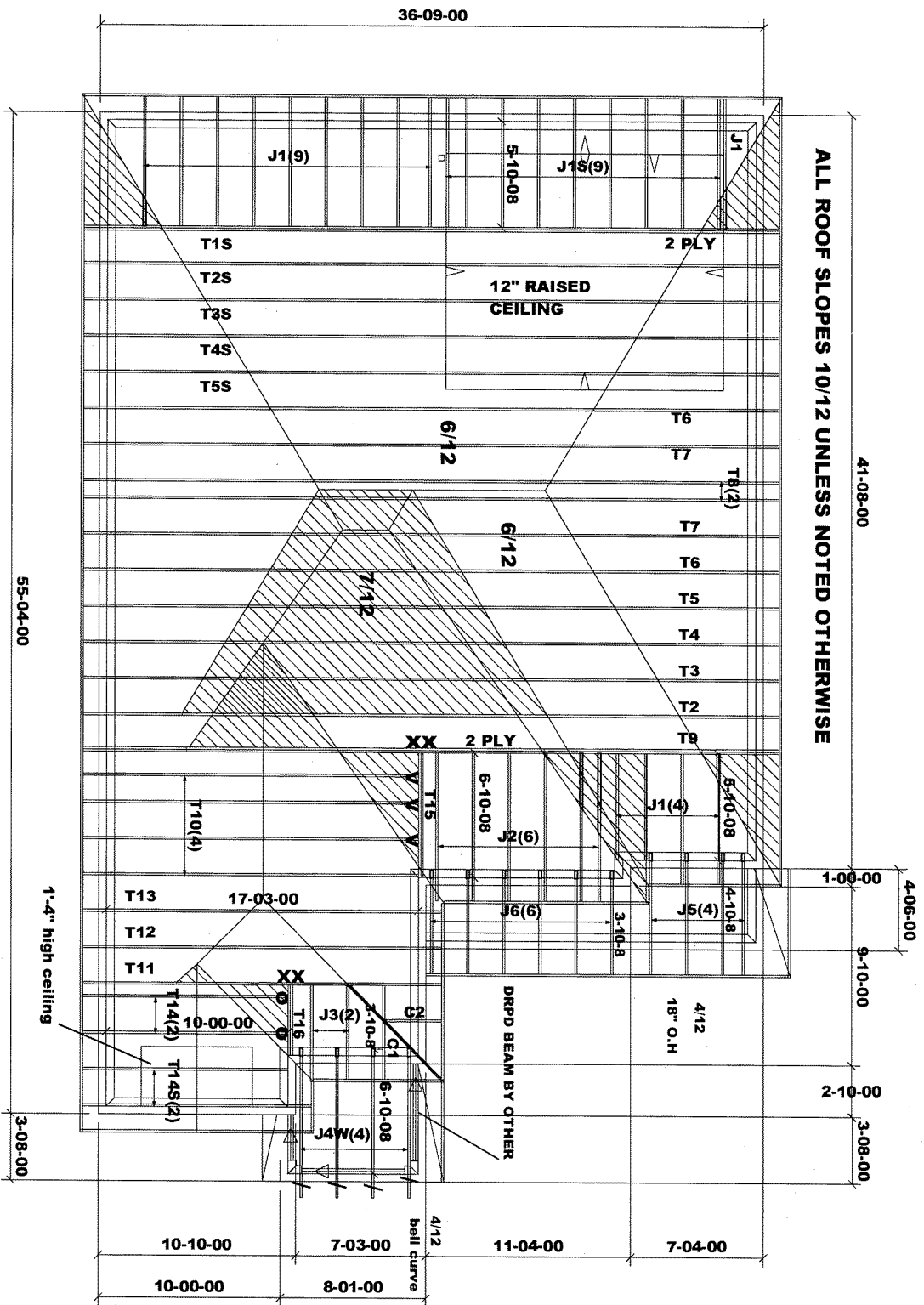




ALL ROOF SLOPES 10/12 UNLESS NOTED OTHERWISE

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:

TCSL = 25.6 psf
TCOL = 6.0 psf
BCIL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:

LUS24 - (O)
LUS26DS - (V)
HGS26-2 - (XX)
H2.5T - (V)



DENOTES:
CONVENTIONAL
FRAMING

M464

		Job Track: 51004 Plan Log: 201943 Layout ID: 406403	
Builder / Location: ROYAL PINE HOMES / RICHMOND HILL Project: CENTREFIELD Date: 2021-05-31 Sales: Mario Dicano Designer: EEO JG		Model / Elevation: UNIT - 4504 / A-STD.WITH OPT.COFF. THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Miter Ver 8.4.2.286	



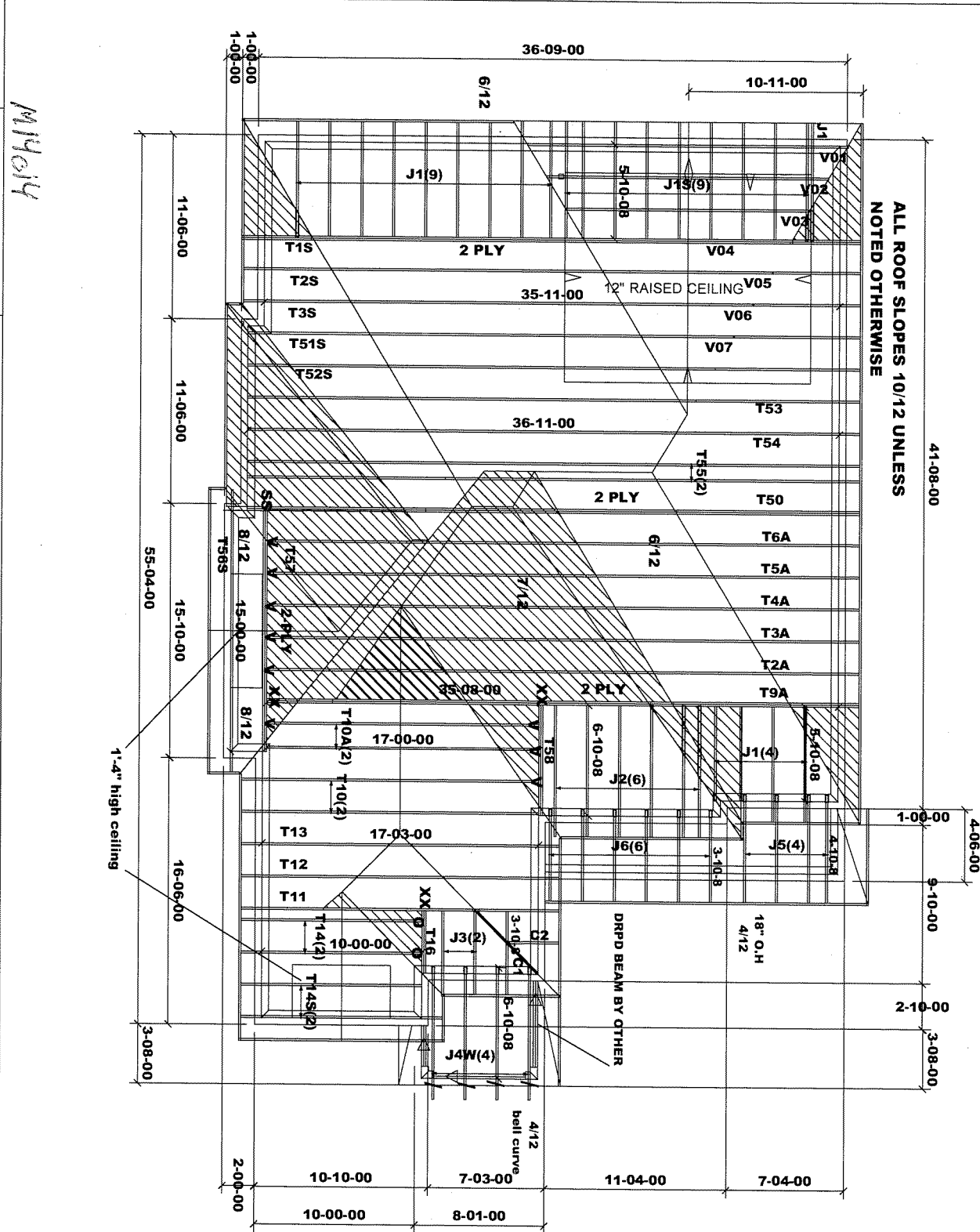
Job Track: **51004**
Plan Log: **201943**
Layout ID: **411069**

Builder / Location: **ROYAL PINE HOMES / RICHMOND HILL**
Project: **CENTREFIELD**
Date: **2021-05-26**
Sales: **Mario DiCaro**
Designer: **EEO**
PURPOSE: **OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.**

Model / Elevation: **UNIT 4504-FLANKAGE / A-STD.WITH OPT.COFF**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Milex ver 8.4.2.286



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment).
Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LUS26DS - (V)
HGUS26-2 - (XX)
LUS26-2 - (VV)
HGUS28-2 - (SS)
H25T - (I)

DENOTES:
 CONVENTIONAL FRAMING



Job Track: **51004**

Plan Log: **201943**

Layout ID: **418988**

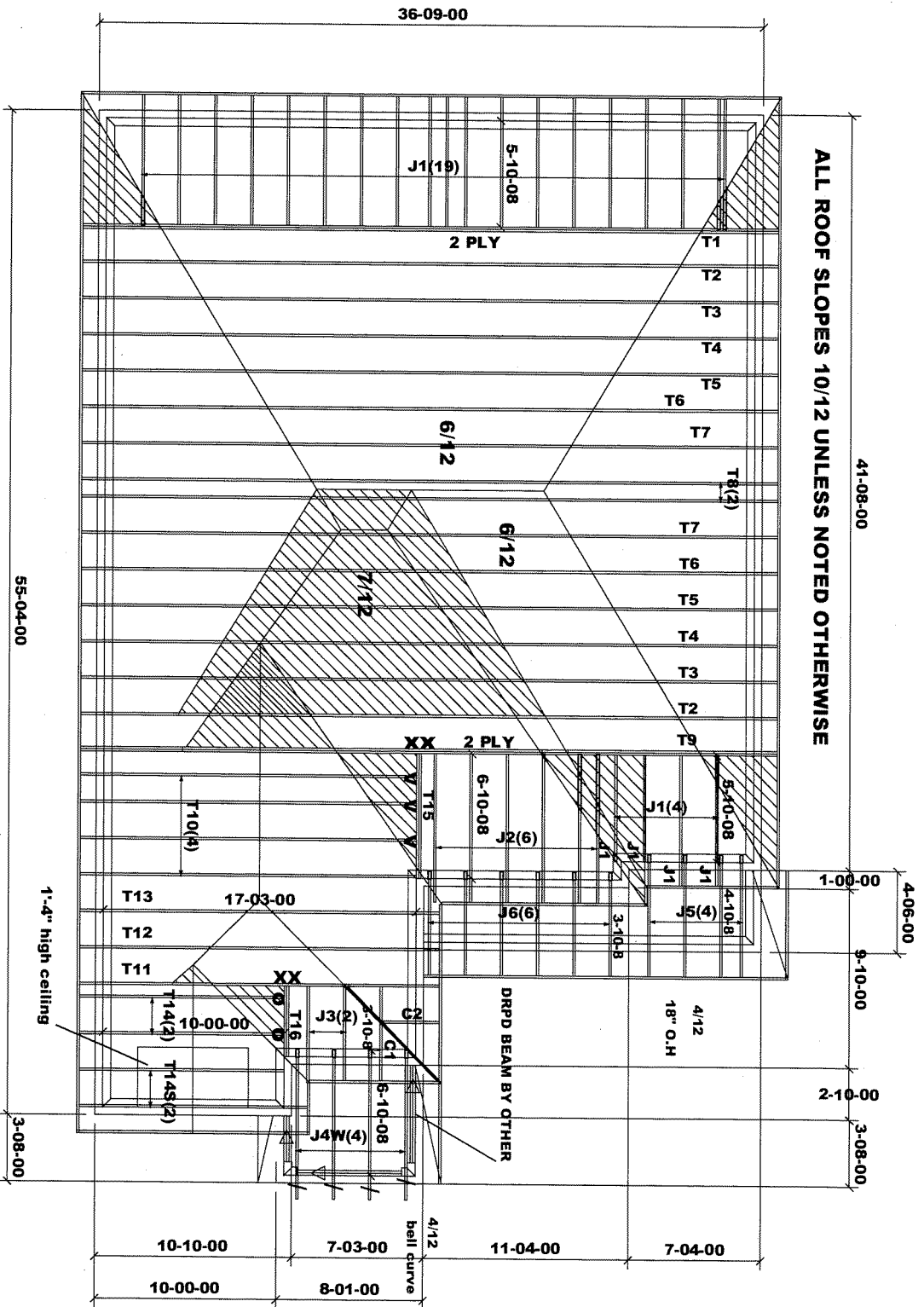
Builder / Location	ROYAL PINE HOMES / RICHMOND HILL		
Project	CENTREFIELD		
Date:	2021-05-26	Salas:	Mario Dujarran
			Designer: EEO
			THESE DRAWINGS COULD BE REDISTRIBUTED IN A MACHINERY ROOM OR TANK ROOM

Model / Elevation:

UNIT - 4504 / A-STD.OR 5 BRM

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.4.2.286



DENOTES:
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T - (I)

DESIGN CONFORMS WITH CBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

ASPHALT SHINGLES
 FINISHED OVERHANG: 12"
 2x6 EXTERIOR WALLS
 2x6 FASCIA BOARD
 HEEL: R.T.M.C.
 All conventional framing to conform with
 Part 9 of O.B.C. 2012 (2019 amendment).
 Roof rafters that cross over or meet trusses
 to be min. 2x4 SPF #2 @ 24" o/c with a
 vertical post to the truss at each cross
 point. Vertical posts longer than 6' to have
 lateral bracing so that the distance between
 the post end points and lateral bracing does
 not exceed 6'.



Job Track: **51004**

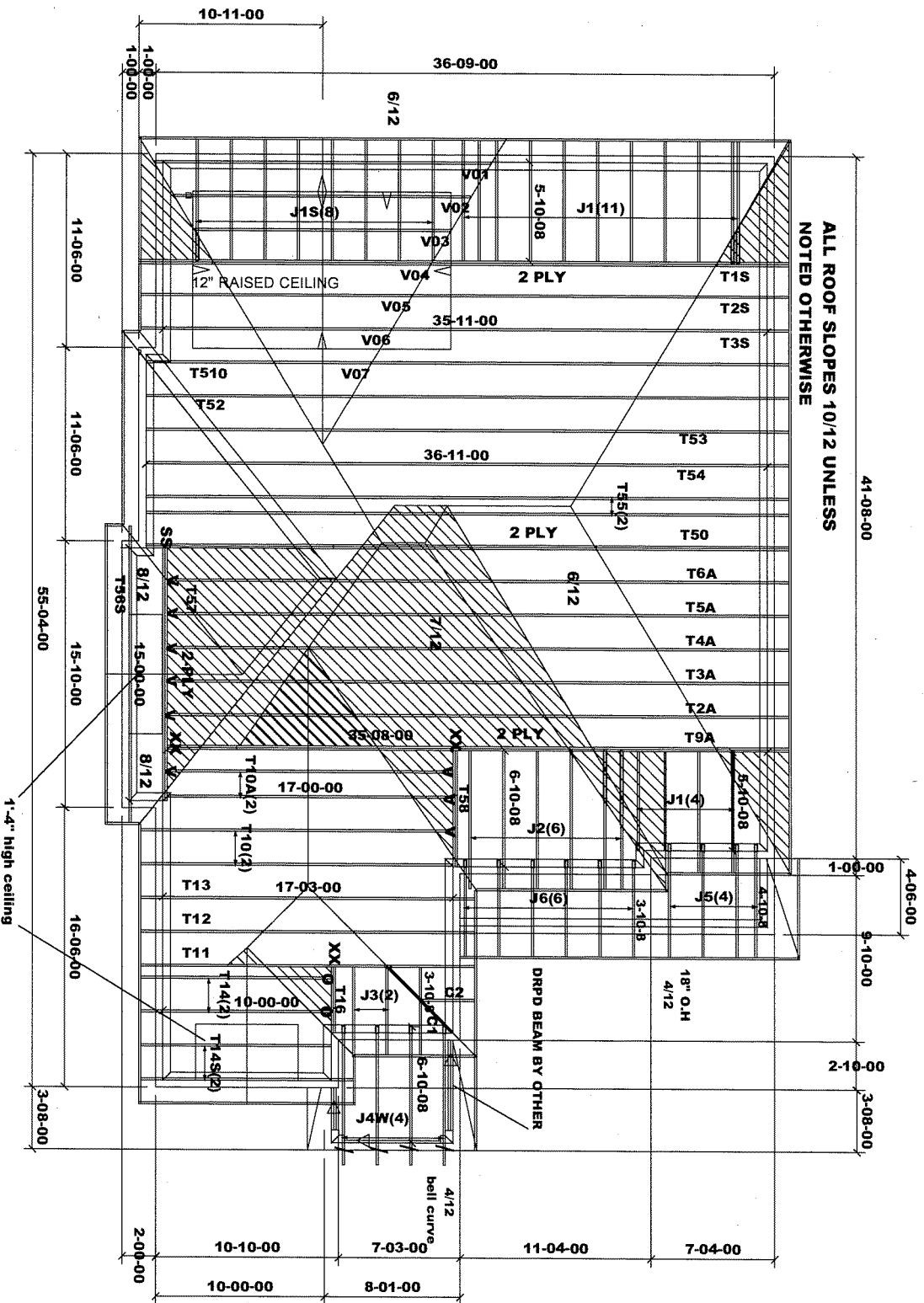
Plan Log: **201943**

Layout ID: **411070**

Builder / Location **ROYAL PINE HOMES / RICHMOND HILL**

Model / Elevation:

UNIT 4504-FLANKAGE / A-5 BRM WITH OPT. COFF.



DENOTES:
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LUS26DS - (V)
HGUS26-2 - (XX)
LUS26-2 - (VV)
HGUS28-2 - (SS)
H2.5T - (I)

DESIGN CONFORMS WITH CBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf
DESIGN LOADS:

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.



Job Track: **51004**

Plan Log: **201943**

Layout ID: **406516**

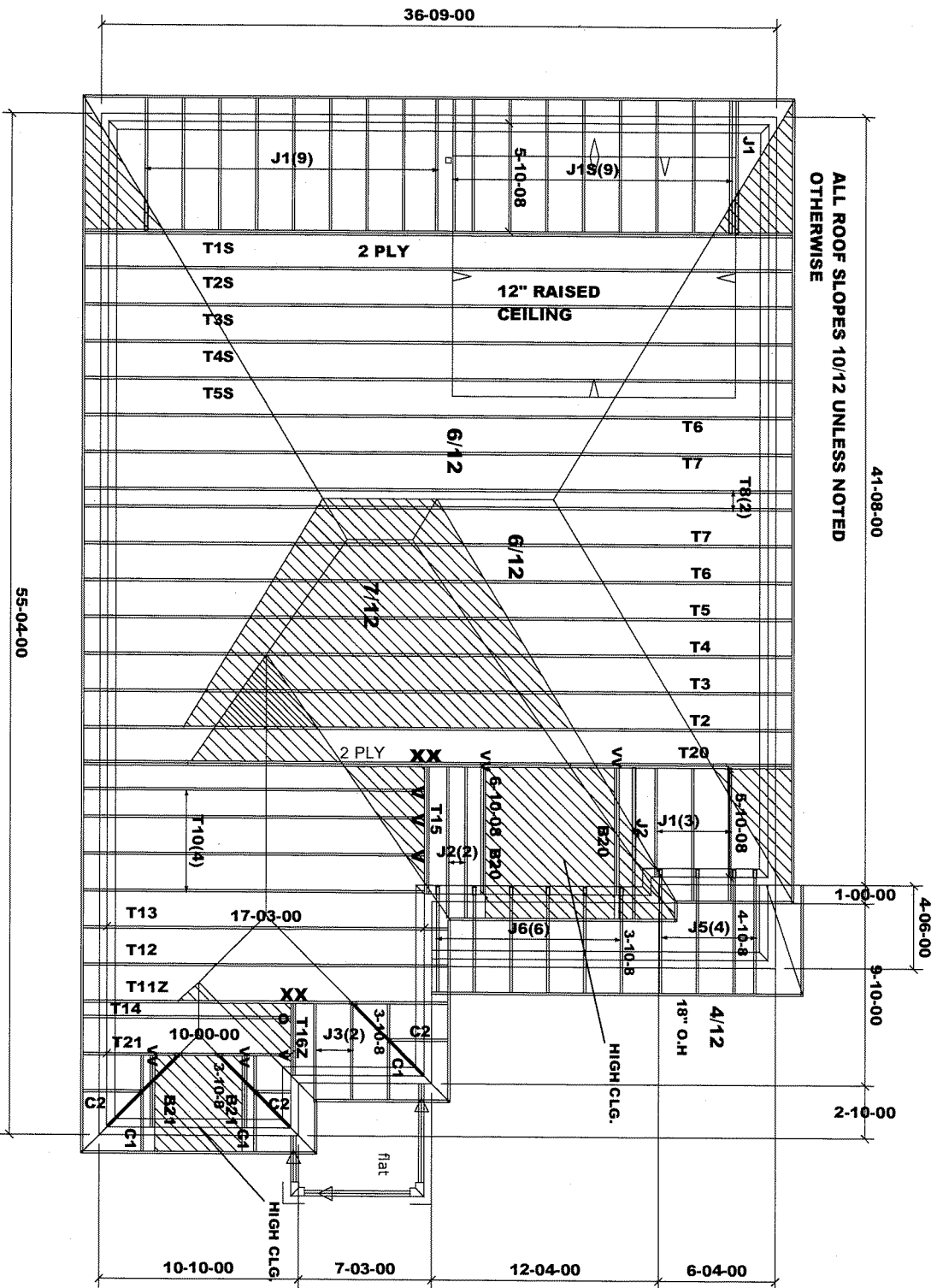
Builder / Location	ROYAL PINE HOMES / RICHMOND HILL		
Project	CENTREFIELD		
Date	2021-05-26	Scale	
	Media: DfGano	Designer: EEO	
	THESE DRAWINGS COME OR REDISTRIBUTED IN TAMAACK ROOF TRU		

Model / Elevation:

UNIT-4504 / B-STD.WITH COFF

ALL NOT BE REPRODUCED, PUBLISHED,
MANUFACTURE OF TRUSSES BY
INC IF UTILIZED FOR ANY OTHER

Mitek ver 8.4.2.286



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:

RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:

TCSL = 25.6 psf

TCDL = 6.0 psf

BCDL = 7.4 psf

HARDWARE:

LUS24 - (O)

LJS26DS - (V)

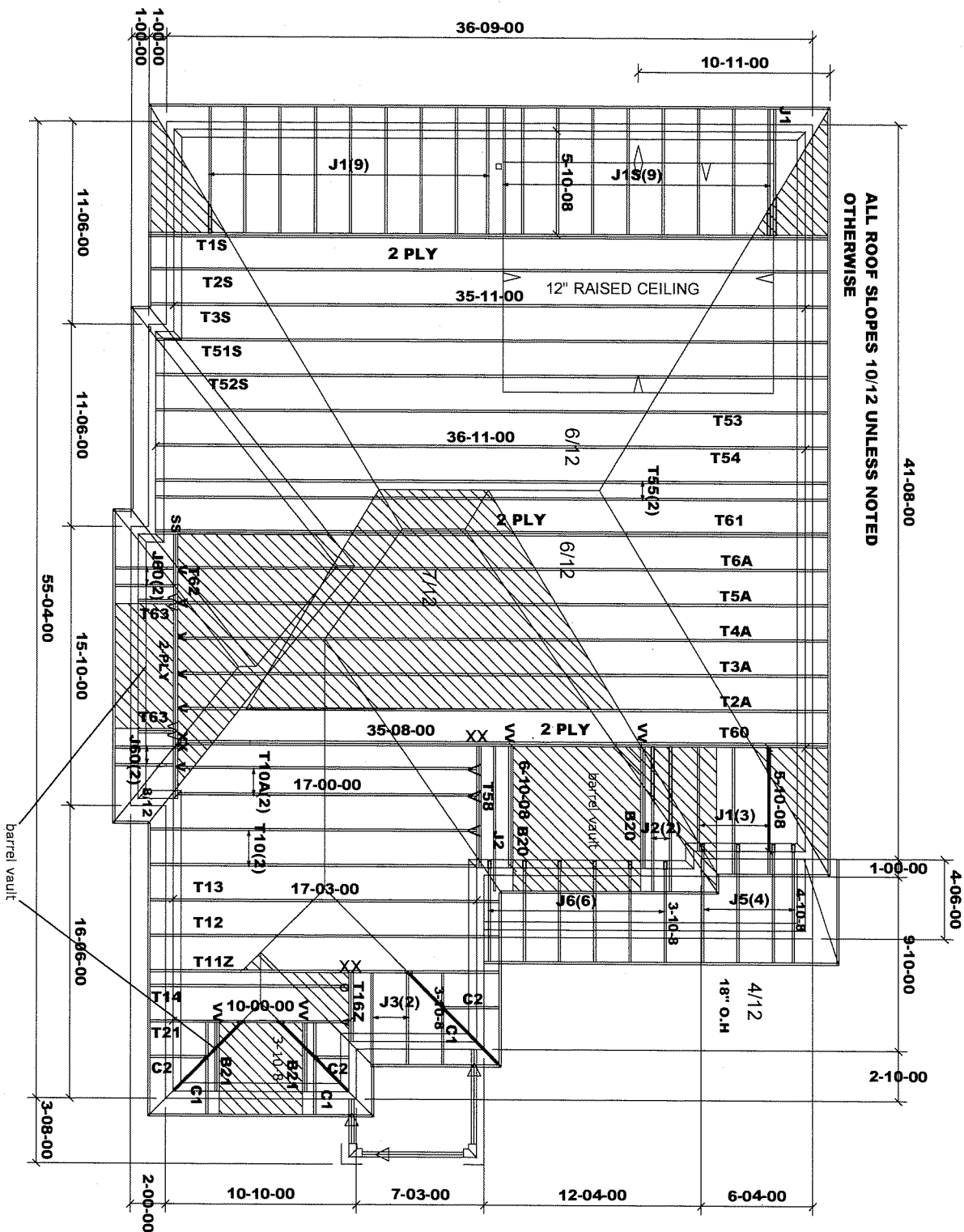
LUS26-2 - (W)

BEAMS:

$$B_{20} = B_{21} =$$

2 - 2x10 SPF #2

DENOTES:
CONVENTIONAL
FRAMING



ALL ROOF SLOPES 10/12 UNLESS NOTED OTHERWISE

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9

Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:

TCSL = 25.6 psf
TCOL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:

LUS24 - (O)
LUS26DS - (W)
HGUS26-2 - (XX)
LUS26-2 - (WV)
HGUS26-2 - (SS)

BEAMS:

B20 = B21 = B22 = B60
2 - 2x10 SPF #2

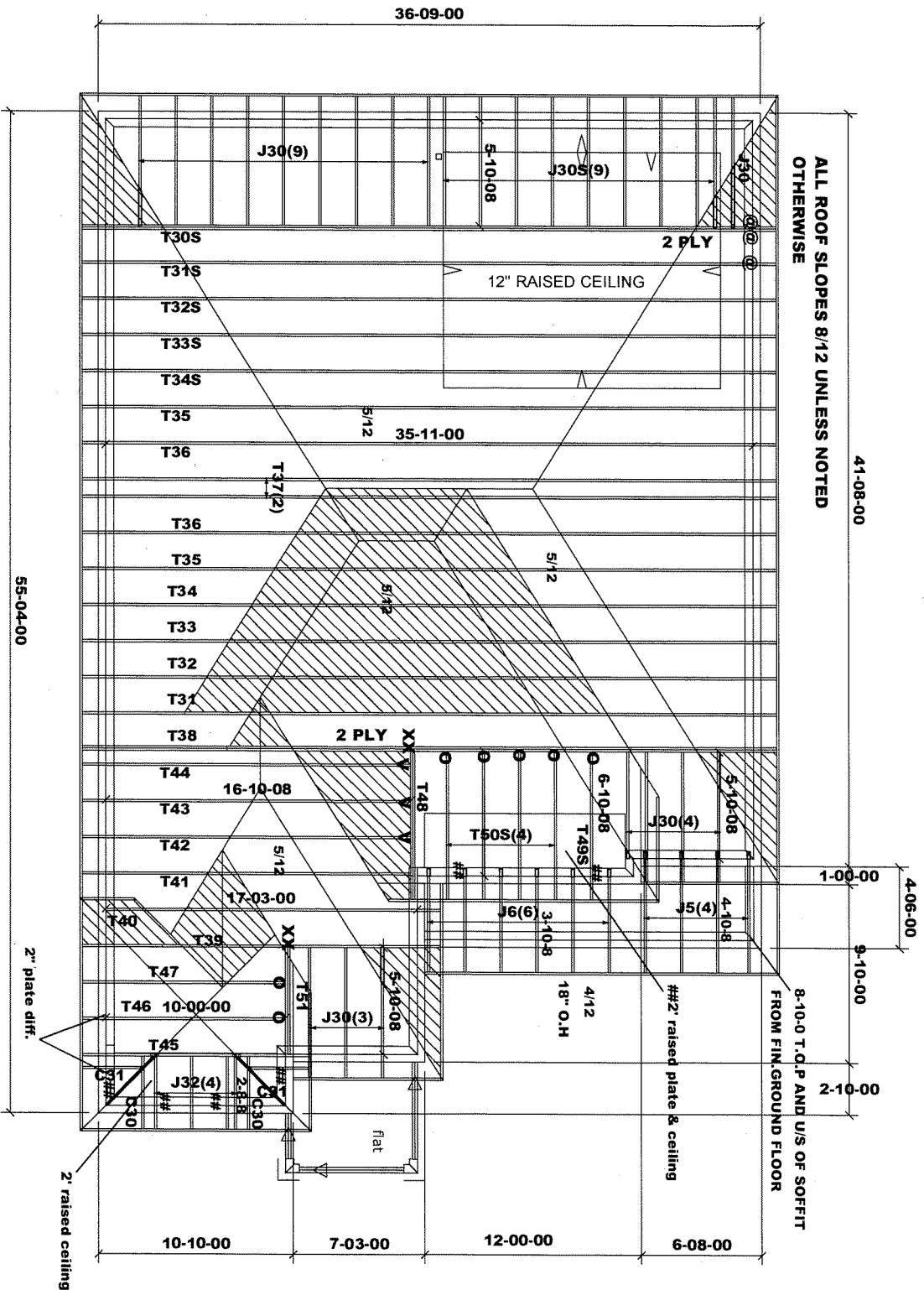
CONVENTIONAL FRAMING

M14044

		Job Track: 51004 Plan Log: 201943 Layout ID: 411071		Builder / Location: ROYAL PINE HOMES / RICHMOND HILL Project: CENTREFIELD Date: 2021-05-26 Sales: Mario DiCaro Designer: EEO JYG		Model / Elevation: UNIT 4504-FLANKAGE / B-STD.WITH OPT. COFF. THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Mitek ver 8.4.2.286	
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CEDED, PUBLISHED,
JESSES BY
ANY OTHER
Mitak ver 8.4.2.28



DENOTES:
CONVENTIONAL
FRAMING



Job Track: **51004**

Plan Log: **201943**

Layout ID: **418992**

Builder / Location: **ROYAL PINE HOMES / RICHMOND HILL**

Project: **CENTREFIELD**

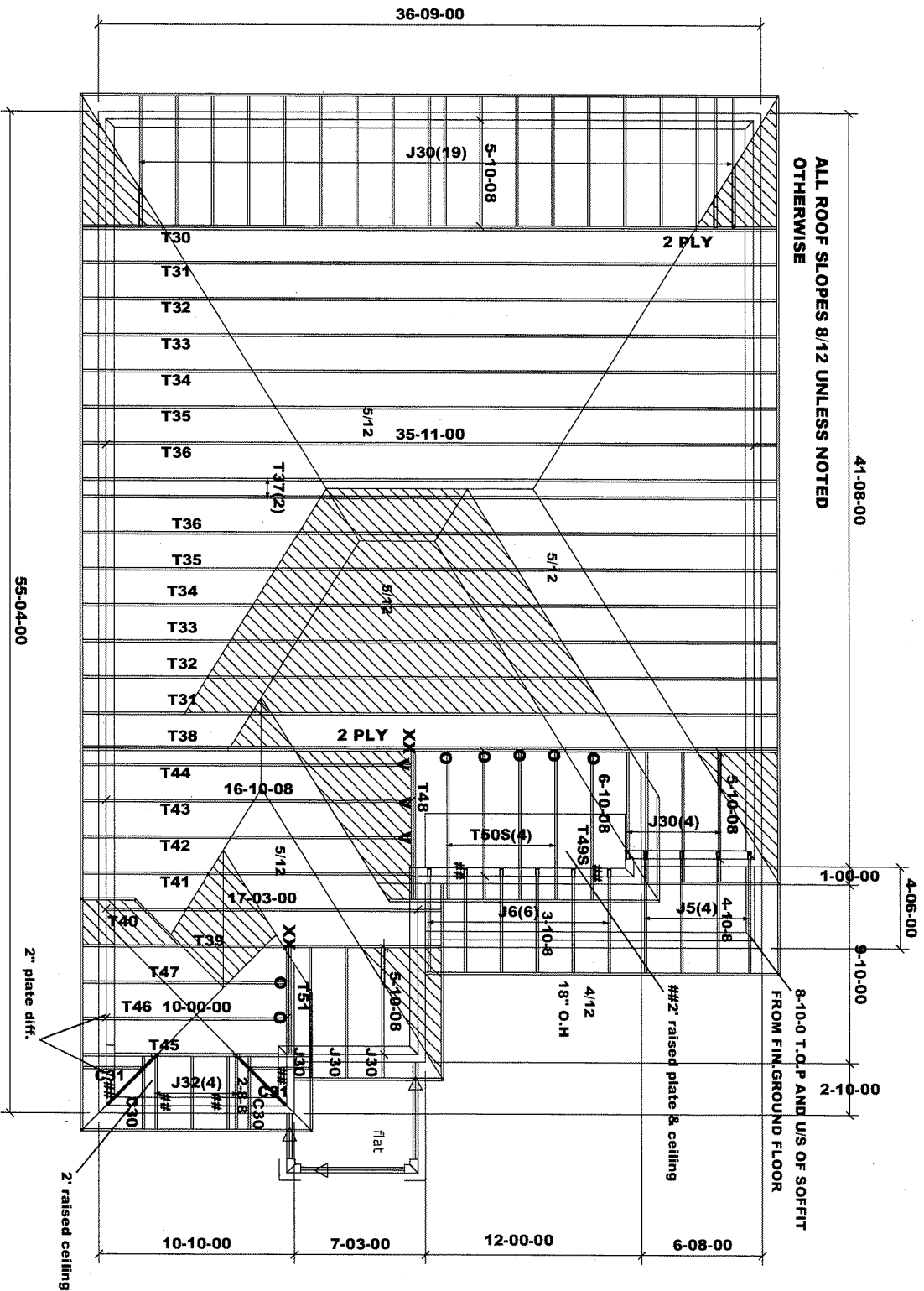
Date: **2021-05-26** Sales: **Mario DiCarro** Designer: **EEO**

THESE DRAWINGS CO
OR REDISTRIBUTED IN
TAMARACK ROOF TRU

UNIT - 4504 / C-STD.OR 5 BRM

ALL NOT BE REPRODUCED, PUBLISHED,
MANUFACTURE OF TRUSSES BY
S INC IF UTILIZED FOR ANY OTHER

ANY OTHER



DENOTES:
CONVENTIONAL
FRAMING

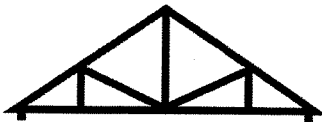
HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DESIGN LOADS:

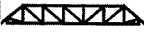
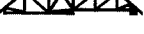
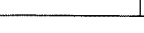
DESIGN CONFORMS WITH CBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf
DESIGN LOADS:

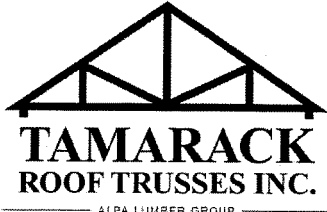
All conventional framing to conform with Part 9 of O.B.C. 2012 / (2019 amendment). Roof rafter's that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

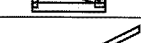
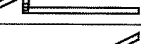
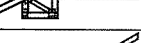
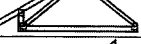
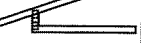
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER	Job Track: 51004	PlanLog: 201943		Layout ID: 406403			
Builder: ROYAL PINE HOMES	Ref #: 11638	Page: 1 of 3		Date: 05-28-2021				
Project: CENTREFIELD	Designer: Jewel Thottakath	Sales Rep: Mario DiCano						
Location: RICHMOND HILL								
Model: UNIT - 4504								
Lot #:								
Elevation: A-STD.WITH OPT.COFF.								

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	10 / 12	35-11-00	4-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	421.03 264.00		
	1	T2 Hip	10 / 12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	148.9 93.50		
	1	T2S Hip	10 / 12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	161.94 104.50		
	1	T3 Hip	10 / 12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.38 95.00		
	1	T3S Hip	10 / 12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	167.69 106.83		
	1	T4 Hip	10 / 12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	166.76 103.83		
	1	T4S Hip	10 / 12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	175.18 109.83		
	1	T5 Hip	10 / 12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	165.21 103.00		
	1	T5S Hip	10 / 12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	180.87 115.67		
	2	T6 Hip	10 / 12	35-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	367.4 230.00		
	2	T7 Hip	10 / 12	35-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	351.11 220.00		
	2	T8 Hip	10 / 12	35-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	384.63 238.00		
	1 2-ply	T9 Roof Special Girder	10 / 12	35-11-00	5-03-15	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	436.66 262.00		
	4	T10 Common	10 / 12	17-03-00	8-09-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	337.62 217.33		

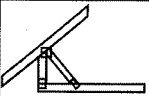
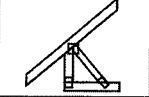
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST										
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT - 4504 Lot #: Elevation: A-STD.WITH OPT.COFF.							Job Track: 51004 PlanLog: 201943 Layout ID: 406403 Ref #: 11638 Page: 2 of 3 Date: 05-28-2021 Designer: Jewel Thottakath Sales Rep: Mario DiCano			

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T11 Hip Girder	10 / 12	17-03-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	87.7 56.50		
	1	T12 Hip	10 / 12	17-03-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	75.92 48.00		
	1	T13 Hip	10 / 12	17-03-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	83.6 54.33		
	2	T14 Common	10 / 12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	90.67 58.67		
	2	T14S Roof Special	10 / 12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	109.39 76.00		
	1 2-ply	T15 Jack-Closed Girder	7 / 12	6-10-08	5-03-09	2 x 4 2 x 6		1-03-07 5-03-09	70.65 45.00		
	1 2-ply	T16 Jack-Closed Girder	10 / 12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	14	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		
	9	J1S Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	187.64 132.00		
	6	J2 Jack-Open	7 / 12	6-10-08	5-03-09	2 x 4	1-03-08	1-03-07 5-03-09	159.83 102.00		
	2	J3 Jack-Open	10 / 12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	31.15 20.33		
	4	J4W Jack-Open	4 / 12	6-10-08	2-10-11	2 x 4	1-03-08	7-03 2-10-11	89.61 58.00		
	4	J5 Jack-Open	4 / 12	4-10-08	2-08-11	2 x 4	1-09-08	1-01-03 2-08-11	57.46 37.33		
	6	J6 Jack-Open	4 / 12	3-10-08	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	72.13 48.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT - 4504 Lot #: Elevation: A-STD.WITH OPT.COFF.	Job Track: 51004 PlanLog: 201943 Layout ID: 406403 Ref #: 11638 Page: 3 of 3 Date: 05-28-2021 Designer: Jewel Thottakath Sales Rep: Mario DiCano

Roof Trusses

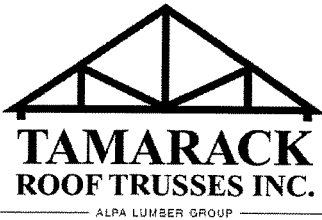
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	C1 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	C2 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.74 7.00		

TOTAL # TRUSS= 80
TOTAL BFT OF ALL TRUSSES= 3193.64
BFT.
TOTAL WEIGHT OF ALL TRSSES 5031.16
LBS

HARDWARE

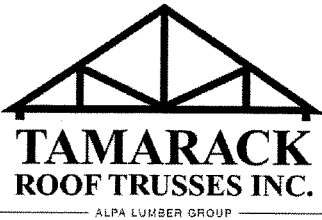
QTY	TYPE	MODEL	LENGTH
4	Hardware	H2.5T	
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
2	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 11

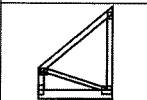
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST						
	Lumber Yard: TAMARACK LUMBER						Job Track: 51004
	Builder: ROYAL PINE HOMES						PlanLog: 201943
	Project: CENTREFIELD						Layout ID: 411069
	Location: RICHMOND HILL						Ref #: 11638
	Model: UNIT 4504- FLANKAGE						Page: 1 of 4
Lot #:						Date: 05-28-2021	
Elevation: A-STD.WITH OPT.COFF						Designer:	
						Sales Rep: Mario DiCano	

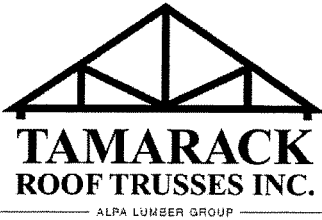
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	10 / 12	35-11-00	4-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	393.25 247.33		
	1	T2A Hip	10 / 12	35-06-08	5-01-04	2 x 4	1-03-08	1-07-11 1-07-11	146.42 91.67		
	1	T2S Hip	10 / 12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	161.65 104.17		
	1	T3A Hip	10 / 12	35-06-08	6-01-04	2 x 4	1-03-08	1-07-11 1-07-11	147.89 93.67		
	1	T3S Hip	10 / 12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.9 104.67		
	1	T4A Hip	10 / 12	35-06-08	7-01-04	2 x 4	1-03-08	1-07-11 1-07-11	164.25 102.50		
	1	T5A Hip	10 / 12	35-06-08	8-01-04	2 x 4	1-03-08	1-07-11 1-07-11	162.69 101.67		
	1	T6A Hip	10 / 12	35-08-00	9-01-04	2 x 4	1-03-08	1-07-11 1-10-03	181.22 113.67		
	1 2-ply	T9A Roof Special Girder	10 / 12	35-06-08	5-03-15	2 x 6	1-03-08	1-07-11 1-07-11	428.89 258.67		
	2	T10 Common	10 / 12	17-03-00	8-09-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	168.81 108.67		
	2	T10A Common	10 / 12	17-00-00	8-09-15	2 x 4		1-10-03 1-07-11	160.09 102.33		
	1	T11 Hip Girder	10 / 12	17-03-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	87.7 56.50		
	1	T12 Hip	10 / 12	17-03-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	75.92 48.00		
	1	T13 Hip	10 / 12	17-03-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	83.6 54.33		

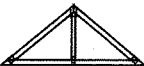
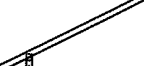
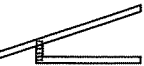
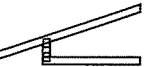
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER						Job Track: 51004	
	Builder: ROYAL PINE HOMES						PlanLog: 201943	
	Project: CENTREFIELD						Layout ID: 411069	
	Location: RICHMOND HILL						Ref #: 11638	
	Model: UNIT 4504- FLANKAGE						Page: 2 of 4	
Lot #:						Date: 05-28-2021		
Elevation: A-STD.WITH OPT.COFF						Designer:		
						Sales Rep: Mario DiCano		

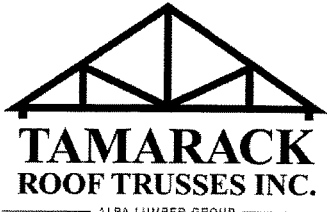
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T14 Common	10 /12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	90.67 58.67		
	2	T14S Roof Special	10 /12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	109.39 76.00		
	1 2-ply	T16 Jack-Closed Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1 2-ply	T50 Hip Girder	10 /12	36-11-00	10-01-00	2 x 4 2 x 8	1-03-08	1-07-11 9-11	512.86 312.00		
	1	T51S Hip	10 /12	36-11-00	7-01-04	2 x 4 2 x 6	1-03-08	9-11 1-07-11	198.6 123.50		
	1	T52S Hip	10 /12	36-11-00	8-01-04	2 x 4 2 x 6	1-03-08	9-11 1-07-11	209.87 130.50		
	1	T53 Hip	10 /12	36-11-00	9-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-11	211.59 131.17		
	1	T54 Hip	10 /12	36-11-00	10-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-11	204.31 124.50		
	2	T55 Hip	10 /12	36-11-00	11-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-11	442.84 273.33		
	1	T56S Roof Special	8 /12	15-00-00	6-04-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	75.59 48.83		
	1 2-ply	T57 Common Girder	8 /12	15-00-00	6-04-13	2 x 4 2 x 8		1-04-13 1-04-13	177 108.00		
	1 2-ply	T58 Jack-Closed Girder	7 /12	6-10-08	5-03-09	2 x 4 2 x 6		1-03-07 5-03-09	70.65 45.00		
	1	V01 Valley	10 /12	19-11-14	8-03-15	2 x 4			66.48 41.67		
	1	V02 Valley	10 /12	17-07-01	7-03-15	2 x 4			55.99 34.17		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST										
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT 4504- FLANKAGE Lot #: Elevation: A-STD.WITH OPT.COFF						Job Track: 51004 PlanLog: 201943 Layout ID: 411069 Ref #: 11638 Page: 3 of 4 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano				

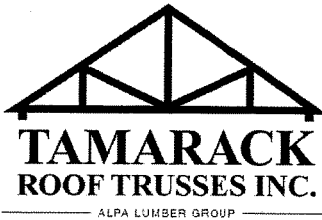
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	V03 Valley	10 /12	15-02-04	6-03-15	2 x 4			47.24 30.00		
	1	V04 Valley	10 /12	12-09-07	5-03-15	2 x 4			38.5 23.83		
	1	V05 Valley	10 /12	10-06-07	4-04-11	2 x 4			29.53 18.67		
	1	V06 Valley	10 /12	8-01-10	3-04-11	2 x 4			22.42 14.17		
	1	V07 Valley	10 /12	5-08-14	2-04-11	2 x 4			15.31 10.33		
	14	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		
	9	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	187.66 132.00		
	6	J2 Jack-Open	7 /12	6-10-08	5-03-09	2 x 4	1-03-08	1-03-07 5-03-09	159.83 102.00		
	2	J3 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	31.15 20.33		
	4	J4W Jack-Open	4 /12	6-10-08	2-10-11	2 x 4	1-03-08	7-03 2-10-11	89.03 58.00		
	4	J5 Jack-Open	4 /12	4-10-08	2-08-11	2 x 4	1-09-08	1-01-03 2-08-11	57.46 37.33		
	6	J6 Jack-Open	4 /12	3-10-08	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	72.13 48.00		
	1	C1 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	C2 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.99 7.00		

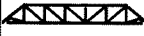
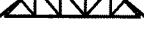
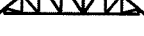
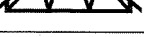
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT 4504- FLANKAGE Lot #: Elevation: A-STD.WITH OPT.COFF	Job Track: 51004 PlanLog: 201943 Layout ID: 411069 Ref #: 11638 Page: 4 of 4 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano

Roof Trusses											
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
TOTAL # TRUSS= 91 TOTAL BFT OF ALL TRUSSES= 3783.84 BFT. TOTAL WEIGHT OF ALL TRSSES 6002.69 LBS											

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
1	Hardware	HGUS28-2	
10	Hardware	LJS26DS	
2	Hardware	LUS24	
1	Hardware	LUS26-2	
TOTAL NUMBER OF ITEMS= 16			

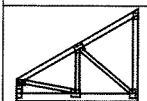
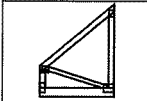
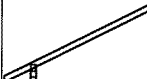
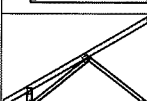
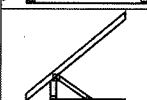
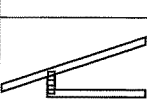
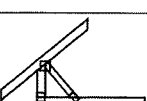
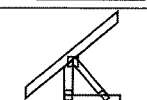
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER	Job Track: 51004	PlanLog: 201943	
Builder: ROYAL PINE HOMES	Layout ID: 418988	Ref #: 11638		
Project: CENTREFIELD	Page: 1 of 2	Date: 05-28-2021		
Location: RICHMOND HILL	Designer:	Sales Rep: Mario DiCano		
Model: UNIT - 4504				
Lot #:				
Elevation: A-STD.OR 5 BRM				

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10 / 12	35-11-00	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	405.42 241.33		
	2	T2 Hip	10 / 12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	297.79 187.00		
	2	T3 Hip	10 / 12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	300.76 190.00		
	2	T4 Hip	10 / 12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	327.29 204.33		
	2	T5 Hip	10 / 12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	323.18 202.00		
	2	T6 Hip	10 / 12	35-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	367.4 230.00		
	2	T7 Hip	10 / 12	35-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	351.11 220.00		
	2	T8 Hip	10 / 12	35-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	384.63 238.00		
	1 2-ply	T9 Roof Special Girder	10 / 12	35-11-00	5-03-15	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	395.43 238.00		
	4	T10 Common	10 / 12	17-03-00	8-09-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	337.62 217.33		
	1	T11 Hip Girder	10 / 12	17-03-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	87.7 56.50		
	1	T12 Hip	10 / 12	17-03-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	75.92 48.00		
	1	T13 Hip	10 / 12	17-03-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	83.6 54.33		
	2	T14 Common	10 / 12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	90.67 58.67		

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
	Builder:	ROYAL PINE HOMES	PlanLog:	201943
	Project:	CENTREFIELD	Layout ID:	418988
	Location:	RICHMOND HILL	Ref #	11638
	Model:	UNIT - 4504	Page:	2 of 2
	Lot #:		Date:	05-28-2021
	Elevation:	A-STD.OR 5 BRM	Designer:	
		Sales Rep:	Mario DiCano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T14S Roof Special	10 / 12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	109.39 76.00		
	1 2-ply	T15 Jack-Closed Girder	7 / 12	6-10-08	5-03-09	2 x 4 2 x 6		1-03-07 5-03-09	70.65 45.00		
	1 2-ply	T16 Jack-Closed Girder	10 / 12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	23	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	386.27 245.33		
	6	J2 Jack-Open	7 / 12	6-10-08	5-03-09	2 x 4	1-03-08	1-03-07 5-03-09	159.83 102.00		
	2	J3 Jack-Open	10 / 12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	31.15 20.33		
	4	J4W Jack-Open	4 / 12	6-10-08	2-10-11	2 x 4	1-03-08	7-03 2-10-11	89.61 58.00		
	4	J5 Jack-Open	4 / 12	4-10-08	2-08-11	2 x 4	1-09-08	1-01-03 2-08-11	57.46 37.33		
	6	J6 Jack-Open	4 / 12	3-10-08	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	72.13 48.00		
	1	C1 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	C2 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.74 7.00		

TOTAL # TRUSS= 80

TOTAL BFT OF ALL TRUSSES= 3062.14

BFT.

TOTAL WEIGHT OF ALL TRSSES 4869.95 LBS

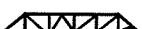
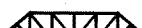
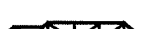
HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	H2.5T	
2	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
2	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 11

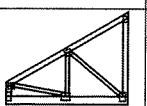
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER	Job Track: 51004			PlanLog: 201943			
Builder: ROYAL PINE HOMES				Layout ID: 411070				
Project: CENTREFIELD				Ref #: 11638				
Location: RICHMOND HILL				Page: 1 of 4				
Model: UNIT 4504-FLANKAGE				Date: 05-28-2021				
Lot #:				Designer:				
Elevation: A-5 BRM WITH OPT. COFF.				Sales Rep: Mario DiCano				

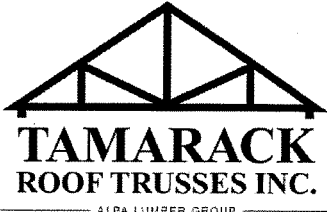
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	10 / 12	35-11-00	4-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	393.25 247.33		
	1	T2A Hip	10 / 12	35-06-08	5-01-04	2 x 4	1-03-08	1-07-11 1-07-11	146.42 91.67		
	1	T2S Hip	10 / 12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	161.65 104.17		
	1	T3A Hip	10 / 12	35-06-08	6-01-04	2 x 4	1-03-08	1-07-11 1-07-11	147.89 93.67		
	1	T3S Hip	10 / 12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.9 104.67		
	1	T4A Hip	10 / 12	35-06-08	7-01-04	2 x 4	1-03-08	1-07-11 1-07-11	161.19 100.83		
	1	T5A Hip	10 / 12	35-06-08	8-01-04	2 x 4	1-03-08	1-07-11 1-07-11	159.12 99.67		
	1	T6A Hip	10 / 12	35-08-00	9-01-04	2 x 4	1-03-08	1-07-11 1-10-03	181.22 113.67		
	1 2-ply	T9A Roof Special Girder	10 / 12	35-06-08	5-03-15	2 x 6	1-03-08	1-07-11 1-07-11	387.79 235.00		
	2	T10 Common	10 / 12	17-03-00	8-09-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	168.81 108.67		
	2	T10A Common	10 / 12	17-00-00	8-09-15	2 x 4		1-10-03 1-07-11	160.09 102.33		
	1	T11 Hip Girder	10 / 12	17-03-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	87.7 56.50		
	1	T12 Hip	10 / 12	17-03-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	75.92 48.00		
	1	T13 Hip	10 / 12	17-03-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	83.6 54.33		

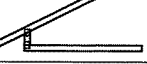
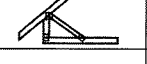
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT 4504-FLANKAGE Lot #: Elevation: A-5 BRM WITH OPT. COFF.	Job Track: 51004 PlanLog: 201943 Layout ID: 411070 Ref #: 11638 Page: 2 of 4 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano						

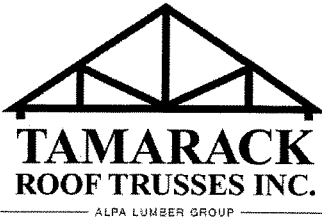
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T14 Common	10 / 12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	90.67 58.67		
	2	T14S Roof Special	10 / 12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	109.39 76.00		
	1 2-ply	T16 Jack-Closed Girder	10 / 12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1 2-ply	T50 Hip Girder	10 / 12	36-11-00	10-01-00	2 x 4 2 x 8	1-03-08	1-07-11 9-11	469.75 287.00		
	1	T52 Hip	10 / 12	36-11-00	8-01-04	2 x 4 2 x 6	1-03-08	9-11 1-07-11	192.37 118.33		
	1	T53 Hip	10 / 12	36-11-00	9-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-11	211.59 131.17		
	1	T54 Hip	10 / 12	36-11-00	10-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-11	204.31 124.50		
	2	T55 Hip	10 / 12	36-11-00	11-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-11	442.84 273.33		
	1	T56S Roof Special	8 / 12	15-00-00	6-04-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	75.59 48.83		
	1 2-ply	T57 Common Girder	8 / 12	15-00-00	6-04-13	2 x 4 2 x 8		1-04-13 1-04-13	162.2 102.33		
	1 2-ply	T58 Jack-Closed Girder	7 / 12	6-10-08	5-03-09	2 x 4 2 x 6		1-03-07 5-03-09	70.65 45.00		
	1	T510 Hip	10 / 12	36-11-00	7-01-10	2 x 6	1-03-08	9-11 1-07-11	202.06 124.33		
	1	V01 Valley	10 / 12	19-11-14	8-03-15	2 x 4			66.48 41.67		
	1	V02 Valley	10 / 12	17-07-01	7-03-15	2 x 4			55.98 34.17		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT 4504-FLANKAGE Lot #: Elevation: A-5 BRM WITH OPT. COFF.	Job Track: 51004 PlanLog: 201943 Layout ID: 411070 Ref #: 11638 Page: 3 of 4 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	V03 Valley	10 /12	15-02-04	6-03-15	2 x 4			47.24 30.00		
	1	V04 Valley	10 /12	12-09-07	5-03-15	2 x 4			38.5 23.83		
	1	V05 Valley	10 /12	10-06-07	4-04-11	2 x 4			29.53 18.67		
	1	V06 Valley	10 /12	8-01-10	3-04-11	2 x 4			22.42 14.17		
	1	V07 Valley	10 /12	5-08-14	2-04-11	2 x 4			15.31 10.33		
	15	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	8	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	166.85 117.33		
	6	J2 Jack-Open	7 /12	6-10-08	5-03-09	2 x 4	1-03-08	1-03-07 5-03-09	159.83 102.00		
	2	J3 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	31.15 20.33		
	4	J4W Jack-Open	4 /12	6-10-08	2-10-11	2 x 4	1-03-08	7-03 2-10-11	89.03 58.00		
	4	J5 Jack-Open	4 /12	4-10-08	2-08-11	2 x 4	1-09-08	1-01-03 2-08-11	57.46 37.33		
	6	J6 Jack-Open	4 /12	3-10-08	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	72.13 48.00		
	1	C1 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	C2 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.99 7.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT 4504-FLANKAGE Lot #: Elevation: A-5 BRM WITH OPT. COFF.	Job Track: 51004 PlanLog: 201943 Layout ID: 411070 Ref #: 11638 Page: 4 of 4 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
TOTAL # TRUSS= 91		TOTAL BFT OF ALL TRUSSES= 3710.49					BFT.	TOTAL WEIGHT OF ALL TRSSES 5878.98 LBS			

HARDWARE

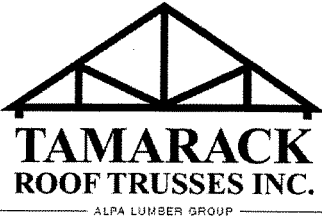
QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
2	Hardware	HGUS26-2	
1	Hardware	HGUS28-2	
10	Hardware	LJS26DS	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 16

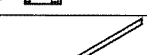
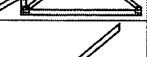
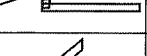
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER	Job Track: 51004						
Builder: ROYAL PINE HOMES	PlanLog: 201943							
Project: CENTREFIELD	Layout ID: 406516							
Location: RICHMOND HILL	Ref #: 11638							
Model: UNIT- 4504	Page: 1 of 3							
Lot #:	Date: 05-28-2021							
Elevation: B-STD.WITH COFF	Designer: Jewel Thottakath							
	Sales Rep: Mario DiCano							

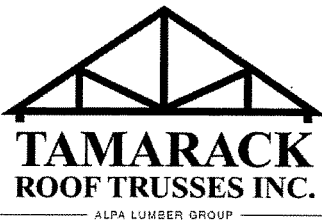
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	10 /12	35-11-00	4-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	393.25 247.33		
	1	T2 Hip	10 /12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	148.9 93.50		
	1	T2S Hip	10 /12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	161.54 104.17		
	1	T3 Hip	10 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.38 95.00		
	1	T3S Hip	10 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.9 104.67		
	1	T4 Hip	10 /12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.65 102.17		
	1	T4S Hip	10 /12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	170.64 107.33		
	1	T5 Hip	10 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	161.59 101.00		
	1	T5S Hip	10 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	178.67 114.33		
	2	T6 Hip	10 /12	35-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	367.4 230.00		
	2	T7 Hip	10 /12	35-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	351.11 220.00		
	2	T8 Hip	10 /12	35-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	384.63 238.00		
	4	T10 Common	10 /12	17-03-00	8-09-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	337.62 217.33		
	1	T11Z Hip Girder	10 /12	17-03-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	87.7 56.50		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST										
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT- 4504 Lot #: Elevation: B-STD.WITH COFF						Job Track: 51004 PlanLog: 201943 Layout ID: 406516 Ref # 11638 Page: 2 of 3 Date: 05-28-2021 Designer: Jewel Thottakath Sales Rep: Mario DiCano				

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T12 Hip	10 /12	17-03-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	75.92 48.00		
	1	T13 Hip	10 /12	17-03-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	83.6 54.33		
	1	T14 Common	10 /12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	45.34 29.33		
	1 2-ply	T15 Jack-Closed Girder	7 /12	6-10-08	5-03-09	2 x 4 2 x 6		1-03-07 5-03-09	70.65 45.00		
	1 2-ply	T16Z Jack-Closed Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1 2-ply	T20 Roof Special Girder	10 /12	35-11-00	5-03-15	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	436.5 262.67		
	1	T21 Hip Girder	10 /12	10-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	56.9 36.83		
	13	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	218.33 138.67		
	9	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	187.64 132.00		
	3	J2 Jack-Open	7 /12	6-10-08	5-03-09	2 x 4	1-03-08	1-03-07 5-03-09	79.92 51.00		
	2	J3 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	31.15 20.33		
	4	J5 Jack-Open	4 /12	4-10-08	2-08-11	2 x 4	1-09-08	1-01-03 2-08-11	57.46 37.33		
	6	J6 Jack-Open	4 /12	3-10-08	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	72.13 48.00		
	3	C1 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	36.06 25.00		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT- 4504 Lot #: Elevation: B-STD.WITH COFF	Job Track: 51004 PlanLog: 201943 Layout ID: 406516 Ref #: 11638 Page: 3 of 3 Date: 05-28-2021 Designer: Jewel Thottakath Sales Rep: Mario DiCano

Roof Trusses

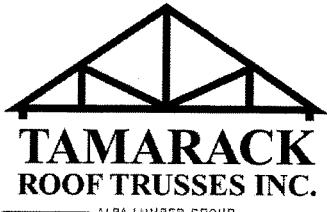
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	C2 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	29.22 21.00		

TOTAL # TRUSS= 74 TOTAL BFT OF ALL TRUSSES= 3010.15 BFT. TOTAL WEIGHT OF ALL TRSSES 4744.93 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
1	Hardware	LUS24	
4	Hardware	LUS26-2	

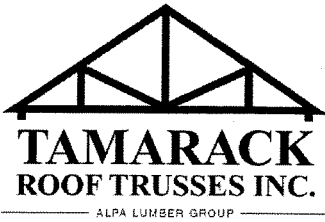
TOTAL NUMBER OF ITEMS= 11

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT 4504- FLANKAGE Lot #: Elevation: B-STD.WITH OPT. COFF.	Job Track: 51004 PlanLog: 201943 Layout ID: 411071 Ref #: 11638 Page: 1 of 3 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	10 / 12	35-11-00	4-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	393.25 247.33		
	1	T2A Hip	10 / 12	35-06-08	5-01-04	2 x 4	1-03-08	1-07-11 1-07-11	146.42 91.67		
	1	T2S Hip	10 / 12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	161.65 104.17		
	1	T3A Hip	10 / 12	35-06-08	6-01-04	2 x 4	1-03-08	1-07-11 1-07-11	147.89 93.67		
	1	T3S Hip	10 / 12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.9 104.67		
	1	T4A Hip	10 / 12	35-06-08	7-01-04	2 x 4	1-03-08	1-07-11 1-07-11	161.19 100.83		
	1	T5A Hip	10 / 12	35-06-08	8-01-04	2 x 4	1-03-08	1-07-11 1-07-11	159.12 99.67		
	1	T6A Hip	10 / 12	35-08-00	9-01-04	2 x 4	1-03-08	1-07-11 1-10-03	181.22 113.67		
	2	T10 Common	10 / 12	17-03-00	8-09-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	168.81 108.67		
	2	T10A Common	10 / 12	17-00-00	8-09-15	2 x 4		1-10-03 1-07-11	160.09 102.33		
	1	T11Z Hip Girder	10 / 12	17-03-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	87.7 56.50		
	1	T12 Hip	10 / 12	17-03-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	75.92 48.00		
	1	T13 Hip	10 / 12	17-03-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	83.6 54.33		
	1	T14 Common	10 / 12	10-00-00	5-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	45.34 29.33		

DELIVERY SHIPLIST

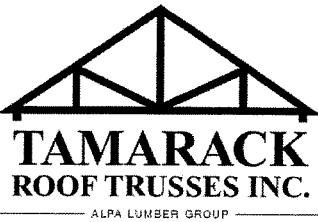


Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: UNIT 4504- FLANKAGE
 Lot #:
 Elevation: B-STD.WITH OPT. COFF.

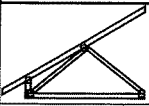
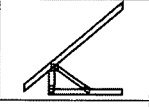
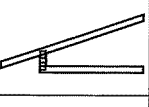
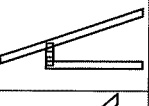
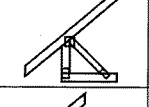
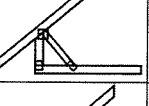
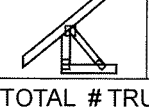
Job Track: 51004
 PlanLog: 201943
 Layout ID: 411071
 Ref #: 11638
 Page: 2 of 3
 Date: 05-28-2021
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T16Z Jack-Closed Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1	T21 Hip Girder	10 /12	10-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	55.23 35.50		
	1	T51S Hip	10 /12	36-11-00	7-01-04	2 x 4 2 x 6	1-03-08	9-11 1-07-11	194.92 121.50		
	1	T52S Hip	10 /12	36-11-00	8-01-04	2 x 4 2 x 6	1-03-08	9-11 1-07-11	206.19 128.17		
	1	T53 Hip	10 /12	36-11-00	9-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-11	211.31 131.17		
	1	T54 Hip	10 /12	36-11-00	10-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-11	204.03 125.00		
	2	T55 Hip	10 /12	36-11-00	11-01-04	2 x 4 2 x 6	1-03-08	1-07-11 9-11	442.3 271.33		
	1 2-ply	T58 Jack-Closed Girder	7 /12	6-10-08	5-03-09	2 x 4 2 x 6		1-03-07 5-03-09	70.65 45.00		
	1 2-ply	T60 Roof Special Girder	10 /12	35-08-00	5-03-15	2 x 6	1-03-08	1-07-11 1-10-03	428.74 259.33		
	1 2-ply	T61 Hip Girder	10 /12	36-11-00	10-01-06	2 x 6 2 x 8	1-03-08	1-07-11 9-11	550 328.67		
	1 2-ply	T62 Hip Girder	8 /12	15-00-00	3-03-11	2 x 6 2 x 8		1-04-13 1-04-13	184.85 114.67		
	2 2-ply	T63 Jack-Closed	10 /12	2-00-00	3-03-11	2 x 4 2 x 6		1-07-11 3-03-11	49.09 33.33		
	13	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	218.33 138.67		
	9	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	187.66 132.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST										
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT 4504- FLANKAGE Lot #: Elevation: B-STD.WITH OPT. COFF.						Job Track: 51004 PlanLog: 201943 Layout ID: 411071 Ref #: 11638 Page: 3 of 3 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano				

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	J2 Jack-Open	7 /12	6-10-08	5-03-09	2 x 4	1-03-08	1-03-07 5-03-09	79.92 51.00		
	2	J3 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	31.15 20.33		
	4	J5 Jack-Open	4 /12	4-10-08	2-08-11	2 x 4	1-09-08	1-01-03 2-08-11	57.46 37.33		
	6	J6 Jack-Open	4 /12	3-10-08	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	72.13 48.00		
	4	J60 Jack-Open	10 /12	2-00-00	3-03-11	2 x 4	1-03-08	1-07-11 3-03-11	41.33 28.00		
	3	C1 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	36.06 25.00		
	3	C2 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	29.22 21.00		

TOTAL # TRUSS= 85

TOTAL BFT OF ALL TRUSSES= 3479.17

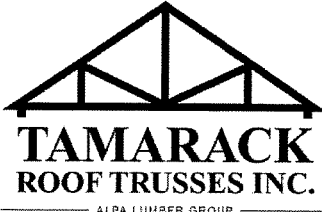
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TOTAL WEIGHT OF ALL TRSSES 5529.81 LBS

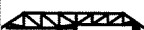
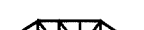
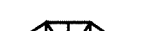
HARDWARE

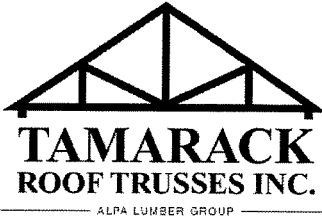
QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
1	Hardware	HGUS28-2	
11	Hardware	LJS26DS	
1	Hardware	LUS24	
6	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 22

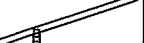
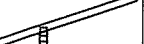
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER	Job Track: 51004	PlanLog: 201943					
Builder: ROYAL PINE HOMES	Layout ID: 411017							
Project: CENTREFIELD	Ref #: 11638							
Location: RICHMOND HILL	Page: 1 of 3							
Model: UNIT-4504	Date: 05-28-2021							
Lot #:	Designer:							
Elevation: C-STD.WITH OPT.COFF.	Sales Rep: Mario DiCano							

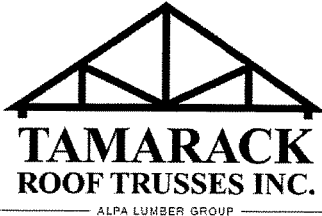
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T30S Hip Girder	8 /12	35-11-00	3-05-15	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	403.42 249.33		
	1	T31 Hip	8 /12	35-11-00	4-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	141.27 89.17		
	1	T31S Hip	8 /12	35-11-00	4-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	152.24 96.33		
	1	T32 Hip	8 /12	35-11-00	5-01-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	142.49 88.50		
	1	T32S Hip	8 /12	35-11-00	5-01-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	152.33 96.33		
	1	T33 Hip	8 /12	35-11-00	5-11-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	152.59 96.33		
	1	T33S Hip	8 /12	35-11-00	5-11-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	161.21 102.83		
	1	T34 Hip	8 /12	35-11-00	6-09-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	154.62 96.17		
	1	T34S Hip	8 /12	35-11-00	6-09-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	167.9 105.83		
	2	T35 Hip	8 /12	35-11-00	7-07-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	321.96 205.00		
	2	T36 Hip	8 /12	35-11-00	8-05-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	326.02 200.00		
	2	T37 Hip	8 /12	35-11-00	9-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	336.59 207.67		
	1 2-ply	T38 Roof Special Girder	8 /12	35-11-00	5-10-15	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	362.73 225.33		
	1	T39 Hip Girder	8 /12	17-03-00	3-05-15	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	80.08 48.83		

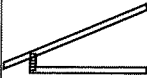
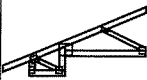
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST									
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT-4504 Lot #: Elevation: C-STD.WITH OPT.COFF.					Job Track: 51004 PlanLog: 201943 Layout ID: 411017 Ref #: 11638 Page: 2 of 3 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano				

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T40 Hip	8 /12	17-03-00	4-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	72.36 47.00		
	1	T41 Hip	8 /12	16-10-08	4-11-11	2 x 4	1-03-08 1-03-08	1-01-13 1-04-13	68.17 43.67		
	1	T42 Hip	8 /12	16-10-08	5-11-15	2 x 4	1-03-08	1-04-13 1-07-13	74.59 48.83		
	1	T43 Hip	8 /12	16-10-08	6-09-15	2 x 4	1-03-08	1-04-13 1-07-13	77.96 49.67		
	1	T44 Common	8 /12	16-10-08	7-01-13	2 x 4	1-03-08	1-04-13 1-07-13	73.73 47.33		
	1	T45 Hip Girder	8 /12	10-00-00	3-02-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	42.73 26.50		
	1	T46 Hip	8 /12	10-00-00	6-06-08	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	51.38 32.00		
	1	T47 Hip	8 /12	10-00-00	5-06-02	2 x 4	1-03-08	3-04-13 3-04-13	52.76 33.00		
	1 2-ply	T48 Jack-Closed Girder	5 /12	6-10-08	5-10-15	2 x 4 2 x 6	1-03-08	3-00-09 5-10-15	82.54 51.00		
	1	T49S Half Hip	5 /12	6-10-08	4-10-13	2 x 4	1-03-08	1-00-09 4-10-13	40.34 27.33		
	4	T50S Jack-Closed	5 /12	6-10-08	5-10-15	2 x 4	1-03-08	1-00-09 5-10-15	157.44 103.33		
	1 2-ply	T51 Jack-Closed Girder	5 /12	5-10-08	3-05-15	2 x 4 2 x 6		1-00-09 3-05-15	55 35.33		
	4	J5 Jack-Open	4 /12	4-10-08	2-08-11	2 x 4	1-09-08	1-01-03 2-08-11	57.46 37.33		
	6	J6 Jack-Open	4 /12	3-10-08	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	72.13 48.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST										
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT-4504 Lot #: Elevation: C-STD.WITH OPT.COFF.							Job Track: 51004 PlanLog: 201943 Layout ID: 411017 Ref #: 11638 Page: 3 of 3 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano			

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	17	J30 Jack-Open	5 /12	5-10-08	3-05-15	2 x 4	1-03-08	1-00-09 3-05-15	277.62 170.00		
	9	J30S Jack-Open	5 /12	5-10-08	3-05-15	2 x 4	1-03-08	1-00-09 2-05-15	186.58 130.50		
	4	J32 Jack-Open	8 /12	2-08-08	3-02-08	2 x 4	1-03-08	1-04-13 3-02-08	44.52 28.00		
	2	C30 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 9-09	1-04-13 2-08-02	20.23 14.00		
	2	C31 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 1-01	1-04-13 2-08-02	18.4 12.67		

TOTAL # TRUSS= 80

TOTAL BFT OF ALL TRUSSES= 2893.14

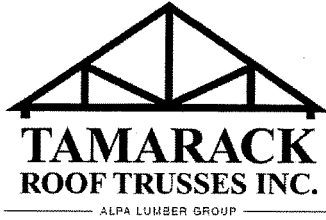
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TOTAL WEIGHT OF ALL TRSSES 4581.38 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
7	Hardware	LUS24	
1	Hardware	LUS26-2	
3	Hardware	TC26	

TOTAL NUMBER OF ITEMS= 16

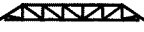


DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: UNIT - 4504
 Lot #:
 Elevation: C-STD.OR 5 BRM

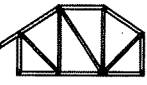
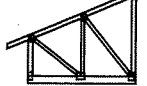
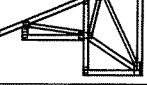
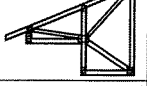
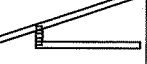
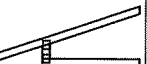
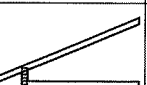
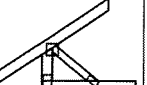
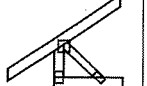
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 PlanLog: 201943
 Layout ID: 418992
 Ref #: 11638
 Page: 1 of 3
 Date: 05-28-2021
 Designer:
 Sales Rep: Mario DiCano

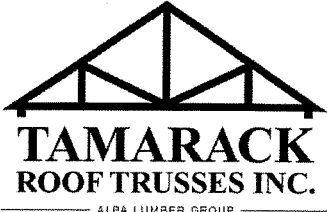
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T30 Hip Girder	8 /12	35-11-00	3-06-01	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	390.75 230.67		
	2	T31 Hip	8 /12	35-11-00	4-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	282.53 178.33		
	2	T32 Hip	8 /12	35-11-00	5-01-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	284.97 177.00		
	2	T33 Hip	8 /12	35-11-00	5-11-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	305.19 192.67		
	2	T34 Hip	8 /12	35-11-00	6-09-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	309.25 192.33		
	2	T35 Hip	8 /12	35-11-00	7-07-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	321.96 205.00		
	2	T36 Hip	8 /12	35-11-00	8-05-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	326.02 200.00		
	2	T37 Hip	8 /12	35-11-00	9-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	336.59 207.67		
	1 2-ply	T38 Roof Special Girder	8 /12	35-11-00	5-10-15	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	356.28 220.67		
	1	T39 Hip Girder	8 /12	17-03-00	3-05-15	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	80.08 48.83		
	1	T40 Hip	8 /12	17-03-00	4-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	72.36 47.00		
	1	T41 Hip	8 /12	16-10-08	4-11-11	2 x 4	1-03-08 1-03-08	1-01-13 1-04-13	68.17 43.67		
	1	T42 Hip	8 /12	16-10-08	5-11-15	2 x 4	1-03-08	1-04-13 1-07-13	74.59 48.83		
	1	T43 Hip	8 /12	16-10-08	6-09-15	2 x 4	1-03-08	1-04-13 1-07-13	77.96 49.67		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT - 4504 Lot #: Elevation: C-STD.OR 5 BRM	Job Track: 51004 PlanLog: 201943 Layout ID: 418992 Ref #: 11638 Page: 2 of 3 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano						

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T44 Common	8 /12	16-10-08	7-01-13	2 x 4	1-03-08	1-04-13 1-07-13	73.73 47.33		
	1	T45 Hip Girder	8 /12	10-00-00	3-02-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	42.73 26.50		
	1	T46 Hip	8 /12	10-00-00	6-06-08	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	51.38 32.00		
	1	T47 Hip	8 /12	10-00-00	5-06-02	2 x 4	1-03-08	3-04-13 3-04-13	52.76 33.00		
	1 2-ply	T48 Jack-Closed Girder	5 /12	6-10-08	5-10-15	2 x 4 2 x 6	1-03-08	3-00-09 5-10-15	82.54 51.00		
	1	T49S Half Hip	5 /12	6-10-08	4-10-13	2 x 4	1-03-08	1-00-09 4-10-13	40.34 27.33		
	4	T50S Jack-Closed	5 /12	6-10-08	5-10-15	2 x 4	1-03-08	1-00-09 5-10-15	157.44 103.33		
	1 2-ply	T51 Jack-Closed Girder	5 /12	5-10-08	3-05-15	2 x 4 2 x 6		1-00-09 3-05-15	54.49 34.67		
	4	J5 Jack-Open	4 /12	4-10-08	2-08-11	2 x 4	1-09-08	1-01-03 2-08-11	57.46 37.33		
	6	J6 Jack-Open	4 /12	3-10-08	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	72.13 48.00		
	26	J30 Jack-Open	5 /12	5-10-08	3-05-15	2 x 4	1-03-08	1-00-09 3-05-15	424.59 260.00		
	4	J32 Jack-Open	8 /12	2-08-08	3-02-08	2 x 4	1-03-08	1-04-13 3-02-08	44.52 28.00		
	2	C30 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 9-09	1-04-13 2-08-02	20.23 14.00		
	2	C31 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 1-01	1-04-13 2-08-02	18.4 12.67		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: UNIT - 4504 Lot #: Elevation: C-STD.OR 5 BRM	Job Track: 51004 PlanLog: 201943 Layout ID: 418992 Ref #: 11638 Page: 3 of 3 Date: 05-28-2021 Designer: Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
TOTAL # TRUSS= 80		TOTAL BFT OF ALL TRUSSES= 2797.5					BFT.	TOTAL WEIGHT OF ALL TRSSES 4479.41 LBS			

HARDWARE

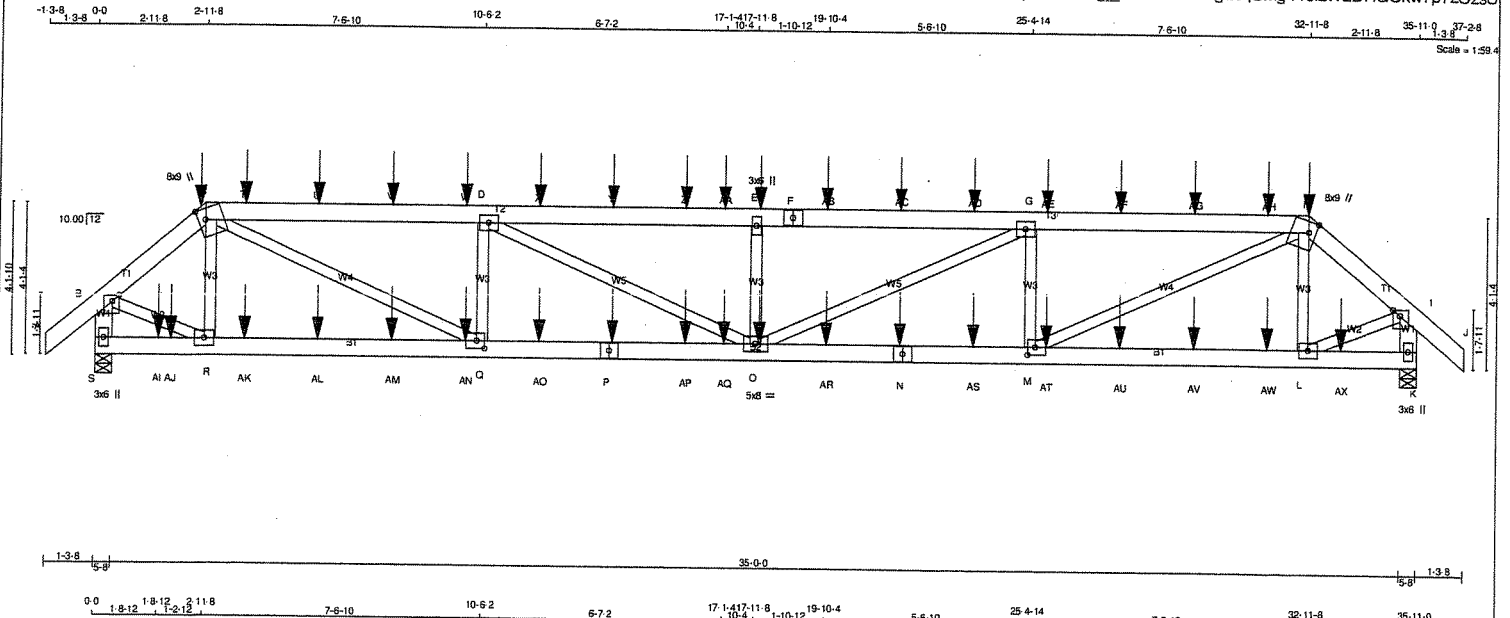
QTY	TYPE	MODEL	LENGTH
7	Hardware	LUS24	
3	Hardware	LJS26DS	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 12

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
418988	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 12:12:17 2021 Page 1
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A - C	2x6	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - J	2x6	DRY	No.2	SPF	
S - B	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	
S - P	2x6	DRY	No.2	SPF	
P - N	2x6	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122\"X3\") SPIRAL NAILS		
A - C 2	12	SIDE(122.0)
C - F 2	12	SIDE(183.1)
F - H 2	12	SIDE(61.0)
H - J 2	12	SIDE(122.0)
S - B 2	12	TOP
K - I 2	12	TOP
BOTTOM CHORDS : (0.122\"X3\") SPIRAL NAILS		
S - P 2	12	SIDE(0.0)
P - N 2	12	SIDE(183.1)
N - K 2	12	SIDE(0.0)
WEBS : (0.122\"X3\") SPIRAL NAILS		
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIROER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
S	3489	0	3489	0
K	3459	0	3459	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2469	1614 / 0	0 / 0	0 / 0	0 / 0	855 / 0	0 / 0
K	2445	1611 / 0	0 / 0	0 / 0	0 / 0	834 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
A-B	0 / 42	-91.8	-91.8	0.04 (1)	10.00	R-C	-700 / 0
B-C	-3518 / 0	-91.8	-91.8	0.05 (1)	5.93	C-Q	0 / 4937
C-T	-7113 / 0	-91.8	-91.8	0.43 (1)	4.13	Q-D	-1918 / 0
T-U	-7113 / 0	-91.8	-91.8	0.43 (1)	4.13	D-O	0 / 1575
U-V	-7113 / 0	-91.8	-91.8	0.43 (1)	4.13	O-E	-1130 / 0
V-W	-7113 / 0	-91.8	-91.8	0.43 (1)	4.13	E-G	0 / 1609
W-D	-7113 / 0	-91.8	-91.8	0.43 (1)	4.13	M-G	-1915 / 0
D-X	-8527 / 0	-91.8	-91.8	0.48 (1)	3.78	M-H	0 / 4921
X-Y	-8527 / 0	-91.8	-91.8	0.48 (1)	3.78	L-H	-709 / 0
Y-Z	-8527 / 0	-91.8	-91.8	0.48 (1)	3.78	B-R	0 / 2855
Z-AA	-8527 / 0	-91.8	-91.8	0.48 (1)	3.78	L-I	0 / 2838
AA-E	-8527 / 0	-91.8	-91.8	0.48 (1)	3.78		
E-F	-8527 / 0	-91.8	-91.8	0.47 (1)	3.79		
F-AB	-8527 / 0	-91.8	-91.8	0.47 (1)	3.79		
AB-AC	-8527 / 0	-91.8	-91.8	0.47 (1)	3.79		
AC-AD	-8527 / 0	-91.8	-91.8	0.47 (1)	3.79		
AD-G	-8527 / 0	-91.8	-91.8	0.47 (1)	3.79		
G-AE	-7082 / 0	-91.8	-91.8	0.42 (1)	4.16		
AE-AF	-7082 / 0	-91.8	-91.8	0.42 (1)	4.16		
AF-AG	-7082 / 0	-91.8	-91.8	0.42 (1)	4.16		
AG-AH	-7082 / 0	-91.8	-91.8	0.42 (1)	4.16		
AH-H	-7082 / 0	-91.8	-91.8	0.42 (1)	4.16		
H-I	-3497 / 0	-91.8	-91.8	0.05 (1)	5.94		
I-J	0 / 42	-91.8	-91.8	0.04 (1)	10.00		
S-B	-3503 / 0	0.0	0.0	0.13 (1)	7.48		
K-I	-3483 / 0	0.0	0.0	0.13 (1)	7.49		
S-AI	0 / 0	-18.5	-18.5	0.09 (4)	10.00		
AI-AJ	0 / 0	-18.5	-18.5	0.09 (4)	10.00		
AJ-R	0 / 0	-18.5	-18.5	0.09 (4)	10.00		
R-AK	0 / 2659	-18.5	-18.5	0.24 (1)	10.00		
AK-AL	0 / 2659	-18.5	-18.5	0.24 (1)	10.00		
AL-AM	0 / 2659	-18.5	-18.5	0.24 (1)	10.00		
AM-AN	0 / 2659	-18.5	-18.5	0.24 (1)	10.00		
AN-Q	0 / 2659	-18.5	-18.5	0.24 (1)	10.00		
Q-AO	0 / 7113	-18.5	-18.5	0.54 (1)	10.00		
AO-P	0 / 7113	-18.5	-18.5	0.54 (1)	10.00		
P-AP	0 / 7113	-18.5	-18.5	0.54 (1)	10.00		
AP-AQ	0 / 7113	-18.5	-18.5	0.54 (1)	10.00		
AQ-O	0 / 7113	-18.5	-18.5	0.54 (1)	10.00		
O-AR	0 / 7082	-18.5	-18.5	0.53 (1)	10.00		
AR-N	0 / 7082	-18.5	-18.5	0.53 (1)	10.00		
N-AS	0 / 7082	-18.5	-18.5	0.53 (1)	10.00		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF	

SPACING = 240 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TC=0.48/1.00 (D-E:1), BC=0.54/1.00 (C-Q:1),
WB=0.44/1.00 (C-Q:1), SS=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.69 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2115540

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
418988	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	8.0	9.0	3.50	2.25
D	TMVW-t	MT20	5.0	6.0		
E	TMVW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.50	2.25
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	5.0	6.0	2.50	2.50
N	BS-t	MT20	5.0	6.0		
O	BMVWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
O	BMVW-t	MT20	5.0	6.0	2.50	2.50
R	BMVW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
AS-M	0 / 7082	-18.5	-18.5	0.53 (1)	10.00		
M-AT	0 / 2642	-18.5	-18.5	0.24 (1)	10.00		
AT-AU	0 / 2642	-18.5	-18.5	0.24 (1)	10.00		
AU-AV	0 / 2642	-18.5	-18.5	0.24 (1)	10.00		
AV-AW	0 / 2642	-18.5	-18.5	0.24 (1)	10.00		
AW-L	0 / 2642	-18.5	-18.5	0.24 (1)	10.00		
L-AX	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
AX-K	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
E	18-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
H	32-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
H	32-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
N	21-10-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
O	18-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
P	14-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	4-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
U	6-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
V	8-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
W	10-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
X	12-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Y	14-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Z	16-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AA	17-1-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AB	19-10-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	21-10-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AD	23-10-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AE	25-10-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AF	27-10-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AG	29-10-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AH	31-10-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AI	1-8-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	2-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	4-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	6-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	8-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	10-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	12-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	16-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	17-1-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	19-10-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	23-10-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	25-10-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	27-10-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	29-10-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AW	31-10-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AX	33-10-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

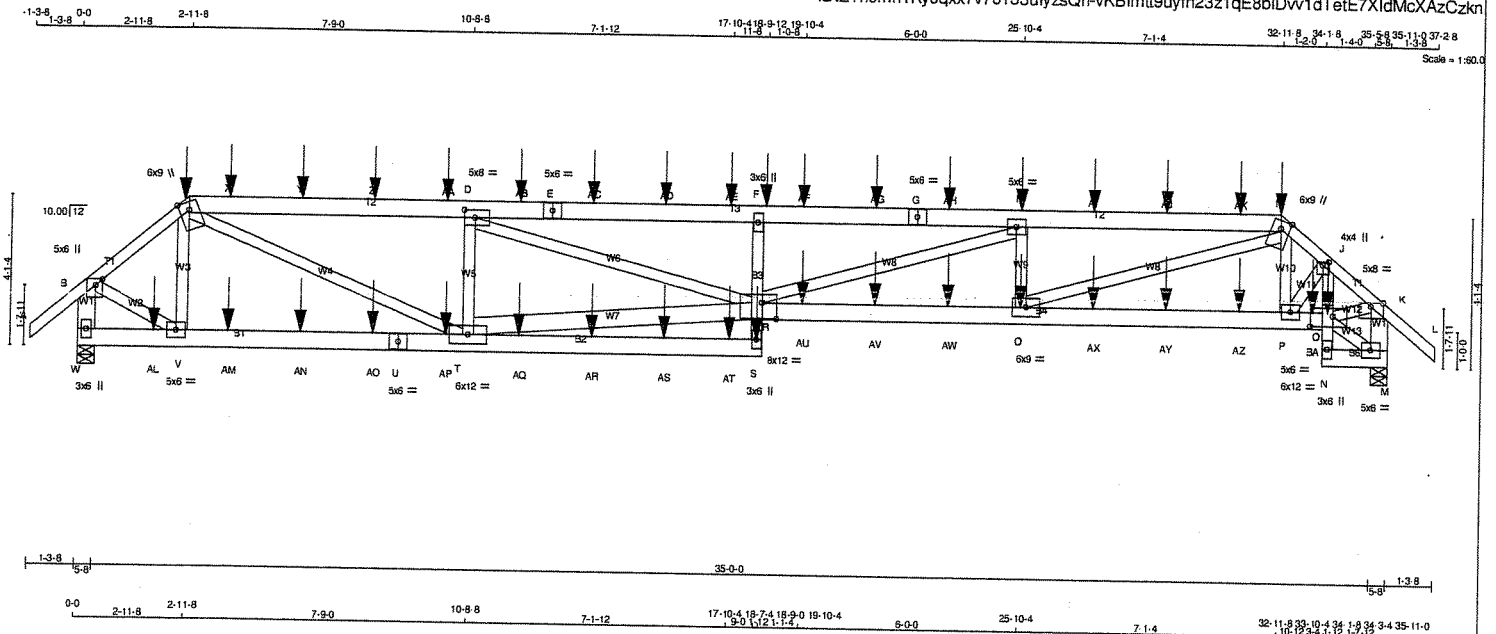


Structural component only
DWG# T-2115540

JOB NAME 406403	TRUSS NAME T1S	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x6	DRY	No.2	SPF	
E - G	2x6	DRY	No.2	SPF	
G - I	2x6	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
L - M	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
K - U	2x6	DRY	No.2	SPF	
U - S	2x6	DRY	No.2	SPF	
S - F	2x4	DRY	No.2	SPF	
F - R	2x6	DRY	No.2	SPF	
R - J	2x4	DRY	No.2	SPF	
J - N	2x6	DRY	No.2	SPF	
N - M	2x6	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
T - R	2x6	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
I - L	12	SIDE(61.0)
C - E	12	SIDE(183.1)
E - G	12	SIDE(183.1)
G - I	12	TOP
W - B	12	TOP
M - K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W - U	12	SIDE(0.0)
U - S	12	SIDE(122.0)
R - O	12	SIDE(183.1)
N - M	12	TOP
S - F	12	TOP
J - N	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	
2x6	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	DOWN	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
W	3615	0	0	3615	0	0	5-8	5-8	5-8
M	3915	0	0	3915	0	0	5-8	5-8	5-8
UNFACTORED REACTIONS		1ST LOASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
W	2555	1681 / 0	0 / 0	0 / 0	0 / 0	874 / 0	0 / 0		
M	2770	1811 / 0	0 / 0	0 / 0	0 / 0	959 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE	CSI (LC)
(LBS)							(LBS)	
FR-TO								
A-B	0 / 41	-91.8	-91.8	0.07 (1)	10.00	V-C	-935 / 0	0.08 (1)
B-C	-3474 / 0	-91.8	-91.8	0.12 (1)	4.93	P-I	0 / 181	0.02 (4)
C-X	-7567 / 0	-91.8	-91.8	0.51 (1)	3.92	P-J	0 / 449	0.04 (1)
X-Y	-7567 / 0	-91.8	-91.8	0.51 (1)	3.92	B-V	0 / 2919	0.26 (1)
Y-Z	-7567 / 0	-91.8	-91.8	0.51 (1)	3.92	O-M	-130 / 0	0.01 (1)
Z-AA	-7567 / 0	-91.8	-91.8	0.51 (1)	3.92	O-K	0 / 3776	0.33 (1)
AA-D	-7567 / 0	-91.8	-91.8	0.51 (1)	3.92	Q-I	0 / 6907	0.61 (1)
D-AB	-12607 / 0	-91.8	-91.8	0.78 (1)	2.91	R-H	0 / 2372	0.21 (1)
AB-E	-12607 / 0	-91.8	-91.8	0.78 (1)	2.91	Q-H	-1994 / 0	0.13 (1)
E-AC	-12607 / 0	-91.8	-91.8	0.78 (1)	2.91	C-T	0 / 5398	0.48 (1)
AC-AD	-12607 / 0	-91.8	-91.8	0.78 (1)	2.91	T-D	-3007 / 0	0.26 (1)
AD-AE	-12607 / 0	-91.8	-91.8	0.78 (1)	2.91	T-R	0 / 7318	0.48 (1)
AE-F	-12607 / 0	-91.8	-91.8	0.78 (1)	2.91	D-R	0 / 5150	0.46 (1)
F-AF	-12780 / 0	-91.8	-91.8	0.58 (1)	3.09			
AF-AG	-12780 / 0	-91.8	-91.8	0.58 (1)	3.09			
AG-G	-12780 / 0	-91.8	-91.8	0.58 (1)	3.09			
G-AH	-12780 / 0	-91.8	-91.8	0.58 (1)	3.09			
AH-H	-12780 / 0	-91.8	-91.8	0.58 (1)	3.09			
H-AI	-10552 / 0	-91.8	-91.8	0.51 (1)	3.42			
AI-AJ	-10552 / 0	-91.8	-91.8	0.51 (1)	3.42			
AJ-AK	-10552 / 0	-91.8	-91.8	0.51 (1)	3.42			
AK-I	-10552 / 0	-91.8	-91.8	0.51 (1)	3.42			
I-J	-5251 / 0	-91.8	-91.8	0.15 (1)	4.12			
J-K	-4955 / 0	-91.8	-91.8	0.14 (1)	4.23			
K-L	0 / 41	-91.8	-91.8	0.07 (1)	10.00			
W-B	-3640 / 0	0.0	0.0	0.13 (1)	7.37			
M-K	-3830 / 0	0.0	0.0	0.14 (1)	7.23			
W-AL	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AL-V	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
V-AM	0 / 2714	-18.5	-18.5	0.24 (1)	10.00			
AM-AN	0 / 2714	-18.5	-18.5	0.24 (1)	10.00			
AN-AO	0 / 2714	-18.5	-18.5	0.24 (1)	10.00			
AO-U	0 / 2714	-18.5	-18.5	0.24 (1)	10.00			
U-AP	0 / 2714	-18.5	-18.5	0.24 (1)	10.00			
AP-T	0 / 2714	-18.5	-18.5	0.24 (1)	10.00			
T-AQ	0 / 459	-18.5	-18.5	0.13 (4)	10.00			
AQ-AR	0 / 459	-18.5	-18.5	0.13 (4)	10.00			
AR-AS	0 / 459	-18.5	-18.5	0.13 (4)	10.00			
AS-AT	0 / 459	-18.5	-18.5	0.13 (4)	10.00			
AT-S	0 / 459	-18.5	-18.5	0.13 (4)	10.00			
S-R	0 / 226	0.0	0.0	0.36 (1)	10.00			
R-F	-1102 / 0	0.0	0.0	0.36 (1)	7.81			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.39")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/591 (0.73")

CSI: TC=0.78/1.00 (D-F:1), BC=0.84/1.00 (Q-R:1), WB=0.61/1.00 (I-Q:1), SS=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



Structural component only
DWG# T-2115450

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
406403	T1S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 12:20:28 2021 Page 2
ID:E?n9mn1Ky9qx7v78133ulyzsQn-vKBlmt9uyfh23z1qE8biDw1dTetE7XldMcXAzCzkn

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	8.0	2.50	3.50
E	TS-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-I	MT20	6.0	12.0	3.25	7.50
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	6.0	9.0		
R	BVMWWW-I	MT20	8.0	12.0	5.25	4.75
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	6.0	12.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	CS1 (LC)	FR-TO	
R-AU	0 / 10552	-18.5 -18.5	0.84 (1)	10.00	
AU-AV	0 / 10552	-18.5 -18.5	0.84 (1)	10.00	
AV-AW	0 / 10552	-18.5 -18.5	0.84 (1)	10.00	
AW-Q	0 / 10552	-18.5 -18.5	0.84 (1)	10.00	
Q-AX	0 / 4085	-18.5 -18.5	0.43 (1)	10.00	
AX-AY	0 / 4085	-18.5 -18.5	0.43 (1)	10.00	
AY-AZ	0 / 4085	-18.5 -18.5	0.43 (1)	10.00	
AZ-P	0 / 4085	-18.5 -18.5	0.43 (1)	10.00	
P-BA	0 / 3818	-18.5 -18.5	0.41 (1)	10.00	
BA-O	0 / 3818	-18.5 -18.5	0.41 (1)	10.00	
N-O	0 / 17	0.0 0.0	0.09 (1)	10.00	
O-J	-553 / 0	0.0 0.0	0.09 (1)	7.81	
N-M	0 / 109	-18.5 -18.5	0.01 (1)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
F	18-9-12	-73	-73	---	BACK	VERT	TOTAL	---	C1
H	25-10-4	-73	-73	---	BACK	VERT	TOTAL	---	C1
I	32-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
I	32-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
O	34-3-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
O	25-10-4	-68	-68	---	BACK	VERT	TOTAL	---	C1
S	18-7-4	-74	-74	---	BACK	VERT	TOTAL	---	C1
X	4-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Y	6-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Z	8-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AA	10-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AB	12-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AC	14-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AD	16-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AE	17-10-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AF	19-10-4	-73	-73	---	BACK	VERT	TOTAL	---	C1
AG	21-10-4	-73	-73	---	BACK	VERT	TOTAL	---	C1
AH	23-10-4	-73	-73	---	BACK	VERT	TOTAL	---	C1
AI	27-10-4	-73	-73	---	BACK	VERT	TOTAL	---	C1
AJ	29-10-4	-73	-73	---	BACK	VERT	TOTAL	---	C1
AK	31-10-4	-73	-73	---	BACK	VERT	TOTAL	---	C1
AL	2-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AM	4-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AN	6-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AO	8-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AP	10-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AQ	12-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AR	14-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AS	16-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AT	17-10-4	-23	-23	---	BACK	VERT	TOTAL	---	C1
AU	19-10-4	-68	-68	---	BACK	VERT	TOTAL	---	C1
AV	21-10-4	-68	-68	---	BACK	VERT	TOTAL	---	C1
AW	23-10-4	-68	-68	---	BACK	VERT	TOTAL	---	C1
AX	27-10-4	-68	-68	---	BACK	VERT	TOTAL	---	C1
AY	29-10-4	-68	-68	---	BACK	VERT	TOTAL	---	C1
AZ	31-10-4	-68	-68	---	BACK	VERT	TOTAL	---	C1
BA	33-10-4	-73	-73	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.74 (I) (INPUT = 1.00)

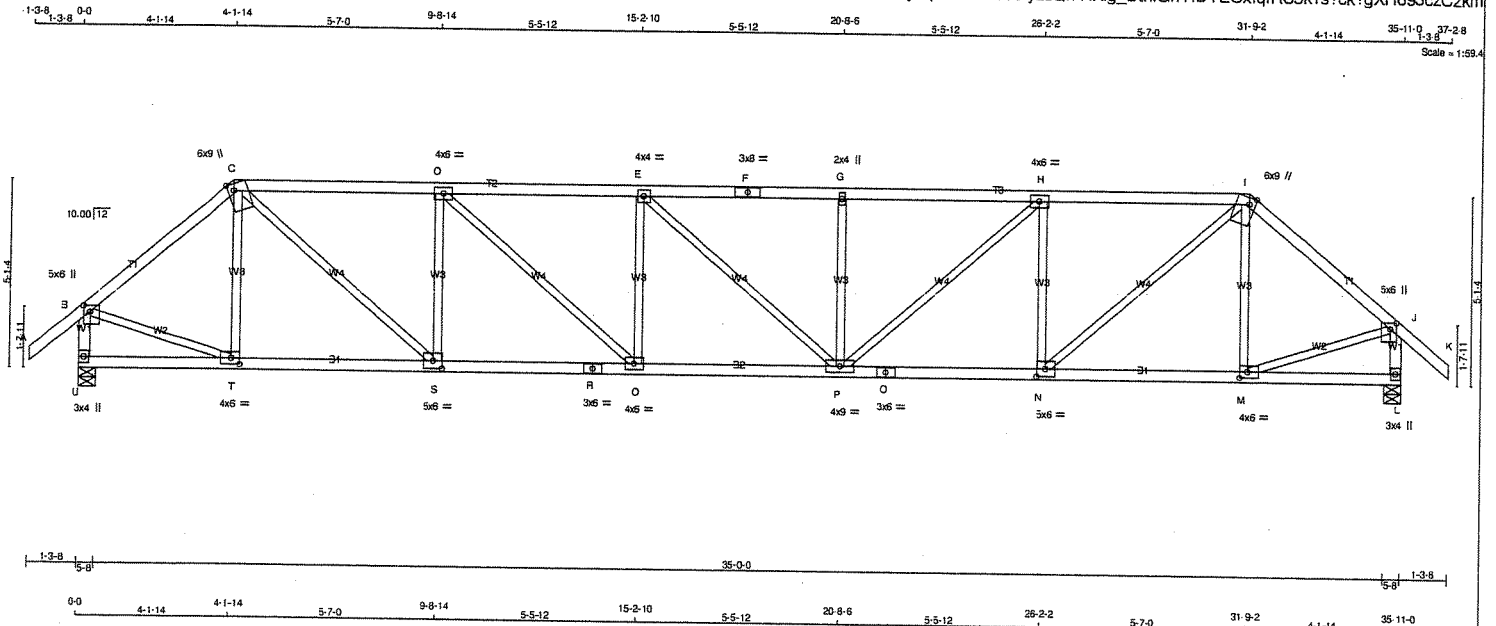


Structural component only
DWG# T-2115450 2/2

JOB NAME 406403	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 12:20:29 2021 Page 1
ID:E?n9mn1Ky9qx7v78133ulyzsQn-NXlg_DtnGnYfDYEOxfqRS5k1s?ck?gXH693czCzkm



LUMBER	N. L. G. A. RULES	CHOROS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
I - K	2x4	DRY	No.2	SPF	
U - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
U - R	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMVW-t	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMVW+w	MT20	2.0	4.0		
H	TMVW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMVW+p	MT20	5.0	6.0	2.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-t	MT20	4.0	6.0	2.00	2.50
N	BMVW-t	MT20	5.0	6.0	2.50	2.75
O	BS-t	MT20	3.0	6.0		
P	BMVW-w	MT20	4.0	9.0		
Q	BMVW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMVW-t	MT20	5.0	6.0	2.50	2.75
T	BMVW-t	MT20	4.0	6.0	2.00	2.50
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	DOWN	BRG	BRG
U	2107	0	0	5-8
L	2107	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
U	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0
L	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO			
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	T-C	-368 / 0	0.14 (1)
B-C	-2007 / 0	-91.8	-91.8	0.36 (1)	4.41	C-S	0 / 1918	0.43 (1)
C-D	-2985 / 0	-91.8	-91.8	0.62 (1)	3.48	S-D	-1150 / 0	0.44 (1)
D-E	-3657 / 0	-91.8	-91.8	0.72 (1)	3.09	D-O	0 / 894	0.20 (1)
E-F	-3656 / 0	-91.8	-91.8	0.59 (1)	3.26	O-E	-488 / 0	0.19 (1)
F-G	-3656 / 0	-91.8	-91.8	0.59 (1)	3.26	E-P	-2 / 0	0.00 (1)
G-H	-3656 / 0	-91.8	-91.8	0.72 (1)	3.09	P-G	-487 / 0	0.19 (1)
H-I	-2986 / 0	-91.8	-91.8	0.62 (1)	3.48	P-H	0 / 891	0.20 (1)
I-J	-2007 / 0	-91.8	-91.8	0.36 (1)	4.41	H-N	-1149 / 0	0.44 (1)
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	N-I	0 / 1919	0.43 (1)
U-B	-2078 / 0	0.0	0.0	0.22 (1)	5.87	M-I	-368 / 0	0.14 (1)
L-J	-2078 / 0	0.0	0.0	0.22 (1)	5.87	B-T	0 / 1608	0.36 (1)
U-T	0 / 0	-18.5	-18.5	0.10 (4)	10.00	M-J	0 / 1608	0.36 (1)
T-S	0 / 1530	-18.5	-18.5	0.31 (1)	10.00			
S-R	0 / 2986	-18.5	-18.5	0.53 (1)	10.00			
R-O	0 / 2986	-18.5	-18.5	0.53 (1)	10.00			
Q-P	0 / 3657	-18.5	-18.5	0.64 (1)	10.00			
P-O	0 / 2986	-18.5	-18.5	0.53 (1)	10.00			
O-N	0 / 2986	-18.5	-18.5	0.53 (1)	10.00			
N-M	0 / 1530	-18.5	-18.5	0.31 (1)	10.00			
M-L	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.42")

CSI: TC=0.72/1.00 (G-H:1), BC=0.64/1.00 (P-O:1),
WB=0.44/1.00 (D-S:1), SS=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.89 (O) (INPUT = 1.00)

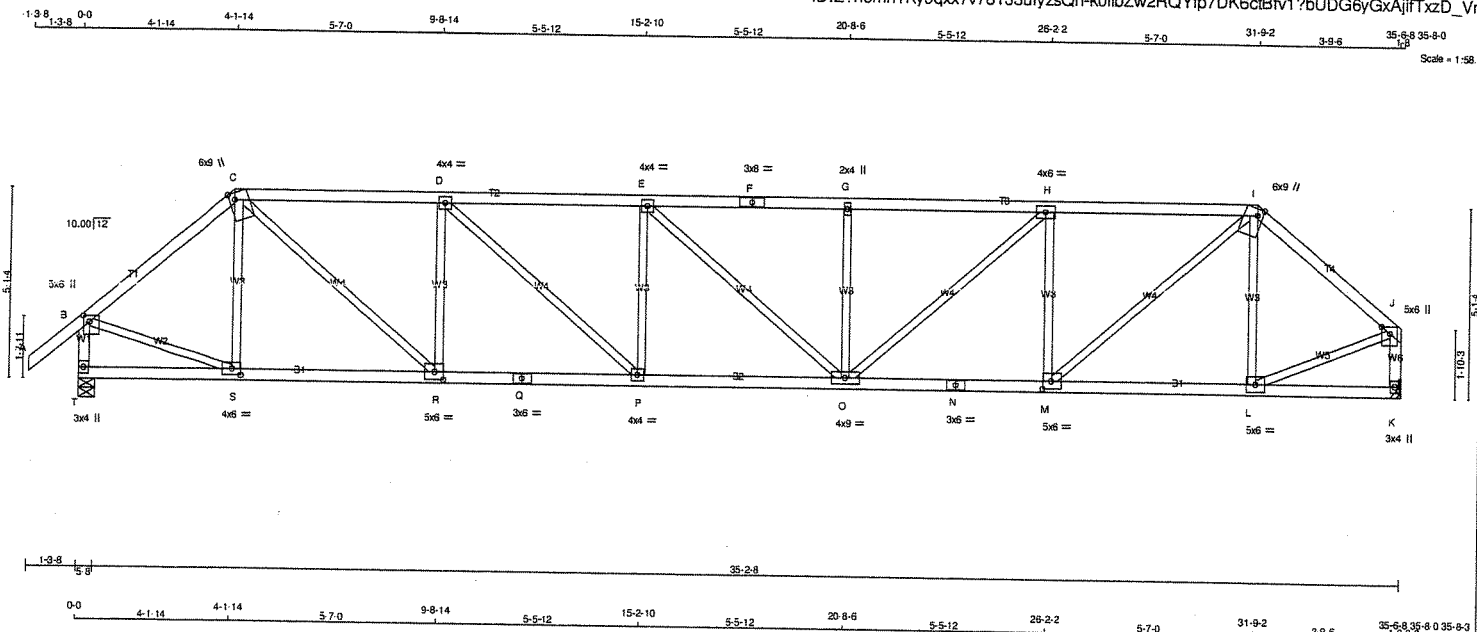


Structural component only
DWG# T-2115451

JOB NAME 411069	TRUSS NAME T2A	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 11:28:12 2021 Page 1
ID:E?n9mn1Ky9qxx7v78133ulyzsQn-k0ilbZw2RQYtp7DK6ctBfv1?bUDG6yGxAjifTxD_Vn



LUMBER	N. L. G. A. RULES	CHOROS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
T - Q	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
K - J	2x4	DRY	No.2	SPF	

ORY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2085	0	5-8
T	1958	0	5-8
K	1958	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LOASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1472	980 / 0	0 / 0	0 / 0	0 / 0	493 / 0	0 / 0
K	1385	909 / 0	0 / 0	0 / 0	0 / 0	476 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO	A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	S-C	-362 / 0	0.14 (1)
	B-C	-1982 / 0	-91.8	-91.8 0.36 (1)	4.44	C-R	0 / 1885	0.42 (1)
	C-D	-2941 / 0	-91.8	-91.8 0.62 (1)	3.50	R-D	-1128 / 0	0.43 (1)
	D-E	-3588 / 0	-91.8	-91.8 0.71 (1)	3.12	D-P	0 / 861	0.19 (1)
	E-F	-3562 / 0	-91.8	-91.8 0.57 (1)	3.31	P-E	-466 / 0	0.18 (1)
	F-G	-3562 / 0	-91.8	-91.8 0.57 (1)	3.31	E-O	-35 / 0	0.04 (1)
	G-H	-3562 / 0	-91.8	-91.8 0.70 (1)	3.14	O-G	-487 / 0	0.19 (1)
	H-I	-2867 / 0	-91.8	-91.8 0.61 (1)	3.55	O-H	0 / 924	0.21 (1)
	I-J	-1822 / 0	-91.8	-91.8 0.29 (1)	4.68	M-H	-1172 / 0	0.45 (1)
	T-B	-2057 / 0	0.0	0.0 0.22 (1)	5.90	M-I	0 / 1953	0.44 (1)
	T-S	0 / 0	-18.5	-18.5 0.10 (4)	10.00	L-I	-490 / 0	0.19 (1)
	S-R	0 / 1511	-18.5	-18.5 0.31 (1)	10.00	B-S	0 / 1588	0.36 (1)
	R-Q	0 / 2941	-18.5	-18.5 0.52 (1)	10.00	L-J	0 / 1515	0.34 (1)
	Q-P	0 / 2941	-18.5	-18.5 0.52 (1)	10.00	K-J	-1934 / 0	0.20 (1)
	P-O	0 / 3588	-18.5	-18.5 0.63 (1)	10.00			
	O-N	0 / 2867	-18.5	-18.5 0.51 (1)	10.00			
	N-M	0 / 2867	-18.5	-18.5 0.51 (1)	10.00			
	M-L	0 / 1385	-18.5	-18.5 0.28 (1)	10.00			
	L-K	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.18")
CALCULATED VERT. DEFL.(LL)= L/999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.18")
CALCULATED VERT. DEFL.(TL)= L/999 (0.40")

CSI: TC=0.71/1.00 (D-E-1), BC=0.63/1.00 (O-P-1),
WB=0.45/1.00 (H-M-1), SSI=0.24/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (S) (INPUT = 0.90)
JSI METAL= 0.91 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2115510

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 12:20:30 2021 Page 1
ID:E?n9mn1Ky9qxx7v78133ulyzsQn-rjJ2BYuPQavPHN7QxfA3qe IORBaL9Ogmxcric2zCzk



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z. N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.20")
CALCULATED VERT. DEFL(LL) = L/ 999 (0.30")
ALLOWABLE DEFL(TL)= L/360 (1.20")
CALCULATED VERT. DEFL(TL) = L/ 787 (0.55")

CSI: TC=0.60/1.00 (E-F:1), BC=0.75/1.00 (S-T:1),
WB=0.56/1.00 (T-V:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTQSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

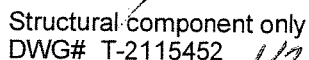
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (X) (INPUT = 0.90)
JSI METAL= 0.85 (W) (INPUT = 1.00)



JOB NAME 406403	TRUSS NAME T2S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 12:20:30 2021 Page 2
 ID:E?n9mn1Ky9qxx7v78133uilyzsQn-rjJ2BYuPQavPHN7OxfA3qe iORBaL9Oqmxic2zCzki

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
X	BMWV-t	MT20	5.0	6.0		
Y	BMWV-t	MT20	4.0	6.0	2.00	2.75
Z	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.

NOTES- (1)
 1) Lateral braces to be a minimum of 2x4 SPF #2

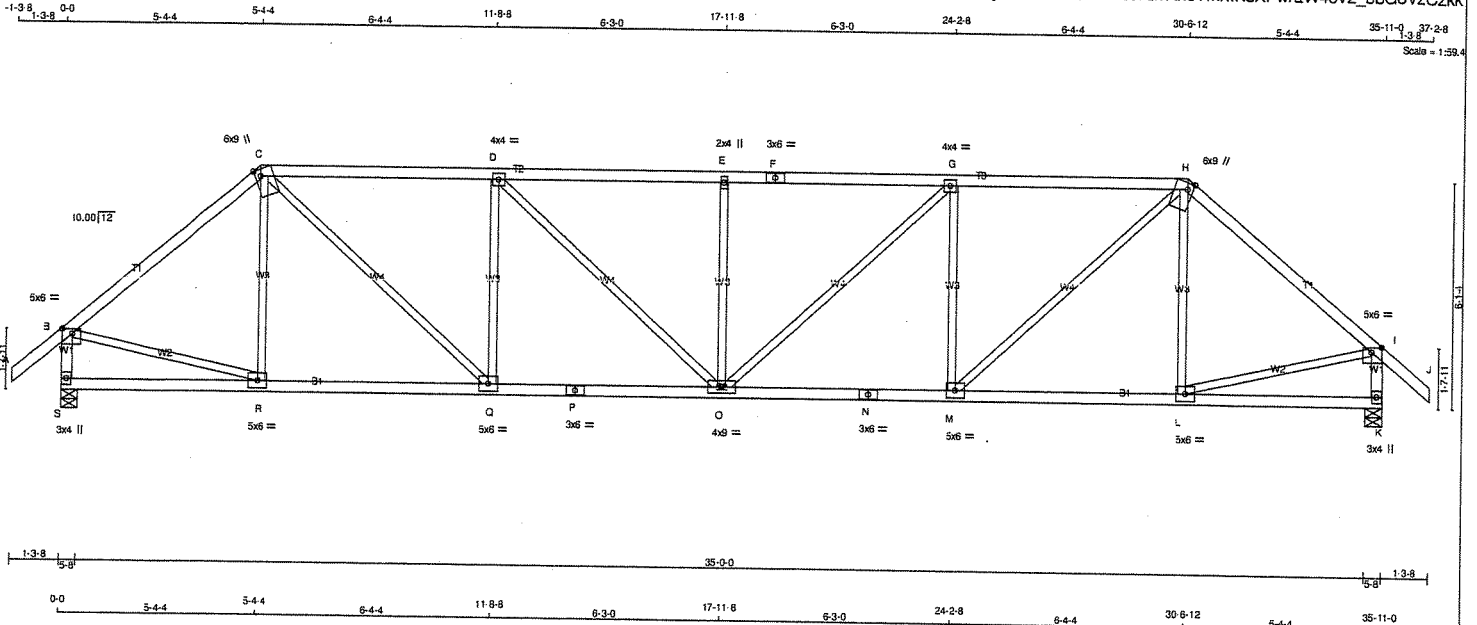


Structural component only
 DWG# T-2115452 *TH*

JOB NAME 406403	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 12:20:31 2021 Page 1
ID:E?n9mn1Ky9qxx7v78133ulyzsQn-KvtQPuv1Bt1GvXhcVMhInSPwraW4cVz_bbG8VzCzkk



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
S - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
S - P	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	6.0 1.50 3.25
C	TTWW+m	MT20	6.0	9.0 Edge 1.75
D	TMVW-t	MT20	4.0	4.0
E	TMVW-w	MT20	2.0	4.0
F	TS-t	MT20	3.0	6.0
G	TMVW-t	MT20	4.0	4.0
H	TTWW+m	MT20	6.0	9.0 Edge 1.75
I	TMVW-p	MT20	5.0	6.0 1.50 3.25
K	BMV1+p	MT20	3.0	4.0
L, M, Q, R				
L	BMVW-t	MT20	5.0	6.0
N	BS-t	MT20	3.0	6.0
O	BMVWW-t	MT20	4.0	9.0
P	BS-t	MT20	3.0	6.0
S	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
S	2107	0	2107	0	0	5-8	5-8	5-8	5-8
K	2107	0	2107	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LOOSE		MAX. MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
S	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0		
K	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 41	R-C	-254 / 6
B-C	-2054 / 0	C-Q	0 / 1612
C-D	-2762 / 0	Q-D	-972 / 0
D-E	-3108 / 0	D-O	0 / 472
E-F	-3108 / 0	O-E	-529 / 0
F-G	-3108 / 0	O-G	0 / 472
G-H	-2762 / 0	M-H	-972 / 0
H-I	-2054 / 0	L-H	0 / 1612
I-J	0 / 41	L-I	-254 / 6
S-B	-2068 / 0	B-R	0 / 1619
K-I	-2068 / 0	L-I	0 / 1619
S-R	0 / 0		
R-O	0 / 1570		
O-P	0 / 2762		
P-O	0 / 2762		
O-N	0 / 2762		
N-M	0 / 2762		
M-L	0 / 1570		
L-K	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.31")

CSI: TC=0.87/1.00 (E-G:1), BC=0.51/1.00 (M-O:1),
WB=0.57/1.00 (D-Q:1), SS=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.87 (P) (INPUT = 1.00)

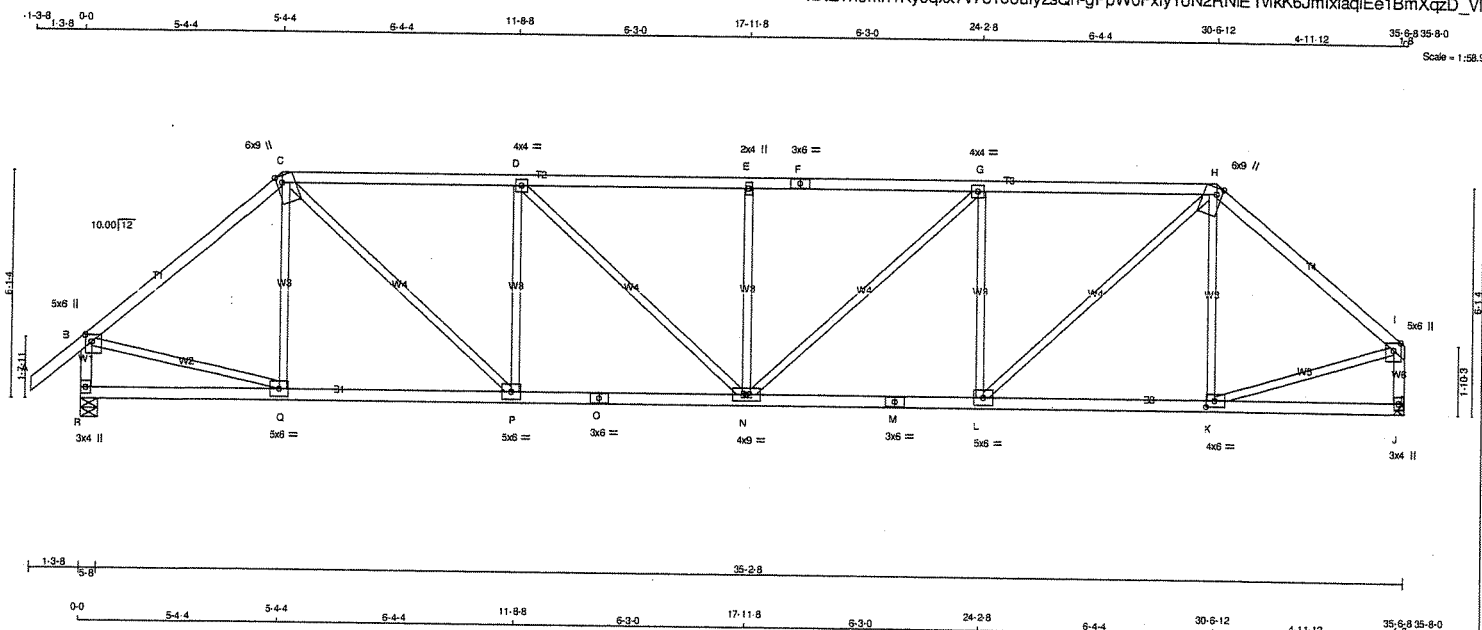


Structural component only
DWG# T-2115453

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411069	T3A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 11:28:14 2021 Page 1
ID:E?n9mn1Ky9qx7v78133ulyzsQn-gPpW0Fxy1oN2RNiE1vfkK6JmxiqIeE1BmXqzD_VI



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
O - F	2x4	DRY	No.2	SPF	
H - I	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
E - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
J - I	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.25	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-w	MT20	5.0	6.0	2.50	2.25
J	BMV1+w	MT20	3.0	4.0		
K	BMVW-t	MT20	4.0	6.0	2.00	2.75
L, P, O						
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-w	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	HORIZ	UPLIFT
R	2085	0	0	5-8
J	1958	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
R	1472	980 / 0	0 / 0	0 / 0	0 / 0	493 / 0
J	1385	909 / 0	0 / 0	0 / 0	0 / 0	476 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	0 / 1580	0.36 (1)	0.56 (1)	0.10 (1)
B-C	-2028 / 0	-91.8	-91.8	0.81 (1)	3.39	0 / 440	0.31 (1)	0.11 (1)	0.58 (1)
C-D	-2718 / 0	-91.8	-91.8	0.85 (1)	3.17	0 / 504	0.37 (1)	0.21 (1)	0.36 (1)
D-E	-3040 / 0	-91.8	-91.8	0.85 (1)	3.17	0 / 1535	0.20 (1)		
E-F	-3040 / 0	-91.8	-91.8	0.85 (1)	3.17				
F-G	-3040 / 0	-91.8	-91.8	0.85 (1)	3.17				
G-H	-2671 / 0	-91.8	-91.8	0.80 (1)	3.42				
H-I	-1906 / 0	-91.8	-91.8	0.52 (1)	4.33				
R-B	-2046 / 0	0.0	0.0	0.22 (1)	5.91				
R-O	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
O-P	0 / 1550	-18.5	-18.5	0.33 (1)	10.00				
P-O	0 / 2718	-18.5	-18.5	0.50 (1)	10.00				
O-N	0 / 2718	-18.5	-18.5	0.50 (1)	10.00				
N-M	0 / 2671	-18.5	-18.5	0.50 (1)	10.00				
M-L	0 / 2671	-18.5	-18.5	0.50 (1)	10.00				
L-K	0 / 1455	-18.5	-18.5	0.32 (1)	10.00				
K-J	0 / 0	-18.5	-18.5	0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.85/1.00 (E-G:1), BC=0.50/1.00 (N-P:1),
WB=0.58/1.00 (G-L:1), SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

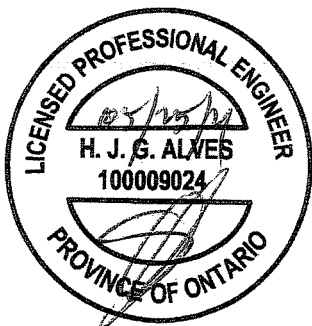
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.85 (O) (INPUT = 1.00)

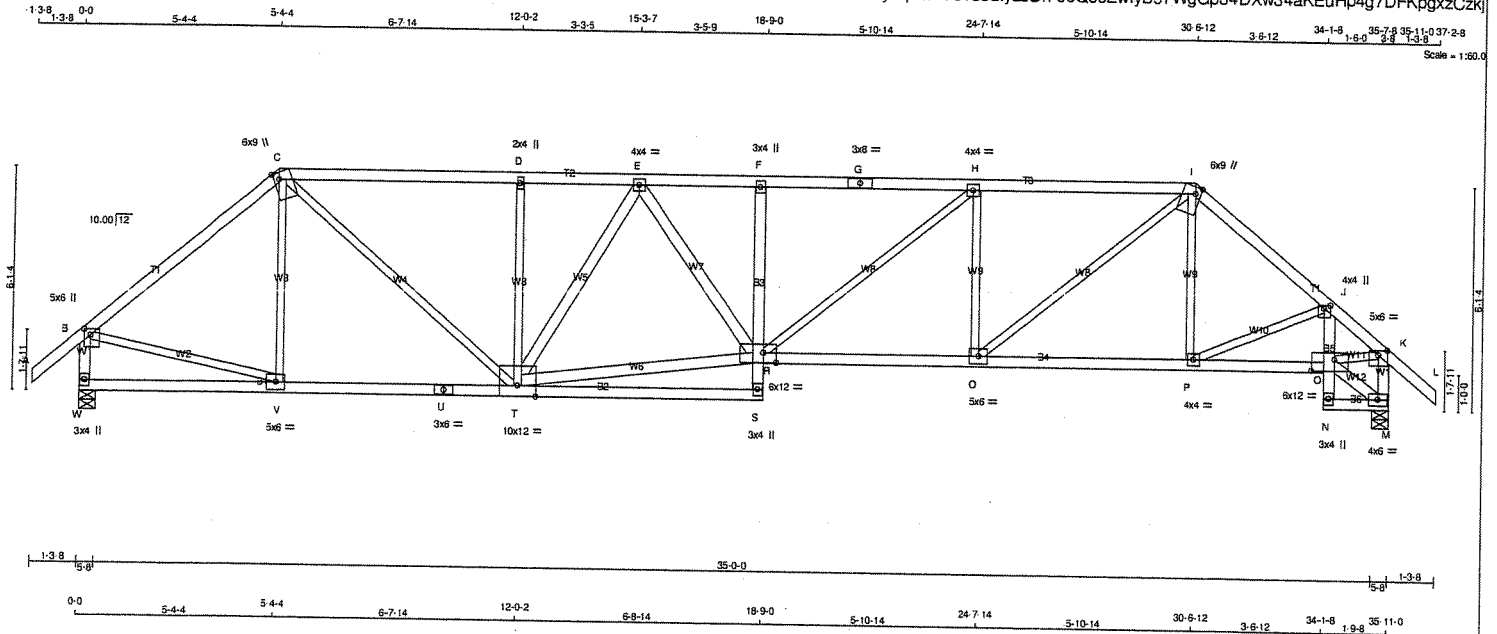


Structural component only
DWG# T-2115511

JOB NAME 406403	TRUSS NAME T3S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Tue May 25 12:20:32 2021 Page 1
ID:E?n9mn1Ky9qxx7v78133ulyzsOn-o6QocEwfyB97WgGp34DXw34aKEuHp4g7DFKpgxzCzkj



LUMBER			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF
W - B	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
W - U	2x4	DRY No.2	SPF
U - S	2x4	DRY No.2	SPF
S - F	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
N - J	2x4	DRY No.2	SPF
N - M	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
O - M	2x4	DRY No.2	SPF
T - R	2x4	DRY No.2	SPF
T - E	2x4	DRY No.2	SPF
E - R	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	6.0 2.00 2.00
C	TTWW+m	MT20	6.0	9.0 Edge 1.75
D	TMVW+w	MT20	2.0	4.0
E	TMVW-t	MT20	4.0	4.0
F	TMV-p	MT20	3.0	4.0
G	TS-t	MT20	3.0	8.0
H	TMVW-t	MT20	4.0	4.0
I	TTWW+m	MT20	6.0	9.0 Edge 1.75
J	TMVW+p	MT20	4.0	4.0 1.00 2.00
K	TMVW-p	MT20	5.0	6.0 1.50 3.00
M	BMVW1-t	MT20	4.0	6.0
N	BMV+p	MT20	3.0	4.0
O	BMVWW-t	MT20	6.0	12.0 3.75 7.50
P	BMVW-t	MT20	4.0	4.0
Q	BMVW-t	MT20	5.0	6.0
R	BMVWW-t	MT20	6.0	12.0 3.25 4.25
S	BMV+p	MT20	3.0	4.0
T	BMVWW-t	MT20	10.0	12.0 Edge
U	BS-t	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG		IN-SX	
JT	VERT	JT	HORZ	JT	UP/LIFT	JT	IN-SX	JT	IN-SX
W	2105	0	2105	0	0	5-8	5-8	5-8	5-8
M	2109	0	2109	0	0	5-8	5-8	5-8	5-8
UNFACTORED REACTIONS									
1ST LOOSE		MAX/MIN COMPONENT REACTIONS							
JT	COMBINED	JT	SNOW	JT	LIVE	JT	PERM LIVE	JT	WIND
W	1486	988 / 0	0 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0	0 / 0
M	1489	991 / 0	0 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8 0.13 (1)	10.00	V-C	-254 / 5	0.15 (1)	
B-C	-2051 / 0	-91.8 -91.8 0.64 (1)	4.06	R-H	0 / 578	0.13 (1)	
C-D	-2794 / 0	-91.8 -91.8 0.82 (1)	3.39	Q-H	-987 / 0	0.38 (1)	
D-E	-2793 / 0	-91.8 -91.8 0.82 (1)	3.39	O-I	0 / 1774	0.40 (1)	
E-F	-3695 / 0	-91.8 -91.8 0.43 (1)	3.31	P-I	0 / 99	0.03 (4)	
F-G	-3716 / 0	-91.8 -91.8 0.89 (1)	2.89	P-J	-151 / 0	0.04 (1)	
G-H	-3716 / 0	-91.8 -91.8 0.89 (1)	2.89	B-V	0 / 1617	0.36 (1)	
H-I	-3266 / 0	-91.8 -91.8 0.81 (1)	3.14	O-M	-78 / 0	0.01 (1)	
I-J	-2462 / 0	-91.8 -91.8 0.28 (1)	4.16	O-K	0 / 1978	0.45 (1)	
J-K	-2571 / 0	-91.8 -91.8 0.19 (1)	4.14	C-T	0 / 1624	0.37 (1)	
K-L	0 / 41	-91.8 -91.8 0.13 (1)	10.00	T-D	-617 / 0	0.36 (1)	
W-B	-2066 / 0	0.0 0.0 0.22 (1)	5.89	T-R	0 / 3174	0.51 (1)	
M-K	-2051 / 0	0.0 0.0 0.22 (1)	5.91	T-E	-908 / 0	0.48 (1)	
				E-R	0 / 805	0.13 (1)	
W-V	0 / 0	-18.5 -18.5 0.15 (4)	10.00				
V-U	0 / 1568	-18.5 -18.5 0.34 (1)	10.00				
U-T	0 / 1568	-18.5 -18.5 0.34 (1)	10.00				
T-S	0 / 100	-18.5 -18.5 0.20 (4)	10.00				
S-R	0 / 55	0.0 0.0 0.14 (1)	10.00				
R-F	-452 / 0	0.0 0.0 0.19 (1)	7.81				
R-Q	0 / 3266	-18.5 -18.5 0.50 (1)	10.00				
O-P	0 / 1879	-18.5 -18.5 0.37 (1)	10.00				
P-O	0 / 2016	-18.5 -18.5 0.39 (1)	10.00				
N-O	0 / 17	0.0 0.0 0.10 (1)	10.00				
O-J	-124 / 0	0.0 0.0 0.10 (1)	7.81				
N-M	0 / 62	-18.5 -18.5 0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TC=0.89/1.00 (F-H:1), BC=0.60/1.00 (Q-R:1), WB=0.51/1.00 (R-T:1), SS=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.52 (R) (INPUT = 1.00)



Structural component only
DWG# T-2115454

JOB NAME 406403	TRUSS NAME T3S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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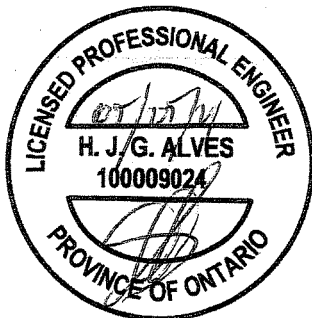
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

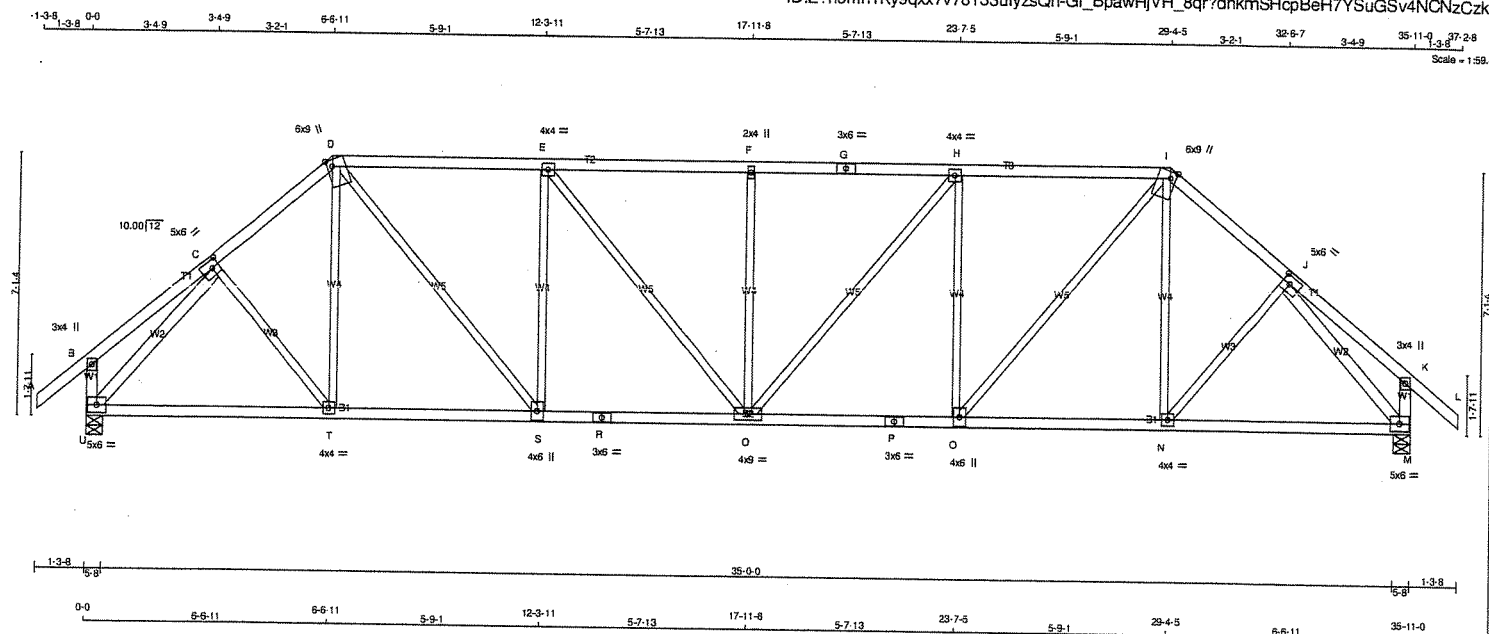
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115454 *ML*

JOB NAME 406403	TRUSS NAME T4	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS		2x3	DRY	No.2
EXCEPT				SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TTWW-t	MT20	5.0	6.0 2.50 2.50
D	TTWW+m	MT20	6.0	9.0 Edge 1.75
E	TTWW-t	MT20	4.0	4.0
F	TTWW+m	MT20	2.0	4.0
G	TS-t	MT20	3.0	6.0
H	TTWW-t	MT20	4.0	4.0
I	TTWW+m	MT20	6.0	9.0 Edge 1.75
J	TTWW-t	MT20	5.0	6.0 2.50 2.50
K	TMV+p	MT20	3.0	4.0
M	BMVW1-t	MT20	5.0	6.0
N	BMVW1-t	MT20	4.0	4.0
O	BMVW1-t	MT20	4.0	6.0
P	BS-t	MT20	3.0	6.0
Q	BMVW1-t	MT20	4.0	9.0
R	BS-t	MT20	3.0	6.0
S	BMVW1-t	MT20	4.0	6.0
T	BMVW1-t	MT20	4.0	4.0
U	BMVW1-t	MT20	5.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1)



Structural component only
DWG# T-2115455

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS	REACTION	GROSS	REACTION	BRG	IN-SX	BRG	IN-SX
U	2107	0	2107	0	0	5-8	5-8	5-8	5-8
M	2107	0	2107	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
U	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0	1488	990 / 0	0 / 0	0 / 0
M	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0	1488	990 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRACED LENGTH	FR-TO
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	C-T	0 / 166	0.04 (1)	-91.8	-91.8 0.13 (1)	10.00	C-T
B-C	0 / 20	-91.8	-91.8 0.14 (1)	10.00	T-D	0 / 75	0.03 (4)	-91.8	-91.8 0.14 (1)	10.00	T-D
C-D	-2076 / 0	-91.8	-91.8 0.20 (1)	4.53	D-S	0 / 1280	0.29 (1)	-91.8	-91.8 0.20 (1)	4.53	D-S
D-E	-2402 / 0	-91.8	-91.8 0.60 (1)	3.81	S-E	-880 / 0	0.77 (1)	-91.8	-91.8 0.60 (1)	3.81	S-E
E-F	-2644 / 0	-91.8	-91.8 0.63 (1)	3.63	E-Q	0 / 378	0.09 (1)	-91.8	-91.8 0.63 (1)	3.63	E-Q
F-G	-2644 / 0	-91.8	-91.8 0.63 (1)	3.63	Q-F	-477 / 0	0.42 (1)	-91.8	-91.8 0.63 (1)	3.63	Q-F
G-H	-2644 / 0	-91.8	-91.8 0.63 (1)	3.63	O-H	0 / 378	0.09 (1)	-91.8	-91.8 0.63 (1)	3.63	O-H
H-I	-2402 / 0	-91.8	-91.8 0.60 (1)	3.81	Q-H	-880 / 0	0.77 (1)	-91.8	-91.8 0.60 (1)	3.81	Q-H
I-J	-2076 / 0	-91.8	-91.8 0.20 (1)	4.53	O-I	0 / 1280	0.29 (1)	-91.8	-91.8 0.20 (1)	4.53	O-I
J-K	0 / 20	-91.8	-91.8 0.14 (1)	10.00	N-I	0 / 75	0.03 (4)	-91.8	-91.8 0.14 (1)	10.00	N-I
K-L	0 / 41	-91.8	-91.8 0.13 (1)	10.00	N-J	0 / 166	0.04 (1)	-91.8	-91.8 0.13 (1)	10.00	N-J
U-B	-244 / 0	0.0	0.0 0.03 (1)	7.81	U-C	-2332 / 0	0.72 (1)	0.0	0.0 0.03 (1)	7.81	U-C
M-K	-244 / 0	0.0	0.0 0.03 (1)	7.81	J-M	-2332 / 0	0.72 (1)	0.0	0.0 0.03 (1)	7.81	J-M
U-T	0 / 1467	-18.5	-18.5 0.34 (1)	10.00				-18.5	-18.5 0.34 (1)	10.00	
T-S	0 / 1575	-18.5	-18.5 0.36 (1)	10.00				-18.5	-18.5 0.36 (1)	10.00	
S-R	0 / 2402	-18.5	-18.5 0.44 (1)	10.00				-18.5	-18.5 0.44 (1)	10.00	
R-Q	0 / 2402	-18.5	-18.5 0.44 (1)	10.00				-18.5	-18.5 0.44 (1)	10.00	
Q-P	0 / 2402	-18.5	-18.5 0.44 (1)	10.00				-18.5	-18.5 0.44 (1)	10.00	
P-O	0 / 2402	-18.5	-18.5 0.44 (1)	10.00				-18.5	-18.5 0.44 (1)	10.00	
O-N	0 / 1575	-18.5	-18.5 0.36 (1)	10.00				-18.5	-18.5 0.36 (1)	10.00	
N-M	0 / 1467	-18.5	-18.5 0.34 (1)	10.00				-18.5	-18.5 0.34 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TQP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.13")
ALLOWABLE DEFL. (TL) = $L/360$ (1.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.24")

CSI: TC=0.63/1.00 (E-F:1), BC=0.44/1.00 (Q-S:1), WB=0.77/1.00 (E-S:1), SS=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

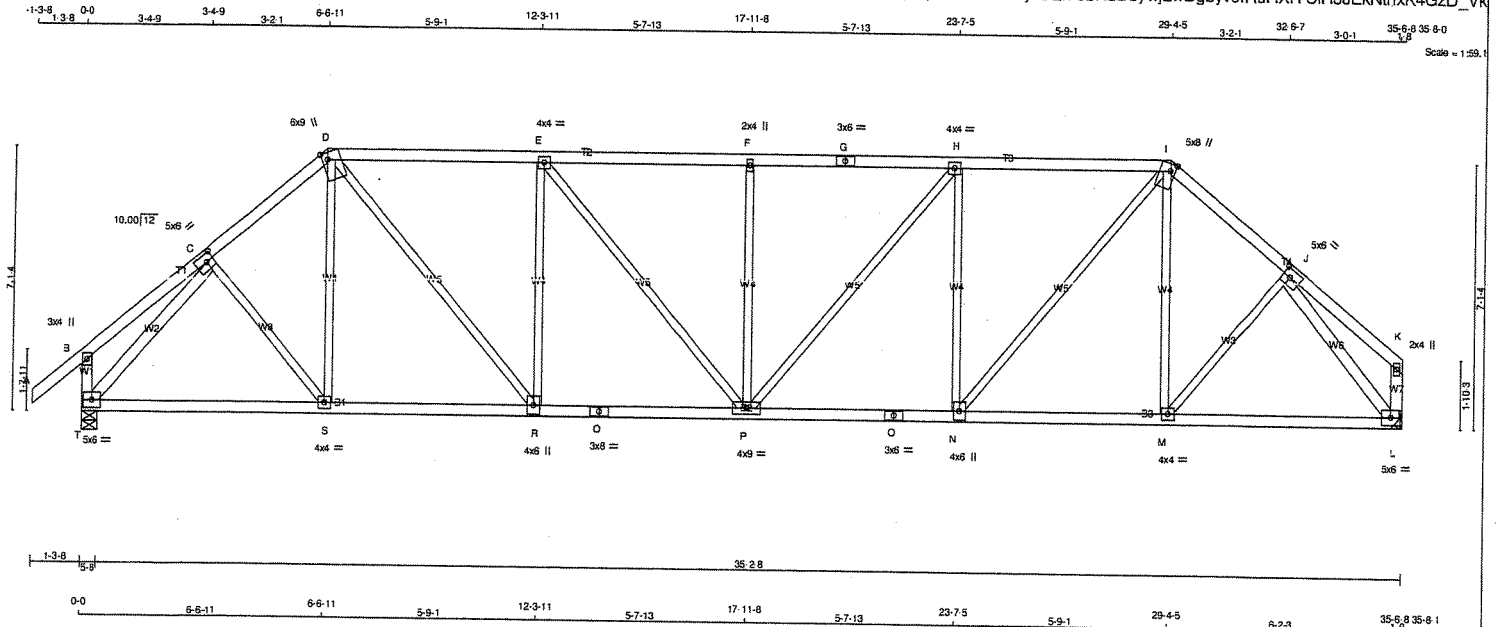
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (J) (INPUT = 0.90)
JSI METAL = 0.74 (R) (INPUT = 1.00)

JOB NAME 411069	TRUSS NAME T4A	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
T - O	2x4	DRY	No.2	SPF
O - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW-w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	8.0	2.25	1.50
J	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	TMW-w	MT20	2.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES

1)



Structural component only
DWG# T-2115512

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	VERT	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
T	2085	0	2085	0	0	5-8	5-8	
L	1967	0	1967	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1472	980 / 0	0 / 0	0 / 0	0 / 0	0 / 0	493 / 0	0 / 0
L	1391	914 / 0	0 / 0	0 / 0	0 / 0	0 / 0	477 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	C-S	0 / 161	0.04 (1)	
B-C	0 / 20	-91.8	-91.8 0.14 (1)	D-R	0 / 76	0.03 (4)	
C-D	-2049 / 0	-91.8	-91.8 0.20 (1)	E-P	0 / 1252	0.28 (1)	
D-E	-2363 / 0	-91.8	-91.8 0.60 (1)	F-G	-858 / 0	0.75 (1)	
E-F	-2586 / 0	-91.8	-91.8 0.62 (1)	G-H	0 / 350	0.08 (1)	
F-G	-2586 / 0	-91.8	-91.8 0.62 (1)	H-I	-477 / 0	0.42 (1)	
G-H	-2586 / 0	-91.8	-91.8 0.62 (1)	I-J	0 / 406	0.09 (1)	
H-I	-2327 / 0	-91.8	-91.8 0.59 (1)	J-K	-902 / 0	0.79 (1)	
I-J	-1953 / 0	-91.8	-91.8 0.19 (1)	K-L	0 / 1311	0.29 (1)	
J-K	0 / 28	-91.8	-91.8 0.13 (1)	L-K	-98 / 38	0.09 (1)	
T-B	-244 / 0	0.0	0.0 0.03 (1)	M-J	0 / 296	0.07 (1)	
T-S	0 / 1449	-18.5	-18.5 0.34 (1)	N-I	-2304 / 0	0.71 (1)	
S-R	0 / 1554	-18.5	-18.5 0.35 (1)	O-P	-2236 / 0	0.64 (1)	
R-Q	0 / 2363	-18.5	-18.5 0.43 (1)	P-Q	-98 / 0	0.01 (1)	
O-P	0 / 2363	-18.5	-18.5 0.43 (1)				
P-O	0 / 2327	-18.5	-18.5 0.43 (1)				
O-N	0 / 2327	-18.5	-18.5 0.43 (1)				
N-M	0 / 1480	-18.5	-18.5 0.33 (1)				
M-L	0 / 1289	-18.5	-18.5 0.30 (1)				

TOTAL WEIGHT = 164 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.18")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.13")
ALLOWABLE DEFL. (TL) = $L/360$ (1.18")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.23")

CSI: TC=0.62/1.00 (E-F:1), BC=0.43/1.00 (P-R:1), WB=0.79/1.00 (H-N:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

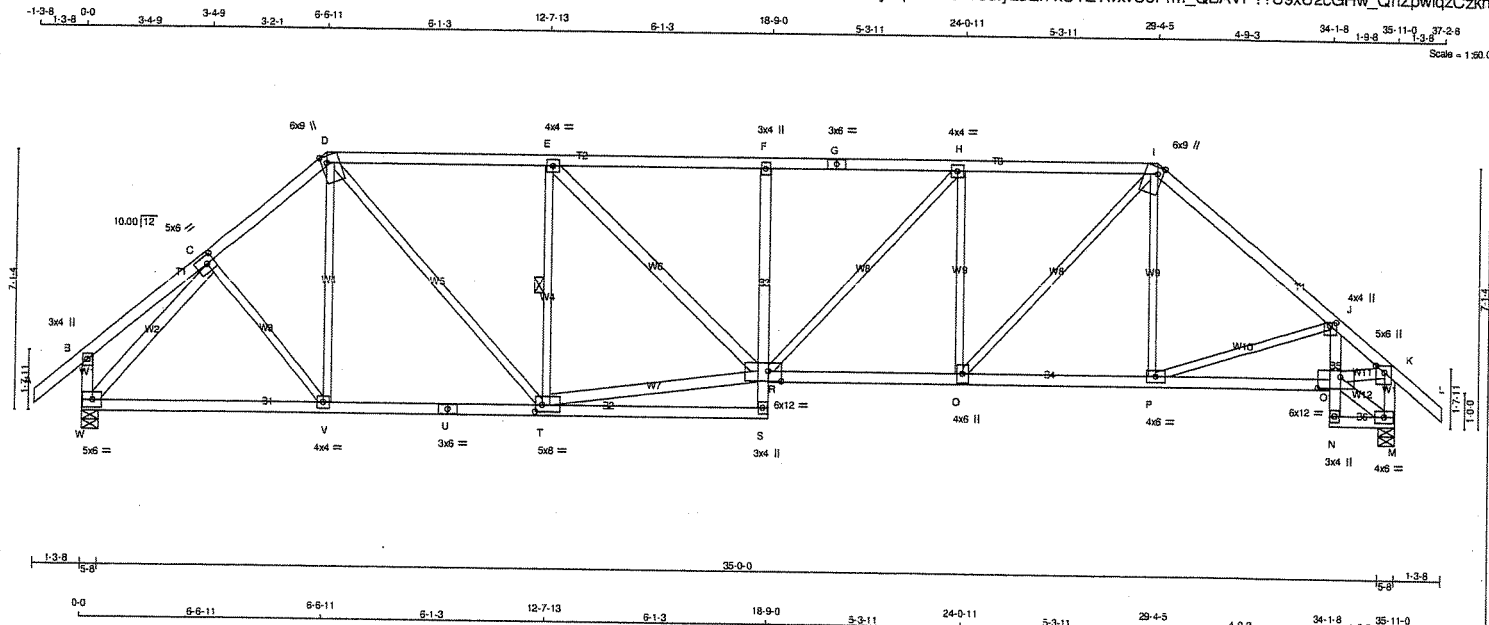
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.73 (O) (INPUT = 1.00)

JOB NAME 406403	TRUSS NAME T4S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
W - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
W - U	2x4	DRY	No.2	SPF	
U - S	2x4	DRY	No.2	SPF	
S - F	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
N - M	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
T - R	2x4	DRY	No.2	SPF	
E - R	2x4	DRY	No.2	SPF	
W - C	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
O - K	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW+p	MT20	5.0	6.0	Edge	2.75
N	BMVW-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
O	BMVW-t	MT20	6.0	12.0	3.75	7.50
P	BMVW-t	MT20	4.0	6.0		
Q	BMVW-t	MT20	4.0	6.0		
R	BMVW-t	MT20	6.0	12.0	3.25	4.25
S	BMV+p	MT20	3.0	4.0		
T	BMVW-t	MT20	5.0	8.0	2.25	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED		INPUT	REQ'D
	GROSS REACTION		GROSS REACTION			
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
W	2105	0	2105	0	5-8	5-8
M	2109	0	2109	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1496	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 41	-91.8	-91.8	C-V	0 / 166
B-C	0 / 20	-91.8	-91.8	V-D	0 / 73
C-D	-2073 / 0	-91.8	-91.8	D-T	0 / 1289
D-E	-2428 / 0	-91.8	-91.8	T-E	-1242 / 0
E-F	-3069 / 0	-91.8	-91.8	F-T	0 / 2407
F-G	-3080 / 0	-91.8	-91.8	G-R	0 / 878
G-H	-3080 / 0	-91.8	-91.8	H-R	0 / 471
H-I	-2761 / 0	-91.8	-91.8	I-R	-892 / 0
I-J	-2405 / 0	-91.8	-91.8	R-I	0 / 1365
J-K	-2622 / 0	-91.8	-91.8	K-I	0 / 132
K-L	0 / 41	-91.8	-91.8	P-J	-267 / 0
W-B	-244 / 0	0.0	0.0	W-C	-2328 / 0
M-K	-2054 / 0	0.0	0.0	C-M	-70 / 0
				O-K	0 / 2048
W-V	0 / 1465	-18.5	-18.5		
V-U	0 / 1572	-18.5	-18.5		
U-T	0 / 1572	-18.5	-18.5		
T-S	0 / 67	-18.5	-18.5		
S-R	0 / 51	0.0	0.0		
R-F	-478 / 0	0.0	0.0		
R-Q	0 / 2761	-18.5	-18.5		
O-P	0 / 1829	-18.5	-18.5		
P-O	0 / 2081	-18.5	-18.5		
N-O	0 / 17	0.0	0.0		
O-J	-130 / 0	0.0	0.0		
N-M	0 / 58	-18.5	-18.5		

TOTAL WEIGHT = 175 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.78/1.00 (E-F:1), BC=0.51/1.00 (Q-R:1), WB=0.71/1.00 (C-W:1), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (K) (INPUT = 0.90)
JSI METAL = 0.52 (T) (INPUT = 1.00)



Structural component only
DWG# T-2115456

CONTINUED ON PAGE 2

JOB NAME 406403	TRUSS NAME T4S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BS-t	MT20	3.0	6.0		
V	BMWV-t	MT20	4.0	4.0		
W	BMWV1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

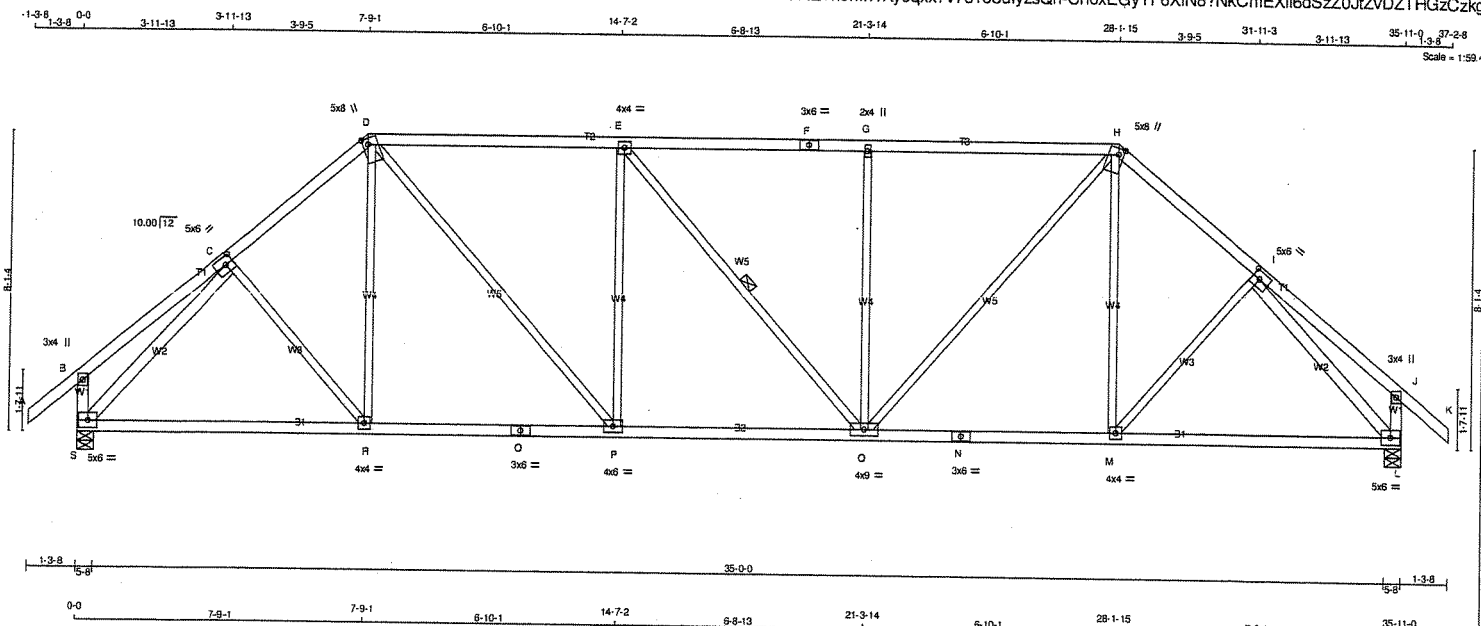


Structural component only
DWG# T-2115456 *mn*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
406403	T5	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
S - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
S - Q	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
S - C	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWV-t	MT20	5.0	6.0	2.50	2.50
D	TTWV+m	MT20	5.0	8.0	2.00	1.75
E	TMWV-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWV+m	MT20	5.0	8.0	2.00	1.75
I	TMWV-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMWV-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWVW-t	MT20	4.0	9.0		
P	BMWV-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
R	BMWV-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0		

NOTES-

- (1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
S	2107	2107	0	5-8
L	2107	2107	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
S	1488	990 / 0	0 / 0	498 / 0	0 / 0
L	1488	990 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1	MAX. FACTORED (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	C-R	0 / 67	0.02 (4)	
B-C	0 / 26	-91.8	-91.8	0.21 (1)	10.00	R-D	0 / 125	0.04 (4)	
C-D	-2060 / 0	-91.8	-91.8	0.29 (1)	4.46	D-P	0 / 1055	0.24 (1)	
D-E	-2255 / 0	-91.8	-91.8	0.82 (1)	3.63	P-E	-675 / 0	0.66 (1)	
E-F	-2255 / 0	-91.8	-91.8	0.81 (1)	3.63	E-O	-2 / 0	0.00 (1)	
F-G	-2255 / 0	-91.8	-91.8	0.81 (1)	3.63	O-G	-675 / 0	0.85 (1)	
G-H	-2255 / 0	-91.8	-91.8	0.81 (1)	3.64	H-I	0 / 1053	0.24 (1)	
H-I	-2060 / 0	-91.8	-91.8	0.29 (1)	4.46	I-H	0 / 125	0.04 (4)	
I-J	0 / 26	-91.8	-91.8	0.21 (1)	10.00	M-I	0 / 67	0.02 (4)	
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	S-C	-2345 / 0	0.99 (1)	
S-B	-264 / 0	0.0	0.0	0.03 (1)	7.81	I-L	-2345 / 0	0.99 (1)	
L-J	-264 / 0	0.0	0.0	0.03 (1)	7.81				
S-R	0 / 1521	-18.5	-18.5	0.39 (1)	10.00				
R-Q	0 / 1560	-18.5	-18.5	0.40 (1)	10.00				
Q-P	0 / 1560	-18.5	-18.5	0.40 (1)	10.00				
P-O	0 / 2256	-18.5	-18.5	0.44 (1)	10.00				
O-N	0 / 1560	-18.5	-18.5	0.40 (1)	10.00				
N-M	0 / 1560	-18.5	-18.5	0.40 (1)	10.00				
M-L	0 / 1522	-18.5	-18.5	0.39 (1)	10.00				

TOTAL WEIGHT = 165 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
DL	= 6.0	PSF	
BOT CH.	LL	= 0.0	PSF
DL	= 7.4	PSF	
TOTAL LOAD	= 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL.(TL) = $L/360$ (1.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.22")

CSI: $TC=0.82/1.00$ (D-E:1), $BC=0.44/1.00$ (O-P:1),
 $WB=0.99/1.00$ (H-L:1), $SSI=0.29/1.00$ (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

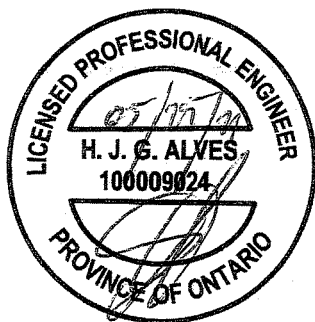
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)

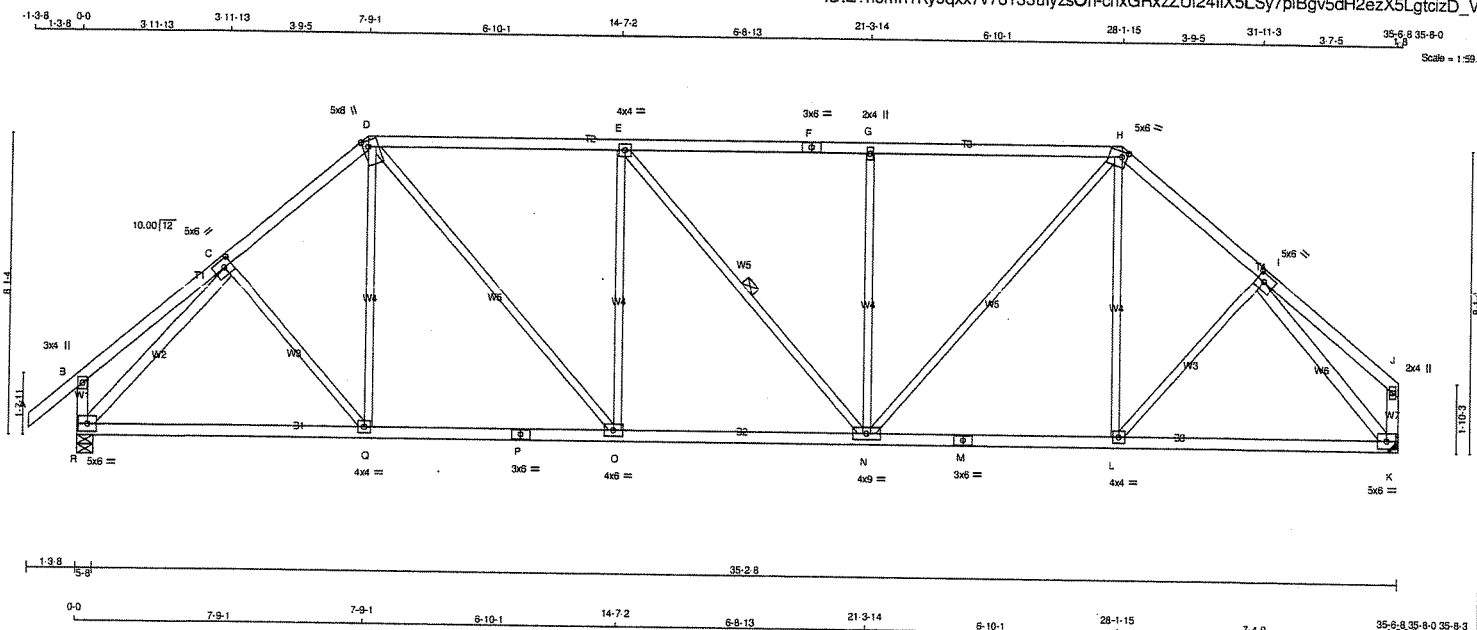
JSI METAL= 0.53 (I) (INPUT = 1.00)



Structural component only
DWG# T-2115457

JOB NAME 411069	TRUSS NAME T5A	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - H	2x4	DRY	No.2 SPF
H - J	2x4	DRY	No.2 SPF
R - B	2x4	DRY	No.2 SPF
R - P	2x4	DRY	No.2 SPF
P - M	2x4	DRY	No.2 SPF
M - K	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF
R - C	2x4	DRY	No.2 SPF
I - K	2x4	DRY	No.2 SPF
K - J	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	2085	0	2085	0	0
K	1967	0	1967	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1472	980 / 0	0 / 0	0 / 0	0 / 0	493 / 0	0 / 0
K	1391	914 / 0	0 / 0	0 / 0	0 / 0	477 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	C-O	0 / 66	0.02 (4)	
B-C	0 / 26	-91.8	-91.8 0.21 (1)	O-D	0 / 126	0.04 (4)	
C-D	-2031 / 0	-91.8	-91.8 0.29 (1)	D-O	0 / 1026	0.23 (1)	
D-E	-2215 / 0	-91.8	-91.8 0.81 (1)	O-E	-654 / 0	0.83 (1)	
E-F	-2196 / 0	-91.8	-91.8 0.80 (1)	E-N	-30 / 0	0.02 (1)	
F-G	-2196 / 0	-91.8	-91.8 0.80 (1)	N-G	-675 / 0	0.85 (1)	
G-H	-2195 / 0	-91.8	-91.8 0.80 (1)	H-N	0 / 1084	0.24 (1)	
H-I	-1956 / 0	-91.8	-91.8 0.27 (1)	L-H	0 / 92	0.03 (4)	
I-J	0 / 35	-91.8	-91.8 0.20 (1)	L-I	0 / 172	0.04 (1)	
R-B	-264 / 0	0.0	0.0 0.03 (1)	R-C	-2316 / 0	0.98 (1)	
				I-K	-2260 / 0	0.90 (1)	
				K-J	-116 / 0	0.01 (1)	
R-O	0 / 1503	-18.5	-18.5 0.39 (1)				
O-P	0 / 1538	-18.5	-18.5 0.39 (1)				
P-O	0 / 1538	-18.5	-18.5 0.39 (1)				
O-N	0 / 2215	-18.5	-18.5 0.43 (1)				
N-M	0 / 1480	-18.5	-18.5 0.37 (1)				
M-L	0 / 1480	-18.5	-18.5 0.37 (1)				
L-K	0 / 1366	-18.5	-18.5 0.36 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.81/1.00 (D-E:1), BC=0.43/1.00 (N-O:1), WB=0.98/1.00 (C-R:1), SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

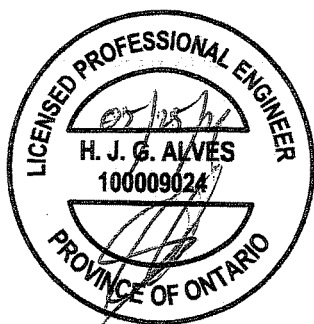
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (H) (INPUT = 0.90)
JSI METAL = 0.52 (C) (INPUT = 1.00)

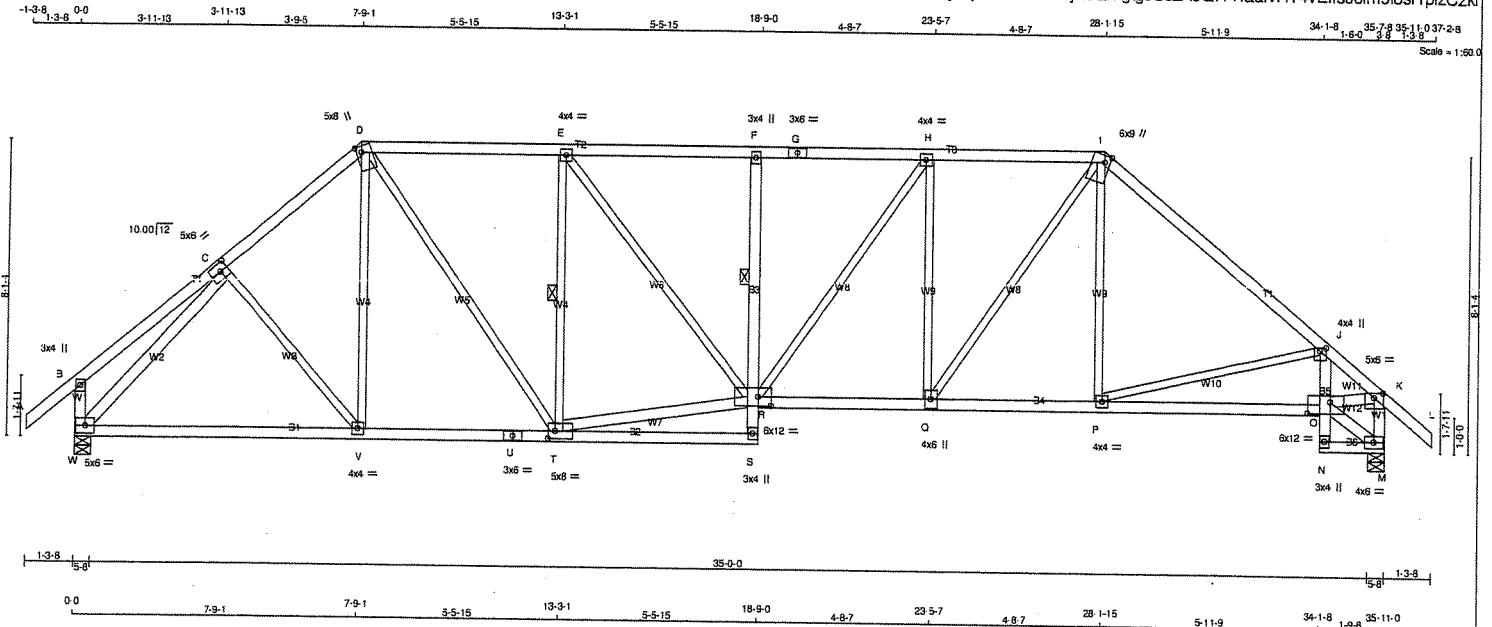


Structural component only
DWG# T-2115513

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
406403	T5S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 12:20:36 2021 Page 1
ID:E?n9mn1Ky9qxx7v78133ulyzsQn-gtJSczA0QY?laalvHT4vElsJolm9I8sl1pizCzklf



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.		
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
W - U	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
W - U	2x4	DRY	No.2	SPF	
U - S	2x4	DRY	No.2	SPF	
S - F	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
N - M	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
T - R	2x4	DRY	No.2	SPF	
W - C	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
O - K	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	UPLIFT
W	2105	0	2105	0
M	2109	0	2109	0

UNFACTORED REACTIONS

	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1496	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-R, E-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (LC)	
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	C-V	0 / 67	0.02 (4)	
B-C	0 / 26	-91.8	-91.8 0.21 (1)	V-D	0 / 114	0.04 (4)	
C-D	-2055 / 0	-91.8	-91.8 0.29 (1)	D-T	0 / 1038	0.23 (1)	
D-E	-2150 / 0	-91.8	-91.8 0.52 (1)	T-E	-1146 / 0	0.48 (1)	
E-F	-2615 / 0	-91.8	-91.8 0.57 (1)	T-R	0 / 2146	0.34 (1)	
F-G	-2622 / 0	-91.8	-91.8 0.42 (1)	E-R	0 / 737	0.17 (1)	
G-H	-2622 / 0	-91.8	-91.8 0.42 (1)	R-H	0 / 387	0.09 (1)	
H-I	-2401 / 0	-91.8	-91.8 0.40 (1)	Q-H	-801 / 0	0.70 (1)	
I-J	-2340 / 0	-91.8	-91.8 0.73 (1)	Q-I	0 / 1086	0.24 (1)	
J-K	-2681 / 0	-91.8	-91.8 0.49 (1)	P-I	0 / 176	0.05 (4)	
K-L	0 / 41	-91.8	-91.8 0.13 (1)	P-J	-409 / 0	0.31 (1)	
W-B	-264 / 0	0.0	0.0 0.03 (1)	W-C	-2340 / 0	0.99 (1)	
M-K	-2053 / 0	0.0	0.0 0.22 (1)	O-M	-72 / 0	0.01 (1)	
				O-K	0 / 2132	0.34 (1)	
W-V	0 / 1518	-18.5	-18.5 0.38 (1)				
V-U	0 / 1557	-18.5	-18.5 0.38 (1)				
U-T	0 / 1557	-18.5	-18.5 0.38 (1)				
T-S	0 / 50	-18.5	-18.5 0.12 (4)				
S-R	0 / 48	0.0	0.0 0.07 (1)				
R-F	-426 / 0	0.0	0.0 0.08 (1)				
R-Q	0 / 2401	-18.5	-18.5 0.44 (1)				
O-P	0 / 1773	-18.5	-18.5 0.37 (1)				
P-O	0 / 2166	-18.5	-18.5 0.44 (1)				
N-O	0 / 17	0.0	0.0 0.09 (1)				
O-J	-138 / 4	0.0	0.0 0.09 (1)				
N-M	0 / 59	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.73/1.00 (I-J:1), BC=0.44/1.00 (O-R:1), WB=0.99/1.00 (C-W:1), SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (K) (INPUT = 0.90)

JSI METAL = 0.52 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115458

CONTINUED ON PAGE 2

JOB NAME 406403	TRUSS NAME T5S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

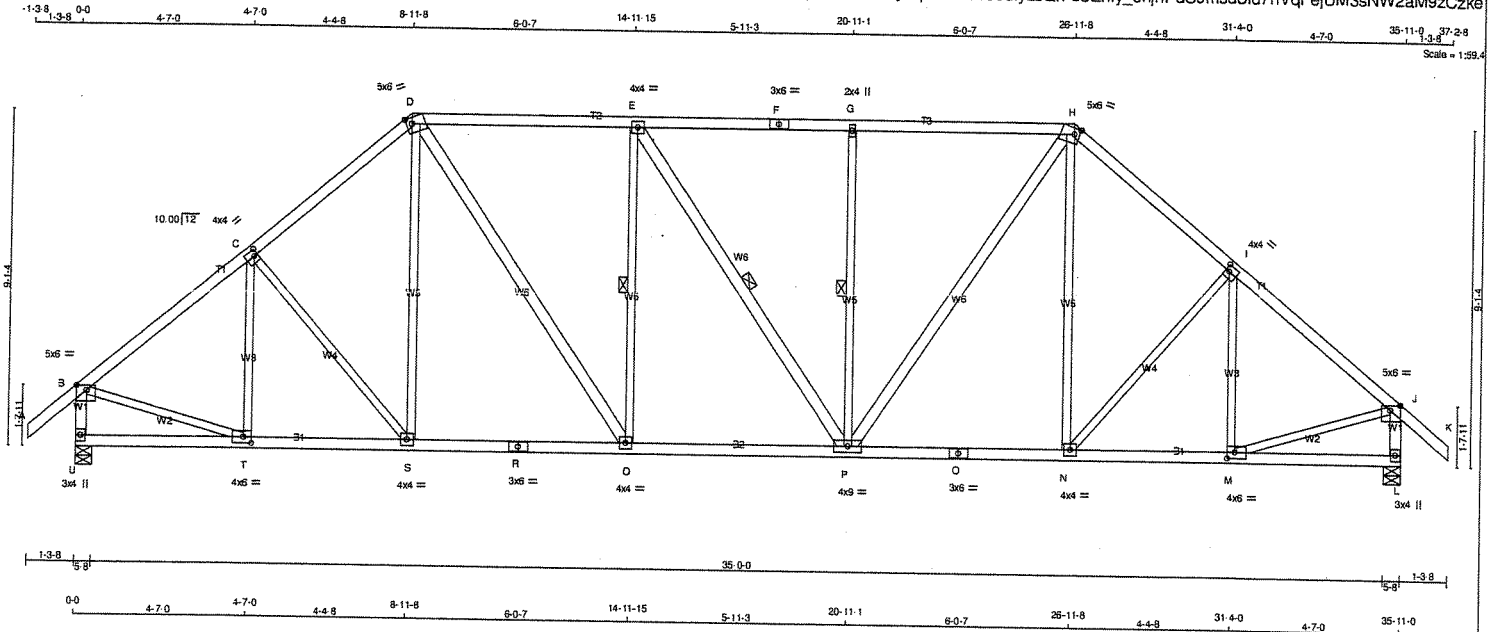


Structural component only
DWG# T-2115458 *m*

JOB NAME 406403	TRUSS NAME T6	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Tue May 25 12:20:37 2021 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
U - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
U - R	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - O	2x4	DRY	No.2	SPF	
E - P	2x4	DRY	No.2	SPF	
P - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	1.75
I	TMVW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	6.0	1.50	3.25
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-t	MT20	4.0	6.0	2.00	2.50
N, Q, S						
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
T	BMVW-t	MT20	4.0	6.0	2.00	2.50
U	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
U	VERT	DOWN	IN-SX	IN-SX
U	2107	0	5-8	5-8
L	2107	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
U	COMBINED	SNOW	LIVE	PERM. LIVE	
U	1488	990 / 0	0 / 0	0 / 0	498 / 0
L	1488	990 / 0	0 / 0	0 / 0	498 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO						FR-TO			
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	T-C	-369 / 0	0.16 (1)	
B-C	-2041 / 0	-91.8	-91.8	0.41 (1)	4.35	C-S	-112 / 0	0.10 (1)	
C-D	-2012 / 0	-91.8	-91.8	0.40 (1)	4.37	S-D	0 / 184	0.05 (4)	
D-E	-2005 / 0	-91.8	-91.8	0.58 (1)	4.13	D-O	0 / 858	0.14 (1)	
E-F	-2004 / 0	-91.8	-91.8	0.58 (1)	4.13	Q-E	-594 / 0	0.32 (1)	
F-G	-2004 / 0	-91.8	-91.8	0.58 (1)	4.13	E-P	-2 / 0	0.00 (1)	
G-H	-2004 / 0	-91.8	-91.8	0.57 (1)	4.14	P-G	-593 / 0	0.32 (1)	
H-I	-2012 / 0	-91.8	-91.8	0.40 (1)	4.37	P-H	0 / 855	0.14 (1)	
I-J	-2041 / 0	-91.8	-91.8	0.41 (1)	4.35	N-H	0 / 185	0.05 (4)	
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	N-I	-112 / 0	0.10 (1)	
U-B	-2069 / 0	0.0	0.0	0.22 (1)	5.88	M-I	-370 / 0	0.17 (1)	
L-J	-2069 / 0	0.0	0.0	0.22 (1)	5.88	B-T	0 / 1655	0.37 (1)	
						M-J	0 / 1654	0.37 (1)	
U-T	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
T-S	0 / 1591	-18.5	-18.5	0.31 (1)	10.00				
S-R	0 / 1520	-18.5	-18.5	0.32 (1)	10.00				
R-Q	0 / 1520	-18.5	-18.5	0.32 (1)	10.00				
Q-P	0 / 2005	-18.5	-18.5	0.40 (1)	10.00				
P-O	0 / 1520	-18.5	-18.5	0.31 (1)	10.00				
O-N	0 / 1520	-18.5	-18.5	0.31 (1)	10.00				
N-M	0 / 1591	-18.5	-18.5	0.32 (1)	10.00				
M-L	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

TOTAL WEIGHT = 2 X 184 = 367 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.58/1.00 (D-E:1), BC=0.40/1.00 (P-Q:1), WB=0.37/1.00 (B-T:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

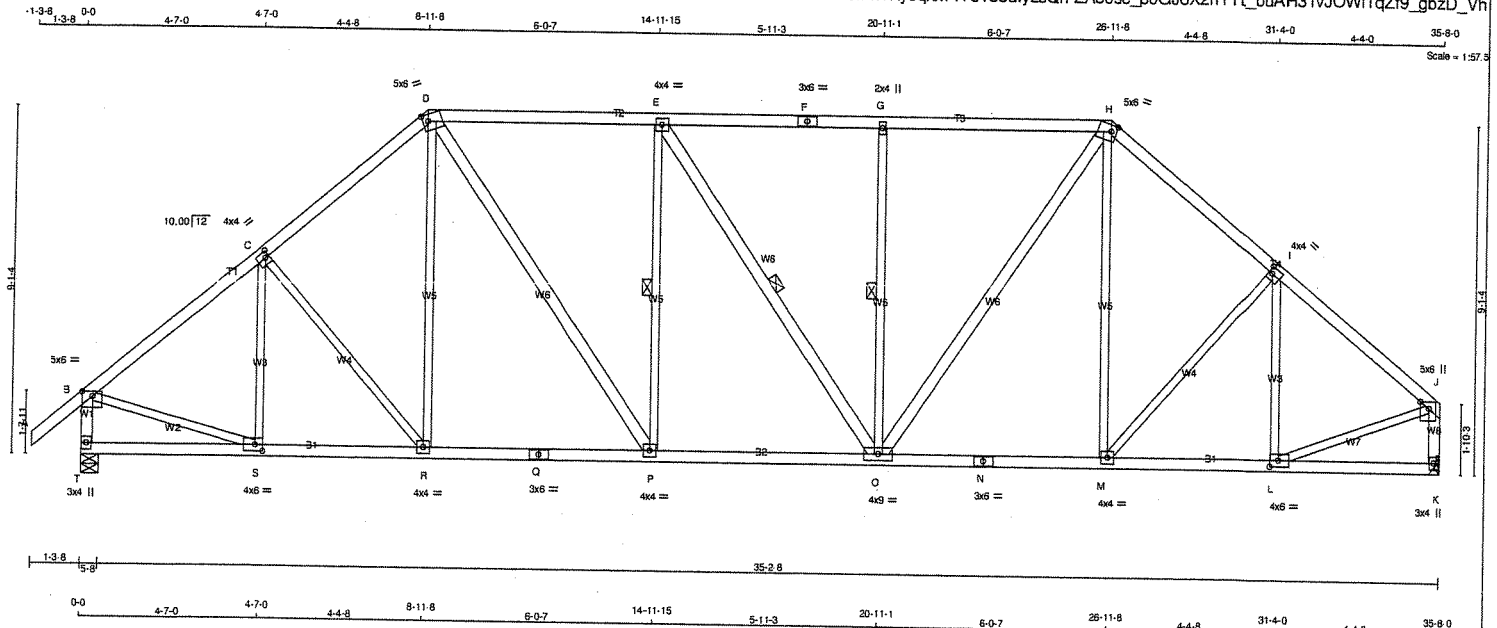
JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.52 (O) (INPUT = 1.00)



Structural component only
DWG# T-2115459

JOB NAME 411069	TRUSS NAME T6A	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitak Industries, Inc. Tue May 25 11:28:18 2021 Page 1
ID:E?n9mn1Ky9qxx7v78133ulyzsQn-ZA30sc_p0GJoX2hTtTt_buAH31vJOWi1qZf9_gbzD_Vh



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.		
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
T - Q	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
D - P	2x4	DRY	No.2	SPF	
E - O	2x4	DRY	No.2	SPF	
O - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	1.50 3.25
C	TMVW-i	MT20	4.0	4.0	2.00 1.25
D	TTWW-m	MT20	5.0	6.0	2.00 1.75
E	TMVW-i	MT20	4.0	4.0	
F	TS-i	MT20	3.0	6.0	
G	TMVW-w	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	6.0	2.00 1.75
I	TMVW-i	MT20	4.0	4.0	2.00 1.25
J	TMVW-p	MT20	5.0	6.0	Edge 2.75
K	BMV1-p	MT20	3.0	4.0	
L	BMVW-i	MT20	4.0	6.0	2.00 2.75
M, P, R					
M	BMVW-i	MT20	4.0	4.0	
N	BS-i	MT20	3.0	6.0	
O	BMVW-w	MT20	4.0	9.0	
O	BS-i	MT20	3.0	6.0	
S	BMVW-i	MT20	4.0	6.0	2.00 2.50
T	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1)



Structural component only
DWG# T-2115514

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP/LIFT	IN-SX
T	2093	0	0	5-8
K	1966	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

	1ST LCASE COMBINED	MAX./MIN. SNOW	MAX./MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1478	983 / 0	0 / 0	0 / 0	0 / 0	495 / 0	0 / 0
K	1390	913 / 0	0 / 0	0 / 0	0 / 0	478 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, E-O, G-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	S-C	-366 / 0	0.16 (1)
B-C	-2025 / 0	-91.8	-91.8 0.41 (1)	4.36	C-R	-115 / 0	0.10 (1)
C-D	-1994 / 0	-91.8	-91.8 0.40 (1)	4.39	R-D	0 / 186	0.05 (4)
D-E	-1982 / 0	-91.8	-91.8 0.58 (1)	4.14	D-P	0 / 841	0.14 (1)
E-F	-1971 / 0	-91.8	-91.8 0.58 (1)	4.16	P-E	-581 / 0	0.31 (1)
F-G	-1971 / 0	-91.8	-91.8 0.58 (1)	4.16	E-O	-19 / 0	0.01 (1)
G-H	-1971 / 0	-91.8	-91.8 0.57 (1)	4.17	O-G	-594 / 0	0.32 (1)
H-I	-1956 / 0	-91.8	-91.8 0.38 (1)	4.45	O-H	0 / 872	0.14 (1)
I-J	-1928 / 0	-91.8	-91.8 0.37 (1)	4.48	M-H	0 / 134	0.04 (4)
T-B	-2056 / 0	0.0	0.0 0.22 (1)	5.90	M-I	-43 / 0	0.04 (1)
K-J	-1931 / 0	0.0	0.0 0.21 (1)	6.06	L-I	-445 / 0	0.20 (1)
					B-S	0 / 1642	0.37 (1)
					L-J	0 / 1593	0.36 (1)
T-S	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
S-R	0 / 1579	-18.5	-18.5 0.31 (1)	10.00			
R-Q	0 / 1506	-18.5	-18.5 0.32 (1)	10.00			
Q-P	0 / 1506	-18.5	-18.5 0.32 (1)	10.00			
P-O	0 / 1982	-18.5	-18.5 0.39 (1)	10.00			
O-N	0 / 1478	-18.5	-18.5 0.30 (1)	10.00			
N-M	0 / 1478	-18.5	-18.5 0.30 (1)	10.00			
M-L	0 / 1504	-18.5	-18.5 0.30 (1)	10.00			
L-K	0 / 0	-18.5	-18.5 0.07 (4)	10.00			

TOTAL WEIGHT = 181 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (1.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.17")

CSI: TC=0.58/1.00 (D-E:1), BC=0.39/1.00 (O-P:1), WB=0.37/1.00 (B-S:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

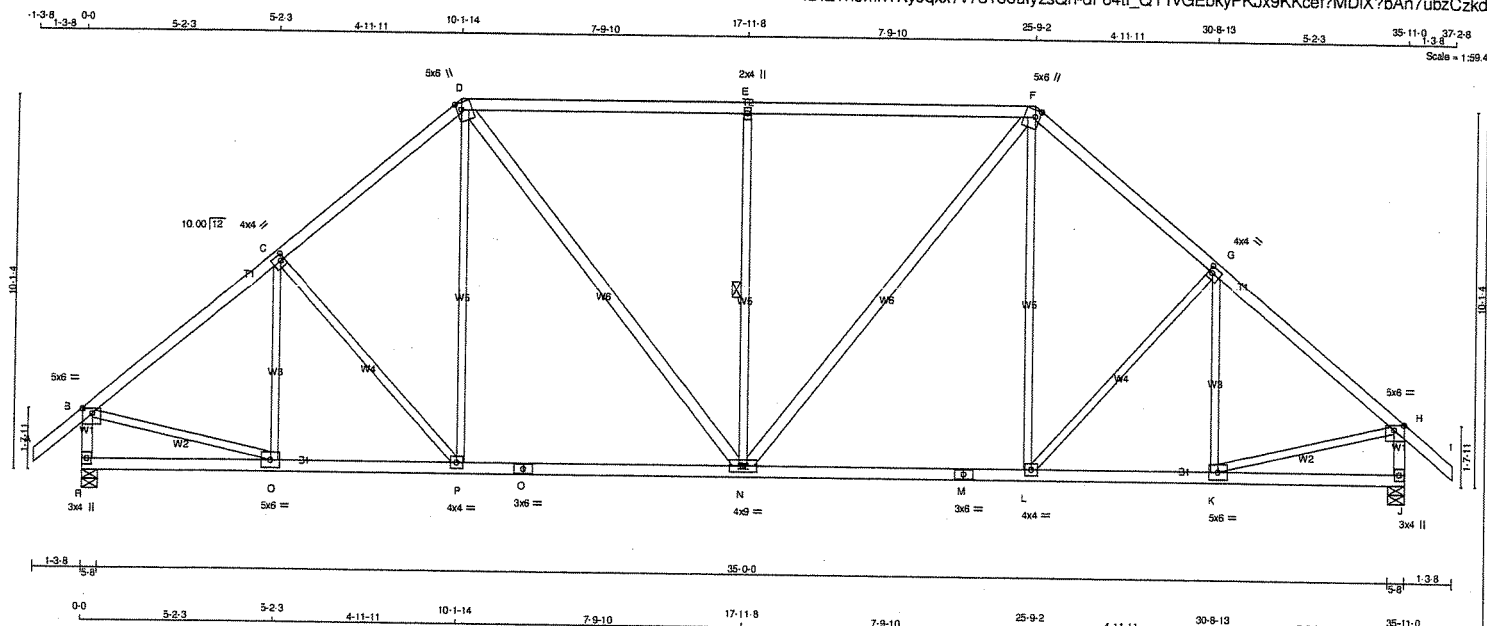
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (H) (INPUT = 0.90)
JSI METAL = 0.51 (Q) (INPUT = 1.00)

JOB NAME 406403	TRUSS NAME T7	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 12:20:38 2021 Page 1
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LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - D	2x4	DRY	No.2	SPF	SPF
D - F	2x4	DRY	No.2	SPF	SPF
F - I	2x4	DRY	No.2	SPF	SPF
R - B	2x4	DRY	No.2	SPF	SPF
J - H	2x4	DRY	No.2	SPF	SPF
R - O	2x4	DRY	No.2	SPF	SPF
O - M	2x4	DRY	No.2	SPF	SPF
M - J	2x4	DRY	No.2	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF
EXCEPT					
D - N	2x4	DRY	No.2	SPF	SPF
N - F	2x4	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
GROSS REACTION	GROSS REACTION	DOWN	UP	BRG	BRG
VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
JT	2107	0	2107	0	5-8
R	2107	0	2107	0	5-8
J	2107	0	2107	0	5-8

UNFACTORED REACTIONS

1ST LOOSE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	UVE	PERM. LIVE	WIND	DEAD	SOIL
R	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
J	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	MAX	LC1
(LBS)	(PLF)	FROM	TO	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO				FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	Q-C	-314 / 0	0.17 (1)
B-C	-2070 / 0	-91.8	-91.8 0.39 (1)	4.36	C-P	-207 / 0	0.24 (1)
C-D	-1964 / 0	-91.8	-91.8 0.37 (1)	4.47	P-D	0 / 276	0.06 (1)
D-E	-1891 / 0	-91.8	-91.8 0.84 (1)	3.64	D-N	0 / 659	0.11 (1)
E-F	-1891 / 0	-91.8	-91.8 0.84 (1)	3.64	N-E	-881 / 0	0.61 (1)
F-G	-1964 / 0	-91.8	-91.8 0.37 (1)	4.47	N-F	0 / 659	0.11 (1)
G-H	-2070 / 0	-91.8	-91.8 0.39 (1)	4.36	L-F	0 / 276	0.06 (1)
H-I	0 / 41	-91.8	-91.8 0.13 (1)	10.00	L-G	-207 / 0	0.24 (1)
R-B	-2065 / 0	0.0	0.0 0.22 (1)	5.89	K-G	-314 / 0	0.17 (1)
J-H	-2065 / 0	0.0	0.0 0.22 (1)	5.89	B-O	0 / 1667	0.38 (1)
					K-H	0 / 1667	0.38 (1)
R-O	0 / 0	-18.5	-18.5 0.10 (4)	10.00			
Q-P	0 / 1617	-18.5	-18.5 0.35 (1)	10.00			
P-O	0 / 1481	-18.5	-18.5 0.37 (1)	10.00			
O-N	0 / 1481	-18.5	-18.5 0.37 (1)	10.00			
N-M	0 / 1481	-18.5	-18.5 0.37 (1)	10.00			
M-L	0 / 1481	-18.5	-18.5 0.37 (1)	10.00			
L-K	0 / 1617	-18.5	-18.5 0.35 (1)	10.00			
K-J	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (1.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.84/1.00 (D-E:1), BC=0.37/1.00 (N-P:1),
WB=0.61/1.00 (E-N:1), SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	6.0	1.50	3.25
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0		
R	BMV1-p	MT20	3.0	4.0		

NOTES-

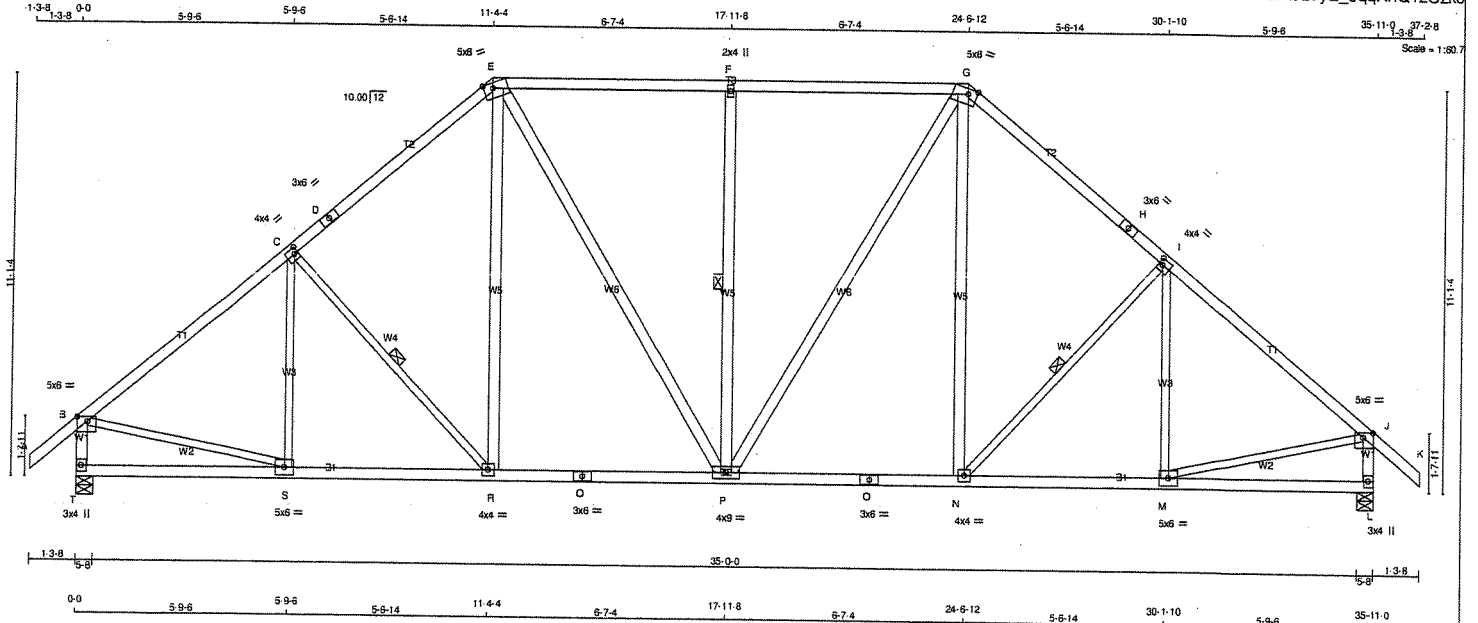
- (1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115460

JOB NAME 406403	TRUSS NAME T8	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER				DESCR	
N. L. G. A. RULES				SPF	
CHORDS	SIZE	LUMBER		SPF	
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
T - Q	2x4	DRY	No.2	SPF	
Q - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
S - C	2x3	DRY	No.2	SPF	
C - R	2x3	DRY	No.2	SPF	
N - I	2x3	DRY	No.2	SPF	
M - I	2x3	DRY	No.2	SPF	
B - S	2x3	DRY	No.2	SPF	
M - J	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMVW-l	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TS-t	MT20	3.0	6.0		
I	TMVW-l	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	6.0	1.50	3.25
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-l	MT20	5.0	6.0		
N	BMVW-l	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMVW-l	MT20	4.0	4.0		
S	BMVW-l	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT GROSS REACTION	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
T	2107	0	0	5-8
L	2107	0	0	5-8

UNFACTORED REACTIONS

	1ST LOSE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	S-C	-255 / 6	0.17 (1)
B-C	-2089 / 0	-91.8 -91.8	0.50 (1)	4.22	C-R	-306 / 0	0.15 (1)
C-D	-1901 / 0	-91.8 -91.8	0.47 (1)	4.41	R-E	0 / 336	0.05 (1)
D-E	-1901 / 0	-91.8 -91.8	0.47 (1)	4.41	E-P	0 / 507	0.08 (1)
E-F	-1695 / 0	-91.8 -91.8	0.57 (1)	4.40	P-F	-743 / 0	0.47 (1)
F-G	-1695 / 0	-91.8 -91.8	0.57 (1)	4.40	P-G	0 / 507	0.08 (1)
G-H	-1901 / 0	-91.8 -91.8	0.47 (1)	4.41	N-G	0 / 336	0.05 (1)
H-I	-1901 / 0	-91.8 -91.8	0.47 (1)	4.41	I-N	-306 / 0	0.15 (1)
I-J	-2089 / 0	-91.8 -91.8	0.50 (1)	4.22	M-I	-255 / 6	0.17 (1)
J-K	0 / 41	-91.8 -91.8	0.13 (1)	10.00	B-S	0 / 1677	0.38 (1)
T-B	-2062 / 0	0.0 0.0	0.22 (1)	5.89	M-J	0 / 1677	0.38 (1)
L-J	-2062 / 0	0.0 0.0	0.22 (1)	5.89			
T-S	0 / 0	-18.5 -18.5	0.14 (4)	10.00			
S-R	0 / 1635	-18.5 -18.5	0.34 (1)	10.00			
R-Q	0 / 1431	-18.5 -18.5	0.32 (1)	10.00			
O-P	0 / 1431	-18.5 -18.5	0.32 (1)	10.00			
P-O	0 / 1431	-18.5 -18.5	0.32 (1)	10.00			
O-N	0 / 1431	-18.5 -18.5	0.32 (1)	10.00			
N-M	0 / 1635	-18.5 -18.5	0.34 (1)	10.00			
M-L	0 / 0	-18.5 -18.5	0.14 (4)	10.00			

TOTAL WEIGHT = 2 X 192 = 385 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.57/1.00 (E-F:1), BC=0.34/1.00 (M-N:1), WB=0.47/1.00 (F-P:1), SS=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (J) (INPUT = 0.90)
JSI METAL = 0.49 (Q) (INPUT = 1.00)

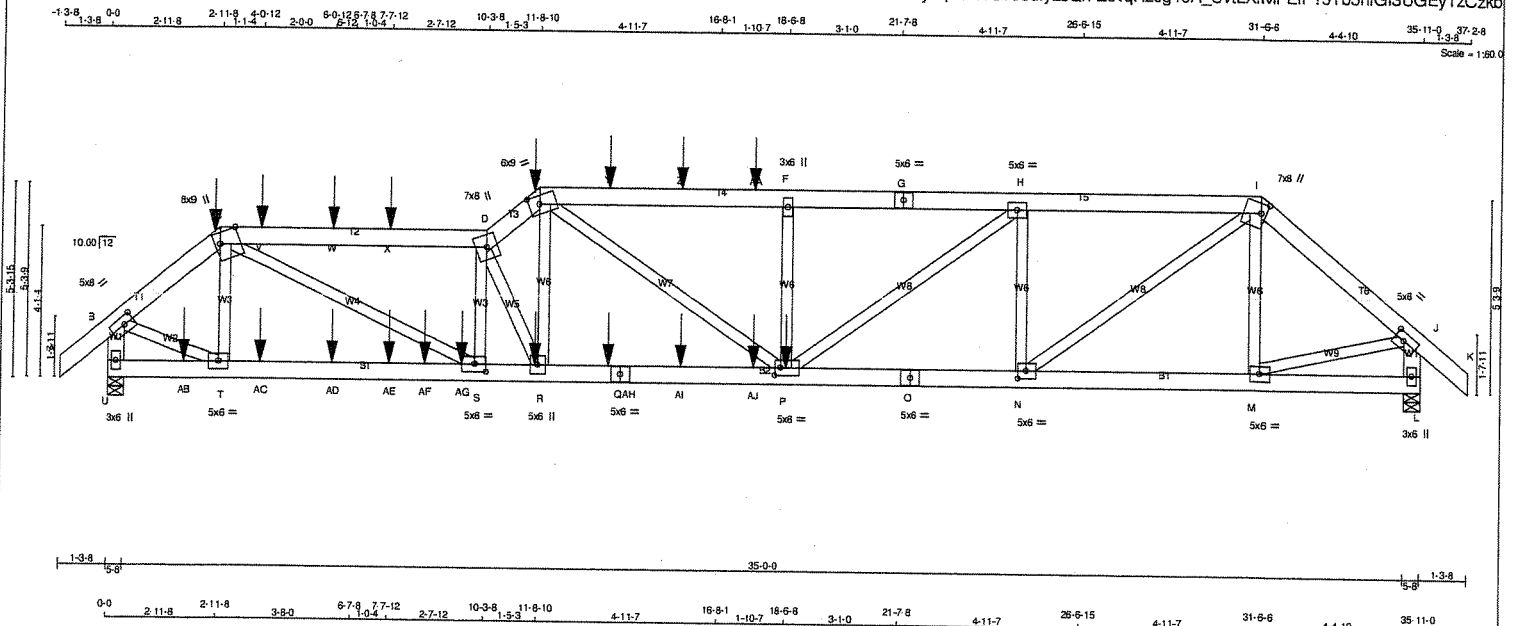


Structural component only
DWG# T-2115461

JOB NAME 406403	TRUSS NAME T9	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x6	DRY	No.2
C - D	2x6	DRY	No.2
D - E	2x6	DRY	No.2
E - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
I - K	2x6	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - O	2x6	DRY	No.2
O - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2

DRY: SEASONED LUMBER

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	2	12
C - D	2	12
D - E	2	12
E - G	2	12
G - I	2	12
I - K	2	12
U - B	2	12
L - J	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U - O	2	12
O - O	2	12
O - L	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6
F - P	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REORD BRG IN-SX
JT VERT	4502	4502	0	0
U	3773	3773	0	0
L	0	0	0	0

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
U	3177	2122 / 0	0 / 0	0 / 0	1055 / 0	0 / 0	
L	2861	1786 / 0	0 / 0	0 / 0	875 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 42	T-C	-927 / 0
B-C	-4875 / 0	C-S	0 / 6785
C-V	-9598 / 0	S-D	-3177 / 0
V-W	-9598 / 0	D-R	-4372 / 0
W-X	-9598 / 0	R-E	0 / 4398
X-D	-9598 / 0	E-P	0 / 1977
D-E	-10380 / 0	N-H	-2672 / 0
E-Y	-9724 / 0	N-I	0 / 4719
Y-Z	-9724 / 0	M-I	-641 / 0
Z-AA	-9724 / 0	B-T	0 / 3794
AA-F	-9724 / 0	M-J	0 / 3236
F-G	-9724 / 0	P-F	-909 / 0
G-H	-9724 / 0	P-H	0 / 3496
H-I	-6932 / 0		
I-J	-4107 / 0		
J-K	0 / 42		
U-B	-4566 / 0		
L-J	-3749 / 0		

U-AB	0 / 0	-18.5	-18.5	0.12 (1)	10.00
AB-T	0 / 0	-18.5	-18.5	0.12 (1)	10.00
T-AC	0 / 3533	-18.5	-18.5	0.35 (1)	10.00
AC-AD	0 / 3533	-18.5	-18.5	0.35 (1)	10.00
AD-AE	0 / 3533	-18.5	-18.5	0.35 (1)	10.00
AE-AF	0 / 3533	-18.5	-18.5	0.35 (1)	10.00
AF-AG	0 / 3533	-18.5	-18.5	0.35 (1)	10.00
AG-S	0 / 3533	-18.5	-18.5	0.35 (1)	10.00
S-R	0 / 9724	-18.5	-18.5	0.74 (1)	10.00
R-AH	0 / 8108	-18.5	-18.5	0.60 (1)	10.00
AH-O	0 / 8108	-18.5	-18.5	0.60 (1)	10.00
O-AI	0 / 8108	-18.5	-18.5	0.60 (1)	10.00
AI-AJ	0 / 8108	-18.5	-18.5	0.60 (1)	10.00
AJ-P	0 / 8108	-18.5	-18.5	0.60 (1)	10.00
P-O	0 / 6932	-18.5	-18.5	0.49 (1)	10.00
O-N	0 / 6932	-18.5	-18.5	0.49 (1)	10.00
N-M	0 / 3123	-18.5	-18.5	0.23 (1)	10.00
M-L	0 / 0	-18.5	-18.5	0.04 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
E	11-8-10	-53	-53	---	FRONT	VERT	DEAD	---	C1

TOTAL WEIGHT = 2 X 218 = 437 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADD'TL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")

CSI: TC=0.58/1.00 (C-D:1), BC=0.74/1.00 (R-S:1),
WB=0.60/1.00 (C-S:1), SS=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)

JSI METAL= 0.80 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2115462

JOB NAME 406403	TRUSS NAME T9	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TTWW+m	MT20	7.0	8.0		
E	TTWW-m	MT20	6.0	9.0	2.75	3.50
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	7.0	8.0	3.25	1.50
J	TMVW-t	MT20	5.0	8.0	2.50	3.25
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	2.00
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	8.0	2.50	3.75
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

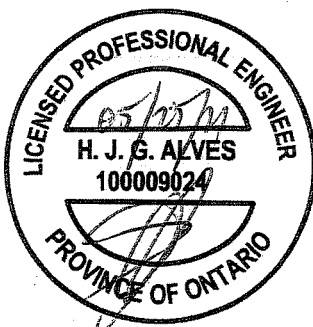
1) Lateral braces to be a minimum of 2x4 SPF #2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	11-8-10	-105	-105	---	BACK	VERT	TOTAL	---	C1
E	11-8-10	-233	-233	---	FRONT	VERT	SNOW	---	C1
P	18-6-8	-1534	-1534	---	BACK	VERT	TOTAL	---	C1
R	11-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
V	4-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
W	6-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
X	7-7-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Y	13-7-12	-100	-100	---	BACK	VERT	TOTAL	---	C1
Z	15-7-12	-100	-100	---	BACK	VERT	TOTAL	---	C1
AA	17-7-12	-100	-100	---	BACK	VERT	TOTAL	---	C1
AB	2-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	4-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	6-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	7-7-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AF	8-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
AG	9-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
AH	13-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
AI	15-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
AJ	17-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

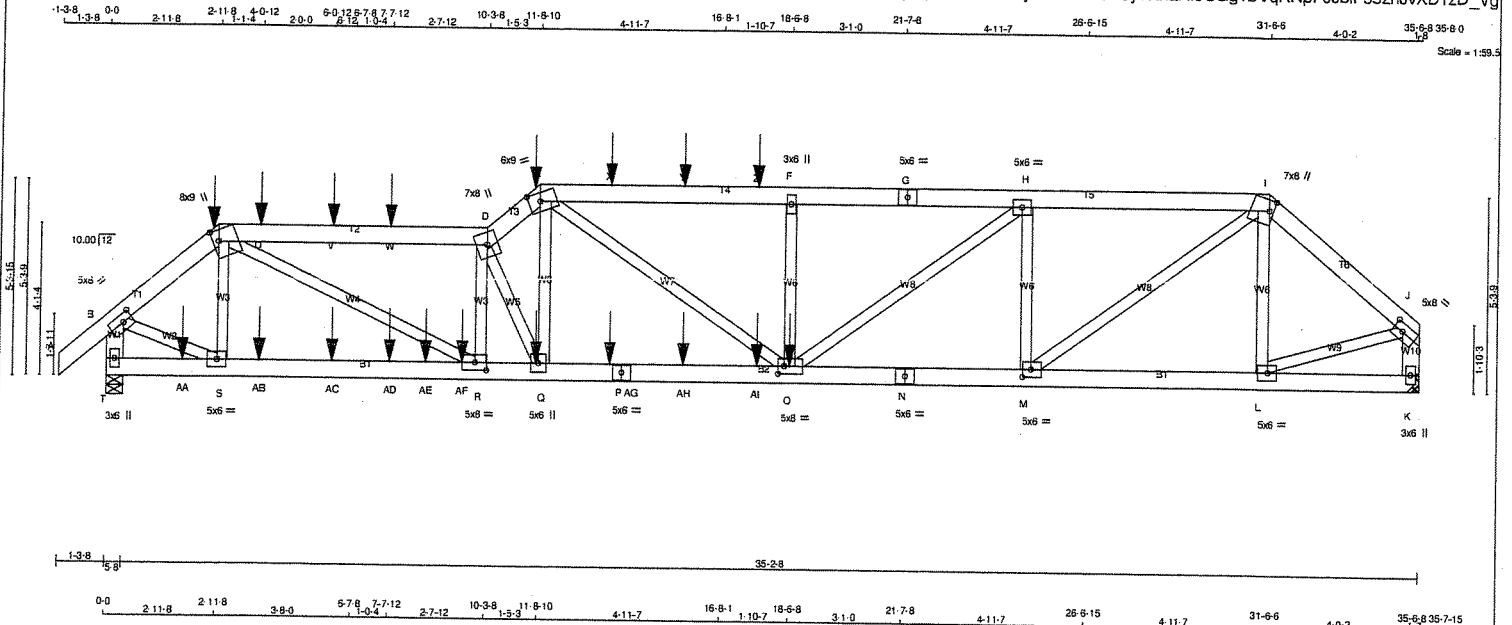
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2115462

JOB NAME 411069	TRUSS NAME T9A	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x6	DRY	No.2 SPF
C - D	2x6	DRY	No.2 SPF
D - E	2x6	DRY	No.2 SPF
E - G	2x6	DRY	No.2 SPF
G - I	2x6	DRY	No.2 SPF
I - J	2x6	DRY	No.2 SPF
T - B	2x6	DRY	No.2 SPF
T - P	2x6	DRY	No.2 SPF
P - N	2x6	DRY	No.2 SPF
N - K	2x6	DRY	No.2 SPF
ALL WEBS EXCEPT K - J	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS.

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2	12	SIDE(122.0)
C-D 2	12	SIDE(61.0)
D-E 2	12	SIDE(122.0)
E-G 2	12	SIDE(61.0)
G-I 2	12	TOP
I-J 2	12	TOP
T-B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
T-P 2	12	SIDE(183.1)
P-N 2	12	SIDE(183.1)
N-K 2	12	SIDE(334.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	
F-O 1	6	
2x6 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT VERT	4358	4358	0	5-8
T HORZ	0	0	0	5-8
K UPLIFT	3533	3533	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 4-0.

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT COMBINED	3077	2047 / 0	0 / 0	0 / 0	0 / 0	1030 / 0	0 / 0
T SNOW	2496	1654 / 0	0 / 0	0 / 0	0 / 0	841 / 0	0 / 0
K LIVE							
K PERM. LIVE							
K WIND							
K DEAD							
K SOIL							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO				FR-TO			
A-B	0 / 42	-91.8	-91.8 0.04 (1)	10.00	S-C	-890 / 0	0.08 (1)
B-C	-4515 / 0	-91.8	-91.8 0.06 (1)	5.38	C-R	0 / 6473	0.57 (1)
C-U	-9197 / 0	-91.8	-91.8 0.57 (1)	3.51	R-D	-3021 / 0	0.26 (1)
U-V	-9197 / 0	-91.8	-91.8 0.57 (1)	3.51	D-O	-4227 / 0	0.39 (1)
V-W	-9197 / 0	-91.8	-91.8 0.57 (1)	3.51	O-E	0 / 4251	0.38 (1)
W-D	-9197 / 0	-91.8	-91.8 0.57 (1)	3.51	E-O	0 / 1738	0.15 (1)
D-E	-9928 / 0	-91.8	-91.8 0.13 (1)	3.82	M-H	-2622 / 0	0.35 (1)
E-X	-9177 / 0	-91.8	-91.8 0.45 (1)	3.67	M-I	0 / 4627	0.41 (1)
X-Y	-9177 / 0	-91.8	-91.8 0.45 (1)	3.67	L-I	-892 / 0	0.12 (1)
Y-Z	-9177 / 0	-91.8	-91.8 0.45 (1)	3.67	B-S	0 / 3664	0.32 (1)
Z-F	-9177 / 0	-91.8	-91.8 0.45 (1)	3.67	L-J	0 / 2932	0.26 (1)
F-G	-9178 / 0	-91.8	-91.8 0.34 (1)	3.78	O-F	-915 / 0	0.12 (1)
G-H	-9178 / 0	-91.8	-91.8 0.34 (1)	3.78	O-H	0 / 3409	0.30 (1)
H-I	-6455 / 0	-91.8	-91.8 0.22 (1)	4.53	K-J	-3518 / 0	0.13 (1)
I-J	-3592 / 0	-91.8	-91.8 0.08 (1)	5.85			
T-B	-4420 / 0	0.0	0.0 0.16 (1)	6.84			

T-AA	0 / 0	-18.5	-18.5 0.11 (4)	10.00
AA-S	0 / 0	-18.5	-18.5 0.11 (4)	10.00
S-AB	0 / 3413	-18.5	-18.5 0.34 (1)	10.00
AB-AC	0 / 3413	-18.5	-18.5 0.34 (1)	10.00
AC-AD	0 / 3413	-18.5	-18.5 0.34 (1)	10.00
AD-AE	0 / 3413	-18.5	-18.5 0.34 (1)	10.00
AE-AF	0 / 3413	-18.5	-18.5 0.34 (1)	10.00
AF-R	0 / 3413	-18.5	-18.5 0.34 (1)	10.00
R-O	0 / 9319	-18.5	-18.5 0.70 (1)	10.00
Q-AG	0 / 7756	-18.5	-18.5 0.57 (1)	10.00
AG-P	0 / 7756	-18.5	-18.5 0.57 (1)	10.00
P-AH	0 / 7756	-18.5	-18.5 0.57 (1)	10.00
AH-AI	0 / 7756	-18.5	-18.5 0.57 (1)	10.00
AI-O	0 / 7756	-18.5	-18.5 0.57 (1)	10.00
O-N	0 / 6455	-18.5	-18.5 0.46 (1)	10.00
N-M	0 / 6455	-18.5	-18.5 0.46 (1)	10.00
M-L	0 / 2720	-18.5	-18.5 0.21 (1)	10.00
L-K	0 / 0	-18.5	-18.5 0.04 (4)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TC=0.57/1.00 (C-D:1), BC=0.70/1.00 (Q-R:1), WB=0.57/1.00 (C-R:1), SS=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX MIN
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (I) (INPUT = 0.90)
JSI METAL = 0.76 (E) (INPUT = 1.00)



Structural component only
DWG# T-2115515

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411069	T9A	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:E?n9mn1Ky9gxx7v78133ulvzsQn-1MdP3y?RnaRf9CGq1bVgRNpF0JblF55znJvXD1zD Vg

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	3.50	1.75
D	TTWW+m	MT20	7.0	8.0		
E	TTWW-m	MT20	6.0	9.0	2.75	3.75
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	7.0	8.0	3.50	1.50
J	TMWW-t	MT20	5.0	8.0	2.50	3.25
K	BMW1+w	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0	2.50	2.00
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	8.0	2.50	3.75
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
E	11-8-10	-53	-53	---	FRONT	VERT	DEAD	---	C1
E	11-8-10	-105	-105	---	BACK	VERT	TOTAL	---	C1
E	11-8-10	-233	-233	---	FRONT	VERT	SNOW	---	C1
O	18-6-8	-1396	-1396	---	BACK	VERT	TOTAL	---	C1
Q	11-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
U	4-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
V	6-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
W	7-7-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
X	13-7-12	-100	-100	---	BACK	VERT	TOTAL	---	C1
Y	15-7-12	-100	-100	---	BACK	VERT	TOTAL	---	C1
Z	17-7-12	-100	-100	---	BACK	VERT	TOTAL	---	C1
AA	2-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AB	4-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	6-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	7-7-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	8-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
AF	9-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
AG	13-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
AH	15-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
AI	17-7-12	-27	-27	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

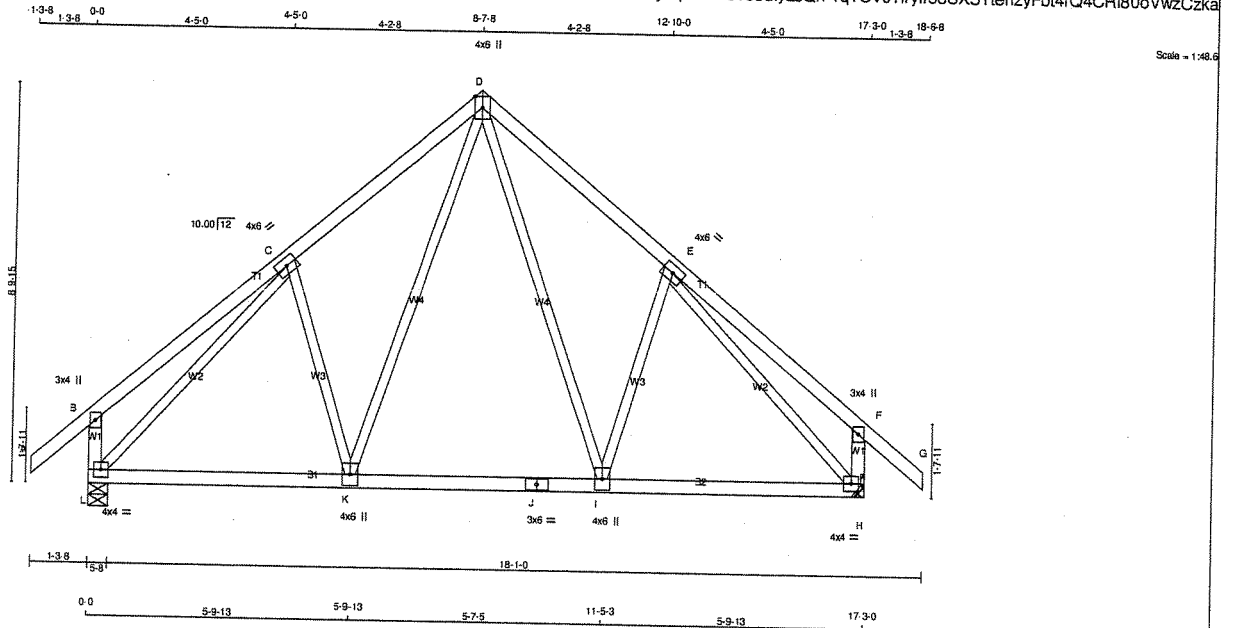
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2115515 *m*

JOB NAME 406403	TRUSS NAME T10	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-t	MT20	4.0	6.0
D	TTWW+p	MT20	4.0	6.0
E	TMWW-t	MT20	4.0	6.0
F	TMV+p	MT20	4.0	4.0
H	BMVW1-t	MT20	4.0	4.0
I	BMVW-t	MT20	4.0	6.0
J	BS-t	MT20	3.0	6.0
K	BMVW-t	MT20	4.0	6.0
L	BMVW1-t	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
L	VERT	DOWN	UP	IN-SX	IN-SX
L	1078	0	1078	0	5-8
H	1078	0	1078	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	COMBINED	512 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
H		512 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8	0.13 (1)	10.00	D-I	0 / 358	0.08 (1)
B-C	0 / 30	-91.8 -91.8	0.28 (1)	10.00	I-E	-236 / 0	0.11 (1)
C-D	-801 / 0	-91.8 -91.8	0.22 (1)	6.25	K-D	0 / 358	0.08 (1)
D-E	-801 / 0	-91.8 -91.8	0.22 (1)	6.25	C-K	-236 / 0	0.11 (1)
E-F	0 / 30	-91.8 -91.8	0.28 (1)	10.00	L-C	-1008 / 0	0.75 (1)
F-G	0 / 41	-91.8 -91.8	0.13 (1)	10.00	E-H	-1008 / 0	0.75 (1)
L-B	-276 / 0	0.0 0.0	0.03 (1)	7.81			
H-F	-276 / 0	0.0 0.0	0.03 (1)	7.81			
L-K	0 / 665	-18.5 -18.5	0.19 (4)	10.00			
K-J	0 / 481	-18.5 -18.5	0.18 (4)	10.00			
J-I	0 / 481	-18.5 -18.5	0.18 (4)	10.00			
I-H	0 / 665	-18.5 -18.5	0.19 (4)	10.00			

TOTAL WEIGHT = 4 X 84 = 338 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.57")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.57")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSI: TC=0.28/1.00 (B-C:1), BC=0.19/1.00 (K-L:4),
WB=0.75/1.00 (C-L:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.77 (H) (INPUT = 0.90)
JSI METAL = 0.25 (C) (INPUT = 1.00)

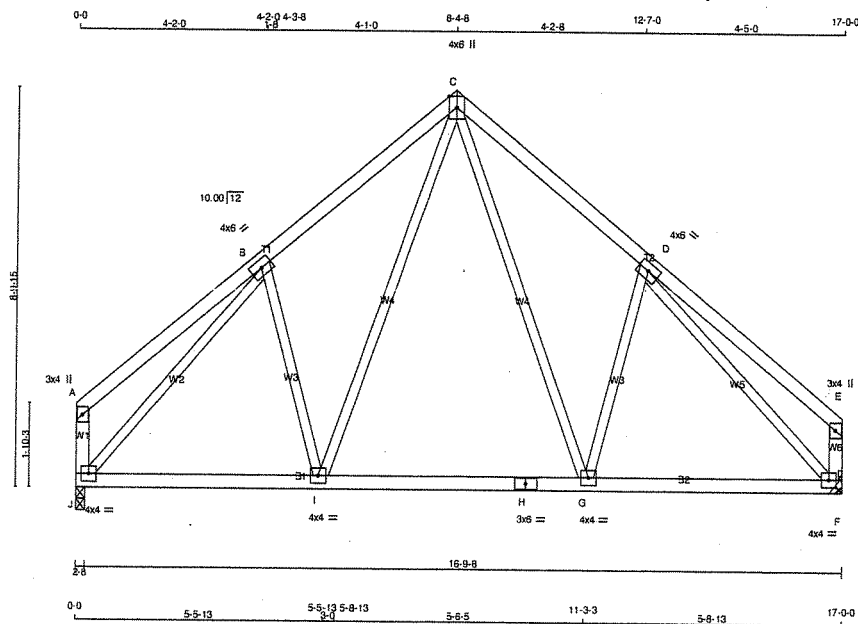


Structural component only
DWG# T-2115463

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411069	T10A	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:E?n9mn1Ky9qx7v78133ulyzsQn-zil9Ue1hJhNOWP28?XIWovf06NAjy2GFdOeHwzD_Ve



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2	
C - E	2x4	DRY	No.2	
J - A	2x4	DRY	No.2	
F - E	2x4	DRY	No.2	
J - H	2x4	DRY	No.2	
H - F	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMW-t	MT20	4.0	6.0		
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMW-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
J	937	0	937	0
F	937	0	937	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
J	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
J	663	435 / 0	0 / 0	0 / 0	228 / 0	0 / 0
F	663	435 / 0	0 / 0	0 / 0	228 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO									
A-B	0 / 30	-91.8	-91.8	0.26 (1)	10.00	C-G	0 / 370	0.08 (1)	
B-C	-772 / 0	-91.8	-91.8	0.21 (1)	6.25	G-D	-241 / 0	0.11 (1)	
C-D	-793 / 0	-91.8	-91.8	0.22 (1)	6.25	J-B	-985 / 0	0.68 (1)	
D-E	0 / 30	-91.8	-91.8	0.28 (1)	10.00	D-F	-992 / 0	0.74 (1)	
J-A	-137 / 0	0.0	0.0	0.02 (1)	7.81	I-C	0 / 323	0.07 (1)	
F-E	-149 / 0	0.0	0.0	0.02 (1)	7.81	B-I	-199 / 0	0.09 (1)	
J-I	0 / 628	-18.5	-18.5	0.18 (4)	10.00				
I-H	0 / 466	-18.5	-18.5	0.18 (4)	10.00				
H-G	0 / 466	-18.5	-18.5	0.18 (4)	10.00				
G-F	0 / 654	-18.5	-18.5	0.19 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.28/1.00 (D-E:1), BC=0.19/1.00 (F-G:4),
WB=0.74/1.00 (D-F:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

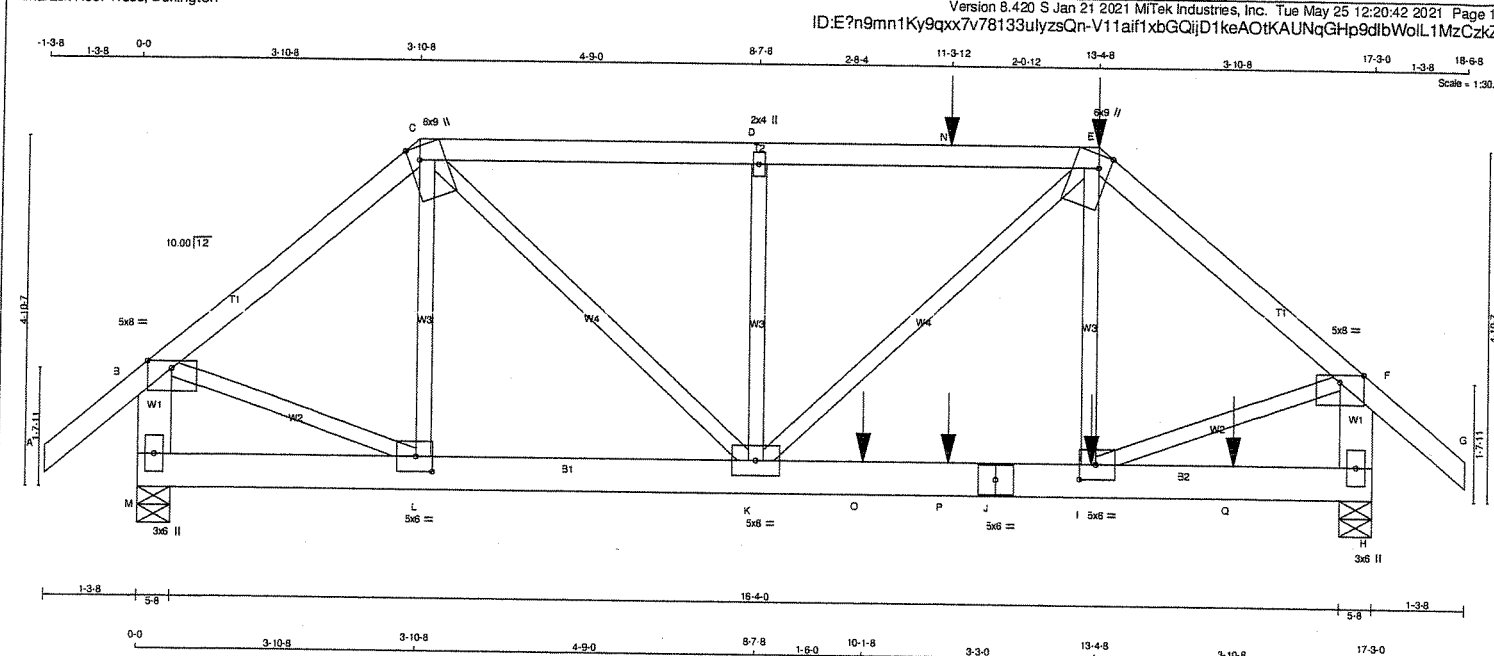
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.24 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115516



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
J - I	2x6	DRY	No.2
M - H	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMVW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	9.0	Edge	1.75
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMVW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	5.0	6.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	1571	1571	5-8	5-8
H VERT	1931	1931	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX / MIN. COMPONENT REACTIONS
JT COMBINED	1106	755 / 0
M	1359	928 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 41	-91.8 -91.8 0.14 (1)	10.00	L-C -333 / 0 0.11 (1)
B-C	-1389 / 0	-91.8 -91.8 0.31 (1)	5.13	C-K 0 / 1280 0.32 (1)
C-D	-1988 / 0	-91.8 -91.8 0.50 (1)	4.16	K-D -568 / 0 0.19 (1)
D-N	-1988 / 0	-91.8 -91.8 0.50 (1)	4.16	K-E 0 / 750 0.19 (1)
N-E	-1988 / 0	-91.8 -91.8 0.50 (1)	4.16	E-I 0 / 105 0.04 (4)
E-F	-1881 / 0	-91.8 -91.8 0.34 (1)	4.52	I-F 0 / 1113 0.28 (1)
F-G	0 / 41	-91.8 -91.8 0.14 (1)	10.00	
M-B	-1524 / 0	0.0 0.0 0.11 (1)	7.81	
H-F	-1956 / 0	0.0 0.0 0.14 (1)	7.16	
M-L	0 / 0	-18.5 -18.5 0.04 (1)	10.00	
L-K	0 / 1056	-18.5 -18.5 0.37 (1)	10.00	
K-O	0 / 1442	-18.5 -18.5 0.72 (1)	10.00	
O-P	0 / 1442	-18.5 -18.5 0.72 (1)	10.00	
P-J	0 / 1442	-18.5 -18.5 0.72 (1)	10.00	
J-I	0 / 1442	-18.5 -18.5 0.72 (1)	10.00	
I-O	0 / 0	-18.5 -18.5 0.21 (1)	10.00	
O-H	0 / 0	-18.5 -18.5 0.21 (1)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LQC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	13-4-8	-174	-174	---	FRONT	VERT	TOTAL	---	C1
I	13-3-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
N	11-3-12	-59	-59	---	FRONT	VERT	TOTAL	---	C1
O	10-1-8	-671	-671	---	FRONT	VERT	TOTAL	---	C1
P	11-3-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
Q	15-3-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TQP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.50/1.00 (D-E:1), BC=0.72/1.00 (I-K:1), WB=0.37/1.00 (F-I:1), SS=0.47/1.00 (H-K:1)

DQL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

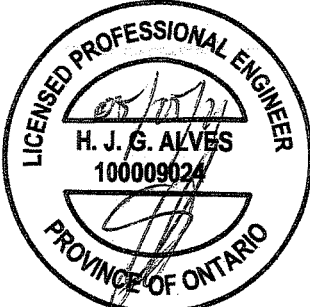
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

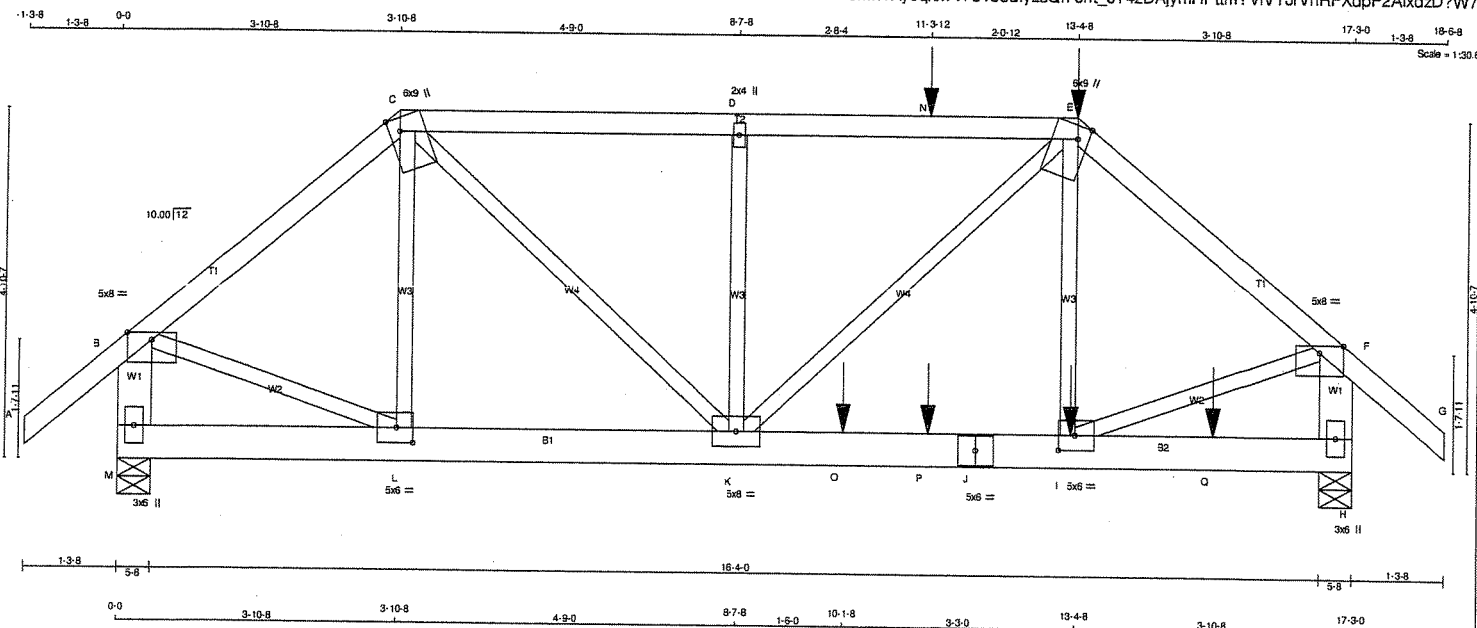
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (L) (INPUT = 0.90)
JSI METAL= 0.40 (J) (INPUT = 1.00)



JOB NAME 406516	TRUSS NAME T11Z	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 10:19:34 2021 Page 1 ID:E?n9mn1Ky9qxx7v78133ulyzsQn-Jnt_JT4zDAjymHPtm?VIVTJrVhRFxdpF2AldzD?W7	



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
M - B	2x6	DRY No.2	SPF
H - F	2x6	DRY No.2	SPF
M - J	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	9.0	Edge	1.75
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMVW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	5.0	6.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
M	1654	0	1654	0	5-8	5-8
H	2029	0	2029	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
M	1164	794 / 0	0 / 0	0 / 0	0 / 0	370 / 0	0 / 0
H	1429	972 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 41	-91.8 -91.8 0.14 (1)	10.00	L-C -379 / 0 0.13 (1)
B-C	-1478 / 0	-91.8 -91.8 0.31 (1)	5.00	C-K 0 / 1428 0.35 (1)
C-D	-2163 / 0	-91.8 -91.8 0.51 (1)	4.00	K-D -566 / 0 0.19 (1)
D-N	-2163 / 0	-91.8 -91.8 0.51 (1)	4.00	K-E 0 / 854 0.21 (1)
N-E	-2163 / 0	-91.8 -91.8 0.51 (1)	4.00	E-F 0 / 121 0.05 (4)
E-F	-2009 / 0	-91.8 -91.8 0.35 (1)	4.38	B-L 0 / 1184 0.29 (1)
F-G	0 / 41	-91.8 -91.8 0.14 (1)	10.00	I-F 0 / 1610 0.40 (1)
M-B	-1603 / 0	0.0 0.0 0.12 (1)	7.73	
H-F	-2069 / 0	0.0 0.0 0.15 (1)	7.01	
M-L	0 / 0	-18.5 -18.5 0.05 (1)	10.00	
L-K	0 / 1124	-18.5 -18.5 0.43 (1)	10.00	
K-O	0 / 1542	-18.5 -18.5 0.85 (1)	10.00	
O-P	0 / 1542	-18.5 -18.5 0.85 (1)	10.00	
P-J	0 / 1542	-18.5 -18.5 0.85 (1)	10.00	
J-I	0 / 1542	-18.5 -18.5 0.85 (1)	10.00	
I-O	0 / 0	-18.5 -18.5 0.25 (1)	10.00	
O-H	0 / 0	-18.5 -18.5 0.25 (1)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	13-4-8	-145	-145	---	FRONT	VERT	TOTAL	---	C1
I	13-3-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
N	11-3-12	-59	-59	---	FRONT	VERT	TOTAL	---	C1
O	10-1-8	-828	-828	---	FRONT	VERT	TOTAL	---	C1
P	11-3-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
O	15-3-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.51/1.00 (D-E:1), BC=0.85/1.00 (I-K:1), WB=0.40/1.00 (F+1), SS=0.57/1.00 (H-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

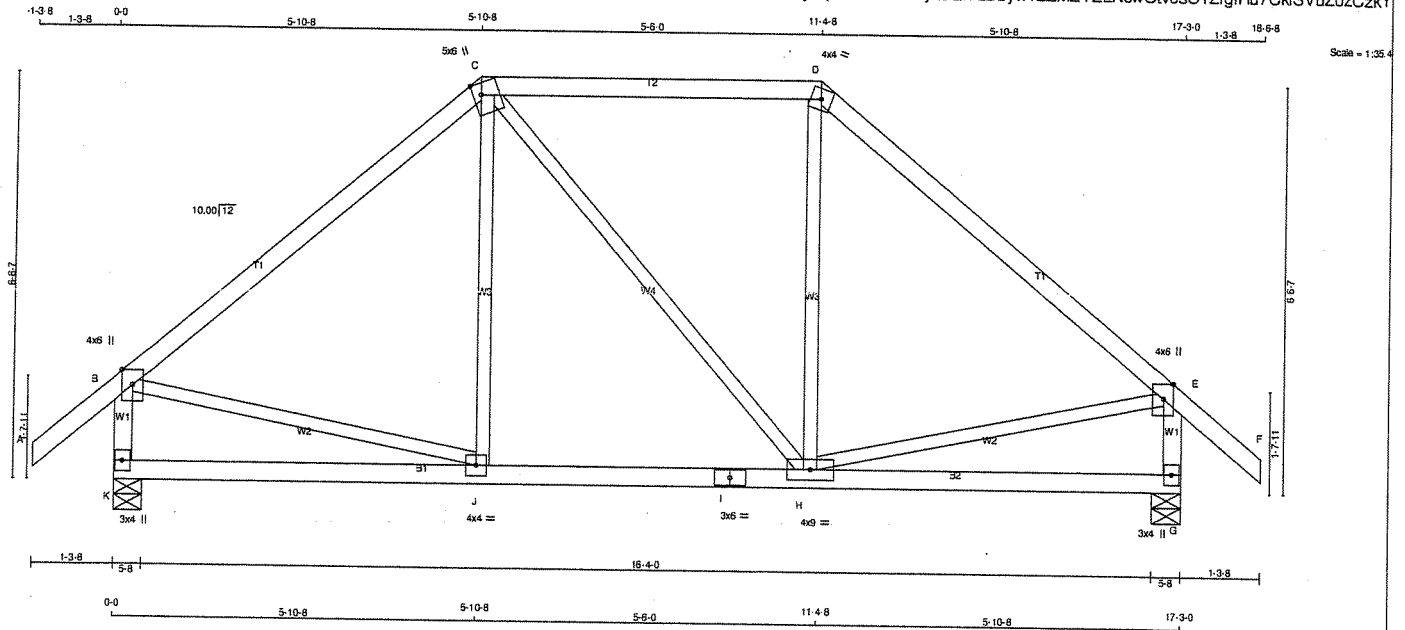
JSI GRIP= 0.81 (L) (INPUT = 0.90)
JSI METAL= 0.45 (J) (INPUT = 1.00)



JOB NAME 406403	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:E?n9mn1Ky9qxx7v78133ulyzsQn-zDbyw?2ZMZYLNCwCtv6sO1ZrgiHu7CKiSVuZozCzKv



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
K - B	2x4 DRY	No.2	SPF		
G - E	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
K	1078	0	1078	0	5-8	5-8
G	1078	0	1078	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	760	512 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
G	760	512 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	J-C	-19 / 80	0.03 (4)	
B-C	-785 / 0	-91.8	-91.8	0.42 (1)	6.22	C-H	0 / 2	0.00 (4)	
C-D	-602 / 0	-91.8	-91.8	0.36 (1)	6.25	H-D	-17 / 82	0.03 (4)	
D-E	-788 / 0	-91.8	-91.8	0.42 (1)	6.21	B-J	0 / 616	0.14 (1)	
E-F	0 / 41	-91.8	-91.8	0.13 (1)	10.00	H-E	0 / 617	0.14 (1)	
K-B	-1032 / 0	0.0	0.0	0.11 (1)	7.72				
G-E	-1031 / 0	0.0	0.0	0.11 (1)	7.72				
K-J	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
J-I	0 / 602	-18.5	-18.5	0.19 (4)	10.00				
I-H	0 / 602	-18.5	-18.5	0.19 (4)	10.00				
H-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00				

TOTAL WEIGHT = 76 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.42/1.00 (D-E:1), BC=0.19/1.00 (H-J:4),
WB=0.14/1.00 (E-H:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.67 (J) (INPUT = 0.90)
JSI METAL = 0.43 (E) (INPUT = 1.00)

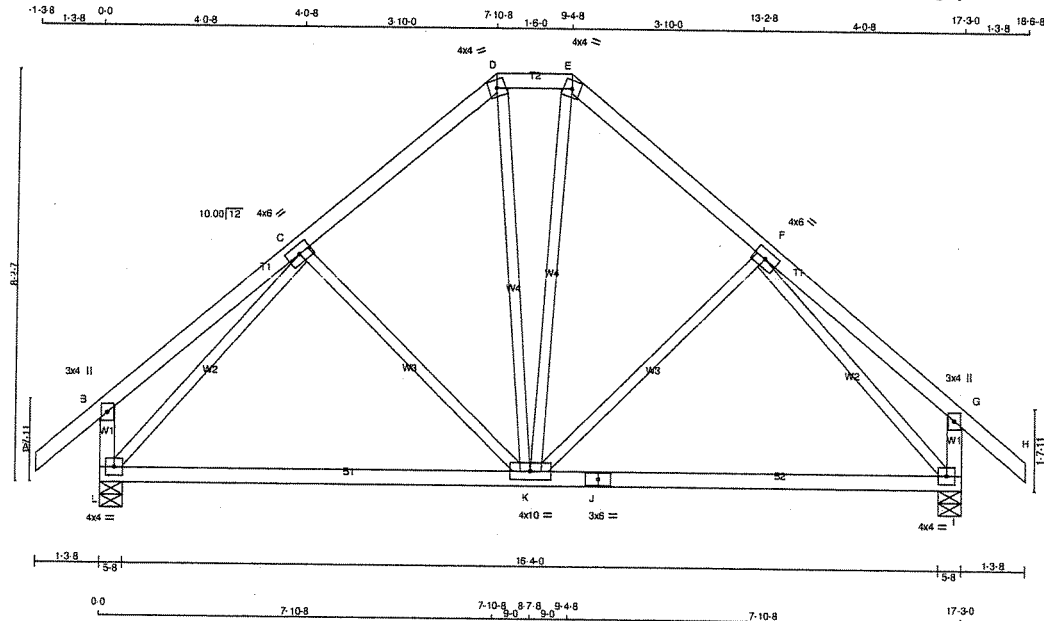


Structural component only
DWG# T-2115465

JOB NAME 406403	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/4" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	10.0		
L	BMVW-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
L	1078	0	0	5-8
I	1078	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	760	512 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
I	760	512 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	C-K	-180 / 0	0.14 (1)	
B-C	0 / 27	-91.8	-91.8	0.23 (1)	10.00	K-F	-180 / 0	0.14 (1)	
C-D	-701 / 0	-91.8	-91.8	0.18 (1)	6.25	L-C	-992 / 0	0.61 (1)	
D-E	-538 / 0	-91.8	-91.8	0.03 (1)	6.25	F-I	-992 / 0	0.61 (1)	
E-F	-701 / 0	-91.8	-91.8	0.18 (1)	6.25	D-K	0 / 225	0.05 (1)	
F-G	0 / 27	-91.8	-91.8	0.23 (1)	10.00	K-E	0 / 225	0.05 (1)	
G-H	0 / 41	-91.8	-91.8	0.13 (1)	10.00				
L-B	-264 / 0	0.0	0.0	0.03 (1)	7.81				
I-G	-264 / 0	0.0	0.0	0.03 (1)	7.81				
L-K	0 / 646	-18.5	-18.5	0.45 (4)	10.00				
K-J	0 / 646	-18.5	-18.5	0.45 (4)	10.00				
J-I	0 / 646	-18.5	-18.5	0.45 (4)	10.00				

TOTAL WEIGHT = 84 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TO=0.23/1.00 (B-C-1), BC=0.45/1.00 (I-K-4),
WB=0.61/1.00 (C-L-1), SS=0.13/1.00 (K-L-4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (L) (INPUT = 0.90)
JSI METAL= 0.33 (J) (INPUT = 1.00)

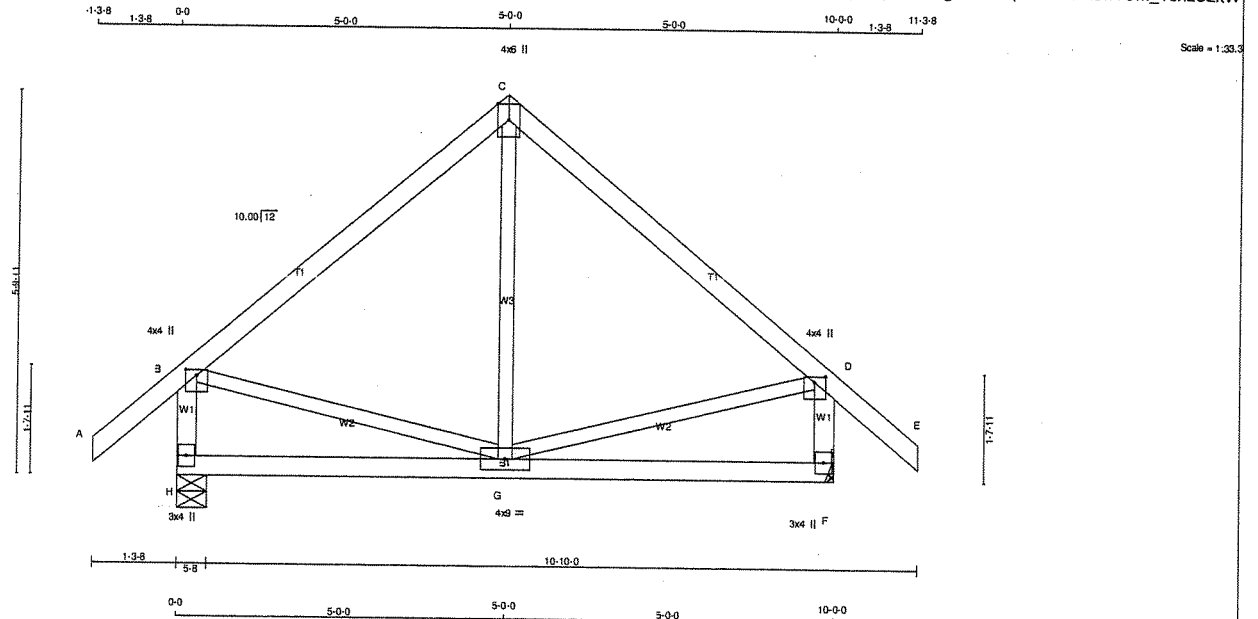


Structural component only
DWG# T-2115466

JOB NAME 406403	TRUSS NAME T14	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
H	678	0	678	0
F	678	0	678	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	477	327	0	0	0	0	151	0
F	477	327	0	0	0	0	151	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	G-C	-23 / 78
B-C	-341 / 0	-91.8	-91.8	0.29 (1)	6.25	B-G	0 / 270
C-D	-341 / 0	-91.8	-91.8	0.29 (1)	6.25	G-D	0 / 270
D-E	0 / 41	-91.8	-91.8	0.13 (1)	10.00		
H-B	-643 / 0	0.0	0.0	0.07 (1)	7.81		
F-D	-643 / 0	0.0	0.0	0.07 (1)	7.81		
H-G	0 / 0	-18.5	-18.5	0.13 (4)	10.00		
G-F	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

TOTAL WEIGHT = 2 X 45 = 91 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.29/1.00 (B-C:1), BC=0.13/1.00 (F-G:4), WB=0.06/1.00 (B-G:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

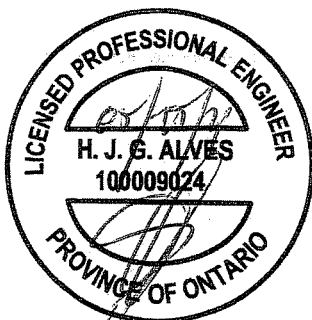
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (D) (INPUT= 0.90)
JSI METAL= 0.14 (D) (INPUT= 1.00)

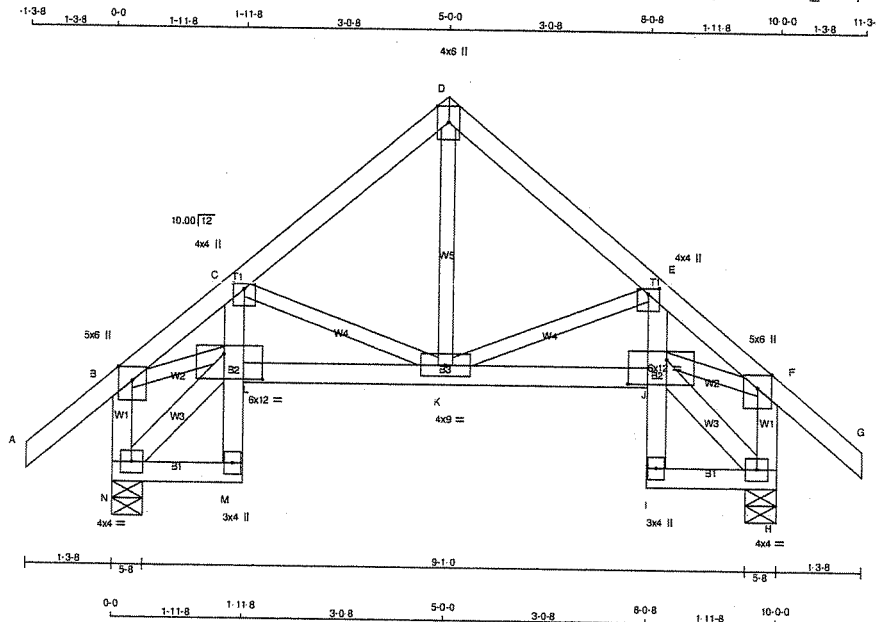


Structural component only
DWG# T-2115467

JOB NAME 411069	TRUSS NAME T14S	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Tue May 25 11:28:22 2021 Page 1
ID:E?n9mn1Ky9qxx7v78133u1yzsQn-RxIXh_1J4VpE0g_Fij3X30Rt7WkDSYNPUH7BpMzD_Vd



TOTAL WEIGHT = 2 X 55 = 109 lb
[M][F]

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x4	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
N - L	2x4	DRY	No.2
J - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN X Y
B	TMVW+p	MT20	5.0	6.0 Edge
C	TMVW+p	MT20	4.0	4.0 1.00 2.00
D	TTW+p	MT20	4.0	6.0 Edge
E	TMVW+p	MT20	4.0	4.0 1.00 2.00
F	TMVW+p	MT20	5.0	6.0 Edge
H	BMVW-1	MT20	4.0	4.0
I	BMV+p	MT20	3.0	4.0
J	BMVW-1	MT20	6.0	12.0 4.50 7.00
K	BMVW-1	MT20	4.0	9.0
L	BMVW-1	MT20	6.0	12.0 4.50 7.00
M	BMV+p	MT20	3.0	4.0
N	BMVW-1	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS	REACTION	GROSS	REACTION	BRG	IN-SX	BRG	IN-SX
N	678	0	678	0	0	5-8	5-8	5-8	5-8
H	678	0	678	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
N	477	327 / 0	0 / 0	0 / 0	0 / 0
H	477	327 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	LC1 MAX. (PL)	UNBRAC. LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO		FR-TO				
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	K-D	0 / 286	0.08 (1)	
B-C	-889 / 0	-91.8	-91.8	0.10 (1)	6.25	K-E	-404 / 0	0.09 (1)	
C-D	-439 / 0	-91.8	-91.8	0.12 (1)	6.25	C-K	-404 / 0	0.09 (1)	
D-E	-439 / 0	-91.8	-91.8	0.12 (1)	6.25	N-L	-29 / 0	0.00 (1)	
E-F	-889 / 0	-91.8	-91.8	0.10 (1)	6.25	B-L	0 / 688	0.15 (1)	
F-G	0 / 41	-91.8	-91.8	0.13 (1)	10.00	J-H	-29 / 0	0.00 (1)	
N-B	-642 / 0	0.0	0.0	0.07 (1)	7.81	J-F	0 / 688	0.15 (1)	
H-F	-642 / 0	0.0	0.0	0.07 (1)	7.81				
N-M	0 / 21	-18.5	-18.5	0.02 (4)	10.00				
M-L	0 / 19	0.0	0.0	0.05 (1)	10.00				
L-C	0 / 219	0.0	0.0	0.08 (1)	10.00				
L-K	0 / 705	-18.5	-18.5	0.14 (1)	10.00				
K-J	0 / 705	-18.5	-18.5	0.14 (1)	10.00				
I-J	0 / 19	0.0	0.0	0.05 (1)	10.00				
J-E	0 / 219	0.0	0.0	0.08 (1)	10.00				
I-H	0 / 21	-18.5	-18.5	0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C.C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.13/1.00 (F-G:1), BC=0.14/1.00 (K-L:1), WB=0.15/1.00 (B-L:1), SS=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

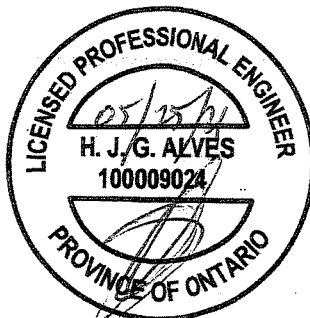
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (B) (INPUT = 0.90)
JSI METAL= 0.34 (F) (INPUT = 1.00)

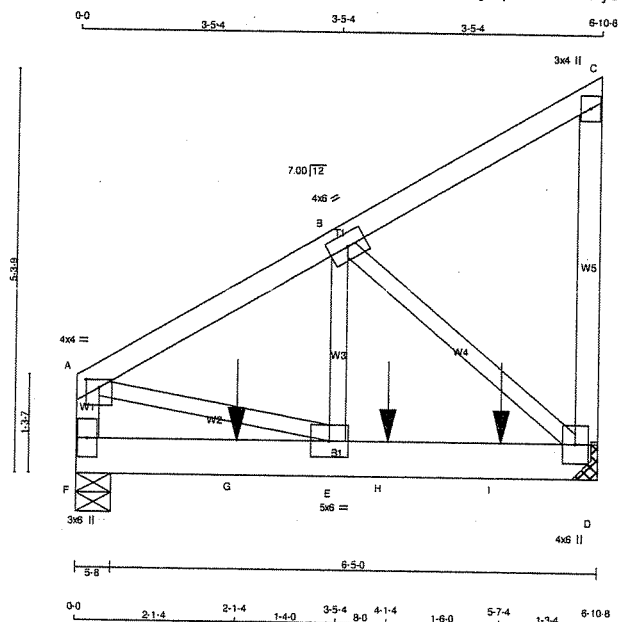


Structural component only
DWG# T-2115517

JOB NAME 406403	TRUSS NAME T15	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 12:20:46 2021 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4 DRY	No.2	SPF		
A - C	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.25	2.00
B	TMVW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD	
	VERT	HORZ	DOWN	HORZ	BRG	BRG
F	1737	0	1737	0	5-8	5-8
D	2199	0	2199	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LOASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1223	832 / 0	0 / 0	0 / 0	0 / 0	391 / 0	0 / 0
D	1548	1056 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
F-A	-1502 / 0	0.0	0.0	0.08 (1)	7.81	A-E	0 / 1687
A-B	-1875 / 0	-91.8	-91.8	0.09 (1)	6.25	E-B	0 / 1951
B-C	-15 / 0	-91.8	-91.8	0.08 (1)	6.25	B-D	-2135 / 0
D-C	-127 / 0	0.0	0.0	0.03 (1)	7.81		
F-G	0 / 0	-18.5	-18.5	0.26 (1)	10.00		
G-E	0 / 0	-18.5	-18.5	0.26 (1)	10.00		
E-H	0 / 1633	-18.5	-18.5	0.43 (1)	10.00		
H-I	0 / 1633	-18.5	-18.5	0.43 (1)	10.00		
I-D	0 / 1633	-18.5	-18.5	0.43 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-1-4	-745	-745	---	BACK	VERT	TOTAL	---	C1
H	4-1-4	-745	-745	---	BACK	VERT	TOTAL	---	C1
I	5-7-4	-745	-745	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 35 = 71 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BOBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.09/1.00 (A-B:1), BC=0.43/1.00 (D-E:1),
WB=0.34/1.00 (B-D:1), SS=0.49/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	(PSI)	(PLI)	SHEAR	SECTION
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (B) (INPUT = 0.90)
JSI METAL = 0.37 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115469

JOB NAME 406403	TRUSS NAME T15	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 12:20:46 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

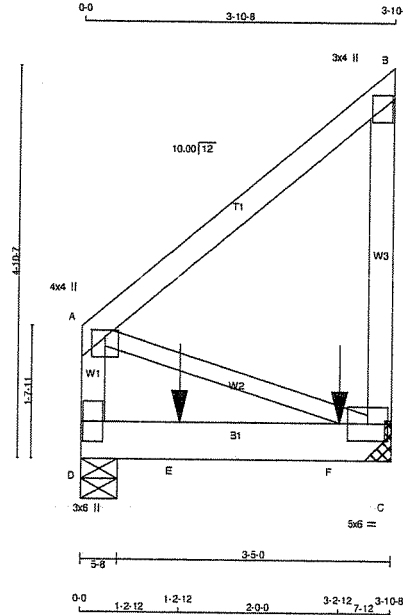


Structural component only
DWG# T-2115469 2/2

JOB NAME 406403	TRUSS NAME T16	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Tue May 25 12:20:47 2021 Page 1
ID:E?n9mn1Ky9qxx7v78133ulyzsQn-s_rTmM53Qo2?q_whRi_20ECJPH55qzMKg4T6iZzCzkU



LUMBER				
CHORDS	SIZE	LUMBER	DESCR.	
D - A	2x4 DRY	No.2	SPF	
A - B	2x4 DRY	No.2	SPF	
C - B	2x4 DRY	No.2	SPF	
D - C	2x6 DRY	No.2	SPF	
ALL WEBS	2x3 DRY	No.2	SPF	
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 1	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
D	775	0	775	0	0	5-8	5-8	
C	975	0	975	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	UVE	PERM LIVE	WIND	DEAD	SOIL
D	544	377 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0
C	685	475 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		W E B S MAX. FACTORED FORCE (LBS)	
	FR-TO	TO	FR-TO	TO	FR-TO	TO
D-A	-178 / 0	0.0	0.0	0.0	0.0	0.0
A-B	0 / 0	-91.8	-91.8	0.13 (1)	10.00	7.81
C-B	-178 / 0	0.0	0.0	0.03 (1)	7.81	
D-E	0 / 0	-18.5	-18.5	0.26 (1)	10.00	
E-F	0 / 0	-18.5	-18.5	0.26 (1)	10.00	
F-C	0 / 0	-18.5	-18.5	0.26 (1)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-2-12	-463	-463	---	BACK	VERT	TOTAL	---	C1
F	3-2-12	-465	-465	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 22 = 43 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.13/1.00 (A-B:1), BC=0.26/1.00 (C-D:1),
WB=0.00/1.00 (A-C:1), SS=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.07 (A) (INPUT = 0.90)
JSI METAL = 0.03 (B) (INPUT = 1.00)

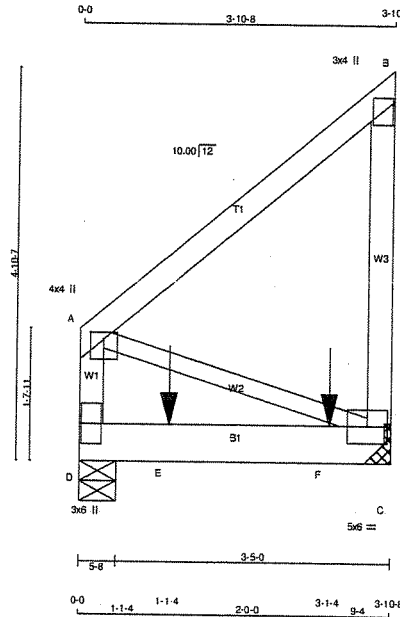


Structural component only
DWG# T-2115470

JOB NAME 406516	TRUSS NAME T16Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:27.3

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2' TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 1	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TMV+p	MT20	3.0	4.0	
C	BMVW1-t	MT20	5.0	6.0	
D	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
D	1487	0	1487	0	0	5-8	5-8	
C	1199	0	1199	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

1ST LOASE	COMBINED	SNOW	UVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1046	717 / 0	0 / 0	0 / 0	0 / 0	329 / 0	0 / 0
D	843	581 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
C							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		
D-A	-178 / 0	0.0 0.0 0.01 (1)	7.81	0.00 (1)
A-B	0 / 0	-91.8 -91.8 0.13 (1)	10.00	
C-B	-178 / 0	0.0 0.0 0.03 (1)	7.81	
D-E	0 / 0	-18.5 -18.5 0.53 (1)	10.00	
E-F	0 / 0	-18.5 -18.5 0.53 (1)	10.00	
F-C	0 / 0	-18.5 -18.5 0.53 (1)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIRL	TYPE	HEEL	CONN.
E	1-1-4	-1123	-1123	---	BACK	VERT	TOTAL	---	C1
F	3-1-4	-464	-464	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 22 = 43 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 898 (0.05")

CSI: TC=0.13/1.00 (A-B:1), BC=0.53/1.00 (C-D:1),
WB=0.00/1.00 (A-C:1), SSI=0.43/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

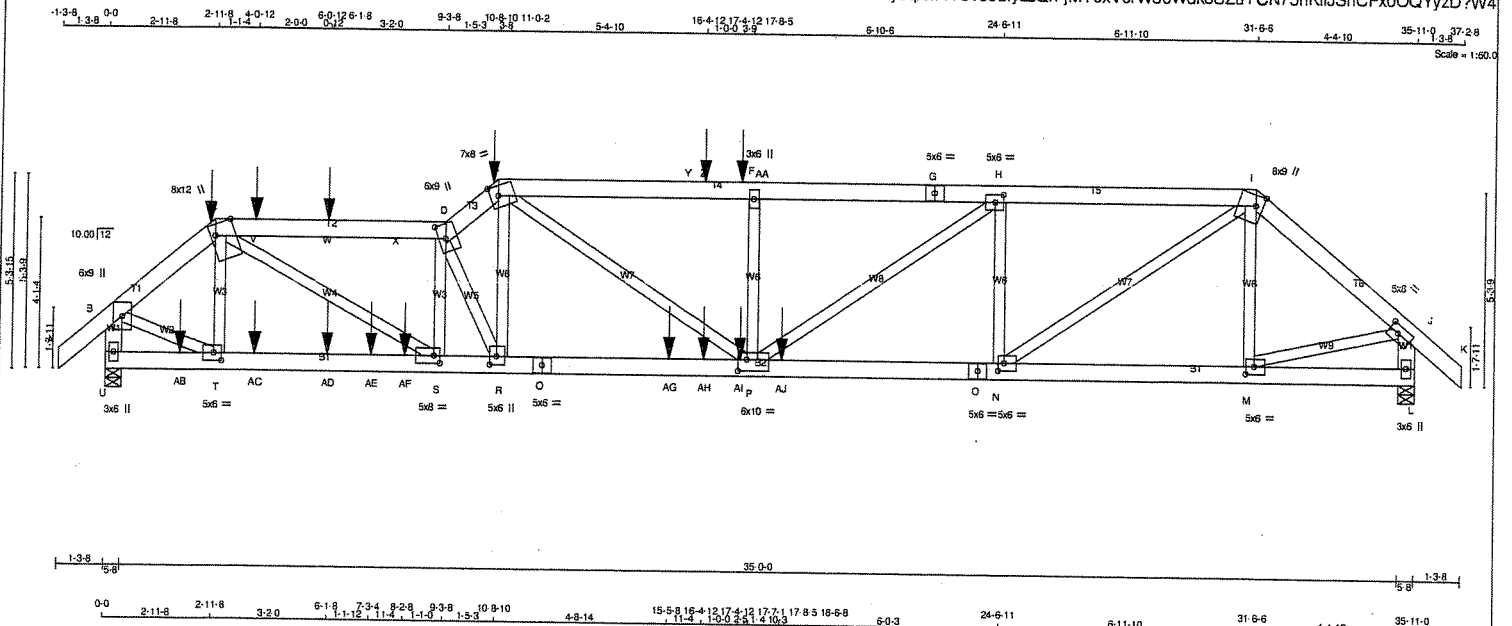
JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115474

JOB NAME 406516	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY	No.2	SPF		
C - D	2x6 DRY	No.2	SPF		
D - E	2x6 DRY	No.2	SPF		
E - G	2x6 DRY	No.2	SPF		
G - I	2x6 DRY	No.2	SPF		
I - K	2x6 DRY	No.2	SPF		
U - B	2x6 DRY	No.2	SPF		
L - J	2x6 DRY	No.2	SPF		
U - O	2x6 DRY	1650F 1.5E	SPF		
Q - O	2x6 DRY	1650F 1.5E	SPF		
O - L	2x6 DRY	1650F 1.5E	SPF		
ALL WEBS EXCEPT	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	SIDE(122.0)
C - D 2	12	SIDE(61.0)
D - E 2	12	SIDE(96.2)
E - G 2	12	SIDE(183.1)
G - I 2	12	TOP
I - K 2	12	TOP
U - B 2	12	TOP
L - J 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U - O 2	12	SIDE(0.0)
O - O 2	12	SIDE(183.1)
O - L 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	SIDE(53.9)
R - E 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	6032	6032	0	5-8
U VERT	0	0	0	5-8
L VERT	4529	4529	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT COMBINED	2895 / 0	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
U	2895 / 0	0 / 0	0 / 0	0 / 0	1351 / 0	0 / 0	
L	3191	2164 / 0	0 / 0	0 / 0	1027 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 42	-91.8	-91.8	0.04 (1)	10.00	T-C	-1211 / 0	0.10 (1)	
B-C	-6399 / 0	-91.8	-91.8	0.09 (1)	4.66	C-S	0 / 8972	0.79 (1)	
C-V	-12588 / 0	-91.8	-91.8	0.56 (1)	3.02	S-D	-3947 / 0	0.34 (1)	
V-W	-12588 / 0	-91.8	-91.8	0.56 (1)	3.02	D-R	-5870 / 0	0.54 (1)	
W-X	-12588 / 0	-91.8	-91.8	0.56 (1)	3.02	R-E	0 / 5865	0.52 (1)	
X-D	-12588 / 0	-213.5	-213.5	0.56 (1)	3.02	M-I	-834 / 0	0.11 (1)	
D-E	-13535 / 0	-213.5	-213.5	0.24 (1)	3.20	B-T	0 / 5193	0.46 (1)	
E-Y	-12754 / 0	-213.5	-213.5	0.75 (1)	2.82	M-J	0 / 3949	0.35 (1)	
Y-Z	-12754 / 0	-91.8	-91.8	0.75 (1)	2.82	N-I	0 / 6247	0.55 (1)	
Z-AA	-12754 / 0	-91.8	-91.8	0.75 (1)	2.82	E-P	0 / 2641	0.23 (1)	
AA-F	-12754 / 0	-91.8	-91.8	0.75 (1)	2.82	N-H	-3275 / 0	0.44 (1)	
F-G	-12755 / 0	-91.8	-91.8	0.57 (1)	3.05	P-F	-1215 / 0	0.16 (1)	
G-H	-12755 / 0	-91.8	-91.8	0.57 (1)	3.05	P-H	0 / 4657	0.41 (1)	
H-I	-8963 / 0	-91.8	-91.8	0.33 (1)	3.85				
I-J	-5012 / 0	-91.8	-91.8	0.12 (1)	5.11				
J-K	0 / 42	-91.8	-91.8	0.04 (1)	10.00				
U-B	-6152 / 0	0.0	0.0	0.22 (1)	5.97				
L-J	-4501 / 0	0.0	0.0	0.16 (1)	6.78				

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
U-AB	0 / 0	-18.5	-18.5	0.12 (1)	10.00				
AB-T	0 / 0	-18.5	-18.5	0.12 (1)	10.00				
T-AC	0 / 4839	-18.5	-18.5	0.40 (1)	10.00				
AC-AD	0 / 4839	-18.5	-18.5	0.40 (1)	10.00				
AD-AE	0 / 4839	-18.5	-18.5	0.40 (1)	10.00				
AE-AF	0 / 4839	-18.5	-18.5	0.40 (1)	10.00				
AF-S	0 / 4839	-18.5	-18.5	0.40 (1)	10.00				
S-R	0 / 12747	-18.5	-18.5	0.59 (1)	10.00				
R-O	0 / 10576	-18.5	-18.5	0.71 (1)	10.00				
Q-AG	0 / 10576	-18.5	-18.5	0.71 (1)	10.00				
AG-AH	0 / 10576	-18.5	-18.5	0.71 (1)	10.00				
AH-AI	0 / 10576	-18.5	-18.5	0.71 (1)	10.00				
AI-P	0 / 10576	-18.5	-18.5	0.71 (1)	10.00				
P-AJ	0 / 8963	-18.5	-18.5	0.64 (1)	10.00				
AJ-O	0 / 8963	-18.5	-18.5	0.64 (1)	10.00				
O-N	0 / 8963	-18.5	-18.5	0.64 (1)	10.00				
N-M	0 / 3810	-18.5	-18.5	0.19 (1)	10.00				
M-L	0 / 0	-18.5	-18.5	0.03 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
E	10-8-10	-286	-286	---	FRONT	VERT	SNOW	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder

START DISTANCE = 8-0-0
START SPAN CARRIED = 6-10-8
END DISTANCE = 16-0-0
END SPAN CARRIED = 6-10-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 096-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.29")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/810 (0.53")

CSI: TC=0.75/1.00 (E-F-1), BC=0.71/1.00 (P-R-1),
WB=0.79/1.00 (C-S-1), SSI=0.70/1.00 (N-P-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

CONTINUED ON PAGE 2



Structural component only
DWG# T-2115475 112

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
406516	T20	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0		
C	TTWW+m	MT20	8.0	12.0	Edge	6.50
D	TTWW+m	MT20	6.0	9.0	4.75	2.25
E	TTWW-m	MT20	7.0	8.0	3.25	2.75
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0	2.50	2.75
I	TTWW+m	MT20	8.0	9.0	3.50	2.25
J	TMVW-t	MT20	5.0	8.0	2.50	3.25
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	6.0	10.0	3.75	3.00
O	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.75	2.25
S	BMWW-t	MT20	5.0	8.0	2.50	2.00
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	4-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
W	6-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Z	16-4-12	-100	-100	---	BACK	VERT	TOTAL	---	C1
AA	17-4-12	-100	-100	---	BACK	VERT	TOTAL	---	C1
AB	2-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	4-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	6-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	7-3-4	-27	-27	---	BACK	VERT	TOTAL	---	C1
AF	8-2-8	-647	-647	---	BACK	VERT	TOTAL	---	C1
AG	15-5-8	-647	-647	---	BACK	VERT	TOTAL	---	C1
AH	16-4-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
AI	17-4-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
AJ	18-6-8	-1534	-1534	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

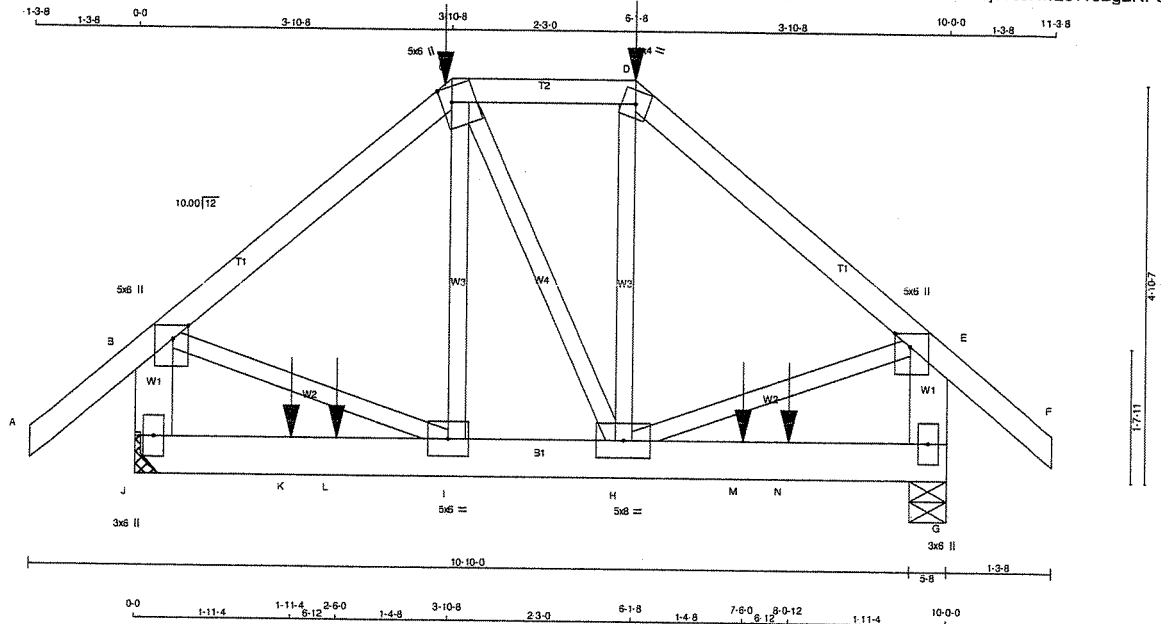
JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.94 (E) (INPUT = 1.00)



Structural component only
DWG# T-2115475

JOB NAME 406516	TRUSS NAME T21	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 57 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x6	DRY	No.2	SPF	
G - E	2x6	DRY	No.2	SPF	
J - G	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	5.0	6.0	2.00	2.25
G	BMV1+p	MT20	3.0	6.0		
H	BMVWW-t	MT20	5.0	8.0		
I	BMVWW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	1615	0	1615	0
G	1615	0	1615	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	1137	773 / 0	0 / 0	0 / 0	0 / 0	365 / 0	0 / 0
G	1137	773 / 0	0 / 0	0 / 0	0 / 0	365 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED LC1 MAX. (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (LBS)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 41	-91.8	-91.8	0.14 (1)	10.00	I-C	0 / 193
B-C	-1158 / 0	-140.0	-140.0	0.44 (1)	5.28	C-H	0 / 8
C-D	-896 / 0	-140.0	-140.0	0.14 (1)	6.25	H-D	0 / 203
D-E	-1163 / 0	-140.0	-140.0	0.44 (1)	5.27	B-I	0 / 928
E-F	0 / 41	-91.8	-91.8	0.14 (1)	10.00	H-E	0 / 932
J-B	-1415 / 0	0.0	0.0	0.10 (1)	7.81		
G-E	-1419 / 0	0.0	0.0	0.10 (1)	7.81		
J-K	0 / 0	-18.5	-18.5	0.32 (1)	10.00		
K-L	0 / 0	-18.5	-18.5	0.32 (1)	10.00		
L-I	0 / 0	-18.5	-18.5	0.32 (1)	10.00		
I-H	0 / 892	-18.5	-18.5	0.29 (1)	10.00		
H-M	0 / 0	-18.5	-18.5	0.31 (1)	10.00		
M-N	0 / 0	-18.5	-18.5	0.31 (1)	10.00		
N-G	0 / 0	-18.5	-18.5	0.31 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-80	-80		BACK	VERT	TOTAL		C1
D	6-1-8	-80	-80		BACK	VERT	TOTAL		C1
K	1-11-4	-14	-14		BACK	VERT	TOTAL		C1
L	2-6-0	-396	-396		BACK	VERT	TOTAL		C1
M	7-6-0	-396	-396		BACK	VERT	TOTAL		C1
N	8-0-12	-14	-14		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStd Girder

START DISTANCE = 0-0

START SPAN CARRIED = 3-10-8

END DISTANCE = 10-0-0

END SPAN CARRIED = 3-10-8

END WALL WIDTH = 1-8

APPLIED TO FRONT SIDE OF TOP CHORD.

- ADDTL LOADS BASED ON 55% OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")

CALCULATED VERT. DEFL.(LL) = L/999 (0.02")

ALLOWABLE DEFL.(TL) = L/360 (0.33")

CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.44/1.00 (D-E:1), BC=0.32/1.00 (I-J:1), WB=0.23/1.00 (E-H:1), SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

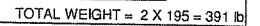
JSI GRIP=0.68 (E) (INPUT = 0.90)

JSI METAL=0.23 (E) (INPUT = 1.00)



Structural component only
DWG# T-2115476

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CONTINUED ON PAGE 2

JOB NAME 418992	TRUSS NAME T30	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TMVW-t	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge	6.50
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	5.0	6.0	2.50	2.25
N	BS-t	MT20	5.0	6.0		
O	BMVW-w	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.25
R	BMVW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
M-AS	0 / 3393	-18.5	-18.5	0.29 (1)	10.00		
AS-AT	0 / 3393	-18.5	-18.5	0.29 (1)	10.00		
AT-AU	0 / 3393	-18.5	-18.5	0.29 (1)	10.00		
AU-L	0 / 3393	-18.5	-18.5	0.29 (1)	10.00		
L-AV	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
AV-AW	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
AW-K	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-1-11	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	3-1-11	-127	-127	---	FRONT	VERT	SNOW	---	C1
E	17-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
H	32-9-5	-30	-30	---	FRONT	VERT	DEAD	---	C1
H	32-9-5	-82	-82	---	BACK	VERT	TOTAL	---	C1
H	32-9-5	-127	-127	---	FRONT	VERT	SNOW	---	C1
L	32-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
O	17-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
P	11-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
T	3-11-4	-82	-82	---	BACK	VERT	TOTAL	---	C1
U	5-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
V	7-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
W	9-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
X	11-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
Y	13-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
Z	15-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AA	18-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AB	20-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AC	22-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AD	24-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AE	26-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AF	28-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AG	30-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AH	1-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AI	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AJ	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AK	7-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AL	9-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AM	13-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AN	15-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AO	18-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AP	20-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AQ	22-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AR	24-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AS	26-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AT	28-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AU	30-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AV	33-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AW	34-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

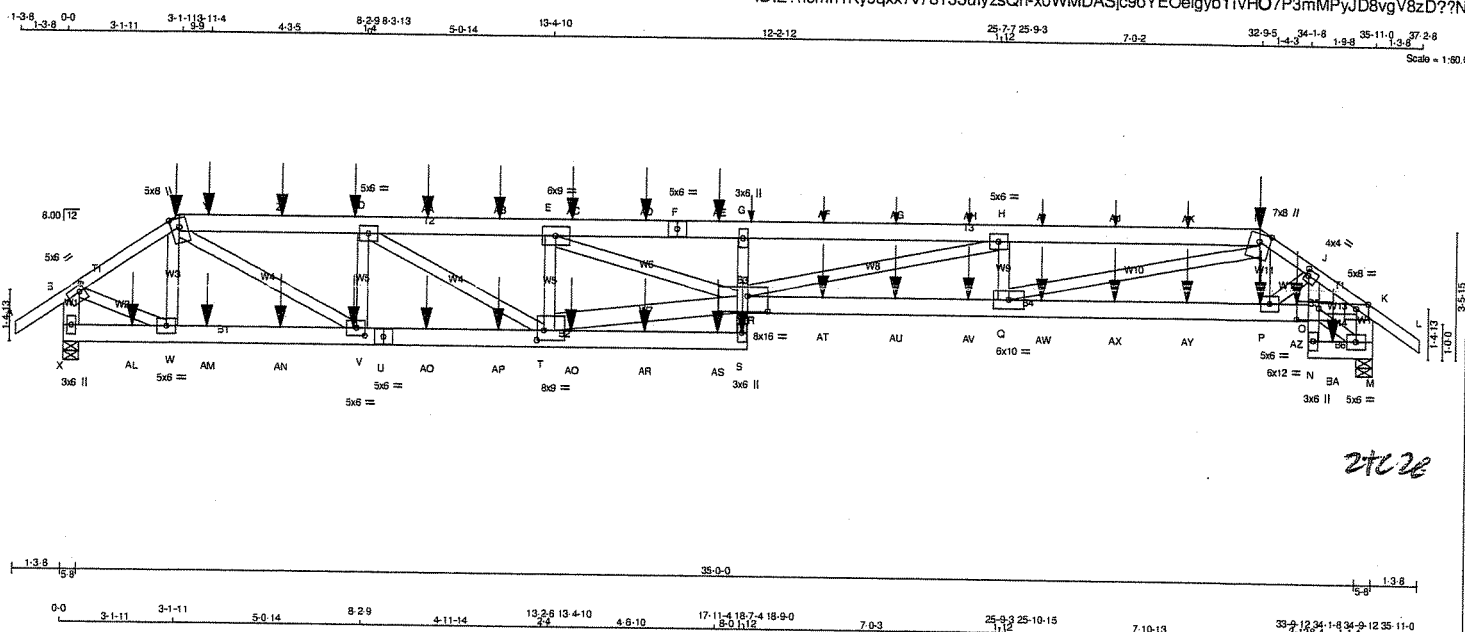
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2115541 302

Tamarack Roof Truss, Burlington

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27C-2E

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
I-L	1	12	SIDE(61.0)
C-F	2	12	SIDE(183.)
F-I	2	12	SIDE(61.0)
X-B	2	12	TDP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
X-U	2	12	SIDE(183.)
U-S	2	12	SIDE(122.)
R-O	2	12	SIDE(183.)
N-M	2	12	SIDE(0.0)
S-G	1	12	TOP
J-N	1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	
NAILS TO BE DRIVEN FROM ONE SIDE ONLY			

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED			MAXIMUM FACTORED			INPUT	REORD
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
X	3686	0	3686	0	0	5-8	5-8
M	4323	0	4323	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	2606	1715 / 0	0 / 0	0 / 0	0 / 0	891 / 0	0 / 0
M	3058	2001 / 0	0 / 0	0 / 0	0 / 0	1057 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT DR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 35	-91.8	-91.8 0.07 (1)	10.00	W-C	-966 / 0	0.07 (1)
B-C	-4112 / 0	-91.8	-91.8 0.15 (1)	4.58	P-I	0 / 1027	0.09 (1)
C-Y	-7677 / 0	-91.8	-91.8 0.24 (1)	4.19	P-J	0 / 149	0.01 (1)
Y-Z	-7677 / 0	-91.8	-91.8 0.24 (1)	4.19	B-W	0 / 3620	0.32 (1)
Z-D	-7677 / 0	-91.8	-91.8 0.24 (1)	4.19	O-M	-303 / 0	0.02 (1)
D-AA	-10350 / 0	-91.8	-91.8 0.33 (1)	3.81	O-K	0 / 5943	0.53 (1)
AA-AB	-10350 / 0	-91.8	-91.8 0.33 (1)	3.81	C-V	0 / 4913	0.43 (1)
AB-E	-10350 / 0	-91.8	-91.8 0.33 (1)	3.61	T-E	-3281 / 0	0.23 (1)
E-AC	-16869 / 0	-91.8	-91.8 0.70 (1)	2.56	V-D	-2434 / 0	0.17 (1)
AC-AD	-16869 / 0	-91.8	-91.8 0.70 (1)	2.56	D-T	0 / 3123	0.28 (1)
AD-F	-16869 / 0	-91.8	-91.8 0.70 (1)	2.56	T-R	0 / 10106	0.66 (1)
F-AE	-16869 / 0	-91.8	-91.8 0.70 (1)	2.56	E-R	0 / 6596	0.58 (1)
AE-G	-16869 / 0	-91.8	-91.8 0.70 (1)	2.56	O-I	0 / 8532	0.75 (1)
G-AF	-16954 / 0	-91.8	-91.8 0.88 (1)	2.42	R-H	0 / 2629	0.23 (1)
AF-AG	-16954 / 0	-91.8	-91.8 0.88 (1)	2.42	O-H	-1490 / 0	0.09 (1)
AG-AH	-16954 / 0	-91.8	-91.8 0.88 (1)	2.42			
AH-H	-16954 / 0	-91.8	-91.8 0.88 (1)	2.42			
H-AI	-14426 / 0	-91.8	-91.8 0.64 (1)	2.86			
AI-AJ	-14426 / 0	-91.8	-91.8 0.64 (1)	2.86			
AJ-AK	-14426 / 0	-91.8	-91.8 0.64 (1)	2.86			
AK-I	-14426 / 0	-91.8	-91.8 0.64 (1)	2.86			
I-J	-7460 / 0	-91.8	-91.8 0.31 (1)	3.34			
J-K	-7228 / 0	-91.8	-91.8 0.30 (1)	3.41			
K-L	0 / 35	-91.8	-91.8 0.07 (1)	10.00			
X-B	-3674 / 0	0.0	0.0 0.13 (1)	7.34			
M-K	-4134 / 0	0.0	0.0 0.15 (1)	7.01			

X-AL	0 / 0	-18.5	-18.5	0.04 (4)	10.00
AL-W	0 / 0	-18.5	-18.5	0.04 (4)	10.00
W-AM	0 / 3481	-18.5	-18.5	0.27 (1)	10.00
AM-AN	0 / 3481	-18.5	-18.5	0.27 (1)	10.00
AN-V	0 / 3481	-18.5	-18.5	0.27 (1)	10.00
V-U	0 / 7677	-18.5	-18.5	0.55 (1)	10.00
U-LO	0 / 7677	-18.5	-18.5	0.55 (1)	10.00
LO-AP	0 / 7677	-18.5	-18.5	0.55 (1)	10.00
AP-T	0 / 7677	-18.5	-18.5	0.55 (1)	10.00
T-AT	0 / 580	-18.5	-18.5	0.08 (1)	10.00
AT-AQ	0 / 580	-18.5	-18.5	0.08 (1)	10.00
AQ-AR	0 / 580	-18.5	-18.5	0.08 (1)	10.00
AR-AS	0 / 580	-18.5	-18.5	0.08 (1)	10.00
AS-S	0 / 580	-18.5	-18.5	0.08 (1)	10.00
S-R	0 / 342	0.0	0.0	0.46 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY
USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TDP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

*** NDN STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL
LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-DVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % DF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.20")
CALCULATED VERT. DEFL(LL) = L/ 740 (0.58")
ALLOWABLE DEFL(TL)= L/360 (1.20")
CALCULATED VERT. DEFL(TL) = L/ 393 (1.10")

CSI: TC=0.88/1.00 (G-H:1), BC=0.79/1.00 (O-R:1),
WB=0.75/1.00 (I-O:1), SSI=0.25/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg



Structural component only
DWG# T-2115484 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411017	T30S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	1.75
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMVW-t	MT20	5.0	6.0		
E	TMVW-t	MT20	6.0	9.0		
F	TS-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	7.0	8.0	Edge	
J	TMVW-t	MT20	4.0	4.0	2.00	1.00
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-t	MT20	6.0	12.0	3.75	7.25
P	BMVW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	6.0	10.0		
R	BVMWWW-t	MT20	8.0	16.0	5.00	6.50
S	BMV+p	MT20	3.0	6.0		
T	BMVWW-t	MT20	8.0	9.0	3.25	2.25
U	BS-t	MT20	5.0	6.0		
V	8MWV-t	MT20	5.0	6.0	2.50	2.75
W	8MWV-t	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. MEMB. FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH FR-TO	MAX. MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO					
R-G	-585 / 0	0.0	0.0	0.44 (1)	7.81		
R-AT	0 / 14425	-18.5	-18.5	0.79 (1)	10.00		
AT-AU	0 / 14425	-18.5	-18.5	0.79 (1)	10.00		
AU-AV	0 / 14425	-18.5	-18.5	0.79 (1)	10.00		
AV-Q	0 / 14425	-18.5	-18.5	0.79 (1)	10.00		
Q-AW	0 / 6241	-18.5	-18.5	0.44 (1)	10.00		
AW-AX	0 / 6241	-18.5	-18.5	0.44 (1)	10.00		
AX-AY	0 / 6241	-18.5	-18.5	0.44 (1)	10.00		
AY-P	0 / 6241	-18.5	-18.5	0.44 (1)	10.00		
P-AZ	0 / 6182	-18.5	-18.5	0.44 (1)	10.00		
AZ-O	0 / 6182	-18.5	-18.5	0.44 (1)	10.00		
N-O	0 / 40	0.0	0.0	0.20 (1)	10.00		
O-J	-93 / 0	0.0	0.0	0.20 (1)	7.81		
N-BA	0 / 256	-18.5	-18.5	0.02 (1)	10.00		
BA-M	0 / 256	-18.5	-18.5	0.02 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-1-11	-30	-30		FRONT	VERT	DEAD		C1
C	3-1-11	-127	-127		FRONT	VERT	SNOW		C1
D	7-11-4	-76	-76		BACK	VERT	TOTAL		C1
G	18-9-12	1	1		BACK	VERT	TOTAL		C1
I	32-9-5	-30	-30		FRONT	VERT	DEAD		C1
I	32-9-5	1	1		BACK	VERT	TOTAL		C1
P	32-9-12	-127	-127		FRONT	VERT	SNOW		C1
S	32-9-12	-171	-171		BACK	VERT	TOTAL		C1
S	18-7-4	-177	-177		BACK	VERT	TOTAL		C1
V	7-11-4	-21	-21		BACK	VERT	TOTAL		C1
V	3-11-4	-82	-82		BACK	VERT	TOTAL		C1
Z	5-11-4	-76	-76		BACK	VERT	TOTAL		C1
AA	9-11-4	-76	-76		BACK	VERT	TOTAL		C1
AB	11-11-4	-76	-76		BACK	VERT	TOTAL		C1
AC	13-11-4	-76	-76		BACK	VERT	TOTAL		C1
AD	15-11-4	-76	-76		BACK	VERT	TOTAL		C1
AE	17-11-4	-76	-76		BACK	VERT	TOTAL		C1
AF	20-9-12	1	1		BACK	VERT	TOTAL		C1
AG	22-9-12	1	1		BACK	VERT	TOTAL		C1
AH	24-9-12	1	1		BACK	VERT	TOTAL		C1
AI	26-9-12	1	1		BACK	VERT	TOTAL		C1
AJ	28-9-12	1	1		BACK	VERT	TOTAL		C1
AK	30-9-12	1	1		BACK	VERT	TOTAL		C1
AL	1-11-4	-21	-21		BACK	VERT	TOTAL		C1
AM	3-11-4	-21	-21		BACK	VERT	TOTAL		C1
AN	5-11-4	-21	-21		BACK	VERT	TOTAL		C1
AO	9-11-4	-21	-21		BACK	VERT	TOTAL		C1
AP	11-11-4	-21	-21		BACK	VERT	TOTAL		C1
AQ	13-11-4	-21	-21		BACK	VERT	TOTAL		C1
AR	15-11-4	-21	-21		BACK	VERT	TOTAL		C1
AS	17-11-4	-23	-23		BACK	VERT	TOTAL		C1
AT	20-9-12	-171	-171		BACK	VERT	TOTAL		C1
AU	22-9-12	-171	-171		BACK	VERT	TOTAL		C1
AV	24-9-12	-171	-171		BACK	VERT	TOTAL		C1
AW	26-9-12	-171	-171		BACK	VERT	TOTAL		C1
AX	28-9-12	-171	-171		BACK	VERT	TOTAL		C1
AY	30-9-12	-171	-171		BACK	VERT	TOTAL		C1
AZ	33-9-12	-175	-175		BACK	VERT	TOTAL		C1
BA	34-9-12	-24	-24		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

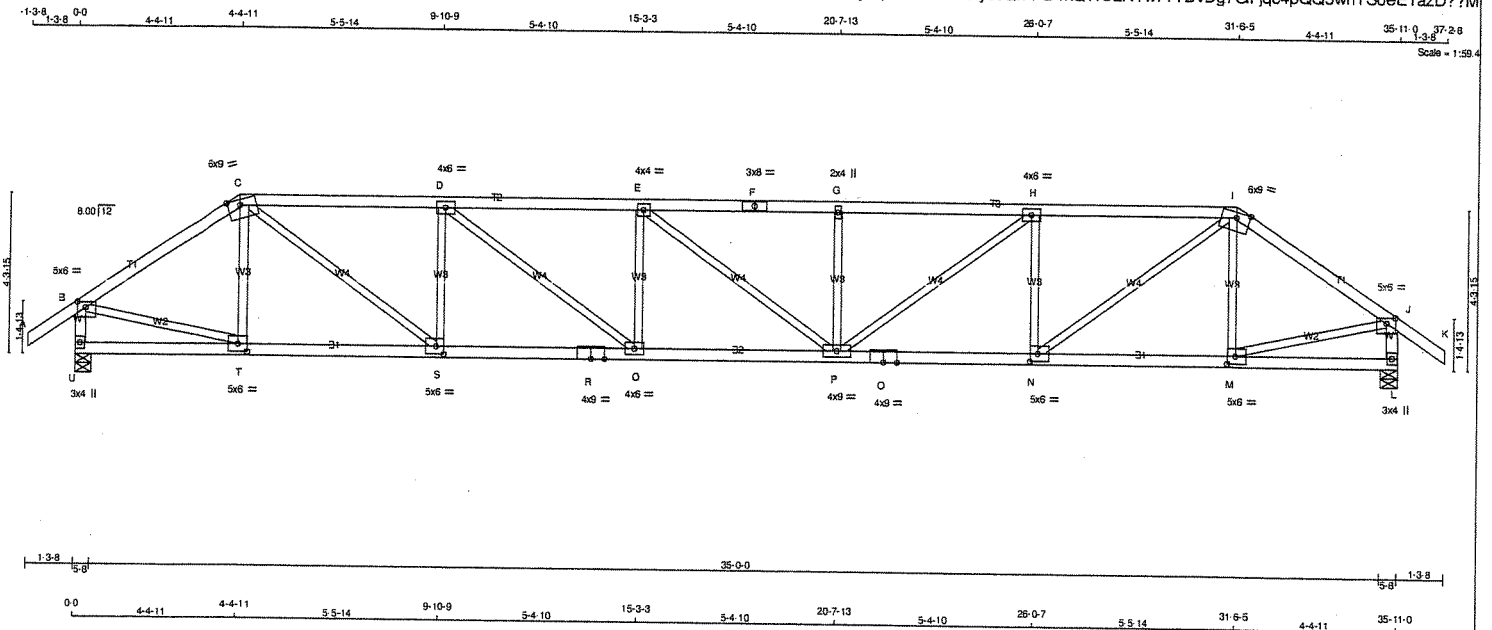
JSI GRIP= 0.89 (V) (INPUT = 0.90)
JSI METAL= 0.94 (I) (INPUT = 1.00)



Structural component only
DWG# T-2115484

JOB NAME 411017	TRUSS NAME T31	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
U - 8	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
U	2106	0	2106	0	0
L	2106	0	2106	0	0

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO 8 E SHEATHED OR MAX. PURLIN SPACING = 2.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	MAX. FACTORED	FR-TO	MAX. FACTORED
A-B	0 / 35	T-C	-356 / 0
B-C	-2310 / 0	C-S	0 / 2077
C-D	-3585 / 0	S-D	-1131 / 0
D-E	-4359 / 0	D-Q	0 / 967
E-F	-4358 / 0	Q-E	-480 / 0
F-G	-4358 / 0	E-P	-2 / 0
G-H	-4358 / 0	P-G	-479 / 0
H-I	-3586 / 0	P-H	0 / 965
I-J	-2310 / 0	N-H	-1131 / 0
J-K	0 / 35	N-I	0 / 2078
U-B	-2074 / 0	M-I	-357 / 0
L-J	-2074 / 0	B-T	0 / 1974
		M-J	0 / 1974
U-T	0 / 0		
T-S	0 / 1910		
S-R	0 / 3585		
R-Q	0 / 3585		
O-P	0 / 4359		
P-O	0 / 3586		
O-N	0 / 3586		
N-M	0 / 1910		
M-L	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 QF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.30")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/776 (0.56")

CSI: TC=0.80/1.00 (G-H-1), BC=0.76/1.00 (P-O-1),
WB=0.47/1.00 (I-H-1), SS=0.23/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (8) (INPUT = 0.90)
JSI METAL = 0.80 (0) (INPUT = 1.00)

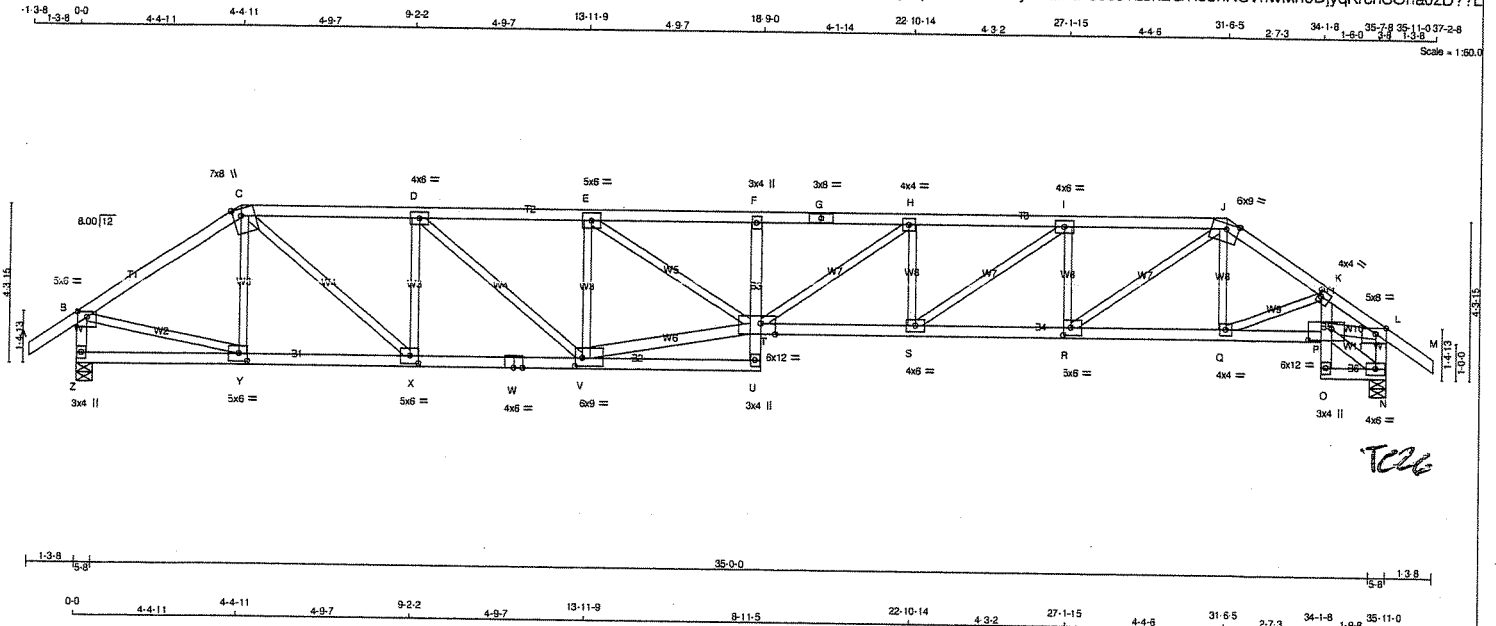


Structural component only
DWG# T-2115485

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411017	T31S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
J - M	2x4	DRY	No.2	SPF	
Z - B	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
Z - W	2x4	DRY	No.2	SPF	
W - U	2x4	DRY	No.2	SPF	
U - F	2x4	DRY	No.2	SPF	
T - P	2x4	DRY	No.2	SPF	
O - K	2x4	DRY	No.2	SPF	
O - N	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
V - T	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
P - L	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	UP
Z	2103	0	2103	0
N	2108	0	2108	0

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

	1ST LOOSE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Z	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
N	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	Y-C	-361 / 0	0.10 (1)	
B-C	-2305 / 0	-91.8	-91.8	0.42 (1)	4.10	C-X	0 / 1958	0.44 (1)	
C-D	-3411 / 0	-91.8	-91.8	0.51 (1)	3.39	X-D	-1174 / 0	0.33 (1)	
D-E	-4209 / 0	-91.8	-91.8	0.61 (1)	2.99	D-V	0 / 1056	0.24 (1)	
E-F	-5806 / 0	-91.8	-91.8	0.78 (1)	2.42	V-E	-1454 / 0	0.41 (1)	
F-G	-5868 / 0	-91.8	-91.8	0.69 (1)	2.48	V-T	0 / 4140	0.67 (1)	
G-H	-5868 / 0	-91.8	-91.8	0.69 (1)	2.48	E-T	0 / 1873	0.42 (1)	
H-I	-5425 / 0	-91.8	-91.8	0.68 (1)	2.57	O-J	0 / 97	0.03 (4)	
I-J	-4345 / 0	-91.8	-91.8	0.52 (1)	3.03	O-K	-411 / 0	0.08 (1)	
J-K	-3015 / 0	-91.8	-91.8	0.26 (1)	3.81	B-Y	0 / 1970	0.44 (1)	
K-L	-3365 / 0	-91.8	-91.8	0.24 (1)	3.64	P-N	-137 / 0	0.01 (1)	
L-M	0 / 35	-91.8	-91.8	0.12 (1)	10.00	P-L	0 / 2767	0.44 (1)	
Z-B	-2071 / 0	0.0	0.0	0.21 (1)	5.88	R-J	0 / 2229	0.50 (1)	
N-L	-2015 / 0	0.0	0.0	0.21 (1)	5.95	T-H	0 / 542	0.12 (1)	
Z-Y	0 / 0	-18.5	-18.5	0.09 (4)	10.00	R-I	-1197 / 0	0.23 (1)	
Y-X	0 / 1906	-18.5	-18.5	0.36 (1)	10.00	S-H	-692 / 0	0.13 (1)	
X-W	0 / 3412	-18.5	-18.5	0.59 (1)	10.00	S-I	0 / 1327	0.30 (1)	
W-V	0 / 3412	-18.5	-18.5	0.59 (1)	10.00				
V-U	0 / 189	-18.5	-18.5	0.11 (4)	10.00				
U-T	0 / 40	0.0	0.0	0.26 (1)	10.00				
T-F	-402 / 0	0.0	0.0	0.27 (1)	7.81				
T-S	0 / 5425	-18.5	-18.5	0.93 (1)	10.00				
S-R	0 / 4345	-18.5	-18.5	0.76 (1)	10.00				
R-Q	0 / 2513	-18.5	-18.5	0.45 (1)	10.00				
O-P	0 / 2892	-18.5	-18.5	0.51 (1)	10.00				
O-P	0 / 17	0.0	0.0	0.18 (1)	10.00				
P-K	0 / 202	0.0	0.0	0.21 (1)	10.00				
O-N	0 / 112	-18.5	-18.5	0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.43")
ALLOWABLE DEFL.(TL) = $L/360$ (1.20")
CALCULATED VERT. DEFL.(TL) = $L/542$ (0.80")

CSI: $TC=0.78/1.00$ (E-F:1), $BC=0.93/1.00$ (S-T:1), $WB=0.67/1.00$ (T-V:1), $SS=0.21/1.00$ (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

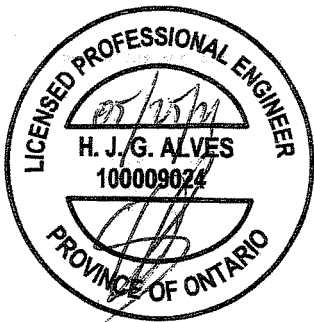
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.90)
JSI METAL = 0.82 (T) (INPUT = 1.00)



Structural component only
DWG# T-2115486

CONTINUED ON PAGE 2

JOB NAME 411017	TRUSS NAME T31S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BS-t	MT20	4.0	6.0		
X	BMWVW-t	MT20	5.0	6.0	2.50	2.75
Y	BMWVW-t	MT20	5.0	6.0	2.50	2.75
Z	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115486 *2/2*

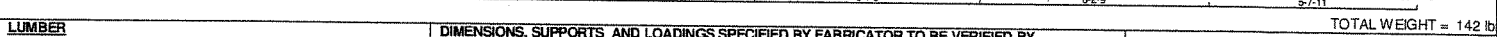
amarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Tue May 25 10:54:33 2021 Page 1

ID:E?n9mn1Ky9qxx7v78133ulyzsqn-LbCUBv4A75rNHL59kK8vwd68UyPumv67K6TzD??K

The drawing shows a roof truss structure with various dimensions and labels. The dimensions are as follows:

- 1-3-8
- 0-0
- 1-3-8
- 5-7-11
- 5-7-11
- 6-2-9
- 11-10-3
- 6-1-5
- 17-11-8
- 6-1-5
- 24-0-13
- 6-2-9
- 30-3-5
- 5-7-11
- 35-11-0
- 37-2-8
- 1-3-8



DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S. K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

S-R	0/0	-18.5	-18.5	0.15 (4)	10.00
R-O	0/1953	-18.5	-18.5	0.40 (1)	10.00
Q-P	0/3311	-18.5	-18.5	0.59 (1)	10.00
P-O	0/3311	-18.5	-18.5	0.59 (1)	10.00
Q-N	0/3311	-18.5	-18.5	0.59 (1)	10.00
N-M	0/3311	-18.5	-18.5	0.59 (1)	10.00
M-L	0/1953	-18.5	-18.5	0.40 (1)	10.00
L-K	0/0	-18.5	-18.5	0.15 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.20")
CALCULATED VERT. DEFL(LL) = L/ 999 (0.22")
ALLOWABLE DEFL(TL)= L/360 (1.20")
CALCULATED VERT. DEFL(TL) = L/ 999 (0.41")

CSI: TC=0.93/1.00 (D-E:1), BC=0.59/1.00 (M-O:1),
WB=0.45/1.00 (B-R:1), SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

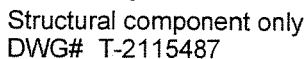
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

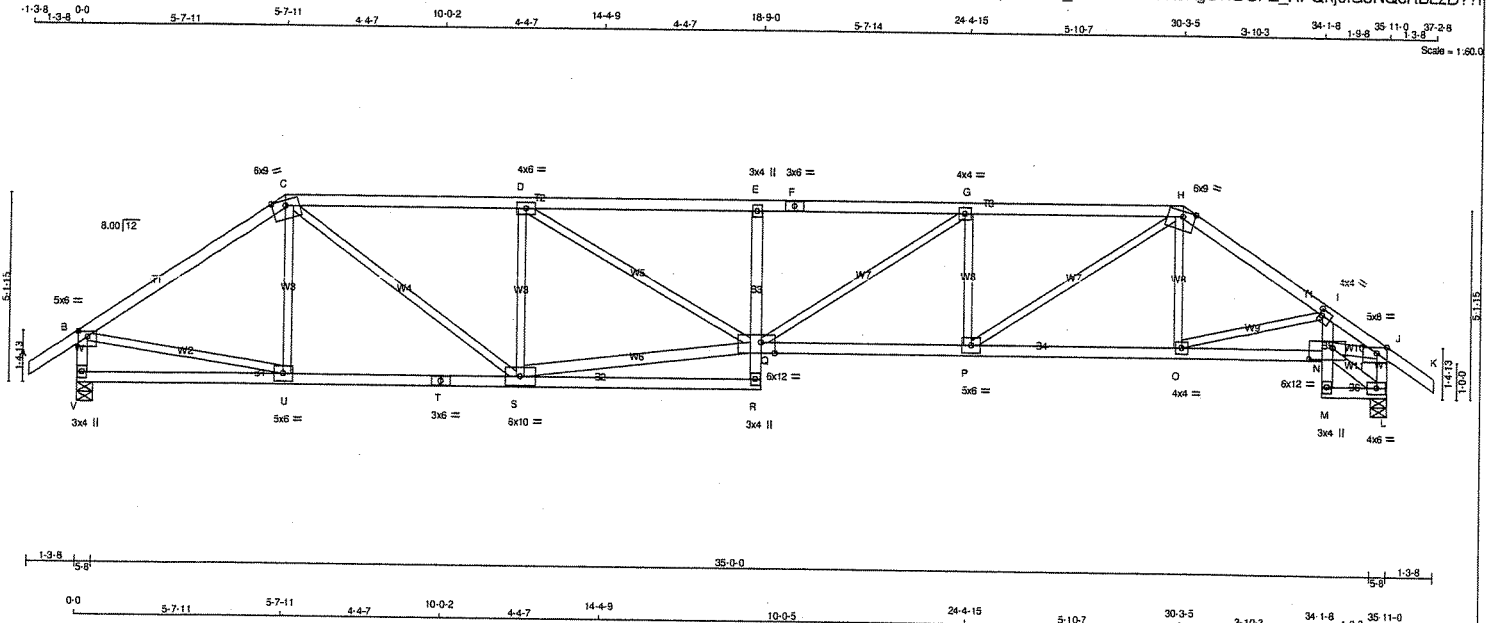
JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.76 (P) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411017	T32S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 10:54:35 2021 Page 1
ID:E?n9mn1Ky9qpx7v78133ulyzsQn-H_KFGtVsRiQrK9XgSWBCPZ_HPQnj0iG3NQcRBLzD??!



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	1650F 1.5E	SPF	
F - H	2x4	DRY	1650F 1.5E	SPF	
H - K	2x4	DRY	No.2	SPF	
V - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
V - T	2x4	DRY	No.2	SPF	
T - R	2x4	DRY	No.2	SPF	
R - E	2x4	DRY	No.2	SPF	
O - N	2x4	DRY	No.2	SPF	
M - I	2x4	DRY	No.2	SPF	
M - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
S - O	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
V	2103	0	2103	0
L	2108	0	2108	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
L	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO						FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	U-C	-247 / 8	0.10 (1)	
B-C	-2353 / 0	-91.8	-91.8	0.74 (1)	3.71	C-S	0 / 1746	0.39 (1)	
C-D	-3343 / 0	-91.8	-91.8	0.70 (1)	3.71	S-D	-1415 / 0	0.56 (1)	
D-E	-4594 / 0	-91.8	-91.8	0.85 (1)	3.15	S-Q	0 / 3258	0.52 (1)	
E-F	-4632 / 0	-91.8	-91.8	0.62 (1)	3.34	D-O	0 / 1430	0.32 (1)	
F-G	-4632 / 0	-91.8	-91.8	0.62 (1)	3.34	O-H	0 / 154	0.04 (4)	
G-H	-4075 / 0	-91.8	-91.8	0.57 (1)	3.56	O-I	-554 / 0	0.17 (1)	
H-I	-2934 / 0	-91.8	-91.8	0.37 (1)	3.76	B-U	0 / 1989	0.45 (1)	
I-J	-3427 / 0	-91.8	-91.8	0.24 (1)	3.60	N-L	-139 / 0	0.01 (1)	
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00	N-J	0 / 2841	0.46 (1)	
V-B	-2061 / 0	0.0	0.0	0.21 (1)	5.89	P-H	0 / 1960	0.44 (1)	
L-J	-2014 / 0	0.0	0.0	0.21 (1)	5.95	Q-G	0 / 670	0.15 (1)	
						P-G	-964 / 0	0.25 (1)	
V-U	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
U-T	0 / 1950	-18.5	-18.5	0.40 (1)	10.00				
T-S	0 / 1950	-18.5	-18.5	0.40 (1)	10.00				
S-R	0 / 148	-18.5	-18.5	0.19 (4)	10.00				
R-Q	0 / 55	0.0	0.0	0.21 (1)	10.00				
Q-E	-517 / 0	0.0	0.0	0.23 (1)	7.81				
Q-P	0 / 4075	-18.5	-18.5	0.74 (1)	10.00				
P-O	0 / 2437	-18.5	-18.5	0.46 (1)	10.00				
O-N	0 / 2966	-18.5	-18.5	0.54 (1)	10.00				
M-N	0 / 17	0.0	0.0	0.18 (1)	10.00				
N-I	0 / 173	0.0	0.0	0.21 (1)	10.00				
M-L	0 / 114	-18.5	-18.5	0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL)= L/774 (0.56")

CSI: TC=0.85/1.00 (D-E:1), BC=0.74/1.00 (P-O:1), WB=0.56/1.00 (D-S:1), SS=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTDSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.65 (O) (INPUT = 1.00)

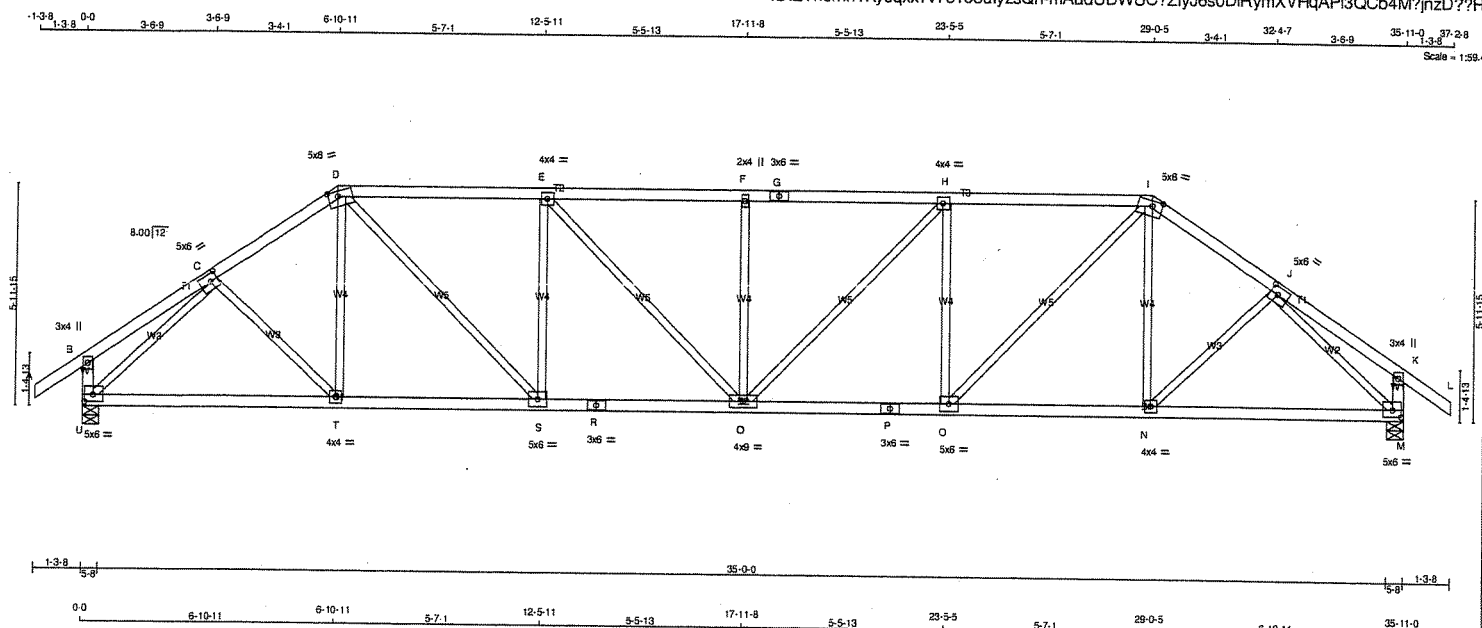


Structural component only
DWG# T-2115488

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411017	T33	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.4.20 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 10:54:36 2021 Page 1
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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF
U - B	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
U - R	2x4	DRY No.2	SPF
R - P	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-i	MT20	5.0	6.0	2.50	2.50
D TTWW-m	MT20	5.0	8.0	1.75	3.25
E TMVW-i	MT20	4.0	4.0		
F TMVW-w	MT20	2.0	4.0		
G TS-i	MT20	3.0	6.0		
H TMVW-i	MT20	4.0	4.0		
I TTWW-m	MT20	5.0	8.0	1.75	3.25
J TMVW-i	MT20	5.0	6.0	2.50	2.50
K TMV+p	MT20	3.0	4.0		
M BMVW1-i	MT20	5.0	6.0	2.25	2.75
N BMVW-i	MT20	4.0	4.0		
O BMVW-i	MT20	5.0	6.0		
P BS-i	MT20	3.0	6.0		
Q BMVW-w	MT20	4.0	9.0		
R BS-i	MT20	3.0	6.0		
S BMVW-i	MT20	5.0	6.0		
T BMVW-i	MT20	4.0	4.0		
U BMVW1-i	MT20	5.0	6.0	2.25	2.75

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	2106	2106	0	5-8
U	0	0	0	5-8
M	0	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
U	1487	989 / 0
M	1487	989 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	C-T	0 / 163	0.04 (1)
B-C	0 / 17	-91.8	-91.8 0.15 (1)	10.00	T-D	0 / 83	0.03 (4)
C-D	-2368 / 0	-91.8	-91.8 0.22 (1)	4.27	D-S	0 / 1326	0.30 (1)
D-E	-2882 / 0	-91.8	-91.8 0.62 (1)	3.52	S-E	-855 / 0	0.48 (1)
E-F	-3155 / 0	-91.8	-91.8 0.65 (1)	3.35	E-O	0 / 393	0.09 (1)
F-G	-3155 / 0	-91.8	-91.8 0.65 (1)	3.35	O-F	-463 / 0	0.26 (1)
G-H	-3155 / 0	-91.8	-91.8 0.65 (1)	3.35	Q-H	0 / 393	0.09 (1)
H-I	-2882 / 0	-91.8	-91.8 0.62 (1)	3.52	O-H	-855 / 0	0.48 (1)
I-J	-2368 / 0	-91.8	-91.8 0.22 (1)	4.27	O-I	0 / 1326	0.30 (1)
J-K	0 / 17	-91.8	-91.8 0.15 (1)	10.00	N-I	0 / 83	0.03 (4)
K-L	0 / 35	-91.8	-91.8 0.12 (1)	10.00	N-J	0 / 163	0.04 (1)
U-B	-253 / 0	0.0	0.0 0.03 (1)	7.81	U-C	-2570 / 0	0.95 (1)
M-K	-253 / 0	0.0	0.0 0.03 (1)	7.81	J-M	-2570 / 0	0.95 (1)
U-T	0 / 1834	-18.5	-18.5 0.40 (1)	10.00			
T-S	0 / 1953	-18.5	-18.5 0.42 (1)	10.00			
S-R	0 / 2883	-18.5	-18.5 0.52 (1)	10.00			
R-O	0 / 2883	-18.5	-18.5 0.52 (1)	10.00			
Q-P	0 / 2883	-18.5	-18.5 0.52 (1)	10.00			
P-O	0 / 2883	-18.5	-18.5 0.52 (1)	10.00			
O-N	0 / 1953	-18.5	-18.5 0.42 (1)	10.00			
N-M	0 / 1834	-18.5	-18.5 0.40 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.32")

CSI: TO=0.65/1.00 (E-F:1), BO=0.52/1.00 (O-Q:1), WB=0.95/1.00 (J-M:1), SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.87 (R) (INPUT = 1.00)

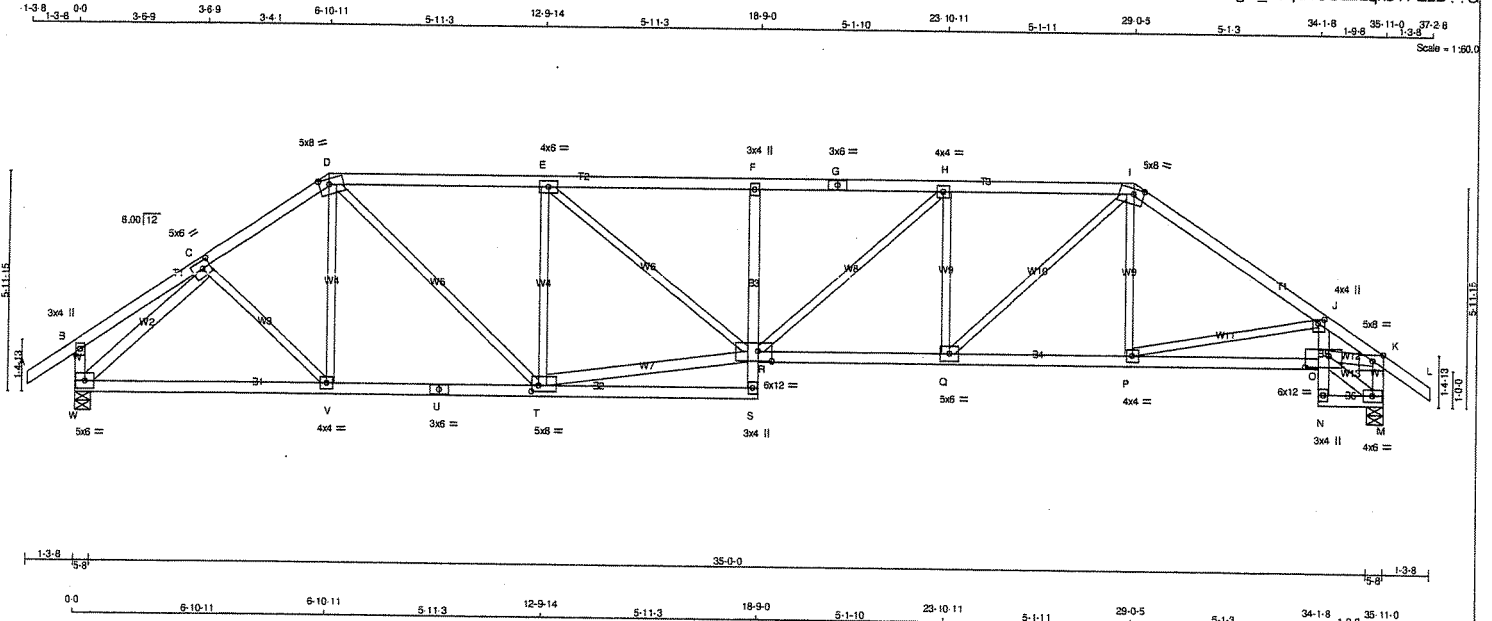


Structural component only
DWG# T-2115489

JOB NAME 411017	TRUSS NAME T33S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF
D - G	2x4 DRY	No.2	SPF
G - I	2x4 DRY	No.2	SPF
I - L	2x4 DRY	No.2	SPF
W - B	2x4 DRY	No.2	SPF
M - K	2x4 DRY	No.2	SPF
W - U	2x4 DRY	No.2	SPF
U - S	2x4 DRY	No.2	SPF
S - F	2x4 DRY	No.2	SPF
R - O	2x4 DRY	No.2	SPF
N - J	2x4 DRY	No.2	SPF
N - M	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			
T - R	2x4 DRY	No.2	SPF
W - C	2x4 DRY	No.2	SPF
O - M	2x4 DRY	No.2	SPF
O - K	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
W	2103	0	2103	0	0	5-8	5-8		
M	2108	0	2108	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	MAX	MIN	COMPONENT REACTIONS									
W	1485	988 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0				
M	1489	990 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MEMB.		WEBS	
MAX. FACTORED	FORCE (LBS)	FR-TO	TO	MAX. FACTORED	FORCE (LBS)
A-B	0 / 35	-91.8	-91.8 0.12 (1)	C-V	0 / 160
B-C	0 / 18	-91.8	-91.8 0.15 (1)	V-D	0 / 81
C-D	-2366 / 0	-91.8	-91.8 0.23 (1)	D-T	0 / 1341
D-E	-2912 / 0	-91.8	-91.8 0.72 (1)	T-E	-1307 / 0
E-F	-3783 / 0	-91.8	-91.8 0.85 (1)	T-R	0 / 2872
F-G	-3804 / 0	-91.8	-91.8 0.65 (1)	E-R	0 / 1101
G-H	-3804 / 0	-91.8	-91.8 0.65 (1)	R-H	0 / 522
H-I	-3414 / 0	-91.8	-91.8 0.58 (1)	O-H	-875 / 0
I-J	-2837 / 0	-91.8	-91.8 0.57 (1)	O-I	0 / 1442
J-K	-3513 / 0	-91.8	-91.8 0.34 (1)	P-I	0 / 187
K-L	0 / 35	-91.8	-91.8 0.12 (1)	P-J	-744 / 0
W-B	-251 / 0	0.0	0.0 0.03 (1)	W-C	-2568 / 0
M-K	-2012 / 0	0.0	0.0 0.21 (1)	O-M	-143 / 0
				O-K	0 / 2943
W-V	0 / 1833	-18.5	-18.5 0.40 (1)		
V-U	0 / 1950	-18.5	-18.5 0.42 (1)		
U-T	0 / 1950	-18.5	-18.5 0.42 (1)		
T-S	0 / 98	-18.5	-18.5 0.14 (4)		
S-R	0 / 50	0.0	0.0 0.14 (1)		
R-F	-469 / 0	0.0	0.0 0.19 (1)		
R-O	0 / 3414	-18.5	-18.5 0.62 (1)		
O-P	0 / 2349	-18.5	-18.5 0.45 (1)		
P-O	0 / 3072	-18.5	-18.5 0.56 (1)		
N-O	0 / 17	0.0	0.0 0.18 (1)		
O-J	0 / 168	0.0	0.0 0.21 (1)		
N-M	0 / 117	-18.5	-18.5 0.03 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, A8C 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.23")
ALLOWABLE DEFL. (TL) = L/360 (1.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.42")

CSI: TC=0.85/1.00 (E-F:1), BC=0.62/1.00 (Q-R:1),
WB=0.73/1.00 (E-T:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)

JSI METAL= 0.58 (U) (INPUT = 1.00)



Structural component only
DWG# T-2115490

CONTINUED ON PAGE 2

JOB NAME 411017	TRUSS NAME T33S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 10:54:37 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWV1-	MT20	4.0	4.0		
W	BMWV1-	MT20	5.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

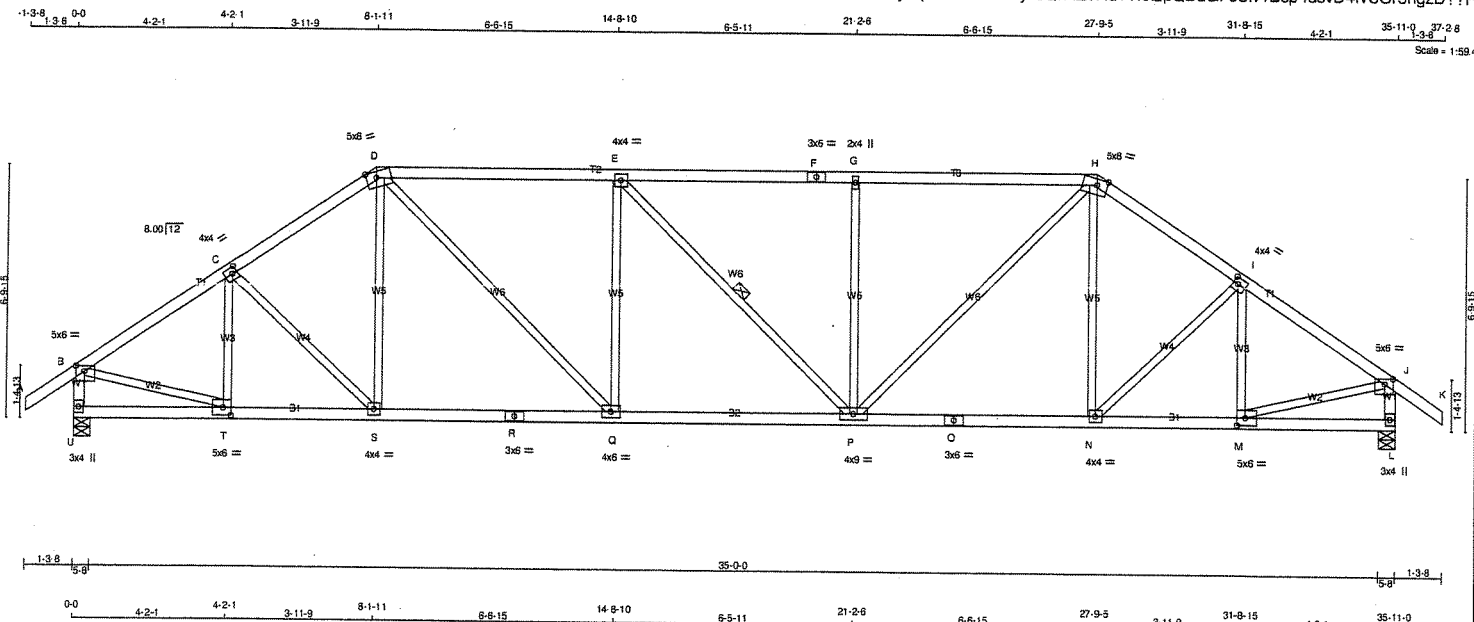
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115490 2/2

JOB NAME 411017	TRUSS NAME T34	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 10:54:38 2021 Page 1
ID:E?n9mn1Ky9qx7v78133ulyzsQn-IZ?NuvYkdpQBdGF8elv1Bcp4dsVd4IV3Or5ngzD??F



TOTAL WEIGHT = 155 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
U	2106	0	0	2106	0	0	5-8	5-8	5-8
L	2106	0	0	2106	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	MAX. CS1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	T-C	-435 / 0	0.12 (1)
B-C	-2304 / 0	-91.8	-91.8 0.33 (1)	4.21	C-S	-25 / 2	0.01 (1)
C-D	-2333 / 0	-91.8	-91.8 0.33 (1)	4.18	S-D	0 / 125	0.04 (4)
D-E	-2699 / 0	-91.8	-91.8 0.82 (1)	3.40	D-O	0 / 1096	0.25 (1)
E-F	-2697 / 0	-91.8	-91.8 0.81 (1)	3.40	Q-E	-650 / 0	0.51 (1)
F-G	-2697 / 0	-91.8	-91.8 0.81 (1)	3.40	E-P	-2 / 0	0.00 (1)
G-H	-2697 / 0	-91.8	-91.8 0.81 (1)	3.41	P-G	-650 / 0	0.51 (1)
H-I	-2334 / 0	-91.8	-91.8 0.33 (1)	4.18	P-H	0 / 1094	0.25 (1)
I-J	-2304 / 0	-91.8	-91.8 0.33 (1)	4.21	N-H	0 / 126	0.04 (4)
J-K	0 / 35	-91.8	-91.8 0.12 (1)	10.00	N-I	-24 / 2	0.01 (1)
U-B	-2068 / 0	0.0	0.0 0.21 (1)	5.88	M-I	-436 / 0	0.12 (1)
L-J	-2068 / 0	0.0	0.0 0.21 (1)	5.88	B-T	0 / 2000	0.45 (1)
					M-J	0 / 2000	0.45 (1)
U-T	0 / 0	-18.5	-18.5 0.07 (4)	10.00			
T-S	0 / 1936	-18.5	-18.5 0.38 (1)	10.00			
S-R	0 / 1920	-18.5	-18.5 0.39 (1)	10.00			
R-Q	0 / 1920	-18.5	-18.5 0.39 (1)	10.00			
Q-P	0 / 2699	-18.5	-18.5 0.51 (1)	10.00			
P-O	0 / 1920	-18.5	-18.5 0.38 (1)	10.00			
O-N	0 / 1920	-18.5	-18.5 0.38 (1)	10.00			
N-M	0 / 1936	-18.5	-18.5 0.38 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CS1: TC=0.82/1.00 (D-E:1), BC=0.51/1.00 (P-O:1),
WB=0.51/1.00 (E-Q:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JS1 GRIP = 0.85 (B) (INPUT = 0.90)
JS1 METAL = 0.65 (O) (INPUT = 1.00)

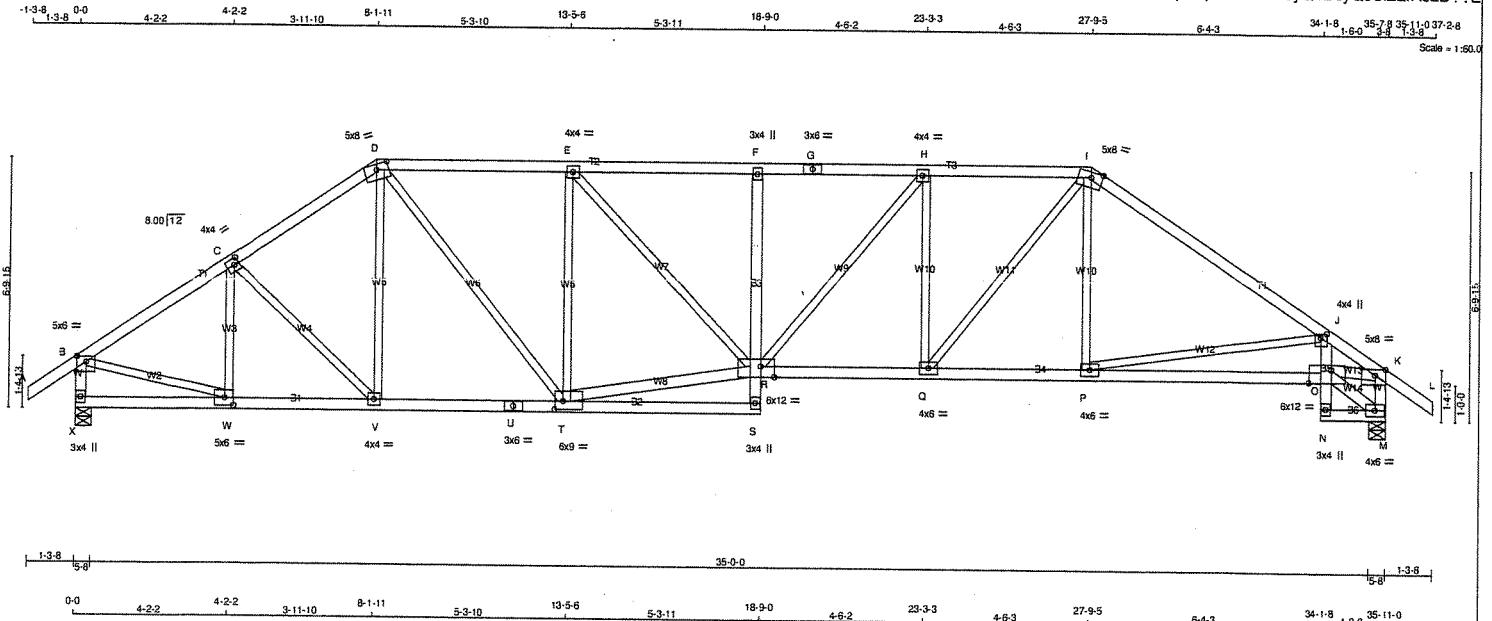


Structural component only
DWG# T-2115491

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411017	T34S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.		
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
X - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
X - U	2x4	DRY	No.2	SPF	
U - S	2x4	DRY	No.2	SPF	
S - F	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
N - M	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
T - R	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
O - K	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	1.75	4.00
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.75
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	3.50
M	BMVW-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWV-t	MT20	6.0	12.0	4.25	7.50
P	BMVW-t	MT20	4.0	6.0		
Q	BMVW-t	MT20	4.0	6.0		
R	BVMWV-t	MT20	6.0	12.0	Edge	4.50
S	BMV+p	MT20	3.0	4.0		
T	BVMWV-t	MT20	6.0	9.0	2.75	2.75
U	BS-t	MT20	3.0	6.0		
V	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ
X	2103	0	2103	0
M	2108	0	2108	0

UNFACTORED REACTIONS

	1ST LOASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
X	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
M	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8 0.12 (1)	10.00	W-C	-428 / 0	0.11 (1)	
B-C	-2303 / 0	-91.8 -91.8 0.33 (1)	4.21	C-V	-31 / 0	0.02 (1)	
C-D	-2326 / 0	-91.8 -91.8 0.33 (1)	4.19	V-D	0 / 109	0.03 (4)	
D-E	-2583 / 0	-91.8 -91.8 0.52 (1)	3.79	D-T	0 / 1068	0.24 (1)	
E-F	-3212 / 0	-91.8 -91.8 0.59 (1)	3.38	T-E	-1209 / 0	0.95 (1)	
F-G	-3225 / 0	-91.8 -91.8 0.44 (1)	3.53	T-R	0 / 2572	0.41 (1)	
G-H	-3225 / 0	-91.8 -91.8 0.44 (1)	3.53	E-R	0 / 905	0.20 (1)	
H-I	-2958 / 0	-91.8 -91.8 0.41 (1)	3.70	R-H	0 / 414	0.09 (1)	
I-J	-2738 / 0	-91.8 -91.8 0.91 (1)	3.47	Q-H	-779 / 0	0.41 (1)	
J-K	-3625 / 0	-91.8 -91.8 0.54 (1)	3.08	O-I	0 / 1104	0.25 (1)	
K-L	0 / 35	-91.8 -91.8 0.12 (1)	10.00	P-I	0 / 225	0.06 (4)	
X-B	-2067 / 0	0.0 0.0 0.21 (1)	5.88	P-J	-968 / 0	0.81 (1)	
M-K	-2009 / 0	0.0 0.0 0.21 (1)	5.95	B-W	0 / 1999	0.45 (1)	
				O-M	-147 / 0	0.02 (1)	
X-W	0 / 0	-18.5 -18.5 0.07 (4)	10.00	O-K	0 / 3076	0.49 (1)	
W-V	0 / 1935	-18.5 -18.5 0.36 (1)	10.00				
V-U	0 / 1914	-18.5 -18.5 0.37 (1)	10.00				
U-T	0 / 1914	-18.5 -18.5 0.37 (1)	10.00				
T-S	0 / 69	-18.5 -18.5 0.13 (4)	10.00				
S-R	0 / 43	0.0 0.0 0.10 (1)	10.00				
R-F	-417 / 0	0.0 0.0 0.19 (1)	7.81				
R-Q	0 / 2958	-18.5 -18.5 0.53 (1)	10.00				
Q-P	0 / 2259	-18.5 -18.5 0.46 (1)	10.00				
P-O	0 / 3208	-18.5 -18.5 0.61 (1)	10.00				
N-O	0 / 17	0.0 0.0 0.19 (1)	10.00				
O-J	0 / 168	0.0 0.0 0.22 (1)	10.00				
N-M	0 / 121	-18.5 -18.5 0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.35")

CSI: TC=0.91/1.00 (I-J:1), BC=0.61/1.00 (O-P:1),
WB=0.95/1.00 (E-T:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (D) (INPUT = 0.90)
JSI METAL = 0.55 (K) (INPUT = 1.00)



Structural component only
DWG# T-2115492

CONTINUED ON PAGE 2

JOB NAME 411017	TRUSS NAME T34S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMW-t	MT20	5.0	6.0	2.50	2.75
X	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

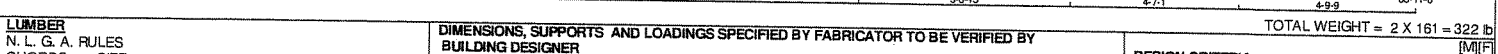
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115492 *W*

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-1-3.8 0-0 4-9.9 4-9.9 4-7.1 9-4-11 5-8-15 15-1-10 5-7-11 20-9-6 5-8-15 25-9-5 4-7.1 31-1-7 4-9.9 35-11-0 37-2.8



NOTES- (1)
) Lateral braces to be a minimum of 2x4 SPF #2

BEARINGS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

BEARING MATERIAL TO BE SPE NO 2 OR BETTER AT JOINT(S) U/I

BRACING
TOP CHORD TO BE SHEATHED OR RAIL ENHANCED

OR IF EXERCISED FROM STANDARD LENGTH $H \cong 10.00$ FT. OR RIGID CEILING DIRECTLY APP.

1 LATERAL BRACE(S) AT 1/3 LENGTH OF E.B.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN

LOADING

CHOPPS

MEMB.		FORCE		VERT. LOAD		LC1		MAX		MAX.		MEMB		FORCE		MAX.	
-------	--	-------	--	------------	--	-----	--	-----	--	------	--	------	--	-------	--	------	--

FR-TO	(LBS)	(PLF)	CSI (LC)	UNBRAC	FR-TO	(LBS)	CSI (LC)	
		FROM	TO	LENGTH				
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	T-C	-359 / 0	0.11 (1)
B-C	-2355 / 0	-91.8	-91.8	0.34 (1)	4.18	C-S	-152 / 0	0.11 (1)
C-D	-2280 / 0	-91.8	-91.8	0.33 (1)	4.25	S-D	0 / 203	0.05 (4)
D-E	-2398 / 0	-91.8	-91.8	0.43 (1)	4.03	D-Q	0 / 852	0.19 (1)
E-F	-2397 / 0	-91.8	-91.8	0.40 (1)	4.08	Q-E	-566 / 0	0.61 (1)
F-G	-2397 / 0	-91.8	-91.8	0.40 (1)	4.08	E-P	-2 / 0	0.00 (1)
G-H	-2397 / 0	-91.8	-91.8	0.43 (1)	4.03	P-G	-565 / 0	0.61 (1)
H-I	-2280 / 0	-91.8	-91.8	0.33 (1)	4.25	P-H	0 / 849	0.19 (1)
I-J	-2356 / 0	-91.8	-91.8	0.34 (1)	4.18	H-N	0 / 204	0.05 (4)
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00	N-I	-152 / 0	0.11 (1)
K-L	-2066 / 0	0.0	0.0	0.21 (1)	5.89	M-I	-359 / 0	0.11 (1)
L-J	-2066 / 0	0.0	0.0	0.21 (1)	5.89	B-T	0 / 2033	0.46 (1)
						M-J	0 / 2033	0.46 (1)
J-T	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
T-S	0 / 1983	-18.5	-18.5	0.38 (1)	10.00			
S-R	0 / 1874	-18.5	-18.5	0.36 (1)	10.00			
R-Q	0 / 1874	-18.5	-18.5	0.36 (1)	10.00			
Q-P	0 / 2398	-18.5	-18.5	0.45 (1)	10.00			
P-O	0 / 1874	-18.5	-18.5	0.36 (1)	10.00			
O-N	0 / 1874	-18.5	-18.5	0.36 (1)	10.00			
N-M	0 / 1983	-18.5	-18.5	0.38 (1)	10.00			
M-L	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

100% 99% 98% 97% 96% 95% 94% 93% 92% 91% 90% 89% 88% 87% 86% 85% 84% 83% 82% 81% 80% 79% 78% 77% 76% 75% 74% 73% 72% 71% 70% 69% 68% 67% 66% 65% 64% 63% 62% 61% 60% 59% 58% 57% 56% 55% 54% 53% 52% 51% 50% 49% 48% 47% 46% 45% 44% 43% 42% 41% 40% 39% 38% 37% 36% 35% 34% 33% 32% 31% 30% 29% 28% 27% 26% 25% 24% 23% 22% 21% 20% 19% 18% 17% 16% 15% 14% 13% 12% 11% 10% 9% 8% 7% 6% 5% 4% 3% 2% 1% 0%

TOP	CH.	LL	=	25.6	PSF
		DL	=	6.0	PSF

TOTAL LOAD = 7.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE

1. *Journal of Management Studies*, 1997, 34, 1, 1-14.

SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:
PART 2 OF BS2036 2015 - 1200 2015

- CSA 086-14

(55% OF 31.3 BSE GSI PLUS 3.4 BSE BAW)

LIVE LOAD

CALCULATED VERT. DEF. (LL) = 1/999 (0.12")

CALCULATED VERT. DEFL(TL) = $L/999$ (0.23")

 $WB=0.61/1.00$ (E-Q:1), $SSI=0.25/1.00$ (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT

TRUSS MANUFACTURING PLANT.

PLATE GRIP(DRY) SHEAR SECTION

	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1227	1278

PLATE PLACEMENT TOL = 0.250 inches

LATE ROTATION TOL. = 5.0 Deg.

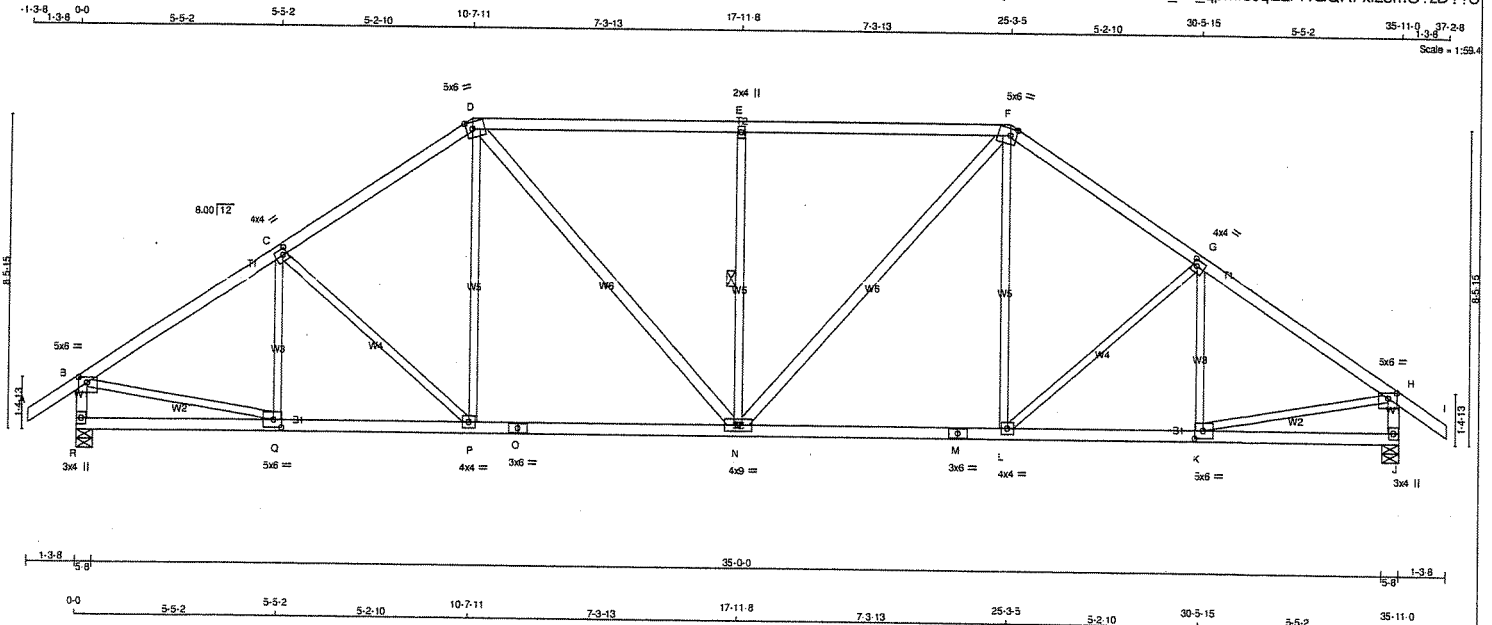
ISI METAL= 0.61 (O) (INPUT = 1.00)

1. *Journal of Management Studies*, 1996, 33, 1, 1-14.

100

JOB NAME 411017	TRUSS NAME T36	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:E?n9mn1Ky9qxx7v78133ulyzsQn-68hWXxac1YB_24_qpmiceqELFrvGQRFxIL3mO?zD??C



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	2.75
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1-p	MT20	3.0	4.0		

NOTES-

- (1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
R	2106	0	2106	0	0	5-8	5-8	5-8
J	2106	0	2106	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX / MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
	SNOW	LIVE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	SNOW	LIVE	PERM. LIVE	WIND
R	1487	989 / 0	0 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0	498 / 0	0 / 0	0 / 0	0 / 0
J	1487	989 / 0	0 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0	498 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		W E B S		FACTORED	
	MAX. FORCE (LBS)	VERT. (PLF)	MAX. (LC)	UNBRACED LENGTH	MAX. FORCE (LBS)	VERT. (PLF)	MAX. (LC)	UNBRACED LENGTH
FR-TO								
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	O-C	-302 / 0	0.11 (1)	
B-C	-2389 / 0	-91.8	-91.8 0.44 (1)	4.05	C-P	-264 / 0	0.25 (1)	
C-D	-2217 / 0	-91.8	-91.8 0.41 (1)	4.20	P-D	0 / 294	0.07 (1)	
D-E	-2250 / 0	-91.8	-91.8 0.76 (1)	3.60	D-N	0 / 647	0.10 (1)	
E-F	-2250 / 0	-91.8	-91.8 0.76 (1)	3.60	N-E	-827 / 0	0.38 (1)	
F-G	-2217 / 0	-91.8	-91.8 0.41 (1)	4.20	N-F	0 / 647	0.10 (1)	
G-H	-2389 / 0	-91.8	-91.8 0.44 (1)	4.05	L-F	0 / 294	0.07 (1)	
H-I	0 / 35	-91.8	-91.8 0.12 (1)	10.00	L-G	-264 / 0	0.25 (1)	
R-B	-2061 / 0	0.0	0.0 0.21 (1)	5.89	K-G	-302 / 0	0.11 (1)	
J-H	-2061 / 0	0.0	0.0 0.21 (1)	5.89	B-O	0 / 2053	0.46 (1)	
					K-H	0 / 2053	0.46 (1)	
R-O	0 / 0	-18.5	-18.5 0.11 (4)	10.00				
Q-P	0 / 2014	-18.5	-18.5 0.41 (1)	10.00				
P-O	0 / 1819	-18.5	-18.5 0.40 (1)	10.00				
O-N	0 / 1819	-18.5	-18.5 0.40 (1)	10.00				
N-M	0 / 1819	-18.5	-18.5 0.40 (1)	10.00				
M-L	0 / 1819	-18.5	-18.5 0.40 (1)	10.00				
L-K	0 / 2014	-18.5	-18.5 0.41 (1)	10.00				
K-J	0 / 0	-18.5	-18.5 0.11 (4)	10.00				

TOTAL WEIGHT = 2 X 163 = 326 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.76/1.00 (E-F:1), BC=0.41/1.00 (K-L:1), WB=0.46/1.00 (B-O:1), SSI=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (H) (INPUT = 0.90)
JSI METAL= 0.52 (O) (INPUT = 1.00)

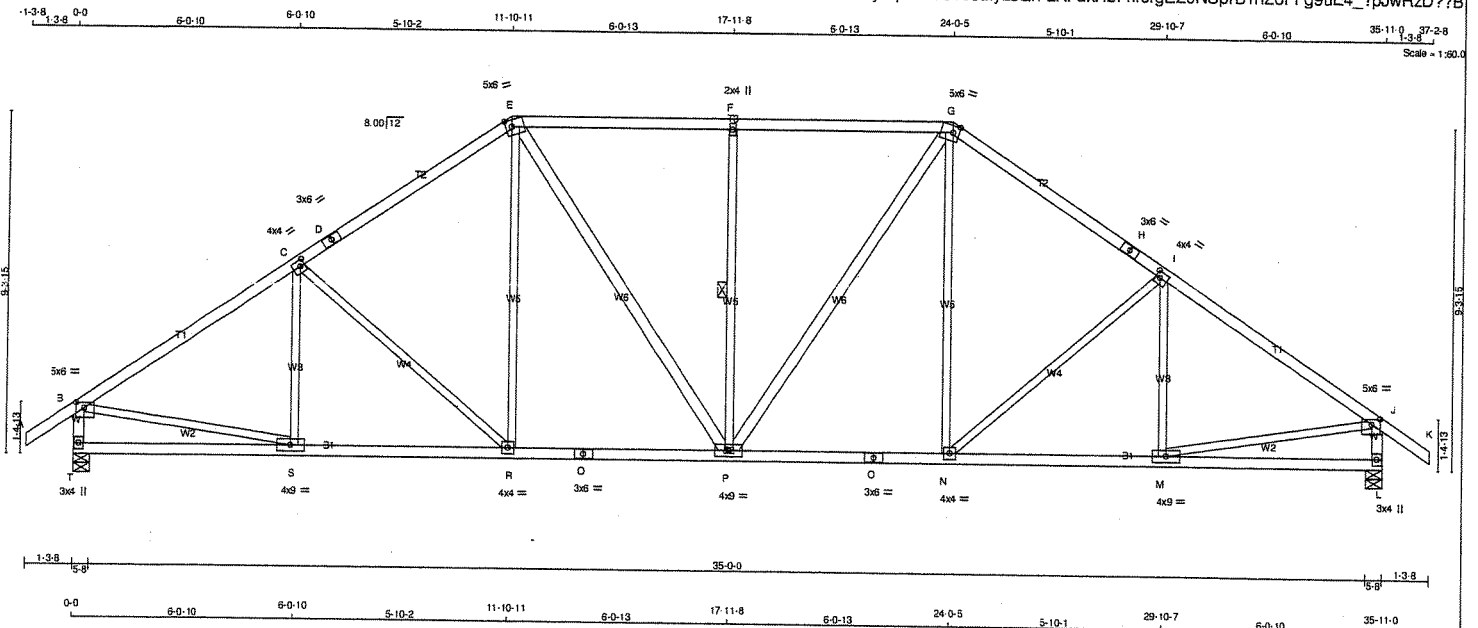


Structural component only
DWG# T-2115494

JOB NAME 411017	TRUSS NAME T37	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - K	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - B	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
E - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG
T	2106	0	2106	0	0	5-8	5-8	5-8	5-8
L	2106	0	2106	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1487	989 / 0	0 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1487	989 / 0	0 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO 8E SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	S-C	-244 / 16 0.11 (1)
B-C	-2409 / 0	-91.8	-91.8	0.56 (1)	3.90	C-R	-378 / 0 0.48 (1)
C-D	-2140 / 0	-91.8	-91.8	0.51 (1)	4.14	R-E	0 / 357 0.08 (1)
D-E	-2140 / 0	-91.8	-91.8	0.51 (1)	4.14	E-P	0 / 477 0.08 (1)
E-F	-2018 / 0	-91.8	-91.8	0.50 (1)	4.22	P-F	-682 / 0 0.39 (1)
F-G	-2018 / 0	-91.8	-91.8	0.50 (1)	4.22	P-G	0 / 477 0.08 (1)
G-H	-2140 / 0	-91.8	-91.8	0.51 (1)	4.14	N-G	0 / 357 0.08 (1)
H-I	-2140 / 0	-91.8	-91.8	0.51 (1)	4.14	N-I	-378 / 0 0.48 (1)
I-J	-2409 / 0	-91.8	-91.8	0.56 (1)	3.90	M-I	-244 / 16 0.11 (1)
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00	B-S	0 / 2067 0.47 (1)
T-B	-2058 / 0	0.0	0.0	0.21 (1)	5.89	M-J	0 / 2067 0.47 (1)
L-J	-2058 / 0	0.0	0.0	0.21 (1)	5.89		

T-S	0 / 0	-18.5	-18.5	0.15 (4)	10.00
S-R	0 / 2034	-18.5	-18.5	0.40 (1)	10.00
R-O	0 / 1752	-18.5	-18.5	0.35 (1)	10.00
O-P	0 / 1752	-18.5	-18.5	0.35 (1)	10.00
P-O	0 / 1752	-18.5	-18.5	0.35 (1)	10.00
O-N	0 / 1752	-18.5	-18.5	0.35 (1)	10.00
N-M	0 / 2034	-18.5	-18.5	0.40 (1)	10.00
M-L	0 / 0	-18.5	-18.5	0.15 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.58/1.00 (B-C:1), BC=0.40/1.00 (R-S:1), WB=0.48/1.00 (H-N:1), SI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS 8 END=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

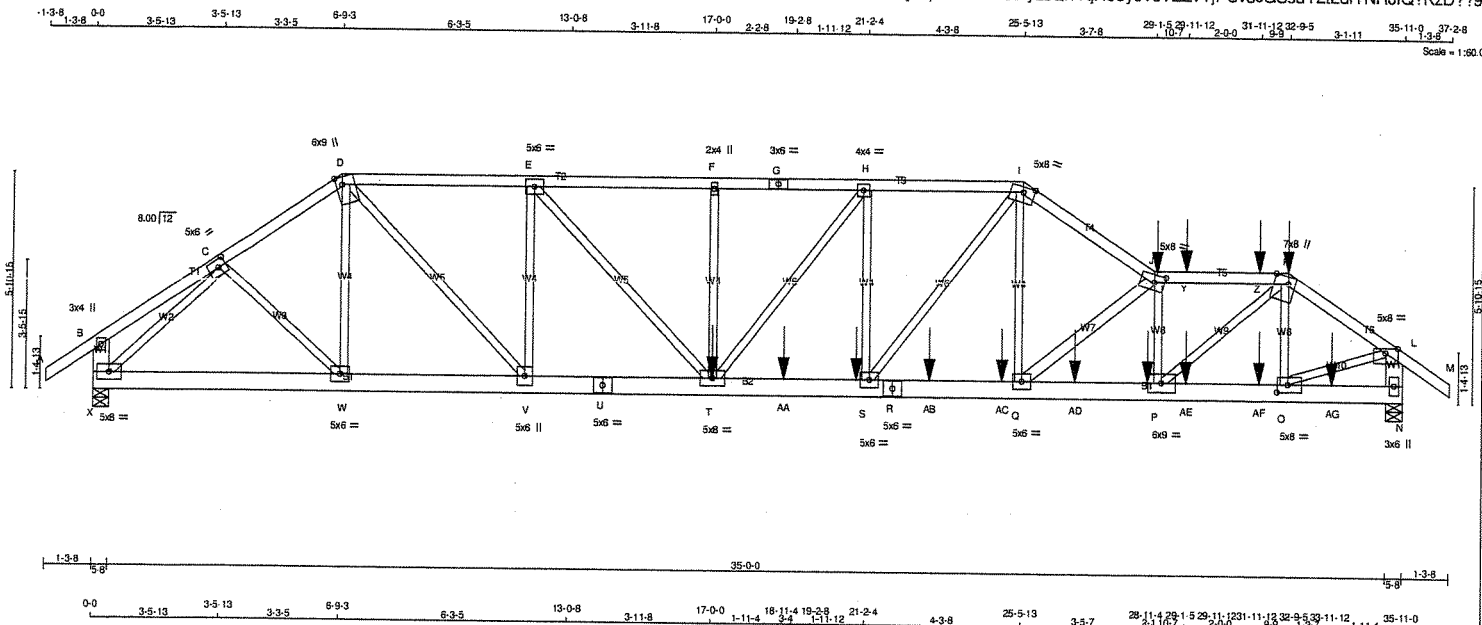
JSI GRIP = 0.85 (8) (INPUT = 0.90)
JSI METAL = 0.55 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2115495

JOB NAME 411017	TRUSS NAME T38	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:E?n9mn1Ky9qxx7v78133ulyzsQn-XjNe9ycVJTZZvYjPUvsJGSSuY2tLdiYNRJQ?KzD??9



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
J - K	2x4	DRY	No.2	SPF	
K - M	2x4	DRY	No.2	SPF	
X - B	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
X - U	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - N	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
Q - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-G 1	12	TOP
G-I 1	12	TOP
I-J 1	12	TOP
J-K 1	12	TOP
K-M 1	12	TOP
X-B 2	12	TOP
N-L 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X-U 2	12	TOP
U-R 2	12	TOP
R-N 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	TOP
2x4 1	6	TOP
2x4 1	6	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	4801	4801	0	0
N	0	0	5-8	5-8
X	3893	3893	0	0

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	3385	2281 / 0	0 / 0	0 / 0	1104 / 0
N	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
X	2745	1846 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0 / 35	-91.8	-91.8 0.07 (1)	10.00	W-D	-295 / 0
B-C	0 / 13	-91.8	-91.8 0.07 (1)	10.00	D-V	0 / 3773
C-D	-5041 / 0	-91.8	-91.8 0.17 (1)	4.19	V-E	-2641 / 0
D-E	-6748 / 0	-91.8	-91.8 0.43 (1)	3.48	Q-I	0 / 2494
E-F	-8683 / 0	-91.8	-91.8 0.57 (1)	2.97	O-J	-2471 / 0
F-G	-8683 / 0	-91.8	-91.8 0.42 (1)	3.07	P-J	-3611 / 0
G-H	-8683 / 0	-91.8	-91.8 0.42 (1)	3.07	P-K	0 / 5477
H-I	-8361 / 0	-91.8	-91.8 0.41 (1)	3.14	Q-K	-1156 / 0
I-J	-8337 / 0	-91.8	-91.8 0.39 (1)	3.14	O-L	0 / 4838
J-Y	-8714 / 0	-91.8	-91.8 0.47 (1)	2.95	T-F	-384 / 0
Y-Z	-8714 / 0	-91.8	-91.8 0.47 (1)	2.95	E-T	0 / 2866
Z-K	-8714 / 0	-91.8	-91.8 0.47 (1)	2.95	S-H	-829 / 0
K-L	-5570 / 0	-91.8	-91.8 0.20 (1)	3.98	T-H	0 / 532
L-M	0 / 35	-91.8	-91.8 0.07 (1)	10.00	S-I	0 / 2245
X-B	-258 / 0	0.0	0.0 0.01 (1)	7.81	C-W	0 / 620
N-L	-4795 / 0	0.0	0.0 0.17 (1)	6.61	X-C	-5163 / 0

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
X-W	0 / 3721	-18.5	-18.5 0.27 (1)	10.00		
W-V	0 / 4170	-18.5	-18.5 0.29 (1)	10.00		
V-U	0 / 6748	-18.5	-18.5 0.49 (1)	10.00		
U-T	0 / 6748	-18.5	-18.5 0.49 (1)	10.00		
T-AA	0 / 8361	-18.5	-18.5 0.68 (1)	10.00		
AA-S	0 / 8361	-18.5	-18.5 0.68 (1)	10.00		
S-R	0 / 6983	-18.5	-18.5 0.55 (1)	10.00		
R-AB	0 / 6983	-18.5	-18.5 0.55 (1)	10.00		
AB-AC	0 / 6983	-18.5	-18.5 0.55 (1)	10.00		
AC-Q	0 / 6983	-18.5	-18.5 0.55 (1)	10.00		
O-AD	0 / 8835	-18.5	-18.5 0.70 (1)	10.00		
AD-P	0 / 8835	-18.5	-18.5 0.70 (1)	10.00		
P-AE	0 / 4586	-18.5	-18.5 0.42 (1)	10.00		
AE-AF	0 / 4586	-18.5	-18.5 0.42 (1)	10.00		
AF-Q	0 / 4586	-18.5	-18.5 0.42 (1)	10.00		
O-AG	0 / 0	-18.5	-18.5 0.04 (1)	10.00		
AG-N	0 / 0	-18.5	-18.5 0.04 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	29-1-5	-81	-81	---	FRONT	VERT	TOTAL	---	C1
K	32-9-5	-30	-30	---	FRONT	VERT	DEAD	---	C1
K	32-9-5	-127	-127	---	FRONT	VERT	SNOW	---	C1
P	28-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	20-11-4	-253	-253	---	FRONT	VERT	TOTAL	---	C1
T	17-0-0	-1406	-1406	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD = 39.0 PSF			

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.43")

CSI: TC=0.57/1.00 (E-F:1), BC=0.70/1.00 (P-Q:1),
WB=0.87/1.00 (C-X:1), SS=0.17/1.00 (Q-S:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (J) (INPUT = 0.90)
JSI METAL = 0.69 (I) (INPUT = 1.00)



Structural component only
DWG# T-2115496

CONTINUED ON PAGE 2

JOB NAME 411017	TRUSS NAME T38	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 10:54:44 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.75
J	TTWW-m	MT20	5.0	8.0	2.50	3.25
K	TTWW+m	MT20	7.0	8.0	Edge	4.75
L	TMVW-p	MT20	5.0	8.0	Edge	
N	BMV1+p	MT20	3.0	5.0		
O	BMWW-t	MT20	5.0	8.0	2.50	3.50
P	BMWW-t	MT20	6.0	9.0		
O, S, W						
O	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
T	BMWWW-t	MT20	5.0	8.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0		
X	BMW1-t	MT20	5.0	8.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	29-11-12	-80	-80	---	FRONT	VERT	TOTAL	---	C1
Z	31-11-12	-82	-82	---	FRONT	VERT	TOTAL	---	C1
AA	18-11-4	-253	-253	---	FRONT	VERT	TOTAL	---	C1
AB	22-11-4	-253	-253	---	FRONT	VERT	TOTAL	---	C1
AC	24-11-4	-253	-253	---	FRONT	VERT	TOTAL	---	C1
AD	26-11-4	-253	-253	---	FRONT	VERT	TOTAL	---	C1
AE	29-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AF	31-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	33-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

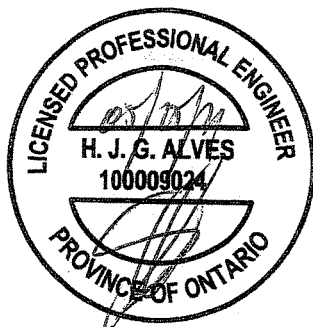
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

- 1) Lateral braces to be a minimum of 2x4 SPF #2

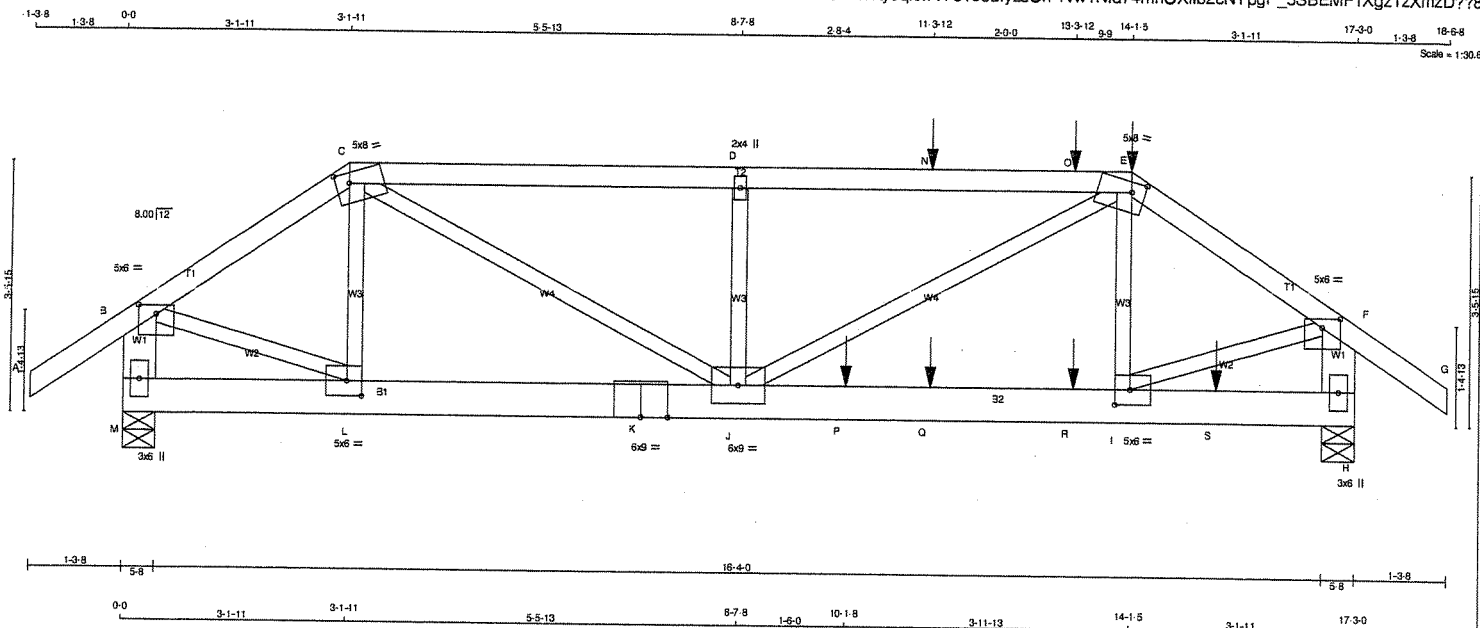


Structural component only
DWG# T-2115496

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411017	T39	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - C	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
M - K	2x6	DRY	No.2
K - H	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	6.0 1.50 3.00
C	TTWW-m	MT20	5.0	8.0 1.75 2.25
D	TMW-w	MT20	2.0	4.0
E	TTWW-m	MT20	5.0	8.0 1.75 2.25
F	TMVW-p	MT20	5.0	6.0 1.50 3.00
H	BMV1-p	MT20	3.0	6.0
I	BMWW-t	MT20	5.0	6.0 2.50 2.50
J	BMWW-t	MT20	6.0	9.0
K	BS-t	MT20	6.0	9.0
L	BMWW-t	MT20	5.0	6.0 2.50 2.50
M	BMV1-p	MT20	3.0	6.0

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
M	1565	0	1565	0	5-8
H	2005	0	2005	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
M	1102	748 / 0	0 / 0
H	1412	959 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 35	-91.8	-91.8	0.14 (1)	L-C	-375 / 0	0.08 (1)
B-C	-1548 / 0	-91.8	-91.8	0.21 (1)	C-J	0 / 1835	0.45 (1)
C-D	-2870 / 0	-91.8	-91.8	0.83 (1)	J-D	-702 / 0	0.15 (1)
D-N	-2870 / 0	-91.8	-91.8	0.83 (1)	J-E	0 / 1189	0.29 (1)
N-O	-2870 / 0	-91.8	-91.8	0.83 (1)	I-E	-70 / 87	0.03 (4)
O-E	-2870 / 0	-91.8	-91.8	0.83 (1)	B-L	0 / 1345	0.33 (1)
E-F	-2214 / 0	-91.8	-91.8	0.25 (1)	I-F	0 / 1923	0.48 (1)
F-G	0 / 35	-91.8	-91.8	0.14 (1)			
M-B	-1528 / 0	0.0	0.0	0.11 (1)			
H-F	-2068 / 0	0.0	0.0	0.15 (1)			
M-L	0 / 0	-18.5	-18.5	0.03 (1)			
L-K	0 / 1273	-18.5	-18.5	0.36 (1)			
K-J	0 / 1273	-18.5	-18.5	0.36 (1)			
J-P	0 / 1835	-18.5	-18.5	0.79 (1)			
P-Q	0 / 1835	-18.5	-18.5	0.79 (1)			
Q-R	0 / 1835	-18.5	-18.5	0.79 (1)			
R-I	0 / 1835	-18.5	-18.5	0.79 (1)			
I-S	0 / 0	-18.5	-18.5	0.25 (1)			
S-H	0 / 0	-18.5	-18.5	0.25 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	14-1-5	-30	-30	---	FRONT	VERT	DEAD	---	C1
E	14-1-5	-127	-127	---	FRONT	VERT	SNOW	---	C1
N	11-3-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
O	13-3-12	-82	-82	---	FRONT	VERT	TOTAL	---	C1
P	10-1-8	-618	-618	---	FRONT	VERT	TOTAL	---	C1
Q	11-3-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	13-3-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	15-3-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CSI: TC=0.83/1.00 (D-E:1), BC=0.79/1.00 (I-J:1),
WB=0.48/1.00 (F-I:1), SS=0.48/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

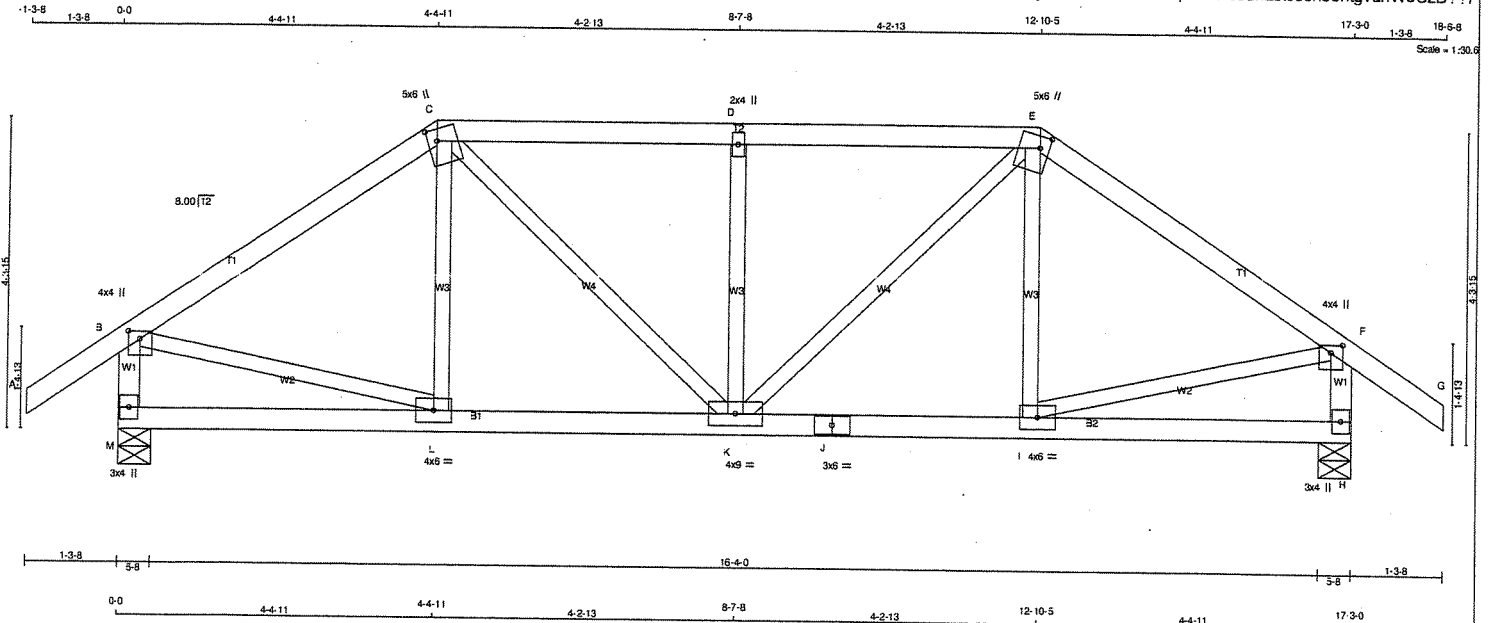
JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.74 (K) (INPUT = 1.00)



Structural component only
DWG# T-2115497

JOB NAME 411017	TRUSS NAME T40	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	BRG	IN-SX
M	1077	0	1077	0	0	248	0	5-8	5-8
H	1077	0	1077	0	0	248	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	759	512	0	0	0	248	0
H	759	512	0	0	0	248	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/35	-91.8	-91.8 0.12 (1)	10.00	L-C	-96/38	0.03 (1)
B-C	-946/0	-91.8	-91.8 0.24 (1)	6.09	C-K	0/389	0.09 (1)
C-D	-1066/0	-91.8	-91.8 0.21 (1)	5.86	K-D	-472/0	0.13 (1)
D-E	-1066/0	-91.8	-91.8 0.21 (1)	5.86	K-E	0/389	0.09 (1)
E-F	-946/0	-91.8	-91.8 0.24 (1)	6.09	I-E	-96/38	0.03 (1)
F-G	0/35	-91.8	-91.8 0.12 (1)	10.00	B-L	0/809	0.18 (1)
M-B	-1043/0	0.0	0.0 0.11 (1)	7.69	I-F	0/809	0.18 (1)
H-F	-1043/0	0.0	0.0 0.11 (1)	7.69			
M-L	0/0	-18.5	-18.5 0.09 (4)	10.00			
L-K	0/784	-18.5	-18.5 0.17 (1)	10.00			
K-J	0/784	-18.5	-18.5 0.17 (1)	10.00			
J-I	0/784	-18.5	-18.5 0.17 (1)	10.00			
I-H	0/0	-18.5	-18.5 0.09 (4)	10.00			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMVWW-t	MT20	4.0	9.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

NOTES-

1) Lateral braces to be a minimum of 2x4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CS1: TC=0.24/1.00 (B-C:1), BC=0.17/1.00 (K-L:1), WB=0.18/1.00 (B-L:1), SS=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.26 (J) (INPUT = 1.00)

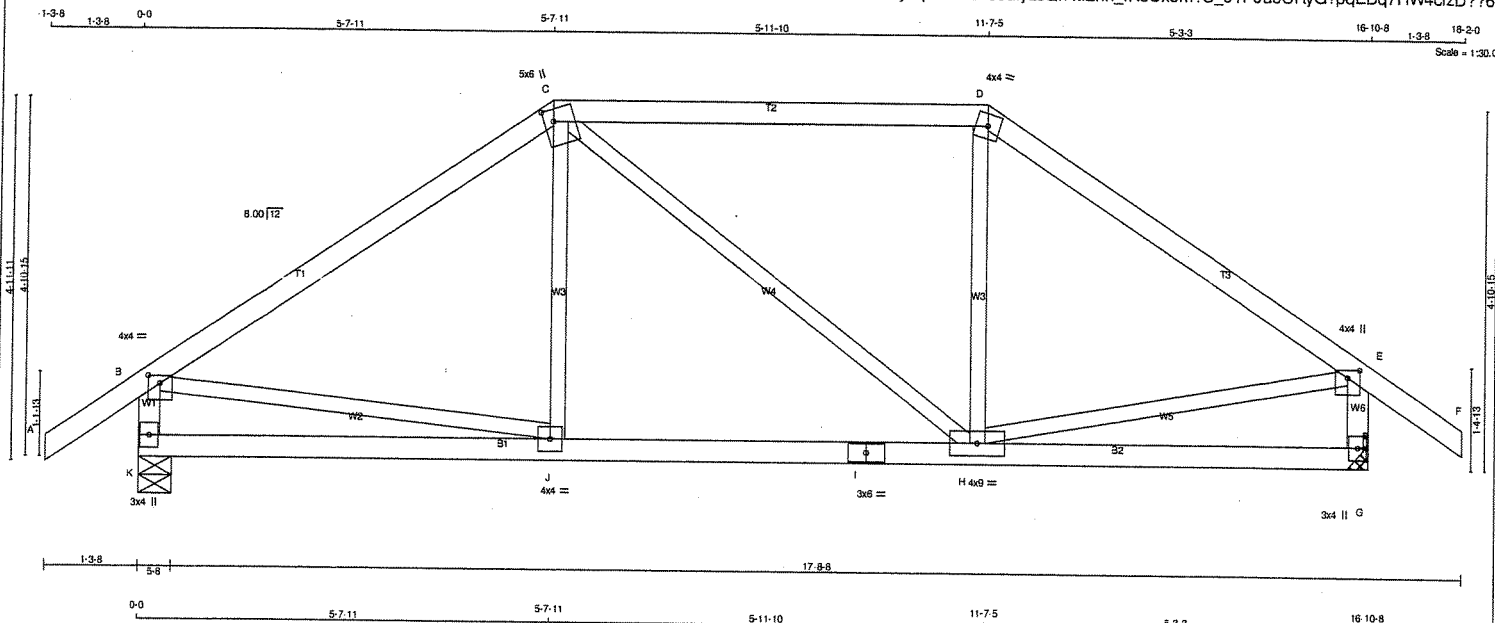


Structural component only
DWG# T-2115498

JOB NAME 411017	TRUSS NAME T41	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.		
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWV+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
K	1056	0	1056	0	5-8
G	1056	0	1056	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
K	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
K	745	502 / 0	0 / 0	0 / 0	0 / 0	243 / 0	0 / 0	
G	745	502 / 0	0 / 0	0 / 0	0 / 0	243 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	J-C	0 / 92	0.03 (4)	
B-C	-929 / 0	-91.8	-91.8 0.39 (1)	C-H	-33 / 0	0.04 (1)	
C-D	-746 / 0	-91.8	-91.8 0.43 (1)	H-D	-16 / 81	0.03 (4)	
D-E	-898 / 0	-91.8	-91.8 0.34 (1)	B-J	0 / 780	0.18 (1)	
E-F	0 / 35	-91.8	-91.8 0.12 (1)	H-E	0 / 761	0.17 (1)	
K-B	-1013 / 0	0.0	0.0 0.10 (1)				
G-E	-1015 / 0	0.0	0.0 0.10 (1)				
K-J	0 / 0	-18.5	-18.5 0.14 (4)				
J-I	0 / 772	-18.5	-18.5 0.21 (4)				
I-H	0 / 772	-18.5	-18.5 0.21 (4)				
H-G	0 / 0	-18.5	-18.5 0.13 (4)				

TOTAL WEIGHT = 68 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.43/1.00 (C-D:1), BC=0.21/1.00 (H-J:4), WB=0.18/1.00 (B-J:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747

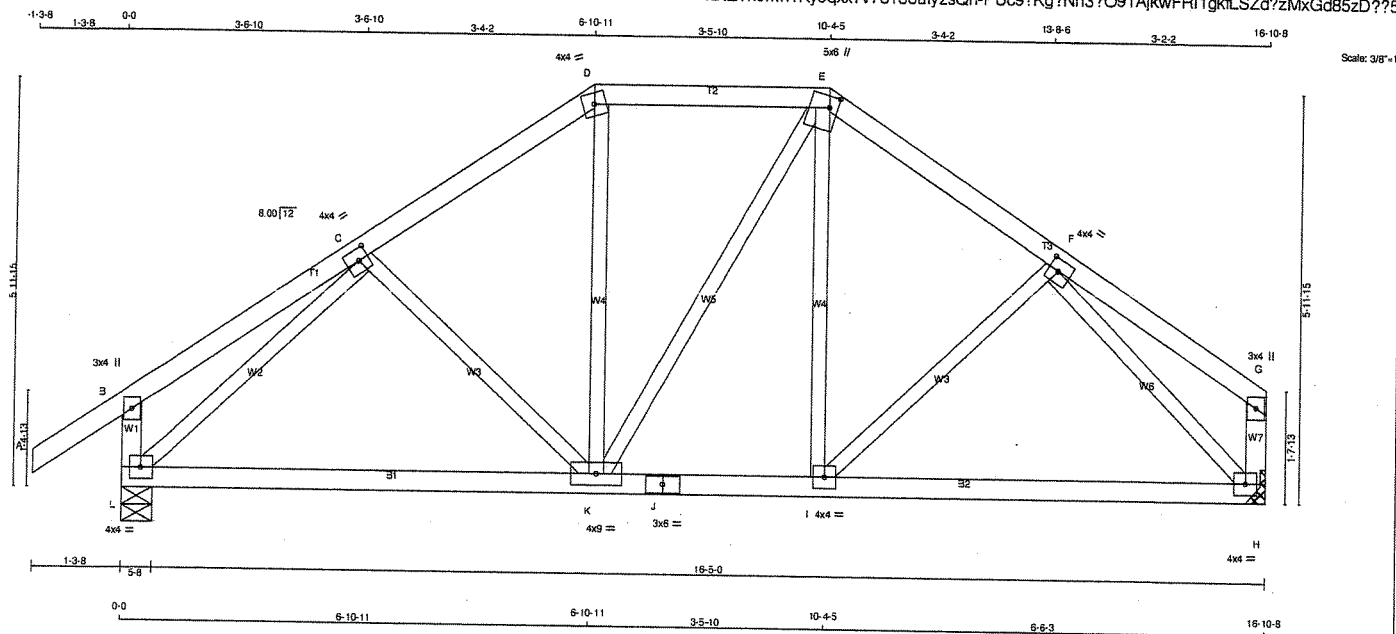
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115499



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWV-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWV+m	MT20	5.0	6.0	2.00	1.50
F	TMWV-t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWV-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMVWV-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	4.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
L	1056	0	1056	0	5-8	5-8
H	930	0	930	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	745	502 / 0	0 / 0	0 / 0	0 / 0	243 / 0	0 / 0
H	658	432 / 0	0 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	C-K	-126 / 3	0.05 (1)
B-C	0 / 20	-91.8 -91.8	0.17 (1)	10.00	K-D	0 / 171	0.04 (1)
C-D	-834 / 0	-91.8 -91.8	0.14 (1)	6.25	K-E	0 / 28	0.01 (1)
D-E	-679 / 0	-91.8 -91.8	0.15 (1)	6.25	E-F	0 / 134	0.03 (4)
E-F	-815 / 0	-91.8 -91.8	0.12 (1)	6.25	F-G	-54 / 23	0.02 (1)
F-G	0 / 20	-91.8 -91.8	0.15 (1)	10.00	L-C	-1075 / 0	0.40 (1)
L-B	-248 / 0	0.0 0.0	0.03 (1)	7.81	F-H	-1044 / 0	0.34 (1)
H-G	-105 / 0	0.0 0.0	0.01 (1)	7.81			
L-K	0 / 768	-18.5 -18.5	0.24 (4)	10.00			
K-J	0 / 665	-18.5 -18.5	0.23 (4)	10.00			
J-I	0 / 665	-18.5 -18.5	0.23 (4)	10.00			
I-H	0 / 702	-18.5 -18.5	0.22 (4)	10.00			

TOTAL WEIGHT = 75 lb [MIF]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL)= L/999 (0.09")

CSI: TC=0.17/1.00 (B-C:1), BC=0.24/1.00 (K-L:4), WB=0.40/1.00 (C-L:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

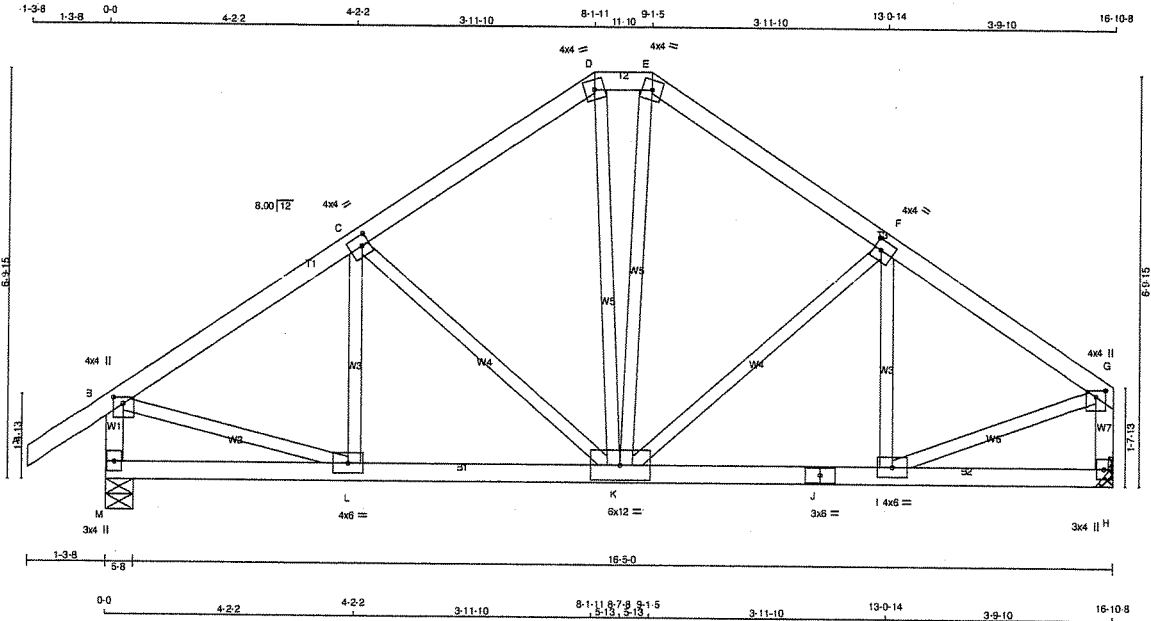
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (L) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)



JOB NAME 411017	TRUSS NAME T43	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 78 lb
(M/F)

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMVWWW-t	MT20	6.0	12.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
M	1056	0	1056	0
H	930	0	930	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	745	502 / 0	0 / 0	0 / 0	0 / 0	243 / 0	0 / 0
H	658	432 / 0	0 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	L-C	-126 / 27	0.03 (1)
B-C	-949 / 0	-91.8	-91.8 0.20 (1)	6.13	C-K	-292 / 0	0.17 (1)
C-D	-734 / 0	-91.8	-91.8 0.20 (1)	6.25	K-F	-222 / 0	0.13 (1)
D-E	-605 / 0	-91.8	-91.8 0.01 (1)	6.25	I-F	-185 / 4	0.05 (1)
E-F	-732 / 0	-91.8	-91.8 0.18 (1)	6.25	B-L	0 / 836	0.19 (1)
F-G	-886 / 0	-91.8	-91.8 0.18 (1)	6.25	I-G	0 / 802	0.18 (1)
M-B	-1023 / 0	0.0	0.0 0.11 (1)	7.75	D-K	0 / 214	0.05 (1)
H-G	-901 / 0	0.0	0.0 0.10 (1)	7.81	K-E	0 / 210	0.05 (1)
M-L	0 / 0	-18.5	-18.5 0.07 (4)	10.00			
L-K	0 / 809	-18.5	-18.5 0.17 (1)	10.00			
K-J	0 / 757	-18.5	-18.5 0.16 (1)	10.00			
J-I	0 / 757	-18.5	-18.5 0.16 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.20/1.00 (B-C:1), BC=0.17/1.00 (K-L:1),
WB=0.19/1.00 (B-L:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

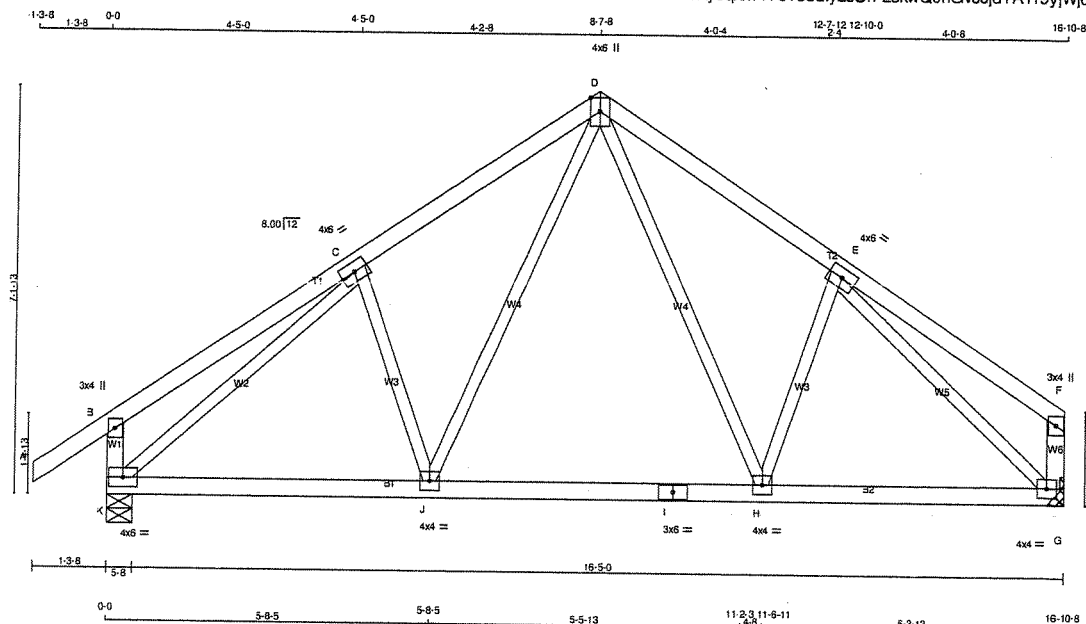
JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115501

JOB NAME 411017	TRUSS NAME T44	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/32"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
K	1056	0	1056	0
G	930	0	930	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	745	502 / 0	0 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	658	432 / 0	0 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	C-J	-245 / 0	0.07 (1)
B-C	0 / 26	-91.8	-91.8 0.27 (1)	10.00	J-D	0 / 379	0.09 (1)
C-D	-890 / 0	-91.8	-91.8 0.22 (1)	6.25	D-H	0 / 307	0.07 (1)
D-E	-854 / 0	-91.8	-91.8 0.20 (1)	6.25	H-E	-181 / 0	0.05 (1)
E-F	0 / 26	-91.8	-91.8 0.25 (1)	10.00	K-C	-1089 / 0	0.62 (1)
K-B	-276 / 0	0.0	0.0 0.03 (1)	7.81	E-G	-1068 / 0	0.54 (1)
G-F	-133 / 0	0.0	0.0 0.01 (1)	7.81			
K-J	0 / 802	-18.5	-18.5 0.21 (4)	10.00			
J-I	0 / 570	-18.5	-18.5 0.19 (4)	10.00			
I-H	0 / 570	-18.5	-18.5 0.19 (4)	10.00			
H-G	0 / 753	-18.5	-18.5 0.19 (1)	10.00			

TOTAL WEIGHT = 74 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.27/1.00 (B-C:1), BC=0.21/1.00 (J-K:4),
WB=0.62/1.00 (C-K:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (G) (INPUT = 0.90)
JSI METAL = 0.26 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115502

Tamarack Roof Truss, Burlington

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DRY; SEASONED LUMBER.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J. G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	I-C	-104 / 34 0.02 (1)
B-C	-620 / 0	-91.8	-91.8	0.13 (1)	8.25	C-H	0 / 21 0.01 (1)
C-K	-528 / 0	-91.8	-91.8	0.45 (1)	8.25	H-D	-119 / 34 0.02 (1)
K-L	-528 / 0	-91.8	-91.8	0.45 (1)	8.25	B-I	0 / 551 0.14 (1)
L-D	-528 / 0	-91.8	-91.8	0.45 (1)	8.25	H-E	0 / 570 0.14 (1)
D-E	-642 / 0	-91.8	-91.8	0.13 (1)	8.25		
E-F	0 / 35	-91.8	-91.8	0.14 (1)	10.00		
J-B	-793 / 0	0.0	0.0	0.09 (1)	7.81		
G-E	-813 / 0	0.0	0.0	0.09 (1)	7.81		
J-M	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
M-I	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
I-N	0 / 510	-18.5	-18.5	0.13 (1)	10.00		
N-O	0 / 510	-18.5	-18.5	0.13 (1)	10.00		
O-H	0 / 510	-18.5	-18.5	0.13 (1)	10.00		
H-P	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
P-G	0 / 0	-18.5	-18.5	0.07 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-8-8	-56	-56	100	FRONT	VERT	TOTAL	---	C1
D	7-3-8	-56	-56	100	FRONT	VERT	TOTAL	---	C1
H	7-2-12	-5	-5	---	FRONT	VERT	TOTAL	---	C1
H	2-9-4	-5	-5	---	FRONT	VERT	TOTAL	---	C1
K	4-9-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
K	6-9-4	-38	-38	---	FRONT	VERT	TOTAL	---	C1
M	2-0-12	-7	-7	---	FRONT	VERT	TOTAL	---	C1
N	4-9-4	-5	-5	---	FRONT	VERT	TOTAL	---	C1
O	6-9-4	-5	-5	---	FRONT	VERT	TOTAL	---	C1
O	7-11-4	-7	-7	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.45/1.00 (C-D:1), BC=0.13/1.00 (H-I:1),
WB=0.14/1.00 (E-H:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

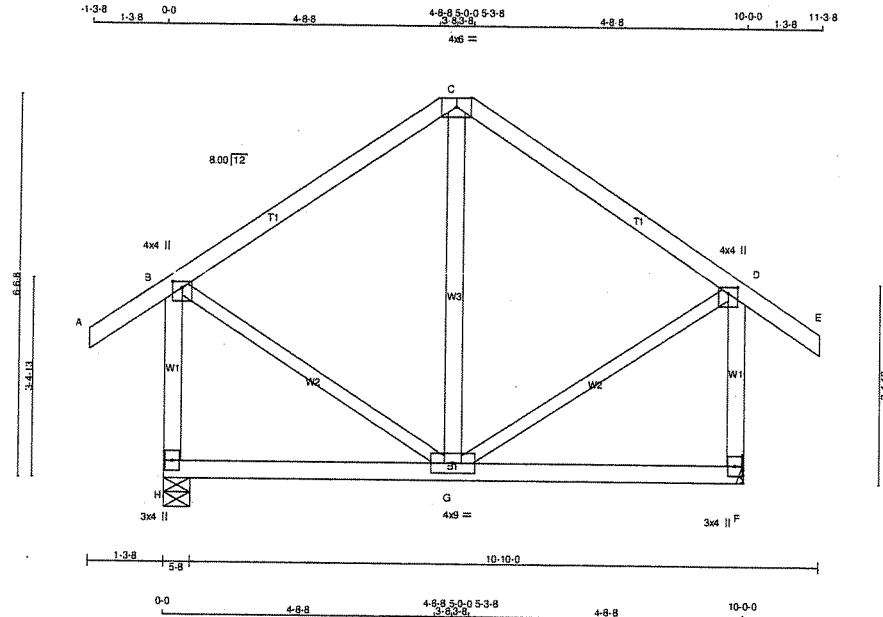
JSI GRIP= 0.61 (I) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)



Structural component only
DWG# T-2115503

JOB NAME 411017	TRUSS NAME T46	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/2\"/>

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
G - C	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
C	TMTWm	MT20	4.0	6.0	Edge
O	TMVW+p	MT20	4.0	4.0	1.25 2.00
F	BMV1+p	MT20	3.0	4.0	
G	BMVW+t	MT20	4.0	9.0	
H	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
H	677	0	677	0	0	5-8	5-8		
F	677	0	677	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	477	326 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0
F	477	326 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO					FR-TO		
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	B-G	0 / 262	0.06 (1)
B-C	-268 / 0	-91.8	-91.8 0.29 (1)	6.25	G-D	0 / 262	0.06 (1)
C-D	-268 / 0	-91.8	-91.8 0.29 (1)	6.25	G-C	-161 / 27	0.08 (1)
D-E	0 / 35	-91.8	-91.8 0.12 (1)	10.00			
H-B	-642 / 0	0.0	0.0 0.13 (1)	7.81			
F-D	-642 / 0	0.0	0.0 0.13 (1)	7.81			
H-G	0 / 0	-18.5	-18.5 0.13 (4)	10.00			
G-F	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

TOTAL WEIGHT = 51 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.29/1.00 (B-C:1), BC=0.13/1.00 (G-H:4), WB=0.08/1.00 (C-G:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(ORY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115504



TOTAL WEIGHT = 53 lb

DESCR.	SPF	SPF	SPF	SPF	SPF

DRY: SEASONED LUMBER.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

BUILDING BEARINGS

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

1ST LCASE MAX./MIN. COMPONENT REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) !

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. (PLF)	LOAD LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	H-C	-154 / 0	0.07 (1)
B-C	-285 / 0	-91.8	-91.8 0.12 (1)	6.25	C-G	0 / 0	0.00 (1)
C-D	-233 / 0	-91.8	-91.8 0.16 (1)	6.25	G-D	-155 / 0	0.07 (1)
D-E	-284 / 0	-91.8	-91.8 0.12 (1)	6.25	B-H	0 / 325	0.07 (1)
I-B	-653 / 0	0.0	0.0 0.13 (1)	7.81	G-E	0 / 324	0.07 (1)
F-E	-526 / 0	0.0	0.0 0.10 (1)	7.81			
I-H	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
H-G	0 / 233	-18.5	-18.5 0.07 (4)	10.00			
G-F	0 / 0	-18.5	-18.5 0.05 (4)	10.00			

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 0B6-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (0.33")
CALCULATED VERT. DEFL(LL) = L/ 999 (0.00")
ALLOWABLE DEFL(TL)= L/360 (0.33")
CALCULATED VERT. DEFL(TL) = L/ 999 (0.01")

CSI: TC=0.16/1.00 (C-D:1), BC=0.07/1.00 (G-H:4),
WB=0.07/1.00 (B-H:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

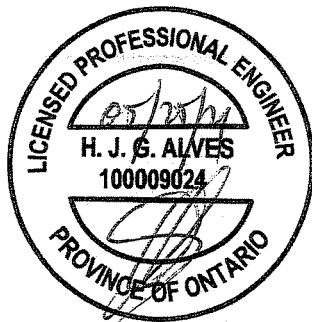
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

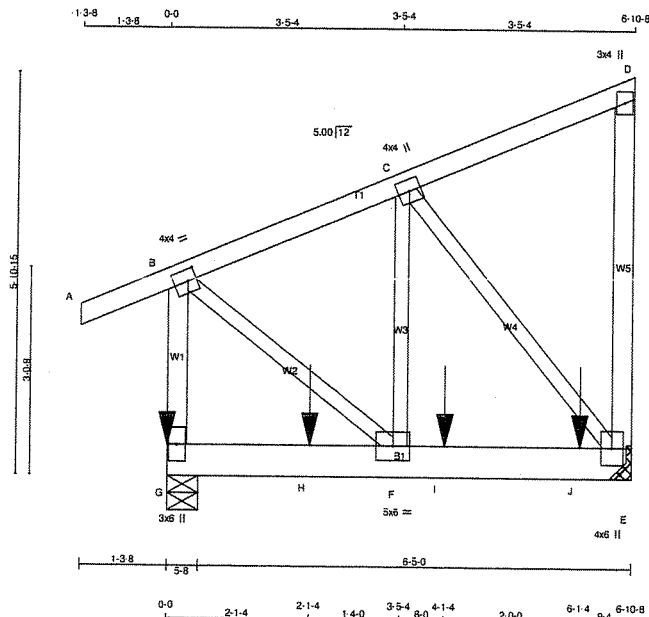
JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115505

JOB NAME 411017	TRUSS NAME T48	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/32"

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
G - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
SPACING (IN)		
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
G-B 1 12	TOP	
A-D 1 12	TOP	
D-E 1 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G-E 2 12	SIDE(0.0)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.50
C TMVW-t	MT20	4.0	4.0		
D TMV-p	MT20	3.0	4.0		
E BMVW-t-p	MT20	4.0	6.0		
F BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	DOWN	UP	BRG	BRG
JT VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
G 2651 0	2651 0	0 0	5-8	5-8	
E 2014 0	2014 0	0 0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
G 1867	1269 / 0	0 / 0	0 / 0	0 / 0	598 / 0	0 / 0	
E 1421	949 / 0	0 / 0	0 / 0	0 / 0	472 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
G-B	-1387 / 0	G-B	0 / 1211
A-B	0 / 24	A-B	0 / 1155
B-C	-1024 / 0	C-E	-1489 / 0
C-D	-13 / 0		
E-D	-123 / 0		
G-H	0 / 0		
H-F	0 / 0		
F-I	0 / 958		
I-J	0 / 958		
J-E	0 / 958		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	0-0		-736		BACK	VERT	TOTAL		C1
H	2-1-4		-643		BACK	VERT	TOTAL		C1
I	4-1-4		-643		BACK	VERT	TOTAL		C1
J	6-1-4		-645		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 41 = 83 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.11/1.00 (B-G:1), BC=0.27/1.00 (E-F:1),
WB=0.34/1.00 (C-E:1), SSI=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.79 (C) (INPUT = 0.90)
JSI METAL = 0.22 (E) (INPUT = 1.00)



Structural component only
DWG# T-2115506

CONTINUED ON PAGE 2

JOB NAME 411017	TRUSS NAME T48	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 10:54:54 2021 Page 2
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PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
G BMV1+p MT20 3.0 6.0

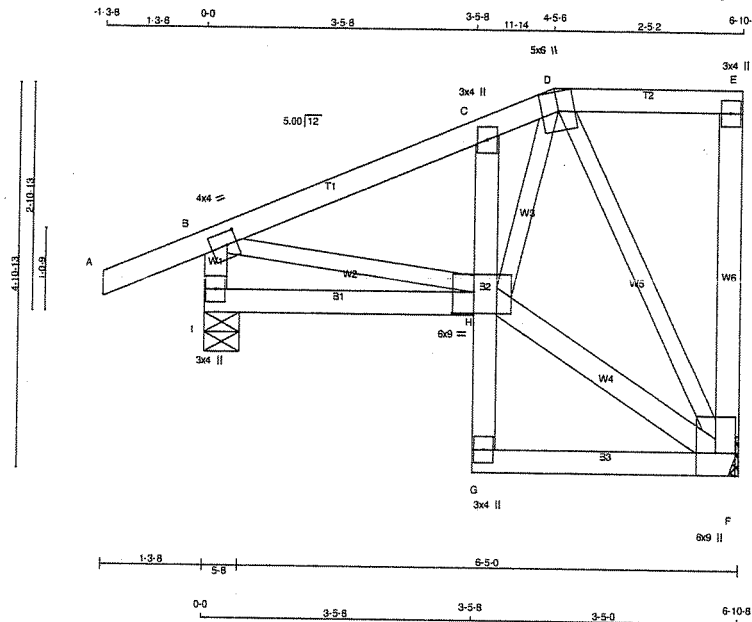
NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115506 *en*

JOB NAME 411017	TRUSS NAME T49S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version B.420 S Jan 21 2021 Mittek Industries, Inc. Tue May 25 10:54:55 2021 Page 1
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Scale = 1/28.0

TOTAL WEIGHT = 40 lb
[MIF]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - C	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS EXCEPT H - F	2x3	DRY	No.2
	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVWW1+p	MT20	6.0	9.0	Edge	
G	BMV+p	MT20	3.0	4.0		
H	BMVWW-I	MT20	6.0	9.0	3.25	3.25
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	379	0	379	0	MECHANICAL	5-8
I	503	0	503	0	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. PERM. LIVE	MAX./MIN. WIND	MAX./MIN. DEAD	MAX./MIN. SOIL
F	268	176 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
I	353	245 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO
A-B	0 / 24	-91.8 -91.8 0.12 (1)	10.00	H-F 0 / 198 0.03 (1)
B-C	-354 / 0	-91.8 -91.8 0.13 (1)	6.25	H-D 0 / 448 0.10 (1)
C-D	-386 / 0	-91.8 -91.8 0.10 (1)	6.25	D-F -386 / 0 0.16 (1)
D-E	0 / 0	-91.8 -91.8 0.09 (1)	10.00	B-H 0 / 344 0.08 (1)
F-E	-110 / 0	0.0 0.0 0.04 (1)	7.81	
I-B	-469 / 0	0.0 0.0 0.05 (1)	7.81	
I-H	0 / 0	-18.5 -18.5 0.07 (4)	10.00	
G-H	0 / 34	0.0 0.0 0.03 (1)	10.00	
H-C	-328 / 0	0.0 0.0 0.02 (1)	7.81	
G-F	0 / 9	-18.5 -18.5 0.06 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13/1.00 (B-C:1), BC=0.07/1.00 (H-I:4),
WB=0.16/1.00 (D-F:1), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

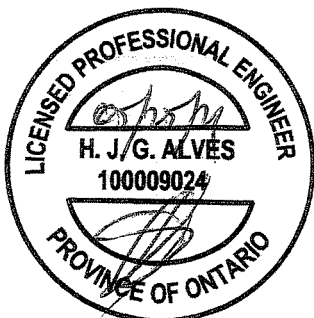
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.46 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115507

JOB NAME 411069	TRUSS NAME T50	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 11:28:23 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	3.00
G	TMWWWW-t	MT20	8.0	12.0	3.75	4.75
G	TP-m	MT20	4.0	9.0	33.75	3.50
H	TMBW1-l	MT20	8.0	12.0	3.75	3.25
I	BMW+w	MT20	3.0	6.0		
J, N, O						
J	BMW-t	MT20	5.0	6.0		
K	BS-t	MT20	6.0	9.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	6.0	9.0		
P	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

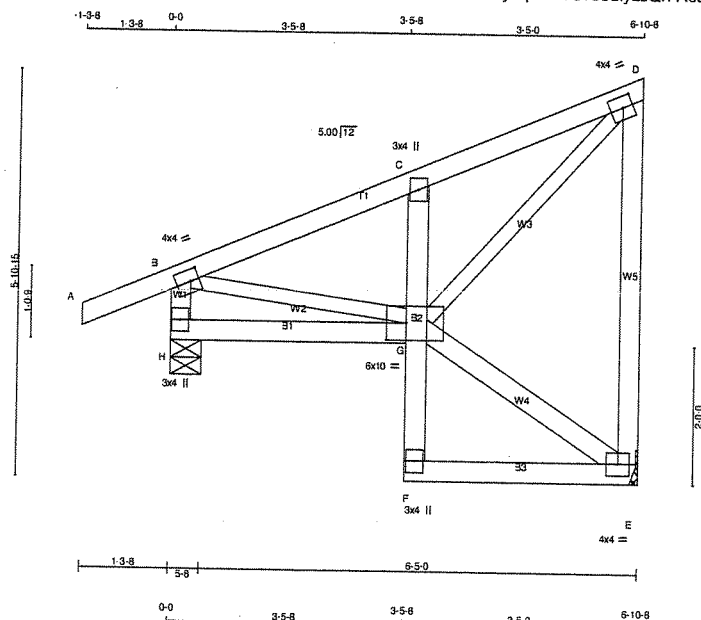


Structural component only
DWG# T-2115518 2/1

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411017	T50S	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale: 3/8\"/>

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - D	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
G - E	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMW1-t	MT20	6.0	10.0	3.00	3.50
H	BMV1+p	MT20	3.0	4.0		

NOTES-

- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ
H	503	0	503	0
E	379	0	379	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	353	245 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
E	268	176 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO	H-B	-469 / 0	0.0	0.0	0.05 (1)	7.81	B-G	0 / 356	0.08 (1)
	A-B	0 / 24	-91.8	-91.8	0.12 (1)	10.00	G-E	-11 / 0	0.00 (1)
	B-C	-365 / 0	-91.8	-91.8	0.13 (1)	6.25	G-D	0 / 493	0.11 (1)
	C-D	-373 / 0	-91.8	-91.8	0.13 (1)	6.25			
	E-D	-343 / 0	0.0	0.0	0.22 (1)	7.81			
	H-G	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
	F-G	0 / 34	0.0	0.0	0.03 (1)	10.00			
	G-C	-379 / 0	0.0	0.0	0.02 (1)	7.81			
	F-E	0 / 9	-18.5	-18.5	0.06 (4)	10.00			

TOTAL WEIGHT = 4 X 39 = 157 lb

[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.23")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL)= $L/360$ (0.23")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TG=0.22/1.00 (D-E:1), BC=0.07/1.00 (G-H:4), WB=0.11/1.00 (D-G:1), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

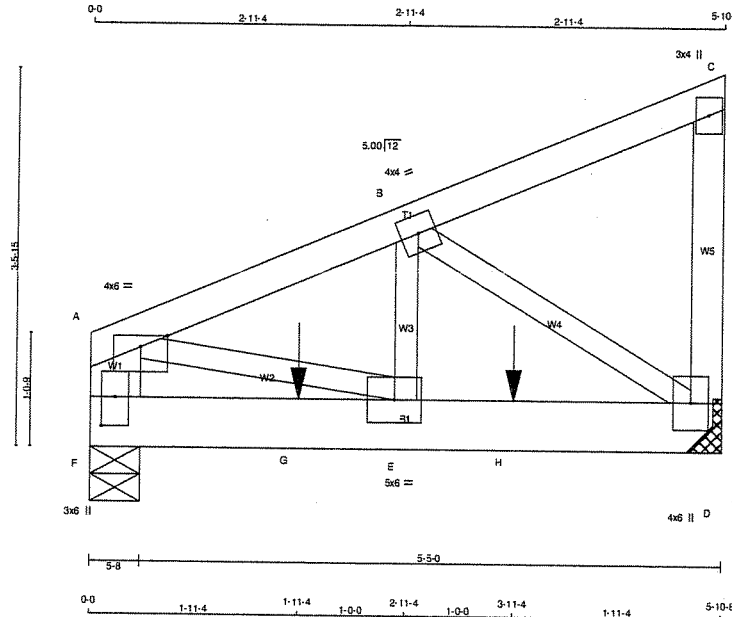
JSI GRIP= 0.49 (D) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115508

JOB NAME 411017	TRUSS NAME T51	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:20.3

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 2	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
F	941	0	941	0	5-8	5-8
D	898	0	898	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	662	453 / 0	0 / 0	0 / 0	0 / 0	209 / 0	0 / 0
D	633	429 / 0	0 / 0	0 / 0	0 / 0	203 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX	
FR-TO		FROM TO		FR-TO			
F-A	-758 / 0	0.0	0.0	0.03 (1)	7.81	A-E	0 / 1035
A-B	-1085 / 0	-91.8	-91.8	0.06 (1)	6.25	E-B	0 / 751
B-C	-9 / 0	-91.8	-91.8	0.05 (1)	10.00	B-D	-1200 / 0
D-C	-110 / 0	0.0	0.0	0.01 (1)	7.81		
F-G	0 / 0	-18.5	-18.5	0.12 (1)	10.00		
G-E	0 / 0	-18.5	-18.5	0.12 (1)	10.00		
E-H	0 / 1010	-18.5	-18.5	0.15 (1)	10.00		
H-D	0 / 1010	-18.5	-18.5	0.15 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-462	-462	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	-375	-375	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 28 = 55 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.15/1.00 (D-E:1), WB=0.13/1.00 (B-D:1), SS=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.57 (B) (INPUT = 0.90)
JSI METAL = 0.23 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115509

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411017	T51	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 10:54:57 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0	3.25	1.50

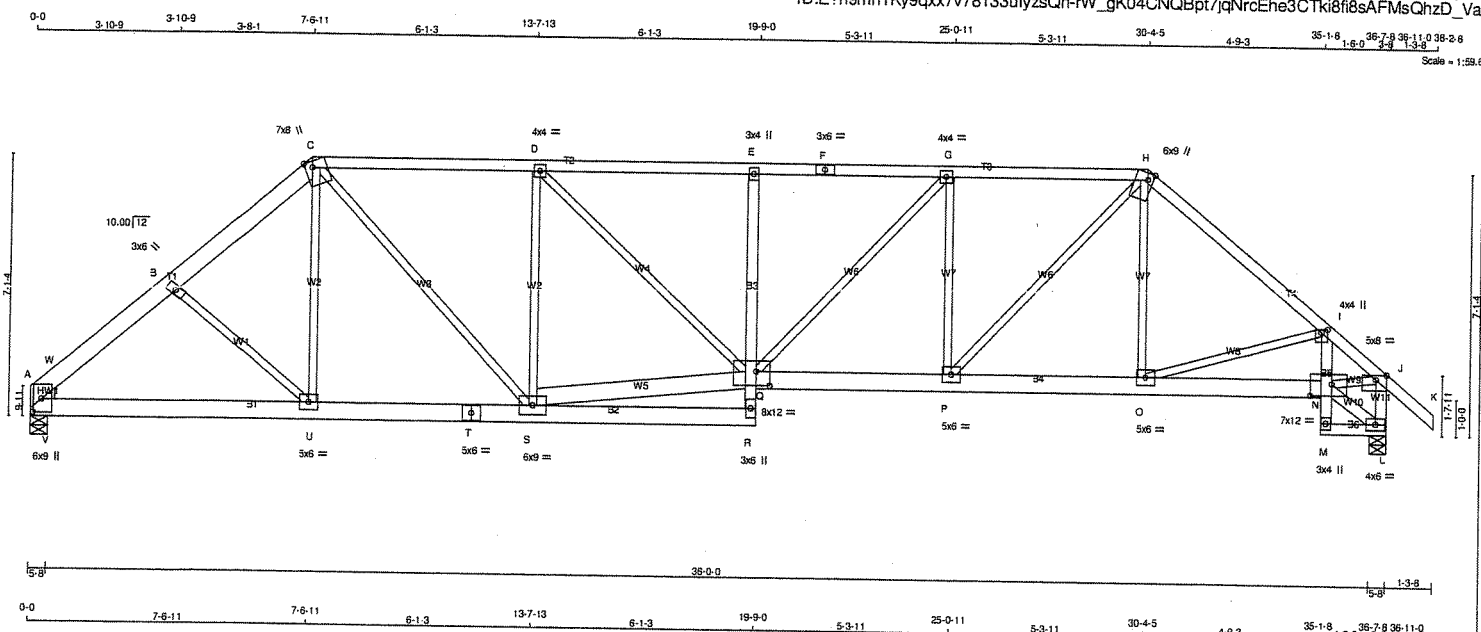
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115509 *mn*



TOTAL WEIGHT = 199 lb

DESCR.
SPF
SPF
SPF
SPF
SPF
SPF
SPF
SPF
SPF
SPF
SPF
SPF

ALL WEBS	2x3	DRY
EXCEPT		
S - O	2x6	DRY
N - L	2x4	DRY

DRY: SEASONED LUMBER

J	T	PLATES	W	L	E	N	Y	X
A	TMBH1+l	MT20	6.0	9.0	4.25	2.75		
B	TMW+w	MT20	3.0	6.0				
C	T1TWW-m	MT20	7.0	8.0	Edge	2.25		
D	TMWW+t	MT20	4.0	4.0				
E	TMV+p	MT20	3.0	4.0				
F	TS-t	MT20	3.0	6.0				
G	TMWW-t	MT20	4.0	4.0				
H	T1TWW-m	MT20	6.0	9.0	Edge	1.75		
I	TMWV+p	MT20	4.0	4.0	1.00	2.00		
J	TMWV-p	MT20	5.0	8.0	1.50	3.50		
L	BMVW-1-t	MT20	4.0	6.0				
M	BMV+p	MT20	3.0	4.0				
N	BVMWV-l	MT20	7.0	12.0	3.75	7.00		
O	P							
A	BMWV-t	MT20	5.0	6.0				
O	BVMWVW-l	MT20	8.0	12.0	4.50	4.50		
R	BMV+p	MT20	3.0	6.0				
S	BMWVW-t	MT20	6.0	9.0				
T	BS-t	MT20	5.0	6.0				

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
A	2033	0	2033	0	0	5-8	5-8	2x4 L
L	2165	0	2165	0	0	5-8	5-8	

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1438	943 / 0	0 / 0	0 / 0	0 / 0	494 / 0	0 / 0
L	1528	1017 / 0	0 / 0	0 / 0	0 / 0	512 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A. L

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-W	-3019 / 0	-91.8	-91.8 0.21 (1)	4.65	B-U	-219 / 0	0.09 (1)
W-B	-2659 / 0	-91.8	-91.8 0.21 (1)	4.90	U-C	0 / 247	0.06 (4)
B-C	-2416 / 0	-91.8	-91.8 0.14 (1)	5.18	C-S	0 / 1204	0.27 (1)
C-D	-2649 / 0	-91.8	-91.8 0.73 (1)	3.50	S-D	-1202 / 0	0.98 (1)
D-E	-3281 / 0	-91.8	-91.8 0.82 (1)	3.09	S-Q	0 / 2641	0.31 (1)
E-F	-3291 / 0	-91.8	-91.8 0.62 (1)	3.32	D-O	0 / 859	0.19 (1)
F-G	-3291 / 0	-91.8	-91.8 0.62 (1)	3.32	Q-G	0 / 556	0.13 (1)
G-H	-2911 / 0	-91.8	-91.8 0.58 (1)	3.55	P-G	-953 / 0	0.52 (1)
H-I	-2523 / 0	-91.8	-91.8 0.46 (1)	3.97	P-H	0 / 1441	0.32 (1)
I-J	-2694 / 0	-91.8	-91.8 0.31 (1)	3.91	Q-H	0 / 160	0.04 (4)
J-K	0 / 41	-91.8	-91.8 0.13 (1)	10.00	C-I	-236 / 0	0.11 (1)
L-J	-2104 / 0	0.0	0.0 0.22 (1)	5.84	N-L	-79 / 0	0.01 (1)
					N-J	0 / 2104	0.47 (1)
					V-W	0 / 359	0.00 (1)
A-V	0 / 2003	-18.5	-18.5 0.33 (1)	10.00			
V-U	0 / 2003	-18.5	-18.5 0.33 (1)	10.00			
U-T	0 / 1841	-18.5	-18.5 0.25 (1)	10.00			
T-S	0 / 1841	-18.5	-18.5 0.25 (1)	10.00			
S-R	0 / 56	-18.5	-18.5 0.07 (4)	10.00			
R-O	0 / 51	0.0	0.0 0.08 (1)	10.00			
Q-E	-475 / 0	0.0	0.0 0.22 (1)	7.81			
Q-P	0 / 2911	-18.5	-18.5 0.38 (1)	10.00			
P-O	0 / 1920	-18.5	-18.5 0.26 (1)	10.00			
O-N	0 / 2142	-18.5	-18.5 0.29 (1)	10.00			
M-N	0 / 17	0.0	0.0 0.10 (1)	10.00			
N-I	-175 / 0	0.0	0.0 0.10 (1)	7.81			
M-L	0 / 65	-18.5	-18.5 0.02 (4)	10.00			

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

-OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.23")
CALCULATED VERT. DEFL(LL) = L/ 999 (0.16")
ALLOWABLE DEFL(TL)= L/360 (1.23")
CALCULATED VERT. DEFL(TL) = L/ 999 (0.30")

CSI: TC=0.82/1.00 (D-E:1), BC=0.38/1.00 (P-Q:1),
WB=0.98/1.00 (D-S:1), SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.76 (A) (INPUT = 1.00)



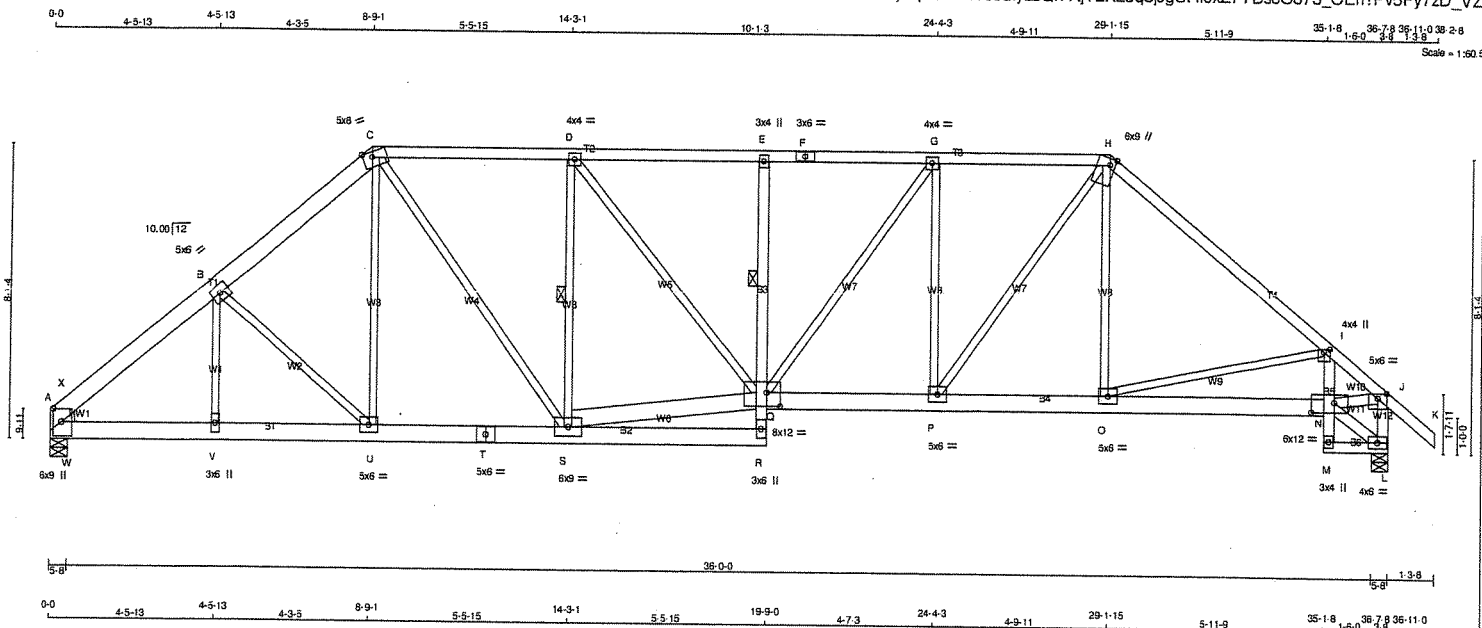
Structural component only
DWG# T-2115519

EWING 7-24-5500		
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411069	T52S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 210 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF	
O - T	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
A - T	2x6	DRY	No.2	SPF	
T - R	2x6	DRY	No.2	SPF	
R - E	2x4	DRY	No.2	SPF	
Q - N	2x6	DRY	No.2	SPF	
M - I	2x4	DRY	No.2	SPF	
M - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
S - Q	2x6	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBHtm	MT20	6.0	9.0	2.75	4.25
B	TMWW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TMV-p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	1.75
I	TMVW-p	MT20	4.0	4.0	1.00	2.00
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMVW-t	MT20	4.0	6.0		
M	BMV-p	MT20	3.0	4.0		
N	BVMWV-t	MT20	6.0	12.0	3.25	7.50
O, P, U						
Q	BMWW-t	MT20	5.0	6.0		
R	BVMWVW-t	MT20	8.0	12.0	4.50	4.50
S	BMV-p	MT20	3.0	6.0		
R	BVMWVW-t	MT20	6.0	9.0		
T	BS-t	MT20	5.0	6.0		
V	BMV-w	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	HEEL
JT	VERT	DOWN	UPLIFT	IN-SX	WEDGE
A	2033	0	0	5-8	5-8
L	2165	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	1438	943 / 0	0 / 0	0 / 0	0 / 0	494 / 0	0 / 0
L	1528	1017 / 0	0 / 0	0 / 0	0 / 0	512 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, D-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-X	-3066 / 0	-91.8	-91.8	0.19 (1)	4.66	V-B	-24 / 53	0.02 (4)
X-B	-2612 / 0	-91.8	-91.8	0.19 (1)	4.97	B-U	-286 / 0	0.17 (1)
B-C	-2347 / 0	-91.8	-91.8	0.15 (1)	5.24	U-C	0 / 324	0.07 (1)
C-D	-2336 / 0	-91.8	-91.8	0.54 (1)	3.92	C-S	0 / 956	0.22 (1)
D-E	-2790 / 0	-91.8	-91.8	0.59 (1)	3.58	S-D	-1103 / 0	0.44 (1)
E-F	-2796 / 0	-91.8	-91.8	0.44 (1)	3.75	S-Q	0 / 2344	0.28 (1)
F-G	-2796 / 0	-91.8	-91.8	0.42 (1)	3.94	D-O	0 / 714	0.16 (1)
G-H	-2531 / 0	-91.8	-91.8	0.42 (1)	3.94	O-H	0 / 196	0.05 (4)
H-I	-2451 / 0	-91.8	-91.8	0.77 (1)	3.80	O-I	-385 / 0	0.29 (1)
I-J	-2759 / 0	-91.8	-91.8	0.50 (1)	3.58	N-L	-73 / 0	0.01 (1)
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	N-J	0 / 2194	0.35 (1)
L-J	-2107 / 0	0.0	0.0	0.22 (1)	5.84	P-H	0 / 1154	0.26 (1)
						Q-G	0 / 459	0.10 (1)
A-W	0 / 1989	-18.5	-18.5	0.35 (1)	10.00	P-G	-859 / 0	0.70 (1)
W-V	0 / 1989	-18.5	-18.5	0.35 (1)	10.00	W-X	0 / 449	0.00 (1)
V-U	0 / 1989	-18.5	-18.5	0.30 (1)	10.00			
U-T	0 / 1783	-18.5	-18.5	0.25 (1)	10.00			
T-S	0 / 1783	-18.5	-18.5	0.25 (1)	10.00			
S-R	0 / 40	-18.5	-18.5	0.06 (4)	10.00			
R-Q	0 / 45	0.0	0.0	0.06 (1)	10.00			
Q-E	-425 / 0	0.0	0.0	0.07 (1)	6.25			
Q-P	0 / 2531	-18.5	-18.5	0.33 (1)	10.00			
P-O	0 / 1859	-18.5	-18.5	0.26 (1)	10.00			
O-N	0 / 2228	-18.5	-18.5	0.30 (1)	10.00			
M-N	0 / 17	0.0	0.0	0.10 (1)	10.00			
N-I	-173 / 0	0.0	0.0	0.09 (1)	7.81			
M-L	0 / 60	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.77/1.00 (H-I:1), BC=0.35/1.00 (V-W:1), WB=0.70/1.00 (G-P:1), SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

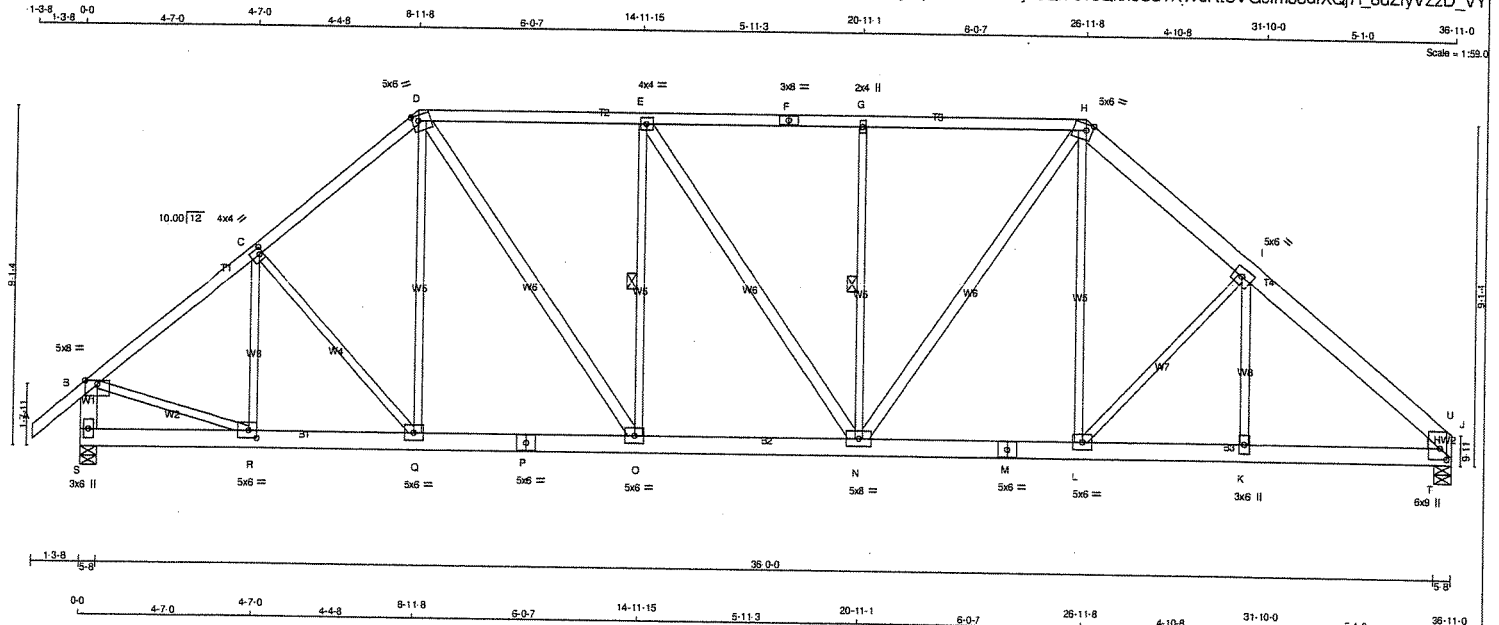
JSI GRIP = 0.89 (J) (INPUT = 0.90)
JSI METAL = 0.76 (A) (INPUT = 1.00)



Structural component only
DWG# T-2115520

JOB NAME 411069	TRUSS NAME T53	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x6	DRY	No.2
S - B	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
M - J	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - O	2x4	DRY	No.2
E - N	2x4	DRY	No.2
N - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTVW-m	MT20	5.0	6.0	1.75	2.00
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTVW-m	MT20	5.0	6.0	2.00	2.00
I	TMVW-t	MT20	5.0	6.0		
J	TMBH-t	MT20	6.0	9.0	3.50	2.00
K	BMVW-w	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-w	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
R	BMVW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
JT	VERT	2162	2162	0	5-8	
S	HORZ	0	0	0	5-8	
J	UPLIFT	2035	2035	0	5-8	2x4 R

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1527	1015 / 0	0 / 0	0 / 0	0 / 0	511 / 0	0 / 0
J	1439	944 / 0	0 / 0	0 / 0	0 / 0	495 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O, G-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MEMB.	FORCE (LBS)	MAX UNBRAC LENGTH	MAX CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	R-C	-377 / 0	0.16 (1)
B-C	-2130 / 0	-91.8	-91.8 0.31 (1)	4.40	C-Q	-106 / 0	0.09 (1)
C-D	-2105 / 0	-91.8	-91.8 0.30 (1)	4.43	Q-D	0 / 190	0.05 (4)
D-E	-2118 / 0	-91.8	-91.8 0.46 (1)	4.21	D-O	0 / 925	0.15 (1)
E-F	-2152 / 0	-91.8	-91.8 0.42 (1)	4.23	O-E	-645 / 0	0.33 (1)
F-G	-2152 / 0	-91.8	-91.8 0.42 (1)	4.23	E-N	0 / 59	0.01 (1)
G-H	-2151 / 0	-91.8	-91.8 0.46 (1)	4.18	N-G	-574 / 0	0.30 (1)
H-I	-2270 / 0	-91.8	-91.8 0.12 (1)	5.33	N-H	0 / 757	0.12 (1)
I-U	-2580 / 0	-91.8	-91.8 0.17 (1)	5.03	L-H	0 / 416	0.09 (1)
U-J	-2982 / 0	-91.8	-91.8 0.17 (1)	4.74	L-I	-383 / 0	0.30 (1)
S-B	-2115 / 0	0.0	0.0 0.14 (1)	6.99	K-I	0 / 76	0.03 (4)
					B-R	0 / 1718	0.39 (1)
					T-U	0 / 336	0.00 (1)
S-R	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
R-Q	0 / 1660	-18.5	-18.5 0.23 (1)	10.00			
O-P	0 / 1591	-18.5	-18.5 0.22 (1)	10.00			
P-O	0 / 1591	-18.5	-18.5 0.22 (1)	10.00			
O-N	0 / 2118	-18.5	-18.5 0.28 (1)	10.00			
N-M	0 / 1718	-18.5	-18.5 0.25 (1)	10.00			
M-L	0 / 1718	-18.5	-18.5 0.25 (1)	10.00			
L-K	0 / 1971	-18.5	-18.5 0.28 (1)	10.00			
K-T	0 / 1971	-18.5	-18.5 0.31 (1)	10.00			
T-J	0 / 1971	-18.5	-18.5 0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TO=0.46/1.00 (G-H:1), BC=0.31/1.00 (K-T:1),
WB=0.39/1.00 (B-R:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

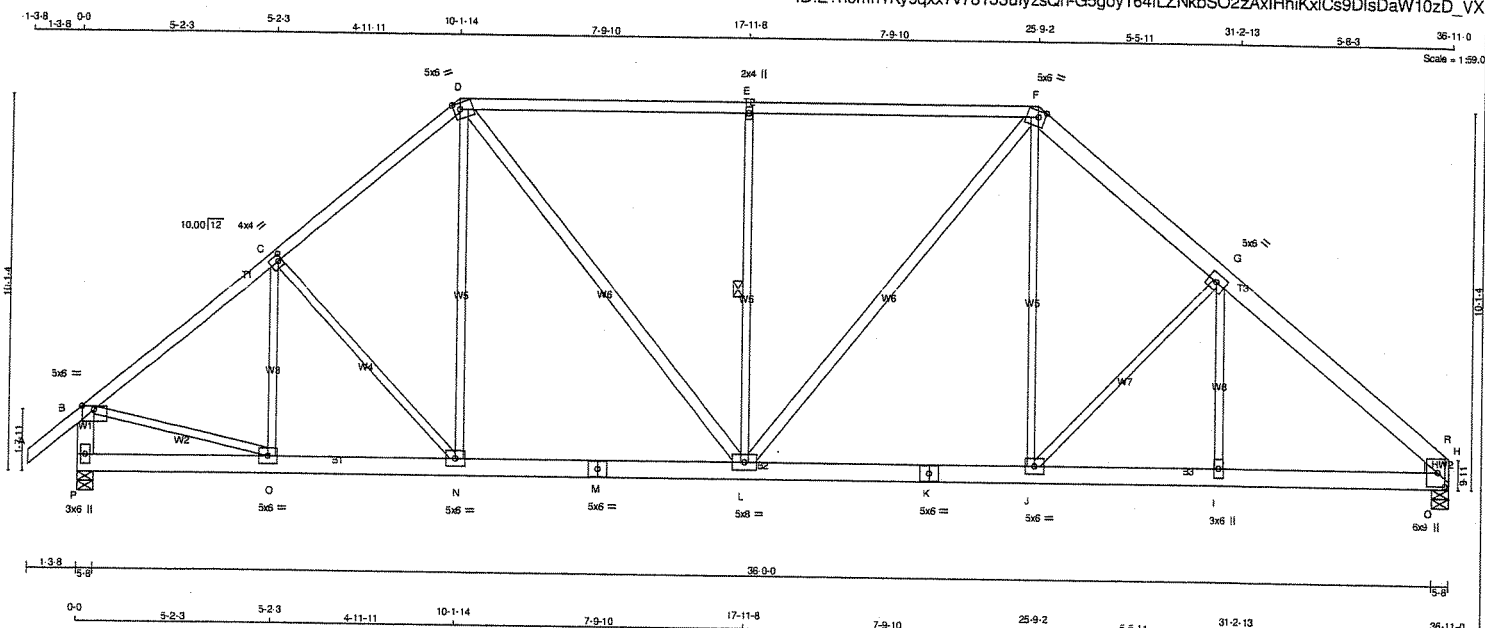
JSI GRIP = 0.83 (H) (INPUT = 0.90)
JSI METAL = 0.76 (J) (INPUT = 1.00)



Structural component only
DWG# T-2115521

JOB NAME 411069	TRUSS NAME T54	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - H	2x6	DRY	No.2 SPF
P - B	2x6	DRY	No.2 SPF
P - M	2x6	DRY	No.2 SPF
M - K	2x6	DRY	No.2 SPF
K - H	2x6	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
EXCEPT			
D - L	2x4	DRY	No.2 SPF
L - F	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMVW-t	MT20	5.0	6.0		
H	TMBH1-t	MT20	6.0	9.0	4.50	2.50
I	BMVW-w	MT20	3.0	6.0		
J	BMVW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMVW-w-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
P	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
P	2162	0	2162	0	5-8	5-8	
H	2035	0	2035	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	LIVE			
P	1527	1015 / 0	0 / 0	0 / 0	0 / 0	511 / 0	0 / 0
H	1439	944 / 0	0 / 0	0 / 0	0 / 0	495 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) P, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.23")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.23")
CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.85/1.00 (D-E:1), BC=0.30/1.00 (H-O:1), WB=0.58/1.00 (E-L:1), SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (O) (INPUT = 0.90)
JSI METAL= 0.76 (H) (INPUT = 1.00)

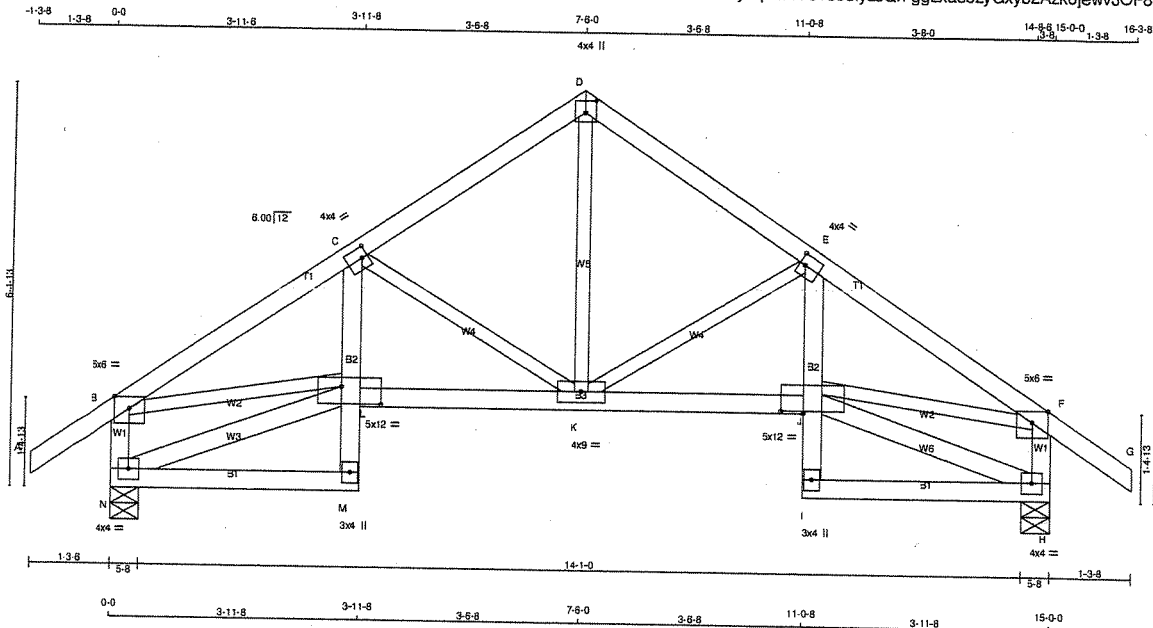


Structural component only
DWG# T-2115522

JSI GRIP = 0.81 (Q) (INPUT = 0.90)
JSI METAL = 0.75 (J) (INPUT = 1.00)

JOB NAME 411069	TRUSS NAME T56S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/32"

TOTAL WEIGHT = 76 lb [M/F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x4	DRY	No.2
L - E	2x4	DRY	No.2
I - J	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
N - L	2x4	DRY	No.2
J - H	2x4	DRY	No.2

DRY: SEASONED LUMBER

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	12.0	0.50	4.25
K	BMVW1-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	5.0	12.0	3.25	7.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
N	953	0	953	0	5-8
H	953	0	953	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
N	671	454 / 0	0 / 0
H	671	454 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO			
A-B	0 / 35	-91.8	-91.8
B-C	-1270 / 0	-91.8	-91.8
C-D	-813 / 0	-91.8	-91.8
D-E	-813 / 0	-91.8	-91.8
E-F	-1270 / 0	-91.8	-91.8
F-G	0 / 35	-91.8	-91.8
G-H	-909 / 0	-91.8	-91.8
H-I	-909 / 0	-91.8	-91.8
I-J	0 / 25	-18.5	-18.5
J-K	0 / 39	-18.5	-18.5
K-L	0 / 124	-18.5	-18.5
L-M	0 / 1088	-18.5	-18.5
M-N	0 / 1088	-18.5	-18.5
N-O	0 / 39	-18.5	-18.5
O-P	0 / 124	-18.5	-18.5
P-Q	0 / 25	-18.5	-18.5

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.17/1.00 (E-F:1), BC=0.21/1.00 (K-L:1),
WB=0.24/1.00 (B-L:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

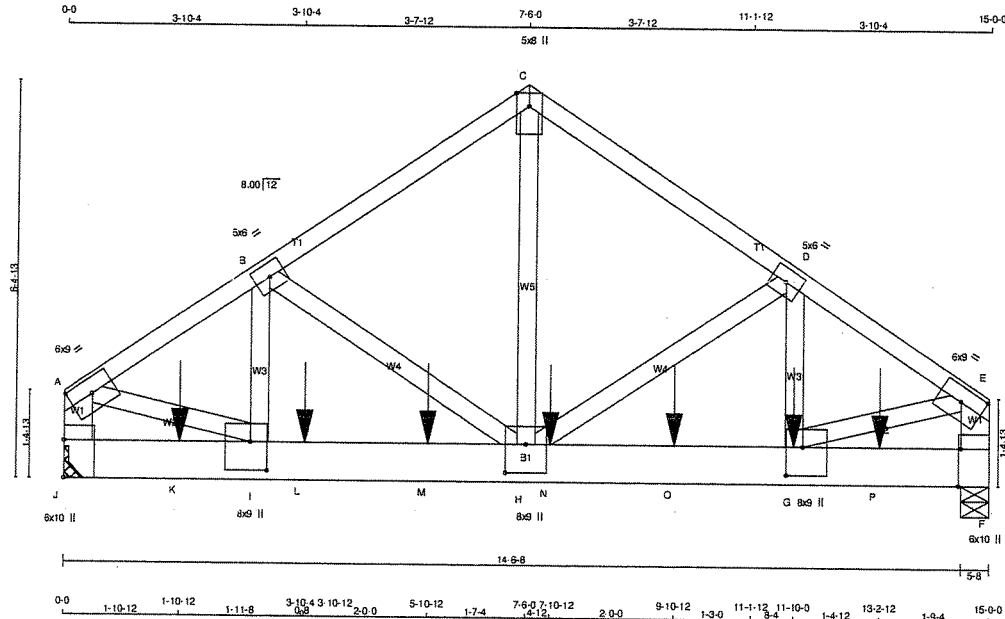
JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)



Structural component only
DWG# T-2115524

JOB NAME 411069	TRUSS NAME T57	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/32

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
J - A	2x6	DRY	No.2	SPF	
F - E	2x6	DRY	No.2	SPF	
J - F	2x8	DRY	1950F 1.7E	SPF	

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - E 1	12	TOP
J - A 2	12	TOP
F - E 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - F 2	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
D - G 2	3	SIDE (733.8)
2x4 1	6	
B - I 2	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	7582	7582	0	MECHANICAL
F 8228	0	8228	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 4-0.

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	5355	3552 / 0	0 / 0
J 5355	3552 / 0	0 / 0	0 / 0
F 5810	3862 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F
BEARING SIZE FACTOR = 1.15 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A - B	-8507 / 0	-91.8 -91.8 0.32 (1)	H - C	0 / 7397
B - C	-7004 / 0	-91.8 -91.8 0.37 (1)	H - D	-2560 / 0
C - D	-7002 / 0	-91.8 -91.8 0.37 (1)	G - D	0 / 2481
D - E	-9500 / 0	-91.8 -91.8 0.38 (1)	B - H	-1553 / 0
J - A	-6913 / 0	0.0 0.0 0.24 (1)	I - B	0 / 1348
F - E	-7707 / 0	0.0 0.0 0.27 (1)	A - I	0 / 7386
			G - E	0 / 8245
J - K	0 / 0	-18.5 -18.5 0.15 (1)		
K - I	0 / 0	-18.5 -18.5 0.15 (1)		
I - L	0 / 7090	-18.5 -18.5 0.38 (1)		
L - M	0 / 7090	-18.5 -18.5 0.38 (1)		
M - H	0 / 7090	-18.5 -18.5 0.38 (1)		
H - N	0 / 7915	-18.5 -18.5 0.41 (1)		
N - O	0 / 7915	-18.5 -18.5 0.41 (1)		
O - G	0 / 7915	-18.5 -18.5 0.41 (1)		
G - P	0 / 0	-18.5 -18.5 0.11 (1)		
P - F	0 / 0	-18.5 -18.5 0.11 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LQC	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.
G	11-10-0	-2481	-2481	---	BACK	VERT	TOTAL	C1
K	1-10-12	-1376	-1376	---	BACK	VERT	TOTAL	C1
L	3-10-12	-1376	-1376	---	BACK	VERT	TOTAL	C1
M	5-10-12	-1376	-1376	---	BACK	VERT	TOTAL	C1
N	7-10-12	-1370	-1370	---	BACK	VERT	TOTAL	C1
O	9-10-12	-1370	-1370	---	BACK	VERT	TOTAL	C1
P	13-2-12	-648	-648	---	TOP	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.38/1.00 (D-E:1), BC=0.41/1.00 (G-H:1),
WB=0.73/1.00 (E-G:1), SS=0.92/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

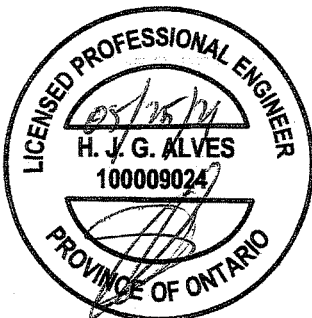
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (C) (INPUT = 0.90)
JSI METAL = 0.89 (I) (INPUT = 1.00)



Structural component only
DWG# T-2115525

CONTINUED ON PAGE 2

JOB NAME 411069	TRUSS NAME T57	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 11:28:32 2021 Page 2
ID:E?n9mn1Ky9qxx/v78133ulyzsQn-8svJoP9bZ3pDCIAHpEtT7sWbY5QoxtungYjAnzD VT

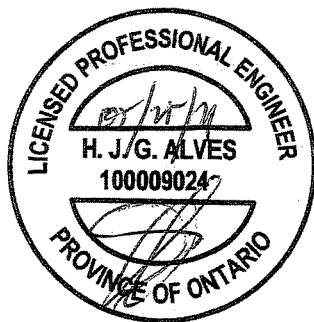
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	2.75	4.50
B	TMVW-t	MT20	5.0	6.0		
C	TTW+p	MT20	5.0	8.0	Edge	
D	TMVW-t	MT20	5.0	6.0		
E	TMVW-t	MT20	6.0	9.0	2.75	4.50
F	BMV1+t	MT20	6.0	10.0	Edge	0.50
G	BMVW+t	MT20	8.0	9.0	5.50	3.25
H	BMVW+t	MT20	8.0	9.0	5.50	4.00
I	BMVW+t	MT20	8.0	9.0	5.50	3.25
J	BMV1+t	MT20	6.0	10.0	7.25	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

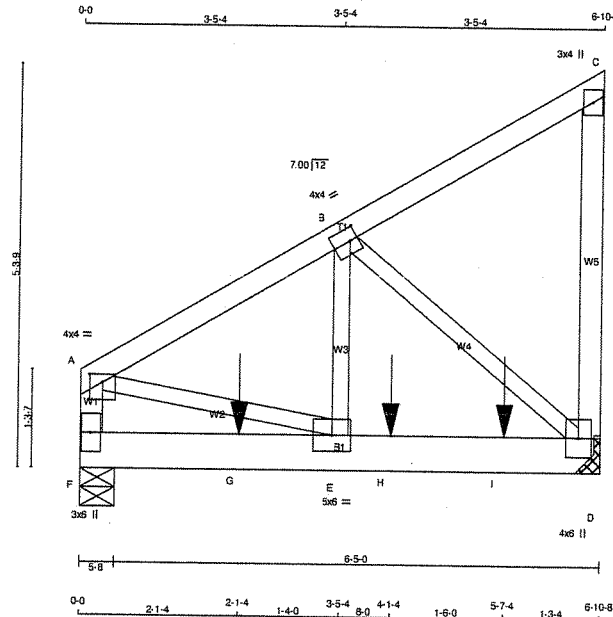


Structural component only
DWG# T-2115525 *mn*

JOB NAME 411069	TRUSS NAME T58	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Tue May 25 11:28:33 2021 Page 1
ID:E?n9mn1Ky9qxx7V78133ulyzsQn-c3Th?IADUtCgqMKMrXl6?KOItYSCXVZ10UJHIDzD_VS



Scale = 1/2" = 1'-0"

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A	1	12
A-C	1	12
C-D	1	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.25	2.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REORD	
	VERT	HORZ	DOWN	HORZ	BRG	IN-SX
F	1654	0	1654	0	5-8	5-8
D	2000	0	2000	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	1166	787 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0
D	1411	947 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
F-A	-1402 / 0	0.0	0.0 0.08 (1)	7.81	A-E	0 / 1565	0.19 (1)
A-B	-1739 / 0	-91.8	-91.8 0.09 (1)	6.25	E-B	0 / 1782	0.22 (1)
B-C	-16 / 0	-91.8	-91.8 0.08 (1)	6.25	B-D	-1981 / 0	0.32 (1)
D-C	-126 / 0	0.0	0.0 0.03 (1)	7.81			
F-G	0 / 0	-18.5	-18.5 0.24 (1)	10.00			
G-E	0 / 0	-18.5	-18.5 0.24 (1)	10.00			
E-H	0 / 1516	-18.5	-18.5 0.37 (1)	10.00			
H-I	0 / 1516	-18.5	-18.5 0.37 (1)	10.00			
I-D	0 / 1516	-18.5	-18.5 0.37 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-1-4	-745	-745	---	BACK	VERT	TOTAL	---	C1
H	4-1-4	-648	-648	---	BACK	VERT	TOTAL	---	C1
I	5-7-4	-648	-648	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 35 = 71 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(\$5 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.09/1.00 (A-B:1), BC=0.37/1.00 (D-E:1),
WB=0.32/1.00 (B-D:1), SS=0.43/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.34 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115526

CONTINUED ON PAGE 2

JOB NAME 411069	TRUSS NAME T58	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

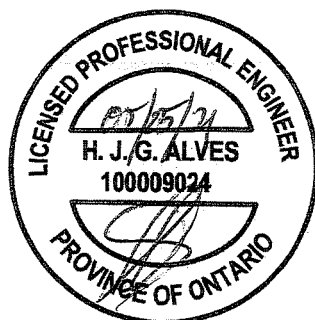
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 11:28:33 2021 Page 2
ID:E?n9mn1Ky9qxx7v78133ulyzsQn-c3Th?ADUtCggMKMrXi6?KOlySCXVZ10UIHiDzD VS

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

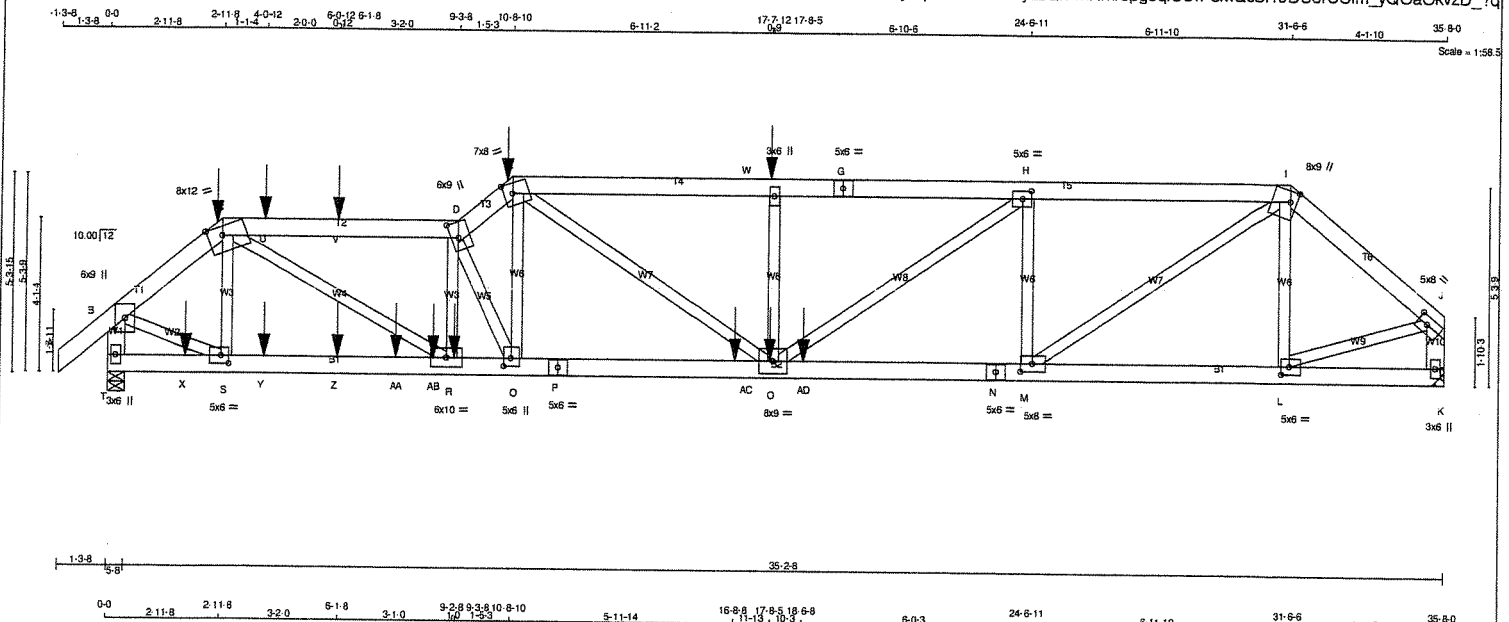


Structural component only
DWG# T-2115526 *mn*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411071	T60	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:E?n9mn1Ky9qxx7V78133ulyzsqn-MRmropgoqCUfP3MQc3I19DUerCCIm_yQaOkvD_?q



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF	
C - D	2x6	DRY	No.2	SPF	
D - E	2x6	DRY	No.2	SPF	
E - G	2x6	DRY	No.2	SPF	
G - I	2x6	DRY	No.2	SPF	
I - J	2x6	DRY	No.2	SPF	
J - B	2x6	DRY	No.2	SPF	
K - J	2x6	DRY	No.2	SPF	
T - P	2x6	DRY	2100F 1.8E	SPF	
P - N	2x6	DRY	2100F 1.8E	SPF	
N - K	2x6	DRY	2100F 1.8E	SPF	

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	SIDE(122.0)
C - D 2	12	SIDE(61.0)
D - E 2	12	SIDE(85.2)
E - G 2	12	SIDE(183.1)
G - I 2	12	TOP
I - J 2	12	TOP
T - B 2	12	TOP
K - J 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
T - P 2	12	SIDE(197.8)
P - N 2	12	SIDE(197.8)
N - K 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	SIDE(53.9)
Q - E 1	6	SIDE(66.2)
R - D 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
T	6032	0	6032	0
K	4491	0	4491	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 4-0.

UNFACTORED REACTIONS

	1ST LOASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
T	4274	2756 / 0	0 / 0	0 / 0	0 / 0	1518 / 0	0 / 0	
K	3181	2059 / 0	0 / 0	0 / 0	0 / 0	1122 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO								
A-B	0 / 42	-91.8	-91.8	0.04 (1)	10.00	FR-TO		
B-C	-6378 / 0	-91.8	-91.8	0.09 (1)	4.66	C-R	-1281 / 0	0.11 (1)
C-D	-12693 / 0	-91.8	-91.8	0.55 (1)	3.03	R-D	0 / 9116	0.81 (1)
D-E	-12693 / 0	-91.8	-91.8	0.55 (1)	3.03	D-O	-5754 / 0	0.35 (1)
E-F	-12693 / 0	-91.8	-91.8	0.55 (1)	3.03	O-E	0 / 5975	0.53 (1)
F-G	-12693 / 0	-91.8	-91.8	0.55 (1)	3.03	E-L	-1037 / 0	0.14 (1)
G-H	-12693 / 0	-91.8	-91.8	0.55 (1)	3.03	L-S	0 / 5176	0.46 (1)
H-I	-9021 / 0	-91.8	-91.8	0.33 (1)	3.84	S-B	0 / 3894	0.34 (1)
I-J	-4864 / 0	-91.8	-91.8	0.11 (1)	5.18	B-M	0 / 6463	0.57 (1)
J-K	-6133 / 0	0.0	0.0	0.22 (1)	5.98	M-I	0 / 2682	0.24 (1)
K-L	-4470 / 0	0.0	0.0	0.16 (1)	6.81	I-O	-3381 / 0	0.45 (1)
L-M	0 / 0	-18.5	-18.5	0.09 (1)	10.00	O-F	-1201 / 0	0.16 (1)
M-N	0 / 0	-18.5	-18.5	0.09 (1)	10.00	F-H	0 / 4823	0.43 (1)
N-O	0 / 4820	-18.5	-18.5	0.26 (1)	10.00			
O-P	0 / 4820	-18.5	-18.5	0.26 (1)	10.00			
P-Q	0 / 4820	-18.5	-18.5	0.26 (1)	10.00			
Q-R	0 / 12857	-56.0	-56.0	0.47 (1)	10.00			
R-S	0 / 10735	-56.0	-56.0	0.45 (1)	10.00			
S-T	0 / 10735	-56.0	-56.0	0.45 (1)	10.00			
T-U	0 / 10735	-18.5	-18.5	0.45 (1)	10.00			
U-V	0 / 9021	-18.5	-18.5	0.40 (1)	10.00			
V-W	0 / 9021	-18.5	-18.5	0.40 (1)	10.00			
W-X	0 / 9021	-18.5	-18.5	0.40 (1)	10.00			
X-Y	0 / 9021	-18.5	-18.5	0.40 (1)	10.00			
Y-Z	0 / 3690	-18.5	-18.5	0.13 (1)	10.00			
Z-AA	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
E	10-8-10	-54	-54	---	FRONT	VERT	DEAD	---	C1
E	10-8-10	-234	-234	---	FRONT	VERT	SNOW	---	C1
F	17-7-12	-100	-100	---	BACK	VERT	TOTAL	---	C1

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
E	10-8-10	-54	-54	---	FRONT	VERT	DEAD	---	C1
E	10-8-10	-234	-234	---	FRONT	VERT	SNOW	---	C1
F	17-7-12	-100	-100	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 9-0-0
START SPAN CARRIED = 6-10-8
END DISTANCE = 17-0-0
END SPAN CARRIED = 6-10-8
END WALL WIDTH = 4-0
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDTL LOADS BASED ON 55% OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL)= L/862 (0.50")

CSI: TC=0.76/1.00 (E-F-1), BC=0.47/1.00 (Q-R-1),
WB=0.81/1.00 (C-R-1), SSI=0.63/1.00 (M-O-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

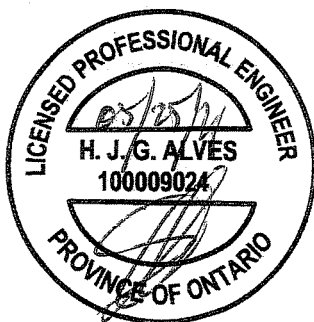
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSQLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

CONTINUED ON PAGE 2



Structural component only
DWG# T-2115536

JOB NAME 411071	TRUSS NAME T60	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:E?n9mn1Ky9qxx7v78133ulyzsQn-MRmropqgqICUfP3MQc3i19DUerCCIm yQOaOkvzD ?q

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
8	TMVW+p	MT20	6.0	9.0		
C	TTWW+m	MT20	8.0	12.0	2.75	4.50
D	TTWW+m	MT20	6.0	9.0	5.00	2.25
E	TTWW-m	MT20	7.0	8.0	3.25	2.75
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0	2.50	2.75
I	TTWW+m	MT20	8.0	9.0	3.50	2.00
J	TMVW-t	MT20	5.0	8.0	2.50	3.25
K	8MV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.50
M	BMVW-t	MT20	5.0	8.0	2.50	3.75
N	BS-t	MT20	5.0	6.0		
O	8MWW-t	MT20	8.0	9.0		
P	8S-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.25
R	8MWW-t	MT20	6.0	10.0		
S	8MWW-t	MT20	5.0	6.0	2.50	2.50
T	8MV1+p	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	17-7-12	-27	-27	---	8ACK	VERT	TOTAL	---	C1
R	9-2-8	-714	-714	---	8ACK	VERT	TOTAL	---	C1
U	4-0-12	-76	-76	---	8ACK	VERT	TOTAL	---	C1
V	6-0-12	-76	-76	---	8ACK	VERT	TOTAL	---	C1
X	2-0-12	-21	-21	---	8ACK	VERT	TOTAL	---	C1
Y	4-0-12	-21	-21	---	8ACK	VERT	TOTAL	---	C1
Z	6-0-12	-21	-21	---	8ACK	VERT	TOTAL	---	C1
AA	7-7-12	-27	-27	---	8ACK	VERT	TOTAL	---	C1
AB	8-7-12	-27	-27	---	8ACK	VERT	TOTAL	---	C1
AC	16-8-8	-714	-714	---	8ACK	VERT	TOTAL	---	C1
AD	18-6-8	-1396	-1396	---	8ACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (E) (INPUT = 0.90)
JSI METAL = 0.96 (E) (INPUT = 1.00)

NOTES- (1)

- 1) Lateral braces to be a minimum of 2x4 SPF #2

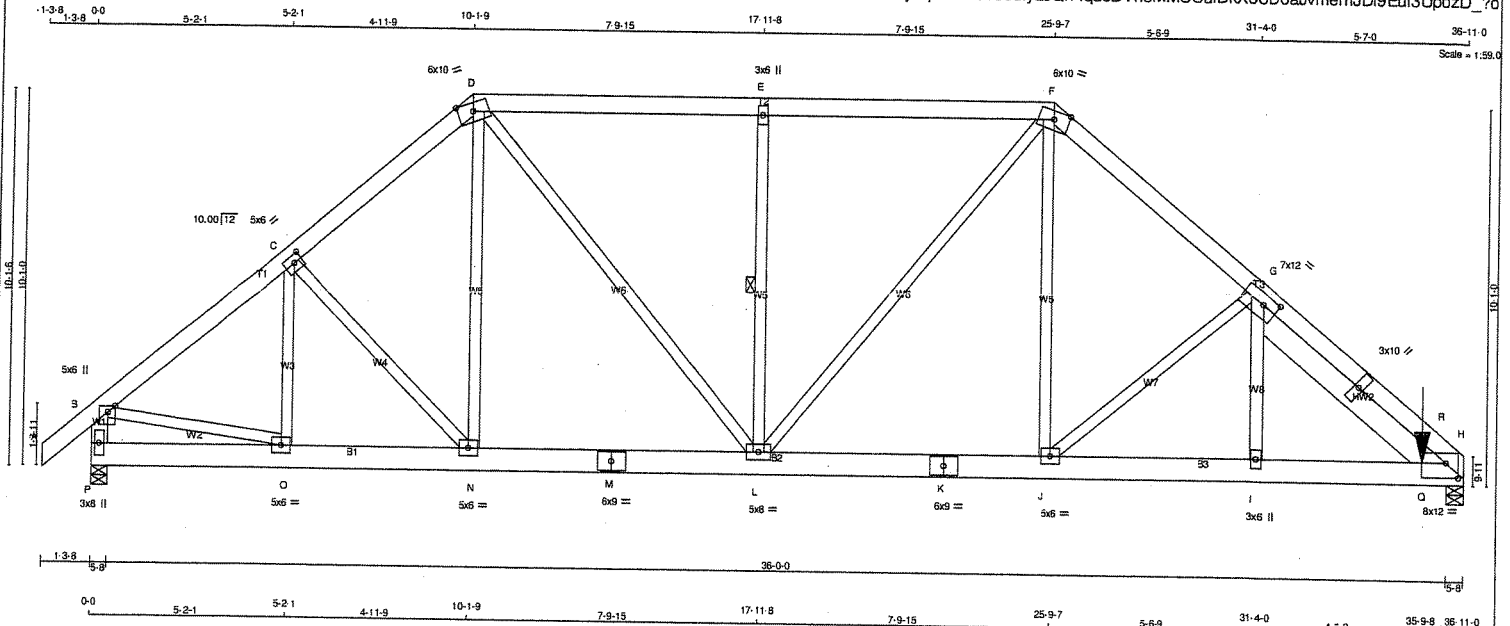


Structural component only
DWG# T-2115536 *W*

JOB NAME 411071	TRUSS NAME T61	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x6	DRY	No.2
D - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
P - B	2x6	DRY	No.2
P - M	2x8	DRY	No.2
M - K	2x8	DRY	No.2
K - H	2x8	DRY	No.2

REINFORCING MEMBERS			
HW2	2x8	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-D 2	12	TOP
D-F 2	12	TOP
F-H 2	12	TOP
P-B 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
P-M 2	12	TOP
M-K 2	12	TOP
K-H 2	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x4 1	6	
2x8 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD	
VERT	HORZ	DOWN	HORZ	BRG	BRG
JT 10003	0	10003	0	5-8	5-8
H 2403	0	2403	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS				
JT COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD
H 7066	4683 / 0	0 / 0	0 / 0	0 / 0	2383 / 0
P 1697	1129 / 0	0 / 0	0 / 0	0 / 0	568 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 42	C-C	-338 / 0
B-C	-2514 / 0	C-N	-185 / 0
C-D	-2421 / 0	N-D	0 / 279
D-E	-2487 / 0	D-L	0 / 1031
E-F	-2487 / 0	L-E	-866 / 0
F-G	-3024 / 0	L-F	0 / 302
G-R	-8437 / 0	J-F	0 / 1221
R-H	-9953 / 0	J-G	-1872 / 0
P-B	-2343 / 0	I-G	0 / 916
		B-O	0 / 1995
		Q-R	0 / 1948
		Q-G	0 / 4259

SPECIFIED CONCENTRATED LOADS (LBS)		FACE DIR.		TYPE		HEEL CONN.	
JT	LOC.	LC1	MAX.	BACK	VERT	TOTAL	C1
Q	35-9-8	-5793	-5793	---	---	---	---

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.23")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (1.23")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CSI: TC=0.40/1.00 (H-R:1), BC=1.00/1.00 (H-O:1),
WB=0.70/1.00 (G-J:1), SS=0.73/1.00 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS 8END=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.47 (H) (INPUT = 1.00)



Structural component only
DWG# T-2115537

JOB NAME 411071	TRUSS NAME T61	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	6.0	10.0	Edge	
E	TMW+w	MT20	3.0	6.0		
F	TTWW-m	MT20	6.0	10.0	Edge	
G	TMWWWW-t	MT20	7.0	12.0	3.25	4.75
G	TP-m	MT20	3.0	10.0		
H	TMBW1-t	MT20	8.0	12.0	4.75	4.00
I	BMW+w	MT20	3.0	6.0		
J, N, O						
J	BMW-t	MT20	5.0	6.0		
K	BS-t	MT20	6.0	9.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	6.0	9.0		
P	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

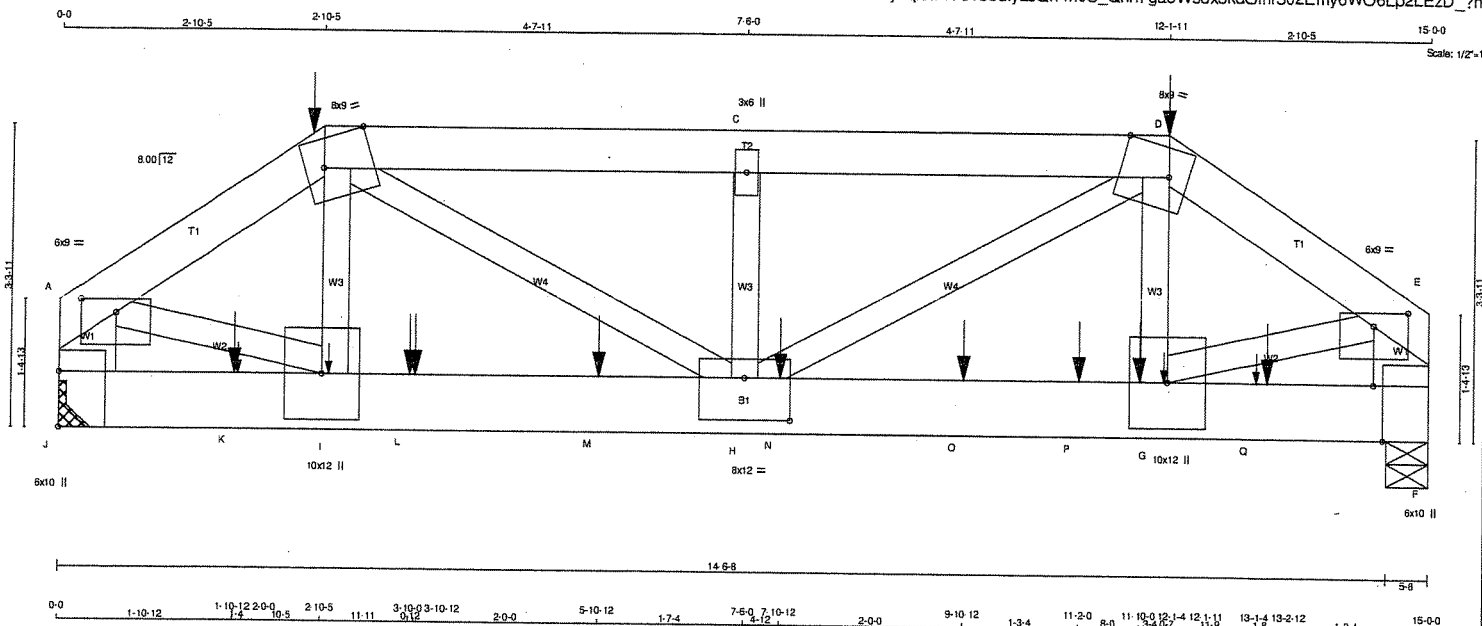


Structural component only
DWG# T-2115537 *2/2*

JOB NAME 411071	TRUSS NAME T62	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x6	DRY No.2	SPF
B - D	2x6	DRY No.2	SPF
D - E	2x6	DRY No.2	SPF
J - A	2x6	DRY No.2	SPF
F - E	2x6	DRY No.2	SPF
J - F	2x8	DRY 1950F 1.7E	SPF
ALL WEBS	2x4	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B	2	12
B - D	2	12
D - E	2	12
J - A	2	12
F - E	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - F	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	2	2
G - D	2	2
H - B	1	6
C - H	1	6
H - D	1	6
G - E	1	6
A - I	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
JT 8222 0	8222 0 0	MECHANICAL	
F 9487 0	9487 0 0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
JT 5808	3849 / 0	0 / 0 0 / 0 1959 / 0 0 / 0
F 6705	4423 / 0	0 / 0 0 / 0 2283 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F
BEARING SIZE FACTOR = 1.15 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A - B	-9914 / 0	-102.1 -102.1	0.16 (1)
B - C	-13082 / 0	-102.1 -102.1	0.37 (1)
C - D	-13082 / 0	-102.1 -102.1	0.37 (1)
D - E	-11631 / 0	-102.1 -102.1	0.21 (1)
J - A	-7943 / 0	0.0 0.0	0.21 (1)
F - E	-9294 / 0	0.0 0.0	0.25 (1)
J - K	0 / 0	-18.5 -18.5	0.17 (1)
K - I	0 / 0	-18.5 -18.5	0.17 (1)
I - L	0 / 8334	-18.5 -18.5	0.40 (1)
L - M	0 / 8334	-18.5 -18.5	0.40 (1)
M - H	0 / 8334	-18.5 -18.5	0.40 (1)
H - N	0 / 9856	-18.5 -18.5	0.48 (1)
N - O	0 / 9856	-18.5 -18.5	0.48 (1)
O - P	0 / 9856	-18.5 -18.5	0.48 (1)
P - G	0 / 9856	-18.5 -18.5	0.48 (1)
G - O	0 / 0	-18.5 -18.5	0.06 (1)
Q - F	0 / 0	-18.5 -18.5	0.06 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-10-5	-6	-6		FRONT	VERT	DEAD		C1
B	2-10-5	-6	-6		FRONT	VERT	TOTAL		C1
B	2-10-5	-26	-26		FRONT	VERT	SNOW		C1
D	12-1-11	-6	-6		FRONT	VERT	DEAD		C1
D	12-1-11	-6	-6		FRONT	VERT	TOTAL		C1
D	12-1-11	-26	-26		FRONT	VERT	SNOW		C1
G	11-10-0	-3167	-3167		BACK	VERT	TOTAL		C1
G	12-1-4	1	1		FRONT	VERT	TOTAL		C1
I	2-11-4	1	1		FRONT	VERT	TOTAL		C1
K	1-10-12	-1376	-1376		BACK	VERT	TOTAL		C1
K	1-11-4	1	1		FRONT	VERT	TOTAL		C1
L	3-10-0	-238	-238		FRONT	VERT	TOTAL		C1
L	3-10-12	-1376	-1376		BACK	VERT	TOTAL		C1
M	5-10-12	-1376	-1376		BACK	VERT	TOTAL		C1
N	7-10-12	-1370	-1370		BACK	VERT	TOTAL		C1
O	9-10-12	-1370	-1370		BACK	VERT	TOTAL		C1
P	11-2-0	-238	-238		FRONT	VERT	TOTAL		C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStd Girder
START DISTANCE = 0-0
START SPAN CARRIED = 2-6-0
END DISTANCE = 15-0-0
END SPAN CARRIED = 2-6-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADD'L LOADS BASED ON 55% OF GSL.

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.37/1.00 (B-C:1), BC=0.48/1.00 (G-H:1), WB=0.88/1.00 (E-G:1), SS=0.84/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

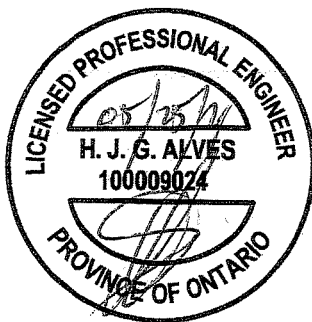
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

CONTINUED ON PAGE 2



Structural component only
DWG# T-2115538

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411071	T62	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	9.0	1.75	4.50
B	TTWW-m	MT20	8.0	9.0	Edge	6.50
C	TMW+w	MT20	3.0	6.0		
D	TTWW-m	MT20	8.0	9.0	Edge	6.50
E	TMVW-p	MT20	6.0	9.0	1.75	4.50
F	BMV1+t	MT20	6.0	10.0	Edge	1.25
G	BMWW+t	MT20	10.0	12.0		
H	BMWWW+t	MT20	8.0	12.0	5.50	6.00
I	BMWWW+t	MT20	10.0	12.0		
J	BMV1+t	MT20	6.0	10.0	7.25	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	13-1-4	1	1	---	FRONT	VERT	TOTAL	---	C1
O	13-2-12	-648	-648	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.89 (G) (INPUT = 1.00)

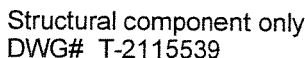


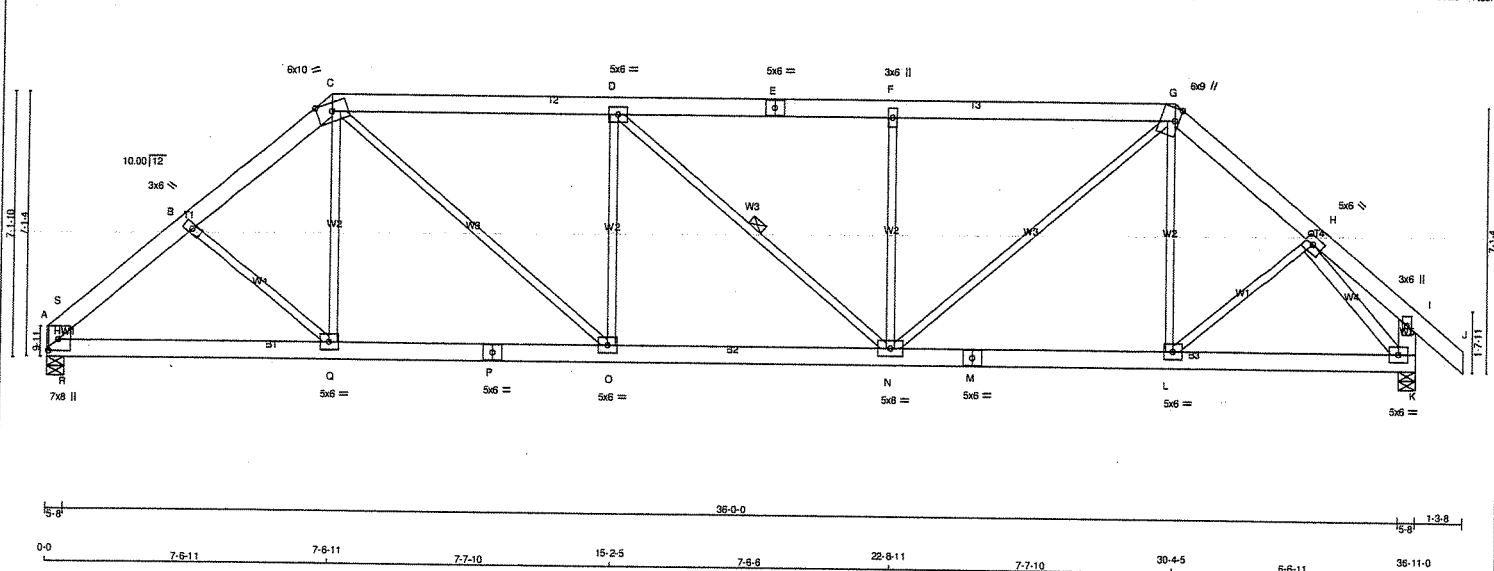
Structural component only
DWG# T-2115538

Tamarack Roof Truss, Burlington



PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.03 (A) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - E	2x6	DRY	No.2
E - G	2x6	DRY	No.2
G - J	2x6	DRY	No.2
K - I	2x6	DRY	No.2
A - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
M - K	2x6	DRY	No.2
ALL WEBS EXCEPT H - K	2x3	DRY	No.2
	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-i	MT20	7.0	8.0	3.75	3.00
B	TMW-w	MT20	3.0	6.0		
C	TTWW-m	MT20	6.0	10.0	Edge	
D	MTWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW-w	MT20	3.0	6.0		
G	TTWW-m	MT20	6.0	9.0	4.00	1.25
H	MTWW-t	MT20	5.0	6.0	2.50	2.75
I	TMV-p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0		
L, O, Q						
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
JT VERT	JT HORZ	JT UPLIFT	JT IN-SX	JT IN-SX
A 2035	0	2035	0	0
K 2167	0	2167	0	0

UNFACTORED REACTIONS

1ST LOASE	MAX / MIN COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	JT SNOW	JT LIVE	JT PERM. LIVE	JT WIND	JT DEAD
A 1439	944 / 0	0 / 0	0 / 0	495 / 0	0 / 0
K 1530	1018 / 0	0 / 0	0 / 0	512 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MAX. FACTORED		FACTORED		WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-S	-3012 / 0	-91.8	-91.8	0.22 (1)	4.65	B-Q	-200 / 0
S-B	-2668 / 0	-91.8	-91.8	0.22 (1)	4.89	O-C	0 / 256
B-C	-2426 / 0	-91.8	-91.8	0.14 (1)	5.17	C-O	0 / 1274
C-D	-2823 / 0	-91.8	-91.8	0.42 (1)	4.57	O-D	-689 / 0
D-E	-2754 / 0	-91.8	-91.8	0.41 (1)	4.61	D-N	-91 / 0
E-F	-2754 / 0	-91.8	-91.8	0.41 (1)	4.61	N-F	-745 / 0
F-G	-2754 / 0	-91.8	-91.8	0.40 (1)	4.63	N-G	0 / 1427
G-H	-2201 / 0	-91.8	-91.8	0.12 (1)	5.40	L-G	0 / 93
H-I	0 / 15	-91.8	-91.8	0.04 (1)	10.00	L-H	0 / 209
I-J	0 / 42	-91.8	-91.8	0.07 (1)	10.00	H-K	-2421 / 0
K-I	-233 / 0	0.0	0.0	0.02 (1)	7.81	R-S	0 / 341

A-R	0 / 2007	-18.5 -18.5	0.32 (1)	10.00
R-O	0 / 2007	-18.5 -18.5	0.32 (1)	10.00
Q-P	0 / 1860	-18.5 -18.5	0.27 (1)	10.00
P-Q	0 / 1860	-18.5 -18.5	0.27 (1)	10.00
O-N	0 / 2823	-18.5 -18.5	0.38 (1)	10.00
N-M	0 / 1676	-18.5 -18.5	0.26 (1)	10.00
M-L	0 / 1676	-18.5 -18.5	0.26 (1)	10.00
L-K	0 / 1517	-18.5 -18.5	0.24 (1)	10.00

TOTAL WEIGHT = 202 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.23")
CALCULATED VERT. DEFL.(LL)= L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.23")
CALCULATED VERT. DEFL.(TL)= L/999 (0.21")

CSI: TC=0.42/1.00 (C-D:1), BC=0.38/1.00 (N-O:1), WB=0.57/1.00 (F-N:1), SS=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

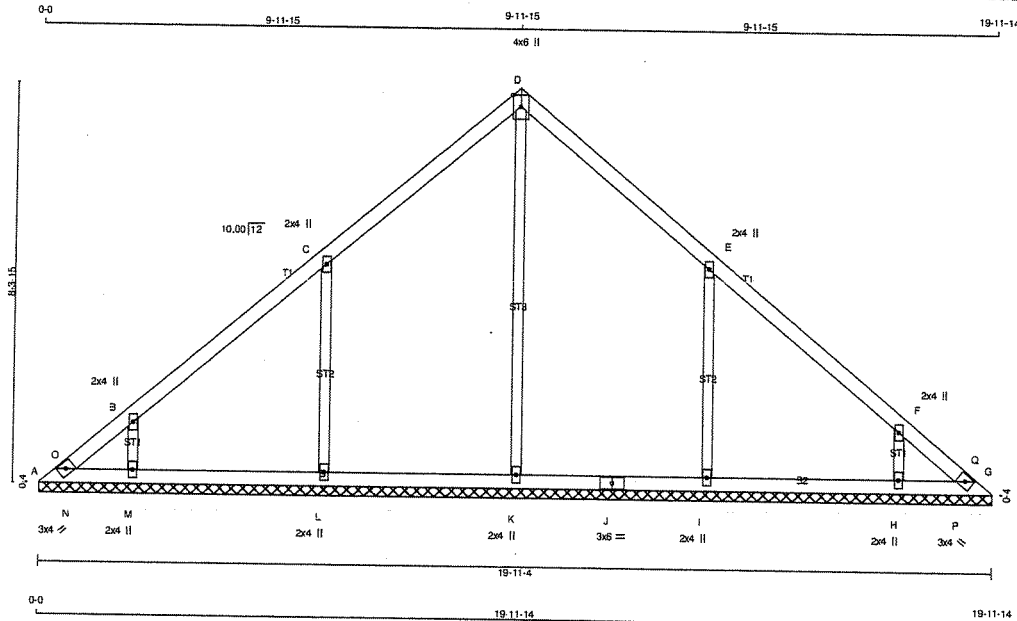
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.66 (A) (INPUT = 1.00)



JOB NAME 411069	TRUSS NAME V01	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/4" = 1'-0"

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
A - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B, C, E, F					
D	TTW+w	MT20	2.0	4.0	
E	TTW+p	MT20	4.0	6.0	Edge
G	TBM1-h	MT20	3.0	4.0	
H, I, K, L, M					
H	BMW1+w	MT20	2.0	4.0	
J	BS-t	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
JT								
A	103	0	103	0	0	0	19-11-4 (7-11-4)	11-4
G	103	0	103	0	0	0	19-11-4 (7-11-4)	11-4
K	284	0	284	0	0	0	19-11-4 (7-11-4)	11-4
L	501	0	501	0	0	0	19-11-4 (7-11-4)	11-4
M	354	0	354	0	0	0	19-11-4 (7-11-4)	11-4
I	501	0	501	0	0	0	19-11-4 (7-11-4)	11-4
H	354	0	354	0	0	0	19-11-4 (7-11-4)	11-4

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
A	72	52/0	0/0	0/0	0/0	0/0	19/0
G	72	52/0	0/0	0/0	0/0	0/0	19/0
K	203	118/0	0/0	0/0	0/0	0/0	85/0
L	353	237/0	0/0	0/0	0/0	0/0	117/0
M	251	162/0	0/0	0/0	0/0	0/0	89/0
I	353	237/0	0/0	0/0	0/0	0/0	117/0
H	251	162/0	0/0	0/0	0/0	0/0	89/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, K, L, M, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC1 MAX (LC)	MEMB.	MAX. FORCE (LBS)	LC1 MAX (PLF)	LC1 MAX (LC)
FR-TO					FR-TO			
A-O	-105/0	-91.8	-91.8	0.02 (1)	6.25	K-D	-210/0	0.28 (1)
O-B	-52/0	-91.8	-91.8	0.11 (1)	6.25	L-C	-425/0	0.16 (1)
B-C	-64/0	-91.8	-91.8	0.23 (1)	6.25	M-B	-284/0	0.04 (1)
C-D	-92/0	-91.8	-91.8	0.23 (1)	6.25	I-E	-425/0	0.16 (1)
D-E	-92/0	-91.8	-91.8	0.23 (1)	6.25	H-F	-284/0	0.04 (1)
E-F	-64/0	-91.8	-91.8	0.23 (1)	6.25	N-O	-2/0	0.00 (1)
F-O	-52/0	-91.8	-91.8	0.11 (1)	6.25	P-O	-2/0	0.00 (1)
Q-G	-105/0	-91.8	-91.8	0.02 (1)	6.25			
A-N	0/79	-18.5	-18.5	0.03 (1)	10.00			
N-M	0/80	-18.5	-18.5	0.05 (4)	10.00			
M-L	0/59	-18.5	-18.5	0.06 (4)	10.00			
L-K	0/50	-18.5	-18.5	0.06 (4)	10.00			
K-J	0/50	-18.5	-18.5	0.06 (4)	10.00			
J-I	0/50	-18.5	-18.5	0.06 (4)	10.00			
I-H	0/59	-18.5	-18.5	0.06 (4)	10.00			
H-P	0/80	-18.5	-18.5	0.05 (4)	10.00			
P-G	0/79	-18.5	-18.5	0.03 (1)	10.00			

TOTAL WEIGHT = 66 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-D:1), BC=0.06/1.00 (H-I:4), WB=0.28/1.00 (D-K:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

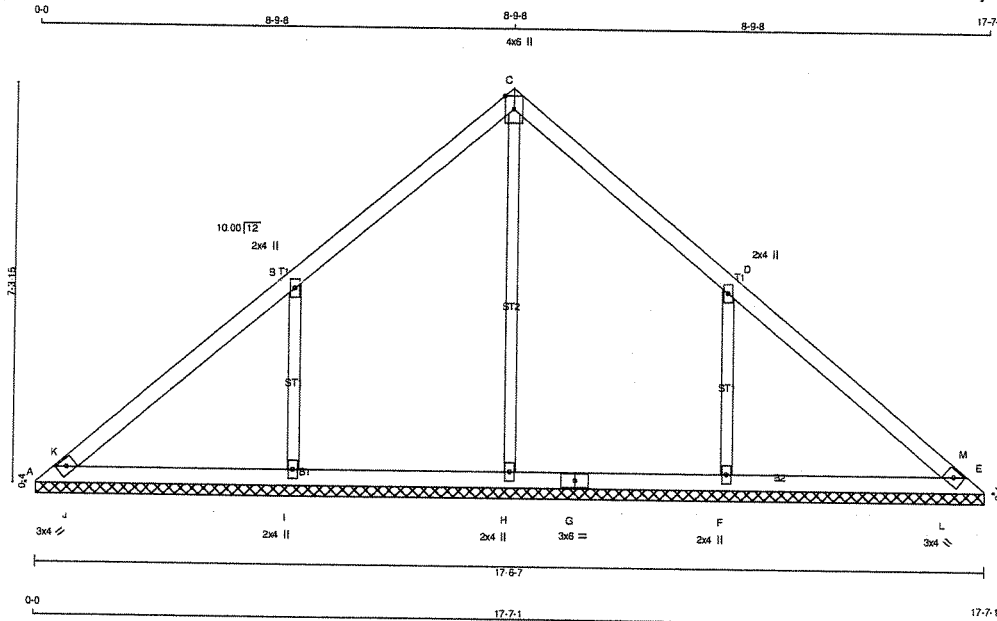
JSI GRIP = 0.38 (F) (INPUT = 0.90)
JSI METAL = 0.23 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115527

JOB NAME 411069	TRUSS NAME V02	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	UPFT	IN-SX
A	143	0	143	0	17-6-7 (7-6-11)7-6-7
E	143	0	143	0	17-6-7 (7-6-11)7-6-7
H	472	0	472	0	17-6-7 (7-6-11)7-6-7
I	587	0	587	0	17-6-7 (7-6-11)7-6-7
F	587	0	587	0	17-6-7 (7-6-11)7-6-7

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	UVE			
A	101	71 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	101	71 / 0	0 / 0	0 / 0	30 / 0	0 / 0
H	337	204 / 0	0 / 0	0 / 0	133 / 0	0 / 0
I	415	276 / 0	0 / 0	0 / 0	139 / 0	0 / 0
F	415	276 / 0	0 / 0	0 / 0	139 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-K	0 / 92	H-C	-431 / 0
K-B	0 / 132	F-B	-457 / 0
B-C	0 / 87	F-D	-457 / 0
C-D	0 / 87	J-K	-150 / 5
D-M	0 / 132	L-M	-150 / 5
M-E	0 / 92		
A-J	-110 / 0		
J-I	-78 / 0		
I-H	-90 / 0		
H-G	-90 / 0		
G-F	-90 / 0		
F-L	-78 / 0		
L-E	-110 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF UVE LOAD

CSI: TC=0.28/1.00 (D-M:1), BC=0.13/1.00 (F-L:1), WB=0.40/1.00 (C-H:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.38 (B) (INPUT = 0.90)

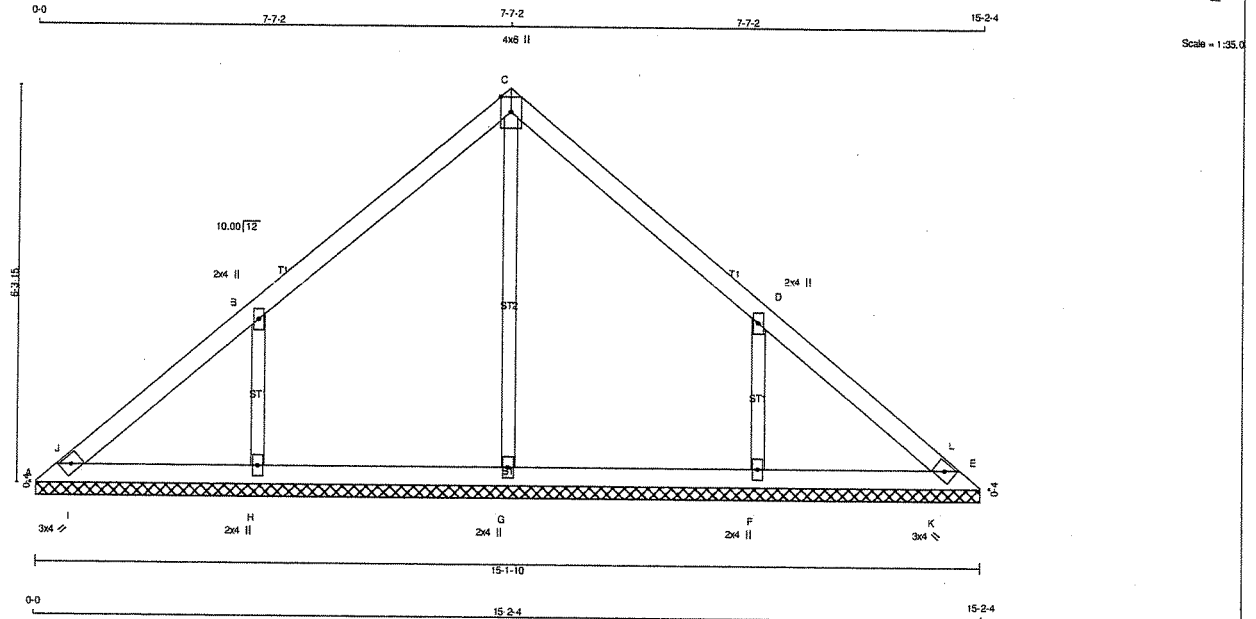
JSI METAL = 0.24 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115528

JOB NAME 411069	TRUSS NAME V03	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 47 lb [M]

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS				SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
A	147	0	147	0	0	15-1-10	15-1-10	15-1-10	15-1-10
E	147	0	147	0	0	15-1-10	15-1-10	15-1-10	15-1-10
G	363	0	363	0	0	15-1-10	15-1-10	15-1-10	15-1-10
H	506	0	506	0	0	15-1-10	15-1-10	15-1-10	15-1-10
F	506	0	506	0	0	15-1-10	15-1-10	15-1-10	15-1-10

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (D-L:1), BC=0.06/1.00 (F-K:1), WB=0.19/1.00 (C-G:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.29 (B) (INPUT = 0.90)
JSI METAL = 0.22 (B) (INPUT = 1.00)

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TMW+w	MT20	2.0	4.0	
C	TTW+p	MT20	4.0	6.0	Edge
D	TMW+w	MT20	2.0	4.0	
E	TBM1-h	MT20	3.0	4.0	
F, G, H					
F	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

- (1) Lateral braces to be a minimum of 2x4 SPF #2

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	SNOW	LIVE	PERM. LIVE	WIND
A	103	72 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
E	103	72 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
G	260	152 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
H	357	240 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
F	357	240 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

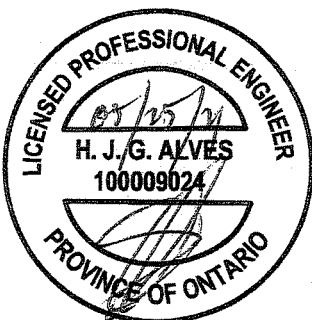
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

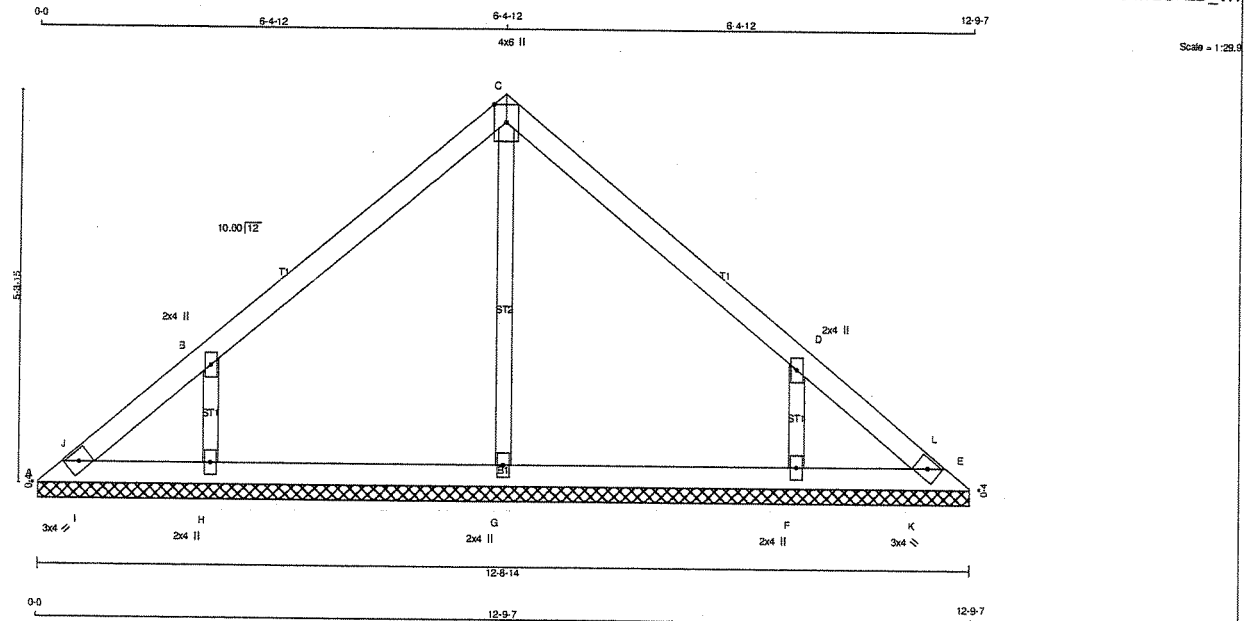
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-J	-34 / 0	-91.8 -91.8 0.03 (1)	6.25		G-C	-300 / 0	0.19 (1)
J-B	0 / 24	-91.8 -91.8 0.22 (1)	10.00		H-B	-409 / 0	0.07 (1)
B-C	-24 / 8	-91.8 -91.8 0.22 (1)	6.25		F-D	-409 / 0	0.07 (1)
C-D	-24 / 8	-91.8 -91.8 0.22 (1)	6.25		I-J	-61 / 6	0.00 (1)
D-L	0 / 24	-91.8 -91.8 0.22 (1)	10.00		K-L	-61 / 6	0.00 (1)
L-E	-34 / 0	-91.8 -91.8 0.03 (1)	6.25				
A-I	-11 / 0	-18.5 -18.5 0.06 (1)	6.25				
I-H	-7 / 14	-18.5 -18.5 0.06 (1)	10.00				
H-G	-10 / 0	-18.5 -18.5 0.06 (4)	6.25				
G-F	-10 / 0	-18.5 -18.5 0.06 (4)	6.25				
F-K	-7 / 14	-18.5 -18.5 0.06 (1)	10.00				
K-E	-11 / 0	-18.5 -18.5 0.06 (1)	6.25				



Structural component only
DWG# T-2115529

JOB NAME 411069	TRUSS NAME V04	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2		SPF
C - E	2x4	DRY	No.2		SPF
A - E	2x4	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0			
B	TTW+w	MT20	2.0	4.0			
C	TTW+p	MT20	4.0	6.0	Edge		
D	TTW+w	MT20	2.0	4.0			
E	TBM1-h	MT20	3.0	4.0			
F, G, H	BMW1+w	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	107	0	107	0	12-8-14	12-8-14
E	107	0	107	0	12-8-14	12-8-14
G	291	0	291	0	12-8-14	12-8-14
H	450	0	450	0	12-8-14	12-8-14
F	450	0	450	0	12-8-14	12-8-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	75	52 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
E	75	52 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
G	209	117 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
H	317	215 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
F	317	215 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-J	-110 / 0	-91.8	-91.8 0.04 (1)	6.25	G-C	-212 / 0	0.09 (1)
J-B	-31 / 0	-91.8	-91.8 0.20 (1)	6.25	H-8	-389 / 0	0.06 (1)
B-C	-94 / 0	-91.8	-91.8 0.20 (1)	6.25	F-D	-389 / 0	0.06 (1)
C-D	-94 / 0	-91.8	-91.8 0.20 (1)	6.25	I-J	0 / 25	0.00 (1)
D-L	-31 / 0	-91.8	-91.8 0.20 (1)	6.25	K-L	0 / 25	0.00 (1)
L-E	-110 / 0	-91.8	-91.8 0.04 (1)	6.25			
A-I	0 / 79	-18.5	-18.5 0.02 (1)	10.00			
I-H	0 / 78	-18.5	-18.5 0.05 (4)	10.00			
H-G	0 / 55	-18.5	-18.5 0.07 (4)	10.00			
G-F	0 / 55	-18.5	-18.5 0.07 (4)	10.00			
F-K	0 / 78	-18.5	-18.5 0.05 (4)	10.00			
K-E	0 / 79	-18.5	-18.5 0.02 (1)	10.00			

TOTAL WEIGHT = 38 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (B-C:1), BC=0.07/1.00 (G-H:4), WB=0.09/1.00 (C-G:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

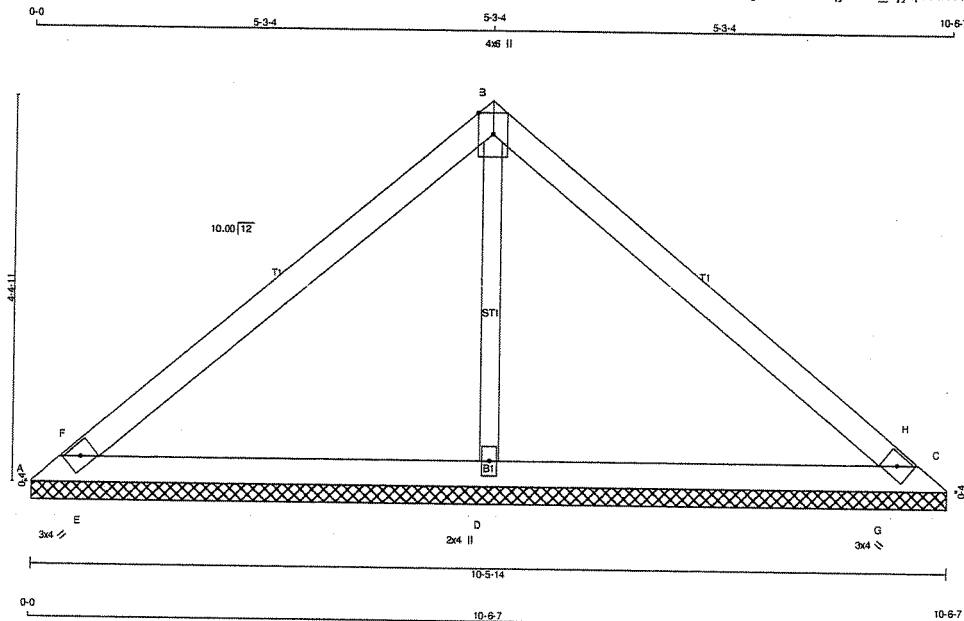


Structural component only
DWG# T-2115530

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
411069	V05	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:25.0

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
B - C	2x4 DRY	No.2	SPF		
A - C	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - B	TBM1-h	MT20	3.0	4.0			
B - C	TTW+p	MT20	4.0	6.0	Edge		
C - B	TBM1-h	MT20	3.0	4.0			
D - B	BMW1+w	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
A	9	0	10-5-14	10-5-14
C	9	0	10-5-14	10-5-14
D	1139	0	10-5-14	10-5-14

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	6	3/0	0/0	0/0	0/0	3/0	0/0
C	6	3/0	0/0	0/0	0/0	3/0	0/0
D	805	530/0	0/0	0/0	0/0	275/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO	A-F	0/456	-91.8	-91.8	0.22 (1)	10.00	D-B	-916/0	0.25 (1)
	F-B	0/409	-91.8	-91.8	0.32 (1)	10.00	E-F	-340/0	0.00 (1)
	B-H	0/409	-91.8	-91.8	0.32 (1)	10.00	G-H	-340/0	0.00 (1)
	H-C	0/456	-91.8	-91.8	0.22 (1)	10.00			
	A-E	-386/0	-18.5	-18.5	0.23 (1)	6.25			
	E-D	-325/0	-18.5	-18.5	0.23 (1)	6.25			
	D-G	-325/0	-18.5	-18.5	0.23 (1)	6.25			
	G-C	-386/0	-18.5	-18.5	0.23 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (B-H:1), BC=0.23/1.00 (D-G:1), WB=0.25/1.00 (B-D:1), SS=0.19/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

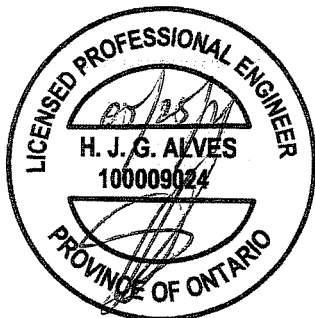
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

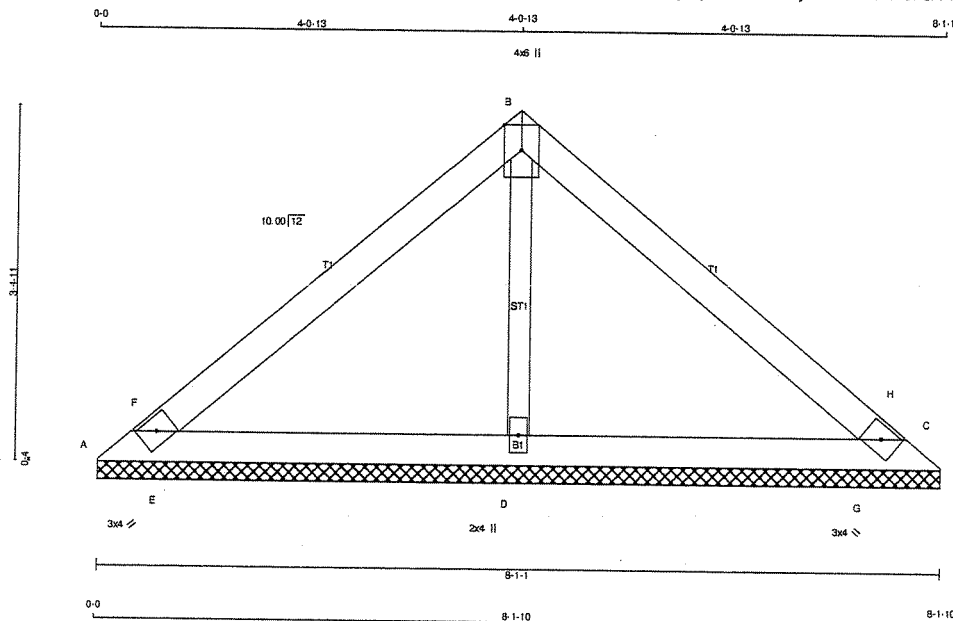
JSI GRIP = 0.46 (D) (INPUT = 0.90)
JSI METAL = 0.19 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115531

JOB NAME 411069	TRUSS NAME V06	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/2"=1'-0"

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TTW+p	MT20	4.0	6.0	Edge
C	TBM1-h	MT20	3.0	4.0	
D	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
A	60	0	60	0	8-1-1	8-1-1
C	60	0	60	0	8-1-1	8-1-1
D	772	0	772	0	8-1-1	8-1-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. LIVE		PERM. LIVE		WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE			
A	42	28 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0	0 / 0
C	42	28 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0	0 / 0
D	546	357 / 0	0 / 0	0 / 0	0 / 0	0 / 0	189 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		W E B S		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MEMB.	MAX. FORCE (LBS)	MAX	CS1 (LC)
FR-TO					FR-TO			
A-F	0 / 247	-91.8	-91.8	0.10 (1)	10.00	D-B	-590 / 0	0.11 (1)
F-B	0 / 231	-91.8	-91.8	0.19 (1)	10.00	E-F	-221 / 0	0.00 (1)
B-H	0 / 231	-91.8	-91.8	0.19 (1)	10.00	G-H	-221 / 0	0.00 (1)
H-C	0 / 247	-91.8	-91.8	0.10 (1)	10.00			
A-E	-222 / 0	-18.5	-18.5	0.14 (1)	6.25			
E-D	-184 / 0	-18.5	-18.5	0.14 (1)	6.25			
D-G	-184 / 0	-18.5	-18.5	0.14 (1)	6.25			
G-C	-222 / 0	-18.5	-18.5	0.14 (1)	6.25			

TOTAL WEIGHT = 22 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (B-H:1), BC=0.14/1.00 (D-G:1), WB=0.11/1.00 (B-D:1), SS=0.13/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

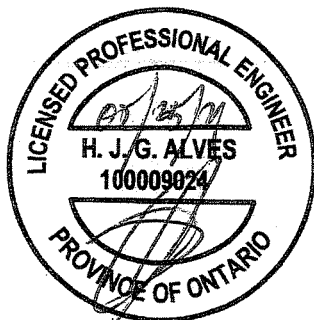
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

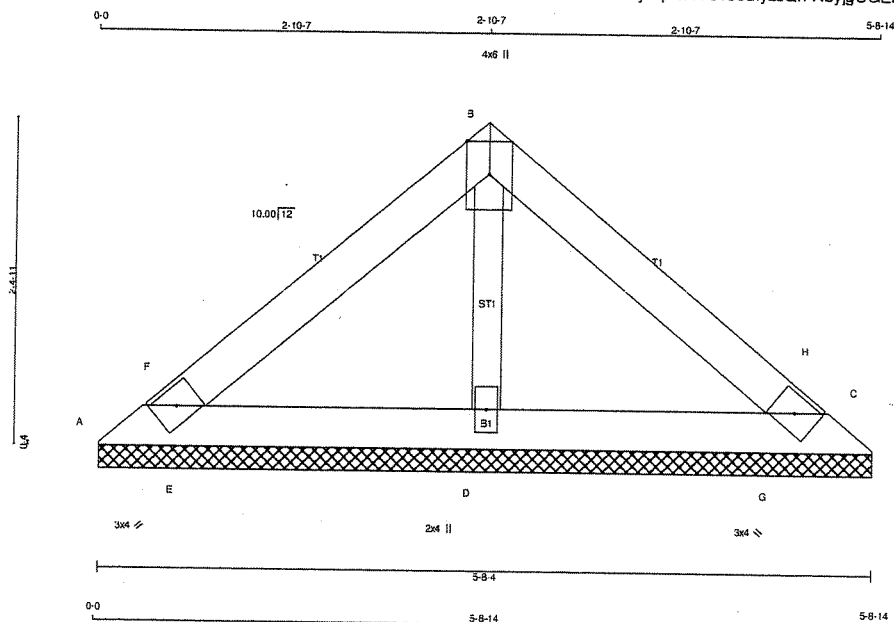


Structural component only
DWG# T-2115532

JOB NAME 411069	TRUSS NAME V07	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 15 lb

LUMBER			
N. L. & A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4 DRY	No.2	SPF
B - C	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
A	79	0	79	0	0	5-8-4	5-8-4	5-8-4	5-8-4
C	79	0	79	0	0	5-8-4	5-8-4	5-8-4	5-8-4
D	468	0	468	0	0	5-8-4	5-8-4	5-8-4	5-8-4

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	56	38 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	0 / 0
C	56	38 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	0 / 0
D	332	215 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM	TO	LENGTH	FR-TO		
A-F	0 / 101	-91.8	-91.8 0.03 (1)	10.00	D-B	-326 / 0	0.05 (1)
F-B	0 / 102	-91.8	-91.8 0.08 (1)	10.00	E-F	-129 / 0	0.00 (1)
B-H	0 / 102	-91.8	-91.8 0.08 (1)	10.00	G-H	-129 / 0	0.00 (1)
H-C	0 / 101	-91.8	-91.8 0.03 (1)	10.00			
A-E	-101 / 0	-18.5	-18.5 0.08 (1)	6.25			
E-D	-81 / 0	-18.5	-18.5 0.08 (1)	6.25			
D-G	-81 / 0	-18.5	-18.5 0.08 (1)	6.25			
G-C	-101 / 0	-18.5	-18.5 0.08 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (B-H:1), BC=0.08/1.00 (D-G:1), WB=0.05/1.00 (B-D:1), SSI=0.07/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

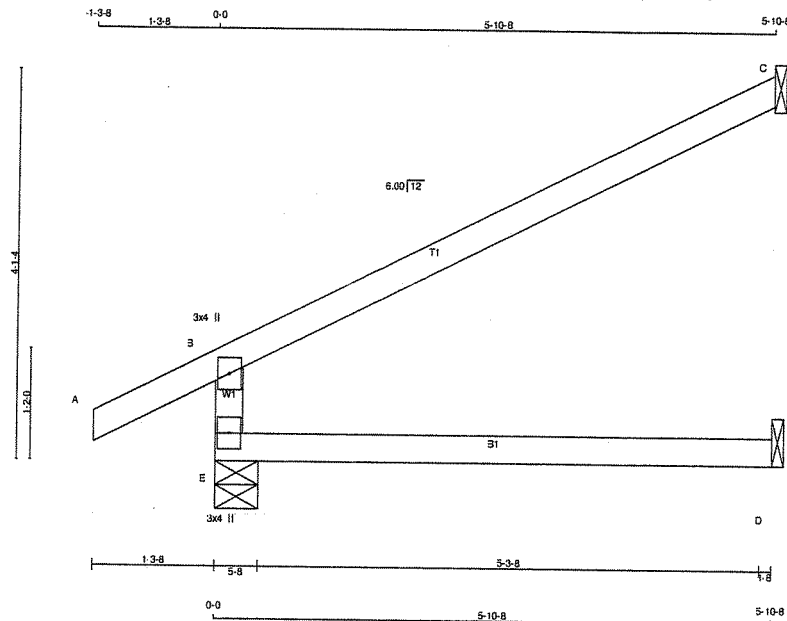
JSI GRIP = 0.16 (D) (INPUT = 0.90)
JSI METAL = 0.07 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115533

JOB NAME 406403	TRUSS NAME J1	QUANTITY 14	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/2" = 1'-0"

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX			MAX. UNBRAC LENGTH FR-TO	MEMB. WEBS MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO	TO			
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

TOTAL WEIGHT = 14 X 17 = 235 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TO=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

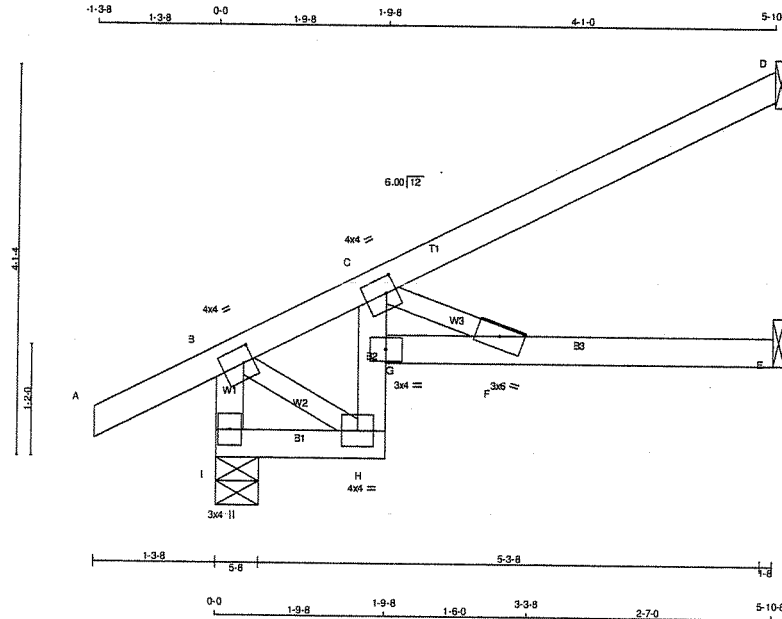
JSI GRIP = 0.18 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115443

JOB NAME 406403	TRUSS NAME J1S	QUANTITY 9	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1/2" = 1'-0"

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT
I	463	0	463	0	0
D	196	0	196	0	0
E	113	0	113	0	0

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	JT	COMBINED
I	325	I	325
D	136	D	136
E	83	E	83

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
I-B	-448 / 0	0.0	0.0	I-B	0 / 210
A-B	0 / 28	-91.8	-91.8	C-F	-515 / 0
B-C	-234 / 0	-91.8	-91.8		
C-D	0 / 4	-91.8	-91.8		
I-H	0 / 0	-18.5	-18.5		
H-G	-82 / 0	0.0	0.0		
G-C	0 / 116	0.0	0.0		
G-F	0 / 460	-18.5	-18.5		
F-E	0 / 0	-18.5	-18.5		

TOTAL WEIGHT = 9 X 21 = 188 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/567$ (0.12")

CSI: TC=0.29/1.00 (C-D:1), BC=0.41/1.00 (F-G:1), WB=0.08/1.00 (C-F:1), SSI=0.21/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.40 (C) (INPUT = 0.90)
JSI METAL = 0.16 (G) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV-w	MT20	3.0	6.0		
G	BVM-I	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

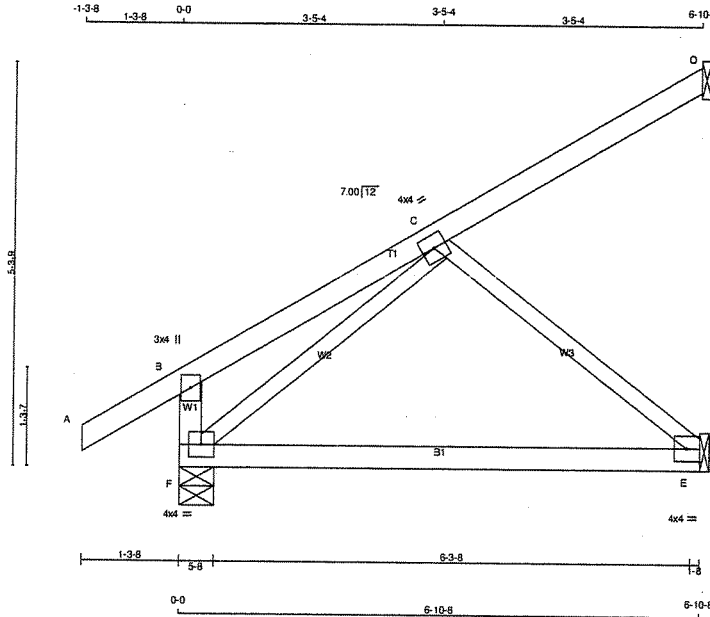
NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115444

JOB NAME 406403	TRUSS NAME J2	QUANTITY 6	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/8" = 1'-0"

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
F - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	
E	BMW1-t	MT20	4.0	4.0	2.00 Edge
F	BMW1-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
F	501	0	0	501	0	0	5-8	5-8	
D	126	0	0	126	0	0	1-8	1-8	
E	255	0	0	255	0	0	1-8	1-8	

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	352	244 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
D	86	70 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0
E	183	107 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD	LC1	MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO		LENGTH	FR-TO			
F-B	-239 / 0	0.0	0.0	0.02 (1)	7.81	C-E	-298 / 0	0.11 (1)	
A-B	0 / 32	-91.8	-91.8	0.12 (1)	10.00	F-C	-302 / 0	0.10 (1)	
B-C	0 / 20	-91.8	-91.8	0.18 (1)	10.00				
C-D	-19 / 0	-91.8	-91.8	0.14 (1)	6.25				
F-E	0 / 228	-18.5	-18.5	0.26 (4)	10.00				

TOTAL WEIGHT = 6 X 27 = 160 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL	LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL. (TL) = $L/850$ (0.10")

CSI: TC=0.18/1.00 (B-C:1), BC=0.26/1.00 (E-F:4),
WB=0.11/1.00 (C-E:1), SS=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

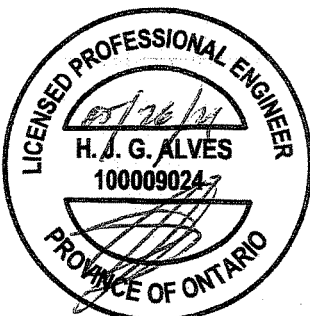
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

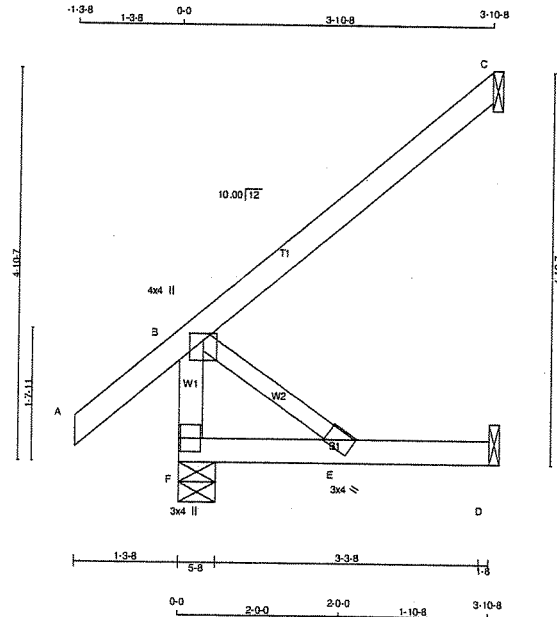
JSI GRIP= 0.27 (C) (INPUT= 0.90)
JSI METAL= 0.08 (E) (INPUT= 1.00)



Structural component only
DWG# T-2115445

JOB NAME 406403	TRUSS NAME J3	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:27.3

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
F - B	2x4 DRY	No.2	SPF	
A - C	2x4 DRY	No.2	SPF	
F - D	2x4 DRY	No.2	SPF	
ALL WEBS	2x3 DRY	No.2	SPF	
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMW+w	MT20	3.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	341	0	341	0	5-8	5-8
C	178	0	178	0	1-8	1-8
D	36	0	40	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND				
F	239	170 / 0	0 / 0	0 / 0	69 / 0	0 / 0		
C	122	99 / 0	0 / 0	0 / 0	23 / 0	0 / 0		
D	29	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CS1 (LC)		W E B S MAX. FACTORED FORCE (LBS)		MAX. CS1 (LC)	
	FR-TO		FROM	TO	LENGTH		FR-TO		LENGTH	
F-B	-305 / 0		0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 41		-91.8	-91.8	0.14 (5)	10.00				
B-C	0 / 0		-91.8	-91.8	0.23 (1)	10.00				
F-E	0 / 0		-18.5	-18.5	0.08 (4)	10.00				
E-D	0 / 0		-18.5	-18.5	0.08 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 16 = 31 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CS1: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

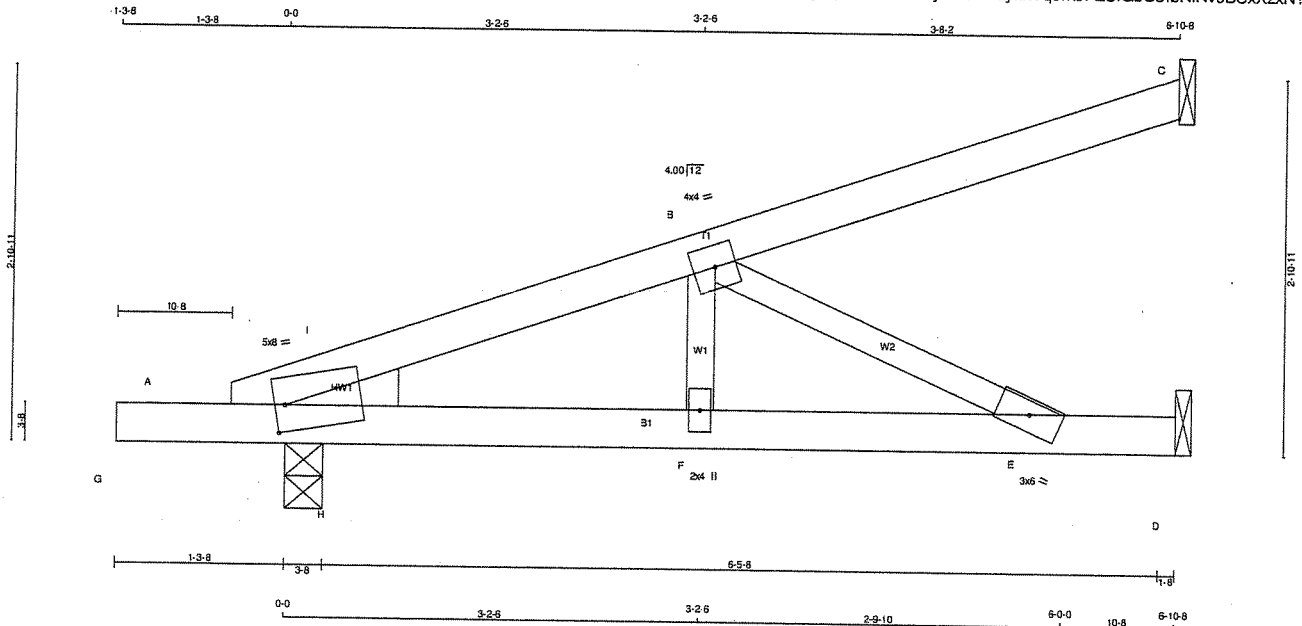


Structural component only
DWG# T-2115446

JOB NAME 406403	TRUSS NAME J4W	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:16.9



LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBMH1m	MT20	5.0	8.0	2.50 1.00
B	TMWW-t	MT20	4.0	4.0	
E	BMW+w	MT20	3.0	6.0	
F	BMW+w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG		HEEL WEDGE
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
A	471	0	471	0	0	3-8	3-8		2x4 L
C	147	0	147	0	0	1-8	1-8		
D	282	0	282	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN COMPONENT REACTIONS		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	VERT	HORZ	DOWN	HORZ						
A	333	218 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0	0 / 0
C	102	81 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0	0 / 0
D	202	119 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (LBS)		MAX. FACTORED VERT. LOAD (LBS)		WEBS MAX. FACTORED FORCE (LBS)		MAX. FACTORED VERT. LOAD (LBS)	
	FR-TO	FROM-TO	FR-TO	FROM-TO	FR-TO	FROM-TO	FR-TO	FROM-TO	FR-TO	FROM-TO
A-I	-792 / 0	-91.8	-91.8	0.07 (1)	6.25	6.25	F-B	0 / 285	0.06 (1)	0.06 (1)
I-B	-770 / 0	-91.8	-91.8	0.10 (1)	6.25	6.25	B-E	-843 / 0	0.17 (1)	0.17 (1)
B-C	-7 / 0	-91.8	-91.8	0.15 (1)	10.00	10.00	H-I	-117 / 0	0.00 (1)	0.00 (1)
G-A	0 / 0	-110.3	-110.3	0.01 (1)	10.00	10.00				
A-H	0 / 743	-18.5	-18.5	0.27 (1)	10.00	10.00				
H-F	0 / 743	-18.5	-18.5	0.27 (1)	10.00	10.00				
F-E	0 / 743	-18.5	-18.5	0.54 (1)	10.00	10.00				
E-D	0 / 0	-18.5	-18.5	0.42 (1)	10.00	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (B-C:1), BC=0.54/1.00 (E-F:1), WB=0.17/1.00 (B-E:1), SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

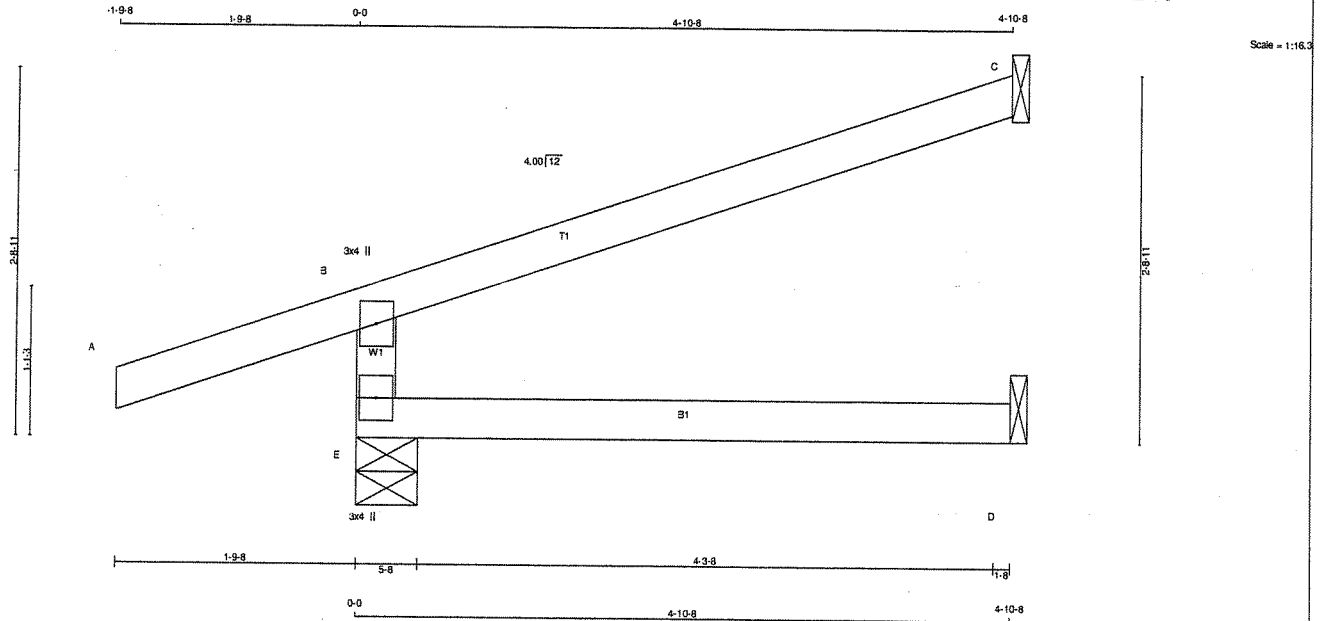
JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)



Structural component only
DWG# T-2115447

JOB NAME 411017	TRUSS NAME J5	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 10:54:24 2021 Page 1
ID:dUe55qQ4UglDof?jKp4h7uzFYDI-6s94y7Ny1J2PWTBZJiVdSE1Xe_BFysFRrCSMIUzD??T



LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	501	0	501	0	5-8	5-8
C	168	0	168	0	1-8	1-8
D	38	0	42	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS				
	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
E	351	250 / 0	0 / 0	0 / 0	101 / 0	0 / 0	
C	116	94 / 0	0 / 0	0 / 0	22 / 0	0 / 0	
D	30	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED				WEBS	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LOAD LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	MAX. FACTORED CSI (LC)
E-B	-448 / 0	0.0	0.0	0.09 (4)	7.81			
A-B	0 / 27	-91.8	-91.8	0.24 (5)	10.00			
B-C	-18 / 0	-91.8	-91.8	0.37 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.37/1.00 (B-C:1), BC=0.09/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



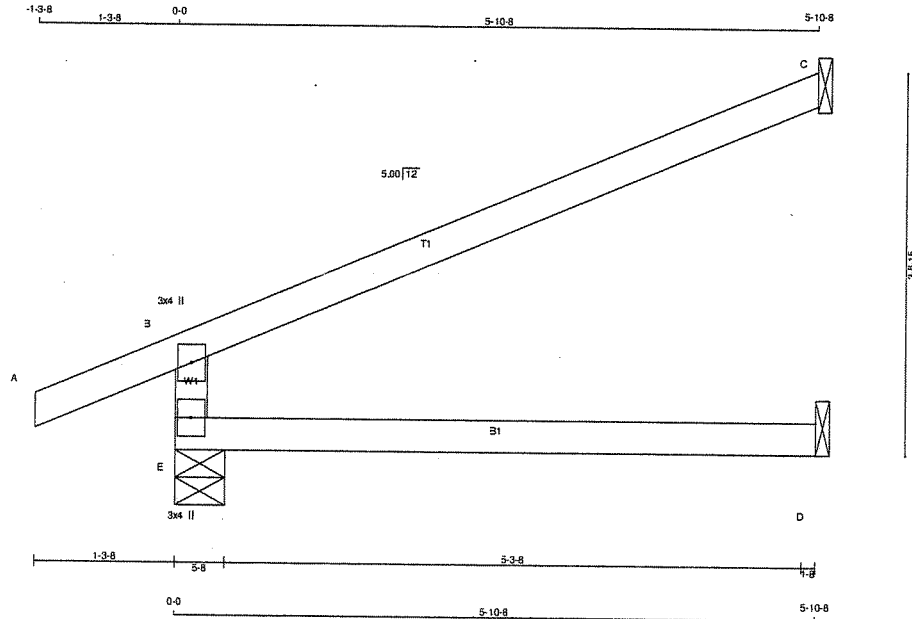
Structural component only
DWG# T-2115479

Structural component only
DWG# T-2115480

JOB NAME 411017	TRUSS NAME J30	QUANTITY 17	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/20.1

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS			
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
E	525	0	525	0	0	0	5-8	5-8	
C	202	0	202	0	0	1-8	1-8		
D	44	0	50	0	0	1-8	1-8		

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	368	257 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 625 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (PLF)	MAX. UNBRAC LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. UNBRAC LENGTH (LC)
FR-TO					FR-TO		
E-B	-461 / 0	0.0	0.0	0.14 (4)	7.81		
A-B	0 / 24	-91.8	-91.8	0.12 (1)	10.00		
B-C	-26 / 0	-91.8	-91.8	0.54 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00		

TOTAL WEIGHT = 17 X 16 = 278 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

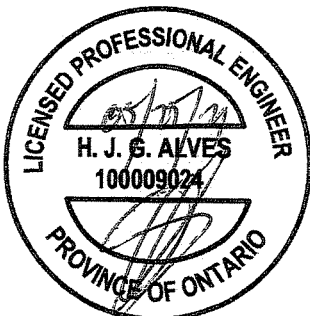
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (E) (INPUT = 0.90)
JSI METAL = 0.11 (B) (INPUT = 1.00)

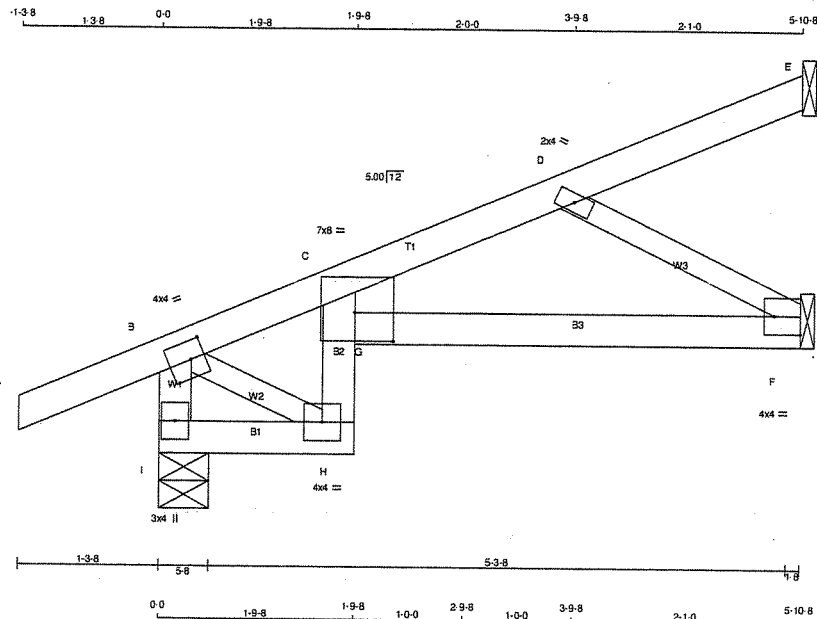


Structural component only
DWG# T-2115481

JOB NAME 411017	TRUSS NAME J30S	QUANTITY 9	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/20.1

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
I - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY SEASONED LUMBER				SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C					
C	TMBV*m	MT20	7.0	8.0	3.25 4.25
D	TMVW+w	MT20	2.0	4.0	
F	BMW1-t	MT20	4.0	4.0	2.00 Edge
G					
H	BMVW-t	MT20	4.0	4.0	
I	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
I	456	0	456	0	0	5-8	5-8	5-8	5-8
E	53	0	53	0	0	1-8	1-8	1-8	1-8
F	259	0	259	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) E, F

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	320	225 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
E	36	35 / 0	0 / 0	0 / 0	0 / 0	1 / 0	0 / 0
F	186	110 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
FR-TO				FR-TO			
I-B	-466 / 0	0.0	0.0 0.02 (4)	B-H	0 / 185	0.04 (1)	
A-B	0 / 24	-91.8	-91.8 0.12 (1)	D-F	-388 / 0	0.07 (1)	
B-C	-235 / 0	-91.8	-91.8 0.14 (1)				
C-D	-313 / 0	-91.8	-91.8 0.18 (1)				
D-E	-17 / 0	-91.8	-91.8 0.13 (1)				
I-H	0 / 18	-18.5	-18.5 0.06 (1)				
H-G	-36 / 7	0.0	0.0 0.18 (1)				
G-C	0 / 54	0.0	0.0 0.12 (1)				
G-F	0 / 326	-18.5	-18.5 0.14 (1)				

TOTAL WEIGHT = 9 X 21 = 187 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.18/1.00 (C-D:1), BC=0.18/1.00 (G-H:1), WB=0.07/1.00 (D-F:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

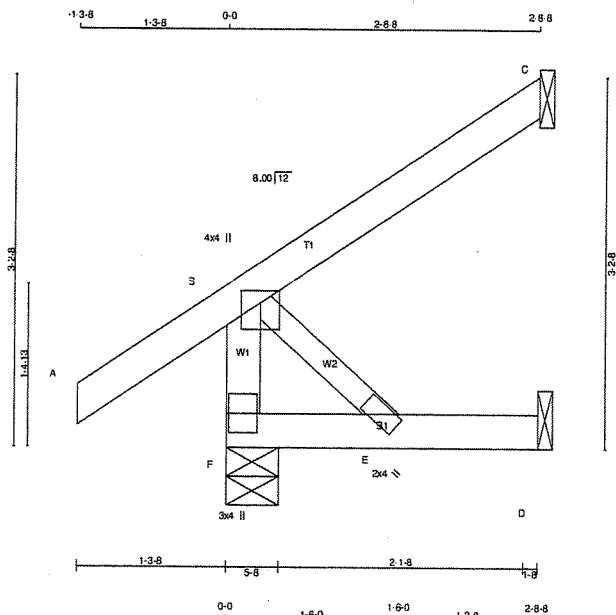
JSI GRIP = 0.29 (B) (INPUT = 0.90)
JSI METAL = 0.19 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115482

JOB NAME 411017	TRUSS NAME J32	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:19.0

TOTAL WEIGHT = 4 X 11 = 45 lb

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
E	BMVW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	275	0	275	0	5-8	5-8
C	124	0	124	0	1-8	1-8
D	25	0	28	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	192	140 / 0	0 / 0	0 / 0	0 / 0	0 / 0	53 / 0
C	86	69 / 0	0 / 0	0 / 0	0 / 0	0 / 0	16 / 0
D	20	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	20 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO		
F-B	-250 / 0	0.0	0.0	0.03 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 35	-91.8	-91.8	0.13 (5)			
B-C	0 / 0	-91.8	-91.8	0.11 (1)			
F-E	0 / 0	-18.5	-18.5	0.04 (4)			
E-D	0 / 0	-18.5	-18.5	0.04 (4)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B-5), BC=0.04/1.00 (D-E-4), WB=0.00/1.00 (B-E-1), SS=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

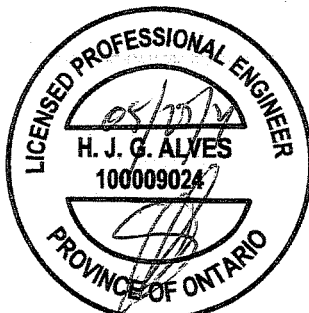
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

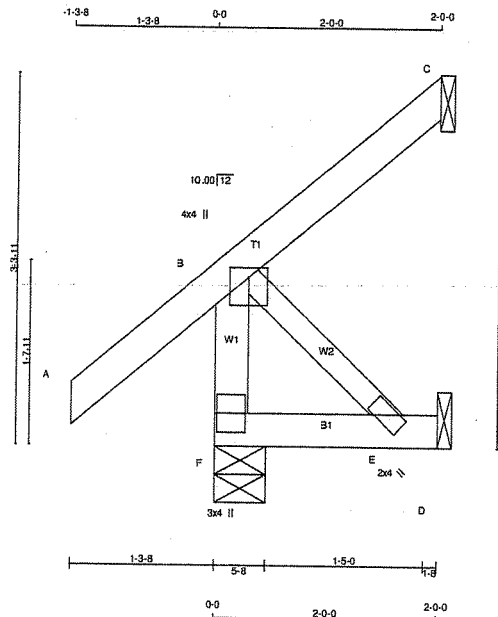
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115483

JOB NAME 411071	TRUSS NAME J60	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu May 27 12:11:46 2021 Page 1			
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		Scale = 1:19.6			



LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS				SPF	
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
F	237	0	237	0	5-8
C	92	0	92	0	1-8
D	18	0	21	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
F	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	165	122 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
C	63	51 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
D	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
F-B	-219 / 0	0.0	0.02 (1)	B-E	0 / 0	0.00 (1)	
A-B	0 / 41	-91.8	-91.8 0.13 (1)				
B-C	0 / 0	-91.8	-91.8 0.06 (1)				
F-E	0 / 0	-18.5	-18.5 0.02 (4)				
E-D	0 / 0	-18.5	-18.5 0.02 (4)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 10 = 41 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

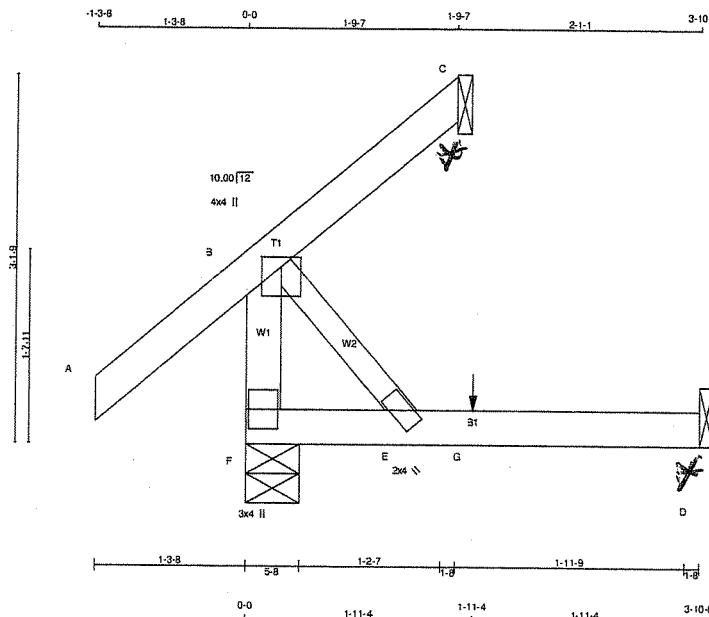
JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2116275

JOB NAME 406516	TRUSS NAME C1	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 3 X 12 = 36 lb

LUMBER				DESCR	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS					
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	4.0	4.0	1.00 2.00
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
F	294	0	294	0	0	5-8	5-8	5-8	
C	33	0	33	0	-40	1-8	1-8	1-8	
D	36	0	40	0	0	1-8	1-8	1-8	

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	207	144 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	23	18 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT.	LC1 MAX (PLF)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
F-B	-258 / 0	0.0	0.0	0.03 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 41	-91.8	-91.8	0.14 (5)			
B-C	-32 / 0	-91.8	-91.8	0.14 (5)			
F-E	0 / 0	-18.5	-18.5	0.07 (4)			
E-G	0 / 0	-18.5	-18.5	0.08 (4)			
G-D	0 / 0	-18.5	-18.5	0.08 (4)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TG=0.14/1.00 (A-B-5), BC=0.08/1.00 (D-E-4),
WB=0.00/1.00 (B-E-1), SS=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

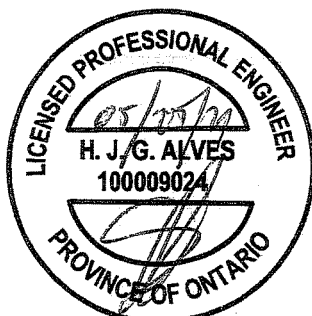
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

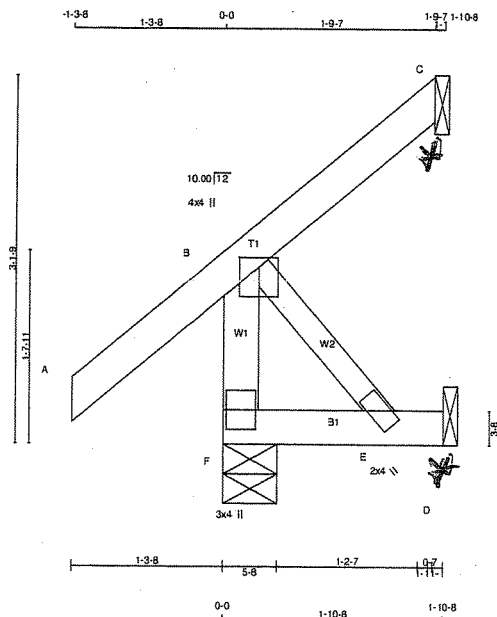
JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115471

JOB NAME 406516	TRUSS NAME C2	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:18.7

TOTAL WEIGHT = 3 X 10 = 29 lb

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMW+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
F	276	0	276	0	0	5-8	5-8		
C	33	0	33	0	-40	1-8	1-8		
D	17	0	19	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	192	144 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	23	18 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED				WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD	LC1	MAX	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO			FR-TO			
F-B	-258 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00				
B-C	-32 / 0	-91.8	-91.8	0.12 (1)	6.25				
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL	LD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.90)

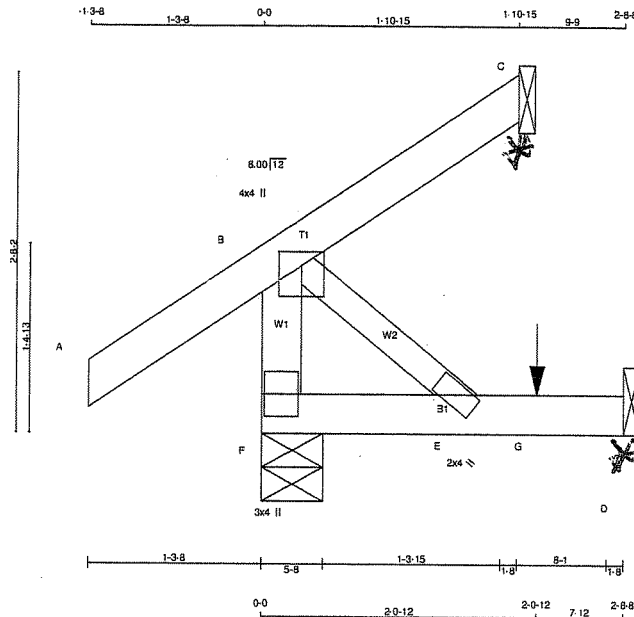
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115472

JOB NAME 411017	TRUSS NAME C30	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 2 X 10 = 20 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

DESCR.
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ
JT	285	0	285	0
F	42	0	42	0
C	42	0	42	0
D	28	0	31	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE	SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
F	199	144 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	29	24 / -25	0 / 0	0 / 0	6 / 0	0 / 0
D	22	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MAX. UNBRAC. LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
F-B	-259 / 0	0.0	0.0	0.03 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 35	-91.8	-91.8	0.14 (5)			
B-C	-25 / 0	-91.8	-91.8	0.13 (5)			
F-E	0 / 0	-18.5	-18.5	0.04 (4)			
E-G	0 / 0	-18.5	-18.5	0.04 (4)			
G-D	0 / 0	-18.5	-18.5	0.04 (4)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-3	-3	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B-5), BC=0.04/1.00 (E-F-4),
WB=0.00/1.00 (B-E-1), SS=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

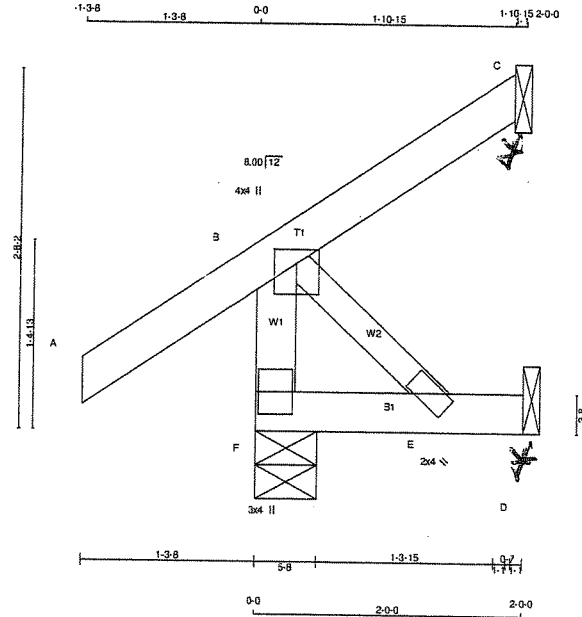


Structural component only
DWG# T-2115477

JOB NAME 411017	TRUSS NAME C31	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue May 25 10:54:23 2021 Page 1
ID:E?n9mn1Ky9qxx7v78133ulyzsQn-egbiinMKG0wYUcMl_zOw1UQlas4DP?HcYipl2zD??U



Scale = 1:16.4

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4 DRY	No.2		
A - C	2x4 DRY	No.2		
F - D	2x4 DRY	No.2		
ALL WEBS	2x3 DRY	No.2		
DRY: SEASONED LUMBER.				

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REORD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	DOWN	HORZ	UPLIFT	IN-SX
F	277	0	0	5-8
C	42	0	-35	1-8
D	18	0	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	193	144 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
C	29	24 / -25	0 / 0	0 / 0	0 / 0	6 / 0	0 / 0
D	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CSI (LC)	MEMB. FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO
F-B	-259 / 0	0.0	0.03 (1)	7.81
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00
B-C	-25 / 0	-91.8	-91.8 0.12 (1)	6.25
F-E	0 / 0	-18.5	-18.5 0.02 (4)	10.00
E-D	0 / 0	-18.5	-18.5 0.02 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 9 = 18 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

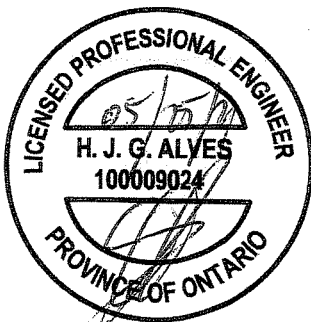
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT= 0.90)
JSI METAL= 0.05 (B) (INPUT= 1.00)



Structural component only
DWG# T-2115478



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
MINIMUM NUMBER OF TOE-NAILS					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

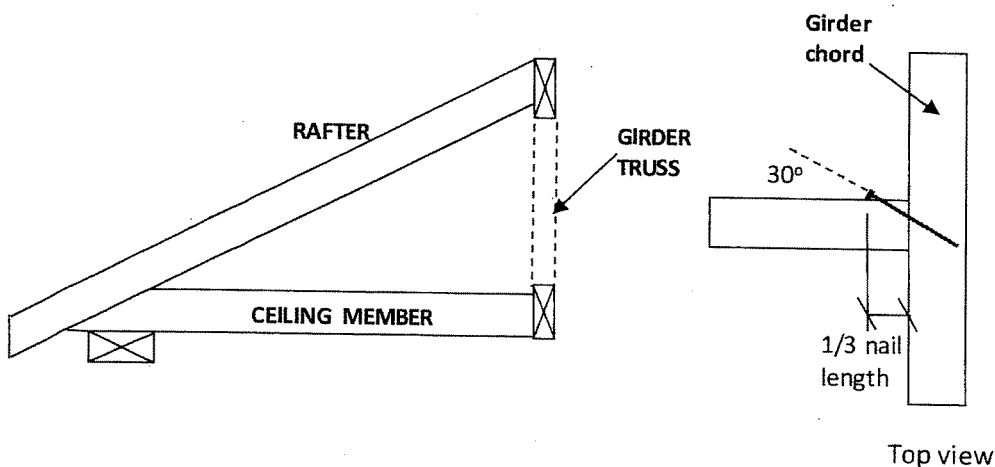
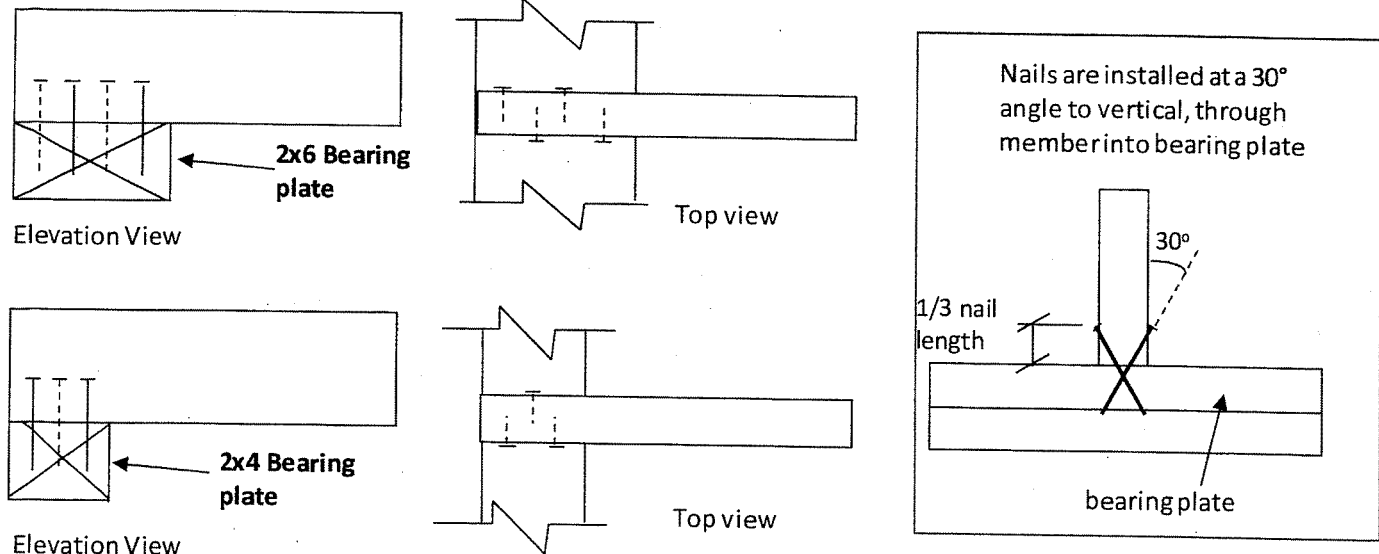


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



TOE-NAIL CAPACITY DETAILS

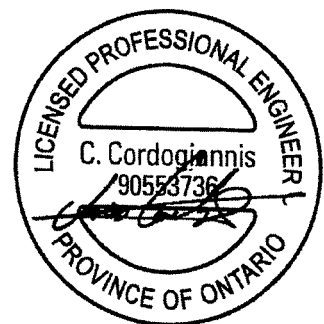
Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

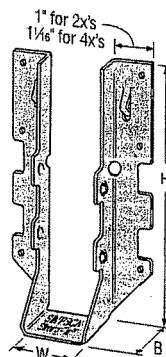
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

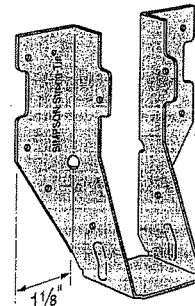
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

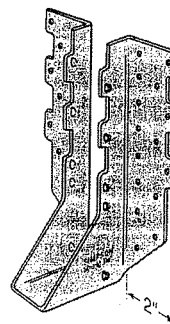
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



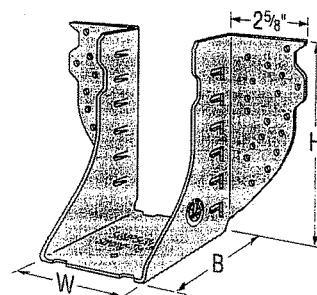
LUS28



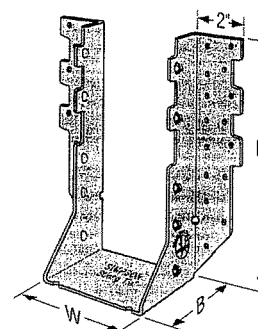
LU26L



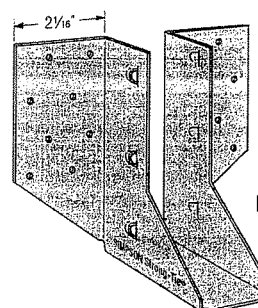
HUS210
(HUS26, HUS28,
and HHUS similar)



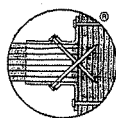
HGUS28-2



HHUS210-2



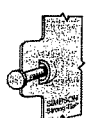
LJS26DS



Double-Shear
Nailing
Top View

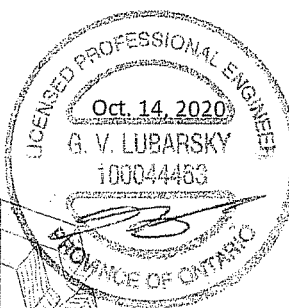
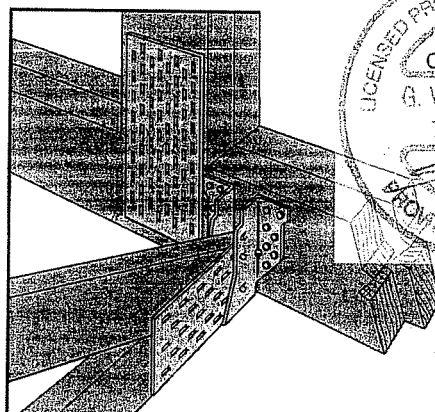


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing
Side View
(available on
some models)

**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

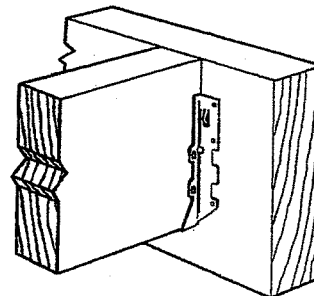
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

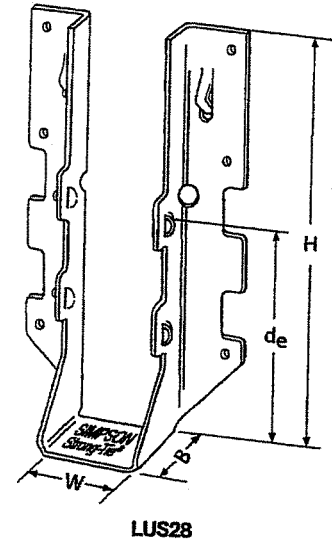
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

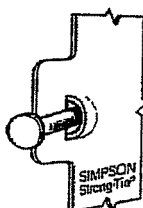


Typical LUS Installation

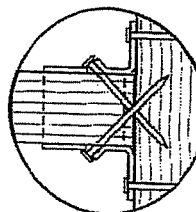


Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1⅞	3⅞	1¾	1⅞	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅞	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⅞	4¾	1¾	3⅞	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅞	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8⅞	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

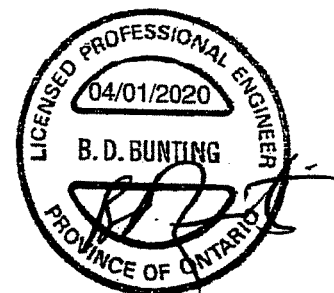
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

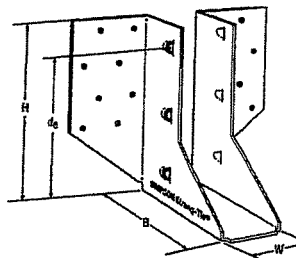
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

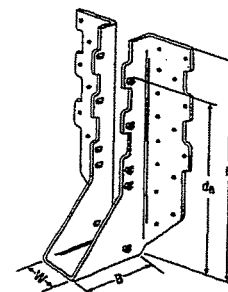
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

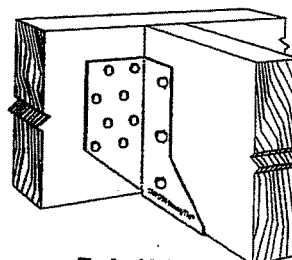
- See current catalogue for options



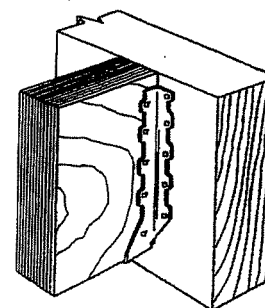
LJS26DS



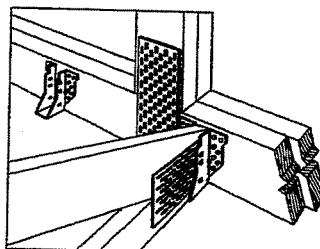
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



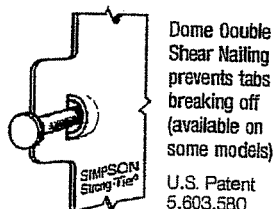
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

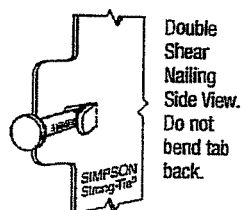
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _o =1.15) lb.	Normal (K _o =1.00) lb.	Uplift (K _o =1.15) lb.	Normal (K _o =1.00) lb.
LJS26DS	18	1 1/16	5	3 1/2	4 3/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 1/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/32	3	7 3/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

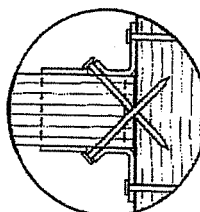


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

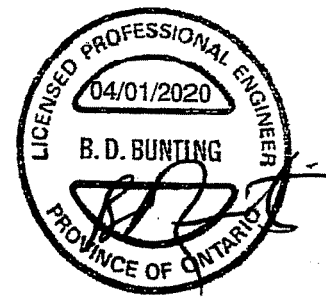
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

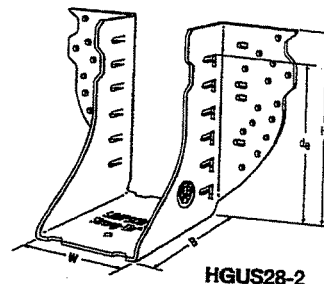
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

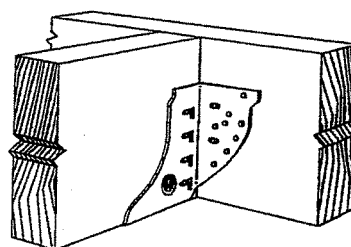
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

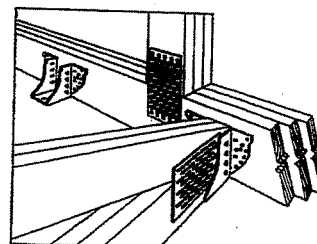
- See current catalogue for options



HGUS28-2



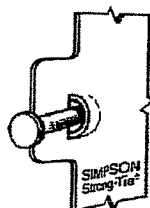
Typical HGUS
Installation



Typical HGUS Installation
(Truss Designer to
provide fastener quantity
for connecting multiple
members together)

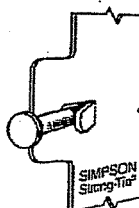
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_o is the distance from the seat of the hanger to the highest joist nail.

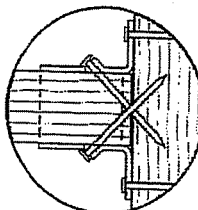


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

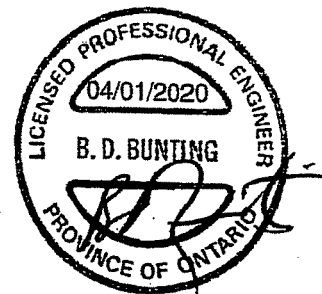
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

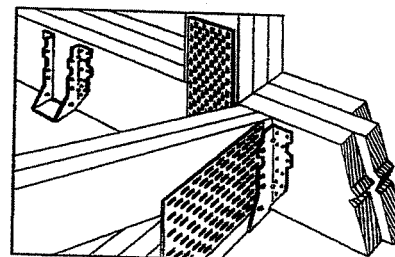
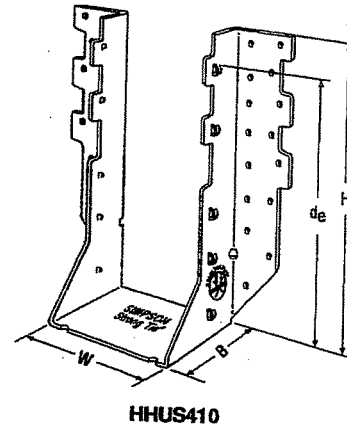
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

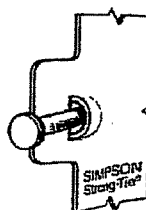
Options:

- See current catalogue for options

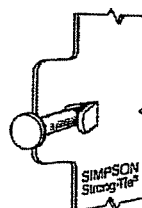


Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HHUS26-2	14	3 1/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 1/16	7 1/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 1/16	9 3/32	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/16	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 3/16	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 3/16	7 1/16	3	6 1/16	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 3/16	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 3/16	7 29/32	(30) 16d	(10) 16d	4670	10155	3370	7210

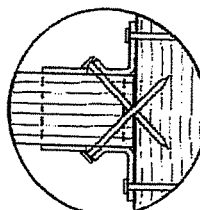
1. d_e is the distance from the seat of the hanger to the highest joist nail.



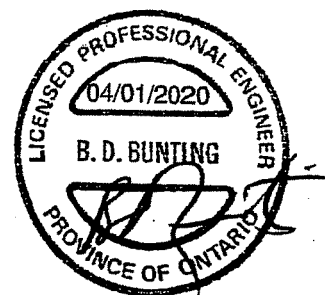
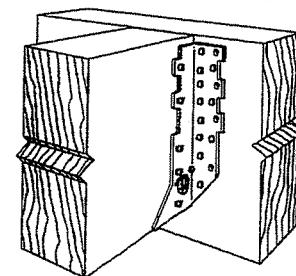
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

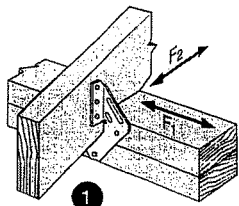
© 2020 Simpson Strong-Tie Company Inc.

T-SPECHUS20 3/20 exp. 6/22

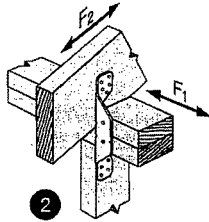
(800) 999-5099
strongtie.com

H/TSP

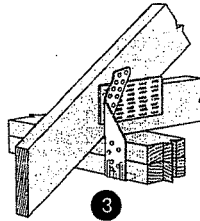
Seismic and Hurricane Ties (cont.)



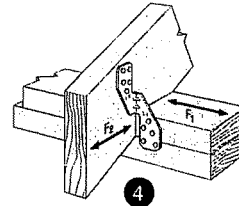
1 H1 Installation



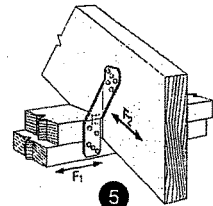
2 H2A Installation



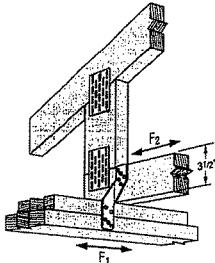
3 TSP Installation



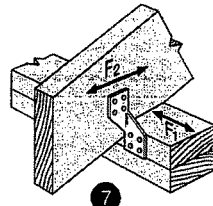
4 H2.5A Installation
(Nails into both top plates)



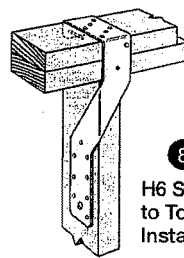
5 H2.5T Installation
(Nails into both top plates)



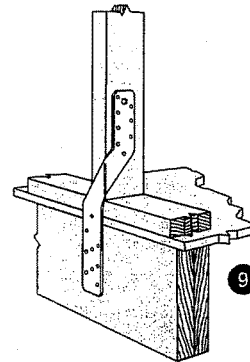
6 H2.5T Installation



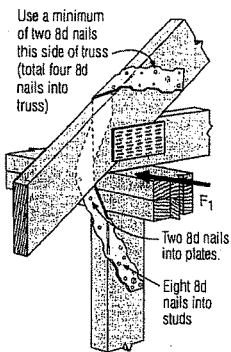
7 H3 Installation
(Nails into upper top plate)



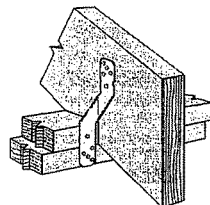
8 H6 Stud to Top Plate Installation



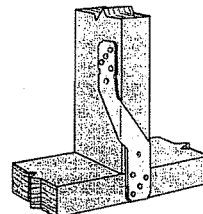
9 H6 Stud to Band Joist Installation



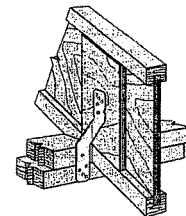
10 H7Z Installation



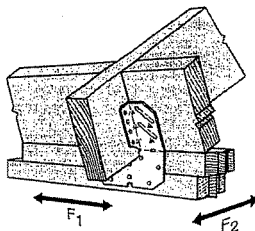
11 H8 Attaching Rafter to Double Top Plates



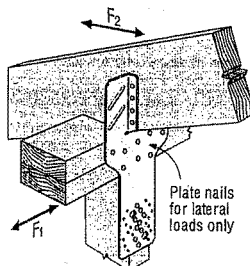
12 H8 attaching Stud to Sill
(4 8d into plate, 5 8d into stud)



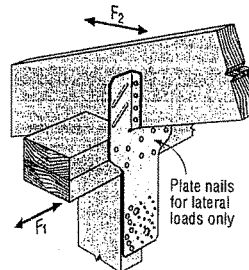
13 H8 attaching I-Joist to Double Top Plates



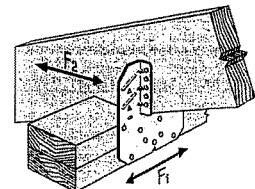
14 H10A Field-Bent Installation



15 H10S Installation

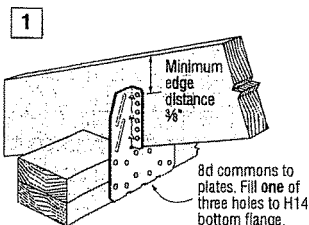


16 H10S Installation with Stud Offset

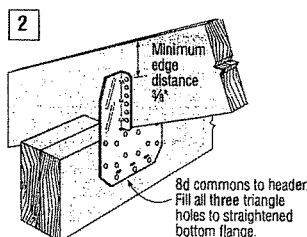


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

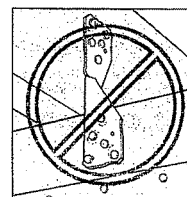


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

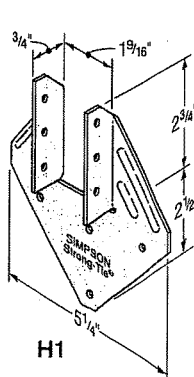
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

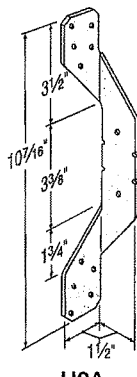
Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

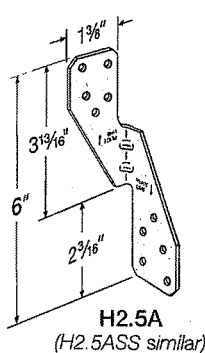
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12; use H10A sloped loads for field bent installation.



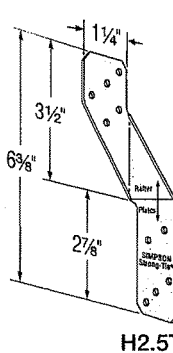
H1



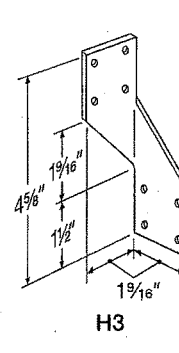
H2A
(H2ASS similar)



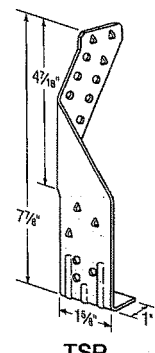
H2.5A
(H2.5ASS similar)



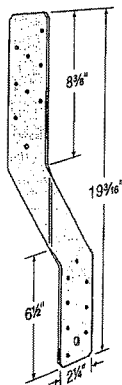
H2.5T



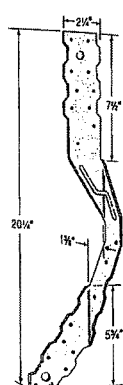
H3



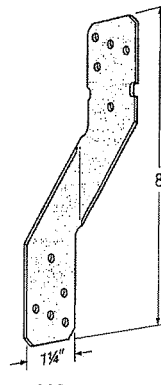
TSP



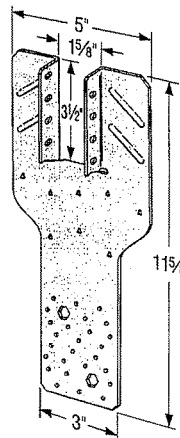
H6



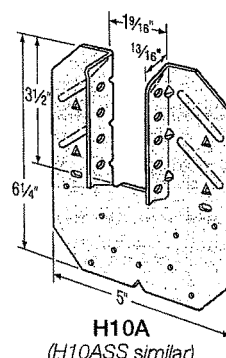
H7Z



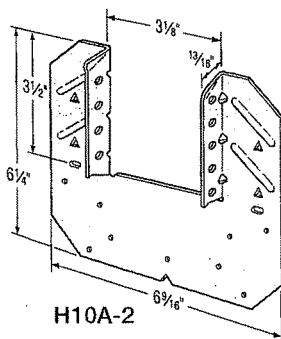
H8



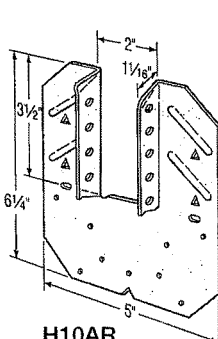
H10S



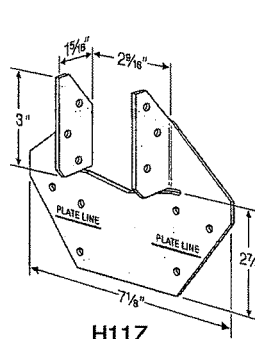
H10A
(H10ASS similar)



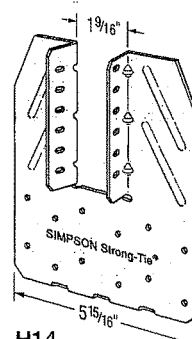
H10A-2



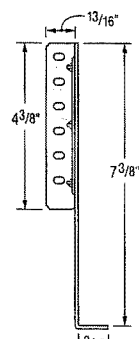
H10AR



H11Z



H14



H14
Profile

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

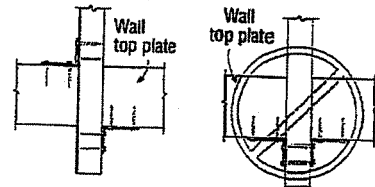
Design: • Factored resistances are in accordance with CSA 086-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

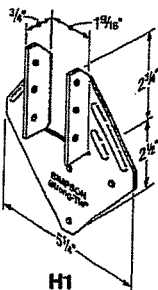
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

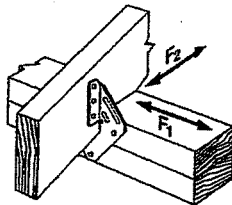


Install diagonally across from each other for minimum 2x truss.

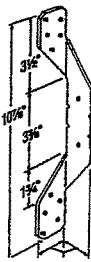
Nailing into both sides of a single ply 2x truss may cause the wood to split.



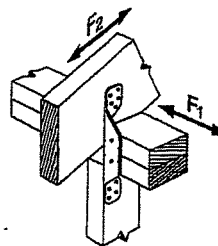
H1



H1 Installation



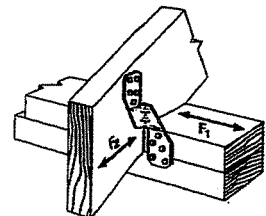
H2A



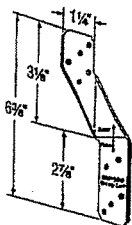
H2A Installation



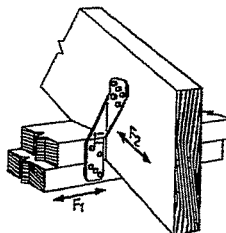
H2.5A



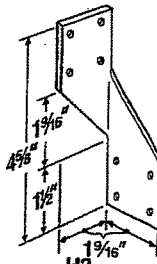
H2.5A Installation



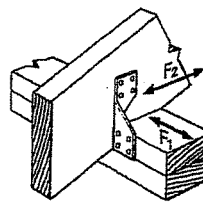
H2.5T



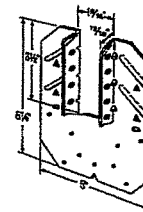
H2.5T Installation
(Nails into both top plates)



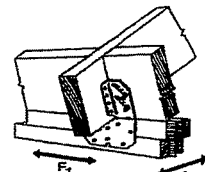
H3



H3 Installation



H10A

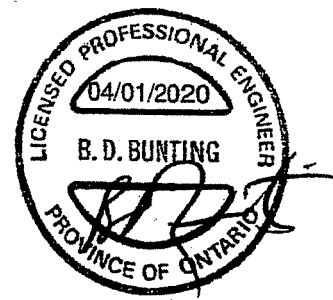


H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
					(K _p =1.15)					(K _p =1.15)
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TC – Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

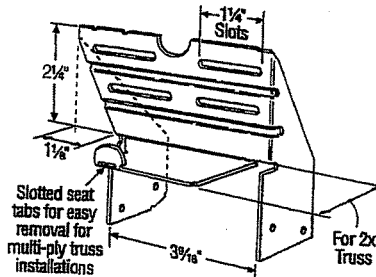
Design: Factored resistances are in accordance with CSA 086-14

Installation:

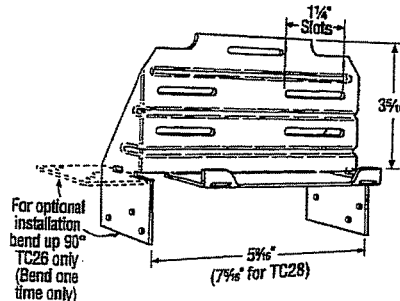
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

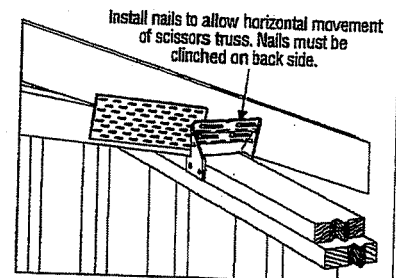
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



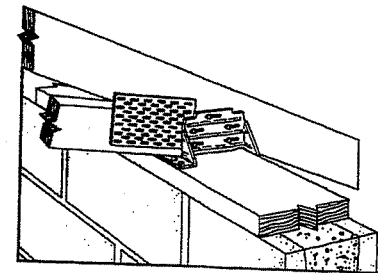
TC24
U.S. Patent 4,932,173



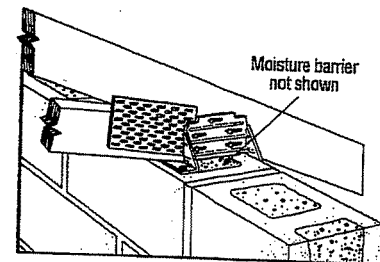
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



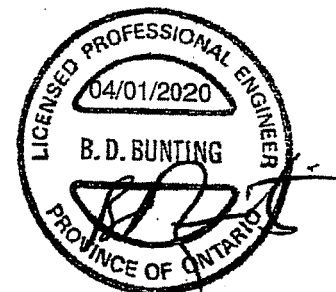
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed, reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



LIMIT
STATES
DESIGN

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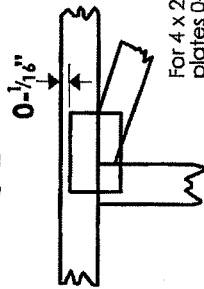
T-SPECTC20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

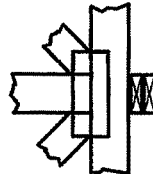
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



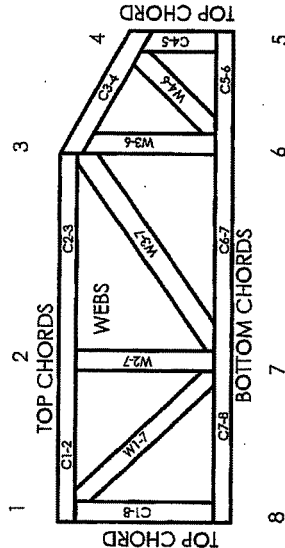
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek

MiTek

POWER TO PERFORM.™

Mitek Engineering Reference Sheet, MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Comber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to comber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.