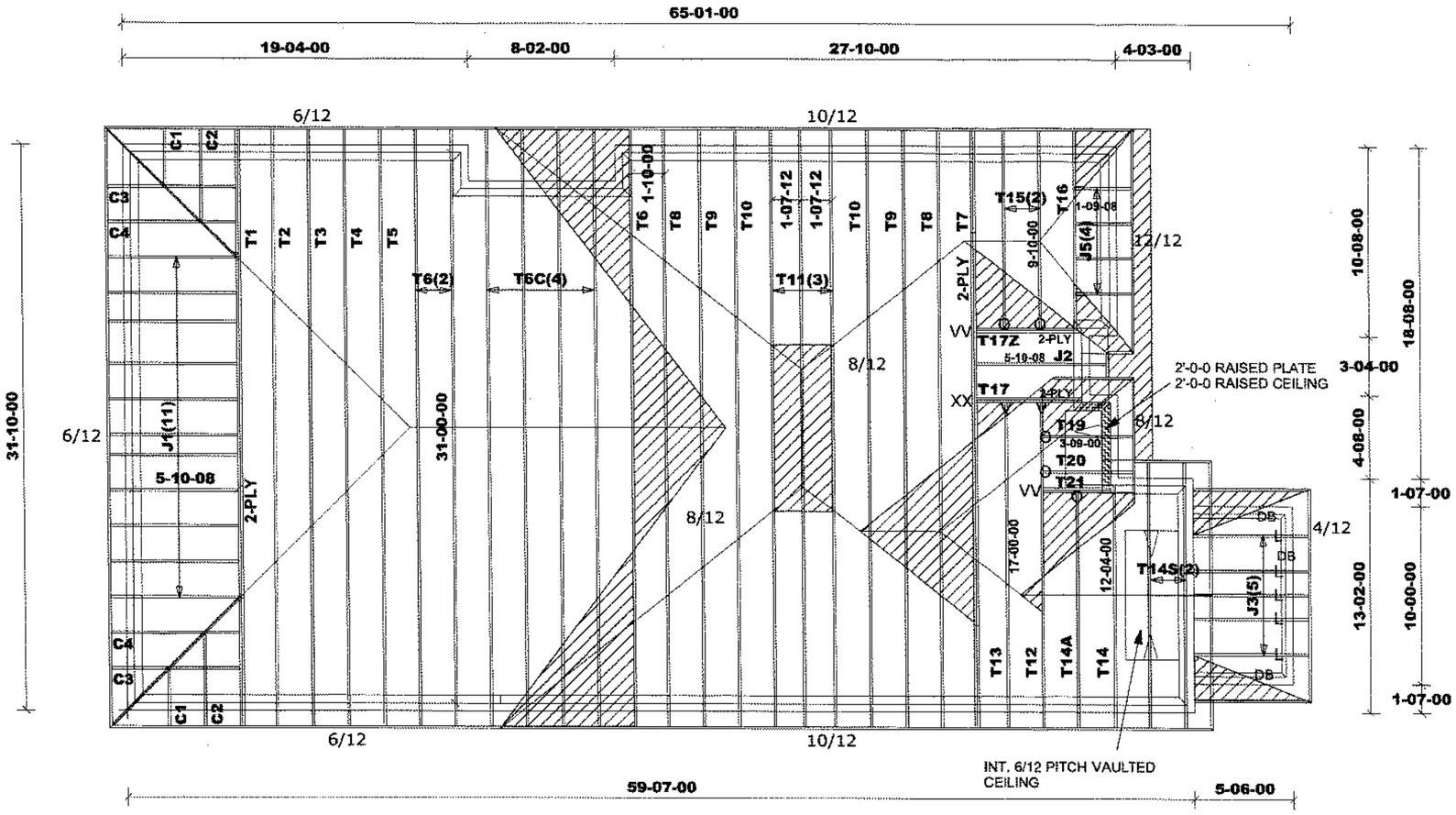


ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

10/12 ROOF PITCHES UNLESS OTHERWISE NOTED

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (L)

DENOTES:
CONVENTIONAL FRAMING

DB - DROPPED BEAMS BY OTHERS

DWG# TR22070145 TO TR22070176

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENTS ONLY
DWG# TR22070215

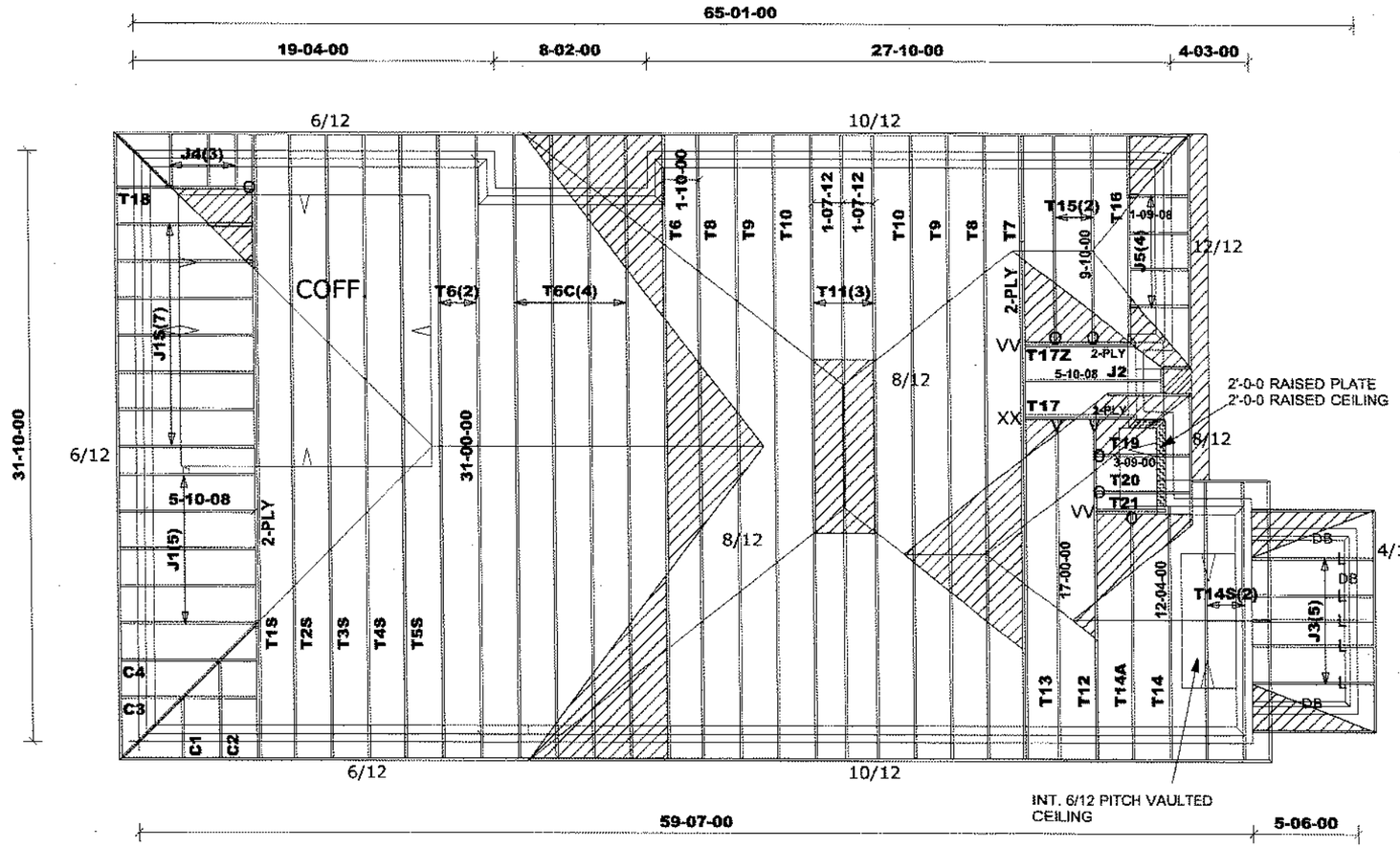
TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK ROOF TRUSSES INC.

Job Track: 52647	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 3803 / A STD	Mitek ver 8.5.3.233
Layout ID: 425212	Project: FORESTSIDE ESTATES		
Plan Log: 205921	Date: 7/18/22 Designer:		



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

10/12 ROOF PITCHES UNLESS OTHERWISE NOTED

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (L)

DENOTES:
CONVENTIONAL FRAMING

DB - DROPPED BEAMS BY OTHERS

DWG# TR22070145 TO TR22070152
DWG# TR22070158 TO TR22070184

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENTS ONLY
DWG# TR22070216

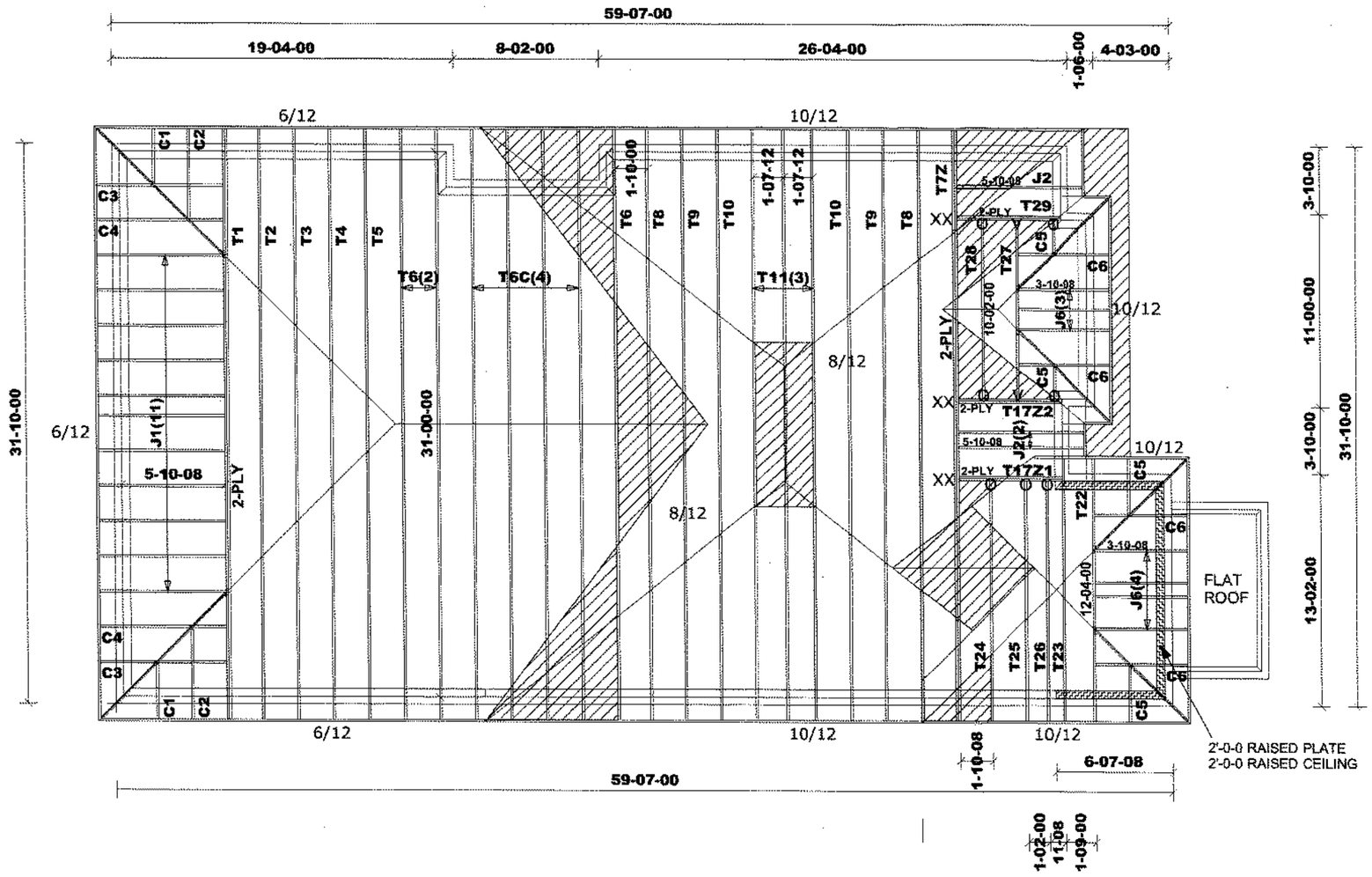
TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK ROOF TRUSSES INC.

Job Track: 52647	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 3803 / A OPT RAISED CEILING	Mitek ver 8.5.3.233
Layout ID: 425213	Project: FORESTSIDE ESTATES		
Plan Log: 205921	Date: 7/18/22 Designer:		



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

10/12 ROOF PITCHES UNLESS OTHERWISE NOTED

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)

DENOTES:
CONVENTIONAL FRAMING

DWG# TR22070145 TO TR22070150
DWG# TR22070153 TO TR22070159
DWG# TR22070161 TO TR22070164
DWG# TR22070185 TO TR22070198

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENTS ONLY
DWG# TR22070217

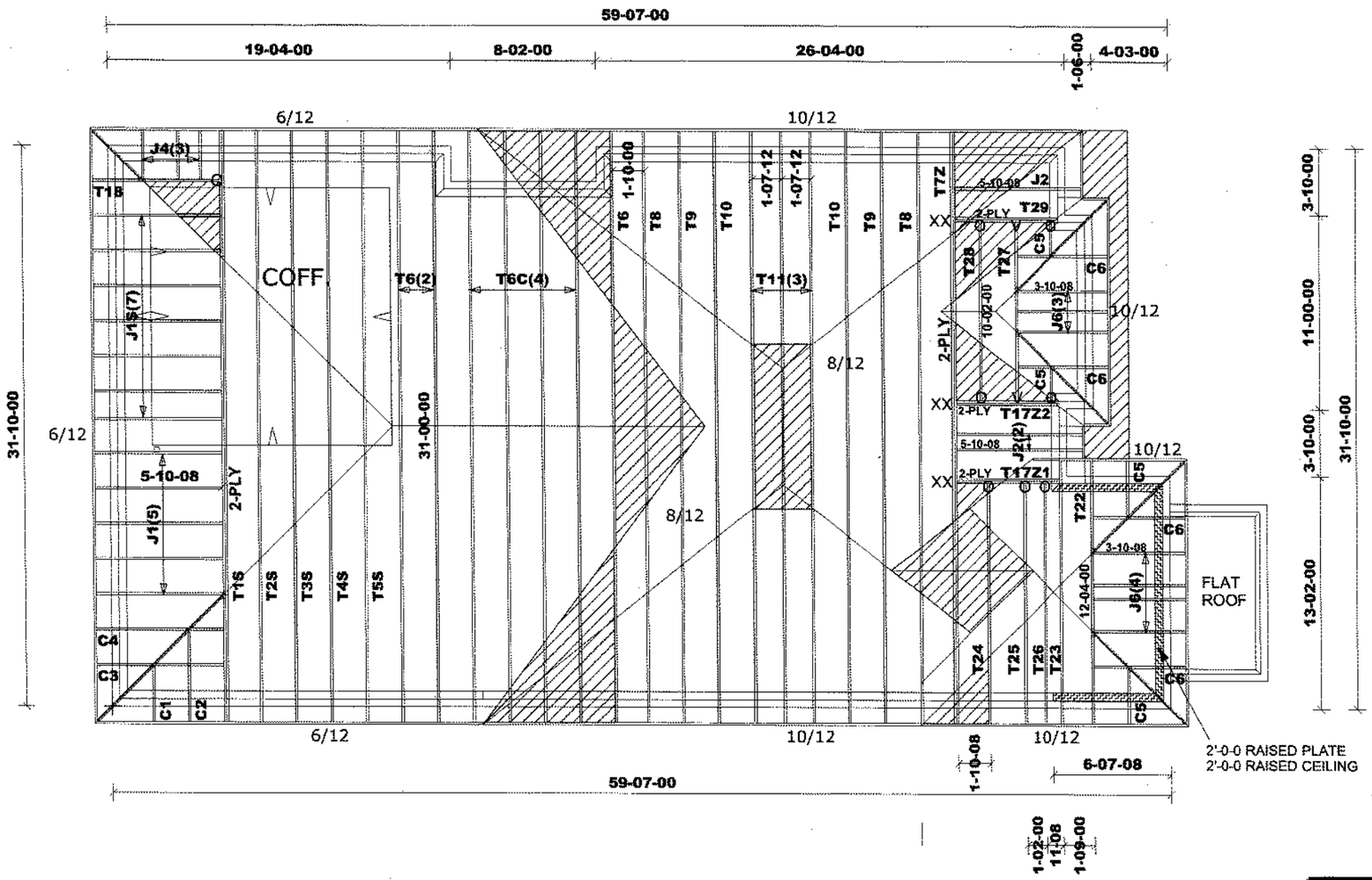
TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK ROOF TRUSSES INC.

Job Track: 52647	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 3803 / B STD	Mitek ver 8.5.3.233
Layout ID: 425214	Project: FORESTSIDE ESTATES		
Plan Log: 205921	Date: 7/18/22 Designer:		



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

10/12 ROOF PITCHES UNLESS OTHERWISE NOTED

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)

DENOTES:
CONVENTIONAL FRAMING

DWG# TR22070145 TO TR22070150
DWG# TR22070158 TO TR22070159
DWG# TR22070161 TO TR22070164
DWG# TR22070177 TO TR22070198

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENTS ONLY
DWG# TR22070218

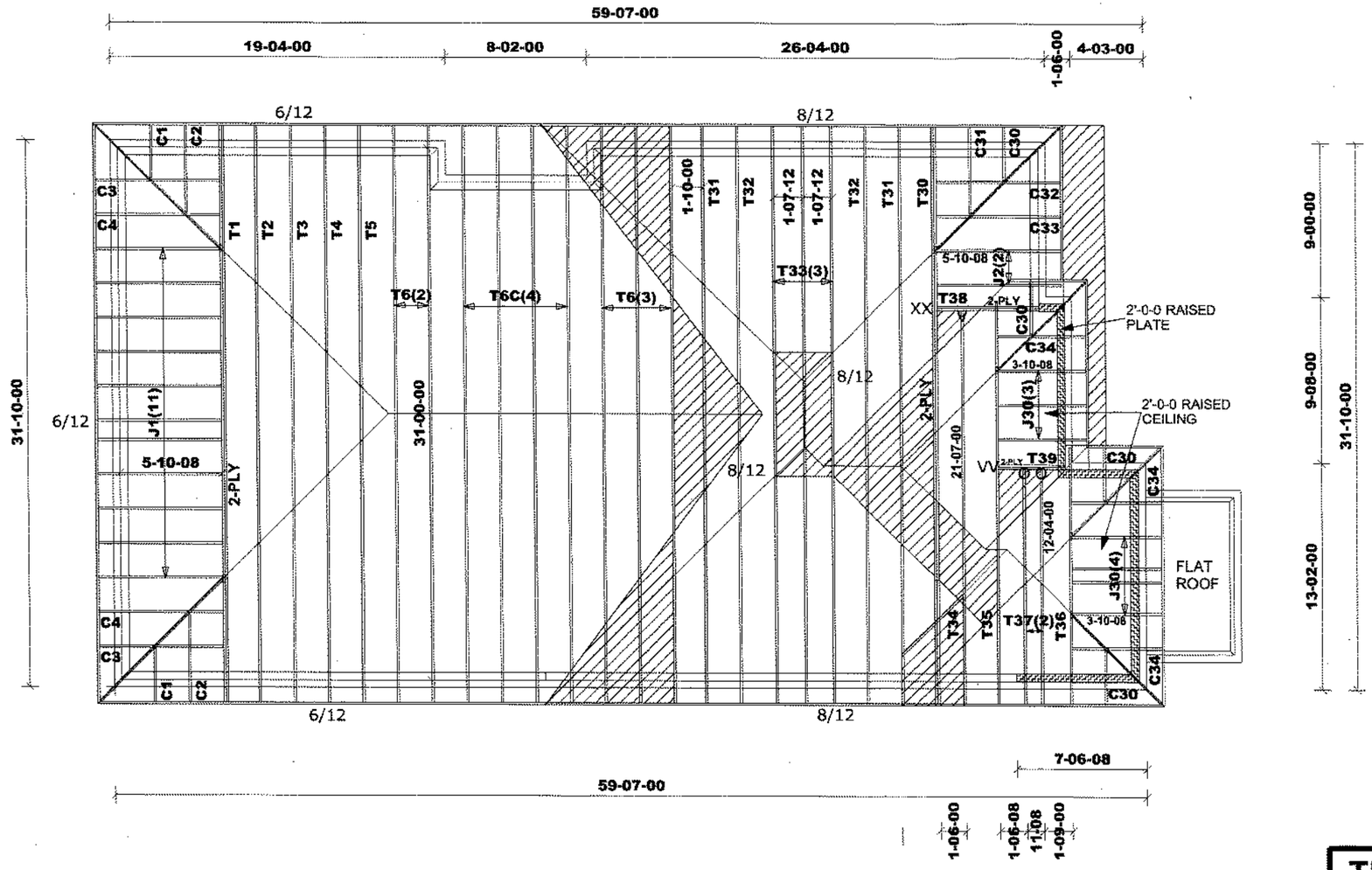
TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK ROOF TRUSSES INC.

Job Track: 52647	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 3803 / B OPT RAISED CEILING	Mitek ver 8.5.3.233
Layout ID: 425215	Project: FORESTSIDE ESTATES		
Plan Log: 205921	Date: 7/18/22 Designer:		



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

8/12 ROOF PITCHES UNLESS OTHERWISE NOTED

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)
H2.5T - (L)

DENOTES:
CONVENTIONAL FRAMING

DWG# TR22070145 TO TR22070150
DWG# TR22070153 TO TR22070159
DWG# TR22070199 TO TR22070214

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENTS ONLY
DWG# TR22070219

TRUSS PLACEMENT PLAN.

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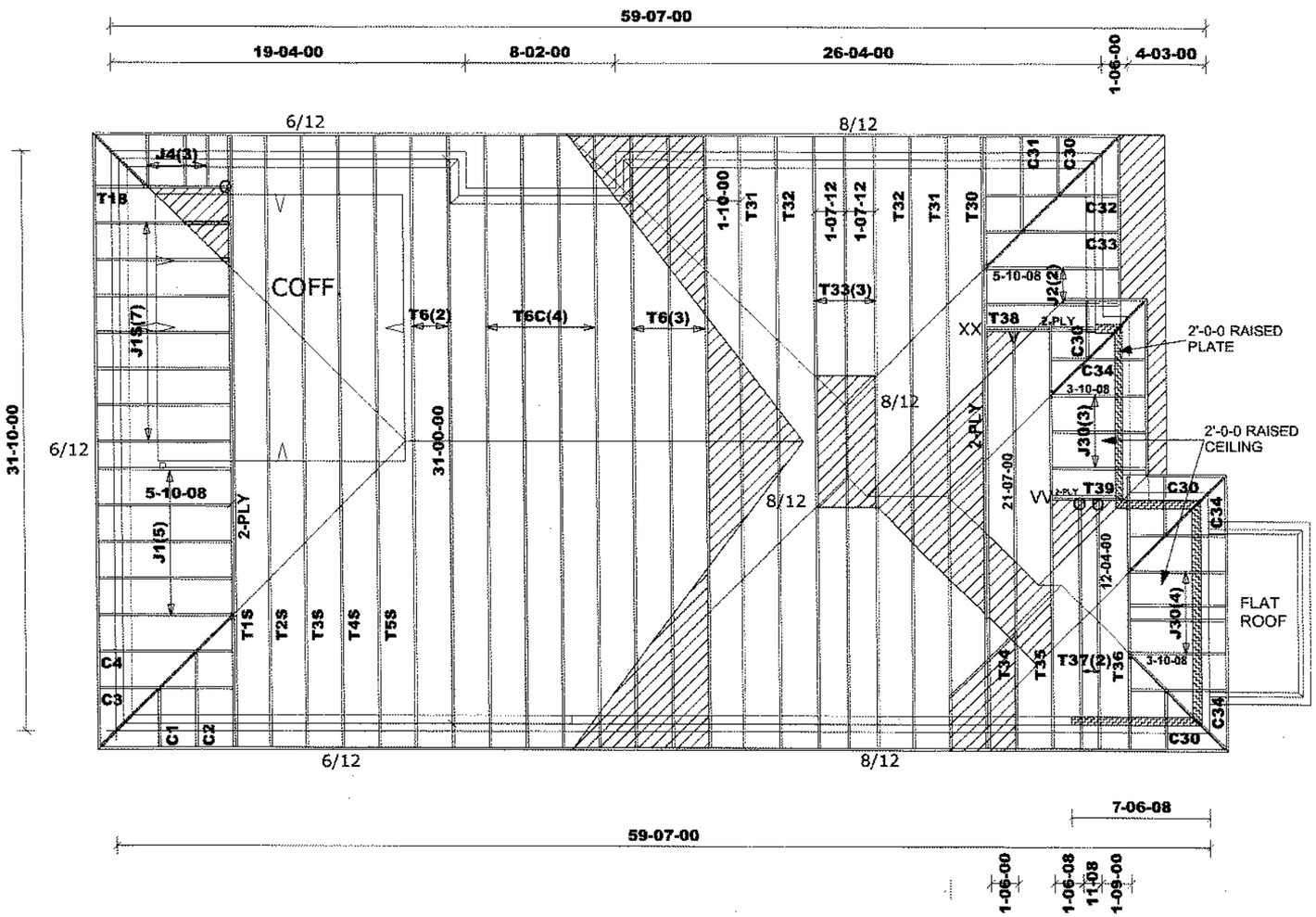
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK ROOF TRUSSES INC.

Job Track: 52647	Builder / Location: ROYAL PINE HOMES / BRAMPTON		Model / Elevation: UNIT 3803 / C STD
Layout ID: 425216	Project: FORESTSIDE ESTATES		Mitek ver 8.5.3.233
Plan Log: 205921	Date: 7/18/22	Designer:	



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

8/12 ROOF PITCHES UNLESS OTHERWISE NOTED

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)
H2.5T - (L)

DENOTES:
CONVENTIONAL FRAMING

DWG# TR22070145 TO TR22070150
DWG# TR22070158 TO TR22070159
DWG# TR22070177 TO TR22070184
DWG# TR22070199 TO TR22070214

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505085
PROVINCE OF ONTARIO

STRUCTURAL COMPONENTS ONLY
DWG# TR22070220

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

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TAMARACK ROOF TRUSSES INC.

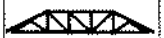
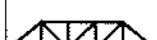
Job Track: 52647	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 3803 / C OPT RAISED CEILING	Mitek ver 8.5.3.233
Layout ID: 425217	Project: FORESTSIDE ESTATES		
Plan Log: 205921	Date: 7/18/22 Designer:		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425212
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	1 of 3
Lot #:		Date:	07-21-2022
Elevation:	A STD	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

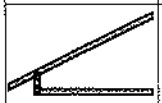
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	6 /12	31-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	304.3 190.00		
	1	T2 Hip	6 /12	31-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.87 80.17		
	1	T3 Hip	6 /12	31-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.51 80.00		
	1	T4 Hip	6 /12	31-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.93 80.00		
	1	T5 Hip	6 /12	31-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	128.89 81.83		
	3	T6 Common	6 /12	31-00-00	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	378.72 237.50		
	4	T6C Common	6 /12	31-00-00	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	504.96 316.67		
	1 2-ply	T7 Hip Girder	10 /12	31-00-00	5-04-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	384.31 232.00		
	2	T8 Hip	10 /12	31-00-00	6-07-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	264.5 167.00		
	2	T9 Hip	10 /12	31-00-00	7-11-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	301.31 188.33		
	2	T10 Hip	10 /12	31-00-00	9-03-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	311.88 193.00		
	3	T11 Hip	10 /12	31-00-00	10-07-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	524.89 330.00		
	1	T12 Hip Girder	10 /12	17-00-00	5-10-14	2 x 4 2 x 6	1-03-08	1-07-11 3-07-11	90.38 57.83		
	1	T13 Hip	10 /12	17-00-00	7-02-14	2 x 4	1-03-08	1-07-11 3-07-11	80.27 51.33		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425212
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	2 of 3
Lot #:		Date:	07-21-2022
Elevation:	A STD	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

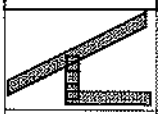
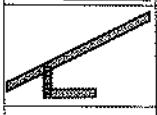
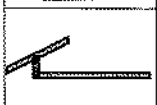
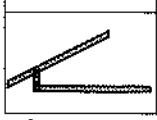
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T14 Common	10 /12	**12-04-00**	6-09-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	59.43 38.17		
	1	T14A Common	10 /12	**11-11-00**	6-09-06	2 x 4	1-03-08	1-07-11 1-07-11	56.62 36.17		
	2	T14S Roof Special	10 /12 6 /12	**12-04-00**	6-09-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	118.6 80.33		
	2	T15 Common	10 /12	**9-10-00**	5-08-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	89.42 58.67		
	1	T16 Hip Girder	10 /12	**9-10-00**	3-08-00	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	44.76 30.67		
	1 2-ply	T17 Monopitch Girder	8 /12	**5-10-08**	5-03-14	2 x 4 2 x 6		1-04-14 5-03-14	64.27 40.67		
	1 2-ply	T17Z Monopitch Girder	8 /12	**5-10-08**	5-03-14	2 x 4 2 x 6		1-04-14 5-03-14	64.27 40.67		
	1	T19 Half Hip	8 /12	**3-09-00**	5-02-07	2 x 4	1-03-08	1-04-14 5-02-07	28.85 19.83		
	1	T20 Half Hip	8 /12	**3-09-00**	5-10-14	2 x 4	1-03-08	1-04-14 5-10-14	30.25 21.00		
	1 2-ply	T21 Monopitch Girder	8 /12	**3-09-00**	5-10-14	2 x 4 2 x 6		3-04-14 5-10-14	49.03 31.67		
	11	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	1	J2 Jack-Open	8 /12	**5-10-08**	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	19.09 11.67		
	5	J3 Jack-Open	4 /12	**5-01-00**	2-08-08	2 x 4	1-03-08	3-15 2-00-04	77.61 46.67		
	4	J5 Jack-Open	12 /12	**1-09-08**	3-08-00	2 x 4	1-03-08	1-10-08 3-08-00	42.29 28.00		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425212
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	3 of 3
Lot #:		Date:	07-21-2022
Elevation:	A STD	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	C1 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	C2 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	C3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	C4 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		

TOTAL # TRUSS= **70** TOTAL BFT OF ALL TRUSSES= **2940.51** BFT. TOTAL WEIGHT OF ALL TRSSES **4669.57** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	H2.5T	
1	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
5	Hardware	LUS24	
2	Hardware	LUS26-2	

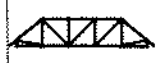
TOTAL NUMBER OF ITEMS= **15**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425213
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	1 of 3
Lot #:		Date:	07-21-2022
Elevation:	A OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

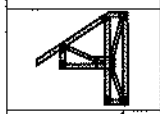
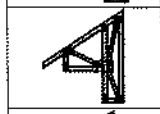
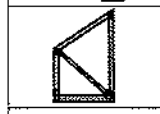
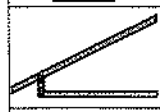
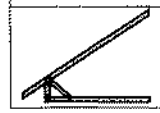
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S Hip Girder	6 /12	31-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	322.18 205.67		
	1	T2S Hip	6 /12	31-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.95 84.50		
	1	T3S Hip	6 /12	31-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.78 84.50		
	1	T4S Hip	6 /12	31-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	138.74 87.83		
	1	T5S Hip	6 /12	31-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	144.67 93.67		
	3	T6 Common	6 /12	31-00-00	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	378.72 237.50		
	4	T6C Common	6 /12	31-00-00	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	504.96 316.67		
	1 2-ply	T7 Hip Girder	10 /12	31-00-00	5-04-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	384.31 232.00		
	2	T8 Hip	10 /12	31-00-00	6-07-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	264.5 167.00		
	2	T9 Hip	10 /12	31-00-00	7-11-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	301.31 188.33		
	2	T10 Hip	10 /12	31-00-00	9-03-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	311.88 193.00		
	3	T11 Hip	10 /12	31-00-00	10-07-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	524.89 330.00		
	1	T12 Hip Girder	10 /12	17-00-00	5-10-14	2 x 4 2 x 6	1-03-08	1-07-11 3-07-11	90.38 57.83		
	1	T13 Hip	10 /12	17-00-00	7-02-14	2 x 4	1-03-08	1-07-11 3-07-11	80.27 51.33		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425213
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	2 of 3
Lot #:		Date:	07-21-2022
Elevation:	A OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

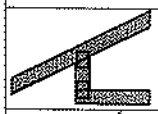
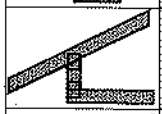
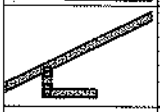
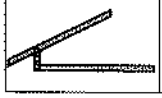
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T14 Common	10 /12	**12-04-00**	6-09-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	59.43 38.17		
	1	T14A Common	10 /12	**11-11-00**	6-09-06	2 x 4	1-03-08	1-07-11 1-07-11	56.62 36.17		
	2	T14S Roof Special	10 /12 6 /12	**12-04-00**	6-09-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	118.6 80.33		
	2	T15 Common	10 /12	**9-10-00**	5-08-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	89.42 58.67		
	1	T16 Hip Girder	10 /12	**9-10-00**	3-08-00	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	44.76 30.67		
	1 2-ply	T17 Monopitch Girder	8 /12	**5-10-08**	5-03-14	2 x 4 2 x 6		1-04-14 5-03-14	64.27 40.67		
	1 2-ply	T17Z Monopitch Girder	8 /12	**5-10-08**	5-03-14	2 x 4 2 x 6		1-04-14 5-03-14	64.27 40.67		
	1	T18 Half Hip Girder	6 /12	**5-10-08**	1-11-00	2 x 4	1-03-08	1-02-00 1-11-00	23.29 16.50		
	1	T19 Half Hip	8 /12	**3-09-00**	5-02-07	2 x 4	1-03-08	1-04-14 5-02-07	28.85 19.83		
	1	T20 Half Hip	8 /12	**3-09-00**	5-10-14	2 x 4	1-03-08	1-04-14 5-10-14	30.25 21.00		
	1 2-ply	T21 Monopitch Girder	8 /12	**3-09-00**	5-10-14	2 x 4 2 x 6		3-04-14 5-10-14	49.03 31.67		
	5	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	83.97 53.33		
	7	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	146.04 98.00		
	1	J2 Jack-Open	8 /12	**5-10-08**	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	19.09 11.67		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425213
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	3 of 3
Lot #:		Date:	07-21-2022
Elevation:	A OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	5	J3 Jack-Open	4 /12	**5-01-00**	2-08-08	2 x 4	1-03-08	3-15 2-00-04	77.61 46.67		
	3	J4 Jack-Open	6 /12	**1-06-00**	1-11-00	2 x 4	1-03-08	1-02-00 1-11-00	18.68 14.00		
	4	J5 Jack-Open	12 /12	**1-09-08**	3-08-00	2 x 4	1-03-08	1-10-08 3-08-00	42.29 28.00		
	1	C1 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	1	C2 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	C4 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		

TOTAL # TRUSS= **71** TOTAL BFT OF ALL TRUSSES= **3022.52** BFT. TOTAL WEIGHT OF ALL TRSSES **4771.34** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	H2.5T	
1	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
6	Hardware	LUS24	
2	Hardware	LUS26-2	

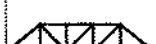
TOTAL NUMBER OF ITEMS= **16**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425214
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	1 of 3
Lot #:		Date:	07-21-2022
Elevation:	B STD	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	**31-00-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	304.3 190.00		
	1	T2 Hip	6 /12	**31-00-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.87 80.17		
	1	T3 Hip	6 /12	**31-00-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.51 80.00		
	1	T4 Hip	6 /12	**31-00-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.93 80.00		
	1	T5 Hip	6 /12	**31-00-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	128.89 81.83		
	3	T6 Common	6 /12	**31-00-00**	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	378.72 237.50		
	4	T6C Common	6 /12	**31-00-00**	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	504.96 316.67		
	1 2-ply	T7Z Hip Girder	10 /12	**31-00-00**	5-04-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	384.31 232.00		
	2	T8 Hip	10 /12	**31-00-00**	6-07-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	264.5 167.00		
	2	T9 Hip	10 /12	**31-00-00**	7-11-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	301.31 188.33		
	2	T10 Hip	10 /12	**31-00-00**	9-03-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	311.88 193.00		
	3	T11 Hip	10 /12	**31-00-00**	10-07-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	524.89 330.00		
	1 2-ply	T17Z1 Monopitch Girder	8 /12	**5-10-08**	5-03-14	2 x 4 2 x 6		1-04-14 5-03-14	64.27 40.67		
	1 2-ply	T17Z2 Monopitch Girder	8 /12	**5-10-08**	5-03-14	2 x 4 2 x 6		1-04-14 5-03-14	64.27 40.67		

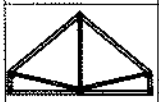
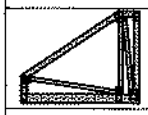
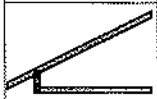
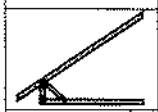
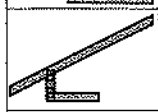
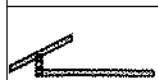
TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	ROYAL PINE HOMES
Project:	FORESTSIDE ESTATES
Location:	BRAMPTON
Model:	UNIT 3803
Lot #:	
Elevation:	B STD

Job Track:	52647
PlanLog:	205921
Layout ID:	425214
Ref #	
Page:	2 of 3
Date:	07-21-2022
Designer:	
Sales Rep:	Rick DiCiano

Roof Trusses

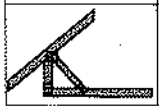
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T22 Hip Girder	10 /12	**12-04-00**	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	58.86 38.00		
	1	T23 Hip	10 /12	**12-04-00**	6-03-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	62.13 41.00		
	1	T24 Hip	10 /12	**12-04-00**	5-08-04	2 x 4	1-03-08 1-03-08	3-07-11 3-07-11	66.26 44.33		
	1	T25 Hip	10 /12	**12-04-00**	7-04-04	2 x 4	1-03-08 1-03-08	3-07-11 3-07-11	68.55 44.33		
	1	T26 Hip	10 /12	**12-04-00**	8-03-15	2 x 4	1-03-08 1-03-08	3-07-11 3-07-11	72.8 47.67		
	1	T27 Hip Girder	10 /12	**10-02-00**	4-10-07	2 x 4 2 x 6		1-07-11 1-07-11	51.94 36.17		
	1	T28 Common	10 /12	**10-02-00**	5-10-08	2 x 4		1-07-11 1-07-11	42.13 27.33		
	1 2-ply	T29 Half Hip Girder	8 /12	**5-10-08**	4-07-08	2 x 4 2 x 6		1-04-14 4-07-08	67.95 42.33		
	11	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	3	J2 Jack-Open	8 /12	**5-10-08**	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	57.27 35.00		
	7	J6 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	105.97 67.67		
	2	C1 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	C2 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425214
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	3 of 3
Lot #:		Date:	07-21-2022
Elevation:	B STD	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	C4 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	4	C5 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	39.03 28.00		
	4	C6 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	48.16 33.33		

TOTAL # TRUSS= **74** TOTAL BFT OF ALL TRUSSES= **2913.66** BFT. TOTAL WEIGHT OF ALL TRSSES **4624.01** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
7	Hardware	LUS24	

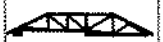
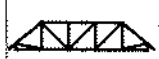
TOTAL NUMBER OF ITEMS= **12**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425215
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	1 of 3
Lot #:		Date:	07-21-2022
Elevation:	B OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

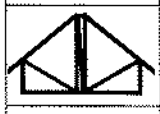
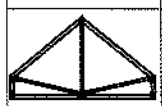
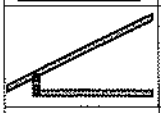
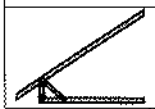
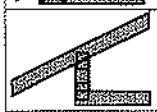
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	6 /12	**31-00-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	322.18 205.67		
	1	T2S Hip	6 /12	**31-00-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.95 84.50		
	1	T3S Hip	6 /12	**31-00-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.78 84.50		
	1	T4S Hip	6 /12	**31-00-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	138.74 87.83		
	1	T5S Hip	6 /12	**31-00-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	144.67 93.67		
	3	T6 Common	6 /12	**31-00-00**	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	378.72 237.50		
	4	T6C Common	6 /12	**31-00-00**	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	504.96 316.67		
	1 2-ply	T7Z Hip Girder	10 /12	**31-00-00**	5-04-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	384.31 232.00		
	2	T8 Hip	10 /12	**31-00-00**	6-07-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	264.5 167.00		
	2	T9 Hip	10 /12	**31-00-00**	7-11-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	301.31 188.33		
	2	T10 Hip	10 /12	**31-00-00**	9-03-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	311.88 193.00		
	3	T11 Hip	10 /12	**31-00-00**	10-07-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	524.89 330.00		
	1 2-ply	T17Z1 Monopitch Girder	8 /12	**5-10-08**	5-03-14	2 x 4 2 x 6		1-04-14 5-03-14	64.27 40.67		
	1 2-ply	T17Z2 Monopitch Girder	8 /12	**5-10-08**	5-03-14	2 x 4 2 x 6		1-04-14 5-03-14	64.27 40.67		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425215
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	2 of 3
Lot #:		Date:	07-21-2022
Elevation:	B OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

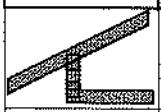
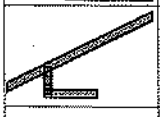
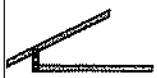
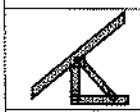
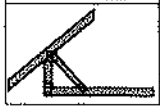
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T18 Half Hip Girder	6 /12	5-10-08	1-11-00	2 x 4	1-03-08	1-02-00 1-11-00	23.29 16.50		
	1	T22 Hip Girder	10 /12	12-04-00	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	58.86 38.00		
	1	T23 Hip	10 /12	12-04-00	6-03-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	62.13 41.00		
	1	T24 Hip	10 /12	12-04-00	5-08-04	2 x 4	1-03-08 1-03-08	3-07-11 3-07-11	66.26 44.33		
	1	T25 Hip	10 /12	12-04-00	7-04-04	2 x 4	1-03-08 1-03-08	3-07-11 3-07-11	68.55 44.33		
	1	T26 Hip	10 /12	12-04-00	8-03-15	2 x 4	1-03-08 1-03-08	3-07-11 3-07-11	72.8 47.67		
	1	T27 Hip Girder	10 /12	10-02-00	4-10-07	2 x 4 2 x 6		1-07-11 1-07-11	51.94 36.17		
	1	T28 Common	10 /12	10-02-00	5-10-08	2 x 4		1-07-11 1-07-11	42.13 27.33		
	1 2-ply	T29 Half Hip Girder	8 /12	5-10-08	4-07-08	2 x 4 2 x 6		1-04-14 4-07-08	67.95 42.33		
	5	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	83.97 53.33		
	7	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	146.04 98.00		
	3	J2 Jack-Open	8 /12	5-10-08	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	57.27 35.00		
	3	J4 Jack-Open	6 /12	1-06-00	1-11-00	2 x 4	1-03-08	1-02-00 1-11-00	18.68 14.00		
	7	J6 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	105.97 67.67		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425215
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	3 of 3
Lot #:		Date:	07-21-2022
Elevation:	B OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	C1 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	1	C2 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	C4 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		
	4	C5 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	39.03 28.00		
	4	C6 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	48.16 33.33		

TOTAL # TRUSS= **75** TOTAL BFT OF ALL TRUSSES= **2995.67** BFT. TOTAL WEIGHT OF ALL TRSSES **4725.77** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
8	Hardware	LUS24	

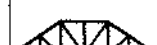
TOTAL NUMBER OF ITEMS= **13**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425216
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	1 of 3
Lot #:		Date:	07-21-2022
Elevation:	C STD	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

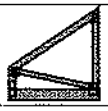
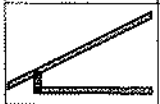
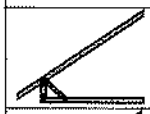
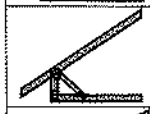
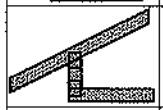
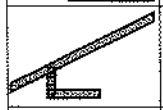
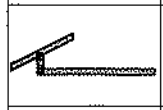
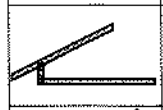
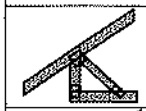
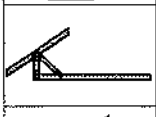
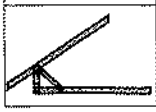
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	6 /12	31-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	304.3 190.00		
	1	T2 Hip	6 /12	31-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.87 80.17		
	1	T3 Hip	6 /12	31-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.51 80.00		
	1	T4 Hip	6 /12	31-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.93 80.00		
	1	T5 Hip	6 /12	31-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	128.89 81.83		
	5	T6 Common	6 /12	31-00-00	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	631.2 395.83		
	4	T6C Common	6 /12	31-00-00	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	504.96 316.67		
	1 2-ply	T30 Hip Girder	8 /12	31-00-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-14 1-04-14	381.31 234.67		
	2	T31 Hip	8 /12	31-00-00	6-07-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	278.24 178.67		
	2	T32 Hip	8 /12	31-00-00	7-11-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	278.43 176.67		
	3	T33 Hip	8 /12	31-00-00	9-03-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	424.03 266.00		
	1	T34 Roof Special	8 /12	21-07-00	7-03-14	2 x 4	1-03-08	3-04-14 3-08-03	107.87 69.00		
	1	T35 Hip Girder	8 /12	22-00-00	5-11-14	2 x 4 2 x 6	1-03-08	3-04-14 1-04-14	128.6 82.50		
	1	T36 Hip Girder	8 /12	12-04-00	3-11-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	54.17 34.67		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425216
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	2 of 3
Lot #:		Date:	07-21-2022
Elevation:	C STD	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T37 Hip	8 /12	12-04-00	5-01-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	113.84 73.67		
	1 2-ply	T38 Monopitch Girder	0 /12	5-10-08	2-00-00	2 x 4 2 x 6		2-00-00 2-00-00	51.42 33.33		
	1 2-ply	T39 Monopitch Girder	8 /12	3-10-08	3-11-14	2 x 4 2 x 6		1-04-14 3-11-14	39.52 25.33		
	11	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	2	J2 Jack-Open	8 /12	5-10-08	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	38.18 23.33		
	7	J30 Jack-Open	8 /12	3-10-08	3-11-14	2 x 4	1-03-08	1-04-14 3-11-14	98.47 63.00		
	2	C1 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	C2 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	C3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	C4 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	4	C30 Jack-Open	8 /12	1-09-07	2-07-03	2 x 4	1-03-08 1-01	1-04-14 2-07-03	35.7 22.67		
	1	C31 Jack-Open	8 /12	1-10-08	3-11-03	2 x 4	1-03-08 1-10-15	1-04-14 2-07-14	11.67 7.67		
	1	C32 Jack-Open	8 /12	1-09-07	2-07-03	2 x 4	1-03-08 4-01-01	1-04-14 2-07-03	13.49 8.33		
	1	C33 Jack-Open	8 /12	3-09-07	3-11-03	2 x 4	1-03-08 2-01-01	1-04-14 3-11-03	16.23 10.33		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425216
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	3 of 3
Lot #:		Date:	07-21-2022
Elevation:	C STD	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	3	C34 Jack-Open	8 /12	**1-09-07**	2-07-03	2 x 4	1-03-08 2-01-01	1-04-14 2-07-03	33.62 21.00		

TOTAL # TRUSS= **71** TOTAL BFT OF ALL TRUSSES= **2726** BFT. TOTAL WEIGHT OF ALL TRSSES **4324.81** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
2	Hardware	LUS24	
1	Hardware	LUS26-2	

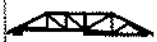
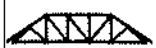
TOTAL NUMBER OF ITEMS= **5**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425217
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	1 of 3
Lot #:		Date:	07-21-2022
Elevation:	C OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

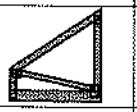
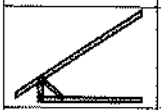
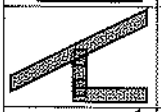
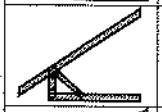
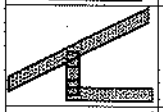
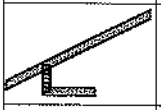
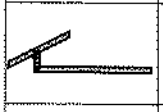
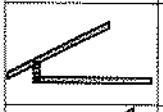
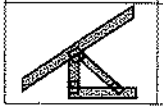
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S Hip Girder	6 /12	**31-00-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	322.18 205.67		
	1	T2S Hip	6 /12	**31-00-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.95 84.50		
	1	T3S Hip	6 /12	**31-00-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.78 84.50		
	1	T4S Hip	6 /12	**31-00-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	138.74 87.83		
	1	T5S Hip	6 /12	**31-00-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	144.67 93.67		
	5	T6 Common	6 /12	**31-00-00**	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	631.2 395.83		
	4	T6C Common	6 /12	**31-00-00**	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	504.96 316.67		
	1	T18 Half Hip Girder	6 /12	**5-10-08**	1-11-00	2 x 4	1-03-08	1-02-00 1-11-00	23.29 16.50		
	1 2-ply	T30 Hip Girder	8 /12	**31-00-00**	5-03-15	2 x 6	1-03-08 1-03-08	1-04-14 1-04-14	381.31 234.67		
	2	T31 Hip	8 /12	**31-00-00**	6-07-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	278.24 178.67		
	2	T32 Hip	8 /12	**31-00-00**	7-11-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	278.43 176.67		
	3	T33 Hip	8 /12	**31-00-00**	9-03-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	424.03 266.00		
	1	T34 Roof Special	8 /12	**21-07-00**	7-03-14	2 x 4	1-03-08	3-04-14 3-08-03	107.87 69.00		
	1	T35 Hip Girder	8 /12	**22-00-00**	5-11-14	2 x 4 2 x 6	1-03-08	3-04-14 1-04-14	128.6 82.50		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425217
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	2 of 3
Lot #:		Date:	07-21-2022
Elevation:	C OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

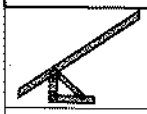
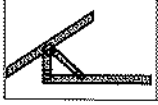
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T36 Hip Girder	8 /12	**12-04-00**	3-11-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	54.17 34.67		
	2	T37 Hip	8 /12	**12-04-00**	5-01-14	2 x 4	1-03-08 1-03-08	1-04-14 1-04-14	113.84 73.67		
	1 2-ply	T38 Monopitch Girder	0 /12	**5-10-08**	2-00-00	2 x 4 2 x 6		2-00-00 2-00-00	51.42 33.33		
	1 2-ply	T39 Monopitch Girder	8 /12	**3-10-08**	3-11-14	2 x 4 2 x 6		1-04-14 3-11-14	39.52 25.33		
	5	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	83.97 53.33		
	7	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	146.04 98.00		
	2	J2 Jack-Open	8 /12	**5-10-08**	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	38.18 23.33		
	3	J4 Jack-Open	6 /12	**1-06-00**	1-11-00	2 x 4	1-03-08	1-02-00 1-11-00	18.68 14.00		
	7	J30 Jack-Open	8 /12	**3-10-08**	3-11-14	2 x 4	1-03-08	1-04-14 3-11-14	98.47 63.00		
	1	C1 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	1	C2 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	C4 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		
	4	C30 Jack-Open	8 /12	**1-09-07**	2-07-03	2 x 4	1-03-08 1-01	1-04-14 2-07-03	35.7 22.67		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52647
Builder:	ROYAL PINE HOMES	PlanLog:	205921
Project:	FORESTSIDE ESTATES	Layout ID:	425217
Location:	BRAMPTON	Ref #	
Model:	UNIT 3803	Page:	3 of 3
Lot #:		Date:	07-21-2022
Elevation:	C OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	C31 Jack-Open	8 /12	**1-10-08**	3-11-03	2 x 4	1-03-08 1-10-15	1-04-14 2-07-14	11.67 7.67		
	1	C32 Jack-Open	8 /12	**1-09-07**	2-07-03	2 x 4	1-03-08 4-01-01	1-04-14 2-07-03	13.49 8.33		
	1	C33 Jack-Open	8 /12	**3-09-07**	3-11-03	2 x 4	1-03-08 2-01-01	1-04-14 3-11-03	16.23 10.33		
	3	C34 Jack-Open	8 /12	**1-09-07**	2-07-03	2 x 4	1-03-08 2-01-01	1-04-14 2-07-03	33.62 21.00		

TOTAL # TRUSS= **72** TOTAL BFT OF ALL TRUSSES= **2808.01** BFT. TOTAL WEIGHT OF ALL TRSSES **4426.58** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
3	Hardware	LUS24	
1	Hardware	LUS26-2	

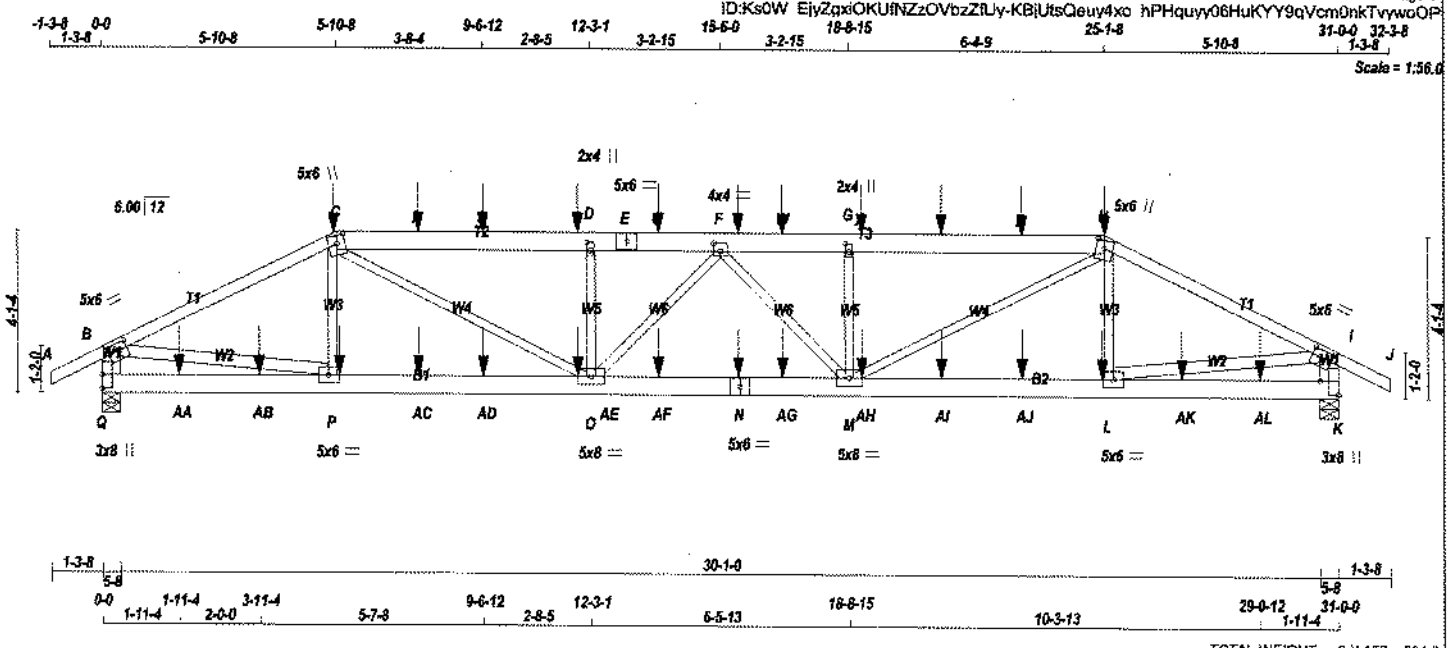
TOTAL NUMBER OF ITEMS= **6**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T1	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 18 15:56:28 2022 Page 1

ID:Ks0W EjyZqxiOKUfNZzOVbzZfUy-KBjUtsQeuy4xo hPHquyy06HuKYY9qVcm0nkTvywoOP



TOTAL WEIGHT = 2 X 152 = 304 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
H-J	1	12	SIDE(61.0)
C-E	2	12	SIDE(183.1)
E-H	2	12	SIDE(183.1)
Q-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q-N	2	12	SIDE(183.1)
N-K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070153 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2799	0	2799	0	0	5-8	5-8
K	2800	0	2800	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1987	1263 / 0	0 / 0	0 / 0	0 / 0	724 / 0	0 / 0
K	1987	1263 / 0	0 / 0	0 / 0	0 / 0	724 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI	LC	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.06	(1)	10.00
B-C	-4202 / 0	-84.9	-84.9	0.43	(1)	4.27
C-R	-5814 / 0	-84.9	-84.9	0.26	(1)	4.66
R-S	-5814 / 0	-84.9	-84.9	0.26	(1)	4.66
S-T	-5814 / 0	-84.9	-84.9	0.26	(1)	4.66
T-D	-5814 / 0	-84.9	-84.9	0.26	(1)	4.66
D-E	-5813 / 0	-84.9	-84.9	0.18	(1)	4.74
E-U	-5813 / 0	-84.9	-84.9	0.18	(1)	4.74
U-F	-5813 / 0	-84.9	-84.9	0.18	(1)	4.74
F-V	-5830 / 0	-84.9	-84.9	0.19	(1)	4.74
V-W	-5830 / 0	-84.9	-84.9	0.19	(1)	4.74
W-G	-5830 / 0	-84.9	-84.9	0.19	(1)	4.74
G-X	-5830 / 0	-84.9	-84.9	0.26	(1)	4.67
X-Y	-5830 / 0	-84.9	-84.9	0.26	(1)	4.67
Y-Z	-5830 / 0	-84.9	-84.9	0.26	(1)	4.67
Z-H	-5830 / 0	-84.9	-84.9	0.26	(1)	4.67
H-I	-4203 / 0	-84.9	-84.9	0.43	(1)	4.27
I-J	0 / 26	-84.9	-84.9	0.06	(1)	10.00
Q-B	-2732 / 0	0.0	0.0	0.10	(1)	7.81
K-I	-2733 / 0	0.0	0.0	0.10	(1)	7.81
Q-AA	0 / 0	-18.5	-18.5	0.06	(4)	10.00
AA-AB	0 / 0	-18.5	-18.5	0.06	(4)	10.00
AB-P	0 / 0	-18.5	-18.5	0.06	(4)	10.00
P-AC	0 / 3775	-18.5	-18.5	0.29	(1)	10.00
AC-AD	0 / 3775	-18.5	-18.5	0.29	(1)	10.00
AD-AE	0 / 3775	-18.5	-18.5	0.29	(1)	10.00
AE-O	0 / 3775	-18.5	-18.5	0.29	(1)	10.00
O-AF	0 / 5919	-18.5	-18.5	0.45	(1)	10.00
AF-N	0 / 5919	-18.5	-18.5	0.45	(1)	10.00
N-AG	0 / 5919	-18.5	-18.5	0.45	(1)	10.00
AG-M	0 / 5919	-18.5	-18.5	0.45	(1)	10.00
M-AH	0 / 3776	-18.5	-18.5	0.29	(1)	10.00
AH-AI	0 / 3776	-18.5	-18.5	0.29	(1)	10.00
AI-AJ	0 / 3776	-18.5	-18.5	0.29	(1)	10.00
AJ-L	0 / 3776	-18.5	-18.5	0.29	(1)	10.00
L-AK	0 / 0	-18.5	-18.5	0.06	(4)	10.00
AK-AL	0 / 0	-18.5	-18.5	0.06	(4)	10.00
AL-K	0 / 0	-18.5	-18.5	0.06	(4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
P-C	-290 / 57	0.04	(1)
L-H	-292 / 54	0.04	(1)
B-P	0 / 3801	0.34	(1)
L-I	0 / 3801	0.34	(1)
M-H	0 / 2371	0.29	(1)
C-O	0 / 2352	0.29	(1)
M-G	-378 / 0	0.11	(1)
O-D	-856 / 0	0.10	(1)
O-F	-160 / 0	0.03	(1)
F-M	-135 / 0	0.03	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.43/1.00 (H-I:1) , BC=0.45/1.00 (M-O:1) , WB=0.34/1.00 (I-L:1) , SSI=0.17/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS SEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.58 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425212	T1	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.25
C	TTWW+m	MT20	5.0	6.0	2.50	2.50
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.50	2.00
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TTWW+m	MT20	5.0	6.0	2.50	2.50
I	TMVW-t	MT20	5.0	6.0	2.25	2.25
K	BMV1+p	MT20	3.0	8.0	4.25	Edge
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	8.0	4.25	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

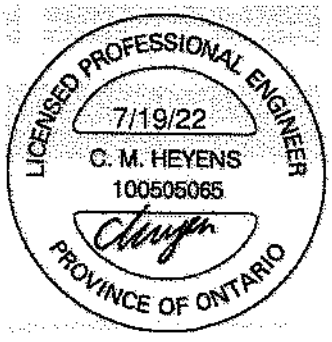
SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-295	-295	--	FRONT	VERT	TOTAL	--	C1
H	25-1-8	-295	-295	--	FRONT	VERT	TOTAL	--	C1
L	25-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
N	15-11-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
P	5-11-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
R	7-11-4	-71	-71	--	FRONT	VERT	TOTAL	--	C1
S	9-6-12	-71	-71	--	FRONT	VERT	TOTAL	--	C1
T	11-11-4	-71	-71	--	FRONT	VERT	TOTAL	--	C1
U	13-11-4	-71	-71	--	FRONT	VERT	TOTAL	--	C1
V	15-11-4	-71	-71	--	FRONT	VERT	TOTAL	--	C1
W	17-0-12	-71	-71	--	FRONT	VERT	TOTAL	--	C1
X	19-0-12	-71	-71	--	FRONT	VERT	TOTAL	--	C1
Y	21-0-12	-71	-71	--	FRONT	VERT	TOTAL	--	C1
Z	23-0-12	-71	-71	--	FRONT	VERT	TOTAL	--	C1
AA	1-11-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AB	3-11-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AC	7-11-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AD	9-6-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AE	11-11-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AF	13-11-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AG	17-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AH	19-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AI	21-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AJ	23-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AK	27-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AL	29-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

STRUCTURAL COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425213	T1S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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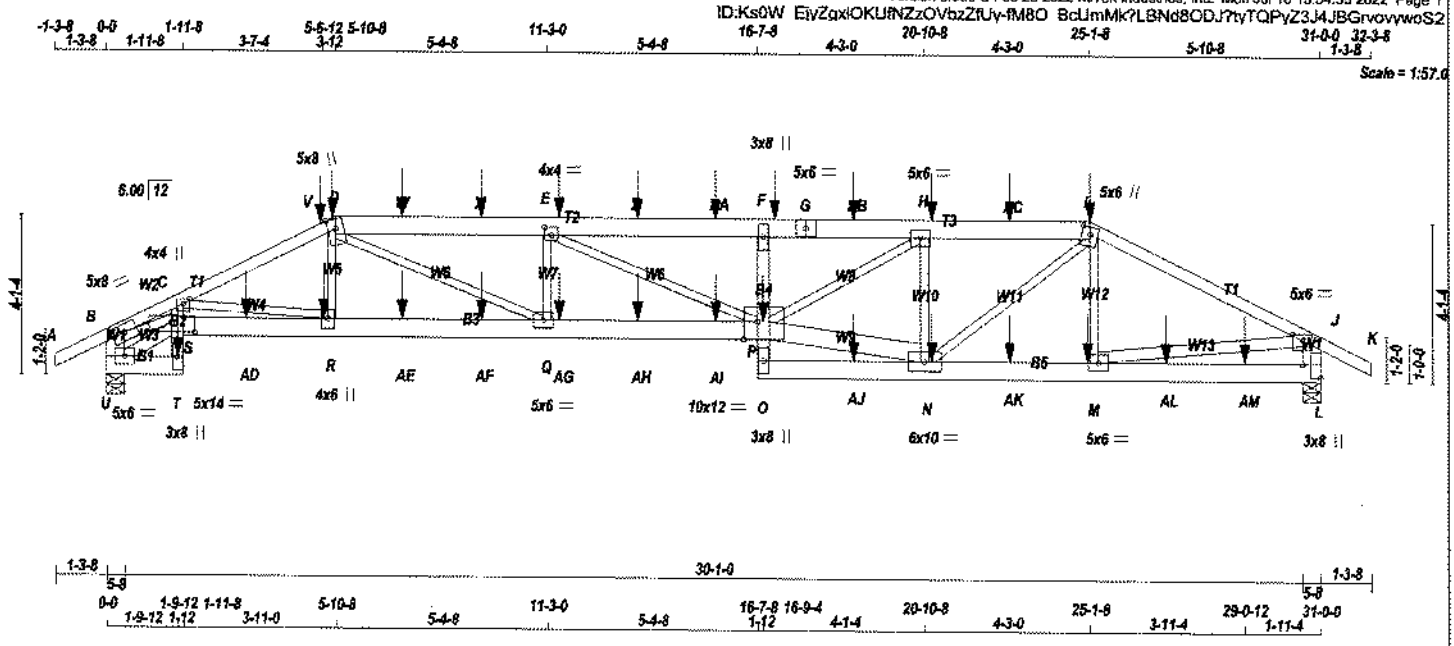

TOTAL WEIGHT = 2 X 161 = 322 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - T	2x6	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	1	12	SIDE(61.0)
I-K	1	12	SIDE(61.0)
D-G	2	12	SIDE(183.1)
G-I	2	12	SIDE(183.1)
U-B	2	12	TOP
L-J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U-T	2	12	SIDE(122.0)
S-P	2	12	SIDE(183.1)
O-L	2	12	SIDE(183.1)
T-C	1	12	TOP
F-O	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

STRUCTURAL COMPONENT ONLY

DWG # TR22070179 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2954	0	2954	0	0	5-8	5-8
U	3429	0	3429	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2095	1337 / 0	0 / 0	0 / 0	0 / 0	759 / 0	0 / 0
U	2427	1580 / 0	0 / 0	0 / 0	0 / 0	847 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	1	0.08 (1)	10.00
B-C	-8097 / 0	-84.9	-84.9	1	0.36 (1)	3.16
C-V	-6783 / 0	-84.9	-84.9	1	0.44 (1)	3.41
V-D	-6783 / 0	-84.9	-84.9	1	0.44 (1)	3.41
D-W	-8761 / 0	-84.9	-84.9	1	0.27 (1)	3.93
W-X	-8761 / 0	-84.9	-84.9	1	0.27 (1)	3.93
X-E	-8761 / 0	-84.9	-84.9	1	0.27 (1)	3.93
E-Y	-9049 / 0	-84.9	-84.9	1	0.29 (1)	3.86
Y-Z	-9049 / 0	-84.9	-84.9	1	0.29 (1)	3.86
Z-AA	-9049 / 0	-84.9	-84.9	1	0.29 (1)	3.86
AA-F	-9049 / 0	-84.9	-84.9	1	0.29 (1)	3.86
F-G	-8962 / 0	-84.9	-84.9	1	0.20 (1)	3.96
G-AB	-8962 / 0	-84.9	-84.9	1	0.20 (1)	3.96
AB-H	-8962 / 0	-84.9	-84.9	1	0.20 (1)	3.96
H-AC	-5739 / 0	-84.9	-84.9	1	0.14 (1)	4.83
AC-I	-5739 / 0	-84.9	-84.9	1	0.14 (1)	4.83
I-J	-4457 / 0	-84.9	-84.9	1	0.45 (1)	4.16
J-K	0 / 26	-84.9	-84.9	1	0.08 (1)	10.00
U-B	-3042 / 0	0.0	0.0	1	0.11 (1)	7.81
L-J	-2876 / 0	0.0	0.0	1	0.10 (1)	7.81
U-T	0 / 671	-18.5	-18.5	1	0.05 (1)	10.00
T-S	0 / 291	0.0	0.0	1	0.53 (1)	10.00
S-C	0 / 871	0.0	0.0	1	0.58 (1)	10.00
S-AD	0 / 8167	-18.5	-18.5	1	0.58 (1)	10.00
AD-R	0 / 8167	-18.5	-18.5	1	0.58 (1)	10.00
R-AE	0 / 6099	-18.5	-18.5	1	0.47 (1)	10.00
AE-AF	0 / 6099	-18.5	-18.5	1	0.47 (1)	10.00
AF-Q	0 / 6099	-18.5	-18.5	1	0.47 (1)	10.00
Q-AG	0 / 8761	-18.5	-18.5	1	0.67 (1)	10.00
AG-AH	0 / 8761	-18.5	-18.5	1	0.67 (1)	10.00
AH-AI	0 / 8761	-18.5	-18.5	1	0.67 (1)	10.00
AI-P	0 / 8761	-18.5	-18.5	1	0.67 (1)	10.00
O-P	0 / 48	0.0	0.0	1	0.18 (1)	10.00
P-F	-593 / 0	0.0	0.0	1	0.17 (1)	7.81
O-AJ	0 / 229	-18.5	-18.5	1	0.04 (4)	10.00
AJ-N	0 / 229	-18.5	-18.5	1	0.04 (4)	10.00
N-AK	0 / 4006	-18.5	-18.5	1	0.28 (1)	10.00
AK-M	0 / 4006	-18.5	-18.5	1	0.28 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-R	-2070 / 0	0.30 (1)
R-D	0 / 720	0.09 (1)
D-Q	0 / 2975	0.37 (1)
Q-E	-1002 / 0	0.09 (1)
E-P	0 / 319	0.04 (1)
P-N	0 / 5730	0.37 (1)
P-H	0 / 3760	0.47 (1)
N-H	-2722 / 0	0.33 (1)
N-I	0 / 2311	0.29 (1)
M-I	-380 / 3	0.05 (1)
M-J	0 / 4032	0.36 (1)
U-S	-766 / 0	0.04 (1)
B-S	0 / 7237	0.90 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(66 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 814 (0.46")

CSI: TC=0.45/1.00 (I-J:1) , BC=0.67/1.00 (P-Q:1) , WB=0.90/1.00 (B-S:1) , SSI=0.42/1.00 (C-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425213	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TTWW+m	MT20	5.0	8.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.50	2.00
F	TMV+p	MT20	3.0	8.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	5.0	6.0	2.25	2.50
J	TMVW-p	MT20	5.0	6.0	2.00	3.00
L	BMV1+p	MT20	3.0	8.0	4.25	Edge
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	6.0	10.0		
O	BMV+p	MT20	3.0	8.0		
P	BVMWWW-t	MT20	10.0	12.0	5.50	4.25
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW+t	MT20	4.0	6.0		
S	BVMWW-t	MT20	5.0	14.0	3.00	7.00
T	BMV+p	MT20	3.0	8.0		
U	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-AL	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
AL-AM	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
AM- L	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-58	-58	--	FRONT	VERT	DEAD	--	C1
D	5-10-8	-223	-223	--	FRONT	VERT	SNOW	--	C1
F	17-0-12	-71	-71	--	FRONT	VERT	TOTAL	--	C1
H	21-0-12	-71	-71	--	FRONT	VERT	TOTAL	--	C1
I	25-1-8	-295	-295	--	FRONT	VERT	TOTAL	--	C1
M	25-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
N	21-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
P	16-9-4	-26	-26	--	FRONT	VERT	TOTAL	--	C1
R	5-6-12	-67	-67	--	FRONT	VERT	TOTAL	--	C1
T	1-9-12	-194	-194	--	FRONT	VERT	TOTAL	--	C1
V	5-6-12	-87	-87	--	FRONT	VERT	TOTAL	--	C1
W	7-6-12	-66	-66	--	FRONT	VERT	TOTAL	--	C1
X	9-6-12	-66	-66	--	FRONT	VERT	TOTAL	--	C1
Y	11-6-12	-66	-66	--	FRONT	VERT	TOTAL	--	C1
Z	13-6-12	-66	-66	--	FRONT	VERT	TOTAL	--	C1
AA	15-6-12	-66	-66	--	FRONT	VERT	TOTAL	--	C1
AB	19-0-12	-71	-71	--	FRONT	VERT	TOTAL	--	C1
AC	23-0-12	-71	-71	--	FRONT	VERT	TOTAL	--	C1
AD	3-6-12	-67	-67	--	FRONT	VERT	TOTAL	--	C1
AE	7-6-12	-67	-67	--	FRONT	VERT	TOTAL	--	C1
AF	9-6-12	-67	-67	--	FRONT	VERT	TOTAL	--	C1
AG	11-6-12	-67	-67	--	FRONT	VERT	TOTAL	--	C1
AH	13-6-12	-67	-67	--	FRONT	VERT	TOTAL	--	C1
AI	15-6-12	-67	-67	--	FRONT	VERT	TOTAL	--	C1
AJ	19-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AK	23-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AL	27-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AM	29-0-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.73 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

DWG # TR22070179 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T2	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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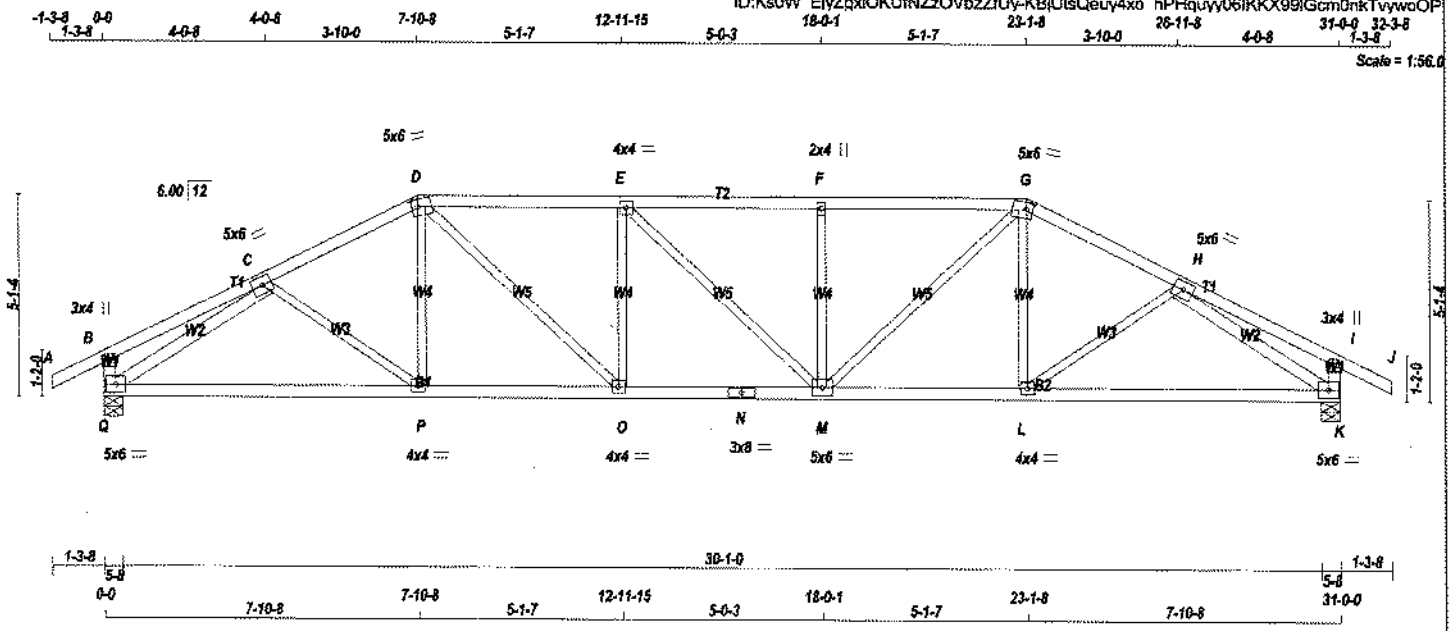


TOTAL WEIGHT = 127 lb (MiTek)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

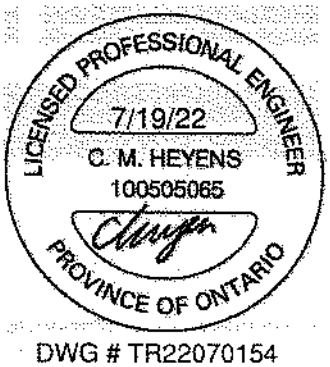
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	1.75
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L, O, P						
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	8.0		
Q	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY



DWG # TR22070154

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1717	0	1717	0	0	5-8	5-8
K	1717	0	1717	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	C-P	0 / 69	0.02 (4)
B-C	0 / 15	-84.9	-84.9	0.19 (1)	10.00	P-D	0 / 138	0.04 (4)
C-D	-2229 / 0	-84.9	-84.9	0.20 (1)	4.41	D-O	0 / 793	0.18 (1)
D-E	-2559 / 0	-84.9	-84.9	0.34 (1)	4.03	O-E	-465 / 0	0.18 (1)
E-F	-2557 / 0	-84.9	-84.9	0.31 (1)	4.06	E-M	-2 / 0	0.00 (1)
F-G	-2557 / 0	-84.9	-84.9	0.34 (1)	4.03	M-F	-465 / 0	0.18 (1)
G-H	-2229 / 0	-84.9	-84.9	0.20 (1)	4.41	M-G	0 / 796	0.18 (1)
H-I	0 / 15	-84.9	-84.9	0.19 (1)	10.00	L-G	0 / 139	0.04 (4)
I-J	0 / 26	-84.9	-84.9	0.11 (1)	10.00	L-H	0 / 89	0.02 (4)
Q-B	-249 / 0	0.0	0.0	0.03 (1)	7.81	Q-C	-2415 / 0	0.67 (1)
K-I	-249 / 0	0.0	0.0	0.03 (1)	7.81	H-K	-2415 / 0	0.67 (1)
Q-P	0 / 1963	-18.5	-18.5	0.46 (1)	10.00			
P-O	0 / 1981	-18.5	-18.5	0.45 (1)	10.00			
O-N	0 / 2559	-18.5	-18.5	0.47 (1)	10.00			
N-M	0 / 2559	-18.5	-18.5	0.47 (1)	10.00			
M-L	0 / 1981	-18.5	-18.5	0.45 (1)	10.00			
L-K	0 / 1963	-18.5	-18.5	0.45 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.34/1.00 (F-G:1) , BC=0.47/1.00 (M-O:1) , WB=0.67/1.00 (H-K:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.83 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425213	T2S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamatack Roof Truss, Burlington

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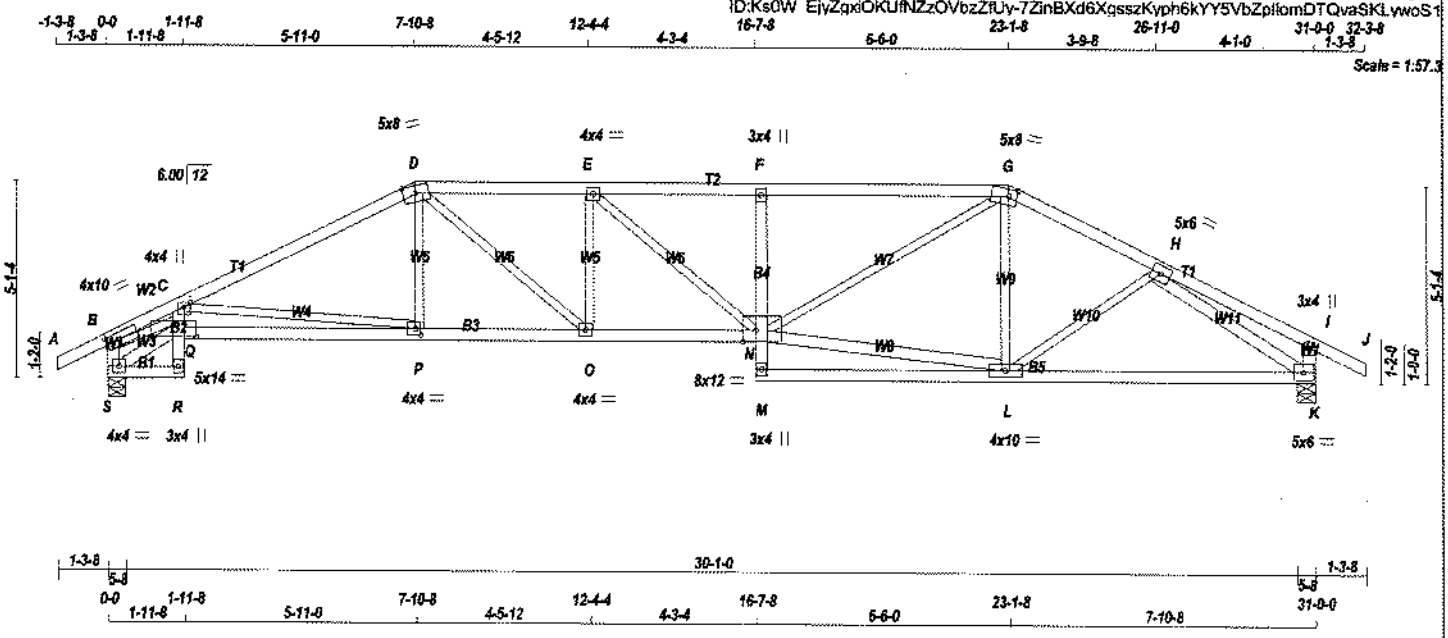

TOTAL WEIGHT = 133 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	10.0	2.00	4.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.25	2.50
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	10.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWWW-l	MT20	8.0	12.0	3.50	4.00
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BVMWW-l	MT20	5.0	14.0	3.50	7.25
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) STRUCTURAL COMPONENT ONLY

DWG # TR22070180

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	1720	0	1720	0	0	5-8	5-8
K	1715	0	1715	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1219	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1215	784 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	-3701 / 0	-84.9	-84.9	0.29 (1)	3.44
C-D	-2772 / 0	-84.9	-84.9	0.61 (1)	3.59
D-E	-3143 / 0	-84.9	-84.9	0.33 (1)	3.69
E-F	-3300 / 0	-84.9	-84.9	0.44 (1)	3.48
F-G	-3274 / 0	-84.9	-84.9	0.61 (1)	3.33
G-H	-2229 / 0	-84.9	-84.9	0.21 (1)	4.41
H-I	0 / 15	-84.9	-84.9	0.19 (1)	10.00
I-J	0 / 26	-84.9	-84.9	0.11 (1)	10.00
S-B	-1558 / 0	0.0	0.0	0.16 (1)	6.60
K-I	-251 / 0	0.0	0.0	0.03 (1)	7.81
S-R	0 / 262	-18.5	-18.5	0.05 (1)	10.00
R-Q	0 / 19	0.0	0.0	0.36 (1)	10.00
Q-C	0 / 180	0.0	0.0	0.39 (1)	10.00
Q-P	0 / 3667	-18.5	-18.5	0.67 (1)	10.00
P-O	0 / 2476	-18.5	-18.5	0.48 (1)	10.00
O-N	0 / 3144	-18.5	-18.5	0.56 (1)	10.00
M-N	0 / 45	0.0	0.0	0.14 (1)	10.00
N-F	-535 / 0	0.0	0.0	0.14 (1)	7.81
M-L	0 / 98	-18.5	-18.5	0.29 (4)	10.00
L-K	0 / 1967	-18.5	-18.5	0.47 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-P	-1208 / 0	0.84 (1)
P-D	0 / 268	0.06 (4)
D-O	0 / 875	0.20 (1)
O-E	-495 / 0	0.13 (1)
E-N	0 / 206	0.05 (1)
N-L	0 / 1903	0.31 (1)
N-G	0 / 1511	0.34 (1)
L-G	-141 / 34	0.05 (1)
S-Q	-300 / 0	0.03 (1)
B-Q	0 / 3334	0.75 (1)
L-H	0 / 87	0.02 (4)
H-K	-2418 / 0	0.69 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 966 (0.39")

CSI: TC=0.61/1.00 (F-G:1) , BC=0.67/1.00 (P-Q:1) , WB=0.84/1.00 (C-P:1) , SSI=0.27/1.00 (C-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Q) (INPUT = 0.90)
JSI METAL= 0.62 (Q) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T3	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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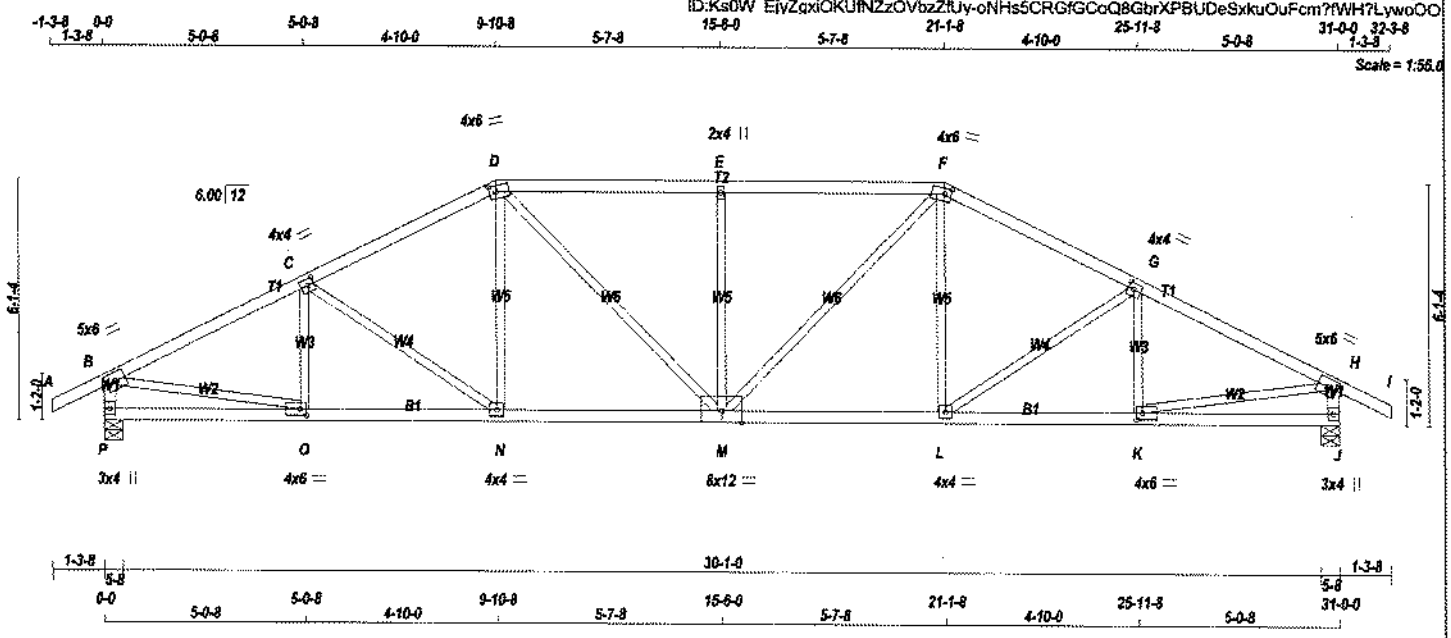

TOTAL WEIGHT = 127 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMWW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.75
L	BMWW-t	MT20	4.0	4.0		
M	BSWWW-t	MT20	8.0	12.0	Edge	5.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.75
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral bracas to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070155

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1717	0	1717	0	0	5-8	5-8
J	1717	0	1717	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B- C	-2301 / 0	-84.9	-84.9	0.33 (1)		4.22
C- D	-2106 / 0	-84.9	-84.9	0.31 (1)		4.40
D- E	-2191 / 0	-84.9	-84.9	0.41 (1)		4.21
E- F	-2191 / 0	-84.9	-84.9	0.41 (1)		4.21
F- G	-2106 / 0	-84.9	-84.9	0.31 (1)		4.40
G- H	-2301 / 0	-84.9	-84.9	0.33 (1)		4.22
H- I	0 / 26	-84.9	-84.9	0.11 (1)		10.00
P- B	-1674 / 0	0.0	0.0	0.17 (1)		6.42
J- H	-1674 / 0	0.0	0.0	0.17 (1)		6.42
P- O	0 / 0	-18.5	-18.5	0.10 (4)		10.00
O- N	0 / 2076	-18.5	-18.5	0.41 (1)		10.00
N- M	0 / 1868	-18.5	-18.5	0.37 (1)		10.00
M- L	0 / 1868	-18.5	-18.5	0.37 (1)		10.00
L- K	0 / 2076	-18.5	-18.5	0.41 (1)		10.00
K- J	0 / 0	-18.5	-18.5	0.10 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-265 / 0	0.06 (1)
C- N	-258 / 0	0.16 (1)
N- D	0 / 260	0.06 (4)
D- M	0 / 464	0.10 (1)
M- E	-584 / 0	0.34 (1)
M- F	0 / 464	0.10 (1)
L- F	0 / 260	0.06 (4)
L- G	-258 / 0	0.16 (1)
K- G	-265 / 0	0.06 (1)
B- O	0 / 2106	0.47 (1)
K- H	0 / 2106	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.41/1.00 (N-O:1) , WB=0.47/1.00 (B-O:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425213	T3S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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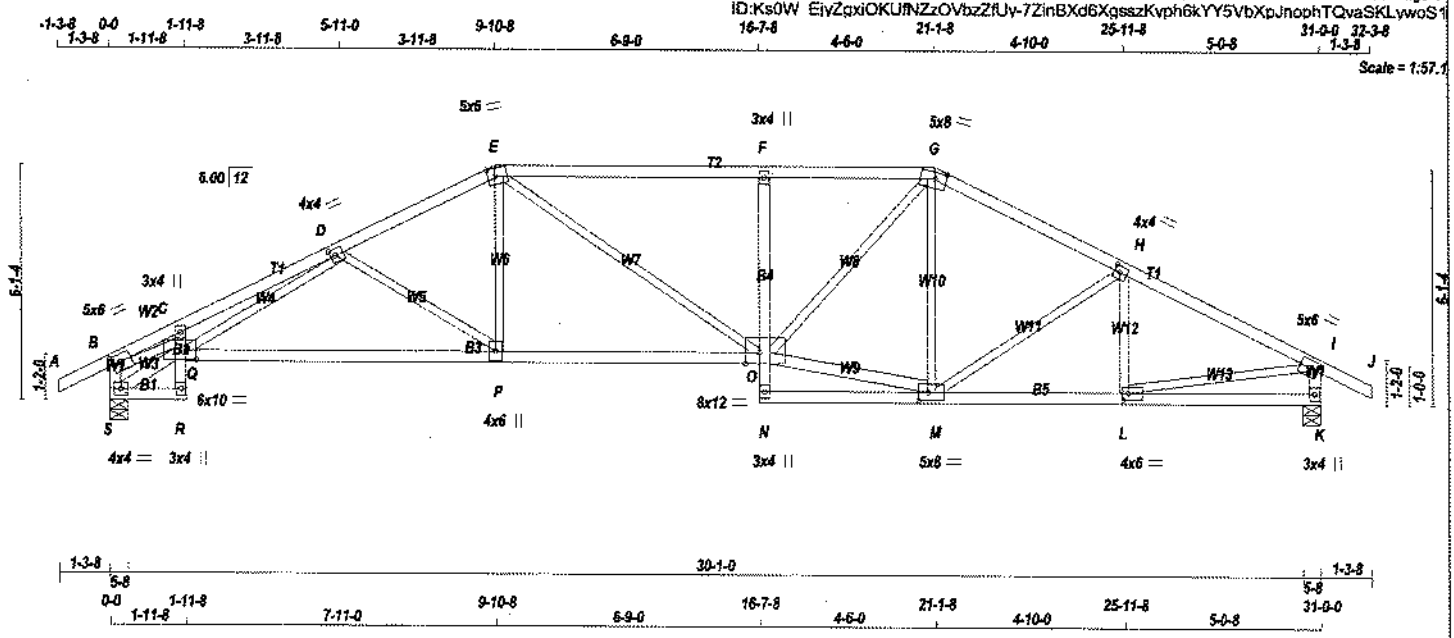

TOTAL WEIGHT = 133 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER		DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	5.0	6.0	2.50	2.25
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.00	3.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMVW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	1.75
M	BMWWW-t	MT20	5.0	8.0	2.50	3.00
N	BMV+p	MT20	3.0	4.0		
O	BVMWWW-t	MT20	8.0	12.0	3.50	4.00
P	BMWW+t	MT20	4.0	6.0		
Q	BVMWWW-t	MT20	6.0	10.0	3.00	3.00
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070181

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1715	0	1715	0	0	5-8	5-8
S	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1215	784 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
S	1219	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	
B-C	-3422 / 0	-84.9	-84.9	0.24 (1)	3.62	
C-D	-3664 / 0	-84.9	-84.9	0.40 (1)	3.36	
D-E	-2537 / 0	-84.9	-84.9	0.28 (1)	4.11	
E-F	-2631 / 0	-84.9	-84.9	0.62 (1)	3.66	
F-G	-2620 / 0	-84.9	-84.9	0.47 (1)	3.80	
G-H	-2103 / 0	-84.9	-84.9	0.31 (1)	4.40	
H-I	-2297 / 0	-84.9	-84.9	0.33 (1)	4.23	
I-J	0 / 26	-84.9	-84.9	0.11 (1)	10.00	
S-B	-1571 / 0	0.0	0.0	0.16 (1)	6.58	
K-I	-1672 / 0	0.0	0.0	0.17 (1)	6.42	
S-R	0 / 239	-18.5	-18.5	0.05 (1)	10.00	
R-Q	0 / 19	0.0	0.0	0.33 (1)	10.00	
Q-C	-189 / 0	0.0	0.0	0.25 (1)	7.81	
Q-P	0 / 2745	-18.5	-18.5	0.61 (1)	10.00	
P-O	0 / 2260	-18.5	-18.5	0.53 (1)	10.00	
N-O	0 / 35	0.0	0.0	0.08 (1)	10.00	
O-F	-606 / 0	0.0	0.0	0.16 (1)	7.81	
N-M	0 / 52	-18.5	-18.5	0.09 (4)	10.00	
M-L	0 / 2074	-18.5	-18.5	0.39 (1)	10.00	
L-K	0 / 0	-18.5	-18.5	0.10 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-P	-596 / 0	0.25 (1)
P-E	0 / 502	0.11 (1)
E-O	0 / 452	0.10 (1)
O-M	0 / 1856	0.30 (1)
O-G	0 / 1132	0.25 (1)
M-G	-168 / 2	0.10 (1)
M-H	-260 / 0	0.16 (1)
L-H	-262 / 0	0.06 (1)
L-I	0 / 2103	0.47 (1)
Q-D	0 / 704	0.16 (1)
S-Q	-273 / 0	0.03 (1)
B-Q	0 / 3026	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.62/1.00 (E-F:1) , BC=0.61/1.00 (P-Q:1) , WB=0.68/1.00 (B-Q:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.80 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T4	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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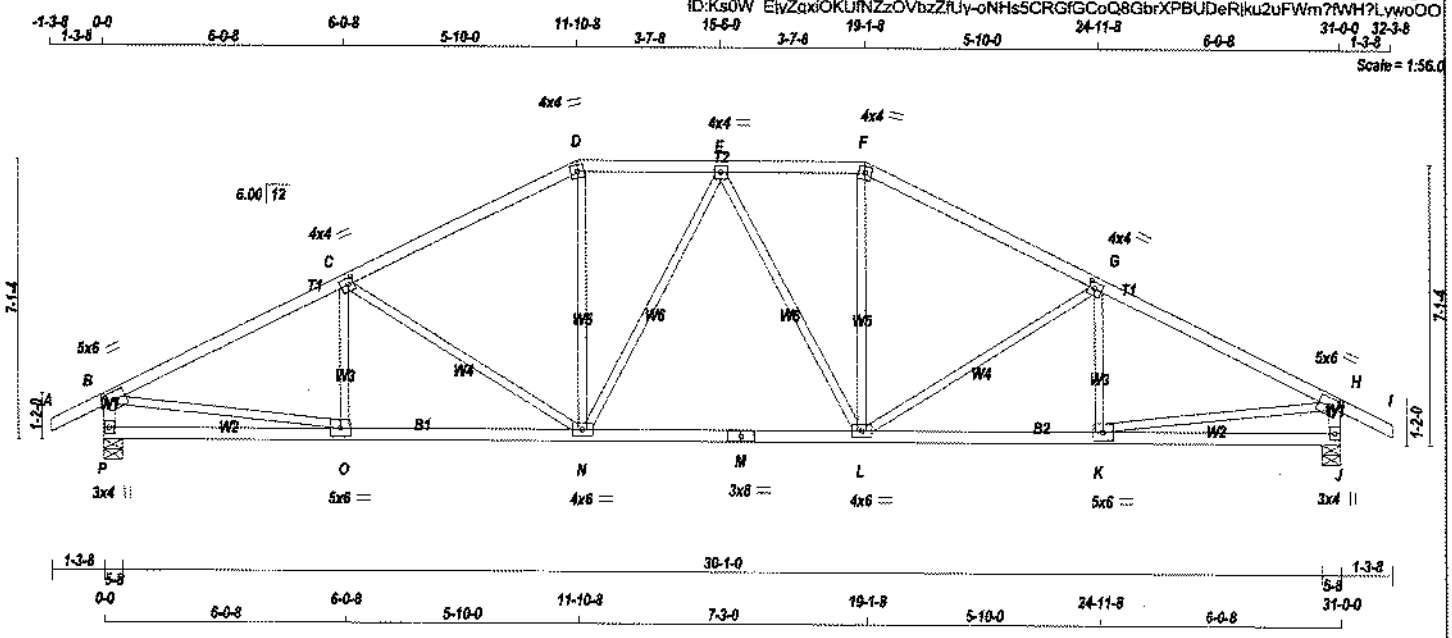


TOTAL WEIGHT = 128 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

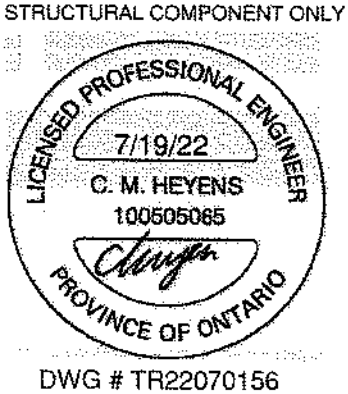
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY



DWG # TR22070156

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1717	0	1717	0	0	5-8	5-8
J	1717	0	1717	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	O- C	-186 / 25	0.05 (1)
B- C	-2334 / 0	-84.9	-84.9	0.49 (1)	4.03	C- N	-454 / 0	0.44 (1)
C- D	-1962 / 0	-84.9	-84.9	0.44 (1)	4.37	N- D	0 / 556	0.13 (1)
D- E	-1738 / 0	-84.9	-84.9	0.16 (1)	4.92	N- E	-206 / 0	0.25 (1)
E- F	-1738 / 0	-84.9	-84.9	0.16 (1)	4.92	E- L	-206 / 0	0.25 (1)
F- G	-1962 / 0	-84.9	-84.9	0.44 (1)	4.37	L- F	0 / 556	0.13 (1)
G- H	-2334 / 0	-84.9	-84.9	0.49 (1)	4.03	L- G	-454 / 0	0.44 (1)
H- I	0 / 26	-84.9	-84.9	0.11 (1)	10.00	K- G	-186 / 25	0.05 (1)
P- B	-1668 / 0	0.0	0.0	0.17 (1)	6.42	B- O	0 / 2132	0.48 (1)
J- H	-1668 / 0	0.0	0.0	0.17 (1)	6.42	K- H	0 / 2132	0.48 (1)
P- O	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
O- N	0 / 2111	-18.5	-18.5	0.43 (1)	10.00			
N- M	0 / 1833	-18.5	-18.5	0.38 (1)	10.00			
M- L	0 / 1833	-18.5	-18.5	0.38 (1)	10.00			
L- K	0 / 2111	-18.5	-18.5	0.43 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA O86-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.49/1.00 (B-C:1) , BC=0.43/1.00 (N-O:1) , WB=0.48/1.00 (B-O:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	660	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425213	T4S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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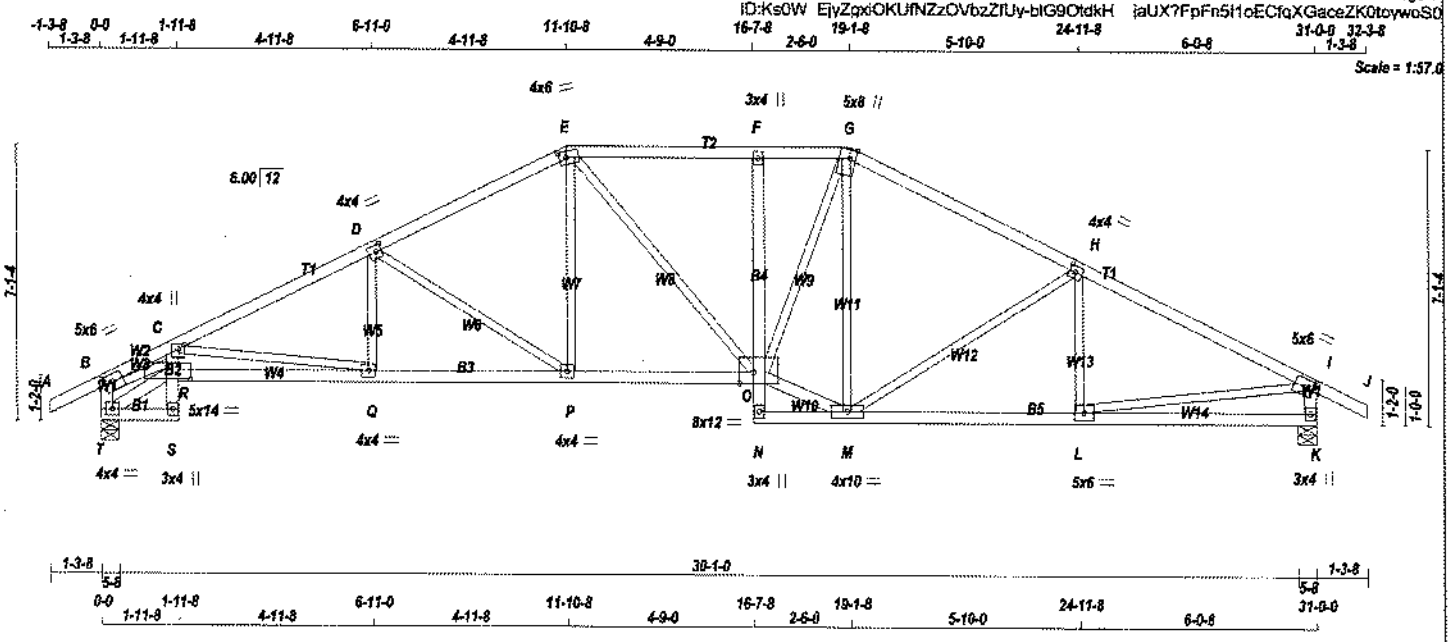

TOTAL WEIGHT = 139 lb

MiTek

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TMV+p	MT20	3.0	4.0		
G	TTWW+m	MT20	5.0	8.0	2.75	1.75
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMVW-t	MT20	5.0	6.0	2.50	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	4.0	10.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWWW-I	MT20	8.0	12.0	3.50	4.50
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BVMWW-I	MT20	5.0	14.0	3.50	7.25
S	BMV+p	MT20	3.0	4.0		
T	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) STRUCTURAL COMPONENT ONLY

DWG # TR22070182

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1715	0	1715	0	0	5-8	5-8
T	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1215	784 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
T	1219	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	
B-C	-3510 / 0	-84.9	-84.9	0.21 (1)	3.61	
C-D	-2952 / 0	-84.9	-84.9	0.41 (1)	3.72	
D-E	-2275 / 0	-84.9	-84.9	0.35 (1)	4.22	
E-F	-2144 / 0	-84.9	-84.9	0.30 (1)	4.38	
F-G	-2137 / 0	-84.9	-84.9	0.22 (1)	4.46	
G-H	-1956 / 0	-84.9	-84.9	0.45 (1)	4.37	
H-I	-2331 / 0	-84.9	-84.9	0.49 (1)	4.03	
I-J	0 / 26	-84.9	-84.9	0.11 (1)	10.00	
T-B	-1566 / 0	0.0	0.0	0.16 (1)	6.58	
K-I	-1666 / 0	0.0	0.0	0.17 (1)	6.42	
T-S	0 / 248	-18.5	-18.5	0.05 (1)	10.00	
S-R	0 / 19	0.0	0.0	0.34 (1)	10.00	
R-C	0 / 166	0.0	0.0	0.36 (1)	10.00	
R-Q	0 / 3439	-18.5	-18.5	0.62 (1)	10.00	
Q-P	0 / 2663	-18.5	-18.5	0.49 (1)	10.00	
P-O	0 / 2022	-18.5	-18.5	0.38 (1)	10.00	
N-O	-4 / 3	0.0	0.0	0.04 (1)	10.00	
O-F	-399 / 0	0.0	0.0	0.15 (1)	7.81	
N-M	0 / 38	-18.5	-18.5	0.10 (4)	10.00	
M-L	0 / 2108	-18.5	-18.5	0.41 (1)	10.00	
L-K	0 / 0	-18.5	-18.5	0.15 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-Q	-785 / 0	0.35 (1)
Q-D	0 / 212	0.05 (4)
D-P	-778 / 0	0.50 (1)
P-E	0 / 529	0.12 (1)
E-O	0 / 189	0.04 (1)
O-M	0 / 1839	0.30 (1)
O-G	0 / 1116	0.25 (1)
M-G	-394 / 0	0.34 (1)
M-H	-459 / 0	0.44 (1)
L-H	-183 / 32	0.05 (1)
L-I	0 / 2129	0.48 (1)
T-R	-283 / 0	0.03 (1)
B-R	0 / 3124	0.70 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L., PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.49/1.00 (H-I:1) , BC=0.62/1.00 (Q-R:1) , WB=0.70/1.00 (B-R:1) , SSI=0.25/1.00 (C-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.82 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T5	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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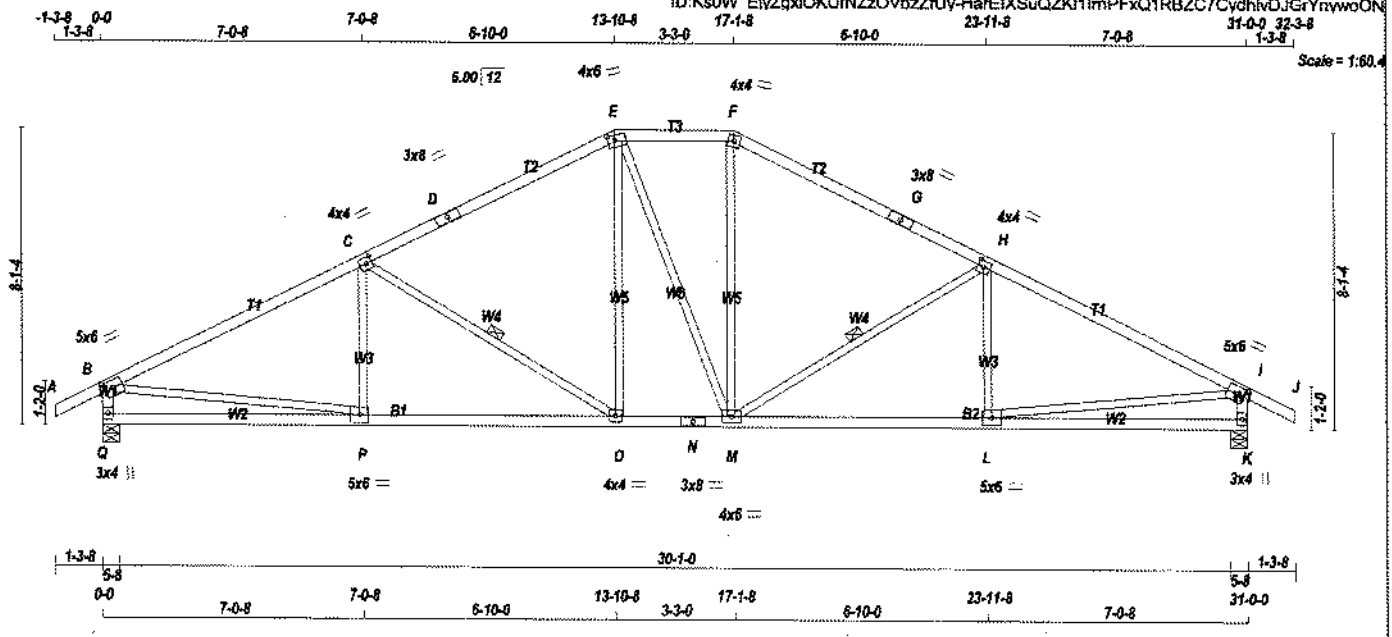


TOTAL WEIGHT = 129 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

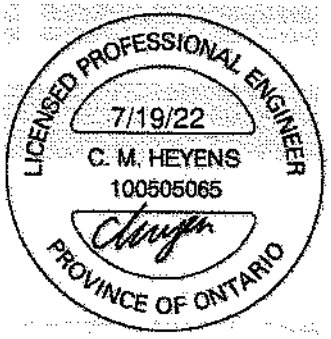
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	8.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY



DWG # TR22070157

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	1717	0	1717	0	0	5-8	5-8
K	1717	0	1717	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	P- C	-112 / 75	0.04 (1)	
B- C	-2341 / 0	-84.9	-84.9	0.70 (1)	3.75	C- O	-648 / 0	0.30 (1)	
C- D	-1792 / 0	-84.9	-84.9	0.61 (1)	4.28	O- E	0 / 430	0.10 (1)	
D- E	-1792 / 0	-84.9	-84.9	0.61 (1)	4.28	E- M	0 / 3	0.00 (1)	
E- F	-1580 / 0	-84.9	-84.9	0.14 (1)	5.13	M- F	0 / 433	0.10 (1)	
F- G	-1793 / 0	-84.9	-84.9	0.61 (1)	4.28	M- H	-647 / 0	0.30 (1)	
G- H	-1793 / 0	-84.9	-84.9	0.61 (1)	4.28	L- H	-114 / 74	0.04 (1)	
H- I	-2340 / 0	-84.9	-84.9	0.70 (1)	3.75	B- P	0 / 2137	0.48 (1)	
I- J	0 / 26	-84.9	-84.9	0.11 (1)	10.00	L- I	0 / 2137	0.48 (1)	
Q- B	-1663 / 0	0.0	0.0	0.17 (1)	6.43				
K- I	-1663 / 0	0.0	0.0	0.17 (1)	6.43				
Q- P	0 / 0	-18.5	-18.5	0.22 (4)	10.00				
P- O	0 / 2122	-18.5	-18.5	0.45 (1)	10.00				
O- N	0 / 1579	-18.5	-18.5	0.32 (1)	10.00				
N- M	0 / 1579	-18.5	-18.5	0.32 (1)	10.00				
M- L	0 / 2121	-18.5	-18.5	0.45 (1)	10.00				
L- K	0 / 0	-18.5	-18.5	0.22 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.22")

CSI: TC=0.70/1.00 (B-C:1) , BC=0.45/1.00 (O-P:1) , WB=0.48/1.00 (B-P:1) , SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.57 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425213	T5S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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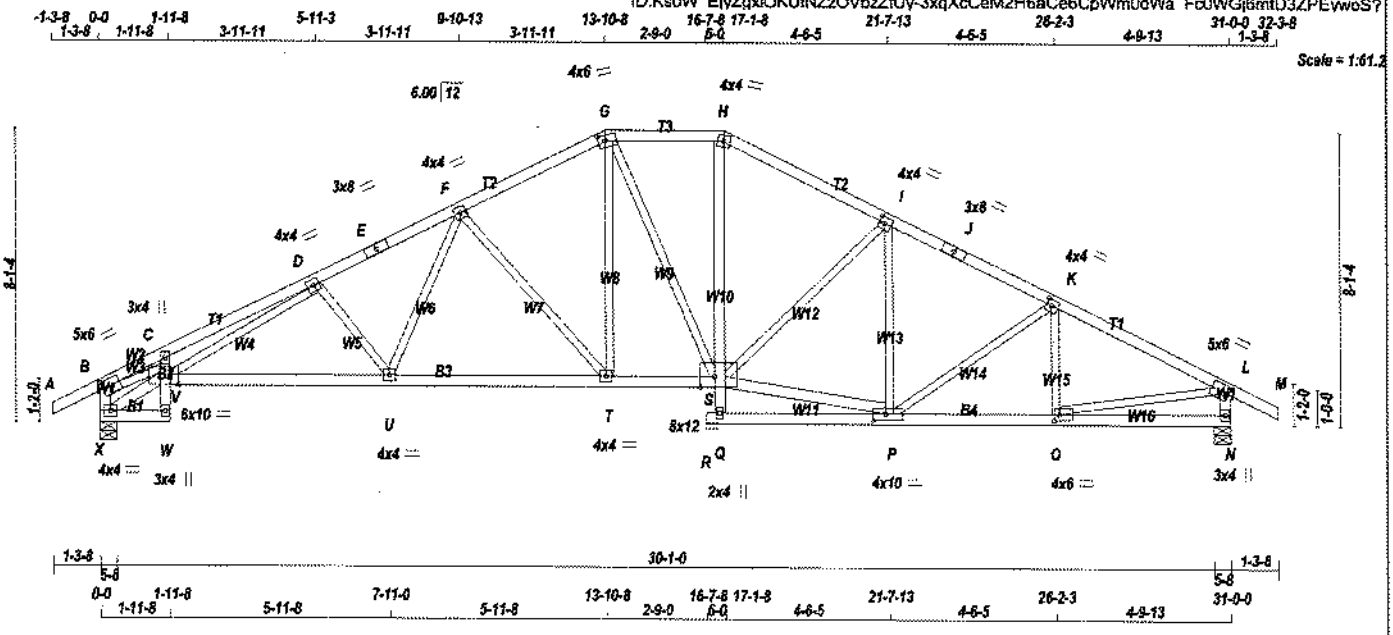

TOTAL WEIGHT = 145 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
X - W	2x4	DRY	No.2	SPF
W - C	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
X - V	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	4.0	6.0	1.75	2.25
H	TTW-m	MT20	4.0	4.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TS-t	MT20	3.0	8.0		
K	TMWW-t	MT20	4.0	4.0	2.00	1.75
L	TMVW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.75
P	BMWWW-t	MT20	4.0	10.0	2.00	4.25
Q	BMW+w	MT20	2.0	4.0		
S	BWMWWW-t	MT20	8.0	12.0	3.50	4.50
T	BMWW-t	MT20	4.0	4.0		
U	BMWW-t	MT20	4.0	4.0		
V	BVMWWW-t	MT20	6.0	10.0	3.25	2.75

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070183 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1718	0	1718	0	0	IN-SX	IN-SX
X	1723	0	1723	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1218	784 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
X	1221	786 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-3440 / 0	-84.9	-84.9	0.23 (1)	3.62
C- D	-3709 / 0	-84.9	-84.9	0.41 (1)	3.34
D- E	-2766 / 0	-84.9	-84.9	0.26 (1)	3.98
E- F	-2766 / 0	-84.9	-84.9	0.26 (1)	3.98
F- G	-2028 / 0	-84.9	-84.9	0.20 (1)	4.59
G- H	-1608 / 0	-84.9	-84.9	0.15 (1)	4.85
H- I	-2020 / 0	-84.9	-84.9	0.25 (1)	4.55
I- J	-2160 / 0	-84.9	-84.9	0.24 (1)	4.43
J- K	-2160 / 0	-84.9	-84.9	0.24 (1)	4.43
K- L	-2280 / 0	-84.9	-84.9	0.30 (1)	4.27
L- M	0 / 26	-84.9	-84.9	0.11 (1)	10.00
X- B	-1573 / 0	0.0	0.0	0.16 (1)	6.58
N- L	-1677 / 0	0.0	0.0	0.17 (1)	6.41
X- W	0 / 240	-18.5	-18.5	0.05 (1)	10.00
W- V	0 / 19	0.0	0.0	0.33 (1)	10.00
V- C	-205 / 0	0.0	0.0	0.25 (1)	7.81
V- U	0 / 2760	-18.5	-18.5	0.62 (1)	10.00
U- T	0 / 2260	-18.5	-18.5	0.44 (1)	10.00
T- S	0 / 1810	-18.5	-18.5	0.34 (1)	10.00
R- Q	0 / 0	-18.5	-18.5	0.00 (4)	10.00
Q- P	0 / 31	-18.5	-18.5	0.09 (4)	10.00
P- O	0 / 2053	-18.5	-18.5	0.38 (1)	10.00
O- N	0 / 0	-18.5	-18.5	0.00 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	0 / 633	0.14 (1)
G- S	-10 / 0	0.01 (4)
X- V	-274 / 0	0.03 (1)
B- V	0 / 3044	0.69 (1)
Q- S	0 / 48	0.05 (1)
S- H	0 / 605	0.14 (1)
S- I	-261 / 0	0.15 (1)
P- I	-213 / 0	0.11 (1)
P- K	-151 / 0	0.08 (1)
O- K	-252 / 0	0.06 (1)
O- L	0 / 2086	0.47 (1)
S- P	0 / 1946	0.31 (1)
V- D	0 / 708	0.16 (1)
D- U	-482 / 0	0.11 (1)
U- F	0 / 553	0.12 (1)
F- T	-710 / 0	0.51 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.41/1.00 (C-D:1) , BC=0.52/1.00 (U-V:1) , WB=0.69/1.00 (B-V:1) , SSI=0.26/1.00 (C-V:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (X) (INPUT = 0.90)
JSI METAL= 0.80 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425213	T5S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMV+p	MT20	3.0	4.0		
X	BMVW1-I	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070183 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T6	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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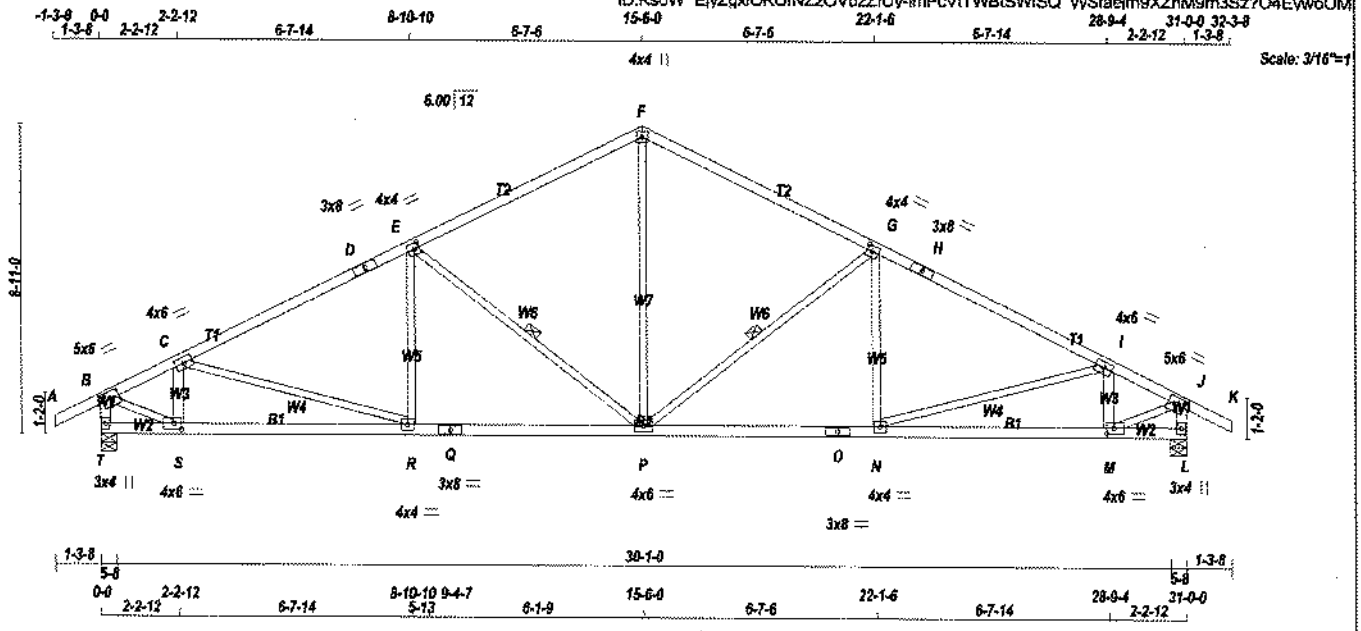

TOTAL WEIGHT = 3 X 126 = 379 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTW+p	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	4.0	6.0	2.00	2.50
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070158

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION				INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT		IN-SX	IN-SX
T	1717	0	1717	0	0		5-8	5-8
L	1717	0	1717	0	0		5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-P, E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00		P- F	0 / 1011	0.23 (1)
B- C	-1964 / 0	-84.9	-84.9	0.24 (1)	4.60		P- G	-710 / 0	0.35 (1)
C- D	-2227 / 0	-84.9	-84.9	0.56 (1)	4.03		N- G	0 / 121	0.04 (4)
D- E	-2227 / 0	-84.9	-84.9	0.56 (1)	4.03		B- S	0 / 1919	0.43 (1)
E- F	-1649 / 0	-84.9	-84.9	0.52 (1)	4.58		S- C	-599 / 0	0.07 (1)
F- G	-1649 / 0	-84.9	-84.9	0.52 (1)	4.58		C- R	0 / 223	0.05 (1)
G- H	-2227 / 0	-84.9	-84.9	0.56 (1)	4.03		R- E	0 / 121	0.04 (4)
H- I	-2227 / 0	-84.9	-84.9	0.56 (1)	4.03		E- P	-710 / 0	0.35 (1)
I- J	-1964 / 0	-84.9	-84.9	0.24 (1)	4.60		M- I	-599 / 0	0.07 (1)
J- K	0 / 26	-84.9	-84.9	0.11 (1)	10.00		M- J	0 / 1919	0.43 (1)
T- S	-1709 / 0	0.0	0.0	0.17 (1)	6.36		N- I	0 / 223	0.05 (1)
L- J	-1709 / 0	0.0	0.0	0.17 (1)	6.36				
T- S	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
S- R	0 / 1791	-18.5	-18.5	0.38 (1)	10.00				
R- Q	0 / 2004	-18.5	-18.5	0.41 (1)	10.00				
Q- P	0 / 2004	-18.5	-18.5	0.41 (1)	10.00				
P- O	0 / 2004	-18.5	-18.5	0.41 (1)	10.00				
O- N	0 / 2004	-18.5	-18.5	0.41 (1)	10.00				
N- M	0 / 1791	-18.5	-18.5	0.38 (1)	10.00				
M- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.56/1.00 (C-E:1) , BC=0.41/1.00 (P-R:1) , WB=0.43/1.00 (B-S:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.56 (Q) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425212	T6C	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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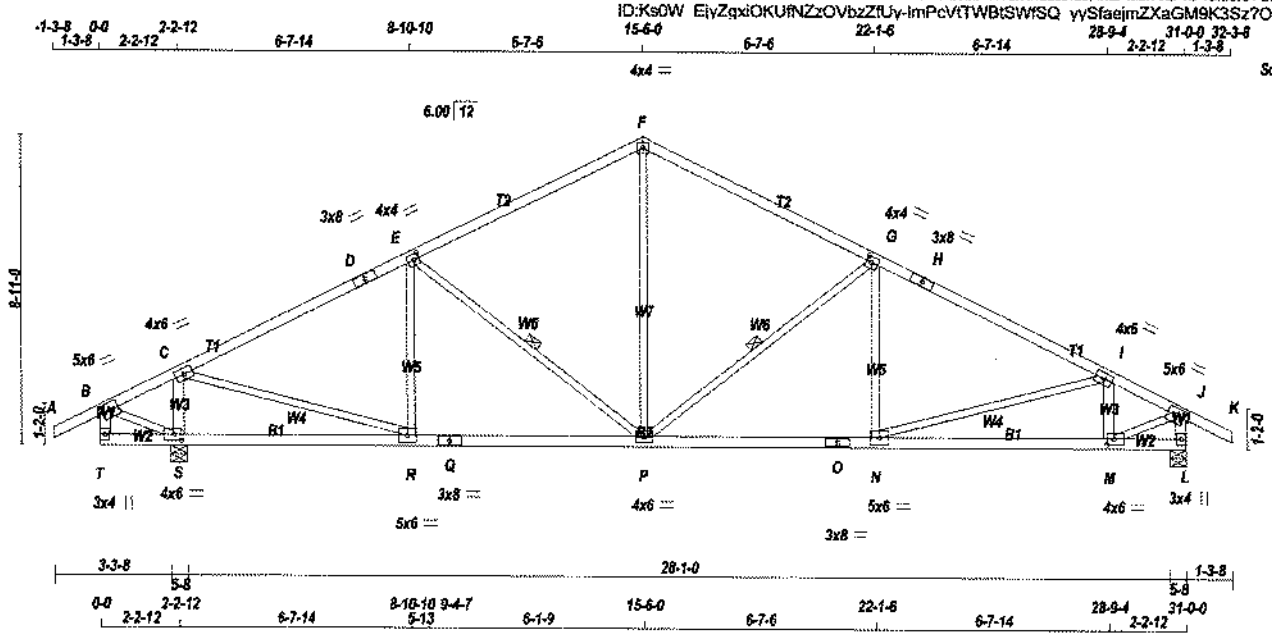


TOTAL WEIGHT = 4 X 126 = 505 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

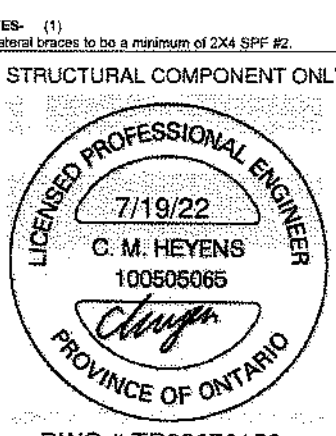
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTW-p	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMWW1-t	MT20	4.0	6.0	2.00	2.50
T	BMV+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY



DWG # TR22070159

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
S	1850	0	1850	0	0	IN-SX	IN-SX
L	1584	0	1584	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1311	846 / 0	0 / 0	0 / 0	0 / 0	465 / 0	0 / 0
L	1123	724 / 0	0 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-P, E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 157	-84.9	-84.9	0.34 (1)	10.00
C- D	-1604 / 0	-84.9	-84.9	0.48 (1)	4.69
D- E	-1604 / 0	-84.9	-84.9	0.48 (1)	4.69
E- F	-1380 / 0	-84.9	-84.9	0.49 (1)	4.94
F- G	-1381 / 0	-84.9	-84.9	0.50 (1)	4.93
G- H	-1977 / 0	-84.9	-84.9	0.53 (1)	4.26
H- I	-1977 / 0	-84.9	-84.9	0.53 (1)	4.26
I- J	-1797 / 0	-84.9	-84.9	0.23 (1)	4.77
J- K	0 / 26	-84.9	-84.9	0.11 (1)	10.00
T- B	-18 / 0	0.0	0.0	0.00 (1)	7.81
L- J	-1577 / 0	0.0	0.0	0.16 (1)	6.57
T- S	0 / 0	-18.5	-18.5	0.15 (4)	10.00
S- R	-98 / 0	-18.5	-18.5	0.12 (4)	6.25
R- Q	0 / 1445	-18.5	-18.5	0.33 (1)	10.00
Q- P	0 / 1445	-18.5	-18.5	0.33 (1)	10.00
P- O	0 / 1782	-18.5	-18.5	0.38 (1)	10.00
O- N	0 / 1782	-18.5	-18.5	0.38 (1)	10.00
N- M	0 / 1641	-18.5	-18.5	0.35 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	0 / 771	0.17 (1)
P- G	-732 / 0	0.36 (1)
N- G	0 / 128	0.04 (4)
B- S	-105 / 0	0.02 (1)
S- C	-1687 / 0	0.19 (1)
C- R	0 / 1609	0.36 (1)
R- E	-343 / 0	0.16 (1)
E- P	-300 / 0	0.15 (1)
M- I	-541 / 0	0.06 (1)
M- J	0 / 1759	0.40 (1)
N- I	0 / 146	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.53/1.00 (G-I:1) , BC=0.38/1.00 (N-P:1) , WB=0.40/1.00 (J-M:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	550	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.52 (Q) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T7	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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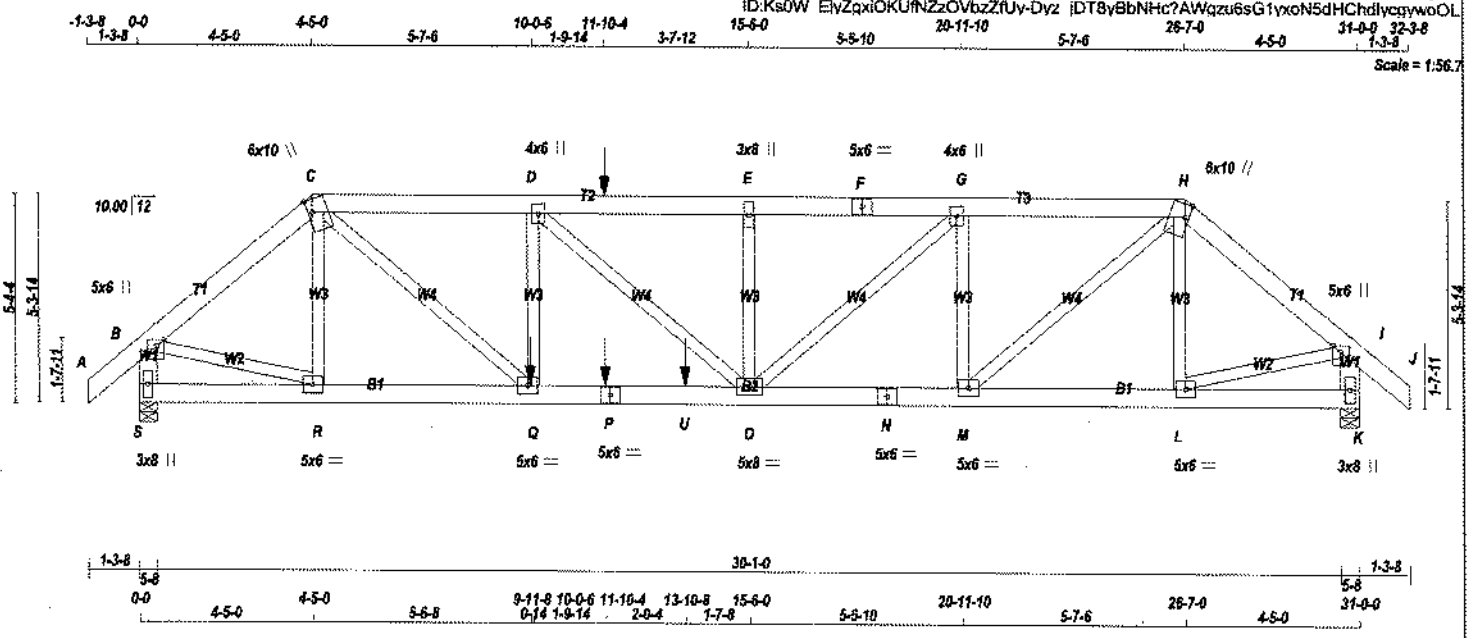


TOTAL WEIGHT = 2 X 192 = 384 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	2	12	TOP
C-F	2	12	SIDE(0.0)
F-H	2	12	TOP
H-J	2	12	TOP
S-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-P	2	12	SIDE(183.1)
P-N	2	12	SIDE(0.0)
N-K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
D-Q	1	6	SIDE(72.2)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070160 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3357	0	3357	0	0	5-8	5-8
K	2804	0	2804	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2373	1562 / 0	0 / 0	0 / 0	0 / 0	611 / 0	0 / 0
K	1984	1299 / 0	0 / 0	0 / 0	0 / 0	685 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 39	-84.9	-84.9	0.03 (1)	10.00	
B-C	-3608 / 0	-84.9	-84.9	0.10 (1)	5.82	
C-D	-5735 / 0	-84.9	-84.9	0.16 (1)	4.82	
D-T	-6156 / 0	-84.9	-84.9	0.17 (1)	4.66	
T-E	-6156 / 0	-84.9	-84.9	0.17 (1)	4.66	
E-F	-6156 / 0	-84.9	-84.9	0.15 (1)	4.69	
F-G	-6156 / 0	-84.9	-84.9	0.15 (1)	4.69	
G-H	-4473 / 0	-84.9	-84.9	0.12 (1)	5.34	
H-I	-2980 / 0	-84.9	-84.9	0.09 (1)	6.25	
I-J	0 / 39	-84.9	-84.9	0.03 (1)	10.00	
S-B	-3305 / 0	0.0	0.0	0.12 (1)	7.84	
K-I	-2784 / 0	0.0	0.0	0.10 (1)	7.81	
S-R	0 / 0	-18.5	-18.5	0.03 (1)	10.00	
R-Q	0 / 2741	-18.5	-18.5	0.32 (1)	10.00	
Q-P	0 / 5735	-18.5	-18.5	0.84 (1)	10.00	
P-U	0 / 5735	-18.5	-18.5	0.84 (1)	10.00	
U-O	0 / 5735	-18.5	-18.5	0.84 (1)	10.00	
O-N	0 / 4473	-18.5	-18.5	0.53 (1)	10.00	
N-M	0 / 4473	-18.5	-18.5	0.53 (1)	10.00	
M-L	0 / 2268	-18.5	-18.5	0.21 (1)	10.00	
L-K	0 / 0	-18.5	-18.5	0.04 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-670 / 0	0.09 (1)
C-Q	0 / 3950	0.35 (1)
Q-D	-984 / 0	0.13 (1)
D-O	0 / 564	0.05 (1)
O-E	-486 / 0	0.07 (1)
O-G	0 / 2253	0.20 (1)
M-G	-1993 / 0	0.27 (1)
M-H	0 / 2909	0.26 (1)
L-H	-393 / 0	0.05 (1)
B-R	0 / 2842	0.25 (1)
L-I	0 / 2347	0.21 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	11-10-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
Q	9-11-8	-688	-688	---	BACK	VERT	TOTAL	---	C1
T	11-10-4	-113	-113	---	BACK	VERT	TOTAL	---	C1
U	13-10-8	-1083	-1083	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.17/1.00 (D-E:1) , BC=0.84/1.00 (O-Q:1) , WB=0.35/1.00 (C-Q:1) , SSI=0.39/1.00 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.80)
JSI METAL= 0.73 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425212	T7	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

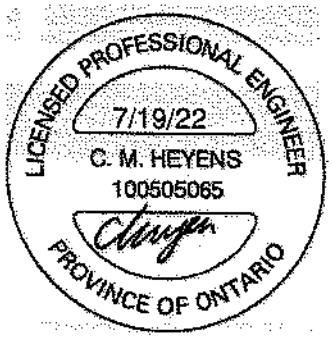
ID:Ks0W_EjyZqxiOKUfNZzOVbzZfUy-Dyz_iDT8yBbNHc?AWg2u6sG1yxoN5dHChdlycgywoOL

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	10.0	4.00	1.25
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	10.0	4.00	1.25
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	8.0		
L, M, Q, R						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY



DWG # TR22070160

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	T7Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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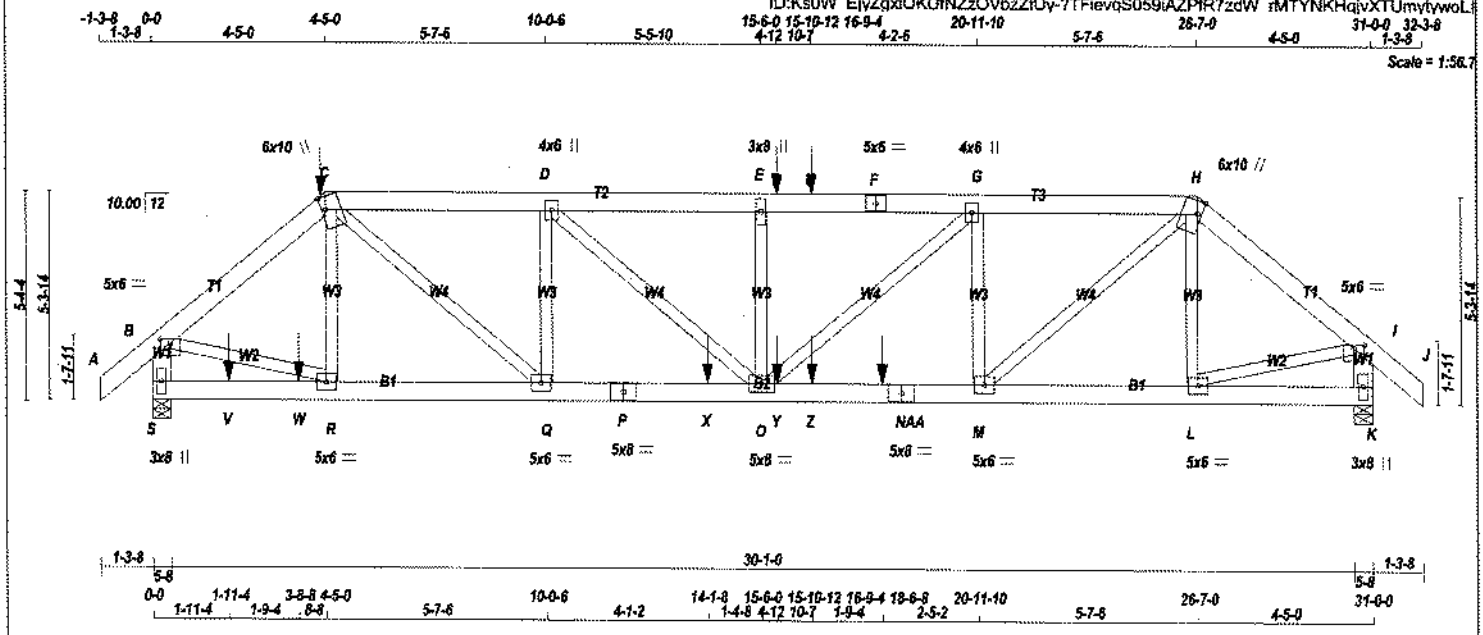


TOTAL WEIGHT = 2 X 192 = 384 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- F	2	12	SIDE(61.0)
F- H	2	12	TOP
H- J	2	12	TOP
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	SIDE(0.0)
P- N	2	12	SIDE(0.0)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070188 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	4166	0	4166	0	0	5-8	5-8
K	3279	0	3279	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2944	1944 / 0	0 / 0	0 / 0	0 / 0	999 / 0	0 / 0
K	2318	1525 / 0	0 / 0	0 / 0	0 / 0	793 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-84.9	-84.9	0.03 (1)	10.00	
B- C	-4416 / 0	-84.9	-84.9	0.11 (1)	5.38	
C- D	-5551 / 0	-84.9	-84.9	0.15 (1)	4.89	
D- E	-6919 / 0	-84.9	-84.9	0.16 (1)	4.46	
E- T	-6919 / 0	-84.9	-84.9	0.19 (1)	4.43	
T- U	-6919 / 0	-84.9	-84.9	0.19 (1)	4.43	
U- F	-6919 / 0	-84.9	-84.9	0.19 (1)	4.43	
F- G	-6919 / 0	-84.9	-84.9	0.19 (1)	4.43	
G- H	-5578 / 0	-84.9	-84.9	0.15 (1)	4.87	
H- I	-3513 / 0	-84.9	-84.9	0.10 (1)	5.88	
I- J	0 / 39	-84.9	-84.9	0.03 (1)	10.00	
S- B	-3976 / 0	0.0	0.0	0.14 (1)	7.13	
K- I	-3226 / 0	0.0	0.0	0.12 (1)	7.71	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	0 / 250	0.02 (4)
C- Q	0 / 2855	0.25 (1)
Q- D	-1697 / 0	0.23 (1)
D- O	0 / 1831	0.16 (1)
O- E	-719 / 0	0.10 (1)
O- G	0 / 1797	0.16 (1)
M- G	-1742 / 0	0.24 (1)
M- H	0 / 3836	0.34 (1)
L- H	-656 / 0	0.09 (1)
B- R	0 / 3478	0.31 (1)
L- I	0 / 2767	0.24 (1)

S- V	0 / 0	-18.5	-18.5	0.19 (1)	10.00
V- W	0 / 0	-18.5	-18.5	0.19 (1)	10.00
W- R	0 / 0	-18.5	-18.5	0.19 (1)	10.00
R- Q	0 / 3387	-18.5	-18.5	0.34 (1)	10.00
Q- P	0 / 5551	-18.5	-18.5	0.68 (1)	10.00
P- X	0 / 5551	-18.5	-18.5	0.68 (1)	10.00
X- O	0 / 5551	-18.5	-18.5	0.68 (1)	10.00
O- Y	0 / 5576	-18.5	-18.5	0.75 (1)	10.00
Y- Z	0 / 5576	-18.5	-18.5	0.75 (1)	10.00
Z-AA	0 / 5576	-18.5	-18.5	0.75 (1)	10.00
AA- N	0 / 5576	-18.5	-18.5	0.75 (1)	10.00
N- M	0 / 5576	-18.5	-18.5	0.75 (1)	10.00
M- L	0 / 2668	-18.5	-18.5	0.32 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.03 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-5-0	-44	-44	---	FRONT	VERT	DEAD	---	C1
C	4-5-0	-168	-168	---	FRONT	VERT	SNOW	---	C1
T	15-10-12	-113	-113	---	BACK	VERT	TOTAL	---	C1
U	16-9-4	-113	-113	---	BACK	VERT	TOTAL	---	C1
V	1-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
W	3-8-8	-736	-736	---	BACK	VERT	TOTAL	---	C1
X	14-1-8	-736	-736	---	BACK	VERT	TOTAL	---	C1
Y	15-10-12	-29	-29	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.19/1.00 (E-G:1) , BC=0.75/1.00 (M-O:1) , WB=0.34/1.00 (H-M:1) , SSI=0.33/1.00 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	T7Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:Ks0W_EjyZgxiOKUfNZzOVbzZfUy-7TFievqS059iAZPfR7zdW_rMTYNKHqjvXTUmytywoLt

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	6.0	10.0	4.00	1.25
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	10.0	4.00	1.25
I	TMVW-p	MT20	5.0	6.0	1.25	3.00
K	BMV1+p	MT20	3.0	8.0		
L, M, Q, R						
L	BMWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	8.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	8.0		
S	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

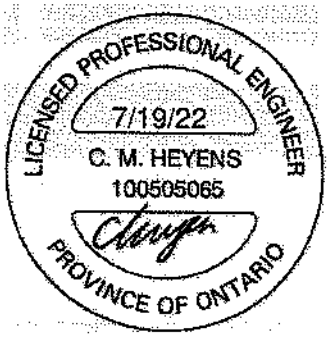
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Z	16-9-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
AA	18-6-8	-821	-821	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.82 (i) (INPUT = 0.90)
JSI METAL= 0.84 (P) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY



DWG # TR22070188 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T8	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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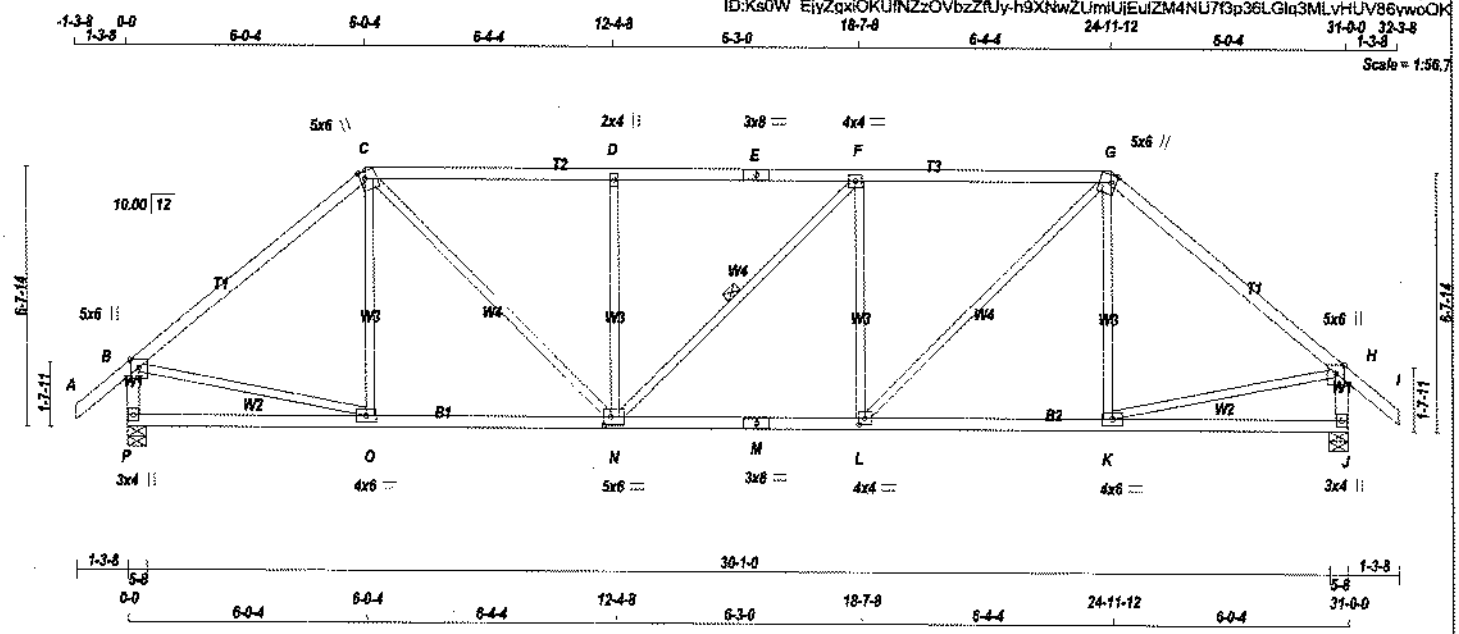

TOTAL WEIGHT = 2 X 132 = 264 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070161

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1720	0	1720	0	0	5-8	5-8
J	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	O-C	-134 / 47	0.10 (1)
B-C	-1617 / 0	-84.9	-84.9	0.72 (1)	4.36	C-N	0 / 978	0.22 (1)
C-D	-1930 / 0	-84.9	-84.9	0.59 (1)	4.19	N-D	-579 / 0	0.42 (1)
D-E	-1930 / 0	-84.9	-84.9	0.59 (1)	4.18	N-F	-2 / 0	0.00 (1)
E-F	-1930 / 0	-84.9	-84.9	0.59 (1)	4.18	L-F	-579 / 0	0.42 (1)
F-G	-1931 / 0	-84.9	-84.9	0.60 (1)	4.18	L-G	0 / 980	0.22 (1)
G-H	-1616 / 0	-84.9	-84.9	0.72 (1)	4.36	K-G	-135 / 47	0.10 (1)
H-I	0 / 38	-84.9	-84.9	0.12 (1)	10.00	B-O	0 / 1268	0.29 (1)
P-B	-1675 / 0	0.0	0.0	0.18 (1)	6.41	K-H	0 / 1267	0.29 (1)
J-H	-1675 / 0	0.0	0.0	0.18 (1)	6.41			
P-O	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
O-N	0 / 1237	-18.5	-18.5	0.30 (1)	10.00			
N-M	0 / 1931	-18.5	-18.5	0.38 (1)	10.00			
M-L	0 / 1931	-18.5	-18.5	0.38 (1)	10.00			
L-K	0 / 1237	-18.5	-18.5	0.30 (1)	10.00			
K-J	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.38/1.00 (L-N:1) , WB=0.42/1.00 (F-L:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T9	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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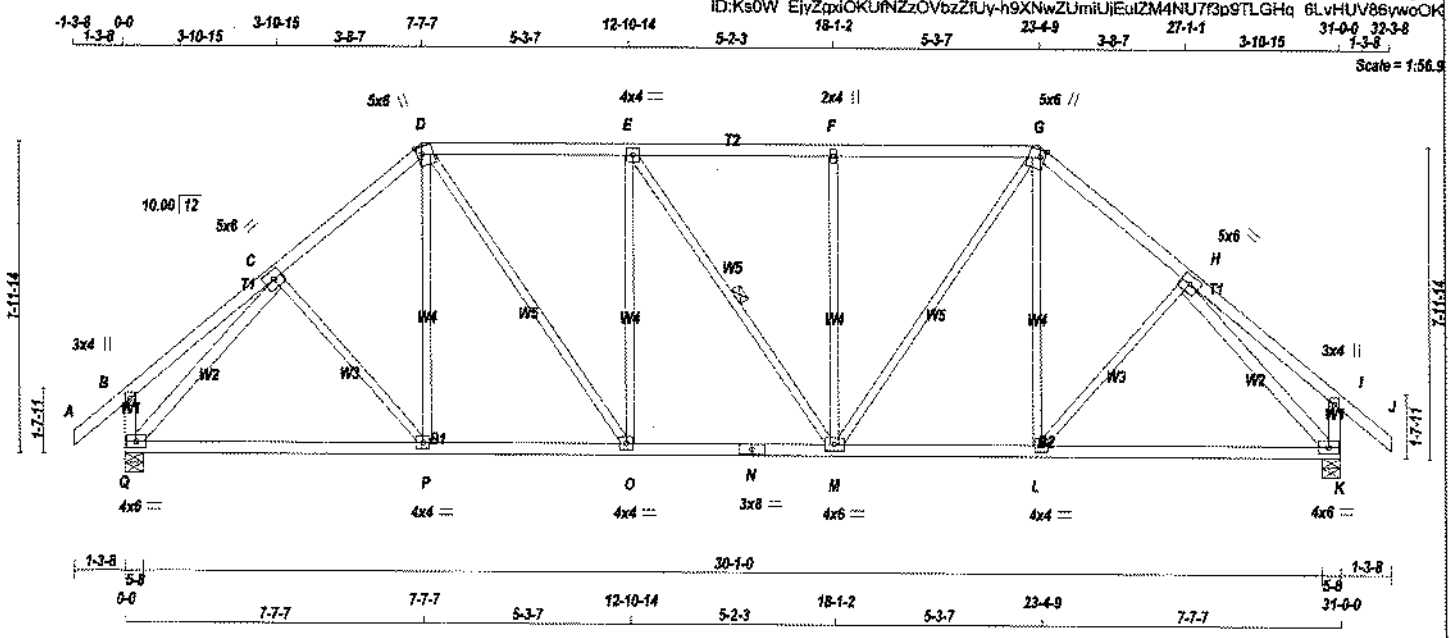


TOTAL WEIGHT = 2 X 151 = 301 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

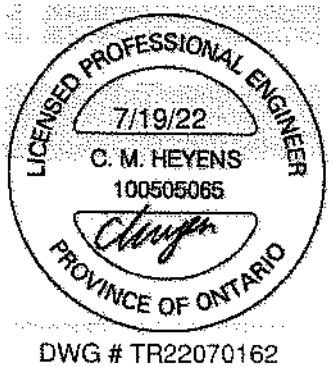
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L, O, P						
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
Q	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY



DWG # TR22070162

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1720	0	1720	0	0	5-8	5-8
K	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC†	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	
B- C	0 / 23	-84.9	-84.9	0.19 (1)	10.00	
C- D	-1604 / 0	-84.9	-84.9	0.24 (1)	4.98	
D- E	-1601 / 0	-84.9	-84.9	0.38 (1)	4.80	
E- F	-1599 / 0	-84.9	-84.9	0.37 (1)	4.80	
F- G	-1599 / 0	-84.9	-84.9	0.37 (1)	4.81	
G- H	-1604 / 0	-84.9	-84.9	0.24 (1)	4.98	
H- I	0 / 23	-84.9	-84.9	0.19 (1)	10.00	
I- J	0 / 38	-84.9	-84.9	0.12 (1)	10.00	
Q- B	-241 / 0	0.0	0.0	0.03 (1)	7.81	
K- I	-241 / 0	0.0	0.0	0.03 (1)	7.81	
Q- P	0 / 1204	-18.5	-18.5	0.34 (4)	10.00	
P- O	0 / 1213	-18.5	-18.5	0.34 (4)	10.00	
O- N	0 / 1601	-18.5	-18.5	0.31 (1)	10.00	
N- M	0 / 1601	-18.5	-18.5	0.31 (1)	10.00	
M- L	0 / 1213	-18.5	-18.5	0.35 (4)	10.00	
L- K	0 / 1204	-18.5	-18.5	0.34 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 49	0.02 (4)
P- D	0 / 138	0.04 (4)
D- O	0 / 684	0.15 (1)
O- E	-480 / 0	0.58 (1)
E- M	-3 / 0	0.00 (1)
M- F	-479 / 0	0.58 (1)
M- G	0 / 681	0.15 (1)
L- G	0 / 139	0.04 (4)
L- H	0 / 49	0.02 (4)
Q- C	-1861 / 0	0.76 (1)
H- K	-1862 / 0	0.76 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.35/1.00 (L-M:4) , WB=0.76/1.00 (H-K:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T10	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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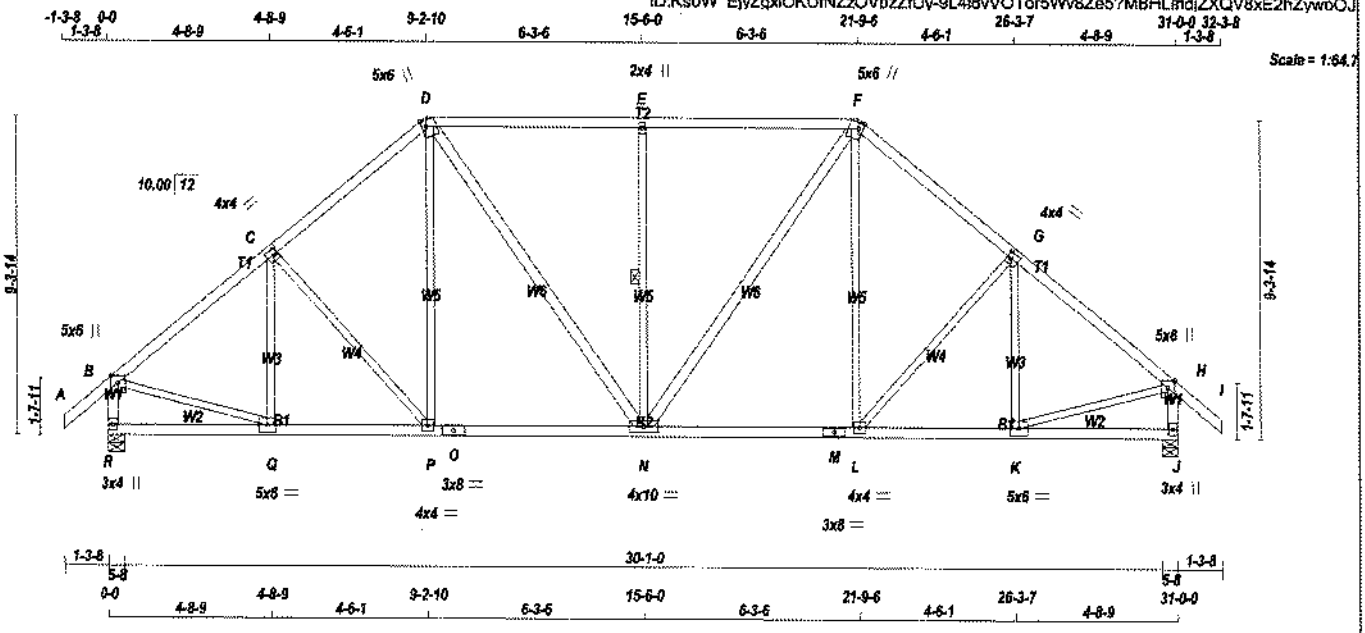

TOTAL WEIGHT = 2 X 156 = 312 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	10.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070163

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1720	0	1720	0	0	5-8	5-8
J	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	
B-C	-1624 / 0	-84.9	-84.9	0.27 (1)	4.94	
C-D	-1537 / 0	-84.9	-84.9	0.26 (1)	5.05	
D-E	-1426 / 0	-84.9	-84.9	0.46 (1)	4.90	
E-F	-1426 / 0	-84.9	-84.9	0.46 (1)	4.90	
F-G	-1537 / 0	-84.9	-84.9	0.26 (1)	5.05	
G-H	-1624 / 0	-84.9	-84.9	0.27 (1)	4.94	
H-I	0 / 38	-84.9	-84.9	0.12 (1)	10.00	
R-B	-1682 / 0	0.0	0.0	0.18 (1)	6.40	
J-H	-1682 / 0	0.0	0.0	0.18 (1)	6.40	
R-O	0 / 0	-18.5	-18.5	0.08 (4)	10.00	
O-P	0 / 1270	-18.5	-18.5	0.26 (1)	10.00	
P-O	0 / 1157	-18.5	-18.5	0.27 (1)	10.00	
O-N	0 / 1157	-18.5	-18.5	0.27 (1)	10.00	
N-M	0 / 1157	-18.5	-18.5	0.27 (1)	10.00	
M-L	0 / 1157	-18.5	-18.5	0.27 (1)	10.00	
L-K	0 / 1270	-18.5	-18.5	0.26 (1)	10.00	
K-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-264 / 0	0.12 (1)
C-P	-174 / 0	0.16 (1)
P-D	0 / 232	0.05 (4)
D-N	0 / 471	0.08 (1)
N-E	-653 / 0	0.37 (1)
N-F	0 / 471	0.08 (1)
L-F	0 / 232	0.05 (4)
L-G	-174 / 0	0.16 (1)
K-G	-264 / 0	0.12 (1)
B-Q	0 / 1317	0.30 (1)
K-H	0 / 1317	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.46/1.00 (D-E:1) , BC=0.27/1.00 (N-P:1) , WB=0.37/1.00 (E-N:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T11	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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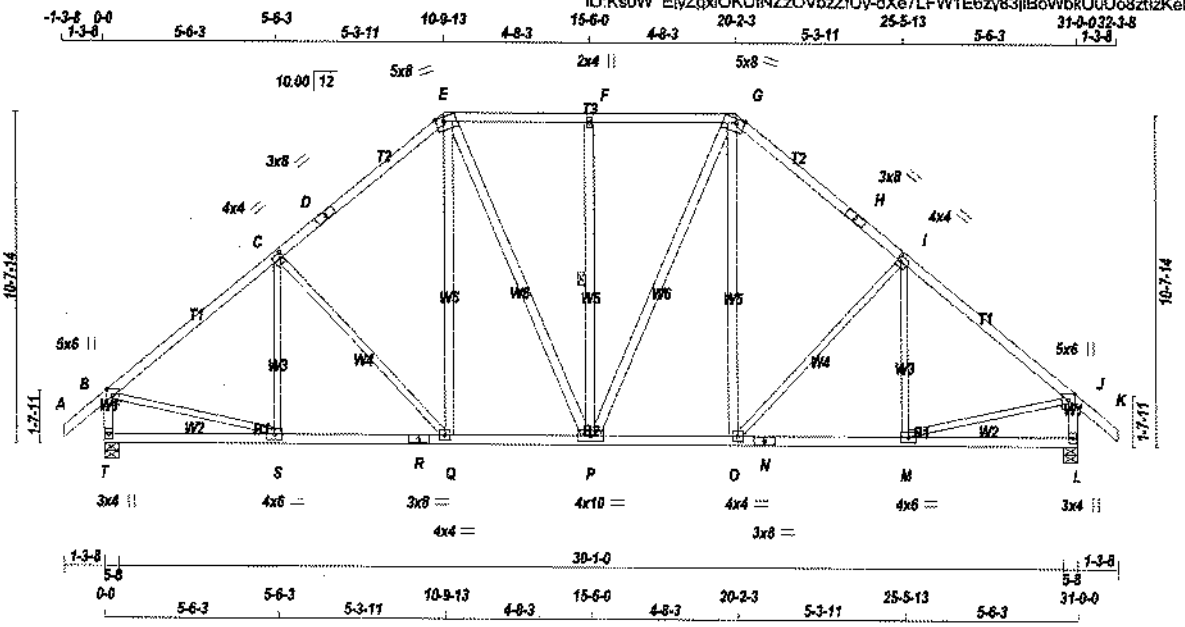


TOTAL WEIGHT = 3 X 175 = 525 lb

[MiTek]

LUMBER

N. L. G. A. RULES

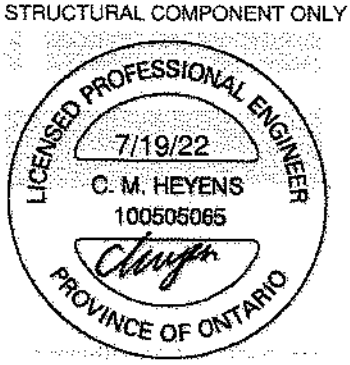
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
C - Q	2x3	DRY	No.2	SPF
O - I	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	8.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMWW+p	MT20	5.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	4.0	10.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	8.0		
S	BMWW-t	MT20	4.0	6.0		

STRUCTURAL COMPONENT ONLY



DWG # TR22070164 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	1720	0	1720	0	0	5-8	5-8
L	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1645 / 0	-84.9	-84.9	0.39 (1)	4.78
C- D	-1459 / 0	-84.9	-84.9	0.36 (1)	5.03
D- E	-1459 / 0	-84.9	-84.9	0.36 (1)	5.03
E- F	-1222 / 0	-84.9	-84.9	0.25 (1)	5.52
F- G	-1222 / 0	-84.9	-84.9	0.25 (1)	5.52
G- H	-1459 / 0	-84.9	-84.9	0.36 (1)	5.03
H- I	-1459 / 0	-84.9	-84.9	0.36 (1)	5.03
I- J	-1645 / 0	-84.9	-84.9	0.39 (1)	4.78
J- K	0 / 38	-84.9	-84.9	0.12 (1)	10.00
T- B	-1677 / 0	0.0	0.0	0.18 (1)	6.41
L- J	-1677 / 0	0.0	0.0	0.18 (1)	6.41
T- S	0 / 0	-18.5	-18.5	0.13 (4)	10.00
S- R	0 / 1291	-18.5	-18.5	0.27 (1)	10.00
R- Q	0 / 1291	-18.5	-18.5	0.27 (1)	10.00
Q- P	0 / 1095	-18.5	-18.5	0.23 (1)	10.00
P- O	0 / 1095	-18.5	-18.5	0.23 (1)	10.00
O- N	0 / 1291	-18.5	-18.5	0.27 (1)	10.00
N- M	0 / 1291	-18.5	-18.5	0.27 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.13 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-193 / 19	0.12 (1)
C- Q	-294 / 0	0.39 (1)
Q- E	0 / 309	0.05 (1)
E- P	0 / 307	0.05 (1)
P- F	-480 / 0	0.27 (1)
P- G	0 / 307	0.05 (1)
O- G	0 / 309	0.05 (1)
O- I	-294 / 0	0.39 (1)
M- I	-193 / 19	0.12 (1)
B- S	0 / 1326	0.30 (1)
M- J	0 / 1326	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.39/1.00 (I-J:1) , BC=0.27/1.00 (M-O:1) , WB=0.39/1.00 (I-O:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.62 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425212	T11	3	1	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070164 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T12	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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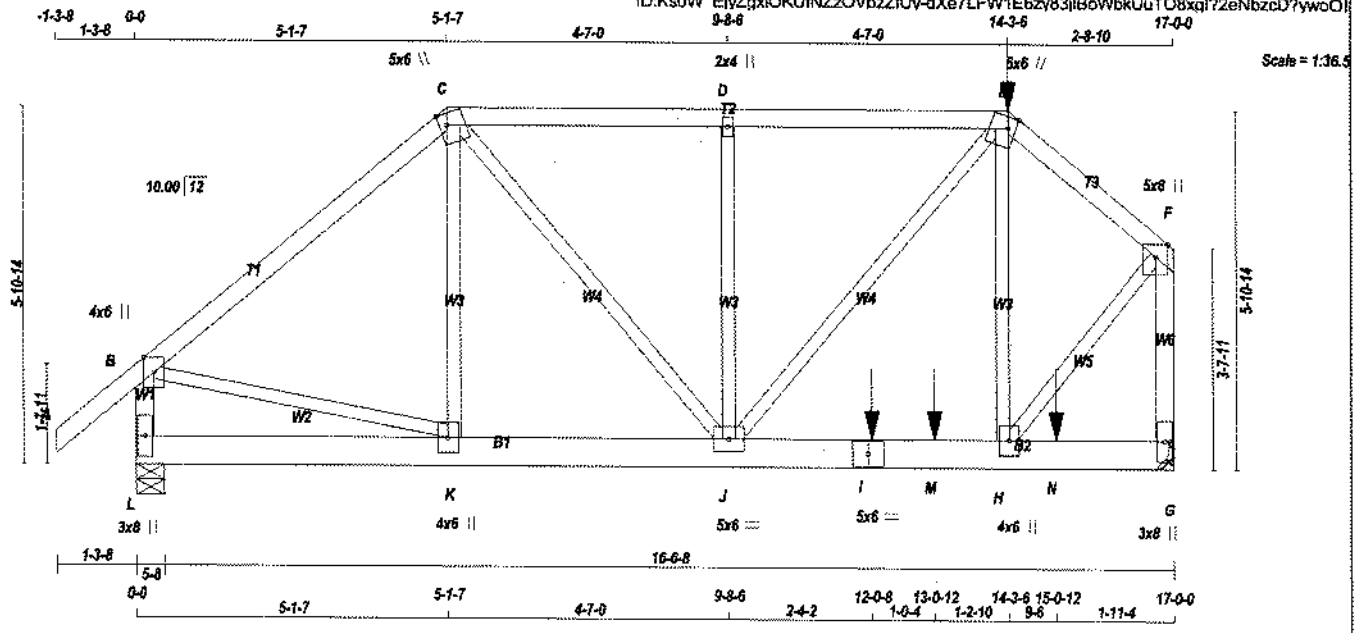

TOTAL WEIGHT = 90 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	8.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070165

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1211	0	1211	0	0	5-8	5-8
G	1587	0	1587	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	858	565 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0
G	1120	746 / 0	0 / 0	0 / 0	0 / 0	374 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.13 (1)	10.00	K- C	-124 / 36	0.06 (1)
B- C	-1010 / 0	-84.9	-84.9	0.48 (1)	5.53	C- J	0 / 521	0.13 (1)
C- D	-1104 / 0	-84.9	-84.9	0.34 (1)	5.54	J- D	-469 / 0	0.24 (1)
D- E	-1104 / 0	-84.9	-84.9	0.34 (1)	5.54	J- E	0 / 549	0.14 (1)
E- F	-989 / 0	-84.9	-84.9	0.13 (1)	6.10	H- E	-164 / 0	0.06 (1)
L- B	-1164 / 0	0.0	0.0	0.13 (1)	7.35	B- K	0 / 795	0.20 (1)
G- F	-1615 / 0	0.0	0.0	0.35 (1)	6.47	H- F	0 / 1148	0.28 (1)
L- K	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
K- J	0 / 772	-18.5	-18.5	0.22 (1)	10.00			
J- I	0 / 753	-18.5	-18.5	0.41 (1)	10.00			
I- M	0 / 753	-18.5	-18.5	0.41 (1)	10.00			
M- H	0 / 753	-18.5	-18.5	0.41 (1)	10.00			
H- N	0 / 0	-18.5	-18.5	0.23 (1)	10.00			
N- G	0 / 0	-18.5	-18.5	0.23 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	14-3-6	-16	-16	—	FRONT	VERT	DEAD	—	C1
E	14-3-6	-59	-59	—	FRONT	VERT	SNOW	—	C1
I	12-0-8	-327	-327	—	FRONT	VERT	TOTAL	—	C1
M	13-0-12	-123	-123	—	FRONT	VERT	TOTAL	—	C1
N	15-0-12	-123	-123	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.41/1.00 (H-J:1) , WB=0.28/1.00 (F-H:1) , SSI=0.29/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	550	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (I) (INPUT = 0.90)
JSI METAL= 0.54 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T13	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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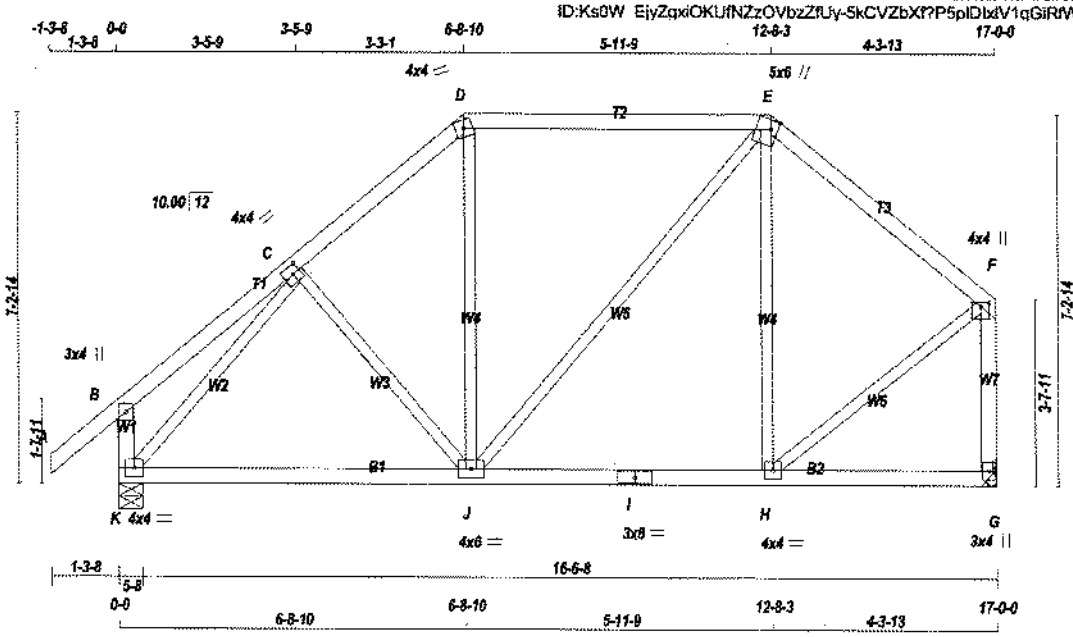

TOTAL WEIGHT = 80 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070166

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	996	0	996	0	0	5-8	5-8
G	879	0	879	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	705	460 / 0	0 / 0	0 / 0	0 / 0	244 / 0	0 / 0
G	624	396 / 0	0 / 0	0 / 0	0 / 0	228 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	C- J	-95 / 6	0.05 (1)	
B- C	0 / 21	-84.9	-84.9	0.15 (1)	10.00	J- D	0 / 95	0.03 (4)	
C- D	-706 / 0	-84.9	-84.9	0.12 (1)	6.25	J- E	0 / 172	0.04 (1)	
D- E	-526 / 0	-84.9	-84.9	0.39 (1)	6.25	H- E	-217 / 0	0.20 (1)	
E- F	-546 / 0	-84.9	-84.9	0.21 (1)	6.25	K- C	-927 / 0	0.42 (1)	
K- B	-226 / 0	0.0	0.0	0.02 (1)	7.81	H- F	0 / 522	0.12 (1)	
G- F	-846 / 0	0.0	0.0	0.19 (1)	7.81				
K- J	0 / 586	-18.5	-18.5	0.24 (4)	10.00				
J- I	0 / 415	-18.5	-18.5	0.23 (4)	10.00				
I- H	0 / 415	-18.5	-18.5	0.23 (4)	10.00				
H- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.39/1.00 (D-E:1) , BC=0.24/1.00 (J-K:4) , WB=0.42/1.00 (C-K:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T14	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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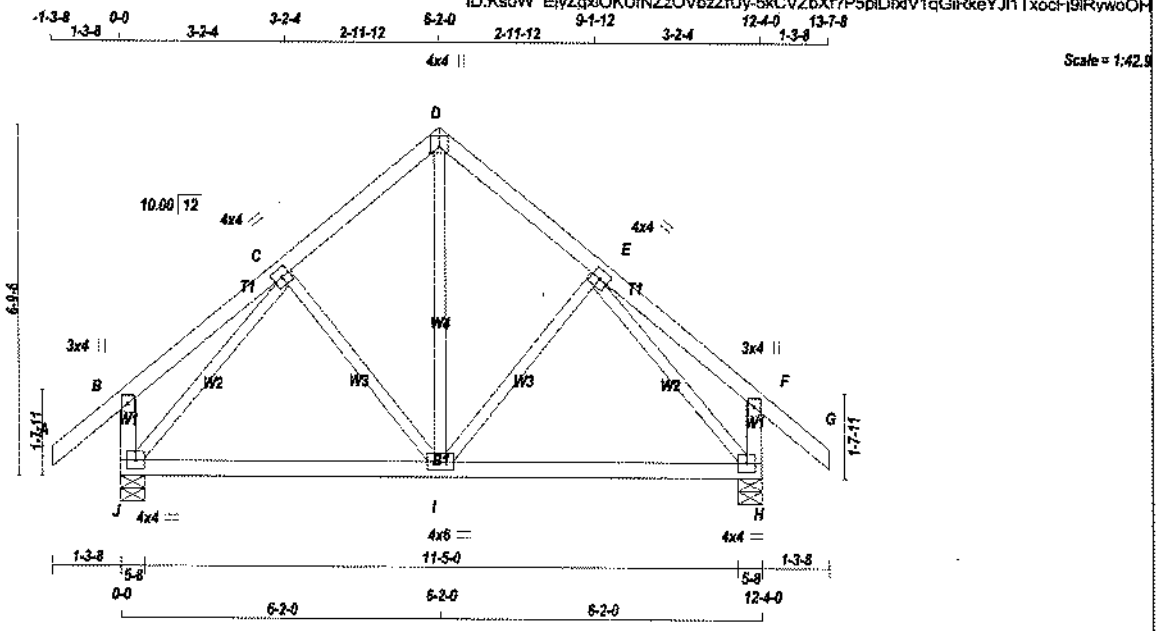

TOTAL WEIGHT = 59 lb

[MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	4.0	1.50	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

DWG # TR22070167

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	755	0	755	0	0	5-8	5-8
H	755	0	755	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00		I-D	0 / 323	0.07 (1)
B-C	0 / 19	-84.9	-84.9	0.13 (1)	10.00		I-E	-117 / 0	0.05 (1)
C-D	-432 / 0	-84.9	-84.9	0.10 (1)	6.25		C-I	-117 / 0	0.05 (1)
D-E	-432 / 0	-84.9	-84.9	0.10 (1)	6.25		J-C	-630 / 0	0.24 (1)
E-F	0 / 19	-84.9	-84.9	0.13 (1)	10.00		E-H	-630 / 0	0.24 (1)
F-G	0 / 38	-84.9	-84.9	0.12 (1)	10.00				
J-B	-218 / 0	0.0	0.0	0.02 (1)	7.81				
H-F	-218 / 0	0.0	0.0	0.02 (1)	7.81				
J-I	0 / 391	-18.5	-18.5	0.23 (4)	10.00				
I-H	0 / 391	-18.5	-18.5	0.23 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(56 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.23/1.00 (I-J:4) , WB=0.24/1.00 (C-J:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T14A	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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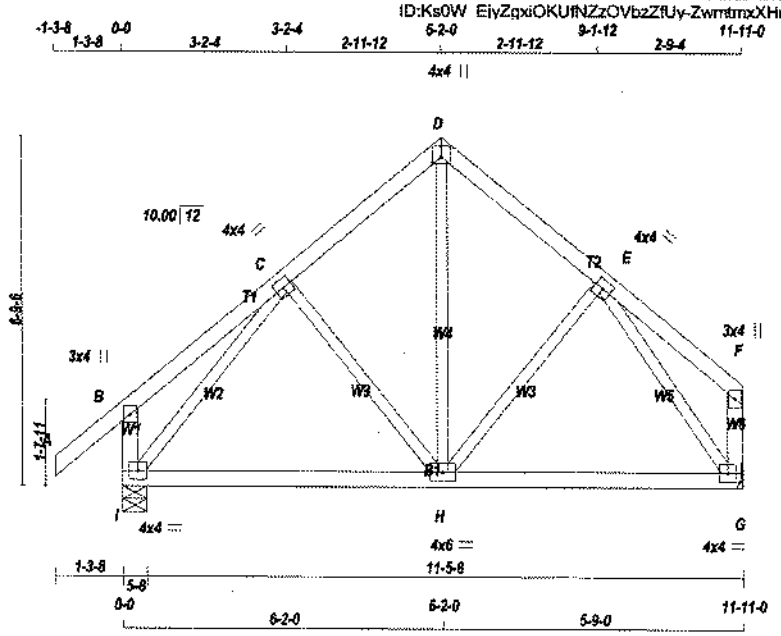

TOTAL WEIGHT = 57 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	4.0	1.50	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070168

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
I	733	0	733	0	0	5-8	5-8
G	616	0	616	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS.					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	518	342 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0
G	437	278 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 36	-84.9	-84.9	0.12 (1)	10.00	
B- C	0 / 19	-84.9	-84.9	0.13 (1)	10.00	
C- D	-406 / 0	-84.9	-84.9	0.10 (1)	6.25	
D- E	-402 / 0	-84.9	-84.9	0.09 (1)	6.25	
E- F	0 / 19	-84.9	-84.9	0.11 (1)	10.00	
I- B	-220 / 0	0.0	0.0	0.02 (1)	7.81	
G- F	-84 / 0	0.0	0.0	0.01 (1)	7.81	
I- H	0 / 374	-18.5	-18.5	0.22 (4)	10.00	
H- G	0 / 339	-18.5	-18.5	0.22 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- D	0 / 288	0.06 (1)
H- E	-69 / 8	0.03 (1)
C- H	-125 / 0	0.05 (1)
I- C	-600 / 0	0.23 (1)
E- G	-597 / 0	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.22/1.00 (H-I:4) , WB=0.23/1.00 (C-I:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T14S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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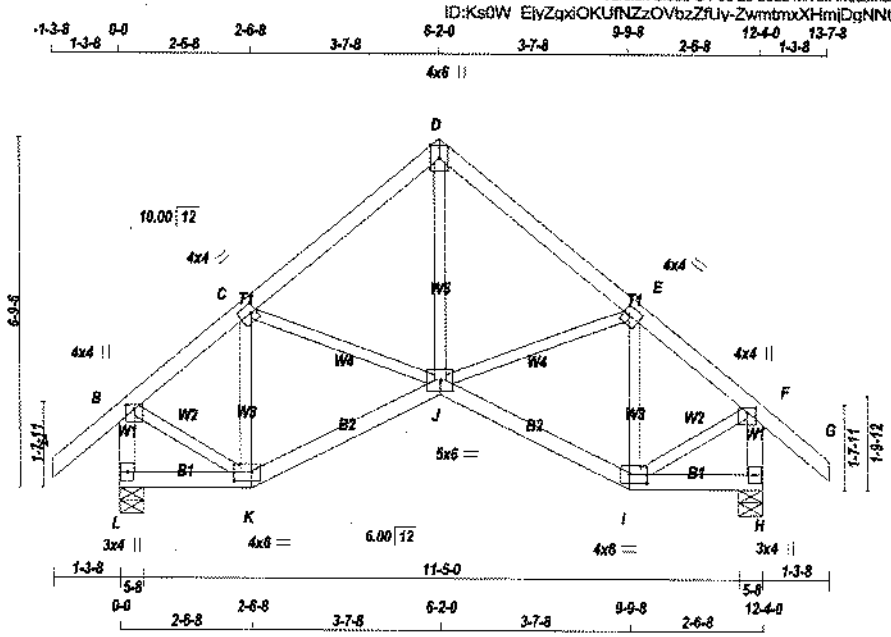

TOTAL WEIGHT = 2 X 59 = 119 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-p	MT20	4.0	6.0	2.00	4.00
J	BBWWW-p	MT20	5.0	6.0		
K	BBWW-p	MT20	4.0	6.0	2.00	4.00
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	755	0	755	0	0	5-8	5-8
H	755	0	755	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
A-B	0 / 38	-84.9	-84.9	0.12	(1)		10.00	J-D	0 / 450	0.10	(1)
B-C	-509 / 0	-84.9	-84.9	0.11	(1)		6.25	J-E	0 / 27	0.01	(1)
C-D	-573 / 0	-84.9	-84.9	0.13	(1)		6.25	I-E	-355 / 0	0.08	(1)
D-E	-573 / 0	-84.9	-84.9	0.13	(1)		6.25	C-J	0 / 27	0.01	(1)
E-F	-509 / 0	-84.9	-84.9	0.11	(1)		6.25	K-C	-355 / 0	0.08	(1)
F-G	0 / 38	-84.9	-84.9	0.12	(1)		10.00	B-K	0 / 463	0.10	(1)
L-B	-731 / 0	0.0	0.0	0.08	(1)		7.81	I-F	0 / 463	0.10	(1)
H-F	-731 / 0	0.0	0.0	0.08	(1)		7.81				
L-K	0 / 0	-18.5	-18.5	0.04	(4)		10.00				
K-J	0 / 449	-18.5	-18.5	0.11	(1)		10.00				
J-I	0 / 449	-18.5	-18.5	0.11	(1)		10.00				
I-H	0 / 0	-18.5	-18.5	0.04	(4)		10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.13/1.00 (C-D:1) , BC=0.11/1.00 (J-K:1) , WB=0.10/1.00 (B-K:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (K) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070169

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T15	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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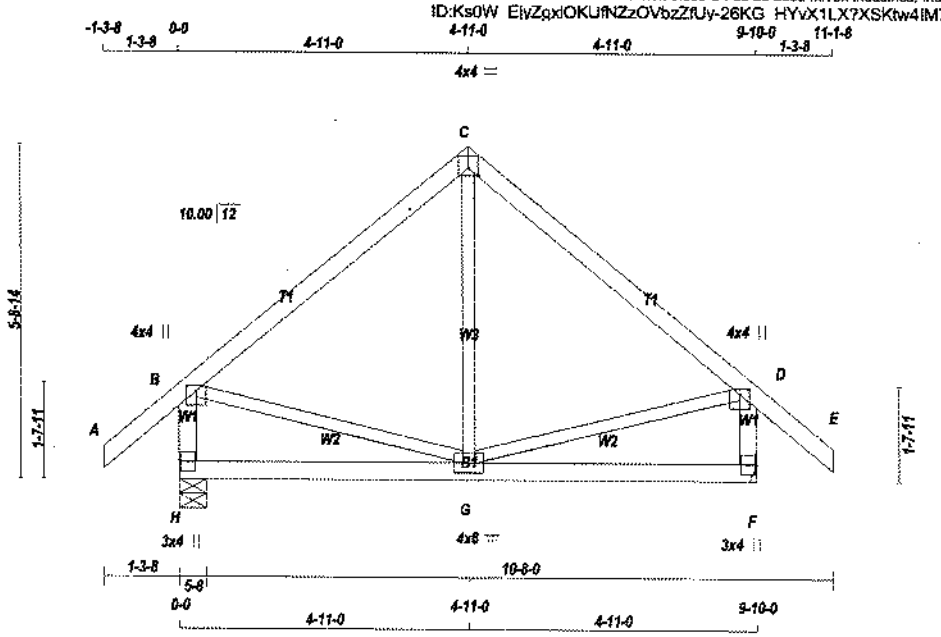

TOTAL WEIGHT = 2 X 45 = 89 lb

MiTek

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	626	0	626	0	0	5-8	5-8
F	626	0	626	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
H	442	293 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
F	442	293 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	G- C	-16 / 76	0.03 (4)
B- C	-314 / 0	-84.9	-84.9	0.26 (1)	6.25	B- G	0 / 249	0.06 (1)
C- D	-314 / 0	-84.9	-84.9	0.26 (1)	6.25	G- D	0 / 249	0.06 (1)
D- E	0 / 38	-84.9	-84.9	0.12 (1)	10.00			
H- B	-591 / 0	0.0	0.0	0.06 (1)	7.81			
F- D	-591 / 0	0.0	0.0	0.06 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	28.3	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.26/1.00 (C-D:1) , BC=0.13/1.00 (F-G:4) , WB=0.06/1.00 (B-G:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070170

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
425212	T16	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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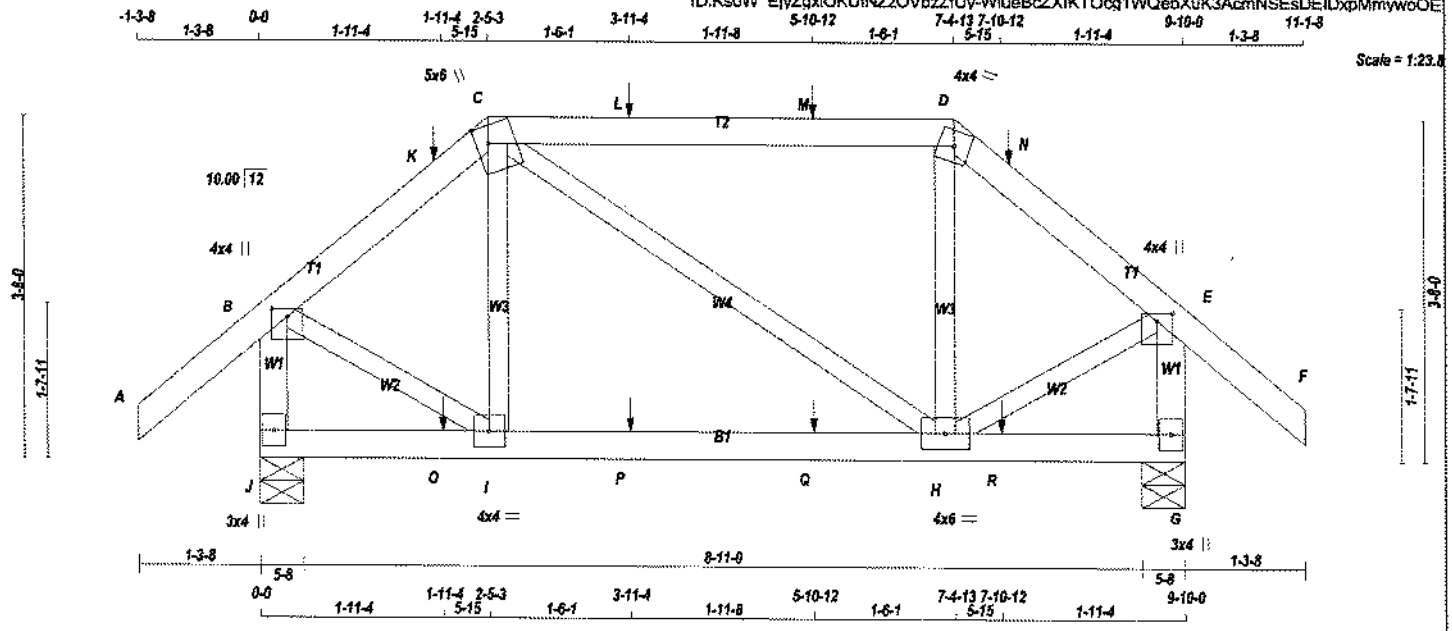

TOTAL WEIGHT = 45 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070171

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	626	0	626	0	0	5-8	5-8
G	626	0	626	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	442	293 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
G	442	293 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-84.9	-84.9	0.13 (1)	10.00	
B-K	-378 / 0	-84.9	-84.9	0.10 (1)	6.25	
K-C	-378 / 0	-84.9	-84.9	0.10 (1)	6.25	
C-L	-287 / 0	-84.9	-84.9	0.40 (1)	6.25	
L-M	-287 / 0	-84.9	-84.9	0.40 (1)	6.25	
M-D	-287 / 0	-84.9	-84.9	0.40 (1)	6.25	
D-N	-378 / 0	-84.9	-84.9	0.10 (1)	6.25	
N-E	-378 / 0	-84.9	-84.9	0.10 (1)	6.25	
E-F	0 / 38	-84.9	-84.9	0.13 (1)	10.00	
J-B	-613 / 0	0.0	0.0	0.07 (1)	7.81	
G-E	-613 / 0	0.0	0.0	0.07 (1)	7.81	
J-O	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
O-I	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
I-P	0 / 287	-18.5	-18.5	0.10 (4)	10.00	
P-Q	0 / 287	-18.5	-18.5	0.10 (4)	10.00	
Q-H	0 / 287	-18.5	-18.5	0.10 (4)	10.00	
H-R	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
R-G	0 / 0	-18.5	-18.5	0.07 (4)	10.00	

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	-71 / 31	0.02 (1)
C-H	0 / 0	0.00 (1)
H-D	-71 / 31	0.02 (1)
B-I	0 / 325	0.08 (1)
H-E	0 / 325	0.08 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	1-11-4	1	1	60	BACK	VERT	TOTAL	—	C1
L	3-11-4	1	1	72	BACK	VERT	TOTAL	—	C1
M	5-10-12	1	1	72	BACK	VERT	TOTAL	—	C1
N	7-10-12	1	1	60	BACK	VERT	TOTAL	—	C1
O	1-11-4	1	1	—	BACK	VERT	TOTAL	—	C1
P	3-11-4	1	1	—	BACK	VERT	TOTAL	—	C1
Q	5-10-12	1	1	—	BACK	VERT	TOTAL	—	C1
R	7-10-12	1	1	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = **24.0** **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.40/1.00 (C-D:1) , BC=0.10/1.00 (H-I:4) , WB=0.08/1.00 (B-I:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T17	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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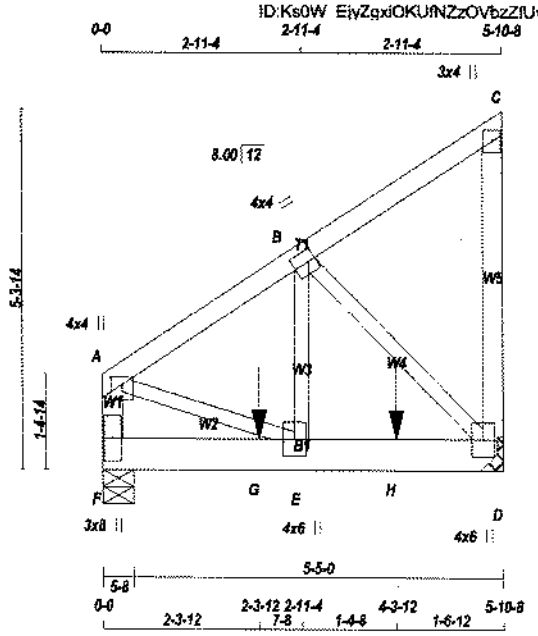

Scale = 1:32.7

TOTAL WEIGHT = 2 X 32 = 64 lb

(M)

LUMBER

N. L. G. A. RULES

			LUMBER	DESCR.
CHORDS	SIZE			
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-D	1	12	TOP
F-A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F-D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00

STRUCTURAL COMPONENT ONLY

DWG # TR22070172 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1552	0	1552	0	0	MECHANICAL	
F	1483	0	1483	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1098	721 / 0	0 / 0	0 / 0	0 / 0	377 / 0	0 / 0
F	1048	694 / 0	0 / 0	0 / 0	0 / 0	353 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A-B	-1374 / 0	-84.9	-84.9	0.06 (1)	6.25		E-B	0 / 1636	0.20 (1)
B-C	-13 / 0	-84.9	-84.9	0.05 (1)	6.25		B-D	-1639 / 0	0.23 (1)
D-C	-101 / 0	0.0	0.0	0.02 (1)	7.81		A-E	0 / 1220	0.15 (1)
F-A	-1264 / 0	0.0	0.0	0.07 (1)	7.81				
F-G	0 / 0	-18.5	-18.5	0.17 (1)	10.00				
G-E	0 / 0	-18.5	-18.5	0.17 (1)	10.00				
E-H	0 / 1154	-18.5	-18.5	0.23 (1)	10.00				
H-D	0 / 1154	-18.5	-18.5	0.23 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-3-12	-1105	-1105	---	BACK	VERT	TOTAL	---	C1
H	4-3-12	-609	-609	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.07/1.00 (A-F:1) , BC=0.23/1.00 (D-E:1) , WB=0.23/1.00 (B-D:1) , SSI=0.47/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL = 0.26 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425212	T17	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070172 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T17Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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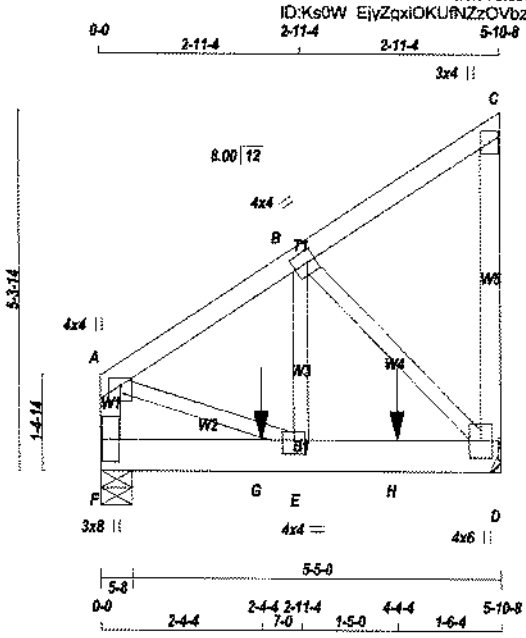


Scale = 1:32.7

TOTAL WEIGHT = 2 X 32 = 64 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

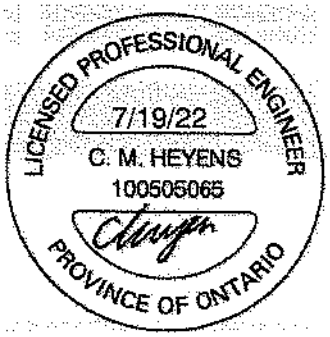
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	4.0	1.25	2.00

STRUCTURAL COMPONENT ONLY



DWG # TR22070173 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	997	0	997	0	0	MECHANICAL	
F	825	0	825	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	703	472 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0
F	582	389 / 0	0 / 0	0 / 0	0 / 0	193 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	-763 / 0	-84.9	-84.9	0.06 (1)		6.25	E- B	0 / 761	0.10 (1)
B- C	-15 / 0	-84.9	-84.9	0.06 (1)		6.25	B- D	-919 / 0	0.13 (1)
D- C	-98 / 0	0.0	0.0	0.02 (1)		7.81	A- E	0 / 884	0.09 (1)
F- A	-750 / 0	0.0	0.0	0.04 (1)		7.81			
F- G	0 / 0	-18.5	-18.5	0.06 (1)		10.00			
G- E	0 / 0	-18.5	-18.5	0.06 (1)		10.00			
E- H	0 / 648	-18.5	-18.5	0.17 (1)		10.00			
H- D	0 / 648	-18.5	-18.5	0.17 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-4-4	-427	-427	—	FRONT	VERT	TOTAL	—	C1
H	4-4-4	-427	-427	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.17/1.00 (D-E:1) , WB=0.13/1.00 (B-D:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (E) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425212	T17Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:Ks0W_EjyZgxlOKUfNZzOVbzZfUy-_VS0Oya93ebFEgcl_L6mRYbRd9idzlhNWthNuCywoOD

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMWW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	4.0	4.0	2.50	2.00
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070173 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	T17Z1	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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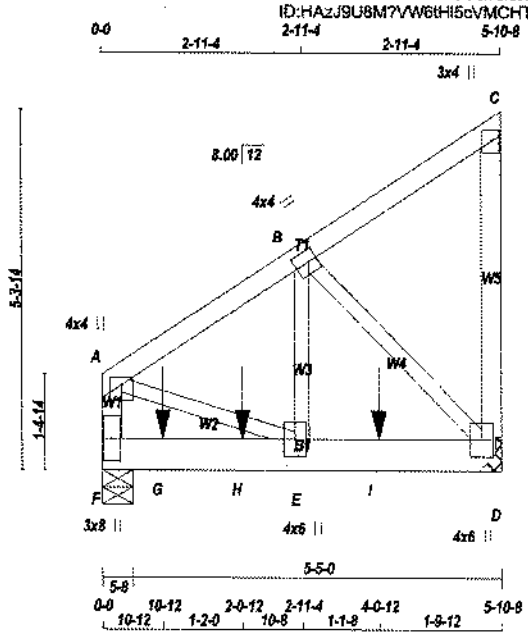

TOTAL WEIGHT = 2 X 32 = 64 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	4.0	1.25	2.00

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070189 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1184	0	1184	0	0	MECHANICAL	
F	1634	0	1634	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D, MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	836	557 / 0	0 / 0	0 / 0	0 / 0	279 / 0	0 / 0
F	1153	772 / 0	0 / 0	0 / 0	0 / 0	381 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	-1082 / 0	-84.9	-84.9	0.06 (1)	6.25	E- B	0 / 1227	0.15 (1)	
B- C	-14 / 0	-84.9	-84.9	0.06 (1)	6.25	B- D	-1294 / 0	0.18 (1)	
D- C	-99 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 983	0.12 (1)	
F- A	-1018 / 0	0.0	0.0	0.06 (1)	7.81				
F- G	0 / 0	-18.5	-18.5	0.20 (1)	10.00				
G- H	0 / 0	-18.5	-18.5	0.20 (1)	10.00				
H- E	0 / 0	-18.5	-18.5	0.20 (1)	10.00				
E- I	0 / 912	-18.5	-18.5	0.21 (1)	10.00				
I- D	0 / 912	-18.5	-18.5	0.21 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	10-12	-520	-520	—	BACK	VERT	TOTAL	—	C1
H	2-0-12	-519	-519	—	BACK	VERT	TOTAL	—	C1
I	4-0-12	-519	-519	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF SCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.21/1.00 (D-E:1) , WB=0.18/1.00 (B-D:1) , SSI=0.30/1.00 (E-F:1)

OOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (B) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425214	T17Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:HAzJ9U8M?VW6tHi5cVMCHTzYmI6-Ugp5sEr5nPHZoj_r?qUs3BOZCvry0KP2I7DJVJywoLH

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

DWG # TR22070189 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	T17Z2	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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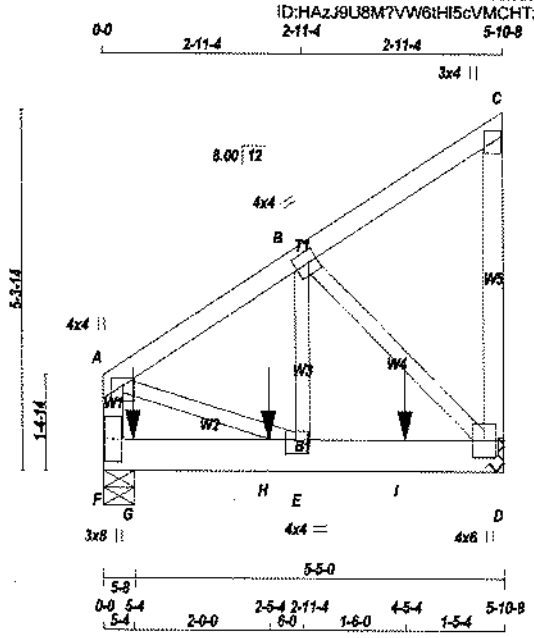

Scale = 1:32.7

TOTAL WEIGHT = 2 X 32 = 64 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	4.0	1.25	2.00

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070190 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1062	0	1062	0	0	MECHANICAL	
F	1159	0	1159	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	751	493 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0
F	817	552 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	-887 / 0	-84.9	-84.9	0.06 (1)	6.25	E- B	0 / 954	0.12 (1)	
B- C	-15 / 0	-84.9	-84.9	0.06 (1)	6.25	B- D	-1065 / 0	0.15 (1)	
D- C	-98 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 792	0.10 (1)	
F- A	-854 / 0	0.0	0.0	0.05 (1)	7.81				
F- G	0 / 0	-18.5	-18.5	0.08 (1)	10.00				
G- H	0 / 0	-18.5	-18.5	0.08 (1)	10.00				
H- E	0 / 0	-18.5	-18.5	0.08 (1)	10.00				
E- I	0 / 750	-18.5	-18.5	0.15 (1)	10.00				
I- D	0 / 750	-18.5	-18.5	0.15 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	5-4	-170	-170	—	TOP	VERT	TOTAL	—	C1
H	2-5-4	-608	-608	—	FRONT	VERT	TOTAL	—	C1
I	4-5-4	-358	-358	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.15/1.00 (D-E:1) , WB=0.15/1.00 (B-D:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425214	T17Z2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	4.0	4.0	2.50	1.75
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

DWG # TR22070190 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425213	T18	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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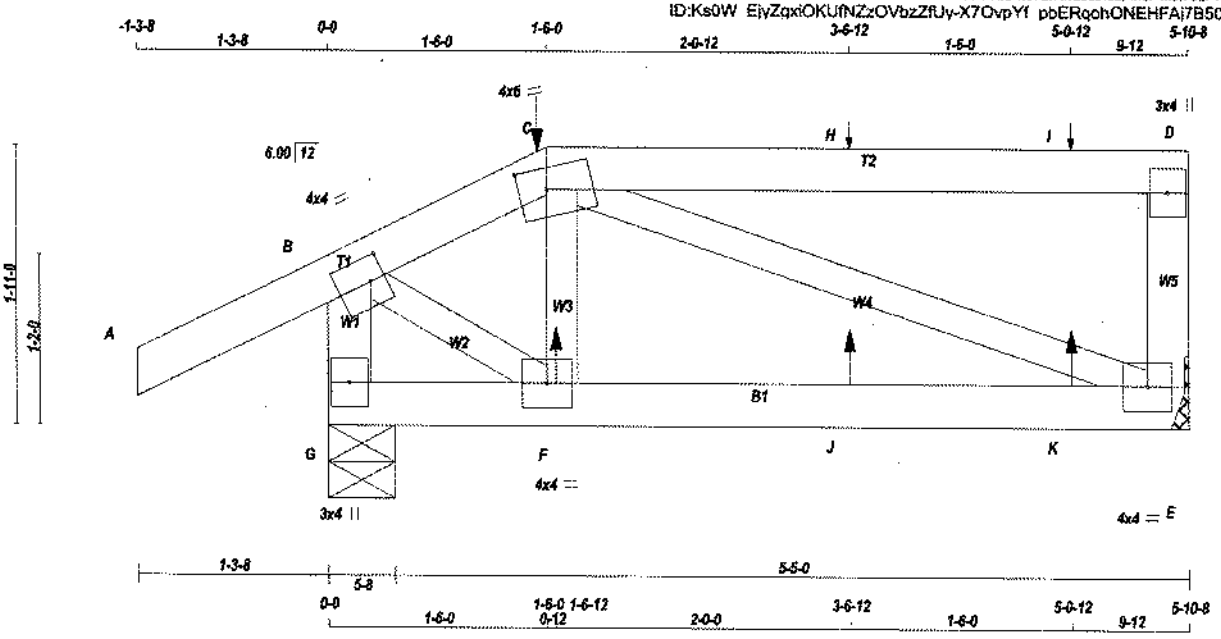


TOTAL WEIGHT = 23 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

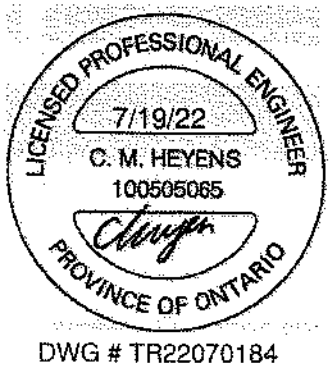
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	4.0	6.0	1.75	2.26
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY



DWG # TR22070184

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	286	0	285	0	0	MECHANICAL	
G	452	0	452	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	203	125 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
G	318	217 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00		F-C	-59 / 37	0.01 (4)
B-C	-257 / 0	-84.9	-84.9	0.11 (1)	6.25		C-E	-218 / 0	0.07 (1)
C-H	0 / 0	-84.9	-84.9	0.28 (1)	10.00		B-F	0 / 236	0.05 (1)
H-I	0 / 0	-84.9	-84.9	0.28 (1)	10.00				
I-D	0 / 0	-84.9	-84.9	0.28 (1)	10.00				
E-D	-186 / 0	0.0	0.0	0.02 (1)	7.81				
G-B	-447 / 0	0.0	0.0	0.05 (1)	7.81				
G-F	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
F-J	0 / 204	-18.5	-18.5	0.09 (4)	10.00				
J-K	0 / 204	-18.5	-18.5	0.09 (4)	10.00				
K-E	0 / 204	-18.5	-18.5	0.09 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-6-0	-29	-29	67	BACK	VERT	TOTAL	—	C1
F	1-6-12	6	1	8	BACK	VERT	TOTAL	—	C1
H	3-6-12	1	1	67	BACK	VERT	TOTAL	—	C1
I	5-0-12	1	1	62	BACK	VERT	TOTAL	—	C1
J	3-6-12	6	1	8	BACK	VERT	TOTAL	—	C1
K	5-0-12	6	1	8	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28/1.00 (C-D:1) , BC=0.09/1.00 (E-F:4) , WB=0.07/1.00 (C-E:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T19	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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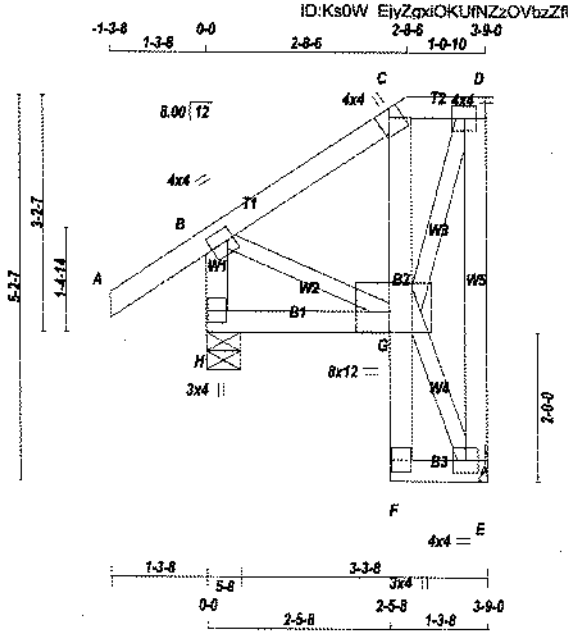

TOTAL WEIGHT = 29 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.66
C	TMTV*+h	MT20	4.0	4.0	Edge	
D	TMVW-t	MT20	4.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-I	MT20	8.0	12.0	3.25	5.25
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070174

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	194	0	194	0	0	MECHANICAL	
H	310	0	310	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	138	87 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
H	218	151 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.13 (5)	10.00		G-E	-6 / 0	0.00 (1)
B-C	-71 / 0	-84.9	-84.9	0.09 (1)	6.25		G-D	0 / 136	0.03 (1)
C-D	-50 / 0	-84.9	-84.9	0.02 (1)	6.25		B-G	0 / 63	0.01 (1)
E-D	-178 / 0	0.0	0.0	0.08 (1)	7.81				
H-B	-286 / 0	0.0	0.0	0.03 (1)	7.81				
H-G	0 / 0	-18.5	-18.5	0.04 (4)	10.00				
F-G	0 / 12	0.0	0.0	0.01 (1)	10.00				
G-C	-122 / 0	0.0	0.0	0.01 (1)	7.81				
F-E	0 / 3	-18.5	-18.5	0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.04/1.00 (G-H:4) , WB=0.03/1.00 (D-G:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425212	T20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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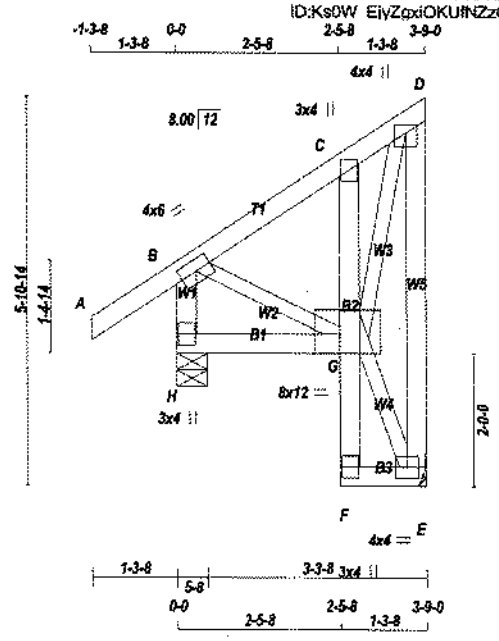

Scale = 1:33.6

TOTAL WEIGHT = 30 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.50	2.00
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWWm	MT20	8.0	12.0	3.50	4.50
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070175

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	194	0	194	0	0	MECHANICAL	
H	310	0	310	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	138	87 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
H	218	151 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.13 (5)	10.00	
B-C	-78 / 0	-84.9	-84.9	0.07 (1)	6.25	
C-D	-94 / 0	-84.9	-84.9	0.06 (1)	6.25	
E-D	-174 / 0	0.0	0.0	0.11 (1)	7.81	
H-B	-286 / 0	0.0	0.0	0.03 (1)	7.81	
H-G	0 / 0	-18.5	-18.5	0.04 (4)	10.00	
F-G	0 / 12	0.0	0.0	0.02 (1)	10.00	
G-C	-202 / 0	0.0	0.0	0.02 (1)	7.81	
F-E	0 / 5	-18.5	-18.5	0.01 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-E	-10 / 0	0.00 (1)
G-D	0 / 207	0.05 (1)
B-G	0 / 79	0.02 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.04/1.00 (G-H:4) , WB=0.05/1.00 (D-G:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	T21	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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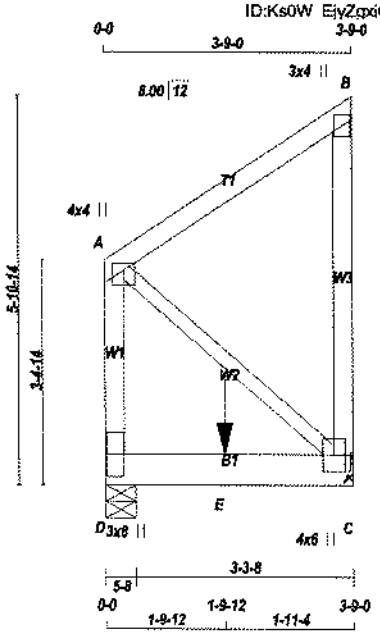

Scale = 1:33.6

TOTAL WEIGHT = 2 X 25 = 49 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	TOP
D- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		

STRUCTURAL COMPONENT ONLY

DWG # TR22070176 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	483	0	483	0	0	MECHANICAL	
D	502	0	502	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	342	221 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0
D	356	231 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			LENGTH	FR-TO		
A- B	0 / 0	-84.9	-84.9	0.11 (1)	10.00		A- C	0 / 0	0.00 (1)
C- B	-159 / 0	0.0	0.0	0.05 (1)	7.81				
D- A	-159 / 0	0.0	0.0	0.02 (1)	7.81				
D- E	0 / 0	-18.5	-18.5	0.22 (1)	10.00				
E- C	0 / 0	-18.5	-18.5	0.22 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-9-12	-422	-422	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.22/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425212	T21	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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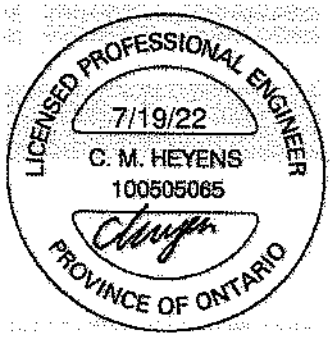
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY



DWG # TR22070176 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	T22	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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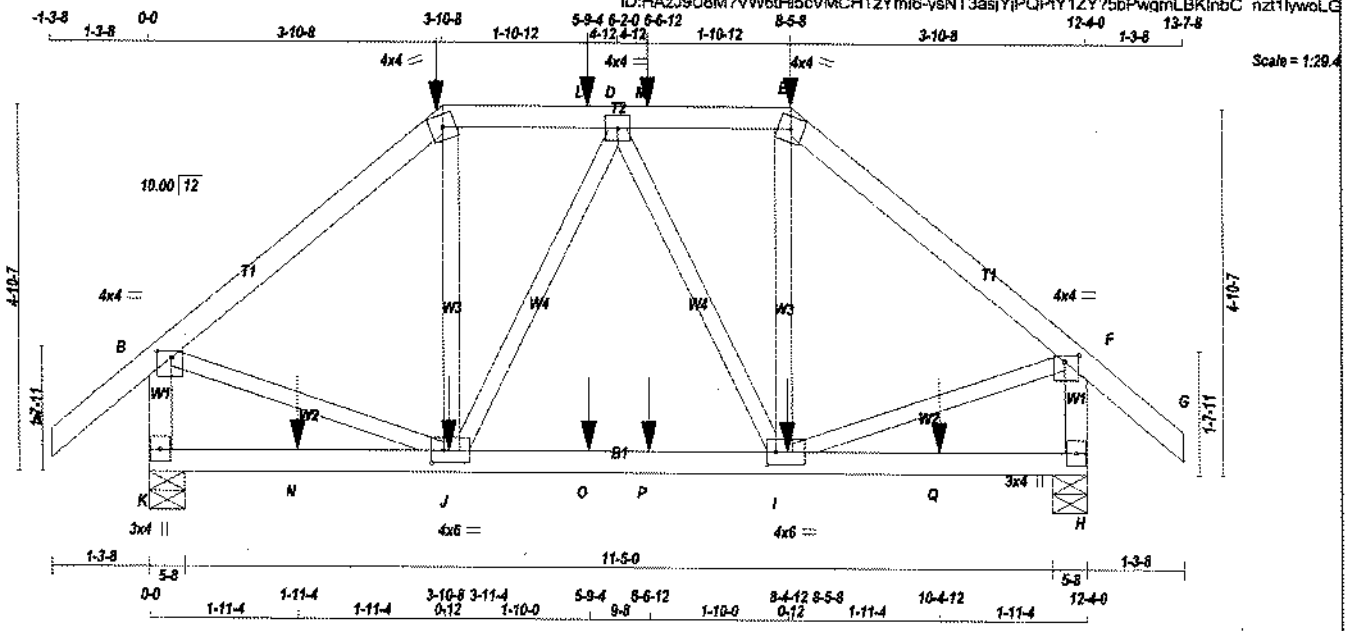

TOTAL WEIGHT = 59 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.00	2.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-p	MT20	4.0	4.0	1.00	2.25
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	6.0	2.00	1.50
J	BMWWW-t	MT20	4.0	6.0	2.00	2.00
K	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070191

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1158	0	1158	0	0	5-8	5-8
H	1158	0	1158	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	818	540 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0
H	818	540 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 38	-84.9	-84.9	0.13 (1)	10.00	J- C	0 / 100	0.04 (4)
B- C	-940 / 0	-84.9	-84.9	0.26 (1)	6.02	J- D	-206 / 0	0.09 (1)
C- L	-722 / 0	-84.9	-84.9	0.11 (1)	6.25	D- I	-206 / 0	0.09 (1)
L- D	-722 / 0	-84.9	-84.9	0.11 (1)	6.25	I- E	0 / 100	0.04 (4)
D- M	-722 / 0	-84.9	-84.9	0.11 (1)	6.25	B- J	0 / 758	0.19 (1)
M- E	-722 / 0	-84.9	-84.9	0.11 (1)	6.25	I- F	0 / 758	0.19 (1)
E- F	-940 / 0	-84.9	-84.9	0.26 (1)	6.02			
F- G	0 / 38	-84.9	-84.9	0.13 (1)	10.00			
K- B	-1122 / 0	0.0	0.0	0.13 (1)	7.45			
H- F	-1122 / 0	0.0	0.0	0.13 (1)	7.45			
K- N	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
N- J	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
J- O	0 / 811	-18.5	-18.5	0.21 (1)	10.00			
O- P	0 / 811	-18.5	-18.5	0.21 (1)	10.00			
P- I	0 / 811	-18.5	-18.5	0.21 (1)	10.00			
I- Q	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
Q- H	0 / 0	-18.5	-18.5	0.12 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-188	-188	---	FRONT	VERT	TOTAL	---	C1
E	8-5-8	-188	-188	---	FRONT	VERT	TOTAL	---	C1
I	8-4-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
J	3-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
L	5-9-4	-55	-55	---	FRONT	VERT	TOTAL	---	C1
M	6-6-12	-55	-55	---	FRONT	VERT	TOTAL	---	C1
N	1-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
O	5-9-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
P	6-6-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
Q	10-4-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD.		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.26/1.00 (E-F:1) , BC=0.21/1.00 (I-J:1) , WB=0.19/1.00 (F-I:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	T23	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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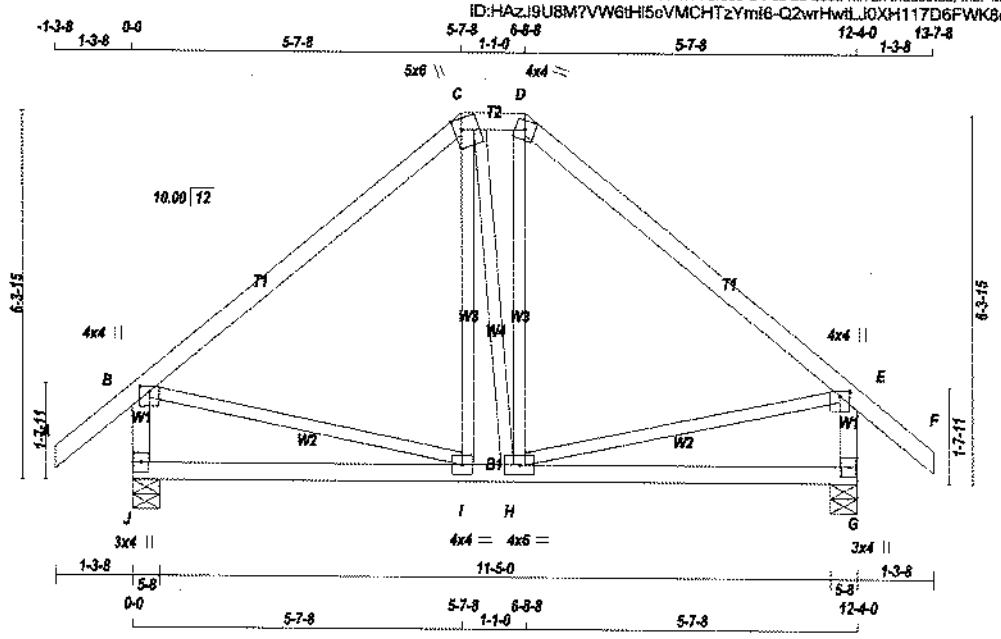

Scale = 1:37.5

TOTAL WEIGHT = 62 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070192

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	755	0	755	0	0	5-8	5-8
G	755	0	755	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
G	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	
B- C	-436 / 0	-84.9	-84.9	0.35 (1)	6.25	
C- D	-335 / 0	-84.9	-84.9	0.01 (1)	6.25	
D- E	-437 / 0	-84.9	-84.9	0.35 (1)	6.25	
E- F	0 / 38	-84.9	-84.9	0.12 (1)	10.00	
J- B	-712 / 0	0.0	0.0	0.08 (1)	7.81	
G- E	-711 / 0	0.0	0.0	0.08 (1)	7.81	
J- I	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
I- H	0 / 334	-18.5	-18.5	0.19 (4)	10.00	
H- G	0 / 0	-18.5	-18.5	0.15 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-8 / 43	0.01 (4)
C- H	0 / 9	0.00 (4)
H- D	-4 / 51	0.02 (4)
B- I	0 / 343	0.08 (1)
H- E	0 / 343	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.35/1.00 (D-E:1) , BC=0.19/1.00 (H-I:4) , WB=0.08/1.00 (E-H:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	660	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (E) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	T24	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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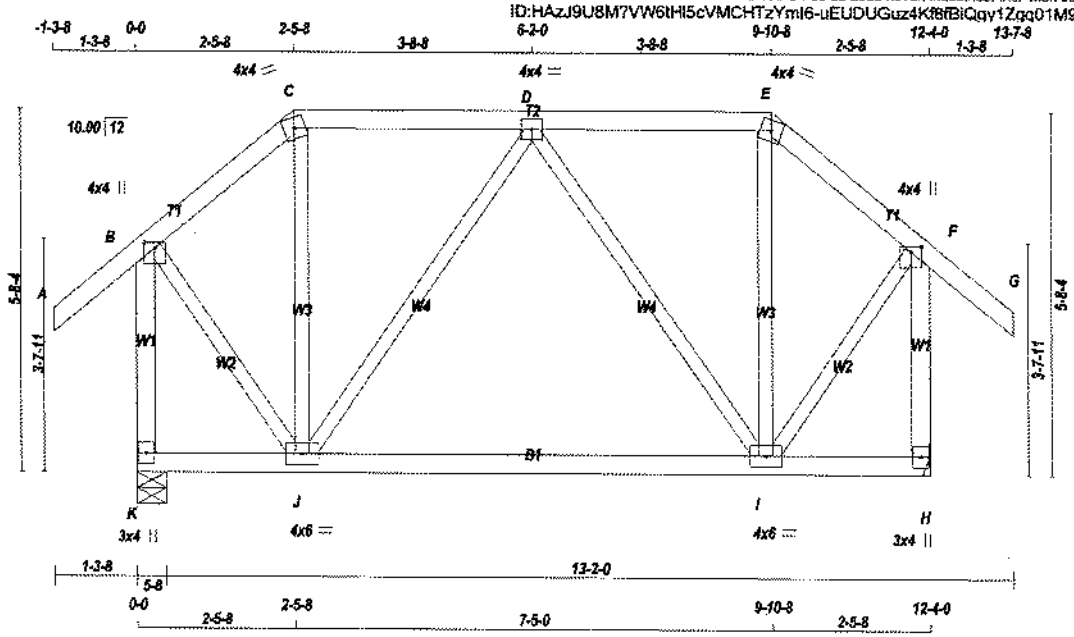

TOTAL WEIGHT = 66 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070193

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	756	0	755	0	0	5-8	5-8
H	755	0	755	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)		10.00
B- C	-328 / 0	-84.9	-84.9	0.09 (1)		6.25
C- D	-250 / 0	-84.9	-84.9	0.19 (1)		6.25
D- E	-250 / 0	-84.9	-84.9	0.19 (1)		6.25
E- F	-328 / 0	-84.9	-84.9	0.09 (1)		6.25
F- G	0 / 38	-84.9	-84.9	0.12 (1)		10.00
K- B	-756 / 0	0.0	0.0	0.17 (1)		7.81
H- F	-756 / 0	0.0	0.0	0.17 (1)		7.81
K- J	0 / 0	-18.5	-18.5	0.16 (4)		10.00
J- I	0 / 380	-18.5	-18.5	0.19 (4)		10.00
I- H	0 / 0	-18.5	-18.5	0.16 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-14 / 39	0.01 (4)
J- D	-233 / 0	0.18 (1)
I- D	-233 / 0	0.18 (1)
I- E	-14 / 39	0.01 (4)
B- J	0 / 409	0.09 (1)
I- F	0 / 409	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = **24.0** **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.19/1.00 (D-E:1) , BC=0.19/1.00 (I-J:4) , WB=0.18/1.00 (D-J:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	660	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	T25	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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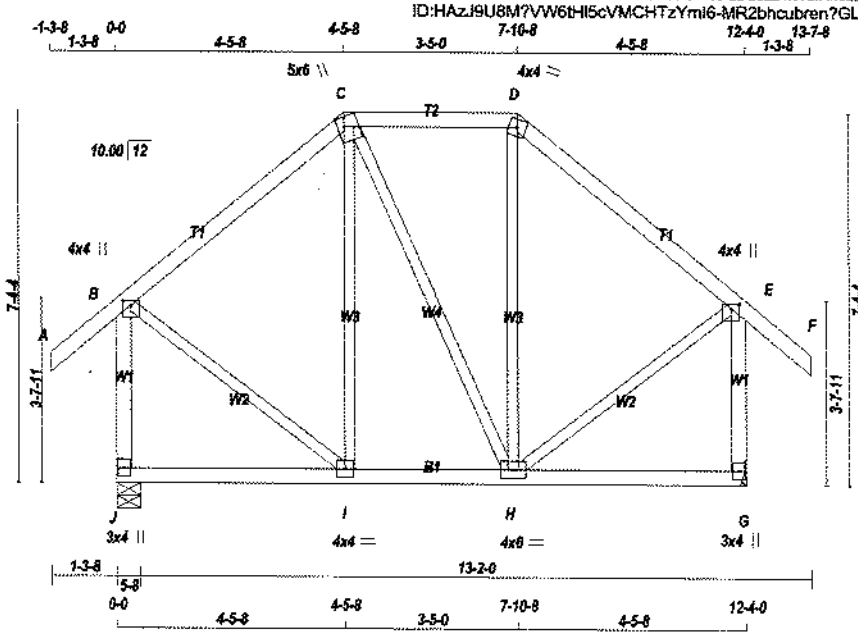

TOTAL WEIGHT = 69 lb

MiTek

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070194

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	755	0	755	0	0	5-8	5-8
G	755	0	755	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
G	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00		I- C	-112 / 16	0.11 (1)
B- C	-344 / 0	-84.9	-84.9	0.22 (1)	6.25		C- H	0 / 0	0.00 (1)
C- D	-262 / 0	-84.9	-84.9	0.13 (1)	6.25		H- D	-113 / 17	0.11 (1)
D- E	-344 / 0	-84.9	-84.9	0.22 (1)	6.25		B- I	0 / 326	0.07 (1)
E- F	0 / 38	-84.9	-84.9	0.12 (1)	10.00		H- E	0 / 326	0.07 (1)
J- B	-719 / 0	0.0	0.0	0.16 (1)	7.81				
G- E	-719 / 0	0.0	0.0	0.16 (1)	7.81				
J- I	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
I- H	0 / 262	-18.5	-18.5	0.10 (4)	10.00				
H- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.10/1.00 (H-I:4) , WB=0.11/1.00 (D-H:1) , SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425214	T26	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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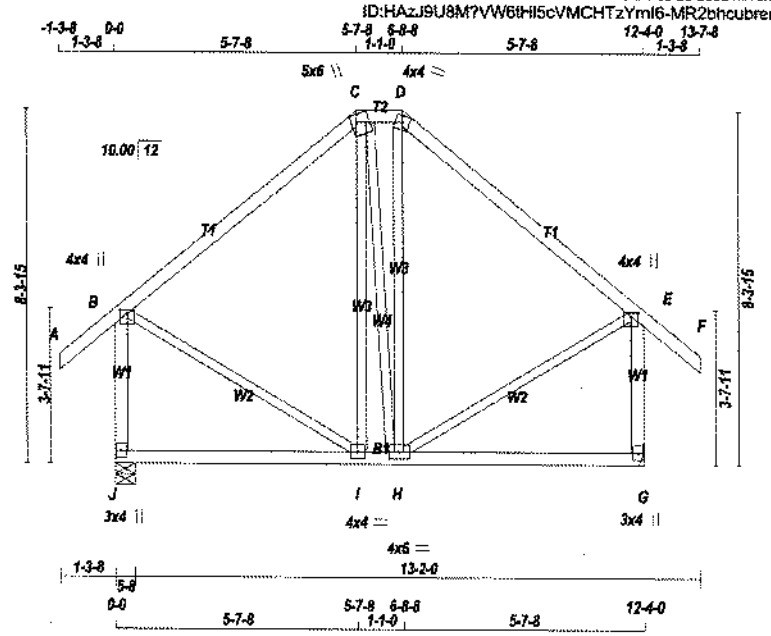

Scale = 1:52.0

TOTAL WEIGHT = 73 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	755	0	755	0	0	5-8	5-8
G	755	0	755	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
G	534	352 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B-C	-329 / 0	-84.9	-84.9	0.35 (1)	6.25
C-D	-250 / 0	-84.9	-84.9	0.01 (1)	6.25
D-E	-328 / 0	-84.9	-84.9	0.35 (1)	6.26
E-F	0 / 38	-84.9	-84.9	0.12 (1)	10.00
J-B	-713 / 0	0.0	0.0	0.16 (1)	7.81
G-E	-712 / 0	0.0	0.0	0.16 (1)	7.81
J-I	0 / 0	-18.5	-18.5	0.14 (4)	10.00
I-H	0 / 251	-18.5	-18.5	0.17 (4)	10.00
H-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	-67 / 22	0.09 (1)
C-H	-6 / 2	0.01 (1)
H-D	-74 / 25	0.10 (1)
B-I	0 / 291	0.07 (1)
H-E	0 / 290	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.17/1.00 (H-I:4) , WB=0.10/1.00 (D-H:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070195

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	T27	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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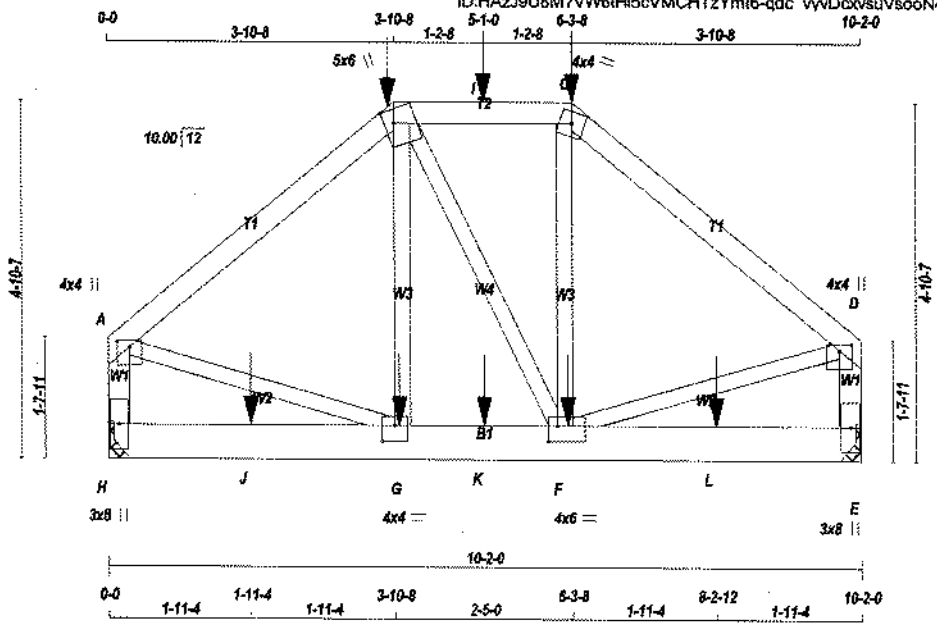

TOTAL WEIGHT = 52 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	8.0		
F	BMWWW-t	MT20	4.0	6.0	2.50	1.50
G	BMWW-t	MT20	4.0	4.0	2.50	2.00
H	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070196

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	880	0	880	0	0	IN-SX	IN-SX
E	880	0	880	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT E = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	623	404 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0
E	623	404 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-766 / 0	-84.9	-84.9	0.26 (1)	6.25	G-B	-82 / 46	0.03 (1)
B-I	-563 / 0	-84.9	-84.9	0.17 (1)	6.25	B-F	-4 / 2	0.00 (1)
I-C	-583 / 0	-84.9	-84.9	0.17 (1)	6.25	F-C	-87 / 46	0.03 (1)
C-D	-763 / 0	-84.9	-84.9	0.26 (1)	6.25	A-G	0 / 614	0.15 (1)
H-A	-837 / 0	0.0	0.0	0.10 (1)	7.81	F-D	0 / 612	0.15 (1)
E-D	-835 / 0	0.0	0.0	0.10 (1)	7.81			
H-J	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
J-G	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
G-K	0 / 585	-18.5	-18.5	0.09 (1)	10.00			
K-F	0 / 585	-18.5	-18.5	0.09 (1)	10.00			
F-L	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
L-E	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-10-8	-188	-188	---	BACK	VERT	TOTAL	---	C1
C	6-3-8	-188	-188	---	BACK	VERT	TOTAL	---	C1
F	6-2-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
G	3-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
I	5-1-0	-55	-55	---	BACK	VERT	TOTAL	---	C1
J	1-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
K	5-1-0	-14	-14	---	BACK	VERT	TOTAL	---	C1
L	8-2-12	-14	-14	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = **24.0** **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.26/1.00 (A-B:1) , BC=0.09/1.00 (F-G:1) , WB=0.15/1.00 (A-G:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.22 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	T28	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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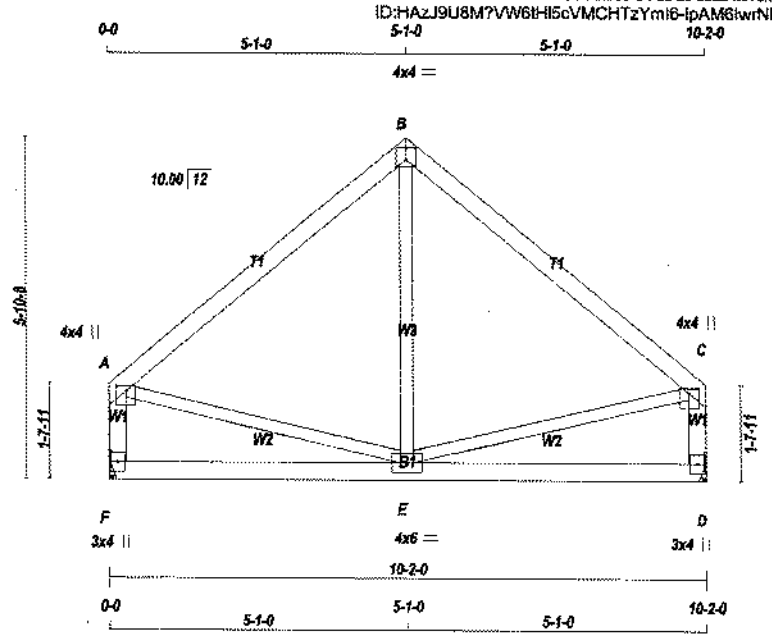

TOTAL WEIGHT = 42 lb

[MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	525	0	525	0	0	MECHANICAL	
D	525	0	525	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	373	237 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0
D	373	237 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-327 / 0	-84.9	-84.9	0.28 (1)	6.25	E-B	-13 / 80	0.03 (4)
B-C	-327 / 0	-84.9	-84.9	0.28 (1)	6.25	A-E	0 / 259	0.06 (1)
F-A	-469 / 0	0.0	0.0	0.05 (1)	7.81	E-C	0 / 259	0.06 (1)
D-C	-469 / 0	0.0	0.0	0.05 (1)	7.81			
F-E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF SCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.28/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) , WB=0.06/1.00 (A-E:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (A) (INPUT = 0.90)
JSI METAL= 0.12 (A) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

DWG # TR22070197

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425214	T29	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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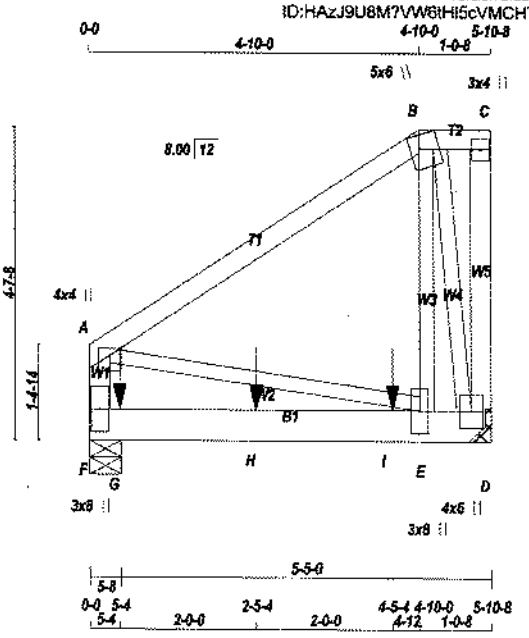

Scale = 1:32.8

TOTAL WEIGHT = 2 X 34 = 68 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-B	1	12	TOP
B-C	1	12	TOP
C-D	1	12	TOP
F-A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F-D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070198 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1062	0	1062	0	0	MECHANICAL	
F	1159	0	1159	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	751	493 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0
F	817	552 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A-B	-438 / 0	-84.9	-84.9	0.19 (1)	6.25		E-B	0 / 1636	0.20 (1)
B-C	0 / 0	-84.9	-84.9	0.01 (1)	10.00		B-D	-1690 / 0	0.26 (1)
D-C	-44 / 0	0.0	0.0	0.01 (1)	7.81		A-E	0 / 372	0.05 (1)
F-A	-523 / 0	0.0	0.0	0.03 (1)	7.81				
F-G	0 / 0	-18.5	-18.5	0.37 (1)	10.00				
G-H	0 / 0	-18.5	-18.5	0.37 (1)	10.00				
H-I	0 / 0	-18.5	-18.5	0.37 (1)	10.00				
I-E	0 / 0	-18.5	-18.5	0.37 (1)	10.00				
E-D	0 / 404	-18.5	-18.5	0.24 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	5-4	-170	-170	---	TOP	VERT	TOTAL	---	C1
H	2-5-4	-608	-608	---	BACK	VERT	TOTAL	---	C1
I	4-5-4	-358	-358	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.19/1.00 (A-B:1) , BC=0.37/1.00 (E-F:1) , WB=0.26/1.00 (B-D:1) , SSI=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (E) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425214	T29	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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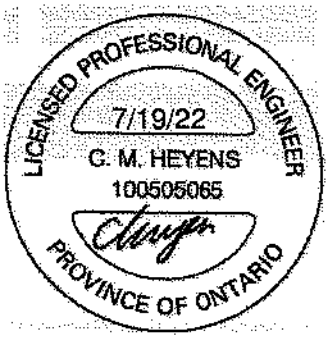
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW+m	MT20	5.0	6.0	2.50	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	3.0	8.0		
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY



DWG # TR22070198 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	T30	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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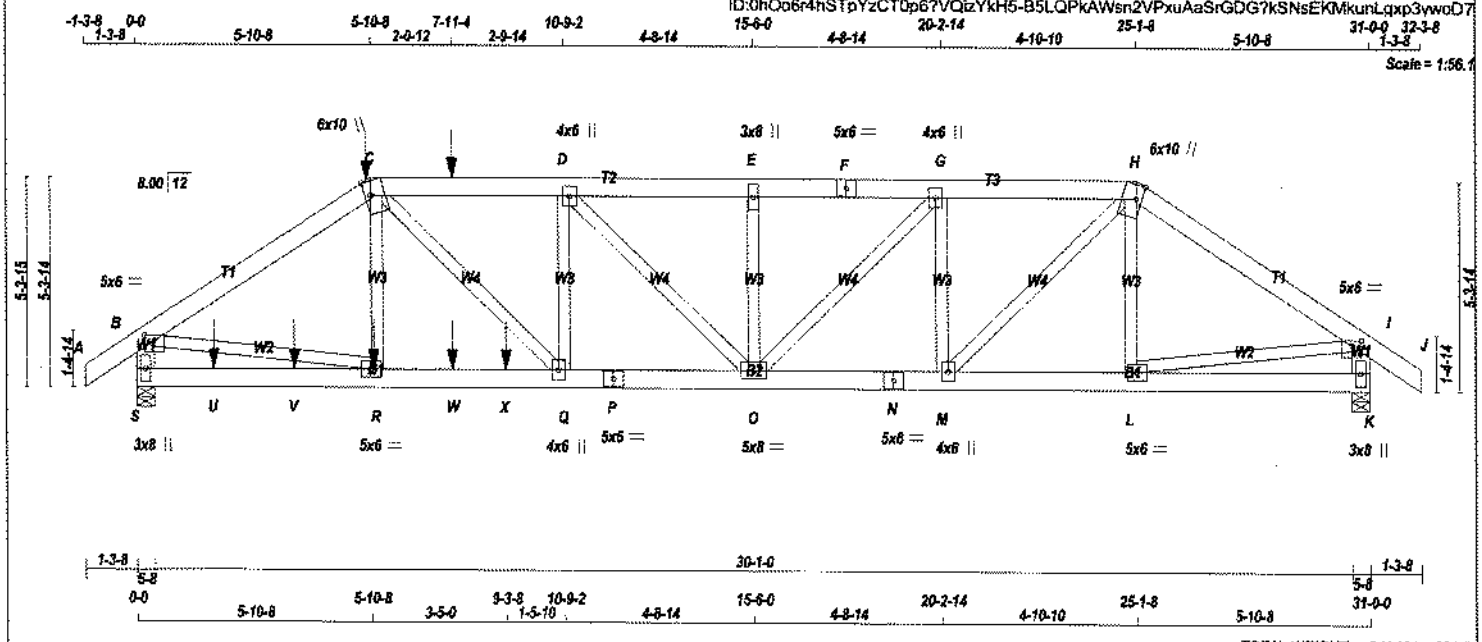


TOTAL WEIGHT = 2 X 191 = 381 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY- SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- F	2	12	SIDE(61.0)
F- H	2	12	TOP
H- J	2	12	TOP
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	SIDE(183.1)
P- N	2	12	TOP
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070205 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HOR2	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3764	0	3764	0	0	5-8	5-8
K	2472	0	2472	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2667	1720 / 0	0 / 0	0 / 0	0 / 0	947 / 0	0 / 0
K	1751	1135 / 0	0 / 0	0 / 0	0 / 0	816 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 33	-84.9	-84.9	0.03 (1)	10.00	R- C	0 / 241	0.03 (4)
B- C	-4956 / 0	-84.9	-84.9	0.19 (1)	5.07	C- Q	0 / 2103	0.19 (1)
C- T	-5617 / 0	-84.9	-84.9	0.19 (1)	4.81	Q- D	0 / 210	0.02 (4)
T- D	-5617 / 0	-84.9	-84.9	0.19 (1)	4.81	D- O	-1006 / 0	0.29 (1)
D- E	-4915 / 0	-84.9	-84.9	0.11 (1)	5.16	O- E	-366 / 0	0.05 (1)
E- F	-4915 / 0	-84.9	-84.9	0.10 (1)	5.18	O- G	0 / 1293	0.11 (1)
F- G	-4915 / 0	-84.9	-84.9	0.10 (1)	5.18	M- G	-1363 / 0	0.19 (1)
G- H	-4013 / 0	-84.9	-84.9	0.09 (1)	5.60	M- H	0 / 2140	0.18 (1)
H- I	-3012 / 0	-84.9	-84.9	0.16 (1)	6.15	L- H	-272 / 0	0.04 (1)
I- J	0 / 33	-84.9	-84.9	0.03 (1)	10.00	B- R	0 / 4163	0.37 (1)
S- B	-3748 / 0	0.0	0.0	0.13 (1)	7.29	L- I	0 / 2529	0.22 (1)
K- I	-2422 / 0	0.0	0.0	0.09 (1)	7.81			
S- U	0 / 0	-18.5	-18.5	0.17 (1)	10.00			
U- V	0 / 0	-18.5	-18.5	0.17 (1)	10.00			
V- R	0 / 0	-18.5	-18.5	0.17 (1)	10.00			
R- W	0 / 4124	-18.5	-18.5	0.78 (1)	10.00			
W- X	0 / 4124	-18.5	-18.5	0.78 (1)	10.00			
X- Q	0 / 4124	-18.5	-18.5	0.78 (1)	10.00			
Q- P	0 / 5617	-18.5	-18.5	0.63 (1)	10.00			
P- O	0 / 5617	-18.5	-18.5	0.63 (1)	10.00			
O- N	0 / 4013	-18.5	-18.5	0.34 (1)	10.00			
N- M	0 / 4013	-18.5	-18.5	0.34 (1)	10.00			
M- L	0 / 2493	-18.5	-18.5	0.18 (1)	10.00			
L- K	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-376	-376	—	BACK	VERT	TOTAL	—	C1
R	5-11-4	-29	-29	—	BACK	VERT	TOTAL	—	C1
T	7-11-4	-113	-113	—	BACK	VERT	TOTAL	—	C1
U	1-11-4	-29	-29	—	BACK	VERT	TOTAL	—	C1
V	3-11-4	-29	-29	—	BACK	VERT	TOTAL	—	C1
W	7-11-4	-29	-29	—	BACK	VERT	TOTAL	—	C1
X	9-3-8	-1373	-1373	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.19/1.00 (C-D:1) , BC=0.78/1.00 (Q-R:1) , WB=0.37/1.00 (B-R:1) , SSI=0.46/1.00 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (R) (INPUT = 0.90)
JSI METAL= 0.68 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	6.0	10.0	Edge	1.75
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	10.0	Edge	1.75
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

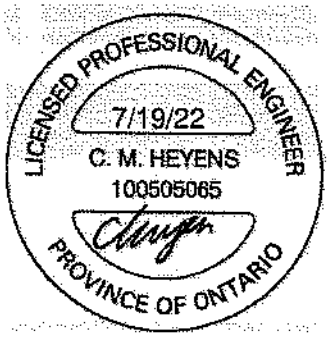
NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

STRUCTURAL COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	T31	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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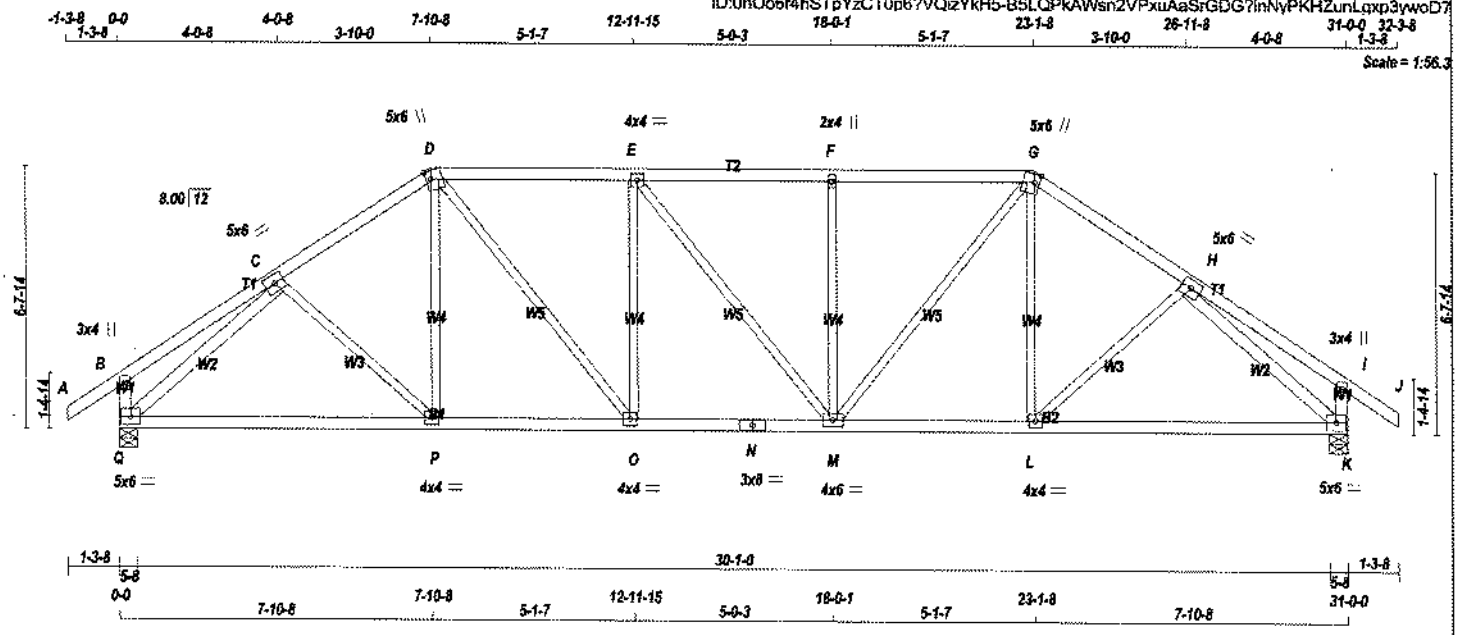

TOTAL WEIGHT = 2 X 139 = 278 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.50	1.50
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L, O, P						
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
Q	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070206

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	1719	0	1719	0	0	5-8	5-8
K	1719	0	1719	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C- P	0 / 53	0.02 (4)
B- C	0 / 20	-84.9	-84.9	0.20 (1)	10.00	P- D	0 / 149	0.05 (4)
C- D	-1822 / 0	-84.9	-84.9	0.20 (1)	4.80	D- O	0 / 696	0.16 (1)
D- E	-1936 / 0	-84.9	-84.9	0.30 (1)	4.57	O- E	-465 / 0	0.34 (1)
E- F	-1935 / 0	-84.9	-84.9	0.27 (1)	4.60	E- M	-2 / 0	0.00 (1)
F- G	-1935 / 0	-84.9	-84.9	0.30 (1)	4.57	M- F	-464 / 0	0.34 (1)
G- H	-1822 / 0	-84.9	-84.9	0.20 (1)	4.80	M- G	0 / 694	0.16 (1)
H- I	0 / 20	-84.9	-84.9	0.20 (1)	10.00	L- G	0 / 150	0.05 (4)
I- J	0 / 32	-84.9	-84.9	0.11 (1)	10.00	L- H	0 / 53	0.02 (4)
Q- B	-246 / 0	0.0	0.0	0.03 (1)	7.81	Q- C	-2057 / 0	0.70 (1)
K- I	-246 / 0	0.0	0.0	0.03 (1)	7.81	H- K	-2057 / 0	0.70 (1)
Q- P	0 / 1497	-18.5	-18.5	0.38 (1)	10.00			
P- O	0 / 1499	-18.5	-18.5	0.38 (4)	10.00			
O- N	0 / 1936	-18.5	-18.5	0.37 (1)	10.00			
N- M	0 / 1936	-18.5	-18.5	0.37 (1)	10.00			
M- L	0 / 1499	-18.5	-18.5	0.39 (4)	10.00			
L- K	0 / 1497	-18.5	-18.5	0.38 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = **24.0** **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.30/1.00 (F-G:1) , BC=0.39/1.00 (L-M:4) , WB=0.70/1.00 (H-K:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (M) (INPUT = 0.90)
JSI METAL= 0.64 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	T32	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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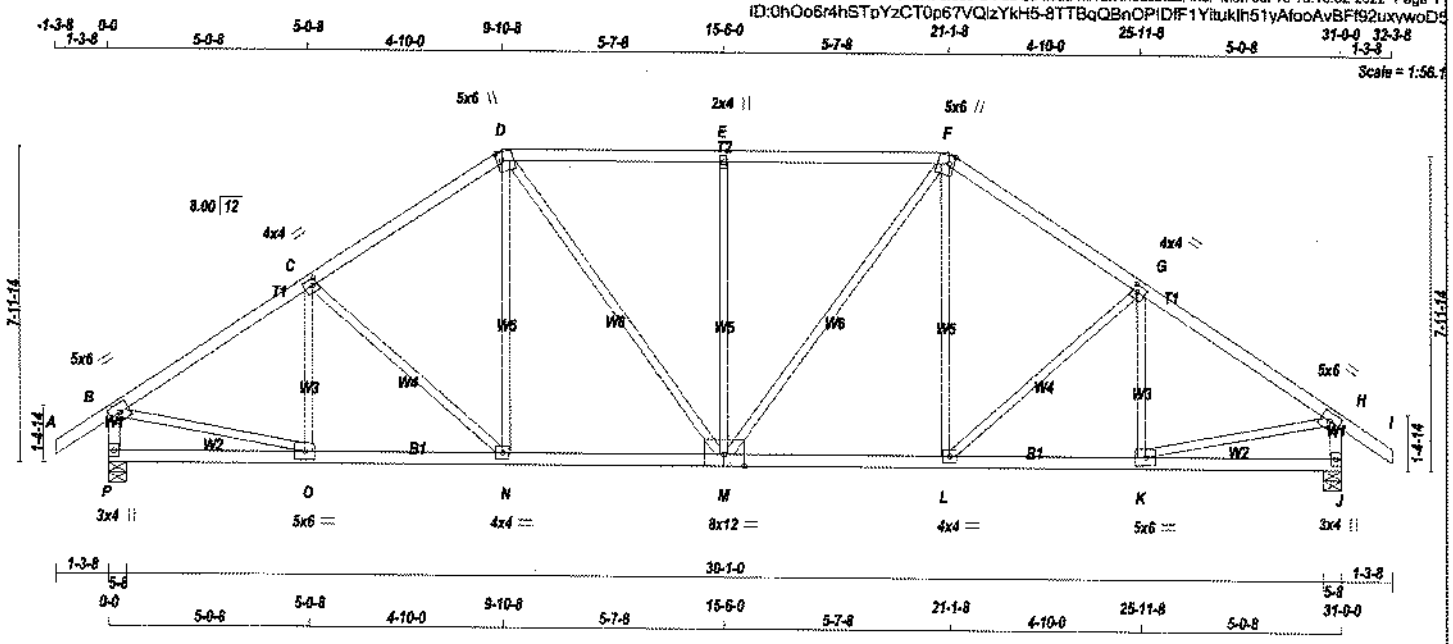

TOTAL WEIGHT = 2 X 139 = 278 lb (MiTek)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BSWWW-I	MT20	8.0	12.0	3.50	6.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070207

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1719	0	1719	0	0	5-8	5-8
J	1719	0	1719	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00
B-C	-1675 / 0	-84.9	-84.9	1	0.32 (1)	4.61
C-D	-1722 / 0	-84.9	-84.9	1	0.30 (1)	4.78
D-E	-1655 / 0	-84.9	-84.9	1	0.38 (1)	4.74
E-F	-1655 / 0	-84.9	-84.9	1	0.38 (1)	4.74
F-G	-1722 / 0	-84.9	-84.9	1	0.30 (1)	4.78
G-H	-1675 / 0	-84.9	-84.9	1	0.32 (1)	4.61
H-I	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00
P-B	-1677 / 0	0.0	0.0	1	0.17 (1)	6.41
J-H	-1677 / 0	0.0	0.0	1	0.17 (1)	6.41
P-O	0 / 0	-18.5	-18.5	4	0.10 (4)	10.00
O-N	0 / 1582	-18.5	-18.5	1	0.33 (1)	10.00
N-M	0 / 1411	-18.5	-18.5	1	0.30 (1)	10.00
M-L	0 / 1411	-18.5	-18.5	1	0.30 (1)	10.00
L-K	0 / 1582	-18.5	-18.5	1	0.33 (1)	10.00
K-J	0 / 0	-18.5	-18.5	4	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-248 / 0	0.08 (1)
C-N	-235 / 0	0.19 (1)
N-D	0 / 269	0.06 (1)
D-M	0 / 413	0.09 (1)
M-E	-583 / 0	0.71 (1)
M-F	0 / 413	0.09 (1)
L-F	0 / 269	0.06 (1)
L-G	-235 / 0	0.19 (1)
K-G	-248 / 0	0.08 (1)
B-O	0 / 1618	0.36 (1)
K-H	0 / 1618	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.33/1.00 (N-O:1) , WB=0.71/1.00 (E-M:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (H) (INPUT = 0.90)
JSI METAL= 0.47 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	T33	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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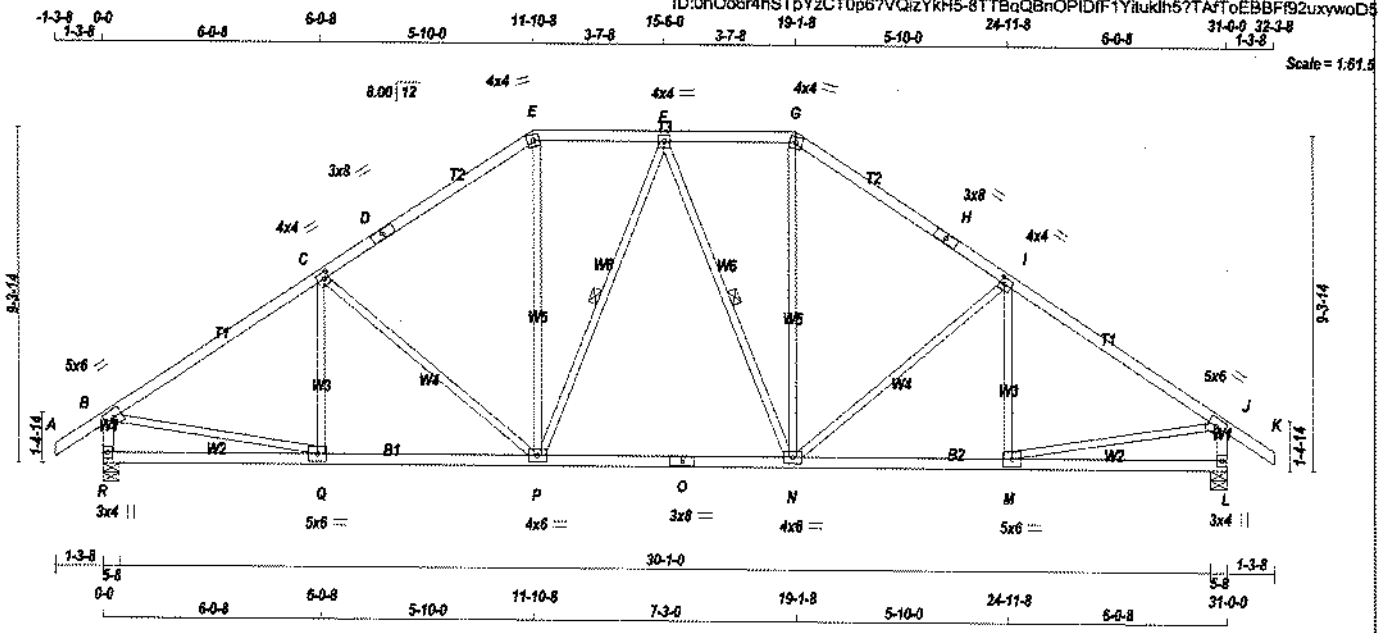

TOTAL WEIGHT = 3 X 141 = 424 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070208

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1719	0	1719	0	0	5-8	5-8
L	1719	0	1719	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P, F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B- C	-1896 / 0	-84.9	-84.9	0.48 (1)	4.41	
C- D	-1605 / 0	-84.9	-84.9	0.44 (1)	4.74	
D- E	-1605 / 0	-84.9	-84.9	0.44 (1)	4.74	
E- F	-1312 / 0	-84.9	-84.9	0.15 (1)	5.51	
F- G	-1312 / 0	-84.9	-84.9	0.15 (1)	5.51	
G- H	-1605 / 0	-84.9	-84.9	0.44 (1)	4.74	
H- I	-1605 / 0	-84.9	-84.9	0.44 (1)	4.74	
I- J	-1896 / 0	-84.9	-84.9	0.48 (1)	4.41	
J- K	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
R- B	-1671 / 0	0.0	0.0	0.17 (1)	6.42	
L- J	-1671 / 0	0.0	0.0	0.17 (1)	6.42	
R- Q	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
Q- P	0 / 1605	-18.5	-18.5	0.35 (1)	10.00	
P- O	0 / 1383	-18.5	-18.5	0.31 (1)	10.00	
O- N	0 / 1383	-18.5	-18.5	0.31 (1)	10.00	
N- M	0 / 1605	-18.5	-18.5	0.35 (1)	10.00	
M- L	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-172 / 30	0.08 (1)
C- P	-395 / 0	0.50 (1)
P- E	0 / 562	0.13 (1)
P- F	-195 / 0	0.13 (1)
F- N	-195 / 0	0.13 (1)
N- G	0 / 562	0.13 (1)
N- I	-395 / 0	0.50 (1)
M- I	-172 / 30	0.08 (1)
B- Q	0 / 1631	0.37 (1)
M- J	0 / 1631	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = **24.0** **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.35/1.00 (M-N:1) , WB=0.50/1.00 (C-P:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.51 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	T34	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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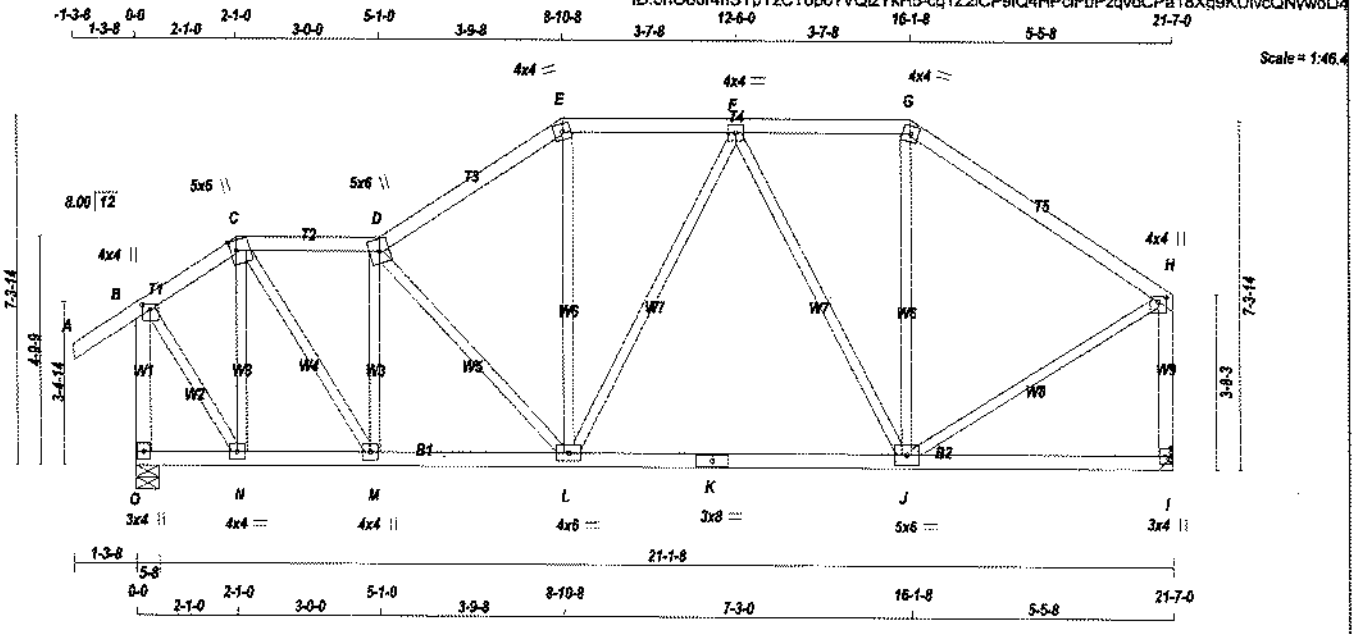

TOTAL WEIGHT = 108 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TTWW+m	MT20	5.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW+t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070209

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
O	1232	0	1232	0	0	5-8	5-8
I	1115	0	1115	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	872	567 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0
I	792	503 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	N-C	-623 / 0		0.21 (1)
B-C	-588 / 0	-84.9	-84.9	0.05 (1)	6.25	C-M	0 / 875		0.20 (1)
C-D	-947 / 0	-84.9	-84.9	0.11 (1)	6.25	M-O	-699 / 0		0.24 (1)
D-E	-1005 / 0	-84.9	-84.9	0.17 (1)	6.05	D-L	-195 / 0		0.12 (1)
E-F	-840 / 0	-84.9	-84.9	0.14 (1)	6.25	L-E	0 / 275		0.06 (1)
F-G	-685 / 0	-84.9	-84.9	0.14 (1)	6.25	L-F	-13 / 28		0.02 (1)
G-H	-799 / 0	-84.9	-84.9	0.34 (1)	6.25	F-J	-404 / 0		0.52 (1)
O-B	-1221 / 0	0.0	0.0	0.24 (1)	7.25	J-G	0 / 96		0.03 (4)
I-H	-1077 / 0	0.0	0.0	0.24 (1)	7.60	B-N	0 / 843		0.19 (1)
						J-H	0 / 776		0.17 (1)
O-N	0 / 0	-18.5	-18.5	0.03 (4)	10.00				
N-M	0 / 473	-18.5	-18.5	0.10 (1)	10.00				
M-L	0 / 963	-18.5	-18.5	0.23 (1)	10.00				
L-K	0 / 846	-18.5	-18.5	0.26 (4)	10.00				
K-J	0 / 846	-18.5	-18.5	0.26 (4)	10.00				
J-I	0 / 0	-18.5	-18.5	0.18 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = **24.0** **IN. C/C**

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.34/1.00 (G-H:1) , BC=0.26/1.00 (J-L:4) , WB=0.52/1.00 (F-J:1) , SSI=0.15/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.36 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	T35	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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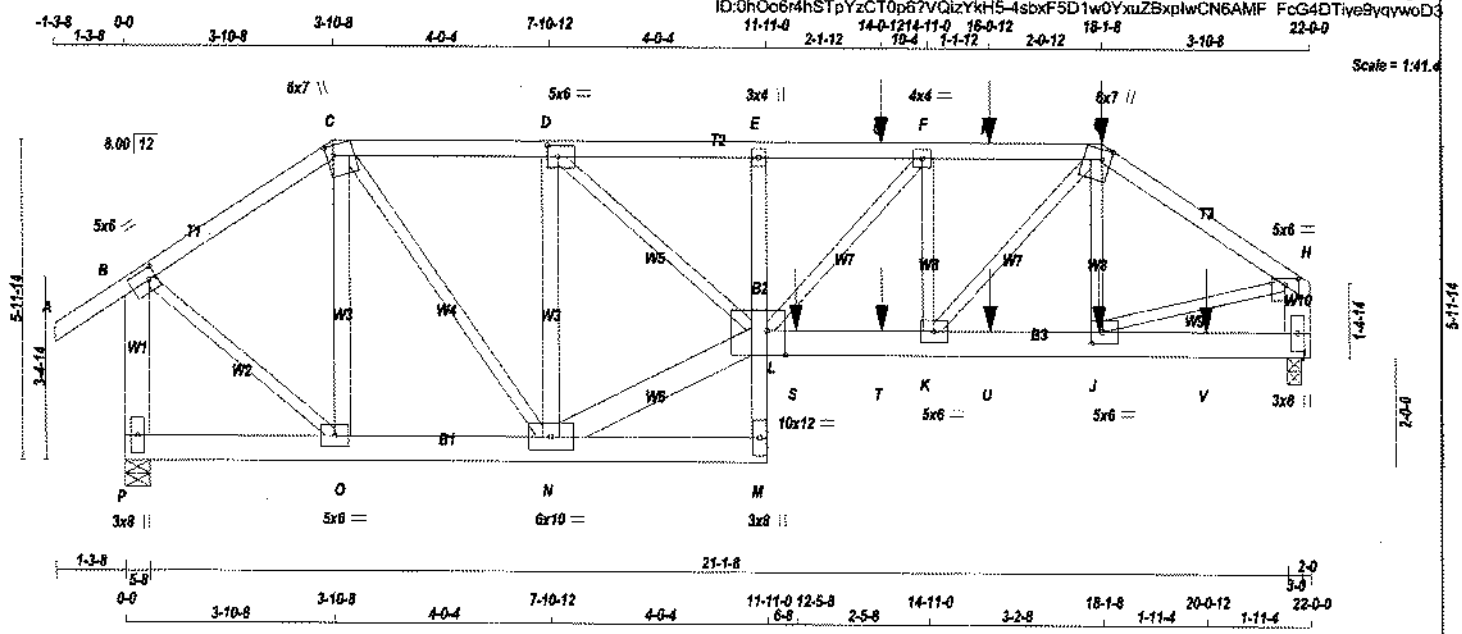

TOTAL WEIGHT = 129 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
N - D	2x4	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW+m	MT20	6.0	7.0	2.25	1.50
D	TMWW-t	MT20	5.0	6.0	2.50	2.25
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	2.25	2.00
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
I	BMV1+p	MT20	3.0	8.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.50
K	BMWW-t	MT20	5.0	6.0		
L	BVMWWW-l	MT20	10.0	12.0	Edge	4.00
M	BMV+p	MT20	3.0	8.0		
N	BMWWW-t	MT20	6.0	10.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070210 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1763	0	1763	0	0	5-8	5-8
I	2040	0	2040	0	0	3-0	3-0

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1248	819 / 0	0 / 0	0 / 0	0 / 0	427 / 0	0 / 0
I	1444	939 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.13 (1)	10.00	O-C	-874 / 0	0.26 (1)
B-C	-1237 / 0	-84.9	-84.9	0.27 (1)	5.41	C-N	0 / 1300	0.32 (1)
C-D	-1768 / 0	-84.9	-84.9	0.27 (1)	4.71	N-D	-1889 / 0	0.72 (1)
D-E	-3374 / 0	-84.9	-84.9	0.39 (1)	3.44	N-L	0 / 1956	0.25 (1)
E-Q	-3391 / 0	-84.9	-84.9	0.34 (1)	3.48	D-L	0 / 2197	0.54 (1)
Q-F	-3391 / 0	-84.9	-84.9	0.34 (1)	3.48	L-F	0 / 554	0.14 (1)
F-R	-3027 / 0	-84.9	-84.9	0.30 (1)	3.70	K-F	-842 / 0	0.21 (1)
R-G	-3027 / 0	-84.9	-84.9	0.30 (1)	3.70	K-G	0 / 1691	0.39 (1)
G-H	-2385 / 0	-84.9	-84.9	0.35 (1)	4.08	J-G	-438 / 0	0.11 (1)
P-B	-1736 / 0	0.0	0.0	0.22 (1)	7.50	B-O	0 / 1282	0.32 (1)
I-H	-1991 / 0	0.0	0.0	0.14 (1)	7.11	J-H	0 / 2043	0.51 (1)
P-O	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
O-N	0 / 1010	-18.5	-18.5	0.16 (1)	10.00			
N-M	0 / 29	-18.5	-18.5	0.03 (4)	10.00			
M-L	0 / 34	0.0	0.0	0.09 (1)	10.00			
L-E	-269 / 0	0.0	0.0	0.09 (1)	7.81			
L-S	0 / 3027	-18.5	-18.5	0.75 (1)	10.00			
S-T	0 / 3027	-18.5	-18.5	0.75 (1)	10.00			
T-K	0 / 3027	-18.5	-18.5	0.75 (1)	10.00			
K-U	0 / 1968	-18.5	-18.5	0.33 (1)	10.00			
U-J	0 / 1968	-18.5	-18.5	0.33 (1)	10.00			
J-V	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
V-I	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	18-1-8	-188	-188	—	FRONT	VERT	TOTAL	—	C1
J	18-0-12	-14	-14	—	FRONT	VERT	TOTAL	—	C1
Q	14-0-12	-55	-55	—	FRONT	VERT	TOTAL	—	C1
R	16-0-12	-55	-55	—	FRONT	VERT	TOTAL	—	C1
S	12-5-8	-642	-642	—	FRONT	VERT	TOTAL	—	C1
T	14-0-12	-14	-14	—	FRONT	VERT	TOTAL	—	C1
U	16-0-12	-14	-14	—	FRONT	VERT	TOTAL	—	C1
V	20-0-12	-14	-14	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.39/1.00 (D-E:1) , BC=0.75/1.00 (K-L:1) , WB=0.72/1.00 (D-N:1) , SSI=0.48/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.45 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425216	T35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

STRUCTURAL COMPONENT ONLY

DWG # TR22070210

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	T36	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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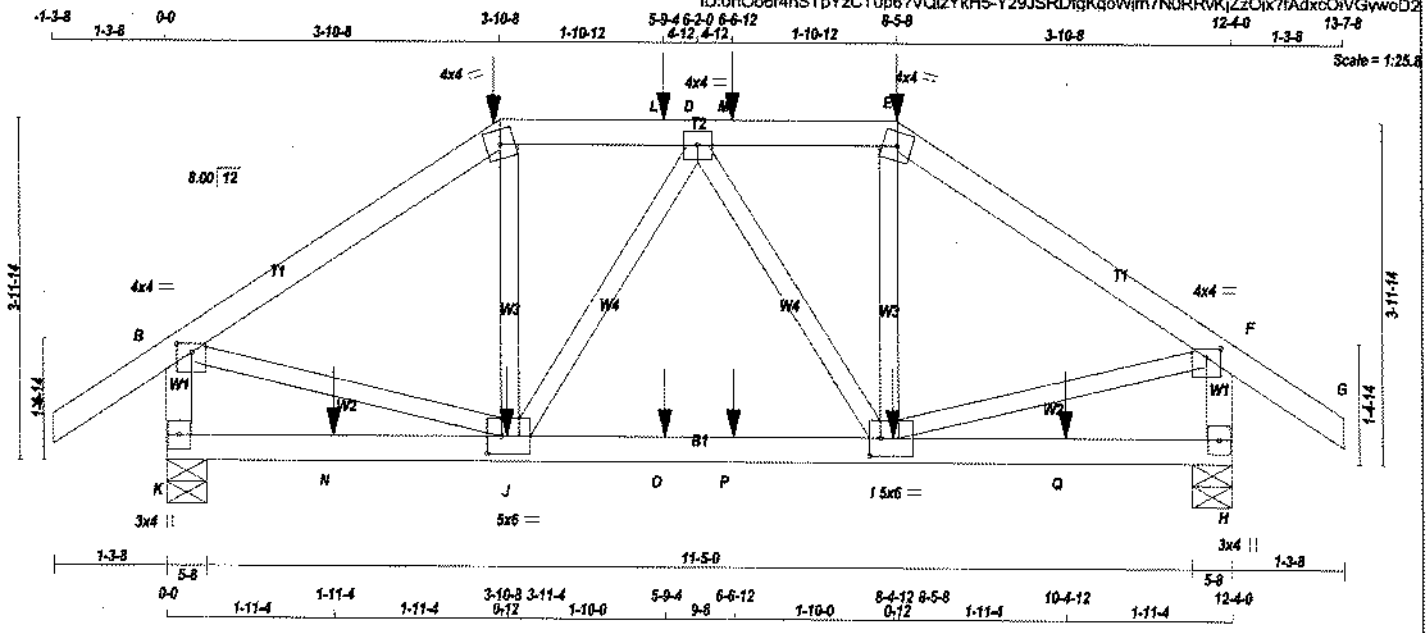

TOTAL WEIGHT = 54 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	6.0	2.50	1.50
J	BMWWW-t	MT20	5.0	6.0	2.50	2.00
K	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070211

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1157	0	1157	0	0	5-8	5-8
H	1157	0	1157	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	817	540 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0
H	817	540 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.13 (1)	10.00
B-C	-1076 / 0	-84.9	-84.9	0.27 (1)	5.72
C-L	-896 / 0	-84.9	-84.9	0.11 (1)	6.25
L-D	-896 / 0	-84.9	-84.9	0.11 (1)	6.25
D-M	-896 / 0	-84.9	-84.9	0.11 (1)	6.25
M-E	-896 / 0	-84.9	-84.9	0.11 (1)	6.25
E-F	-1076 / 0	-84.9	-84.9	0.27 (1)	5.72
F-G	0 / 32	-84.9	-84.9	0.13 (1)	10.00
K-B	-1120 / 0	0.0	0.0	0.13 (1)	7.46
H-F	-1120 / 0	0.0	0.0	0.13 (1)	7.46
K-N	0 / 0	-18.5	-18.5	0.12 (4)	10.00
N-J	0 / 0	-18.5	-18.5	0.12 (4)	10.00
J-O	0 / 1004	-18.5	-18.5	0.23 (1)	10.00
O-P	0 / 1004	-18.5	-18.5	0.23 (1)	10.00
P-I	0 / 1004	-18.5	-18.5	0.23 (1)	10.00
I-Q	0 / 0	-18.5	-18.5	0.12 (4)	10.00
Q-H	0 / 0	-18.5	-18.5	0.12 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	0 / 98	0.04 (4)
J-D	-215 / 0	0.07 (1)
D-I	-215 / 0	0.07 (1)
I-E	0 / 98	0.04 (4)
B-J	0 / 925	0.23 (1)
I-F	0 / 925	0.23 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-188	-188	---	FRONT	VERT	TOTAL	---	C1
E	8-5-8	-188	-188	---	FRONT	VERT	TOTAL	---	C1
I	8-4-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
J	3-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
L	5-9-4	-55	-55	---	FRONT	VERT	TOTAL	---	C1
M	6-6-12	-55	-55	---	FRONT	VERT	TOTAL	---	C1
N	1-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
O	5-9-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
P	6-6-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
Q	10-4-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.23/1.00 (I-J:1) , WB=0.23/1.00 (B-J:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	T37	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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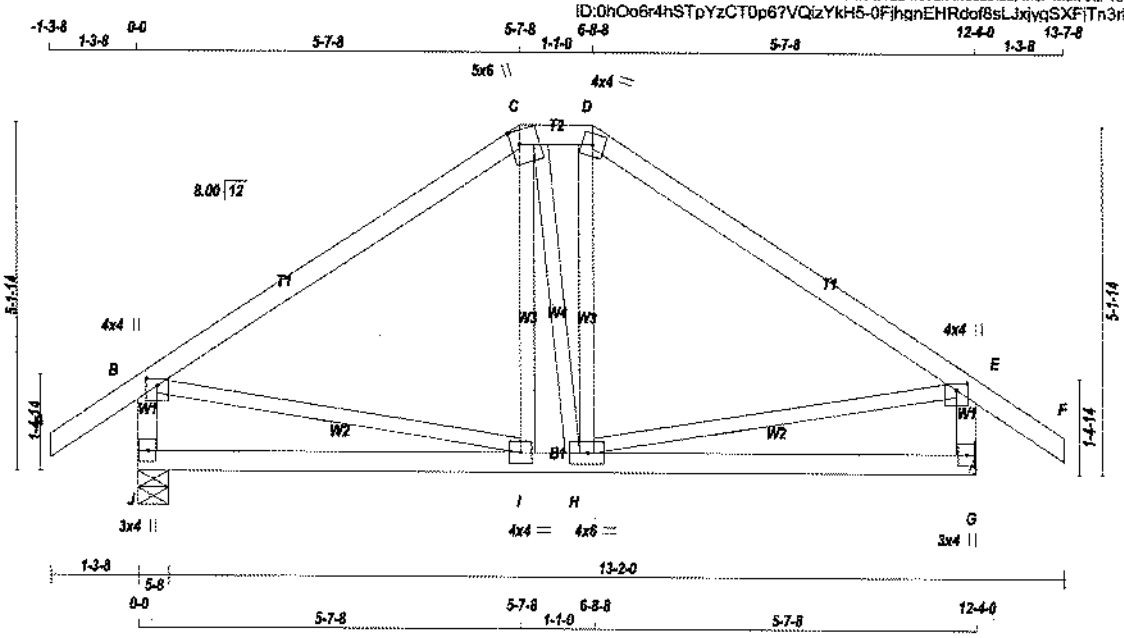

TOTAL WEIGHT = 2 X 57 = 114 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
J	754	0	754	0	0	5-8	5-8
G	754	0	754	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	533	351 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
G	533	351 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	I- C	-11 / 42	0.01 (4)	
B- C	-499 / 0	-84.9	-84.9	0.35 (1)	6.25	C- H	0 / 8	0.00 (4)	
C- D	-415 / 0	-84.9	-84.9	0.01 (1)	6.25	H- D	-7 / 50	0.02 (4)	
D- E	-500 / 0	-84.9	-84.9	0.35 (1)	6.25	B- I	0 / 422	0.09 (1)	
E- F	0 / 32	-84.9	-84.9	0.11 (1)	10.00	H- E	0 / 422	0.10 (1)	
J- B	-711 / 0	0.0	0.0	0.07 (1)	7.81				
G- E	-710 / 0	0.0	0.0	0.07 (1)	7.81				
J- I	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
I- H	0 / 414	-18.5	-18.5	0.19 (4)	10.00				
H- G	0 / 0	-18.5	-18.5	0.15 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.35/1.00 (D-E:1) , BC=0.19/1.00 (H-I:4) , WB=0.10/1.00 (E-H:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (H) (INPUT = 0.90)
JSI METAL= 0.16 (E) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070212

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	T38	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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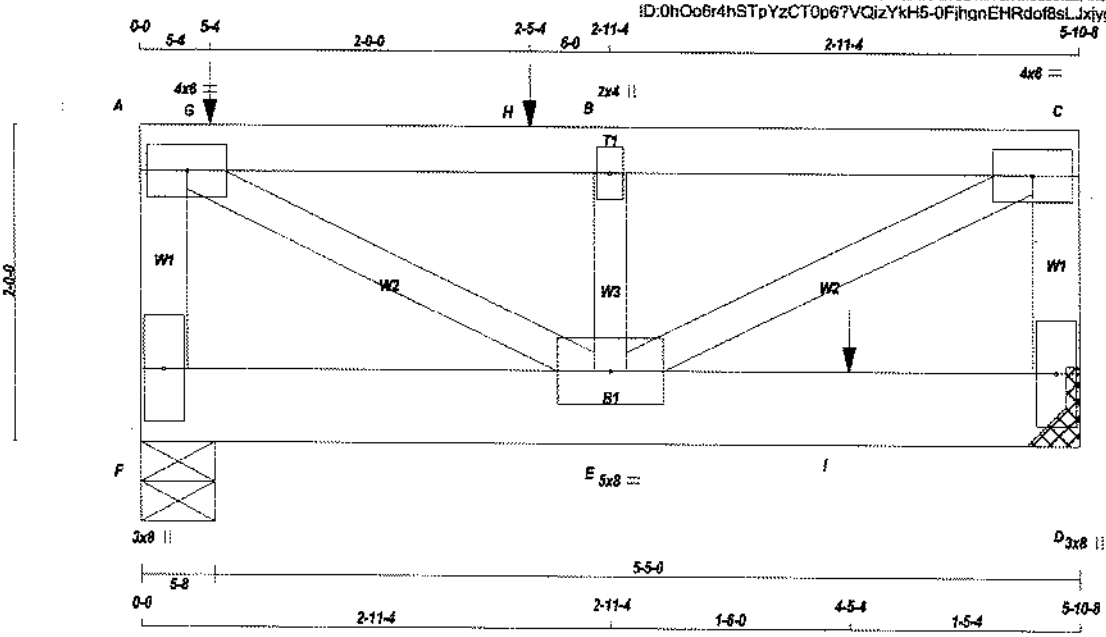

Scale = 1:14.0

TOTAL WEIGHT = 2 X 26 = 51 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	SIDE(0.0)
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-I	MT20	4.0	6.0	
B	TMW-Hw	MT20	2.0	4.0	
C	TMVW-I	MT20	4.0	6.0	

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070213 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1907	0	1907	0	0	5-8	5-8
D	1958	0	1958	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1350	878 / 0	0 / 0	0 / 0	0 / 0	472 / 0	0 / 0
D	1388	894 / 0	0 / 0	0 / 0	0 / 0	493 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1850 / 0	0.0	0.0	0.11 (1)	7.81	A- E	0 / 2778	0.34 (1)
A- G	-2431 / 0	-84.9	-84.9	0.45 (1)	5.11	E- B	-2157 / 0	0.17 (1)
G- H	-2431 / 0	-84.9	-84.9	0.45 (1)	5.11	E- C	0 / 2778	0.34 (1)
H- B	-2431 / 0	-84.9	-84.9	0.45 (1)	5.11			
B- C	-2431 / 0	-84.9	-84.9	0.30 (1)	5.38			
D- C	-1342 / 0	0.0	0.0	0.08 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.03 (1)	10.00			
E- I	0 / 0	-18.5	-18.5	0.32 (1)	10.00			
I- D	0 / 0	-18.5	-18.5	0.32 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	5-4	-144	-144	---	TOP	VERT	TOTAL	---	C1
H	2-5-4	-1385	-1385	---	BACK	VERT	TOTAL	---	C1
I	4-5-4	-777	-777	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.45/1.00 (A-B:1) , BC=0.32/1.00 (D-E:1) , WB=0.34/1.00 (A-E:1) , SSI=0.82/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.30 (A) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425216	T38	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMV1+p	MT20	3.0	8.0		
E	BMWWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070213 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425216	T39	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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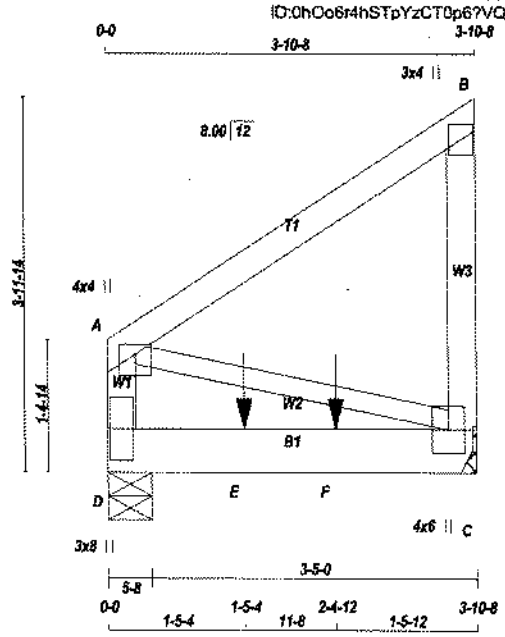


Scale = 1:23.5

TOTAL WEIGHT = 2 X 20 = 40 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	TOP
D- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

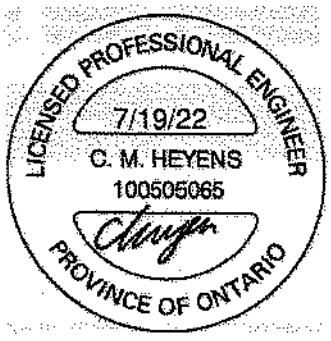
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		

STRUCTURAL COMPONENT ONLY



DWG # TR22070214 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
C	928	0	928	0	0	MECHANICAL	
D	943	0	943	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	655	438 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0
D	666	445 / 0	0 / 0	0 / 0	0 / 0	221 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 0	-84.9	-84.9	0.12 (1)		10.00	A- C	0 / 0	0.00 (1)
C- B	-164 / 0	0.0	0.0	0.02 (1)		7.81			
D- A	-164 / 0	0.0	0.0	0.01 (1)		7.81			
D- E	0 / 0	-18.5	-18.5	0.41 (1)		10.00			
E- F	0 / 0	-18.5	-18.5	0.41 (1)		10.00			
F- C	0 / 0	-18.5	-18.5	0.41 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-5-4	-518	-518	---	BACK	VERT	TOTAL	---	C1
F	2-4-12	-518	-518	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.41/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425216	T39	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070214 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	J1	11	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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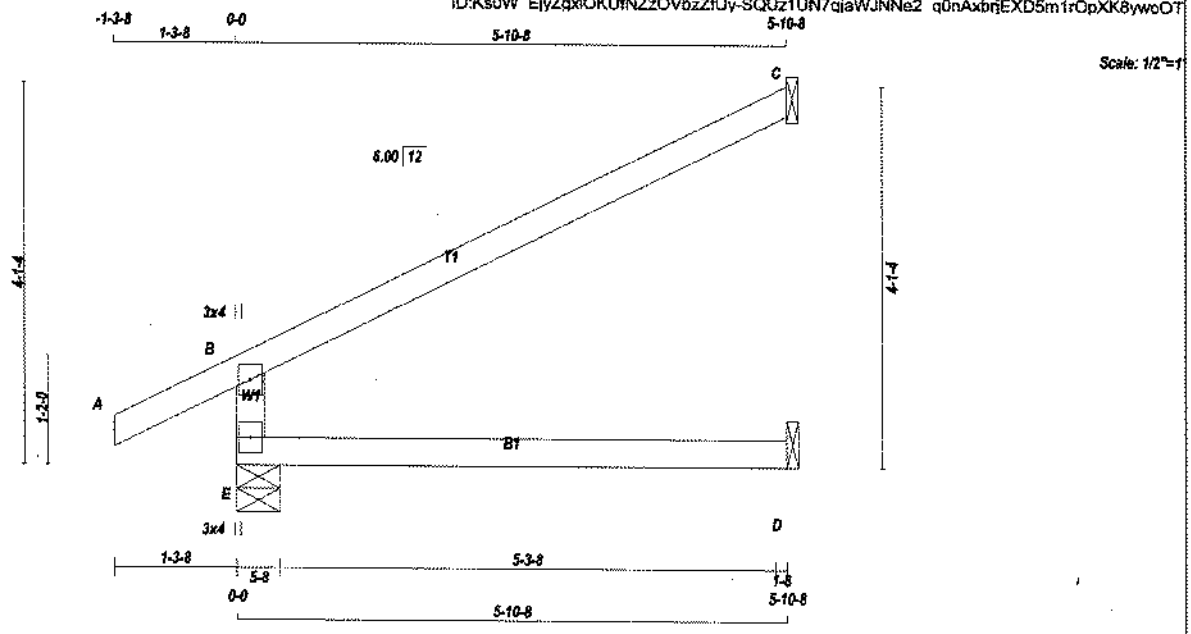

TOTAL WEIGHT = 11 X 17 = 185 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
E	490	0	490	0	0	5-8	5-8
C	187	0	187	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	346	234 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	129	103 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-427 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00			
B- C	-26 / 0	-84.9	-84.9	0.50 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070149

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425213	J1S	7	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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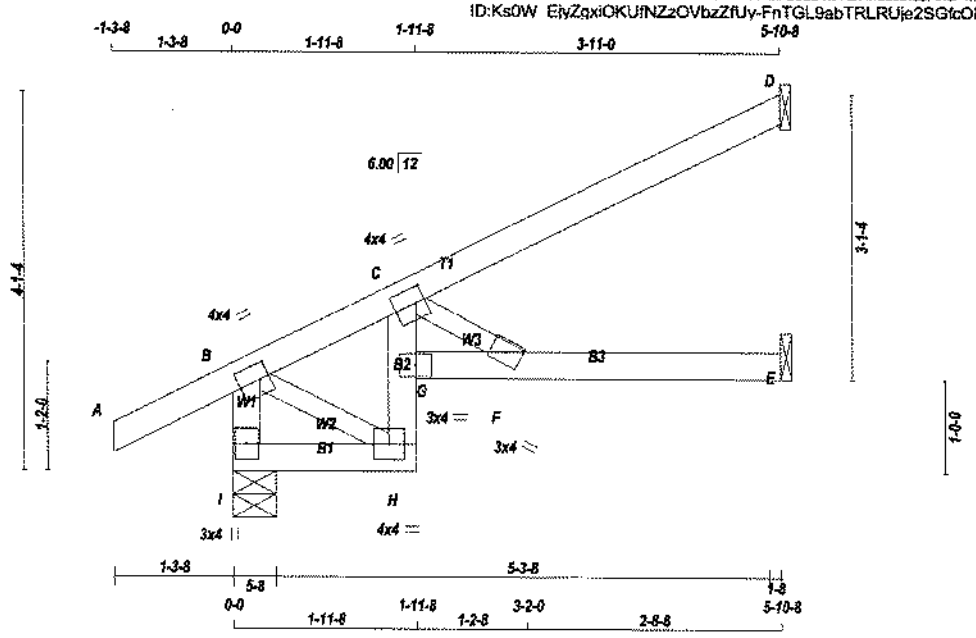

TOTAL WEIGHT = 7 X 21 = 146 lb

[M

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW+w	MT20	3.0	4.0		
G	BVM-I	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	432	0	432	0	0	5-8	5-8
D	180	0	180	0	0	1-8	1-8
E	111	0	111	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	304	207 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	125	94 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	81	38 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-415 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-215 / 0	-84.9	-84.9	0.11 (1)	6.25
C- D	0 / 4	-84.9	-84.9	0.26 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.02 (4)	10.00
H- G	-63 / 0	0.0	0.0	0.23 (1)	7.81
G- C	0 / 163	0.0	0.0	0.26 (1)	10.00
G- F	0 / 388	-18.5	-18.5	0.40 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.33 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- H	0 / 186	0.04 (1)
C- F	-468 / 0	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 594 (0.12")

CSI: TC=0.26/1.00 (C-D:1) , BC=0.40/1.00 (F-G:1) , WB=0.07/1.00 (C-F:1) , SSI=0.18/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (C) (INPUT = 0.90)
JSI METAL= 0.17 (F) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070177

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	J2	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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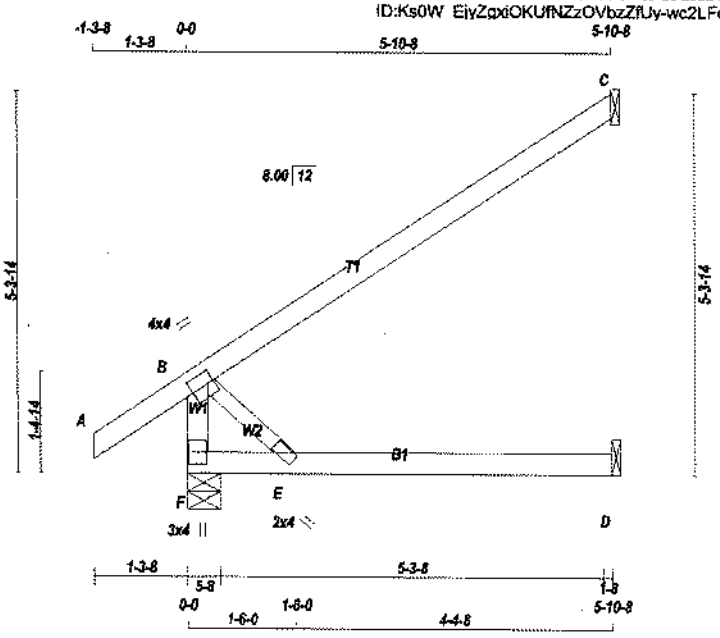

TOTAL WEIGHT = 19 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	420	0	420	0	0	5-8	5-8
C	249	0	249	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	296	201 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	172	137 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM (PLF)	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-366 / 0	0.0	0.0	0.04 (1)	7.81	
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B- C	0 / 0	-84.9	-84.9	0.50 (1)	10.00	
F- E	0 / 0	-18.5	-18.5	0.13 (4)	10.00	
E- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	786	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070150

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	J3	5	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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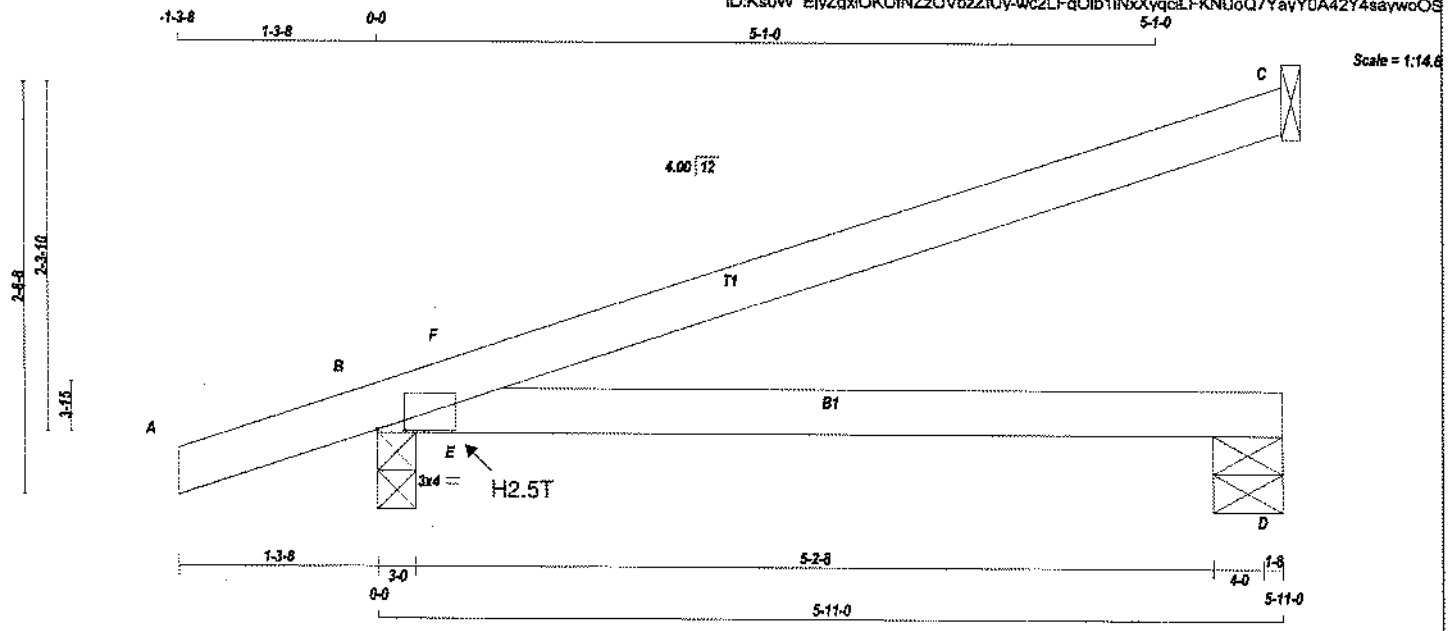

TOTAL WEIGHT = 5 X 16 = 78 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	0.25	2.00

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070151

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	220	0	220	0	-156	1-8	1-8
B	419	0	419	141	-273	3-0	3-0
D	86	0	86	0	-110	5-8	5-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 156 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 273 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 141 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	153	116 / 0	0 / 0	0 / 0	0 / -135	36 / 0	0 / 0
B	295	200 / 0	0 / 0	0 / 0	0 / -256	95 / 0	0 / 0
D	64	21 / 0	0 / 0	0 / 0	0 / -106	43 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.9	-84.9	0.11 (1)	10.00	E- F	-321 / 128	0.00 (1)
B- F	-104 / 19	-84.9	-84.9	0.06 (3)	6.25			
F- C	-58 / 2	-84.9	-84.9	0.38 (1)	6.25			
B- E	0 / 0	-18.5	-18.5	0.27 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.27 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (20-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 640 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 510 (0.14")

CSI: TC=0.38/1.00 (C-F:1) , BC=0.27/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.25/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425213	J4	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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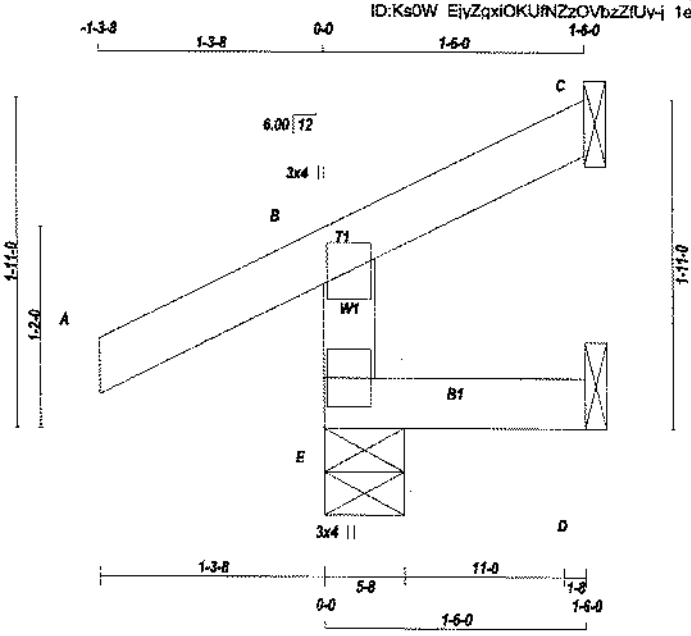

TOTAL WEIGHT = 3 X 6 = 19 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	245	0	245	0	0	5-8	5-8
C	23	0	23	0	-28	1-8	1-8
D	2	0	12	0	-6	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	170	127 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0
C	16	12 / -20	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	3	0 / -8	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-219 / 0	0.0	0.0	0.03 (5)	7.81			
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00			
B-C	-18 / 0	-84.9	-84.9	0.08 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070178

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	J5	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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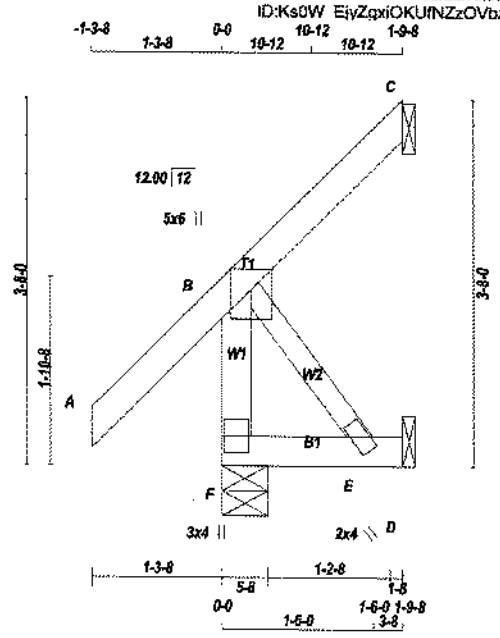

TOTAL WEIGHT = 4 X 11 = 42 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	257	0	257	0	0	5-8	5-8
C	30	0	30	0	-36	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	178	132 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	21	16 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-240 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 42	-84.9	-84.9	0.12 (1)	10.00			
B- C	-33 / 0	-84.9	-84.9	0.11 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070152

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	J6	7	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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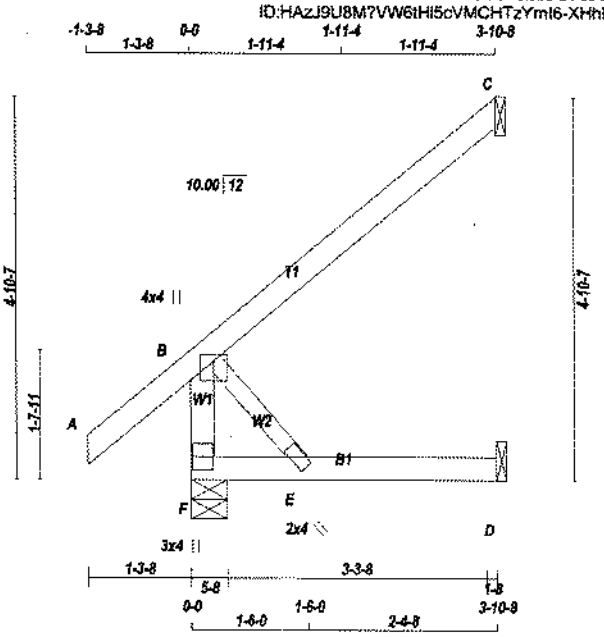

TOTAL WEIGHT = 7 X 15 = 106 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	318	0	318	0	0	5-8	5-8
C	164	0	164	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	223	155 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
C	113	90 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-282 / 0	0.0	0.0	0.03 (1)		7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.14 (5)		10.00			
B- C	0 / 0	-84.9	-84.9	0.22 (1)		10.00			
F- E	0 / 0	-18.5	-18.5	0.07 (4)		10.00			
E- D	0 / 0	-18.5	-18.5	0.08 (4)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF SCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070187

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
425216	J30	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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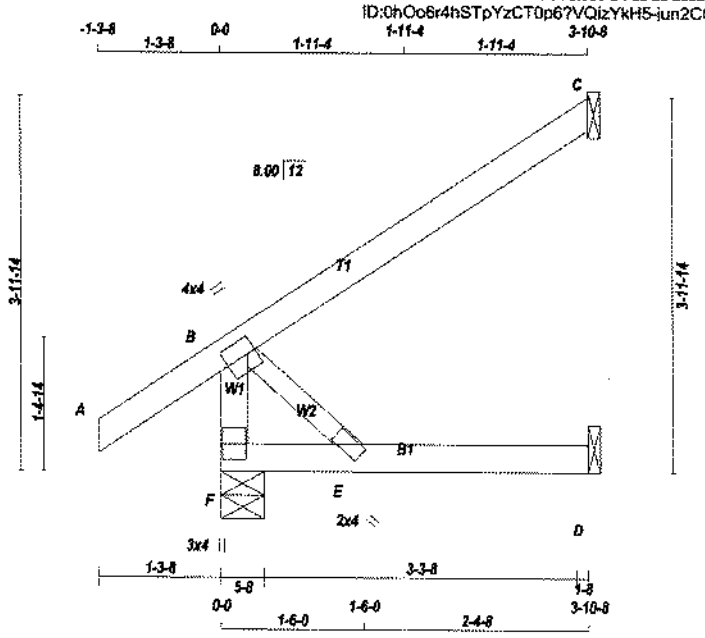

TOTAL WEIGHT = 7 X 14 = 98 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	317	0	317	0	0	5-8	5-8
C	164	0	164	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS. SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	223	154 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	113	90 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-281 / 0	0.0	0.0	0.03	(1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 32	-84.9	-84.9	0.13	(5)	10.00			
B-C	0 / 0	-84.9	-84.9	0.22	(1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.07	(4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.08	(4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070204

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	C1	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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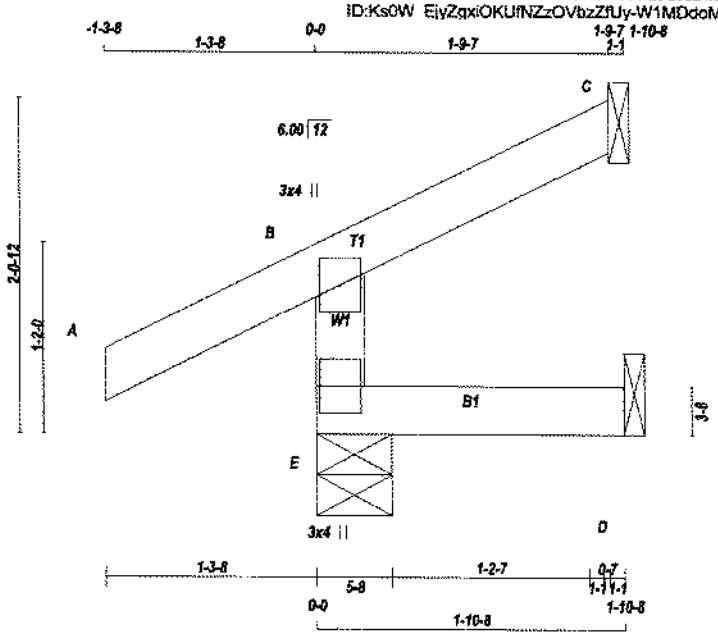

TOTAL WEIGHT = 2 X 7 = 14 lb

[M

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	252	0	252	0	0	5-8	5-8
C	41	0	41	0	-20	1-8	1-8
D	9	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	178	129 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	29	22 / -16	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	8	0 / -7	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-225 / 0	0.0	0.0	0.03 (5)	7.81			
A- B	0 / 26	-84.9	-84.9	0.11 (5)	10.00			
B- C	-15 / 0	-84.9	-84.9	0.08 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:5) , BC=0.03/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.06/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070145

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	C2	2	1	ROYAL PINE HOMES TRUSS DESC.	

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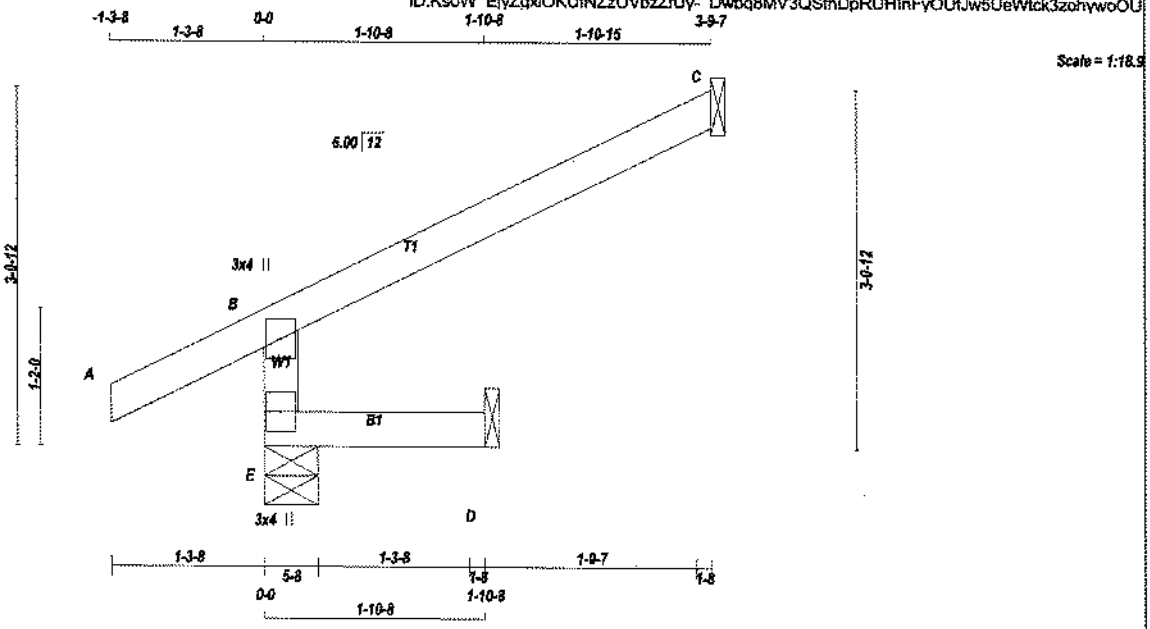

TOTAL WEIGHT = 2 X 10 = 19 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	335	0	335	0	0	5-8	5-8
C	121	0	121	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	233	173 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	83	66 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-316 / 0	0.0	0.0	0.01 (4)	7.81			
A- B	0 / 26	-84.9	-84.9	0.12 (5)	10.00			
B- C	-18 / 0	-84.9	-84.9	0.21 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	660	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070146

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	C3	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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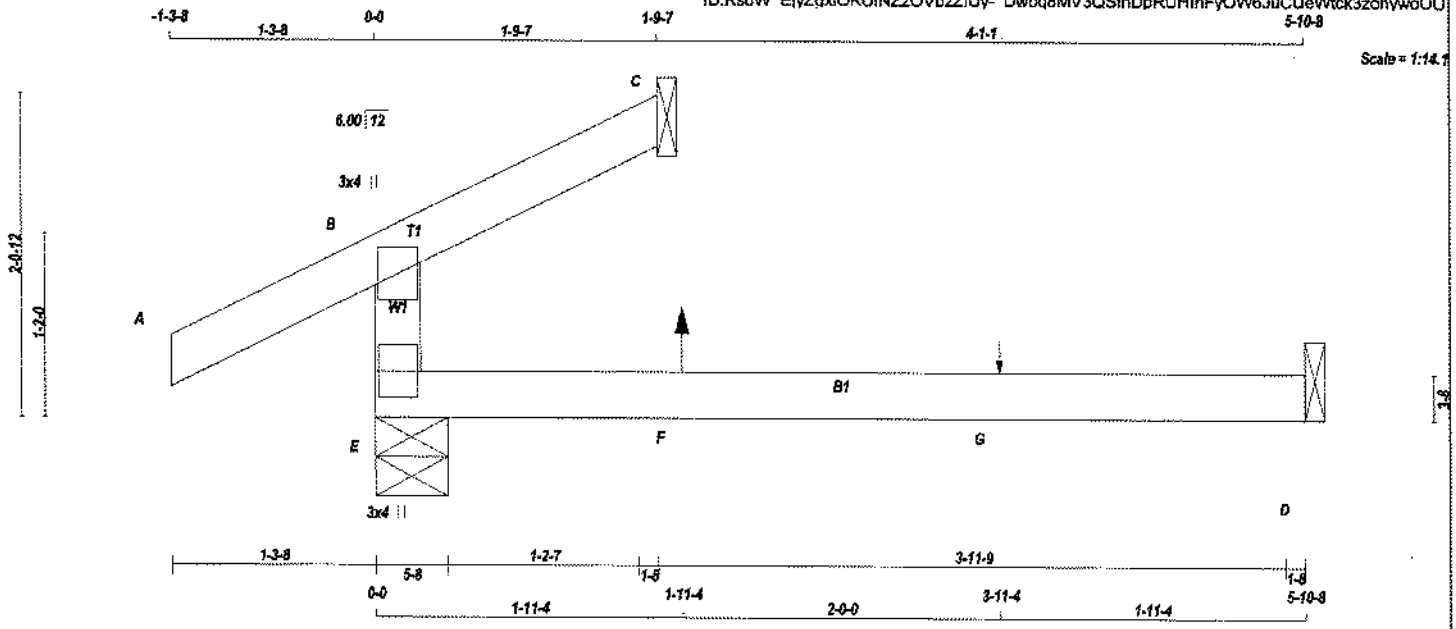

TOTAL WEIGHT = 2 X 12 = 23 lb

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	266	0	265	0	0	5-8	5-8
C	60	0	50	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	187	125 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	44	19 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -2	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-208 / 0	0.0	0.0	0.11 (4)		7.81			
A-B	0 / 26	-84.9	-84.9	0.11 (1)		10.00			
B-C	-8 / 9	-84.9	-84.9	0.08 (4)		10.00			
E-F	0 / 0	-18.5	-18.5	0.14 (4)		10.00			
F-G	0 / 0	-18.5	-18.5	0.14 (4)		10.00			
G-D	0 / 0	-18.5	-18.5	0.14 (4)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	4	1	7	FRONT	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (D-E:4)

DOL LUMBER=0.97 NAIL=0.97 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070147

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425212	C4	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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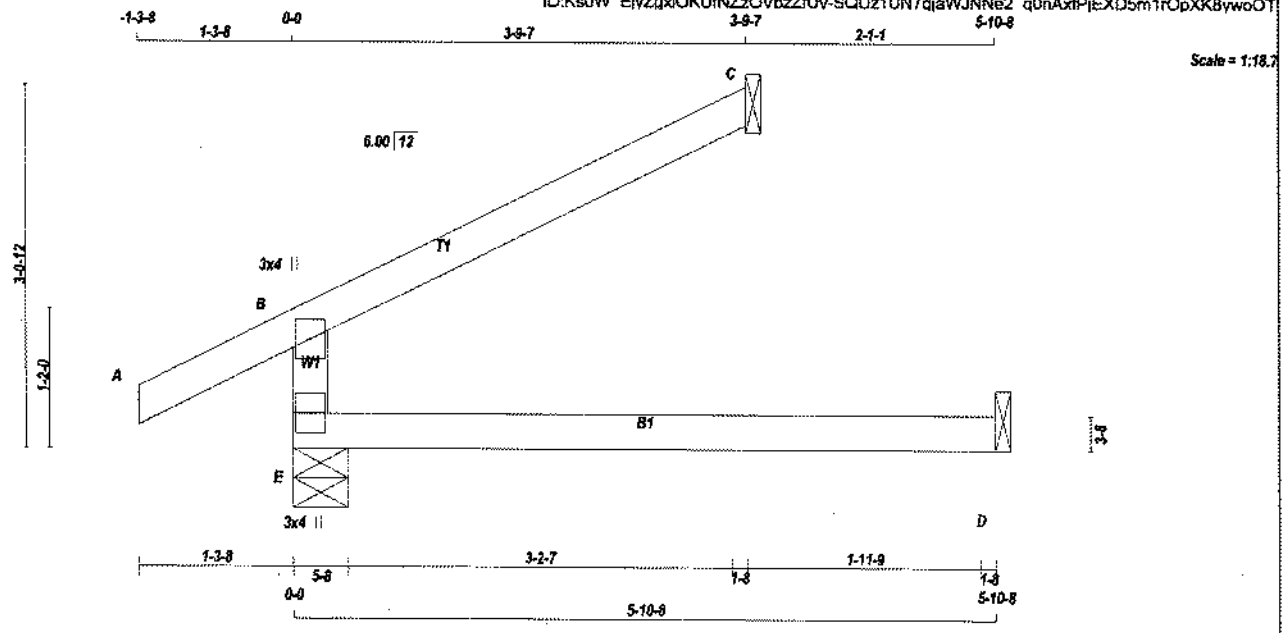

TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	380	0	380	0	0	5-8	5-8
C	121	0	121	0	0	1-8	1-8
D	46	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	269	173 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	83	56 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-316 / 0	0.0	0.0	0.13 (4)		7.81			
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00			
B- C	-18 / 0	-84.9	-84.9	0.21 (1)		6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070148

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	C5	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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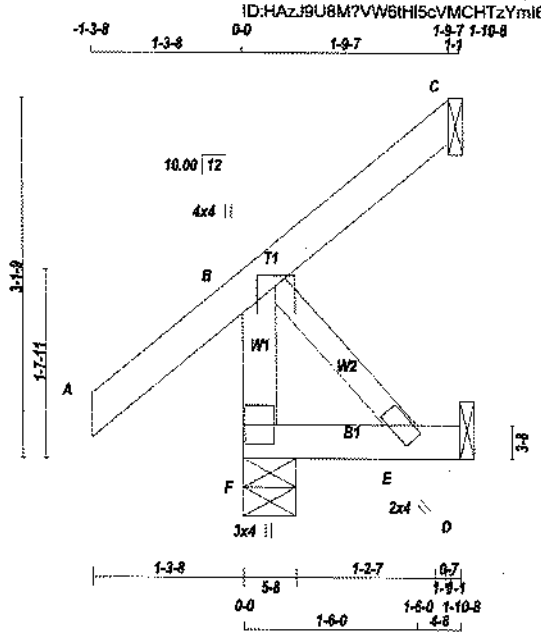

TOTAL WEIGHT = 4 X 10 = 39 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	266	0	266	0	0	5-8	5-8
C	30	0	30	0	-36	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	179	131 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-239 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 36	-84.9	-84.9	0.12 (1)	10.00			
B-C	-29 / 0	-84.9	-84.9	0.11 (1)	6.25			
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070185

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425214	C6	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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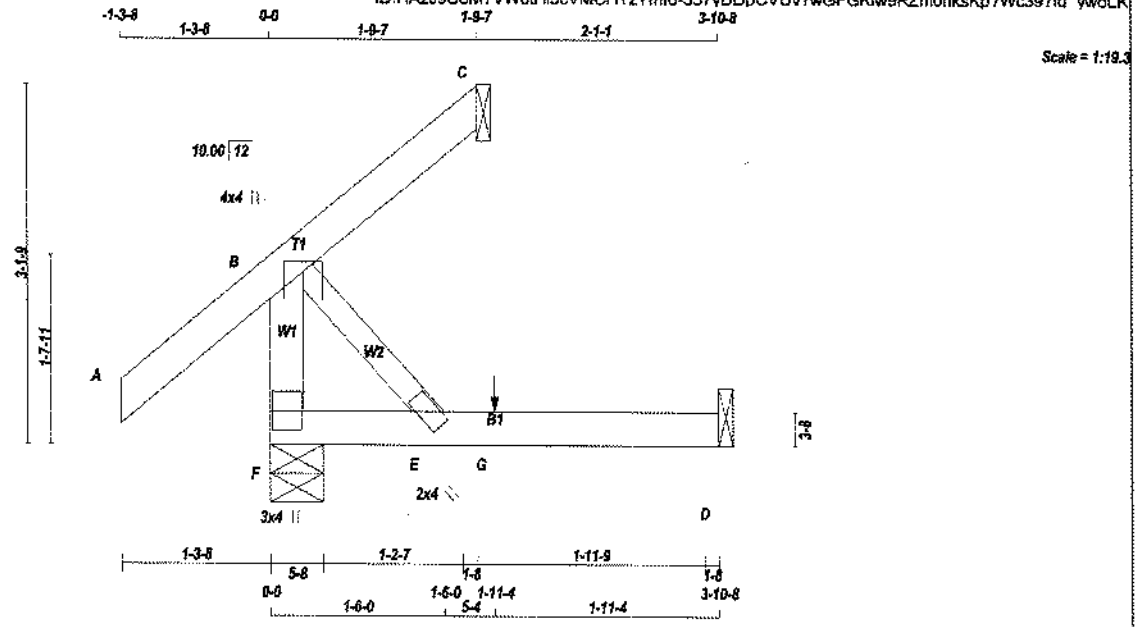

TOTAL WEIGHT = 4 X 12 = 48 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070186

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	275	0	275	0	0	5-8	5-8
C	30	0	30	0	-36	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	194	131 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-239 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-84.9	-84.9	0.14 (5)	10.00			
B- C	-29 / 0	-84.9	-84.9	0.13 (5)	6.25			
F- E	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	660	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	C30	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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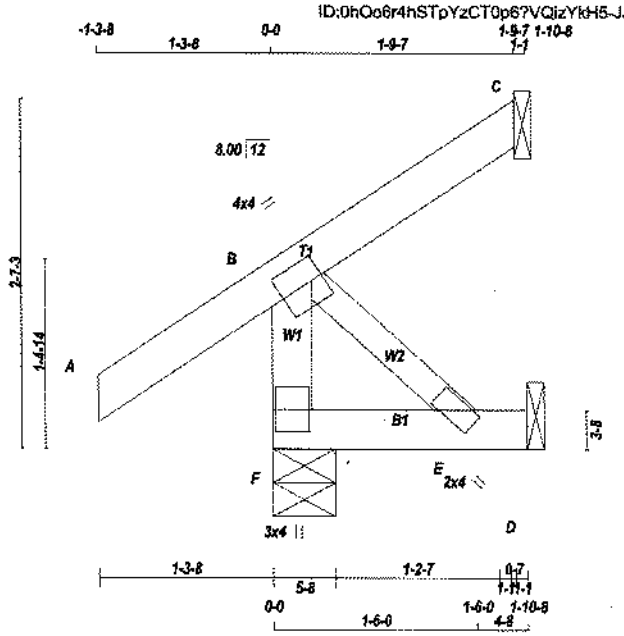

TOTAL WEIGHT = 4 X 9 = 36 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	254	0	254	0	0	5-8	5-8
C	31	0	31	0	-35	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	177	130 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 0.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-237 / 0	0.0	0.0	0.02 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 32	-84.9	-84.9	0.12 (5)	10.00			
B-C	-25 / 0	-84.9	-84.9	0.11 (5)	6.25			
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:5) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070199

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	C31	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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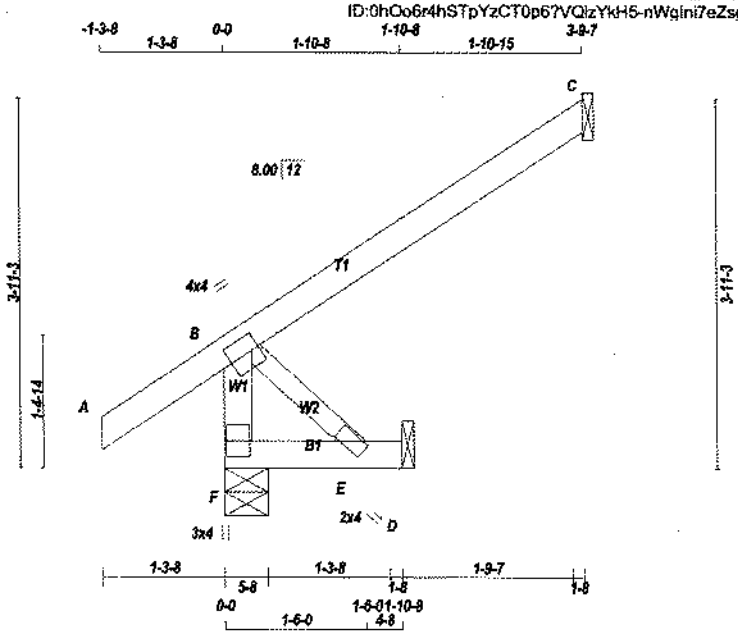

TOTAL WEIGHT = 12 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	294	0	294	0	0	5-8	5-8
C	161	0	161	0	0	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	205	152 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	111	88 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-277 / 0	0.0	0.0	0.03 (1)		7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 32	-84.9	-84.9	0.12 (5)		10.00			
B- C	0 / 0	-84.9	-84.9	0.21 (1)		10.00			
F- E	0 / 0	-18.5	-18.5	0.02 (4)		10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = **24.0** **IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070200

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	C32	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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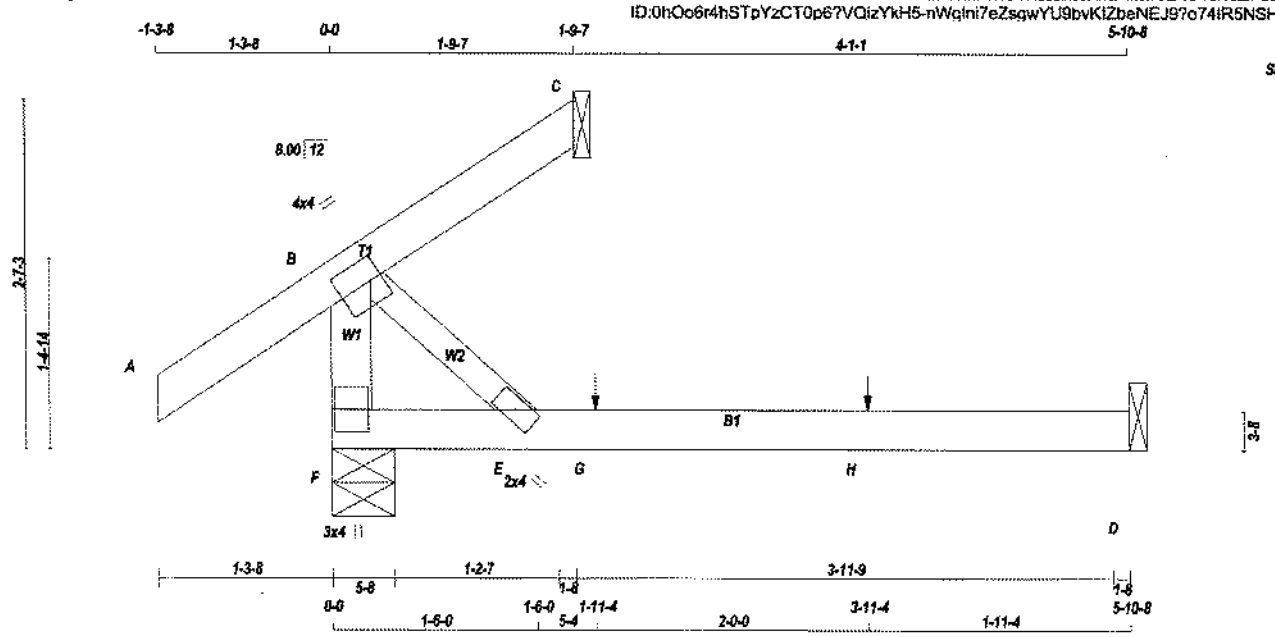

TOTAL WEIGHT = 13 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	291	0	291	0	0	5-8	5-8
C	31	0	31	0	0	1-8	1-8
D	54	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	207	130 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	21	17 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-237 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 32	-84.9	-84.9	0.12 (1)	10.00				
B-C	-25 / 0	-84.9	-84.9	0.11 (1)	6.25				
F-E	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
E-G	0 / 0	-18.5	-18.5	0.19 (4)	10.00				
G-H	0 / 0	-18.5	-18.5	0.19 (4)	10.00				
H-D	0 / 0	-18.5	-18.5	0.19 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=0.96 NAIL=0.96 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.80)
JSI METAL= 0.05 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070201

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	C33	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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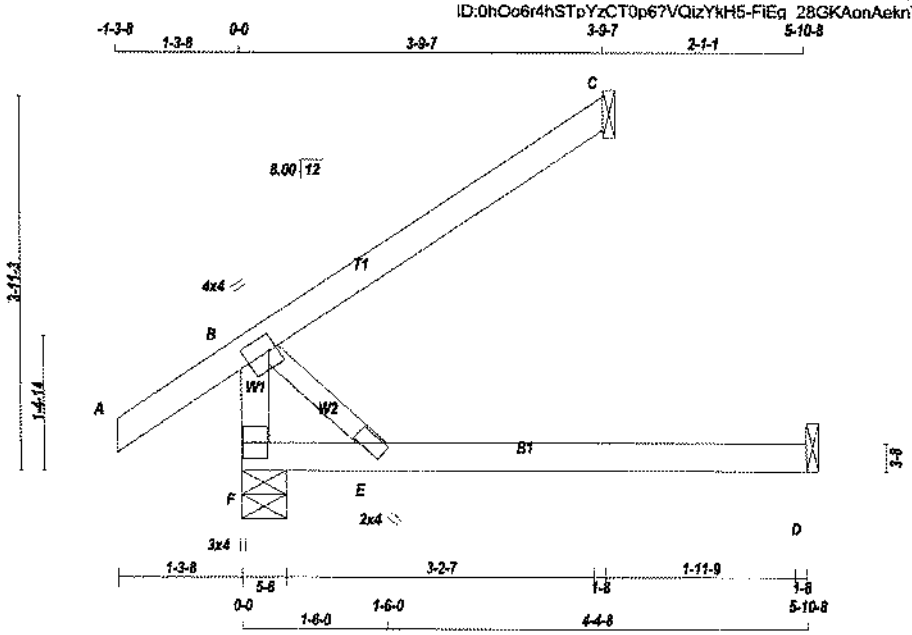

TOTAL WEIGHT = 16 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	331	0	331	0	0	5-8	5-8
C	161	0	161	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	235	152 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
C	111	86 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-277 / 0	0.0	0.0	0.03 (1)		7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00			
B- C	0 / 0	-84.9	-84.9	0.21 (1)		10.00			
F- E	0 / 0	-18.5	-18.5	0.13 (4)		10.00			
E- D	0 / 0	-18.5	-18.5	0.19 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070202

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
425216	C34	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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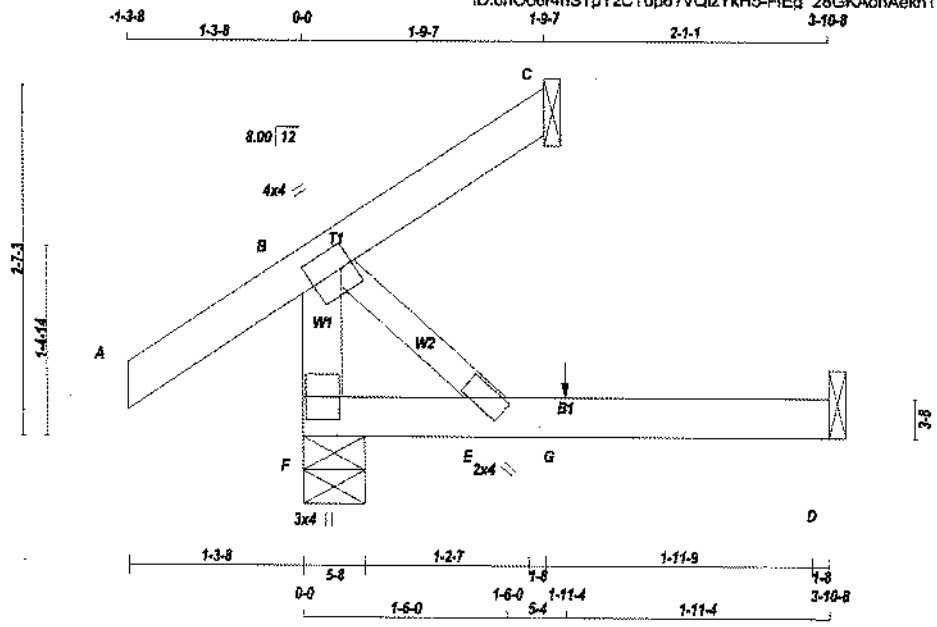


TOTAL WEIGHT = 3 X 11 = 34 lb

(M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	273	0	273	0	0	5-8	5-8
C	31	0	31	0	-35	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	192	130 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-237 / 0	0.0	0.0	0.02 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 32	-84.9	-84.9	0.13 (5)	10.00			
B- C	-26 / 0	-84.9	-84.9	0.13 (5)	6.25			
F- E	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
7/19/22
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

DWG # TR22070203

EWP DESIGN INC.

(905) 832-2250 FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

February 1, 2019

MiTek®

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
MINIMUM NUMBER OF NAILS					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

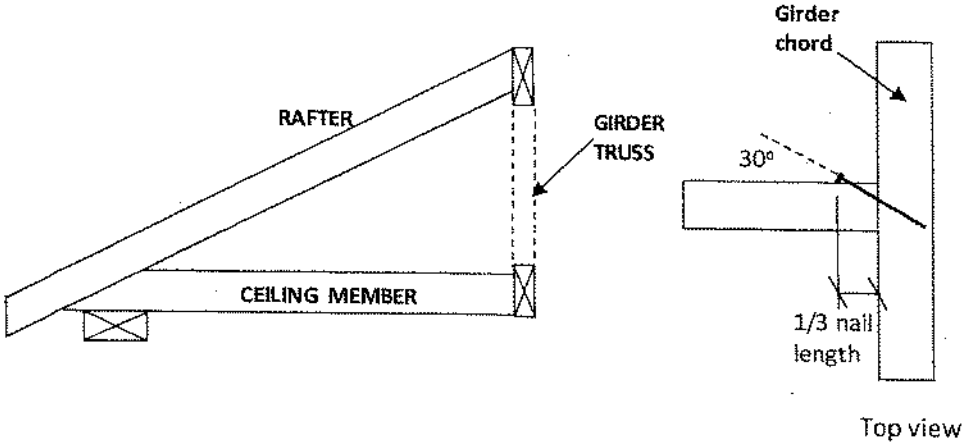


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485
LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

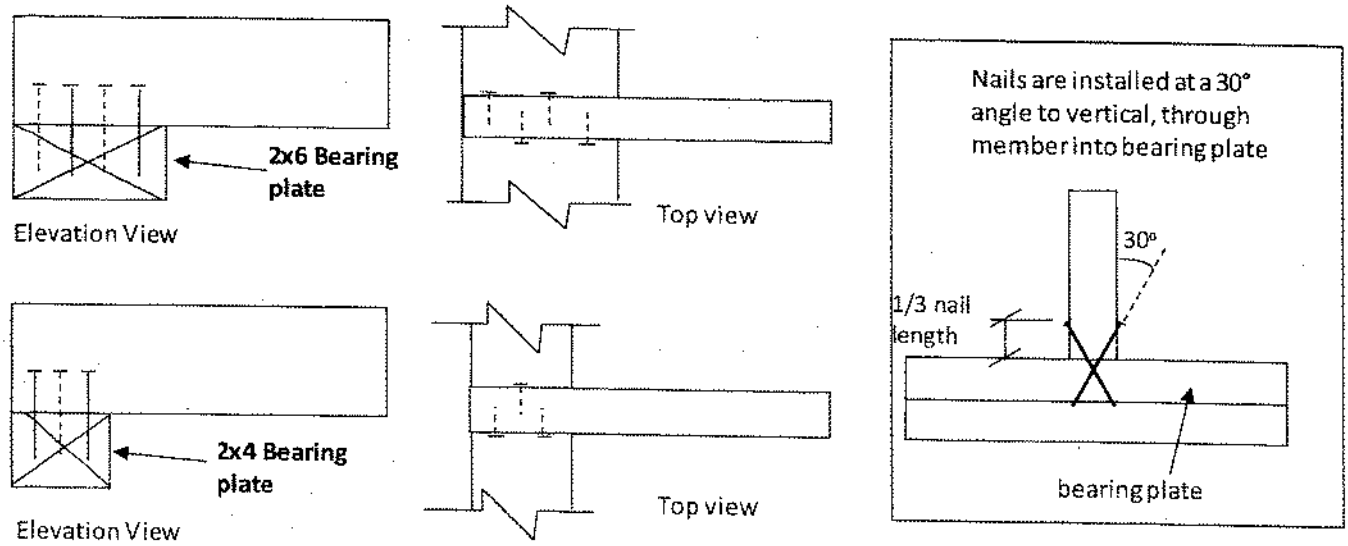
MiTek®

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).

2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).

3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.

4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).

5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.

6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.

7. Lumber must be dry (< 19% moisture content) at the time of nail installation.

8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485

December 21, 2020

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

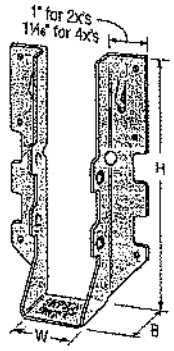
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

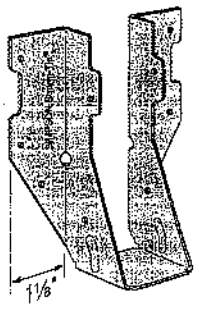
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

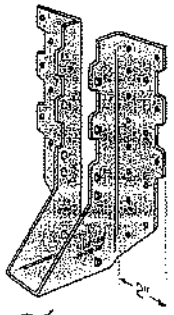
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



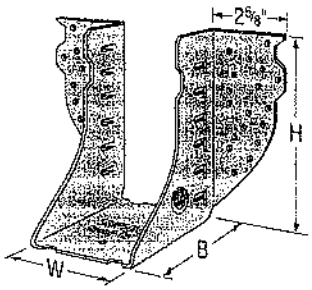
LUS28



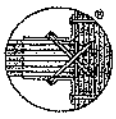
LU26L



HUS210
(HUS26, HUS28, and HHUS similar)

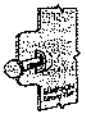


HGUS28-2

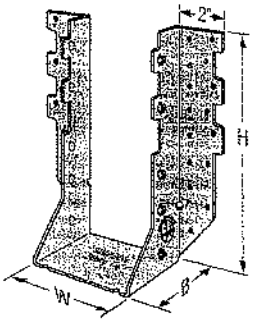


Double-Shear Nailing Top View

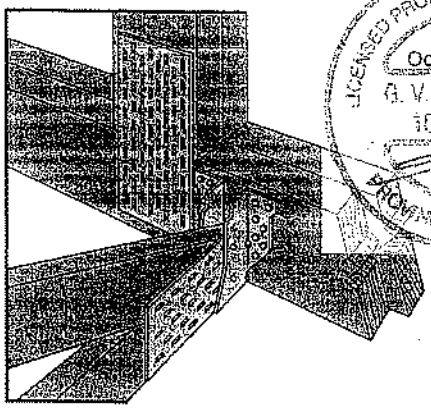
Double-Shear Nailing Side View; Do not bend tab



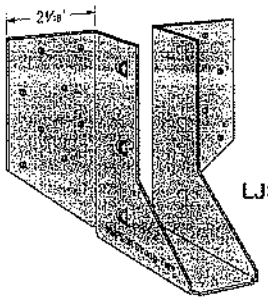
Dome Double-Shear Nailing Side View *(available on some models)*



HHUS210-2



Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

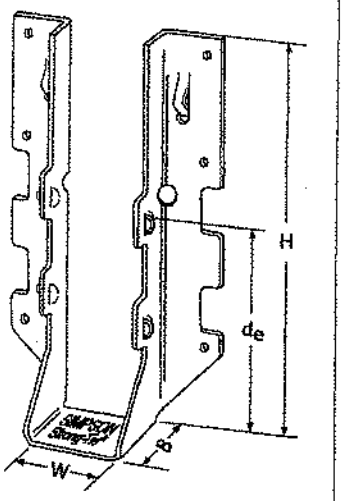
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

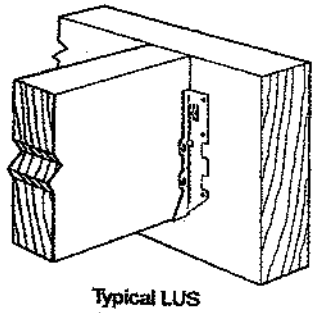
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

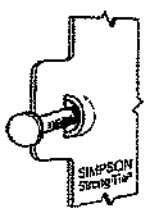


LUS28


Typical LUS Installation

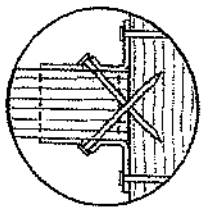
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⁹⁄₁₆	3⅛	1¾	1 ¹⁵⁄₁₆	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1 ¹⁵⁄₁₆	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⁹⁄₁₆	4¾	1¾	3⅝	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4⁹⁄₁₆	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⁹⁄₁₆	6⅝	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⁹⁄₁₆	7 ¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8⁹⁄₁₆	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

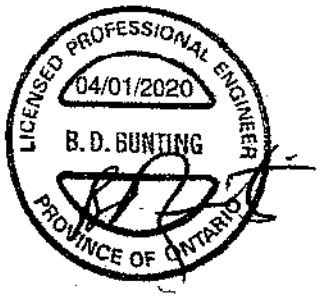
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

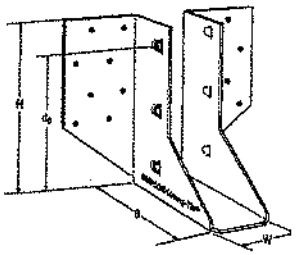
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

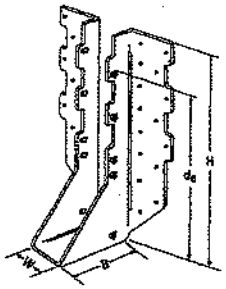
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

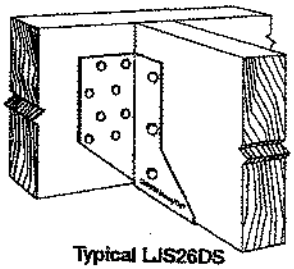
- See current catalogue for options



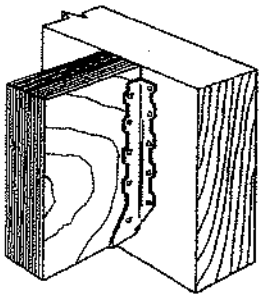
LJS26DS



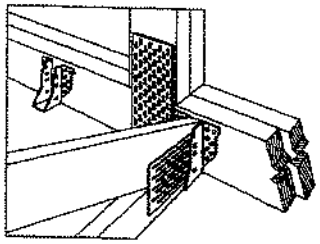
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



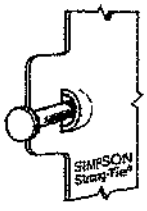
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

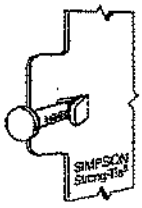
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_s[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1⅜	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅜	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅜	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅜	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_s is the distance from the seat of the hanger to the highest joist nail.

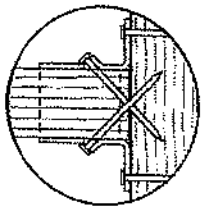


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

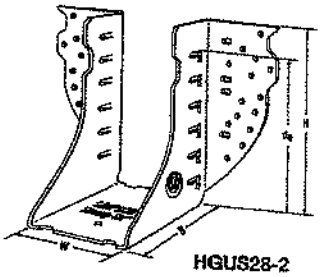
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

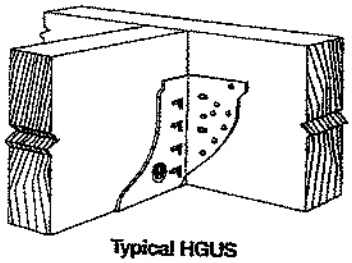
Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2



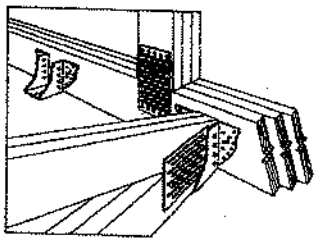
Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

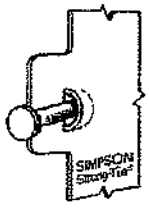
- See current catalogue for options



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

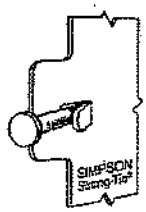
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅝₁₆	5⅞₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4⅛₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅝₁₆	5⅞₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅝₁₆	7⅜₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4⅛₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅝₁₆	7⅜₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⅝₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4⅛₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅝₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅝₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅝₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

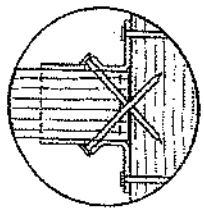


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

UNITED STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

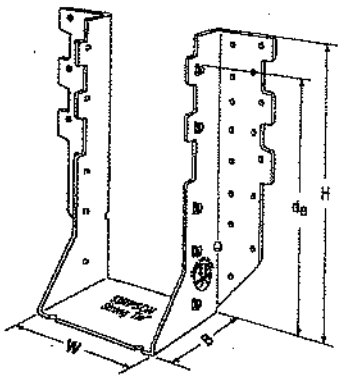
All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

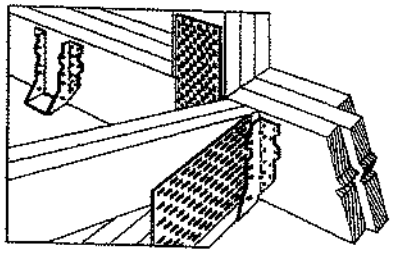
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HHUS410

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



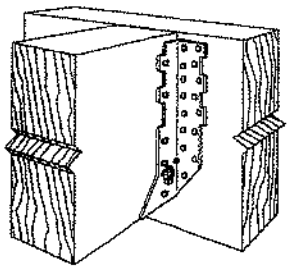
Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Options:

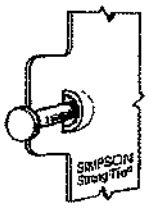
- See current catalogue for options

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 5/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 5/8	9	3	8	(30) 16d	(10) 16d	4670	9655	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_e is the distance from the seat of the hanger to the highest joist nail.

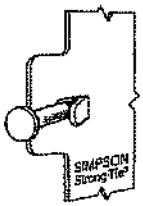


Typical HHUS Installation

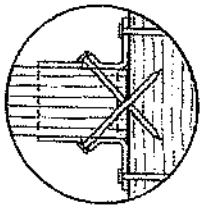


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

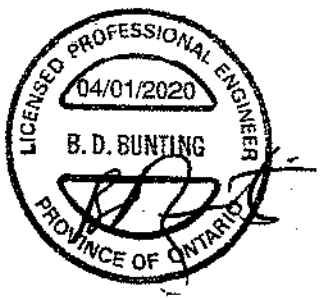
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

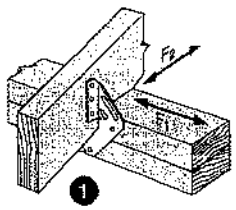
This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHHUS20 3/20 exp. 6/22

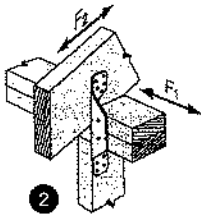
(800) 999-5099
strongtie.com

H/TSP

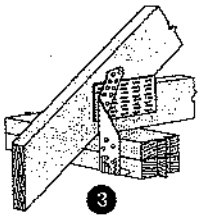
Seismic and Hurricane Ties (cont.)



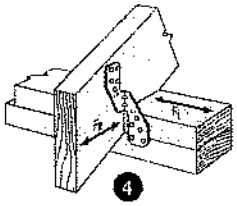
1 H1 Installation



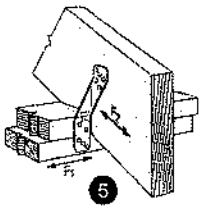
2 H2A Installation



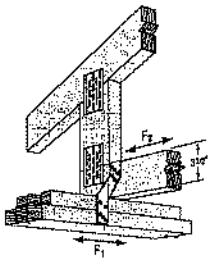
3 TSP Installation



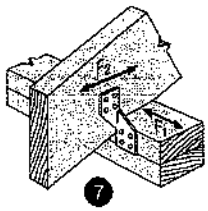
4 H2.5A Installation
(Nails into both top plates)



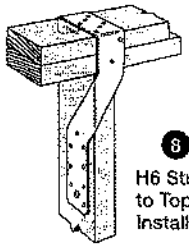
5 H2.5T Installation
(Nails into both top plates)



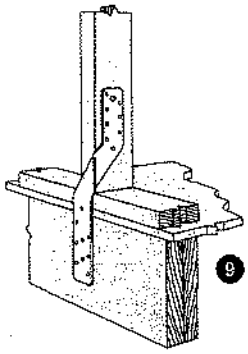
6 H2.5T Installation



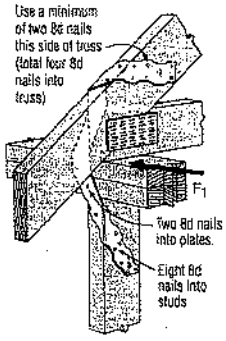
7 H3 Installation
(Nails into upper top plate)



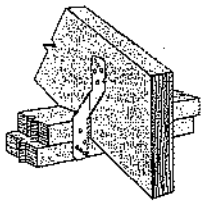
8 H6 Stud to Top Plate Installation



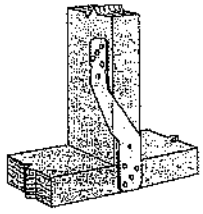
9 H6 Stud to Band Joist Installation



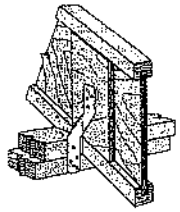
10 H7Z Installation



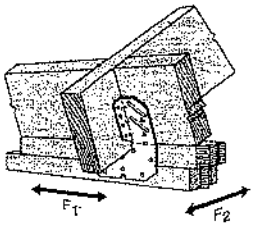
11 H8 Attaching Rafter to Double Top Plates



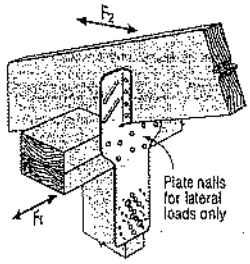
12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)



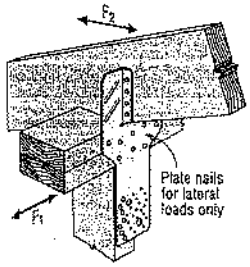
13 H8 attaching I-Joist to Double Top Plates



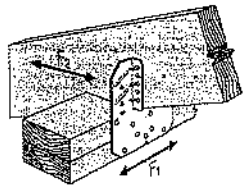
14 H10A Field-Bent Installation



15 H10S Installation



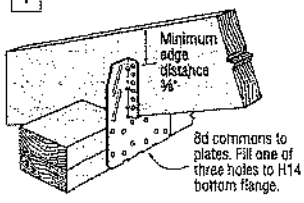
16 H10S Installation with Stud Offset



17 H10A Installation

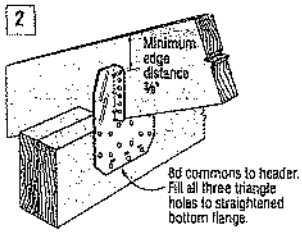
H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

1



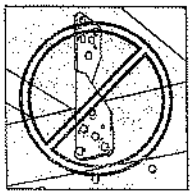
18 H14 Installation to Double Top Plates

2



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

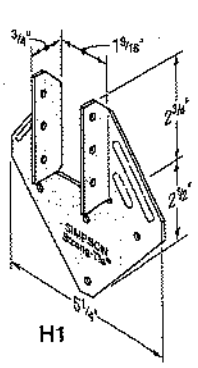
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

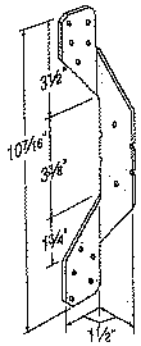
Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit **strongtie.com**.

Installation:

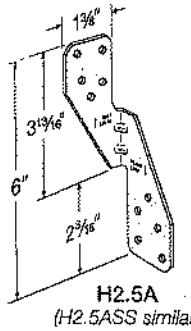
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



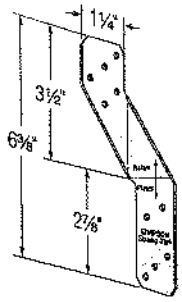
H1



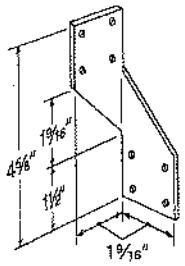
H2A
(H2ASS similar)



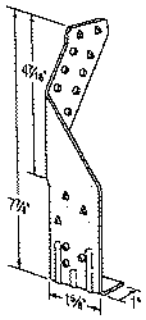
H2.5A
(H2.5ASS similar)



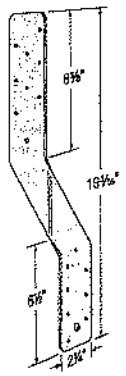
H2.5T



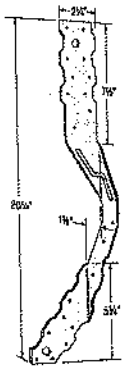
H3



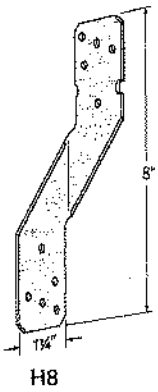
TSP



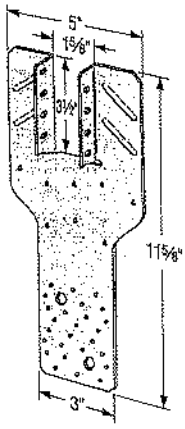
H6



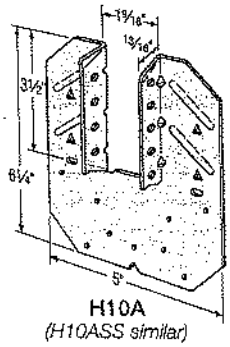
H7Z



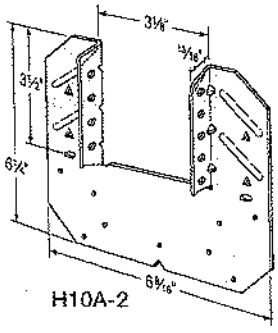
H8



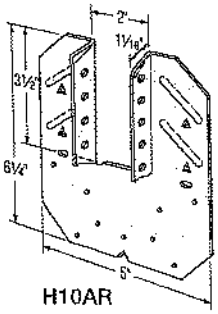
H10S



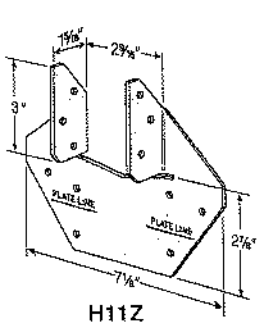
H10A
(H10ASS similar)



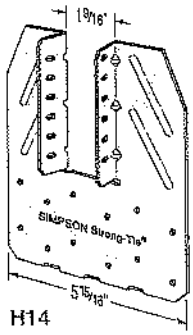
H10A-2



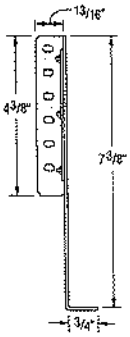
H10AR



H11Z



H14



H14
Profile

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON Strong-Tie

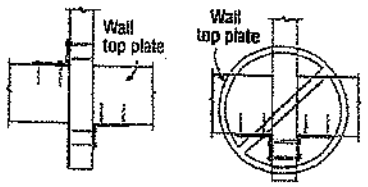
The H connector series provides wind and seismic ties for trusses and rafters.
Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners
- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

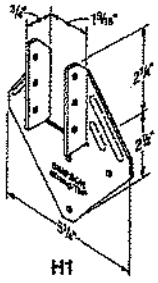
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

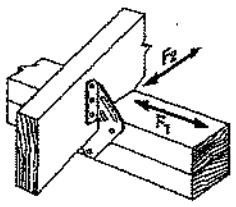
Hurricane Tie Installations to Achieve Twice the Load (Top View)



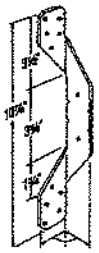
Install diagonally across from each other for minimum 2x truss.

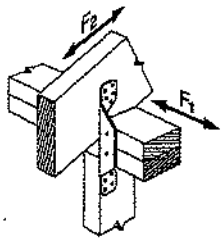
Nailing into both sides of a single ply 2x truss may cause the wood to split.


H1

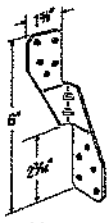


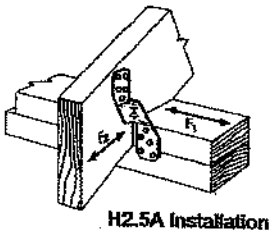
H1 Installation


H2A

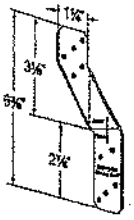


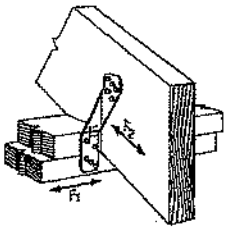
H2A Installation


H2.5A

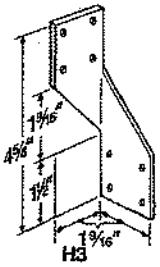


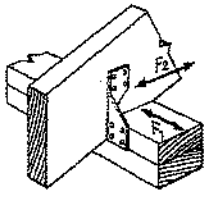
H2.5A Installation


H2.5T

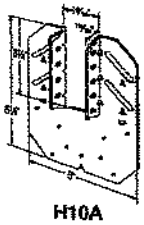


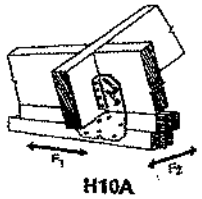
H2.5T Installation (Nails into both top plates)


H3



H3 Installation

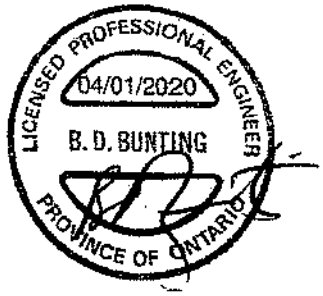

H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal F_1	Normal F_2	Uplift	Normal F_1	Normal F_2
					(K_D=1.15)			(K_D=1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

TC – Truss Connectors

SIMPSON Strong-Tie®

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

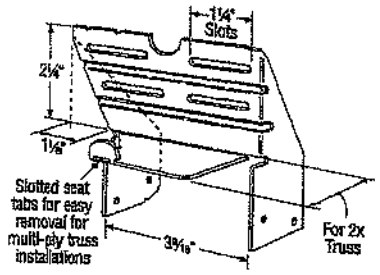
Design: Factored resistances are in accordance with CSA 086-14

Installation:

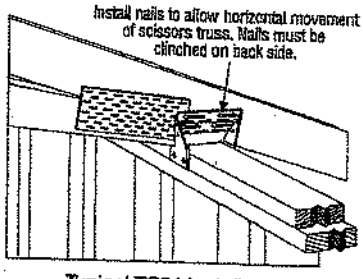
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss-allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

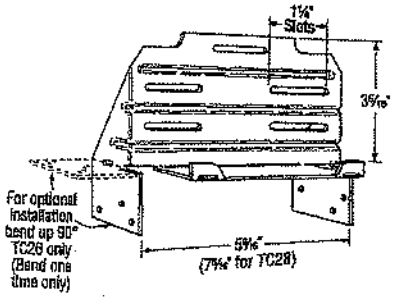
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



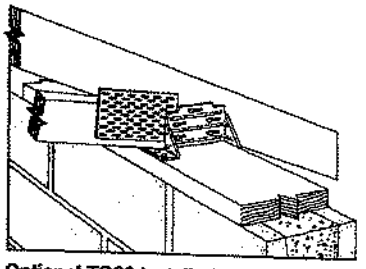
TC24
U.S. Patent 4,932,173



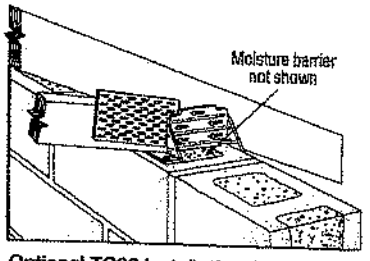
Typical TC24 Installation



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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(800) 999-5099
strongtie.com

HTU

SIMPSON Strong-Tie

Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (in.)	Minimum Header Size	Fasteners		Factored Resistance			
					D.Fir-L		S-P-F	
			Header	Joist	Uplift (K_D = 1.15)	Normal (K_D = 1.00)	Uplift (K_D = 1.15)	Normal (K_D = 1.00)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
HTU26 (Min.)	3⅞	(2) 2x4	(10) 16d	(14) 10d x 1½"	1740	3340	1235	2370
					7.74	14.86	5.49	10.54
HTU26 (Max.)	5⅛	(2) 2x4	(10) 16d	(20) 10d x 1½"	2470	4016	1755	2850
					10.99	17.86	7.81	12.68
HTU28 (Max.)	3⅞	(2) 2x6	(20) 16d	(26) 10d x 1½"	4150	6395	2945	4540
					18.46	28.46	13.10	20.19
HTU210 (Max.)	7¼	(2) 2x6	(20) 16d	(32) 10d x 1½"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19

See table footnotes on p. 260.

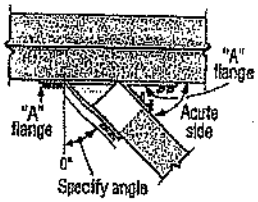
LICENSED PROFESSIONAL ENGINEER 5/29/19 B. D. BUNTING PROVINCE OF ONTARIO

Hanger Options

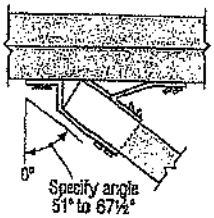
See Hanger Options information on pp. 125–127.

Skewed Seat

- Skewable up to 67½°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
				D.Fir-L		S-P-F	
		Header	Joist	Uplift (KD=1.15)	Normal (KD=1.00)	Uplift (KD=1.15)	Normal (KD=1.00)
				lbs	lbs	lbs	lbs
				kN	kN	kN	kN
HTU26	< 51	(20) 16d	(14) 10d x 1½"	1835	4110	1300	2905
				8.16	18.28	5.78	12.92
	51–67½	(20) 16d	(12) 10d x 1½"	1350	3620	955	2560
				6.01	16.10	4.25	11.39
HTU28	< 51	(26) 16d	(20) 10d x 1½"	2810	4270	1985	3030
				12.50	18.99	8.83	13.48
	51–67½	(26) 16d	(17) 10d x 1½"	2075	3930	1465	2780
				9.23	17.48	6.52	12.37
HTU210	< 51	(32) 16d	(26) 10d x 1½"	3785	4430	2675	3135
				16.84	19.71	11.90	13.95
	51–67½	(32) 16d	(22) 10d x 1½"	2795	4240	1980	3000
				12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
				9.52	16.53	6.74	11.68
	51–67½	(20) 16d	(12) 10d	1610	3920	1140	2785
				7.16	17.44	5.07	12.39
HTU28-2	< 51	(26) 16d	(20) 10d	3960	5425	2815	3855
				17.62	24.13	12.52	17.15
	51–67½	(26) 16d	(17) 10d	2385	5425	1695	3855
				10.61	24.13	7.54	17.15
HTU210-2	< 51	(32) 16d	(26) 10d	5025	6890	3570	4890
				22.35	30.65	15.88	21.75
	51–67½	(36) 16d	(22) 10d	3145	6680	2225	4745
				13.99	29.72	9.90	21.10

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3½" long, 10d x 1½" = 0.148" dia. x 1½" long, 10d = 0.148" dia. x 3" long. See pp. 27–28 for other nail sizes and information.

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

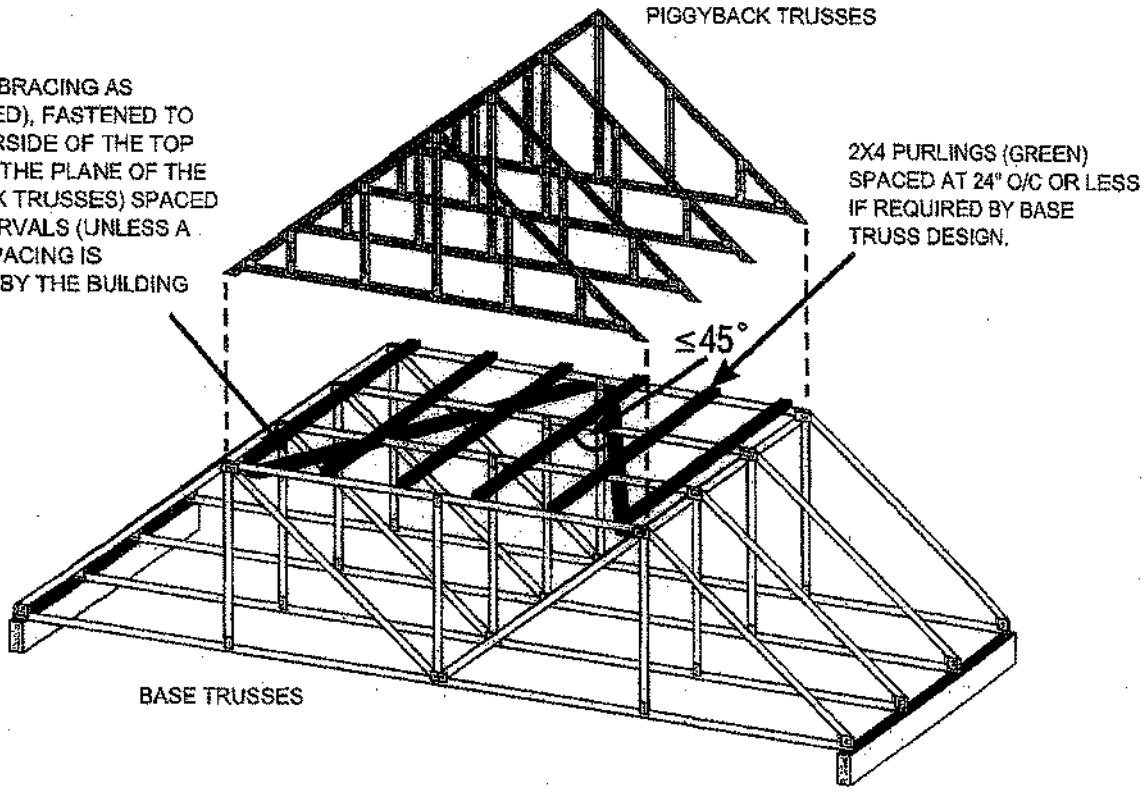
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

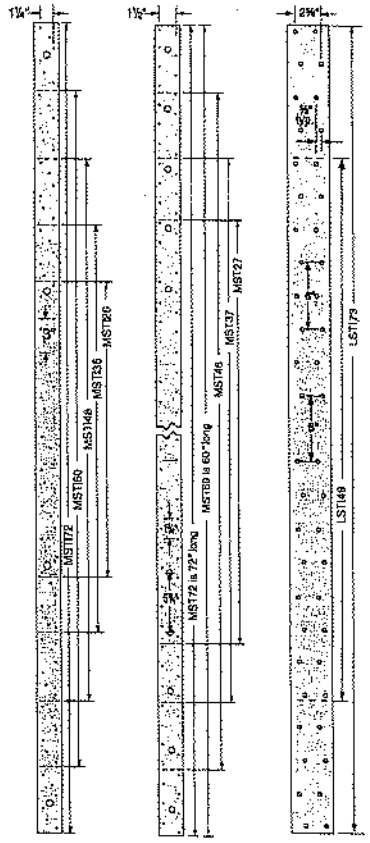
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

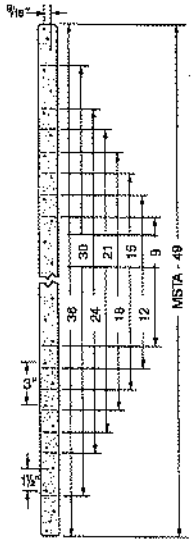
MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

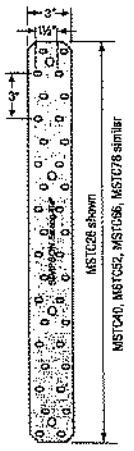
Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion information, pp. 18–20.

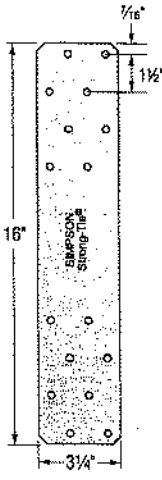
Installation: Use all specified fasteners; see General Notes

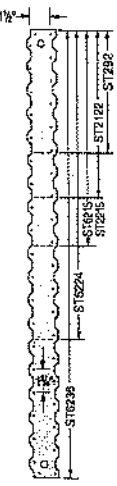
Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

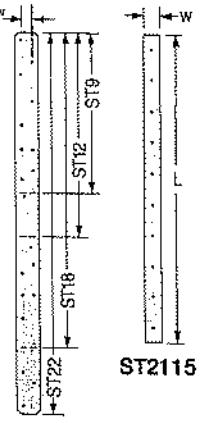


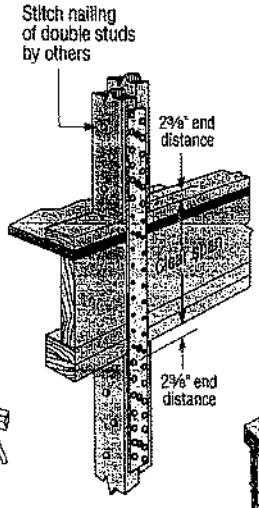


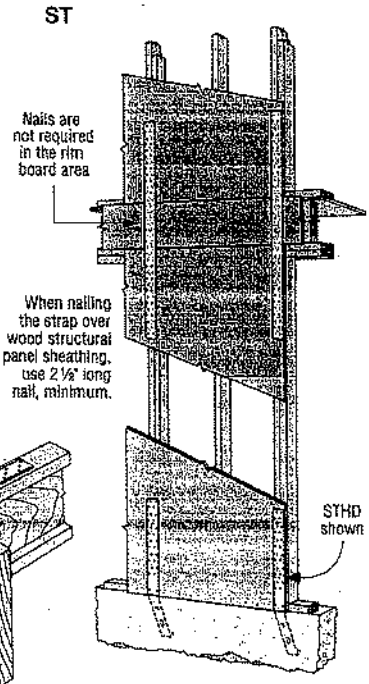


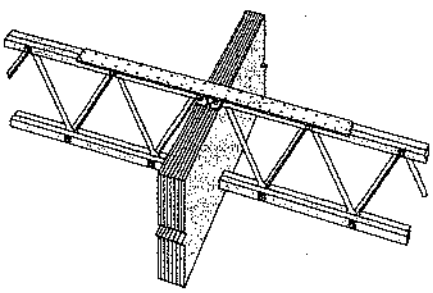


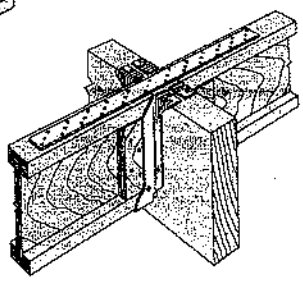









Typical LSTI Installation


Typical MSTI Installation
(MIT hanger shown)
LSTI similar

Straps and Ties

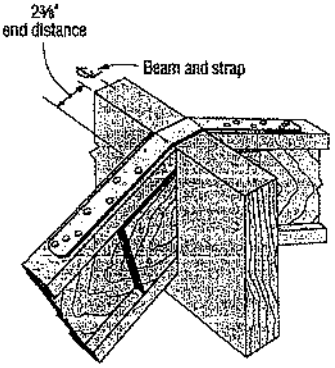
HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties (cont.)

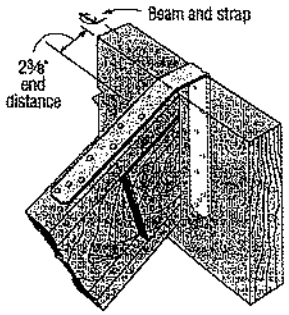
These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.) W	Dimensions (in.) L	Fasteners (Total)	Factored Tensile Resistance D.Fir–L (K_D = 1.00) lb. kN	D.Fir–L (K_D = 1.15) lb. kN	S-P-F (K_D = 1.00) lb. kN	S-P-F (K_D = 1.15) lb. kN
LSTA9	20	1¼	9	(6) 10d	600 2.67	690 3.07	555 2.47	635 2.82
LSTA12		1¼	12	(8) 10d	800 3.56	920 4.09	735 3.27	845 3.76
LSTA15		1¼	15	(10) 10d	1000 4.45	1150 5.12	920 4.09	1060 4.72
LSTA18		1¼	18	(12) 10d	1200 5.34	1380 6.14	1105 4.92	1270 5.65
LSTA21		1¼	21	(14) 10d	1400 6.23	1610 7.16	1290 5.74	1485 6.61
LSTA24		1¼	24	(16) 10d	1600 7.12	1840 8.19	1475 6.56	1695 7.54
ST292		2⅛	9⅝	(8) 8d	585 2.60	675 3.00	535 2.38	615 2.74
ST2122		2⅛	12⅜	(12) 8d	940 4.18	1085 4.83	865 3.85	995 4.43
ST2115		¾	16⅝	(8) 8d	670 2.98	770 3.43	615 2.74	710 3.16
ST2215		2⅛	16⅝	(16) 8d	1335 5.94	1540 6.85	1235 5.49	1420 6.32
LSTA30	18	1¼	30	(20) 10d	2235 9.94	2465 10.97	2075 9.23	2385 10.61
LSTA36		1¼	36	(24) 10d	2465 10.97	2465 10.97	2465 10.97	2465 10.97
LSTI49		3¾	49	(32) 10d x 1½"	3115 13.86	3580 15.93	2852 12.69	3280 14.59
LSTI73		3¾	73	(48) 10d x 1½"	4670 20.77	5370 23.89	4280 19.04	4920 21.89
MSTA9		1¼	9	(6) 10d	670 2.98	770 3.43	625 2.78	715 3.18
SD MSTA12		1¼	12	(8) 10d	895 3.98	1030 4.58	830 3.69	955 4.25
MSTA15		1¼	15	(10) 10d	1120 4.98	1285 5.72	1040 4.63	1195 5.32
SD MSTA18		1¼	18	(12) 10d	1340 5.96	1545 6.87	1245 5.54	1430 6.36
MSTA21		1¼	21	(14) 10d	1565 6.96	1800 8.01	1455 6.47	1670 7.43
SD MSTA24		1¼	24	(16) 10d	1790 7.96	2060 9.16	1660 7.38	1910 8.50
MSTA30	16	1¼	30	(20) 10d	2470 10.99	2840 12.63	2260 10.05	2595 11.54
SD MSTA36		1¼	36	(24) 10d	2965 13.19	3070 13.66	2710 12.06	3070 13.66
MSTA49		1¼	49	(28) 8d	2725 12.12	2725 12.12	2545 11.32	2725 12.12
ST6215		2⅛	16⅝	(16) 8d	1405 6.25	1615 7.18	1300 5.78	1500 6.67
ST6224		2⅛	23⅜	(24) 8d	2305 10.25	2650 11.79	2155 9.59	2475 11.01
ST9		1⅛	9	(6) 8d	525 2.34	605 2.69	490 2.18	560 2.49
ST12		1¼	11⅞	(8) 8d	700 3.11	805 3.58	650 2.89	750 3.34
ST18		1¼	17¾	(12) 8d	1050 4.67	1210 5.38	975 4.34	1125 5.00
ST22		1¼	21⅝	(18) 8d	1580 7.03	1790 7.96	1465 6.52	1685 7.50



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. **Nails:** 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

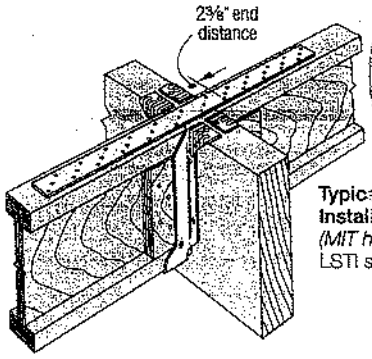
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
					D.Fir-L		S-P-F	
		W	L		(K_D = 1.00)	(K_D = 1.15)	(K_D = 1.00)	(K_D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
MSTC28	16	3	28¼	(32) 10d	3955	4545	3615	4155
					17.59	20.22	16.08	18.48
MSTC40		3	40¼	(48) 10d	5930	6820	5420	6235
					26.38	30.34	24.11	27.74
MSTC52		3	52¼	(54) 10d	6670	6940	6100	6940
					29.67	30.87	27.14	30.87
MSTC66	14	3	65¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
MSTC78		3	77¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
ST6236		2⅛	33⁹⁄₁₆	(36) 8d	3735	4295	3270	3760
					16.61	19.11	14.55	16.73
MSTI26	12	2⅛	26	(22) 10d x 1½"	2825	3250	2475	2850
					12.57	14.46	11.01	12.68
MSTI36		2⅛	36	(32) 10d x 1½"	4110	4725	3600	4140
					18.28	21.02	16.01	18.42
MSTI48		2⅛	48	(44) 10d x 1½"	5650	6500	4955	5695
					25.13	28.91	22.04	25.33
MSTI60		2⅛	60	(56) 10d x 1½"	7195	7360	6305	7250
					32.01	32.74	28.05	32.25
MSTI72		2⅛	72	(68) 10d x 1½"	7360	7360	7240	7360
					32.74	32.74	32.21	32.74
MST27		2⅛	27	(26) 8d	2685	3090	2365	2710
					11.94	13.75	10.48	12.06
MST37		2⅛	37½	(38) 8d	3930	4515	3440	3960
					17.48	20.08	15.30	17.62
MSTI48		2⅛	48	(50) 8d	5170	5945	4530	5210
					23.00	26.45	20.15	23.18
HRS416Z		3¼	16	(16) ¼" x 1½" SDS	2400	2760	2120	2440
					10.68	12.28	9.43	10.85
MST60	10	2⅛	60	(64) 8d	6620	7610	5800	6670
					29.45	33.85	25.80	29.67
MST72		2⅛	72	(78) 8d	8065	9135	7065	8125
					35.88	40.64	31.43	36.14

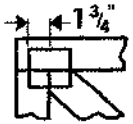
1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. **Nails:** 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.



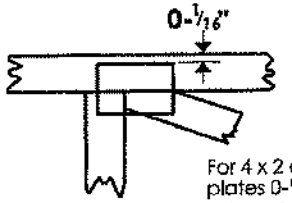
Typical MSTI Installation
(MIT hanger shown)
LSTI similar

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

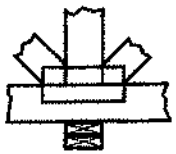
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

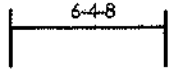
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

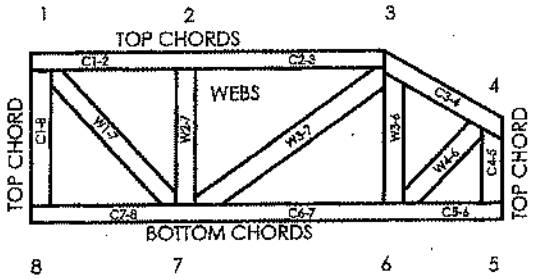
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.®

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.