"x3") SPIRAL NAILS		
I-M	3	SIDE(183.1)
N-O	3	SIDE(183.1)
P-Q	3	SIDE(183.1)
R-S	3	SIDE(183.1)
T-U	3	SIDE(183.1)
V-W	3	SIDE(183.1)
X-Y	3	SIDE(183.1)
Z	3	SIDE(183.1)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT
L	6284	0	0
W	2525	0	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
L	4441	2932 / 0
W	1788	1162 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-R, G-R, H-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	FORCE (LBS)	FROM TO	PLF	CSI (LC)	UNBRAC	LENGTH	FORCE (LBS)
FR-TO								
A-B	0 / 38	-84.9	-84.9	0.06 (1)	10.00	C-T	-170 / 0	0.01 (1)
B-C	-3334 / 0	-84.9	-84.9	0.10 (1)	5.03	T-D	-45 / 24	0.01 (1)
C-D	-3157 / 0	-84.9	-84.9	0.09 (1)	5.15	D-S	-212 / 0	0.03 (1)
D-E	-3045 / 0	-84.9	-84.9	0.09 (1)	5.22	S-E	0 / 295	0.03 (1)
E-F	-3361 / 0	-84.9	-84.9	0.21 (1)	4.91	E-R	0 / 1722	0.15 (1)
F-G	-3355 / 0	-84.9	-84.9	0.25 (1)	4.86	R-P	0 / 3533	0.23 (1)
G-H	-3510 / 0	-84.9	-84.9	0.26 (1)	4.77	P-G	-287 / 0	0.08 (1)
H-I	-4772 / 0	-84.9	-84.9	0.18 (1)	4.27	G-H	-306 / 0	0.12 (1)
I-J	-5889 / 0	-84.9	-84.9	0.46 (1)	3.53	H-I	-306 / 0	0.11 (1)
J-K	0 / 38	-84.9	-84.9	0.06 (1)	10.00	N-H	0 / 3045	0.27 (1)
K-L	-2464 / 0	0.0	0.0	0.09 (1)	7.81	N-I	-3334 / 0	0.77 (1)
L-M	-5855 / 0	0.0	0.0	0.22 (1)	6.10	M-I	0 / 3806	0.34 (1)
M-N	0 / 70	-18.5	-18.5	0.01 (1)	10.00	N-J	0 / 5303	0.47 (1)
N-O	0 / 19	0.0	0.0	0.06 (1)	10.00	O-J	-83 / 0	0.09 (1)
O-P	0 / 46	0.0	0.0	0.06 (1)	10.00	P-U	0 / 2539	0.22 (1)
P-Q	0 / 2591	-18.5	-18.5	0.19 (1)	10.00	Q-U		
Q-R	0 / 2440	-18.5	-18.5	0.18 (1)	10.00			
R-S	0 / 2324	-18.5	-18.5	0.17 (1)	10.00			
S-T	0 / 43	0.0	0.0	0.03 (1)	10.00			
T-U	-448 / 0	0.0	0.0	0.04 (1)	7.81			
U-V	0 / 41	-18.5	-18.5	0.04 (1)	10.00			
V-W	0 / 3685	-18.5	-18.5	0.26 (1)	10.00			
W-X	0 / 3685	-18.5	-18.5	0.26 (1)	10.00			
X-Y	0 / 5151	-18.5	-18.5	0.35 (1)	10.00			
Y-Z	0 / 0	-18.5	-18.5	0.24 (1)	10.00			
Z-L	0 / 0	-18.5	-18.5	0.24 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.46/1.00 (I-J:1), BC=0.35/1.00 (M-N:1), WB=0.77/1.00 (N-N:1), SS=0.16/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.77 (M) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070592 PG 1/2

JOB NAME 425138	TRUSS NAME T2S	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
--------------------	-------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:15:36 2022 Page 2
ID: o0V9YTq5oz7QIFCPkyhtuzaO6Z-9MxxCuhw 0CZkABP Oz fqwyaMILRhPFxhAzPXyWDD

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TMVW-t	MT20	4.0	6.0	2.00	2.75
E	TTVW-m	MT20	5.0	8.0	1.75	2.75
F	TMV+p	MT20	3.0	4.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTVW-m	MT20	5.0	8.0	Edge	3.00
I	TMVW-t	MT20	4.0	6.0	2.00	1.50
J	TMVW-p	MT20	6.0	7.0	Edge	
L	BMV1+p	MT20	3.0	8.0		
M	BMVW-t	MT20	6.0	10.0		
N, S, T						
N	BMVW-t	MT20	4.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	6.0	10.0		
Q	BMV+p	MT20	3.0	8.0		
R	BVMW-t	MT20	10.0	12.0	5.50	3.75
U	BVMW-m	MT20	10.0	12.0	6.50	7.75
V	BMV+p	MT20	3.0	8.0		
W	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

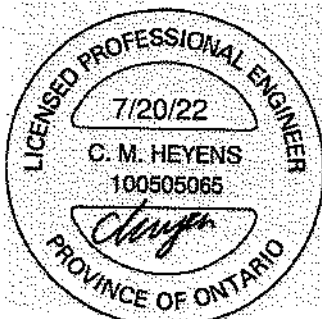
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	25-10-0	-3228	-3228	—	FRONT	VERT	TOTAL	—	C1
X	25-7-4	-199	-199	—	TOP	VERT	TOTAL	—	C1
Y	28-1-12	-199	-199	—	TOP	VERT	TOTAL	—	C1
Z	29-10-12	-177	-177	—	TOP	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



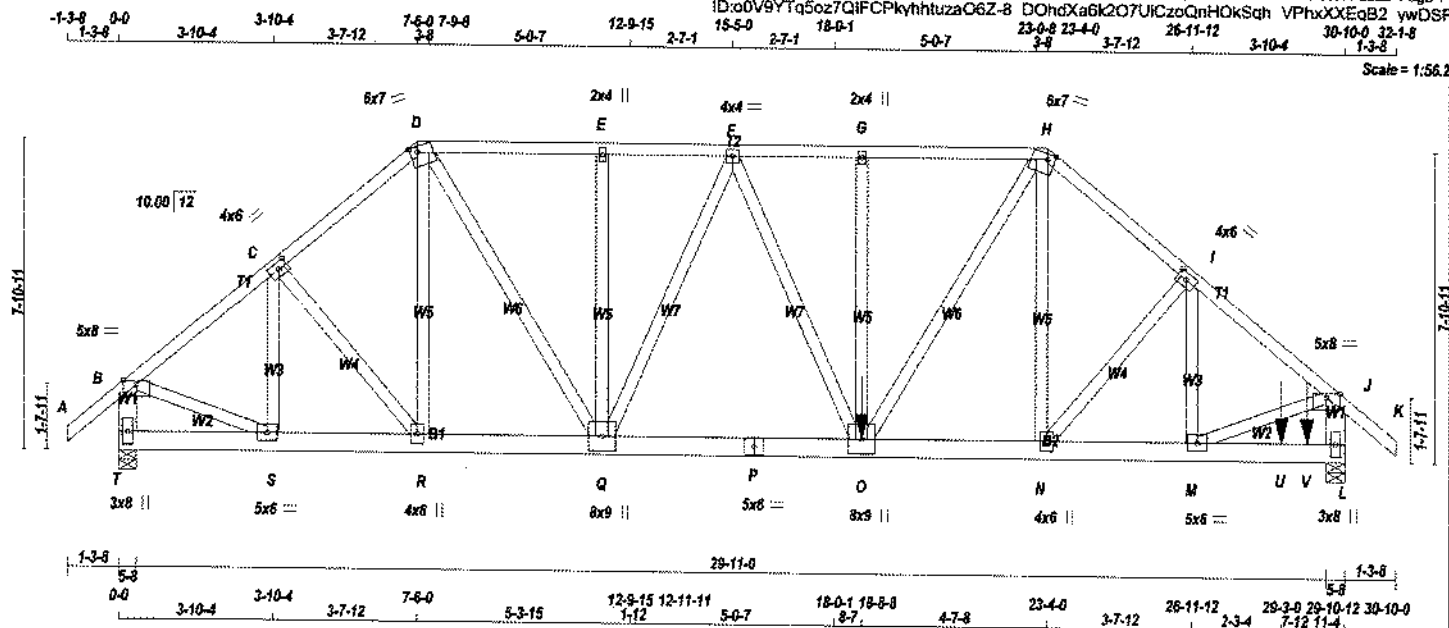
STRUCTURAL COMPONENT ONLY
DWG # TR22070592 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T3	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:00:14 2022 Page 1

ID:00V9YTq5oz7QIFCPkyhtuza06Z-8 DOndXa6k2OTUJCzoQnHokSqh VPhXXEqB2 ywDSF



Scale = 1:56.2

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
T - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
T - P	2x6	DRY	No.2	SPF	
P - L	2x6	DRY	No.2	SPF	

ALL WEBS 2x4 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D	12	TOP
D - H	12	TOP
H - K	12	TOP
T - B	2	TOP
L - J	2	TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS

T - P 2 12 TOP

P - L 2 12 SIDE(183.1)

WEBS : (0.122"x3") SPIRAL NAILS

O - G 1 6 SIDE(611.0)

2x4 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
T	3033	0	0	5-8
L	5963	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
T	2147	1395 / 0	0 / 0	0 / 0	0 / 0	752 / 0
L	4215	2778 / 0	0 / 0	0 / 0	0 / 0	1437 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO			
A-B	0 / 38	-84.9	-84.9	0.06 (1)	10.00	S-C	-724 / 0	0.09 (1)	
B-C	-3075 / 0	-84.9	-84.9	0.15 (1)	5.15	C-R	0 / 246	0.02 (1)	
C-D	-3338 / 0	-84.9	-84.9	0.15 (1)	4.88	R-D	-79 / 22	0.03 (1)	
D-E	-3717 / 0	-84.9	-84.9	0.18 (1)	4.74	N-H	0 / 244	0.02 (1)	
E-F	-3717 / 0	-84.9	-84.9	0.17 (1)	4.74	N-I	-517 / 0	0.11 (1)	
F-G	-4863 / 0	-84.9	-84.9	0.18 (1)	4.23	M-I	0 / 270	0.02 (1)	
G-H	-4863 / 0	-84.9	-84.9	0.23 (1)	4.19	B-S	0 / 2496	0.22 (1)	
H-I	-4495 / 0	-84.9	-84.9	0.18 (1)	4.39	M-J	0 / 3947	0.35 (1)	
I-J	-4879 / 0	-84.9	-84.9	0.20 (1)	4.23	O-H	0 / 2726	0.24 (1)	
J-K	0 / 38	-84.9	-84.9	0.06 (1)	10.00	D-Q	0 / 2246	0.20 (1)	
T-B	-2964 / 0	0.0	0.0	0.11 (1)	7.31	O-G	-369 / 0	0.15 (1)	
L-J	-4579 / 0	0.0	0.0	0.17 (1)	8.74	Q-E	-387 / 0	0.15 (1)	
						Q-F	-1547 / 0	0.78 (1)	
						F-O	0 / 1310	0.12 (1)	
T-S	0 / 0	-18.5	-18.5	0.02 (1)	10.00				
S-R	0 / 2379	-18.5	-18.5	0.17 (1)	10.00				
R-Q	0 / 2538	-18.5	-18.5	0.18 (1)	10.00				
Q-P	0 / 4338	-18.5	-18.5	0.32 (1)	10.00				
P-O	0 / 4338	-18.5	-18.5	0.32 (1)	10.00				
O-N	0 / 3432	-18.5	-18.5	0.27 (1)	10.00				
N-M	0 / 3763	-18.5	-18.5	0.50 (1)	10.00				
M-U	0 / 0	-18.5	-18.5	0.74 (1)	10.00				
U-V	0 / 0	-18.5	-18.5	0.74 (1)	10.00				
V-L	0 / 0	-18.5	-18.5	0.74 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	18-9-8	-2154	-2154	---	BACK	VERT	TOTAL	---	C1
U	29-3-0	-1617	-1617	---	BACK	VERT	TOTAL	---	C1
V	29-10-12	-167	-167	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 203 = 406 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 8 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03')
CALCULATED VERT. DEFL.(LL) = L/999 (0.07')
ALLOWABLE DEFL.(TL) = L/360 (1.03')
CALCULATED VERT. DEFL.(TL) = L/999 (0.14')

CSI: TC=0.23/1.00 (G-H-1), BC=0.74/1.00 (L-M-1), WB=0.78/1.00 (F-Q-1), SSI=0.46/1.00 (L-M-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.85 (D) (INPUT = 0.90)
JSI METAL=0.45 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070567 PG 1/2

JOB NAME 425136	TRUSS NAME T3	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:00:14 2022 Page 2
ID:00V9YTq5oz7QIFCPkyhhtuza06Z-8 D0hdXa8k207UICzoCnHOkSgh VPhxXXEqB2 ywDSF

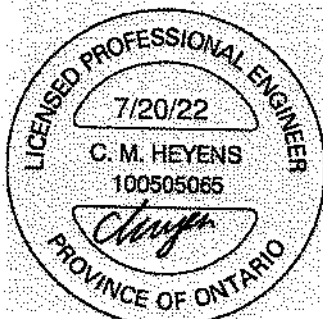
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW-m	MT20	6.0	7.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	6.0	7.0	1.75	2.25
I	TMWW-t	MT20	4.0	6.0	2.00	2.75
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMVt+p	MT20	3.0	8.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWWW-t	MT20	8.0	9.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	8.0	9.0		
R	BMWW-t	MT20	4.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMVt+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070567 PG 2/2

STRUCTURAL COMPONENT ONLY
DWG # TR22070612 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T3Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 5 Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 12:08:07 2022 Page 2
ID: o0V9YTq6oz7QIFCPkyhhtuza06Z- 8M54ICa6FmgAfnuzia7mT1hkpNiznwwV0lskyw8aM

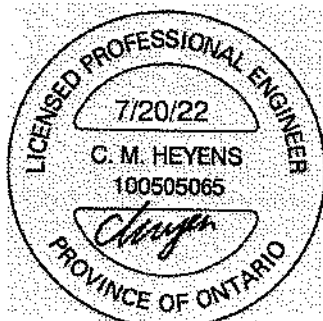
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	8.0	2.00	2.75
D	TTWW+m	MT20	6.0	7.0	2.00	1.75
E	TMW-w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW-w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	7.0	2.00	1.75
I	TMWW-t	MT20	4.0	6.0	2.00	2.75
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	8.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW+L	MT20	4.0	6.0		
O	BMWWW-t	MT20	8.0	10.0	3.75	5.00
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	6.0	10.0	3.75	5.00
R	BMWW+L	MT20	4.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070612 PG 2/2



TOTAL WEIGHT = $4 \times 154 = 617$ lb

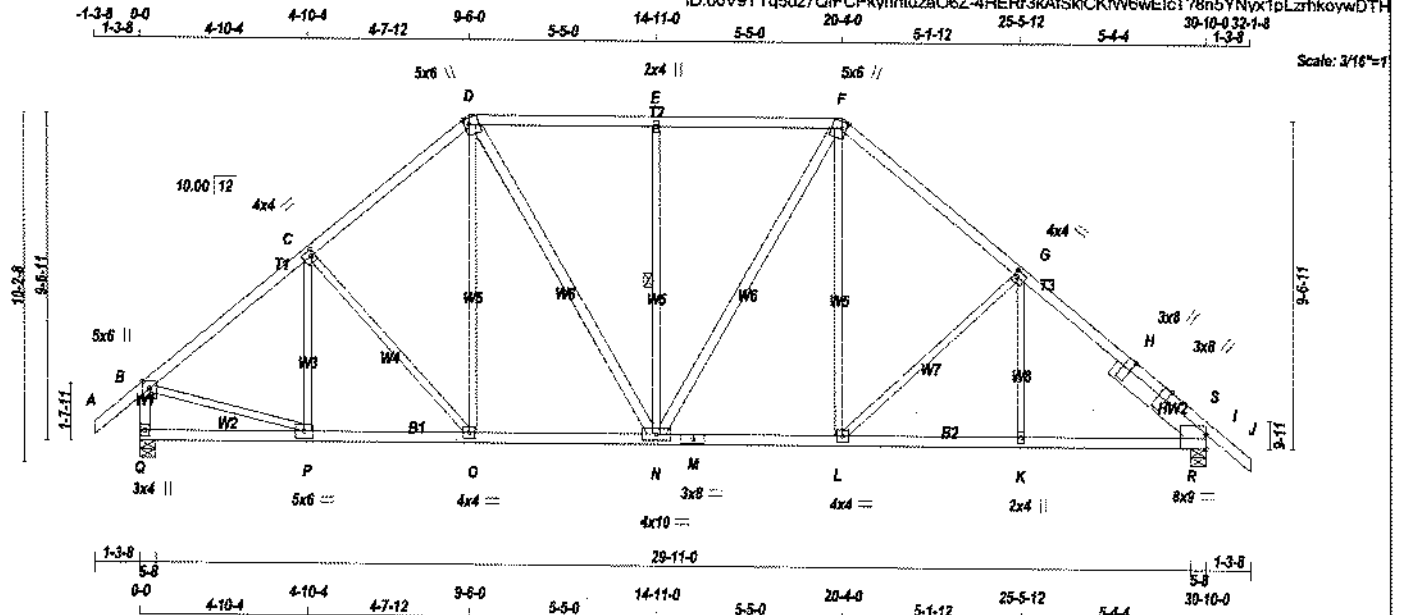
```
JSI GRIP= 0.82 (H) (INPUT = 0.90 )
JSI METAL= 0.59 (B) (INPUT = 1.00 )
```

STRUCTURAL COMPONENT ONLY
DWG # TR22070557

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T4X	11	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.330 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 09:59:08 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhhtuza06Z-4HER3kAtSkCKW6wEicT78n5YNyx1PLzhkoywDTH



TOTAL WEIGHT = 11 X 157 = 1732 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - J	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
Q - M	2x4	DRY	No.2	SPF	
M - I	2x4	DRY	No.2	SPF	

REINFORCING MEMBERS	HW2	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - N	2x4	DRY	No.2	SPF	
N - F	2x4	DRY	No.2	SPF	

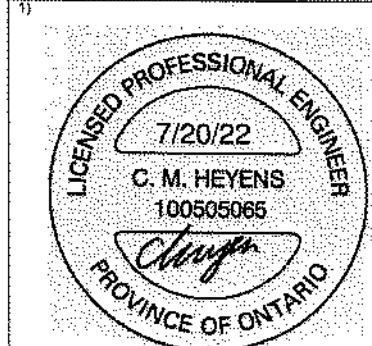
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	V	X
B	TMAW+p	MT20	5.0	6.0	Edge	
C	TMAW+4	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMAW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMAW+4	MT20	4.0	4.0	2.00	1.25
I	TMBMR+4	MT20	8.0	9.0	5.25	
I	RT+4	MT20	3.0	8.0	Edge	18.25
I	RT+4	MT20	3.0	8.0	Edge	34.25
K	BMW+4	MT20	2.0	4.0		
L	BMW+4	MT20	4.0	4.0		
M	BS+4	MT20	3.0	8.0		
N	BMW+4	MT20	4.0	10.0		
O	BMW+4	MT20	4.0	4.0		
P	BMW+4	MT20	5.0	6.0		
Q	BMV+4	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)



STRUCTURAL COMPONENT ONLY
DWG # TR22070558

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REGRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
Q	1711	0	1711	0
I	1711	0	1711	0

UNFACTORED REACTIONS							
	1ST LOASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SC
Q	1212	783 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0
I	1212	783 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CS (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO						FR-TO			
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	P-C	-248 / 0	0.12 (1)	
B-C	-1619 / 0	-84.9	-84.9	0.29 (1)	4.93	C-O	-198 / 0	0.20 (1)	
C-D	-1612 / 0	-84.9	-84.9	0.28 (1)	5.07	O-D	0 / 242	0.05 (1)	
D-E	-1361 / 0	-84.9	-84.9	0.34 (1)	5.18	D-N	0 / 442	0.07 (1)	
E-F	-1361 / 0	-84.9	-84.9	0.34 (1)	5.18	N-E	-589 / 0	0.34 (1)	
F-G	-1579 / 0	-84.9	-84.9	0.34 (1)	4.91	N-F	0 / 320	0.05 (1)	
G-H	-1475 / 0	-84.9	-84.9	0.25 (1)	5.15	L-F	0 / 358	0.08 (1)	
H-S	-1475 / 0	-84.9	-84.9	0.25 (1)	5.15	L-G	-387 / 0	0.37 (1)	
S-I	-1855 / 0	-84.9	-84.9	0.06 (1)	4.80	K-G	0 / 98	0.03 (4)	
I-J	0 / 28	-84.9	-84.9	0.11 (1)	10.00	B-P	0 / 1312	0.30 (1)	
Q-B	-1673 / 0	0.0	0.0	0.18 (1)	6.42	R-S	0 / 994	0.00 (1)	
						R-H	-959 / 0	0.10 (1)	
Q-P	0 / 0	-18.5	-18.5	0.09 (4)	10.00				
P-O	0 / 1267	-18.5	-18.5	0.25 (1)	10.00				
O-N	0 / 1138	-18.5	-18.5	0.24 (1)	10.00				
N-M	0 / 1199	-18.5	-18.5	0.26 (1)	10.00				
M-L	0 / 1199	-18.5	-18.5	0.26 (1)	10.00				
L-K	0 / 1463	-18.5	-18.5	0.30 (1)	10.00				
K-R	0 / 1463	-18.5	-18.5	0.34 (1)	10.00				
R-I	0 / 866	-18.5	-18.5	0.24 (1)	10.00				

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(L1) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.34/1.00 (D-E:1), BC=0.34/1.00 (K-R:1), WB=0.37/1.00 (G-L:1), SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.63 (I) (INPUT = 1.00)

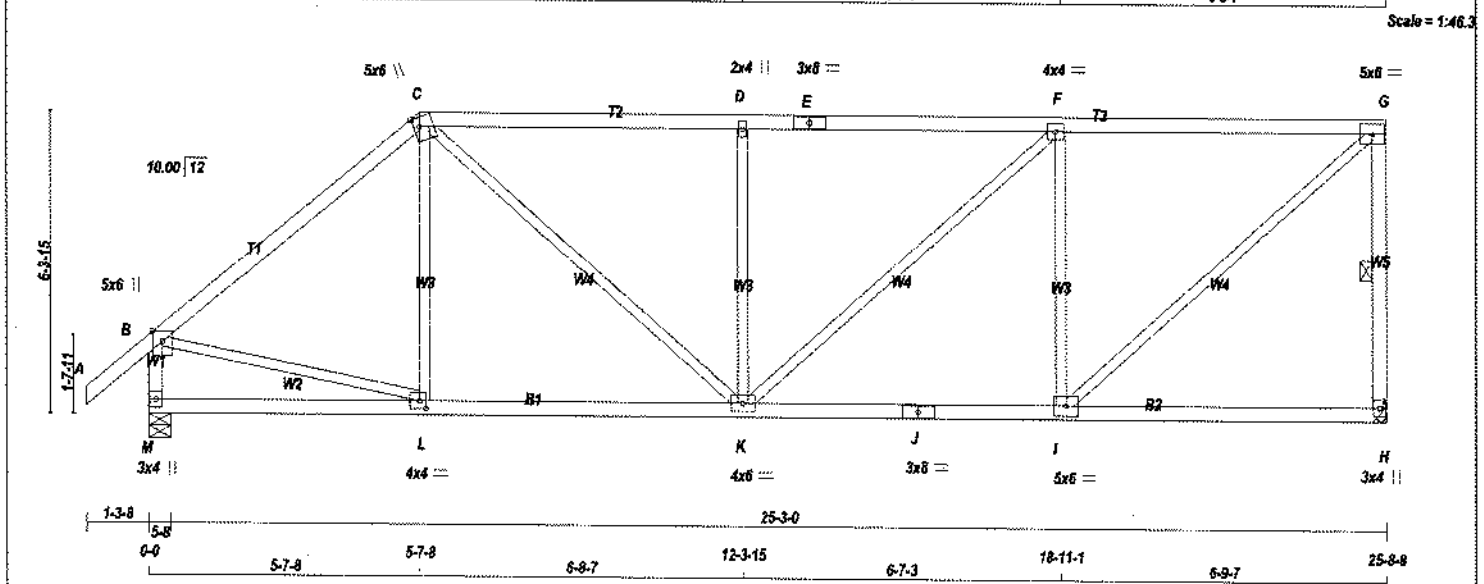
JOB NAME 425136	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
--------------------	------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 09:58:09 2022 Page 1

ID: c0V9YTq5oz7QIFCPkyhhtuzaO6Z-JwA8Suo0AYKI7dGeB91hTj137BJR0d878nH23AiywDUC

Scale = 1:46.3



TOTAL WEIGHT = 110 LB

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	4.0	2.00	1.50
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	RECORD
JT	VERT	HORZ	BRG	BRG
H	1329	0	1329	0
M	1446	0	1446	0
			MECHANICAL	
			5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	943	599 / 0	0 / 0	0 / 0	0 / 0	344 / 0	0 / 0
M	1024	663 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 38	-84.9	-84.9 0.12 (1)	L-C	-95 / 61	0.06 (1)	
B-C	-1285 / 0	-84.9	-84.9 0.57 (1)	B-L	0 / 1011	0.23 (1)	
C-D	-1481 / 0	-84.9	-84.9 0.61 (1)	I-G	0 / 1573	0.35 (1)	
D-E	-1481 / 0	-84.9	-84.9 0.63 (1)	C-K	0 / 868	0.15 (1)	
E-F	-1481 / 0	-84.9	-84.9 0.63 (1)	I-F	-905 / 0	0.58 (1)	
F-G	-1175 / 0	-84.9	-84.9 0.60 (1)	K-D	-612 / 0	0.39 (1)	
H-G	-1278 / 0	0.0	0.0 0.24 (1)	K-F	0 / 414	0.09 (1)	
M-B	-1406 / 0	0.0	0.0 0.15 (1)				
M-L	0 / 0	-18.5	-18.5 0.17 (4)				
L-K	0 / 984	-18.5	-18.5 0.26 (4)				
K-J	0 / 1175	-18.5	-18.5 0.31 (4)				
J-I	0 / 1175	-18.5	-18.5 0.31 (4)				
I-H	0 / 0	-18.5	-18.5 0.20 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.63/1.00 (D-F:1), BC=0.31/1.00 (I-K:4), WB=0.58/1.00 (F-t:1), SS=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	PLI
		MAX MIN	MAX MIN
MT20	850	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

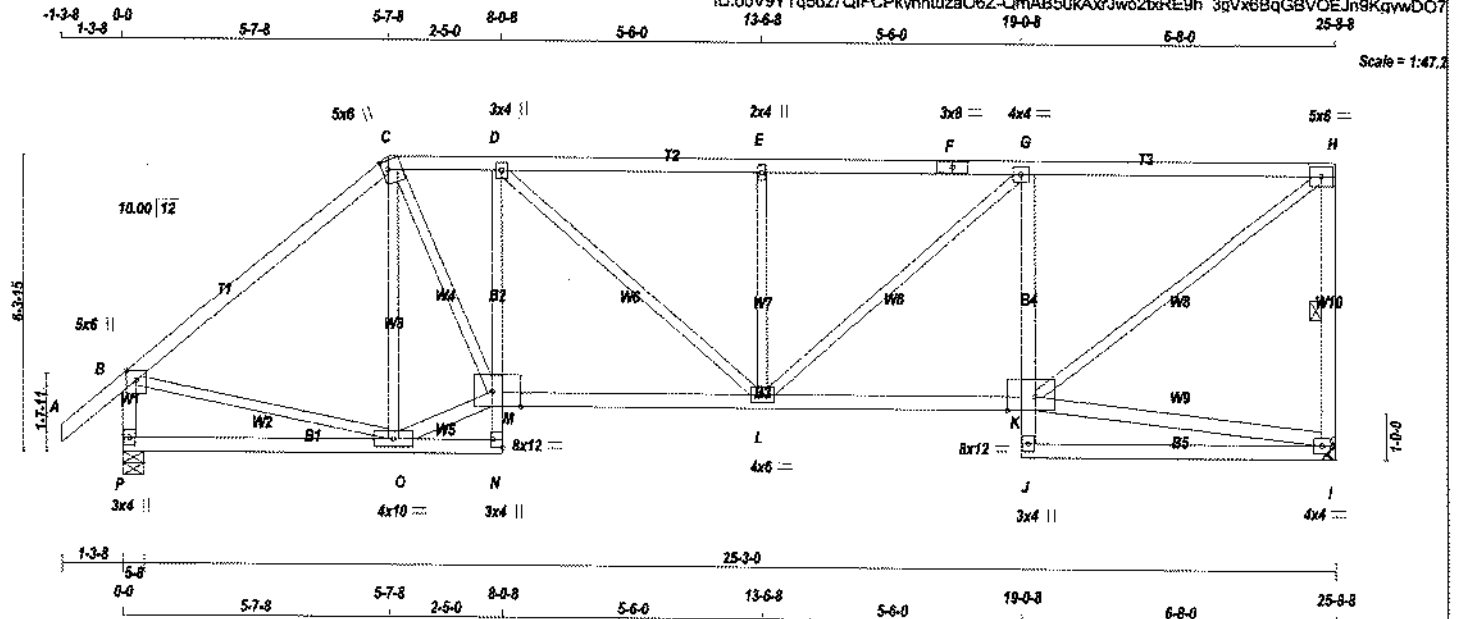
JSI GRIP= 0.83 (L) (INPUT = 0.80)
JSI METAL = 0.50 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY
DWG # TR22070550

JOB NAME 425137	TRUSS NAME T5S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
--------------------	-------------------	---------------	----------	-------------------------------	----------

Tamareck Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:04:38 2022 Page 1
ID:00V9YTq5oz7QIFCPkvhtuza06Z-QmAB5ukAxxJwo2bRE9H 3gVx6BqGBVOEJn9KqywDO7



TOTAL WEIGHT = 127 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
P - B	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
N - D	2x3	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
O - M	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	

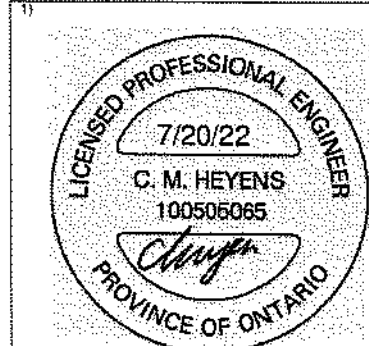
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TFWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+p	MT20	3.0	4.0		
E	TMVW+m	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMVW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	BMVW-t	MT20	8.0	12.0	3.50	7.25
L	BMVW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	8.0	12.0	Edge	7.25
N	BMV+p	MT20	3.0	4.0	2.00	Edge
O	BMVW-t	MT20	4.0	10.0		
P	BMV-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)



STRUCTURAL COMPONENT ONLY
DWG # TR22070587

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
I	1329	0	1329	0	0	0
P	1446	0	1446	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	MAX. MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	943	599 / 0	0 / 0	0 / 0	0 / 0	344 / 0	0 / 0
P	1024	863 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO		
A-B	0 / 38	-84.9	-84.9 0.12 (1)	10.00	O-C	-595 / 0
B-C	-1291 / 0	-84.9	-84.9 0.57 (1)	4.96	O-M	0 / 1070
C-D	-1484 / 0	-84.9	-84.9 0.25 (1)	5.07	C-M	0 / 1261
D-E	-1741 / 0	-84.9	-84.9 0.35 (1)	4.70	D-L	0 / 328
E-F	-1741 / 0	-84.9	-84.9 0.57 (1)	4.35	L-E	-457 / 0
F-G	-1741 / 0	-84.9	-84.9 0.57 (1)	4.35	L-G	0 / 515
G-H	-1351 / 0	-84.9	-84.9 0.56 (1)	4.85	K-I	-26 / 0
H-I	-1284 / 0	0.0	0.0 0.24 (1)	5.73	K-H	0 / 1705
P-B	-1403 / 0	0.0	0.0 0.15 (1)	6.67	B-O	0 / 1014
P-O	0 / 0	-18.5	-18.5 0.15 (4)	10.00		
O-N	0 / 13	-18.5	-18.5 0.15 (4)	10.00		
N-M	-16 / 0	0.0	0.0 0.04 (1)	7.81		
M-D	-606 / 0	0.0	0.0 0.25 (1)	7.81		
M-L	0 / 1487	-18.5	-18.5 0.32 (1)	10.00		
L-K	0 / 1358	-18.5	-18.5 0.30 (1)	10.00		
J-K	0 / 56	0.0	0.0 0.05 (1)	10.00		
K-G	-937 / 0	0.0	0.0 0.32 (1)	7.81		
J-I	0 / 26	-18.5	-18.5 0.23 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.86")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.07")
ALLOWABLE DEFL. (TL) = $L/360$ (0.86")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.15")

CSI: TC=0.57/1.00 (E-G:1), SC=0.32/1.00 (L-M:1)
WB=0.38/1.00 (I-K:1), SS=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747

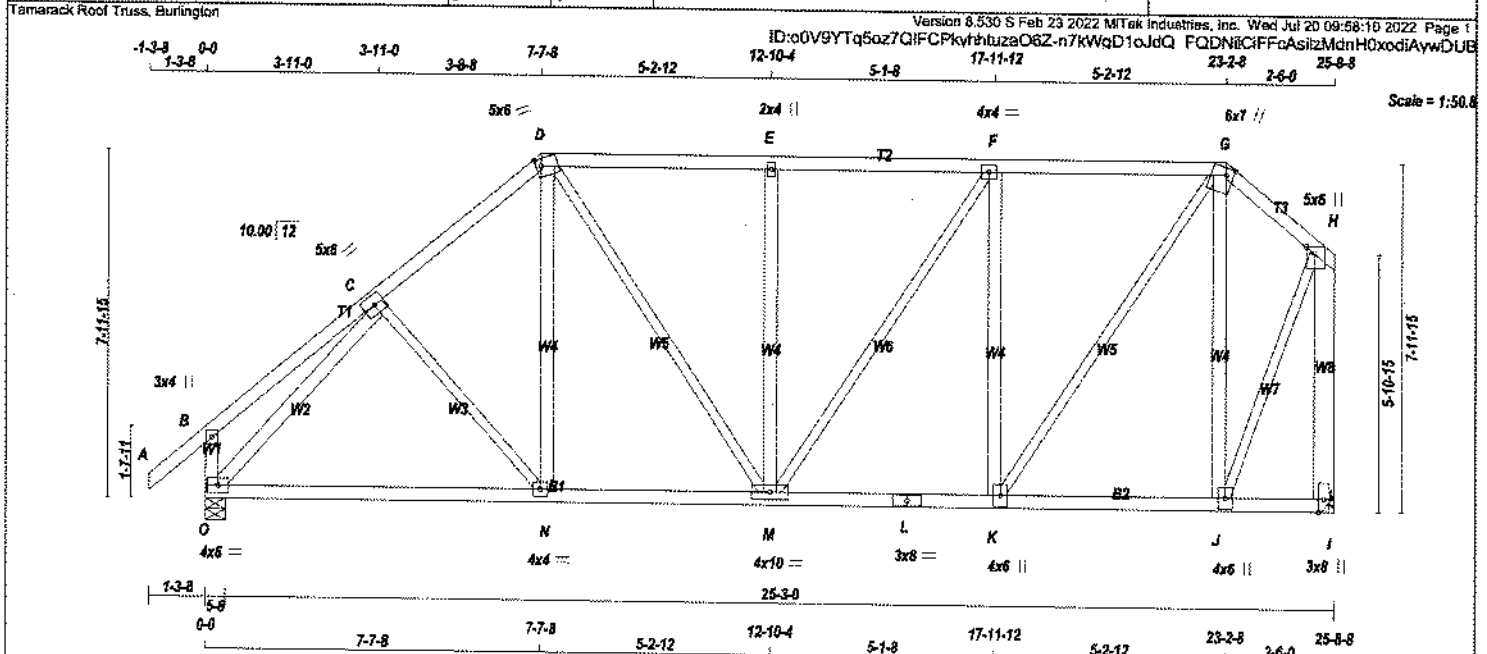
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.51 (B) (INPUT = 1.00)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME 425136	TRUSS NAME T6	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 3 X 147 = 441 lb		
N. L. G. A. RULES					BEARINGS										DESIGN CRITERIA		
CHORDS		SIZE	LUMBER	DESCR.	FACTORED		MAXIMUM FACTORED		INPUT	REQRD		SPECIFIED LOADS:					
A - D	2x4	DRY	No.2	SPF	GROSS REACTION		GROSS REACTION		BRG	BRG		TOP CH. LL = 23.3 PSF					
D - G	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	DL = 6.0 PSF				
G - H	2x4	DRY	No.2	SPF	O	1446	0	1446	0	5-8	5-8	BOT CH. LL = 0.0 PSF					
O - B	2x4	DRY	No.2	SPF	I	1329	0	1329	0	0	MECHANICAL	DL = 7.4 PSF					
I - H	2x6	DRY	No.2	SPF	A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-6.										TOTAL LOAD = 36.7 PSF		
O - L	2x4	DRY	No.2	SPF											SPACING = 24.0 IN./C		
L - I	2x4	DRY	No.2	SPF											LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12		
ALL WEBS EXCEPT					UNFACTORED REACTIONS										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015		
C - N	2x3	DRY	No.2	SPF	1ST CASE		MAX./MIN. COMPONENT REACTIONS										
D - M	2x3	DRY	No.2	SPF	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL					
M - F	2x3	DRY	No.2	SPF	O	1024	663 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0					
K - G	2x3	DRY	No.2	SPF	I	643	559 / 0	0 / 0	0 / 0	0 / 0	344 / 0	0 / 0					
J - H	2x3	DRY	No.2	SPF													
DRY: SEASONED LUMBER																	

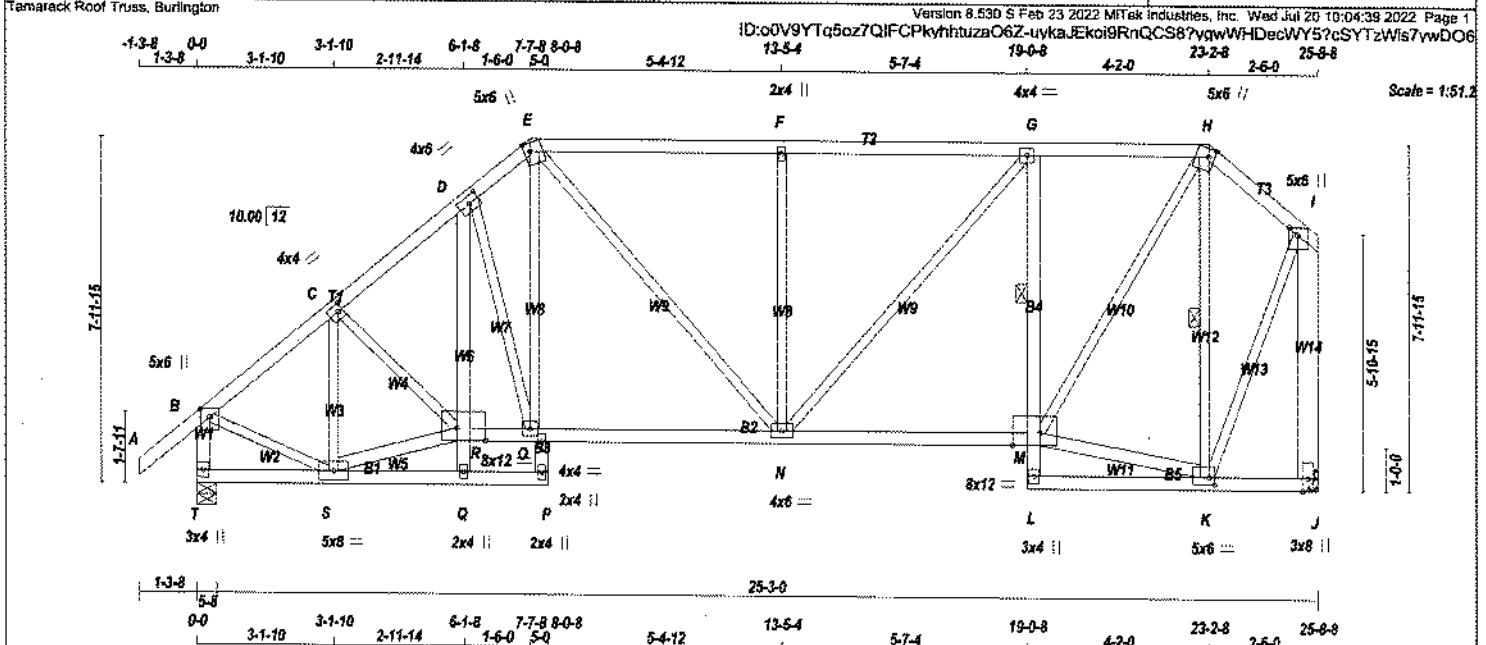
PLATES (table is in inches)				UNFACTORED REACTIONS				LOADING			
JT TYPE	PLATES	W	LEN Y X	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B TMV+p	MT20	3.0	4.0	O	1024	663 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0
C TMVW+t	MT20	5.0	6.0	I	943	599 / 0	0 / 0	0 / 0	0 / 0	344 / 0	0 / 0
D TMVW+m	MT20	5.0	6.0	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O							
E TMVW+w	MT20	2.0	4.0								
F TMVW-t	MT20	4.0	4.0								
G TMVW+m	MT20	6.0	7.0								
H TMVW+p	MT20	5.0	6.0								
I BMV+p	MT20	3.0	6.0								
J BMVW+t	MT20	4.0	6.0								
K BMVW+t	MT20	4.0	6.0								
L BS-t	MT20	3.0	6.0								
M BMVW+t	MT20	4.0	10.0								
N BMVW-t	MT20	4.0	4.0								
O BMVW-t	MT20	4.0	6.0								
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.											
NOTES: (1)											
1) Lateral braces to be a minimum of 2x4 SPF #2.											

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH (FR-TO)	MAX. FACTORED UNBRAC LENGTH (FR-TO)
FR-TO				FR-TO			
A-B	0 / 38	-84.9	-84.9 0.12 (1)	10.00	C-N	-51 / 27	0.03 (1)
B-C	0 / 24	-84.9	-84.9 0.20 (1)	10.00	N-D	0 / 186	0.04 (4)
C-D	-1248 / 0	-84.9	-84.9 0.23 (1)	5.49	D-M	0 / 350	0.08 (1)
D-E	-1139 / 0	-84.9	-84.9 0.34 (1)	5.52	M-E	-478 / 0	0.41 (1)
E-F	-1139 / 0	-84.9	-84.9 0.34 (1)	5.51	M-F	0 / 312	0.07 (1)
F-G	-966 / 0	-84.9	-84.9 0.33 (1)	5.87	K-F	-738 / 0	0.64 (1)
G-H	-536 / 0	-84.9	-84.9 0.08 (1)	6.25	K-G	0 / 1015	0.23 (1)
H-I	-241 / 0	0.0	0.0 0.03 (1)	7.81	J-G	-783 / 0	0.68 (1)
I-H	-1316 / 0	0.0	0.0 0.63 (1)	7.81	O-C	-1504 / 0	0.62 (1)
					J-H	0 / 957	0.22 (1)
O-N	0 / 973	-18.5	-18.5 0.32 (4)	10.00			
N-M	0 / 942	-18.5	-18.5 0.33 (4)	10.00			
M-L	0 / 966	-18.5	-18.5 0.21 (1)	10.00			
L-K	0 / 966	-18.5	-18.5 0.21 (1)	10.00			
K-J	0 / 395	-18.5	-18.5 0.14 (4)	10.00			
J-I	0 / 0	-18.5	-18.5 0.07 (4)	10.00			

STRUCTURAL COMPONENT ONLY
DWG # TR22070551

ALLOWABLE DEFL. (LL) = L/360 (0.86")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.86")
CALCULATED VERT. DEFL. (TL) = L/999 (0.13")
CSI: TC=0.63/1.00 (H-I), BC=0.33/1.00 (M-N), WB=0.68/1.00 (G-J), SSI=0.21/1.00 (F-G, I)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.80 (G) (INPUT = 0.90)
JSI METAL= 0.34 (L) (INPUT = 1.00)

JOB NAME 425137	TRUSS NAME T6S	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
T - B	2x4	DRY	No.2
J - I	2x6	DRY	No.2
T - P	2x4	DRY	No.2
R - M	2x4	DRY	No.2
L - G	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
M - K	2x4	DRY	No.2
Q - D	2x4	DRY	No.2
S - R	2x4	DRY	No.2

PLATES (table is in inches)			
JT	TYPE	PLATES	W LEN Y X
B	TMVW+p	MT20	5.0 6.0 Edge
C	TMVW-l	MT20	4.0 4.0 2.00 1.25
D	TMVW-l	MT20	4.0 6.0 2.00 2.75
E	TTVW+m	MT20	5.0 6.0 2.25 1.50
F	TMVW-w	MT20	2.0 4.0
G	TMVW-l	MT20	4.0 4.0
H	TTVW+m	MT20	5.0 6.0 2.25 1.50
I	TMVW+p	MT20	5.0 6.0 2.00 2.25
J	BMVl+p	MT20	3.0 8.0 Edge
K	BMVWV-l	MT20	5.0 6.0 2.00 1.50
L	BMV+p	MT20	3.0 4.0
M	BMVWV-l	MT20	8.0 12.0 3.50 7.50
N	BMVWV-l	MT20	4.0 6.0
O	BMVWV-l	MT20	4.0 4.0
P	NP+w	MT20	2.0 4.0
Q	BMVW-w	MT20	2.0 4.0
R	BMVWV-l	MT20	8.0 12.0 3.50 7.75
S	BMVWV-l	MT20	5.0 8.0
T	BMVl+p	MT20	3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX
T	1473	0	0
J	1337	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-B.

UNFACTORED REACTIONS			
JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW LIVE PERM. LIVE WIND DEAD SOIL	
T	1048	663 / 0	0 / 0
J	950	599 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH FR-TO
A-B	0 / 38	-84.9	10.00
B-C	-1242 / 0	-84.9	5.62
C-D	-1557 / 0	-84.9	5.15
D-E	-1438 / 0	-84.9	5.41
E-F	-1339 / 0	-84.9	4.99
F-G	-1340 / 0	-84.9	4.99
G-H	-1001 / 0	-84.9	5.90
H-I	-527 / 0	-84.9	6.25
I-J	-1460 / 0	0.0	6.79
J-K	-1328 / 0	0.0	7.81
T-S	0 / 0	-18.5	10.00
S-O	0 / 21	-18.5	10.00
Q-P	0 / 0	-18.5	10.00
R-O	0 / 1190	-18.5	10.00
O-N	0 / 1096	-18.5	10.00
N-M	0 / 1004	-18.5	10.00
L-M	0 / 33	0.0	10.00
M-G	-824 / 0	0.0	6.25
L-K	0 / 22	-18.5	10.00
K-J	0 / 0	-18.5	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.64/1.00 (I-J:1), BC=0.25/1.00 (N-O:4), WB=0.47/1.00 (F-N:1), SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

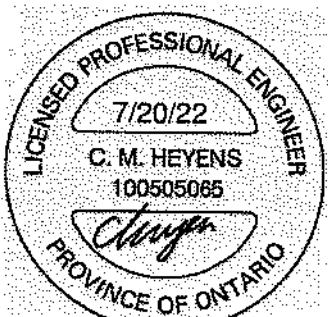
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070588 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425137	T6S	3	1	TRUSS DESC.		

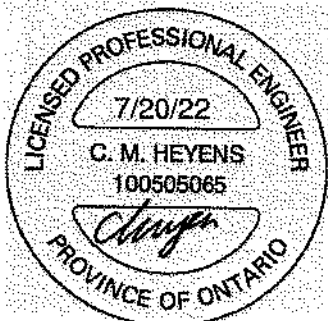
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 10:04:39 2022 Page 2
ID:90V9YTg5oz7QIFCPkvhhtuzaO6Z-uykaJEkoi9RnQCS8?vgwWHDecWY5?cSYTzWis7ywDO6

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070588 PG 2/2

[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425138	T6S1	3	1	TRUSS DESC.		

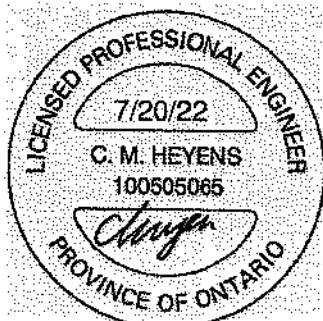
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:15:38 2022 Page 2
ID: c0V9YTq5oz7QIFCPkyhluzaO6Z-5k2idajAWdSH TL06r7SkF7GPASsvacX77f4TCvwDDp

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

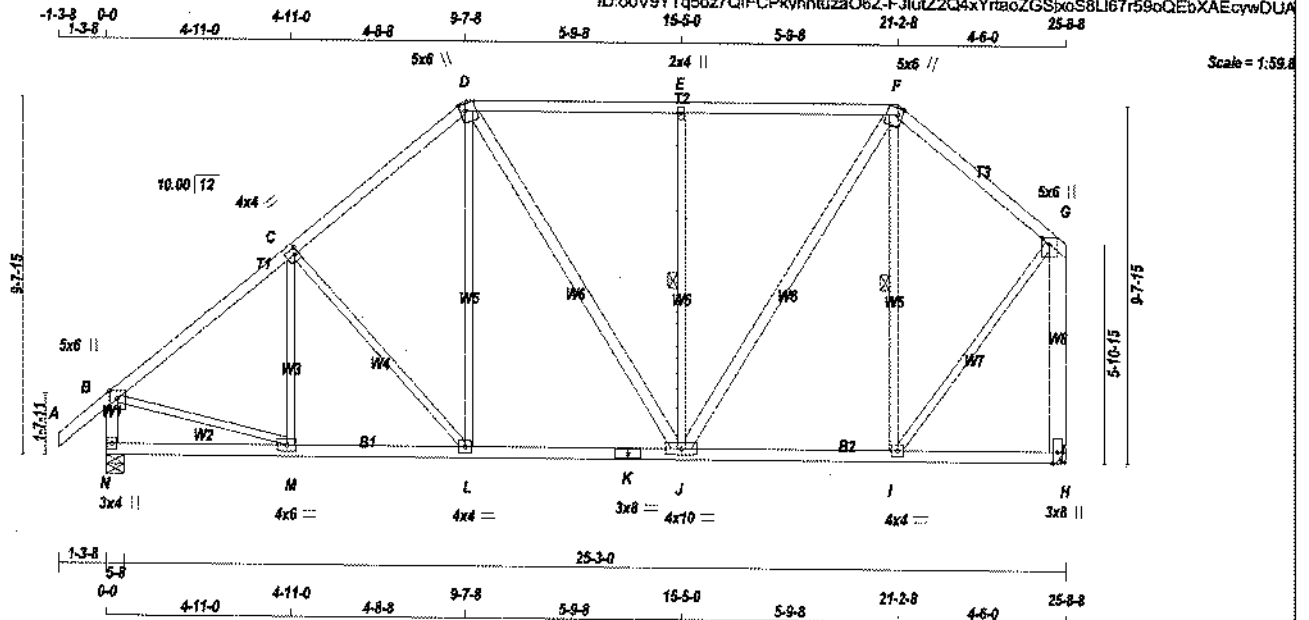


STRUCTURAL COMPONENT ONLY
DWG # TR22070594 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T7	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Jul 20 09:56:11 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhltuzaO6Z-FJluZ2Q4xYrtaoZGSboS8Ll67r59oQEhXAEcywDUA



TOTAL WEIGHT = 143 lb

LUMBER					N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.			
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
F - G	2x4	DRY	No.2	SPF		
N - B	2x4	DRY	No.2	SPF		
H - G	2x6	DRY	No.2	SPF		
N - K	2x4	DRY	No.2	SPF		
K - H	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		
D - J	2x4	DRY	No.2	SPF		
J - F	2x4	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)						
JT TYPE	PLATES	W	LEN	Y	X	
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-i	MT20	4.0	4.0	2.00	1.25
D	TMVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TMVW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	5.0	6.0	2.00	2.25
H	BMV1+p	MT20	3.0	8.0	Edge	
I	BMVW-i	MT20	4.0	4.0		
J	BMVW-w	MT20	4.0	10.0		
K	BS-i	MT20	3.0	8.0		
L	BMVW-i	MT20	4.0	4.0		
M	BMVW-i	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS	REACTION	DOWN	HORZ	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
N	1448	0	1448	0	0	5-8
H	1329	0	1329	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	MAX/MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1024	663 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0
H	943	599 / 0	0 / 0	0 / 0	0 / 0	344 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-J, F-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 38	-84.9	-84.9 0.12 (1)	10.00	M-C	-177 / 12	0.09 (1)
B-C	-1307 / 0	-84.9	-84.9 0.28 (1)	5.35	C-L	-256 / 0	0.28 (1)
C-D	-1151 / 0	-84.9	-84.9 0.27 (1)	5.63	L-D	0 / 289	0.07 (1)
D-E	-917 / 0	-84.9	-84.9 0.38 (1)	5.94	D-J	0 / 106	0.02 (1)
E-F	-917 / 0	-84.9	-84.9 0.38 (1)	5.94	J-E	-604 / 0	0.38 (1)
F-G	-704 / 0	-84.9	-84.9 0.23 (1)	6.25	J-F	0 / 727	0.12 (1)
N-B	-1408 / 0	0.0	0.0 0.15 (1)	6.87	I-F	-546 / 0	0.34 (1)
H-G	-1297 / 0	0.0	0.0 0.62 (1)	7.81	B-M	0 / 1063	0.24 (1)
					I-G	0 / 848	0.19 (1)
N-M	0 / 0	-18.5	-18.5 0.10 (4)	10.00			
M-L	0 / 1028	-18.5	-18.5 0.22 (1)	10.00			
L-K	0 / 861	-18.5	-18.5 0.20 (1)	10.00			
K-J	0 / 861	-18.5	-18.5 0.20 (1)	10.00			
J-I	0 / 534	-18.5	-18.5 0.17 (4)	10.00			
I-H	0 / 0	-18.5	-18.5 0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 27.2 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.62/1.00 (G-H:1), BC=0.22/1.00 (L-M:1), WB=0.38/1.00 (E-J:1), SS=0.24/1.00 (F-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

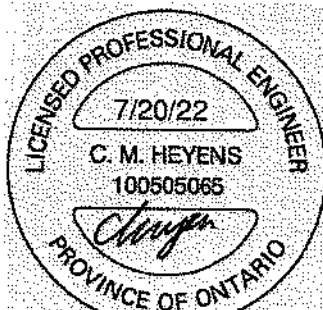
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)



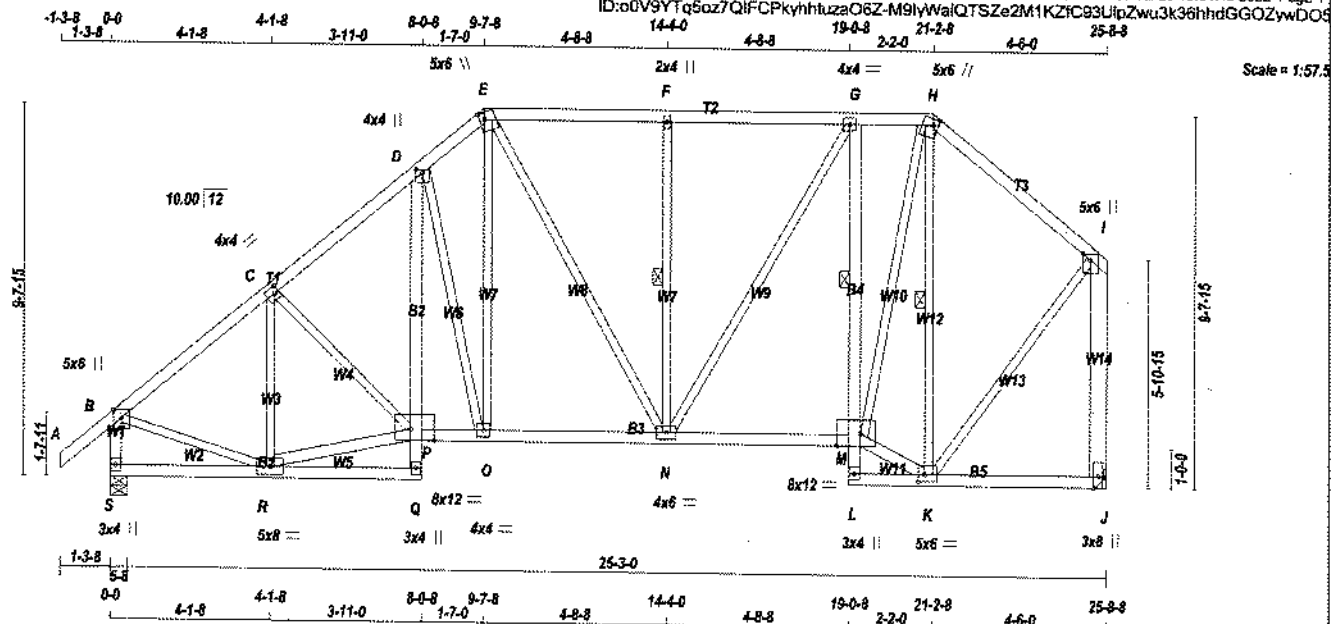
STRUCTURAL COMPONENT ONLY
DWG # TR22070552

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425137	T7S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:04:40 2022 Page 1

ID:0GV9Ytq5oz7QIFCPkyhhtuza06Z-M9lyWaiQTSZe2M1KZfC93UpZwu3k36hhdGG0ZywDO6



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
S - B	2x4	DRY	No.2
J - I	2x6	DRY	No.2
S - Q	2x4	DRY	No.2
O - D	2x4	DRY	No.2
P - M	2x4	DRY	No.2
L - G	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
R - P	2x4	DRY	No.2
M - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	TTVW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW-t	MT20	2.0	4.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTVW+m	MT20	5.0	6.0	2.00	1.50
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
J	BMV1+p	MT20	3.0	6.0	Edge	
K	BMVWV-t	MT20	5.0	6.0	2.25	2.00
L	BMV+p	MT20	3.0	4.0		
M	BMVWV-t	MT20	8.0	12.0	3.75	7.25
N	BMVWV-t	MT20	4.0	6.0		
O	BMVWV-t	MT20	4.0	4.0		
P	BMVWV-t	MT20	8.0	12.0	3.50	7.25
Q	BMV+p	MT20	3.0	4.0		
R	BMVWV-t	MT20	5.0	8.0		
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
S	1448	1448	0	5-8
J	1329	1329	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
S	1024	863 / 0	0 / 0	361 / 0
J	943	590 / 0	0 / 0	344 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, F-N, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	R-C	-503 / 0
B-C	-1275 / 0	-84.9	-84.9	0.18 (1)	5.52	R-P	0 / 1031
C-D	-1403 / 0	-84.9	-84.9	0.18 (1)	5.33	C-P	0 / 70
D-E	-1268 / 0	-84.9	-84.9	0.06 (1)	5.67	D-O	-430 / 0
E-F	-1036 / 0	-84.9	-84.9	0.23 (1)	5.90	O-E	0 / 468
F-G	-1036 / 0	-84.9	-84.9	0.23 (1)	5.90	E-N	0 / 150
G-H	-780 / 0	-84.9	-84.9	0.10 (1)	6.25	N-F	-468 / 0
H-I	-706 / 0	-84.9	-84.9	0.23 (1)	6.25	H-G	0 / 503
S-B	-1416 / 0	0.0	0.0	0.15 (1)	6.85	M-K	0 / 565
J-I	-1300 / 0	0.0	0.0	0.63 (1)	7.81	M-H	0 / 1053
S-R	0 / 0	-18.5	-18.5	0.08 (4)	10.00	B-R	0 / 1049
R-Q	0 / 17	-18.5	-18.5	0.08 (4)	10.00	K-I	0 / 860
Q-P	0 / 31	0.0	0.0	0.03 (1)	10.00		
P-D	0 / 263	0.0	0.0	0.07 (1)	10.00		
P-O	0 / 1067	-18.5	-18.5	0.21 (1)	10.00		
O-N	0 / 962	-18.5	-18.5	0.21 (1)	10.00		
N-M	0 / 784	-18.5	-18.5	0.18 (1)	10.00		
L-M	-16 / 0	0.0	0.0	0.03 (1)	7.81		
M-G	-741 / 0	0.0	0.0	0.12 (1)	6.25		
L-K	0 / 31	-18.5	-18.5	0.11 (4)	10.00		
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

TOTAL WEIGHT = 167 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCSC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(L1) = L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.63/1.00 (I-J:1), BC=0.21/1.00 (O-P:1),
WB=0.50/1.00 (H-K:1), SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

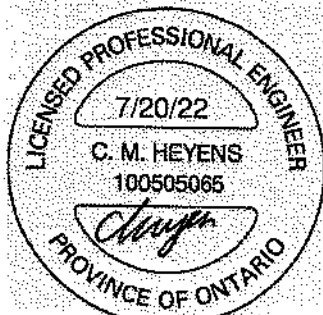
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070589 PG 1/2

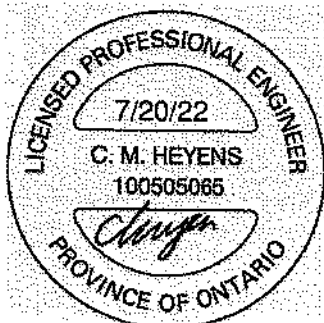
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425137	T7S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 10:04:40 2022 Page 2

ID: a0V9YTg5oz7QIFCPkyhhtuzaO6Z-M9IvWaiQTSZe2M1KZTC93UpZwu3k38hdGGOZywDO5

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

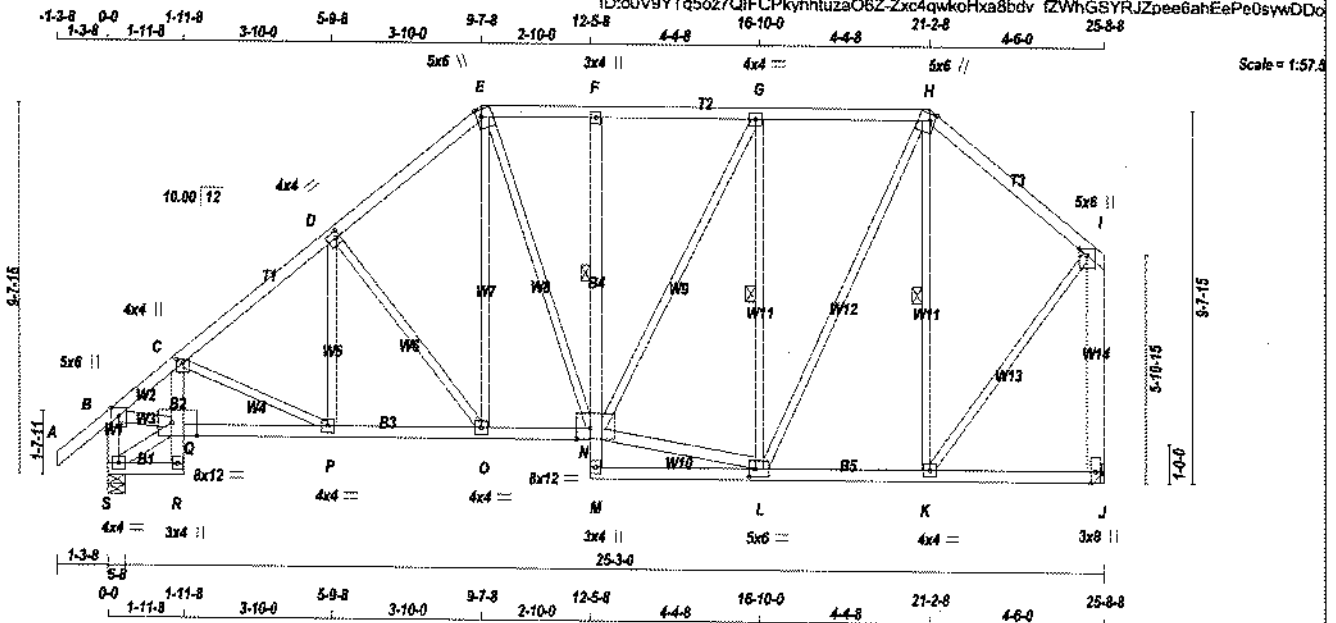


STRUCTURAL COMPONENT ONLY
DWG # TR22070589 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425138	T7S1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Jul 20 10:15:39 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhtuza06Z-Zxc4qwk0Hxa8Bdv 6ZWhGSYRJZpsee6ahEePe0sywDDo



TOTAL WEIGHT = 161 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
H - I	2x4	DRY	No.2	SPF	
S - B	2x4	DRY	No.2	SPF	
J - I	2x6	DRY	No.2	SPF	
S - C	2x4	DRY	No.2	SPF	
R - C	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
M - F	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
S - Q	2x4	DRY	No.2	SPF	
B - Q	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	TTVW+tm	MT20	5.0	6.0	2.25	1.50
F	TMV+p	MT20	3.0	4.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTVW+tm	MT20	5.0	6.0	2.25	1.50
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
J	BMV1+p	MT20	3.0	8.0	Edge	
K, O, P						
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.00
M	BMV+p	MT20	3.0	4.0		
N	BMVW-t	MT20	8.0	12.0	3.25	4.25
Q	BMVW-t	MT20	8.0	12.0	4.00	7.75
R	BMV+p	MT20	3.0	4.0		
S	BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
J	1326	1325	0	0
S	1449	1449	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	COMBINED	941	597 / 0	0 / 0	0 / 0	0 / 0	344 / 0	0 / 0
S	COMBINED	1026	665 / 0	0 / 0	0 / 0	0 / 0	382 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N, G-L, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS: (LC)
FR-TO						FR-TO			
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	C-P	-132 / 0	0.04 (1)	
B-C	-1677 / 0	-84.9	-84.9	0.11 (1)	5.04	P-D	0 / 97	0.03 (4)	
C-D	-1521 / 0	-84.9	-84.9	0.18 (1)	5.16	D-O	-379 / 0	0.29 (1)	
D-E	-1284 / 0	-84.9	-84.9	0.17 (1)	5.55	O-E	0 / 370	0.08 (1)	
E-F	-1025 / 0	-84.9	-84.9	0.10 (1)	8.09	E-N	0 / 210	0.05 (1)	
F-G	-1023 / 0	-84.9	-84.9	0.19 (1)	5.99	N-L	0 / 846	0.14 (1)	
G-H	-837 / 0	-84.9	-84.9	0.18 (1)	6.25	L-H	0 / 401	0.09 (1)	
H-I	-701 / 0	-84.9	-84.9	0.23 (1)	6.25	L-G	-775 / 0	0.48 (1)	
S-B	-1411 / 0	0.0	0.0	0.15 (1)	6.87	H-K	0 / 730	0.16 (1)	
J-I	-1292 / 0	0.0	0.0	0.62 (1)	7.81	K-H	-558 / 0	0.35 (1)	
S-R	0 / 35	-18.5	-18.5	0.02 (4)	10.00	K-I	0 / 844	0.19 (1)	
R-Q	0 / 19	0.0	0.0	0.06 (1)	10.00	S-Q	-42 / 0	0.00 (1)	
Q-C	-64 / 11	0.0	0.0	0.04 (1)	7.81	B-Q	0 / 1284	0.21 (1)	
Q-P	0 / 1302	-18.5	-18.5	0.25 (1)	10.00				
P-O	0 / 1181	-18.5	-18.5	0.22 (1)	10.00				
O-N	0 / 955	-18.5	-18.5	0.18 (1)	10.00				
M-N	0 / 36	0.0	0.0	0.03 (1)	10.00				
N-F	-310 / 0	0.0	0.0	0.04 (1)	6.25				
M-L	0 / 18	-18.5	-18.5	0.07 (4)	10.00				
L-K	0 / 531	-18.5	-18.5	0.14 (1)	10.00				
K-J	0 / 0	-18.5	-18.5	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.86")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.05")
ALLOWABLE DEFL. (TL) = $L/360$ (0.86")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.09")

CS: TC=0.62/1.00 (P-J:1), BC=0.25/1.00 (P-Q:1), WB=0.48/1.00 (G-L:1), SS=0.17/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

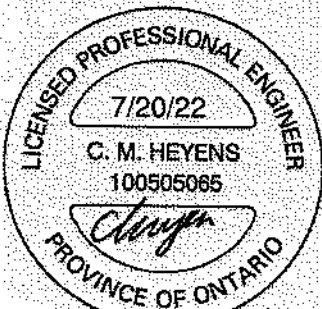
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.60 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR2070595 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425138	T7S1	1	1	TRUSS DESC.		

Tamareck Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:15:39 2022 Page 2
ID:o0V9YTq5oz7QIFCPkyhhtuzaO6Z-Zxc4qwkHba8bdv fZWbGSYRJZpce8ahEePe0sywDDo

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070595 PG 2/2

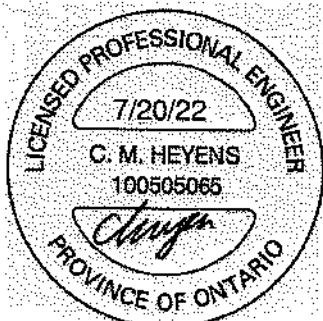
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T8	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:15 2022 Page 2
ID: c0V9Ytq6oz7QIFCPkyhhtuza06Z-cAnrmvzYCt1AFIdHOXvx0qchYJ5Ob8BUhiuakbQywDSE

CONNECTION REQUIREMENTS

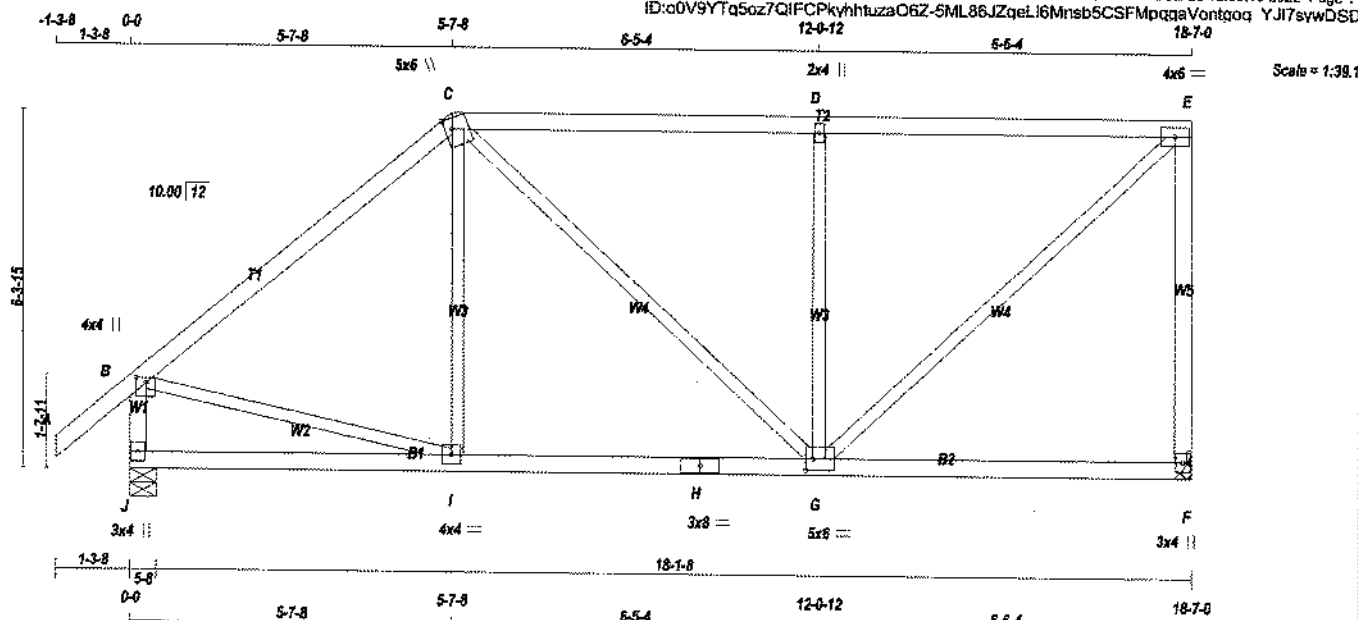
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22070568 PG 2/2

JOB NAME 425136	TRUSS NAME T9	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:00:16 2022 Page 1
ID: a0V9Ytq5oz7QIFCPkyhtuza06Z-5ML86JZqeLi6Mnsb5CSFmpggaVontgoc YJi7sywDSD



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	5.0	6.0	2.50	1.50
H	BS-t	MT20	3.0	6.0		
I	BMVWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
F	960	0	960	0	0	MECHANICAL			
J	1078	0	1078	0	0	5-8	5-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MBL. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	682	433 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0
J	763	497 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO	CSI (LC)	FR-TO			
A-B	0 / 38	-84.9 -84.9	0.12 (1)	1-2	-24 / 80	0.03 (4)	
B-C	-832 / 0	-84.9 -84.9	0.52 (1)	2-3	0 / 164	0.04 (1)	
C-D	-757 / 0	-84.9 -84.9	0.64 (1)	3-4	-681 / 0	0.44 (1)	
D-E	-757 / 0	-84.9 -84.9	0.64 (1)	4-5	0 / 1032	0.23 (1)	
E-F	-911 / 0	0.0 0.0	0.73 (1)	5-6	0 / 654	0.15 (1)	
F-G	-1036 / 0	0.0 0.0	0.11 (1)				
J-I	0 / 0	-18.5 -18.5	0.15 (4)				
I-H	0 / 638	-18.5 -18.5	0.24 (4)				
H-G	0 / 638	-18.5 -18.5	0.24 (4)				
G-F	0 / 0	-18.5 -18.5	0.18 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.62")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.62")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.73/1.00 (E-F:1), BC=0.24/1.00 (G-I:4), WB=0.44/1.00 (D-G:1), SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

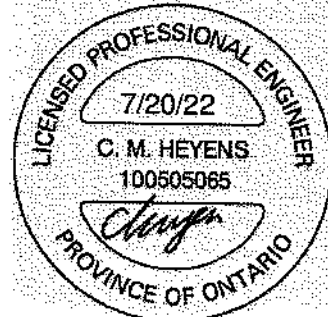
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

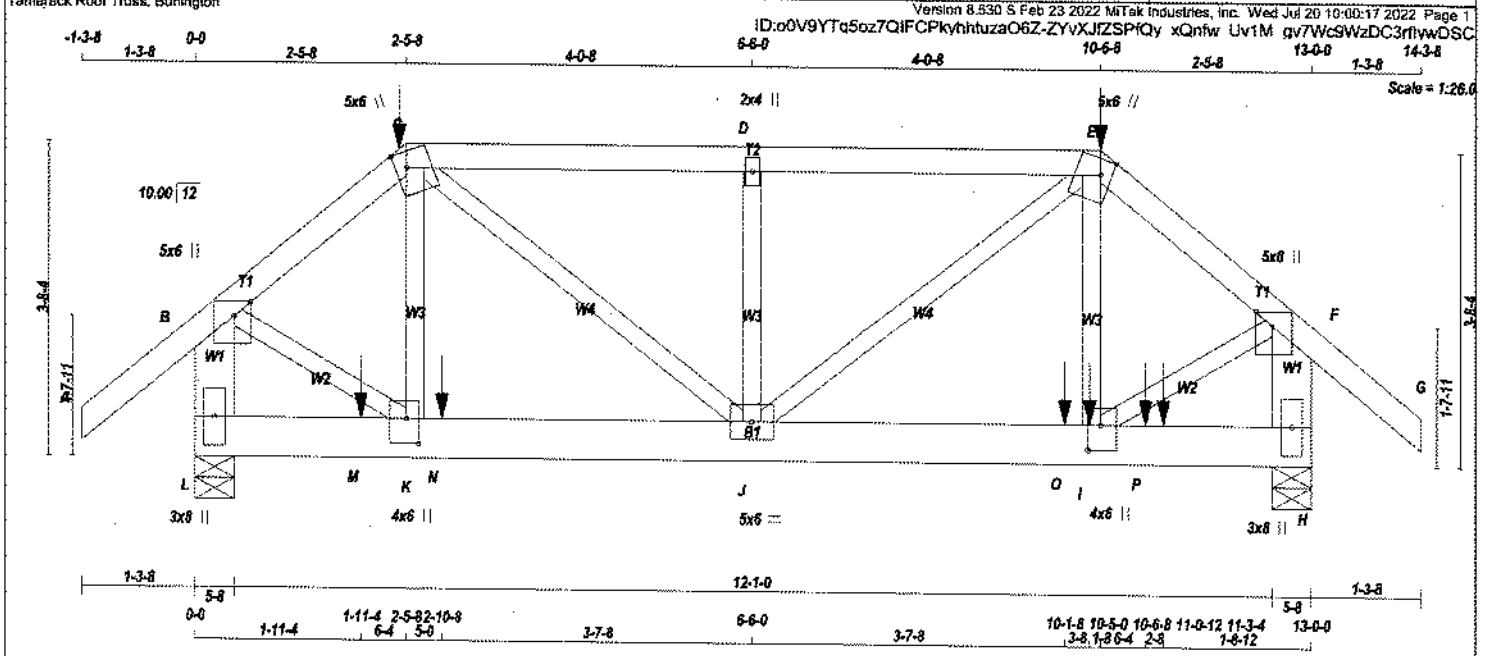
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.99 (I) (INPUT = 0.90)
JSI METAL = 0.25 (G) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070569



TOTAL HEIGHT = 2 X 67 = 134 ft. [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-E	12	SIDE(61.0)
E-G	12	SIDE(61.0)
L-B	2	TOP
H-F	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-H	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
I-E	5	SIDE(275.9)
2x3	5	
J-C	6	
D-J	6	
J-E	6	
K-B	6	
I-F	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT 1683	DOWN 1683	0	5-8
L	0	0	5-8	5-8
H	2925	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
L	1189 788 / 0 0 / 0 0 / 0 401 / 0 0 / 0
H	2066 1369 / 0 0 / 0 0 / 0 697 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 38	K-C	-137 / 8
B-C	-1441 / 0	C-J	0 / 1134
C-D	-1977 / 0	J-D	-396 / 0
D-E	-1977 / 0	J-E	-137 / 0
E-F	-2664 / 0	I-E	0 / 1454
F-G	0 / 38	B-K	0 / 1221
L-B	-1676 / 0	I-F	0 / 2257
H-F	-2909 / 0		
L-M	0 / 0		
M-K	0 / 0		
K-N	0 / 1098		
N-J	0 / 1098		
J-O	0 / 2083		
O-I	0 / 2083		
I-P	0 / 0		
P-H	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-69	-69	---	BACK	VERT	---	C1
E	10-6-8	-69	-69	---	BACK	VERT	---	C1
M	1-11-4	-5	-5	---	BACK	VERT	---	C1
N	2-10-8	-243	-243	---	BACK	VERT	---	C1
O	10-1-8	-243	-243	---	BACK	VERT	---	C1
P	11-0-12	-1243	-1243	---	FRONT	VERT	---	C1
P	11-0-12	-5	-5	---	BACK	VERT	---	C1
P	11-3-4	-262	-262	---	FRONT	VERT	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.43")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.43")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TO=0.13/1.00 (D-E:1), BC=0.27/1.00 (I-J:1), WB=0.28/1.00 (F-I:1), SS=0.87/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

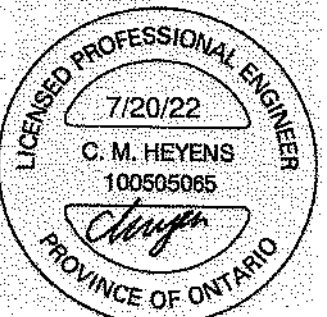
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.47 (I) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY
DWG # TR22070570 PG 1/2

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T10	1	2	TRUSS DESC.		

Tamereck Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitak Industries, Inc. Wed Jul 20 10:00:17 2022 Page 2

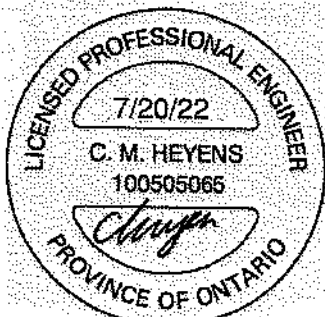
ID:00V9YTq5oz7QIFCPkvhtuzaO6Z-ZYvXJfZSPiQy xQnfw Uv1M gy7Wc9WzDC3rlywDSC

PLATES (table is in inches)

IT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW+p	MT20	5.0	6.0	2.00	2.25
H	BMV1+p	MT20	3.0	8.0		
I	BMWW+1	MT20	4.0	6.0	3.50	1.75
J	BMWWW-1	MT20	5.0	6.0		
K	BMWW+1	MT20	4.0	6.0	3.50	1.75
L	BMV1+p	MT20	3.0	8.0		

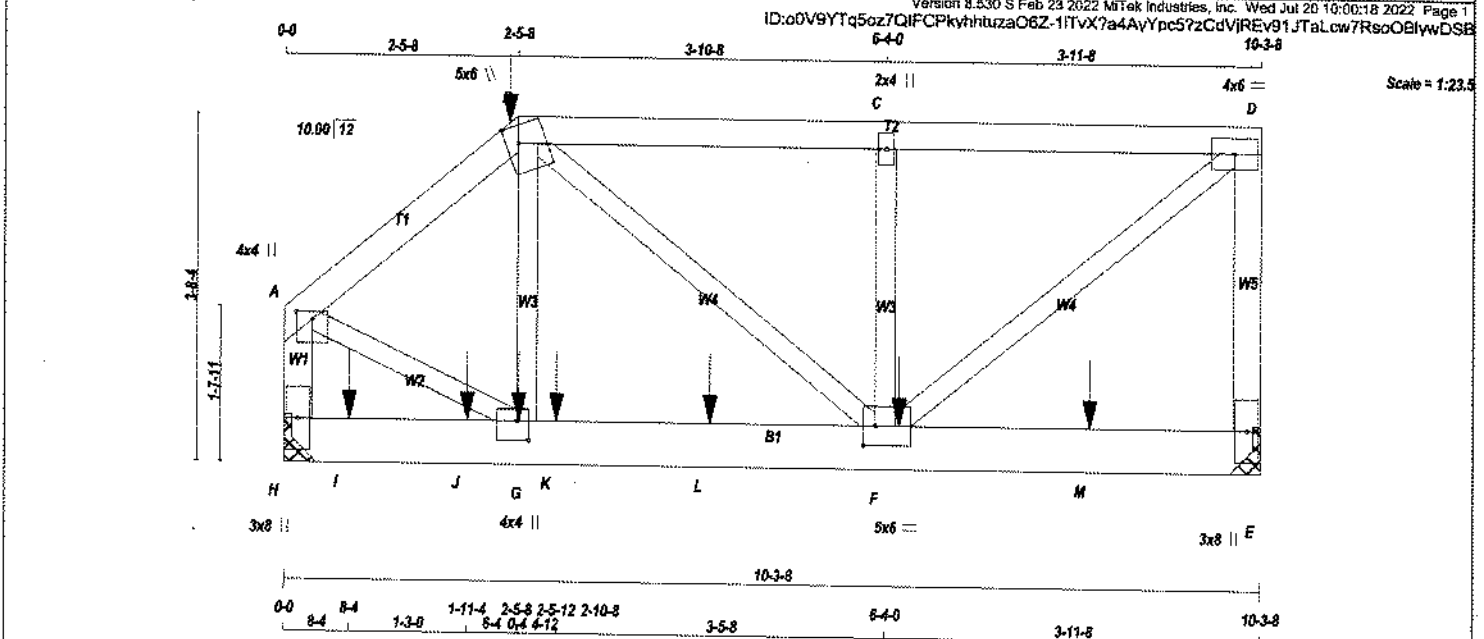
NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070570 PG 2/2

JOB NAME 425136	TRUSS NAME T11	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
H - A	2x4	DRY	No.2	SPF	
H - E	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B	12	SIDE (61.0)
B - D	12	SIDE (61.0)
D - E	12	TOP
H - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H - E	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	1777	0	1777	0	MECHANICAL	
H	2305	0	2305	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E, H. MINIMUM BEARING LENGTH AT JOINT E = 4-0, JOINT H = 4-0.

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1257	822/0	0/0	0/0	0/0	436/0	0/0
H	1630	1068/0	0/0	0/0	0/0	562/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)
FR-TO				
A - B	-1857/0	-84.9	-84.9	0.06 (1)
B - C	-1663/0	-84.9	-84.9	0.11 (1)
C - D	-1663/0	-84.9	-84.9	0.11 (1)
E - D	-1527/0	0.0	0.0	0.16 (1)
H - A	-1978/0	0.0	0.0	0.11 (1)
H - I	0/0	-18.5	-18.5	0.08 (1)
I - J	0/0	-18.5	-18.5	0.08 (1)
J - G	0/0	-18.5	-18.5	0.08 (1)
G - K	0/1441	-18.5	-18.5	0.22 (1)
K - L	0/1441	-18.5	-18.5	0.22 (1)
L - F	0/1441	-18.5	-18.5	0.22 (1)
F - M	0/0	-18.5	-18.5	0.15 (1)
M - E	0/0	-18.5	-18.5	0.15 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
B	2-5-8	-89	-69	---	FRONT	VERT	TOTAL	C1
F	6-5-12	-363	-363	---	BACK	VERT	TOTAL	C1
G	2-5-12	-363	-363	---	BACK	VERT	TOTAL	C1
I	8-4	-365	-365	---	BACK	VERT	TOTAL	C1
J	1-11-4	-5	-5	---	FRONT	VERT	TOTAL	C1
K	2-10-8	-242	-242	---	FRONT	VERT	TOTAL	C1
L	4-5-12	-363	-363	---	BACK	VERT	TOTAL	C1
M	8-5-12	-363	-363	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.34")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.34")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.03")

CSI: TC=0.16/1.00 (D-E:1), BC=0.22/1.00 (F-G:1), WB=0.27/1.00 (D-F:1), SS=0.19/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

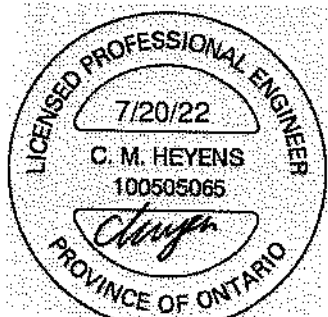
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.32 (G) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T11	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:18 2022 Page 2
ID:00V9YTq5oz7QIFCPkyhhiuza06Z-1fTvX7a4AvYpc57zCdVjREv91JTALcw7RsoOBlywDSB

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-l	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	8.0		
F	BMWW/W-l	MT20	5.0	6.0	2.50	1.50
G	BMWW+l	MT20	4.0	4.0	2.50	1.50
H	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



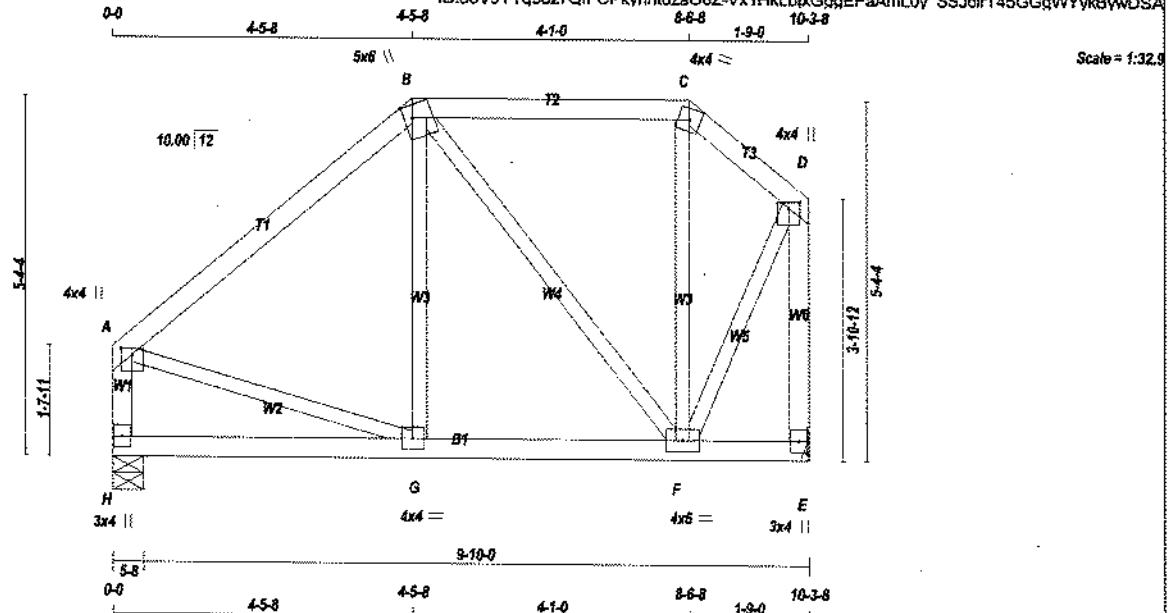
STRUCTURAL COMPONENT ONLY
DWG # TR22070571 PG 2/2

JOB NAME 425136	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:00:19 2022 Page 1

ID:00V9YTq5oz7QIFCPkyhhuza06Z-Vx1HkLbXGggEFaAmL0y SSJoirT45GGgWYyk8ywD5A



Scale = 1:32.0

LUMBER					DESCR.	
CHORDS	SIZE	LUMBER	DRY	NO.2	SPF	
A - B	2x4	DRY	No.2	SPF		
B - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
H - A	2x4	DRY	No.2	SPF		
E - D	2x4	DRY	No.2	SPF		
H - E	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-L	MT20	4.0	6.0		
G	BMVWW-L	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
H	532	0	532	0
E	532	0	532	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	378	240/0	0/0	0/0	0/0	0/0	138/0	0/0
E	378	240/0	0/0	0/0	0/0	0/0	138/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	-358/0	-84.9	-84.9 0.22 (1)	G-B	0/74	0.03 (4)	
B-C	-159/0	-84.9	-84.9 0.18 (1)	B-F	-187/0	0.13 (1)	
C-D	-211/0	-84.9	-84.9 0.03 (1)	F-C	-111/2	0.05 (1)	
H-A	-497/0	0.0	0.0 0.05 (1)	A-G	0/295	0.06 (1)	
E-D	-520/0	0.0	0.0 0.13 (1)	F-D	0/349	0.08 (1)	
H-G	0/0	-18.5	-18.5 0.09 (4)				
G-F	0/274	-18.5	-18.5 0.11 (4)				
F-E	0/0	-18.5	-18.5 0.03 (4)				

TOTAL WEIGHT = 51 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.4	PSF	
TOTAL LOAD =	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA D86-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22/1.00 (A-B:1), BC=0.11/1.00 (F-G:4)
WB=0.13/1.00 (B-F:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

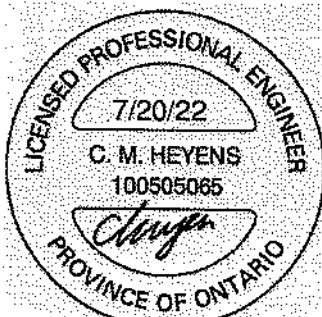
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

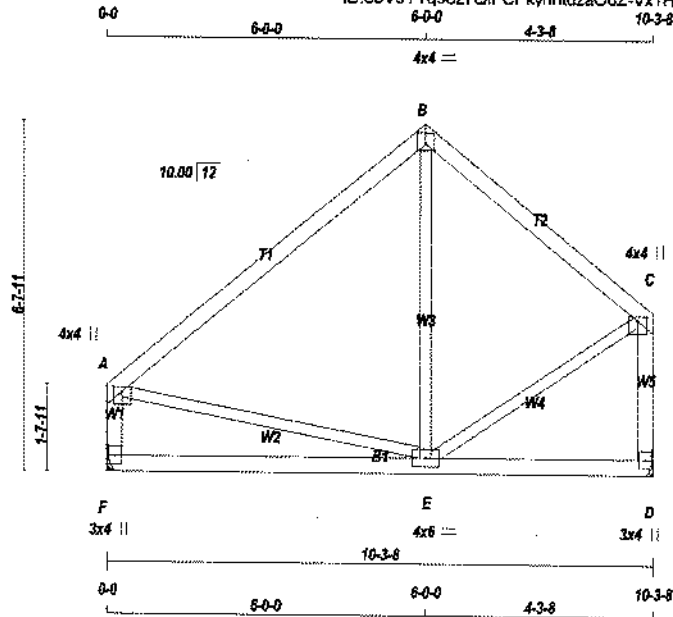
JSI GRIP= 0.45 (F) (INPUT = 0.90)
JSI METAL= 0.12 (A) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070572

JOB NAME 425136	TRUSS NAME T13	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Jul 20 10:00:19 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhhtuza06Z-Vx1Hkl.bixGggEFaAmL0y SSG3iqq47RGqWYykBywDSA



TOTAL WEIGHT = 4 X 45 = 181 lb
[M] [F]

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMW+p	MT20	4.0	4.0	1.00 2.00
B	TTW+p	MT20	4.0	4.0	1.50 2.00
C	TMW+p	MT20	4.0	4.0	1.00 2.00
D	BMV1+p	MT20	3.0	4.0	
E	BMWVVV-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
F	532	0	532	0	0	MECHANICAL
D	532	0	532	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	MAX / MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	378	240 / 0	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0
D	378	240 / 0	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)	UNBRAC
FR-TO		FROM	TO	FR-TO			
A-B	-280 / 0	-84.9	-84.9 0.39 (1)	E-B	-69 / 60	0.05 (1)	
B-C	-286 / 0	-84.9	-84.9 0.20 (1)	A-E	0 / 226	0.05 (1)	
F-A	-486 / 0	0.0	0.0 0.05 (1)	E-C	0 / 262	0.06 (1)	
D-C	-508 / 0	0.0	0.0 0.05 (1)				
F-E	0 / 0	-18.5	-18.5 0.15 (4)				
E-D	0 / 0	-18.5	-18.5 0.15 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.4	PSF	
TOTAL LOAD =	36.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.39/1.00 (A-B:1), EC=0.15/1.00 (E-F:4)
WB=0.06/1.00 (C-E:1), SS=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

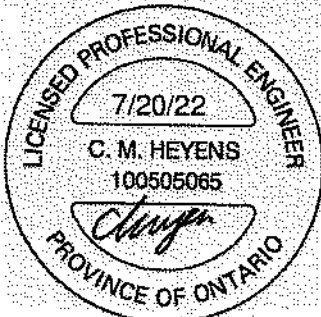
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	PL)	PL)
MT20	650	371	1747	786	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.41 (A) (INPUT = 0.90)
JSI METAL = 0.11 (A) (INPUT = 1.00)



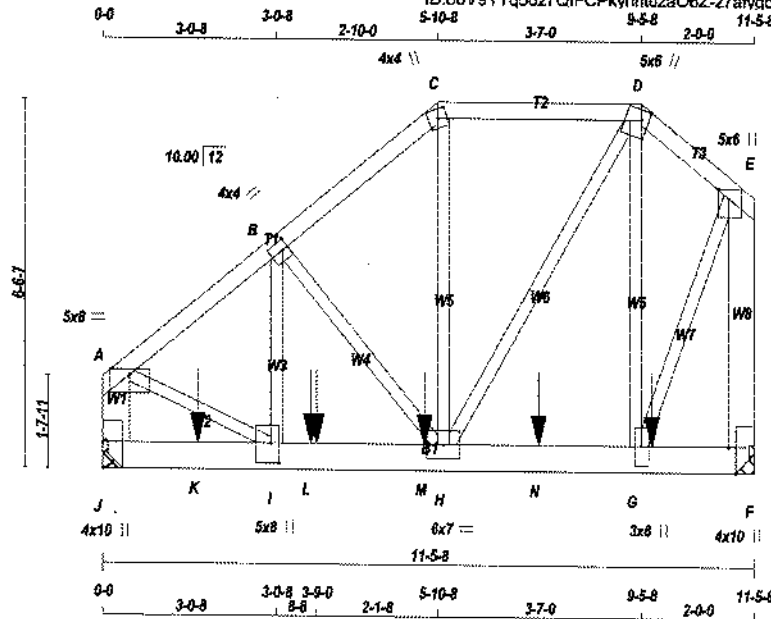
STRUCTURAL COMPONENT ONLY
DWG # TR22070573

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T14	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 9.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:20 2022 Page 1

ID:00V9YTq5oz7QIFCPkYthtza06Z-z7afygcLiaoXrP9MK2XBxI P266cpUsQvAHVGdywDS9



TOTAL WEIGHT = 2 X 74 = 148 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
J - A	2x6	DRY	No.2	SPF	
F - E	2x6	DRY	No.2	SPF	
J - F	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
D - E	12	TOP
J - A	2	TOP
F - E	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - F	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
G - D	6	SIDE(158.7)
H - C	6	SIDE(158.7)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
J	4578	0	0	MECHANICAL
F	4080	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT JOINT J = 4'-0, JOINT F = 4'-0.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
J	3243 2100 / 0 0 / 0 0 / 0 1143 / 0 0 / 0
F	2891 1866 / 0 0 / 0 0 / 0 1025 / 0 0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. (LC)
FR-TO					FR-TO		
A-B	-4218 / 0	-84.9	-84.9 0.13 (1)	4.55	I-B	0 / 1535	0.19 (1)
B-C	-3119 / 0	-84.9	-84.9 0.09 (1)	5.18	B-H	-1405 / 0	0.26 (1)
C-D	-2410 / 0	-84.9	-84.9 0.12 (1)	5.67	H-C	0 / 1744	0.22 (1)
D-E	-1814 / 0	-84.9	-84.9 0.04 (1)	6.25	H-D	0 / 2109	0.26 (1)
J-A	-4128 / 0	0.0	0.0 0.15 (1)	7.02	G-D	-902 / 0	0.30 (1)
F-E	-4186 / 0	0.0	0.0 0.57 (1)	8.98	A-I	0 / 3498	0.43 (1)
					G-E	0 / 3242	0.40 (1)
J-K	0 / 0	-18.5	-18.5 0.26 (1)	10.00			
K-L	0 / 0	-18.5	-18.5 0.28 (1)	10.00			
L-L	0 / 3246	-18.5	-18.5 0.44 (1)	10.00			
L-M	0 / 3246	-18.5	-18.5 0.44 (1)	10.00			
M-H	0 / 3246	-18.5	-18.5 0.44 (1)	10.00			
H-N	0 / 1371	-18.5	-18.5 0.34 (1)	10.00			
N-G	0 / 1371	-18.5	-18.5 0.34 (1)	10.00			
G-F	0 / 0	-18.5	-18.5 0.10 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	9-7-12	-928	-928	---	FRONT	VERT	TOTAL	---	C1
K	1-7-12	-928	-928	---	FRONT	VERT	TOTAL	---	C1
L	3-7-12	-928	-928	---	FRONT	VERT	TOTAL	---	C1
L	3-8-0	-651	-651	---	BACK	VERT	TOTAL	---	C1
M	5-7-12	-928	-928	---	FRONT	VERT	TOTAL	---	C1
N	7-7-12	-928	-928	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.4	PSF	
TOTAL LOAD =	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.06")

CSI: TC=0.57/1.00 (E-F-1) . BC=0.44/1.00 (H-I-1) .
WB=0.43/1.00 (A-I-1) , SI=0.64/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

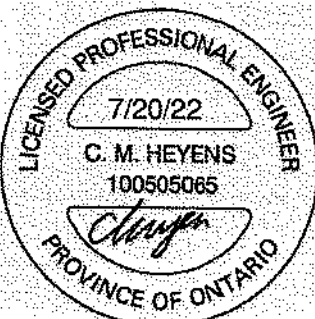
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.59 (I) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070574

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T14	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 6.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:00:20 2022 Page 2
ID:00VBYTq5oz7QIFCPkyhtuzaO6Z-z7afvqcliaoXrP9MK2XBX1 P266cpUsQvAHVGdywDS9

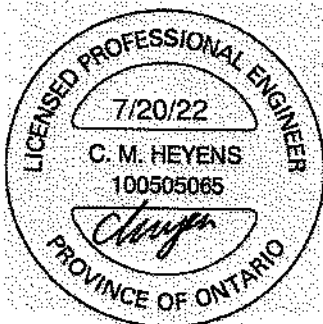
PLATES (table is in inches)

IT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-l	MT20	4.0	4.0	2.00	1.25
C	TTW+m	MT20	4.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	5.0	6.0	2.00	2.25
F	BMV1+t	MT20	4.0	10.0	Edge	1.50
G	BMWW+t	MT20	3.0	8.0		
H	BMWW-l	MT20	6.0	7.0		
I	BMWW+t	MT20	5.0	8.0	4.25	1.75
J	BMV1+t	MT20	4.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



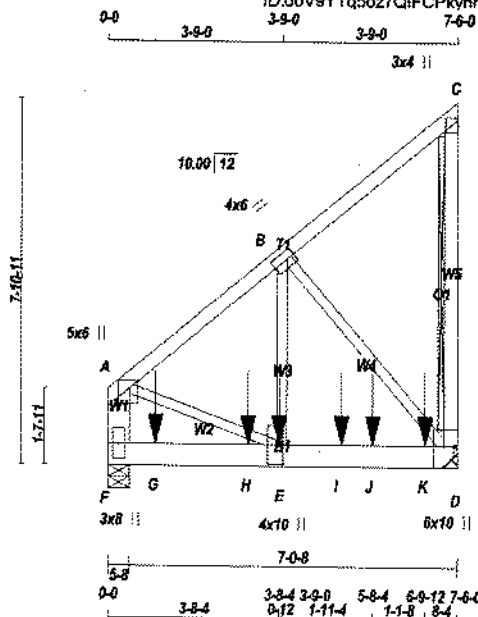
STRUCTURAL COMPONENT ONLY
DWG # TR22070574 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T15	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:00:21 2022 Page 1

ID:00V9YTq5oz7QIFCPkYhtuzaO6Z-RK8190dzTtwOTYkYm2Q3UXhrWQbYbZ8q12o4yWDS8



Scale: 1/4\"=1'

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	

EXCEPT

DRY: SEASONED LUMBER.

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122\"X3\") SPIRAL NAILS		
A - C	1	12
C - D	1	12
F - A	2	12
BOTTOM CHORDS : (0.122\"X3\") SPIRAL NAILS		
F - D	2	12
WEBS : (0.122\"X3\") SPIRAL NAILS		
B - E	1	6
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	3063	0	3063	0	0	MECHANICAL
F	2454	0	2454	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2168	1408 / 0	0 / 0	0 / 0	0 / 0	760 / 0	0 / 0
F	1737	1128 / 0	0 / 0	0 / 0	0 / 0	609 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3\" COMMON WIRE NAILS @ 6\" O.C. WITH 3\" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. UNBRACED LENGTH	FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO					
A-B	-2211 / 0	-84.9	-84.9	0.12 (1)	5.87	E-B	0 / 3032	0.38 (1)
B-C	-22 / 0	-84.9	-84.9	0.09 (1)	6.25	B-D	-2626 / 0	0.70 (1)
D-C	-126 / 0	0.0	0.0	0.03 (1)	7.81	A-E	0 / 1804	0.22 (1)
F-A	-2113 / 0	0.0	0.0	0.08 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.23 (1)	10.00			
G-H	0 / 0	-18.5	-18.5	0.23 (1)	10.00			
H-E	0 / 0	-18.5	-18.5	0.23 (1)	10.00			
E-I	0 / 1715	-18.5	-18.5	0.52 (1)	10.00			
I-J	0 / 1715	-18.5	-18.5	0.52 (1)	10.00			
J-K	0 / 1715	-18.5	-18.5	0.52 (1)	10.00			
K-D	0 / 1715	-18.5	-18.5	0.52 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-8-4	-1235	-1235	---	FRONT	VERT	TOTAL	---	C1
G	1-0-4	-363	-363	---	BACK	VERT	TOTAL	---	C1
H	3-0-4	-363	-363	---	BACK	VERT	TOTAL	---	C1
I	5-0-4	-363	-363	---	BACK	VERT	TOTAL	---	C1
J	5-8-4	-667	-667	---	FRONT	VERT	TOTAL	---	C1
K	6-9-12	-365	-365	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 45 = 91 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(56% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.25\")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03\")
ALLOWABLE DEFL.(TL) = L/360 (0.25\")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05\")

CSI: TC=0.12/1.00 (A-B:1), BC=0.52/1.00 (D-E:1)
WB=0.70/1.00 (B-D:1), SI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

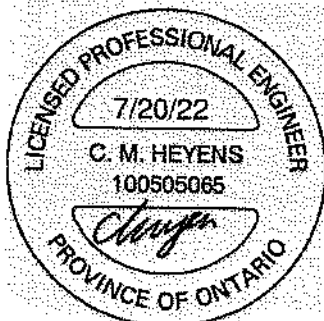
NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.41 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070575 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T15	1	2	TRUSS DESC.		

Tamrack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:21 2022 Page 2
ID:00V9YTq5oz7QIFCPkyhhtuzaO6Z-RK8190dzTlwQTYkYtm2Q3XhrWQbYbZ8q12o4ywDS8

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	6.0	2.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+t	MT20	6.0	10.0	Edge	2.50
E	BMWW-H	MT20	4.0	10.0	5.00	1.50
F	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



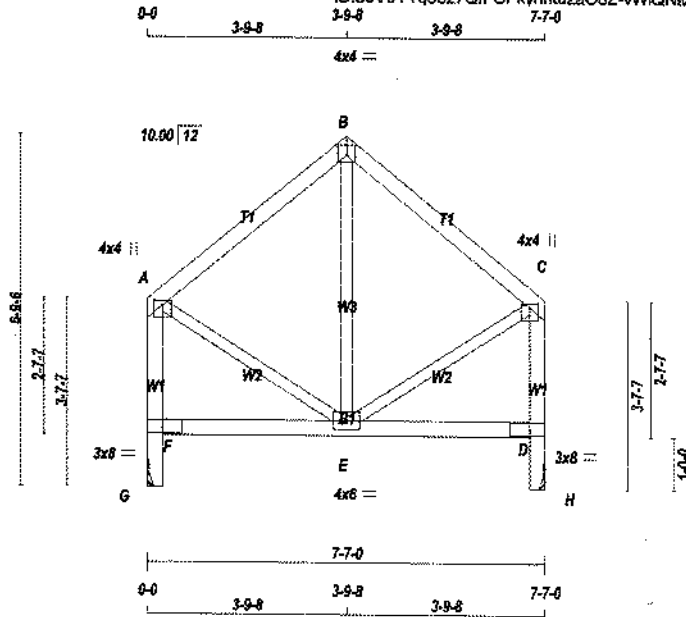
STRUCTURAL COMPONENT ONLY
DWG # TR22070575 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T16	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:22 2022 Page 1

ID: c0V9YTq5oz7QIFCPkyhhtuza06Z-vWQNMdbEB2F5iJRTZfo43rywu0HUWJMUMCKWwvDS7



Scale = 1/42.7

TOTAL WEIGHT = 3 X 38 = 115 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
C - A	2x4	DRY	No.2	SPF	
H - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - TMVW+p	MT20	4.0	4.0	1.00	2.00		
B - TTW-p	MT20	4.0	4.0	1.50	2.00		
C - TMVW+p	MT20	4.0	4.0	1.00	2.00		
D - BVM-H	MT20	3.0	8.0	1.50	4.50		
E - BMVWV-H	MT20	4.0	8.0				
F - BVM-H	MT20	3.0	8.0	1.50	4.50		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			MAXIMUM FACTORED			INPUT	REQD
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	392	0	392	0	0	MECHANICAL	
H	392	0	392	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G, H. MINIMUM BEARING LENGTH AT JOINT G = 1-8, JOINT H = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	278	177 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
H	278	177 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)
A-B	-201 / 0	-84.9	-84.9	0.16 (1)	6.25	E-B	-78 / 31	0.04 (1)	
B-C	-201 / 0	-84.9	-84.9	0.16 (1)	6.25	A-E	0 / 135	0.03 (1)	
C-F	-392 / 0	0.0	0.0	0.04 (1)	7.81	E-C	0 / 135	0.03 (1)	
F-A	-352 / 0	0.0	0.0	0.10 (1)	7.81				
H-D	-392 / 0	0.0	0.0	0.04 (1)	7.81				
D-C	-352 / 0	0.0	0.0	0.10 (1)	7.81				
F-E	0 / 45	-18.5	-18.5	0.06 (4)	10.00				
E-D	0 / 45	-18.5	-18.5	0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.25")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.25")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1), BC=0.06/1.00 (E-F:4)
WB=0.04/1.00 (B-E:1), SS=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

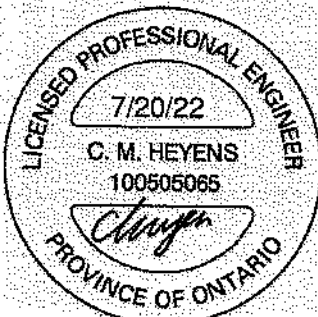
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (C) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

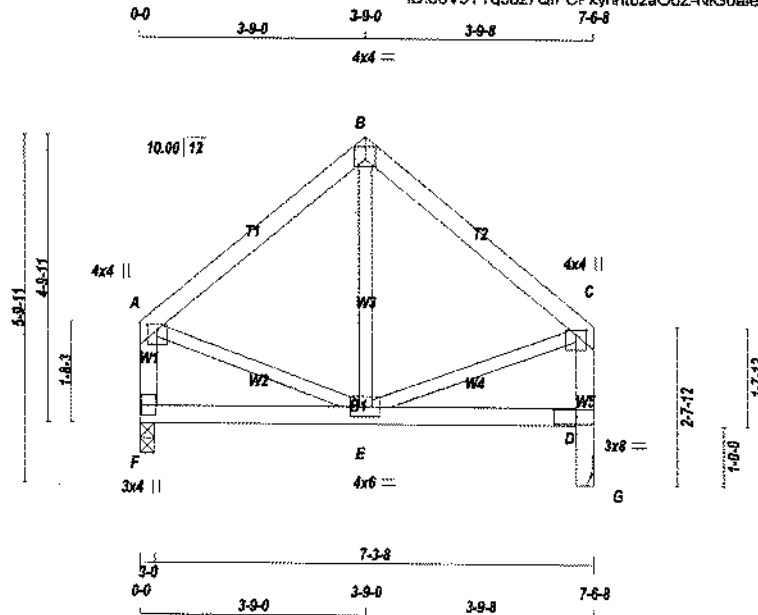


STRUCTURAL COMPONENT ONLY
DWG # TR22070576

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T17	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 10:00:23 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhthtzaO6Z-NiGoaleD7VA6lsux7B4u8lc?vKD10xsb8W9tywDS6



Scale = 1:37.0

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
G - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
	DRY, SEASONED LUMBER.				

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - TMVW+p	MT20	4.0	4.0	1.00	2.00		
B - TTW-p	MT20	4.0	4.0	1.50	2.00		
C - TMVW+p	MT20	4.0	4.0	1.00	2.00		
D - BVM-I	MT20	3.0	8.0	1.50	4.50		
E - BMVWW-I	MT20	4.0	6.0				
F - BMV1+p	MT20	3.0	4.0				

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	390	0	390	0
G	390	0	0	390

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	277	176 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
G	277	176 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)
FR-TO				
A-B	-222 / 0	-84.9	-84.9 0.21 (1)	6.25
B-C	-222 / 0	-84.9	-84.9 0.21 (1)	6.25
F-A	-363 / 0	0.0	0.0 0.04 (1)	7.81
G-D	-390 / 0	0.0	0.0 0.04 (1)	7.81
D-C	-362 / 0	0.0	0.0 0.04 (1)	7.81
F-E	0 / 0	-18.5	-18.5 0.07 (4)	10.00
E-D	0 / 0	-18.5	-18.5 0.07 (4)	10.00

TOTAL WEIGHT = 2 X 34 = 67 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 35.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (B-C:1), BC=0.07/1.00 (E-F:4), WB=0.04/1.00 (A-E:1), SI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

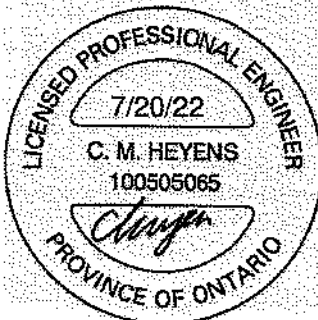
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	
MT20	650	371	1747	788 1967 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (C) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

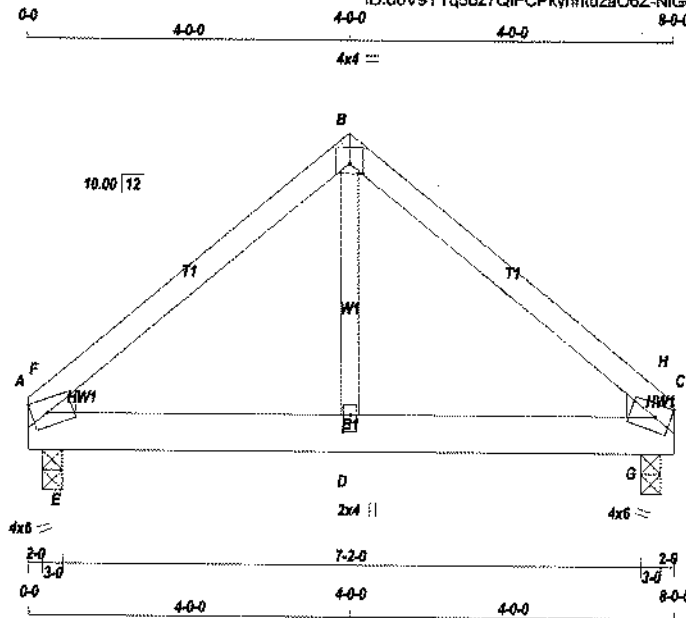


STRUCTURAL COMPONENT ONLY
DWG # TR22070577

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T18	2	1	TRUSS DESC.		

Tamareck Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:00:23 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhhuza06Z-NiGoaieD7VA6isux?B4u8lc1LKDV0xsb8W9tywDS6



Scale = 1:27.6

TOTAL WEIGHT = 2 X 30 = 60 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
A - C	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	6.0	1.50	2.00
B	TTW-p	MT20	4.0	4.0	1.50	2.00
C	TMBH1-m	MT20	4.0	6.0	1.50	2.00
D	BMW-w	MT20	2.0	4.0	2.50	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
A	413	0	413	0	0
B	413	0	413	0	0
C	413	0	413	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
A	293	186 / 0	0 / 0	0 / 0	107 / 0	0 / 0
C	293	186 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	UNBRAC LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	CS1 (LC)
A-F	-256 / 0	-84.9	-84.9	0.10 (1)	6.25	D-B	0 / 177	0.04 (1)			
F-B	-350 / 0	-84.9	-84.9	0.12 (1)	6.25	E-F	-280 / 0	0.09 (1)			
B-H	-350 / 0	-84.9	-84.9	0.12 (1)	6.25	G-H	-280 / 0	0.09 (1)			
H-C	-256 / 0	-84.9	-84.9	0.10 (1)	6.25						
A-E	0 / 256	-18.5	-18.5	0.11 (1)	10.00						
E-D	0 / 256	-18.5	-18.5	0.11 (1)	10.00						
D-G	0 / 256	-18.5	-18.5	0.11 (1)	10.00						
G-C	0 / 256	-18.5	-18.5	0.11 (1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CS1: TC=0.12/1.00 (B-F:1), BC=0.11/1.00 (D-G:1), WB=0.04/1.00 (B-D:1), SB=0.16/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

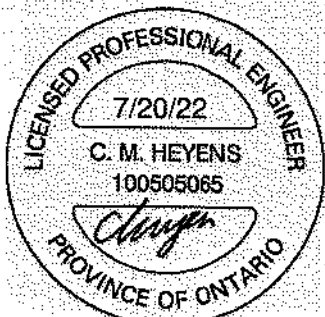
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

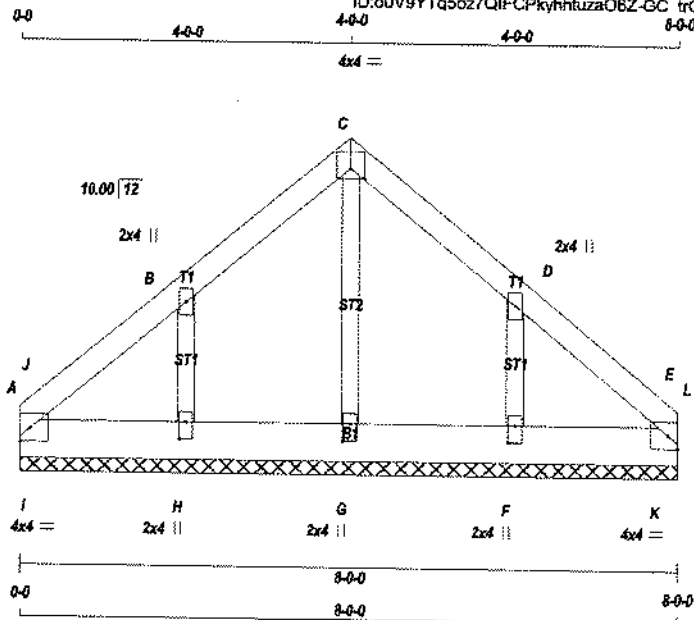
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.56 (B) (INPUT = 0.90)
JSI METAL = 0.09 (A) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070578

JOB NAME 425136	TRUSS NAME G18	QUANTITY 1	PLV 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 24-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	4.0	4.0		Edge
B	TMW1-w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMW1-w	MT20	2.0	4.0		
E	TMB1-I	MT20	4.0	4.0		Edge
F, G, H						
F	BMW1+w	MT20	2.0	4.0	2.50	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LCT MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED UNBRAC LENGTH	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-J	-48 / 0	-84.9	-84.9 0.01 (1)	G-C	-88 / 0	6.25	0.02 (1)
J-B	-48 / 0	-84.9	-84.9 0.05 (1)	H-B	-187 / 0	6.25	0.03 (1)
B-C	-54 / 0	-84.9	-84.9 0.05 (1)	F-D	-187 / 0	6.25	0.03 (1)
C-D	-54 / 0	-84.9	-84.9 0.05 (1)	I-J	-68 / 0	6.25	0.00 (1)
D-L	-48 / 0	-84.9	-84.9 0.05 (1)	K-L	-68 / 0	6.25	0.00 (1)
L-E	-48 / 0	-84.9	-84.9 0.01 (1)				
A-I	0 / 43	-18.5	-18.5 0.01 (1)				
I-H	0 / 43	-18.5	-18.5 0.01 (1)				
H-G	0 / 33	-18.5	-18.5 0.01 (1)				
G-F	0 / 33	-18.5	-18.5 0.01 (1)				
F-K	0 / 43	-18.5	-18.5 0.01 (1)				
K-E	0 / 43	-18.5	-18.5 0.01 (1)				

TOTAL WEIGHT = 32 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (B-C-1), BC=0.01/1.00 (H-E-1),

WB=0.03/1.00 (B-H-1), SS=0.06/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

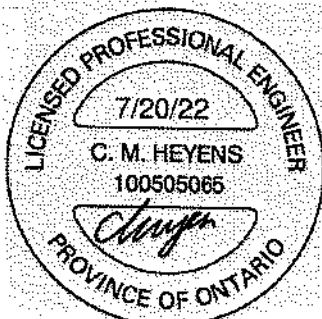
PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (C) (INPUT = 0.90)

JSI METAL= 0.10 (D) (INPUT = 1.00)

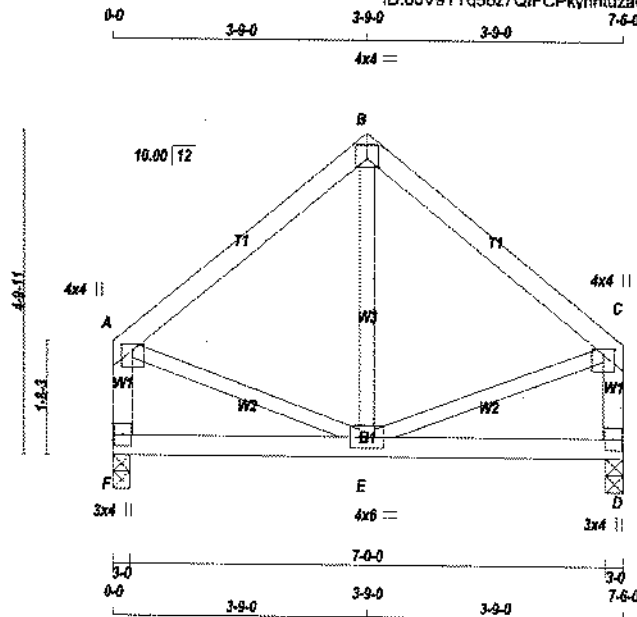


STRUCTURAL COMPONENT ONLY
DWG # TR22070562

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T19	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:24 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhtuzaO6Z-svqAo2frotzK0T7Zuc7hV9AZIYHIOz?qofPpYwDS5



Scale = 1:32.9

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMVt+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	6.0		
F	BMVt+p	MT20	3.0	4.0		

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	388	0	388	0	0	3-0	3-0
D	388	0	388	0	0	3-0	3-0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	275	175 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
D	275	175 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)	CSI (LC)
FR-TO		FROM	TO					
A-B	-220 / 0	-84.9	-84.9	0.15 (1)	6.25	E-B	-37 / 48	0.02 (4)
B-C	-220 / 0	-84.9	-84.9	0.15 (1)	6.25	A-E	0 / 180	0.04 (1)
F-A	-361 / 0	0.0	0.0	0.04 (1)	7.81	E-C	0 / 180	0.04 (1)
D-C	-361 / 0	0.0	0.0	0.04 (1)	7.81			
F-E	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

TOTAL WEIGHT = 2 X 32 = 65 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.15/1.00 (A-B:1), BC=0.07/1.00 (E-F:4)
WB=0.04/1.00 (A-E:1), SS=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

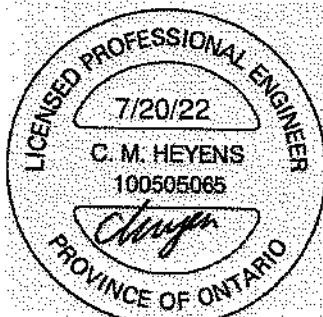
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.29 (A) (INPUT = 0.90)

JSI METAL= 0.08 (A) (INPUT = 1.00)

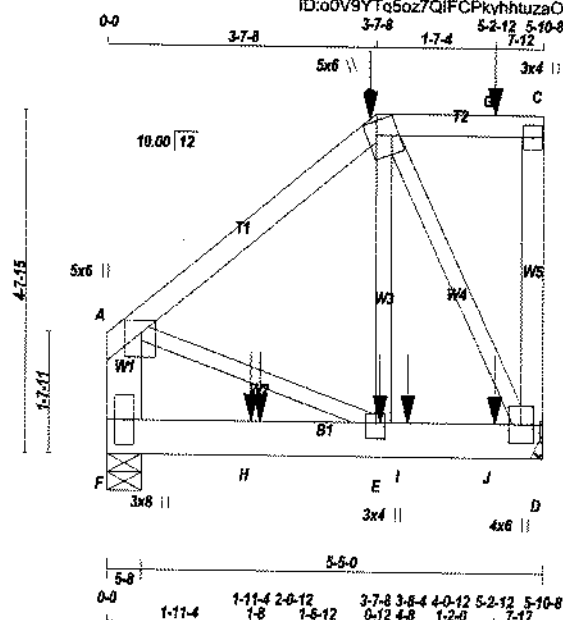


STRUCTURAL COMPONENT ONLY
DWG # TR22070579

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T20	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:00:25 2022 Page 1



Scale = 1/32

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A-B	1	12	SIDE (61.0)
B-C	1	12	SIDE (61.0)
C-D	1	12	TOP
F-A	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
F-D	2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
D	942	0	MECHANICAL	
F	790	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2'-0.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	
D	666
F	559

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
MEMB.							
FR-TO							
A-B	-506 / 0	-84.9	-84.9	0.11 (1)	8.25	E-B	0 / 568
B-C	0 / 0	-84.9	-84.9	0.06 (1)	10.00	B-D	-861 / 0
G-C	0 / 0	-84.9	-84.9	0.06 (1)	10.00	A-E	0 / 408
D-C	-155 / 0	0.0	0.0	0.03 (1)	7.81		
F-A	-807 / 0	0.0	0.0	0.02 (1)	7.81		
F-H	0 / 0	-18.5	-18.5	0.12 (1)	10.00		
H-E	0 / 0	-18.5	-18.5	0.12 (1)	10.00		
E-I	0 / 402	-18.5	-18.5	0.08 (1)	10.00		
I-J	0 / 402	-18.5	-18.5	0.08 (1)	10.00		
J-D	0 / 402	-18.5	-18.5	0.08 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-7-8	-170	-170	---	FRONT	VERT	TOTAL	---	C1
E	3-8-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
G	5-2-12	-58	-58	---	FRONT	VERT	TOTAL	---	C1
H	1-11-4	-12	-12	---	FRONT	VERT	TOTAL	---	C1
H	2-0-12	-263	-263	---	BACK	VERT	TOTAL	---	C1
I	4-0-12	-263	-263	---	BACK	VERT	TOTAL	---	C1
J	5-2-12	-15	-15	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 35 = 70 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.11/1.00 (A-B:1), BC=0.12/1.00 (E-F:1), WB=0.16/1.00 (B-D:1), SS=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

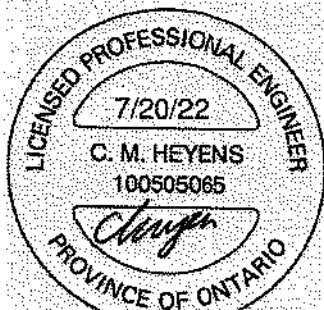
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.65 (E) (INPUT = 0.90)
JSI METAL = 0.12 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070580 PG 1/2

JOB NAME 425136	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.530 S Feb 23 2022 Mittek Industries, Inc. Wed Jul 20 10:00:25 2022 Page 2		ID:o0V9Yf5oz7QIFCPkvtthtzaC6Z-K5OY?CgUW6QqvA2J6b7MEhM07upUpQ93S7GxywDS4	

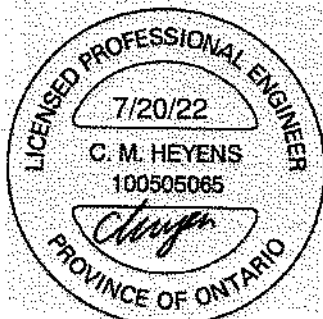
Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW+1	MT20	3.0	4.0	2.50	1.50
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

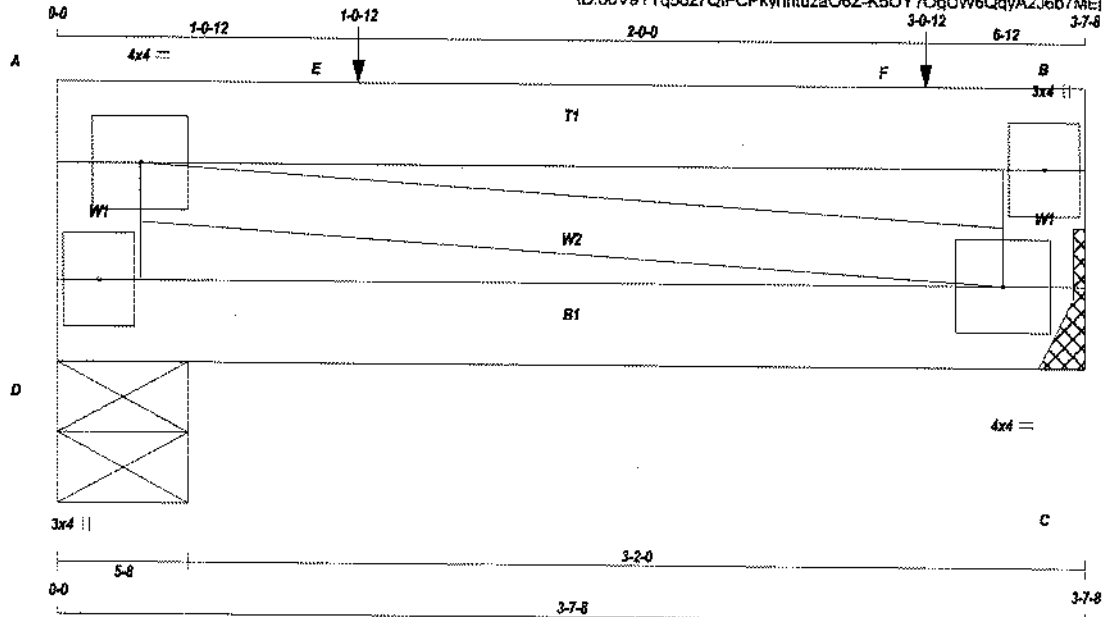
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070580 PG 2/2

JOB NAME 425136	TRUSS NAME T21	QUANTITY 2	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Jul 20 10:00:25 2022 Page 1
ID:ro0V9Ytq5oz7QIFCPkyhhtuzaO6Z-K50Y7OgUW6QqyA2J6b7MEjnhJB7v5Urr93S7GxywDS4



Scale = 1/4" = 1'-0"

TOTAL WEIGHT = 4 X 12 = 47 lb

LUMBER	DESCR.	SPF
N. L. G. A. RULES		
CHORDS	SIZE	
D - A	2x4 DRY	No.2
A - B	2x4 DRY	No.2
C - B	2x4 DRY	No.2
D - C	2x4 DRY	No.2
ALL WEBS	2x3 DRY	No.2
DRY: SEASONED LUMBER.		

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A	12	TOP
A-B	12	TOP
B-C	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMVW-p	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	4.0		
D	BMV1-p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
D	473	0	473	0
C	577	0	577	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2'-0".

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
D	337
C	411

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO												
D-A	-440 / 0	0.0	0.0	0.02 (1)	7.81							
A-E	0 / 0	-84.9	-84.9	0.29 (1)	10.00							
E-F	0 / 0	-84.9	-84.9	0.29 (1)	10.00							
F-B	0 / 0	-84.9	-84.9	0.29 (1)	10.00							
C-B	-543 / 0	0.0	0.0	0.03 (1)	7.81							
D-C	0 / 0	-18.5	-18.5	0.04 (4)	10.00							

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-0-12	-235	-235		TOP	VERT	TOTAL		C1
F	3-0-12	-248	-248		TOP	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.29/1.00 (A-B:1), BC=0.04/1.00 (C-D:4), WB=0.00/1.00 (A-C:1), SS=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	850	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.15 (C) (INPUT = 0.90)
JSI METAL = 0.04 (C) (INPUT = 1.00)

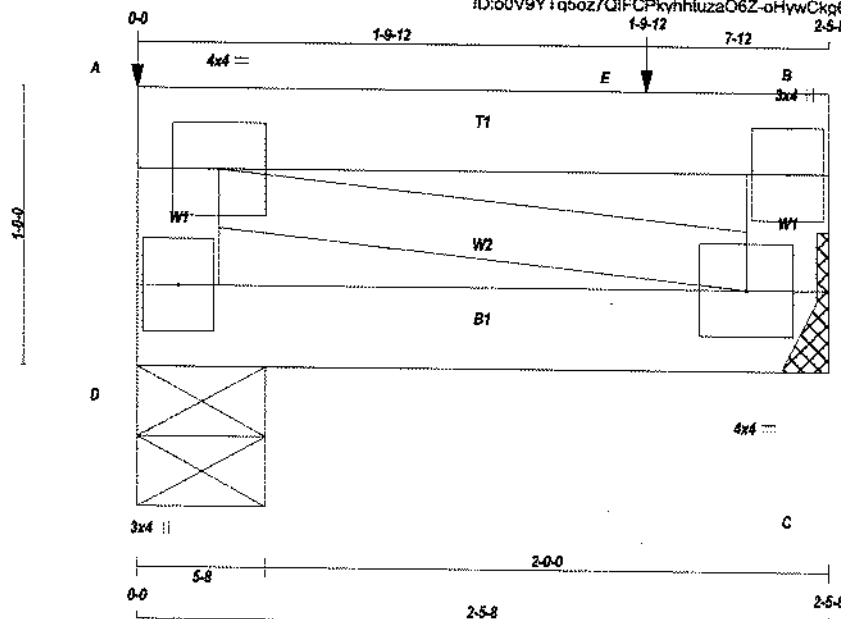


STRUCTURAL COMPONENT ONLY
DWG # TR22070581

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T22	2	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:26 2022 Page 1
ID:o0V9Ytq5oz7QIFCPkyhhuza06Z-oHywCkg6HQZhzKcWqJebmwEWIXF8DI5IH6kpUHywDS3



Scale: 1.5"=1'

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 1	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMVW-p	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	4.0		
D	BMV1-p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
D	553	0	553	0
C	381	0	381	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2'-0".

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	394	243 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0
C	257	158 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						FR-TO			
D-A	-530 / 0	0.0	0.0	0.03 (1)	7.81	A-C	0 / 0	0.00 (1)	
A-E	0 / 0	-84.9	-84.9	0.14 (1)	10.00				
E-B	0 / 0	-84.9	-84.9	0.14 (1)	10.00				
C-B	-339 / 0	0.0	0.0	0.02 (1)	7.81				
D-C	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-244	-244	--	TOP	VERT	TOTAL	--	C1
E	1-9-12	-227	-227	--	TOP	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) G1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 8 = 32 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- THCC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS1: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (C-D:4)
WB=0.00/1.00 (A-C:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

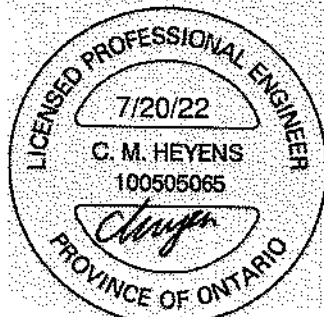
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)

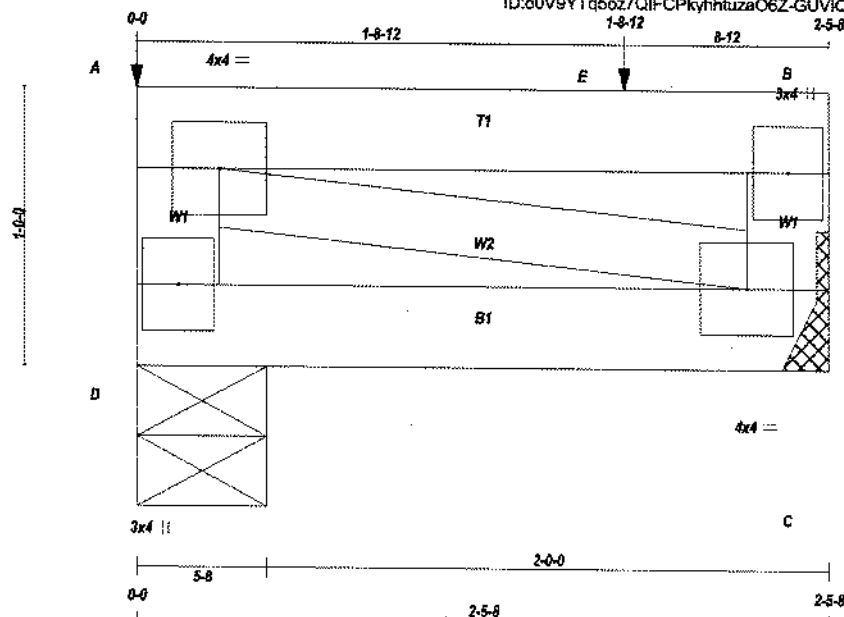


STRUCTURAL COMPONENT ONLY
DWG # TR22070582

JOB NAME 425136	TRUSS NAME T22Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
--------------------	--------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:27 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhtuza06Z-GUVIQ4hk2hYBTBiE09qJ8nhuxbsylLSWmUN0jywDS2



Scale: 1.5"=1'

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 1	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0			
B	TMV+p	MT20	3.0	4.0			
C	BMVW1-t	MT20	4.0	4.0			
D	BMV1+p	MT20	3.0	4.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
D	566	0	566	0
C	350	0	350	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2'-0."

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
D	403	248 / 0
C	248	152 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1	MAX. FACTORED (PLF)	CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
D-A	-543 / 0	0.0	0.0	0.03 (1)	7.81			A-C	0 / 0	0.00 (1)	
A-E	0 / 0	-84.9	-84.9	0.15 (1)	10.00						
E-B	0 / 0	-84.9	-84.9	0.15 (1)	10.00						
C-B	-327 / 0	0.0	0.0	0.02 (1)	7.81						
D-C	0 / 0	-18.5	-18.5	0.02 (4)	10.00						

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-246	-246	---	TOP	VERT	TOTAL	---	C1
E	1-8-12	-226	-226	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CS1: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (C-D:4)
WB=0.00/1.00 (A-C:1), SS1=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

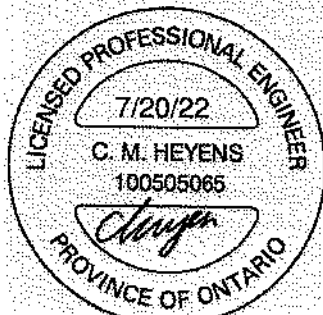
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)

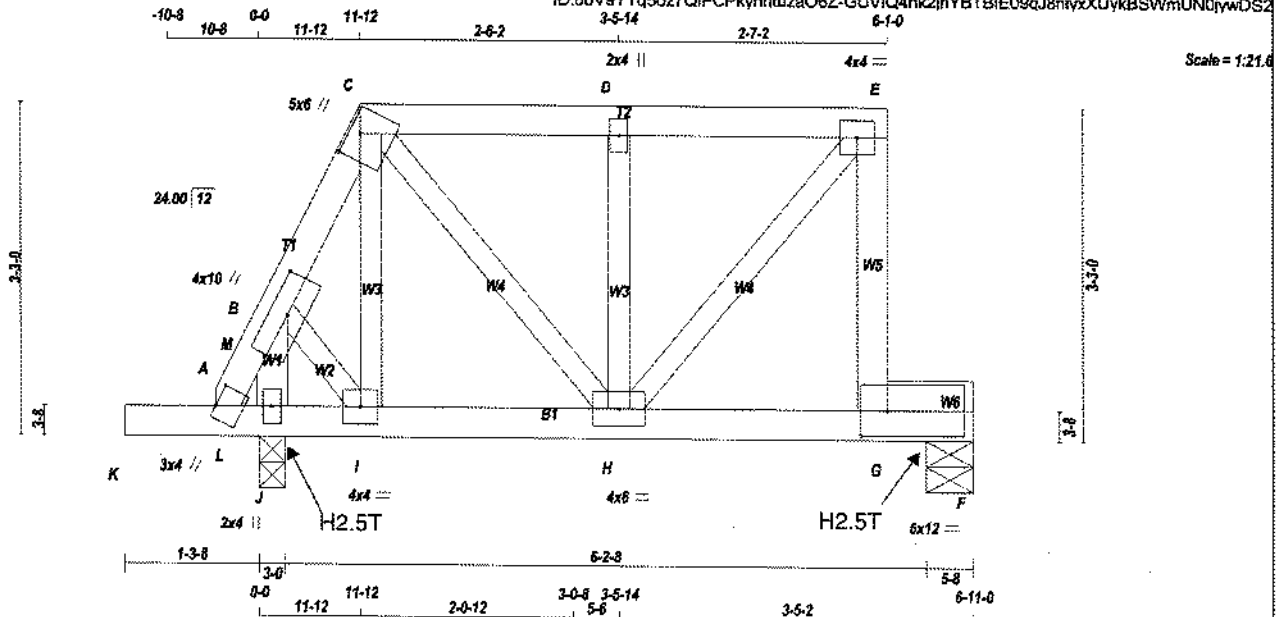


STRUCTURAL COMPONENT ONLY
DWG # TR22070583

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	T23	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Jul 20 10:00:27 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhhtuza06Z-GUVI4h2jhYBTBIE09qJ8niyxXUykBSWmUNQjywDS2



Scale = 1:21.6

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
K - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
J - B	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM-h	MT20	3.0	4.0		2.75
B	TMWW-I	MT20	4.0	10.0	2.00	4.75
C	TTWW-h	MT20	6.0	6.0	1.25	3.00
D	TMWW-w	MT20	2.0	4.0		
E	TMWW-I	MT20	4.0	4.0		
G	BMWW-I	MT20	6.0	12.0	3.00	3.00
H	BMWW-W-I	MT20	4.0	6.0		
I	BMWW-I	MT20	4.0	4.0		
J	BMWW-I+w	MT20	2.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
F	289	0	287	0
J	508	0	513	146

PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT J FOR 150 LBS. FACTORED UPLIFT

PROVIDE FOR 146 LBS. FACTORED HORIZONTAL REACTION AT JOINT J

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	193	115 / 0	0 / 0	0 / 0	45 / -120	78 / 0	0 / 0
J	361	229 / 0	0 / 0	0 / 0	12 / -191	132 / 0	0 / 0

HORIZONTAL REACTIONS

JT	—	0 / 0	0 / 0	104 / -78	0 / 0	0 / 0
----	---	-------	-------	-----------	-------	-------

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. (PLF)	FACTORED HORIZ. (PLF)	MAX. LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. (PLF)	FACTORED HORIZ. (PLF)	MAX. LENGTH (LC)
FR-TO						FR-TO					
A-M	-82 / 233	-84.9	-84.9	0.07 (1)	6.25	I-C	-180 / 34	0.03 (1)			
M-B	-22 / 98	-84.9	-84.9	0.04 (1)	6.25	J-B	-402 / 127	0.04 (1)			
B-C	-176 / 128	-84.9	-84.9	0.04 (7)	6.25	B-I	-25 / 175	0.04 (1)			
C-D	-238 / 147	-84.9	-84.9	0.08 (1)	6.25	H-D	-264 / 160	0.05 (1)			
D-E	-239 / 150	-84.9	-84.9	0.08 (1)	6.25	C-H	-31 / 257	0.06 (1)			
G-E	-363 / 163	0.0	0.0	0.07 (7)	7.81	H-E	-149 / 361	0.07 (1)			
K-A	0 / 0	-103.4	-103.4	0.05 (1)	10.00	L-M	-175 / 49	0.00 (1)			
A-L	-56 / 32	-18.5	-18.5	0.05 (1)	6.25						
L-J	-56 / 32	-18.5	-18.5	0.05 (1)	6.25						
J-I	-134 / 120	-18.5	-18.5	0.05 (1)	6.25						
I-H	-93 / 104	-18.5	-18.5	0.09 (1)	6.25						
H-G	-20 / 52	-18.5	-18.5	0.30 (1)	6.25						
G-F	0 / 0	-18.5	-18.5	0.12 (1)	10.00						

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (20-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, Cp, based on the (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 7.4 PSF RESPECTIVELY.

TOTAL WEIGHT = 5 X 35 = 174 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.23")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.23")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL. (LL) = L/194 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/194 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.08/1.00 (D-E-1), BC=0.30/1.00 (G-H-1)
WB=0.07/1.00 (E-H-1), SI=0.11/1.00 (A-L-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

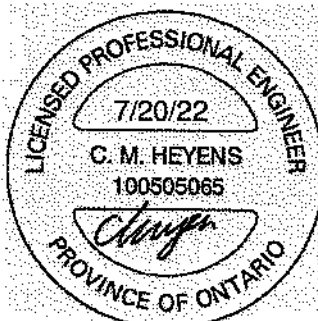
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES	PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX	MIN
MT20	650	371	1747	788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.62 (C) (INPUT = 0.90)
JSI METAL= 0.12 (G) (INPUT = 1.00)

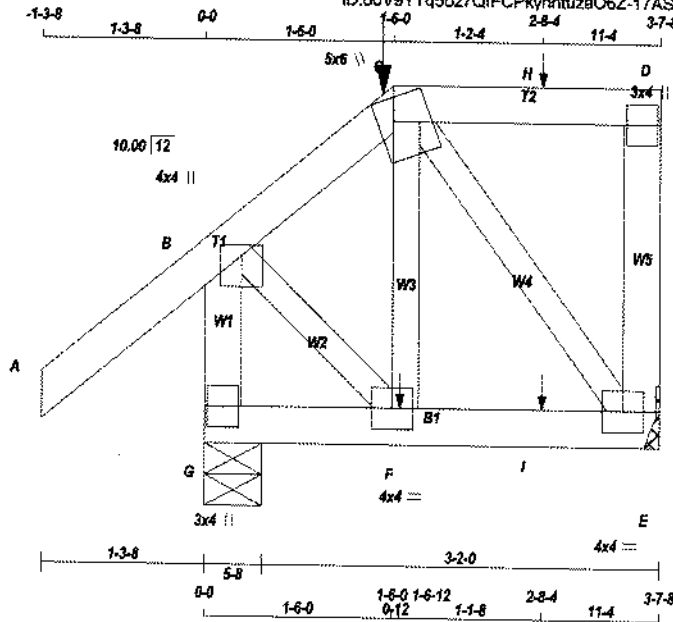


STRUCTURAL COMPONENT ONLY
DWG # TR22070584

JOB NAME 425138	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Jul 20 10:15:40 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhhtuza06Z-17AS1FkQ1Ei7DnUADG1wpq5INzCMNg3qSi8BYJwDDr



Scale = 1:17.8

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - D 2x4 DRY No.2

E - D 2x4 DRY No.2

G - B 2x4 DRY No.2

G - E 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TTVW+m	MT20	5.0	6.0	2.25	1.50
D TMV+p	MT20	3.0	4.0		
E BMVW1-t	MT20	4.0	4.0		
F BMVW1-t	MT20	4.0	4.0		
G BMV1+p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	182	182	0	0
G	352	352	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-6.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
E	130	82 / -15	0 / 0	0 / 0	48 / 0	0 / 0
G	247	175 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (9)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO			
A-B	0 / 38	-84.9	-84.9 0.15 (5)	10.00		F-C	-14 / 46	0.01 (6)	
B-C	-117 / 0	-84.9	-84.9 0.15 (5)	6.25		C-E	-97 / 7	0.02 (1)	
C-H	0 / 0	-84.9	-84.9 0.07 (1)	10.00		B-F	-8 / 81	0.02 (1)	
H-D	0 / 0	-84.9	-84.9 0.07 (1)	10.00					
E-D	-80 / 0	0.0	0.0 0.01 (1)	7.81					
G-B	-339 / 0	0.0	0.0 0.04 (1)	7.81					
G-F	0 / 0	-18.5	-18.5 0.01 (4)	10.00					
F-I	-5 / 62	-18.5	-18.5 0.02 (4)	10.00					
I-E	-5 / 62	-18.5	-18.5 0.02 (4)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-6-0	-29	-29	78	BACK	VERT	TOTAL		C1
F	1-6-12	1	1		BACK	VERT	TOTAL		C1
H	2-8-4	1	1	75	BACK	VERT	TOTAL		C1
I	2-8-4	1	1		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 21 LB

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CSI: TC=0.15/1.00 (A-B-5), BC=0.02/1.00 (E-F-4)
WB=0.02/1.00 (C-E-1), SSI=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

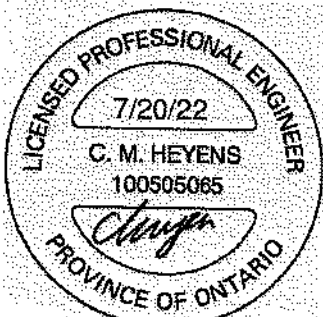
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



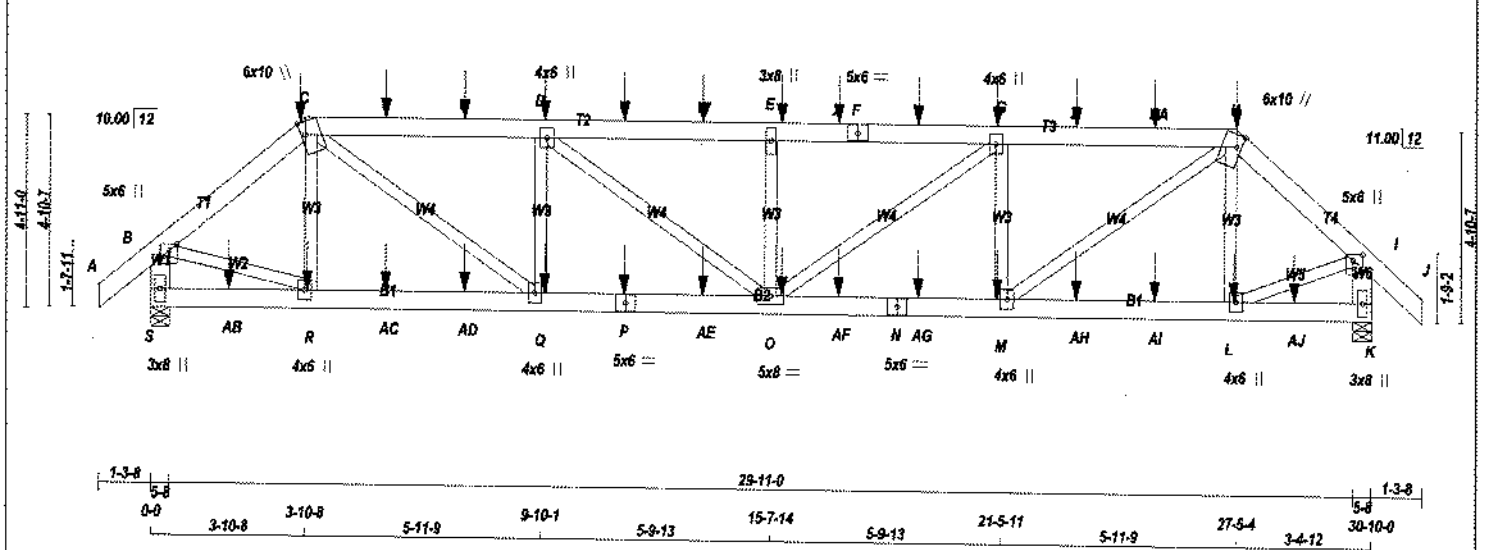
STRUCTURAL COMPONENT ONLY
DWG # TR22070596

JOB NAME 425139	TRUSS NAME T30	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

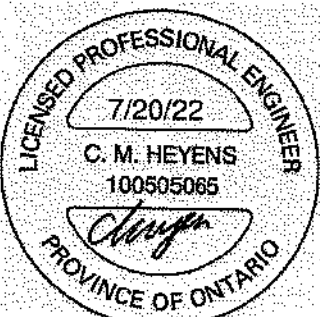
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 10:28:15 2022 Page 1
ID:ld0GLhBwK6p3yzOsvBoilH3za0dF-gwwZ5xoe6OMV1ipTx92LXL7FV17qDpob3ofZywD12

Scale = 1:56.4



LUMBER N. L. G. A. RULES CHORDS SIZE A - C 2x6 DRY No.2 SPF C - F 2x6 DRY No.2 SPF F - H 2x6 DRY No.2 SPF H - J 2x6 DRY No.2 SPF J - L 2x6 DRY No.2 SPF L - N 2x6 DRY No.2 SPF N - K 2x6 DRY No.2 SPF ALL WEBS 2x4 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 2 12 SIDE(122.0) C-F 2 12 SIDE(183.1) F-H 2 12 SIDE(183.1) H-J 2 12 SIDE(122.0) J-L 2 12 TOP L-N 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS S-P 2 12 SIDE(183.1) P-N 2 12 SIDE(183.1) N-K 2 12 SIDE(183.1) WEBS : (0.122"x3") SPIRAL NAILS 2x4 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION JT VERT 2537 0 S 2537 0 K 2527 0 MAXIMUM FACTORED GROSS REACTION DOWN 2537 0 UPLIFT 0 0 INPUT BRG 5-8 5-8 REQD BRG 5-8 5-8 UNFACTORED REACTIONS 1ST CASE MAX/MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL S 1797 1161 / 0 0 / 0 0 / 0 0 / 0 636 / 0 0 / 0 K 1791 1154 / 0 0 / 0 0 / 0 0 / 0 637 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX CS1 (LC) UNBRAC LENGTH FR-TO A-B 0 / 39 -84.9 -84.9 0.03 (1) 10.00 B-C -2595 / 0 -84.9 -84.9 0.07 (1) 6.25 C-T -3797 / 0 -84.9 -84.9 0.18 (1) 5.62 T-U -3797 / 0 -84.9 -84.9 0.18 (1) 5.62 U-D -3797 / 0 -84.9 -84.9 0.18 (1) 5.62 D-V -4301 / 0 -84.9 -84.9 0.18 (1) 5.35 V-W -4301 / 0 -84.9 -84.9 0.18 (1) 5.35 W-E -4301 / 0 -84.9 -84.9 0.18 (1) 5.35 E-X -4301 / 0 -84.9 -84.9 0.18 (1) 5.35 X-F -4301 / 0 -84.9 -84.9 0.18 (1) 5.35 F-Y -4301 / 0 -84.9 -84.9 0.18 (1) 5.35 Y-G -4301 / 0 -84.9 -84.9 0.18 (1) 5.35 G-Z -3685 / 0 -84.9 -84.9 0.17 (1) 5.68 Z-AA -3685 / 0 -84.9 -84.9 0.17 (1) 5.68 AA-H -2404 / 0 -84.9 -84.9 0.05 (1) 6.25 H-I 0 / 41 -84.9 -84.9 0.04 (1) 10.00 I-J -2508 / 0 0.0 0.0 0.09 (1) 7.81 J-K -2508 / 0 0.0 0.0 0.09 (1) 7.81 WEBS MAX. FACTORED FORCE (LBS) MAX CS1 (LC) UNBRAC LENGTH FR-TO R-C -390 / 0 0.04 (1) L-H -444 / 0 0.05 (1) B-R 0 / 2060 0.18 (1) L-I 0 / 1863 0.16 (1) M-H 0 / 2403 0.21 (1) C-Q 0 / 2264 0.20 (1) M-G -1270 / 0 0.14 (1) Q-D -1188 / 0 0.13 (1) Q-G 0 / 773 0.07 (1) D-O 0 / 632 0.06 (1) O-E -689 / 0 0.08 (1)				DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 36.7 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPC 2014 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (1.03") CALCULATED VERT. DEFL.(LL) = L/999 (0.07") ALLOWABLE DEFL.(TL) = L/360 (1.03") CALCULATED VERT. DEFL.(TL) = L/999 (0.13") CS1: TC=0.18/1.00 (E-G:1), BC=0.28/1.00 (O-Q:1), WB=0.21/1.00 (H-M:1), SSI=0.13/1.00 (G-H:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.			
---	--	--	--	---	--	--	--	--	--	--	--	---	--	--	--



STRUCTURAL COMPONENT ONLY
DWG # TR22070597 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T30	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:29:15 2022 Page 2

ID:Id0GLhBWk6p3yzQayBoH3za0dF-qwwZ5xce6fOMV1ipT92LXL7FV1?gDpotb3ofZywD12

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	8.0	2.00	2.25
C	TTWW+m	MT20	6.0	10.0	4.00	1.25
D	TMWW+1	MT20	4.0	8.0		
E	TMWW+w	MT20	3.0	8.0		
F	TS-1	MT20	5.0	6.0		
G	TMWW+1	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	10.0	Edge	1.50
I	TMWW+p	MT20	5.0	6.0	1.75	2.75
K	BMV1+p	MT20	3.0	8.0		
L, M, Q, R						
L	BMWW+1	MT20	4.0	6.0		
N	BS-1	MT20	5.0	6.0		
O	BMWWW-1	MT20	5.0	8.0		
P	BS-1	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

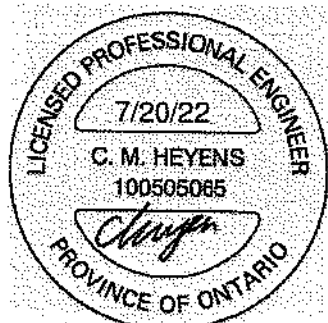
SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-188	-188	---	FRONT	VERT	TOTAL	---	C1
D	9-11-4	-55	-55	---	FRONT	VERT	TOTAL	---	C1
E	15-11-4	-55	-55	---	FRONT	VERT	TOTAL	---	C1
G	21-4-8	-55	-55	---	FRONT	VERT	TOTAL	---	C1
H	27-5-4	-20	-20	---	FRONT	VERT	DEAD	---	C1
H	27-5-4	-61	-61	---	FRONT	VERT	TOTAL	---	C1
H	27-5-4	-76	-76	---	FRONT	VERT	SNOW	---	C1
L	27-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
M	21-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
O	15-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
P	11-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
Q	9-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
R	3-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
T	5-11-4	-55	-55	---	FRONT	VERT	TOTAL	---	C1
U	7-11-4	-55	-55	---	FRONT	VERT	TOTAL	---	C1
V	11-11-4	-55	-55	---	FRONT	VERT	TOTAL	---	C1
W	13-11-4	-55	-55	---	FRONT	VERT	TOTAL	---	C1
X	17-4-8	-55	-55	---	FRONT	VERT	TOTAL	---	C1
Y	19-4-8	-55	-55	---	FRONT	VERT	TOTAL	---	C1
Z	23-4-8	-55	-55	---	FRONT	VERT	TOTAL	---	C1
AA	25-4-8	-55	-55	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AC	5-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AD	7-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AE	13-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AF	17-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AG	19-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AH	23-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AI	25-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AJ	28-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.83 (L) (INPUT = 0.90)
JSI METAL= 0.44 (R) (INPUT = 1.00)

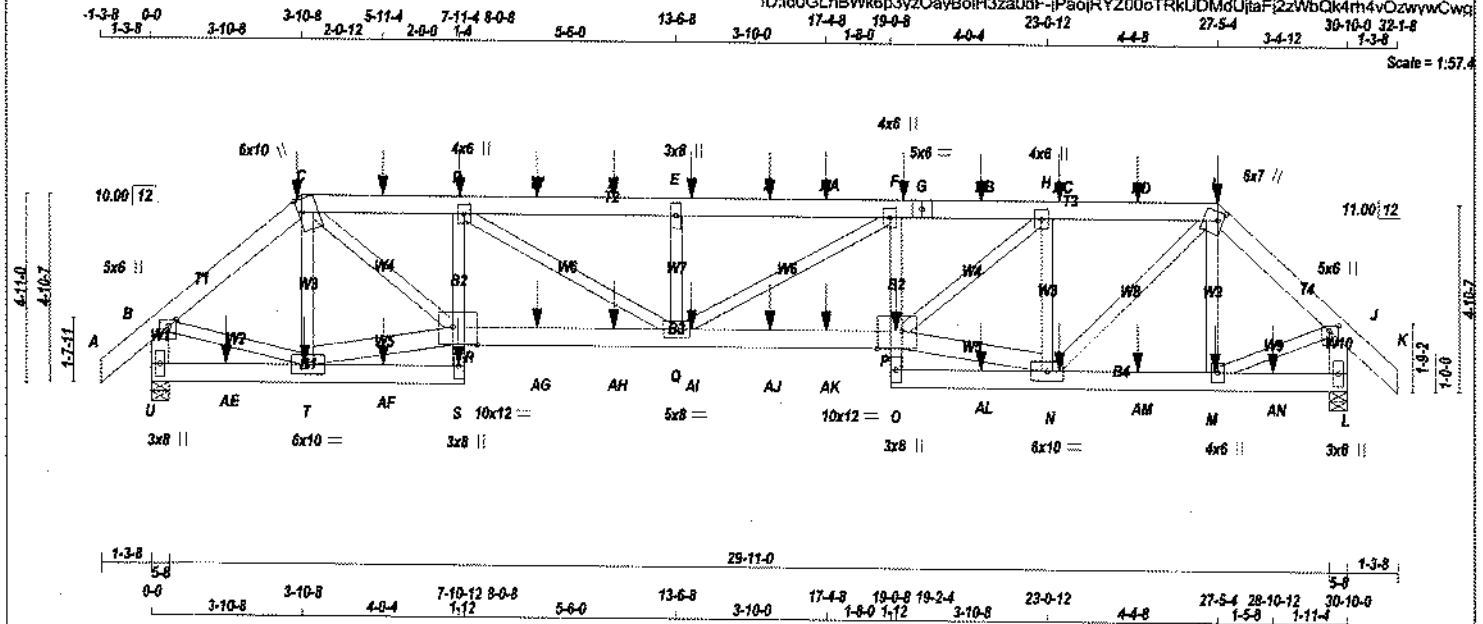


STRUCTURAL COMPONENT ONLY
DWG # TR22070597 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425140	T30S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITak Industries, Inc. Wed Jul 20 10:36:03 2022 Page 1
ID: d0GLhBWk6p3yzOayBoIH3za0dF-iPaoiRYZ00oTRkUDMdUjtaFj2zWbQk4th4vOzwYwCw9



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - D	2x4	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
O - F	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
T - R	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	2	12
C - G	2	12
G - I	2	12
I - K	2	12
U - B	2	12
U - S	2	12
L - J	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
U - S	2	12
R - P	2	12
O - L	2	12
S - D	1	12
F - O	1	12
WEBS: (0.122"x3") SPIRAL NAILS		
2x4	1	6
2x6	2	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
U	2488	0	5-8	5-8
L	2487	0	5-8	5-8

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS						
JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1765	1130 / 0	0 / 0	0 / 0	0 / 0	0 / 0	635 / 0	0 / 0
L	1764	1126 / 0	0 / 0	0 / 0	0 / 0	0 / 0	636 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)
FR-TO		FROM TO			FR-TO	
A-B	0 / 39	-84.9 -84.9	0.03 (1)	10.00	T-C	-889 / 0
B-C	-2533 / 0	-84.9 -84.9	0.07 (1)	6.25	T-R	0 / 1901
C-V	-4160 / 0	-84.9 -84.9	0.09 (1)	5.52	C-R	0 / 2932
V-D	-4160 / 0	-84.9 -84.9	0.09 (1)	5.52	D-Q	0 / 1322
D-W	-5313 / 0	-84.9 -84.9	0.12 (1)	5.00	Q-E	-511 / 0
W-X	-5313 / 0	-84.9 -84.9	0.12 (1)	5.00	Q-F	0 / 264
X-E	-5313 / 0	-84.9 -84.9	0.12 (1)	5.00	P-N	0 / 3207
E-Y	-5313 / 0	-84.9 -84.9	0.13 (1)	4.99	P-H	0 / 2475
Y-Z	-5313 / 0	-84.9 -84.9	0.13 (1)	4.99	N-H	-2168 / 0
Z-AA	-5313 / 0	-84.9 -84.9	0.13 (1)	4.99	N-I	0 / 2059
AA-F	-5313 / 0	-84.9 -84.9	0.13 (1)	4.99	M-I	-466 / 0
F-G	-5060 / 0	-84.9 -84.9	0.11 (1)	5.11	B-T	0 / 2010
G-AB	-5060 / 0	-84.9 -84.9	0.11 (1)	5.11	M-J	0 / 1821
AB-H	-5060 / 0	-84.9 -84.9	0.11 (1)	5.11		
H-AC	-3145 / 0	-84.9 -84.9	0.10 (1)	6.13		
AC-AD	-3145 / 0	-84.9 -84.9	0.10 (1)	6.13		
AD-I	-3145 / 0	-84.9 -84.9	0.10 (1)	6.13		
I-J	-2350 / 0	-84.9 -84.9	0.05 (1)	6.25		
J-K	0 / 41	-84.9 -84.9	0.04 (1)	10.00		
U-B	-2455 / 0	0.0	0.09 (1)	7.81		
L-J	-2459 / 0	0.0	0.09 (1)	7.81		
U-AE	0 / 0	-18.5 -18.5	0.03 (4)	10.00		
AE-T	0 / 0	-18.5 -18.5	0.03 (4)	10.00		
T-AF	0 / 70	-18.5 -18.5	0.03 (4)	10.00		
AF-S	0 / 70	-18.5 -18.5	0.03 (4)	10.00		
S-R	0 / 65	0.0	0.08 (1)	10.00		
R-D	-1217 / 0	0.0	0.08 (1)	7.81		
R-AG	0 / 4181	-18.5 -18.5	0.33 (1)	10.00		
AG-AH	0 / 4181	-18.5 -18.5	0.33 (1)	10.00		
AH-Q	0 / 4181	-18.5 -18.5	0.33 (1)	10.00		
Q-AI	0 / 5087	-18.5 -18.5	0.40 (1)	10.00		
AI-AJ	0 / 5087	-18.5 -18.5	0.40 (1)	10.00		
AJ-AK	0 / 5087	-18.5 -18.5	0.40 (1)	10.00		
AK-P	0 / 5087	-18.5 -18.5	0.40 (1)	10.00		
P-Q	0 / 42	0.0	0.07 (1)	10.00		
P-F	-622 / 0	0.0	0.08 (1)	7.81		
O-AL	0 / 91	-18.5 -18.5	0.03 (4)	10.00		
AL-N	0 / 91	-18.5 -18.5	0.03 (4)	10.00		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/989 (0.16")
ALLOWABLE DEFL. (TL) = L/360 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.19")

CS1: TO=0.13/1.00 (E-F-1), SC=0.40/1.00 (P-Q-1)
WB=0.29/1.00 (C-R-1), SS=0.11/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

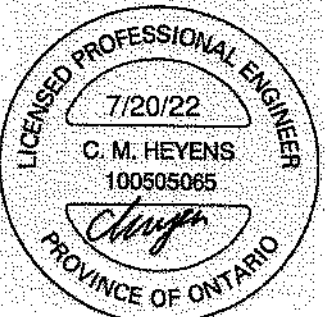
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 785 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070630 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425140	T30S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:36:03 2022 Page 2

ID:d0GLhBWk6p3yzOayBoH3za0dF-jPaciRYZ00eTRkUDMcUjbaFi2zWbQk4rh4vOzwYwCwg

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTVW+m	MT20	5.0	10.0	4.00	1.25
D	TMVW+p	MT20	4.0	6.0		
E	TMVW+w	MT20	3.0	8.0		
F	TMVW+p	MT20	4.0	6.0		
G	TS-I	MT20	5.0	6.0		
H	TMVW+I	MT20	4.0	8.0		
I	TTVW+m	MT20	6.0	7.0	2.75	1.75
J	TMVW+p	MT20	5.0	6.0	1.75	2.75
L	BMV1+p	MT20	3.0	8.0		
M	BMVW+I	MT20	4.0	6.0		
N	BMVWVW-I	MT20	8.0	10.0		
O	BMV+p	MT20	3.0	8.0		
P	BMVWVW-I	MT20	10.0	12.0	5.50	7.50
Q	BMVWVW-I	MT20	5.0	8.0		
R	BMVWVW-I	MT20	10.0	12.0	5.25	7.50
S	BMV+p	MT20	3.0	8.0		
T	BMVWVW-I	MT20	5.0	10.0		
U	BMV1+p	MT20	3.0	8.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
N-AM	0 / 1706	-18.5	-18.5	0.13 (1)	10.00		
AM-M	0 / 1706	-18.5	-18.5	0.13 (1)	10.00		
M-AN	0 / 0	-18.5	-18.5	0.02 (4)	10.00		
AN-L	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

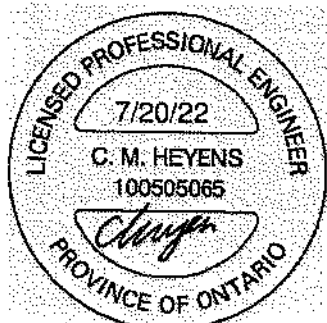
SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-188	-188		FRONT	VERT	TOTAL		C1
D	7-11-4	-55	-55		FRONT	VERT	TOTAL		C1
F	19-4-8	-55	-55		FRONT	VERT	TOTAL		C1
I	27-5-4	-20	-20		FRONT	VERT	DEAD		C1
I	27-5-4	-61	-61		FRONT	VERT	TOTAL		C1
I	27-5-4	-76	-76		FRONT	VERT	SNOW		C1
M	27-4-8	-14	-14		FRONT	VERT	TOTAL		C1
N	23-4-8	-14	-14		FRONT	VERT	TOTAL		C1
P	19-2-4	-20	-20		FRONT	VERT	TOTAL		C1
S	7-10-12	-21	-21		FRONT	VERT	TOTAL		C1
T	3-11-4	-14	-14		FRONT	VERT	TOTAL		C1
V	5-11-4	-55	-55		FRONT	VERT	TOTAL		C1
W	9-11-4	-3	-3		FRONT	VERT	TOTAL		C1
X	11-11-4	-3	-3		FRONT	VERT	TOTAL		C1
Y	13-11-4	-3	-3		FRONT	VERT	TOTAL		C1
Z	15-11-4	-3	-3		FRONT	VERT	TOTAL		C1
AA	17-4-8	-3	-3		FRONT	VERT	TOTAL		C1
AB	21-4-8	-55	-55		FRONT	VERT	TOTAL		C1
AC	23-4-8	-55	-55		FRONT	VERT	TOTAL		C1
AD	25-4-8	-55	-55		FRONT	VERT	TOTAL		C1
AE	1-11-4	-14	-14		FRONT	VERT	TOTAL		C1
AF	5-11-4	-14	-14		FRONT	VERT	TOTAL		C1
AG	9-11-4	-51	-51		FRONT	VERT	TOTAL		C1
AH	11-11-4	-51	-51		FRONT	VERT	TOTAL		C1
AI	13-11-4	-51	-51		FRONT	VERT	TOTAL		C1
AJ	15-11-4	-51	-51		FRONT	VERT	TOTAL		C1
AK	17-4-8	-51	-51		FRONT	VERT	TOTAL		C1
AL	21-4-8	-14	-14		FRONT	VERT	TOTAL		C1
AM	25-4-8	-14	-14		FRONT	VERT	TOTAL		C1
AN	28-10-12	-14	-14		FRONT	VERT	TOTAL		C1

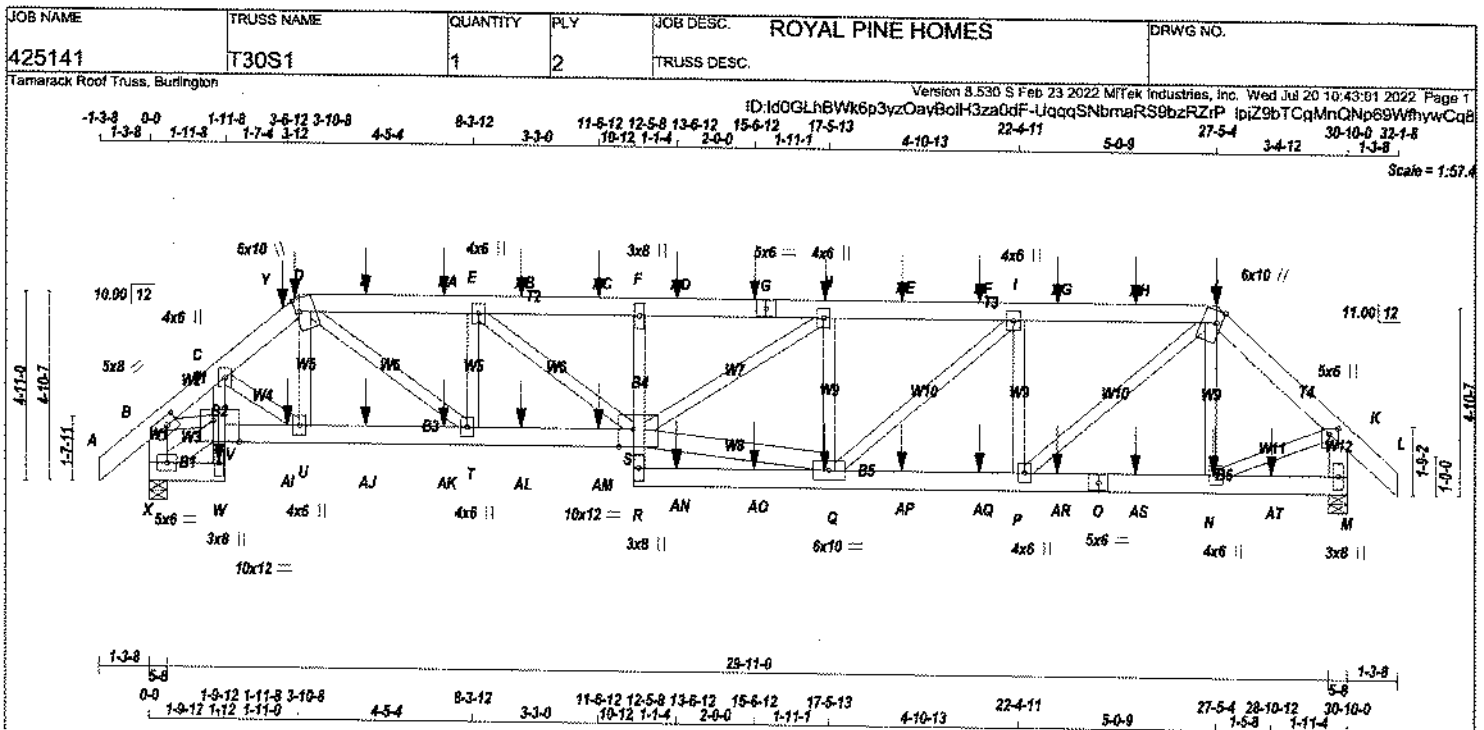
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.43 (H) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070630



TOTAL WEIGHT = 2 X 204 = 407 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF	
D - G	2x6	DRY	No.2	SPF	
G - J	2x6	DRY	No.2	SPF	
J - L	2x6	DRY	No.2	SPF	
X - B	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
X - W	2x6	DRY	No.2	SPF	
W - C	2x4	DRY	No.2	SPF	
V - S	2x6	DRY	No.2	SPF	
R - F	2x4	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
S - V	2x6	DRY	No.2	SPF	
X - V	2x6	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 2 12	SIDE (122.0)	
D-G 2 12	SIDE (61.0)	
G-J 2 12	SIDE (183.1)	
J-L 2 12	SIDE (122.0)	
X-B 2 12	TOP	
M-K 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X-W 2 12	SIDE (122.0)	
V-S 2 12	SIDE (0.0)	
R-O 2 12	SIDE (183.1)	
O-M 2 12	SIDE (183.1)	
W-C 1 12	TOP	
F-R 1 12	TOP	
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1 6		
2x6 2 6		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
M	2502	0	2502	0
X	2621	0	2621	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
M	1773	1141 / 0 0 / 0 0 / 0 832 / 0 0 / 0
X	1855	1208 / 0 0 / 0 0 / 0 649 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
A-B 0 / 39	-84.9	-84.9 0.03 (1)	10.00	C-U -330 / 0	0.02 (1)		
B-C -3640 / 0	-84.9	-84.9 0.08 (1)	5.82	U-D 0 / 245	0.02 (1)		
C-Y 3237 / 0	-84.9	-84.9 0.08 (1)	6.09	D-T 0 / 2198	0.19 (1)		
Y-D -3237 / 0	-84.9	-84.9 0.08 (1)	6.09	T-E -1107 / 0	0.09 (1)		
D-Z -4272 / 0	-84.9	-84.9 0.09 (1)	5.47	E-S 0 / 1178	0.10 (1)		
Z-AA -4272 / 0	-84.9	-84.9 0.09 (1)	5.47	S-Q 0 / 4138	0.27 (1)		
AA-E -4272 / 0	-84.9	-84.9 0.08 (1)	5.47	S-H 0 / 1221	0.11 (1)		
E-AB -5195 / 0	-84.9	-84.9 0.09 (1)	5.08	Q-H -1346 / 0	0.15 (1)		
AB-AC -5195 / 0	-84.9	-84.9 0.09 (1)	5.08	I-Q 0 / 1003	0.09 (1)		
AC-F -5195 / 0	-84.9	-84.9 0.09 (1)	5.08	P-I -1330 / 0	0.15 (1)		
F-AD -5164 / 0	-84.9	-84.9 0.13 (1)	5.05	P-J 0 / 2206	0.20 (1)		
AD-G -5164 / 0	-84.9	-84.9 0.13 (1)	5.05	N-J -450 / 0	0.05 (1)		
G-H -5164 / 0	-84.9	-84.9 0.13 (1)	5.05	N-K 0 / 1838	0.16 (1)		
H-AE -4128 / 0	-84.9	-84.9 0.13 (1)	5.49	X-V -78 / 0	0.00 (1)		
AE-AF -4128 / 0	-84.9	-84.9 0.13 (1)	5.49	B-V 0 / 2727	0.24 (1)		
AF-I -4128 / 0	-84.9	-84.9 0.13 (1)	5.49				
I-AG -3390 / 0	-84.9	-84.9 0.12 (1)	5.93				
AG-AH -3390 / 0	-84.9	-84.9 0.12 (1)	5.93				
AH-J -3390 / 0	-84.9	-84.9 0.12 (1)	5.93				
J-K -2371 / 0	-84.9	-84.9 0.05 (1)	6.25				
K-L 0 / 41	-84.9	-84.9 0.04 (1)	10.00				
X-B -2562 / 0	0.0	0.0 0.09 (1)	7.81				
M-K -2479 / 0	0.0	0.0 0.09 (1)	7.81				
X-W 0 / 66	-18.5	-18.5 0.01 (4)	10.00				
W-V 0 / 201	0.0	0.0 0.07 (1)	10.00				
V-C 0 / 385	0.0	0.0 0.09 (1)	10.00				
V-AI 0 / 2780	-18.5	-18.5 0.22 (1)	10.00				
AI-U 0 / 2780	-18.5	-18.5 0.22 (1)	10.00				
U-AJ 0 / 2523	-18.5	-18.5 0.20 (1)	10.00				
AJ-AK 0 / 2523	-18.5	-18.5 0.20 (1)	10.00				
AK-T 0 / 2523	-18.5	-18.5 0.20 (1)	10.00				
T-AL 0 / 4272	-18.5	-18.5 0.32 (1)	10.00				
AL-AM 0 / 4272	-18.5	-18.5 0.32 (1)	10.00				
AM-S 0 / 4272	-18.5	-18.5 0.32 (1)	10.00				
R-S 0 / 64	0.0	0.0 0.08 (1)	10.00				
S-F -450 / 0	0.0	0.0 0.08 (1)	7.81				
R-AN 0 / 105	-18.5	-18.5 0.04 (4)	10.00				
AN-AO 0 / 105	-18.5	-18.5 0.04 (4)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.16")

CSI: TC=0.13/1.00 (H-I), BC=0.32/1.00 (S-T), WB=0.27/1.00 (Q-S), SS=0.12/1.00 (J-I)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

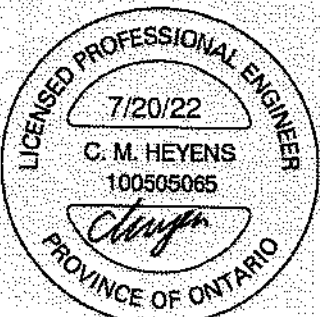
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

STRUCTURAL COMPONENT ONLY
DWG # TR22070636

PG 1/2

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
 DWG # TR22070636 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425141	T30S1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:43:01 2022 Page 2
ID:Id0GLhBWk6p3yzOayBoIH3za0dF-UqqgSNbmaRS9bzRZrP IpiZ9bTCgMnQNP69WthwCq8

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TMVW+p	MT20	4.0	6.0		
D	TTWW+m	MT20	6.0	10.0	4.00	1.25
E, H, I						
E	TMVW+t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	8.0		
G	TS-t	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	10.0	Edge	1.50
K	TMVW+p	MT20	5.0	6.0	1.75	2.75
M	BMV1+p	MT20	3.0	8.0		
N, P, T, U						
N	BMVW+t	MT20	4.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMVW+t	MT20	6.0	10.0		
R	BMV+p	MT20	3.0	8.0		
S	BMVW+t	MT20	10.0	12.0	Edge	4.50
V	BMVW+m	MT20	10.0	12.0	6.50	8.00
W	BMV+p	MT20	3.0	8.0		
X	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
AO-Q	0/106	-18.5	-18.5	0.04 (4)	10.00		
Q-AP	0/3390	-18.5	-18.5	0.24 (1)	10.00		
AP-AQ	0/3390	-18.5	-18.5	0.24 (1)	10.00		
AQ-P	0/3390	-18.5	-18.5	0.24 (1)	10.00		
P-AR	0/1722	-18.5	-18.5	0.13 (1)	10.00		
AR-O	0/1722	-18.5	-18.5	0.13 (1)	10.00		
O-AS	0/1722	-18.5	-18.5	0.13 (1)	10.00		
AS-N	0/1722	-18.5	-18.5	0.13 (1)	10.00		
N-AT	0/0	-18.5	-18.5	0.03 (4)	10.00		
AT-M	0/0	-18.5	-18.5	0.03 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	3-10-8	-23	-23		FRONT	VERT	DEAD		C1
O	3-10-8	-87	-87		FRONT	VERT	SNOW		C1
G	15-6-12	-55	-55		FRONT	VERT	TOTAL		C1
H	17-4-8	-55	-55		FRONT	VERT	TOTAL		C1
J	27-5-4	-20	-20		FRONT	VERT	DEAD		C1
J	27-5-4	-61	-61		FRONT	VERT	TOTAL		C1
J	27-5-4	-76	-76		FRONT	VERT	SNOW		C1
N	27-4-8	-14	-14		FRONT	VERT	TOTAL		C1
Q	17-4-8	-14	-14		FRONT	VERT	TOTAL		C1
W	1-9-12	-130	-130		FRONT	VERT	TOTAL		C1
Y	3-6-12	-23	-23		FRONT	VERT	TOTAL		C1
Z	5-6-12	-3	-3		FRONT	VERT	TOTAL		C1
AA	7-6-12	-3	-3		FRONT	VERT	TOTAL		C1
AB	9-6-12	-3	-3		FRONT	VERT	TOTAL		C1
AC	11-6-12	-3	-3		FRONT	VERT	TOTAL		C1
AD	13-6-12	-55	-55		FRONT	VERT	TOTAL		C1
AE	19-4-8	-55	-55		FRONT	VERT	TOTAL		C1
AF	21-4-8	-55	-55		FRONT	VERT	TOTAL		C1
AG	23-4-8	-55	-55		FRONT	VERT	TOTAL		C1
AH	25-4-8	-55	-55		FRONT	VERT	TOTAL		C1
AI	3-6-12	-51	-51		FRONT	VERT	TOTAL		C1
AJ	5-6-12	-51	-51		FRONT	VERT	TOTAL		C1
AK	7-6-12	-51	-51		FRONT	VERT	TOTAL		C1
AL	9-6-12	-51	-51		FRONT	VERT	TOTAL		C1
AM	11-6-12	-51	-51		FRONT	VERT	TOTAL		C1
AN	13-6-12	-14	-14		FRONT	VERT	TOTAL		C1
AO	15-6-12	-14	-14		FRONT	VERT	TOTAL		C1
AP	19-4-8	-14	-14		FRONT	VERT	TOTAL		C1
AQ	21-4-8	-14	-14		FRONT	VERT	TOTAL		C1
AR	23-4-8	-14	-14		FRONT	VERT	TOTAL		C1
AS	25-4-8	-14	-14		FRONT	VERT	TOTAL		C1
AT	28-10-12	-14	-14		FRONT	VERT	TOTAL		C1

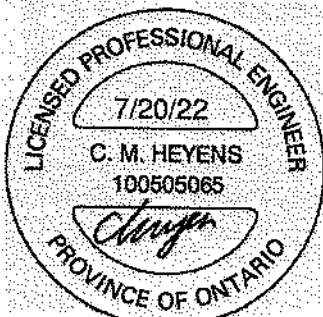
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (N) (INPUT = 0.90)
JSI METAL = 0.39 (T) (INPUT = 1.00)

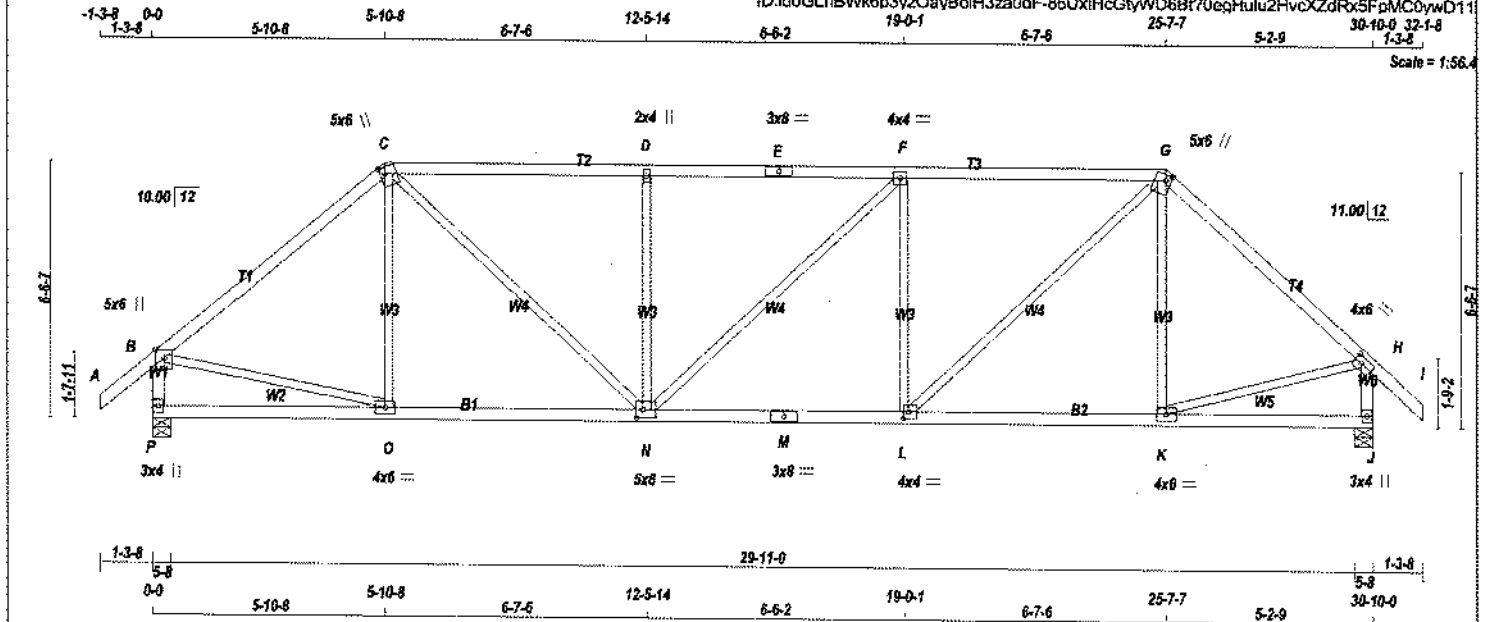


STRUCTURAL COMPONENT ONLY
DWG # TR22070636 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T31	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MitTek Industries, Inc. Wed Jul 20 10:29:16 2022 Page 1
ID:ld0GLhBWWk6p3yzQay8oIH3za0dF-86UxIHcGtyWD68H70eqHulu2HvcXZdR5FpMC0ywd11



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (tab/e is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	3.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.50
H	TMVW-t	MT20	4.0	6.0	2.00	2.25
J	BMVW+p	MT20	3.0	4.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	4.0	2.00	1.50
M	BS-t	MT20	3.0	3.0		
N	BMVW-w	MT20	5.0	6.0	2.50	2.00
O	BMVW-t	MT20	4.0	6.0		
P	BMVW+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
P	1711	0	1711	0	5-8	5-8
J	1711	0	1711	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1212	783/0	0/0	0/0	0/0	430/0	0/0
J	1213	783/0	0/0	0/0	0/0	430/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD LC1	MAX. (PLF)	MAX. (LC)	MEMB. MAX. FORCE (LBS)	MAX. (LC)	MEMB. MAX. FORCE (LBS)	MAX. (LC)
FR-TO								
A-B	0/38	-84.9	-84.9	0.12 (1)	10.00	O-C	-138/46	0.10 (1)
B-C	-1608/0	-84.9	-84.9	0.68 (1)	4.43	C-N	0/1003	0.23 (1)
C-D	-1961/0	-84.9	-84.9	0.66 (1)	4.08	N-D	-604/0	0.42 (1)
D-E	-1961/0	-84.9	-84.9	0.66 (1)	4.07	N-F	0/147	0.01 (1)
E-F	-1961/0	-84.9	-84.9	0.66 (1)	4.07	L-F	-637/0	0.44 (1)
F-G	-1927/0	-84.9	-84.9	0.66 (1)	4.11	L-G	0/1106	0.25 (1)
G-H	-1529/0	-84.9	-84.9	0.51 (1)	4.74	K-G	-177/26	0.12 (1)
H-I	0/40	-84.9	-84.9	0.12 (1)	10.00	B-O	0/1262	0.28 (1)
P-B	-1668/0	0.0	0.0	0.18 (1)	6.42	K-H	0/1163	0.26 (1)
J-H	-1674/0	0.0	0.0	0.18 (1)	6.42			
P-O	0/0	-18.5	-18.5	0.17 (4)	10.00			
O-N	0/1230	-18.5	-18.5	0.30 (1)	10.00			
N-M	0/1927	-18.5	-18.5	0.39 (1)	10.00			
M-L	0/1927	-18.5	-18.5	0.39 (1)	10.00			
L-K	0/1121	-18.5	-18.5	0.27 (1)	10.00			
K-J	0/0	-18.5	-18.5	0.15 (4)	10.00			

TOTAL WEIGHT = 131 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TO=0.68/1.00 (B-C-1), BC=0.39/1.00 (L-N-1), WB=0.44/1.00 (F-L-1), SS=0.26/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

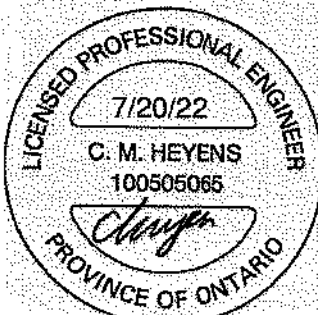
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

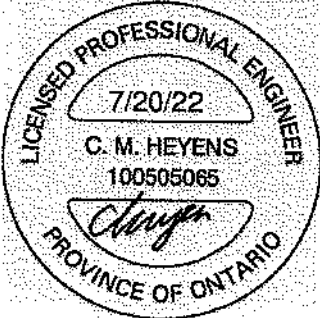
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.65 (M) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070598

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425140	T31S	1	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington				Version 8.530 5 Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:36:04 2022 Page 2 ID:ld0GLhBWk6p3yzQayBoIH3za0dF-Bc8AwnYCnJwK3u3QwK7yQnnmiNs997o7wkexVMwCw		
NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.  STRUCTURAL COMPONENT ONLY DWG # TR22070631 PG 2/2						

S-R	0 / 19	0.0	0.0	0.07 (1)	10.00
R-C	-103 / 0	0.0	0.0	0.07 (1)	7.81
R-Q	0 / 1613	-18.5	-18.5	0.36 (1)	10.00
Q-P	0 / 1454	-18.5	-18.5	0.34 (1)	10.00
O-P	0 / 54	0.0	0.0	0.09 (1)	10.00
P-E	-605 / 0	0.0	0.0	0.23 (1)	7.81
O-N	0 / 60	-18.5	-18.5	0.19 (4)	10.00
N-M	0 / 1118	-18.5	-18.5	0.29 (4)	10.00
M-L	0 / 1118	-18.5	-18.5	0.29 (4)	10.00
L-K	0 / 0	-18.5	-18.5	0.14 (4)	10.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 786	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (D) (INPUT = 0.90)
 JSI METAL = 0.73 (B) (INPUT = 1.00)

CONTINUED ON PAGE

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425141	T31S1	1	1	TRUSS DESC.		

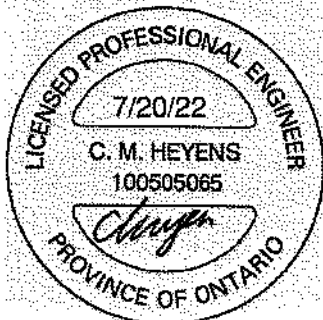
Tamereck Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:43:02 2022 Page 2
ID:1d0GLhBWk6p3yzQav8oltH3za0dF-y0OCfcOLla0D70IP6V Mx5B6tXD59vW1mu3B8ywCq7

Edge - INDICATES REFERENCE CORNER OF PLATE.
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070637 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T32	2	1	TRUSS DESC.		

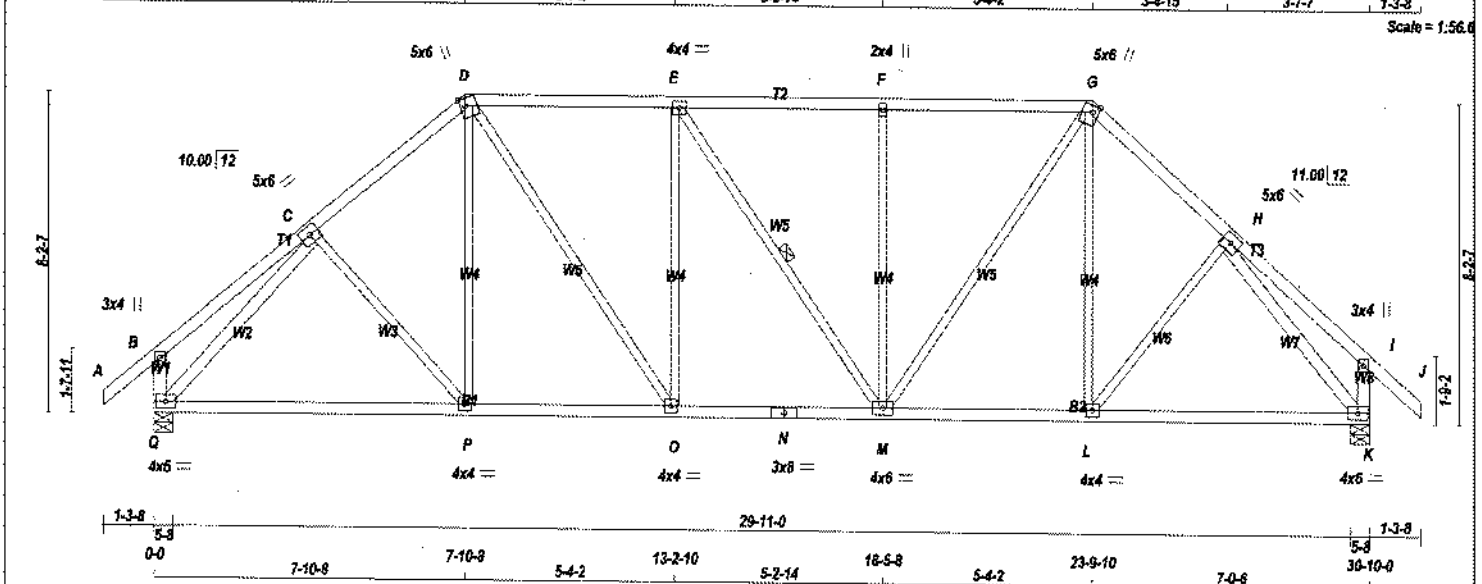
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:29:17 2022 Page 1

ID:ld0GLhBWk6p3yzQayBoIH3za0dF-cl2JWddveGe4kLSCaMBWQyQicJzEI 75KvYvkSwwD10

1-3-8 0-0 4-0-8 4-0-8 3-10-0 7-10-8 5-4-2 13-2-10 5-2-14 18-5-8 5-4-2 23-9-10 3-4-15 27-2-9 3-7-7 30-10-0 32-1-8 1-3-8

Scale = 1:56.0



LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-t	MT20	4.0	4.0		
F	TMVW+m	MT20	2.0	4.0		
G	TTVW+m	MT20	5.0	6.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW-t	MT20	4.0	6.0		
L, O, P						
L	BMVW-t	MT20	4.0	4.0		
M	BMVW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
Q	BMVW-t	MT20	4.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
Q	1711	0	5-8	5-8
K	1711	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
Q	1212	783 / 0	0 / 0 0 / 0 430 / 0 0 / 0
K	1213	783 / 0	0 / 0 0 / 0 430 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	CHORDS		WEBS	
		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
A-B	0 / 38	-84.9	-84.9 0.12 (1)	10.00	C-P -11 / 43
B-C	0 / 24	-84.9	-84.9 0.21 (1)	10.00	P-D 0 / 158
C-D	-1585 / 0	-84.9	-84.9 0.26 (1)	4.98	D-O 0 / 634
D-E	-1583 / 0	-84.9	-84.9 0.38 (1)	4.85	Q-E -443 / 0
E-F	-1524 / 0	-84.9	-84.9 0.36 (1)	4.88	E-M -53 / 0
F-G	-1524 / 0	-84.9	-84.9 0.37 (1)	4.90	M-F -483 / 0
G-H	-1529 / 0	-84.9	-84.9 0.20 (1)	5.12	M-G 0 / 741
H-I	0 / 23	-84.9	-84.9 0.16 (1)	10.00	L-G 0 / 108
I-J	0 / 40	-84.9	-84.9 0.12 (1)	10.00	L-H 0 / 58
Q-B	-245 / 0	0.0	0.0 0.03 (1)	7.81	Q-C -1850 / 0
K-I	-232 / 0	0.0	0.0 0.03 (1)	7.81	H-K -1788 / 0
Q-P	0 / 1203	-18.5	-18.5 0.35 (4)	10.00	
P-O	0 / 1198	-18.5	-18.5 0.36 (4)	10.00	
O-N	0 / 1654	-18.5	-18.5 0.30 (1)	10.00	
N-M	0 / 1554	-18.5	-18.5 0.30 (1)	10.00	
M-L	0 / 1108	-18.5	-18.5 0.30 (4)	10.00	
L-K	0 / 1076	-18.5	-18.5 0.29 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")

CALCULATED VERT. DEFL.(LL) = L/999 (0.06")

ALLOWABLE DEFL.(TL) = L/360 (1.03")

CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.38/1.00 (D-E-1), BC=0.38/1.00 (O-P-4), WB=0.81/1.00 (C-Q-1), SS=0.21/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

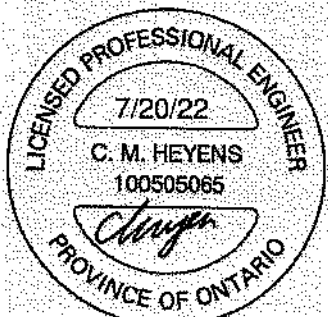
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)

JSI METAL = 0.53 (N) (INPUT = 1.00)

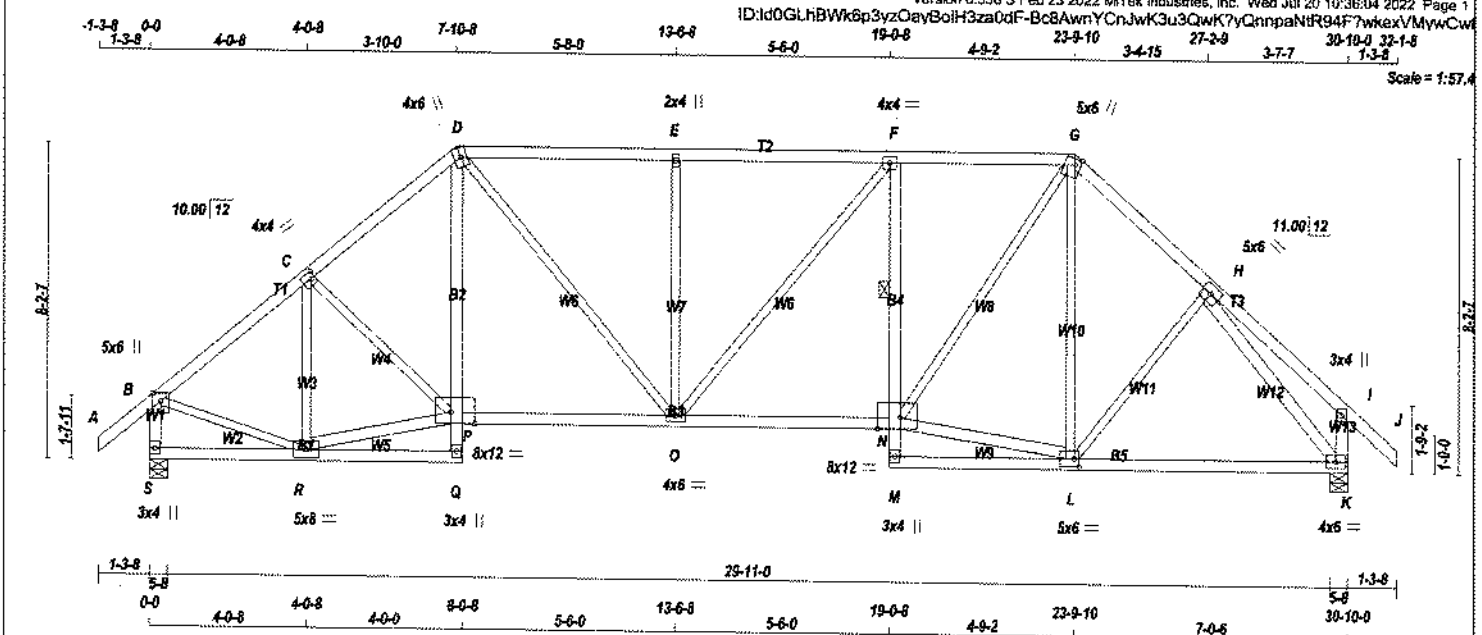


STRUCTURAL COMPONENT ONLY

DWG # TR22070599

JOB NAME 425140	TRUSS NAME T32S	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
---------------------------	---------------------------	----------------------	-----------------	--------------------------------------	----------

Tamarack Roof Truss, Burlington Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:36:04 2022 Page 1



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - B	2x4	DRY	No.2
B - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2
F - K	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2 SPF			
EXCEPT			
R - P	2x4	DRY	No.2
N - L	2x4	DRY	No.2
H - K	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN Y	X
B TMVW+p	MT20	5.0	6.0	Edge
C TMVW-t	MT20	4.0	4.0	2.00 1.25
D TTVW+m	MT20	4.0	6.0	
E TMVW-w	MT20	2.0	4.0	
F TMVW-t	MT20	4.0	4.0	
G TTVW+m	MT20	5.0	6.0	2.00 1.75
H TMVW-t	MT20	5.0	6.0	
I TMV+p	MT20	3.0	4.0	
K BMVW1-t	MT20	4.0	6.0	
L BMVW1-t	MT20	5.0	6.0	2.50 1.50
M BMV+p	MT20	3.0	4.0	
N BMVW-t	MT20	8.0	12.0	3.50 7.00
O BMVW-t	MT20	4.0	6.0	
P BMVW-t	MT20	8.0	12.0	3.50 7.25
Q BMV+p	MT20	3.0	4.0	
R BMVW-t	MT20	5.0	6.0	
S BMV1+p	MT20	3.0	4.0	
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG	BRG	IN-SX	IN-SX
S	1711	0	1711	0	0	5-8	5-8		
K	1711	0	1711	0	0	5-8	5-8		

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
S	1212	783 / 0	0 / 0	0 / 0	0 / 0
K	1213	783 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	CS1 (LC)	UNBRAC LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX CS1 (LC)
FR-TO									
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	R-C	-640 / 0	0.24 (1)	
B-C	-1568 / 0	-84.9	-84.9	0.26 (1)	4.98	R-P	0 / 1263	0.20 (1)	
C-D	-1798 / 0	-84.9	-84.9	0.28 (1)	4.72	C-P	0 / 160	0.04 (1)	
D-E	-1799 / 0	-84.9	-84.9	0.47 (1)	4.46	D-O	0 / 691	0.16 (1)	
E-F	-1799 / 0	-84.9	-84.9	0.47 (1)	4.46	O-E	-539 / 0	0.49 (1)	
F-G	-1703 / 0	-84.9	-84.9	0.20 (1)	5.11	O-F	0 / 147	0.03 (1)	
G-H	-1530 / 0	-84.9	-84.9	0.20 (1)	5.11	N-L	0 / 1108	0.18 (1)	
H-I	0 / 23	-84.9	-84.9	0.16 (1)	10.00	N-G	0 / 1067	0.24 (1)	
I-J	0 / 40	-84.9	-84.9	0.12 (1)	10.00	L-G	-142 / 20	0.19 (1)	
S-B	-1682 / 0	0.0	0.0	0.18 (1)	6.40	L-H	0 / 58	0.02 (4)	
K-I	-231 / 0	0.0	0.0	0.03 (1)	7.81	B-R	0 / 1291	0.29 (1)	
						H-K	-1788 / 0	0.71 (1)	
S-R	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
R-Q	0 / 22	-18.5	-18.5	0.08 (4)	10.00				
Q-P	0 / 33	0.0	0.0	0.04 (1)	10.00				
P-D	0 / 284	0.0	0.0	0.08 (1)	10.00				
P-O	0 / 1364	-18.5	-18.5	0.31 (1)	10.00				
O-N	0 / 1707	-18.5	-18.5	0.36 (1)	10.00				
M-N	0 / 26	0.0	0.0	0.04 (1)	10.00				
N-F	-571 / 0	0.0	0.0	0.06 (1)	6.25				
M-L	0 / 26	-18.5	-18.5	0.21 (4)	10.00				
L-K	0 / 1077	-18.5	-18.5	0.30 (4)	10.00				

TOTAL WEIGHT = 2 X 163 = 327 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.47/1.00 (E-F:1), BC=0.38/1.00 (N-O:1), WB=0.71/1.00 (H-K:1), SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

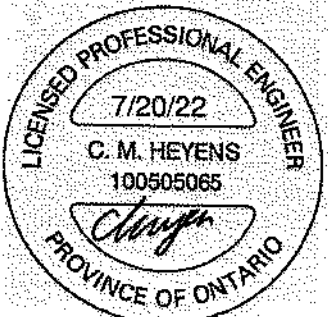
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

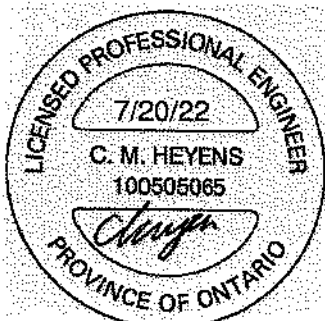
JSI GRIP= 0.87 (R) (INPUT = 0.90)
JSI METAL= 0.58 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070632 PG 1/2

JOB NAME 425140	TRUSS NAME T32S	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamareck Roof Truss, Burlington		Version 8.530 S Feb 23 2022 Mitak Industries, Inc. Wed Jul 20 10:36:04 2022 Page 2		fD:ld0GLhBWk6p3yzQayRoIH3za0dF-Bc8AwnYCnJwK3u3QwK7yQnnpaNBt94F7wkexVMwCw	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070632 PG 2/2

Version 8.530 S Feb 23 2022 MTEK industries, Inc. Wed Jul 20 10:43:02 2022 Page 1
 ID: d0GLhBWA6p3yzOayBcIH3za0dF-y0OCfcOLa0D70IP6V Mx5F90xh5DW1mu3B8YwCq

1-3-8 0-0 1-11-8 1-11-8 2-11-8 4-11-0 7-10-8 12-5-8 5-8-1 18-1-9 5-8-1 23-9-10 3-4-15 27-2-9 3-7-7 30-10-0 32-1-8 1-3-8

Scale = 1/8" = 1'-0"

8'-2-7

1-3-8

10.00 12

4x4 //

3x4 //

5x6 //

W1 W2 W3 W4 W5 W6 W7 W8 W9 W10 W11 W12 W13 W14 W15

8x12 //

4x4 //

3x4 //

5x6 //

11.00 12

4x4 //

5x6 //

1-3-8 1-0-0

1-3-8

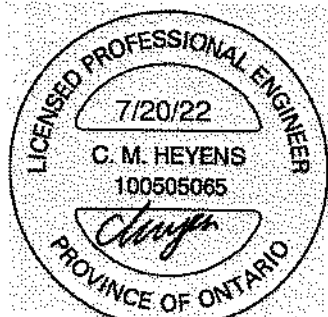
29'-11-0

1-3-8 0-0 1-11-8 1-11-8 2-11-8 4-11-0 7-10-8 12-5-8 5-8-1 18-1-9 5-8-1 23-9-10 3-4-15 27-2-9 3-7-7 30-10-0

PLATES (table is in inches)						
J	TYPE	PLATES	W	LEN	Y	X
B	TMVW+P	MT20	5.0	6.0	Edge	
C	TMV+P	MT20	3.0	4.0		
D	TMVW+T	MT20	4.0	4.0		
E	TTVW+M	MT20	5.0	6.0	2.25	1.50
F	TMV+P	MT20	3.0	4.0		
G	TMVW+T	MT20	4.0	4.0		
H	TTVW+M	MT20	5.0	6.0	2.00	1.75
I	TMVW+T	MT20	4.0	4.0	2.00	1.25
J	TMVW+T	MT20	5.0	6.0	2.50	2.50
L	BMV+P	MT20	3.0	4.0		
M	BMVW+T	MT20	4.0	4.0	2.00	1.50
N	BMVW+T	MT20	4.0	4.0		
O	BS-I	MT20	3.0	8.0		
P	BMVW+T	MT20	4.0	10.0		
Q	BMV+P	MT20	3.0	4.0		
R	BMVW+T	MT20	8.0	12.0	3.50	4.25
S	BMVW+T	MT20	4.0	4.0		
T	BMVW+T	MT20	8.0	12.0	3.25	4.00
U	BMV+P	MT20	3.0	4.0		

MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE	VERT. DISPL	MAX. DISPL	MEMB.	FORCE	MAX. DISPL	MAX. DISPL
	(LBS)	(IN)	(IN)		(LBS)	(IN)	(IN)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 38	-84.9	-84.9	12 (1)	10.00	T-D	-75 / 0
B-C	-1899 / 0	-84.9	-84.9	15 (1)	4.75	D-S	-233 / 0
C-D	-1868 / 0	-84.9	-84.9	17 (1)	4.75	S-E	0 / 312
D-E	-1787 / 0	-84.9	-84.9	17 (1)	4.85	E-R	0 / 674
E-F	-1739 / 0	-84.9	-84.9	33 (1)	4.70	R-P	0 / 1635
F-G	-1735 / 0	-84.9	-84.9	46 (1)	4.53	R-G	0 / 305
G-H	-1538 / 0	-84.9	-84.9	45 (1)	4.77	P-G	-769 / 0
H-I	-1519 / 0	-84.9	-84.9	20 (1)	5.12	P-H	0 / 757
I-J	-1471 / 0	-84.9	-84.9	21 (1)	5.19	N-H	0 / 89
J-K	0 / 40	-84.9	-84.9	12 (1)	10.00	N-I	-5 / 5
V-B	-1674 / 0	0.0	0.0	18 (1)	6.42	M-I	-368 / 0
L-J	-1678 / 0	0.0	0.0	18 (1)	6.41	M-J	0 / 1182
						V-T	-48 / 0
V-U	0 / 42	-18.5	-18.5	02 (4)	10.00	B-T	0 / 1463

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.67 (B) (INPUT = 1.00)



JOB NAME 425141	TRUSS NAME T32S1	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
--------------------	---------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitak Industries, Inc. Wed Jul 20 10:43:02 2022 Page 2
ID:Id0GLhBwk6p3yzOayBoIH3za0dF-y0OCfscOLla0D70IP6V Mx5F9Xh5DW1mu3B8ywCq7

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWV1+p	MT20	4.0	4.0		

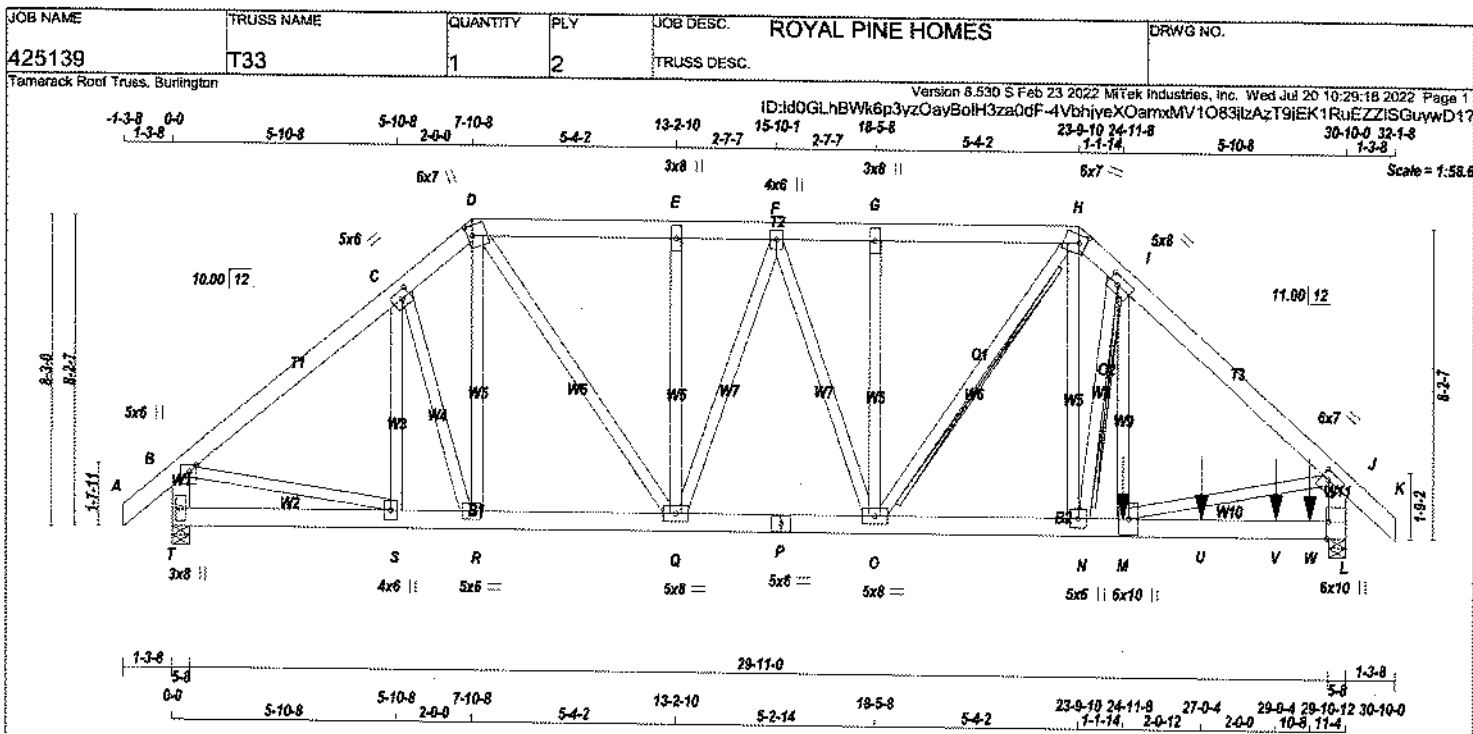
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070638 PG 2/2



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - D 2x6 DRY No.2 SPF
D - H 2x6 DRY No.2 SPF
H - K 2x6 DRY No.2 SPF
T - B 2x6 DRY No.2 SPF
L - J 2x6 DRY No.2 SPF
T - P 2x6 DRY No.2 SPF
P - L 2x6 DRY No.2 SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS
A-D 2 12 TOP
D-H 2 12 TOP
H-K 2 12 TOP
T-B 2 12 TOP
L-J 2 12 TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS
T-P 2 12 TOP
P-L 2 12 SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS
N-I 1 6 SIDE(605.6)
I-M 1 6 SIDE(573.0)
2x4 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
T	2864	0	2864	0	5-8	5-8
L	7696	0	7696	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
T	2029	1312 / 0	0 / 0	0 / 0	0 / 0	717 / 0	0 / 0
L	5448	3552 / 0	0 / 0	0 / 0	0 / 0	1895 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR GRID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT H-O, I-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)		MAX. CSI (LC)
		FROM	TO				FROM	TO	
FR-TO	0 / 39	-84.9	-84.9	0.03 (1)	10.00	S-C	-408 / 0	0.09 (1)	
A-B	-3127 / 0	-84.9	-84.9	0.12 (1)	6.12	C-R	-97 / 0	0.03 (1)	
B-C	-3208 / 0	-84.9	-84.9	0.10 (1)	6.08	R-D	0 / 181	0.02 (1)	
C-D	-3643 / 0	-84.9	-84.9	0.10 (1)	5.85	D-G	0 / 2021	0.18 (1)	
D-E	-3543 / 0	-84.9	-84.9	0.09 (1)	5.87	Q-E	-420 / 0	0.18 (1)	
E-F	-4271 / 0	-84.9	-84.9	0.09 (1)	5.47	G-G	-400 / 0	0.17 (1)	
F-G	-4271 / 0	-84.9	-84.9	0.11 (1)	5.44	O-H	-827 / 0	0.31 (1)	
G-H	-8468 / 0	-84.9	-84.9	0.17 (1)	4.55	N-H	0 / 4723	0.42 (1)	
H-I	-7520 / 0	-84.9	-84.9	0.33 (1)	4.15	N-I	-4525 / 0	0.83 (1)	
I-J	0 / 41	-84.9	-84.9	0.04 (1)	10.00	M-I	0 / 4961	0.44 (1)	
J-K	-2809 / 0	0.0	0.0	0.10 (1)	7.81	B-S	0 / 2464	0.22 (1)	
T-B	-6626 / 0	0.0	0.0	0.24 (1)	5.78	M-J	0 / 5835	0.50 (1)	
L-J	0 / 0	-18.5	-18.5	0.04 (4)	10.00	Q-F	-1182 / 0	0.57 (1)	
T-S	0 / 0	-18.5	-18.5	0.17 (1)	10.00	F-O	0 / 1091	0.10 (1)	
S-R	0 / 2421	-18.5	-18.5	0.17 (1)	10.00				
R-Q	0 / 2392	-18.5	-18.5	0.17 (1)	10.00				
Q-P	0 / 3822	-18.5	-18.5	0.29 (1)	10.00				
P-O	0 / 3822	-18.5	-18.5	0.29 (1)	10.00				
O-N	0 / 4743	-18.5	-18.5	0.52 (1)	10.00				
N-M	0 / 5517	-18.5	-18.5	0.57 (1)	10.00				
M-U	0 / 0	-18.5	-18.5	0.62 (1)	10.00				
U-V	0 / 0	-18.5	-18.5	0.62 (1)	10.00				
V-W	0 / 0	-18.5	-18.5	0.62 (1)	10.00				
W-L	0 / 0	-18.5	-18.5	0.62 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.33/1.00 (I-J:1), BC=0.62/1.00 (L-M:1),
WB=0.83/1.00 (N:1), SS=0.35/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1673

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 9.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.83 (M) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070600 PG 1/2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIRL	TYPE	HEEL	CONN.
M	24-11-8	-3698	-3698	---	BACK	VERT	TOTAL	C1
U	27-0-4	-529	-529	---	BACK	VERT	TOTAL	C1
V	29-0-4	-649	-649	---	BACK	VERT	TOTAL	C1

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T33	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:29:18 2022 Page 2
ID:Id0GLhBWk6p3vzOayBoIH3za0dF-4VbhiyeXCarmxMV1O83ltzAzT9lEK1RuEZZISGuwD1?

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMVW+l	MT20	5.0	6.0	2.50	2.75
D	TTVW+m	MT20	6.0	7.0	3.25	1.50
E	TMVW+w	MT20	3.0	8.0		
F	TMVW+h	MT20	4.0	6.0		
G	TMVW+e	MT20	3.0	8.0		
H	TTVW-m	MT20	5.0	7.0	2.75	2.25
I	TMVW+l	MT20	5.0	6.0	2.50	3.25
J	TMVW-l	MT20	6.0	7.0	2.50	2.25
L	BMV1+H	MT20	6.0	10.0	Edge	0.50
M	BMVW+l	MT20	5.0	10.0		
N	BMVW+l	MT20	5.0	6.0		
O	BMVW+l	MT20	5.0	6.0		
P	BS-4	MT20	5.0	6.0		
Q	BMVW+l	MT20	5.0	6.0		
R	BMVW+l	MT20	5.0	6.0		
S	BMVW+l	MT20	4.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	28-10-12	-167	-167		TOP	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



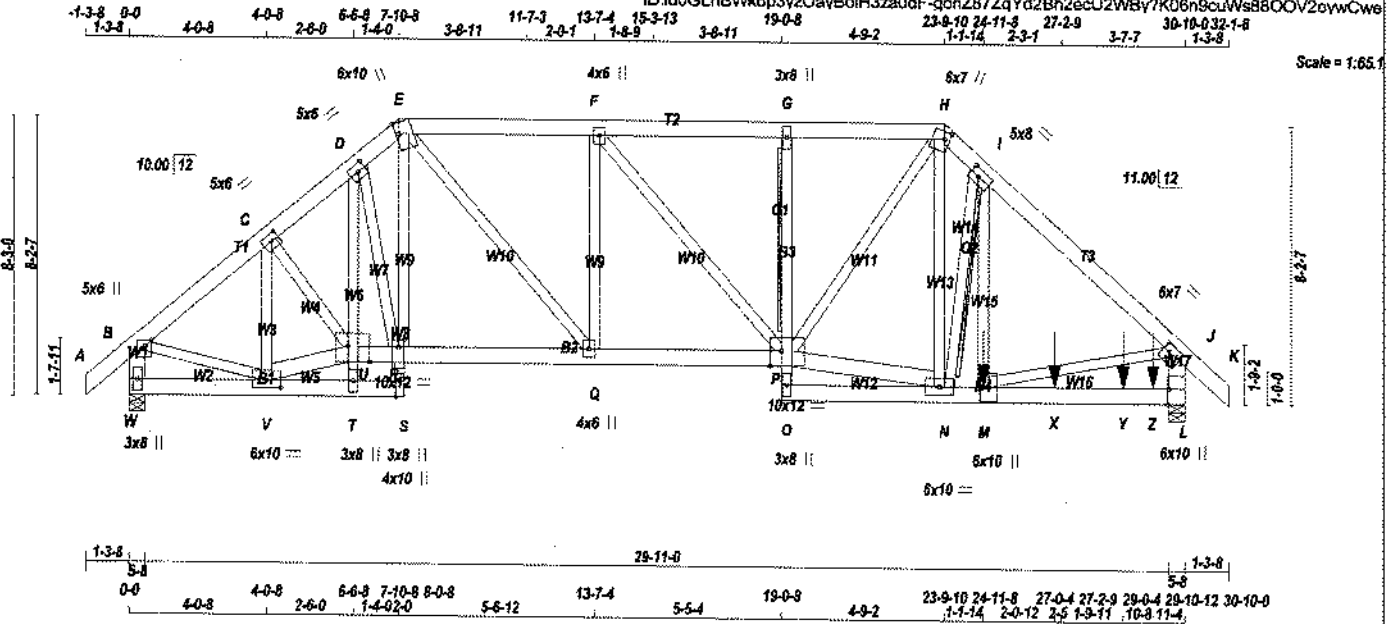
STRUCTURAL COMPONENT ONLY
DWG # TR22070600

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425140	T33S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:36:05 2022 Page 1

ID:1d0GLhBWk6p3yzOay8oH3za0dF-qchZ87ZqYd2Bh2ecU2WBy7K06n9cuW88OOV2oywCw6



TOTAL WEIGHT = 2 X 251 = 501 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - E	2x6	DRY	No.2	SPF	
E - H	2x6	DRY	No.2	SPF	
H - K	2x6	DRY	No.2	SPF	
W - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
W - S	2x6	DRY	No.2	SPF	
U - P	2x6	DRY	No.2	SPF	
O - G	2x4	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
V - U	2x6	DRY	No.2	SPF	
P - N	2x6	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-E	2	12
E-H	2	12
H-K	2	12
W-B	2	12
L-J	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W-S	2	12
U-P	2	12
O-L	2	12
G-O	1	12
WEBS : (0.122"x3") SPIRAL NAILS		
I-M	1	6
N-I	1	6
2x4	1	6
2x6	2	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
W	2886	0	2886	0
L	7702	0	7702	0

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW
W	2046
L	5451

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT G-P, I-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
A-B	0/39	-84.9	-84.9	0.03 (1)	10.00	V-U	-1533/0
B-C	-3012/0	-84.9	-84.9	0.06 (1)	6.25	C-U	0/2484
C-D	-3750/0	-84.9	-84.9	0.08 (1)	5.78	P-N	0/893
D-E	-3805/0	-84.9	-84.9	0.05 (1)	5.88	P-H	0/4829
E-F	-4167/0	-84.9	-84.9	0.12 (1)	5.48	N-H	0/446
F-G	-5010/0	-84.9	-84.9	0.14 (1)	5.10	B-V	0/3608
G-H	-5005/0	-84.9	-84.9	0.10 (1)	5.13	M-J	0/2411
H-I	-6354/0	-84.9	-84.9	0.17 (1)	4.58	N-I	0/5637
I-J	-7522/0	-84.9	-84.9	0.35 (1)	4.13	M-I	0/5005
J-K	0/41	-84.9	-84.9	0.04 (1)	10.00	N-I	-4452/0
W-B	-2649/0	0.0	0.0	0.10 (1)	7.81	T-U	0/48
L-J	-6636/0	0.0	0.0	0.24 (1)	5.78	U-D	0/314
W-V	0/0	-18.5	-18.5	0.02 (4)	10.00	R-E	0/410
V-T	0/32	-18.5	-18.5	0.01 (4)	10.00	D-R	-357/0
T-S	0/0	-18.5	-18.5	0.01 (4)	10.00	Q-F	-1538/0
U-R	0/2842	-18.5	-18.5	0.22 (1)	10.00	E-Q	0/2143
R-Q	0/2776	-18.5	-18.5	0.22 (1)	10.00	F-P	0/1321
Q-P	0/4168	-18.5	-18.5	0.29 (1)	10.00		
O-P	-84/0	0.0	0.0	0.02 (1)	7.81		
P-G	-398/0	0.0	0.0	0.03 (1)	7.81		
O-N	0/33	-18.5	-18.5	0.22 (1)	10.00		
N-M	0/5339	-18.5	-18.5	0.06 (1)	10.00		
M-X	0/0	-18.5	-18.5	0.62 (1)	10.00		
X-Y	0/0	-18.5	-18.5	0.62 (1)	10.00		
Y-Z	0/0	-18.5	-18.5	0.62 (1)	10.00		
Z-L	0/0	-18.5	-18.5	0.62 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 38.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSH TO=0.35/1.00 (I-J), BC=0.82/1.00 (L-M), WB=0.82/1.00 (I-N), SSI=0.35/1.00 (L-M)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

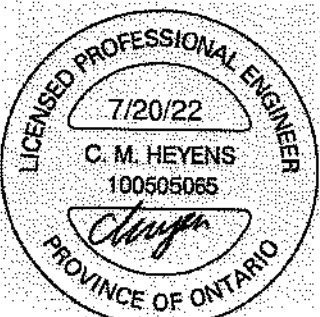
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.80 (M) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070633 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425140	T33S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:36:06 2022 Page 2
ID:Id0GLhBWK6p3yzOayBoIH3za0dF-8 FxLTaSjxA2IBDc211QVCIBsAUrdz6iN272eFywCwd

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMVW-t	MT20	5.0	6.0	2.50	2.75
D	TMVW-l	MT20	5.0	6.0	2.50	2.75
E	TTWW+m	MT20	6.0	10.0	4.00	1.25
F	TMVW+l	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	8.0		
H	TTWW+m	MT20	6.0	7.0	2.75	1.75
I	TMVW-t	MT20	5.0	8.0	2.50	3.25
J	TMVW-l	MT20	6.0	7.0	2.50	2.25
L	BMV1+t	MT20	8.0	10.0	Edge	0.50
M	BMVW+l	MT20	6.0	10.0		
N	BMVWW-l	MT20	6.0	10.0		
O	BMV+p	MT20	3.0	8.0		
P	BMVWWW-l	MT20	10.0	12.0	5.25	4.00
Q	BMVW+l	MT20	4.0	6.0		
R						
R	BMVWW+l	MT20	4.0	10.0	2.50	1.75
S	NP+w	MT20	3.0	8.0	Edge	1.50
T	BMV+w	MT20	3.0	8.0		
U	BMVWWW+l	MT20	10.0	12.0	5.50	7.50
V	BMVWWW-l	MT20	6.0	10.0	2.75	5.00
W	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES (1)

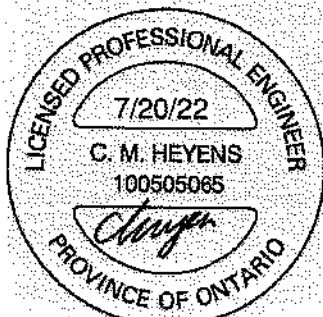
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	24-11-8	-3698	-3698	---	BACK	VERT	TOTAL	---	C1
X	27-0-4	-529	-529	---	BACK	VERT	TOTAL	---	C1
Y	29-0-4	-849	-849	---	BACK	VERT	TOTAL	---	C1
Z	29-10-12	-167	-167	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22070633 PG 2/2

JOB NAME 425141	TRUSS NAME T33S1	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
---------------------------	----------------------------	----------------------	-----------------	--------------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 5 Feb 23 2022 Mittek Industries, Inc. Wed Jul 20 10:43:04 2022 Page 2
ID:1d0GLhBWk6p3yzQayBoiH3za0dF-uPWY4PeetMkSRa8XXSRMBdig9bZ0XpV4NAF0YwCof

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TMVW+p	MT20	4.0	8.0		
D	TMVW-t	MT20	5.0	6.0	2.50	2.75
E	TTVW+m	MT20	6.0	7.0	3.25	1.50
F	TMV+p	MT20	3.0	8.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	4.0	6.0		
I	TTVW-m	MT20	6.0	7.0	2.75	2.25
J	TMVW-t	MT20	5.0	8.0	2.50	3.25
K	TMVW-t	MT20	6.0	7.0	2.50	2.25
M	BMV1-t	MT20	6.0	10.0	Edge	0.50
N	BMVW-t	MT20	6.0	10.0		
O	BMVW-t	MT20	5.0	6.0	3.00	2.25
P	BS-t	MT20	5.0	6.0		
Q, U, V						
Q	BMVW-t	MT20	4.0	6.0		
R	BMVW-t	MT20	6.0	10.0	2.75	5.00
S	BMV+p	MT20	3.0	8.0		
T	BMVW-t	MT20	10.0	12.0	Edge	4.50
W	BMVW-m	MT20	10.0	12.0	6.50	8.00
X	BMV+p	MT20	3.0	8.0		
Y	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

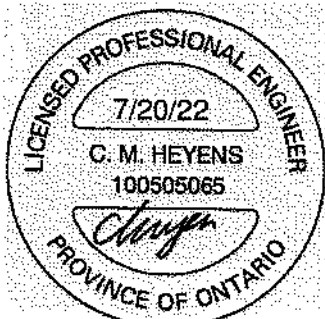
CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRAC	MEMB.	FORCE (LBS)	MAX FACTORED CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
AB-M	0/0	-18.5	-18.5	0.82 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	24-11-8	-3698	-3698		BACK	VERT	TOTAL		C1
Z	27-0-4	-529	-529		BACK	VERT	TOTAL		C1
AA	29-0-4	-649	-649		BACK	VERT	TOTAL		C1
AB	29-10-12	-167	-167		TOP	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



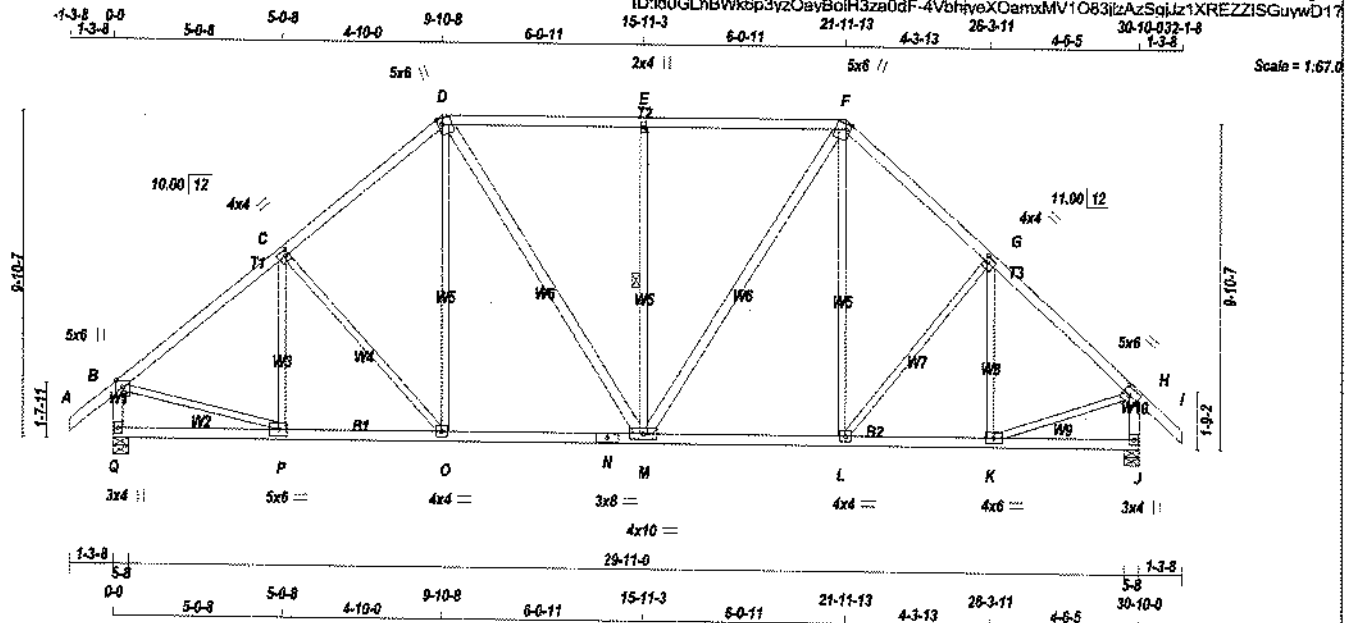
STRUCTURAL COMPONENT ONLY
DWG # TR22070639 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T34	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:29:18 2022 Page 1

ID:ld0GLhBWk6p3yzOayBoIH3za0dF-4VbhyeXQamxMV1083jzAzSgJz1XREZZISGuywD17



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - M	2x4	DRY	No.2	SPF	
M - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	Edge	
C	TMWV-l	MT20	4.0	4.0	2.00	1.25
D	TTWV+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWV+m	MT20	5.0	6.0	2.00	1.50
G	TMWV-l	MT20	4.0	4.0	2.00	1.25
H	TMWV-l	MT20	5.0	6.0	2.50	2.50
J	BMV1+p	MT20	3.0	4.0		
K	BMWV-l	MT20	4.0	6.0		
L	BMWV-l	MT20	4.0	4.0		
M	BMWVW-l	MT20	4.0	10.0		
N	BS-l	MT20	3.0	6.0		
O	BMWV-l	MT20	4.0	4.0		
P	BMWV-l	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
Q	VERT 1711	DOWN 1711	0	5-8
J	HORIZ 1711	UP 1711	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1212	783 / 0	0 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0
J	1213	783 / 0	0 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	P-C	-232 / 0
B-C	-1624 / 0	-84.9	-84.9	0.31 (1)	4.89	C-O	-225 / 0
C-D	-1486 / 0	-84.9	-84.9	0.30 (1)	5.08	O-D	0 / 268
D-E	-1326 / 0	-84.9	-84.9	0.43 (1)	5.10	D-M	0 / 377
E-F	-1326 / 0	-84.9	-84.9	0.43 (1)	5.10	M-E	-628 / 0
F-G	-1467 / 0	-84.9	-84.9	0.24 (1)	5.17	M-F	0 / 502
G-H	-1525 / 0	-84.9	-84.9	0.25 (1)	5.08	L-F	0 / 212
H-I	0 / 40	-84.9	-84.9	0.12 (1)	10.00	L-G	-146 / 0
Q-B	-1671 / 0	0.0	0.0	0.18 (1)	6.42	K-G	-274 / 0
J-H	-1675 / 0	0.0	0.0	0.18 (1)	6.41	B-P	0 / 1314
Q-P	0 / 0	-18.5	-18.5	0.10 (4)	10.00	K-H	0 / 1201
P-O	0 / 1272	-18.5	-18.5	0.26 (1)	10.00		
O-N	0 / 1124	-18.5	-18.5	0.26 (1)	10.00		
N-M	0 / 1124	-18.5	-18.5	0.26 (1)	10.00		
M-L	0 / 1057	-18.5	-18.5	0.25 (1)	10.00		
L-K	0 / 1146	-18.5	-18.5	0.24 (1)	10.00		
K-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

TOTAL WEIGHT = 160 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CSI: TC=0.43/1.00 (D-E-1), SC=0.26/1.00 (O-P-1), WB=0.41/1.00 (E-M-1), SS=0.25/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

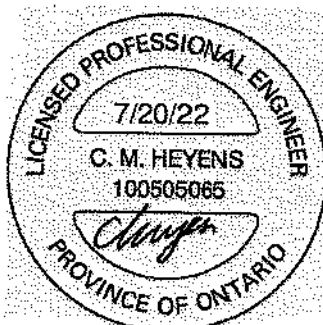
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL = 0.61 (B) (INPUT = 1.00)

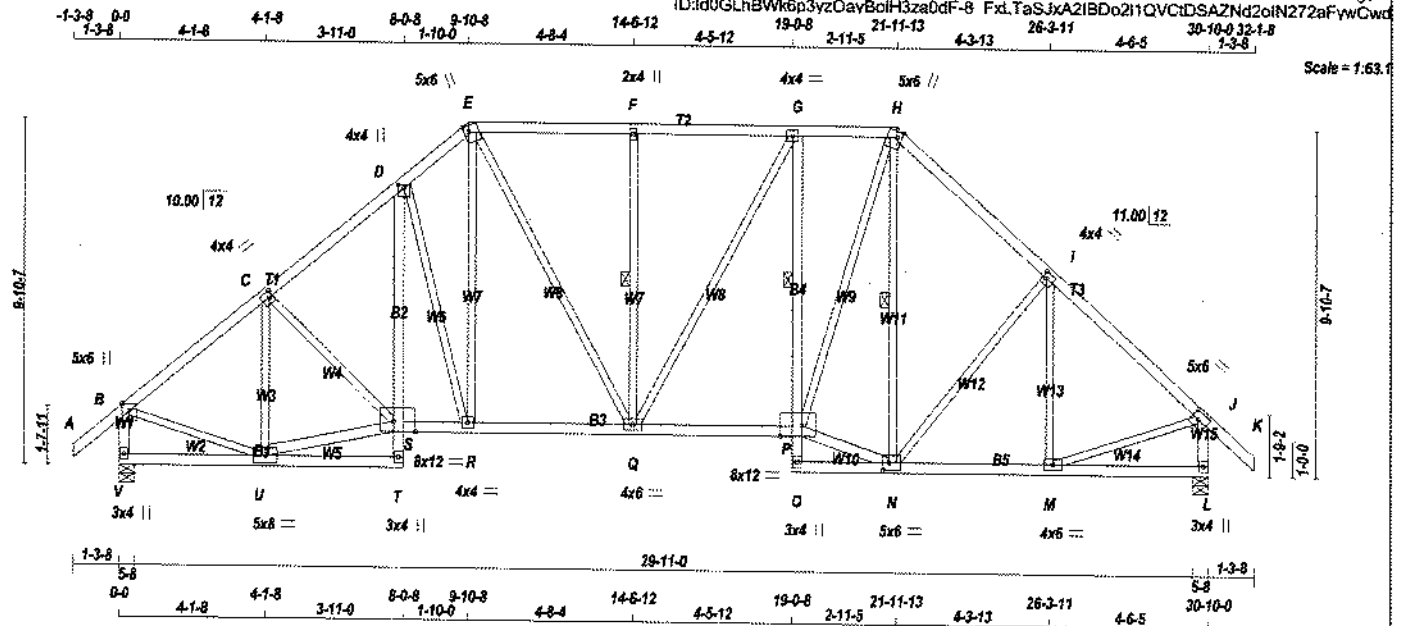


STRUCTURAL COMPONENT ONLY
DWG # TR22070601

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425140	T34S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:36:06 2022 Page 1
ID: d0GLhBWk6p3yzOayBoIH3za0dF-8 Fx.TaSJxkAZIBDo2h1QVCtDSAZNd2oIN272aFywCwd



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
V - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
V - T	2x4	DRY	No.2	SPF	
T - D	2x4	DRY	No.2	SPF	
S - P	2x4	DRY	No.2	SPF	
O - G	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
U - S	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C TMVW-t	MT20	4.0	4.0	2.00	1.25
D TMVW+p	MT20	4.0	4.0	1.00	2.00
E TTWV+m	MT20	5.0	6.0	2.25	1.50
F TMVW-t	MT20	2.0	4.0		
G TMVW-t	MT20	4.0	4.0		
H TTWV+m	MT20	5.0	6.0	2.00	1.75
I TMVW-t	MT20	4.0	4.0	2.00	1.25
J TMVW-t	MT20	5.0	6.0	2.50	2.50
L BMVt+p	MT20	3.0	4.0		
M BMVW-t	MT20	4.0	6.0		
N BMVW-t	MT20	5.0	6.0	2.50	2.00
O BMV+p	MT20	3.0	4.0		
P BMVW-t	MT20	8.0	12.0	3.50	7.25
Q BMVW-t	MT20	4.0	6.0		
R BMVW-t	MT20	4.0	4.0		
S BMVW-t	MT20	8.0	12.0	3.50	7.50
T BMV+p	MT20	3.0	4.0		
U BMVW-t	MT20	5.0	8.0		
V BMVt+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
V	1711	0	5-8	5-8
L	1711	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
V	1212	783 / 0 0 / 0 0 / 0 430 / 0 0 / 0
L	1213	783 / 0 0 / 0 0 / 0 430 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P, F-Q, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO							
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	U-C	-639 / 0 0.24 (1)
B-C	-1570 / 0	-84.9	-84.9	0.20 (1)	5.08	U-S	0 / 1268 0.20 (1)
C-D	-1799 / 0	-84.9	-84.9	0.19 (1)	4.82	C-S	0 / 173 0.04 (1)
D-E	-1649 / 0	-84.9	-84.9	0.08 (1)	5.11	D-R	-430 / 0 0.45 (1)
E-F	-1454 / 0	-84.9	-84.9	0.23 (1)	5.19	R-E	0 / 463 0.10 (1)
F-G	-1454 / 0	-84.9	-84.9	0.23 (1)	5.19	E-Q	0 / 417 0.09 (1)
G-H	-1384 / 0	-84.9	-84.9	0.12 (1)	5.45	Q-F	-450 / 0 0.23 (1)
H-I	-1486 / 0	-84.9	-84.9	0.25 (1)	5.16	Q-G	0 / 183 0.04 (1)
I-J	-1526 / 0	-84.9	-84.9	0.25 (1)	5.08	P-N	0 / 1110 0.18 (1)
J-K	0 / 40	-84.9	-84.9	0.12 (1)	10.00	P-H	0 / 958 0.22 (1)
V-B	-1681 / 0	0.0	0.0	0.18 (1)	6.41	N-H	-209 / 0 0.14 (1)
L-J	-1675 / 0	0.0	0.0	0.18 (1)	6.41	M-I	-272 / 0 0.15 (1)
V-U	0 / 0	-18.5	-18.5	0.08 (4)	10.00	B-U	0 / 1288 0.29 (1)
U-T	0 / 22	-18.5	-18.5	0.08 (4)	10.00	M-J	0 / 1201 0.27 (1)
T-S	0 / 32	0.0	0.0	0.04 (1)	10.00		
S-D	0 / 257	0.0	0.0	0.07 (1)	10.00		
R-Q	0 / 1373	-18.5	-18.5	0.26 (1)	10.00		
Q-P	0 / 1254	-18.5	-18.5	0.25 (1)	10.00		
P-O	0 / 1367	-18.5	-18.5	0.27 (1)	10.00		
O-N	0 / 17	0.0	0.0	0.03 (1)	10.00		
N-M	-477 / 0	0.0	0.0	0.06 (1)	6.25		
M-L	0 / 20	-18.5	-18.5	0.06 (4)	10.00		
	0 / 1147	-18.5	-18.5	0.24 (1)	10.00		
	0 / 0	-18.5	-18.5	0.06 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.25/1.00 (I-J), BC=0.27/1.00 (P-Q), WB=0.45/1.00 (D-R), SS=0.19/1.00 (E-F)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

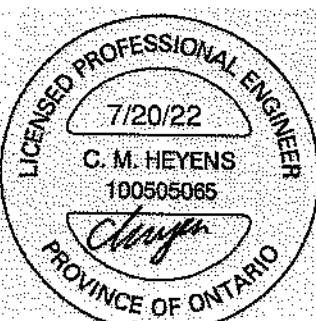
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.58 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070634 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425140	T34S	1	1	TRUSS DESC.		

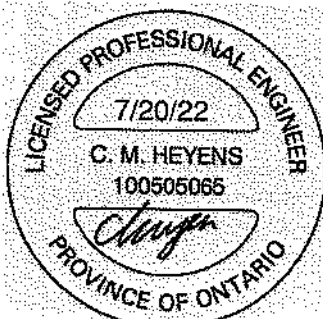
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:36:06 2022 Page 2
ID:Id0GLhBWk6p3yzQayBoIH3za0dF-8 FxLTaSJx2IBDo2H1QVCIDSAZNd2oIN272aFywQwd

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

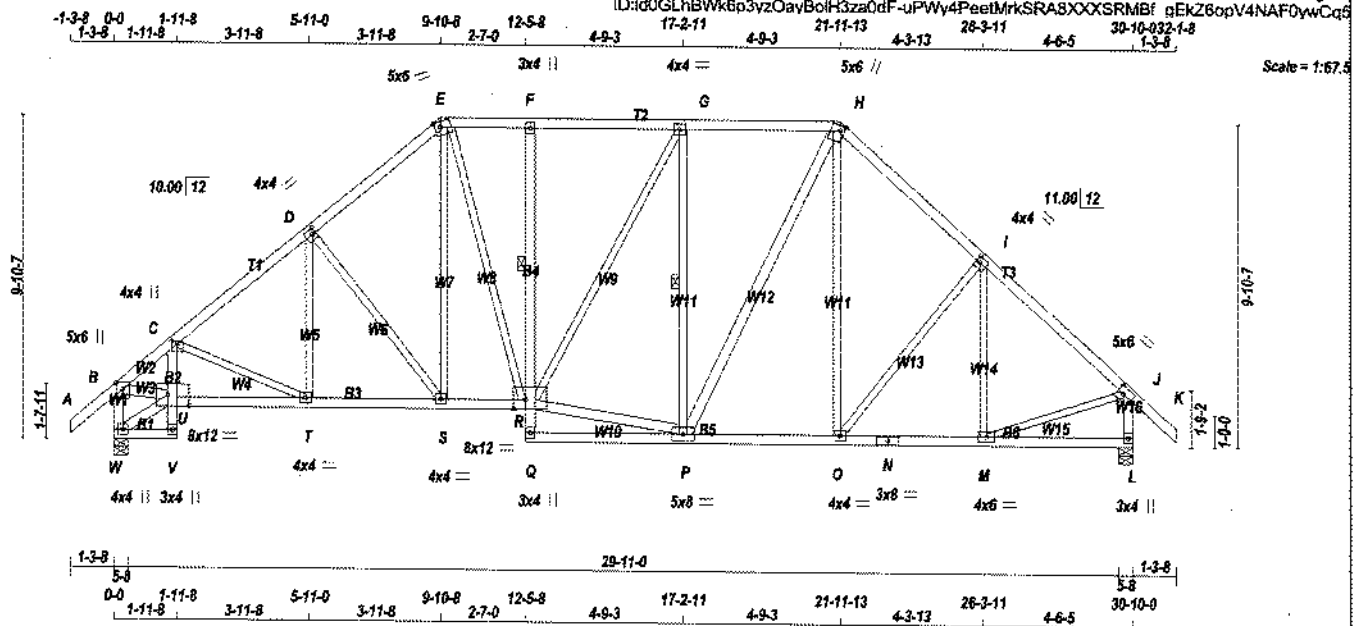
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070634 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425141	T34S1	1	1	ROYAL PINE HOMES	

Tamarack Roof Truss, Burlington Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:43:04 2022 Page 1



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
B - U	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	TMVW-m	MT20	5.0	6.0	2.00	2.25
F	TMVW+p	MT20	3.0	4.0		
G	TMVW-t	MT20	4.0	4.0		
H	TMVW+m	MT20	5.0	6.0	2.00	1.50
I	TMVW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-t	MT20	5.0	6.0	2.50	2.50
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMV+p	MT20	3.0	4.0		
R	BMVW-t	MT20	8.0	12.0	3.25	4.25
U	BMVW-t	MT20	8.0	12.0	4.00	7.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQ'D BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
L	1709	0	1709	0	5-8	5-8
W	1714	0	1714	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1211	781/0	0/0	0/0	0/0	429/0	0/0
W	1214	784/0	0/0	0/0	0/0	430/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-R, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD 1.C1 MAX (PLF)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0/36	-84.9	-84.9 0.12 (1)	10.00
B-C	-2039/0	-84.9	-84.9 0.13 (1)	4.64
C-D	-1904/0	-84.9	-84.9 0.21 (1)	4.70
D-E	-1646/0	-84.9	-84.9 0.20 (1)	4.99
E-F	-1389/0	-84.9	-84.9 0.12 (1)	5.42
F-G	-1387/0	-84.9	-84.9 0.24 (1)	5.27
G-H	-1283/0	-84.9	-84.9 0.24 (1)	5.44
H-I	-1461/0	-84.9	-84.9 0.25 (1)	5.18
I-J	-1523/0	-84.9	-84.9 0.25 (1)	5.09
J-K	0/40	-84.9	-84.9 0.12 (1)	10.00
W-B	-1670/0	0.0	0.0 0.18 (1)	6.42
L-J	-1673/0	0.0	0.0 0.18 (1)	6.42
M-L				
V-V	0/43	-18.5	-18.5 0.02 (4)	10.00
V-U	0/19	0.0	0.0 0.07 (1)	10.00
U-C	-88/3	0.0	0.0 0.05 (1)	7.81
U-T	0/1585	-18.5	-18.5 0.29 (1)	10.00
T-S	0/1475	-18.5	-18.5 0.27 (1)	10.00
S-R	0/1247	-18.5	-18.5 0.23 (1)	10.00
Q-R	0/38	0.0	0.0 0.03 (1)	10.00
R-F	-310/0	0.0	0.0 0.04 (1)	6.25
Q-P	0/21	-18.5	-18.5 0.10 (4)	10.00
P-O	0/1053	-18.5	-18.5 0.23 (1)	10.00
O-N	0/1145	-18.5	-18.5 0.22 (1)	10.00
N-M	0/1145	-18.5	-18.5 0.22 (1)	10.00
M-L	0/0	-18.5	-18.5 0.08 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.12")

CSI: TO=0.25/1.00 (I-J), BC=0.28/1.00 (T-U), WB=0.42/1.00 (G-P), SS=0.19/1.00 (G-H, I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

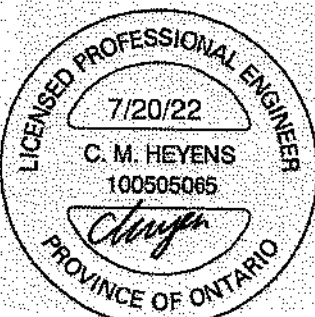
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.80)
JSI METAL= 0.72 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070640 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425141	T34S1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:43:04 2022 Page 2
ID:Id0GLhBWK6p3yzQay8oIH3za0dF-uPWY4PeetMrkSRA8XXXSRMBf gEKZ6opV4NAF0ywCg5

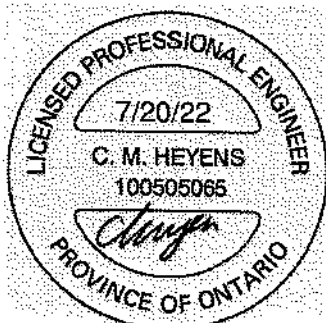
PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
V BMV+p	MT20	3.0	4.0		
W BMVW1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070640 PG 2/2

Version 8.530 S Feb 23 2022 MTask Industries, Inc. Wed Jul 20 10:13 2022 Page 1

ID:1d0GLhBwK6p3yzQayBoIH3za0dF-z4QUgmJ1RTI7Sxkys8Cehm1q2aQKcK66CywD08

1-3-8 0-0 4-10-4 4-10-4 4-7-12 9-6-0 5-5-0 14-11-0 5-5-0 20-4-0 4-7-12 24-11-12 4-10-4 29-10-0 31-1-8 1-3-8

Scale: 3/16"=1'

1-3-8 0-0 4-10-4 4-10-4 4-7-12 9-6-0 5-5-0 14-11-0 5-5-0 20-4-0 4-7-8 24-11-8 24-11-12 27-0-4 29-0-4 29-10-0 9-12 1-3-8

Scale: 3/16"=1'

LUMBER
N.L.G.A. RULES
CHORDS SIZE DRY LUMBER DESCR.
A - D 2x4 DRY No.2 SPF
D - F 2x4 DRY No.2 SPF
F - I 2x4 DRY No.2 SPF
Q - B 2x6 DRY No.2 SPF
J - H 2x6 DRY No.2 SPF
Q - M 2x6 DRY No.2 SPF
M - J 2x6 DRY No.2 SPF
ALL WEBS 2x4 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:
CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS
A-D 1 12 TOP
D-F 1 12 TOP
F-I 1 12 TOP
Q-B 2 12 TOP
J-H 2 12 TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS
Q-M 2 12 TOP
M-J 2 12 SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS
G-K 1 2 SIDE(1155.4)
2x4 2 2
O-C 1 6
O-D 1 6
N-D 1 6
E-N 1 6
N-F 1 6
L-F 1 6
L-G 1 6
P-B 1 6
K-H 1 6
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
FACTORED GROSS REACTION
JT VERT 2590 0
Q 7528 0
J 7528 0
MAXIMUM FACTORED GROSS REACTION
DOWN 2590 0
HORZ 0 0
UPLIFT 0 0
INPUT BRG IN-SX 5-8 5-8
REQRD BRG IN-SX 5-8 5-8
UNFACTORED REACTIONS
1ST LCASE MAX./MIN. COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
Q 1835 1186 / 0 0 / 0 0 / 0 649 / 0 0 / 0
J 5332 3455 / 0 0 / 0 0 / 0 649 / 0 0 / 0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.06 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
2x4 DRY SPF No.2 T-BRACE AT E-N, G-L
2x6 DRY SPF No.2 T-BRACE AT F-N
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW
LOADING
TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 38	-84.9 -84.9 0.06 (1)	10.00	P-C	-452 / 0
B-C	-2687 / 0	-84.9 -84.9 0.23 (1)	5.34	C-O	-43 / 0
C-D	-2716 / 0	-84.9 -84.9 0.22 (1)	5.32	O-D	0 / 132
D-E	-2804 / 0	-84.9 -84.9 0.28 (1)	5.17	D-N	0 / 1461
E-F	-2804 / 0	-84.9 -84.9 0.28 (1)	5.17	N-E	-547 / 0
F-G	-4076 / 0	-84.9 -84.9 0.25 (1)	4.52	N-F	-678 / 0
G-H	-7383 / 0	-84.9 -84.9 0.49 (1)	3.36	L-F	0 / 2842
H-I	0 / 38	-84.9 -84.9 0.06 (1)	10.00	L-G	-3845 / 0
Q-B	-2641 / 0	0.0 0.0 0.09 (1)	7.81	K-G	0 / 4503
J-H	-6460 / 0	0.0 0.0 0.24 (1)	5.84	B-P	0 / 2152
				K-H	0 / 5869
Q-P	0 / 0	-18.5 -18.5 0.03 (4)	10.00		
P-O	0 / 2087	-18.5 -18.5 0.15 (1)	10.00		
O-N	0 / 2060	-18.5 -18.5 0.16 (1)	10.00		
N-M	0 / 3149	-18.5 -18.5 0.23 (1)	10.00		
M-L	0 / 3149	-18.5 -18.5 0.23 (1)	10.00		
L-K	0 / 5691	-18.5 -18.5 0.47 (1)	10.00		
K-R	0 / 0	-18.5 -18.5 0.40 (1)	10.00		
R-S	0 / 0	-18.5 -18.5 0.40 (1)	10.00		
S-J	0 / 0	-18.5 -18.5 0.40 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIRL	TYPE	HEEL	CONN.
K	24-11-8	-3635	-3635	-	FRONT	VERT	TOTAL	-	C1
R	27-0-4	-529	-529	-	FRONT	VERT	TOTAL	-	C1
S	29-0-4	-651	-651	-	FRONT	VERT	TOTAL	-	C1

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF
SPACING = 24.0 IN. C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFL.(LL) = L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/899 (0.14")
CSI: TC=0.49/1.00 (G-H:1), BC=0.47/1.00 (K-L:1), WB=0.76/1.00 (G-L:1), SSI=0.35/1.00 (J-K:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL = 0.85 (K) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY
DWG # TR22070613 PG 1/2

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070613 PG 1/2

CONTINUED ON PAGE 2

JOB NAME 425139	TRUSS NAME T35	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:30:13 2022 Page 2
ID:Id0GLhBWk6p3yzOayBoIH3za0dF-z4QUGmJ1RTI7Sxkys6Cehmigt2aQKeKCd6tiCyw008

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TMVW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	3.00
G	TMVW-t	MT20	5.0	6.0	2.50	1.75
H	TMVW-p	MT20	6.0	7.0	Edge	
J	BMV1-t	MT20	6.0	10.0	Edge	0.50
K	BMVW-t	MT20	6.0	10.0		
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMVW-t	MT20	6.0	10.0		
Q	BMV1-t	MT20	6.0	10.0	5.50	

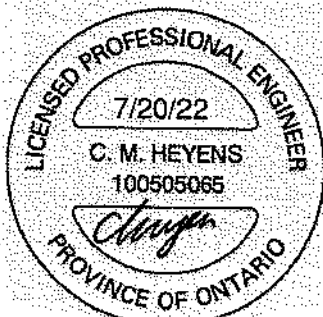
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

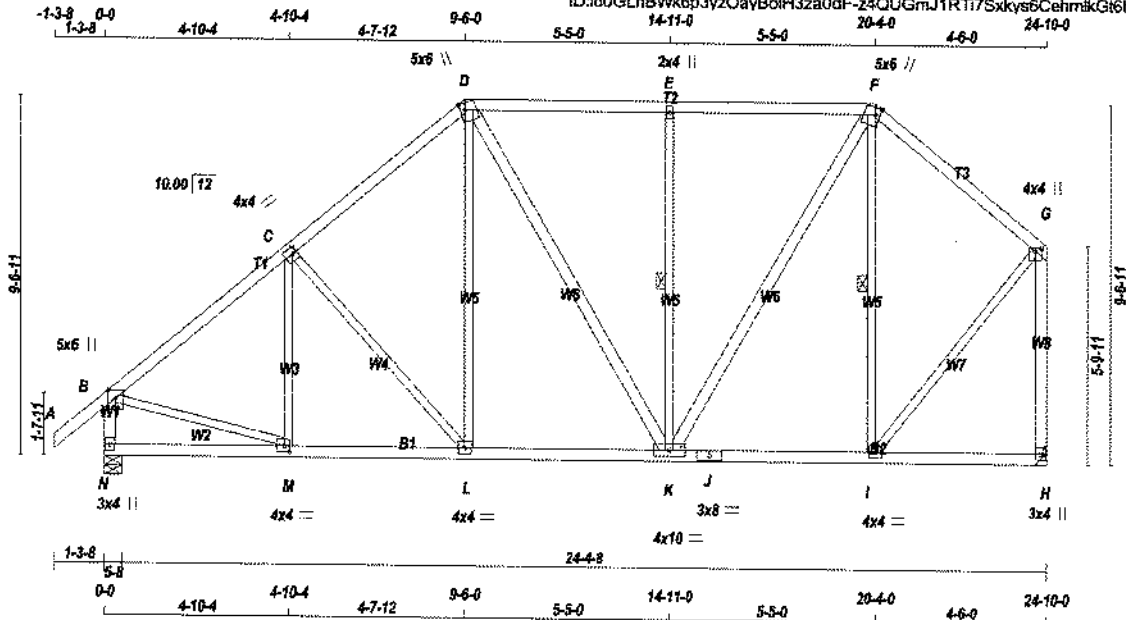


STRUCTURAL COMPONENT ONLY
DWG # TR22070613 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWS NO.
425139	T36	7	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:30:13 2022 Page 1
ID:dd0GLh8Wk6p3yzQayBotH3za0dF-z4QUgmJ1RTi7Skys5CehmkGt6hQQ8Kd6tiCvwD08



Scale = 1:58.8

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - K	2x4	DRY	No.2	SPF	
K - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	Edge	
C	TMVW-i	MT20	4.0	4.0	2.00	1.25
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTVW-i	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-i	MT20	4.0	4.0		
J	BS-i	MT20	3.0	8.0		
K	BMVW-i	MT20	4.0	10.0		
L	BMVW-i	MT20	4.0	4.0		
M	BMVW-i	MT20	4.0	4.0	2.00	1.50
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	1401	0	1401	0	5-8	5-8
H	1283	0	1283	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SORL
	SNOW	LIVE	PERM.LIVE	WIND			
N	992	643 / 0	0 / 0	0 / 0	0 / 0	349 / 0	0 / 0
H	911	576 / 0	0 / 0	0 / 0	0 / 0	333 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K, F-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO					
A-B	0 / 38	-84.8	-84.9 0.12 (1)	10.00	M-C -170 / 14
B-C	-1253 / 0	-84.9	-84.9 0.27 (1)	5.45	C-L -257 / 0
C-D	-1096 / 0	-84.9	-84.9 0.26 (1)	5.74	L-D 0 / 285
D-E	-861 / 0	-84.9	-84.9 0.33 (1)	6.16	D-K 0 / 84
E-F	-881 / 0	-84.9	-84.9 0.33 (1)	6.16	K-E -564 / 0
F-G	-583 / 0	-84.9	-84.9 0.23 (1)	6.25	K-F 0 / 881
N-B	-1363 / 0	0.0	0.0 0.15 (1)	6.96	I-F -518 / 0
H-G	-1251 / 0	0.0	0.0 0.84 (1)	7.19	B-M 0 / 1021
N-M	0 / 0	-18.5	-18.5 0.10 (4)	10.00	I-G 0 / 812
M-L	0 / 986	-18.5	-18.5 0.21 (1)	10.00	
L-K	0 / 819	-18.5	-18.5 0.18 (1)	10.00	
K-J	0 / 517	-18.5	-18.5 0.15 (4)	10.00	
J-I	0 / 517	-18.5	-18.5 0.15 (4)	10.00	
I-H	0 / 0	-18.5	-18.5 0.11 (4)	10.00	

TOTAL WEIGHT = 7 X 136 = 952 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSF: TC=0.84/1.00 (G-H:1), BC=0.21/1.00 (L-M:1)

, WB=0.34/1.00 (E-K:1), SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (M) (INPUT = 0.90)
JSI METAL = 0.49 (B) (INPUT = 1.00)

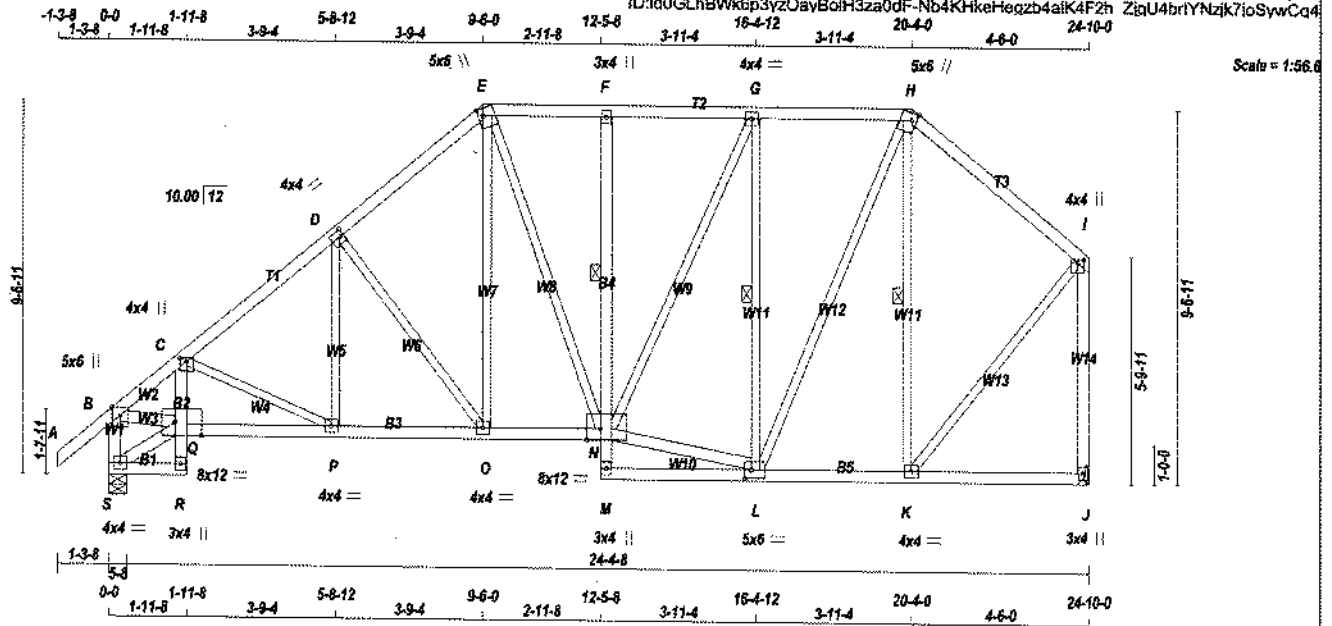


STRUCTURAL COMPONENT ONLY
DWG # TR22070614

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425141	T36S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:43:05 2022 Page 1
ID:1d0GLhBwK6p3yzOayBoIH3za0dF-Nb4KHkeHegzb4aIK4F2h ZlgU4brtYNzk7ioSywCq4



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
H - I	2x4	DRY	No.2	SPF	
S - B	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
S - R	2x4	DRY	No.2	SPF	
R - C	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
M - F	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
N - L	2x4	DRY	No.2	SPF	
S - Q	2x4	DRY	No.2	SPF	
B - Q	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TMVW-i	MT20	4.0	4.0	2.00	1.25
E	TTVW+m	MT20	5.0	6.0	2.25	1.50
F	TMV+p	MT20	3.0	4.0		
G	TMVW-i	MT20	4.0	4.0		
H	TTVW+m	MT20	5.0	6.0	2.25	1.50
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K, O, P						
K	BMVW-i	MT20	4.0	4.0		
L	BMVWW-i	MT20	5.0	6.0	2.50	2.00
M	BMV+p	MT20	3.0	4.0		
N	BMVWW-i	MT20	8.0	12.0	3.50	4.00
Q	BMVW-i	MT20	8.0	12.0	4.00	8.00
R	BMV+p	MT20	3.0	4.0		
S	BMVW-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	1280	0	1280	0
S	1404	0	1404	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
J	908
S	994

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.11 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N, G-L, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CS (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS (LC)
FR-TO						FR-TO			
A-B	0/138	-84.9	-84.9	0.12 (1)	10.00	C-P	-132/0	0.04 (1)	
B-C	-1615/0	-84.9	-84.9	0.11 (1)	5.11	P-D	0/96	0.03 (4)	
C-D	-1458/0	-84.9	-84.9	0.17 (1)	5.26	D-O	-374/0	0.27 (1)	
D-E	-1205/0	-84.9	-84.9	0.16 (1)	5.66	O-E	0/367	0.08 (1)	
E-F	-968/0	-84.9	-84.9	0.09 (1)	6.23	E-N	0/167	0.04 (1)	
F-G	-966/0	-84.9	-84.9	0.14 (1)	6.17	N-L	0/793	0.13 (1)	
G-H	-781/0	-84.9	-84.9	0.14 (1)	6.25	N-G	0/434	0.10 (1)	
H-I	-680/0	-84.9	-84.9	0.23 (1)	6.25	L-G	-763/0	0.46 (1)	
I-J	-1366/0	0.0	0.0	0.15 (1)	6.94	L-H	0/687	0.15 (1)	
J-I	-1247/0	0.0	0.0	0.84 (1)	7.19	K-H	-527/0	0.32 (1)	
S-R	0/134	-18.5	-18.5	0.02 (4)	10.00	K-I	0/808	0.18 (1)	
R-Q	0/19	0.0	0.0	0.05 (1)	10.00	S-Q	-40/0	0.00 (1)	
Q-C	-59/13	0.0	0.0	0.04 (1)	7.81	B-Q	0/1236	0.20 (1)	
Q-P	0/1252	-18.5	-18.5	0.24 (1)	10.00				
P-O	0/1132	-18.5	-18.5	0.21 (1)	10.00				
O-N	0/910	-18.5	-18.5	0.17 (1)	10.00				
M-N	0/33	0.0	0.0	0.03 (1)	10.00				
N-F	-289/0	0.0	0.0	0.03 (1)	6.25				
M-L	0/18	-18.5	-18.5	0.06 (4)	10.00				
L-K	0/515	-18.5	-18.5	0.14 (1)	10.00				
K-J	0/0	-18.5	-18.5	0.09 (4)	10.00				

TOTAL WEIGHT = 153 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.
- (55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

- ALLOWABLE DEFL.(LL) = L/360 (0.83")
- CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
- ALLOWABLE DEFL.(TL) = L/360 (0.83")
- CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.84/1.00 (I-J); BC=0.24/1.00 (P-Q); WB=0.46/1.00 (G-L); SS=0.18/1.00 (G-H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

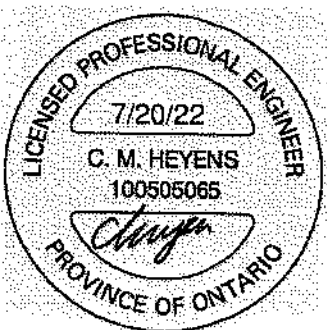
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.58 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2



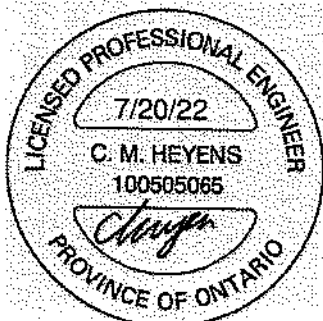
STRUCTURAL COMPONENT ONLY
DWG # TR22070641 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425141	T36S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:43:05 2022 Page 2
ID:Id0GLhBWk6p3yzCevBoIH3za0dF-Nb4KHkeHegzb4aIK4F2h_ZigU4brlYNzjk7icSwwCq4

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070641 PG 2/2



1) C6: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

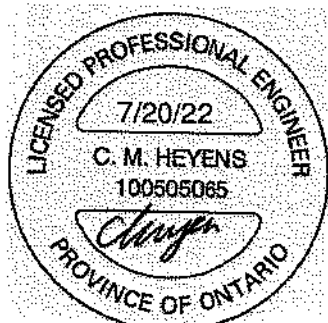
JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.76 (I) (INPUT = 1.00)

JOB NAME 425139	TRUSS NAME T37	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:14 2022 Page 2
ID:ld0GLhEWk6p3yzQay8oIH3za0dF-RG sT6JfCng 45J9QqitEzHy9HJ9g1URHsQEerwD07

CONNECTION REQUIREMENTS

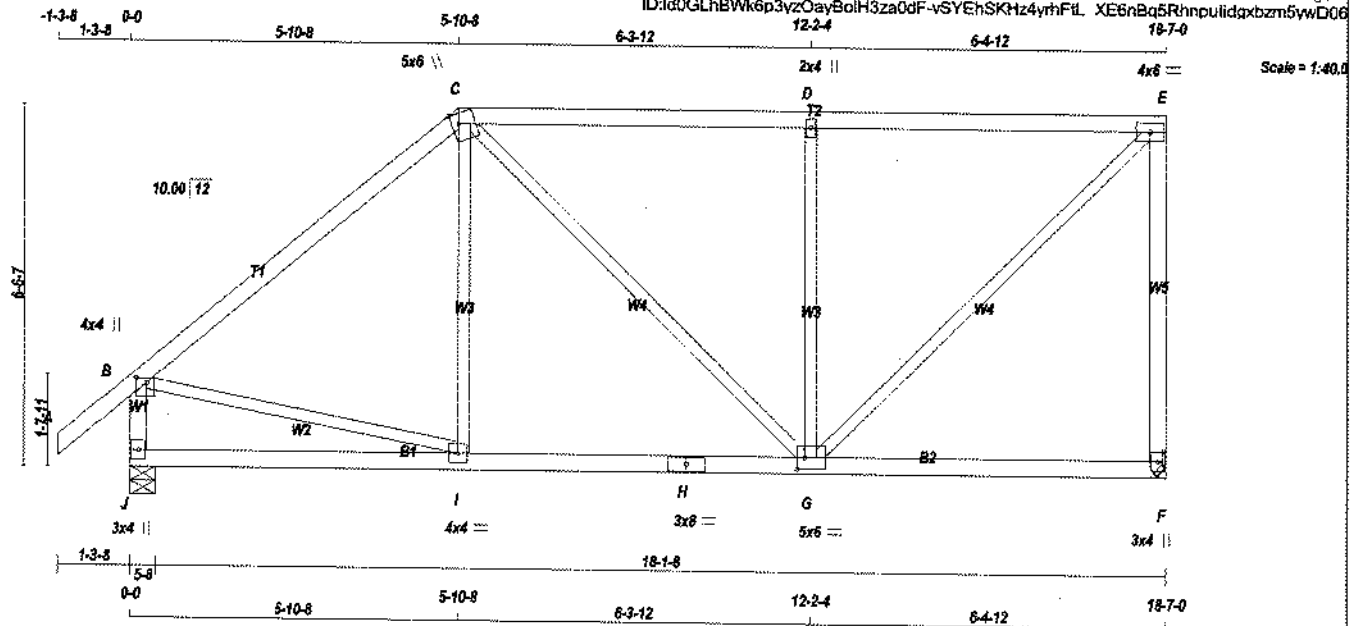
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T38	1	1	TRUSS DESC.		

Temarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:30:15 2022 Page 1
ID:ld0GLhBWk6p3yzOayBotH3za0dF-vSYehSKHz4yvhFtl XE6nBq5Rhnplidgxbzm5yWd06



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TTWV+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-i	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-i	MT20	5.0	6.0	2.50	1.50
H	BS-i	MT20	3.0	6.0		
I	BMVWW-i	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	960	0	960	0
J	1078	0	1078	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	682	433 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0
J	763	497 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	I-C	-15 / 88	0.03 (4)		
B-C	-824 / 0	-84.9	-84.9	0.57 (1)	5.82	C-G	0 / 128	0.03 (1)		
C-D	-722 / 0	-84.9	-84.9	0.61 (1)	5.83	G-D	-668 / 0	0.47 (1)		
D-E	-723 / 0	-84.9	-84.9	0.61 (1)	5.82	G-E	0 / 1010	0.23 (1)		
F-E	-912 / 0	0.0	0.0	0.80 (1)	7.81	B-I	0 / 648	0.15 (1)		
J-B	-1033 / 0	0.0	0.0	0.11 (1)	7.72					
J-I	0 / 0	-18.5	-18.5	0.15 (4)	10.00					
I-H	0 / 531	-18.5	-18.5	0.23 (4)	10.00					
H-G	0 / 531	-18.5	-18.5	0.23 (4)	10.00					
G-F	0 / 0	-18.5	-18.5	0.17 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.80/1.00 (E-F:1), BC=0.23/1.00 (G-E:4), WB=0.47/1.00 (D-G:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

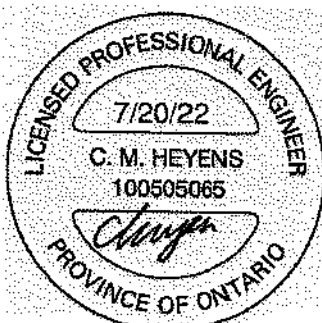
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.25 (G) (INPUT = 1.00)

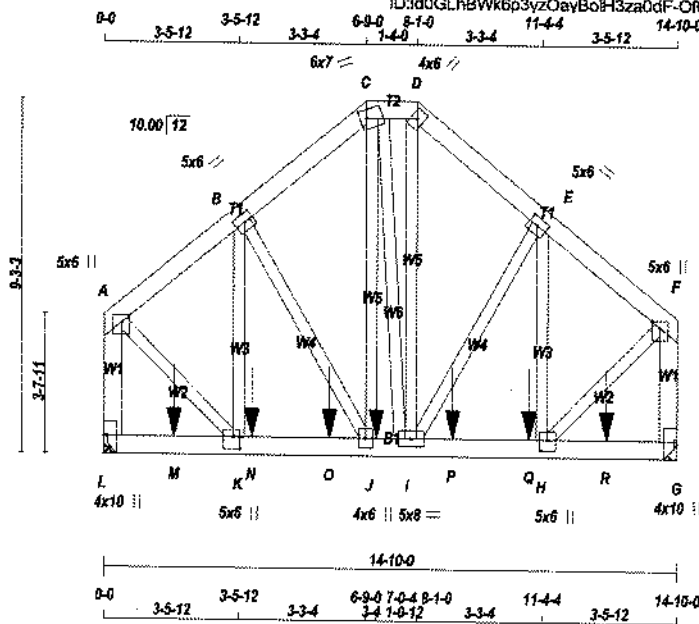


STRUCTURAL COMPONENT ONLY
DWG # TR22070616

JOB NAME 425139	TRUSS NAME T39	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
--------------------	-------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:16 2022 Page 1



Scale = 1:57.8

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY	No.2	SPF		
C - D	2x6 DRY	No.2	SPF		
D - F	2x6 DRY	No.2	SPF		
L - A	2x6 DRY	No.2	SPF		
G - F	2x6 DRY	No.2	SPF		
L - G	2x6 DRY	No.2	SPF		

ALL WEBS 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2 12	TOP	
C-D 2 12	TOP	
D-F 2 12	TOP	
L-A 2 12	TOP	
G-F 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-G 2 12	SIDE(183.1)	
WEBS : (0.122"x3") SPIRAL NAILS		
J-C 1 6	SIDE(73.5)	
I-C 1 6	SIDE(73.4)	
B-K 1 6	SIDE(146.9)	
E-H 1 6	SIDE(146.9)	
2x4 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	VERT	GROSS REACTION	DOWN		
JT	5238	0	5238	0	BRG	BRG
L	5150	0	5150	0	MECHANICAL	MECHANICAL
G	5150	0	5150	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. G. MINIMUM BEARING LENGTH AT JOINT L = 4-0, JOINT G = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERMILIVE WIND DEAD SOIL
L	3712	2390 / 0 0 / 0 0 / 0 0 / 0 1323 / 0 0 / 0
G	3550	2349 / 0 0 / 0 0 / 0 0 / 0 1301 / 0 0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		WEBS	
	MAX. FACTORED	FORCE (LBS)	VERT. LOAD LC1	MAX. (PLF)	MAX. UNBRAC	MAX. FACTORED
A-B	-3522 / 0	-84.9	-84.9	0.06 (1)	5.91	K-B 0 / 268 0.02 (1)
B-C	-3248 / 0	-84.9	-84.9	0.06 (1)	8.10	B-J -455 / 0 0.14 (1)
C-D	-2520 / 0	-84.9	-84.9	0.01 (1)	6.25	J-C 0 / 1838 0.16 (1)
D-E	-3257 / 0	-84.9	-84.9	0.05 (1)	6.10	C-I 0 / 63 0.01 (1)
E-F	-3422 / 0	-84.9	-84.9	0.06 (1)	5.98	I-D 0 / 1907 0.17 (1)
L-A	-4823 / 0	0.0	0.0	0.32 (1)	6.60	I-E -285 / 0 0.09 (1)
G-F	-4680 / 0	0.0	0.0	0.31 (1)	6.68	H-E 0 / 69 0.01 (4)
L-M	0 / 0	-18.5	-18.5	0.29 (1)	10.00	A-K 0 / 3631 0.32 (1)
M-K	0 / 0	-18.5	-18.5	0.29 (1)	10.00	H-F 0 / 3528 0.31 (1)
K-N	0 / 2706	-18.5	-18.5	0.47 (1)	10.00	
N-O	0 / 2706	-18.5	-18.5	0.47 (1)	10.00	
O-J	0 / 2706	-18.5	-18.5	0.47 (1)	10.00	
J-I	0 / 2512	-18.5	-18.5	0.44 (1)	10.00	
I-P	0 / 2629	-18.5	-18.5	0.45 (1)	10.00	
P-Q	0 / 2629	-18.5	-18.5	0.46 (1)	10.00	
Q-H	0 / 2629	-18.5	-18.5	0.46 (1)	10.00	
H-R	0 / 0	-18.5	-18.5	0.30 (1)	10.00	
R-G	0 / 0	-18.5	-18.5	0.30 (1)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	7-0-4	-896	-896	---	BACK	VERT	TOTAL	---	C1
M	1-9-12	-896	-896	---	BACK	VERT	TOTAL	---	C1
N	3-9-12	-896	-896	---	BACK	VERT	TOTAL	---	C1
O	5-9-12	-896	-896	---	BACK	VERT	TOTAL	---	C1
P	9-0-4	-896	-896	---	BACK	VERT	TOTAL	---	C1
Q	11-0-4	-896	-896	---	BACK	VERT	TOTAL	---	C1
R	13-0-4	-896	-896	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.49")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.49")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.32/1.00 (A-L-1), BC=0.47/1.00 (J-K-1),
WB=0.32/1.00 (A-K-1), SS=0.58/1.00 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

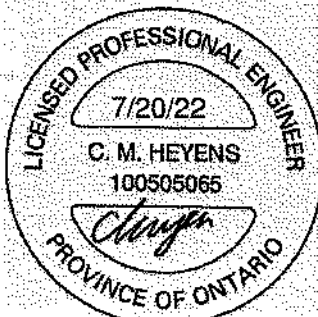
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.49 (K) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070617 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T39	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTak Industries, Inc. Wed Jul 20 10:30:16 2022 Page 2
ID:Id0GLh9Wk6p3yzOay8oiH3za0dF-Of6duoLvk04lJPSXXFLJONOI63NdnOmubLXIXywD05

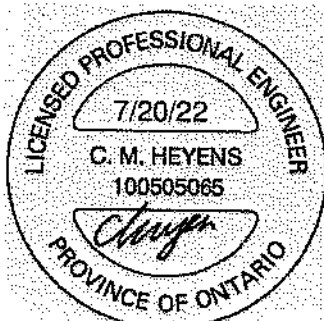
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	8.0	2.00	2.25
B	TMWW-1	MT20	5.0	6.0	2.80	2.75
C	TTWW-m	MT20	6.0	7.0	2.75	2.00
D	TTW+h	MT20	4.0	6.0		
E	TMWW-1	MT20	5.0	8.0	2.50	2.75
F	TMVW+p	MT20	5.0	6.0	2.00	2.25
G	BMV1+1	MT20	4.0	10.0	Edge	1.50
H	BMVWW+1	MT20	5.0	6.0	3.25	2.50
I	BMVWW+1	MT20	5.0	8.0		
J	BMVWW+1	MT20	4.0	6.0		
K	BMVWW+1	MT20	5.0	8.0	3.75	2.00
L	BMV1+1	MT20	4.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070617 PG 2/2



JSI GRIP= 0.53 (A) (INPUT = 0.90)
JSI METAL= 0.15 (A) (INPUT = 1.00)

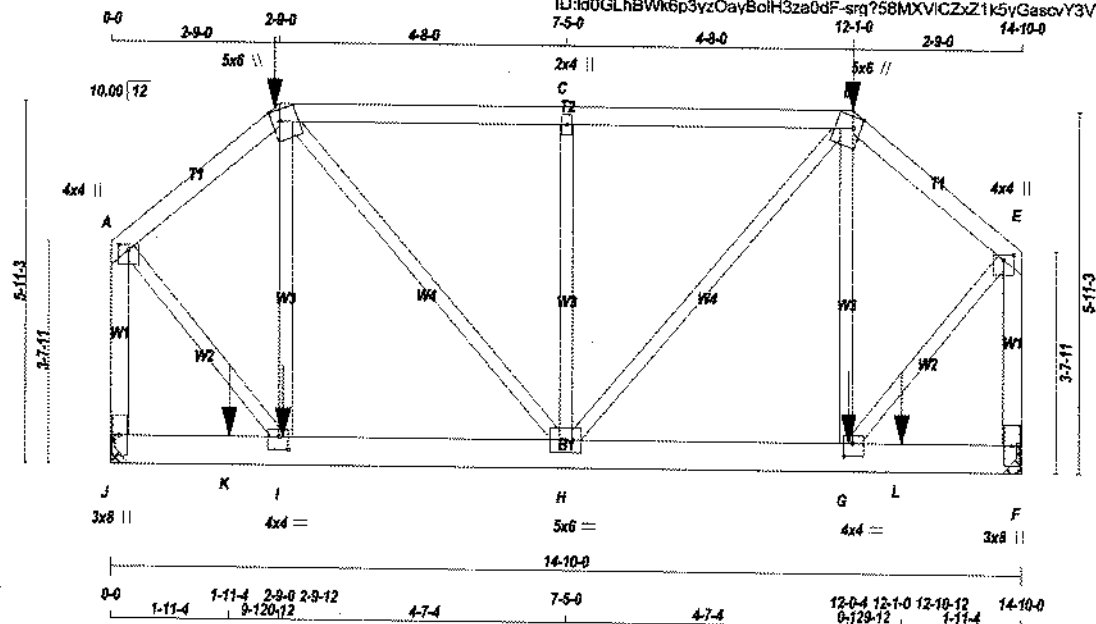


JOB NAME 425139	TRUSS NAME T41	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:17 2022 Page 1

ID:ld0GLhBwK6p3yzOayBoIH3za0dF-srq758MXVICZxZ1k5yGascvY3VVeMGXw7F44rzywD04



Scale = 1/32"

LUMBER	N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
J - A	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
J - F	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTVW+m	MT20	5.0	6.0	2.25	1.50
C	TMVW+w	MT20	2.0	4.0		
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	3.0		
G	BMVW-t	MT20	4.0	4.0	2.50	1.75
H	BMVW-w	MT20	5.0	6.0		
I	BMVW-t	MT20	4.0	4.0	2.50	1.75
J	BMV1+p	MT20	3.0	3.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
J	VERT 937	DOWN 0	0	0
F	VERT 937	DOWN 0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT JOINT J = 3-B, JOINT F = 3-B.

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
J	COMBINED	SNOW 427/0	0/0	0/0
F	COMBINED	SNOW 427/0	0/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	-533/0	-64.9 -64.9 0.13 (1)	I-B	-359/0
B-C	-633/0	-64.9 -64.9 0.34 (1)	B-H	0/360
C-D	-633/0	-64.9 -64.9 0.34 (1)	H-C	-484/0
D-E	-533/0	-64.9 -64.9 0.13 (1)	H-D	0/360
J-A	-920/0	0.0 0.0 0.20 (1)	G-D	-359/0
F-E	-920/0	0.0 0.0 0.20 (1)	A-I	0/615
J-K	0/0	-18.5 -18.5 0.03 (4)	G-E	0/615
K-I	0/0	-18.5 -18.5 0.03 (4)		
I-H	0/401	-18.5 -18.5 0.08 (1)		
H-G	0/401	-18.5 -18.5 0.08 (1)		
G-L	0/0	-18.5 -18.5 0.03 (4)		
L-F	0/0	-18.5 -18.5 0.03 (4)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-9-0	-108	-108		FRONT	VERT	TOTAL		C1
D	12-1-0	-108	-108		FRONT	VERT	TOTAL		C1
G	12-0-4	-6	-6		FRONT	VERT	TOTAL		C1
I	2-9-12	-6	-6		FRONT	VERT	TOTAL		C1
K	1-11-4	-6	-6		FRONT	VERT	TOTAL		C1
L	12-10-12	-6	-6		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 63 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.34/1.00 (B-C-1), BC=0.08/1.00 (H-I-1),
WB=0.25/1.00 (C-H-1), SS=0.21/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

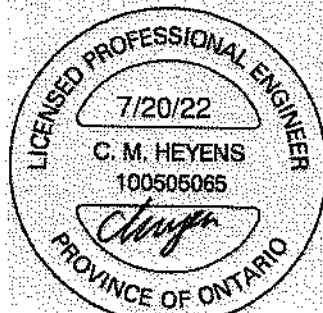
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	(PSI)	(PL)	(PL)
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL= 0.18 (E) (INPUT = 1.00)

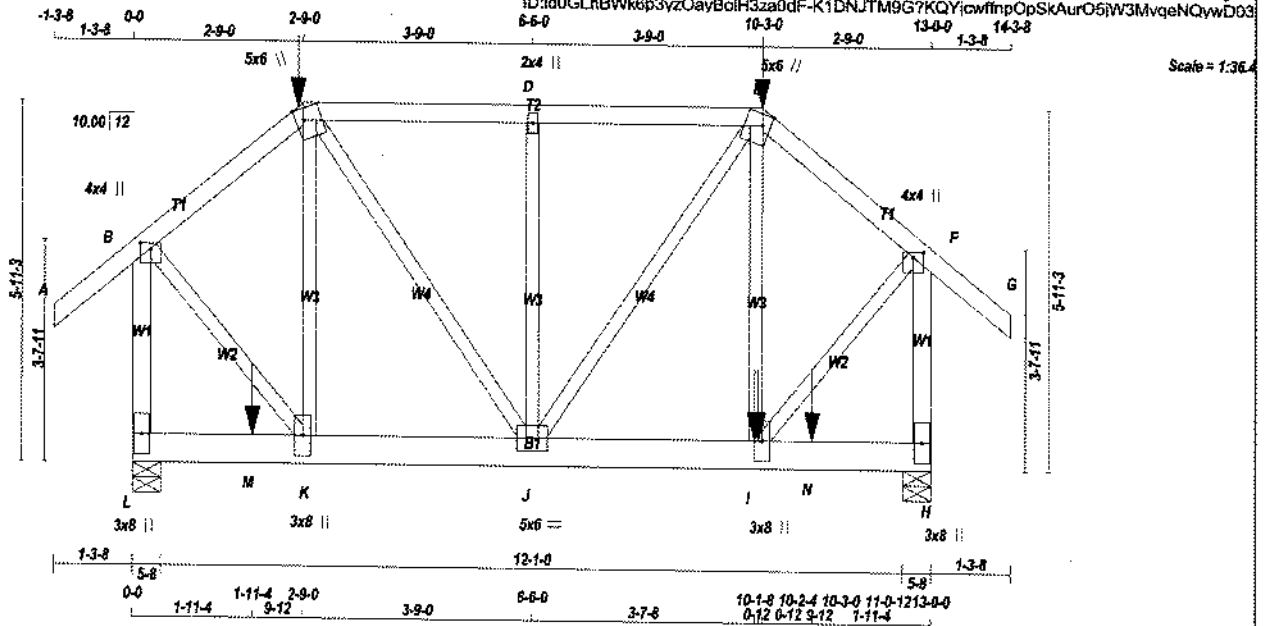


STRUCTURAL COMPONENT ONLY
DWG # TR22070619

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T42	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:18 2022 Page 1
ID:Id0GLhBWk6p3yzOayBotH3za0dF-K1DNJTM9G7KQY(cwffnpOpSkAurO5)W3MvqeNQyWDO3



Scale = 1/32

TOTAL WEIGHT = 2 X 81 = 161 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - H	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE(61.0)
C - E 1	12	SIDE(61.0)
E - G 1	12	SIDE(61.0)
L - B 1	12	TOP
H - F 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L - H 2	12	SIDE(163.1)
WEBS : (0.122"x3") SPIRAL NAILS		
L - E 1	6	SIDE(321.0)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	UP	BRG	IN-SX	BRG	IN-SX
L	1379	0	1379	0	0	5-8	5-8	5-8	5-8
H	2466	0	2466	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	973	647 / 0	0 / 0	0 / 0	0 / 0	326 / 0	0 / 0
H	1742	1154 / 0	0 / 0	0 / 0	0 / 0	588 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT.	MAX. (PLF)	MAX. (LC)	MEMB.	FORCE (LBS)	MAX. (LC)
FR-TO					FR-TO		
A - B	0 / 38	-84.9	-84.9 0.06 (1)	10.00	K - C	-567 / 0	0.15 (1)
B - C	-747 / 0	-84.9	-84.9 0.06 (1)	6.25	C - J	0 / 796	0.10 (1)
C - D	-998 / 0	-84.9	-84.9 0.11 (1)	6.25	J - D	-379 / 0	0.10 (1)
D - E	-998 / 0	-84.9	-84.9 0.11 (1)	6.25	J - E	-204 / 0	0.08 (1)
E - F	-1437 / 0	-84.9	-84.9 0.07 (1)	6.25	E - E	0 / 883	0.09 (1)
F - G	0 / 38	-84.9	-84.9 0.06 (1)	10.00	B - K	0 / 852	0.11 (1)
L - B	-1360 / 0	0.0	0.0 0.15 (1)	7.81	L - F	0 / 1658	0.21 (1)
H - F	-2401 / 0	0.0	0.0 0.26 (1)	7.26			
L - M	0 / 0	-18.5	-18.5 0.01 (4)	10.00			
M - K	0 / 0	-18.5	-18.5 0.01 (4)	10.00			
K - J	0 / 581	-18.5	-18.5 0.05 (1)	10.00			
J - I	0 / 1113	-18.5	-18.5 0.11 (1)	10.00			
I - N	0 / 0	-18.5	-18.5 0.04 (1)	10.00			
N - H	0 / 0	-18.5	-18.5 0.04 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	2-0-0	-108	-108	---	BACK	VERT	TOTAL	---
E	10-3-0	-108	-108	---	BACK	VERT	TOTAL	---
I	10-1-8	-1365	-1365	---	FRONT	VERT	TOTAL	---
L	10-2-4	-6	-6	---	BACK	VERT	TOTAL	---
M	1-11-4	-6	-6	---	BACK	VERT	TOTAL	---
N	11-0-12	-6	-6	---	BACK	VERT	TOTAL	---

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(56% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CS: TC=0.26/1.00 (F-H:1), BC=0.11/1.00 (J-K:1),
WB=0.21/1.00 (F-1:1), SS=0.08/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 L.S.BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL = 0.33 (K) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070620

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T42	1	2	TRUSS DESC.		

Temarack Roof Truss, Burlington

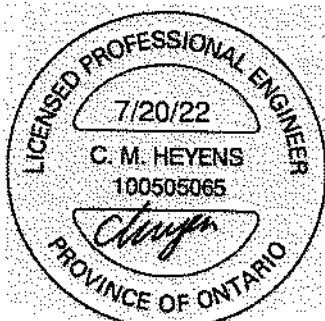
Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:30:18 2022 Page 2
ID:ld0GLhBWk6p3yzOayBcIH3za0dF-K1DNJTM9G7KQYicwfinpOpSkAurOSjW3MvgeNQyWD03

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTVW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTVW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	8.0		
I	BMVW+t	MT20	3.0	8.0		
J	BMVW+t	MT20	5.0	6.0		
K	BMVW+t	MT20	3.0	8.0		
L	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

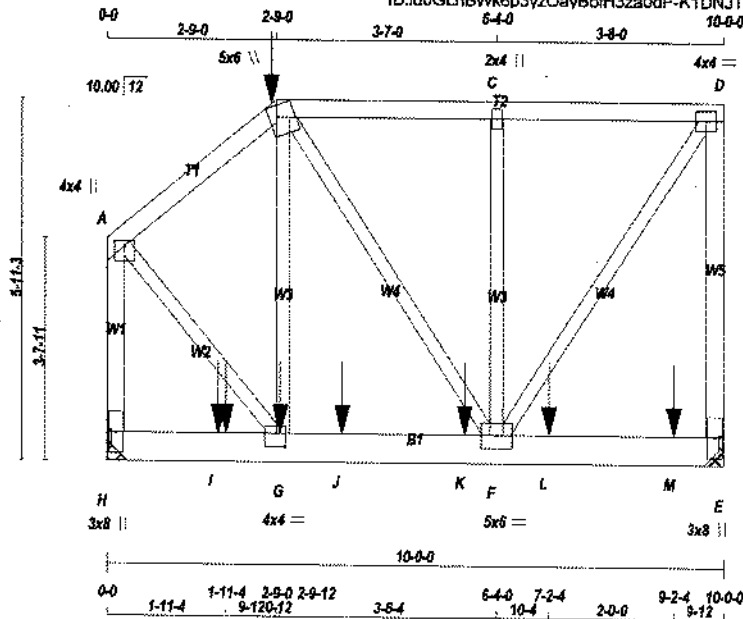
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070620 PG 2/2

JOB NAME 425139	TRUSS NAME T43	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:30:18 2022 Page 1
ID: d0GLhBwK6p3yzOayBoH3za0dF-K1DNJTM9G7KQYJowfnpOpShuuqW5JY3MvqeNQYwD0S



TOTAL WEIGHT = 2 X 62 = 123 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B	12	SIDE(81.0)
B-D	12	SIDE(81.0)
D-E	12	TOP
H-A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H-E	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	DOWN	UP
E	1951	0	1951	0	0
H	1747	0	1747	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E, H. MINIMUM BEARING LENGTH AT JOINT E = 4-0, JOINT H = 4-0.

UNFACTORED REACTIONS		1ST LOASE	MAX MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
E	1380	903 / 0	0 / 0
H	1235	809 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	WEBS		MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						FR-TO			
A-B	-997 / 0	-84.9	-84.9	0.06 (1)	6.25	G-B	0 / 58	0.01 (4)	
B-C	-906 / 0	-84.9	-84.9	0.09 (1)	6.25	B-F	0 / 263	0.03 (1)	
C-D	-906 / 0	-84.9	-84.9	0.09 (1)	6.25	F-C	-365 / 0	0.10 (1)	
E-D	-1498 / 0	0.0	0.0	0.47 (1)	7.81	F-D	0 / 1646	0.20 (1)	
H-A	-1620 / 0	0.0	0.0	0.17 (1)	7.81	A-G	0 / 1151	0.14 (1)	
H-I	0 / 0	-18.5	-18.5	0.09 (1)	10.00				
I-G	0 / 0	-18.5	-18.5	0.09 (1)	10.00				
G-J	0 / 763	-18.5	-18.5	0.17 (1)	10.00				
J-K	0 / 763	-18.5	-18.5	0.17 (1)	10.00				
K-F	0 / 763	-18.5	-18.5	0.17 (1)	10.00				
F-L	0 / 0	-18.5	-18.5	0.13 (1)	10.00				
L-M	0 / 0	-18.5	-18.5	0.13 (1)	10.00				
M-E	0 / 0	-18.5	-18.5	0.13 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT									
B	2-9-0	-108	-108		FRONT	VERT	TOTAL		C1
G	2-9-12	-6	-6		FRONT	VERT	TOTAL		C1
I	1-9-12	-352	-352		BACK	VERT	TOTAL		C1
J	1-11-4	-6	-6		FRONT	VERT	TOTAL		C1
K	3-9-12	-352	-352		BACK	VERT	TOTAL		C1
L	5-9-12	-352	-352		BACK	VERT	TOTAL		C1
M	7-2-4	-352	-352		BACK	VERT	TOTAL		C1
	9-2-4	-353	-353		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.33")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.33")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSF: TC=0.47/1.00 (D-E:1), BC=0.17/1.00 (F-G:1), WB=0.20/1.00 (D-F:1), SS=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 0.23 (D) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070621 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T43	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

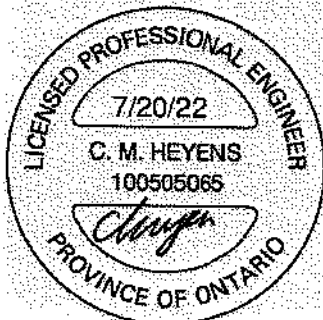
Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:30:18 2022 Page 2
ID:Id0GLhBWk6p3yzQayBoIH3za0dF-oEnfWpNo1JSHAsB6DNJ2x17seIAqAoDaZZBvsywd02

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMVW+w	MT20	2.0	4.0		
D	TMVW-l	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	8.0		
F	BMWWW-l	MT20	5.0	6.0		
G	BMWW-l	MT20	4.0	4.0	2.50	1.75
H	BMV1+p	MT20	3.0	8.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

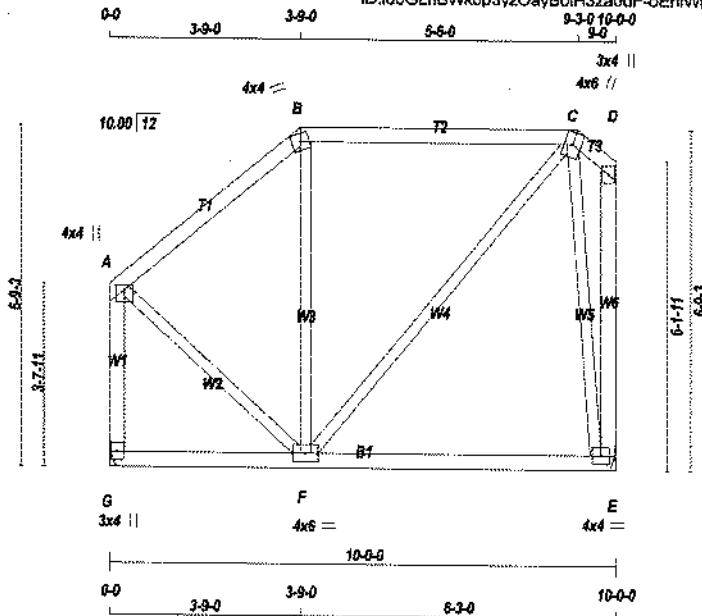


STRUCTURAL COMPONENT ONLY
DWG # TR22070621 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T44	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Jul 20 10:30:19 2022 Page 1
ID:IG0GLhBWK6p3yzOayBoIH3za0dF-oEniWpNo1JSHASB6DNJ2x1?AIvq8fDaZZBvsyW0D2



Scale = 1:44.0

TOTAL WEIGHT = 2 X 56 = 111 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - A	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-m	MT20	4.0	4.0		
C	TTW+m	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVW-1	MT20	4.0	4.0		
F	BMVW-1	MT20	4.0	6.0		
G	BMV-1	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
G	517	0	517	0
E	517	0	517	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G, E. MINIMUM BEARING LENGTH AT JOINT G = 1-8, JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	367	233 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
E	367	233 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. FORCE (LBS)
FR-TO		FROM	TO	FR-TO
A-B	-260 / 0	-64.9	-64.9 0.21 (1)	6.25
B-C	-198 / 0	-64.9	-64.9 0.43 (1)	6.25
C-D	0 / 0	-64.9	-64.9 0.01 (1)	10.00
G-A	-497 / 0	0.0	0.0 0.11 (1)	7.81
E-D	-34 / 0	0.0	0.0 0.03 (1)	7.81
G-F	0 / 0	-18.5	-18.5 0.15 (4)	10.00
F-E	0 / 54	-18.5	-18.5 0.16 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 38.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CS1: TC=0.43/1.00 (B-C-1), BC=0.16/1.00 (E-F-4)
WB=0.34/1.00 (C-E-1), SS=0.18/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

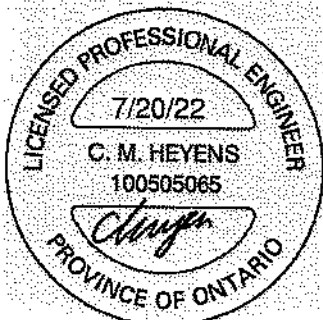
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	850	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (A) (INPUT = 0.90)
JSI METAL = 0.10 (A) (INPUT = 1.00)

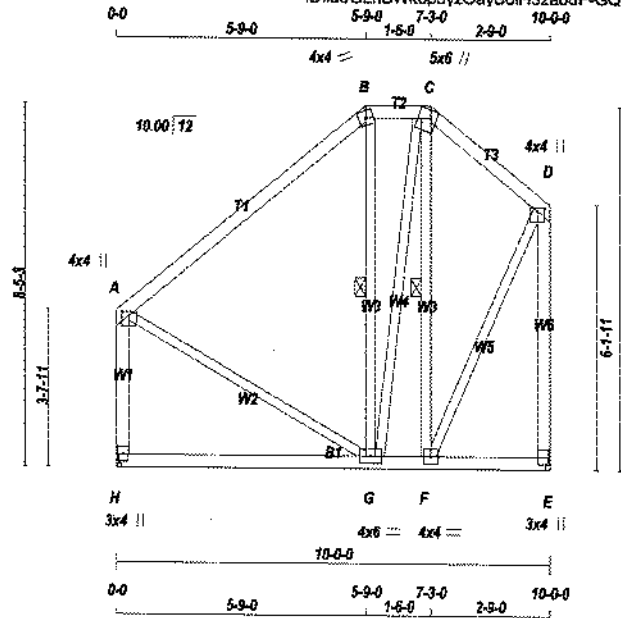


STRUCTURAL COMPONENT ONLY
DWG # TR22070622

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T45	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:20 2022 Page 1
ID: d0GLhBWk6p3yzOayBolt43za0dF-GQL7k9OQodaBo0mIm4qHUEX2aiWEZITMpDJkRlywD01



Scale = 1:51.4

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY, SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-m	MT20	4.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWW-1	MT20	4.0	4.0		
G	BMWW-1	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	517	0	517	0	0	MECHANICAL
E	517	0	517	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOL
H	367	233 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
E	367	233 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-G, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED MEMB. CSI (LC)
FR-TO		FROM TO		FR-TO	
A-B	-212 / 0	-84.9 -84.9 0.36 (1)	6.25	G-B	-172 / 0
B-C	-160 / 0	-84.9 -84.9 0.02 (1)	6.25	G-C	0 / 183
C-D	-173 / 0	-84.9 -84.9 0.08 (1)	6.25	F-C	-250 / 0
H-A	-471 / 0	0.0 0.0 0.10 (1)	7.81	A-G	0 / 186
E-D	-497 / 0	0.0 0.0 0.39 (1)	7.81	F-D	0 / 300
H-G	0 / 0	-18.5 -18.5 0.14 (4)	10.00		
G-F	0 / 129	-18.5 -18.5 0.15 (4)	10.00		
F-E	0 / 0	-18.5 -18.5 0.03 (4)	10.00		

TOTAL WEIGHT = 2 X 66 = 132 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.39/1.00 (D-E:1), SC=0.15/1.00 (F-G:4)
WB=0.11/1.00 (C-F:1), SS=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

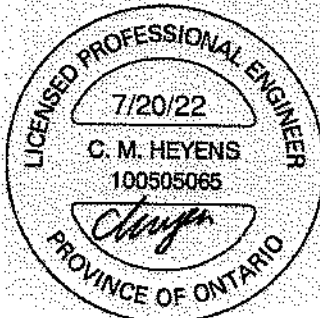
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.35 (F) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



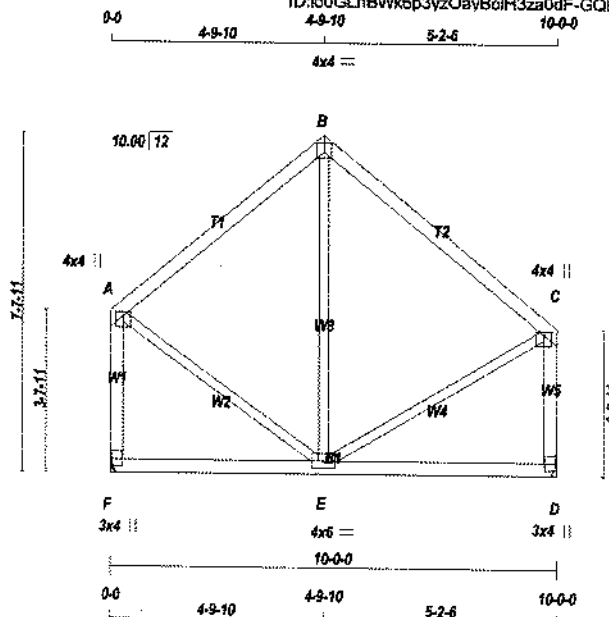
STRUCTURAL COMPONENT ONLY
DWG # TR22070623

JOB NAME 425139	TRUSS NAME T46	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
--------------------	-------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:20 2022 Page 1

ID:ld0GLhBwK6p3yzOay8c1M3za0dF-GQL7k9OQoda8c0mim4qHUEX48iWWZfKMPDJkRtywD01



Scale = 1/4" = 1'-0"

LUMBER	SIZE	DRY	No.2	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
F	517	0	0	MECHANICAL
D	517	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-3.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	367	233 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
D	367	233 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	-240 / 0	-84.9 -84.9 0.25 (1)	E-B	-117 / 37
B-C	-240 / 0	-84.9 -84.9 0.29 (1)	A-E	0 / 224
F-A	-484 / 0	0.0 0.0 0.11 (1)	E-C	0 / 212
D-C	-479 / 0	0.0 0.0 0.09 (1)		
F-E	0 / 0	-18.5 -18.5 0.13 (4)		
E-D	0 / 0	-18.5 -18.5 0.13 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
BOT CH. LL = 6.0 PSF
DL = 0.0 PSF
TOTAL LOAD = 39.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.02")

CSI: TC=0.29/1.00 (B-C:1), BC=0.13/1.00 (E-F:4)
WB=0.12/1.00 (B-E:1), SI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

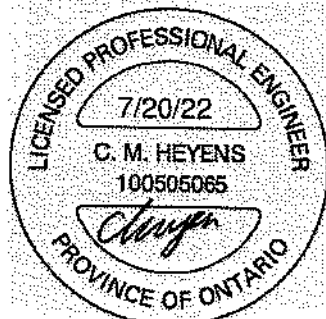
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PLI)	SECTION (PLI)
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 789

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (C) (INPUT = 0.90)
JSI METAL = 0.10 (A) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070624

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T47	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:30:21 2022 Page 2

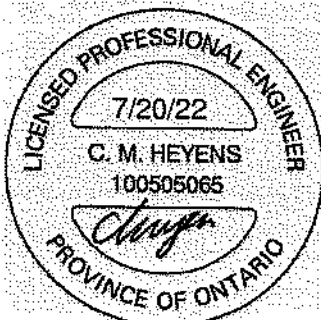
ID:d0GLhBWk6p3yzQavBoiH3za0dF-kcvWxVP2Zwi7QALVKoLW0S4ib6mTlyvW2t2l_kywD00

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMVW-t	MT20	4.0	6.0	2.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	4.0	10.0	5.00	1.50
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070625 PG 2/2

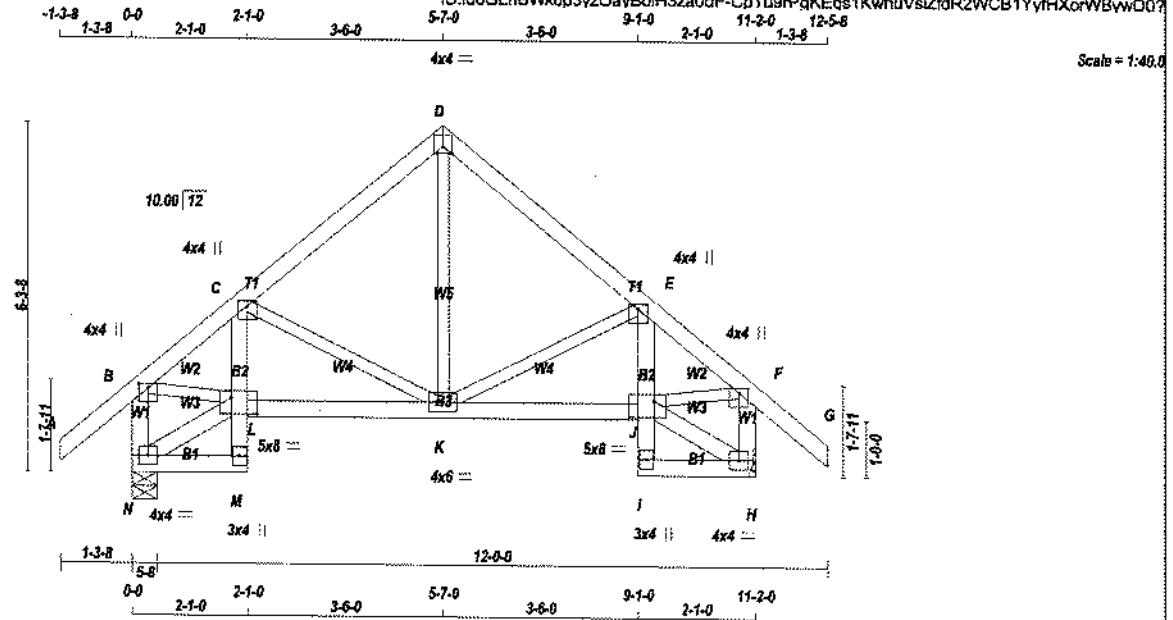
STRUCTURAL COMPONENT ONLY
DWG # TR22070626

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	T48S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:22 2022 Page 1

ID:k0GLh8Wk6p3yzOayBoH3za0dF-CpTu3rPqKEcs1KwhuVsIZfR2WCB1YyfHXorWByw00?



TOTAL WEIGHT = 2 X 59 = 118 lb
[M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
N - M	2x4	DRY	No.2	SPF	
M - C	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
I - E	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
N - L	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B, C, E, F							
B	TMVW+p	MT20	4.0	4.0	1.00	2.00	
D	TTW-p	MT20	4.0	4.0	1.50	2.00	
H	BMVW1-t	MT20	4.0	4.0			
I	BMV+p	MT20	3.0	4.0			
J	BMVWV-I	MT20	5.0	8.0	3.50	5.50	
K	BMVWV-I	MT20	4.0	6.0			
L	BMVWV-I	MT20	5.0	8.0	3.50	5.50	
M	BMV+p	MT20	3.0	4.0			
N	BMVW1-t	MT20	4.0	4.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
N	695	0	695	0
H	695	0	695	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	401	325 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0
H	491	325 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM TO		FR-TO				
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	K-D	0 / 258	0.06 (1)
B-C	-651 / 0	-84.9	-84.9	0.09 (1)	6.25	R-E	-221 / 0	0.08 (1)
C-D	-423 / 0	-84.9	-84.9	0.14 (1)	6.25	C-K	-221 / 0	0.06 (1)
D-E	-423 / 0	-84.9	-84.9	0.14 (1)	6.25	N-L	-18 / 0	0.00 (1)
E-F	-651 / 0	-84.9	-84.9	0.09 (1)	6.25	B-L	0 / 503	0.11 (1)
F-G	0 / 38	-84.9	-84.9	0.12 (1)	10.00	J-H	-18 / 0	0.00 (1)
N-B	-668 / 0	0.0	0.0	0.07 (1)	7.81	J-F	0 / 503	0.11 (1)
H-F	-668 / 0	0.0	0.0	0.07 (1)	7.81			
N-M	0 / 15	-18.5	-18.5	0.02 (4)	10.00			
M-L	0 / 20	0.0	0.0	0.03 (1)	10.00			
L-C	0 / 33	0.0	0.0	0.02 (4)	10.00			
L-K	0 / 509	-18.5	-18.5	0.12 (1)	10.00			
K-J	0 / 509	-18.5	-18.5	0.12 (1)	10.00			
I-J	0 / 20	0.0	0.0	0.03 (1)	10.00			
J-E	0 / 33	0.0	0.0	0.02 (4)	10.00			
I-H	0 / 15	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.37")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.37")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CS1: TC=0.14/1.00 (D-E), BC=0.12/1.00 (J-K), WB=0.11/1.00 (F-J), SS=0.11/1.00 (C-D)

DOL LUMBER=1.00 NAIL=1.00 L.S. BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

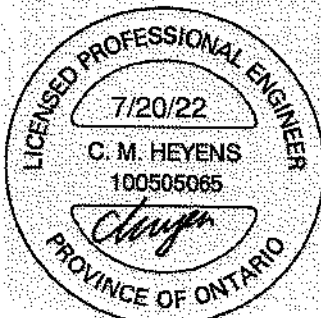
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

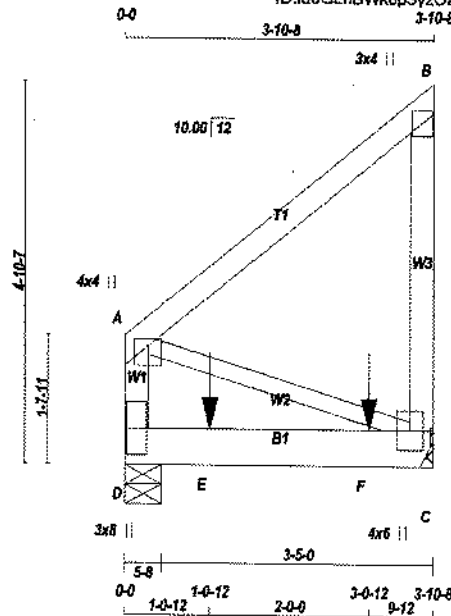
JSI GRIP = 0.81 (D) (INPUT = 0.90)
JSI METAL = 0.20 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070627

JOB NAME 425139	TRUSS NAME T49	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:22 2022 Page 1	

ID:ld0GLhBWk6p3yz0ayBoIH3za0dF-CpTu9rPgKEqstKwhuVslZIdSKW911Z#Hx0rWBWd07



Scale = 1:28.1

LUMBER				
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS		2x3	DRY	No.2
DRY: SEASONED LUMBER.		SPF		

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B	12	TOP
B-C	12	TOP
D-A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TMV+p	MT20	3.0	4.0	
C	BMVW1+p	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP/LIFT		
C	921	0	921	0	MECHANICAL	5-8
D	833	0	833	0	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	850	436 / 0	0 / 0	0 / 0	0 / 0	0 / 0	214 / 0	0 / 0
D	588	394 / 0	0 / 0	0 / 0	0 / 0	0 / 0	194 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO				
A-B	0 / 0	-84.9	0 / 0	-84.9
C-B	-164 / 0	0.0	0 / 0	0.0
D-A	-164 / 0	0.0	0 / 0	0.0
D-E	0 / 0	-18.5	0 / 0	-18.5
E-F	0 / 0	-18.5	0 / 0	-18.5
F-C	0 / 0	-18.5	0 / 0	-18.5

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-6-12	-476	-476	---	FRONT	VERT	TOTAL	---	C1
F	3-0-12	-477	-477	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(56 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.12/1.00 (A-B:1), BC=0.26/1.00 (C-D:1)

WB=0.00/1.00 (A-C:1), SI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

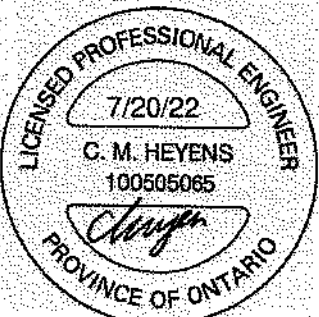
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (A) (INPUT = 0.96)
JSI METAL= 0.03 (B) (INPUT = 1.00)



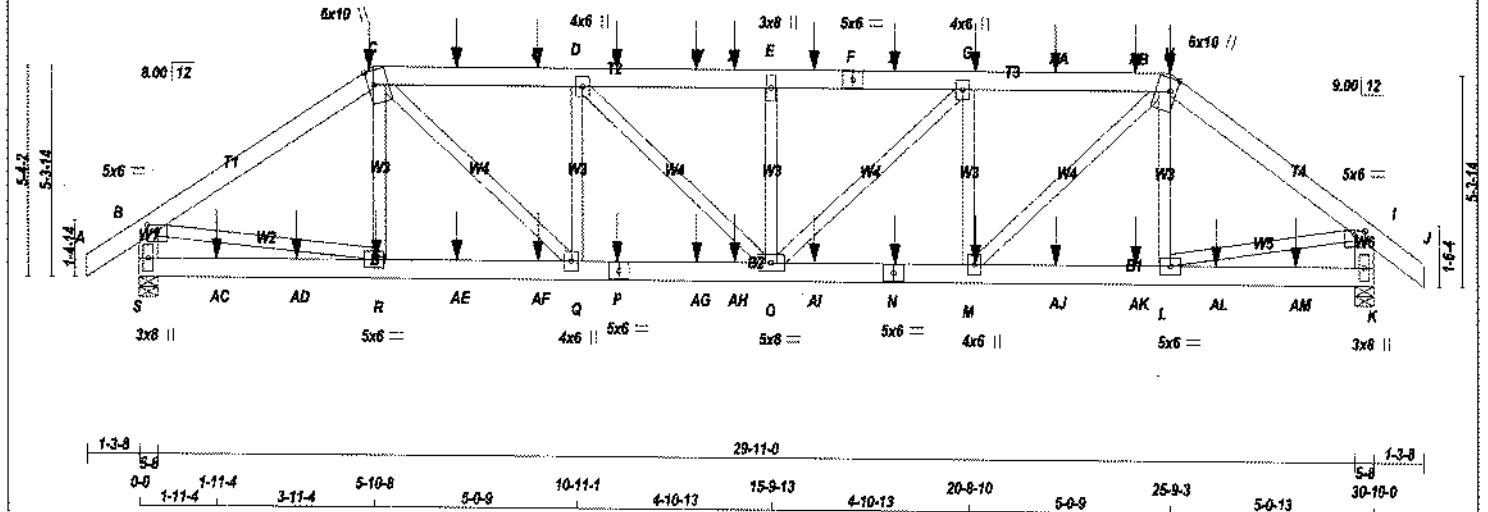
STRUCTURAL COMPONENT ONLY
DWG # TR22070628 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.														
425139	T49	1	2	TRUSS DESC.																
Tamarack Roof Truss, Burlington				Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:23 2022 Page 2 ID:dd0G1hBWk6p3yzOayBoIH3za0dF-g71GMBQI5YyifUVtSDN 5t9d3vVGm0zoVBXP2dywD0																
PLATES (table is in inches) <table border="1"><thead><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>D</td><td>BMV1+P</td><td>MT20</td><td>3.0</td><td>8.0</td><td></td><td></td></tr></tbody></table> NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.							JT	TYPE	PLATES	W	LEN	Y	X	D	BMV1+P	MT20	3.0	8.0		
JT	TYPE	PLATES	W	LEN	Y	X														
D	BMV1+P	MT20	3.0	8.0																
 STRUCTURAL COMPONENT ONLY DWG # TR22070628 PG 2/2																				

JOB NAME 425142	TRUSS NAME T51	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:51:08 2022 Page 1
 ID:9JKWqE1o0hmyukA0GZGHL1zZKTS-WWwVfU3p3WpA9HBDy3QDMcnPqz1P3oNdnEzywCIX
 -1-3-8 0-0 5-10-8 5-10-8 5-0-9 10-11-1 4-10-13 15-9-13 4-10-13 20-8-10 4-2-2 24-10-12 25-9-3 10-7 5-0-13 30-10-0 32-1-8
 1-3-8 1-3-8 1-3-8
 Scale = 1:55.8



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	DESCR.
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
L - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #/ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-F	2	12
F-H	2	12
H-J	2	12
J-L	2	12
L-P	2	12
P-N	2	12
N-K	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
S-P	2	12
P-N	2	12
N-K	2	12
WEBS: (0.122"x3") SPIRAL NAILS		
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PILES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	JT VERT	IN-SX	IN-SX
S 3257	0	0	5-8
K 3255	0	0	5-8

UNFACTORED REACTIONS			
1ST CASE	MAX. MIN. COMPONENT REACTIONS		
JT COMBINED	SNOW	LIVE	PERM. LIVE
S 2310	1480 / 0	0 / 0	0 / 0
K 2308	1482 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)
FR-TO			
A-B	0 / 33	-84.9	-84.9 0.03 (1)
B-C	-4122 / 0	-84.9	-84.9 0.17 (1)
C-T	-4982 / 0	-84.9	-84.9 0.19 (1)
T-U	-4982 / 0	-84.9	-84.9 0.19 (1)
U-D	-4982 / 0	-84.9	-84.9 0.19 (1)
D-V	-5386 / 0	-84.9	-84.9 0.20 (1)
V-W	-5386 / 0	-84.9	-84.9 0.20 (1)
W-X	-5386 / 0	-84.9	-84.9 0.20 (1)
X-E	-5386 / 0	-84.9	-84.9 0.20 (1)
E-Y	-5386 / 0	-84.9	-84.9 0.18 (1)
Y-F	-5386 / 0	-84.9	-84.9 0.18 (1)
F-Z	-5386 / 0	-84.9	-84.9 0.18 (1)
Z-G	-5386 / 0	-84.9	-84.9 0.18 (1)
G-AA	-4765 / 0	-84.9	-84.9 0.17 (1)
AA-AB	-4765 / 0	-84.9	-84.9 0.17 (1)
AB-H	-4765 / 0	-84.9	-84.9 0.17 (1)
H-I	-3772 / 0	-84.9	-84.9 0.13 (1)
I-J	0 / 36	-84.9	-84.9 0.03 (1)
S-B	-3178 / 0	0.0	0.0 0.11 (1)
K-I	-3191 / 0	0.0	0.0 0.11 (1)
S-AC	0 / 0	-18.5	-18.5 0.07 (4)
AC-AD	0 / 0	-18.5	-18.5 0.07 (4)
AD-R	0 / 0	-18.5	-18.5 0.07 (4)
R-AE	0 / 3414	-18.5	-18.5 0.25 (1)
AE-AF	0 / 3414	-18.5	-18.5 0.25 (1)
AF-Q	0 / 3414	-18.5	-18.5 0.25 (1)
Q-P	0 / 4983	-18.5	-18.5 0.37 (1)
P-AQ	0 / 4983	-18.5	-18.5 0.37 (1)
AQ-AH	0 / 4983	-18.5	-18.5 0.37 (1)
AH-O	0 / 4983	-18.5	-18.5 0.37 (1)
O-AI	0 / 4765	-18.5	-18.5 0.35 (1)
AI-N	0 / 4765	-18.5	-18.5 0.35 (1)
N-M	0 / 4765	-18.5	-18.5 0.35 (1)
M-AJ	0 / 2999	-18.5	-18.5 0.22 (1)
AJ-AK	0 / 2999	-18.5	-18.5 0.22 (1)
AK-L	0 / 2999	-18.5	-18.5 0.22 (1)
L-AL	0 / 0	-18.5	-18.5 0.06 (4)
AL-AM	0 / 0	-18.5	-18.5 0.06 (4)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2015, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA D86-14
 - TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
 ALLOWABLE DEFL.(TL) = L/360 (1.03")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.20/1.00 (D-E:1), BC=0.37/1.00 (O-Q:1)
 WB=0.31/1.00 (B-R:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

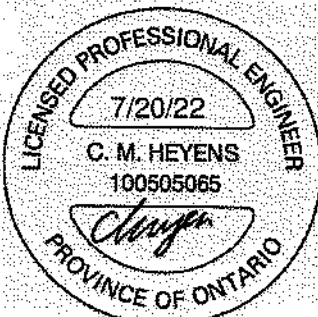
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
 DWG # TR22070643 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T51	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:51:08 2022 Page 2

ID:9UKWqE1o0hmYukA0GZGH1zZkTS-iWwfuVFu3p3WpA9IBDy3QDMcnPqz1P3oNdnEzywCIX

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	6.0	10.0	Edge	1.75
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	10.0	4.00	1.75
I	TMVW-p	MT20	5.0	6.0	1.25	3.00
L	BMWW-t	MT20	3.0	8.0		
M	BMWW+t	MT20	5.0	6.0		
N	BS-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	8.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
AM-K	0 / 0	-18.5	-18.5	0.06 (4)	10.00		

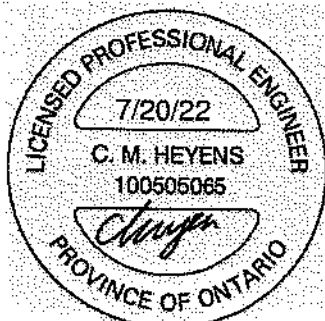
SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-375	-375		FRONT	VERT	TOTAL		C1
G	20-10-12	-113	-113		FRONT	VERT	TOTAL		C1
H	25-9-3	-50	-50		FRONT	VERT	DEAD		C1
H	25-9-3	-194	-194		FRONT	VERT	SNOW		C1
M	20-10-12	-29	-29		FRONT	VERT	TOTAL		C1
N	18-10-12	-29	-29		FRONT	VERT	TOTAL		C1
P	11-11-4	-29	-29		FRONT	VERT	TOTAL		C1
R	5-11-4	-29	-29		FRONT	VERT	TOTAL		C1
T	7-11-4	-113	-113		FRONT	VERT	TOTAL		C1
U	9-11-4	-113	-113		FRONT	VERT	TOTAL		C1
V	11-11-4	-113	-113		FRONT	VERT	TOTAL		C1
W	13-11-4	-113	-113		FRONT	VERT	TOTAL		C1
X	14-10-12	-113	-113		FRONT	VERT	TOTAL		C1
Y	15-10-12	-113	-113		FRONT	VERT	TOTAL		C1
Z	18-10-12	-113	-113		FRONT	VERT	TOTAL		C1
AA	22-10-12	-113	-113		FRONT	VERT	TOTAL		C1
AB	24-10-12	-117	-117		FRONT	VERT	TOTAL		C1
AC	1-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AD	3-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AE	7-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AF	9-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AG	13-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AH	14-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AI	16-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AJ	22-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AK	24-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AL	26-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AM	28-10-12	-29	-29		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.45 (P) (INPUT = 1.00)



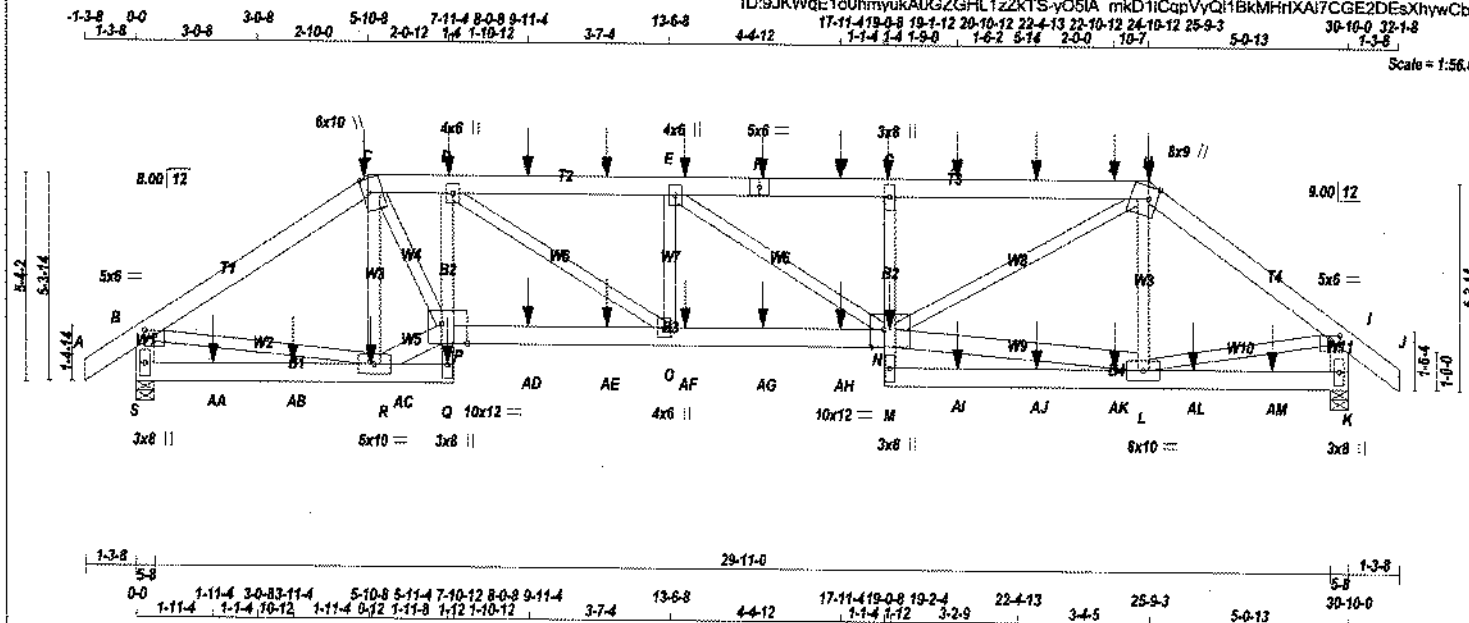
STRUCTURAL COMPONENT ONLY
DWG # TR22070643 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425143	T51S	1	2	ROYAL PINE HOMES	

Tamarack Roof Truss, Burlington

Version 5.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:58:24 2022 Page 1

ID:9JKWqE1o0hmuykADGZGHL1zZKTS-yO5IA mkD1ICqpVyQhBkMHrXAI7CCE2DEsXhywCbr



Scale = 1:56.8

<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - D	2x4	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT		2x4	DRY	No.2
R - P	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425143	T51S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitak Industries, Inc. Wed Jul 20 10:58:24 2022 Page 2
ID:9JKWqE100hmyukA0GZGHL1zZkTS-y05IA mkD1tCqpVvQH1BkMHr1XA7CCE2DEsXhywCb

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	6.0	10.0	Edge	1.75
D	TMVW+p	MT20	4.0	6.0		
E	TMVW-H	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	8.0		
H	TTWW+m	MT20	8.0	9.0	3.75	2.50
I	TMVW-p	MT20	5.0	6.0	1.25	3.00
K	BMV1+p	MT20	3.0	8.0		
L	BMVWW-I	MT20	6.0	10.0		
M	BMV+p	MT20	3.0	8.0		
N	BVMWWW-I	MT20	10.0	12.0	5.25	4.25
O	BMVWW-I	MT20	4.0	6.0		
P	BVMWW-I	MT20	10.0	12.0	6.00	7.75
Q	BMV+p	MT20	3.0	8.0		
R	BMVWW-I	MT20	6.0	10.0		
S	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
L-AL	0/0	-18.5	-18.5	0.10 (4)	10.00		
AL-AM	0/0	-18.5	-18.5	0.10 (4)	10.00		
AM-K	0/0	-18.5	-18.5	0.10 (4)	10.00		

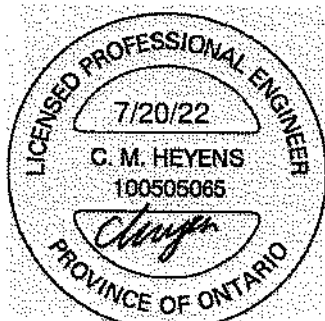
SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	6-10-8	-375	-375		FRONT	VERT	TOTAL		C1
D	7-11-4	-113	-113		FRONT	VERT	TOTAL		C1
F	15-11-4	-63	-63		FRONT	VERT	TOTAL		C1
G	19-1-12	-113	-113		FRONT	VERT	TOTAL		C1
H	25-9-3	-50	-50		FRONT	VERT	DEAD		C1
H	25-9-3	-194	-194		FRONT	VERT	SNOW		C1
N	19-2-4	-36	-36		FRONT	VERT	TOTAL		C1
Q	7-10-12	-36	-36		FRONT	VERT	TOTAL		C1
T	9-11-4	-63	-63		FRONT	VERT	TOTAL		C1
U	11-11-4	-63	-63		FRONT	VERT	TOTAL		C1
V	13-11-4	-63	-63		FRONT	VERT	TOTAL		C1
W	17-11-4	-63	-63		FRONT	VERT	TOTAL		C1
X	20-10-12	-113	-113		FRONT	VERT	TOTAL		C1
Y	22-10-12	-113	-113		FRONT	VERT	TOTAL		C1
Z	24-10-12	-117	-117		FRONT	VERT	TOTAL		C1
AA	1-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AB	3-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AC	5-11-4	-29	-29		FRONT	VERT	TOTAL		C1
AD	9-11-4	-69	-69		FRONT	VERT	TOTAL		C1
AE	11-11-4	-69	-69		FRONT	VERT	TOTAL		C1
AF	13-11-4	-69	-69		FRONT	VERT	TOTAL		C1
AG	15-11-4	-69	-69		FRONT	VERT	TOTAL		C1
AH	17-11-4	-69	-69		FRONT	VERT	TOTAL		C1
AI	20-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AJ	22-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AK	24-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AL	26-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AM	28-10-12	-29	-29		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

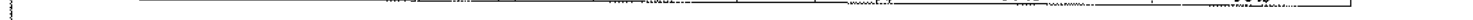
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070681 PG 2/2

Tamarack Roof Truss, Burlington

[illegible]

ME = A	2x6	DR1	NO.2	SFF	2465	163070	070	070	070	83370	070	TOTAL LOAD = 36.7 PSF
--------	-----	-----	------	-----	------	--------	-----	-----	-----	-------	-----	-----------------------

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT. LOADING IN FLAT SECTION BASED ON A

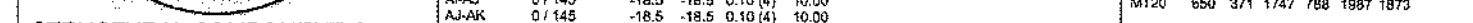
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY
APPLIED

***** WINDS ***** LOADING ***** ALL LOAD CAGES.

CHORDS #ROWS SURFACE LOAD(PLF) TOTAL LOAD CASES: (4) THIS TRUSS IS DESIGNED FOR RESIDENTIAL

AA-AB	-6454 / 0	-84.9	-84.9	0.36 (1)	4.36	CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
-------	-----------	-------	-------	----------	------	--

RAILS TO BE DRIVEN FROM ONE SIDE ONLY.	AB-G	6454.0	-84.0	-84.0	0.36 (1)	4.36	ALLOWABLE DEFL(TL)= L/360 (1.03")
	CA-G	5361.0	-96.0	-96.0	0.32 (1)	4.66	



CONTINUED ON PAGE

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

TABLE 1

SPACING = 24.0 IN. C/C

1. *Journal of Management Studies*, 1997, 34, 1, 1-14.

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL

THIS CROSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

165 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.

RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFLECTION = 1/360 (2.03")

CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL(TL) = $L/360$ (1.03")

CALCULATED VERT. DEFL.(TL) = $1/999$ (0.21")

CSI: TC=0.36/1.00 (E-G:1), BC=0.40/1.00 (Q-R:1)
WB=0.44/1.00 (B-R:1), SSI=0.22/1.00 (E-G:1)

DOI NUMBER=100NAH=100LBEND=100

COL LUMBER=1.00 NAIL=1.00 ES BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT

RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

TEST NO.	PLATE GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
1						
2						
3						
4						
5						
6						
7						
8						
9						
10						
11						
12						
13						
14						
15						
16						
17						
18						
19						
20						
21						
22						
23						
24						
25						
26						
27						
28						
29						
30						
31						
32						
33						
34						
35						
36						
37						
38						
39						
40						
41						
42						
43						
44						
45						
46						
47						
48						
49						
50						
51						
52						
53						
54						
55						
56						
57						
58						
59						
60						
61						
62						
63						
64						
65						
66						
67						
68						
69						
70						
71						
72						
73						
74						
75						
76						
77						
78						
79						
80						
81						
82						
83						
84						
85						
86						
87						
88						

	NO-EX	1974	NO-EX	1974	NO-EX	1974
MT20	650	371	1747	768	1987	1873

© 2006 The Authors
Journal compilation © 2006 Blackwell Publishing Ltd

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425144	T51S1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 11:04:39 2022 Page 2

ID:9JKWqE1o0hmyukA0GZGHL1zZKTS-UYGWR0JwF7 JRq1u0G7QRJwUB1Uo4QH2aCCSwwCVs

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-l	MT20	5.0	8.0	2.50	3.75
C	TMVW+p	MT20	4.0	6.0		
D	TTWW-m	MT20	6.0	10.0	3.00	3.50
E	TMV+p	MT20	3.0	8.0		
F	TS-l	MT20	5.0	6.0		
G	TMVW+l	MT20	4.0	6.0		
H	TTWW-m	MT20	8.0	10.0	2.75	3.75
I	TMVW+p	MT20	5.0	6.0	1.25	3.00
K	BMV1+p	MT20	3.0	8.0		
L	BMVW-l	MT20	5.0	6.0		
M	BS-l	MT20	5.0	6.0		
N	BMVWW-l	MT20	8.0	10.0		
O	BMV+p	MT20	3.0	8.0		
P	BVMVWW-l	MT20	10.0	12.0	Edge	4.50
Q	BMVW+l	MT20	4.0	6.0		
R	BVMVWW-l	MT20	10.0	12.0	4.00	7.75
S	BMV+p	MT20	3.0	8.0		
T	BMVW1-l	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
FR-TO		FROM	TO				
AM-M	0 / 2970	-18.5	-18.5	0.23 (1)	10.00		
M-AN	0 / 2970	-18.5	-18.5	0.23 (1)	10.00		
AN-AO	0 / 2970	-18.5	-18.5	0.23 (1)	10.00		
AQ-L	0 / 2970	-18.5	-18.5	0.23 (1)	10.00		
L-AP	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
AP-AQ	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
AQ-K	0 / 0	-18.5	-18.5	0.07 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-58	-58		FRONT	VERT	DEAD		C1
D	5-10-8	-223	-223		FRONT	VERT	SNOW		C1
H	25-9-3	-50	-50		FRONT	VERT	DEAD		C1
H	25-9-3	-194	-194		FRONT	VERT	SNOW		C1
S	1-9-12	-252	-252		FRONT	VERT	TOTAL		C1
U	5-6-12	-83	-83		FRONT	VERT	TOTAL		C1
V	7-6-12	-63	-63		FRONT	VERT	TOTAL		C1
W	9-6-12	-63	-63		FRONT	VERT	TOTAL		C1
X	11-6-12	-63	-63		FRONT	VERT	TOTAL		C1
Y	13-2-4	-113	-113		FRONT	VERT	TOTAL		C1
Z	15-2-4	-113	-113		FRONT	VERT	TOTAL		C1
AA	17-2-4	-113	-113		FRONT	VERT	TOTAL		C1
AB	18-9-4	-113	-113		FRONT	VERT	TOTAL		C1
AC	20-10-12	-113	-113		FRONT	VERT	TOTAL		C1
AD	22-10-12	-113	-113		FRONT	VERT	TOTAL		C1
AE	24-10-12	-117	-117		FRONT	VERT	TOTAL		C1
AF	3-6-12	-69	-69		FRONT	VERT	TOTAL		C1
AG	9-6-12	-69	-69		FRONT	VERT	TOTAL		C1
AH	11-6-12	-69	-69		FRONT	VERT	TOTAL		C1
AI	13-2-4	-32	-32		FRONT	VERT	TOTAL		C1
AJ	15-2-4	-29	-29		FRONT	VERT	TOTAL		C1
AK	17-2-4	-29	-29		FRONT	VERT	TOTAL		C1
AL	18-9-4	-29	-29		FRONT	VERT	TOTAL		C1
AM	20-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AN	22-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AO	24-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AP	26-10-12	-29	-29		FRONT	VERT	TOTAL		C1
AQ	28-10-12	-29	-29		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

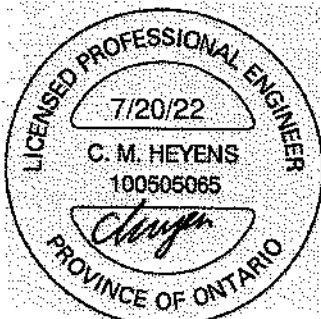
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (D) (INPUT = 0.90)

JSI METAL = 0.55 (B) (INPUT = 1.00)

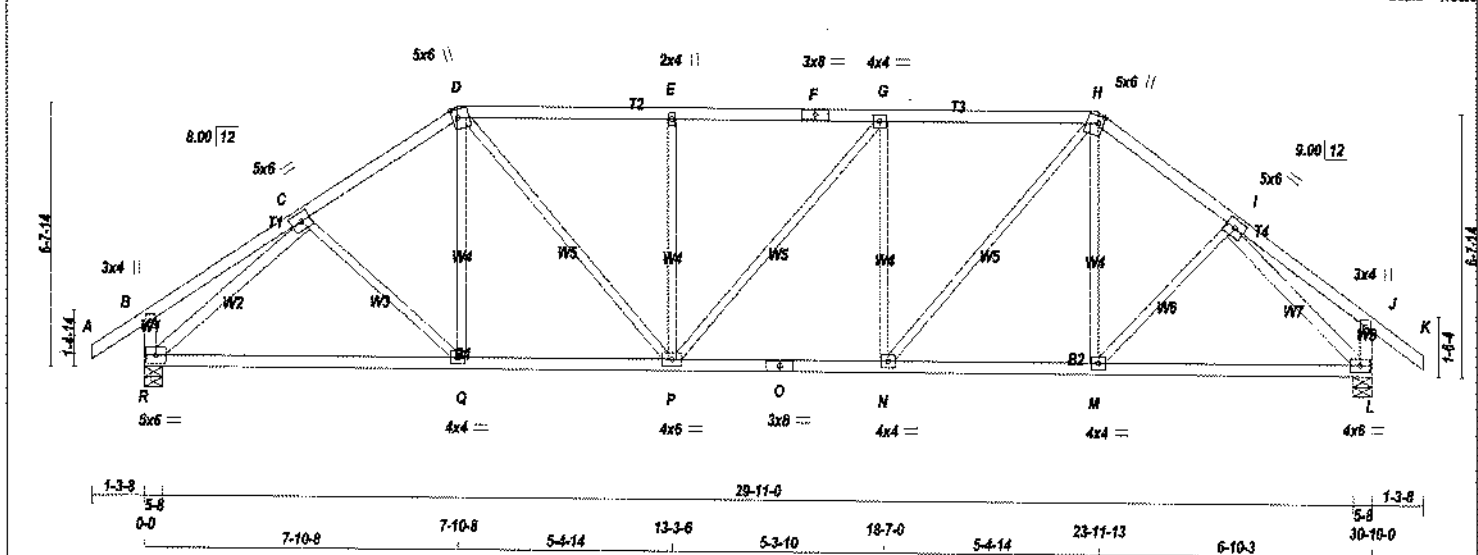


STRUCTURAL COMPONENT ONLY
DWG # TR22070687 PG 2/2

JOB NAME 425142	TRUSS NAME T52	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 10:51:09 2022 Page 1
 ID:9JKWqE1o0hmyukA0GZGHL1zZKTS-CIJJEVfNkw8ZjMRukBbdmU0Bki0ZC01MLnPywCIV

Scale = 1:56.0



TOTAL WEIGHT = 139 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - Q	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
R - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	TTWW+m	MT20	5.0	6.0	2.50 1.50
E	TMW+w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	8.0	
G	TMWW-t	MT20	4.0	4.0	
H	TTWW+m	MT20	5.0	6.0	2.25 1.50
I	TMWW-t	MT20	5.0	6.0	
J	TMV+p	MT20	3.0	4.0	
L	BMWW-t	MT20	4.0	6.0	
M, N, Q					
M	BMWW-t	MT20	4.0	4.0	
O	BS-t	MT20	3.0	8.0	
P	BMWW-t	MT20	4.0	6.0	
R	BMWW-t	MT20	5.0	6.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY
DWG # TR22070644

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REORD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
R		1710	0	1710	0	0	5-8	5-8	
L		1711	0	1711	0	0	5-8	5-8	

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
JT		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R		1212	782/0	0/0	0/0	0/0	430/0	0/0
L		1212	782/0	0/0	0/0	0/0	430/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING				CHORDS				WEBS			
TOTAL LOAD CASES: (4)				MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
				MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	
				FR-TO		FROM TO		FR-TO			
				A-B	0/32	-84.9	-84.9 0.11 (1)	10.00	C-Q	-2/52	0.02 (4)
				B-C	0/20	-84.9	-84.9 0.20 (1)	10.00	Q-D	0/153	0.05 (4)
				C-D	-1810/0	-84.9	-84.9 0.26 (1)	4.72	D-P	0/684	0.15 (1)
				D-E	-1932/0	-84.9	-84.9 0.41 (1)	4.42	P-E	-491/0	0.36 (1)
				E-F	-1932/0	-84.9	-84.9 0.41 (1)	4.41	P-G	0/85	0.01 (1)
				F-G	-1932/0	-84.9	-84.9 0.41 (1)	4.41	N-G	-542/0	0.40 (1)
				G-H	-1891/0	-84.9	-84.9 0.41 (1)	4.45	N-H	0/833	0.19 (1)
				H-I	-1710/0	-84.9	-84.9 0.19 (1)	4.91	M-H	0/99	0.03 (4)
				I-J	0/19	-84.9	-84.9 0.15 (1)	10.00	M-I	0/77	0.02 (4)
				J-K	0/35	-84.9	-84.9 0.12 (1)	10.00	R-C	-2045/0	0.70 (1)
				R-B	-246/0	0.0	0.0 0.03 (1)	7.81	I-L	-1928/0	0.56 (1)
				L-J	-230/0	0.0	0.0 0.02 (1)	7.81			
				R-Q	0/1488	-18.5	-18.5 0.38 (1)	10.00			
				Q-P	0/1489	-18.5	-18.5 0.39 (4)	10.00			
				P-O	0/1891	-18.5	-18.5 0.36 (1)	10.00			
				O-N	0/1891	-18.5	-18.5 0.36 (1)	10.00			
				N-M	0/1351	-18.5	-18.5 0.32 (1)	10.00			
				M-L	0/1297	-18.5	-18.5 0.32 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.41/1.00 (E-G-1), BC=0.39/1.00 (P-Q-4), WB=0.70/1.00 (C-R-1), SS=0.21/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

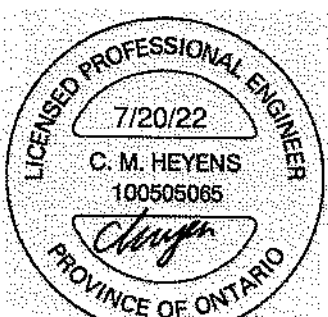
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	850 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.83 (O) (INPUT = 1.00)

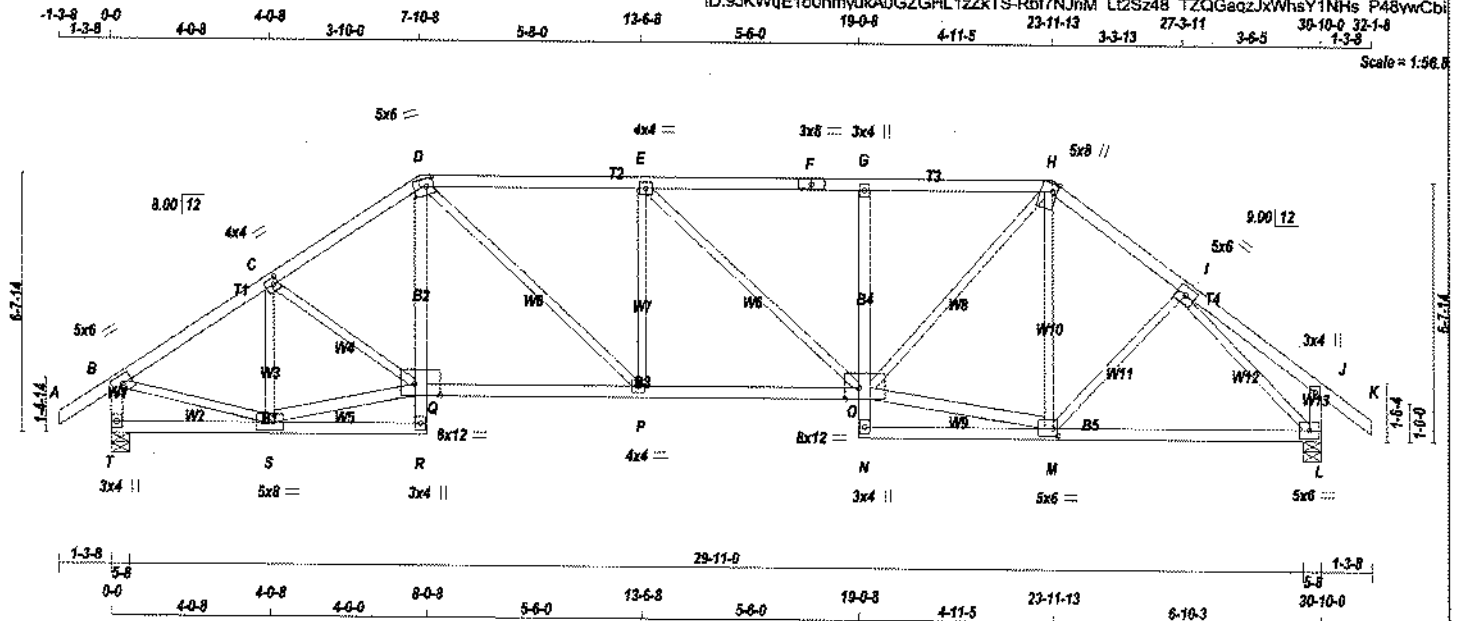


STRUCTURAL COMPONENT ONLY
 DWG # TR22070644

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425143	T52S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:58:25 2022 Page 1
ID:9JKWqE1c0hmyukA0GZGHL1zZkTS-Rbf7NjNm Lt2Sz48 TZQGaozJxWhsY1NHs P48vwCbi



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
T - R	2x4	DRY	No.2	SPF	
R - D	2x4	DRY	No.2	SPF	
Q - O	2x4	DRY	No.2	SPF	
N - G	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
S - Q	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	5.0	6.0	2.50	2.25
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMV+p	MT20	3.0	4.0		
H	TTVW+m	MT20	5.0	8.0	Edge	1.50
I	TMVW-t	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	5.0	6.0	2.25	1.50
N	BMV+p	MT20	3.0	4.0		
O	BMVW-t	MT20	8.0	12.0	Edge	4.25
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	8.0	12.0	3.50	7.75
R	BMV+p	MT20	3.0	4.0		
S	BMVW-t	MT20	5.0	8.0		
T	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	DOWN	GROSS REACTION	UP		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
T	1710	0	1710	0	5-8	5-8
L	1711	0	1711	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1212	782/0	0/0	0/0	0/0	430/0	0/0
L	1212	782/0	0/0	0/0	0/0	430/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	FACTORED		MAX. UNBRACED	MEMB.	MAX. FACTORED
					LC1	MAX			
FR-TO					FROM	TO	LENGTH	FR-TO	
A-B	0/32	-84.9	-84.9	0.11 (1)	10.00	S-C	-725/0	0.19 (1)	
B-C	-1813/0	-84.9	-84.9	0.26 (1)	4.71	S-Q	0/1546	0.25 (1)	
C-D	-2126/0	-84.9	-84.9	0.28 (1)	4.40	C-Q	0/269	0.06 (1)	
D-E	-2318/0	-84.9	-84.9	0.52 (1)	3.97	D-P	0/779	0.18 (1)	
E-F	-2191/0	-84.9	-84.9	0.51 (1)	4.08	P-E	-420/0	0.20 (1)	
F-G	-2191/0	-84.9	-84.9	0.51 (1)	4.08	E-O	-176/0	0.21 (1)	
G-H	-2193/0	-84.9	-84.9	0.37 (1)	4.24	O-M	0/1337	0.21 (1)	
H-I	-1710/0	-84.9	-84.9	0.19 (1)	4.91	O-H	0/1249	0.28 (1)	
I-J	0/18	-84.9	-84.9	0.14 (1)	10.00	M-H	-195/0	0.14 (1)	
J-K	0/35	-84.9	-84.9	0.12 (1)	10.00	M-I	0/77	0.02 (4)	
T-B	-1678/0	0.0	0.0	0.17 (1)	6.41	B-S	0/1579	0.36 (1)	
L-J	-231/0	0.0	0.0	0.02 (1)	7.81	I-L	-1928/0	0.79 (1)	
T-S	0/0	-18.5	-18.5	0.07 (4)	10.00				
S-R	0/32	-18.5	-18.5	0.08 (4)	10.00				
R-Q	0/31	0.0	0.0	0.08 (1)	10.00				
Q-D	0/324	0.0	0.0	0.10 (1)	10.00				
Q-P	0/1752	-18.5	-18.5	0.30 (1)	10.00				
P-O	0/2318	-18.5	-18.5	0.46 (1)	10.00				
N-O	0/30	0.0	0.0	0.06 (1)	10.00				
O-G	-465/0	0.0	0.0	0.15 (1)	7.81				
N-M	0/41	-18.5	-18.5	0.20 (4)	10.00				
M-I	0/1297	-18.5	-18.5	0.32 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.21")

CSI: TC=0.52/1.00 (D-E-1), BC=0.46/1.00 (O-P-1)
WB=0.79/1.00 (I-L-1), SS=0.23/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

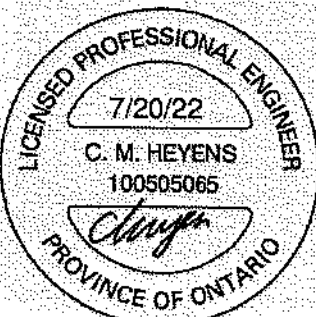
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 660 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425143	T52S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 10:58:25 2022 Page 2
ID:9JKWgE1a0hmyukA0GZGHL1zZkTS-Rbf7Njnm 112Sz48 TZQGaqz.kWhsY1NHs P48vwCbi

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22070682 PG 2/2

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 11:04:40 2022 Page 1

ID:9JKWqE1o0hmyukA0GZGHL1zZkTS-ykpu4YJafZ7nxbFDSjnMzfs2a6WFDsuaWIKkvwCVr

1-3-8 0-0 1-11-8 4-11-0 7-10-8 12-5-8 18-2-11 23-11-13 27-3-11 30-16-0 32-1-8

7-3-8 1-11-8 2-11-8 2-11-8 4-7-0 5-9-3 5-9-3 3-3-13 3-6-5 1-3-8

Scale = 1/56.8



BUILDING DESIGNER						
BEARINGS						
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORIZ	DOWN	HORIZ	IN-SX	IN-SX
M	1708	0	1708	0	5-8	5-8
V	1713	0	1713	0	5-8	5-8

UNFACTORED REACTIONS						
JT	1ST L.CASE COMBINED		MAX./MIN. COMPONENT REACTIONS			
	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1210	781 / 0	0 / 0	0 / 0	429 / 0	0 / 0
V	1213	783 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3 PSF
	DL =	6.0 PSF
BOT CH.	LL =	0.0 PSF
	DL =	7.4 PSF
TOTAL LOAD	=	36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF NBCC 2015 (FLOORING)

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										
LOADING										
TOTAL LOAD CASES: (4)										
CHORDS					WEB S					
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	FACTORED (PLF)	MAX CS (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS (LC)		
FR-TO		FROM TO				FR-TO				
A-B	0 / 32	-84.9	-84.9	0.11 (f)	10.00	T-D	0 / 88	0.02 (4)		
B-C	-2394 / 0	-84.9	-84.9	0.17 (f)	4.29	D-S	-308 / 0	0.11 (f)		
C-D	-2400 / 0	-84.9	-84.9	0.20 (f)	4.27	S-E	0 / 346	0.08 (f)		
D-E	-2115 / 0	-84.9	-84.9	0.19 (f)	4.51	E-R	0 / 744	0.17 (f)		
E-F	-2241 / 0	-84.9	-84.9	0.36 (f)	4.21	R-P	0 / 1892	0.30 (f)		
F-G	-2231 / 0	-84.9	-84.9	0.53 (f)	4.02	R-H	0 / 436	0.10 (f)		
G-H	-2231 / 0	-84.9	-84.9	0.53 (f)	4.02	P-H	-843 / 0	0.82 (f)		
H-I	-1907 / 0	-84.9	-84.9	0.50 (f)	4.32	P-I	0 / 837	0.19 (f)		
I-J	-1705 / 0	-84.9	-84.9	0.18 (f)	4.02	N-I	0 / 97	0.03 (4)		
J-K	0 / 18	-84.9	-84.9	0.14 (f)	10.00	N-J	0 / 77	0.02 (4)		
K-L	0 / 35	-84.9	-84.9	0.12 (f)	10.00	J-M	-1923 / 0	0.79 (f)		
V-B	-1854 / 0	0.0	0.0	0.17 (f)	6.45	V-T	-86 / 0	0.01 (f)		
M-K	-231 / 0	0.0	0.0	0.02 (f)	7.81	B-T	0 / 1965	0.32 (f)		

RA 5/09 CBC 2012 (2019 AMENDMENT)
 CSA 086-14
 TPIC 2014

DESIGN ASSUMPTIONS
 OVERHANG NOT TO BE ALTERED OR CUT
 OFF.

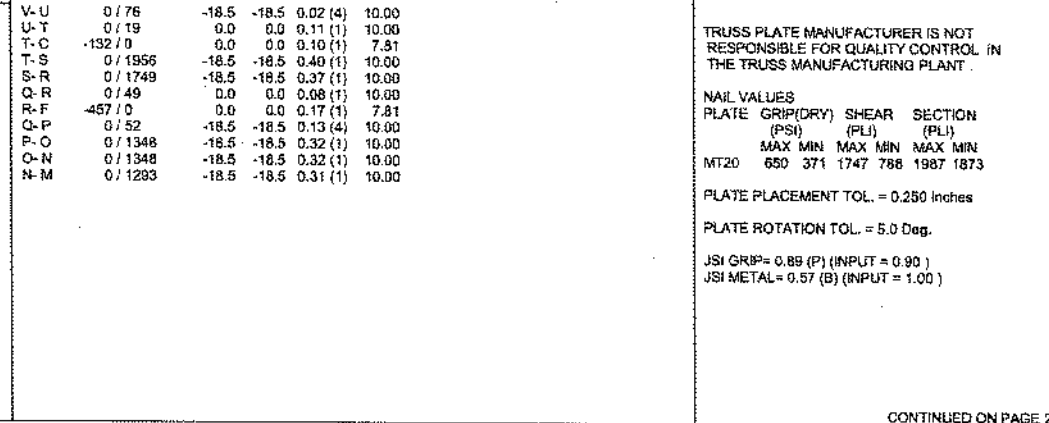
(56 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.
 RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED
 ROOF LIVE LOAD

ALLOWABLE DEFL. (L₁)= L/360 (1.03")
 CALCULATED VERT. DEFL.(L₁)= L/ 999 (0.16")
 ALLOWABLE DEFL. (T₁)= L/360 (1.03")
 CALCULATED VERT. DEFL. (T₁)= L/ 999 (0.21")

CSI: TC=0.53/1.00 (F+H-1), BC=0.40/1.00 (S-T-1),
 WB=0.79/1.00 (J-H-1), SSI=0.23/1.00 (H-E-1)

DOL LUMBER=1.00 NAIL=1.00 L5 BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425144	T52S1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 11:04:40 2022 Page 2
ID:9JKWgE1c0hmyukA0GZGHL1zZkTS-ykpu4YJaIZ7nbFDSjnMzfs2aSWFD9uaWikkywCvr

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMVW1+p	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

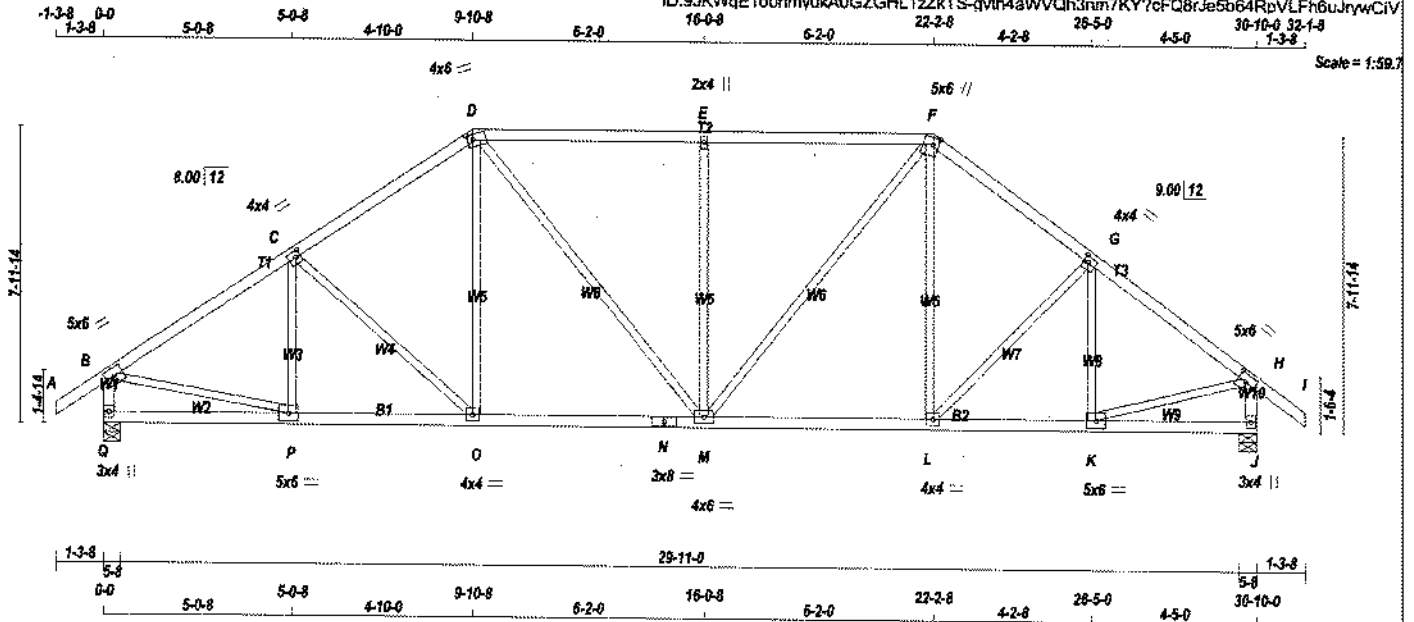


STRUCTURAL COMPONENT ONLY
DWG # TR22070688 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T53	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Jul 20 10:51:10 2022 Page 1
ID:9JkWqE1o0hmyukA0GZGHL1zZkTS-qvth4aWVQh3nm7KY?cFQ8rJe5b64RpVLfH6uJrywCIV



TOTAL WEIGHT = 139 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	5.0	6.0	
C	TMVW-1	MT20	4.0	4.0	2.00 1.50
D	TTVW-m	MT20	4.0	6.0	1.75 1.75
E	TMVW-m	MT20	2.0	4.0	
F	TTVW-m	MT20	5.0	6.0	2.25 1.75
G	TMVW-1	MT20	4.0	4.0	2.00 1.50
H	TMVW-1	MT20	5.0	6.0	2.50 2.75
J	BMV1+p	MT20	3.0	4.0	
K	BMVW-1	MT20	5.0	6.0	
L	BMVW-1	MT20	4.0	4.0	
M	BMVW-1	MT20	4.0	6.0	
N	BS-1	MT20	3.0	8.0	
O	BMVW-1	MT20	4.0	4.0	
P	BMVW-1	MT20	5.0	6.0	
Q	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	DOWN	UP		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
Q	1710	0	1710	0	5-8	5-8
J	1711	0	1711	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
Q	1212	782 / 0	0 / 0	0 / 0	0 / 0	430 / 0
J	1212	782 / 0	0 / 0	0 / 0	0 / 0	430 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX.	MEMB.	MAX. FACTORED	FACTORED	MAX.
FR-TO	FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)	FR-TO	FORCE (LBS)	CS (LC)	CS (LC)
A-B	0 / 32	-84.9	-84.9 0.11 (1)	10.00	P-C	-244 / 0	0.08 (1)
B-C	-1863 / 0	-84.9	-84.9 0.32 (1)	4.83	C-O	-239 / 0	0.19 (1)
C-D	-1707 / 0	-84.9	-84.9 0.30 (1)	4.80	Q-D	0 / 264	0.06 (1)
D-E	-1651 / 0	-84.9	-84.9 0.46 (1)	4.64	D-M	0 / 402	0.09 (1)
E-F	-1651 / 0	-84.9	-84.9 0.46 (1)	4.64	M-E	-841 / 0	0.78 (1)
F-G	-1646 / 0	-84.9	-84.9 0.23 (1)	4.95	M-F	0 / 567	0.13 (1)
G-H	-1704 / 0	-84.9	-84.9 0.24 (1)	4.88	L-F	0 / 195	0.05 (4)
H-I	0 / 35	-84.9	-84.9 0.12 (1)	10.00	L-G	-131 / 0	0.09 (1)
Q-B	-1669 / 0	0.0	0.0 0.17 (1)	6.42	K-G	-295 / 0	0.10 (1)
J-H	-1674 / 0	0.0	0.0 0.17 (1)	6.42	B-P	0 / 1609	0.38 (1)
					K-H	0 / 1433	0.32 (1)
Q-P	0 / 0	-18.5	-18.5 0.10 (4)	10.00			
P-O	0 / 1573	-18.5	-18.5 0.32 (1)	10.00			
Q-N	0 / 1399	-18.5	-18.5 0.30 (1)	10.00			
N-M	0 / 1399	-18.5	-18.5 0.30 (1)	10.00			
M-L	0 / 1286	-18.5	-18.5 0.28 (1)	10.00			
L-K	0 / 1383	-18.5	-18.5 0.28 (1)	10.00			
K-J	0 / 0	-18.5	-18.5 0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	8.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	38.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.46/1.00 (D-E:1), BC=0.32/1.00 (O-P:1), WB=0.78/1.00 (E-M:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

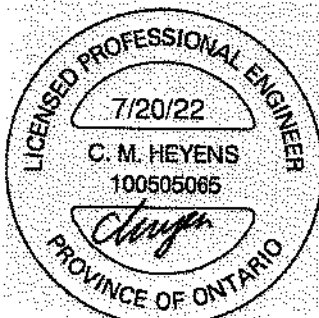
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



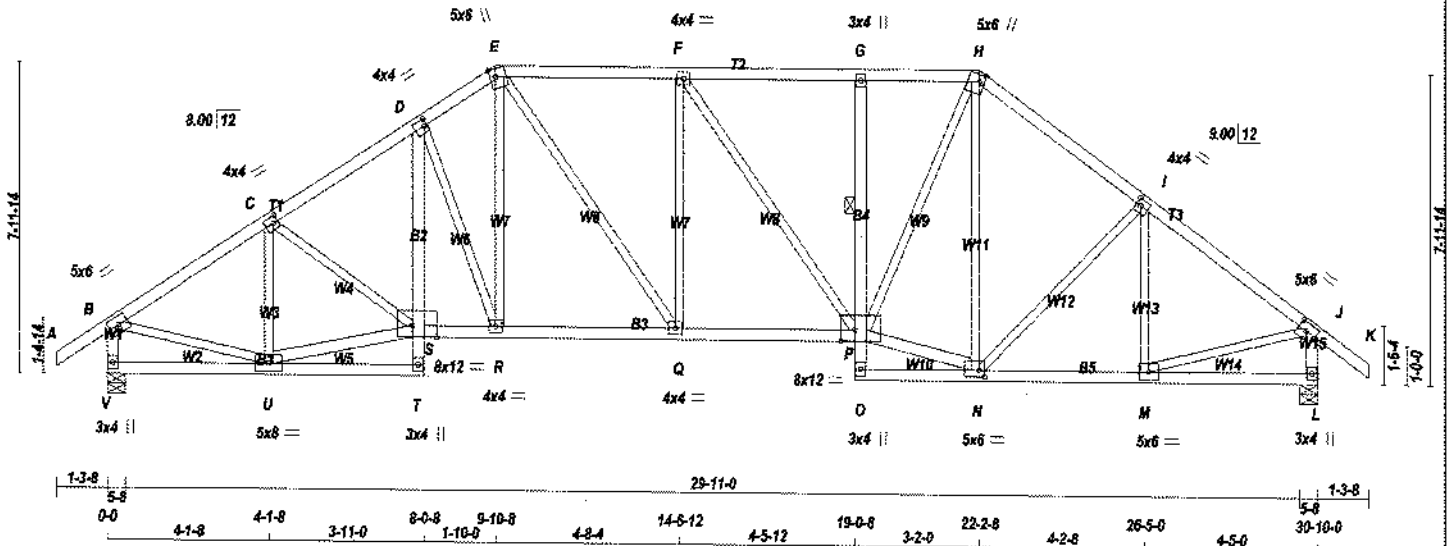
STRUCTURAL COMPONENT ONLY
DWG # TR22070645

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425143	T53S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:58:26 2022 Page 1

ID:9JKWQe1o0hmyukADGZGHL1zZkTS-vnDWafo ke7v37f1XA4fpmMDiLubb63XWwJzaywCbh
 1-3-8 0-0 4-1-8 4-1-8 3-11-0 8-0-8 9-10-8 4-8-4 14-6-12 4-5-12 19-0-8 3-2-0 22-2-8 4-2-8 26-5-0 4-5-0 30-10-0 32-1-8 1-3-8
 Scale = 1:56.8



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - D	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
U - S	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-4	MT20	5.0	6.0		
C	TMVW-4	MT20	4.0	4.0	2.00	1.50
D	TMVW-4	MT20	4.0	4.0	2.00	1.00
E	TTVW+m	MT20	5.0	6.0	Edge	1.75
F	TMVW-4	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	TTVW+m	MT20	5.0	6.0	2.25	1.50
I	TMVW-4	MT20	4.0	4.0	2.00	1.50
J	TMVW-4	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-4	MT20	5.0	6.0		
N	BMVW-4	MT20	5.0	6.0	2.00	1.50
O	BMV+p	MT20	3.0	4.0		
P	BMVW-4	MT20	8.0	12.0	3.50	4.25
Q	BMVW-4	MT20	4.0	4.0		
R	BMVW-4	MT20	4.0	4.0		
S	BMVW-4	MT20	8.0	12.0	3.50	7.50
T	BMV+p	MT20	3.0	4.0		
U	BMVW-4	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
V	1710	0	1710	0	5-8	5-8
L	1711	0	1711	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
V	1212	782 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0
L	1212	782 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO	CSI (LC)	FR-TO			
A-B	0 / 32	-84.9	-84.9 0.11 (1)	10.00	U-C	-727 / 0	0.19 (1)
B-C	-1816 / 0	-84.9	-84.9 0.20 (1)	4.80	U-S	0 / 1549	0.25 (1)
C-D	-2132 / 0	-84.9	-84.9 0.20 (1)	4.49	C-S	0 / 294	0.07 (1)
D-E	-1939 / 0	-84.9	-84.9 0.08 (1)	4.79	D-R	-473 / 0	0.27 (1)
E-F	-1863 / 0	-84.9	-84.9 0.25 (1)	4.89	R-E	0 / 485	0.11 (1)
F-G	-1739 / 0	-84.9	-84.9 0.14 (1)	4.83	E-Q	0 / 449	0.10 (1)
G-H	-1735 / 0	-84.9	-84.9 0.24 (1)	4.95	Q-P	-272 / 0	0.23 (1)
H-I	-1846 / 0	-84.9	-84.9 0.23 (1)	4.95	P-N	-218 / 0	0.30 (1)
I-J	-1705 / 0	-84.9	-84.9 0.24 (1)	4.88	F-P	0 / 1333	0.21 (1)
J-K	0 / 35	-84.9	-84.9 0.12 (1)	10.00	P-H	0 / 1075	0.24 (1)
V-B	-1677 / 0	0.0	0.0 0.17 (1)	6.41	N-H	-244 / 0	0.30 (1)
L-J	-1674 / 0	0.0	0.0 0.17 (1)	6.42	N-I	-133 / 0	0.09 (1)
					M-I	-292 / 0	0.10 (1)
V-U	0 / 0	-18.5	-18.5 0.07 (4)	10.00	B-U	0 / 1578	0.36 (1)
U-T	0 / 32	-18.5	-18.5 0.08 (4)	10.00	M-J	0 / 1434	0.32 (1)
T-S	0 / 30	0.0	0.0 0.05 (1)	10.00			
S-D	0 / 297	0.0	0.0 0.09 (1)	10.00			
S-R	0 / 1769	-18.5	-18.5 0.32 (1)	10.00			
R-Q	0 / 1605	-18.5	-18.5 0.31 (1)	10.00			
Q-P	0 / 1893	-18.5	-18.5 0.35 (1)	10.00			
O-P	0 / 21	0.0	0.0 0.04 (1)	10.00			
P-G	-326 / 0	0.0	0.0 0.04 (1)	6.25			
O-N	0 / 28	-18.5	-18.5 0.06 (4)	10.00			
N-M	0 / 1384	-18.5	-18.5 0.27 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, ABC 2019
 - PART 9 OF CBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
 ALLOWABLE DEFL.(TL) = L/360 (1.03")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.25/1.00 (E-F:1), BC=0.35/1.00 (P-Q:1)
 WB=0.36/1.00 (B-U:1), SI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

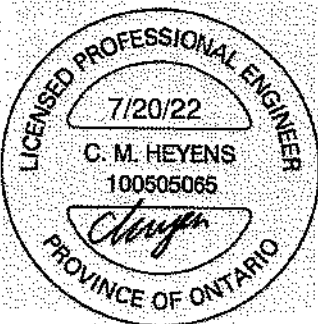
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (N) (INPUT = 0.90)
 JSI METAL = 0.45 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
 DWG # TR22070683 PG 1/2

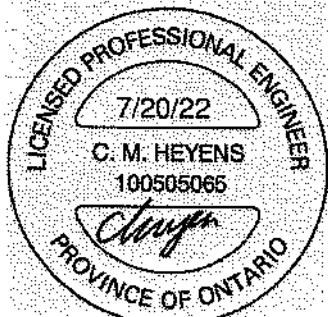
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425143	T53S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington
Version 8.530 S Feb 23 2022 MfTek Industries, Inc. Wed Jul 20 10:58:26 2022 Page 2
ID:9JKWqE1c0hmYukA0GZGHL1zZkTS-vnDWafo ke7v37fLXA4gnMDiLubb63XWWizcawwCbn

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

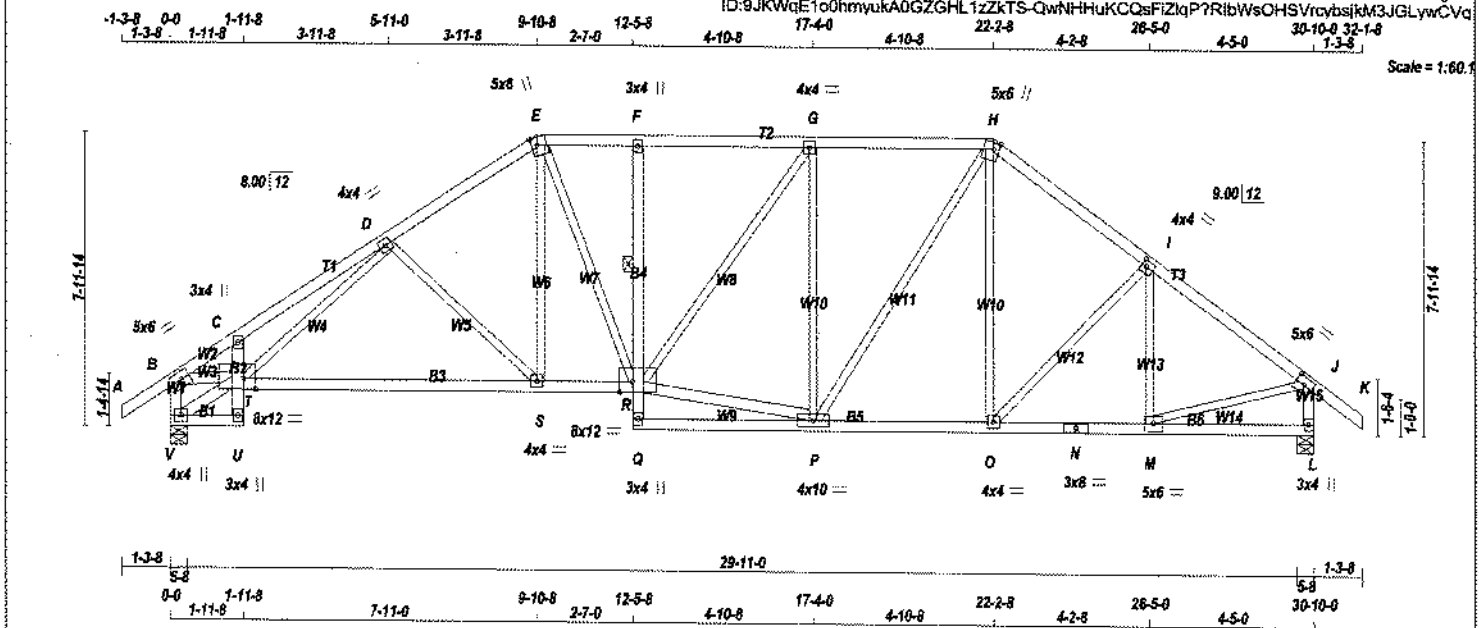


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425144	T53S1	1	1	ROYAL PINE HOMES	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 11:04:41 2022 Page 1

ID:9JKWqE1o0hmyuka0GZGHL1ZZKTS-QwNHuKCCsFizIqP7RibWsOHSVrcvbsjKM3JGLywCVq



TOTAL WEIGHT = 158 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
V - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
V - U	2x4	DRY	No.2	SPF	
U - C	2x4	DRY	No.2	SPF	
T - R	2x4	DRY	No.2	SPF	
Q - F	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
R - P	2x4	DRY	No.2	SPF	
V - T	2x4	DRY	No.2	SPF	
B - T	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	5.0		
C	TMV+p	MT20	3.0	4.0		
D	TMVW-1	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
F	TMV+p	MT20	3.0	4.0		
G	TMVW-1	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.75
I	TMVW-1	MT20	4.0	4.0	2.00	1.50
J	TMVW-1	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-1	MT20	5.0	6.0		
N	BS-1	MT20	3.0	8.0		
O	BMVW-1	MT20	4.0	4.0		
P	BMVWVW-t	MT20	4.0	10.0		
Q	BMV+p	MT20	3.0	4.0		
R	BMVWVW-1	MT20	8.0	12.0	3.50	4.25
S	BMVW-1	MT20	4.0	4.0		
T	BMVWVW-1	MT20	8.0	12.0	3.50	4.25
U	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	1708	0	5-8	5-8
V	1713	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
L	1210	781/0	0/0	0/0
V	1213	793/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/32	-84.9	-84.9 0.11 (1)	T-D	0/194	0.04 (1)	
B-C	-2414/0	-84.9	-84.9 0.14 (1)	D-S	-406/0	0.23 (1)	
C-D	-2425/0	-84.9	-84.9 0.25 (1)	S-E	0/436	0.10 (1)	
D-E	-1953/0	-84.9	-84.9 0.22 (1)	E-R	0/449	0.10 (1)	
E-F	-1780/0	-84.9	-84.9 0.13 (1)	R-P	0/1603	0.26 (1)	
F-G	-1775/0	-84.9	-84.9 0.26 (1)	P-G	0/301	0.07 (1)	
G-H	-1596/0	-84.9	-84.9 0.26 (1)	H-G	-715/0	0.87 (1)	
H-I	-1640/0	-84.9	-84.9 0.23 (1)	P-H	0/575	0.13 (1)	
I-J	-1702/0	-84.9	-84.9 0.24 (1)	G-H	0/180	0.04 (4)	
J-K	0/35	-84.9	-84.9 0.12 (1)	Q-I	-136/0	0.09 (1)	
V-B	-1694/0	0.0	0.0 0.17 (1)	M-I	-289/0	0.10 (1)	
L-J	-1672/0	0.0	0.0 0.17 (1)	M-J	0/1431	0.32 (1)	
				V-T	-87/0	0.01 (1)	
				B-T	0/1900	0.32 (1)	
V-U	0/76	-18.5	-18.5 0.02 (4)				
U-T	0/19	0.0	0.0 0.11 (1)				
T-C	-190/0	0.0	0.0 0.08 (1)				
T-S	0/1899	-18.5	-18.5 0.48 (1)				
S-R	0/1610	-18.5	-18.5 0.42 (4)				
Q-R	0/139	0.0	0.0 0.05 (1)				
R-F	-317/0	0.0	0.0 0.05 (1)				
Q-P	0/132	-18.5	-18.5 0.11 (4)				
P-O	0/1280	-18.5	-18.5 0.27 (1)				
O-N	0/1382	-18.5	-18.5 0.26 (1)				
N-M	0/1382	-18.5	-18.5 0.26 (1)				
M-L	0/0	-18.5	-18.5 0.08 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TRC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (1.03")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.08")
ALLOWABLE DEFL.(TL) = 1/360 (1.03")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.22")

CSF: TC=0.26/1.00 (F-G-1), BC=0.46/1.00 (S-T-1)
WB=0.87/1.00 (G-P-1), SS=0.20/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

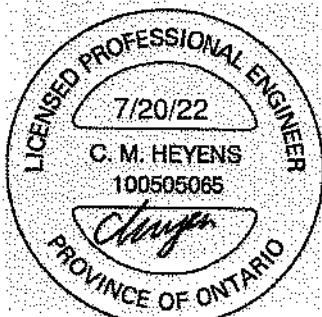
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	850	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (B) (INPUT = 0.90)
JSI METAL = 0.57 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070689 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425144	T53S1	1	1	TRUSS DESC.		

Tamareck Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 11:04:41 2022 Page 2
ID:9JKWqE1c0hmyukA0GZGHL1zZkTS-QwNHuKCCqFIZkP?RIbWsoHSVrcybsikM3JGLywCVq

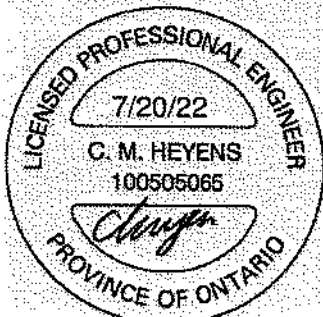
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMVW1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

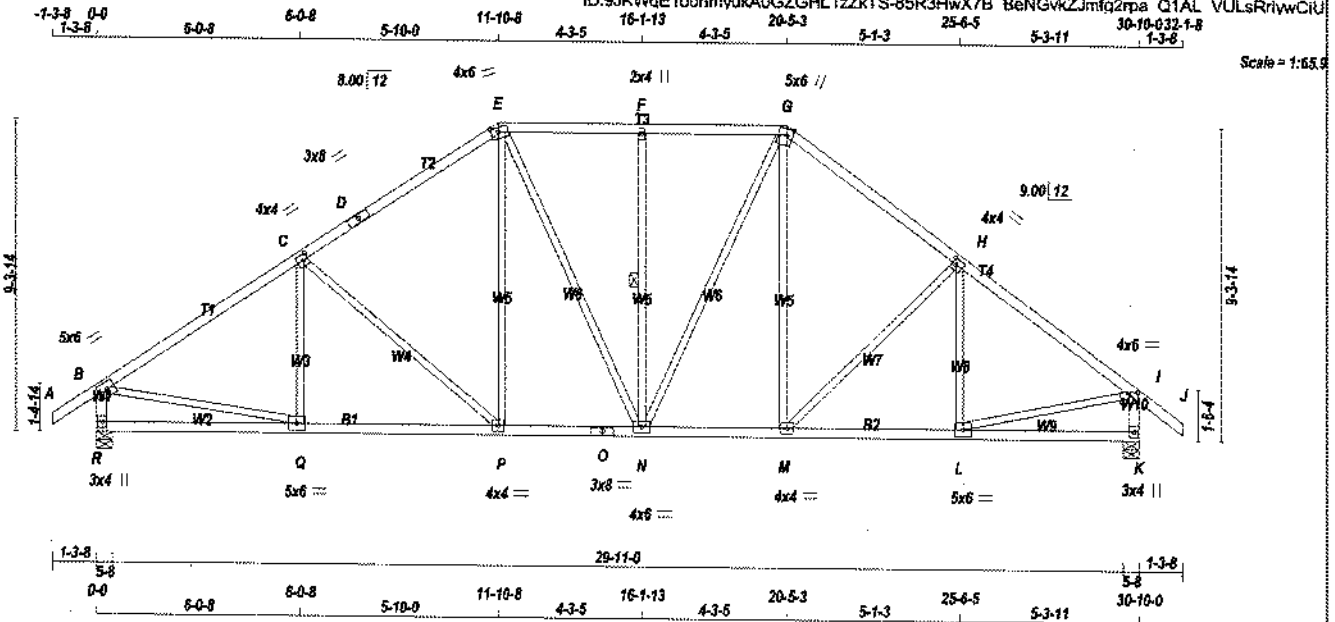


STRUCTURAL COMPONENT ONLY
DWG # TR22070689 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T54	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:51:11 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL1zZKTS-85R3HwX7B BeNGvkZjmfq2mpa Q1AL VULsRrIwCIU



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
R - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - K	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMWW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	Edge	2.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW-p	MT20	4.0	6.0	1.00	3.00
K	BMV1-p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	DOWN	GROSS REACTION	UP		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
R	1710	0	1710	0	5-8	5-8
K	1711	0	1711	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
R	1212	782 / 0	0 / 0	0 / 0	0 / 0	430 / 0
K	1212	782 / 0	0 / 0	0 / 0	0 / 0	430 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED			
		VERT. LOAD (PLF)	CS1 (LC)			FORCE (LBS)	MAX CS1 (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	Q-C	-163 / 39	0.07 (1)	
B-C	-1885 / 0	-84.9	-84.9	0.47 (1)	4.41	C-P	-403 / 0	0.51 (1)	
C-D	-1588 / 0	-84.9	-84.9	0.44 (1)	4.76	E-N	0 / 358	0.08 (1)	
D-E	-1588 / 0	-84.9	-84.9	0.44 (1)	4.76	E-N	0 / 195	0.04 (1)	
E-F	-1379 / 0	-84.9	-84.9	0.21 (1)	5.32	N-F	-438 / 0	0.25 (1)	
F-G	-1379 / 0	-84.9	-84.9	0.21 (1)	5.32	N-G	0 / 364	0.08 (1)	
G-H	-1580 / 0	-84.9	-84.9	0.33 (1)	4.94	M-G	0 / 282	0.06 (1)	
H-I	-1737 / 0	-84.9	-84.9	0.36 (1)	4.72	M-H	-275 / 0	0.29 (1)	
I-J	0 / 35	-84.9	-84.9	0.12 (1)	10.00	L-H	-211 / 9	0.10 (1)	
R-B	-1664 / 0	0.0	0.0	0.17 (1)	6.43	B-Q	0 / 1622	0.36 (1)	
K-I	-1669 / 0	0.0	0.0	0.17 (1)	6.42	L-I	0 / 1450	0.33 (1)	
R-Q	0 / 0	-18.5	-18.5	0.16 (4)	10.00				
Q-P	0 / 1596	-18.5	-18.5	0.33 (1)	10.00				
P-O	0 / 1295	-18.5	-18.5	0.26 (1)	10.00				
O-N	0 / 1295	-18.5	-18.5	0.26 (1)	10.00				
N-M	0 / 1223	-18.5	-18.5	0.24 (1)	10.00				
M-L	0 / 1414	-18.5	-18.5	0.28 (1)	10.00				
L-K	0 / 0	-18.5	-18.5	0.12 (4)	10.00				

TOTAL WEIGHT = 148 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.03")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (1.03")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.12")

CS1: TC=0.47/1.00 (B-C:1), BC=0.33/1.00 (P-Q:1)
WB=0.51/1.00 (C-P:1), SS=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

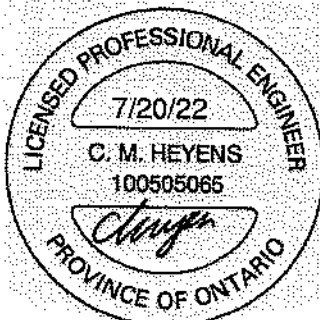
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070646

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
425143	T54S	1	1	ROYAL PINE HOMES		

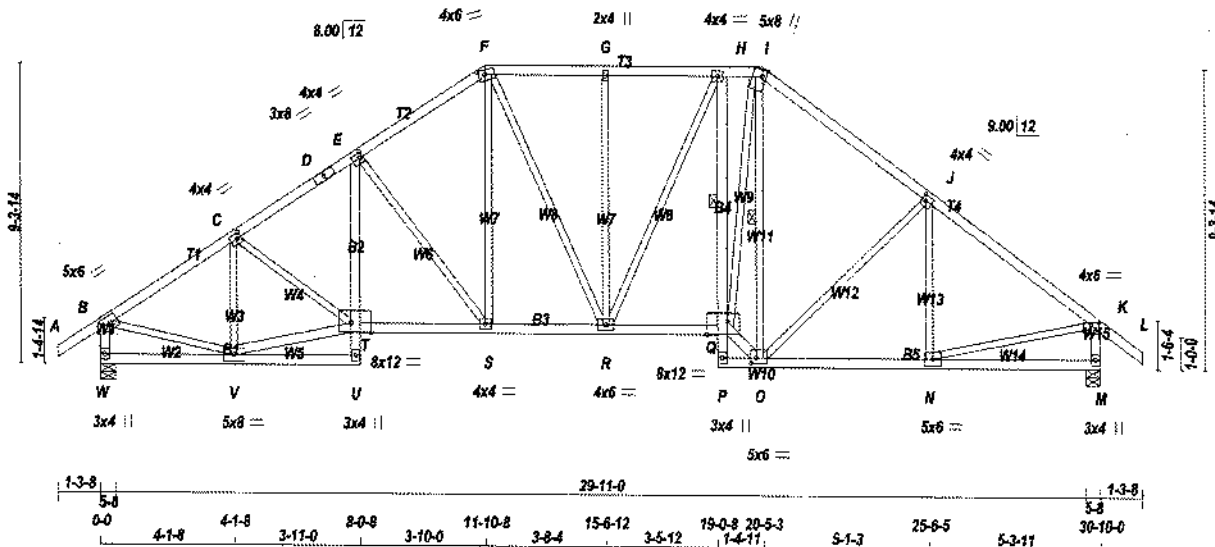
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:58:27 2022 Page 1

ID:9JKWqE1o0hmyukA0GZGHL1ZzKTS-Nznuo?pcVv7mhHEX5ubuL7vMCIEaKXvgkATW80YwCbq

1-3-8 0-0 4-1-8 4-1-8 3-11-0 8-0-8 3-10-0 11-10-8 3-8-4 15-6-12 3-5-12 19-0-8 20-5-3 5-1-3 25-6-5 5-3-11 30-10-0 32-1-8 1-3-8

Scale = 1:68.7



TOTAL WEIGHT = 174 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
W - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
W - U	2x4	DRY	No.2	SPF	
U - E	2x4	DRY	No.2	SPF	
T - Q	2x4	DRY	No.2	SPF	
P - H	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
V - T	2x4	DRY	No.2	SPF	
Q - O	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TTWW-m	MT20	4.0	6.0	1.75	2.50
G	TMVW-t	MT20	2.0	4.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	Edge	1.50
J	TMVW-t	MT20	4.0	4.0	2.00	1.50
K	TMVW-p	MT20	4.0	6.0	1.00	3.00
M	BMV1-p	MT20	3.0	4.0		
N	BMVW-t	MT20	5.0	6.0		
O	BMVW-t	MT20	5.0	6.0	2.00	2.00
P	BMV-p	MT20	3.0	4.0		
Q	BMVW-t	MT20	8.0	12.0	5.00	7.50
R	BMVW-t	MT20	4.0	6.0		
S	BMVW-t	MT20	4.0	4.0		
T	BMVW-t	MT20	8.0	12.0	3.50	7.50
U	BMV-p	MT20	3.0	4.0		
V	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
W	1710	0	0	5-8
M	1711	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
W	1212	782 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0
M	1212	782 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-Q, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	FACTORED	VERT. LOAD	LC1	MAX	MAX. MEMB.	FACTORED	MAX
	MEMB.	FORCE (LBS)	FROM	TO	CSI (LC)	MEMB.	FORCE (LBS)	CSI (LC)
FR-TO						FR-TO		
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	V-C	-724 / 0	0.19 (1)
B-C	-1813 / 0	-84.9	-84.9	0.21 (1)	4.79	V-T	0 / 1545	0.25 (1)
C-D	-2135 / 0	-84.9	-84.9	0.19 (1)	4.51	C-T	0 / 316	0.07 (1)
D-E	-2135 / 0	-84.9	-84.9	0.19 (1)	4.51	E-S	-550 / 0	0.46 (1)
E-F	-1760 / 0	-84.9	-84.9	0.19 (1)	4.86	S-F	0 / 503	0.11 (1)
F-G	-1547 / 0	-84.9	-84.9	0.16 (1)	5.15	Q-O	0 / 1546	0.25 (1)
G-H	-1431 / 0	-84.9	-84.9	0.16 (1)	5.15	Q-I	0 / 1380	0.31 (1)
H-I	-1431 / 0	-84.9	-84.9	0.04 (1)	5.44	C-I	-674 / 0	0.39 (1)
I-J	-1569 / 0	-84.9	-84.9	0.34 (1)	4.52	Q-J	-261 / 0	0.27 (1)
J-K	-1734 / 0	-84.9	-84.9	0.35 (1)	4.72	N-J	-230 / 3	0.10 (1)
K-L	0 / 35	-84.9	-84.9	0.12 (1)	10.00	B-V	0 / 1574	0.35 (1)
W-B	-1677 / 0	0.0	0.0	0.17 (1)	6.41	N-K	0 / 1447	0.33 (1)
M-K	-1666 / 0	0.0	0.0	0.17 (1)	6.42	F-R	0 / 226	0.05 (1)
						R-G	-373 / 0	0.51 (1)
W-V	0 / 0	-18.5	-18.5	0.07 (4)	10.00	R-H	0 / 265	0.06 (1)
V-U	0 / 32	-18.5	-18.5	0.08 (4)	10.00			
U-T	0 / 30	0.0	0.0	0.05 (1)	10.00			
T-E	0 / 294	0.0	0.0	0.09 (1)	10.00			
T-S	0 / 1782	-18.5	-18.5	0.32 (1)	10.00			
S-R	0 / 1455	-18.5	-18.5	0.27 (1)	10.00			
R-Q	0 / 1438	-18.5	-18.5	0.26 (1)	10.00			
P-Q	-76 / 0	0.0	0.0	0.06 (1)	7.81			
Q-H	-393 / 0	0.0	0.0	0.00 (1)	6.25			
P-O	0 / 57	-18.5	-18.5	0.15 (1)	10.00			
O-N	0 / 1412	-18.5	-18.5	0.36 (1)	10.00			
N-M	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (L1) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/960 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.16")

CSI: TC>0.35/1.00 (J-K:1), BC>0.36/1.00 (N-O:1), WB>0.51/1.00 (G-R:1), SI>0.17/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

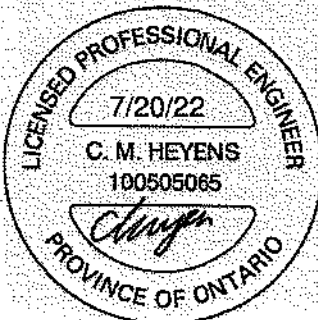
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070684 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425143	T54S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:58:28 2022 Page 2
ID:9JKWqE1o0hmyukA0GZGHL1zZkTS-rALG7LpEGGFdJRpftb67uCSXA8ap3_8p2gC3gSvwCbf

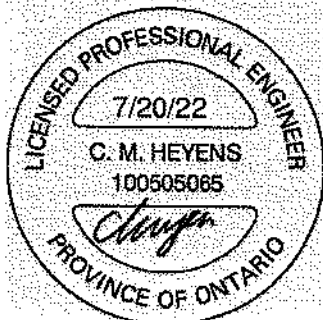
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



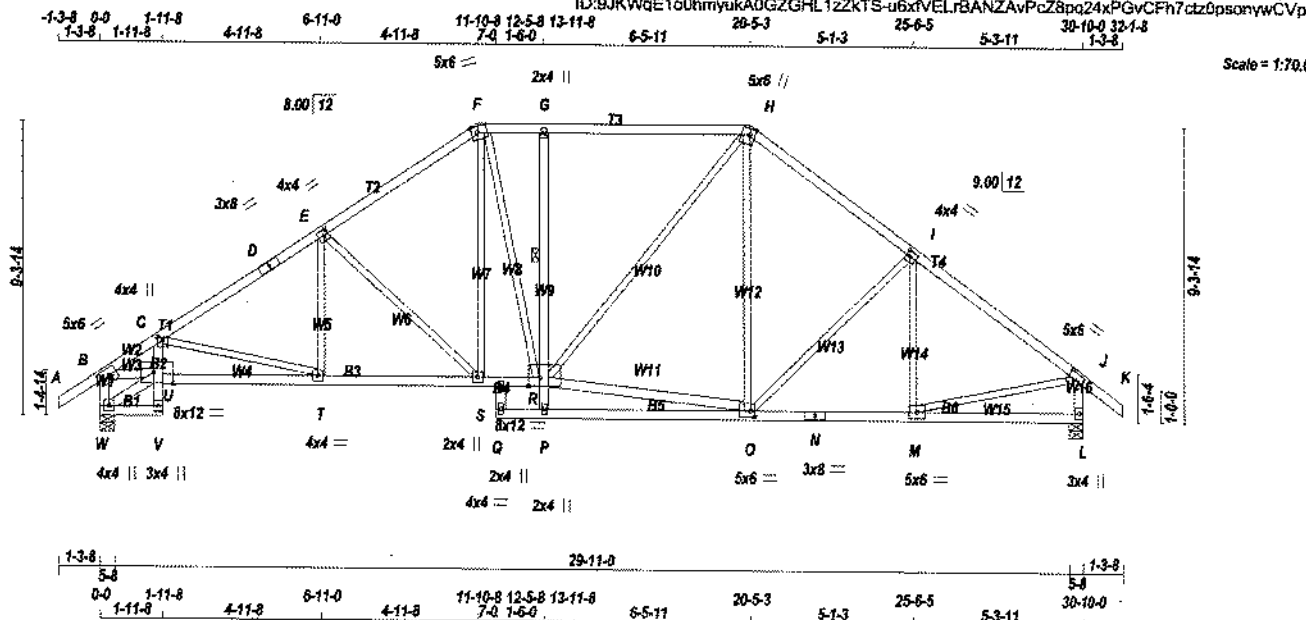
STRUCTURAL COMPONENT ONLY
DWG # TR22070684 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425144	T54S1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 11:04:42 2022 Page 1

ID:9JKWqE1o0hmyukA0GZGHL1ZKTS-uf6xfVELr8ANZAvPcZ8pq24xPGvCFh7ctz0psoywwCVp



TOTAL WEIGHT = 162 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
W - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
W - V	2x4	DRY	No.2	SPF	
V - C	2x4	DRY	No.2	SPF	
U - R	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
R - O	2x4	DRY	No.2	SPF	
W - U	2x4	DRY	No.2	SPF	
B - U	2x4	DRY	No.2	SPF	
P - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TS-t	MT20	3.0	8.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.50
F	TTVW-m	MT20	5.0	6.0	2.25	2.25
G	TMVW-w	MT20	2.0	4.0		
H	TTVW+m	MT20	5.0	6.0	2.25	1.75
I	TMVW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMVW-t	MT20	5.0	6.0	2.25	1.50
P	BMVW-w	MT20	2.0	4.0		
Q	NP-w	MT20	2.0	4.0		
R	BMVW-t	MT20	8.0	12.0	3.25	4.00
S	BMVW-t	MT20	4.0	4.0		
T	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UPLIFT		
L	1720	0	1720	0	5-8	5-8
W	1727	0	1727	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	1220	781/0	0/0	0/0	0/0	439/0	0/0
W	1225	783/0	0/0	0/0	0/0	442/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 6.25 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C-T	-450 / 0	0.21 (1)
B-C	-2704 / 0	-84.9	-84.9	0.16 (1)	4.10	T-E	0 / 159	0.05 (4)
C-D	-2250 / 0	-84.9	-84.9	0.34 (1)	4.26	E-S	-578 / 0	0.52 (1)
D-E	-2250 / 0	-84.9	-84.9	0.34 (1)	4.26	R-O	0 / 1223	0.20 (1)
E-F	-1790 / 0	-84.9	-84.9	0.31 (1)	4.71	R-H	0 / 534	0.12 (1)
F-G	-1573 / 0	-84.9	-84.9	0.37 (1)	4.84	O-H	0 / 127	0.04 (4)
G-H	-1574 / 0	-84.9	-84.9	0.45 (1)	4.74	O-I	-264 / 0	0.28 (1)
H-I	-1581 / 0	-84.9	-84.9	0.34 (1)	4.91	M-I	-225 / 0	0.10 (1)
I-J	-1747 / 0	-84.9	-84.9	0.35 (1)	4.71	M-J	0 / 1458	0.33 (1)
J-K	0 / 35	-84.9	-84.9	0.12 (1)	10.00	W-U	-99 / 0	0.01 (1)
W-B	-1668 / 0	0.0	0.0	0.17 (1)	6.44	B-U	0 / 2244	0.36 (1)
L-J	-1677 / 0	0.0	0.0	0.18 (1)	6.41	S-F	0 / 470	0.11 (1)
						P-R	0 / 86	0.05 (1)
W-V	0 / 84	-18.5	-18.5	0.03 (4)	10.00	R-G	-555 / 0	0.07 (1)
V-U	0 / 19	0.0	0.0	0.13 (1)	10.00	F-R	0 / 420	0.09 (1)
U-C	0 / 103	0.0	0.0	0.15 (1)	10.00			
U-T	0 / 2325	-18.5	-18.5	0.43 (1)	10.00			
T-S	0 / 1889	-18.5	-18.5	0.36 (1)	10.00			
S-R	0 / 1471	-18.5	-18.5	0.27 (1)	10.00			
Q-P	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
P-O	0 / 28	-18.5	-18.5	0.17 (4)	10.00			
O-N	0 / 1422	-18.5	-18.5	0.33 (1)	10.00			
N-M	0 / 1422	-18.5	-18.5	0.33 (1)	10.00			
M-L	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CSI: TC=0.45/1.00 (G-H-I), BC=0.43/1.00 (T-U-V), WB=0.52/1.00 (E-S-1), SS=0.25/1.00 (G-H-I)

DOL LUMBER=1.00 NAL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

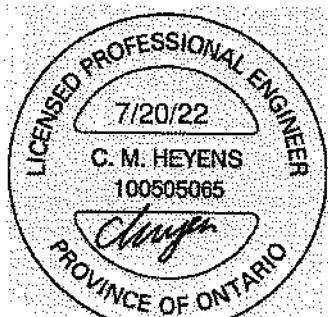
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.64 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070690 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425144	T54S1	1	1	TRUSS DESC.		

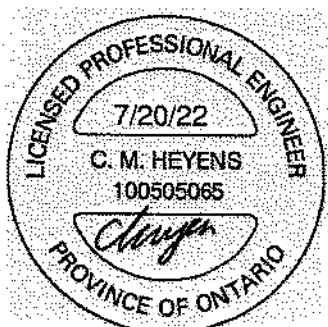
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 11:04:42 2022 Page 2
ID:9JKWqE1c0hmyukA0GZGHL1zZKTS-u6xfVELrBANZAvPcZ8pg24xPGvCFh7ctz0psonywcVp

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BVMWW-I	MT20	8.0	12.0	4.00	7.50
V	BMV+p	MT20	3.0	4.0		
W	BVMW1+p	MT20	4.0	4.0		
X	NP+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T55	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:51:12 2022 Page 2

ID:9JKWqE1c0hmvyukA0GZGHL1zZKTS-cH7RVGYIylJU?QTW61HuDGO2kOhGvmQel?b?NkywCIT

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW-m	MT20	6.0	7.0	2.75	2.00
G	TTW+m	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	8.0	2.50	3.25
I	TS-t	MT20	5.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TMVW-l	MT20	6.0	10.0		
M	BMV1-t	MT20	6.0	10.0	Edge	0.50
N	BMWW-t	MT20	5.0	8.0	2.50	2.25
O	BMWW-H	MT20	4.0	10.0	4.25	2.00
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0	2.50	2.00
R	BMWW-t	MT20	4.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

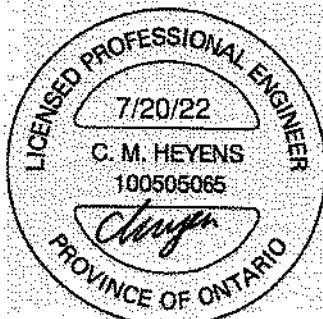
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	25-0-4	-529	-529	—	BACK	VERT	TOTAL	—	C1
X	29-0-4	-867	-867	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22070647 PG 2/2

TOTAL WEIGHT = 2 X 24' = 495 lb

(M)

LUMBER	N.L.G.A. RULES			
CHORDS	SIZE	LUMBER		DESCR.
A - D	2x6	DRY	No.2	SF
D - F	2x6	DRY	No.2	SF
F - G	2x6	DRY	No.2	SF
G - J	2x6	DRY	No.2	SF
J - M	2x6	DRY	No.2	SF
Y - B	2x6	DRY	No.2	SF
N - L	2x6	DRY	No.2	SF
Y - W	2x6	DRY	No.2	SF
W - E	2x4	DRY	No.2	SF
V - R	2x6	DRY	No.2	SF
T - N	2x6	DRY	No.2	SF
ALL WEBS EXCEPT X - V & R - P	2x4 DRY	No.2	SF	SF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 2	12	TOP
D-F 2	12	TOP
F-G 2	12	TOP
G-J 2	12	TOP
J-M 2	12	TOP
Y-B 2	12	TOP
N-L 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Y-W 2	12	TOP
V-R 2	12	TOP
T-N 2	12	SIDE(183.1)
W-E 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
I-P 1	3	SIDE(1080.7)
K-O 1	6	SIDE(12.1)
2x4 1	6	
2x6 2	6	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ		
Y	3246	0	3246	0	5-8	5-8
N	7783	0	7783	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN COMPONENT REACTIONS		DEAD	SOIL
	SNOW	LIVE	PERMALIVE	WIND		
Y	2300	1485 / 0	0 / 0	0 / 0	815 / 0	0 / 0
N	5508	3593 / 0	0 / 0	0 / 0	1914 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT H-S, G-U

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	-94.9	X-C	-1547 / 0
B-C	-3950 / 0	X-V	0 / 3359
C-D	-5024 / 0	C-V	0 / 1100
D-E	-8024 / 0	E-U	-711 / 0
E-F	-4453 / 0	U-F	0 / 2024
F-G	-3719 / 0	P-I	0 / 2868
G-H	-6388 / 0	P-K	-480 / 0
H-I	-6905 / 0	O-K	0 / 336
I-J	-8169 / 0	B-X	0 / 3369
J-K	-8169 / 0	O-L	0 / 7098
K-L	-8619 / 0	Q-R	0 / 86
L-M	0 / 36	R-H	0 / 5133
Y-B	-3199 / 0	R-P	0 / 6964
N-L	-7201 / 0	R-I	-2834 / 0
Y-X	0 / 0	S-H	-3744 / 0
X-W	0 / 49	S-G	0 / 4006
W-V	0 / 45	U-G	-1834 / 0
V-E	0 / 367		
U-U	0 / 4203		
U-S	0 / 4733		
S-R	0 / 5191		
T-Q	0 / 0		
Q-P	0 / 15		
P-Z	0 / 6885		
Z-O	0 / 6885		
O-AA	0 / 0		
AA-N	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	35.7	PSF

JOB NAME 425143	TRUSS NAME T55S	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:58:29 2022 Page 2
ID:9JKWqE1o0hmvyukA0GZGHL1zZKTS-JMveDhgt1ZNUwaOwDldMRQ_gMYrioNyzCUydDwywCbe

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMVW+p	MT20	4.0	6.0		
F	TTW+m	MT20	5.0	6.0		
G	TTWV+m	MT20	6.0	7.0	3.00	2.00
H	TMVW-t	MT20	5.0	6.0	2.50	3.25
I	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	TS-t	MT20	5.0	6.0		
K	TMVW-t	MT20	5.0	6.0	2.50	2.75
L	TMVW-t	MT20	6.0	10.0		
N	BMV1-t	MT20	6.0	10.0	Edge	0.50
O	BMVW-t	MT20	5.0	8.0	2.50	2.25
P	BMVWV-t	MT20	8.0	9.0	4.25	3.25
Q	BMV+w	MT20	3.0	8.0		
R	BMVWV-t	MT20	10.0	12.0	5.50	7.25
S						
S	BMVWV+p	MT20	4.0	12.0	4.50	2.00
T	NP+w	MT20	3.0	8.0	Edge	1.50
U	BMVWV-t	MT20	5.0	8.0		
V	BMVWV-t	MT20	10.0	12.0	5.50	7.25
W	BMV+p	MT20	3.0	8.0		
X	BMVWV-t	MT20	6.0	10.0	2.75	5.00
Y	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

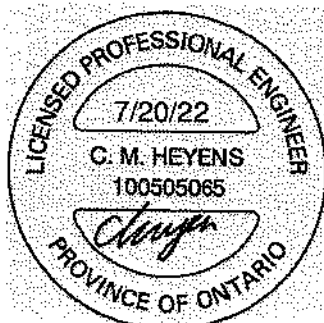
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	27-0-4	-529	-529	—	BACK	VERT	TOTAL	—	C1
P	22-11-8	-3431	-3431	—	BACK	VERT	TOTAL	—	C1
Z	25-0-4	-529	-529	—	BACK	VERT	TOTAL	—	C1
AA	29-0-4	-867	-867	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



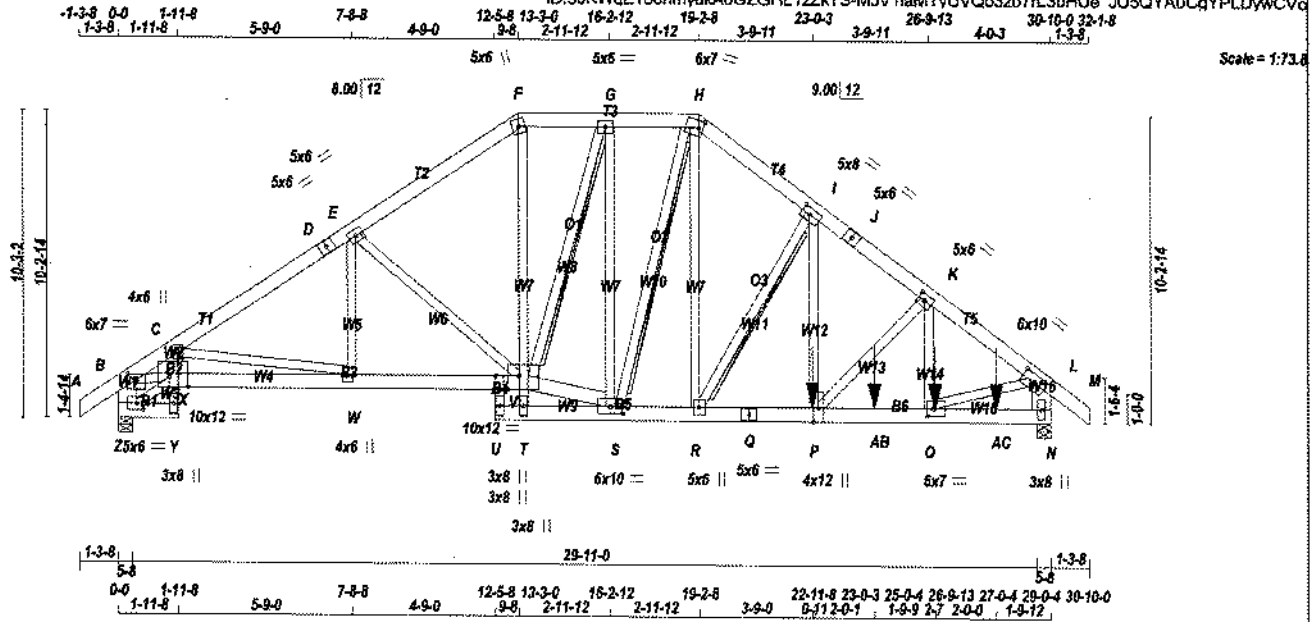
STRUCTURAL COMPONENT ONLY
DWG # TR22070685 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425144	T55S1	1	2	TRUSS DESC.		

Tamareck Roof Truss, Burlington

ID:9JKWqE1o0hmyukADGZGHL1ZKTS-MJVF1iaMTyUVQo3zo7L3bHJc JU5QYADcQYPLDyWCvQ

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 11:04:43 2022 Page 1



TOTAL WEIGHT = 2 X 245 = 491 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6 DRY	No.2	SPF		
D - F	2x6 DRY	No.2	SPF		
F - H	2x6 DRY	No.2	SPF		
H - J	2x6 DRY	No.2	SPF		
J - M	2x6 DRY	No.2	SPF		
Z - B	2x8 DRY	No.2	SPF		
N - L	2x8 DRY	No.2	SPF		
Z - Y	2x6 DRY	No.2	SPF		
X - V	2x4 DRY	No.2	SPF		
U - Q	2x6 DRY	No.2	SPF		
Q - N	2x6 DRY	No.2	SPF		
ALL WEBS EXCEPT V - S	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 2 12	TOP	
D-F 2 12	TOP	
F-H 2 12	TOP	
H-J 2 12	TOP	
J-M 2 12	TOP	
Z-B 2 12	TOP	
N-L 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Z-Y 2 12	TOP	
X-V 2 12	TOP	
U-Q 2 12	TOP	
Q-N 2 12	TOP	
Y-C 1 12	SIDE(183.1)	
WEBS : (0.122"x3") SPIRAL NAILS		
I-P 1 3	SIDE(1080.7)	
K-O 1 6	SIDE(12.1)	
2x4 1 6		
2x6 2 6		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ			
N	7755	0	7755	0	5-8
Z	3266	0	3266	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	5487	3885 / 0	0 / 0	0 / 0	0 / 0	1902 / 0	0 / 0
Z	2314	1494 / 0	0 / 0	0 / 0	0 / 0	820 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, Z

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT H-S, G-V
2x6 DRY SPF No.2 T-BRACE AT I-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 80% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	
FR-TO		FROM TO	CS1 (LC)	FR-TO		CS1 (LC)	
A-B	0 / 33	-84.9 -84.9	0.03 (1)	10.00	C-W	-701 / 0 0.16 (1)	
B-C	-5819 / 0	-84.9 -84.9	0.12 (1)	4.82	W-E	0 / 203 0.02 (4)	
C-D	-5061 / 0	-84.9 -84.9	0.18 (1)	5.03	E-V	-731 / 0 0.29 (1)	
D-E	-5081 / 0	-84.9 -84.9	0.18 (1)	5.03	S-G	0 / 164 0.01 (1)	
E-F	-4491 / 0	-84.9 -84.9	0.16 (1)	5.29	S-H	-1235 / 0 0.59 (1)	
F-G	-3745 / 0	-84.9 -84.9	0.05 (1)	5.80	R-H	0 / 4018 0.36 (1)	
G-H	-3667 / 0	-84.9 -84.9	0.05 (1)	5.73	R-I	-4704 / 0 0.48 (1)	
H-I	-6208 / 0	-84.9 -84.9	0.10 (1)	5.05	P-I	0 / 5456 0.48 (1)	
I-J	-8105 / 0	-84.9 -84.9	0.18 (1)	4.14	P-K	-528 / 0 0.09 (1)	
J-K	-6105 / 0	-84.9 -84.9	0.18 (1)	4.14	O-K	0 / 374 0.03 (1)	
K-L	-8589 / 0	-84.9 -84.9	0.20 (1)	4.03	O-L	0 / 7071 0.63 (1)	
L-M	0 / 36	-84.9 -84.9	0.03 (1)	10.00	T-V	-49 / 13 0.01 (1)	
Z-B	-3181 / 0	0.0 0.0	0.08 (1)	7.81	V-F	0 / 2164 0.20 (1)	
N-L	-7178 / 0	0.0 0.0	0.20 (1)	6.29	V-S	0 / 4661 0.26 (1)	
					V-G	-394 / 0 0.14 (1)	
Z-Y	0 / 170	-18.5 -18.5	0.01 (1)	10.00	Z-X	-187 / 0 0.01 (1)	
Y-X	0 / 19	0.0 0.0	0.13 (1)	10.00	B-X	0 / 4787 0.42 (1)	
X-C	0 / 387	0.0 0.0	0.17 (1)	10.00			
X-W	0 / 4940	-18.5 -18.5	0.35 (1)	10.00			
W-V	0 / 4251	-18.5 -18.5	0.30 (1)	10.00			
U-T	0 / 0	-18.5 -18.5	0.01 (4)	10.00			
T-S	0 / 18	-18.5 -18.5	0.10 (1)	10.00			
S-R	0 / 4211	-18.5 -18.5	0.38 (1)	10.00			
R-Q	0 / 6496	-18.5 -18.5	0.48 (1)	10.00			
Q-P	0 / 6496	-18.5 -18.5	0.48 (1)	10.00			
P-AB	0 / 6859	-18.5 -18.5	0.65 (1)	10.00			
AB-O	0 / 6859	-18.5 -18.5	0.85 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CS1: TC=0.20/1.00 (K-L:1), BC=0.65/1.00 (O-P:1)
WB=0.63/1.00 (L-O:1), SS=0.24/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.79 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070691 PG 1/2

JOB NAME 425144	TRUSS NAME T55S1	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Jul 20 11:04:43 2022 Page 2
ID:9JKWgE1o0hmyukA0GZGHL1zZkTS-MJV1aMTyUVQo3zo7nL3bHue JU5QYA0CqYPLDywCVo

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	6.0	7.0	2.00	3.75
C	TMWW+p	MT20	4.0	6.0		
D	TS-1	MT20	5.0	6.0		
E	TMWW-1	MT20	5.0	6.0		
F	TTW+m	MT20	5.0	6.0		
G	TMWW-1	MT20	5.0	6.0		
H	TTWW-m	MT20	6.0	7.0	2.75	1.75
I	TMWW-1	MT20	5.0	6.0	2.50	3.25
J	TS-1	MT20	5.0	6.0		
K	TMWW-1	MT20	5.0	6.0	2.50	2.75
L	TMWW-1	MT20	6.0	10.0	3.00	4.50
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-1	MT20	6.0	7.0	3.00	2.50
P	BMWW-1	MT20	4.0	12.0	Edge	
Q	BS-1	MT20	5.0	6.0		
R	BMWW-1	MT20	5.0	6.0		
S	BMWW-1	MT20	6.0	10.0	2.75	5.00
T	BMW-w	MT20	3.0	6.0		
U	NP+w	MT20	3.0	6.0		
V	BMWW-1	MT20	10.0	12.0	Edge	4.50
W	BMWW-1	MT20	4.0	6.0		
X	BMWW-1	MT20	10.0	12.0	4.00	7.50
Y	BMV+p	MT20	3.0	6.0		
Z	BMWW-1	MT20	5.0	6.0	2.50	2.25
AA	NP+w	MT20	3.0	6.0	Edge	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX	MAX. MEMB. FORCE (LBS)	FACTORED UNBRAC. LENGTH FR-TO	MAX. MEMB. FORCE (LBS)	LC1 MAX
FR-TO		FROM	TO				
O-AC	0/0	-18.5	-18.5	0.37 (1)	10.00		
AC-N	0/0	-18.5	-18.5	0.37 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	27-0-4	-529	-529	---	BACK	VERT	TOTAL	---	C1
P	22-11-8	-3431	-3431	---	BACK	VERT	TOTAL	---	C1
AB	25-0-4	-529	-529	---	BACK	VERT	TOTAL	---	C1
AC	29-0-4	-867	-867	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22070691 PG 2/2

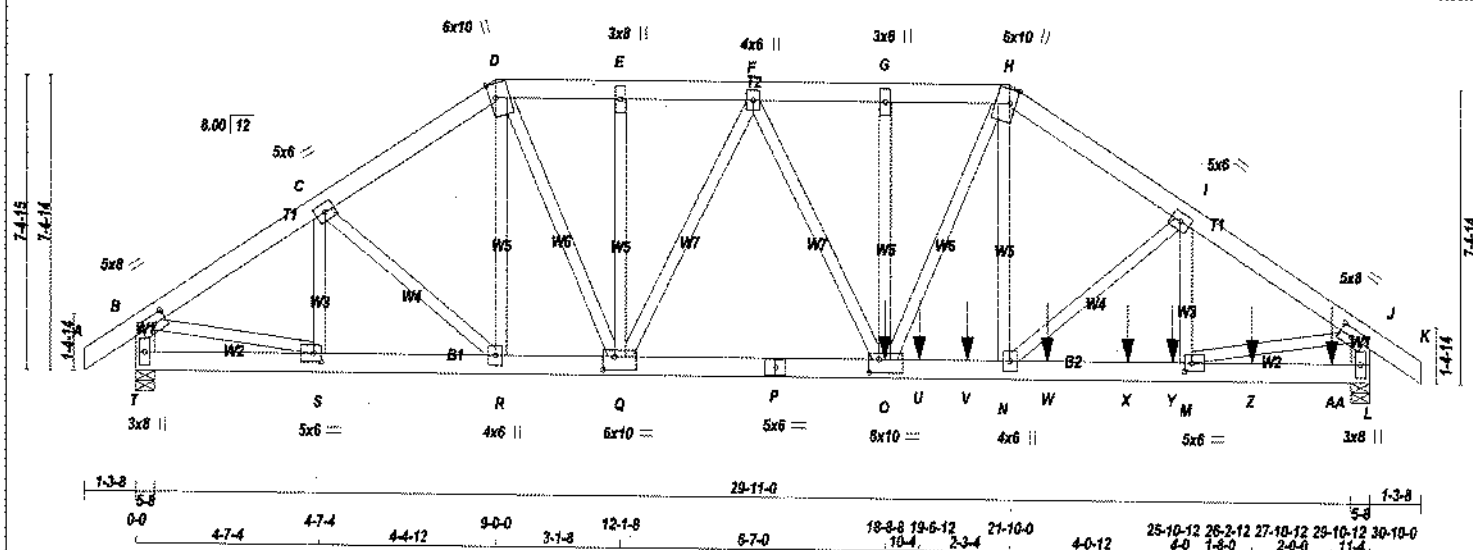
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425142	T56	1	2	ROYAL PINE HOMES	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 12:08:12 2022 Page 1

ID:8JKWqE10hnyukA0GZGHL1zZkTS-aengKWAMoEvStFOHvFMuBDr6w1PRLGV16GeoTywBZL

Scale = 1.557



TOTAL WEIGHT = 2 X 218 = 437 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	BEARINGS			
A - D	2x6	DRY	No.2	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
D - H	2x6	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG
H - K	2x6	DRY	No.2	JT VERT	DOWN	UP	IN-SX
T - B	2x6	DRY	No.2	T 3181	0	0	5-8
L - J	2x6	DRY	No.2	L 4937	0	0	5-8
T - P	2x6	DRY	No.2				
P - L	2x6	DRY	No.2				
ALL WEBS	2x4	DRY	No.2	UNFACTORED REACTIONS			
EXCEPT			SPF	1ST LCASE MAX/MIN COMPONENT REACTIONS			
				JT COMBINED	SNOW	LIVE	PERM. LIVE
				T 2248	1487 / 0	0 / 0	0 / 0
				L 3477	2363 / 0	0 / 0	0 / 0

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

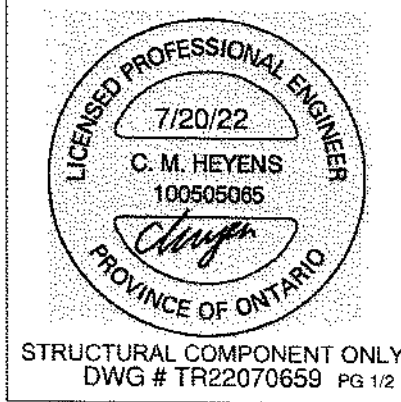
TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS

TOP CHORDS : (0.122"x3") SPIRAL NAILS



STRUCTURAL COMPONENT ONLY
DWG # TR22070659 PG 1/2

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FACTORED

VERT. LOAD LC1 MAX

MAX. FACTORED

WEBS

MAX. FACTORED

FORCE (LBS)

MAX. FACTORED

CS (LC)

UNBRACED

LENGTH FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

FR-TO

JOB NAME 425142	TRUSS NAME T56	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.630 S Feb 23 2022 Mittek Industries, Inc. Wed Jul 20 12:09:12 2022 Page 2		ID: 9JKWqE1c0hmYukA0GZGHL1zZkTS-aengKWAMcEySTeFOHvFMuBDrw1PRLGV16GeeTywBZL	

Tamarack Roof Truss, Burlington

Version 8.630 S Feb 23 2022 Mittek Industries, Inc. Wed Jul 20 12:09:12 2022 Page 2

ID: 9JKWqE1c0hmYukA0GZGHL1zZkTS-aengKWAMcEySTeFOHvFMuBDrw1PRLGV16GeeTywBZL

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	8.0	2.50	3.75
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	6.0	10.0	Edge	1.75
E	TMWW+w	MT20	3.0	8.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMWW+w	MT20	3.0	8.0		
H	TTWW+m	MT20	6.0	10.0	Edge	1.75
I	TMWW-t	MT20	5.0	6.0		
J	TMWW-t	MT20	5.0	8.0	2.50	3.75
L	BMV1+p	MT20	3.0	8.0		
M	BMWW-t	MT20	5.0	8.0	2.50	2.25
N	BMWW+t	MT20	4.0	6.0		
O	BMWWW-t	MT20	6.0	10.0	4.00	3.00
P	SS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	6.0	10.0	4.00	3.50
R	BMWW+t	MT20	4.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.25
T	BMV1+p	MT20	3.0	6.0		

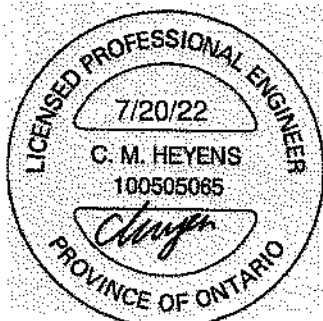
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

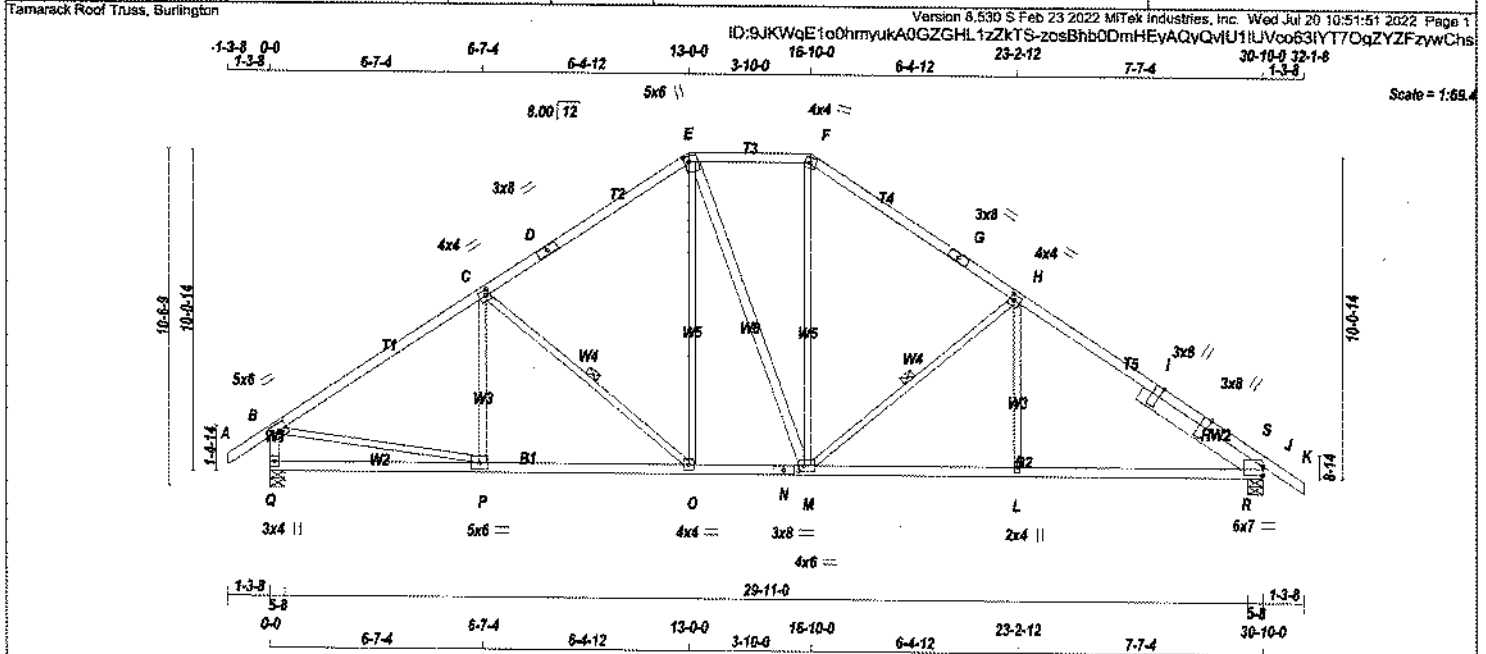
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22070659 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425142	T57	1	1	ROYAL PINE HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



TOTAL WEIGHT = 144 lb

[M/F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - K	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - J	2x4	DRY	No.2

REINFORCING MEMBERS			
HW2	2x6	DRY	No.2
ALL WEBS EXCEPT E - M	2x3	DRY	No.2
	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	6.0		
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	8.0		
E TTWW+m	MT20	5.0	6.0	2.00	1.50
F TTW-m	MT20	4.0	4.0		
G TS-t	MT20	3.0	8.0		
H TMVW-t	MT20	4.0	4.0	2.00	1.50
J TMBMR-t	MT20	6.0	7.0	3.50	
J RT-t	MT20	3.0	8.0	Edge	25.00
J RT-t	MT20	3.0	8.0	Edge	46.25
L BMVW-w	MT20	2.0	4.0		
M BMVW-w	MT20	4.0	6.0	2.00	2.00
N BS-t	MT20	3.0	8.0		
O BMVW-t	MT20	4.0	4.0		
P BMVW-t	MT20	5.0	6.0		
Q BMV-t+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQ'D
JT	VERT	DOWN	BRG	BRG
Q	1710	0	5-8	5-8
J	1710	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1212	782/0	0/0	0/0	0/0	430/0	0/0
J	1212	782/0	0/0	0/0	0/0	430/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0/32	-84.9 -84.9	0.11 (1)	10.00	P-C	-124/63	0.06 (1)
B-C	-1887/0	-84.9 -84.9	0.56 (1)	4.27	C-O	-489/0	0.24 (1)
C-D	-1513/0	-84.9 -84.9	0.53 (1)	4.72	O-E	0/403	0.09 (1)
D-E	-1513/0	-84.9 -84.9	0.53 (1)	4.72	E-M	0/56	0.01 (1)
E-F	-1251/0	-84.9 -84.9	0.18 (1)	5.57	M-F	0/456	0.10 (1)
F-G	-1530/0	-84.9 -84.9	0.48 (1)	4.79	M-H	-854/0	0.32 (1)
G-H	-1530/0	-84.9 -84.9	0.48 (1)	4.79	L-H	0/151	0.05 (4)
H-I	-1896/0	-84.9 -84.9	0.49 (1)	4.64	B-P	0/1622	0.36 (1)
I-S	-1896/0	-84.9 -84.9	0.49 (1)	4.64	R-S	0/603	0.09 (1)
S-J	-1910/0	-84.9 -84.9	0.11 (1)	4.79	R-I	-842/0	0.12 (1)
J-K	0/20	-84.9 -84.9	0.11 (1)	10.00			
Q-B	-1660/0	0.0 0.0	0.17 (1)	6.44			
Q-P	0/0	-18.5 -18.5	0.20 (4)	10.00			
P-O	0/1600	-18.5 -18.5	0.36 (1)	10.00			
O-N	0/1231	-18.5 -18.5	0.25 (1)	10.00			
N-M	0/1231	-18.5 -18.5	0.25 (1)	10.00			
M-L	0/1746	-18.5 -18.5	0.40 (1)	10.00			
L-R	0/1746	-18.5 -18.5	0.40 (1)	10.00			
R-J	0/1127	-18.5 -18.5	0.24 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.58/1.00 (B-C:1), BC=0.40/1.00 (L-R:1), WB=0.36/1.00 (B-P:1), SS=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (M) (INPUT = 0.90)

JSI METAL= 0.68 (J) (INPUT = 1.00)

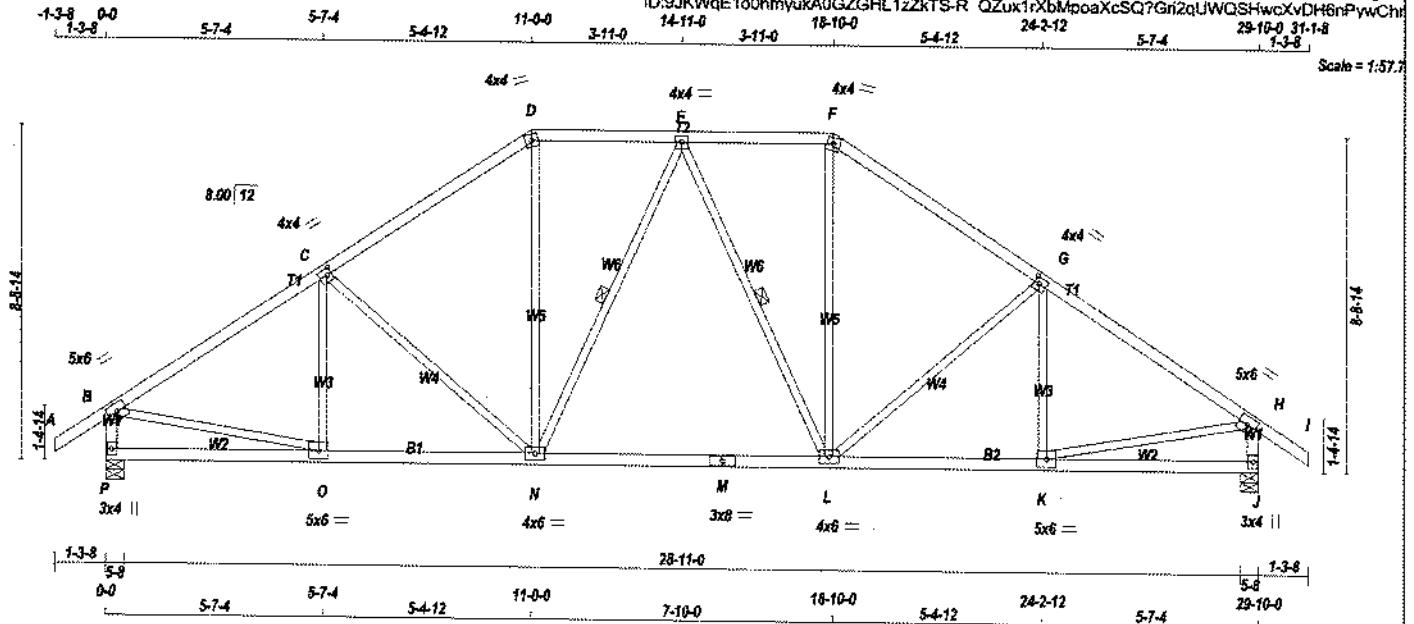
STRUCTURAL COMPONENT ONLY
DWG # TR22070660

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

JOB NAME 425142	TRUSS NAME T58	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
--------------------	-------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 10:51:52 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL1zZkTS-R QZux1rXbMpoaXcSQ7Gn2qUWQSHwcXvDH6nPywChf



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
J - L	2x4	DRY	No.2	SPF	
L - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	6.0		
C	TMVW-1	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW-1	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-1	MT20	4.0	4.0	2.00	1.50
H	TMVW-1	MT20	5.0	6.0		
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-1	MT20	5.0	6.0		
L	BMVW-1	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-1	MT20	4.0	6.0		
O	BMVW-1	MT20	5.0	6.0		
P	BMV1-p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	UPLIFT		
P	1858	0	1858	0	5-8	5-8
J	1858	0	1858	0	5-8	5-8

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
P	1175	759 / 0	0 / 0	0 / 0	0 / 0	416 / 0	0 / 0
J	1175	759 / 0	0 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 32	-84.9	-84.9 0.11 (1)	10.00
B-C	-1805 / 0	-84.9	-84.9 0.39 (1)	4.59
C-D	-1586 / 0	-84.9	-84.9 0.37 (1)	4.88
D-E	-1281 / 0	-84.9	-84.9 0.17 (1)	5.52
E-F	-1281 / 0	-84.9	-84.9 0.17 (1)	5.52
F-G	-1586 / 0	-84.9	-84.9 0.37 (1)	4.88
G-H	-1805 / 0	-84.9	-84.9 0.39 (1)	4.59
H-I	0 / 32	-84.9	-84.9 0.11 (1)	10.00
P-B	-1613 / 0	0.0	0.0 0.17 (1)	6.51
J-H	-1613 / 0	0.0	0.0 0.17 (1)	6.51
P-O	0 / 0	-18.5	-18.5 0.12 (4)	10.00
O-N	0 / 1527	-18.5	-18.5 0.35 (1)	10.00
N-M	0 / 1371	-18.5	-18.5 0.32 (1)	10.00
M-L	0 / 1371	-18.5	-18.5 0.32 (1)	10.00
L-K	0 / 1527	-18.5	-18.5 0.35 (1)	10.00
K-J	0 / 0	-18.5	-18.5 0.12 (4)	10.00

TOTAL WEIGHT = 4 X 135 = 539 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.99")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.99")
CALCULATED VERT. DEFL. (TL) = L/999 (0.16")

CSI: TC=0.39/1.00 (B-C:1), BC=0.35/1.00 (N-O:1)
WB=0.35/1.00 (C-N:1), SS=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

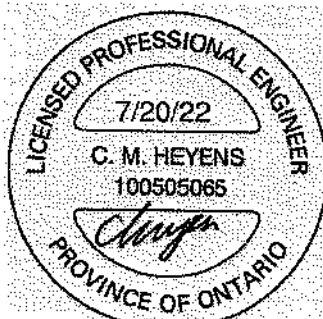
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.77 (H) (INPUT = 0.90)
JSI METAL= 0.52 (M) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070661

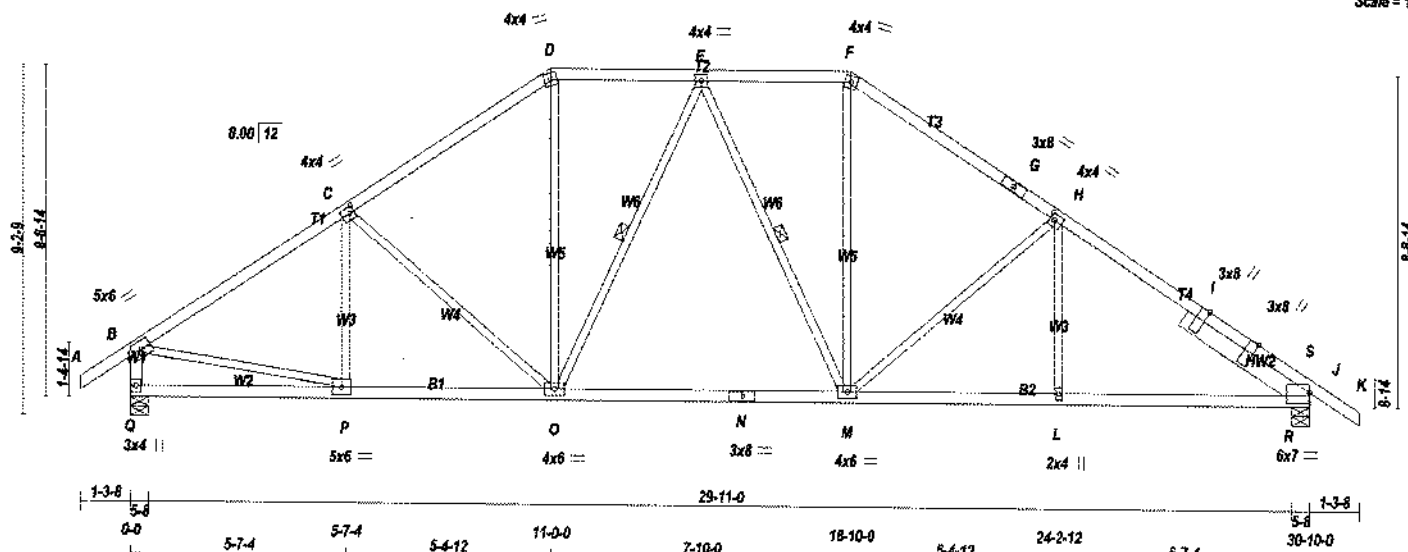
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T58X	4	1	TRUSS DESC.		

Tamereck Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:51:53 2022 Page 1

ID:9JKWqE1o0hmyukADGZGHL1zZkTS-vAzx6H1TluUgCk6p08WVNB79w140K2q81qJsywChq

1-3-8 0-0 5-7-4 5-7-4 5-4-12 11-0-0 3-11-0 14-11-0 3-11-0 18-10-0 5-4-12 24-2-12 6-7-4 30-10-0 32-1-8 1-3-8
Scale = 1/58.3



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - K	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
O - N	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	

REINFORCING MEMBERS	HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW-t	MT20	5.0	6.0		
C TMWW-t	MT20	4.0	4.0	2.00	1.50
D TTW-m	MT20	4.0	4.0		
E TMWW-t	MT20	4.0	4.0		
F TTW-m	MT20	4.0	4.0		
G TS-t	MT20	3.0	8.0		
H TMWW-t	MT20	4.0	4.0	2.00	1.50
J TMBMR-t	MT20	6.0	7.0	3.50	
K RT-t	MT20	3.0	8.0	Edge 21.50	
L RT-t	MT20	3.0	8.0	Edge 38.75	
M BMWW-t	MT20	2.0	4.0		
N BMWW-t	MT20	4.0	6.0		
O BMWW-t	MT20	3.0	8.0		
P BMWW-t	MT20	5.0	6.0		
Q BMV-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
Q	1710	0	1710	0	5-8	5-8
J	1710	0	1710	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS			
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
Q	1212	782 / 0	0 / 0	0 / 0	0 / 0	430 / 0
J	1212	782 / 0	0 / 0	0 / 0	0 / 0	430 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
A-B	0 / 32	-84.9	-84.9	0.11 (1)
B-C	-1877 / 0	-84.9	-84.9	0.40 (1)
C-D	-1647 / 0	-84.9	-84.9	0.37 (1)
D-E	-1350 / 0	-84.9	-84.9	0.17 (1)
E-F	-1396 / 0	-84.9	-84.9	0.17 (1)
F-G	-1694 / 0	-84.9	-84.9	0.34 (1)
G-H	-1694 / 0	-84.9	-84.9	0.34 (1)
H-I	-1700 / 0	-84.9	-84.9	0.31 (1)
I-S	-1700 / 0	-84.9	-84.9	0.31 (1)
S-J	-1959 / 0	-84.9	-84.9	0.06 (1)
J-K	0 / 20	-84.9	-84.9	0.11 (1)
Q-B	-1065 / 0	0.0	0.0	0.17 (1)
Q-P	0 / 0	-18.5	-18.5	0.12 (4)
P-O	0 / 1587	-18.5	-18.5	0.36 (1)
O-N	0 / 1462	-18.5	-18.5	0.34 (1)
N-M	0 / 1462	-18.5	-18.5	0.34 (1)
M-L	0 / 1770	-18.5	-18.5	0.38 (1)
L-R	0 / 1770	-18.5	-18.5	0.36 (1)
R-J	0 / 1108	-18.5	-18.5	0.24 (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CSI: TC=0.40/1.00 (B-C-1), BC=0.36/1.00 (L-M-1)
WB=0.53/1.00 (H-M-1), SS=0.19/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

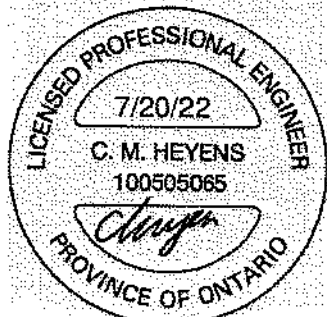
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (B) (INPUT = 0.90)
JSI METAL = 0.69 (J) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070662

JOB NAME 425142	TRUSS NAME T59	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
--------------------	-------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:51:54 2022 Page 2
ID:9JkWqE1o0hmyukA0GZGHL1zZkTS-NMXJJ4253CcX1th?ar2kw77D?K1CiuqNXmDrlywChp

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	6.0	10.0	3.00	4.75
C	TMWW-I	MT20	5.0	6.0		
D	TMWW-I	MT20	5.0	6.0	2.50	1.50
E	TTWW-m	MT20	6.0	7.0	2.75	2.00
F	TMVW-w	MT20	3.0	8.0		
G	TTWW-m	MT20	6.0	7.0	2.75	2.00
H	TMWW-I	MT20	5.0	6.0	2.50	1.50
I	TMWW-I	MT20	5.0	6.0		
J	TMVW-I	MT20	6.0	10.0	3.00	4.75
L	BMV1+p	MT20	3.0	8.0		
M	BMWW-I	MT20	6.0	7.0	3.00	2.50
N	BMWW-I	MT20	4.0	10.0	4.25	2.00
O	BMWW-I	MT20	5.0	6.0		
P	BS-I	MT20	5.0	6.0		
Q	BMWWW-I	MT20	5.0	8.0		
R	BMWW-I	MT20	5.0	6.0		
S	BMWW-I	MT20	4.0	10.0	4.25	2.00
T	BMWW-I	MT20	6.0	7.0	3.00	2.50
U	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	27-0-4	-529	-529	—	FRONT	VERT	TOTAL	—	C1
X	29-0-4	-869	-869	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

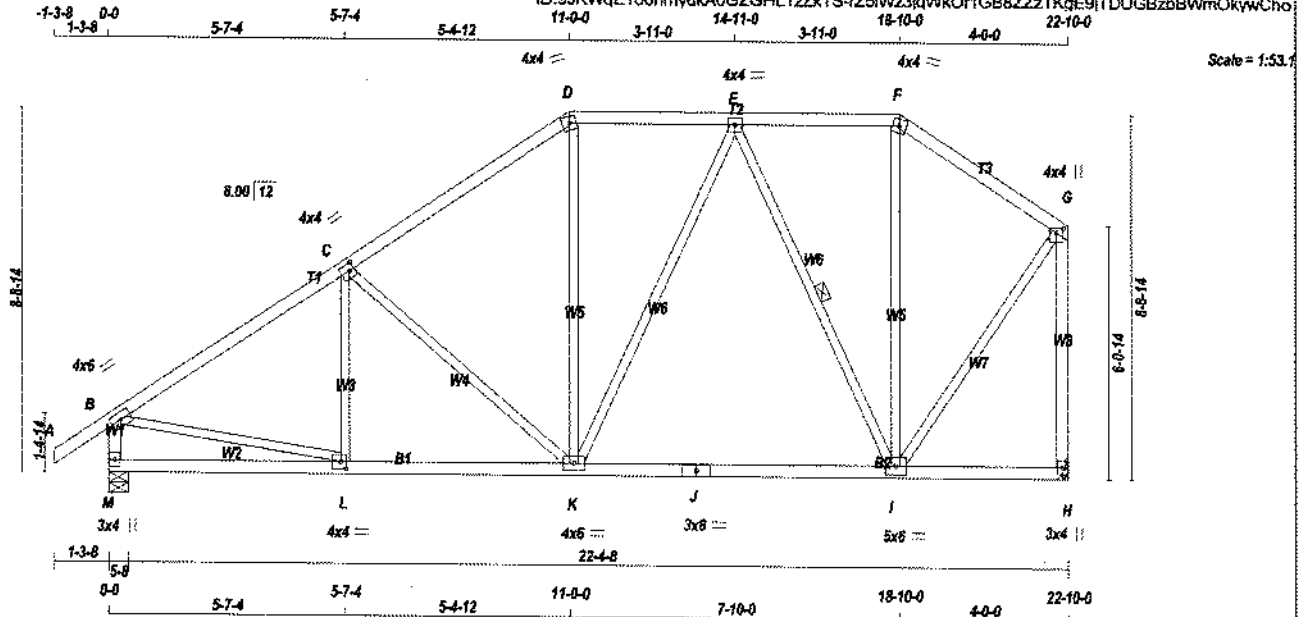


STRUCTURAL COMPONENT ONLY
DWG # TR22070663 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T60	7	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:51:55 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL1zZKTS-rZ5Wz3jqWkOF1GB8ZZzTKgE9jTDUGBzb8WmOkYwCho



TOTAL WEIGHT = 7 X 112 = 786 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - G	2x4	DRY	No.2 SPF
M - B	2x4	DRY	No.2 SPF
H - G	2x4	DRY	No.2 SPF
M - J	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVWV-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMVWV-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	4.0	2.00	1.50
M	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG
M	1207	0	0	1297	0	0	5-8	5-8	
H	1180	0	0	1180	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	918	598 / 0	0 / 0	0 / 0	0 / 0	322 / 0	0 / 0
H	838	532 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.9 -84.9	0.11 (1)	10.00	L-C	-110 / 42	0.04 (1)
B-C	-1301 / 0	-84.9 -84.9	0.36 (1)	5.25	C-K	-408 / 0	0.42 (1)
C-D	-998 / 0	-84.9 -84.9	0.34 (1)	5.82	K-D	0 / 236	0.05 (1)
D-E	-805 / 0	-84.9 -84.9	0.17 (1)	6.25	K-E	0 / 175	0.04 (1)
E-F	-478 / 0	-84.9 -84.9	0.16 (1)	6.25	E-I	-821 / 0	0.38 (1)
F-G	-575 / 0	-84.9 -84.9	0.18 (1)	6.25	I-F	0 / 61	0.02 (4)
M-B	-1252 / 0	0.0 0.0	0.13 (1)	7.19	B-L	0 / 1128	0.25 (1)
H-G	-1160 / 0	0.0 0.0	0.08 (1)	7.40	I-G	0 / 823	0.19 (1)
M-L	0 / 0	-18.5 -18.5	0.12 (4)	10.00			
L-K	0 / 1108	-18.5 -18.5	0.28 (1)	10.00			
K-J	0 / 733	-18.5 -18.5	0.24 (4)	10.00			
J-I	0 / 733	-18.5 -18.5	0.24 (4)	10.00			
I-H	0 / 0	-18.5 -18.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.76")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.76")
CALCULATED VERT. DEFL. (TL) = L/969 (0.10")

CSI: TC=0.88/1.00 (G-H:1), BC=0.28/1.00 (K-L:1)
WB=0.42/1.00 (C-K:1), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

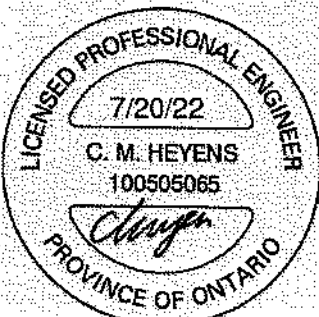
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)

JSI METAL= 0.38 (L) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070664



1945

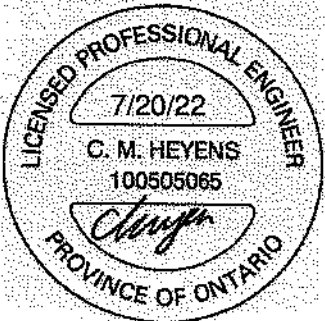
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425144	T60S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITak Industries, Inc. Wed Jul 20 11:04:44 2022 Page 2

ID:9uKWqE1o0hmyukA0GZGHL1zZkTS-qV3PwyN5indHQDY hZsl7V0f8jwZ9259RKlztgywCVn

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



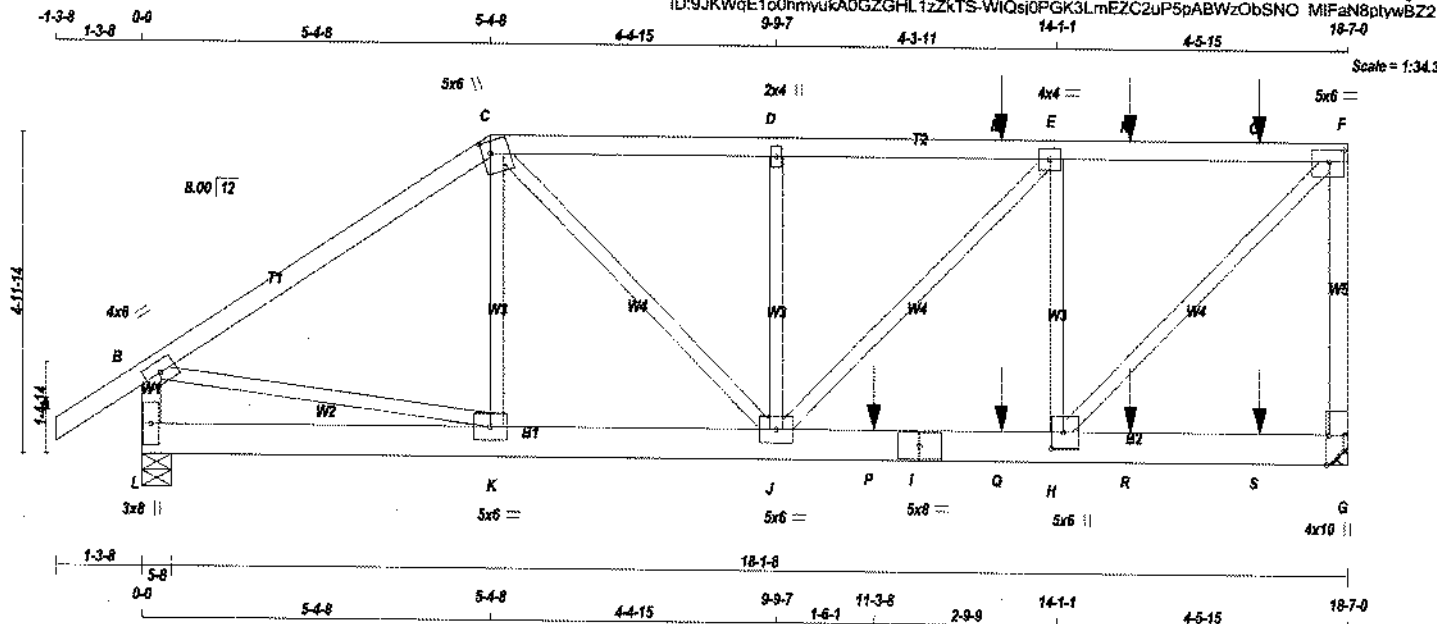
STRUCTURAL COMPONENT ONLY
DWG # TR22070692 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T61	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 12:09:31 2022 Page 1

ID:9JKWqE1o0hmyuka0GZGHL1zZkTS-WiQsJ0PGK3LmEZC2uP5ABWzObSNO MIFa8ptywBZ2



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - F	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMW-1 MT20	4.0	6.0		
C TTWW+m MT20	5.0	6.0	2.25	1.50
D TMW+w MT20	2.0	4.0		
E TMWW-t MT20	4.0	4.0		
F TMW-1 MT20	5.0	6.0	2.25	2.75
G BMV1+t MT20	4.0	10.0	Edge	0.50
H BMWW+t MT20	5.0	6.0	3.00	2.25
I BS-t MT20	5.0	8.0		
J BMWW-t MT20	5.0	6.0		
K BMWW-t MT20	5.0	6.0		
L BMV1+p MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION	GROSS REACTION	DOWN	HORIZ		
JT	VERT	HORIZ	DOWN	HORIZ	BRG	BRG
G	1726	0	1726	0	MECHANICAL	
L	1470	0	1470	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
	JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
G	1222	795 / 0	0 / 0	0 / 0	0 / 0	427 / 0
L	1039	685 / 0	0 / 0	0 / 0	0 / 0	354 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 32	-84.9	-84.9	0.13 (1)	10.00	K-C	-207 / 12	0.07 (1)
B-C	-1512 / 0	-84.9	-84.9	0.57 (1)	4.60	B-K	0 / 1278	0.32 (1)
C-D	-1883 / 0	-84.9	-84.9	0.28 (1)	4.58	H-F	0 / 2097	0.52 (1)
D-M	-1883 / 0	-84.9	-84.9	0.43 (1)	4.36	C-J	0 / 911	0.23 (1)
M-E	-1883 / 0	-84.9	-84.9	0.43 (1)	4.36	H-E	-1044 / 0	0.37 (1)
E-N	-1463 / 0	-84.9	-84.9	0.40 (1)	4.88	J-D	-389 / 0	0.14 (1)
N-O	-1463 / 0	-84.9	-84.9	0.40 (1)	4.88	J-E	0 / 616	0.15 (1)
O-F	-1463 / 0	-84.9	-84.9	0.40 (1)	4.88			
G-F	-1717 / 0	0.0	0.0	0.67 (1)	6.30			
L-B	-1416 / 0	0.0	0.0	0.16 (1)	6.81			

L-K	0 / 0	-18.5	-18.5 0.07 (4)	10.00
K-J	0 / 1252	-18.5	-18.5 0.35 (1)	10.00
J-P	0 / 1463	-18.5	-18.5 0.67 (1)	10.00
P-I	0 / 1463	-18.5	-18.5 0.67 (1)	10.00
I-Q	0 / 1463	-18.5	-18.5 0.67 (1)	10.00
Q-H	0 / 1463	-18.5	-18.5 0.67 (1)	10.00
H-R	0 / 0	-18.5	-18.5 0.16 (1)	10.00
R-S	0 / 0	-18.5	-18.5 0.16 (1)	10.00
S-G	0 / 0	-18.5	-18.5 0.16 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	13-2-12	-55	-55	---	BACK	VERT	TOTAL	---	C1
N	15-2-12	-55	-55	---	BACK	VERT	TOTAL	---	C1
O	17-2-12	-55	-55	---	BACK	VERT	TOTAL	---	C1
P	11-3-8	-610	-610	---	BACK	VERT	TOTAL	---	C1
Q	13-2-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
R	15-2-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
S	17-2-12	-14	-14	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBCG 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.62")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/360$ (0.62")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.13")

CS1: TC=0.67/1.00 (F-G:1), BC=0.67/1.00 (H-J:1),
WB=0.52/1.00 (F-H:1), SS=0.41/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

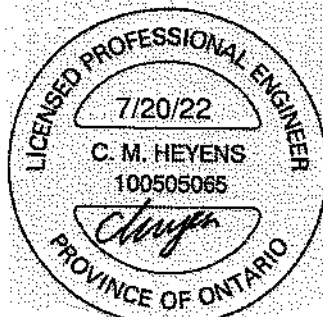
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 6.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.68 (I) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070665 PG 1/2

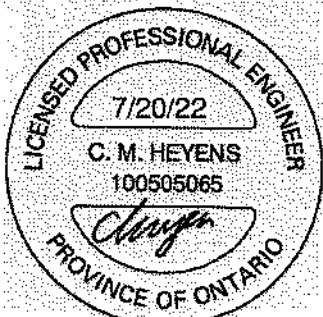
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T61	1	1	TRUSS DESC.		

Temarack Roof Truss, Burlington

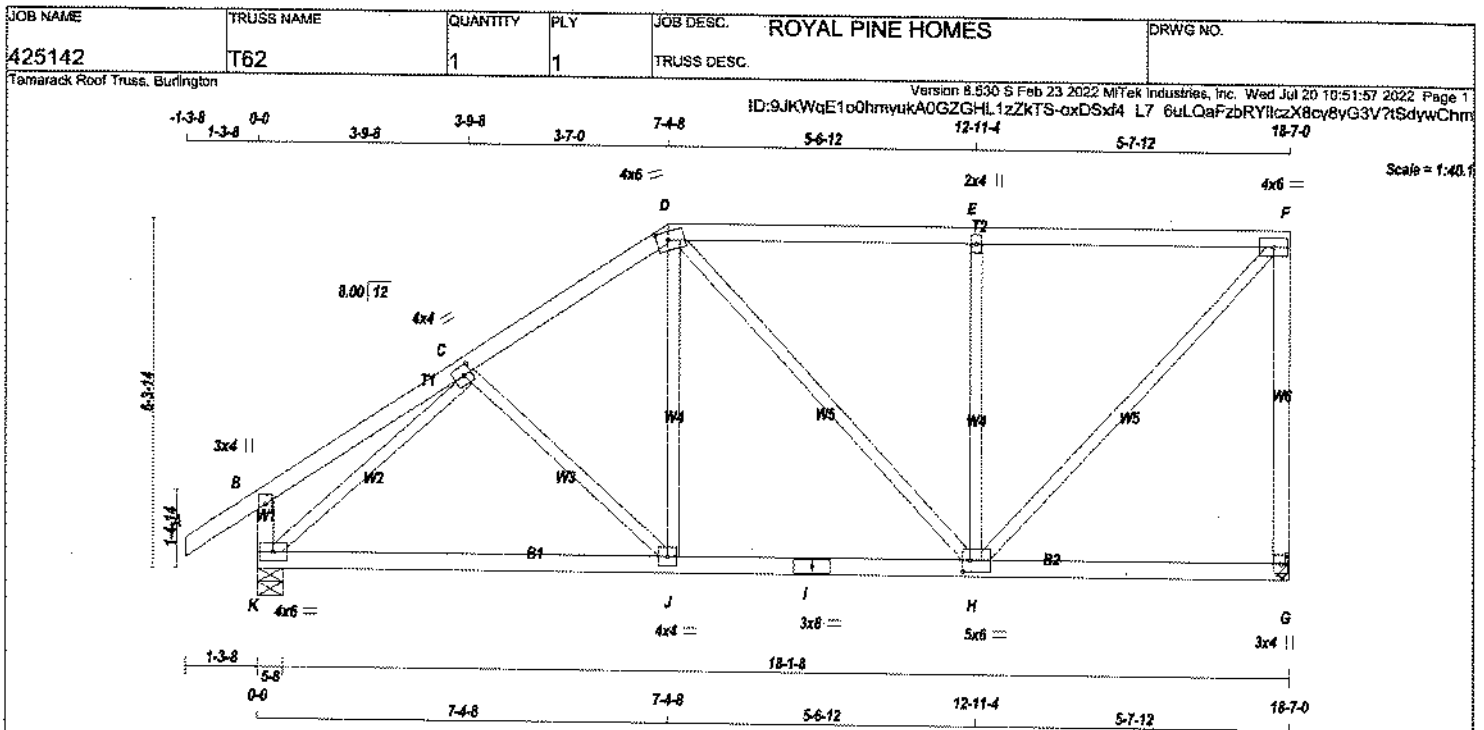
Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 12:09:31 2022 Page 2
ID:9JKWqE1c0hmyukA0GZGHL1zZkTS-WIQsi0PGK3LmEZC2uP5pABWzObSNO MIFaN8ptyw8Z2

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22070665 PG 2/2



LUMBER

N. E. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TMVW-m	MT20	4.0	6.0	1.75	2.50
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
G	960	0	060	0
K	1077	0	1077	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
G	682 433 / 0 0 / 0 0 / 0 0 / 0 249 / 0 0 / 0
K	762 497 / 0 0 / 0 0 / 0 0 / 0 265 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.9 -84.9 0.11 (1)	10.00	C-J	-125 / 4	0.06 (1)	
B-C	0 / 19	-84.9 -84.9 0.18 (1)	10.00	J-D	0 / 225	0.05 (4)	
C-D	-889 / 0	-84.9 -84.9 0.19 (1)	6.25	D-H	-58 / 0	0.09 (1)	
D-E	-686 / 0	-84.9 -84.9 0.46 (1)	6.25	H-E	-589 / 0	0.38 (1)	
E-F	-686 / 0	-84.9 -84.9 0.46 (1)	6.25	H-F	0 / 1004	0.23 (1)	
G-F	-915 / 0	0.0 0.0 0.73 (1)	7.81	K-C	-1128 / 0	0.47 (1)	
K-B	-237 / 0	0.0 0.0 0.02 (1)	7.81				
K-J	0 / 813	-18.5 -18.5 0.28 (4)	10.00				
J-I	0 / 726	-18.5 -18.5 0.28 (4)	10.00				
I-H	0 / 726	-18.5 -18.5 0.28 (4)	10.00				
H-G	0 / 0	-18.5 -18.5 0.12 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 0.0 PSF
TOTAL LOAD	= 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.62")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.62")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CS1: TC=0.73/1.00 (F-G:1), BC=0.28/1.00 (H-J:4),
WB=0.47/1.00 (C-K:1), SS1=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BENO=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

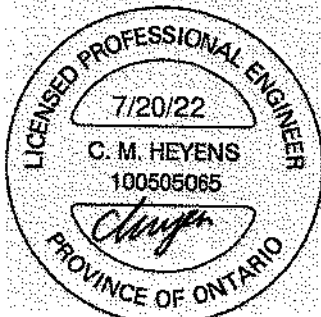
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.37 (C) (INPUT = 1.00)

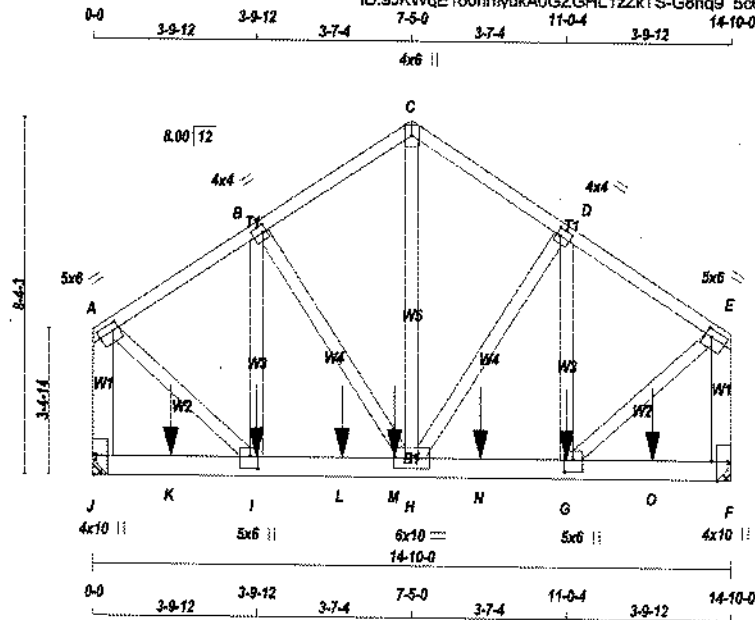


STRUCTURAL COMPONENT ONLY
DWG # TR22070666

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T63	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:51:58 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL1zZKTS-G8nq9 5c6R6zWV7mph6g4ZlumxSRhdQOH9kR 3ywCht



Scale = 1:51.8

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
J - A	2x6	DRY	No.2	SPF	
F - E	2x6	DRY	No.2	SPF	
J - F	2x6	DRY	No.2	SPF	

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-E	12	TOP
J-A	12	TOP
F-E	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J-F	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
D-G	6	SIDE(119.9)
B-I	6	SIDE(119.9)
H-C	6	SIDE(119.9)
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
J	4863	0	0	MECHANICAL
F	4801	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT JOINT J = 4-0, JOINT F = 4-0.

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
J	3446 2221 / 0 0 / 0 0 / 0 1226 / 0 0 / 0
F	3402 2192 / 0 0 / 0 0 / 0 1210 / 0 0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	-3485 / 0	-84.9 -84.9 0.13 (1)	4.91	H-C	0 / 3256	0.29 (1)
B-C	-3174 / 0	-84.9 -84.9 0.12 (1)	5.11	H-D	-427 / 0	0.12 (1)
C-D	-3174 / 0	-84.9 -84.9 0.12 (1)	5.11	G-D	0 / 141	0.01 (4)
D-E	-3422 / 0	-84.9 -84.9 0.13 (1)	4.96	B-H	-539 / 0	0.15 (1)
J-A	-4381 / 0	0.0 0.0 0.28 (1)	6.85	I-B	0 / 276	0.02 (1)
F-E	-4292 / 0	0.0 0.0 0.27 (1)	6.91	A-I	0 / 3717	0.33 (1)
				G-E	0 / 3640	0.32 (1)
J-K	0 / 0	-18.5 -18.5 0.31 (1)	10.00			
K-L	0 / 0	-18.5 -18.5 0.31 (1)	10.00			
L-M	0 / 2921	-18.5 -18.5 0.44 (1)	10.00			
M-H	0 / 2921	-18.5 -18.5 0.44 (1)	10.00			
H-N	0 / 2860	-18.5 -18.5 0.43 (1)	10.00			
N-G	0 / 2860	-18.5 -18.5 0.43 (1)	10.00			
G-O	0 / 0	-18.5 -18.5 0.33 (1)	10.00			
O-F	0 / 0	-18.5 -18.5 0.33 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	11-0-4	-823	-823	---	BACK	VERT	TOTAL	---	C1
I	3-9-12	-823	-823	---	BACK	VERT	TOTAL	---	C1
K	1-9-12	-823	-823	---	BACK	VERT	TOTAL	---	C1
L	5-9-12	-823	-823	---	BACK	VERT	TOTAL	---	C1
M	7-0-4	-823	-823	---	BACK	VERT	TOTAL	---	C1
N	9-0-4	-823	-823	---	BACK	VERT	TOTAL	---	C1
O	13-0-4	-823	-823	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 102 = 205 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.28/1.00 (A-J-1), BC=0.44/1.00 (H-I-1),
WB=0.33/1.00 (A-I-1), SI=0.57/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (i) (INPUT = 0.90)
JSI METAL= 0.53 (i) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070667 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T63	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 10:51:58 2022 Page 2
ID:9JKWqE1o0hrryukA0GZGHL1zZkTS-G8nc9 5c6R6zWV7mph6q4zlumxSRhdQQH9kR 3ywChl

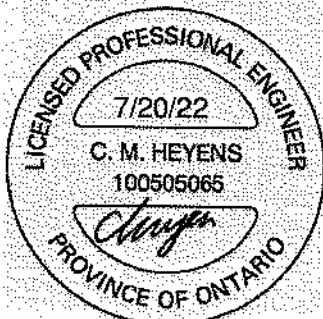
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	6.0	2.00	1.75
B	TMVW-1	MT20	4.0	4.0	2.00	1.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-1	MT20	4.0	4.0	2.00	1.00
E	TMVW-1	MT20	5.0	6.0	2.00	1.75
F	BMV1+1	MT20	4.0	10.0	Edge	1.50
G	BMVW+1	MT20	5.0	6.0	3.50	2.25
H	BMVW+1	MT20	6.0	10.0		
I	BMVW+1	MT20	5.0	6.0	3.50	2.25
J	BMV1+1	MT20	4.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

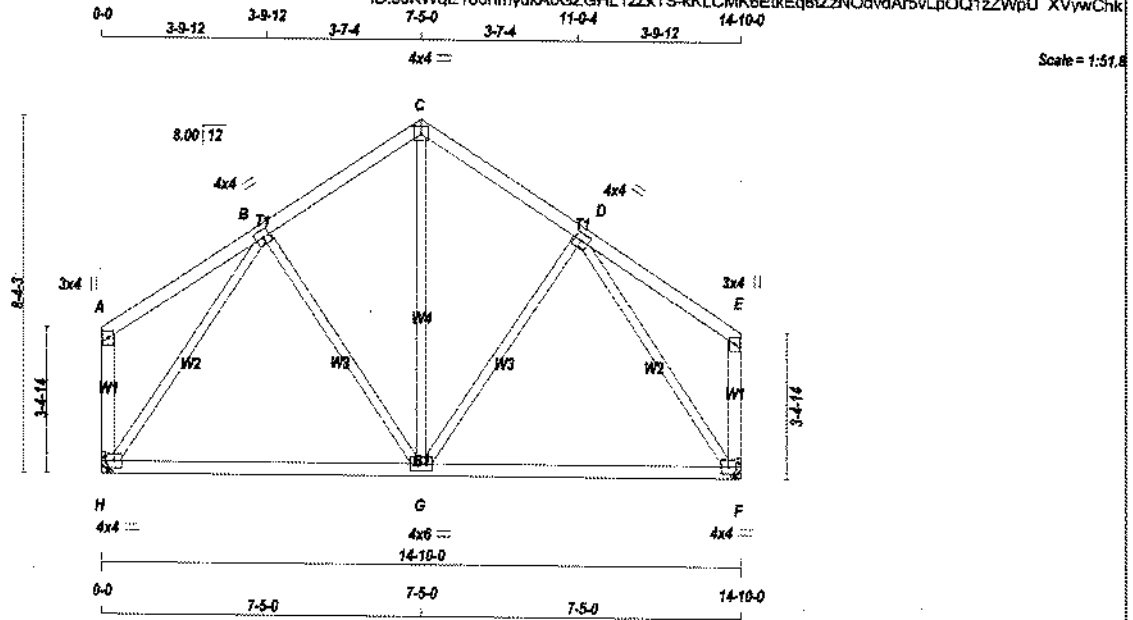


STRUCTURAL COMPONENT ONLY
DWG # TR22070667 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T64	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:51:59 2022 Page 1
ID:9JKWqE1o0hmyukABGZGHL1zZkTS-kKLCMK6EtEq8ZzN0dvdAr5vLpOQ1zZWpU XVYwChk



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - A	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW-t	MT20	4.0	4.0		
G	BMVW-t	MT20	4.0	6.0		
H	BMVW-t	MT20	4.0	4.0		

NOTES (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ
H	767	0	767	0
F	767	0	767	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOL
H	544	345 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0
F	544	345 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 20	-84.9 -84.9 0.19 (1)	G-C	0 / 254
B-C	-464 / 0	-84.9 -84.9 0.15 (1)	G-D	-50 / 27
C-D	-464 / 0	-84.9 -84.9 0.15 (1)	B-G	-50 / 27
D-E	0 / 20	-84.9 -84.9 0.19 (1)	H-B	-713 / 0
H-A	-122 / 0	0.0 0.0 0.02 (1)	D-F	-713 / 0
F-E	-122 / 0	0.0 0.0 0.02 (1)		
H-G	0 / 397	-18.5 -18.5 0.33 (4)		
G-F	0 / 397	-18.5 -18.5 0.33 (4)		

TOTAL WEIGHT = 71 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.19/1.00 (D-E:1), BC=0.33/1.00 (F-G:4)
WB=0.57/1.00 (D-F:1), SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

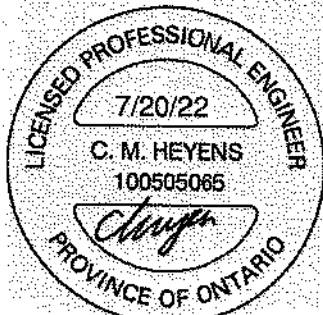
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (D) (INPUT = 0.90)

JSI METAL= 0.22 (B) (INPUT = 1.00)

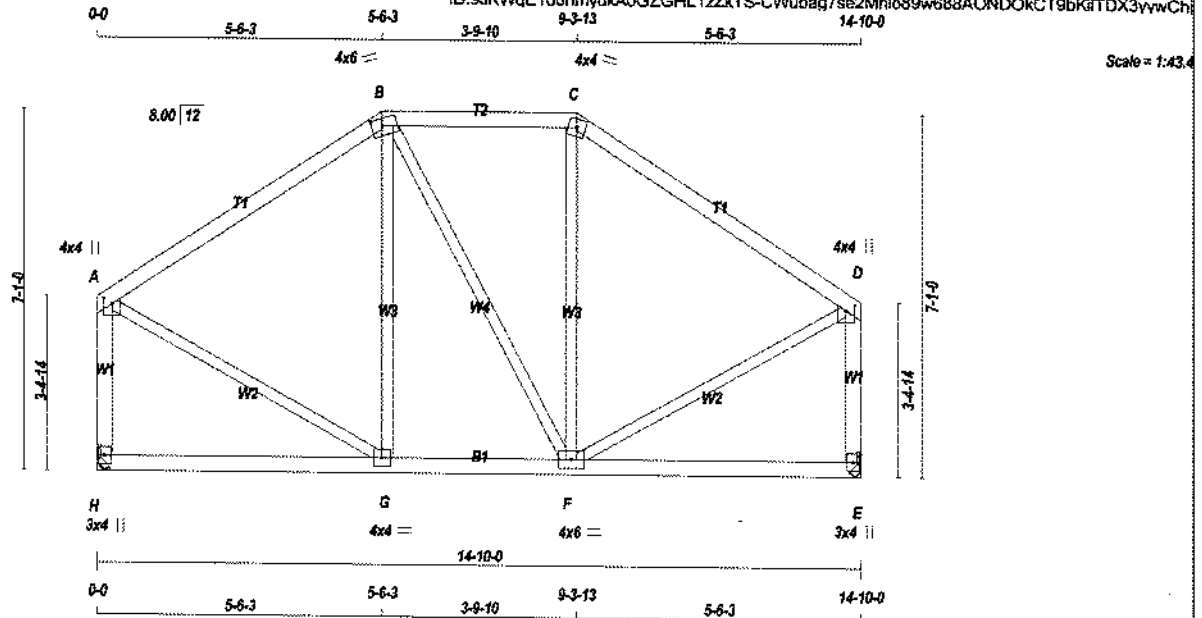


STRUCTURAL COMPONENT ONLY
DWG # TR22070668

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T65	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:52:08 2022 Page 1
ID:9JKWqE1oOhmyukA0GZGHL1zZKTS-CWubag7se2Mhio89w688AONDOKCT9bKGTDX3YvwCh



Scale = 1:43.4

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
B - C	2x4 DRY	No.2	SPF		
C - D	2x4 DRY	No.2	SPF		
H - A	2x4 DRY	No.2	SPF		
E - D	2x4 DRY	No.2	SPF		
H - E	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00	
B	TTWW-m	MT20	4.0	6.0	1.75	2.50	
C	TTWW-m	MT20	4.0	4.0			
D	TMVW+p	MT20	4.0	4.0	1.25	2.00	
E	BMVt+p	MT20	3.0	4.0			
F	BMWWW-t	MT20	4.0	6.0			
G	BMWWW-t	MT20	4.0	4.0			
H	BMVt+p	MT20	3.0	4.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	787	0	787	0	0	MECHANICAL
E	767	0	767	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT E = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	544	345 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0
E	544	345 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)
FR-TO						
A-B	-483 / 0	-84.9	-84.9 0.33 (1)	6.25	G-B	-126 / 22 0.11 (1)
B-C	-399 / 0	-84.9	-84.9 0.16 (1)	6.25	B-F	0 / 0 0.00 (1)
C-D	-482 / 0	-84.9	-84.9 0.33 (1)	6.25	F-C	-127 / 22 0.11 (1)
H-A	-723 / 0	0.0	0.0 0.14 (1)	7.81	A-G	0 / 457 0.10 (1)
E-D	-722 / 0	0.0	0.0 0.14 (1)	7.81	F-D	0 / 457 0.10 (1)
H-G	0 / 0	-18.5	-18.5 0.12 (4)	10.00		
G-F	0 / 399	-18.5	-18.5 0.14 (4)	10.00		
F-E	0 / 0	-18.5	-18.5 0.12 (4)	10.00		

TOTAL WEIGHT = 70 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.33/1.00 (A-B:1), BC=0.14/1.00 (F-G:4)
WB=0.11/1.00 (C-F:1), SI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

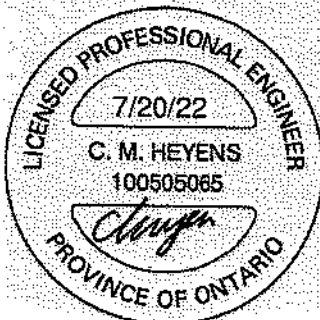
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (F) (INPUT = 0.90)
JSI METAL= 0.14 (A) (INPUT = 1.00)

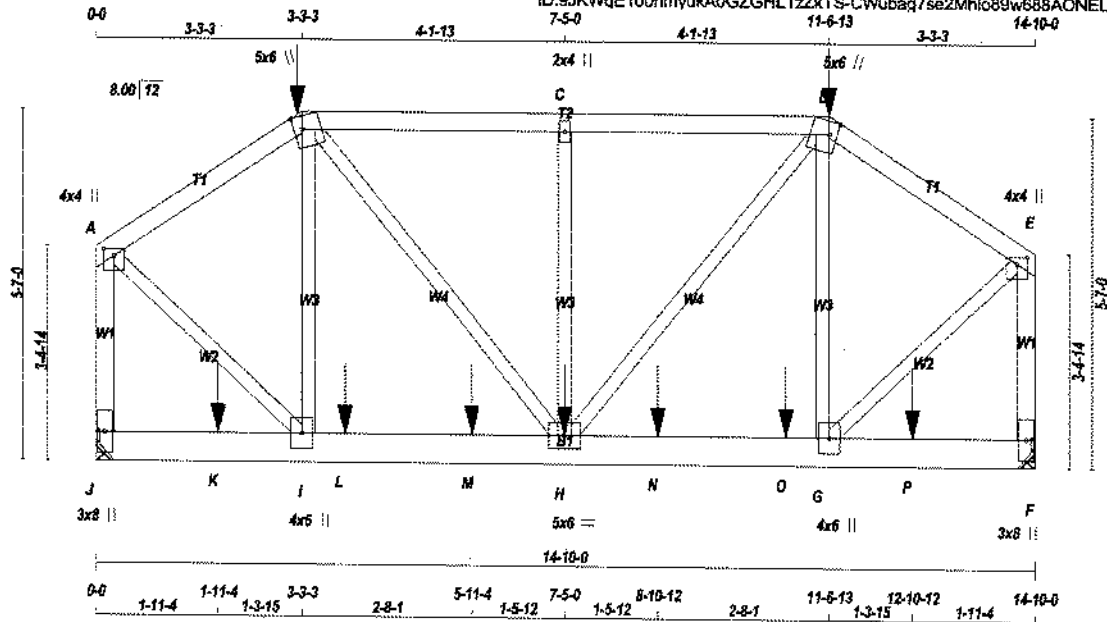


STRUCTURAL COMPONENT ONLY
DWG # TR22070669

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T66	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:52:00 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL1zZkTS-CWubag7se2Mhic89w688AONELKC99ZVITDX3yywCh



Scale = 1:35.2

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - B	TMVW+p	MT20	4.0	4.0	1.25	2.00
B - D	TTWW+m	MT20	5.0	6.0	2.50	1.50
C - E	TMVW+w	MT20	2.0	4.0		
D - E	TTWW+m	MT20	5.0	6.0	2.50	1.50
E - F	TMVW+p	MT20	4.0	4.0	1.25	2.00
F - G	BMVW+t	MT20	3.0	8.0		
G - H	BMVW+t	MT20	4.0	6.0		
H - I	BMVW+t	MT20	5.0	6.0		
I - J	BMVW+t	MT20	4.0	6.0		
J - F	BMVW+t	MT20	3.0	8.0		

NOTES: (1)
1) Lateral bracing to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
J	1252	0	0	0
F	1252	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT JOINT J = 3-8, JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	862	596 / 0	0 / 0	0 / 0	0 / 0	284 / 0	0 / 0
F	862	596 / 0	0 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
A-B	-828 / 0	-84.9 -84.9 0.18 (1)	I-B	-296 / 0
B-C	-908 / 0	-84.9 -84.9 0.27 (1)	B-H	0 / 508
C-D	-988 / 0	-84.9 -84.9 0.27 (1)	H-C	-426 / 0
D-E	-828 / 0	-84.9 -84.9 0.18 (1)	H-D	0 / 508
J-A	-1212 / 0	0.0 0.0 0.23 (1)	G-D	-296 / 0
F-E	-1212 / 0	0.0 0.0 0.23 (1)	A-I	0 / 920
			G-E	0 / 920
J-K	0 / 0	-18.5 -18.5 0.08 (1)		
K-L	0 / 0	-18.5 -18.5 0.08 (1)		
L-L	0 / 881	-18.5 -18.5 0.16 (1)		
L-M	0 / 881	-18.5 -18.5 0.16 (1)		
M-H	0 / 881	-18.5 -18.5 0.16 (1)		
H-N	0 / 881	-18.5 -18.5 0.16 (1)		
N-O	0 / 881	-18.5 -18.5 0.16 (1)		
O-G	0 / 881	-18.5 -18.5 0.16 (1)		
G-P	0 / 0	-18.5 -18.5 0.08 (1)		
P-F	0 / 0	-18.5 -18.5 0.08 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-3-3	-12	-12		FRONT	VERT	DEAD		C1
B	3-3-3	-45	-45		FRONT	VERT	SNOW		C1
D	11-6-13	-12	-12		FRONT	VERT	DEAD		C1
D	11-6-13	-45	-45		FRONT	VERT	SNOW		C1
H	7-5-0	-86	-86		FRONT	VERT	TOTAL		C1
K	1-11-4	-66	-66		FRONT	VERT	TOTAL		C1
L	3-11-4	-66	-66		FRONT	VERT	TOTAL		C1
M	5-11-4	-66	-66		FRONT	VERT	TOTAL		C1
N	8-10-12	-66	-66		FRONT	VERT	TOTAL		C1
O	10-10-12	-66	-66		FRONT	VERT	TOTAL		C1
P	12-10-12	-66	-66		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 0.0 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.27/1.00 (B-C-1), BC=0.18/1.00 (H-I-1),
WB=0.23/1.00 (E-G-1), SSI=0.19/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

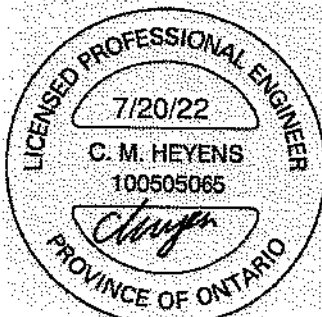
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.31 (I) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070670 PG 1/2

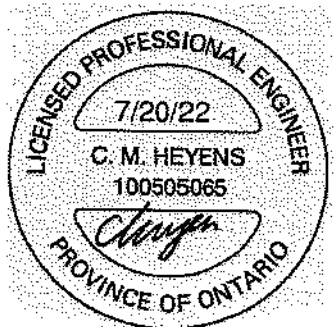
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T66	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:52:01 2022 Page 2
ID:9JKWqE1a0hmyukA0GZGHL1zZkTS-giSzm08LUPMVYNyLUpqNbwP58YQu0ls 7z5bOywCh

CONNECTION REQUIREMENTS

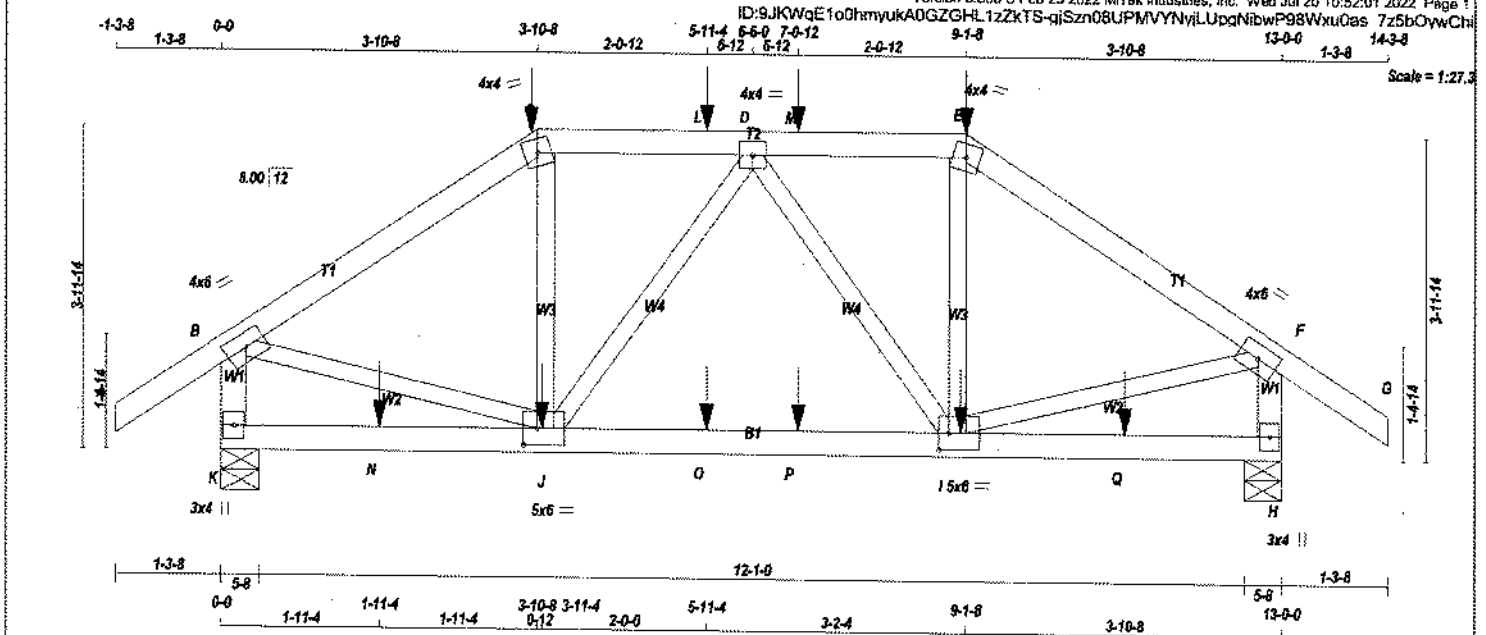
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22070670 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T67	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:52:01 2022 Page 1



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF

A - C	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-1	MT20	4.0	6.0		
C TTW-m	MT20	4.0	4.0		
D TMVW-1	MT20	4.0	4.0		
E TTW-m	MT20	4.0	4.0		
F TMVW-1	MT20	4.0	6.0		
H BMV1-p	MT20	3.0	4.0		
I BMVWV-1	MT20	5.0	6.0	2.50	1.50
J BMVWV-1	MT20	5.0	6.0	2.50	2.00
K BMV1-p	MT20	3.0	4.0		

NOTES: (1) 1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	UPLIFT		
K	1191	0	1191	0	5-8	5-8
H	1191	0	1191	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	842	555 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	842	555 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LOC. MAX. (LC)	MAX. UNBRAC. LENGTH (FT)	MEMB. MAX. FORCE (LBS)	MAX. (LC)		
FR-TO								
A-B	0 / 32	-84.9	-84.9 0.13 (1)	10.00	J-C	0 / 108	0.04 (4)	
B-C	-1122 / 0	-84.9	-84.9 0.27 (1)	5.62	J-D	-244 / 0	0.08 (1)	
C-L	-936 / 0	-84.9	-84.9 0.15 (1)	6.20	D-I	-244 / 0	0.08 (1)	
L-D	-936 / 0	-84.9	-84.9 0.15 (1)	6.20	I-E	0 / 108	0.04 (4)	
D-M	-936 / 0	-84.9	-84.9 0.15 (1)	6.20	B-J	0 / 965	0.24 (1)	
M-E	-936 / 0	-84.9	-84.9 0.15 (1)	6.20	J-F	0 / 965	0.24 (1)	
E-F	-1122 / 0	-84.9	-84.9 0.27 (1)	5.62				
F-G	0 / 32	-84.9	-84.9 0.13 (1)	10.00				
K-B	-1156 / 0	0.0	0.0 0.13 (1)	7.37				
H-F	-1156 / 0	0.0	0.0 0.13 (1)	7.37				
K-N	0 / 0	-18.5	-18.5 0.13 (4)	10.00				
N-J	0 / 0	-18.5	-18.5 0.13 (4)	10.00				
J-O	0 / 1072	-18.5	-18.5 0.26 (1)	10.00				
O-P	0 / 1072	-18.5	-18.5 0.26 (1)	10.00				
P-I	0 / 1072	-18.5	-18.5 0.26 (1)	10.00				
I-Q	0 / 0	-18.5	-18.5 0.13 (4)	10.00				
Q-H	0 / 0	-18.5	-18.5 0.13 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)		FACE	DIR.	TYPE	HEEL	CONN.
JT	LOC. LC1 MAX. MAX.++					
C	3-10-8 -188 -188	FRONT	VERT	TOTAL	---	C1
E	9-1-8 -188 -188	FRONT	VERT	TOTAL	---	C1
I	9-0-12 -14 -14	FRONT	VERT	TOTAL	---	C1
J	3-11-4 -14 -14	FRONT	VERT	TOTAL	---	C1
L	5-11-4 -55 -55	FRONT	VERT	TOTAL	---	C1
M	7-0-12 -55 -55	FRONT	VERT	TOTAL	---	C1
N	1-11-4 -14 -14	FRONT	VERT	TOTAL	---	C1
O	5-11-4 -14 -14	FRONT	VERT	TOTAL	---	C1
P	7-0-12 -14 -14	FRONT	VERT	TOTAL	---	C1
Q	11-0-12 -14 -14	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.27/1.00 (E-F:1), BC=0.28/1.00 (I-J:1), WB=0.24/1.00 (B-J:1), SS=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

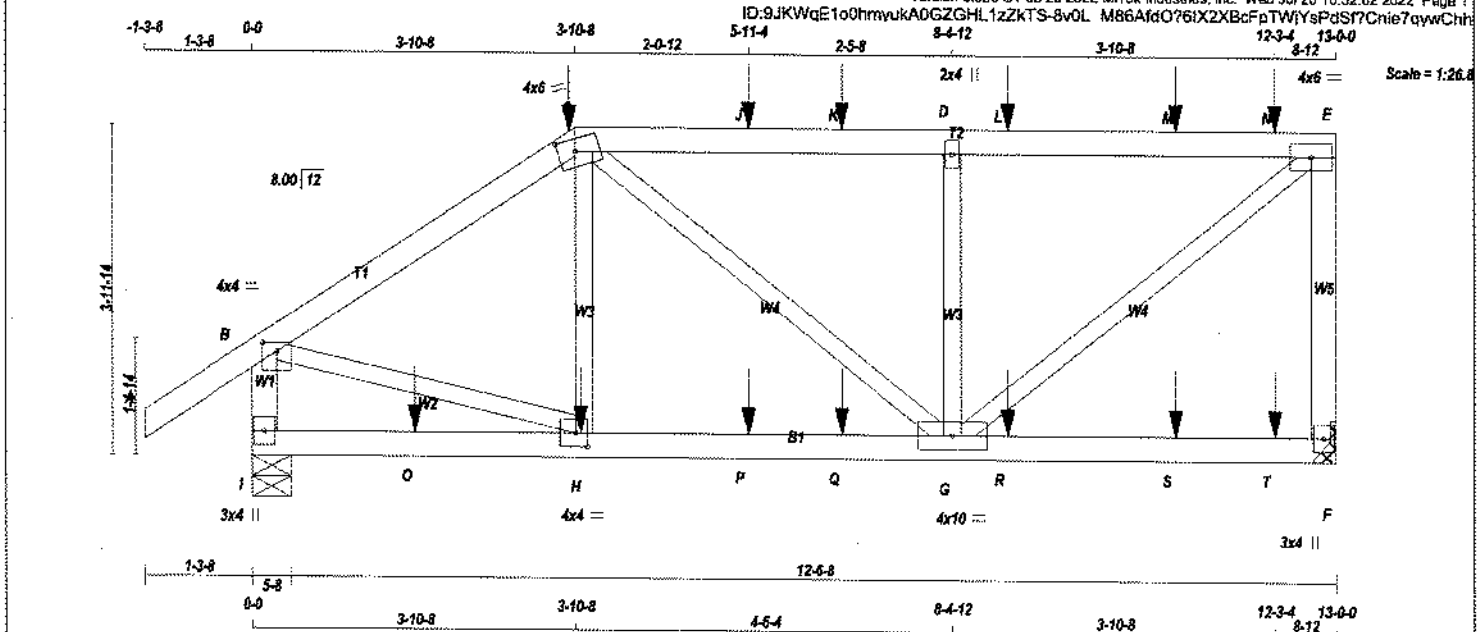
JSI GRIP= 0.78 (I) (INPUT = 0.90)
JSI METAL= 0.29 (F) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070671

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T68	1	1	TRUSS DESC.		

Temerack Roof Truss, Burlington Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:52:02 2022 Page 1



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMVW-w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1-p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	10.0		
H	BMVWW-t	MT20	4.0	4.0	2.00	1.75
I	BMV1-p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
JT 1112 0	1112 0 0	MECHANICAL	
F 1152 0	1152 0 0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
JT 789	504 / 0 0 / 0 0 / 0 0 / 0 284 / 0 0 / 0
F 815	535 / 0 0 / 0 0 / 0 0 / 0 280 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
A-B	0 / 32	-84.9	-84.9	0.13 (1)	10.00	H-C	-116 / 53	0.03 (1)
B-C	-1066 / 0	-84.9	-84.9	0.27 (1)	5.74	C-G	0 / 137	0.03 (1)
C-D	-989 / 0	-84.9	-84.9	0.54 (1)	6.38	G-O	-739 / 0	0.19 (1)
D-E	-989 / 0	-84.9	-84.9	0.54 (1)	5.38	E-H	0 / 1268	0.31 (1)
E-F	-989 / 0	-84.9	-84.9	0.54 (1)	5.38	F-I	0 / 918	0.23 (1)
F-G	-989 / 0	-84.9	-84.9	0.54 (1)	5.38			
G-H	-989 / 0	-84.9	-84.9	0.54 (1)	5.38			
H-I	-989 / 0	-84.9	-84.9	0.54 (1)	5.38			
I-J	-1051 / 0	0.0	0.0	0.27 (1)	7.84			
J-K	-1116 / 0	0.0	0.0	0.13 (1)	7.47			
K-L	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
L-M	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
M-N	0 / 892	-18.5	-18.5	0.24 (4)	10.00			
N-O	0 / 892	-18.5	-18.5	0.24 (4)	10.00			
O-P	0 / 892	-18.5	-18.5	0.24 (4)	10.00			
P-Q	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
Q-R	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
R-S	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
S-T	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
T-F	0 / 0	-18.5	-18.5	0.16 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-188	-188	---	BACK	VERT	TOTAL	C1
H	3-11-4	-14	-14	---	BACK	VERT	TOTAL	C1
J	5-11-4	-55	-55	---	BACK	VERT	TOTAL	C1
K	7-0-12	-55	-55	---	BACK	VERT	TOTAL	C1
L	9-0-12	-55	-55	---	BACK	VERT	TOTAL	C1
M	11-0-12	-55	-55	---	BACK	VERT	TOTAL	C1
N	12-3-4	-63	-63	---	BACK	VERT	TOTAL	C1
O	1-11-4	-14	-14	---	BACK	VERT	TOTAL	C1
P	5-11-4	-14	-14	---	BACK	VERT	TOTAL	C1
Q	7-0-12	-14	-14	---	BACK	VERT	TOTAL	C1
R	9-0-12	-14	-14	---	BACK	VERT	TOTAL	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.43")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.43")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.54/1.00 (D-E-1), BC=0.24/1.00 (G-H-4), WB=0.31/1.00 (E-G-1), SS=0.33/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

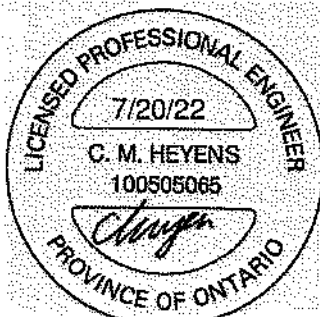
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22070672 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T68	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:52:03 2022 Page 2

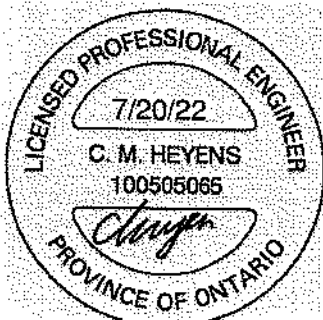
ID:9JKWqE1o0hmyukA0GZGHL1zZKTS-c5ajC18bzIFcGtkeEim07hTyCeMv9RRS8qHywChg

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	11-0-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
T	12-3-4	-16	-16	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

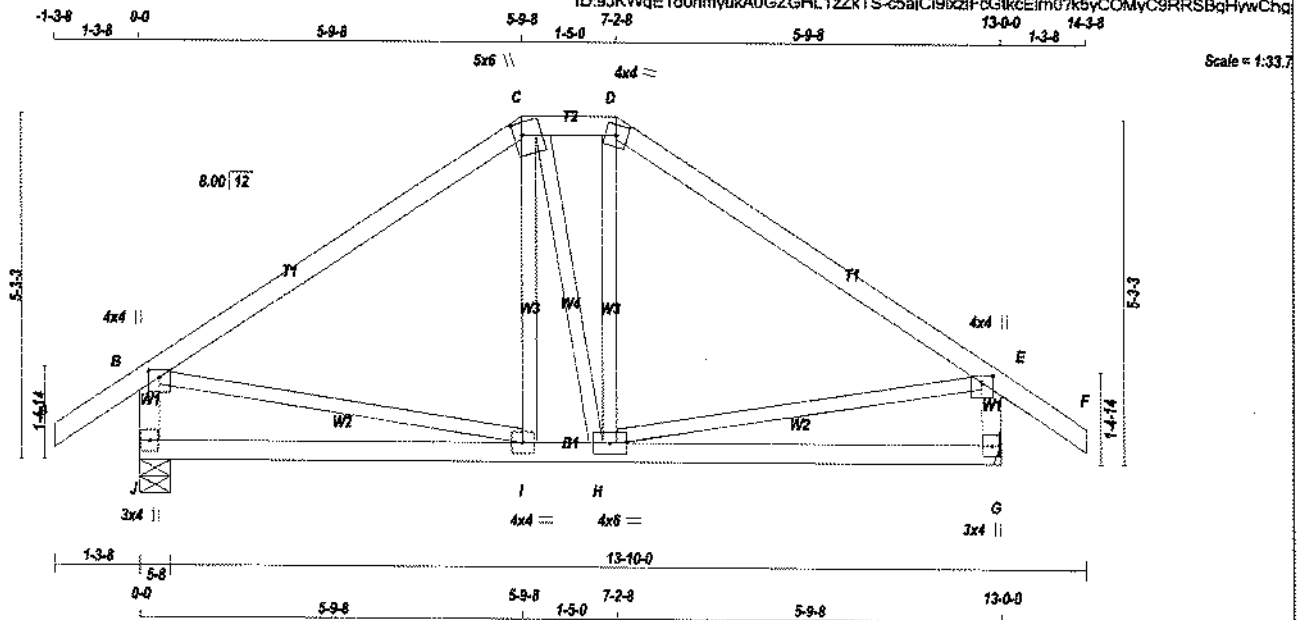


STRUCTURAL COMPONENT ONLY
DWG # TR22070672 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T69	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:52:03 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL1zZkTS-c5aiC19bzF0GkKcElm07k5yCOMyC9RRSBgHywChg



TOTAL WEIGHT = 2 X 59 = 118 lb (M)

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-i	MT20	4.0	6.0		
I	BMVWW-i	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	DOWN	UP		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
J	788	0	788	0	5-8	5-8
G	788	0	788	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	557	367 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0
G	557	367 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	I-C	-10 / 48 0.02 (4)
B-C	-530 / 0	-84.9	-84.9	0.37 (1)	6.25	C-H	0 / 6 0.00 (4)
C-D	-448 / 0	-84.9	-84.9	0.02 (1)	6.25	H-D	-6 / 54 0.02 (4)
D-E	-539 / 0	-84.9	-84.9	0.37 (1)	6.25	B-I	0 / 454 0.10 (1)
E-F	0 / 32	-84.9	-84.9	0.11 (1)	10.00	H-E	0 / 455 0.10 (1)
J-B	-744 / 0	0.0	0.0	0.08 (1)	7.81		
G-E	-743 / 0	0.0	0.0	0.08 (1)	7.81		
J-I	0 / 0	-18.5	-18.5	0.14 (4)	10.00		
I-H	0 / 447	-18.5	-18.5	0.19 (4)	10.00		
H-G	0 / 0	-18.5	-18.5	0.15 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CS: TC=0.37/1.00 (D-E:1), BC=0.19/1.00 (H-I:4), WB=0.10/1.00 (E-H:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

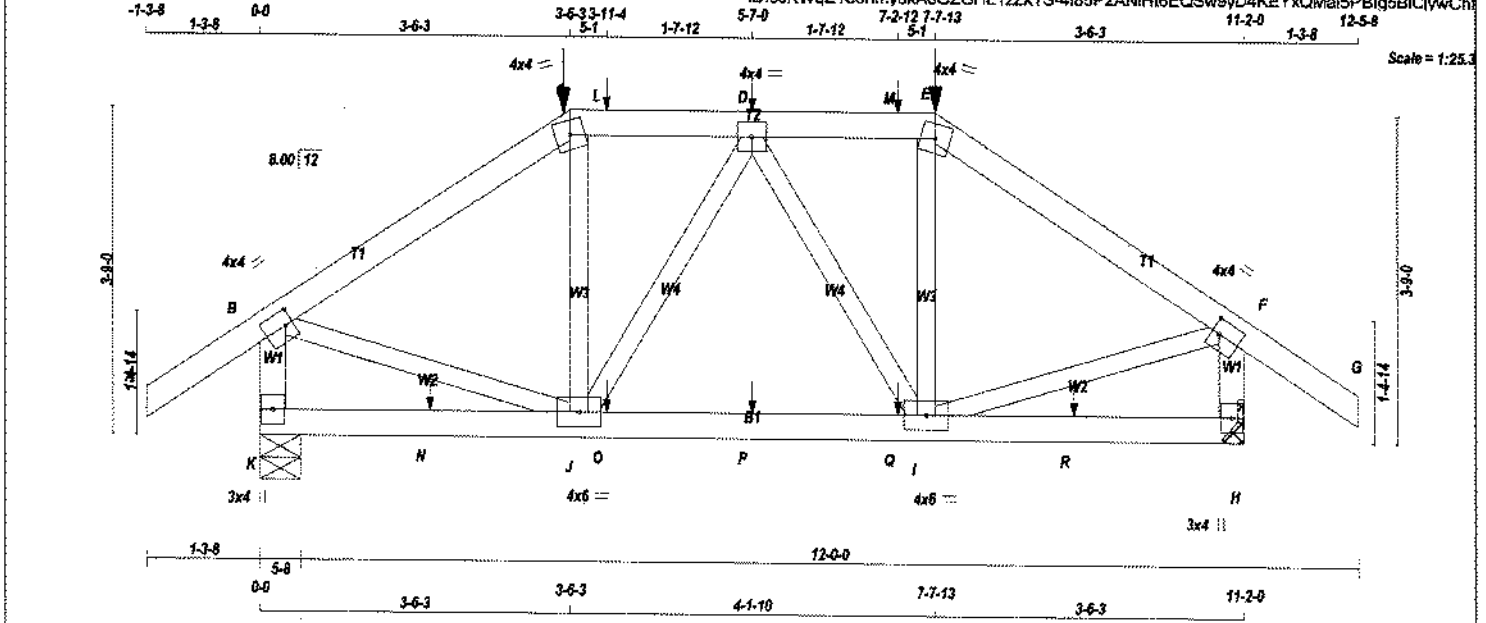
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (H) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070673



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
K - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	4.0	4.0	2.00	1.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-l	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMW-t	MT20	4.0	4.0	2.00	1.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-l	MT20	4.0	6.0		
J	BMWWW-l	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	DOWN	UP	DOWN
K	738	0	738	0
H	738	0	738	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	521	348 / 0	0 / 0	0 / 0	0 / 0	173 / 0	0 / 0
H	521	348 / 0	0 / 0	0 / 0	0 / 0	173 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. CS (LC)
FR-TO						
A-B	0 / 32	-84.9	-84.9 0.13 (1)	10.00	J-C	0 / 55
B-C	-552 / 0	-84.9	-84.9 0.20 (1)	6.25	J-D	-118 / 0
C-L	-460 / 0	-84.9	-84.9 0.06 (1)	6.25	D-I	-118 / 0
L-D	-460 / 0	-84.9	-84.9 0.06 (1)	6.25	I-E	0 / 55
D-M	-460 / 0	-84.9	-84.9 0.06 (1)	6.25	B-J	0 / 479
M-E	-460 / 0	-84.9	-84.9 0.06 (1)	6.25	I-F	0 / 479
E-F	-552 / 0	-84.9	-84.9 0.20 (1)	6.25		
F-G	0 / 32	-84.9	-84.9 0.13 (1)	10.00		
K-B	-711 / 0	0.0	0.0 0.08 (1)	7.81		
H-F	-711 / 0	0.0	0.0 0.08 (1)	7.81		
K-N	0 / 0	-18.5	-18.5 0.06 (4)	10.00		
N-J	0 / 0	-18.5	-18.5 0.06 (4)	10.00		
J-O	0 / 518	-18.5	-18.5 0.12 (1)	10.00		
O-P	0 / 518	-18.5	-18.5 0.12 (1)	10.00		
P-Q	0 / 518	-18.5	-18.5 0.12 (1)	10.00		
Q-I	0 / 518	-18.5	-18.5 0.12 (1)	10.00		
I-R	0 / 0	-18.5	-18.5 0.06 (4)	10.00		
R-H	0 / 0	-18.5	-18.5 0.06 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-6-3	-7	-7	FRONT	VERT	DEAD		C1
C	3-6-3	-24	-24	FRONT	VERT	SNOW		C1
D	5-7-0	1	71	BACK	VERT	TOTAL		C1
E	7-7-13	-7	-7	FRONT	VERT	DEAD		C1
E	7-7-13	-24	-24	FRONT	VERT	SNOW		C1
L	3-11-4	1	57	BACK	VERT	TOTAL		C1
M	7-2-12	1	57	BACK	VERT	TOTAL		C1
N	1-11-4	1	57	BACK	VERT	TOTAL		C1
O	3-11-4	1	57	BACK	VERT	TOTAL		C1
P	5-7-0	1	71	BACK	VERT	TOTAL		C1
Q	7-2-12	1	71	BACK	VERT	TOTAL		C1
R	9-2-12	1	71	BACK	VERT	TOTAL		C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDITIONAL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.20/1.00 (B-C:1), BC=0.12/1.00 (I-J:1), WB=0.12/1.00 (F-I:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

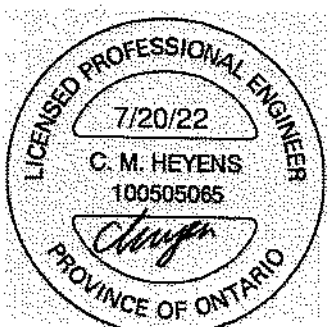


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425142	T70	1	1	ROYAL PINE HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:52:04 2022 Page 2
ID:9JkWqE1o0hmyuka0GZGHL1zZkTS-4l85P2ANiHt6EQSw9yD4KEYxQMai5PBiq5BICjvwCh

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



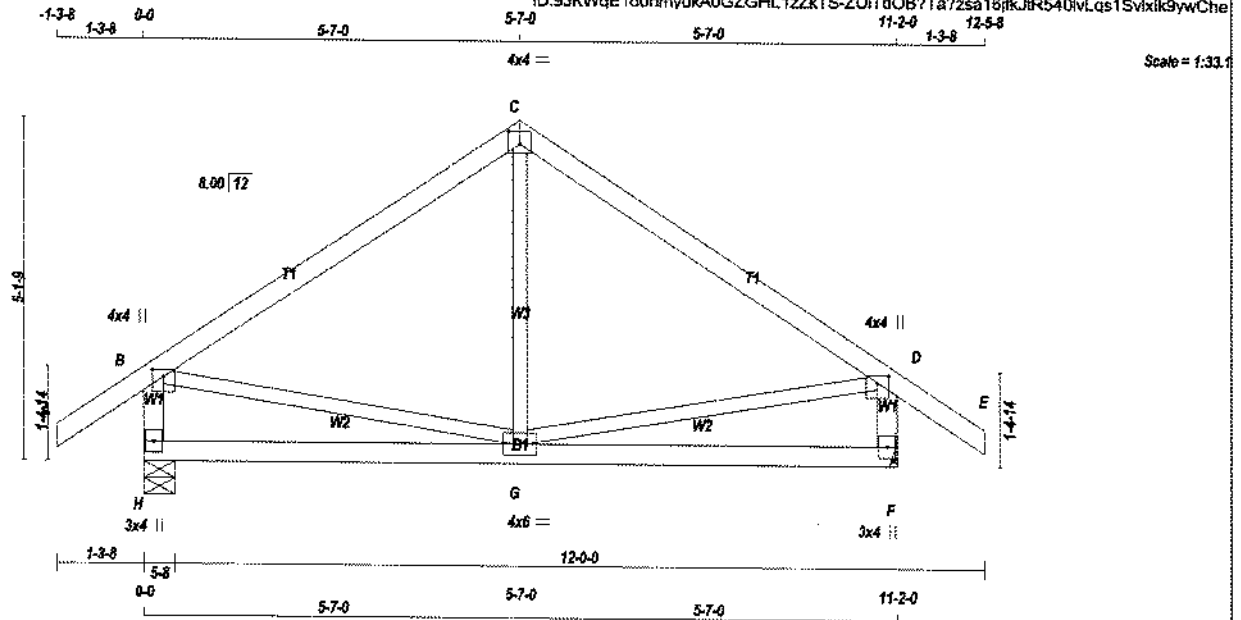
7/20/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR22070674 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T71	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mittek Industries, Inc. Wed Jul 20 10:52:05 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL1zZKTS-ZUITdOB7Ta7zsa18jfkJr540ivLqs1Svixik9ywChe



TOTAL WEIGHT = 46 lb [M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWV-1	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
H	694	0	0	5-8
F	694	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
H	490	324/0
F	490	324/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CS (LC)	MAX. UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO						FR-TO			
A-B	0/32	-84.9	-84.9	0.11 (1)	10.00	G-C	-8/80	0.03 (4)	
B-C	-419/0	-84.9	-84.9	0.34 (1)	6.25	B-G	0/356	0.08 (1)	
C-D	-419/0	-84.9	-84.9	0.34 (1)	6.25	G-D	0/356	0.08 (1)	
D-E	0/32	-84.9	-84.9	0.11 (1)	10.00				
H-B	-654/0	0.0	0.0	0.07 (1)	7.81				
F-D	-654/0	0.0	0.0	0.07 (1)	7.81				
H-G	0/0	-18.5	-18.5	0.16 (4)	10.00				
G-F	0/0	-18.5	-18.5	0.16 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(56% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.37")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.37")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSK TC=0.34/1.00 (B-C:1), BC=0.16/1.00 (G-H:4)
WB=0.08/1.00 (D-G:1), SB=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

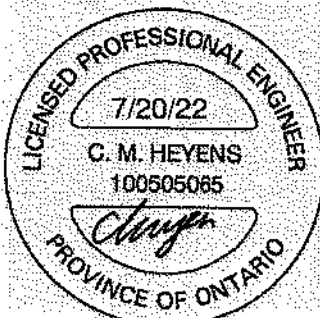
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.47 (B) (INPUT = 0.90)
JSI METAL = 0.14 (D) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070675

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T72	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:52:06 2022 Page 2
ID:9JKWqE1c0hmyukA0GZGHL1zZkTS-1qGsqkBDdEu7qUjcJHMFYPdJK9B7Z9ub7PgsGowChd

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMWW-t	MT20	4.0	8.0	2.00	2.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	5.0		
E	BMWW-t	MT20	4.0	10.0	5.00	1.75
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

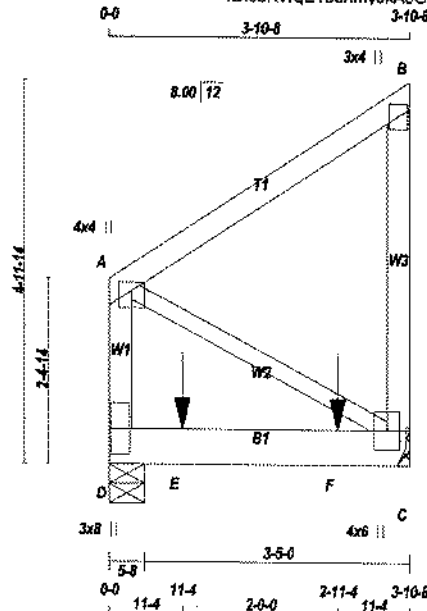


STRUCTURAL COMPONENT ONLY
DWG # TR22070676 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425142	T73	1	2	ROYAL PINE HOMES	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mytek Industries, Inc. Wed Jul 20 10:52:07 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL1zZkTS-VtqE23CF7CFh5tBv4mnyAT_ZaOlnkM3QPp2ywChc



Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 2 X 22 = 45 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
C - B	2x4 DRY	No.2	SPF		
D - A	2x4 DRY	No.2	SPF		
D - C	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B 1	12	TOP
B - C 1	12	TOP
D - A 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D - C 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TMV+p	MT20	3.0	4.0	
C	BMVW1+p	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
C	887	0	887	0
D	910	0	910	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
C	COMBINED	SNOW	LIVE
D	625	420 / 0	0 / 0
D	642	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. CSI (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO								
A-B	0 / 0	-84.9	-84.9	0.12 (1)	10.00	A-C	0 / 0	0.00 (1)
C-B	-164 / 0	0.0	0.0	0.03 (1)	7.81			
D-A	-164 / 0	0.0	0.0	0.01 (1)	7.81			
D-E	0 / 0	-18.5	-18.5	0.25 (1)	10.00			
E-F	0 / 0	-18.5	-18.5	0.25 (1)	10.00			
F-C	0 / 0	-18.5	-18.5	0.25 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	11-4	-507	-507	—	FRONT	VERT	TOTAL	—	C1
F	2-11-4	-476	-476	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.12/1.00 (A-B:1), BC=0.25/1.00 (C-D:1)
WB=0.00/1.00 (A-C:1), SS=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL = 0.03 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070677 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T73	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:52:07 2022 Page 2
ID:9JlKWgE1o0hmyukA0GZGHL1zZkTS-VtqE23CF7CFh5iBv4mnysAT ZaOlnnkM3QPp2ywrChg

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



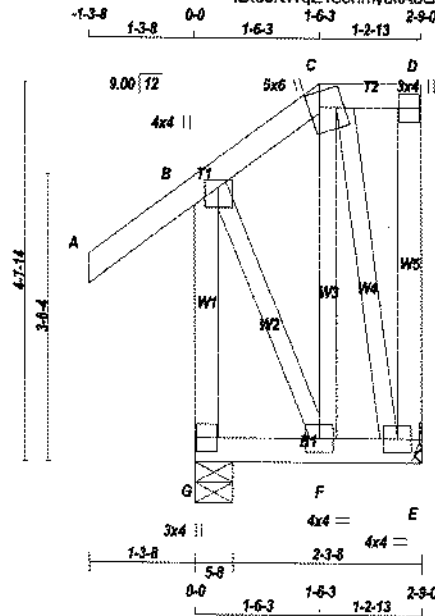
STRUCTURAL COMPONENT ONLY
DWG # TR22070677 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T74	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

ID:9JKWqE1o0hmyukA0GZGHL1zZKTS-VtqE23CF7CFh5BV4mnyATvZdAlnXkM3QPp2ywChc

Version 6.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:52:07 2022 Page 1



Scale = 1:27.2

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW+1	MT20	4.0	4.0		
F	BMVW-1	MT20	4.0	4.0		
G	BMV+1-p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
E	113	0	113	0
G	289	0	289	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	81	48 / -16	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
G	202	144 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	MEMB.	WEBS	MAX. FACTORED	FORCE	MAX	CS1 (LC)
FR-TO							FR-TO					
A-B	0 / 35	-84.9	-84.9	0.12 (5)	10.00		F-C	-5 / 25	0.01 (4)			
B-C	-50 / 0	-84.9	-84.9	0.12 (5)	6.25		C-E	-50 / 11	0.02 (1)			
C-D	0 / 0	-84.9	-84.9	0.02 (1)	10.00		B-F	-8 / 30	0.01 (1)			
E-D	-52 / 0	0.0	0.0	0.02 (1)	7.81							
G-B	-272 / 0	0.0	0.0	0.06 (1)	7.81							
G-F	0 / 0	-18.5	-18.5	0.01 (4)	10.00							
F-E	-3 / 14	-18.5	-18.5	0.01 (1)	10.00							

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS1: TC=0.12/1.00 (A-B:5), BC=0.01/1.00 (F-G:4)
WB=0.02/1.00 (C-E:1), SS=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

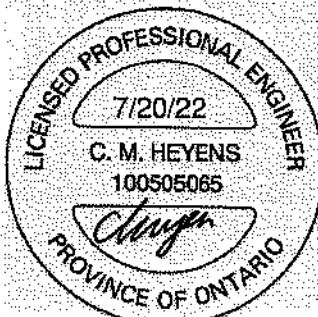
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



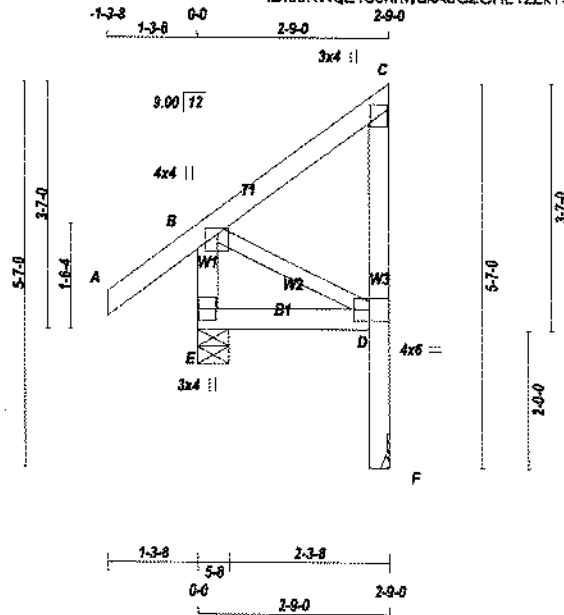
STRUCTURAL COMPONENT ONLY
DWG # TR22070678

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	T75	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

ID:9JKWqE1a0hmyukA0GZGH1zZKTS-z3NcFPDmVNY11hOn10U4iefzyz1E1ub19yLUyWChb

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:52:08 2022 Page 1



Scale: 3/8"=1'

TOTAL WEIGHT = 5 X 18 = 90 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	BVMW-i	MT20	4.0	6.0	2.00	2.50
E	BMV1+p	MT20	3.0	4.0		

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ
F	142	0	142	0
E	259	0	259	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	101	84/0	0/0	0/0	0/0	37/0	0/0
E	182	128/0	0/0	0/0	0/0	53/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	1C1 MAX	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
A-B	0/35	-84.9	-84.9	0.12 (5)	10.00	B-D	0/0	0.00 (1)	
B-C	0/0	-84.9	-84.9	0.11 (1)	10.00				
F-D	-142/0	0.0	0.0	0.02 (1)	7.81				
D-C	-117/0	0.0	0.0	0.03 (1)	7.81				
E-B	-234/0	0.0	0.0	0.02 (1)	7.81				
E-D	0/0	-18.5	-18.5	0.04 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	38.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:5), SC=0.04/1.00 (D-E:4)
WB=0.00/1.00 (B-D:1), SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

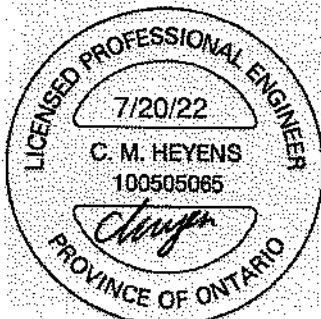
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	786

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

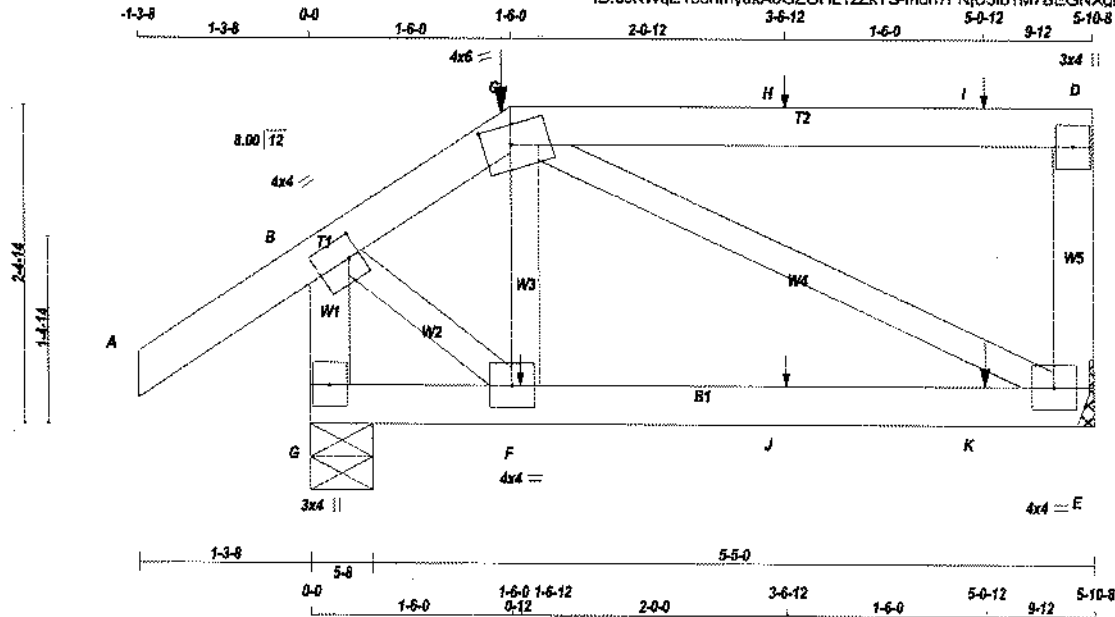


STRUCTURAL COMPONENT ONLY
DWG # TR22070679

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425144	T76	1	1	ROYAL PINE HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 11:04:45 2022 Page 1
ID:9JKWqE1o0hmyuka0GZGHL1ZZkTS-Ihdn7FNJUS181M7BEGNXqZzE7JDuaZJf 1WP6ywCVm



Scale = 1/16.7

TOTAL WEIGHT = 25 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
E - D	2x4	DRY	No.2	SPF		
G - B	2x4	DRY	No.2	SPF		
G - E	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-l	MT20	4.0	4.0	2.00	1.00
C TTVW-m	MT20	4.0	6.0	1.75	2.50
D TMVW-p	MT20	3.0	4.0		
E BMVW1-l	MT20	4.0	4.0		
F BMVW-l	MT20	4.0	4.0		
G BMVW1-p	MT20	3.0	4.0		

NOTES

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
E	370	0	0	MECHANICAL
G	667	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
E	262 173/0 0/0 0/0 0/0 89/0 0/0
G	466 336/0 0/0 0/0 0/0 130/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0/32	-84.9 -84.9 0.11 (1)	10.00	F-C	-144/24 0.02 (1)
B-C	-415/0	-84.9 -84.9 0.11 (1)	6.25	C-E	-346/0 0.12 (1)
C-H	0/0	-84.9 -84.9 0.28 (1)	10.00	B-F	0/385 0.09 (1)
H-I	0/0	-84.9 -84.9 0.28 (1)	10.00		
I-D	0/0	-84.9 -84.9 0.28 (1)	10.00		
E-D	-186/0	0.0 0.0 0.02 (1)	7.81		
G-B	-665/0	0.0 0.0 0.07 (1)	7.81		
G-F	0/0	-18.5 -18.5 0.07 (4)	10.00		
F-J	0/312	-18.5 -18.5 0.09 (1)	10.00		
J-K	0/312	-18.5 -18.5 0.09 (1)	10.00		
K-E	0/312	-18.5 -18.5 0.09 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	1-6-0	-216	-216	TOP	VERT	TOTAL	---	C1
C	1-6-0	1	1	BACK	VERT	TOTAL	---	C1
F	1-8-12	1	1	BACK	VERT	TOTAL	---	C1
H	3-6-12	1	1	BACK	VERT	TOTAL	---	C1
I	5-0-12	1	1	BACK	VERT	TOTAL	---	C1
J	3-6-12	1	1	BACK	VERT	TOTAL	---	C1
K	5-0-12	-0	-0	---	---	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOY CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CS1: TC=0.28/1.00 (C-D:1), BC=0.09/1.00 (E-F:1), WB=0.12/1.00 (C-E:1), SS=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

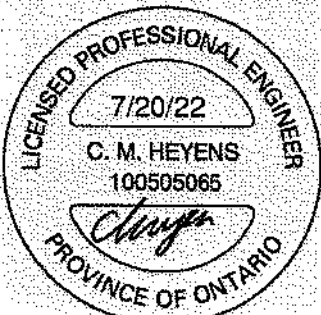
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

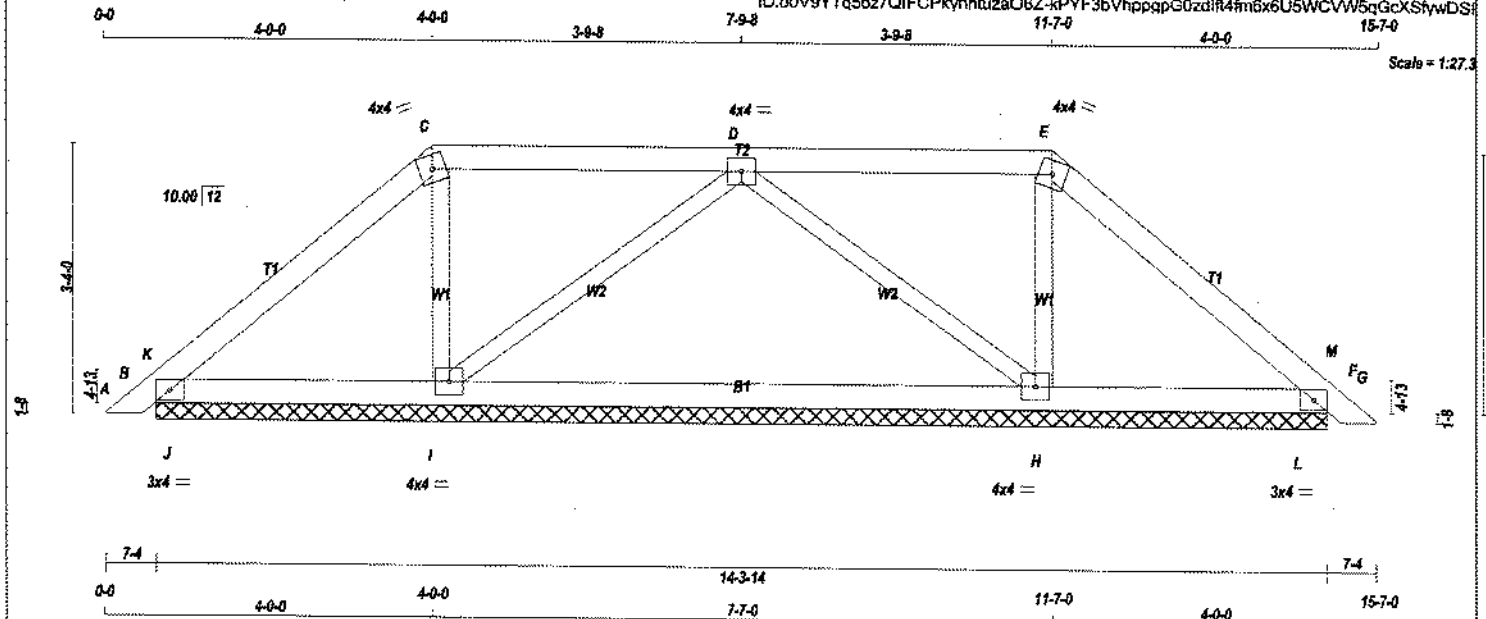


STRUCTURAL COMPONENT ONLY
DWG # TR22070693

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	PB1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:11 2022 Page 1
ID: o0V9YTqS0z7QIFCPkyhtuza06Z-kPYF3bVhpgpG0zdiR4m6x6U5WCVW5qGcXStywDS



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
C - E	2x4	DRY	No.2	SPF	SPF
E - G	2x4	DRY	No.2	SPF	SPF
G - I	2x4	DRY	No.2	SPF	SPF
I - K	2x4	DRY	No.2	SPF	SPF
K - M	2x4	DRY	No.2	SPF	SPF
M - O	2x4	DRY	No.2	SPF	SPF
O - Q	2x4	DRY	No.2	SPF	SPF
Q - S	2x4	DRY	No.2	SPF	SPF
S - U	2x4	DRY	No.2	SPF	SPF
U - W	2x4	DRY	No.2	SPF	SPF
W - Y	2x4	DRY	No.2	SPF	SPF
Y - A	2x4	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
C - E	2x4	DRY	No.2	SPF	SPF
E - G	2x4	DRY	No.2	SPF	SPF
G - I	2x4	DRY	No.2	SPF	SPF
I - K	2x4	DRY	No.2	SPF	SPF
K - M	2x4	DRY	No.2	SPF	SPF
M - O	2x4	DRY	No.2	SPF	SPF
O - Q	2x4	DRY	No.2	SPF	SPF
Q - S	2x4	DRY	No.2	SPF	SPF
S - U	2x4	DRY	No.2	SPF	SPF
U - W	2x4	DRY	No.2	SPF	SPF
W - Y	2x4	DRY	No.2	SPF	SPF
Y - A	2x4	DRY	No.2	SPF	SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-1	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMBW-1	MT20	4.0	4.0	
E	TTW-m	MT20	4.0	4.0	
F	TMB1-1	MT20	3.0	4.0	
H	BMWW1-1	MT20	4.0	4.0	
I	BMWW1-1	MT20	4.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP/LIFT		
B	220	0	220	0	14-3-14	14-3-14
F	220	0	220	0	14-3-14	14-3-14
H	564	0	564	0	14-3-14	14-3-14
I	564	0	564	0	14-3-14	14-3-14

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	153	116 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
F	153	116 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
H	403	242 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0
I	403	242 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO		
A-B	0 / 13	-84.9	-84.9 0.02 (1)	10.00	I-C	-218 / 0	0.04 (1)
B-K	0 / 96	-84.9	-84.9 0.08 (1)	10.00	I-D	-322 / 0	0.12 (1)
K-C	-45 / 0	-84.9	-84.9 0.11 (1)	6.25	D-H	-322 / 0	0.12 (1)
C-D	-19 / 0	-84.9	-84.9 0.20 (1)	6.25	H-E	-218 / 0	0.04 (1)
D-E	-19 / 0	-84.9	-84.9 0.20 (1)	6.25	J-K	-334 / 0	0.00 (1)
E-M	-45 / 0	-84.9	-84.9 0.11 (1)	6.25	L-M	-334 / 0	0.00 (1)
M-F	0 / 96	-84.9	-84.9 0.08 (1)	10.00			
F-G	0 / 13	-84.9	-84.9 0.02 (1)	10.00			
B-J	0 / 27	-18.5	-18.5 0.11 (1)	10.00			
J-I	0 / 27	-18.5	-18.5 0.17 (4)	10.00			
I-H	0 / 272	-18.5	-18.5 0.18 (4)	10.00			
H-L	0 / 27	-18.5	-18.5 0.17 (4)	10.00			
L-F	0 / 27	-18.5	-18.5 0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.20/1.00 (D-E:1), BC=0.18/1.00 (H-I:4), WB=0.12/1.00 (D-I:1), SS=0.25/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

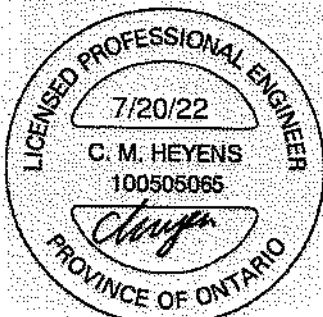
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (H) (INPUT = 0.90)
JSI METAL= 0.08 (H) (INPUT = 1.00)

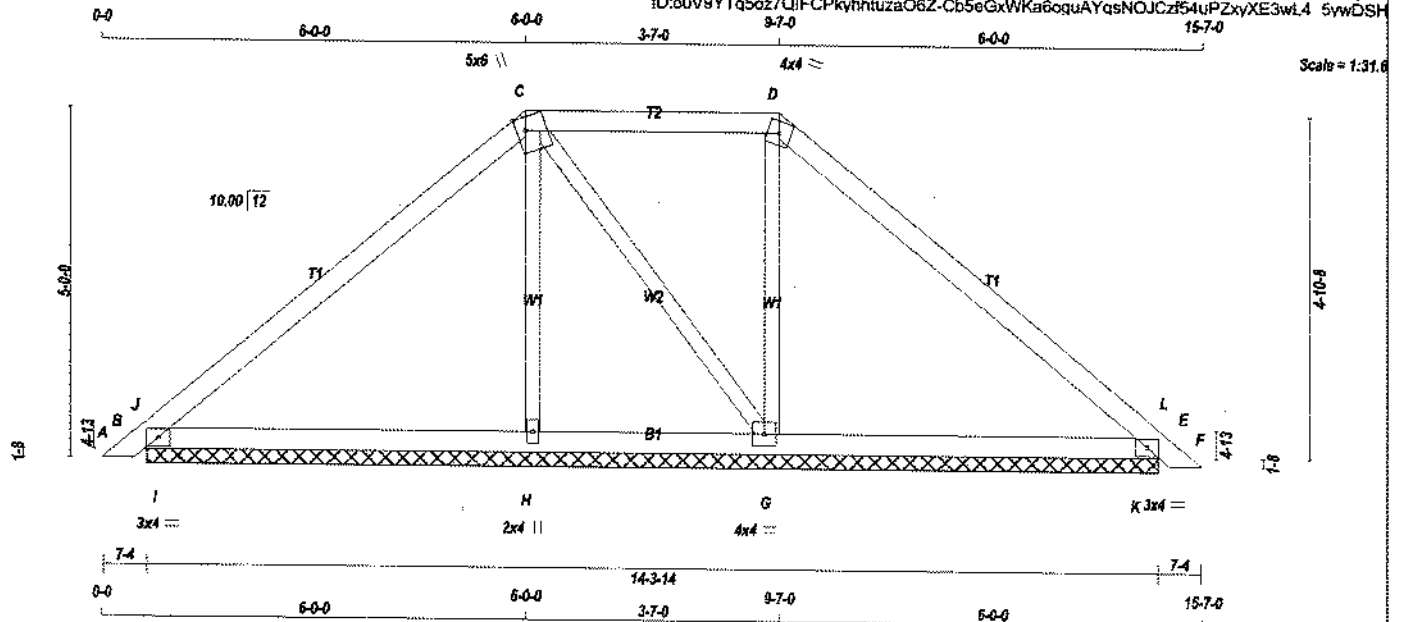


STRUCTURAL COMPONENT ONLY
DWG # TR22070564

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	PB2	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.536 S Feb 23 2022 MITek Industries, Inc. Wed Jul 26 10:00:12 2022 Page 1
ID:00V9YTq5oz7QlFCPkyhtuzaO6Z-Cb5eGxWKa6oguAYqsNOUCzF54uPZxyXE3wL4 5vwDSH



TOTAL WEIGHT = 50 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
	DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TTWW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMW1-I	MT20	4.0	4.0	
H	BMW1+w	MT20	2.0	4.0	

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ UPLIFT		
B	478	0	478	0	14-3-14	14-3-14
E	483	0	453	0	14-3-14	14-3-14
H	265	0	265	0	14-3-14	14-3-14
G	372	0	372	0	14-3-14	14-3-14

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	337	231 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
E	319	216 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
H	192	104 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
G	265	166 / 0	0 / 0	0 / 0	0 / 0	99 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 13	-84.9	-84.9	0.02 (1)	10.00	H-C	-138 / 0	0.05 (1)
B-J	-74 / 58	-84.9	-84.9	0.17 (1)	6.25	C-G	-51 / 0	0.03 (1)
J-C	-277 / 0	-84.9	-84.9	0.31 (1)	6.25	G-D	-203 / 0	0.07 (1)
C-D	-167 / 0	-84.9	-84.9	0.19 (1)	6.25	I-J	-681 / 0	0.00 (1)
D-L	-239 / 0	-84.9	-84.9	0.31 (1)	6.25	K-L	-886 / 0	0.00 (1)
L-E	-68 / 100	-84.9	-84.9	0.18 (1)	6.25			
E-F	0 / 13	-84.9	-84.9	0.02 (1)	10.00			
B-I	0 / 201	-18.5	-18.5	0.26 (1)	10.00			
I-H	0 / 201	-18.5	-18.5	0.26 (1)	10.00			
H-G	0 / 198	-18.5	-18.5	0.15 (1)	10.00			
G-K	0 / 172	-18.5	-18.5	0.26 (1)	10.00			
K-E	0 / 172	-18.5	-18.5	0.26 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-J:1), BC=0.26/1.00 (H-I:1).

WB=0.07/1.00 (D-G:1), SSI=0.54/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

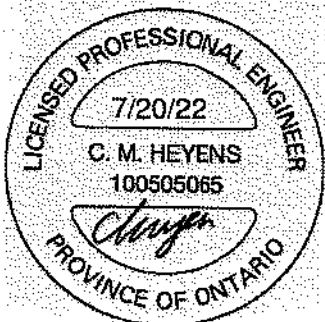
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1967 1873

PLATE PLACEMENT TOL. = 0.250 inches

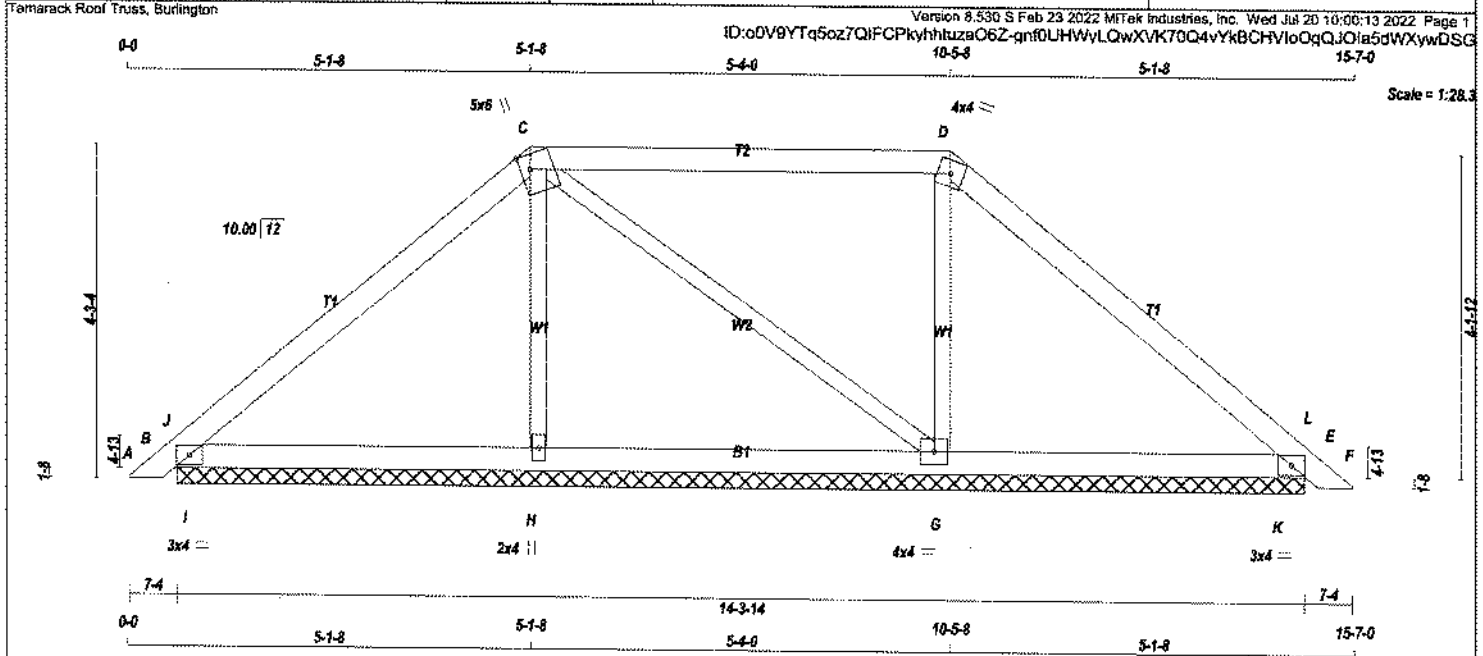
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070565

JOB NAME 425136	TRUSS NAME PB3	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 2 X 48 = 97 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1 12	TOP
C - D	1 12	TOP
D - F	1 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B - E	1 12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-J	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-J	MT20	3.0	4.0		
G	BMWVW1-J	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						
BEARINGS						
JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG IN-SX	REQD BRG IN-SX
B	403	0	403	0	14-3-14	14-3-14
E	375	0	375	0	14-3-14	14-3-14
H	357	0	357	0	14-3-14	14-3-14
G	434	0	434	0	14-3-14	14-3-14

UNFACTORED REACTIONS							
1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	282	198/0	0/0	0/0	0/0	84/0	0/0
E	264	183/0	0/0	0/0	0/0	80/0	0/0
H	257	146/0	0/0	0/0	0/0	111/0	0/0
G	308	198/0	0/0	0/0	0/0	121/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	LC1 MAX (PLF)	
FR-TO		FROM	TO	FR-TO				
A-B	0/13	-84.9	-84.9	0.01 (1)	10.00	H-C	-228/0	0.03 (1)
B-J	-49/0	-84.9	-84.9	0.05 (1)	6.25	C-G	-41/0	0.02 (1)
J-C	-224/0	-84.9	-84.9	0.11 (1)	6.25	G-D	-279/0	0.04 (1)
C-D	-123/0	-84.9	-84.9	0.21 (1)	6.25	I-J	-481/0	0.00 (1)
D-L	-182/0	-84.9	-84.9	0.11 (1)	6.25	K-L	-496/0	0.00 (1)
L-E	-40/36	-84.9	-84.9	0.05 (1)	6.25			
E-F	0/13	-84.9	-84.9	0.01 (1)	10.00			
B-I	0/163	-18.5	-18.5	0.09 (1)	10.00			
I-H	0/163	-18.5	-18.5	0.09 (1)	10.00			
H-G	0/156	-18.5	-18.5	0.06 (4)	10.00			
G-K	0/131	-18.5	-18.5	0.09 (1)	10.00			
K-E	0/131	-18.5	-18.5	0.09 (1)	10.00			

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD		=	38.7 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-D:1), BC=0.09/1.00 (B-I:1), WB=0.04/1.00 (D-G:1), SSI=0.19/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

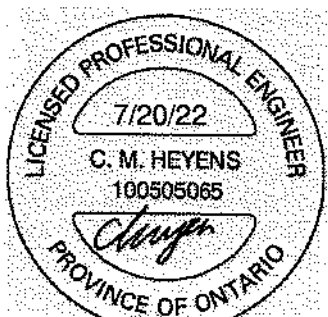
NAIL VALUES					
PLATE	GRIP (DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

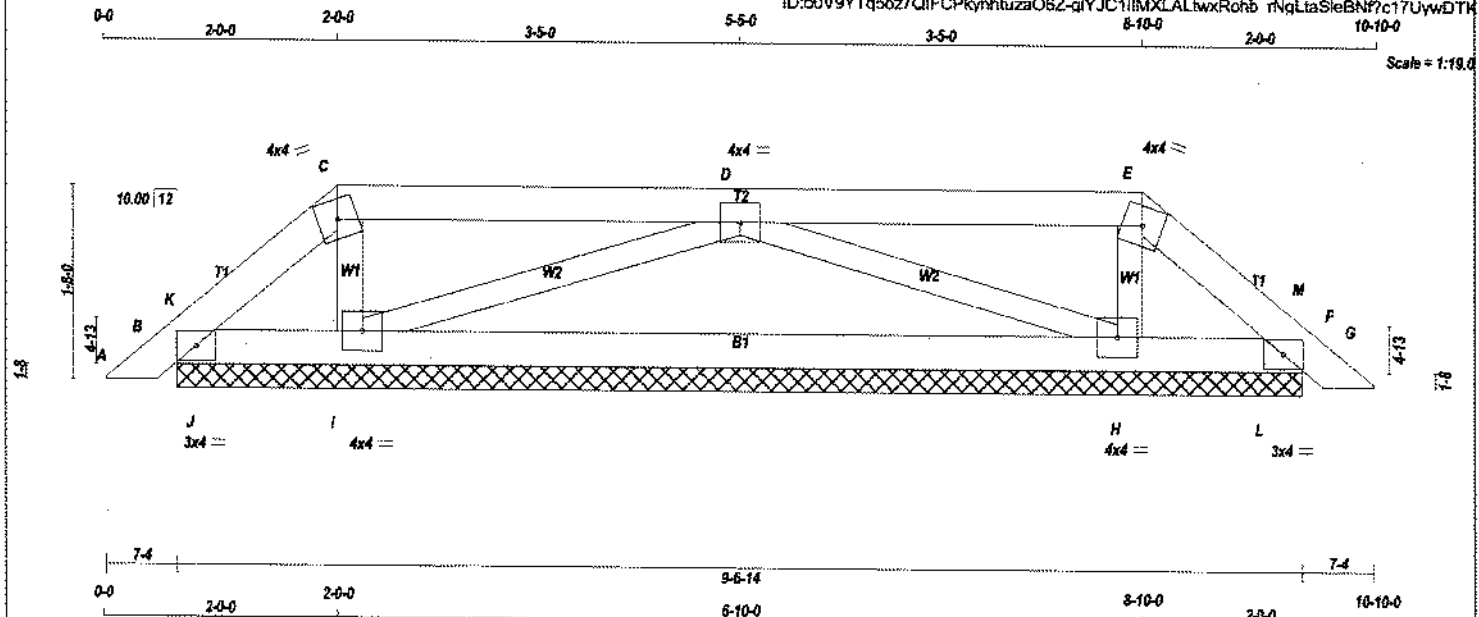
STRUCTURAL COMPONENT ONLY
DWG # TR22070566



STRUCTURAL COMPONENT ONLY
DWG # TR22070566

JOB NAME 425136	TRUSS NAME PB4	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 09:55:05 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhhtuza06Z-qIYJC1IMXLALhwRohb rNgLiaSleBNF?c17UyWDTM



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0			
C	TTW-m	MT20	4.0	4.0			
D	TTWW-t	MT20	4.0	4.0			
E	TTW-m	MT20	4.0	4.0			
F	TMB1-t	MT20	3.0	4.0			
H	BMWW1-t	MT20	4.0	4.0			
I	BMWW1-t	MT20	4.0	4.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
B	-8	0	0	0	9-8-14	9-8-14
F	-8	0	0	0	9-8-14	9-8-14
I	545	0	545	0	9-8-14	9-8-14
H	545	0	545	0	9-8-14	9-8-14

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	-8	14/0	0/0	0/0	0/0	0/-22	0/0
F	-8	14/0	0/0	0/0	0/0	0/-22	0/0
I	390	234/0	0/0	0/0	0/0	156/0	0/0
H	390	234/0	0/0	0/0	0/0	156/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4).

CHORDS				WESS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0/13	-84.9	-84.9	0.02 (1)	10.00	I-C	-229 / 0 0.03 (1)	
B-K	0/174	-84.9	-84.9	0.05 (4)	10.00	I-D	-500 / 0 0.12 (1)	
K-C	0/114	-84.9	-84.9	0.03 (1)	10.00	D-H	-500 / 0 0.12 (1)	
C-D	0/111	-84.9	-84.9	0.16 (1)	10.00	H-E	-229 / 0 0.03 (1)	
D-E	0/111	-84.9	-84.9	0.16 (1)	10.00	J-K	-142 / 0 0.00 (1)	
E-M	0/114	-84.9	-84.9	0.03 (1)	10.00	L-M	-142 / 0 0.00 (1)	
M-F	0/174	-84.9	-84.9	0.05 (4)	10.00			
F-G	0/13	-84.9	-84.9	0.02 (1)	10.00			

B-J	-92/0	-18.5	-18.5	0.03 (1)	6.25
J-I	-92/0	-18.5	-18.5	0.14 (4)	6.25
I-H	0/357	-18.5	-18.5	0.16 (4)	10.00
H-L	-92/0	-18.5	-18.5	0.14 (4)	6.25
L-F	-92/0	-18.5	-18.5	0.03 (1)	6.25

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 8 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TRC 2014

(5% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (C-D:1), BC=0.16/1.00 (H-I:4), WB=0.12/1.00 (D-I:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.60 (H) (INPUT = 0.90)
JSI METAL = 0.15 (I) (INPUT = 1.00)



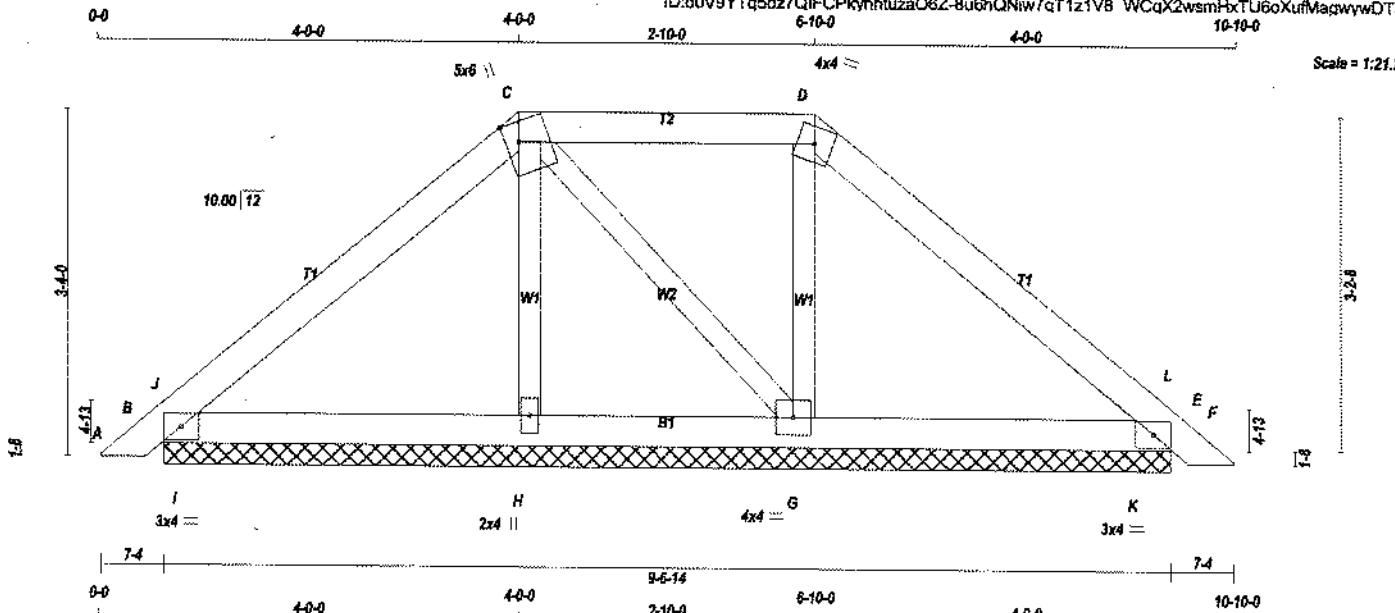
STRUCTURAL COMPONENT ONLY
DWG # TR22070554

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	PB5	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 09:59:06 2022 Page 1

ID:00V9YTq5oz7QlFCPlkyhhtuza06Z-8u6hQNiW7qT1z1V8 WCqX2wsmHxTU6oXufMagwywDTJ



Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-I	MT20	5.0	8.0	2.25	1.50
D	TTWW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW-I	MT20	4.0	4.0		
H	BMWW-I	MT20	2.0	4.0		

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	298	0	298	0	9-6-14	9-6-14
E	278	0	278	0	9-6-14	9-6-14
H	213	0	213	0	9-6-14	9-6-14
G	298	0	298	0	9-6-14	9-6-14

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	210	146 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
E	196	134 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
H	153	86 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
G	205	128 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED	MAX. FACTORED		MAX. FACTORED	MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (PLF)		MEMB.	FORCE (LBS)			LC1 (PLF)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 13	-84.9	-84.9	0.02 (1)	10.00	H-C	-127 / 0	0.02 (1)	
B-J	-44 / 0	-84.9	-84.9	0.05 (1)	6.25	C-G	-38 / 0	0.01 (1)	
J-C	-142 / 0	-84.9	-84.9	0.12 (1)	6.25	G-D	-175 / 0	0.03 (1)	
C-D	-73 / 0	-84.9	-84.9	0.12 (1)	6.25	I-J	-276 / 0	0.00 (1)	
D-L	-111 / 0	-84.9	-84.9	0.12 (1)	6.25	K-L	-281 / 0	0.00 (1)	
L-E	-36 / 0	-84.9	-84.9	0.05 (1)	6.25				
E-F	0 / 13	-84.9	-84.9	0.02 (1)	10.00				
B-I	0 / 103	-18.5	-18.5	0.11 (1)	10.00				
I-H	0 / 103	-18.5	-18.5	0.11 (1)	10.00				
H-G	0 / 99	-18.5	-18.5	0.06 (1)	10.00				
G-K	0 / 79	-18.5	-18.5	0.11 (1)	10.00				
K-E	0 / 79	-18.5	-18.5	0.11 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (D-L:1), SC=0.11/1.00 (B-I:1), WB=0.03/1.00 (D-G:1), SSI=0.22/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

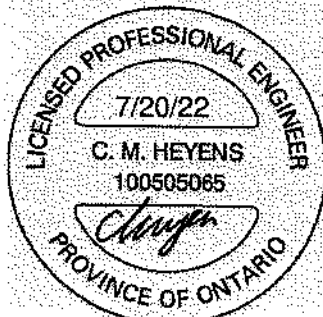
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.23 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070555

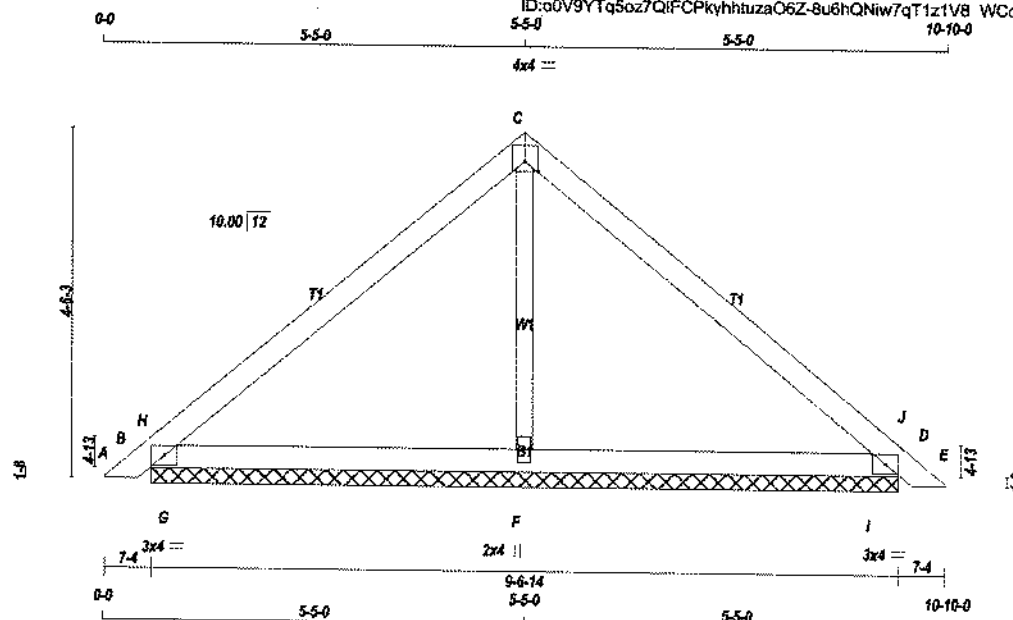
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	PB6	12	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 09:59:06 2022 Page 1

ID:00V9YTq5oz7QfPCPyhhtuza06Z-8u6hQNiW7qT1z1V8 WCqX2woxHvU6wXufMaqwywDT

Scale = 1:28.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	FTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1-w	MT20	2.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	392	0	392	0	9-8-14	9-8-14
D	392	0	392	0	9-8-14	9-8-14
F	295	0	295	0	9-8-14	9-8-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	275	191/0	0/0	0/0	0/0	84/0
D	275	191/0	0/0	0/0	0/0	84/0
F	213	113/0	0/0	0/0	0/0	100/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/13	-84.9	-84.9	0.02 (1)	10.00	F-C	-91/0
B-H	-34/93	-84.9	-84.9	0.16 (1)	6.25	G-H	-613/0
H-C	-209/0	-84.9	-84.9	0.24 (1)	6.25	I-J	-613/0
C-J	-209/0	-84.9	-84.9	0.24 (1)	6.25		0.00 (1)
J-D	-34/93	-84.9	-84.9	0.16 (1)	6.25		0.00 (1)
D-E	0/13	-84.9	-84.9	0.02 (1)	10.00		
B-G	0/149	-18.5	-18.5	0.22 (1)	10.00		
G-F	0/149	-18.5	-18.5	0.22 (1)	10.00		
F-I	0/149	-18.5	-18.5	0.22 (1)	10.00		
I-D	0/149	-18.5	-18.5	0.22 (1)	10.00		

TOTAL WEIGHT = 12 X 30 = 360 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-H:1), BC=0.22/1.00 (B-G:1)
WB=0.02/1.00 (C-F:1), SSI=0.47/1.00 (B-G:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

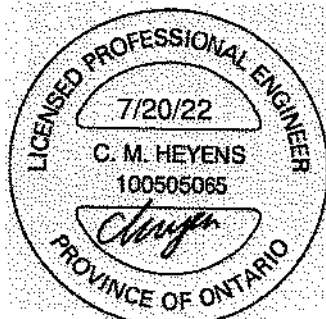
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (D) (INPUT = 0.80)
JSI METAL= 0.09 (D) (INPUT = 1.00)



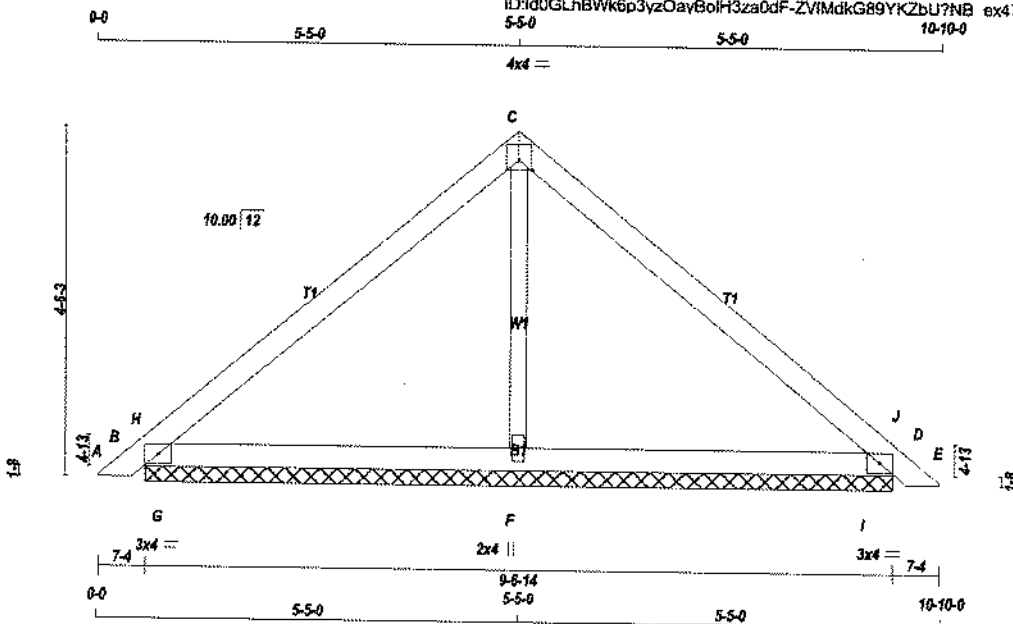
STRUCTURAL COMPONENT ONLY
DWG # TR22070556

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	PB6Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:30:10 2022 Page 1

ID:1d0GLhBWk6p3yzOayBoIH3za0dF-ZVIMdkG89YKZbU7NB ex477PNg6WYD8ZuWfuD5tywD0B



Scale = 1:28.6

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1 12		TOP
C-E 1 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B-D 1 12		TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B-TMB1-I	MT20	3.0	4.0			
C-TTW-p	MT20	4.0	4.0	1.50	2.00	
D-TMB1-I	MT20	3.0	4.0			
F-BMW1+w	MT20	2.0	4.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	393	0	393	0	9-6-14	9-6-14
D	393	0	393	0	9-6-14	9-6-14
F	292	0	292	0	9-6-14	9-6-14

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	276	191 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
D	276	191 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
F	212	112 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB. MAX. FORCE (LBS)	MAX. CS1 (LC)	MEMB. MAX. FORCE (LBS)	MAX. CS1 (LC)
FR-TO								
A-B	0 / 13	-84.9	-84.9	0.01 (1)	10.00	F-C	-89 / 0	0.01 (1)
B-H	-34 / 87	-84.9	-84.9	0.08 (1)	6.25	G-H	-806 / 0	0.00 (1)
H-C	-211 / 0	-84.9	-84.9	0.12 (1)	6.25	I-J	-806 / 0	0.00 (1)
C-J	-211 / 0	-84.9	-84.9	0.12 (1)	6.25			
J-D	-34 / 87	-84.9	-84.9	0.08 (1)	6.25			
D-E	0 / 13	-84.9	-84.9	0.01 (1)	10.00			
B-G	0 / 151	-18.5	-18.5	0.11 (1)	10.00			
G-F	0 / 151	-18.5	-18.5	0.11 (1)	10.00			
F-I	0 / 151	-18.5	-18.5	0.11 (1)	10.00			
I-D	0 / 151	-18.5	-18.5	0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.12/1.00 (C-H:1), BC=0.11/1.00 (F-G:1), WB=0.01/1.00 (C-F:1), SS=0.23/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

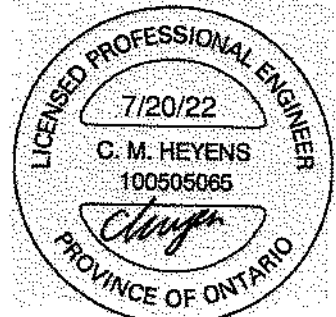
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



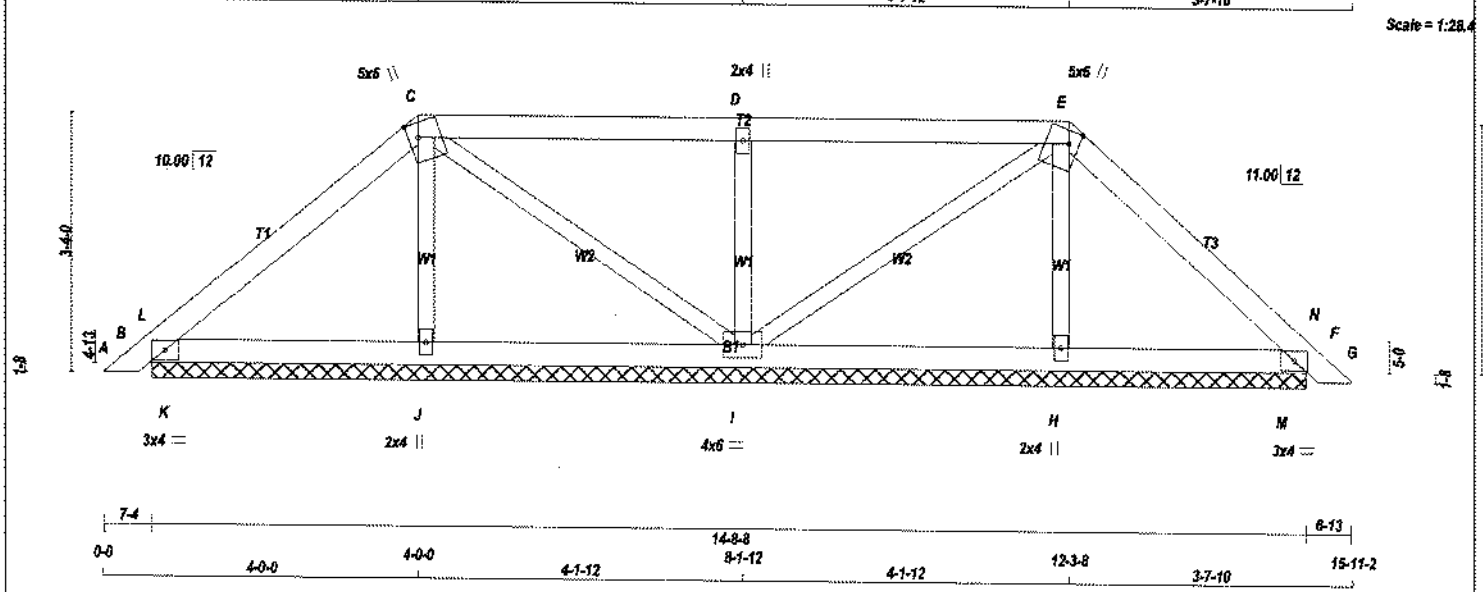
STRUCTURAL COMPONENT ONLY
DWG # TR22070609

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	PB30	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:11 2022 Page 1

ID:ld0GLhBWk6p3yzOayBoilH3za0dF-1hkr4HmwsSPDeaah9AcLqY64Soyai1JdmdKywDOA



LUMBER

N. L. G. A. RULES

CHORDS SIZE

ALL WEBS 2x3 DRY

DRY: SEASONED LUMBER.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 6.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2018

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 686-14

- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (C-D-1), BC=0.11/1.00 (B-K-1), WB=0.08/1.00 (D-I-1), SH=0.23/1.00 (B-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 550 371 1747 766 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)

JSI METAL = 0.09 (D) (INPUT = 1.00)

CHORDS	FACTORED	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CSI (LC)	LENGTH	FR-TO	FORCE (LBS)
FR-TO	0 / 13	-84.9	-84.9 0.02 (1)	10.00	J-C	-154 / 0
A-B	-25 / 1	-84.9	-84.9 0.05 (1)	6.25	C-I	-60 / 0
B-L	-108 / 0	-84.9	-84.9 0.12 (1)	6.25	I-D	-439 / 0
L-C	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	E-E	-54 / 0
C-D	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	H-E	-144 / 0
D-E	-21 / 0	-84.9	-84.9 0.09 (1)	6.25	K-L	-291 / 0
E-N	-105 / 0	-84.9	-84.9 0.04 (1)	6.25	M-N	-265 / 0
N-F	-21 / 0	-84.9	-84.9 0.02 (1)	10.00		
F-G	0 / 12	-84.9	-84.9 0.02 (1)	10.00		
G-K	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
K-J	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
J-I	0 / 71	-18.5	-18.5 0.07 (1)	10.00		
I-H	0 / 66	-18.5	-18.5 0.06 (1)	10.00		
H-M	0 / 71	-18.5	-18.5 0.09 (1)	10.00		
M-F	0 / 71	-18.5	-18.5 0.09 (1)	10.00		

CHORDS	FACTORED	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CSI (LC)	LENGTH	FR-TO	FORCE (LBS)
FR-TO	0 / 13	-84.9	-84.9 0.02 (1)	10.00	J-C	-154 / 0
A-B	-25 / 1	-84.9	-84.9 0.05 (1)	6.25	C-I	-60 / 0
B-L	-108 / 0	-84.9	-84.9 0.12 (1)	6.25	I-D	-439 / 0
L-C	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	E-E	-54 / 0
C-D	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	H-E	-144 / 0
D-E	-21 / 0	-84.9	-84.9 0.09 (1)	6.25	K-L	-291 / 0
E-N	-105 / 0	-84.9	-84.9 0.04 (1)	6.25	M-N	-265 / 0
N-F	-21 / 0	-84.9	-84.9 0.02 (1)	10.00		
F-G	0 / 12	-84.9	-84.9 0.02 (1)	10.00		
G-K	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
K-J	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
J-I	0 / 71	-18.5	-18.5 0.07 (1)	10.00		
I-H	0 / 66	-18.5	-18.5 0.06 (1)	10.00		
H-M	0 / 71	-18.5	-18.5 0.09 (1)	10.00		
M-F	0 / 71	-18.5	-18.5 0.09 (1)	10.00		

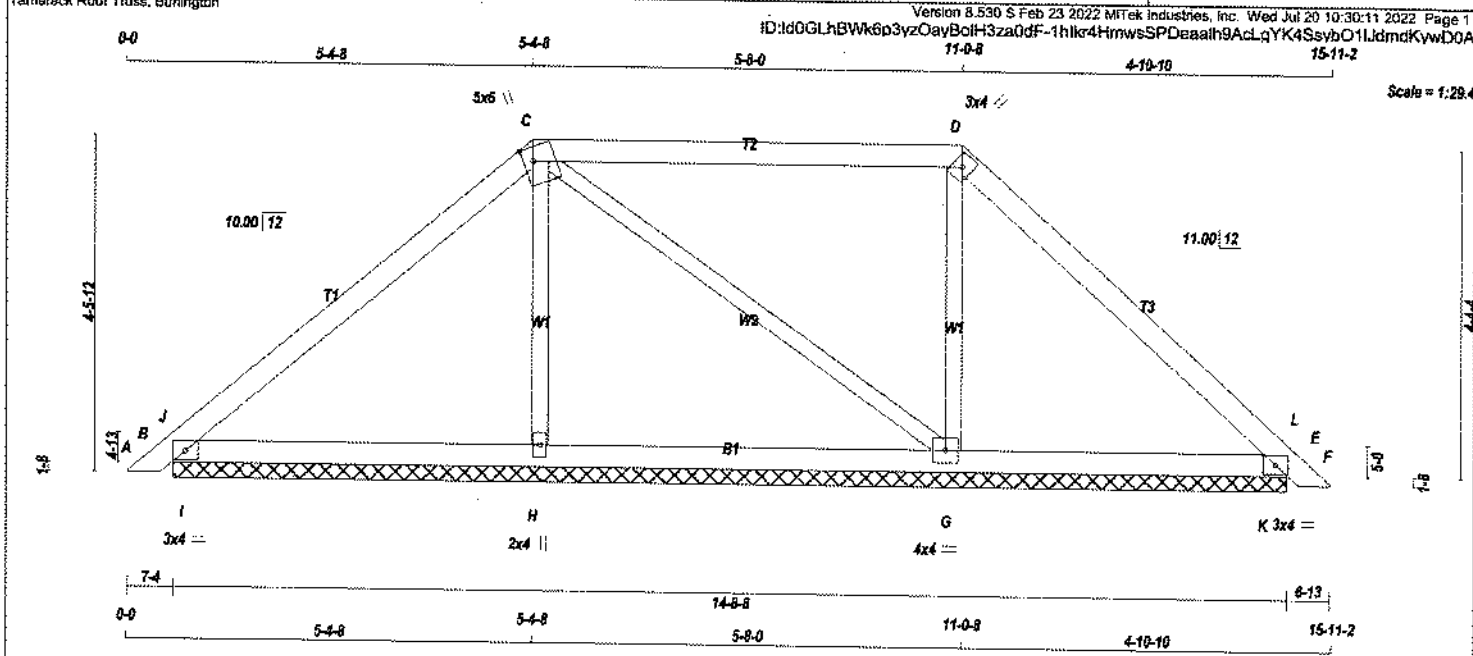
CHORDS	FACTORED	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CSI (LC)	LENGTH	FR-TO	FORCE (LBS)
FR-TO	0 / 13	-84.9	-84.9 0.02 (1)	10.00	J-C	-154 / 0
A-B	-25 / 1	-84.9	-84.9 0.05 (1)	6.25	C-I	-60 / 0
B-L	-108 / 0	-84.9	-84.9 0.12 (1)	6.25	I-D	-439 / 0
L-C	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	E-E	-54 / 0
C-D	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	H-E	-144 / 0
D-E	-21 / 0	-84.9	-84.9 0.09 (1)	6.25	K-L	-291 / 0
E-N	-105 / 0	-84.9	-84.9 0.04 (1)	6.25	M-N	-265 / 0
N-F	-21 / 0	-84.9	-84.9 0.02 (1)	10.00		
F-G	0 / 12	-84.9	-84.9 0.02 (1)	10.00		
G-K	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
K-J	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
J-I	0 / 71	-18.5	-18.5 0.07 (1)	10.00		
I-H	0 / 66	-18.5	-18.5 0.06 (1)	10.00		
H-M	0 / 71	-18.5	-18.5 0.09 (1)	10.00		
M-F	0 / 71	-18.5	-18.5 0.09 (1)	10.00		

CHORDS	FACTORED	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CSI (LC)	LENGTH	FR-TO	FORCE (LBS)
FR-TO	0 / 13	-84.9	-84.9 0.02 (1)	10.00	J-C	-154 / 0
A-B	-25 / 1	-84.9	-84.9 0.05 (1)	6.25	C-I	-60 / 0
B-L	-108 / 0	-84.9	-84.9 0.12 (1)	6.25	I-D	-439 / 0
L-C	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	E-E	-54 / 0
C-D	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	H-E	-144 / 0
D-E	-21 / 0	-84.9	-84.9 0.09 (1)	6.25	K-L	-291 / 0
E-N	-105 / 0	-84.9	-84.9 0.04 (1)	6.25	M-N	-265 / 0
N-F	-21 / 0	-84.9	-84.9 0.02 (1)	10.00		
F-G	0 / 12	-84.9	-84.9 0.02 (1)	10.00		
G-K	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
K-J	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
J-I	0 / 71	-18.5	-18.5 0.07 (1)	10.00		
I-H	0 / 66	-18.5	-18.5 0.06 (1)	10.00		
H-M	0 / 71	-18.5	-18.5 0.09 (1)	10.00		
M-F	0 / 71	-18.5	-18.5 0.09 (1)	10.00		

CHORDS	FACTORED	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CSI (LC)	LENGTH	FR-TO	FORCE (LBS)
FR-TO	0 / 13	-84.9	-84.9 0.02 (1)	10.00	J-C	-154 / 0
A-B	-25 / 1	-84.9	-84.9 0.05 (1)	6.25	C-I	-60 / 0
B-L	-108 / 0	-84.9	-84.9 0.12 (1)	6.25	I-D	-439 / 0
L-C	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	E-E	-54 / 0
C-D	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	H-E	-144 / 0
D-E	-21 / 0	-84.9	-84.9 0.09 (1)	6.25	K-L	-291 / 0
E-N	-105 / 0	-84.9	-84.9 0.04 (1)	6.25	M-N	-265 / 0
N-F	-21 / 0	-84.9	-84.9 0.02 (1)	10.00		
F-G	0 / 12	-84.9	-84.9 0.02 (1)	10.00		
G-K	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
K-J	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
J-I	0 / 71	-18.5	-18.5 0.07 (1)	10.00		
I-H	0 / 66	-18.5	-18.5 0.06 (1)	10.00		
H-M	0 / 71	-18.5	-18.5 0.09 (1)	10.00		
M-F	0 / 71	-18.5	-18.5 0.09 (1)	10.00		

CHORDS	FACTORED	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CSI (LC)	LENGTH	FR-TO	FORCE (LBS)
FR-TO	0 / 13	-84.9	-84.9 0.02 (1)	10.00	J-C	-154 / 0
A-B	-25 / 1	-84.9	-84.9 0.05 (1)	6.25	C-I	-60 / 0
B-L	-108 / 0	-84.9	-84.9 0.12 (1)	6.25	I-D	-439 / 0
L-C	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	E-E	-54 / 0
C-D	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	H-E	-144 / 0
D-E	-21 / 0	-84.9	-84.9 0.09 (1)	6.25	K-L	-291 / 0
E-N	-105 / 0	-84.9	-84.9 0.04 (1)	6.25	M-N	-265 / 0
N-F	-21 / 0	-84.9	-84.9 0.02 (1)	10.00		
F-G	0 / 12	-84.9	-84.9 0.02 (1)	10.00		
G-K	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
K-J	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
J-I	0 / 71	-18.5	-18.5 0.07 (1)	10.00		
I-H	0 / 66	-18.5	-18.5 0.06 (1)	10.00		
H-M	0 / 71	-18.5	-18.5 0.09 (1)	10.00		
M-F	0 / 71	-18.5	-18.5 0.09 (1)	10.00		

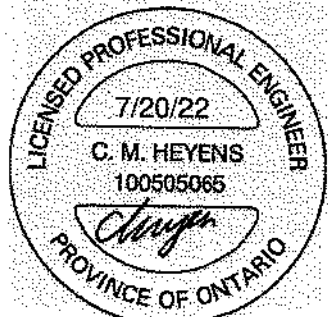
CHORDS	FACTORED	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CSI (LC)	LENGTH	FR-TO	FORCE (LBS)
FR-TO	0 / 13	-84.9	-84.9 0.02 (1)	10.00	J-C	-154 / 0
A-B	-25 / 1	-84.9	-84.9 0.05 (1)	6.25	C-I	-60 / 0
B-L	-108 / 0	-84.9	-84.9 0.12 (1)	6.25	I-D	-439 / 0
L-C	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	E-E	-54 / 0
C-D	-21 / 0	-84.9	-84.9 0.25 (1)	6.25	H-E	-144 / 0
D-E	-21 / 0	-84.9	-84.9 0.09 (1)	6.25	K-L	-291 / 0
E-N	-105 / 0	-84.9	-84.9 0.04 (1)	6.25	M-N	-265 / 0
N-F	-21 / 0	-84.9	-84.9 0.02 (1)	10.00		
F-G	0 / 12	-84.9	-84.9 0.02 (1)	10.00		
G-K	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
K-J	0 / 77	-18.5	-18.5 0.11 (1)	10.00		
J-I	0 / 71	-18.5	-18.5 0.07 (1)	10.00		
I-H	0 / 66	-18.5	-18.5 0.06 (1)	10.00		
H-M	0 / 71	-18.5	-18.5 0.09 (1)	10.00		
M-F	0 / 71	-18.5	-18.5 0.09 (1)	10.00		



TOTAL WEIGHT = 2 X 50 = 100 lb									
DESIGN CRITERIA									
SPECIFIED LOADS:									
TOP CH. LL = 23.3 PSF									
DL = 6.0 PSF									
BOT CH. LL = 0.0 PSF									
DL = 7.4 PSF									
TOTAL LOAD = 36.7 PSF									
SPACING = 24.0 IN. C/C									
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2015									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF BCBC 2018, ABC 2019									
- PART 9 OF OBC 2012 (2010 AMENDMENT)									
- CSA 086-14									
- TPIC 2014									
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD									
CSK TC=0.23/1.00 (C-D:1), BC=0.10/1.00 (B-I:1), WB=0.04/1.00 (D-G:1), SS=0.21/1.00 (B-I:1)									
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10									
COMPANION LIVE LOAD FACTOR = 1.00									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
NAIL VALUES									
PLATE GRIP(DRY) SHEAR SECTION									
(RFL)									

LUMBER									
N.L.G.A. RULES									
CHORDS SIZE LUMBER DESCR.									
A - C 2x4 DRY No.2 SPF									
C - D 2x4 DRY No.2 SPF									
D - F 2x4 DRY No.2 SPF									
B - E 2x4 DRY No.2 SPF									
ALL WEBS 2x3 DRY No.2 SPF									
DRY: SEASONED LUMBER.									
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:									
CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)									
TOP CHORDS : (0.122"x3") SPIRAL NAILS									
A-C 1 12 TOP									
C-D 1 12 TOP									
D-F 1 12 TOP									
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS									
B-E 1 12 TOP									
WEBS : (0.122"x3") SPIRAL NAILS									
2x3 1 6									
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.									
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.									
PLATES (table is in inches)									
JT TYPE PLATES W LEN Y X									
B TMB1+1 MT20 3.0 4.0									
C TTWW+m MT20 5.0 6.0 2.25 1.50									
D TTW+h MT20 3.0 4.0									
E TMB1+1 MT20 3.0 4.0									
G BMWW1+1 MT20 4.0 4.0									
H BMW1+w MT20 2.0 4.0									
NOTES- (1)									
1) Lateral braces to be a minimum of 2X4 SPF #2.									

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT BRG REQD BRG									
JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX									
B 422 0 422 0 0 14-8-8 14-8-8									
E 376 0 376 0 0 14-8-8 14-8-8									
H 380 0 380 0 0 14-8-8 14-8-8									
G 428 0 428 0 0 14-8-8 14-8-8									
UNFACTORED REACTIONS									
1ST LCASE MAX/MIN COMPONENT REACTIONS									
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL									
B 298 206 / 0 0 / 0 0 / 0 0 / 0 88 / 0 0 / 0									
E 264 185 / 0 0 / 0 0 / 0 0 / 0 79 / 0 0 / 0									
H 273 155 / 0 0 / 0 0 / 0 0 / 0 119 / 0 0 / 0									
G 305 184 / 0 0 / 0 0 / 0 0 / 0 121 / 0 0 / 0									
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNGRADED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS WEBS									
MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX. CSI (LC) MAX. UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. CSI (LC)									
FR-TO FROM TO									
A-B 0 / 13 -84.9 -84.9 0.01 (1) 10.00 H-C -239 / 0 0.03 (1)									
B-J -47 / 13 -84.9 -84.9 0.06 (1) 6.25 C-G -49 / 0 0.02 (1)									
J-C -237 / 0 -84.9 -84.9 0.12 (1) 6.25 G-D -272 / 0 0.04 (1)									
C-D -126 / 0 -84.9 -84.9 0.23 (1) 6.25 I-J -539 / 0 0.00 (1)									
D-L -194 / 0 -84.9 -84.9 0.10 (1) 6.25 K-L -489 / 0 0.00 (1)									
L-E -35 / 46 -84.9 -84.9 0.05 (1) 6.25									
E-F 0 / 12 -84.9 -84.9 0.01 (1) 10.00									
B-I 0 / 172 -18.5 -18.5 0.10 (1) 10.00									
F-H 0 / 172 -18.5 -18.5 0.10 (1) 10.00									
H-G 0 / 166 -18.5 -18.5 0.07 (4) 10.00									
G-K 0 / 133 -18.5 -18.5 0.08 (1) 10.00									
K-E 0 / 133 -18.5 -18.5 0.08 (1) 10.00									

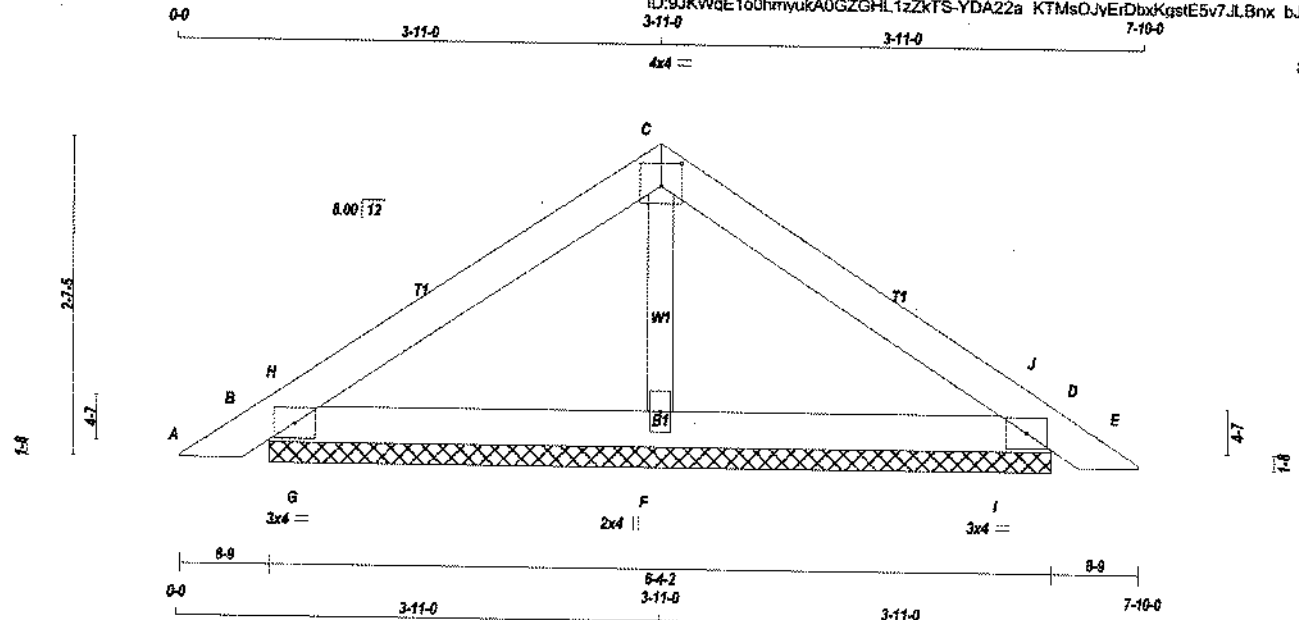


STRUCTURAL COMPONENT ONLY
DWG # TR22070611

JOB NAME 425142	TRUSS NAME PB51	QUANTITY 14	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.530 S Feb 23 2022 Mitak Industries, Inc. Wed Jul 20 10:51:48 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL1zZkTS-YDA22a KTMsoJyErDhxKgstE5v7JL8nx bJveeywChv
3-11-0 7-10-0

Scale = 1:18.1



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
B - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1-w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
B	251	0	0	0
D	251	0	0	0
F	263	0	0	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	176	123 / 0	0 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
D	176	123 / 0	0 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
F	189	109 / 0	0 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 14	-84.9	-84.9 0.03 (1)	10.00	F-C	-123 / 0	0.02 (1)
B-H	-33 / 0	-84.9	-84.9 0.04 (1)	6.25	G-H	-232 / 0	0.00 (1)
H-C	-85 / 0	-84.9	-84.9 0.10 (1)	6.25	I-J	-232 / 0	0.00 (1)
C-J	-85 / 0	-84.9	-84.9 0.10 (1)	6.25			
J-D	-33 / 0	-84.9	-84.9 0.04 (1)	6.25			
D-E	0 / 14	-84.9	-84.9 0.03 (1)	10.00			
B-G	0 / 74	-18.5	-18.5 0.10 (1)	10.00			
G-F	0 / 74	-18.5	-18.5 0.10 (1)	10.00			
F-I	0 / 74	-18.5	-18.5 0.10 (1)	10.00			
I-D	0 / 74	-18.5	-18.5 0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.10/1.00 (C-H:1), BC=0.10/1.00 (B-G:1), WB=0.02/1.00 (C-F:1), SSI=0.17/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

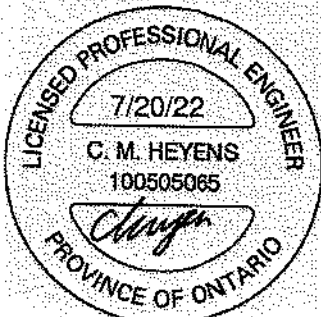
NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)



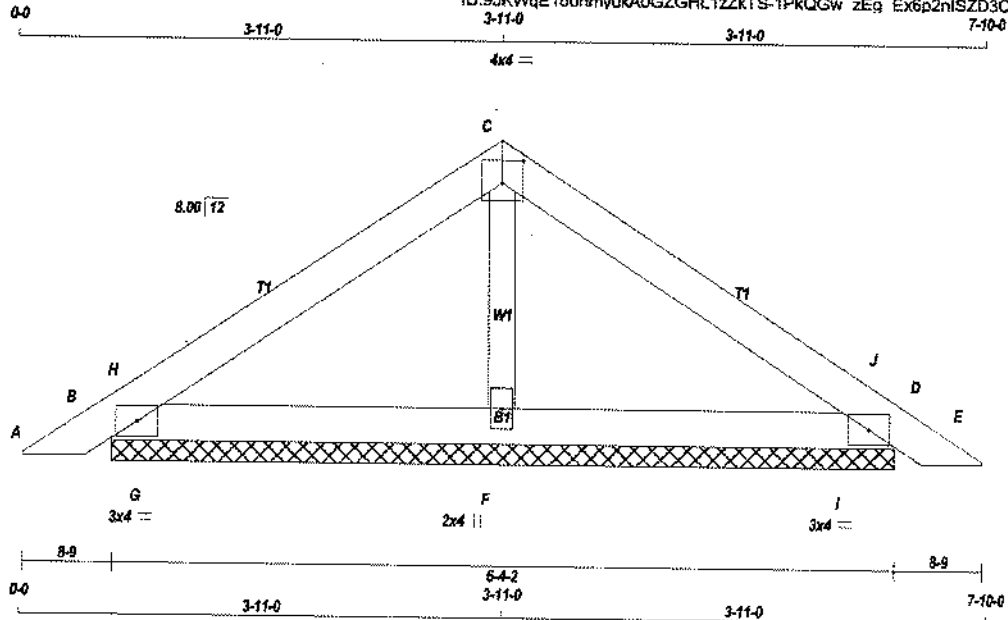
STRUCTURAL COMPONENT ONLY
DWG # TR22070657

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	PB51Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:51:49 2022 Page 1

ID:9JKWqE1o0hmykA0GZGH1zZKTS-1PkQGw zEg Ex6p2nISZD3QOdJTM4eBSDF3SA5ywChu



Scale = 1/8" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B - D	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	YMB1-f	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	YMB1-f	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	251	0	251	0	6-4-2	6-4-2
D	251	0	251	0	6-4-2	6-4-2
F	252	0	252	0	6-4-2	6-4-2

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
B	176	123 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
D	176	123 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
F	188	109 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED	FACTORED			MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED	FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	FORCE (LBS)			MAX. (LC)		
FR-TO		FROM	TO			FR-TO			
A-B	0 / 14	-84.9	-84.9	0.01 (1)	10.00	F-C	-123 / 0		0.01 (1)
B-H	-35 / 0	-84.9	-84.9	0.02 (1)	6.25	G-H	-230 / 0		0.00 (1)
H-C	-95 / 0	-84.9	-84.9	0.05 (1)	6.25	I-J	-230 / 0		0.00 (1)
C-J	-95 / 0	-84.9	-84.9	0.05 (1)	6.25				
J-D	-35 / 0	-84.9	-84.9	0.02 (1)	6.25				
D-E	0 / 14	-84.9	-84.9	0.01 (1)	10.00				
B-G	0 / 75	-18.5	-18.5	0.05 (1)	10.00				
G-F	0 / 75	-18.5	-18.5	0.05 (1)	10.00				
F-I	0 / 75	-18.5	-18.5	0.05 (1)	10.00				
I-D	0 / 75	-18.5	-18.5	0.05 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 38.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TFC 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-J-1), BC=0.05/1.00 (F-I-1), WB=0.01/1.00 (G-F-1), SSI=0.03/1.00 (B-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (D) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

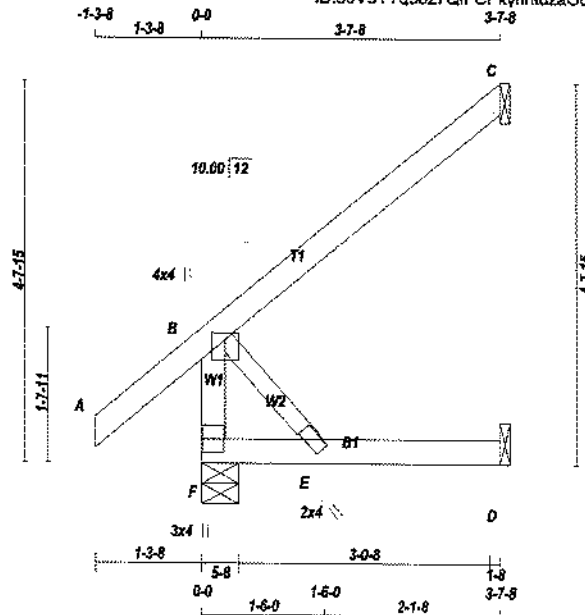


STRUCTURAL COMPONENT ONLY
DWG # TR22070658

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	J1	20	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 10:00:11 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhhtuza06Z-kPYF3bVhppgpG0zdRf4fm6yGU6FCWN5qGcXStywDS



Scale = 1:27.1

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	305	0	305	0	0	5-8	5-8
C	154	0	154	0	0	1-8	1-8
D	34	0	38	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	214	149 / 0	0 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
C	108	84 / 0	0 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0
D	27	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	UNBRAC LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
F-B	-271 / 0	0.0	0.0	0.03 (1)	7.81	10.00	F-B	0.0	0.00 (1)		
A-B	0 / 38	-84.9	-84.9	0.13 (5)	10.00		B-E	0.0	0.00 (1)		
B-C	0 / 0	-84.9	-84.9	0.19 (1)	10.00						
F-E	0 / 0	-18.5	-18.5	0.07 (4)	10.00						
E-D	0 / 0	-18.5	-18.5	0.07 (4)	10.00						

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CS1: TC=0.19/1.00 (B-C:1), BC=0.07/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

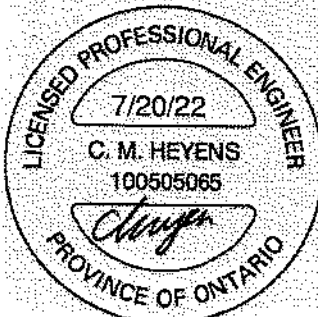
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

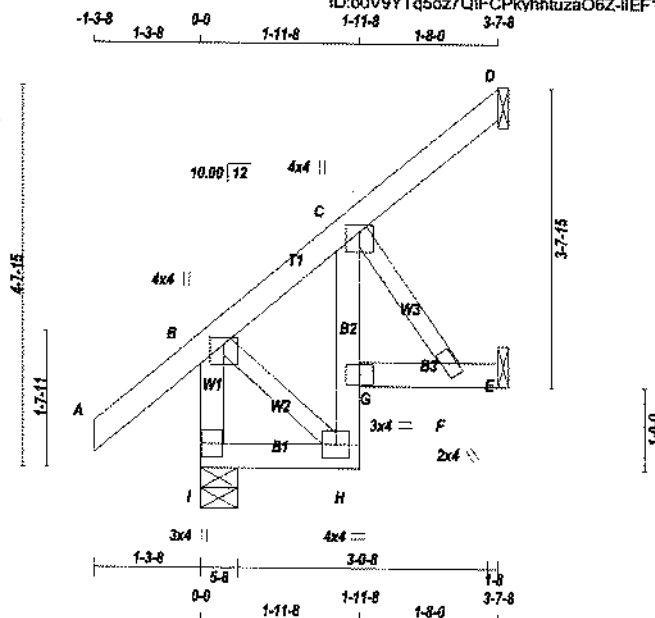


STRUCTURAL COMPONENT ONLY
DWG # TR22070563

JOB NAME 425137	TRUSS NAME J1S	QUANTITY 6	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
---------------------------	--------------------------	----------------------	-----------------	--------------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:02:10 2022 Page 1
ID:00V9YTq5oz7QIFCPkyhtuza06Z-IIeF1bLoNfw2IZVm9Vz sYI7Sk1bKYcCHwK7FywDQR



Scale = 1:27.1

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	No.2	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY, SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TMVW+p	MT20	4.0	4.0	1.00 2.00
F	BMVW+p	MT20	2.0	4.0	
G	BMVW+p	MT20	3.0	4.0	
H	BMVW+p	MT20	4.0	4.0	
I	BMVW+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
I	327	0	327	0	5-8	5-8
D	79	0	79	0	1-8	1-8
E	86	0	86	0	1-8	1-8

SEE MITEK STANDARD DETAIL MS02015-H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	DOWN				
I	230	161/0	0/0	0/0	0/0	0/0	68/0	0/0
D	65	42/0	0/0	0/0	0/0	0/0	13/0	0/0
E	83	30/-16	0/0	0/0	0/0	0/0	33/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FACTORED CS1 (LC)
FR-TO		FROM TO		FR-TO			
I-B	-311/0	0.0	0.0 0.03 (1)	B-H	-7/55	10.00	0.01 (1)
A-B	0/38	-84.9	-84.9 0.13 (5)	C-F	-123/15	10.00	0.02 (1)
B-C	-88/0	-84.9	-84.9 0.13 (5)				
C-D	0/5	-84.9	-84.9 0.05 (1)				
I-H	0/0	-18.5	-18.5 0.02 (4)				
H-G	-15/16	0.0	0.0 0.06 (1)				
G-C	0/38	0.0	0.0 0.07 (1)				
G-F	-9/69	-18.5	-18.5 0.08 (1)				
F-E	0/0	-18.5	-18.5 0.06 (1)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 6 X 19 = 115 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CS1: TC=0.13/1.00 (A-B:5), BC=0.08/1.00 (F-G:1), WB=0.02/1.00 (C-F:1), SS=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

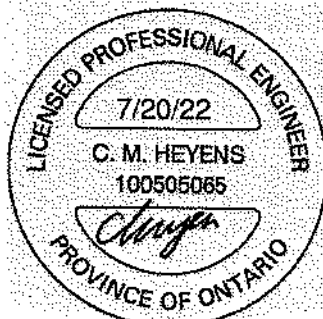
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

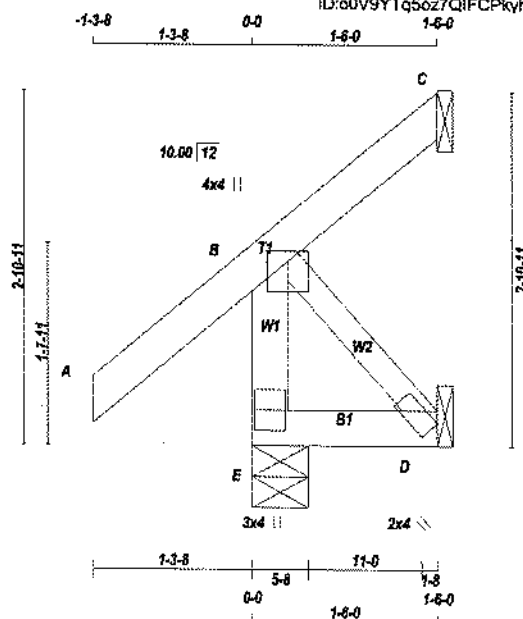


STRUCTURAL COMPONENT ONLY
DWG # TR22070585

JOB NAME 425138	TRUSS NAME J2	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWS NO.
--------------------	------------------	---------------	----------	-------------------------------	----------

Tamarrack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:15:33 2022 Page 1
ID: a0V9YTq5oz7QIFCPkyhhtuzaO6Z-knFpZsf1h5q 6SqJlPH1BIXl0pvEveorjyJoDywdDu



Scale = 1/16\"/>

LUMBER				DESCR.	
N.L.G.A. RULES	SIZE	LUMBER	No.	SPF	
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW1+w	MT20	2.0	4.0		Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
E	248	0	248	0	5-8
C	9	0	9	0	1-8
D	13	0	14	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	173	129 / 0	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
C	6	5 / -30	0 / 0	0 / 0	0 / 0	0 / 0	1 / 0	0 / 0
D	10	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
E-B	-235 / 0	E-D	0 / 0
A-B	0 / 38		
B-C	-35 / 0		
E-D	0 / 0		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), SC=0.01/1.00 (D-E:4)
WB=0.00/1.00 (B-D:1), SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

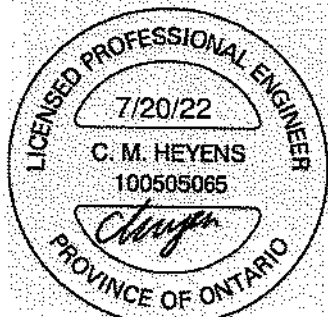
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	850	321	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



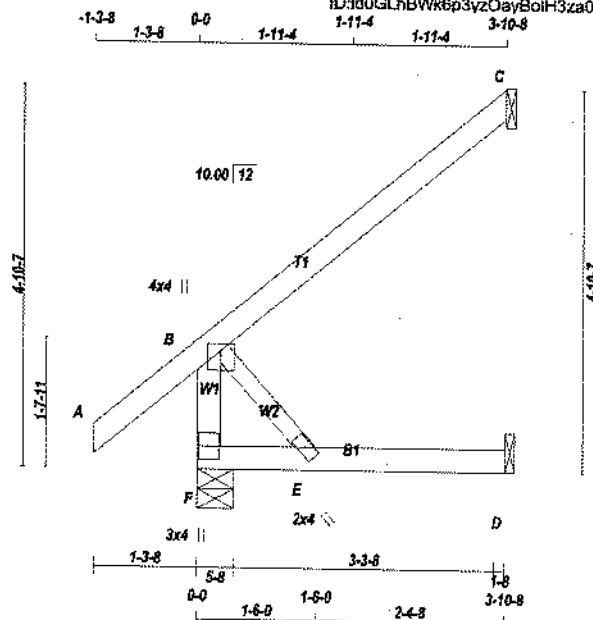
STRUCTURAL COMPONENT ONLY
DWG # TR22070590

JOB NAME 425139	TRUSS NAME J30	QUANTITY 17	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
--------------------	-------------------	----------------	----------	-------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:08 2022 Page 1

ID:Id0GLhBWk6p3yz0ayBoIH3za0dF-d6dbD3Fudx3rMAr73ZcT I21LsRVIEFb3LP607ywd0D0



Scale = 1:28.1

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	DRY
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	318	0	318	0	5-8	5-8
C	164	0	164	0	1-8	1-8
D	36	0	40	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	223	155 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
C	113	90 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LCT (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LCT (LC)
FR-TO		FROM	TO		FR-TO		
F-B	-282 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0
A-B	0 / 38	-84.9	-84.9	0.14 (5)	10.00		0.00 (1)
B-C	0 / 0	-84.9	-84.9	0.22 (1)	10.00		
F-E	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
E-D	0 / 0	-18.5	-18.5	0.06 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 17 X 16 = 257 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.22/1.00 (B-C:1), BC=0.06/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (B-C:1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

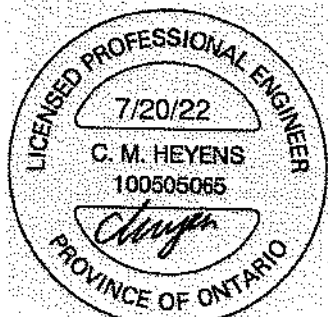
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 786 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

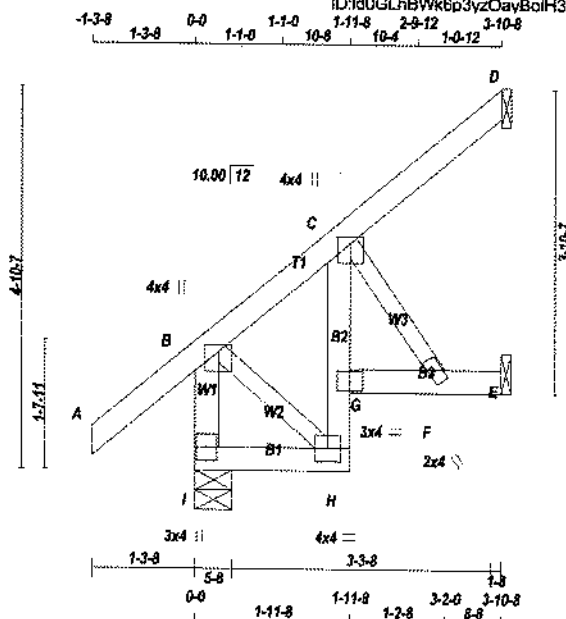


STRUCTURAL COMPONENT ONLY
DWG # TR22070606

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425140	J30S	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:32:41 2022 Page 1
ID:1d0GLhBWk6p3yzOavBoIH3za0dF-eYMRKNSbX17Ch2PXCsMhztC7Yfv5Rm6EfdydwCzq



Scale = 1/8" = 1'-0"

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
I - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - C	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TMVW+p	MT20	4.0	4.0	1.00 2.00
F	BMVW+w	MT20	2.0	4.0	
G	BMVH	MT20	3.0	4.0	
H	BMVW-i	MT20	4.0	4.0	
I	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
I	339	0	339	0	5-8	5-8
D	99	0	99	0	1-8	1-8
E	90	0	90	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
I	238	186 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
D	62	47 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
E	66	31 / -14	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	WEBS		MAX. FACTORED
	FORCE (LBS)	VERT. LOAD (PL)	LC1 MAX	CS1 (LC)		FORCE (LBS)	MAX. CS1 (LC)	
FR-TO								
I-B	-322 / 0	0.0	0.0	0.03 (1)	7.81	B-H	-5 / 64	0.01 (1)
A-B	0 / 38	-84.9	-84.9	0.14 (5)	10.00	C-F	-144 / 12	0.02 (1)
B-C	-98 / 0	-84.9	-84.9	0.13 (5)	6.25			
C-D	0 / 4	-84.9	-84.9	0.06 (1)	10.00			
I-H	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
H-G	-21 / 15	0.0	0.0	0.07 (1)	7.81			
G-C	0 / 47	0.0	0.0	0.08 (1)	10.00			
G-F	-6 / 81	-18.5	-18.5	0.11 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.09 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS1: TC=0.14/1.00 (A-B:5), BC=0.11/1.00 (F-G:1)
WB=0.02/1.00 (C-F:1), SS=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

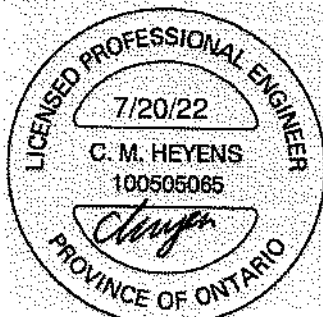
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070629

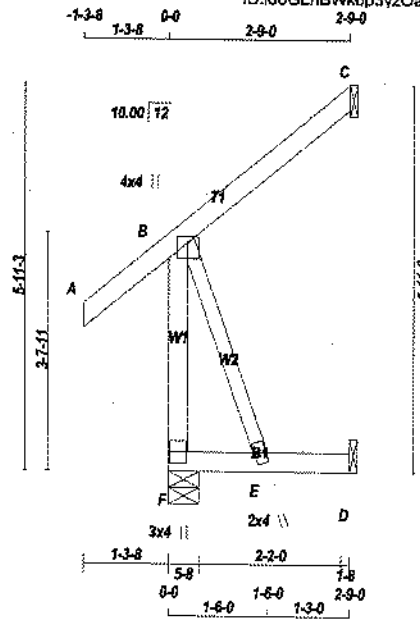
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	J31	5	1	TRUSS DESC.		

Temareck Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:08 2022 Page 1

ID:ldOGLhBWk6p3yzCay8olH3za0dF-d6dbD3Fudx3rMar73ZcT i23msS8IEfb3LP607yWD0D

Scale = 1:33.9



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	260	0	260	0	5-8	5-8
C	117	0	117	0	1-8	1-8
D	26	0	26	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	182	128 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	81	64 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	20	0 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)				
FR-TO								
F-B	-234 / 0	0.0	0.0	0.05 (1)	7.81	0 / 0	0.00 (1)	
A-B	0 / 38	-84.9	-84.9	0.13 (5)	10.00			
B-C	0 / 0	-84.9	-84.9	0.11 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:5), BC=0.04/1.00 (E-F:4)
WB=0.00/1.00 (B-E:1), SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

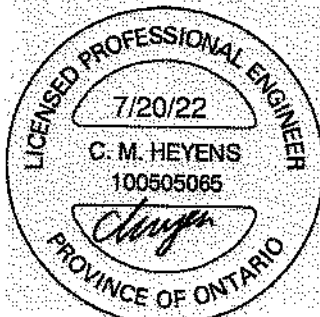
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.65 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070607

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	J32	10	1	TRUSS DESC.		

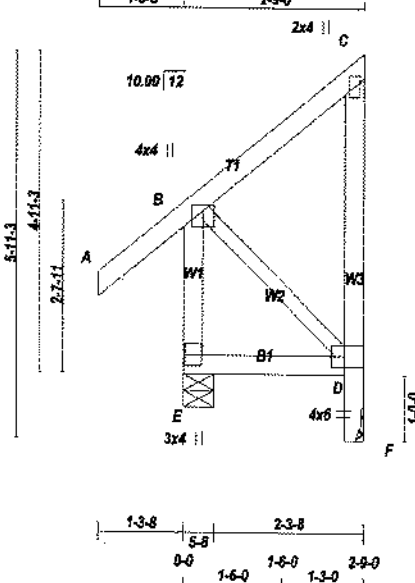
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:30:08 2022 Page 1

ID:ld0GLhBWk6p3yzOayBoIH3za0dF-5JBzQPFWOEBizKQBdG7iXwaEXGnRuh4KH78ZRYwD0C

-1-3-8 0-0 2-9-0 2-9-0

Scale = 1:33.9



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
F - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+r	MT20	2.0	4.0		
D	BWMV+4	MT20	4.0	6.0	2.00	2.50
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP
E	252	0	252	0
F	127	0	127	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-0".

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	177	125 / 0	0 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
F	90	56 / 0	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO						FR-TO			
E-B	-228 / 0	0.0	0.0	0.03 (1)	7.81	B-D	0 / 3	0.00 (1)	
A-B	0 / 38	-84.9	-84.9	0.12 (5)	10.00	F-D	-127 / 0	0.01 (1)	
B-C	-3 / 0	-84.9	-84.9	0.09 (1)	10.00	D-C	-105 / 0	0.03 (1)	
E-D	0 / 0	-18.5	-18.5	0.04 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 10 X 20 = 205 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OGC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B-5), BC=0.04/1.00 (D-E-4)

, WB=0.03/1.00 (C-D-1), SSI=0.07/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

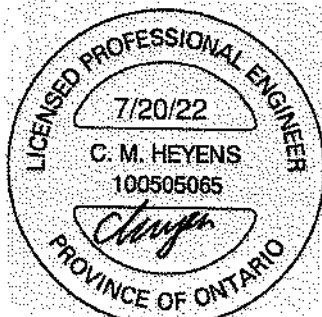
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PLI)	PLI
MT20	650	371	1747

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070608



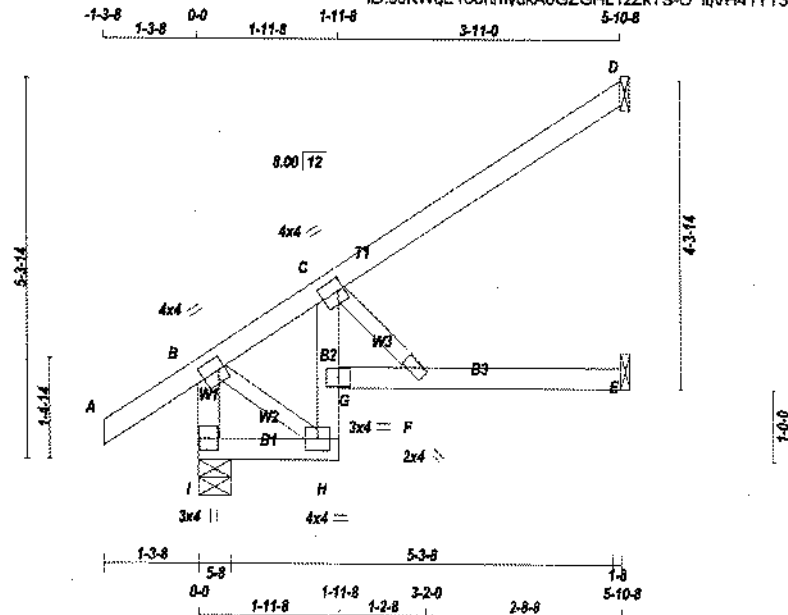
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425143	J51S	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Jul 20 10:53:36 2022 Page 1

ID:9JKWqE1c0hmryukA0GZGHL1zZKTS-O iIVH41YT3HZUcYGF3dMfYRwDe1 rQPTcROyWcQD

Scale = 1:30.9



TOTAL WEIGHT = 5 X 23 = 114 lb

LUMBER					
N.L.G.A. RULES					
CHORDS		SIZE	LUMBER		DESCR
I - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
H - C	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
I	434	0	434	0	5-8	5-8
D	175	0	175	0	1-8	1-8
E	115	0	115	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	305	208 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	122	92 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	84	37 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB. MAX. FORCE (LBS)	MAX. CSI (I.C)
FR-TO						
I-B	-417 / 0	0.0	0.0	0.04 (1)	7.81	0.04 (1)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	0.05 (1)
B-C	-192 / 0	-84.9	-84.9	0.11 (1)	6.25	0.05 (1)
C-D	0 / 4	-84.9	-84.9	0.25 (1)	10.00	
I-H	0 / 0	-18.5	-18.5	0.02 (4)	10.00	
H-G	-85 / 0	0.0	0.0	0.19 (1)	7.81	
G-C	0 / 155	0.0	0.0	0.21 (1)	10.00	
G-F	0 / 242	-18.5	-18.5	0.38 (1)	10.00	
F-E	0 / 0	-18.5	-18.5	0.34 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/989$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/600$ (0.12")

CSI: TC=0.25/1.00 (C-D:1), BC=0.38/1.00 (F-G:1)
WB=0.05/1.00 (C-F:1), SS=0.17/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

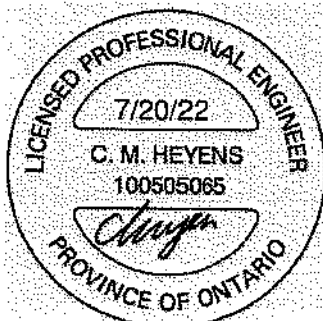
JSI GRIP = 0.30 (C) (INPUT = 0.90)
JSI METAL = 0.18 (F) (INPUT = 1.00)

PLAYES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	BMVW-w	MT20	2.0	4.0		
G	BMVW-t	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMVW-t	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

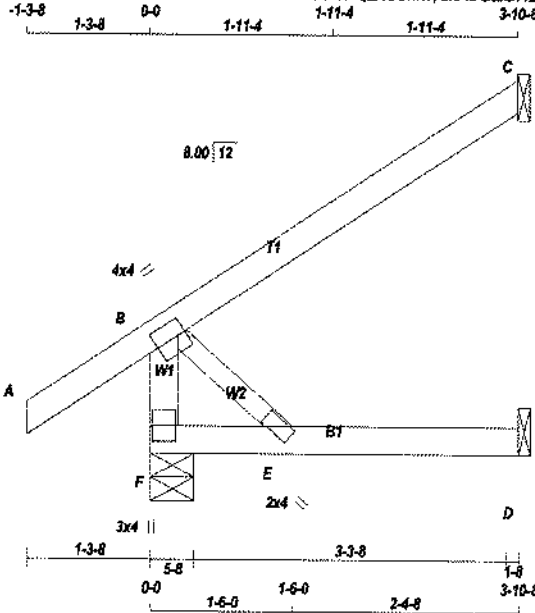


STRUCTURAL COMPONENT ONLY
DWG # TR22070680

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	J52	10	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:51:46 2022 Page 1
ID:9UKWqE1o0hmyukA0GZGHL1zZKTS-cq2leuy4xcq4f4T6AusbRoSm5RAthafXlqoZmywChx



Scale = 1:23.8

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.00
E	BMVW-w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	UP		
F	317	0	317	0	5-8	5-8
C	164	0	164	0	1-8	1-8
D	36	0	40	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	223	154 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
C	113	90 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. VERT. LOAD (LBS)	MAX. HORZ. LOAD (LBS)	MAX. FORCE (LBS)	MAX. HORZ. LOAD (LBS)	MAX. VERT. LOAD (LBS)	MAX. HORZ. LOAD (LBS)
FR-TO								
F-B	-281 / 0	0.0	0.0	0.03 (1)	7.81	0 / 0	0.00 (1)	
A-B	0 / 32	-84.9	-84.9	0.13 (5)	10.00			
B-C	0 / 0	-84.9	-84.9	0.22 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.22/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

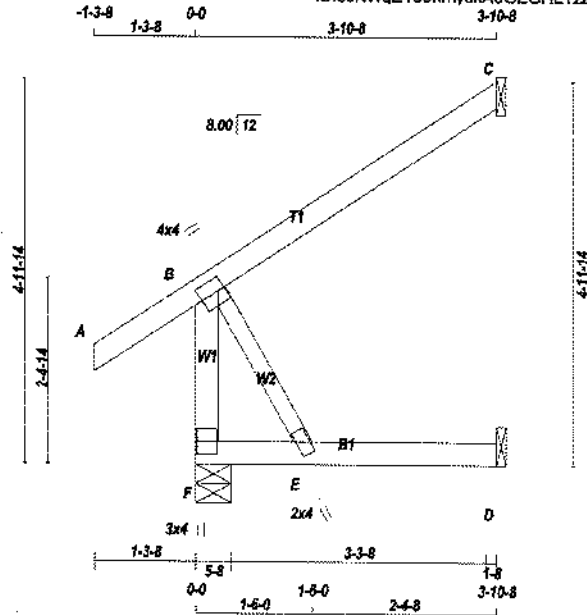


STRUCTURAL COMPONENT ONLY
DWG # TR22070654

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	J53	3	1	TRUSS DESC.		

Tamarack Roof Trusses, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:51:47 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL1zZkTS-40cqrEzi2kXipffgQ58eL1WVnPkqomyaL5CywChw



Scale = 1:28.8

TOTAL WEIGHT = 3 X 16 = 48 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4 DRY	No.2	SPF		
A - C	2x4 DRY	No.2	SPF		
F - D	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.00
E BMVW-w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	317	0	317	0	0	5-8	5-8
C	164	0	164	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE	MAX (MIN) COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
F	223	164 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	113	90 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM	TO		FR-TO			
F-B	-281 / 0	0.0	0.0	0.04 (1)	7.31	B-E	0 / 0	0.00 (1)
A-B	0 / 32	-84.9	-84.9	0.13 (5)	10.00			
B-C	0 / 0	-84.9	-84.9	0.22 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS1: TC=0.22/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

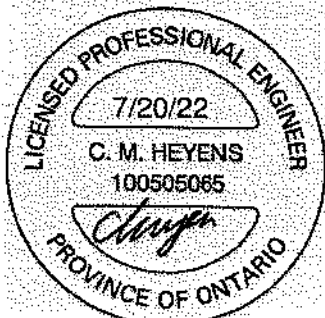
NAIL VALUES

PLATE GRIP(DRY)	PLATE GRIP(WET)	PLATE GRIP(PL)	SECTION
(PSI)	(PSI)	(PL)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



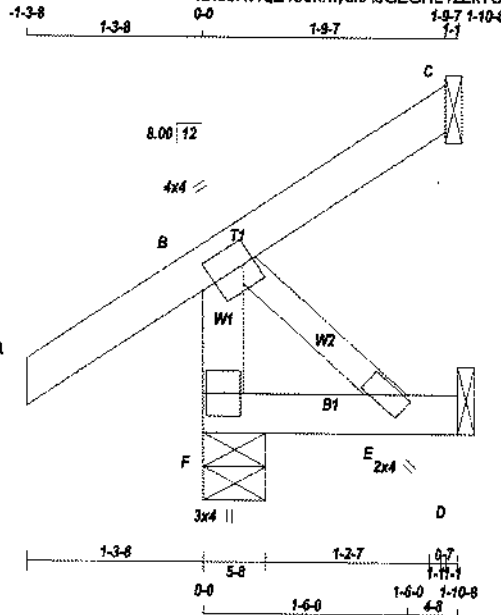
STRUCTURAL COMPONENT ONLY
DWG # TR22070655

JOB NAME 425144	TRUSS NAME J55	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
---------------------------	--------------------------	----------------------	-----------------	--------------------------------------	-------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Jul 20 11:04:37 2022 Page 1

ID:9JKWqE1o0hmyukA0GZGHLfzZKTS-X98mSWHINekG48WembEIL0EenubX0 S8qI557awwCVu



Scale = 1:16.4

TOTAL WEIGHT = 3 X 9 = 27 lb

LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE		LUMBER		DESCR.
F - B	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
F - D	2x4	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMW-1	MT20	4.0	4.0	2.00 1.00
E	BMW-w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
F	254	0	254	0	5-8	5-8		
C	31	0	31	0	1-8	1-8		
D	17	0	19	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE				
F	177	130 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. CSI (LC)		
F-B	-237 / 0	0.0	0.0	0.02 (1)	7.81	0.00 (1)		
A-B	0 / 32	-84.9	-84.9	0.12 (5)	10.00			
B-C	-25 / 0	-84.9	-84.9	0.11 (5)	6.25			
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:5), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

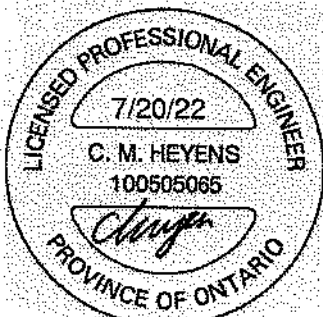
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)

JSI METAL = 0.04 (B) (INPUT = 1.00)



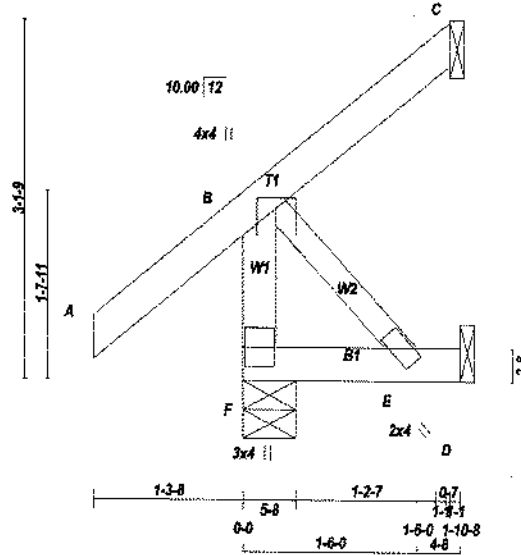
STRUCTURAL COMPONENT ONLY
DWG # TR22070686

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425136	C1	7	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:00:08 2022 Page 1

ID: o0V9YTq5oz7QIFCPkyhhtuzaC6Z-Kqs7RaSpXuHEPZE2dXJM17UT9H6M79ef8INsrKvwDSI



Scale = 1/16\"

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV+w	MT20	2.0	4.0		
F	BMV+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	256	0	256	0
C	30	0	30	0
D	17	0	17	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED					
F	179	131/0	0/0	0/0	0/0	0/0
C	21	17/1-25	0/0	0/0	4/0	0/0
D	14	0/0	0/0	0/0	14/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
F-B	-239/0	0.0	0.0	0.03 (1)	7.81	0/0	0.00 (1)
A-B	0/38	-84.9	-84.9	0.12 (1)	10.00		
B-C	-29/0	-84.9	-84.9	0.11 (1)	6.25		
F-E	0/0	-18.5	-18.5	0.02 (4)	10.00		
E-D	0/0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TFC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4)
WB=0.00/1.00 (B-E:1), SS=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

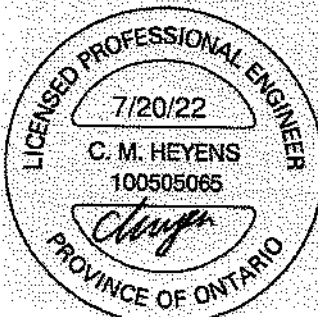
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

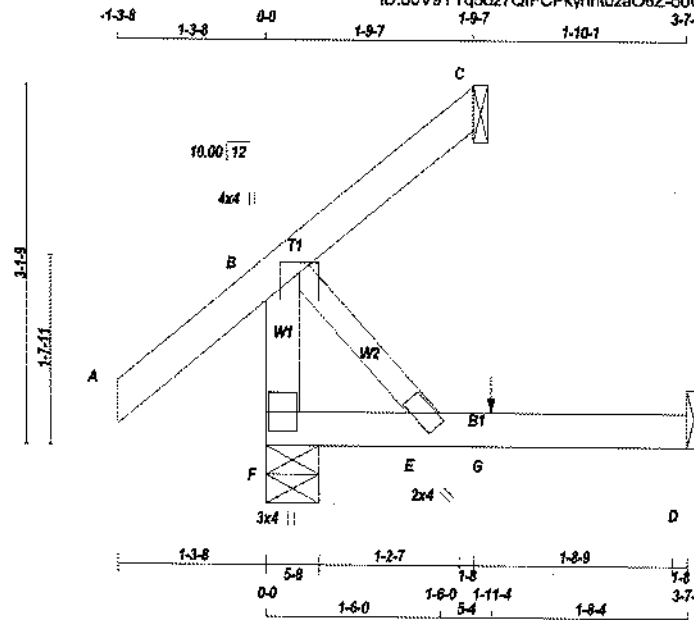
JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.95 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070559

JOB NAME 425136	TRUSS NAME C2	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:09 2022 Page 1	

ID: a0V9YTq5oz7QIFCPkyhhtuzaO6Z-o0QVewTRIBP51pFBEqbaL1cdgRnkcuoNy7QNmywDSK



Scale = 1:19.3

LUMBER					N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.			
F - B	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
F - D	2x4	DRY	No.2	SPF		
ALL WEBS 2x3 DRY No.2 SPF					DRY: SEASONED LUMBER.	

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMW+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	UP	BRG	IN-SX	BRG	IN-SX
F	272	0	272	0	0	5-8	5-8		
C	30	0	30	0	-35	1-8	1-8		
D	34	0	38	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED						
F	192	131/0	0/0	0/0	61/0	0/0
C	21	17/-25	0/0	0/0	4/0	0/0
D	27	0/0	0/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		WEBS	
MEMB.	FACTORED	MEMB.	FACTORED
FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	FORCE (LBS)	MAX. CS1 (LC)
FR-TO	FROM TO	FR-TO	LENGTH
F-B	-239/0	0.0	0.0 0.03 (1)
A-B	0/38	-84.9	-84.9 0.13 (5)
B-C	-29/0	-84.9	-84.9 0.13 (5)
F-E	0/0	-18.5	-18.5 0.07 (4)
E-G	0/0	-18.5	-18.5 0.07 (4)
G-D	0/0	-18.5	-18.5 0.07 (4)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	1	BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD = 36.7 PSF			

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS1: TD=0.13/1.00 (A-B-S), BC=0.07/1.00 (D-E-4), WB=0.00/1.00 (B-E-1), SS1=0.08/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

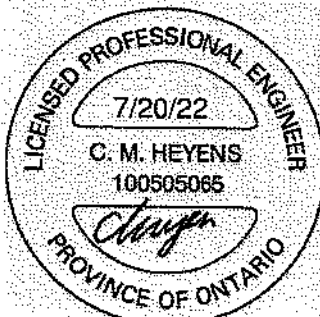
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

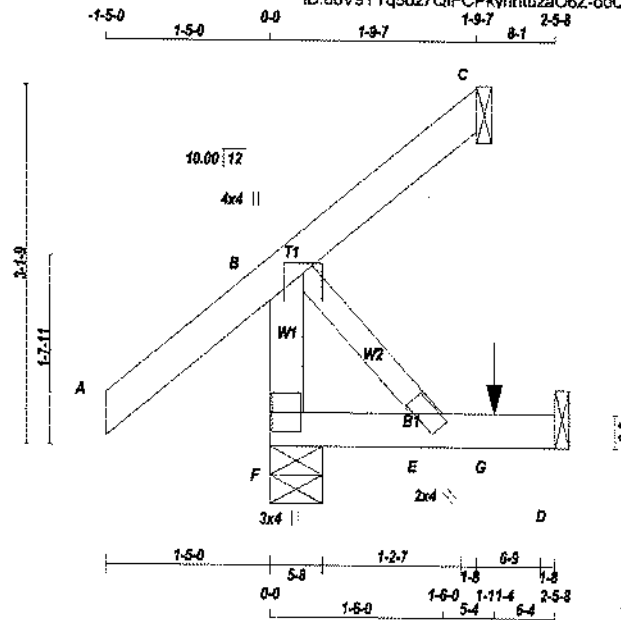


STRUCTURAL COMPONENT ONLY
DWG # TR22070560

JOB NAME 425136	TRUSS NAME C3	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
--------------------	------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:00:09 2022 Page 1
ID: a0V9YTq5oz7QIFCPkyvhtuza06Z-o0QVewTRiBP51lpFBEqba1cNgSMkcuoNy7QNmywDSK



Scale = 1:19.3

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
F	282	0	282	0	0	5-8	5-8	
C	22	0	22	0	-45	1-8	1-8	
D	25	0	25	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	197	142 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	15	12 / -30	0 / 0	0 / 0	0 / 0	3 / 0	0 / 0
D	20	0 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	MAX. UNBRACED LENGTH	MEMB. B-E	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
F-B	-258 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 41	-84.9	-84.9	0.15 (5)	10.00			
B-C	-35 / 0	-84.9	-84.9	0.14 (5)	6.25			
F-E	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
E-G	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
G-D	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LCC	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-3	-3	-	BACK	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CS1: TC=0.15/1.00 (A-B-5), BC=0.03/1.00 (E-F-4), WB=0.00/1.00 (B-E-1), SS=0.00/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

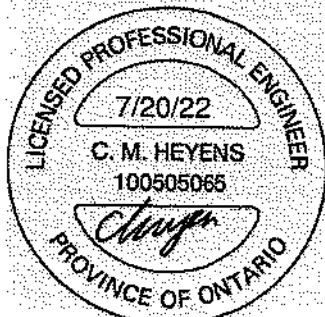
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

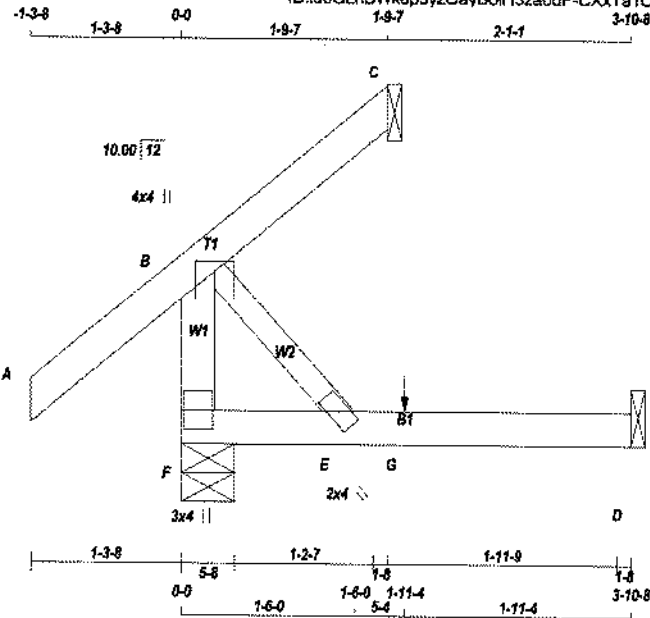
JSI GRIP = 0.16 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070561

JOB NAME 425139	TRUSS NAME C4	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:05 2022 Page 1
ID:Id0GLhBwK6p3yzQayBoH3za0dF-CXxTa1C0K0hGV16QOR3mn4CYMPPpYIV9MNASPgywD0G



Scale = 1/16"

LUMBER	SIZE	QTY	DESCR.
N. L. G. A. RULES			
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMVW+p	MT20	4.0	4.0	1.00 2.00
E BMV+w	MT20	2.0	4.0	
F BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
F	275	0	5-8	5-8
C	30	0	1-8	1-8
D	36	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED						
F	194	131/0	0/0	0/0	82/0	0/0
C	21	17/1-25	0/0	0/0	4/0	0/0
D	29	0/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX.	FACTORED
MEMB.	FORCE	FORCE	FORCE	FORCE	FORCE	FORCE	FORCE	FORCE	FORCE
	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO									
F-B	-239/0	0.0	0.0	0.03 (1)	7.81	0/0	0/0	0.00 (1)	
A-B	0/38	-84.0	-84.9	0.14 (5)	10.00				
B-C	-29/0	-84.9	-84.9	0.13 (5)	6.25				
F-E	0/0	-18.5	-18.5	0.07 (4)	10.00				
E-G	0/0	-18.5	-18.5	0.08 (4)	10.00				
G-D	0/0	-18.5	-18.5	0.08 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	1	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS: TC=0.14/1.00 (A-B:5), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.06/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

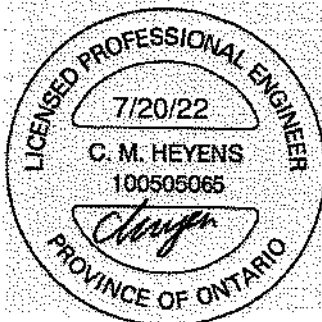
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

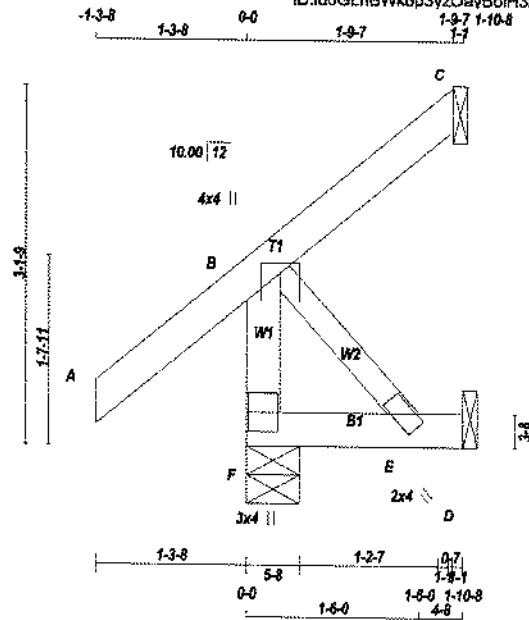
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070602

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425139	C5	1	1	ROYAL PINE HOMES	
Tamarack Roof Truss, Burlington					
Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Jul 20 10:30:06 2022 Page 1					
ID:ld0GLh8Wk6p3yz0ayBoH3za0dF-qkVroNDe5Jp76thcy8a7vHyjP3m0HKib1w7y6YwDOF					
Scale = 1:19.2					



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	256	0	256	0
C	30	0	30	0
D	17	0	17	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	178	131/0	0/0	0/0	0/0	48/0
C	21	17/-25	0/0	0/0	0/0	4/0
D	14	0/0	0/0	0/0	0/0	14/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
F-B	-239/0	0.0	0.0 (1)	7.81	B-E	0/0	0.00 (1)
A-B	0/38	-84.9	-84.9 (1)	10.00			
B-C	-20/0	-84.9	-84.9 (1)	6.25			
F-E	0/0	-18.5	-18.5 (4)	10.00			
E-D	0/0	-18.5	-18.5 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

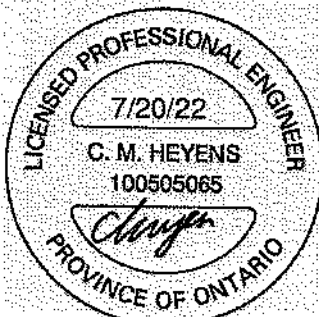
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070603

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425139	C30	5	1	TRUSS DESC.		

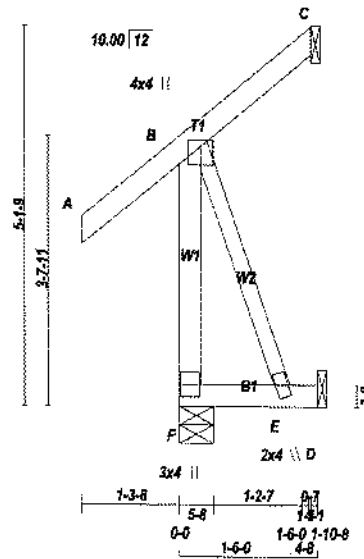
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:30:07 2022 Page 1

ID:IdOGLhBWk6p3yzOayBoitH3za0dF-8w3D7IEGsdX k0GpW5SESVVu9T6E0n7SqfYUyYwDOE

-1-3-8 0-0 1-3-7 1-9-7 1-10-8

Scale = 1:29.9



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	UP		
F	256	0	256	0	5-8	5-8
C	30	0	30	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	179	131 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
F-B	-239 / 0	0.0	0.0	0.05 (1)	7.81	0 / 0	0.00 (1)
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00		
B-C	-29 / 0	-84.9	-84.9	0.11 (1)	6.25		
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00		
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TFC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS1: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4)

, WB=0.00/1.00 (B-E:1), SS=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

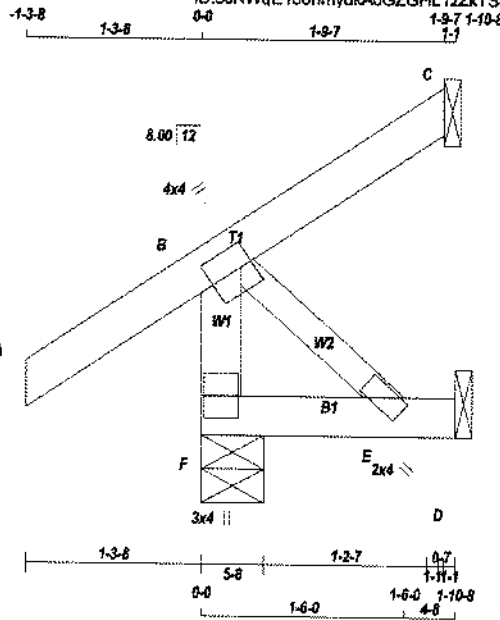


STRUCTURAL COMPONENT ONLY
DWG # TR22070604

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	C51	4	1	TRUSS DESC.		

Temareck Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:51:42 2022 Page 1
ID:9JKWqE1e0hmyukA0GZGHL1zZkTS-k3pnoWvauW5Eb1mhlKqWRbeCMU5Dx7b3cgsaR?ywc7?



TOTAL WEIGHT = 4 X 9 = 36 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMVW-w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	254	0	254	0	5-8	5-8
C	31	0	31	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	177	130 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CS1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)	
		VERT. (PLF)	CS1 (LC)				FORCE (LBS)	MAX. CS1 (LC)
F-B	-237 / 0	0.0	0.0	0.02 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 32	-84.9	-84.9	0.12 (5)	10.00			
B-C	-25 / 0	-84.9	-84.9	0.11 (5)	6.25			
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2010
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA G86-14
- TPC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CS1: TC=0.12/1.00 (A-B:5), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

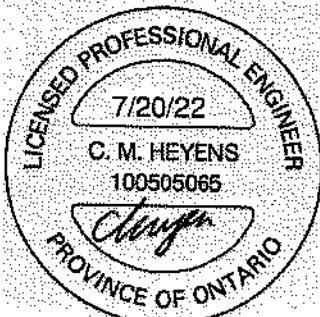
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 789

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.80)
JSI METAL= 0.04 (B) (INPUT = 1.00)



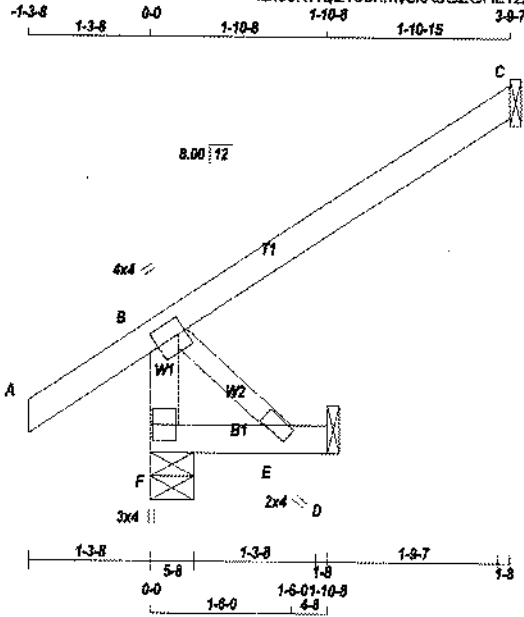
STRUCTURAL COMPONENT ONLY
DWG # TR22070648

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	C52	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

ID: 9JKWqE1o0hmyukA0GZGHL1zZKTS-CFN9?swCqD5DBLUR11.9zoALhuQSqrCkKc8zRywCi

Version 8.630 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:51:43 2022 Page 1



Scale = 1:23.6

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-4	MT20	4.0	4.0	2.00	1.00	
E	BMVW-4	MT20	2.0	4.0			
F	BMV1-p	MT20	3.0	4.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	294	0	294	0	5-8	IN-SX
C	181	0	181	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	206	152/0	0/0	0/0	0/0	53/0	0/0
C	111	88/0	0/0	0/0	0/0	23/0	0/0
D	14	0/0	0/0	0/0	0/0	14/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MAX. CSI (LC)	UNBRAC LENGTH	MEMB.	WEBS		MAX. CSI (LC)
	MAX. FORCE (LBS)	FACTORED	VERT. LOAD	LC1				MAX. FORCE (LBS)	FACTORED	
FR-TO			FROM	TO			FR-TO			
F-B	-277/0	0.0	0.0	0.03 (1)	7.81		B-E	0/0	0.00 (1)	
A-B	0/32	-84.9	-84.9	0.12 (5)	10.00					
B-C	0/0	-84.9	-84.9	0.21 (1)	10.00					
F-E	0/0	-18.5	-18.5	0.02 (4)	10.00					
E-D	0/0	-18.5	-18.5	0.02 (4)	10.00					

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
CL = 6.0 PSF
BOT CH. LL = 0.0 PSF
CL = 7.4 PSF
TOTAL LOAD = 38.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.21/1.00 (B-C:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

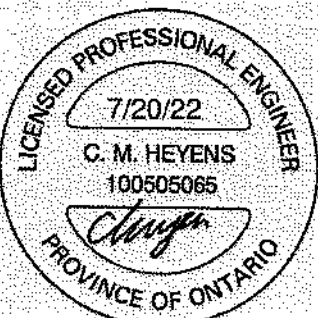
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	789	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

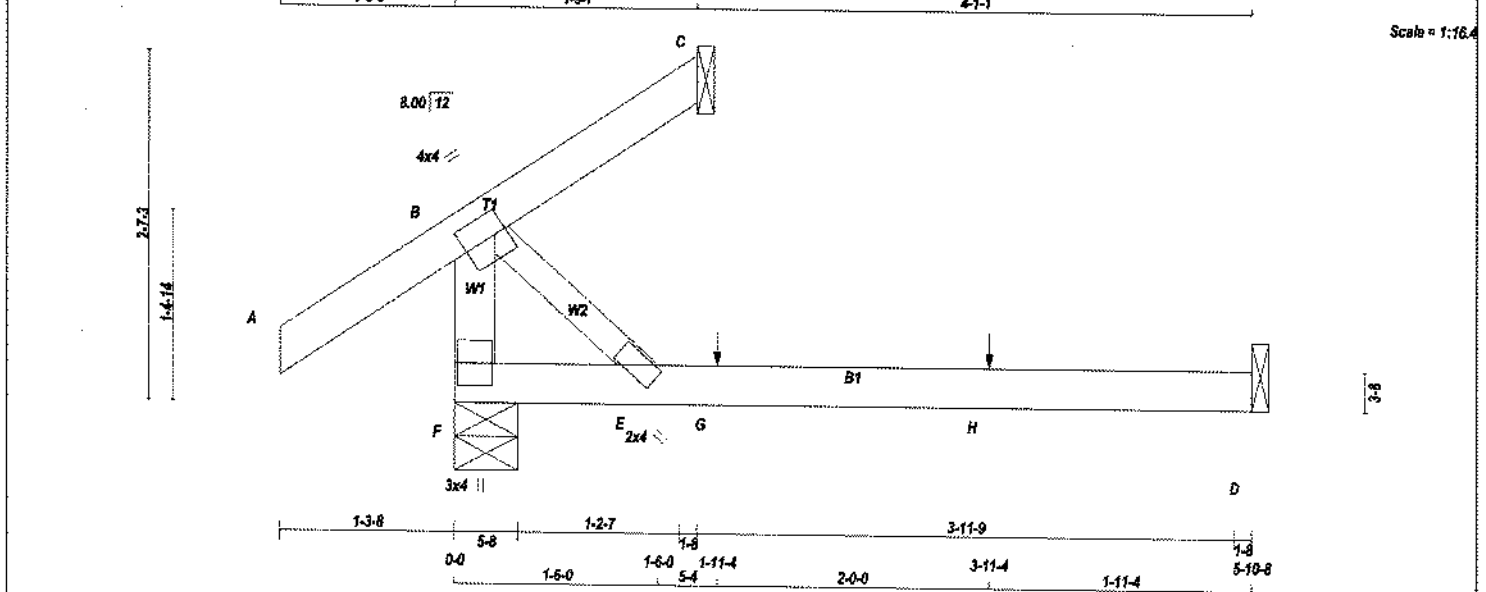
JSI GRIP = 0.17 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070649

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425142	C53	1	1	ROYAL PINE HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Jul 20 10:51:43 2022 Page 1
ID:9JKWqE1o0hmyuka0GZGHL1zZKTS-CFN9?swCtqD5DBLuR1L9zoAN3uOqgwrCrKc8zRywCi
5-10-8



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
F - B 2x4 DRY No.2 SPF
A - C 2x4 DRY No.2 SPF
F - D 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER		TOTAL WEIGHT = 13 lb	
BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG
F	291	291	5-8
C	31	31	1-8
D	54	54	1-8

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH.	LL = 23.3 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF	
SPACING = 24.0 IN. C/C	
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015	
THIS DESIGN COMPLIES WITH:	
- PART 9 OF BCBC 2018, ABC 2019	
- PART 9 OF OBC 2012 (2019 AMENDMENT)	
- CSA 086-14	
- TPIC 2014	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SEE MTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D.
UNFACTORED REACTIONS
1ST CASE MAX. MIN. COMPONENT REACTIONS
JT 1ST CASE MAX. MIN. COMPONENT REACTIONS
F 207 130/0 0/0 0/0 0/0 77/0 0/0
C 21 17/0 0/0 0/0 0/0 4/0 0/0
D 43 0/0 0/0 0/0 0/0 43/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C.
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING	
TOTAL LOAD CASES: (4)	
CHORDS	WEBS
MEMB.	MEMB.
MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
VERT. LOAD (LBS)	VERT. LOAD (LBS)
FROM TO	FROM TO
F-B	B-E
A-B	
B-C	
F-E	
E-G	
G-H	
H-D	

SPECIFIED CONCENTRATED LOADS (LBS)	
JT	LOC.
G	1-11-4
H	3-11-4

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

STRUCTURAL COMPONENT ONLY
DWG # TR22070650

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

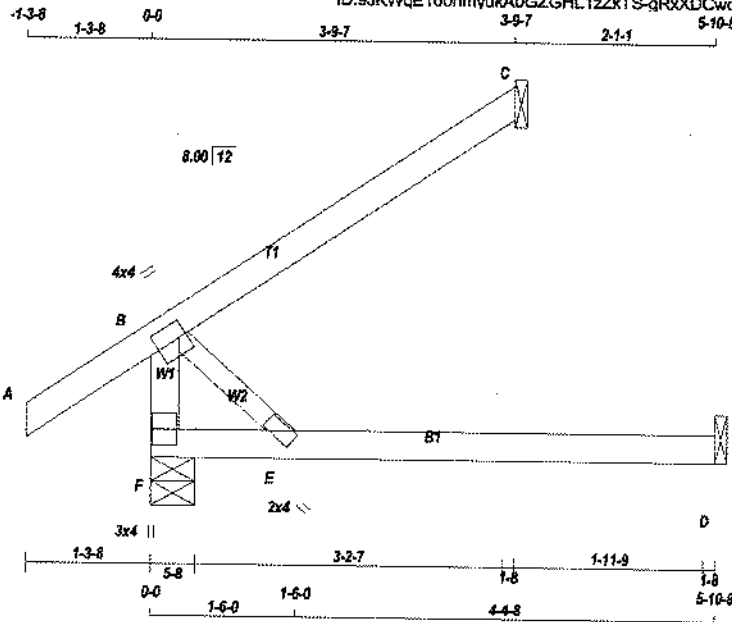
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.15 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425142	C54	1	1	ROYAL PINE HOMES	

Tamarack Roof Truss, Burlington

TRUSS DESC.

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Jul 20 10:51:44 2022 Page 1
ID:9JKWqEto0hmyukA0GZGHL1ZZkTS-qRxXDCwqP7LvtLw4 IsOW0JWRK2PN5M3 LhVtywChz



TOTAL WEIGHT = 16 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-l	MT20	4.0	4.0	2.00	1.00
E	BMV-w	MT20	2.0	4.0		
F	BMV1-p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	331	0	331	0	5-8	5-8
C	161	0	161	0	1-8	1-8
D	64	0	61	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	235	152/0	0/0	0/0	0/0	83/0	0/0
C	111	88/0	0/0	0/0	0/0	23/0	0/0
D	43	0/0	0/0	0/0	0/0	43/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
F-B	-277/0	0.0 0.0	0.03 (1)	7.81	B-E	0/0	0.00 (1)
A-B	0/32	-84.9 -84.9	0.11 (1)	10.00			
B-C	0/0	-84.9 -84.9	0.21 (1)	10.00			
F-E	0/0	-18.5 -18.5	0.13 (4)	10.00			
E-D	0/0	-18.5 -18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.21/1.00 (B-C:1), BC=0.19/1.00 (D-E:4)
WB=0.00/1.00 (B-E:1), SS=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

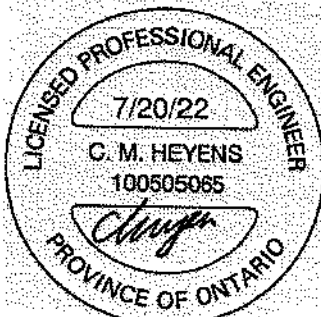
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	MAX	MIN	MAX MIN
	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)

JSI METAL= 0.05 (B) (INPUT = 1.00)

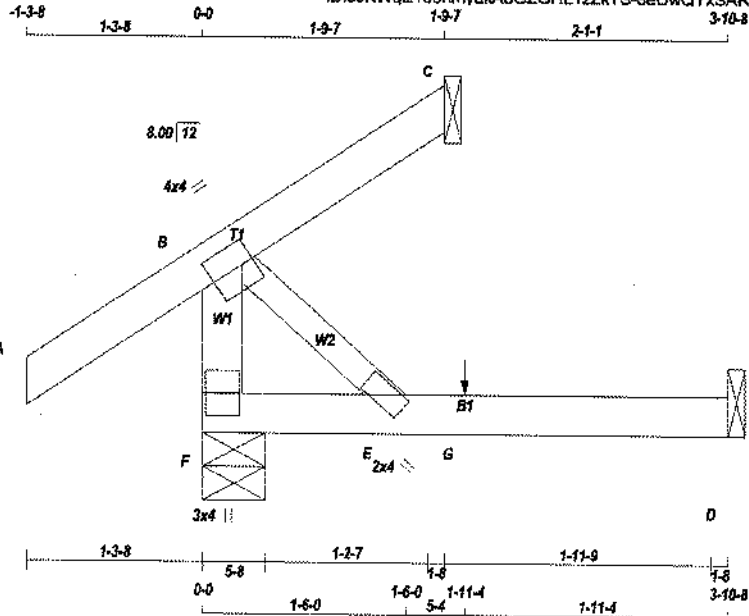


STRUCTURAL COMPONENT ONLY
DWG # TR22070651

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
425142	C55	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.536 S Feb 23 2022 MITek Industries, Inc. Wed Jul 28 10:51:45 2022 Page 1
ID:9JKWqE1o0hmyukA0GZGHL12ZkTS-8eUwQYxSARUpSVVGYsNd3DGkH5x8qKvle5E1JywChy



Scale = 1/16"

TOTAL WEIGHT = 3 X 11 = 34 lb

LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS	SIZE		
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMVW-I MT20	4.0	4.0	2.00	1.00
E BMVW-w MT20	2.0	4.0		
F BMV1+p MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
F	273	0	273	0
C	31	0	31	0
D	38	0	40	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
F	192	130 / 0	0 / 0	0 / 0	62 / 0	0 / 0	
C	21	17 / -25	0 / 0	0 / 0	4 / 0	0 / 0	
D	29	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
F-B	-237 / 0	0.0	0.0	0.02 (1)	7.81	0 / 0	0.00 (1)
A-B	0 / 32	-84.9	-84.9	0.13 (5)	10.00		
B-C	-25 / 0	-84.9	-84.9	0.13 (5)	6.25		
F-E	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
E-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
G-D	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	1	FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.13/1.00 (A-B:5), BC=0.08/1.00 (D-E:4)
WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22070652

EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

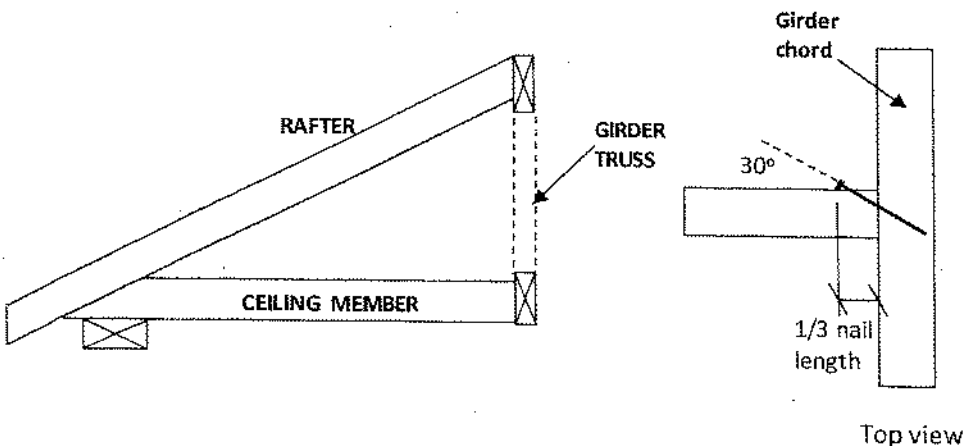
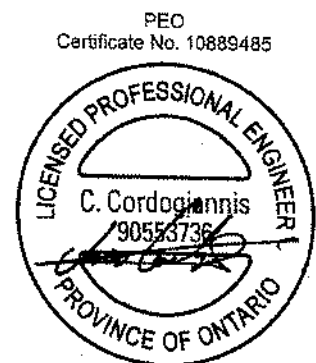


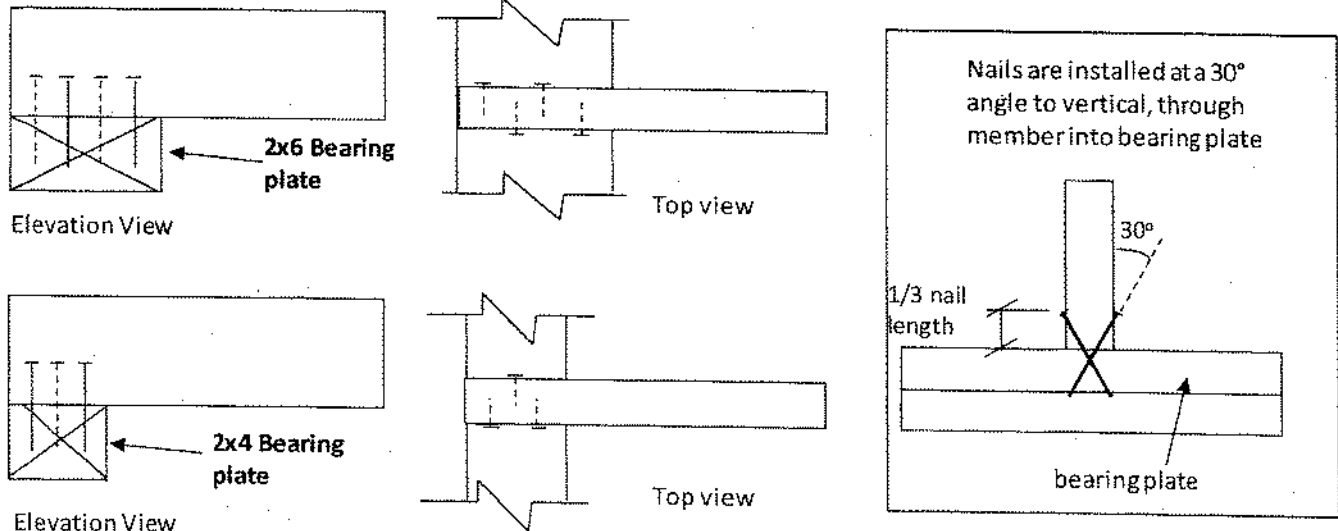
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_0 factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

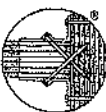
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

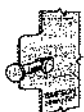
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

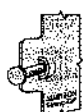
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear Nailing Top View

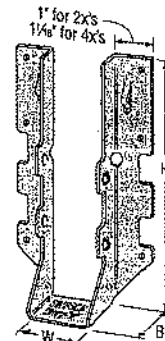
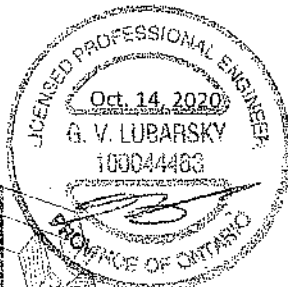
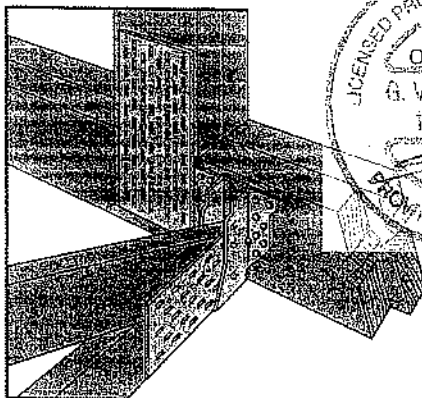


Double-Shear Nailing Side View; Do not bend tab

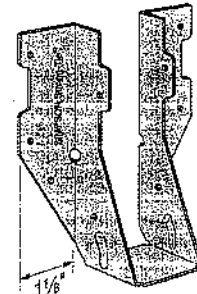


Dome Double-Shear Nailing Side View (available on some models)

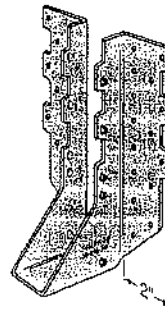
Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



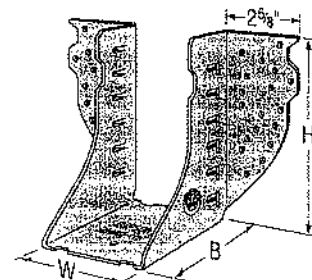
LUS26



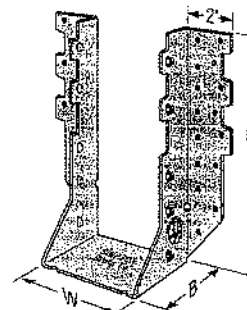
LU26L



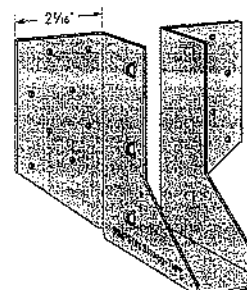
HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



HHUS210-2



LJS26DS

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

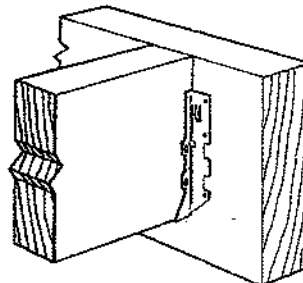
- Factored resistances are in accordance with CSA 086-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

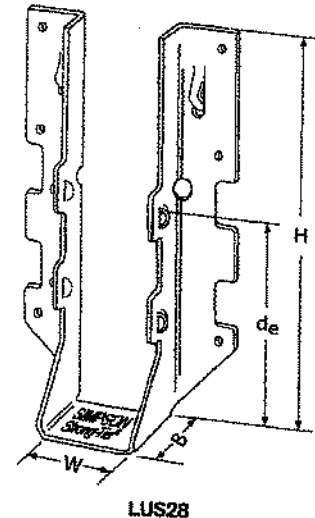
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

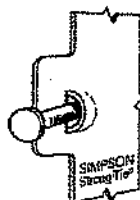


Typical LUS Installation



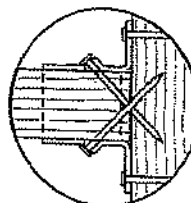
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LUS24	18	1 1/8	3 1/8	1 1/8	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/8	1 1/8	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/8	2	3 3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/8	1 1/8	3 3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/8	2	3 3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 1/8	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

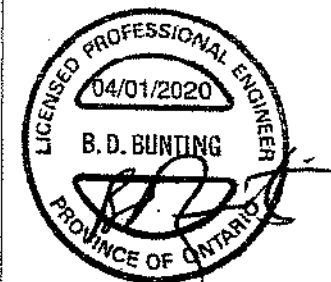


Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

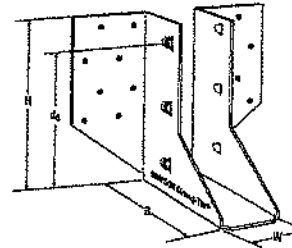
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

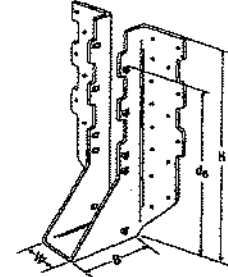
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

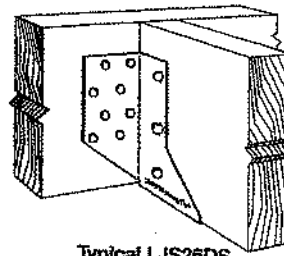
- See current catalogue for options



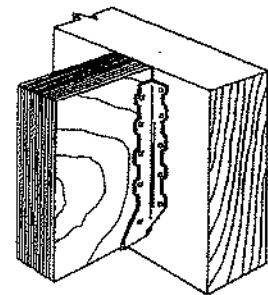
LJS26DS



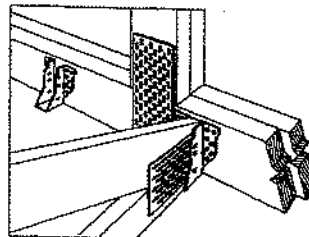
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



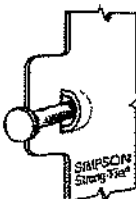
Typical HUS
Installation



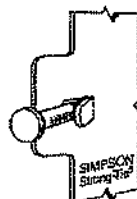
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _a	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1½	5	3½	4½	(16) 16d	(6) 16d		2065	4285	1460	4115
HUS26	16	1½	5½	3	3½	(14) 16d	(6) 16d		2705	4940	2065	3875
HUS28	16	1½	7½	3	6½	(22) 16d	(8) 16d		3605	5365	2675	4345
HUS210	16	1½	9½	3	7½	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

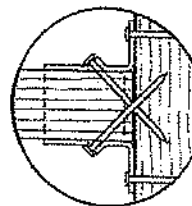
1. d_a is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,803,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPEC HUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

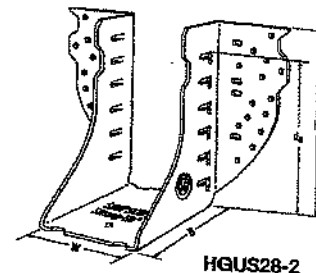
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

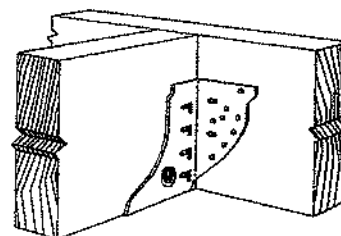
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

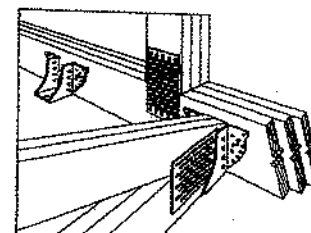
- See current catalogue for options



HGUS26-2



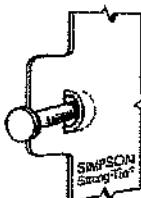
Typical HGUS Installation



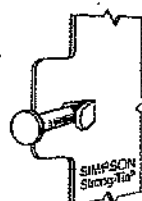
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 1/8	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 1/8	9 1/8	5	7 1/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14895	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

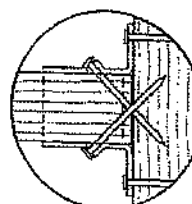
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

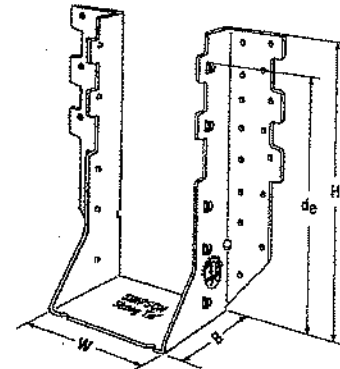
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

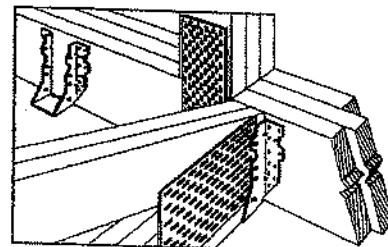
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



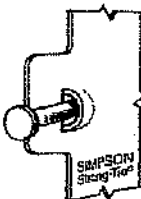
HHUS410



Typical HHUS Installation
(Truss Designer to provide
fastener quantity for connecting
multiple members together)

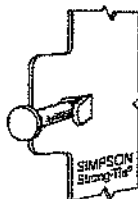
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 1/8	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5 1/8	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/8	9	3 1/8	7 1/8	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_s is the distance from the seat of the hanger to the highest joist nail.

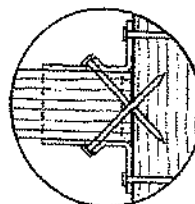


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

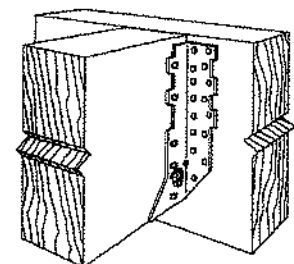
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



Typical HHUS Installation



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

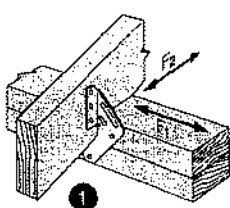
© 2020 Simpson Strong-Tie Company Inc.

T-SPECHHUS20 3/20 exp. 6/22

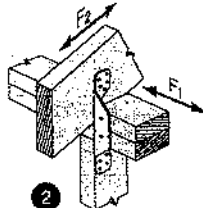
(800) 999-5099
strongtie.com

H/TSP

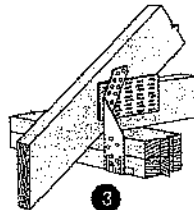
Seismic and Hurricane Ties (cont.)



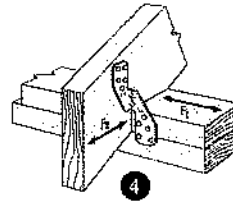
1 H1 Installation



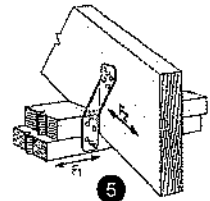
2 H2A Installation



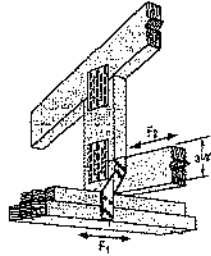
3 TSP Installation



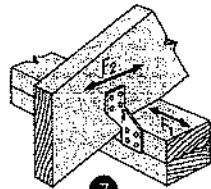
4 H2.5A Installation
(Nails into both top plates)



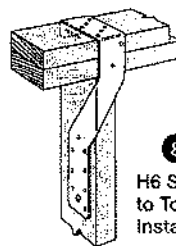
5 H2.5T Installation
(Nails into both top plates)



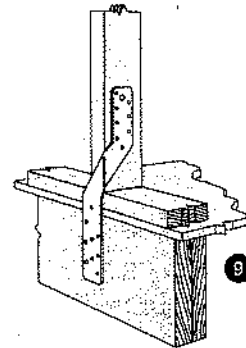
6 H2.5T Installation



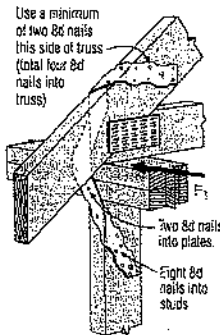
7 H3 Installation
(Nails into upper top plate)



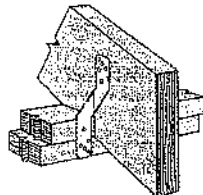
8 H6 Stud to Top Plate Installation



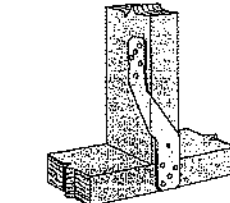
9 H6 Stud to Band Joist Installation



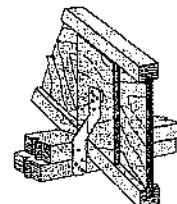
10 H7Z Installation



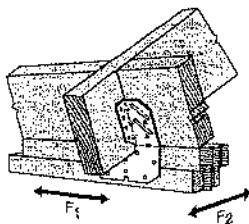
11 H8 Attaching Rafter to Double Top Plates



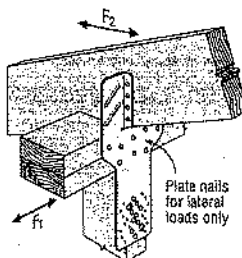
12 H8 attaching Stud to Sill
(4) 8d into plate, (5) 8d into stud



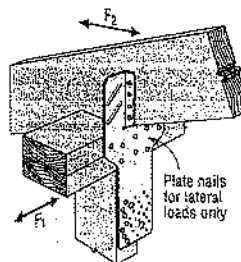
13 H8 attaching I-Joist to Double Top Plates



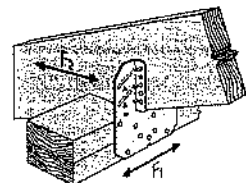
14 H10A Field-Bent Installation



15 H10S Installation

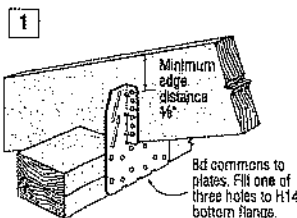


16 H10S Installation with Stud Offset

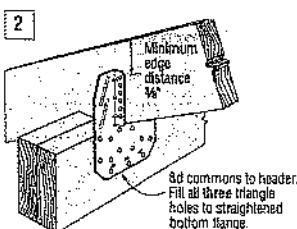


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

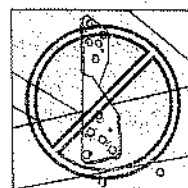


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

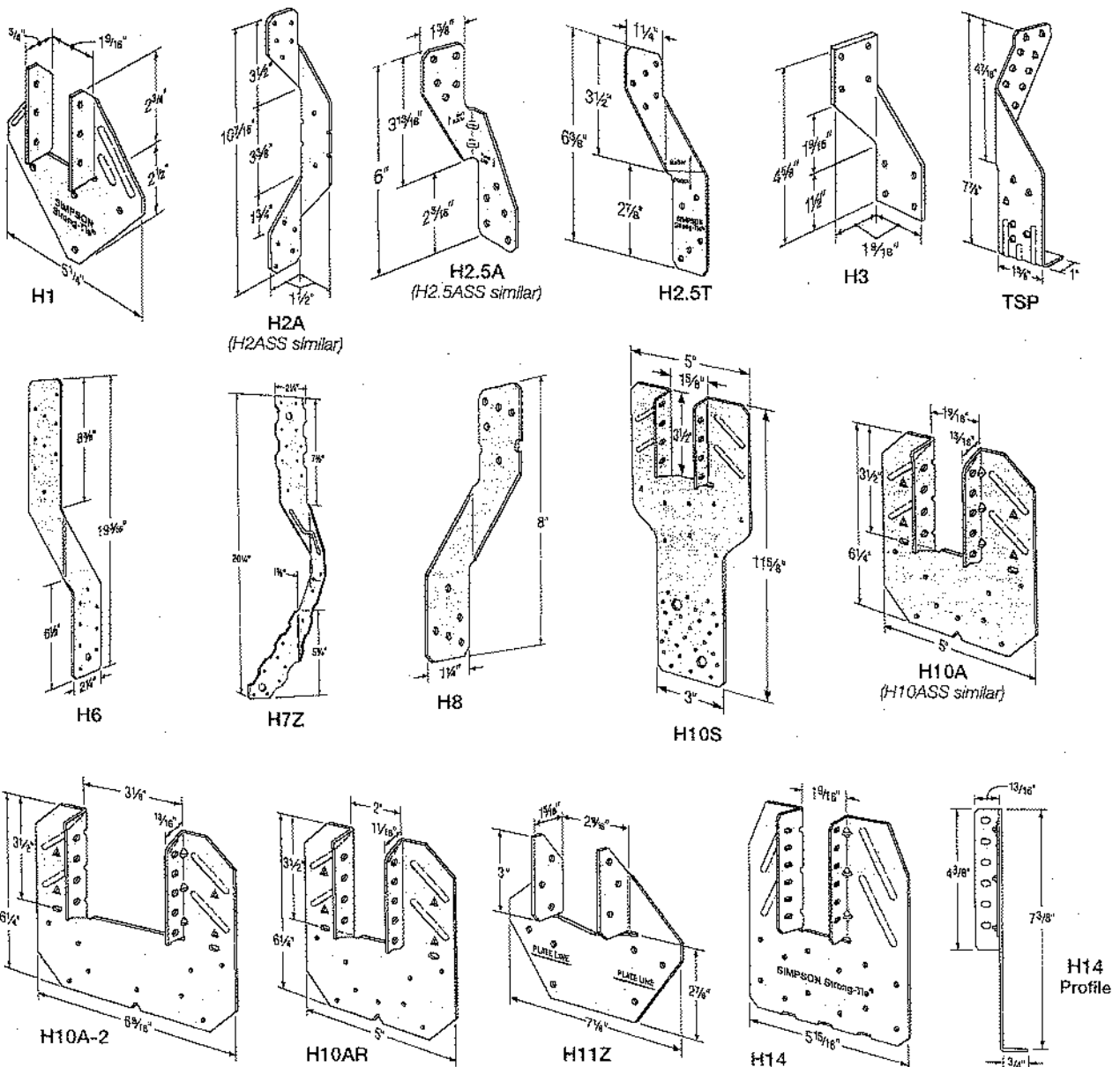
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized, H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12; use H10A sloped loads for field bent installation.



H - Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

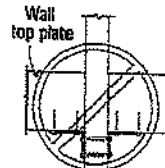
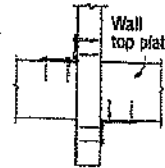
Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA 086-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners
• Nails: 8d = 0.131" dia. x 2 1/2" long common wire, 8d x 1 1/2" = 0.131" x 1 1/2" long, 10d x 1 1/2" = 0.146" x 1 1/2" long
• H1 can be installed with flanges facing outwards
• Hurricane ties do not replace solid blocking

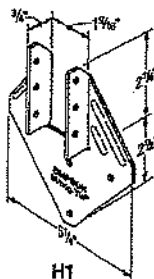
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0 .

Hurricane Tie Installations to Achieve Twice the Load (Top View)

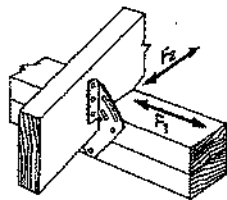


Install diagonally across from each other for minimum 2x truss.

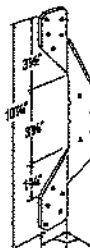
Nailing into both sides of a single ply 2x truss may cause the wood to split.



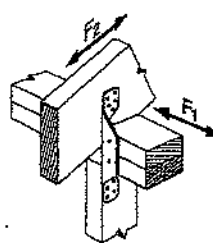
H1



H1 Installation



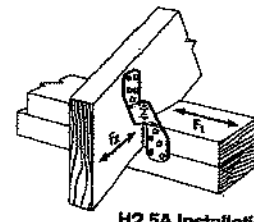
H2A



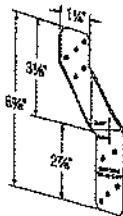
H2A Installation



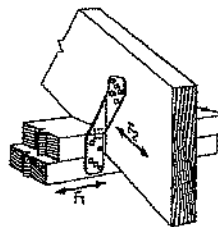
H2.5A



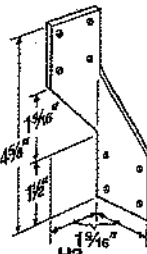
H2.5A Installation



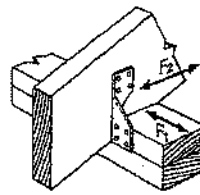
H2.5T



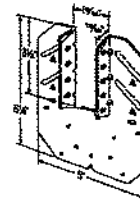
H2.5T Installation
(Nails into both top plates)



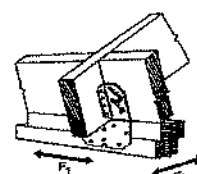
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
		To Rafter	To Plates	To Studs	D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1 1/2"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1 1/2"	(2) 8d x 1 1/2"	(5) 8d x 1 1/2"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1735	795	410	1505	565	290

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

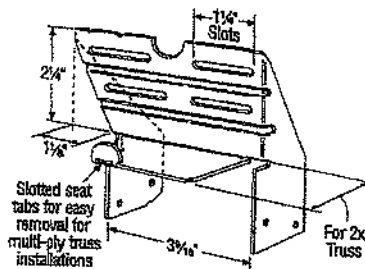
Design: Factored resistances are in accordance with CSA 086-14

Installation:

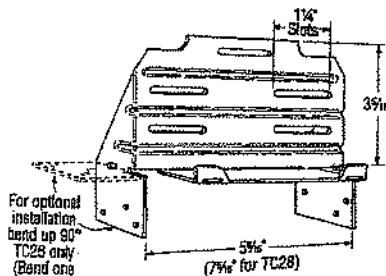
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

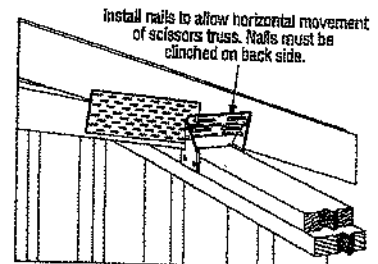
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



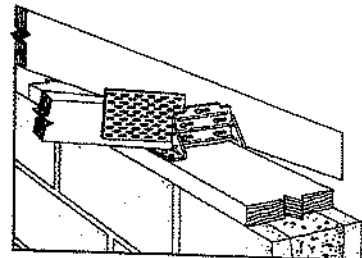
TC24
U.S. Patent 4,932,173



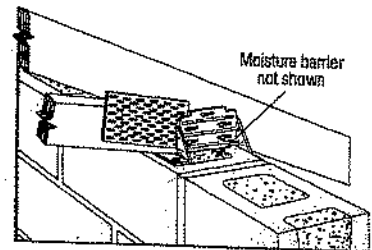
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nail
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	606	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1 1/2"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - 3/8" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECTC20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

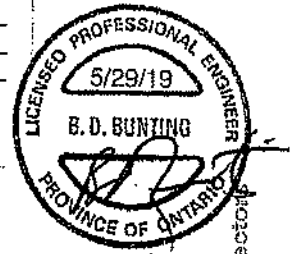
HTU**SIMPSON**
Strong-Tie**Face-Mount Truss Hanger (cont.)**

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (in.)	Minimum Header Size	Fasteners		Factored Resistance			
			Header	Joist	D.Fir-L		S-P-F	
					Uplift	Normal	Uplift	Normal
					($K_D = 1.15$)	($K_D = 1.00$)	($K_D = 1.15$)	($K_D = 1.00$)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
HTU26 (Min.)	3½	(2) 2x4	(10) 16d	(14) 10d x 1½"	1740	3340	1235	2370
					7.74	14.86	5.49	10.54
HTU26 (Max.)	5½	(2) 2x4	(10) 16d	(20) 10d x 1½"	2470	4016	1755	2650
					10.99	17.86	7.81	12.88
HTU28 (Max.)	3½	(2) 2x6	(20) 16d	(26) 10d x 1½"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19
HTU210 (Max.)	7¼	(2) 2x6	(20) 16d	(32) 10d x 1½"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19

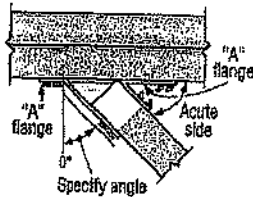
See table footnotes on p. 280.

**Hanger Options**

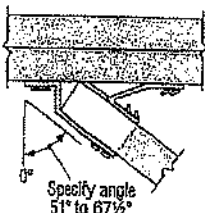
See Hanger Options Information on pp. 125-127.

Skewed Seat

- Skewable up to 67½°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
		Header	Joist	D.Fir-L		S-P-F	
				Uplift	Normal	Uplift	Normal
				($K_D = 1.15$)	($K_D = 1.00$)	($K_D = 1.15$)	($K_D = 1.00$)
				lbs	lbs	lbs	lbs
				kN	kN	kN	kN
HTU26	< 51	(20) 16d	(14) 10d x 1½"	1935	4110	1390	2905
	51-67½	(20) 16d	(12) 10d x 1½"	8.16	16.28	5.78	12.92
HTU28	< 51	(26) 16d	(20) 10d x 1½"	1350	3620	955	2550
	51-67½	(26) 16d	(17) 10d x 1½"	6.01	16.10	4.25	11.39
HTU210	< 51	(32) 16d	(26) 10d x 1½"	2810	4270	1985	3030
	51-67½	(32) 16d	(22) 10d x 1½"	12.60	18.99	8.83	13.48
HTU26-2	< 51	(20) 16d	(14) 10d	2075	3930	1465	2780
	51-67½	(20) 16d	(12) 10d	9.23	17.48	6.52	12.37
HTU28-2	< 51	(26) 16d	(20) 10d	3785	4430	2675	3135
	51-67½	(26) 16d	(17) 10d	16.84	19.71	11.90	13.95
HTU210-2	< 51	(32) 16d	(26) 10d	2795	4240	1980	3000
	51-67½	(32) 16d	(22) 10d	12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
	51-67½	(20) 16d	(12) 10d	9.52	16.53	6.74	11.68
HTU28-2	< 51	(26) 16d	(20) 10d	1610	3920	1140	2785
	51-67½	(26) 16d	(17) 10d	7.16	17.44	5.07	12.39
HTU210-2	< 51	(32) 16d	(26) 10d	3960	5425	2615	3855
	51-67½	(32) 16d	(22) 10d	17.62	24.13	12.52	17.15
HTU26-2	< 51	(20) 16d	(14) 10d	2385	5425	1895	3855
	51-67½	(20) 16d	(12) 10d	10.61	24.13	7.54	17.15
HTU28-2	< 51	(26) 16d	(20) 10d	5025	6690	3570	4890
	51-67½	(26) 16d	(17) 10d	22.35	30.65	15.88	21.75
HTU210-2	< 51	(32) 16d	(26) 10d	3145	6680	2225	4745
	51-67½	(32) 16d	(22) 10d	13.99	29.72	9.90	21.10

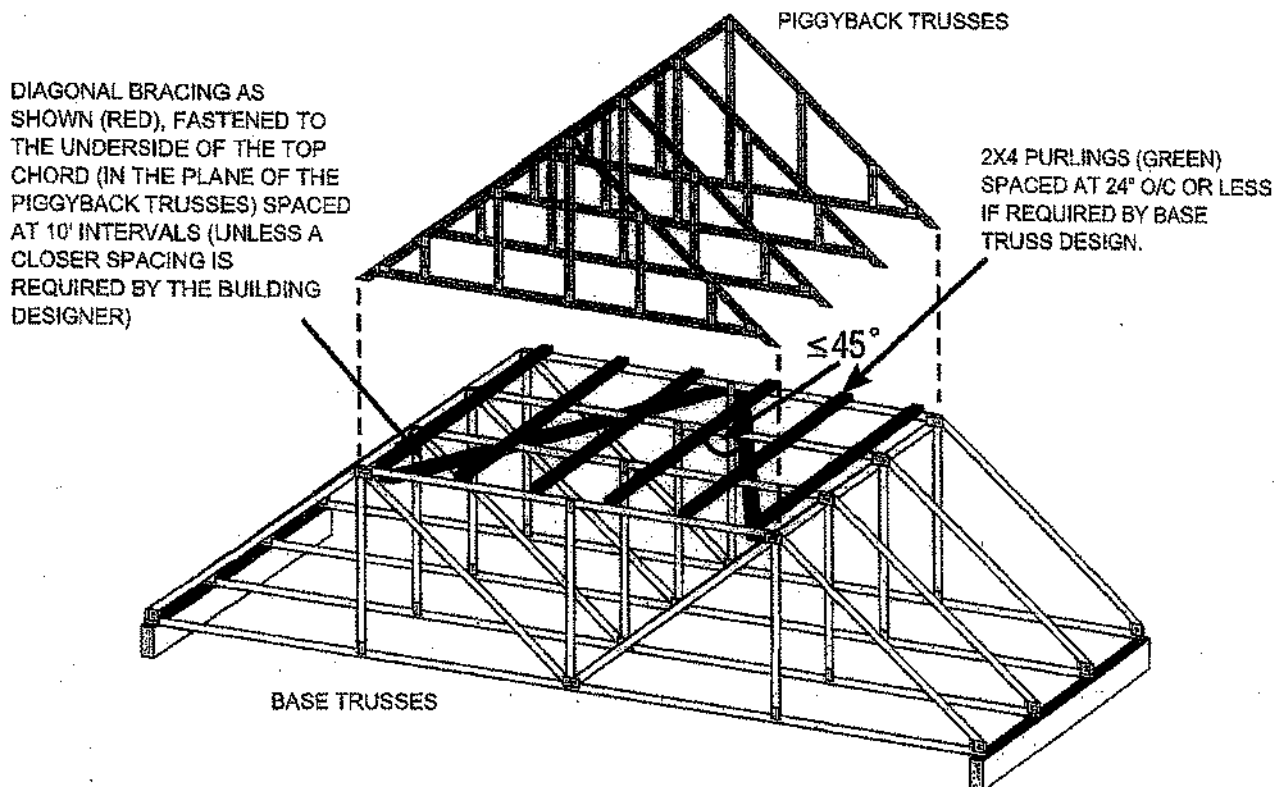
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3½" long. 10d x 1½" = 0.148" dia. x 1½" long. 10d = 0.148" dia. x 3" long. See pp. 27-28 for other nail sizes and information.

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

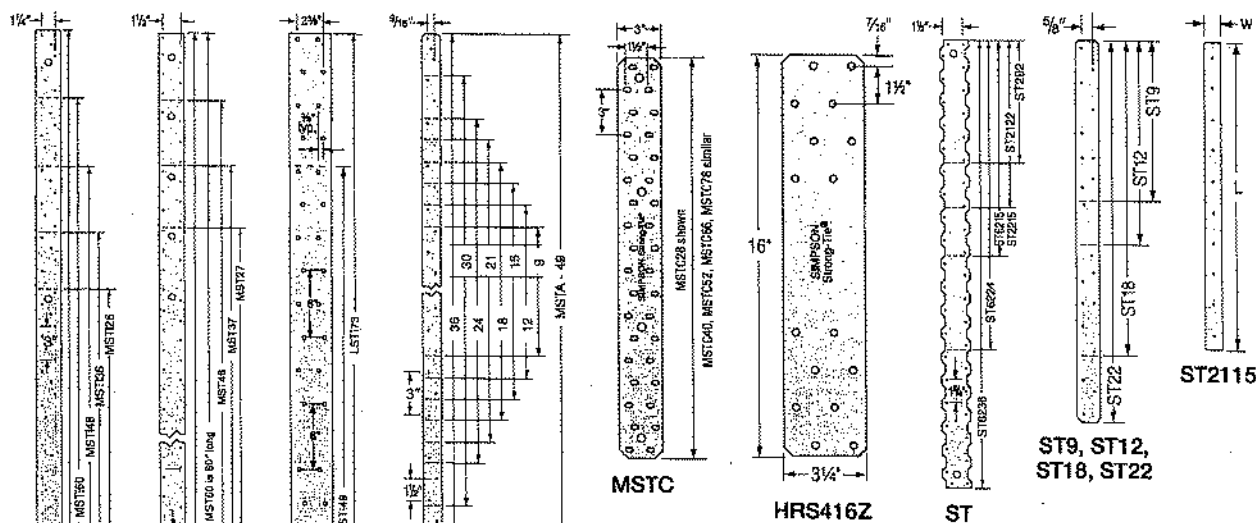
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

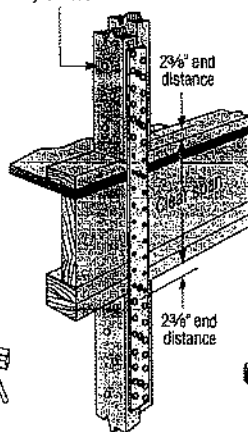
Installation: Use all specified fasteners; see General Notes

Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths



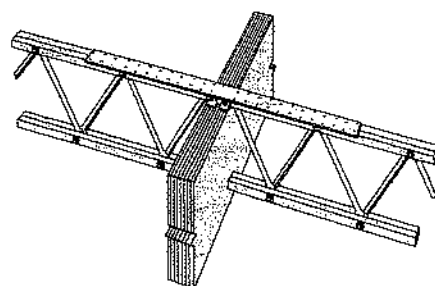
LSTA and MSTA
(Pilot holes not shown)

Stitch nailing
of double studs
by others



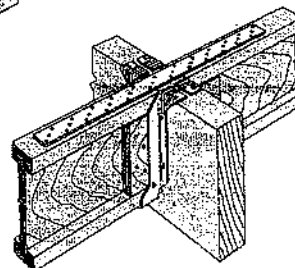
Nails are
not required
in the rim
board area

When nailing
the strap over
wood structural
panel sheathing,
use 2 1/2\" long
nail, minimum.

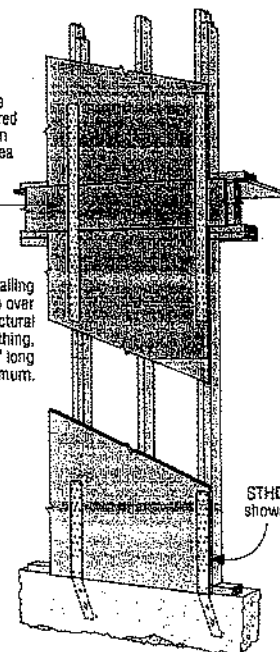


Typical LSTI Installation

Floor-to-Floor Tie
Installation
Showing a
Clear Span



Typical MSTI Installation
(MIT hanger shown)
LSTI similar



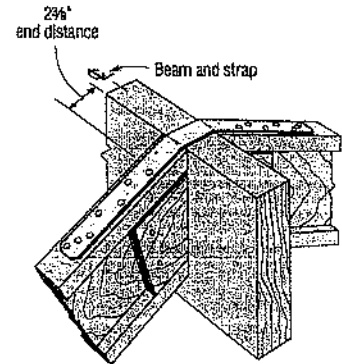
Typical Detail with
Strap Installed over
Wood Structural Panel
Sheathing

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI
Strap Ties (cont.)

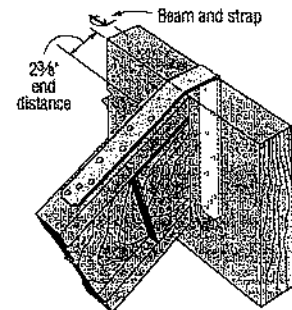
These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366-370 for more information.

	Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance				
			W	L		D.Fir-L		S-P-F		
						(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	
						lb.	lb.	lb.	lb.	
					kN	kN	kN	kN		
SS	LSTA9	20	1¼	9	(6) 10d	600	690	555	635	
	LSTA12		1¼	12	(8) 10d	800	920	735	845	
	LSTA15		1¼	15	(10) 10d	1000	1150	920	1060	
	LSTA18		1¼	18	(12) 10d	1200	1380	1105	1270	
	LSTA21		1¼	21	(14) 10d	1400	1610	1290	1485	
	LSTA24		1¼	24	(16) 10d	1600	1840	1475	1695	
	ST292		2⅞	9⅞	(8) 8d	585	675	535	615	
	ST2122		2⅞	12⅞	(12) 8d	940	1085	865	995	
	ST2115		¾	16⅞	(8) 8d	670	770	615	710	
	ST2215		2⅞	16⅞	(16) 8d	1335	1540	1235	1420	
	LSTA30		18	1¼	30	(20) 10d	2235	2465	2075	2385
	LSTA36			1¼	36	(24) 10d	2465	2465	2465	2465
LSTI49	3¾	49		(32) 10d x 1½"	3115	3580	2852	3280		
LSTI73	3¾	73		(48) 10d x 1½"	4670	5370	4280	4920		
MSTA9	1¼	9		(6) 10d	670	770	625	715		
MSTA12	1¼	12		(8) 10d	895	1030	830	955		
MSTA15	1¼	15		(10) 10d	1120	1285	1040	1195		
MSTA18	1¼	18		(12) 10d	1340	1545	1245	1430		
MSTA21	1¼	21		(14) 10d	1565	1800	1455	1670		
MSTA24	1¼	24		(16) 10d	1790	2060	1660	1910		
MSTA30	16	1¼		30	(20) 10d	2470	2840	2260	2595	
MSTA36		1¼		36	(24) 10d	2965	3070	2710	3070	
MSTA49		1¼	49	(28) 8d	2725	2725	2545	2725		
ST6215		2⅞	16⅞	(16) 8d	1405	1615	1300	1500		
ST6224		2⅞	23⅞	(24) 8d	2305	2650	2155	2475		
ST9		1¼	9	(6) 8d	525	605	490	560		
ST12		1¼	11⅞	(8) 8d	700	805	650	750		
ST18		1¼	17¾	(12) 8d	1050	1210	975	1125		
ST22		1¼	21⅞	(18) 8d	1580	1790	1485	1685		
						7.03	7.96	6.52	7.50	



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Use half of the nails in each member being connected to achieve the listed resistances.
- Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22-23 for other nail sizes and information.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

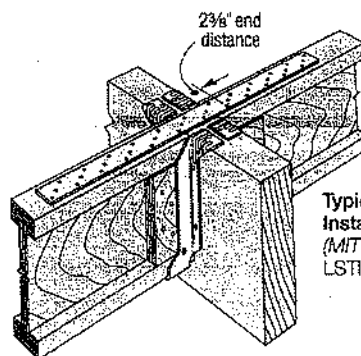
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

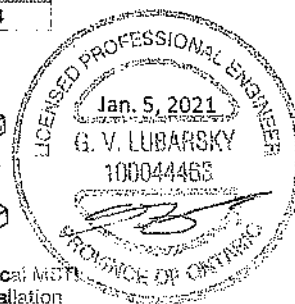
SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (In.)		Fasteners (Total)	Factored Tensile Resistance			
		W	L		D.Fir-L		S-P-F	
					(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
MSTC28	16	3	28¼	(32) 10d	3955	4545	3615	4155
					17.59	20.22	16.08	18.48
MSTC40		3	40¼	(48) 10d	5930	6820	5420	6235
					26.38	30.34	24.11	27.74
MSTC52	14	3	52¼	(54) 10d	6670	6940	6100	6940
					29.67	30.87	27.14	30.87
MSTC66		3	65¼	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
MSTC78	12	3	77¼	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
ST6236		2½	33 13⁄₁₆	(36) 8d	3735	4295	3270	3760
					16.61	19.11	14.55	16.73
MSTI26	12	2½	26	(22) 10d x 1 ½"	2825	3250	2475	2850
					12.57	14.46	11.01	12.68
MSTI36		2½	36	(32) 10d x 1 ½"	4110	4725	3600	4140
					18.28	21.02	16.01	18.42
MSTI48	10	2½	48	(44) 10d x 1 ½"	5650	6500	4955	5695
					25.13	28.91	22.04	25.33
MSTI60		2½	60	(56) 10d x 1 ½"	7195	7360	6305	7250
					32.01	32.74	28.05	32.25
MSTI72	8	2½	72	(68) 10d x 1 ½"	7360	7360	7240	7360
					32.74	32.74	32.21	32.74
MST27		2½	27	(26) 8d	2685	3090	2355	2710
					11.94	13.75	10.48	12.06
MST37	6	2½	37 ½	(38) 8d	3930	4515	3440	3960
					17.48	20.08	15.30	17.62
MST48		2½	48	(50) 8d	5170	5945	4530	5210
					23.00	26.45	20.15	23.18
HRS416Z	10	3¼	16	(16) ¼" x 1 ½" SDS	2400	2760	2120	2440
					10.68	12.28	9.43	10.85
MST60		2½	60	(64) 8d	6620	7610	5800	6670
					29.45	33.85	25.80	29.67
MST72	8	2½	72	(78) 8d	8065	9135	7065	8125
					35.88	40.64	31.43	36.14

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22–23 for other nail sizes and information.



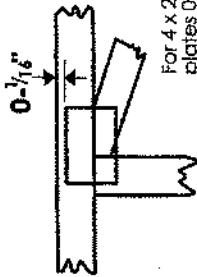
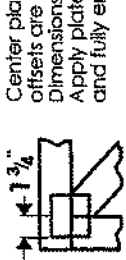
Typical MSTI
Installation
(MIT hanger shown)
LSTI similar



Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths or mm.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



*Plate location details available in Mitek software or upon request.

PLATE SIZE

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

4 x 4

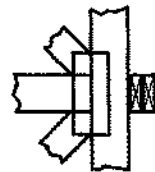
LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, I or Eliminator bracing if indicated.



BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

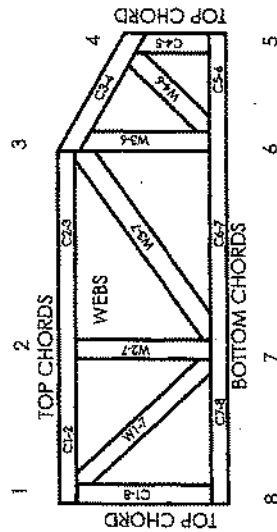


Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12491-R

© 2007 Mitek® All Rights Reserved

Mitek
POWER TO PERFORM.™

Mitek Engineering Reference Sheet: Miti-7473C rev. 10-06

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative 1, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019