

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment).
Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

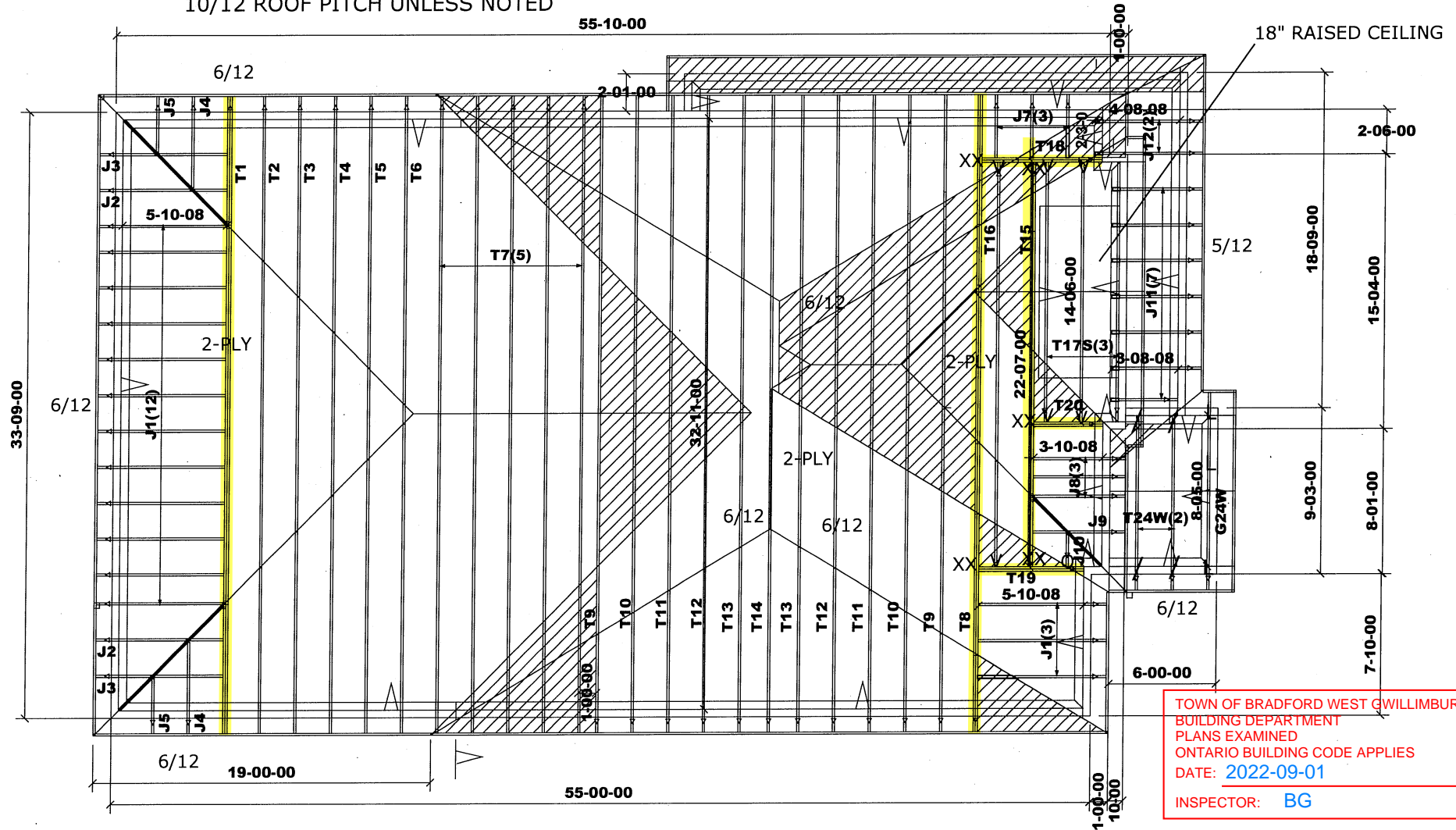
DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

 **DENOTES:
CONVENTIONAL
FRAMING**

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LSTA9- (L)
H2.5T- (I)

10/12 ROOF PITCH UNLESS NOTED



TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2022-09-01
INSPECTOR: BG

REVIEWED



Job Track: **50465**

Plan Log: **205577**

Layout ID: **423666**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: 2022-07-11

Sales: Rick DiCiano

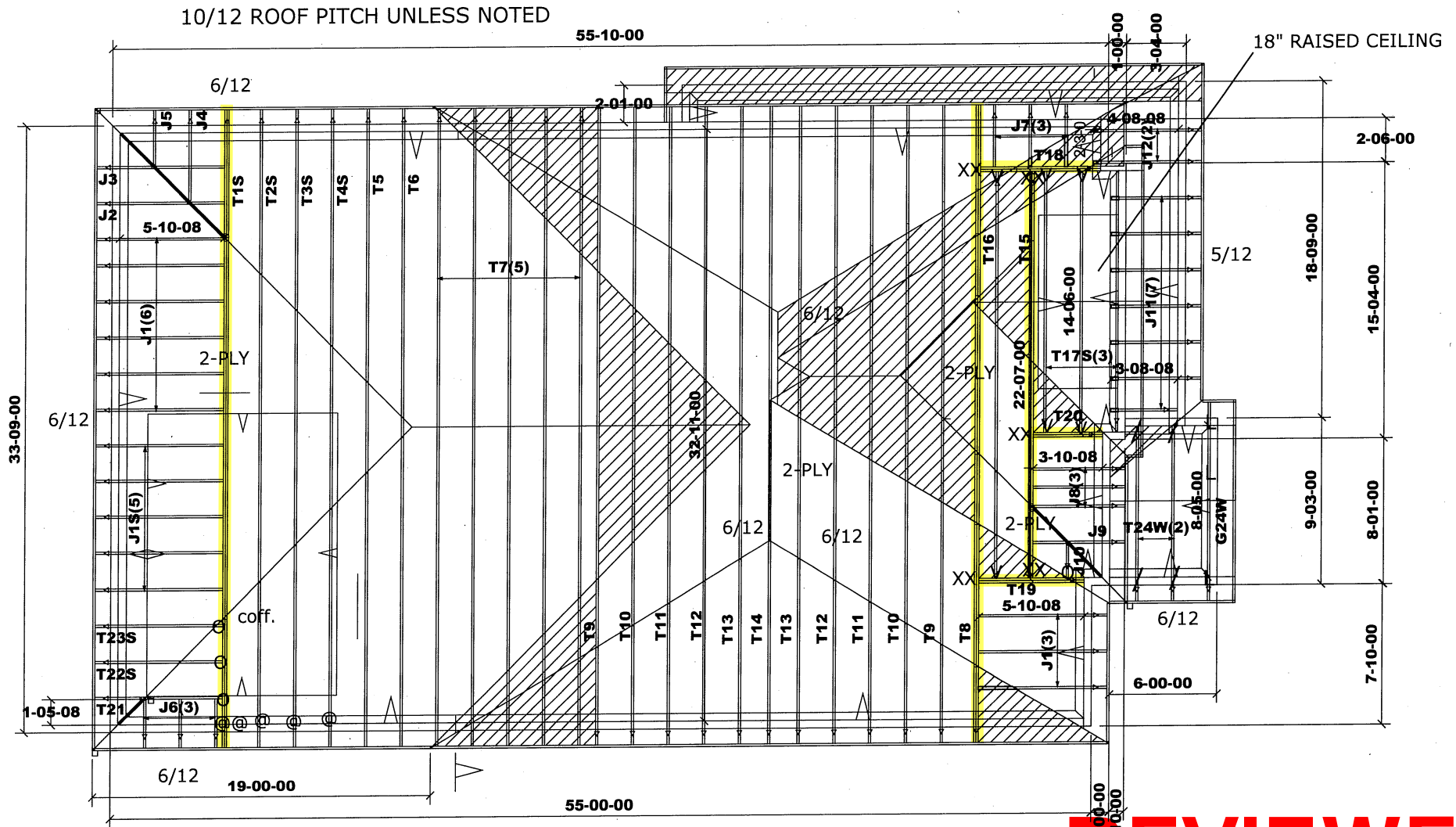
Designer: JG

Model / Elevation:

S42-21 / A

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Mitek ver 8.5.3.233

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LSTA9- (L)
H2.5T- (I)
TC26-(@)



100% REVIEWED

Job Track: **50465**Plan Log: **205577**

Layout ID: 423665

Builder / Location:	
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BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: 2022-07-11

Sales:	Rick DiCiano
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Designer: JG

Model / Elevation:	
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S42-21 / A- OPT.WITH COFF.

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Mitek ver 8.5.3.233

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2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

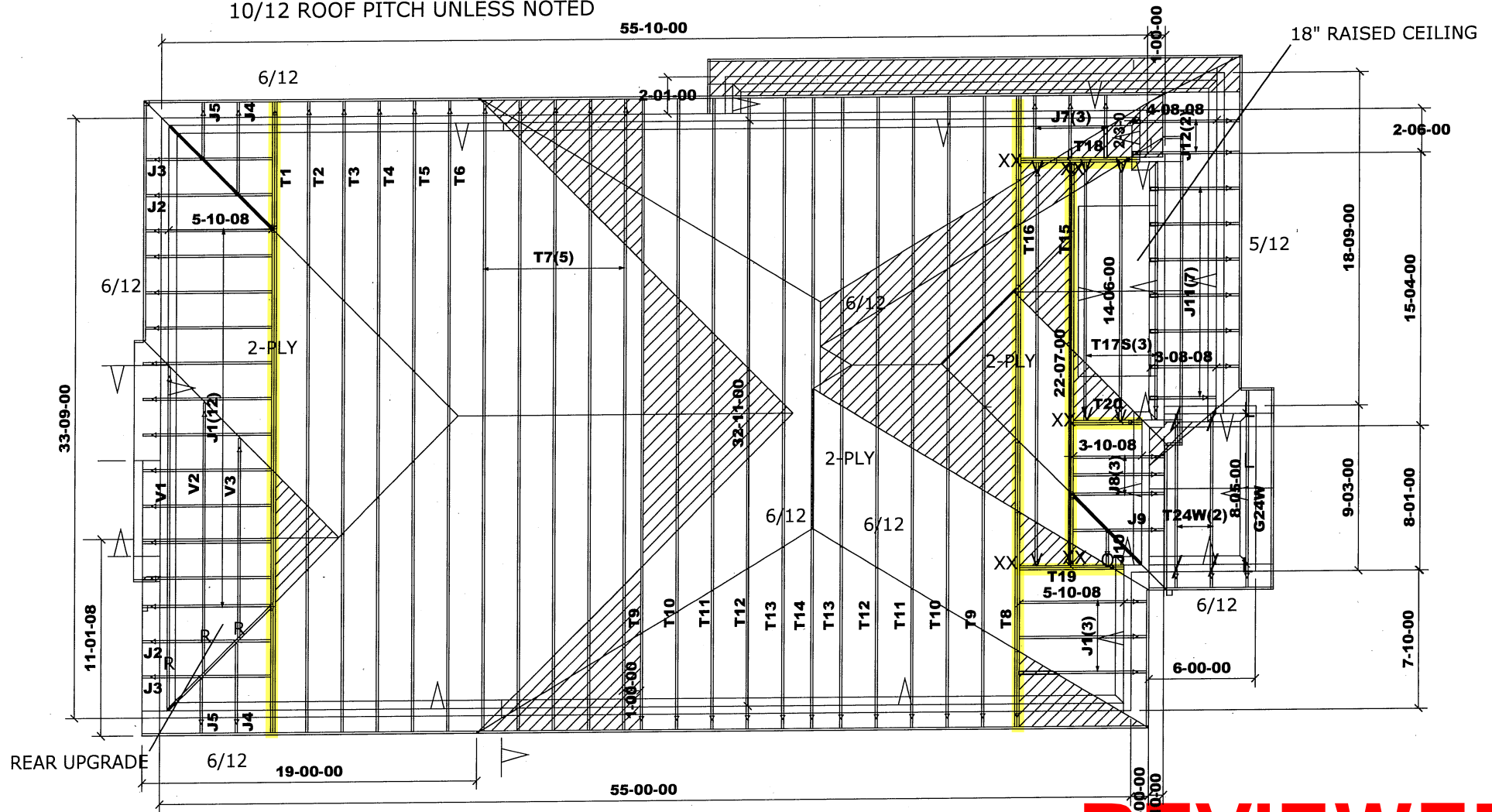
DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

 DENOTES:
CONVENTIONAL
FRAMING

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LSTA9- (L)
H2.5T- (I)
VTCR-(R)

10/12 ROOF PITCH UNLESS NOTED



REVIEWED



Job Track: **50465**
Plan Log: **205577**
Layout ID: **423667**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **2022-07-11** Sales: Rick DiCiano Designer: JG

Model / Elevation:

S42-21 / A-REAR UPGRADE

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Mitek ver 8.5.3.233

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FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

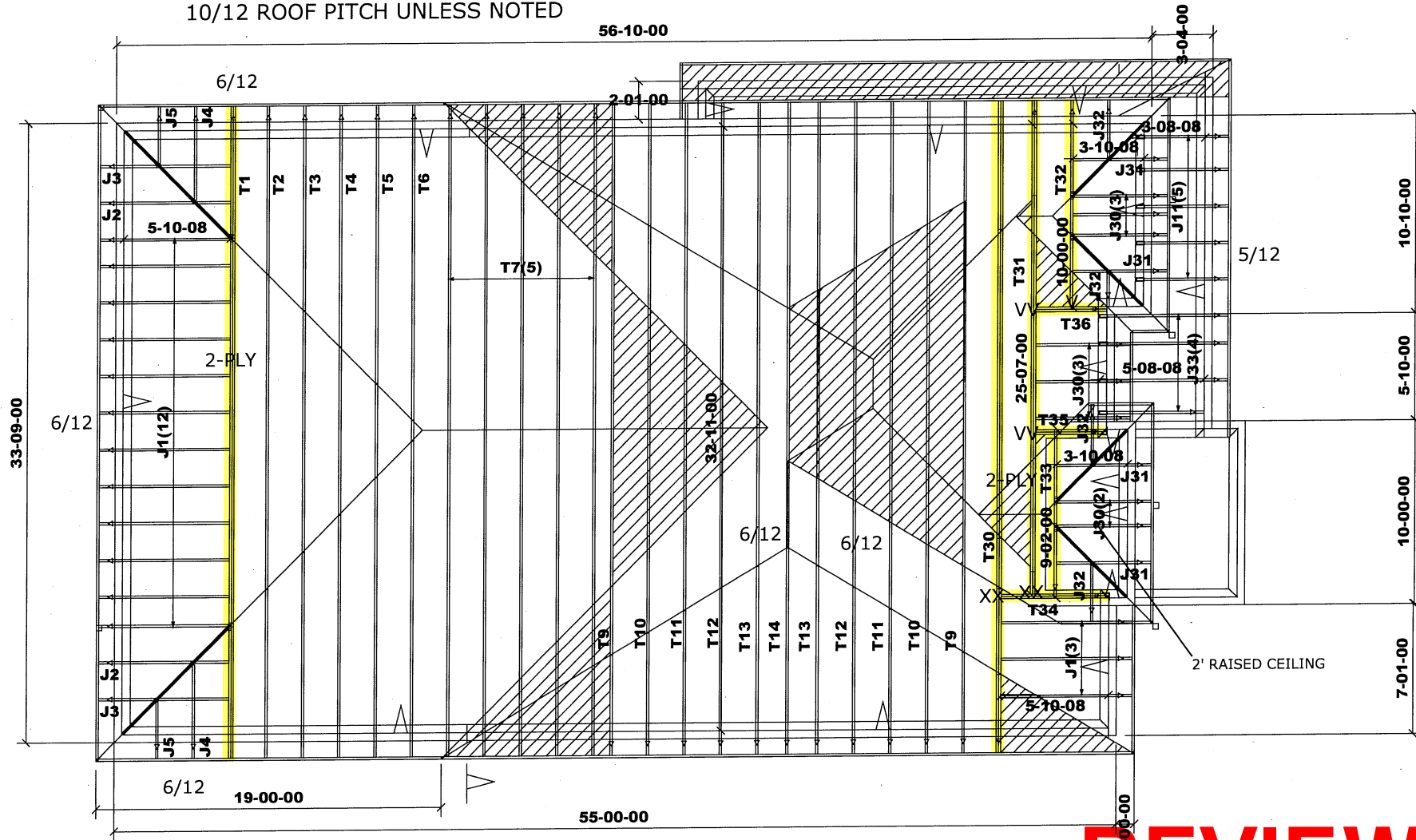
DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
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BCDL = 7.4 psf

 **DENOTES:**
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LSTA9- (L)
H2.5T- (I)

10/12 ROOF PITCH UNLESS NOTED



REVIEWED



Job Track: **50465**
Plan Log: **205577**
Layout ID: **423669**

Builder / Location:
BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **2022-07-11** Sales: Rick DiCiano Designer: JG

Model / Elevation:
S42-21 / B

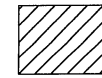
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Mitek ver 8.5.3.233

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ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

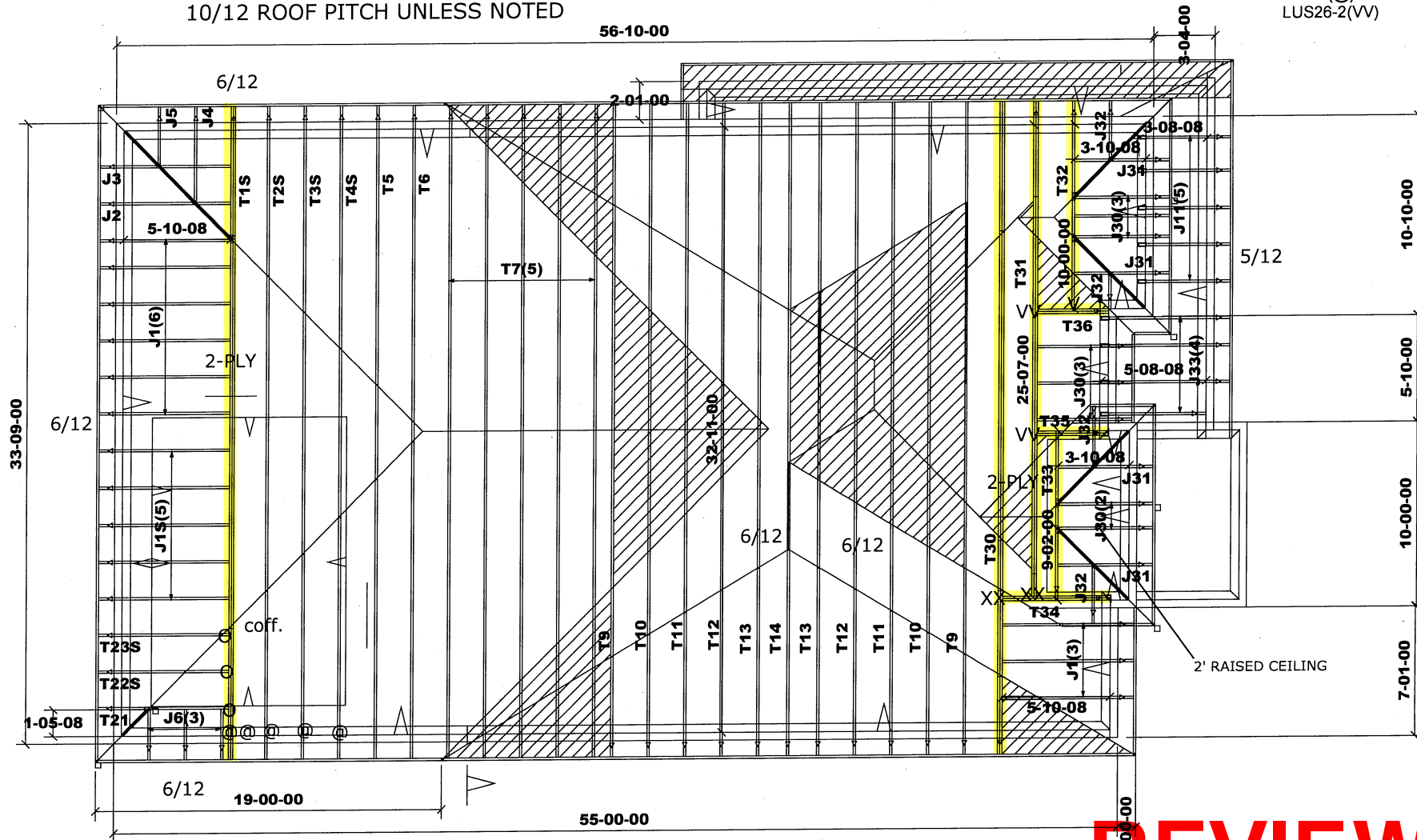


DENOTES:
CONVENTIONAL
FRAMING

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LSTA9- (L)
H2.5T- (I)
TC26- (@)
LUS26-2(VV)

10/12 ROOF PITCH UNLESS NOTED



Job Track: **50465**
Plan Log: **205577**
Layout ID: **423668**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **2022-07-11**

Sales:

Rick DiCiano

Designer: JG

Model / Elevation:

S42-21 / B-OPT.WITH COFF.

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Mitek ver 8.5.3.233

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ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

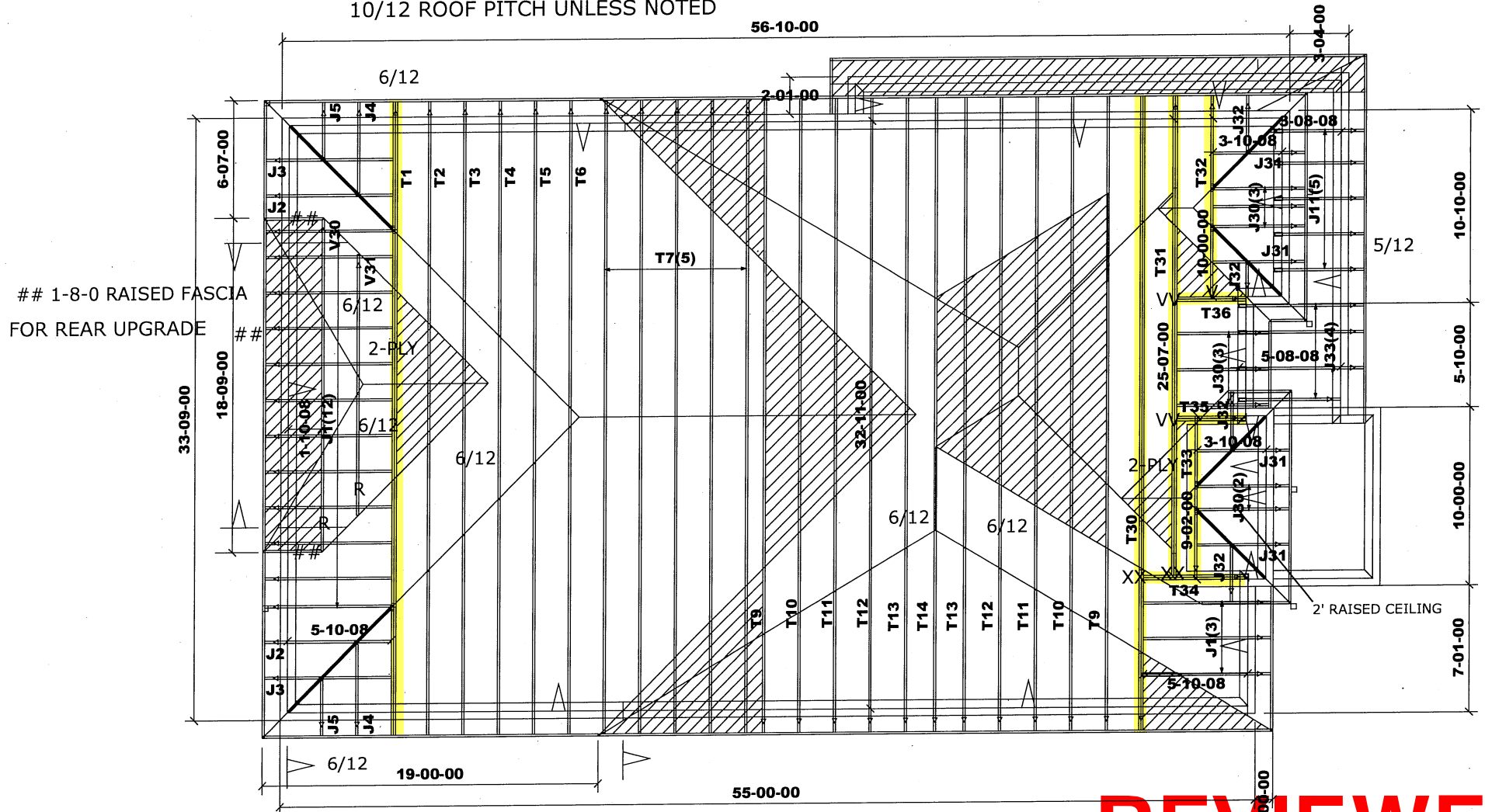
DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

 **DENOTES:**
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
VTCR-(R)

10/12 ROOF PITCH UNLESS NOTED



Job Track: **50465**
Plan Log: **205577**
Layout ID: **423670**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: 2022-07-11 Sales: Rick DiCiano Designer: JG

Model / Elevation:

S42-21 / B-REAR UPGRADE

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Mitek ver 8.5.3.233

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ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

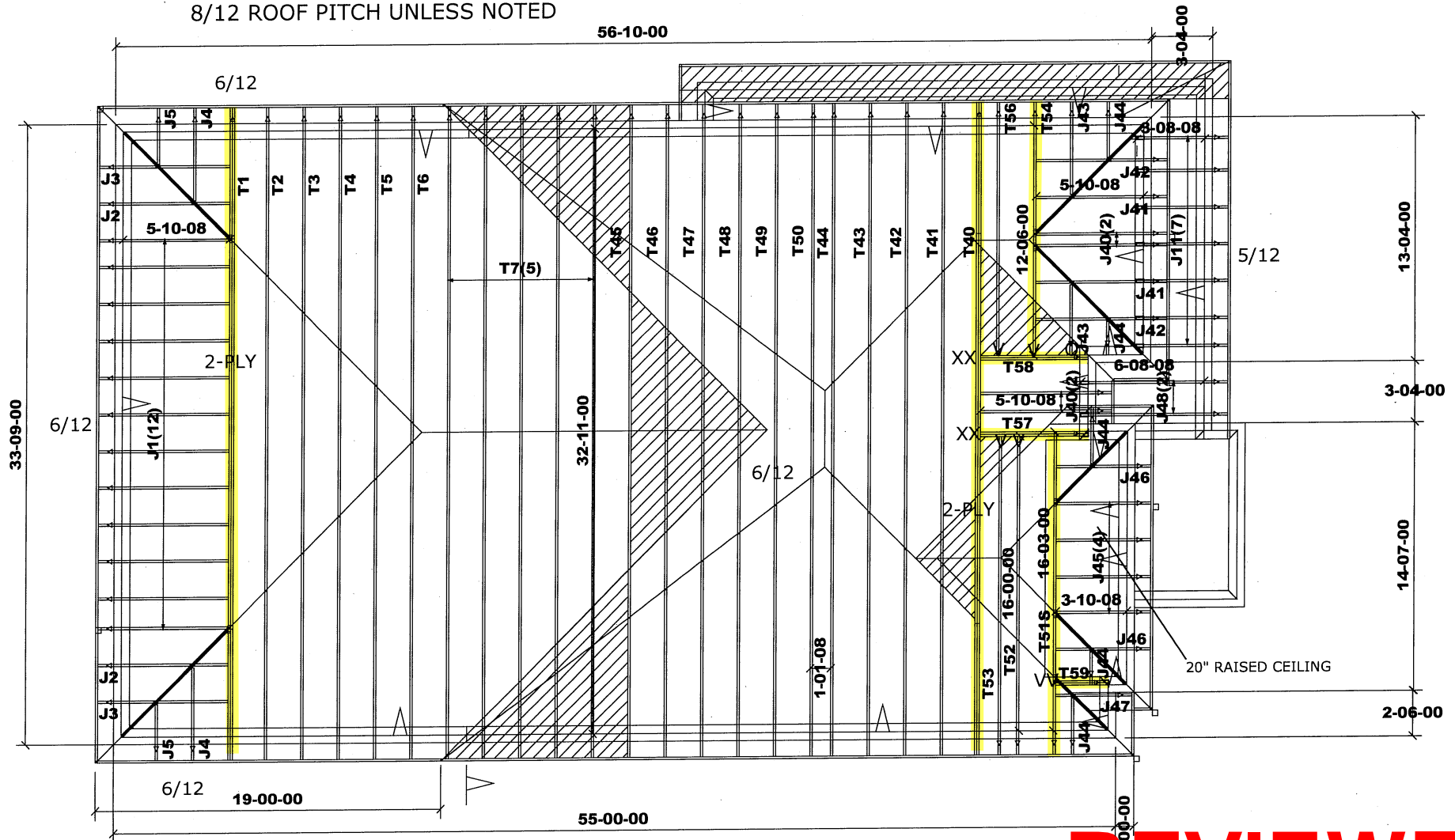
DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

 **DENOTES:**
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

LUS26-2(VV)

8/12 ROOF PITCH UNLESS NOTED



REVIEWED



Job Track: **50465**

Plan Log: **205577**

Layout ID: **423672**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **2022-07-11**

Sales:

Rick DiCiano

Designer: JG

Model / Elevation:

S42-21 / C

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Mitek ver 8.5.3.233

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ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

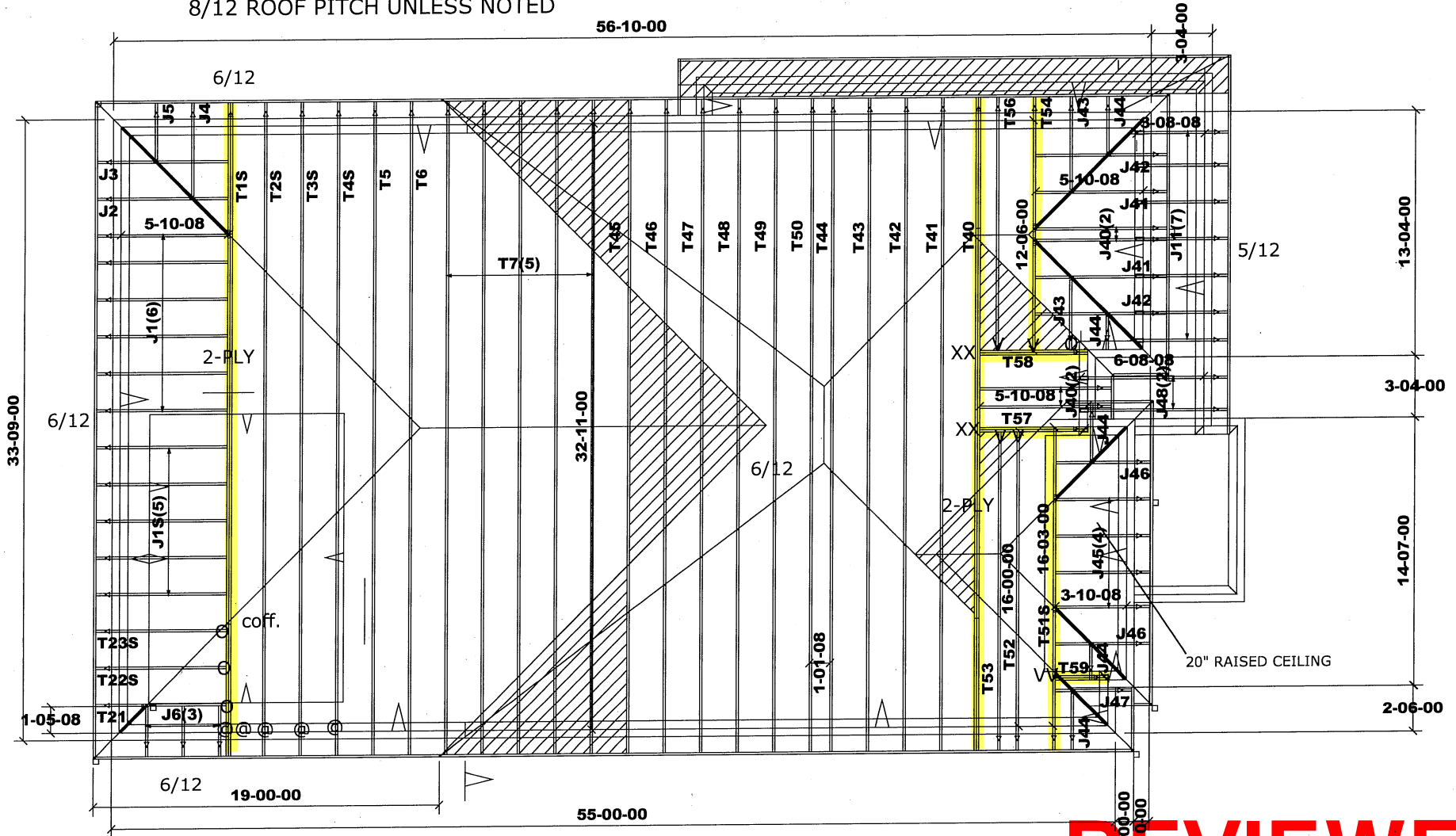
DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

 **DENOTES:**
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
TC26-(@)

LUS26-2 (VV)

8/12 ROOF PITCH UNLESS NOTED



Job Track: **50465**
Plan Log: **205577**
Layout ID: **423671**

Builder / Location:
BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: 2022-07-11 Sales: Rick DiCiano Designer: JG

Model / Elevation:
S42-21 / C-OPT.WITH COFF.

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Mitek ver 8.5.3.233

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ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

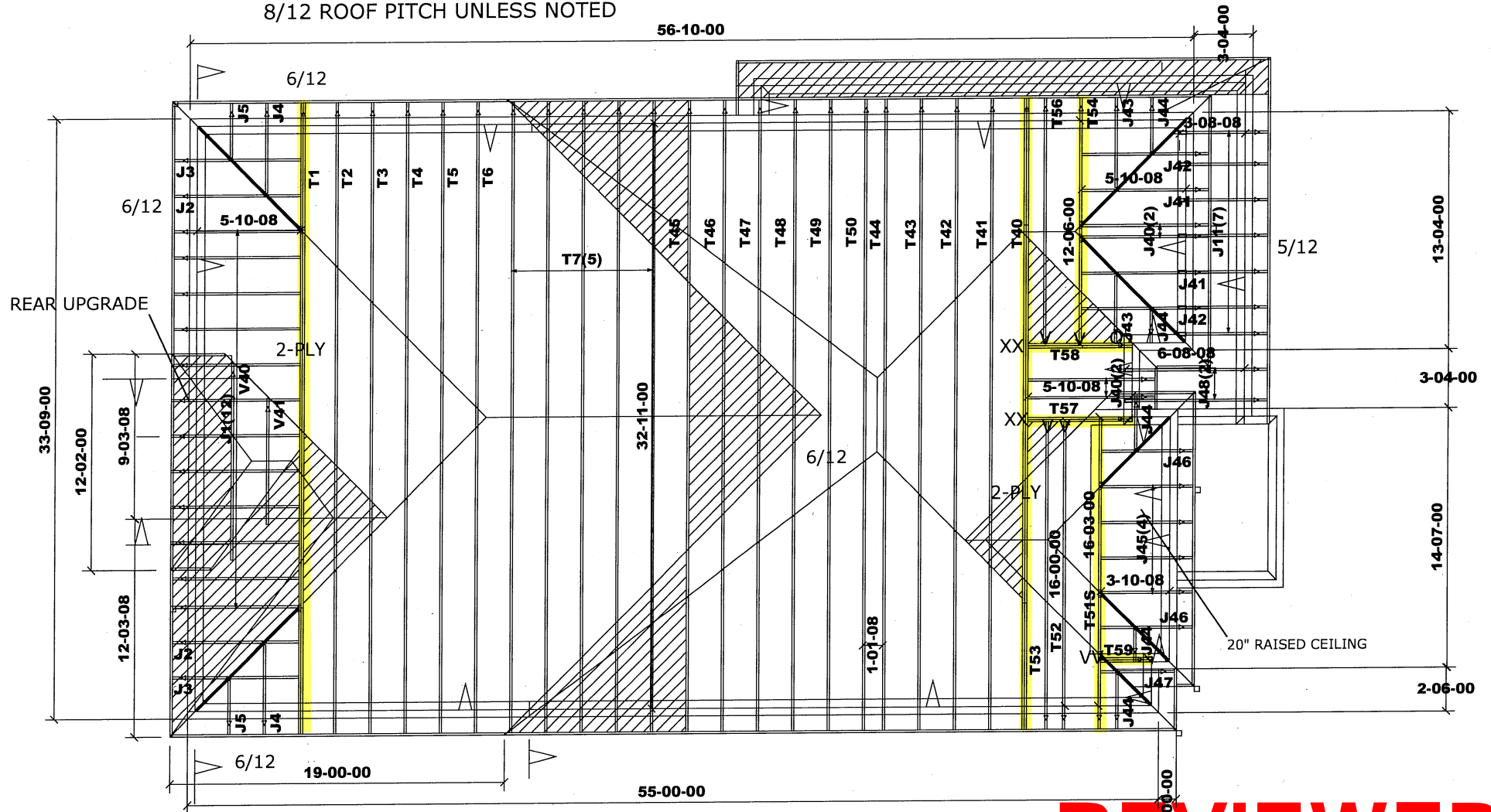
DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

 **DENOTES:
CONVENTIONAL
FRAMING**

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2(VV)

8/12 ROOF PITCH UNLESS NOTED



REVIEWED



Job Track: **50465**
Plan Log: **205577**
Layout ID: **423673**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: 2022-07-11 Sales: Rick DiCiano Designer: JG

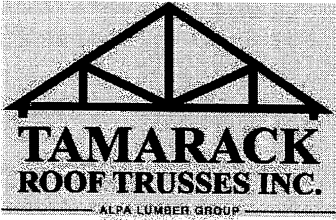
Model / Elevation:

S42-21 / C-REAR UPGRADE

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Mitek ver 8.5.3.233

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: A

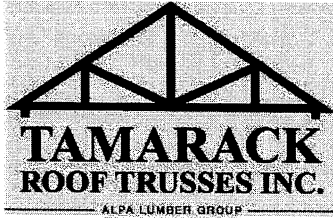
Job Track: 50465
 PlanLog: 205577
 Layout ID: 423666
 Ref #
 Page: 1 of 3
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	292.31 185.33		
	1	T2 Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.13 84.67		
	1	T3 Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.01 83.00		
	1	T4 Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.27 86.17		
	1	T5 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.67 87.50		
	1	T6 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.75 88.00		
	5	T7 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	717.12 451.67		
	1 2-ply	T8 Roof Special Girder	10 /12	32-11-00	7-04-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	359.92 226.67		
	2	T9 Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	284.28 179.67		
	2	T10 Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	287.31 180.67		
	2	T11 Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	320.52 202.33		
	2	T12 Hip	10 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	310.24 196.67		
	2	T13 Hip	10 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	324.22 202.33		
	1	T14 Hip	10 /12	32-11-00	9-11-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	160.12 101.33		

REVIEWED

DELIVERY SHIPLIST



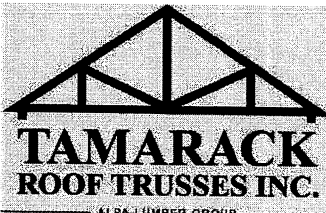
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: A

Job Track: 50465
 PlanLog: 205577
 Layout ID: 423666
 Ref #
 Page: 2 of 3
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano

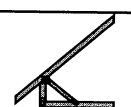
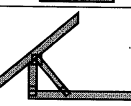
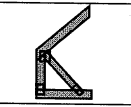
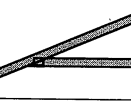

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T15 Hip Girder	10 /12	22-07-00	4-10-07	2 x 4 2 x 6		1-07-11 1-07-11	206.04 131.00		
	1	T16 Hip	10 /12	22-07-00	6-06-07	2 x 4		1-07-11 1-07-11	100.62 63.00		
	3	T17S Roof Special	10 /12	14-06-00	7-08-03	2 x 4		1-07-11 1-07-11	208.55 138.00		
	1 2-ply	T18 Half Hip Girder	6 /12	6-08-08	3-06-03	2 x 4 2 x 6		1-02-00 3-06-03	65.3 43.33		
	1 2-ply	T19 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T20 Jack-Closed Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	2	T24W Common	6 /12	8-05-00	3-00-15	2 x 4	1-03-08 1-03-08	5-07 5-07	53.73 35.33		
	1	G24W GABLE	6 /12	8-05-00	3-00-15	2 x 4	1-03-08 1-03-08	5-07 5-07	25.54 16.33		
	15	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	2	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	3	J7 Jack-Open	10 /12	2-03-00	3-06-03	2 x 4	1-03-08	1-07-11 3-06-03	33.47 24.50		

REVIEWED

 <p>TAMARACK ROOF TRUSSES INC. <small>ALPHA LUMBER GROUP</small></p>	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-21 Lot #: Elevation: A	Job Track: 50465 PlanLog: 205577 Layout ID: 423666 Ref # Page: 3 of 3 Date: 07-11-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	J8 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	46.35 30.50		
	1	J9 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J10 Jack-Open	10 /12	1-10-08	3-01-09	2 x 4		1-07-11 3-02-07	7.88 5.67		
	7	J11 Jack-Open	5 /12	3-08-08	2-04-13	2 x 4	1-03-08	4-01 1-10-09	73.81 46.67		
	2	J12 Jack-Open	5 /12	4-08-08	2-09-13	2 x 4	1-03-08	4-01 2-03-09	25.84 16.00		

TOTAL # TRUSS= 80

TOTAL BFT OF ALL TRUSSES= 3194.33

BFT.

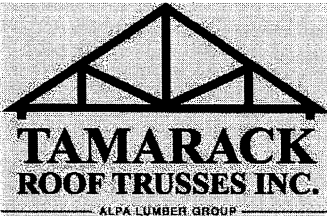
TOTAL WEIGHT OF ALL TRSSES 5036.33 LBS

HARDWARE















QTY	TYPE	MODEL	LENGTH
4	Hardware	H2.5T	
5	Hardware	HGUS26-2	
6	Hardware	LJS26DS	
3	Hardware	LSTA9	
1	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 19

REVIEWED

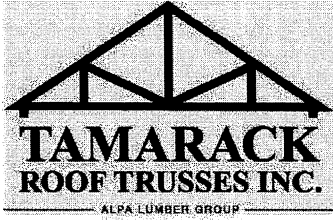
 <p>TAMARACK ROOF TRUSSES INC. <small>ALPA LUMBER GROUP</small></p>	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-21 Lot #: Elevation: A-OPT.WITH COFF.	Job Track: 50465 PlanLog: 205577 Layout ID: 423665 Ref # Page: 1 of 3 Date: 07-11-2022 Designer: Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	305.69 197.33		
	1	T2S Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.32 92.50		
	1	T3S Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.68 91.83		
	1	T4S Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	145.48 92.67		
	1	T5 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.02 89.50		
	1	T6 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.6 90.00		
	5	T7 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	717.12 451.67		
	1 2-ply	T8 Roof Special Girder	10 /12	32-11-00	7-04-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	363.25 228.67		
	2	T9 Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	284.28 179.67		
	2	T10 Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	287.31 180.67		
	2	T11 Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	320.52 202.33		
	2	T12 Hip	10 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	317.34 200.67		
	2	T13 Hip	10 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	329.75 205.67		
	1	T14 Hip	10 /12	32-11-00	9-11-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	167.42 105.33		

REVIEWED

DELIVERY SHIPLIST



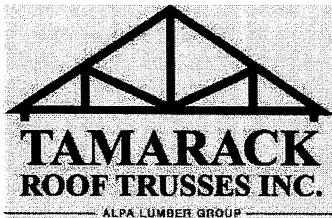
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 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: A-OPT.WITH COFF.

Job Track: 50465
 PlanLog: 205577
 Layout ID: 423665
 Ref #
 Page: 2 of 3
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano



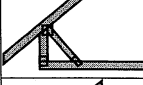


Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T15 Hip Girder	10 / 12	22-07-00	4-10-07	2 x 4 2 x 6		1-07-11 1-07-11	217.9 139.33		
	1	T16 Hip	10 / 12	22-07-00	6-06-07	2 x 4		1-07-11 1-07-11	100.62 63.00		
	3	T17S Roof Special	10 / 12	14-06-00	7-08-03	2 x 4		1-07-11 1-07-11	208.55 138.00		
	1 2-ply	T18 Half Hip Girder	6 / 12	6-08-08	3-06-03	2 x 4 2 x 6		1-02-00 3-06-03	65.3 43.33		
	1 2-ply	T19 Jack-Closed Girder	6 / 12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T20 Jack-Closed Girder	10 / 12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	1	T21 Half Hip Girder	6 / 12	5-10-08	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	23.24 16.50		
	1	T22S Half Hip	6 / 12	5-10-08	2-10-12	2 x 4	1-03-08	1-02-00 1-10-12	26.14 19.50		
	1	T23S Half Hip	6 / 12	5-10-08	3-10-12	2 x 4	1-03-08	1-02-00 2-10-12	26.46 18.67		
	2	T24W Common	6 / 12	8-05-00	3-00-15	2 x 4	1-03-08 1-03-08	5-07 5-07	53.73 35.33		
	1	G24W GABLE	6 / 12	8-05-00	3-00-15	2 x 4	1-03-08 1-03-08	5-07 5-07	25.54 16.33		
	9	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	5	J1S Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	103.94 73.33		
	1	J2 Jack-Open	6 / 12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		

REVIEWED

 <p>TAMARACK ROOF TRUSSES INC. <small>ALPA LUMBER GROUP</small></p>	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-21 Lot #: Elevation: A-OPT.WITH COFF.		Job Track: 50465 PlanLog: 205577 Layout ID: 423665 Ref # Page: 3 of 3 Date: 07-11-2022 Designer: Sales Rep: Rick DiCiano	
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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	3	J6 Jack-Open	6 /12	1-05-08	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	18.38 14.00		
	3	J7 Jack-Open	10 /12	2-03-00	3-06-03	2 x 4	1-03-08	1-07-11 3-06-03	33.47 24.50		
	3	J8 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	46.35 30.50		
	1	J9 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J10 Jack-Open	10 /12	1-10-08	3-01-09	2 x 4		1-07-11 3-02-07	7.88 5.67		
	7	J11 Jack-Open	5 /12	3-08-08	2-04-13	2 x 4	1-03-08	4-01 1-10-09	73.81 46.67		
	2	J12 Jack-Open	5 /12	4-08-08	2-09-13	2 x 4	1-03-08	4-01 2-03-09	25.84 16.00		

TOTAL # TRUSS= 81

TOTAL BFT OF ALL TRUSSES= 3306.5

BFT.

TOTAL WEIGHT OF ALL TRSSES 5175.19 LBS

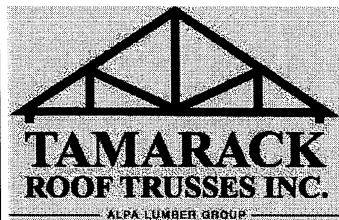
HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	H2.5T	
5	Hardware	HGUS26-2	
6	Hardware	LJS26DS	
3	Hardware	LSTA9	
4	Hardware	LUS24	
5	Hardware	TC26	

TOTAL NUMBER OF ITEMS= 27

REVIEWED

DELIVERY SHIPLIST



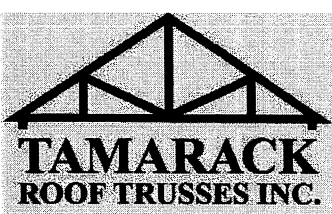
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: A-REAR UPGRADE

Job Track: 50465
 PlanLog: 205577
 Layout ID: 423667
 Ref #
 Page: 1 of 3
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano












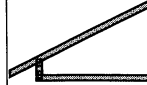


Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	292.31 185.33		
	1	T2 Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.13 84.67		
	1	T3 Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.96 83.67		
	1	T4 Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.27 86.17		
	1	T5 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.02 89.50		
	1	T6 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.75 88.00		
	5	T7 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	717.12 451.67		
	1 2-ply	T8 Roof Special Girder	10 /12	32-11-00	7-04-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	359.92 226.67		
	2	T9 Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	284.28 179.67		
	2	T10 Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	287.31 180.67		
	2	T11 Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	320.52 202.33		
	2	T12 Hip	10 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	310.24 196.67		
	2	T13 Hip	10 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	324.22 202.33		
	1	T14 Hip	10 /12	32-11-00	9-11-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	160.12 101.33		

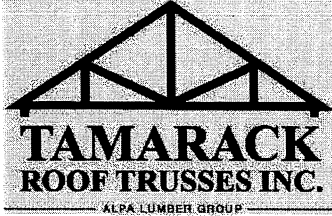
REVIEWED

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. <small>ALPHA LUMBER GROUP</small>	Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
	Builder:	BAYVIEW WELLINGTON	PlanLog:	205577
	Project:	GREEN VALLEY EAST	Layout ID:	423667
	Location:	BRADFORD	Ref #	
	Model:	S42-21	Page:	2 of 3
	Lot #:		Date:	07-11-2022
	Elevation:	A-REAR UPGRADE	Designer:	
		Sales Rep:	Rick DiCiano	






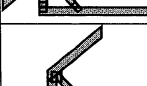


Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T15 Hip Girder	10 /12	22-07-00	4-10-07	2 x 4 2 x 6		1-07-11 1-07-11	206.04 131.00		
	1	T16 Hip	10 /12	22-07-00	6-06-07	2 x 4		1-07-11 1-07-11	100.62 63.00		
	3	T17S Roof Special	10 /12	14-06-00	7-08-03	2 x 4		1-07-11 1-07-11	208.55 138.00		
	1 2-ply	T18 Half Hip Girder	6 /12	6-08-08	3-06-03	2 x 4 2 x 6		1-02-00 3-06-03	65.3 43.33		
	1 2-ply	T19 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T20 Jack-Closed Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	2	T24W Common	6 /12	8-05-00	3-00-15	2 x 4	1-03-08 1-03-08	5-07 5-07	53.73 35.33		
	1	G24W GABLE	6 /12	8-05-00	3-00-15	2 x 4	1-03-08 1-03-08	5-07 5-07	25.54 16.33		
	1	V1 Valley	6 /12	19-02-00	4-09-08	2 x 4			58.89 39.50		
	1	V2 Valley	6 /12	15-02-00	3-09-08	2 x 4			39.54 25.33		
	1	V3 Valley	6 /12	11-02-00	2-09-08	2 x 4			27.06 17.50		
	15	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	2	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		

REVIEWED

 <p>TAMARACK ROOF TRUSSES INC. <small>ALFA LUMBER GROUP</small></p>	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-21 Lot #: Elevation: A-REAR UPGRADE		Job Track: 50465 PlanLog: 205577 Layout ID: 423667 Ref # Page: 3 of 3 Date: 07-11-2022 Designer: Sales Rep: Rick DiCiano	
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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	3	J7 Jack-Open	10 /12	2-03-00	3-06-03	2 x 4	1-03-08	1-07-11 3-06-03	33.47 24.50		
	3	J8 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	46.35 30.50		
	1	J9 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J10 Jack-Open	10 /12	1-10-08	3-01-09	2 x 4		1-07-11 3-02-07	7.88 5.67		
	7	J11 Jack-Open	5 /12	3-08-08	2-04-13	2 x 4	1-03-08	4-01 1-10-09	73.81 46.67		
	2	J12 Jack-Open	5 /12	4-08-08	2-09-13	2 x 4	1-03-08	4-01 2-03-09	25.84 16.00		

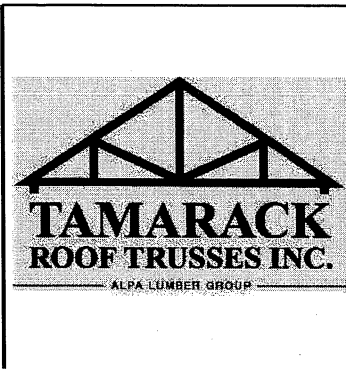
TOTAL # TRUSS= 83 TOTAL BFT OF ALL TRUSSES= 3279.33 BFT. TOTAL WEIGHT OF ALL TRSSES 5166.13 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	H2.5T	
5	Hardware	HGUS26-2	
6	Hardware	LJS26DS	
3	Hardware	LSTA9	
1	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 19

REVIEWED



DELIVERY SHIPLIST

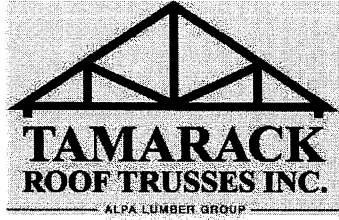
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: B-OPT.WITH COFF.

Job Track: 50465
 PlanLog: 205577
 Layout ID: 423668
 Ref #
 Page: 1 of 3
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	305.7 197.33		
	1	T2S Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.46 92.50		
	1	T3S Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.96 91.83		
	1	T4S Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	145.62 92.67		
	1	T5 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.67 87.50		
	1	T6 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.75 88.00		
	5	T7 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	717.12 451.67		
	2	T9 Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	284.28 179.67		
	2	T10 Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	287.31 180.67		
	2	T11 Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	320.52 202.33		
	2	T12 Hip	10 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	310.24 196.67		
	2	T13 Hip	10 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	324.22 202.33		
	1	T14 Hip	10 /12	32-11-00	9-11-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	167.42 105.33		
	1	T21 Half Hip Girder	6 /12	5-10-08	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	23.24 16.50		

REVIEWED



DELIVERY SHIPLIST

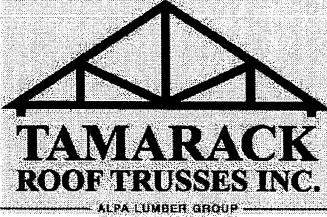
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 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: B-OPT.WITH COFF.

Job Track: 50465
 PlanLog: 205577
 Layout ID: 423668
 Ref #
 Page: 2 of 3
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano

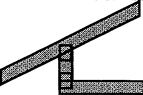
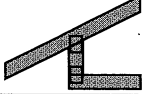
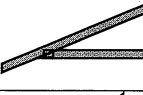
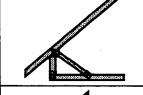
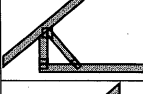


Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T22S Half Hip	6 /12	5-10-08	2-10-12	2 x 4	1-03-08	1-02-00 1-10-12	24.63 18.17		
	1	T23S Half Hip	6 /12	5-10-08	3-10-12	2 x 4	1-03-08	1-02-00 2-10-12	25.33 17.67		
	1 2-ply	T30 Roof Special Girder	10 /12	32-11-00	6-04-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	362.83 222.00		
	1 2-ply	T31 Hip Girder	10 /12	25-07-00	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 3-10-03	246.37 153.33		
	1	T32 Hip Girder	10 /12	10-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	55.23 35.50		
	1	T33 Hip Girder	10 /12	9-02-00	4-10-07	2 x 4 2 x 6		1-07-11 1-07-11	50.4 34.83		
	1 2-ply	T34 Flat Girder	0 /12	5-10-08	2-00-00	2 x 6		2-00-00 2-00-00	57.82 37.33		
	1 2-ply	T35 Flat	0 /12	3-10-08	2-00-00	2 x 6		2-00-00 2-00-00	38.36 25.33		
	1 2-ply	T36 Jack-Closed Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	9	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	5	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	103.94 73.33		
	1	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		
	1	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		

REVIEWED

 <p>TAMARACK ROOF TRUSSES INC. <small>ALFA LUMBER GROUP</small></p>	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-21 Lot #: Elevation: B-OPT.WITH COFF.		Job Track: 50465 PlanLog: 205577 Layout ID: 423668 Ref # Page: 3 of 3 Date: 07-11-2022 Designer: Sales Rep: Rick DiCiano	
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Roof Trusses

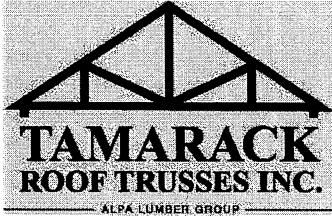
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	3	J6 Jack-Open	6 /12	1-05-08	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	18.38 14.00		
	5	J11 Jack-Open	5 /12	3-08-08	2-04-13	2 x 4	1-03-08	4-01 1-10-09	52.72 33.33		
	8	J30 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	124.58 81.33		
	4	J31 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	48.06 33.33		
	4	J32 Jack-Open	10 /12	1-10-08	3-01-09	2 x 4	1-03-08	1-07-11 3-02-07	39.17 28.00		
	4	J33 Jack-Open	5 /12	5-08-08	3-02-13	2 x 4	1-03-08	4-01 2-08-09	61.18 37.33		
TOTAL # TRUSS= 84 TOTAL BFT OF ALL TRUSSES= 3181.81 BFT. TOTAL WEIGHT OF ALL TRSSES 4998.14 LBS											

HARDWARE







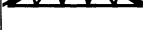



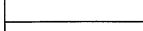
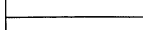
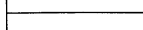
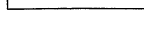
QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
2	Hardware	LUS26-2	
2	Hardware	LUS26-2	
5	Hardware	TC26	

TOTAL NUMBER OF ITEMS= 12

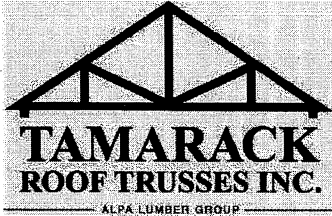
REVIEWED

 <p>TAMARACK ROOF TRUSSES INC. <small>ALFA LUMBER GROUP</small></p>	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-21 Lot #: Elevation: B		Job Track: 50465 Plan Log: 205577 Layout ID: 423669 Ref # Page: 1 of 3 Date: 07-11-2022 Designer: Sales Rep: Rick DiCiano	
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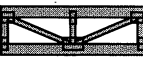
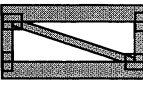
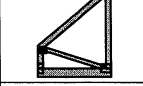

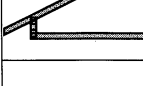






Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	292.31 185.33		
	1	T2 Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.13 84.67		
	1	T3 Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.96 83.67		
	1	T4 Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.27 86.17		
	1	T5 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.67 87.50		
	1	T6 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.75 88.00		
	5	T7 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	717.12 451.67		
	2	T9 Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	284.28 179.67		
	2	T10 Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	287.31 180.67		
	2	T11 Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	320.52 202.33		
	2	T12 Hip	10 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	310.24 196.67		
	2	T13 Hip	10 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	324.22 202.33		
	1	T14 Hip	10 /12	32-11-00	9-11-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	167.42 105.33		
	1 2-ply	T30 Roof Special Girder	10 /12	32-11-00	6-04-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	362.83 222.00		

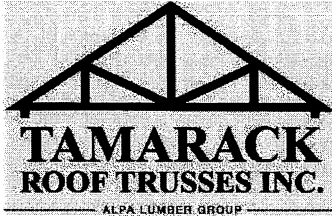
REVIEWED

 <p>TAMARACK ROOF TRUSSES INC. <small>ALFA LUMBER GROUP</small></p>	DELIVERY SHIPLIST					
	Lumber Yard: TAMARACK LUMBER				Job Track: 50465	
	Builder: BAYVIEW WELLINGTON				Plan Log: 205577	
	Project: GREEN VALLEY EAST				Layout ID: 423669	
	Location: BRADFORD				Ref #	
	Model: S42-21				Page: 2 of 3	
Lot #:				Date: 07-11-2022		
Elevation: B				Designer:		
				Sales Rep: Rick DiCiano		



Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T31 Hip Girder	10 /12	25-07-00	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 3-10-03	246.37 153.33		
	1	T32 Hip Girder	10 /12	10-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	55.23 35.50		
	1	T33 Hip Girder	10 /12	9-02-00	4-10-07	2 x 4 2 x 6		1-07-11 1-07-11	50.4 34.83		
	1 2-ply	T34 Flat Girder	0 /12	5-10-08	2-00-00	2 x 6		2-00-00 2-00-00	57.82 37.33		
	1 2-ply	T35 Flat	0 /12	3-10-08	2-00-00	2 x 6		2-00-00 2-00-00	38.36 25.33		
	1 2-ply	T36 Jack-Closed Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	15	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	2	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	5	J11 Jack-Open	5 /12	3-08-08	2-04-13	2 x 4	1-03-08	4-01 1-10-09	52.72 33.33		
	8	J30 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	124.58 81.33		
	4	J31 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	48.06 33.33		

REVIEWED

DELIVERY SHIPLIST		
 <p>TAMARACK ROOF TRUSSES INC. <small>ALPA LUMBER GROUP</small></p>	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-21 Lot #: Elevation: B	Job Track: 50465 PlanLog: 205577 Layout ID: 423669 Ref # Page: 3 of 3 Date: 07-11-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	J32 Jack-Open	10 /12	1-10-08	3-01-09	2 x 4	1-03-08	1-07-11 3-02-07	39.17 28.00		
	4	J33 Jack-Open	5 /12	5-08-08	3-02-13	2 x 4	1-03-08	4-01 2-08-09	61.18 37.33		

TOTAL # TRUSS= 83 TOTAL BFT OF ALL TRUSSES= 3098.31 BFT. TOTAL WEIGHT OF ALL TRSSES 4904.61 LBS

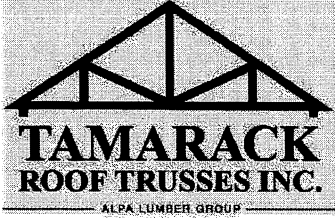
HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 5

REVIEWED

DELIVERY SHIPLIST



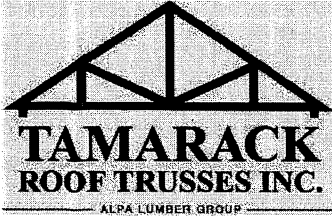
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: B-REAR UPGRADE

Job Track: 50465
 PlanLog: 205577
 Layout ID: 423670
 Ref #
 Page: 1 of 3
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano





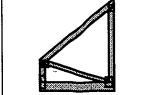



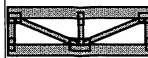





Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	300.17 191.33		
	1	T2 Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.13 84.67		
	1	T3 Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.96 83.67		
	1	T4 Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.27 86.17		
	1	T5 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.67 87.50		
	1	T6 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.75 88.00		
	5	T7 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	717.12 451.67		
	2	T9 Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	284.28 179.67		
	2	T10 Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	287.31 180.67		
	2	T11 Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	320.52 202.33		
	2	T12 Hip	10 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	310.24 196.67		
	2	T13 Hip	10 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	324.22 202.33		
	15	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	1	T14 Hip	10 /12	32-11-00	9-11-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	167.42 105.33		

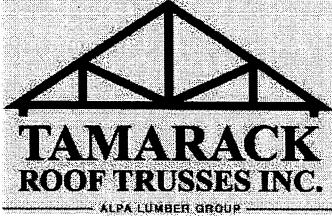
REVIEWED

 <p>TAMARACK ROOF TRUSSES INC. <small>ALPA LUMBER GROUP</small></p>	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER						Job Track: 50465	
	Builder: BAYVIEW WELLINGTON						PlanLog: 205577	
	Project: GREEN VALLEY EAST						Layout ID: 423670	
	Location: BRADFORD						Ref #	
	Model: S42-21						Page: 2 of 3	
Lot #:						Date: 07-11-2022		
Elevation: B-REAR UPGRADE						Designer:		
						Sales Rep: Rick DiCiano		



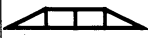

Roof Trusses

PROFILE	QTY. PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	J11 Jack-Open	5 /12	3-08-08	2-04-13	2 x 4	1-03-08	4-01 1-10-09	52.72 33.33		
	4	J33 Jack-Open	5 /12	5-08-08	3-02-13	2 x 4	1-03-08	4-01 2-08-09	61.18 37.33		
	1 2-ply	T30 Roof Special Girder	10 /12	32-11-00	6-04-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	378.12 233.33		
	1 2-ply	T31 Hip Girder	10 /12	25-07-00	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 3-10-03	257.48 163.33		
	1 2-ply	T36 Jack-Closed Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	8	J30 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	124.58 81.33		
	1 2-ply	T35 Flat	0 /12	3-10-08	2-00-00	2 x 6		2-00-00 2-00-00	38.36 25.33		
	1	T33 Hip Girder	10 /12	9-02-00	4-10-07	2 x 4 2 x 6		1-07-11 1-07-11	50.4 34.83		
	1 2-ply	T34 Flat Girder	0 /12	5-10-08	2-00-00	2 x 6		2-00-00 2-00-00	57.82 37.33		
	1	T32 Hip Girder	10 /12	10-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	55.23 35.50		
	2	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		

REVIEWED

 <p>TAMARACK ROOF TRUSSES INC. <small>ALFA LUMBER GROUP</small></p>	DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER		Job Track:	50465
	Builder:	BAYVIEW WELLINGTON		PlanLog:	205577
	Project:	GREEN VALLEY EAST		Layout ID:	423670
	Location:	BRADFORD		Ref #	
	Model:	S42-21		Page:	3 of 3
Lot #:			Date:	07-11-2022	
Elevation:	B-REAR UPGRADE		Designer:		
			Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	J31 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	48.06 33.33		
	4	J32 Jack-Open	10 /12	1-10-08	3-01-09	2 x 4	1-03-08	1-07-11 3-02-07	39.95 28.00		
	1	V30 Valley	6 /12	18-07-00	2-08-07	2 x 4			47.34 30.50		
	1	V31 Valley	6 /12	14-07-00	3-04-07	2 x 4			39.33 24.67		

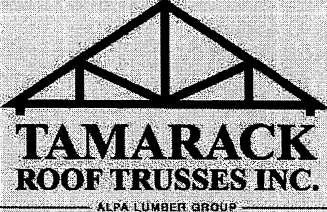
TOTAL # TRUSS= 85 TOTAL BFT OF ALL TRUSSES= 3180.81 BFT. TOTAL WEIGHT OF ALL TRSSES 5026.33 LBS

HARDWARE













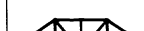

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
2	Hardware	LUS26-2	
2		VTOR	

TOTAL NUMBER OF ITEMS= 7

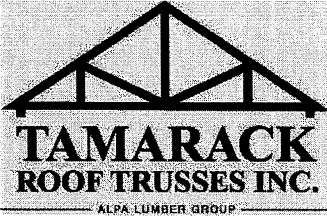
REVIEWED

 <p>TAMARACK ROOF TRUSSES INC. <small>ALFA LUMBER GROUP</small></p>	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER						Job Track: 50465	
	Builder: BAYVIEW WELLINGTON						PlanLog: 205577	
	Project: GREEN VALLEY EAST						Layout ID: 423671	
	Location: BRADFORD						Ref #	
	Model: S42-21						Page: 1 of 4	
Lot #:						Date: 07-11-2022		
Elevation: C-OPT.WITH COFF.						Designer:		
						Sales Rep: Rick DiCiano		



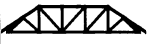



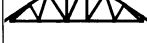




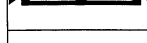
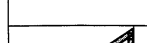

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	311.56 200.67		
	1	T2S Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.46 92.50		
	1	T3S Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.83 91.83		
	1	T4S Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	145.55 92.17		
	1	T5 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.67 87.50		
	1	T6 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.75 88.00		
	5	T7 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	717.12 451.67		
	1	T21 Half Hip Girder	6 /12	5-10-08	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	23.24 16.50		
	1	T22S Half Hip	6 /12	5-10-08	2-10-12	2 x 4	1-03-08	1-02-00 1-10-12	24.63 18.17		
	1	T23S Half Hip	6 /12	5-10-08	3-10-12	2 x 4	1-03-08	1-02-00 2-10-12	25.33 17.67		
	1 2-ply	T40 Hip Girder	8 /12	32-11-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	360.37 219.00		
	1	T41 Hip	8 /12	32-11-00	6-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.8 94.17		
	1	T42 Hip	8 /12	32-11-00	7-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.92 93.00		
	1	T43 Hip	8 /12	32-11-00	9-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	154.75 97.83		

REVIEWED

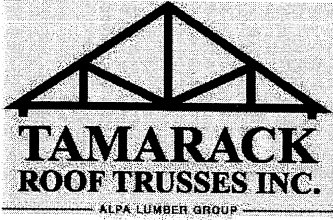
 <p>TAMARACK ROOF TRUSSES INC. <small>ALPA LUMBER GROUP</small></p>	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER						Job Track: 50465	
	Builder: BAYVIEW WELLINGTON						PlanLog: 205577	
	Project: GREEN VALLEY EAST						Layout ID: 423671	
	Location: BRADFORD						Ref #	
	Model: S42-21						Page: 2 of 4	
Lot #:						Date: 07-11-2022		
Elevation: C-OPT.WITH COFF.						Designer:		
						Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T44 Hip	8 /12	32-11-00	10-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	167.1 106.00		
	1	T45 Hip	8 /12	32-11-00	5-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	142.25 91.33		
	1	T46 Hip	8 /12	32-11-00	6-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.44 94.17		
	1	T47 Hip	8 /12	32-11-00	7-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	143.58 90.33		
	1	T48 Hip	8 /12	32-11-00	8-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	149.9 95.50		
	1	T49 Hip	8 /12	32-11-00	9-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	156.78 98.67		
	1	T50 Hip	8 /12	32-11-00	10-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	166.79 106.00		
	1	T51S Hip Girder	8 /12	16-03-00	5-07-13	2 x 4 2 x 6	1-03-08	1-04-13 1-04-13	83.48 52.50		
	1	T52 Hip	8 /12	16-00-00	6-11-13	2 x 4	1-03-08	1-04-13 3-02-13	77.58 49.50		
	1	T53 Common	8 /12	16-00-00	7-07-13	2 x 4	1-03-08	1-04-13 3-02-13	73.48 47.33		
	1	T54 Hip Girder	8 /12	12-06-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	71.03 44.50		
	1	T56 Common	8 /12	12-06-00	5-06-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	54.88 35.83		
	1 2-ply	T57 Flat Girder	0 /12	5-10-08	1-08-00	2 x 6		1-08-00 1-08-00	55.39 33.67		
	1 2-ply	T58 Jack-Closed Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		

REVIEWED

DELIVERY SHIPLIST



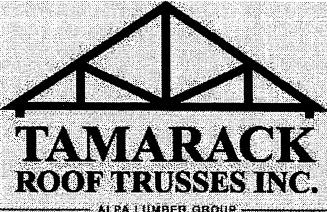
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 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: C-OPT.WITH COFF.

Job Track: 50465
 PlanLog: 205577
 Layout ID: 423671
 Ref #
 Page: 3 of 4
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano

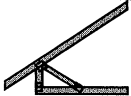

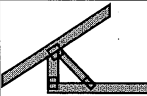

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T59 Flat	0 /12	2-10-08	1-08-00	2 x 4		1-08-00 1-08-00	22.18 16.33		
	6	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00		
	5	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	103.94 73.33		
	1	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		
	1	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	3	J6 Jack-Open	6 /12	1-05-08	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	18.38 14.00		
	7	J11 Jack-Open	5 /12	3-08-08	2-04-13	2 x 4	1-03-08	4-01 1-10-09	73.81 46.67		
	4	J40 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	71.32 42.67		
	2	J41 Jack-Open	8 /12	3-09-08	3-11-02	2 x 4	1-03-08 2-01-00	1-04-13 3-11-02	33.12 20.67		
	2	J42 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	26.74 16.67		
	2	J43 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.41 15.33		
	5	J44 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	44.52 28.33		

REVIEWED

 <p>TAMARACK ROOF TRUSSES INC. <small>ALFA LUMBER GROUP</small></p>	DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER		Job Track:	50465
	Builder:	BAYVIEW WELLINGTON		PlanLog:	205577
	Project:	GREEN VALLEY EAST		Layout ID:	423671
	Location:	BRADFORD		Ref #	
	Model:	S42-21		Page:	4 of 4
Lot #:			Date:	07-11-2022	
Elevation:	C-OPT.WITH COFF.		Designer:		
			Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	J45 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	57.6 36.00		
	2	J46 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-01-01	1-04-13 2-07-02	22.18 14.00		
	1	J47 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01-01	1-04-13 2-07-02	9.95 6.33		
	2	J48 Jack-Open	5 /12	6-08-08	3-07-13	2 x 4	1-03-08	4-01 3-01-09	39.64 24.33		

TOTAL # TRUSS= 87 TOTAL BFT OF ALL TRUSSES= 3092.01 BFT. TOTAL WEIGHT OF ALL TRSSES 4862.78 LBS

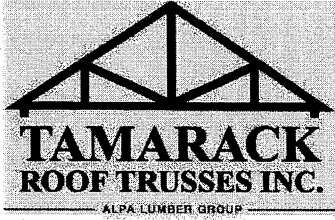
HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
4	Hardware	LUS24	
5	Hardware	TC26	

TOTAL NUMBER OF ITEMS= 15 LUS26-2 1

REVIEWED

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: C

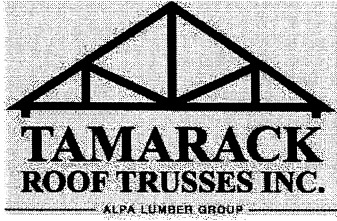
Job Track: 50465
 PlanLog: 205577
 Layout ID: 423672
 Ref #
 Page: 1 of 4
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	292.31 185.33		
	1	T2 Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.13 84.67		
	1	T3 Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.96 83.67		
	1	T4 Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.27 86.17		
	1	T5 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.67 87.50		
	1	T6 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.75 88.00		
	5	T7 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	717.12 451.67		
	1 2-ply	T40 Hip Girder	8 /12	32-11-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	360.37 219.00		
	1	T41 Hip	8 /12	32-11-00	6-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.8 94.17		
	1	T42 Hip	8 /12	32-11-00	7-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.92 93.00		
	1	T43 Hip	8 /12	32-11-00	9-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	154.75 97.83		
	1	T44 Hip	8 /12	32-11-00	10-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	167.1 106.00		
	1	T45 Hip	8 /12	32-11-00	5-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	142.25 91.33		
	1	T46 Hip	8 /12	32-11-00	6-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.44 94.17		

REVIEWED

DELIVERY SHIPLIST



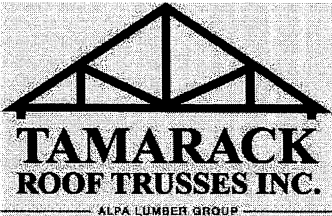
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: C

Job Track: 50465
 PlanLog: 205577
 Layout ID: 423672
 Ref #
 Page: 2 of 4
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano














Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T47 Hip	8 /12	32-11-00	7-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	143.58 90.33		
	1	T48 Hip	8 /12	32-11-00	8-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	149.9 95.50		
	1	T49 Hip	8 /12	32-11-00	9-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	156.78 98.67		
	1	T50 Hip	8 /12	32-11-00	10-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	166.79 106.00		
	1	T51S Hip Girder	8 /12	16-03-00	5-07-13	2 x 4 2 x 6	1-03-08	1-04-13 1-04-13	83.48 52.50		
	1	T52 Hip	8 /12	16-00-00	6-11-13	2 x 4	1-03-08	1-04-13 3-02-13	77.58 49.50		
	1	T53 Common	8 /12	16-00-00	7-07-13	2 x 4	1-03-08	1-04-13 3-02-13	73.48 47.33		
	1	T54 Hip Girder	8 /12	12-06-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	71.03 44.50		
	1	T56 Common	8 /12	12-06-00	5-06-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	54.88 35.83		
	1 2-ply	T57 Flat Girder	0 /12	5-10-08	1-08-00	2 x 6		1-08-00 1-08-00	55.39 33.67		
	1 2-ply	T58 Jack-Closed Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1 2-ply	T59 Flat	0 /12	2-10-08	1-08-00	2 x 4		1-08-00 1-08-00	22.18 16.33		
	12	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.53 128.00		
	2	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		

REVIEWED

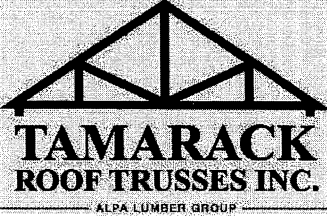
 <p>TAMARACK ROOF TRUSSES INC. <small>ALFA LUMBER GROUP</small></p>	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-21 Lot #: Elevation: C		Job Track: 50465 PlanLog: 205577 Layout ID: 423672 Ref # Page: 3 of 4 Date: 07-11-2022 Designer: Sales Rep: Rick DiCiano	
--	-------------------	--	--	--	--	--	---	--

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	7	J11 Jack-Open	5 /12	3-08-08	2-04-13	2 x 4	1-03-08	4-01 1-10-09	73.81 46.67		
	4	J40 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	71.32 42.67		
	2	J41 Jack-Open	8 /12	3-09-08	3-11-02	2 x 4	1-03-08 2-01-00	1-04-13 3-11-02	33.12 20.67		
	2	J42 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	26.74 16.67		
	2	J43 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.41 15.33		
	5	J44 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	44.52 28.33		
	4	J45 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	57.6 36.00		
	2	J46 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-01-01	1-04-13 2-07-02	22.18 14.00		
	1	J47 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01-01	1-04-13 2-07-02	9.95 6.33		
	2	J48 Jack-Open	5 /12	6-08-08	3-07-13	2 x 4	1-03-08	4-01 3-01-09	39.64 24.33		

TOTAL # TRUSS= 86
 TOTAL BFT OF ALL TRUSSES= 3005.67
 BFT.
 TOTAL WEIGHT OF ALL TRSSES 4763.59 LBS

REVIEWED

DELIVERY SHIPLIST		
 <p>TAMARACK ROOF TRUSSES INC. <small>ALPA LUMBER GROUP</small></p>	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-21 Lot #: Elevation: C	Job Track: 50465 PlanLog: 205577 Layout ID: 423672 Ref # Page: 4 of 4 Date: 07-11-2022 Designer: Sales Rep: Rick DiCiano

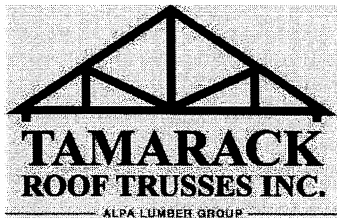
HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
1	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 7 LUS 26-2 1

REVIEWED

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: C-REAR UPGRADE

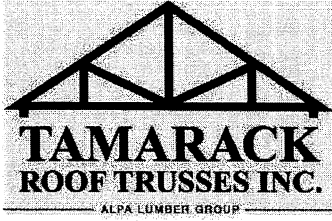
Job Track: 50465
 PlanLog: 205577
 Layout ID: 423673
 Ref #
 Page: 1 of 4
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	292.31 185.33		
	1	T2 Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.13 84.67		
	1	T3 Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.96 83.67		
	1	T4 Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.27 86.17		
	1	T5 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.67 87.50		
	1	T6 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.75 88.00		
	5	T7 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	717.12 451.67		
	1 2-ply	T40 Hip Girder	8 /12	32-11-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	398.71 242.67		
	1	T41 Hip	8 /12	32-11-00	6-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.8 94.17		
	1	T42 Hip	8 /12	32-11-00	7-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.92 93.00		
	1	T43 Hip	8 /12	32-11-00	9-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	154.75 97.83		
	1	T44 Hip	8 /12	32-11-00	10-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	170.83 108.00		
	1	T45 Hip	8 /12	32-11-00	5-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	142.25 91.33		
	1	T46 Hip	8 /12	32-11-00	6-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.44 94.17		

REVIEWED

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: C-REAR UPGRADE

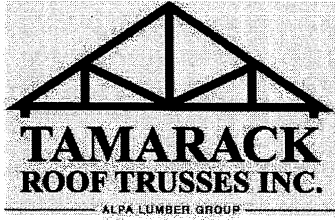
Job Track: 50465
 PlanLog: 205577
 Layout ID: 423673
 Ref #
 Page: 2 of 4
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T47 Hip	8 /12	32-11-00	7-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	143.58 90.33		
	1	T48 Hip	8 /12	32-11-00	8-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	149.9 95.50		
	1	T49 Hip	8 /12	32-11-00	9-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	156.78 98.67		
	1	T50 Hip	8 /12	32-11-00	10-07-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	170.5 108.00		
	1	T51S Hip Girder	8 /12	16-03-00	5-07-13	2 x 4 2 x 6	1-03-08	1-04-13 1-04-13	82.32 52.00		
	1	T52 Hip	8 /12	16-00-00	6-11-13	2 x 4	1-03-08	1-04-13 3-02-13	77.58 49.50		
	1	T53 Common	8 /12	16-00-00	7-07-13	2 x 4	1-03-08	1-04-13 3-02-13	73.48 47.33		
	1	T54 Hip Girder	8 /12	12-06-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	72.45 45.33		
	1	T56 Common	8 /12	12-06-00	5-06-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	54.88 35.83		
	1 2-ply	T57 Flat Girder	0 /12	5-10-08	1-08-00	2 x 6		1-08-00 1-08-00	55.39 33.67		
	1 2-ply	T58 Jack-Closed Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1 2-ply	T59 Flat	0 /12	2-10-08	1-08-00	2 x 4		1-08-00 1-08-00	22.18 16.33		
	1	V40 Valley	6 /12	11-01-00	2-00-01	2 x 4			27 17.33		
	1	V41 Valley	6 /12	7-01-00	1-09-04	2 x 4			16.37 11.00		

REVIEWED

DELIVERY SHIPLIST



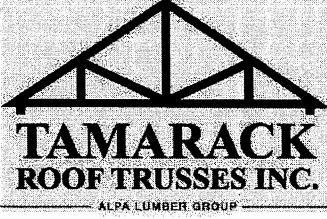
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-21
 Lot #:
 Elevation: C-REAR UPGRADE

Job Track: 50465
 PlanLog: 205577
 Layout ID: 423673
 Ref #
 Page: 3 of 4
 Date: 07-11-2022
 Designer:
 Sales Rep: Rick DiCiano


Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	12	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.53 128.00		
	2	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	7	J11 Jack-Open	5 /12	3-08-08	2-04-13	2 x 4	1-03-08	4-01 1-10-09	73.81 46.67		
	4	J40 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	76.29 49.33		
	2	J41 Jack-Open	8 /12	3-09-08	3-11-02	2 x 4	1-03-08 2-01-00	1-04-13 3-11-02	33.12 20.67		
	2	J42 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	26.74 16.67		
	2	J43 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.41 15.33		
	5	J44 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	44.52 28.33		
	4	J45 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	57.6 36.00		
	2	J46 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-01-01	1-04-13 2-07-02	22.18 14.00		
	1	J47 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01-01	1-04-13 2-07-02	9.95 6.33		

REVIEWED

DELIVERY SHIPLIST		
 <p>TAMARACK ROOF TRUSSES INC. <small>ALPHA LUMBER GROUP</small></p>	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-21 Lot #: Elevation: C-REAR UPGRADE	Job Track: 50465 PlanLog: 205577 Layout ID: 423673 Ref # Page: 4 of 4 Date: 07-11-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J48 Jack-Open	5 /12	6-08-08	3-07-13	2 x 4	1-03-08	4-01 3-01-09	39.64 24.33		

TOTAL # TRUSS= 88 TOTAL BFT OF ALL TRUSSES= 3068.66 BFT. TOTAL WEIGHT OF ALL TRSSES 4857.95 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
1	Hardware	LUS24	
1	Hardware	LUS26-2	

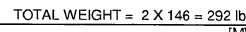
TOTAL NUMBER OF ITEMS= 8

REVIEWED

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Fri Jul 8 11:54:16 2022 Page 1

ID:c3jyj23uDiq 8pvrKbKzpy75XW-eFJQpSS?e5dqhlrPVYEM6K?pmMRJWzok25vKJvz 8vL



JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.67 (P) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423667	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 11:54:16 2022 Page 2
ID:c3jvi23uDiq 8pvRKbkZpy75XW-eFJQpSS?e5dghlrPYEM6K?pMMrJWzok25vKJvz 8vL

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.00
I	TMVW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	8.0	4.50	Edge
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	3.0	8.0	4.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

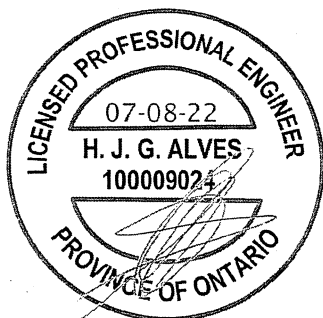
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-308	-308	---	FRONT	VERT	TOTAL	---	C1
D	11-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
H	27-0-8	-308	-308	---	FRONT	VERT	TOTAL	---	C1
L	26-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Q	11-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	7-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
U	9-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
V	13-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
W	15-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
X	17-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Y	19-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Z	21-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AA	23-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AB	25-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AC	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
AD	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AE	7-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AF	9-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	13-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	15-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AI	17-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	19-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	21-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	23-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	25-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	28-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	30-11-12	-20	-20	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

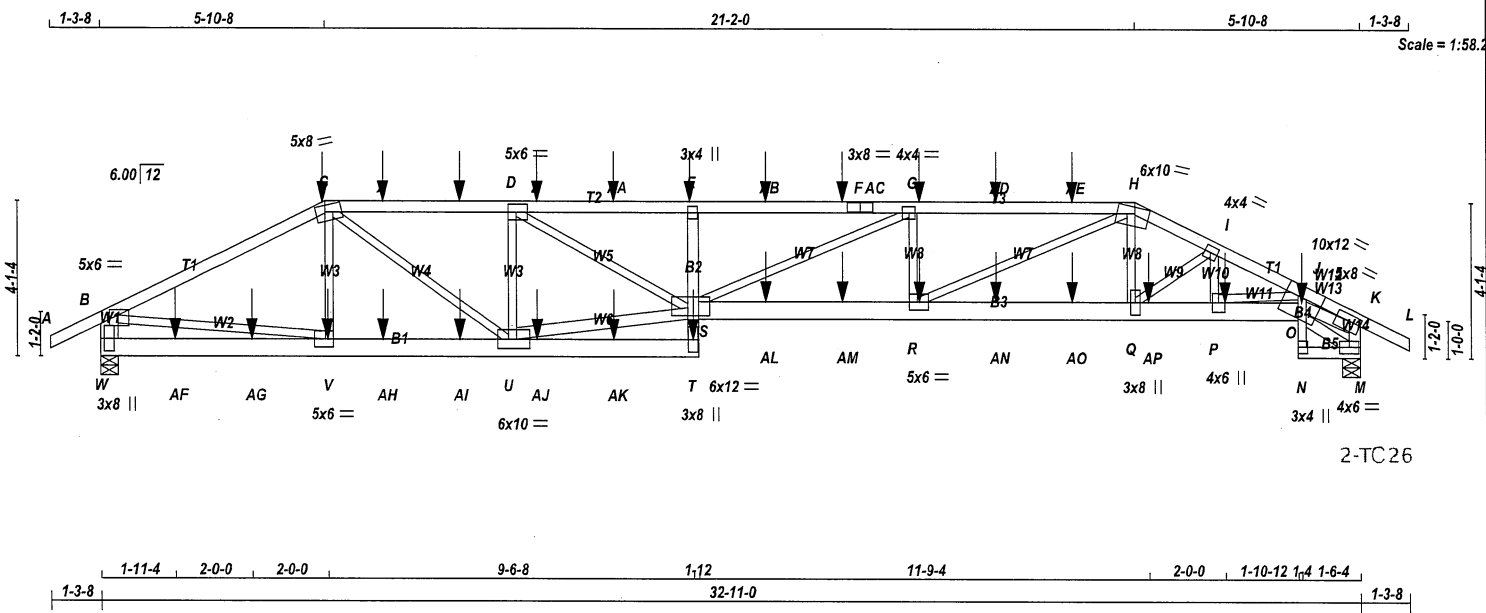


Structural component only
DWG# T-2216627

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T1S	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Fri Jul 8 11:47:58 2022 Page 1
ID:c3jyj23uDijq_8pvrKbkZpy75XW-jXTWTCTufy?TLfymMASjncLGQ?lyyA57HKK2pz_9?F



TOTAL WEIGHT = 2 X 153 = 306 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	2100F 1.8E
C - F	2x4	DRY	2100F 1.8E
F - H	2x4	DRY	2100F 1.8E
H - L	2x4	DRY	2100F 1.8E
W - B	2x6	DRY	No.2
M - K	2x4	DRY	No.2
W - T	2x6	DRY	No.2
T - E	2x4	DRY	No.2
S - O	2x6	DRY	No.2
N - J	2x3	DRY	No.2
N - M	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
U - S	2x4	DRY	No.2
O - M	2x4	DRY	No.2
O - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(65.9)
C-F	12	SIDE(65.9)
F-H	12	SIDE(65.9)
H-L	12	TOP
M-K	12	TOP
W-B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W-T	12	SIDE(183.1)
S-O	12	SIDE(183.1)
T-E	12	TOP
N-J	12	TOP
J-M	12	SIDE(24.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
W 3801	0	5-8	5-8
M 4359	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS			
1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	
JT COMBINED	SNOW	LIVE	PERM.LIVE
W 2665	1878 / 0	0 / 0	0 / 0
M 3051	2183 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 34	-112.4 -112.4 0.05 (1)	10.00
B-C	-5741 / 0	-112.4 -112.4 0.35 (1)	4.64
C-X	-7616 / 0	-112.4 -112.4 0.31 (1)	4.14
X-Y	-7616 / 0	-112.4 -112.4 0.31 (1)	4.14
Y-D	-7616 / 0	-112.4 -112.4 0.31 (1)	4.14
D-Z	-11773 / 0	-112.4 -112.4 0.45 (1)	3.32
Z-AA	-11773 / 0	-112.4 -112.4 0.45 (1)	3.32
AA-E	-11773 / 0	-112.4 -112.4 0.45 (1)	3.32
E-AB	-11920 / 0	-112.4 -112.4 0.61 (1)	3.20
AB-AC	-11920 / 0	-112.4 -112.4 0.61 (1)	3.20
AC-F	-11920 / 0	-112.4 -112.4 0.61 (1)	3.20
F-G	-11920 / 0	-112.4 -112.4 0.61 (1)	3.20
G-AD	-11297 / 0	-112.4 -112.4 0.58 (1)	3.30
AD-AE	-11297 / 0	-112.4 -112.4 0.58 (1)	3.30
AE-H	-11297 / 0	-112.4 -112.4 0.58 (1)	3.30
H-I	-8375 / 0	-112.4 -112.4 0.14 (1)	4.14
I-J	-9162 / 0	-112.4 -112.4 0.42 (1)	3.62
J-K	-11092 / 0	-112.4 -112.4 0.45 (1)	3.26
K-L	0 / 34	-112.4 -112.4 0.05 (1)	10.00
W-B	-3721 / 0	0.0 0.0 0.13 (1)	7.31
M-K	-4152 / 0	0.0 0.0 0.23 (1)	5.82

W-AF	0 / 0	-18.5 -18.5 0.07 (4)	10.00
AF-AG	0 / 0	-18.5 -18.5 0.07 (4)	10.00
AG-V	0 / 0	-18.5 -18.5 0.07 (4)	10.00
V-AH	0 / 5119	-18.5 -18.5 0.37 (1)	10.00
AH-AI	0 / 5119	-18.5 -18.5 0.37 (1)	10.00
AI-U	0 / 5119	-18.5 -18.5 0.37 (1)	10.00
U-AJ	0 / 403	-18.5 -18.5 0.07 (1)	10.00
AJ-AK	0 / 403	-18.5 -18.5 0.07 (1)	10.00
AK-T	0 / 403	-18.5 -18.5 0.07 (1)	10.00
T-S	0 / 105	0.0 0.0 0.31 (1)	10.00
S-E	-850 / 0	0.0 0.0 0.31 (1)	7.81
S-AL	0 / 11297	-18.5 -18.5 0.86 (1)	10.00
AL-AM	0 / 11297	-18.5 -18.5 0.86 (1)	10.00
AM-R	0 / 11297	-18.5 -18.5 0.86 (1)	10.00
R-AN	0 / 7513	-18.5 -18.5 0.61 (1)	10.00
AN-AO	0 / 7513	-18.5 -18.5 0.61 (1)	10.00
AO-Q	0 / 7513	-18.5 -18.5 0.61 (1)	10.00
Q-AP	0 / 8330	-18.5 -18.5 0.68 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.39")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/573 (0.69")

CSI: TC=0.61/1.00 (E-G:1), BC=0.86/1.00 (R-S:1), WB=0.87/1.00 (K-O:1), SSI=0.33/1.00 (J-O:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 1.00 (K) (INPUT = 1.00)



Structural component only
DWG# T-2216603

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	T1S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TTWW-m	MT20	5.0	8.0	2.00	2.75
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	10.0	2.00	4.50
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J						
J	TMBVWWW*-t	MT20	10.0	12.0	Edge	4.00
K	TMVW-t	MT20	5.0	8.0	2.00	3.75
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O						
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW+t	MT20	3.0	8.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BVMWWW-t	MT20	6.0	12.0	4.50	3.50
T	BMV+p	MT20	3.0	8.0		
U	BMWWW-t	MT20	6.0	10.0	3.00	3.50
V	BMWW-t	MT20	5.0	6.0	2.50	2.50
W	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED UNBRACED LENGTH (LC)	MAX. FACTORED LC1 (LC)
FR-TO		FROM TO		FR-TO			
AP-P	0/8330	-18.5	-18.5 0.68 (1)	10.00			
P-O	0/10272	-18.5	-18.5 0.80 (1)	10.00			
N-O	0/16	0.0	0.0 0.43 (1)	10.00			
O-J	0/1772	0.0	0.0 0.65 (1)	10.00			
N-M	0/271	-18.5	-18.5 0.03 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-308	-308	---	FRONT	VERT	TOTAL	---	C1
E	15-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
G	21-4-12	-90	-90	---	FRONT	VERT	TOTAL	---	C1
O	31-4-12	-228	-228	---	FRONT	VERT	TOTAL	---	C1
P	29-4-12	-243	-243	---	FRONT	VERT	TOTAL	---	C1
R	21-4-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
T	15-5-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
V	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	7-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Y	9-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Z	11-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AA	13-4-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AB	17-4-12	-90	-90	---	FRONT	VERT	TOTAL	---	C1
AC	19-4-12	-90	-90	---	FRONT	VERT	TOTAL	---	C1
AD	23-4-12	-90	-90	---	FRONT	VERT	TOTAL	---	C1
AE	25-4-12	-90	-90	---	FRONT	VERT	TOTAL	---	C1
AF	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
AG	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	7-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AI	9-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	11-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	13-4-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	17-4-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AM	19-4-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AN	23-4-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AO	25-4-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AP	27-4-12	-243	-243	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

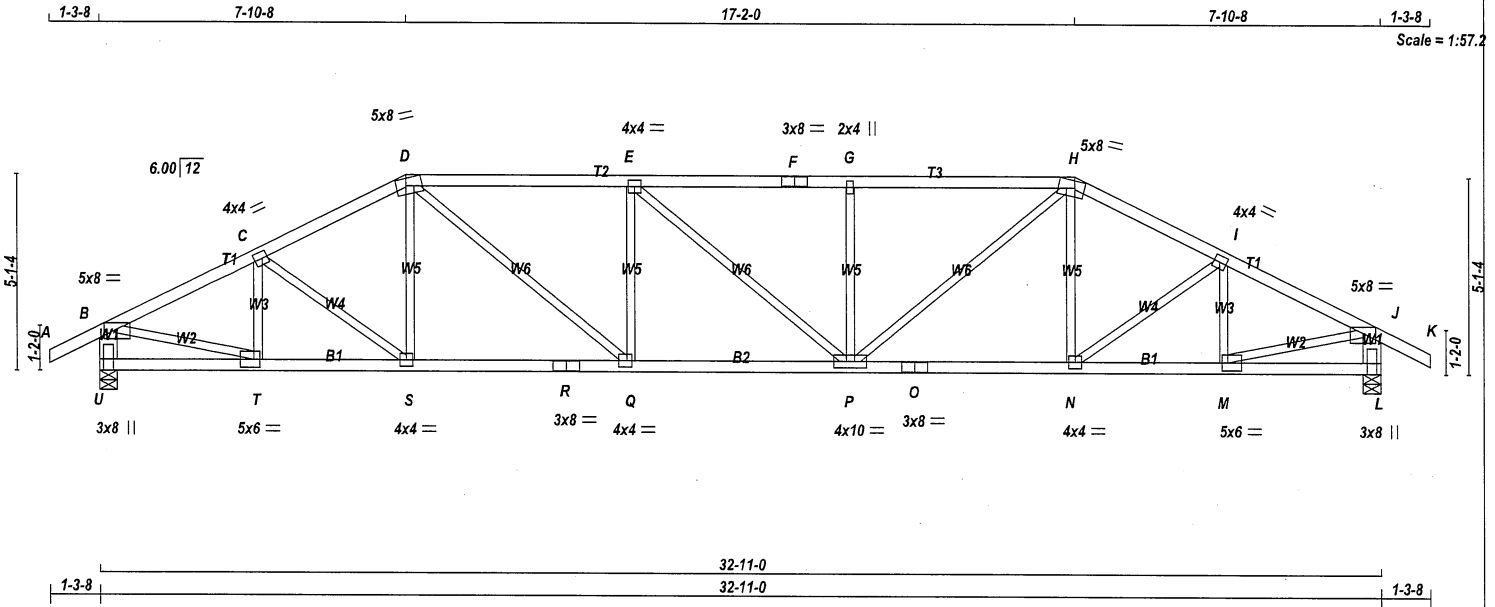


Structural component only
DWG# T-2216603

REVIEWED

JOB NAME 423667	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 133 lb
[M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTVW-m	MT20	5.0	8.0	2.25	3.00
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTVW-m	MT20	5.0	8.0	2.25	3.00
I	TMVW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	8.0	Edge	
M	BMVW-t	MT20	5.0	6.0	2.50	1.75
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	10.0		
Q	BMVW-t	MT20	4.0	4.0	2.00	1.50
R	BS-t	MT20	3.0	8.0		
S	BMVW-t	MT20	4.0	4.0		
T	BMVW-t	MT20	5.0	6.0	2.50	1.75
U	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED		MAXIMUM FACTORED	
GROSS REACTION		GROSS REACTION	
JT	VERT	DOWN	UPLIFT
U	2307	0	0
L	2307	0	0

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
L	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE	VERT. LOAD	MAX	MEMB.	FORCE	MAX	
(LBS)	(PLF)	LC1	CS1 (LC)	(LBS)	(LBS)	CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	T-C	-519 / 0
B-C	-3013 / 0	-112.4	-112.4	0.40 (1)	3.67	C-S	-23 / 4
C-D	-3037 / 0	-112.4	-112.4	0.39 (1)	3.66	S-D	0 / 117
D-E	-3656 / 0	-112.4	-112.4	0.84 (1)	2.91	D-Q	0 / 1248
E-F	-3654 / 0	-112.4	-112.4	0.83 (1)	2.91	Q-E	-696 / 0
F-G	-3654 / 0	-112.4	-112.4	0.83 (1)	2.91	E-P	-2 / 0
G-H	-3654 / 0	-112.4	-112.4	0.83 (1)	2.92	P-G	-695 / 0
H-I	-3037 / 0	-112.4	-112.4	0.39 (1)	3.66	P-H	0 / 1245
I-J	-3013 / 0	-112.4	-112.4	0.40 (1)	3.67	H-N	0 / 119
J-K	0 / 34	-112.4	-112.4	0.15 (1)	10.00	N-I	-22 / 4
U-B	-2267 / 0	0.0	0.0	0.15 (1)	6.81	M-I	-519 / 0
L-J	-2267 / 0	0.0	0.0	0.15 (1)	6.81	B-T	0 / 2774
						M-J	0 / 2774
U-T	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
T-S	0 / 2713	-18.5	-18.5	0.49 (1)	10.00		
S-R	0 / 2698	-18.5	-18.5	0.49 (1)	10.00		
R-Q	0 / 2698	-18.5	-18.5	0.49 (1)	10.00		
Q-P	0 / 3656	-18.5	-18.5	0.64 (1)	10.00		
P-O	0 / 2698	-18.5	-18.5	0.49 (1)	10.00		
O-N	0 / 2698	-18.5	-18.5	0.49 (1)	10.00		
N-M	0 / 2713	-18.5	-18.5	0.50 (1)	10.00		
M-L	0 / 0	-18.5	-18.5	0.07 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.38")
CSI: TC=0.84/1.00 (D-E-1), BC=0.64/1.00 (P-Q-1), WB=0.62/1.00 (B-T-1), SSI=0.30/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

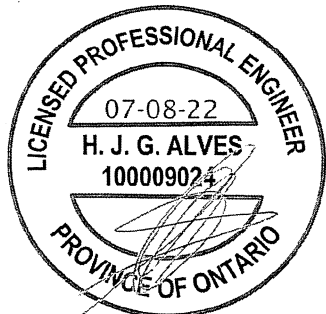
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.83 (O) (INPUT = 1.00)

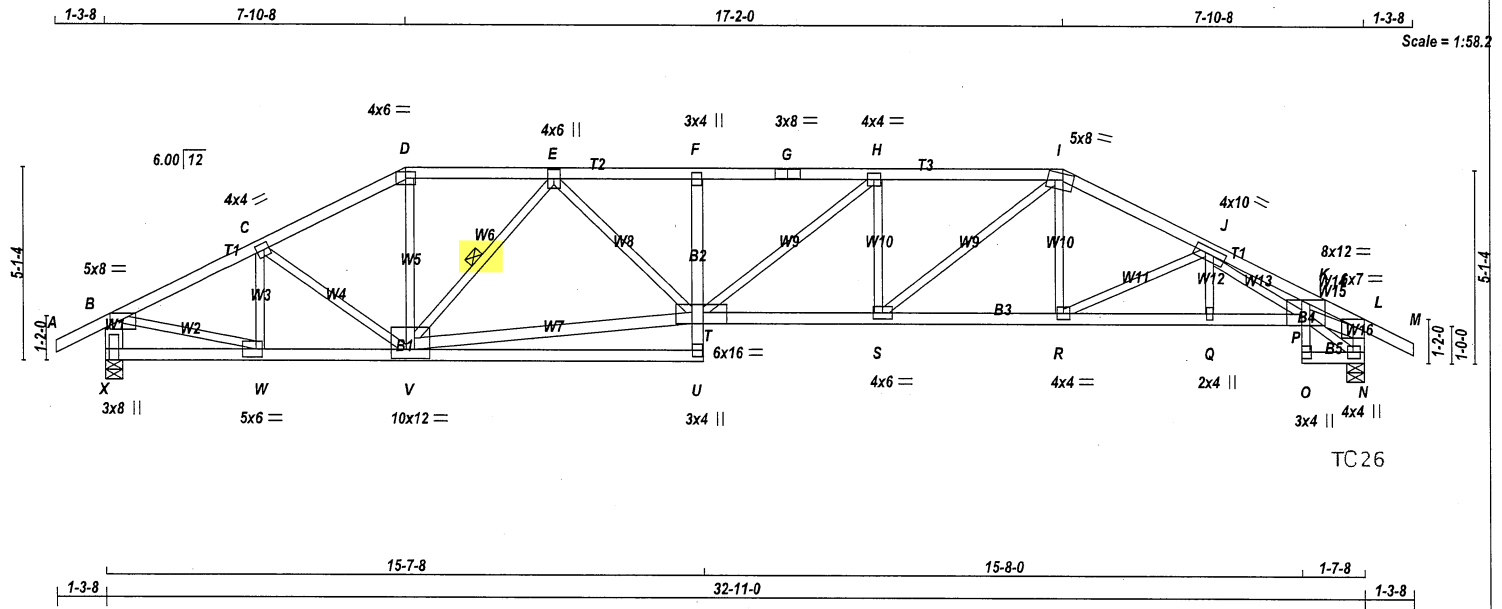


Structural component only
DWG# T-2216628

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T2S	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:c3jy23uDijq_8pVRKbkZpy75XW-fwbGuuv9BaFbay58TaUBt1QatoSkQuWObbpQ6iz_9?D



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2	SPF
	D - G	2x4	DRY	No.2	SPF
	G - I	2x4	DRY	No.2	SPF
	I - M	2x4	DRY	No.2	SPF
	X - B	2x6	DRY	No.2	SPF
	N - L	2x4	DRY	No.2	SPF
	X - U	2x4	DRY	No.2	SPF
	U - F	2x4	DRY	No.2	SPF
	T - P	2x4	DRY	No.2	SPF
	O - K	2x3	DRY	No.2	SPF
	O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2		SPF
V - T	2x4	DRY	No.2		SPF
P - L	2x4	DRY	No.2		SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-l	MT20	4.0	6.0		
E	TMVW+t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTW-m	MT20	5.0	8.0	2.00	3.25
J	TMVW-t	MT20	4.0	10.0		
K	TMVW-t	MT20	4.0	10.0		
K	TMBVWW-t	MT20	8.0	12.0	Edge	4.75
L	TMVW-p	MT20	6.0	7.0	Edge	
N	BMVW1+p	MT20	4.0	4.0	2.00	1.75
O	BMV+p	MT20	3.0	4.0		
P						
Q	BMV+w	MT20	2.0	4.0		
R	BMVW-t	MT20	4.0	4.0		
S	BMVW-t	MT20	4.0	6.0		
T	BMVW-t	MT20	6.0	16.0	Edge	7.50
U	BMV+p	MT20	3.0	4.0		
V	BMVW-t	MT20	10.0	12.0	3.00	6.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
X	2304	0	2304	0
N	2311	0	2311	0

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
X	1612	1155 / 0
N	1617	1159 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-V.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0 / 34	-112.4	-112.4	W-C	-535 / 0	0.10 (1)
B-C	-3003 / 0	-112.4	-112.4	C-V	-5 / 27	0.01 (4)
C-D	-3042 / 0	-112.4	-112.4	V-D	0 / 1001	0.23 (1)
D-E	-2718 / 0	-112.4	-112.4	E-T	0 / 3624	0.58 (1)
E-F	-4609 / 0	-112.4	-112.4	T-E	0 / 1208	0.27 (1)
F-G	-4654 / 0	-112.4	-112.4	G-H	0 / 247	0.06 (1)
G-H	-4654 / 0	-112.4	-112.4	H-I	-766 / 0	0.20 (1)
H-I	-4461 / 0	-112.4	-112.4	I-J	0 / 1373	0.31 (1)
I-J	-3793 / 0	-112.4	-112.4	J-I	0 / 445	0.10 (1)
J-K	-4931 / 0	-112.4	-112.4	K-J	-794 / 0	0.25 (1)
K-L	-4869 / 0	-112.4	-112.4	L-K	0 / 62	0.02 (4)
L-M	0 / 34	-112.4	-112.4	M-L	0 / 2764	0.62 (1)
X-B	-2261 / 0	0.0	0.0	B-W	-1691 / 0	0.43 (1)
N-L	-2214 / 0	0.0	0.0	W-B	-149 / 0	0.02 (1)
X-W	0 / 0	-18.5	-18.5	P-L	0 / 4273	0.69 (1)
W-V	0 / 2704	-18.5	-18.5	L-P	0 / 483	0.11 (1)
V-U	0 / 171	-18.5	-18.5			
U-T	0 / 64	0.0	0.0			
T-F	-441 / 0	0.0	0.0			
T-S	0 / 4461	-18.5	-18.5			
S-R	0 / 3385	-18.5	-18.5			
R-Q	0 / 4089	-18.5	-18.5			
Q-P	0 / 4089	-18.5	-18.5			
O-P	0 / 16	0.0	0.0			
P-K	0 / 102	0.0	0.0			
O-N	0 / 125	-18.5	-18.5			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.32")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/700 (0.56")

CSI: TC=0.74/1.00 (F-H:1) , BC=0.78/1.00 (S-T:1) , WB=0.69/1.00 (L-P:1) , SS=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.85 (E) (INPUT = 1.00)



Structural component only
DWG# T-2216604

REVIEWED CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T2S	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington


Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 11:48:00 2022 Page 2
ID:c3jvi23uDijq 8pvRKbkZpy75XW-fwGGuuv9BaFBay58TaUBt1QatoSkQuWObbpQ6iz 9?D

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMWV-t	MT20	5.0	6.0	2.50	1.75
X	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



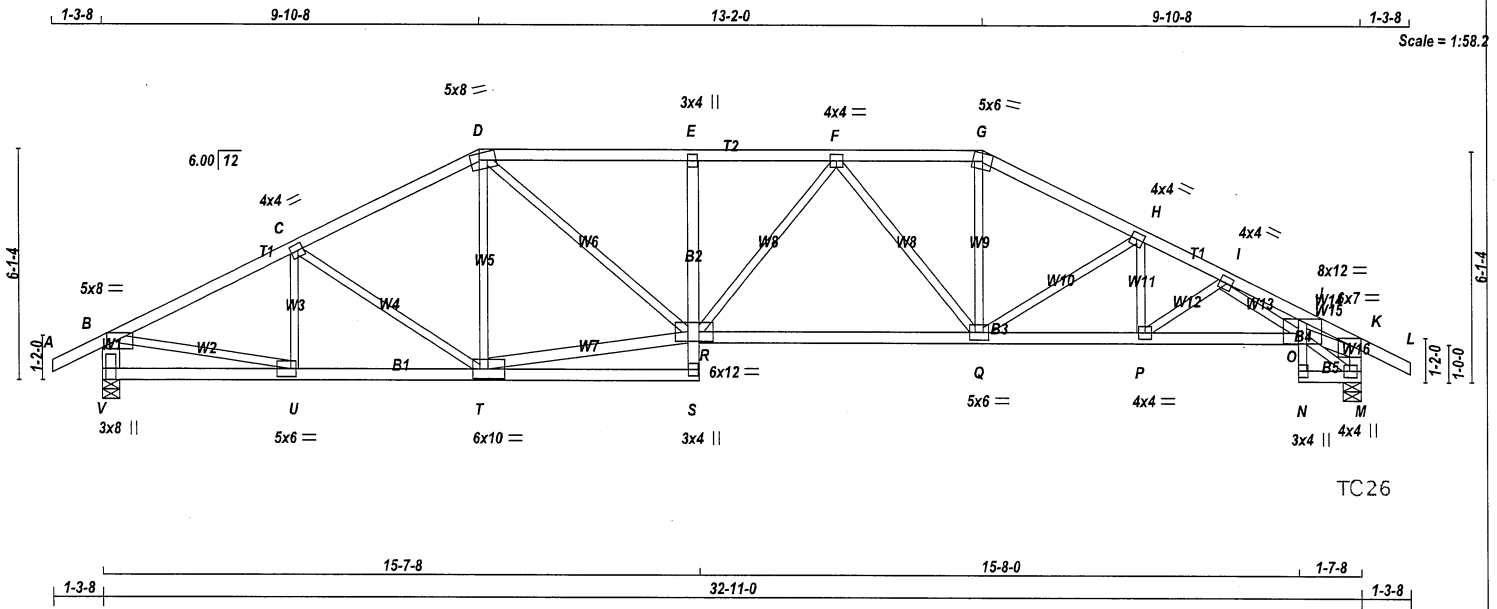
Structural component only
DWG# T-2216604

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	T3S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Fri Jul 8 11:48:01 2022 Page 1
ID:c3iyj23uDijq_8pvRKbkZpy75XW-769f5EwnytN2C6gL1l?QPEznNCpe9LjXpFz_f8z_9?C



TOTAL WEIGHT = 144 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	SPF
D - G	2x4	DRY	SPF
G - L	2x4	DRY	SPF
V - B	2x6	DRY	SPF
M - K	2x4	DRY	SPF
V - S	2x4	DRY	SPF
S - E	2x4	DRY	SPF
R - O	2x4	DRY	SPF
N - J	2x3	DRY	SPF
N - M	2x4	DRY	SPF
ALL WEBS	2x3	DRY	SPF
EXCEPT			
T - R	2x4	DRY	SPF
O - K	2x4	DRY	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	Edge
C, H, I					
D	TMVW-t	MT20	4.0	4.0	2.00 1.75
E	TTVW-m	MT20	5.0	8.0	2.00 2.75
F	TMV+p	MT20	3.0	4.0	
G	TMVW-t	MT20	4.0	4.0	
J	TTVW-m	MT20	5.0	6.0	
K	TMBVWWV+IMT20		8.0	12.0	3.25 4.75
L	TMVW-p	MT20	6.0	7.0	Edge
M	BMVW1+p	MT20	4.0	4.0	2.00 1.75
N	BMV+p	MT20	3.0	4.0	
O					
P	BMVW-t	MT20	4.0	4.0	
Q	BMVWW-t	MT20	5.0	6.0	
R	BMVWW-t	MT20	6.0	12.0	3.00 4.50
S	BMV+p	MT20	3.0	4.0	
T	BMVWW-t	MT20	6.0	10.0	3.00 2.25
U	BMVW-t	MT20	5.0	6.0	2.50 1.75
V	BMV1+p	MT20	3.0	8.0	Edge

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX
V	2304	0	0	2304	0	0	5-8	5-8	
M	2311	0	0	2311	0	0	5-8	5-8	

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1612	1155 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
M	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE	VERT. LOAD	MAX. (PLF)	MEMB.	FORCE	MAX. (LBS)	MAX. (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 34	-112.4 -112.4	0.15 (1)	U-C	-397 / 0	0.09 (1)	
B-C	-3117 / 0	-112.4 -112.4	0.48 (1)	C-T	-294 / 0	0.18 (1)	
C-D	-2905 / 0	-112.4 -112.4	0.45 (1)	T-D	-158 / 36	0.09 (1)	
D-E	-3705 / 0	-112.4 -112.4	0.63 (1)	T-R	0 / 2526	0.41 (1)	
E-F	-3722 / 0	-112.4 -112.4	0.47 (1)	D-R	0 / 1494	0.34 (1)	
F-G	-3119 / 0	-112.4 -112.4	0.31 (1)	Q-G	0 / 1161	0.26 (1)	
G-H	-3476 / 0	-112.4 -112.4	0.39 (1)	Q-H	-783 / 0	0.33 (1)	
H-I	-4175 / 0	-112.4 -112.4	0.32 (1)	P-H	0 / 315	0.07 (1)	
I-J	-4957 / 0	-112.4 -112.4	0.54 (1)	B-U	0 / 2853	0.64 (1)	
J-K	-4882 / 0	-112.4 -112.4	0.51 (1)	R-F	0 / 244	0.05 (1)	
K-L	0 / 34	-112.4 -112.4	0.15 (1)	F-Q	-743 / 0	0.48 (1)	
V-B	-2259 / 0	0.0 0.0	0.15 (1)	O-M	-150 / 0	0.02 (1)	
M-K	-2214 / 0	0.0 0.0	0.22 (1)	O-K	0 / 4287	0.69 (1)	
				I-O	0 / 477	0.11 (1)	
				P-I	-457 / 0	0.08 (1)	
V-U	0 / 0	-18.5 -18.5	0.10 (4)				
U-T	0 / 2812	-18.5 -18.5	0.53 (1)				
T-S	0 / 84	-18.5 -18.5	0.13 (4)				
S-R	0 / 47	0.0 0.0	0.12 (1)				
R-E	-622 / 0	0.0 0.0	0.20 (1)				
R-Q	0 / 3572	-18.5 -18.5	0.70 (1)				
Q-P	0 / 3751	-18.5 -18.5	0.72 (1)				
P-O	0 / 4120	-18.5 -18.5	0.74 (1)				
N-O	0 / 16	0.0 0.0	0.34 (1)				
O-J	0 / 100	0.0 0.0	0.36 (1)				
N-M	0 / 125	-18.5 -18.5	0.03 (1)				

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/788 (0.50")

CSI: TC=0.63/1.00 (D-E:1), BC=0.74/1.00 (O-P:1), WB=0.69/1.00 (K-O:1), SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.80 (K) (INPUT = 1.00)



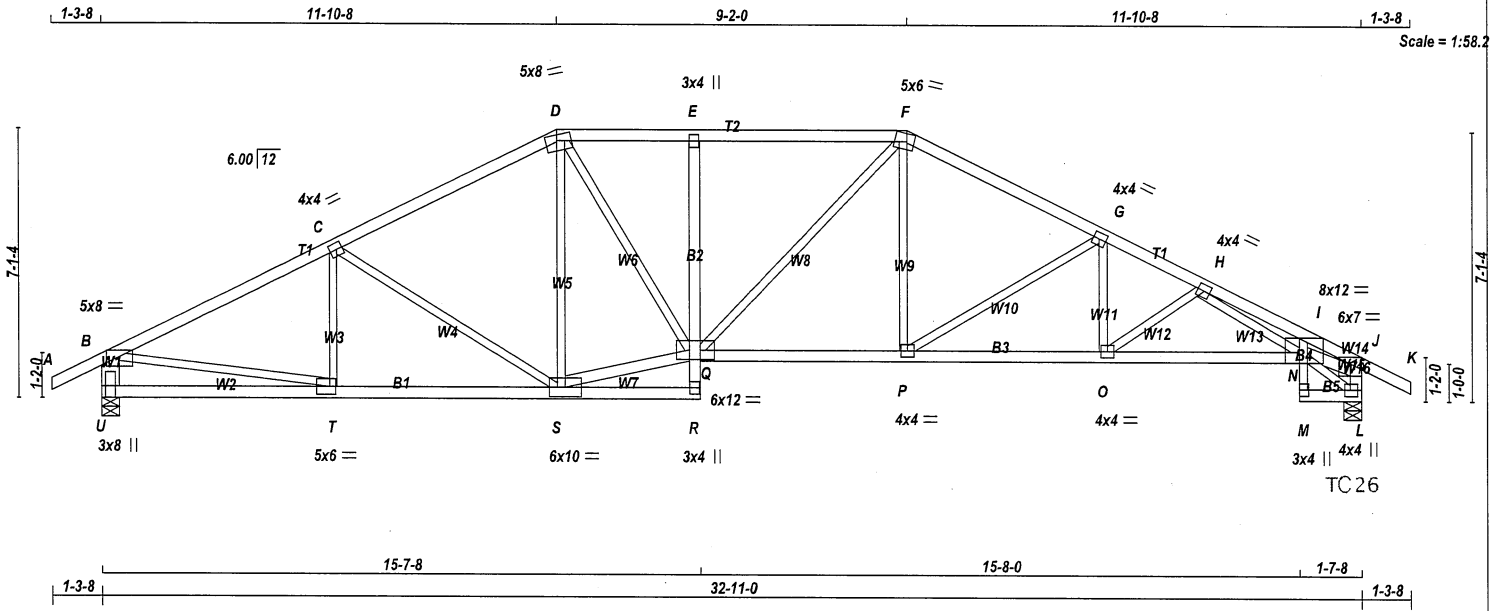
Structural component only
DWG# T-2216605

REVIEWED CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	T4S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:c3jyj23uDijq_8pvRKbkZpy75XW-blj1JZxPiBVvqGFxb?WfySVwrc97uotg2viXBaz_9?B



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - E	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
M - I	2x3	DRY	No.2
M - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - Q	2x4	DRY	No.2
N - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C, G, H						
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TTVW-m	MT20	5.0	8.0	1.75	3.50
E	TMV+p	MT20	3.0	4.0		
F	TTVW-m	MT20	5.0	6.0	2.50	2.25
I						
J	TMBVWW*-IMT20		8.0	12.0	Edge	4.50
L	TMVW-p	MT20	6.0	7.0	Edge	
J	BMVW1+p	MT20	4.0	4.0	2.00	1.75
M	BMV+p	MT20	3.0	4.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	6.0	12.0	3.00	4.50
R	BMV+p	MT20	3.0	4.0		
S	BMVW-t	MT20	6.0	10.0	3.00	2.50
T	BMVW-t	MT20	5.0	6.0	2.50	2.00
U	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	UPLIFT	IN-SX	IN-SX	BRG
U	2304	0	2304	0	0	5-8	5-8	5-8	
L	2311	0	2311	0	0	5-8	5-8	5-8	

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1612	1155 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
L	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 34	-112.4 -112.4	0.15 (1)	10.00	T-C	-293 / 23	0.08 (1)
B-C	-3169 / 0	-112.4 -112.4	0.71 (1)	3.23	C-S	-552 / 0	0.53 (1)
C-D	-2727 / 0	-112.4 -112.4	0.64 (1)	3.54	S-D	-246 / 0	0.22 (1)
D-E	-3076 / 0	-112.4 -112.4	0.43 (1)	3.58	S-Q	0 / 2445	0.39 (1)
E-F	-3085 / 0	-112.4 -112.4	0.54 (1)	3.48	D-Q	0 / 1272	0.29 (1)
F-G	-3160 / 0	-112.4 -112.4	0.51 (1)	3.49	Q-F	0 / 386	0.09 (1)
G-H	-4026 / 0	-112.4 -112.4	0.42 (1)	3.16	P-F	0 / 625	0.14 (1)
H-I	-5004 / 0	-112.4 -112.4	0.60 (1)	2.59	P-G	-967 / 0	0.64 (1)
I-J	-4915 / 0	-112.4 -112.4	0.49 (1)	2.76	O-G	0 / 364	0.08 (1)
J-K	0 / 34	-112.4 -112.4	0.15 (1)	10.00	B-T	0 / 2894	0.65 (1)
U-B	-2254 / 0	0.0 0.0	0.15 (1)	6.82	N-L	-151 / 0	0.02 (1)
L-J	-2214 / 0	0.0 0.0	0.22 (1)	5.71	N-J	0 / 4323	0.69 (1)
					O-H	-464 / 0	0.10 (1)
					H-N	0 / 640	0.14 (1)
U-T	0 / 0	-18.5 -18.5	0.15 (4)	10.00			
T-S	0 / 2865	-18.5 -18.5	0.53 (1)	10.00			
S-R	0 / 49	-18.5 -18.5	0.10 (4)	10.00			
R-Q	0 / 25	0.0 0.0	0.07 (1)	10.00			
Q-E	-644 / 0	0.0 0.0	0.34 (1)	7.81			
Q-P	0 / 2817	-18.5 -18.5	0.53 (1)	10.00			
P-O	0 / 3629	-18.5 -18.5	0.66 (1)	10.00			
O-N	0 / 4004	-18.5 -18.5	0.72 (1)	10.00			
M-N	0 / 16	0.0 0.0	0.34 (1)	10.00			
N-I	0 / 36	0.0 0.0	0.34 (1)	10.00			
M-L	0 / 126	-18.5 -18.5	0.03 (1)	10.00			

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/987 (0.40")

CSI: TC=0.71/1.00 (B-C:1), BC=0.72/1.00 (N-O:1),
WB=0.69/1.00 (J-N:1), SS=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.80 (J) (INPUT = 1.00)



Structural component only
DWG# T-2216606

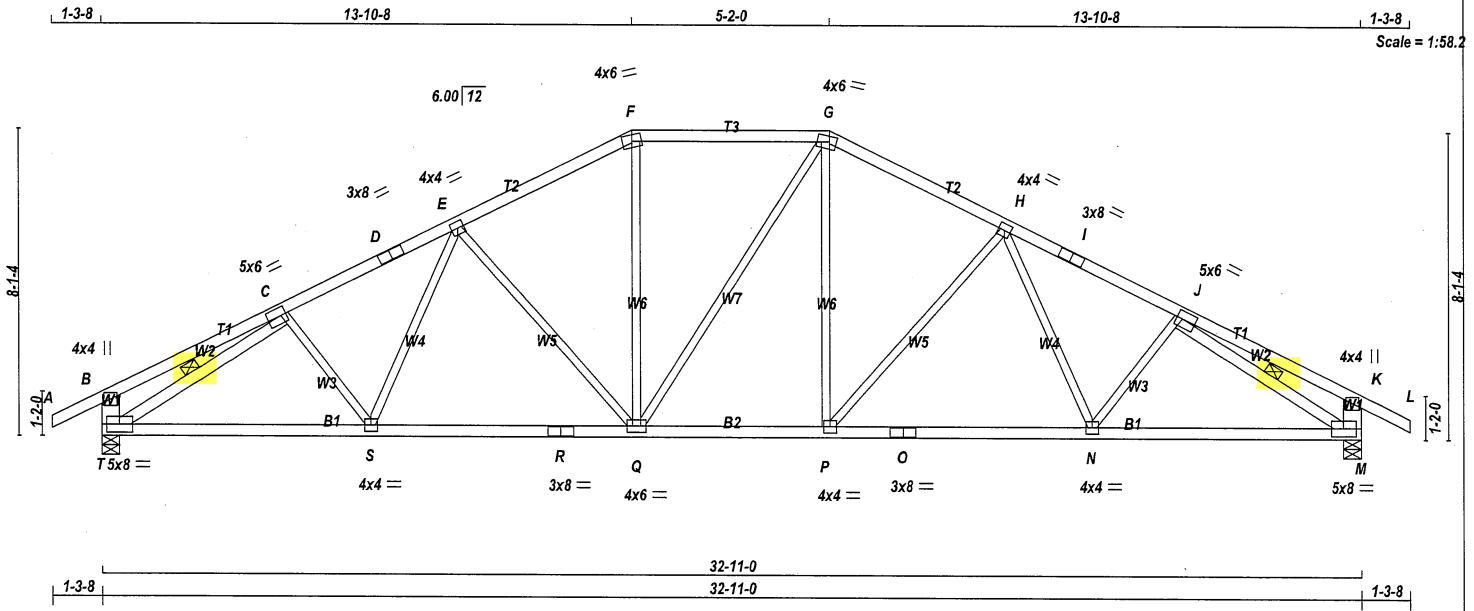
REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T5	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 143 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
T - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
T - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
T - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	4.0	6.0	1.75	2.25
H	TMWW-t	MT20	4.0	4.0		
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	8.0		
N, P, S						
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
Q	BMVWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	8.0		
T	BMVW1-t	MT20	5.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED DOWN REACTION	INPUT BRG UPLIFT	REQRD BRG IN-SX
JT	VERT	2307	0	0	5-8
T	HORZ	2307	0	0	5-8
M	HORZ	2307	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
M	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 34	C-S	-53 / 53
B-C	0 / 20	S-E	0 / 201
C-D	-3041 / 0	E-Q	-666 / 0
D-E	-3041 / 0	Q-F	0 / 616
E-F	-2501 / 0	F-G	0 / 2
F-G	-2224 / 0	G-H	0 / 614
G-H	-2500 / 0	H-I	-667 / 0
H-I	-3042 / 0	I-J	0 / 202
I-J	-3042 / 0	J-K	-53 / 54
J-K	0 / 20	K-L	-3337 / 0
K-L	0 / 34	L-M	-3337 / 0
L-M	-368 / 0		
M-K	-368 / 0		
T-S	0 / 2754		
S-R	0 / 2645		
R-Q	0 / 2645		
Q-P	0 / 2223		
P-O	0 / 2645		
O-N	0 / 2645		
N-M	0 / 2754		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/999 (0.27")

CSI: TC=0.45/1.00 (F-G:1), BC=0.55/1.00 (S-T:1), WB=0.68/1.00 (H-P:1), SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.84 (O) (INPUT = 1.00)



Structural component only
DWG# T-2216607

REVIEWED

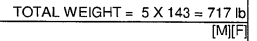
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 11:48:03 2022 Page 1
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JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.87 (Q) (INPUT = 1.00)



REVIEWED



JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.77 (C) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	T8	1	2	TRUSS DESC.		

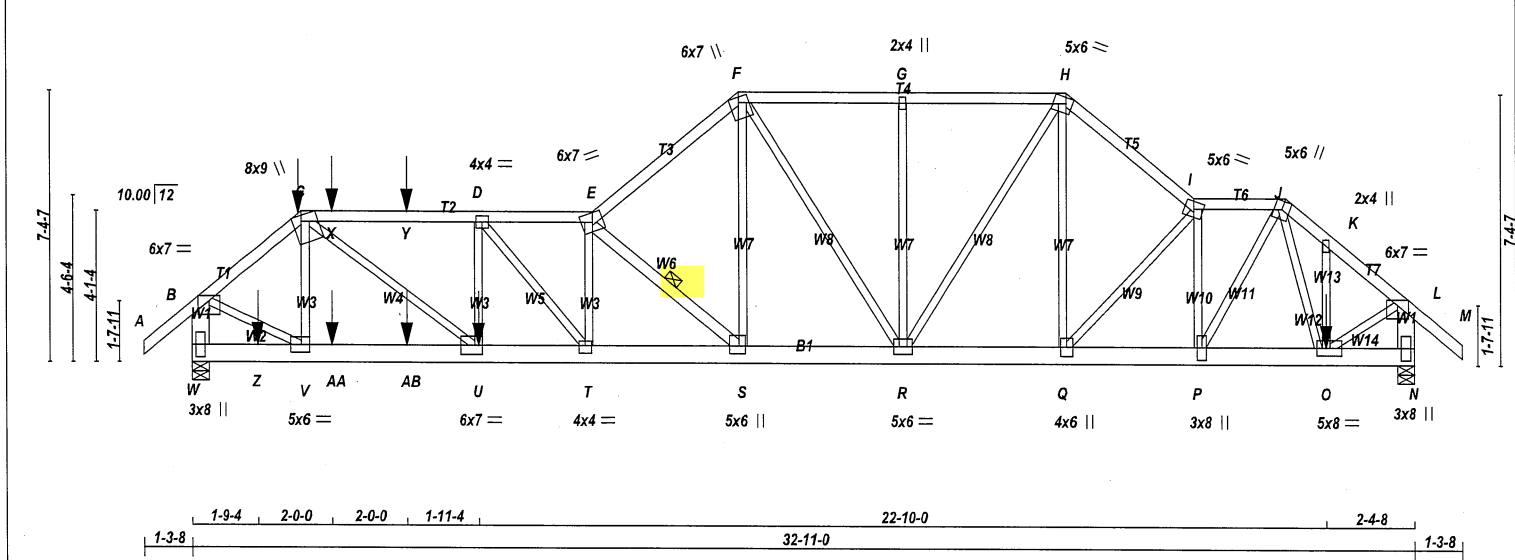
Tamarack Roof Truss, Burlington

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1-3-8 2-11-8 9-12 2-0-0 5-0-4 3-11-0 8-10-0 3-5-0 5-11-8 1-3-8

Scale = 1:59.8



<div>LUMBER</div> <div>N. L. G. A. RULES</div> <table><tr><th>CHORDS</th><th>SIZE</th><th>LUMBER</th><th>DESCR.</th></tr><tr><td>A - C</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>C - E</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>E - F</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>F - H</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>H - I</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>I - J</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>J - M</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>W - B</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>N - L</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>W - N</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr></table> <div>ALL WEBS 2x3 DRY No.2 SPF</div> <div>EXCEPT</div> <div>E - S 2x4 DRY No.2 SPF</div>										CHORDS	SIZE	LUMBER	DESCR.	A - C	2x4	DRY No.2	SPF	C - E	2x4	DRY No.2	SPF	E - F	2x4	DRY No.2	SPF	F - H	2x4	DRY No.2	SPF	H - I	2x4	DRY No.2	SPF	I - J	2x4	DRY No.2	SPF	J - M	2x4	DRY No.2	SPF	W - B	2x6	DRY No.2	SPF	N - L	2x6	DRY No.2	SPF	W - N	2x6	DRY No.2	SPF	<div>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</div> <div>BEARINGS</div> <table><tr><th>FACTORED</th><th>MAXIMUM FACTORED</th><th>INPUT</th><th>REQD</th></tr><tr><th>GROSS REACTION</th><th>GROSS REACTION</th><th>BRG</th><th></th></tr><tr><th>JT VERT</th><th>DOWN</th><th>HORZ</th><th>UPLIFT</th></tr><tr><td>W 5407 0</td><td>5407 0</td><td>0 0</td><td>5-8</td></tr><tr><td>N 6364 0</td><td>6364 0</td><td>0 0</td><td>5-8</td></tr></table> <div>UNFACTORED REACTIONS</div> <table><tr><th>1ST LCASE</th><th>MAX./MIN.</th><th>COMPONENT REACTIONS</th><th></th><th></th><th></th><th></th></tr><tr><th>JT COMBINED</th><th>SNOW</th><th>LIVE</th><th>PERM.LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr><tr><td>W 3781</td><td>2724 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>1057 / 0</td><td>0 / 0</td></tr><tr><td>N 4449</td><td>3213 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>1236 / 0</td><td>0 / 0</td></tr></table> <div>BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N</div> <div>BRACING</div> <div>TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.</div> <div>MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.</div> <div>ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.</div> <div>1 LATERAL BRACE(S) AT 1 / 2 LENGTH OF E-S.</div> <div>END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW</div> <div>LOADING</div> <div>TOTAL LOAD CASES: (4)</div> <table><tr><th colspan="4">C H O R D S</th><th colspan="4">W E B S</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. UNBRACED LENGTH (LC)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. UNBRACED LENGTH (LC)</th><th></th></tr><tr><td>FR-TO</td><td></td><td></td><td></td><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 50</td><td>-112.4 -112.4</td><td>0.09 (1)</td><td>10.00</td><td>V-C</td><td>-1344 / 0</td><td>0.17 (1)</td></tr><tr><td>B-C</td><td>-5389 / 0</td><td>-112.4 -112.4</td><td>0.20 (1)</td><td>4.03</td><td>C-U</td><td>0 / 6405</td><td>0.79 (1)</td></tr><tr><td>C-X</td><td>-9137 / 0</td><td>-112.4 -112.4</td><td>0.68 (1)</td><td>2.70</td><td>T-E</td><td>-365 / 0</td><td>0.05 (1)</td></tr><tr><td>X-Y</td><td>-9137 / 0</td><td>-112.4 -112.4</td><td>0.68 (1)</td><td>2.70</td><td>E-S</td><td>-6147 / 0</td><td>0.53 (1)</td></tr><tr><td>Y-D</td><td>-9137 / 0</td><td>-112.4 -112.4</td><td>0.68 (1)</td><td>2.70</td><td>S-F</td><td>0 / 4204</td><td>0.52 (1)</td></tr><tr><td>D-E</td><td>-9496 / 0</td><td>-112.4 -112.4</td><td>0.51 (1)</td><td>2.75</td><td>F-R</td><td>-574 / 0</td><td>0.40 (1)</td></tr><tr><td>E-F</td><td>-6490 / 0</td><td>-112.4 -112.4</td><td>0.37 (1)</td><td>3.58</td><td>R-G</td><td>-561 / 0</td><td>0.26 (1)</td></tr><tr><td>F-G</td><td>-4741 / 0</td><td>-112.4 -112.4</td><td>0.29 (1)</td><td>4.16</td><td>H-H</td><td>0 / 1457</td><td>0.18 (1)</td></tr><tr><td>G-H</td><td>-4741 / 0</td><td>-112.4 -112.4</td><td>0.29 (1)</td><td>4.16</td><td>Q-H</td><td>0 / 1643</td><td>0.20 (1)</td></tr><tr><td>H-I</td><td>-5126 / 0</td><td>-112.4 -112.4</td><td>0.23 (1)</td><td>4.10</td><td>Q-I</td><td>-2130 / 0</td><td>0.52 (1)</td></tr><tr><td>I-J</td><td>-5270 / 0</td><td>-112.4 -112.4</td><td>0.15 (1)</td><td>4.11</td><td>P-I</td><td>-2040 / 0</td><td>0.30 (1)</td></tr><tr><td>J-K</td><td>-5686 / 0</td><td>-112.4 -112.4</td><td>0.15 (1)</td><td>3.97</td><td>B-V</td><td>0 / 4441</td><td>0.55 (1)</td></tr><tr><td>K-L</td><td>-5746 / 0</td><td>-112.4 -112.4</td><td>0.19 (1)</td><td>3.92</td><td>J-O</td><td>0 / 1327</td><td>0.16 (1)</td></tr><tr><td>L-M</td><td>0 / 50</td><td>-112.4 -112.4</td><td>0.09 (1)</td><td>10.00</td><td>P-J</td><td>0 / 2490</td><td>0.31 (1)</td></tr><tr><td>W-B</td><td>-5436 / 0</td><td>0.0 0.0</td><td>0.20 (1)</td><td>6.29</td><td>U-D</td><td>-1026 / 0</td><td>0.13 (1)</td></tr><tr><td>N-L</td><td>-6240 / 0</td><td>0.0 0.0</td><td>0.23 (1)</td><td>5.94</td><td>D-T</td><td>0 / 576</td><td>0.07 (1)</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td>O-K</td><td>-108 / 2</td><td>0.01 (1)</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td>O-L</td><td>0 / 4945</td><td>0.61 (1)</td></tr><tr><td>W-Z</td><td>0 / 0</td><td>-18.5 -18.5</td><td>0.08 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>Z-V</td><td>0 / 0</td><td>-18.5 -18.5</td><td>0.08 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>V-AA</td><td>0 / 4087</td><td>-18.5 -18.5</td><td>0.38 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>AA-AB</td><td>0 / 4087</td><td>-18.5 -18.5</td><td>0.38 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>AB-U</td><td>0 / 4087</td><td>-18.5 -18.5</td><td>0.38 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>U-T</td><td>0 / 9137</td><td>-18.5 -18.5</td><td>0.71 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>T-S</td><td>0 / 9506</td><td>-18.5 -18.5</td><td>0.70 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>S-R</td><td>0 / 5049</td><td>-18.5 -18.5</td><td>0.38 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>R-Q</td><td>0 / 3962</td><td>-18.5 -18.5</td><td>0.31 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>Q-P</td><td>0 / 5321</td><td>-18.5 -18.5</td><td>0.38 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>P-O</td><td>0 / 4045</td><td>-18.5 -18.5</td><td>0.35 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>O-N</td><td>0 / 0</td><td>-18.5 -18.5</td><td>0.09 (1)</td><td>10.00</td><td></td><td></td><td></td></tr></table> <div>SPECIFIED CONCENTRATED LOADS (LBS)</div> <table><tr><th>JT</th><th>LOC.</th><th>LC1</th><th>MAX-</th><th>MAX+</th><th>FACE</th><th>DIR.</th><th>TYPE</th><th>HEEL</th><th>CONN.</th></tr><tr><td>C</td><td>2-11-8</td><td>-30</td><td>-30</td><td>---</td><td>FRONT</td><td>VERT</td><td>DEAD</td><td>---</td><td>C1</td></tr><tr><td>C</td><td>2-11-8</td><td>-159</td><td>-159</td><td>---</td><td>FRONT</td><td>VERT</td><td>SNOW</td><td>---</td><td>C1</td></tr><tr><td>O</td><td>30-6-8</td><td>-2471</td><td>-2471</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr><tr><td>U</td><td>7-8-8</td><td>-2080</td><td>-2080</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr><tr><td>X</td><td>3-9-4</td><td>-100</td><td>-100</td><td>---</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>---</td><td>C1</td></tr></table>										FACTORED	MAXIMUM FACTORED	INPUT	REQD	GROSS REACTION	GROSS REACTION	BRG		JT VERT	DOWN	HORZ	UPLIFT	W 5407 0	5407 0	0 0	5-8	N 6364 0	6364 0	0 0	5-8	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS					JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	W 3781	2724 / 0	0 / 0	0 / 0	0 / 0	1057 / 0	0 / 0	N 4449	3213 / 0	0 / 0	0 / 0	0 / 0	1236 / 0	0 / 0	C H O R D S				W E B S				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)		FR-TO				FR-TO				A-B	0 / 50	-112.4 -112.4	0.09 (1)	10.00	V-C	-1344 / 0	0.17 (1)	B-C	-5389 / 0	-112.4 -112.4	0.20 (1)	4.03	C-U	0 / 6405	0.79 (1)	C-X	-9137 / 0	-112.4 -112.4	0.68 (1)	2.70	T-E	-365 / 0	0.05 (1)	X-Y	-9137 / 0	-112.4 -112.4	0.68 (1)	2.70	E-S	-6147 / 0	0.53 (1)	Y-D	-9137 / 0	-112.4 -112.4	0.68 (1)	2.70	S-F	0 / 4204	0.52 (1)	D-E	-9496 / 0	-112.4 -112.4	0.51 (1)	2.75	F-R	-574 / 0	0.40 (1)	E-F	-6490 / 0	-112.4 -112.4	0.37 (1)	3.58	R-G	-561 / 0	0.26 (1)	F-G	-4741 / 0	-112.4 -112.4	0.29 (1)	4.16	H-H	0 / 1457	0.18 (1)	G-H	-4741 / 0	-112.4 -112.4	0.29 (1)	4.16	Q-H	0 / 1643	0.20 (1)	H-I	-5126 / 0	-112.4 -112.4	0.23 (1)	4.10	Q-I	-2130 / 0	0.52 (1)	I-J	-5270 / 0	-112.4 -112.4	0.15 (1)	4.11	P-I	-2040 / 0	0.30 (1)	J-K	-5686 / 0	-112.4 -112.4	0.15 (1)	3.97	B-V	0 / 4441	0.55 (1)	K-L	-5746 / 0	-112.4 -112.4	0.19 (1)	3.92	J-O	0 / 1327	0.16 (1)	L-M	0 / 50	-112.4 -112.4	0.09 (1)	10.00	P-J	0 / 2490	0.31 (1)	W-B	-5436 / 0	0.0 0.0	0.20 (1)	6.29	U-D	-1026 / 0	0.13 (1)	N-L	-6240 / 0	0.0 0.0	0.23 (1)	5.94	D-T	0 / 576	0.07 (1)						O-K	-108 / 2	0.01 (1)						O-L	0 / 4945	0.61 (1)	W-Z	0 / 0	-18.5 -18.5	0.08 (1)	10.00				Z-V	0 / 0	-18.5 -18.5	0.08 (1)	10.00				V-AA	0 / 4087	-18.5 -18.5	0.38 (1)	10.00				AA-AB	0 / 4087	-18.5 -18.5	0.38 (1)	10.00				AB-U	0 / 4087	-18.5 -18.5	0.38 (1)	10.00				U-T	0 / 9137	-18.5 -18.5	0.71 (1)	10.00				T-S	0 / 9506	-18.5 -18.5	0.70 (1)	10.00				S-R	0 / 5049	-18.5 -18.5	0.38 (1)	10.00				R-Q	0 / 3962	-18.5 -18.5	0.31 (1)	10.00				Q-P	0 / 5321	-18.5 -18.5	0.38 (1)	10.00				P-O	0 / 4045	-18.5 -18.5	0.35 (1)	10.00				O-N	0 / 0	-18.5 -18.5	0.09 (1)	10.00				JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1	C	2-11-8	-159	-159	---	FRONT	VERT	SNOW	---	C1	O	30-6-8	-2471	-2471	---	FRONT	VERT	TOTAL	---	C1	U	7-8-8	-2080	-2080	---	FRONT	VERT	TOTAL	---	C1	X	3-9-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
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MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)																																																																																																																																																																																																																																																																																																																																																																																																																																													
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C-X	-9137 / 0	-112.4 -112.4	0.68 (1)	2.70	T-E	-365 / 0	0.05 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
X-Y	-9137 / 0	-112.4 -112.4	0.68 (1)	2.70	E-S	-6147 / 0	0.53 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
Y-D	-9137 / 0	-112.4 -112.4	0.68 (1)	2.70	S-F	0 / 4204	0.52 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
D-E	-9496 / 0	-112.4 -112.4	0.51 (1)	2.75	F-R	-574 / 0	0.40 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
E-F	-6490 / 0	-112.4 -112.4	0.37 (1)	3.58	R-G	-561 / 0	0.26 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
F-G	-4741 / 0	-112.4 -112.4	0.29 (1)	4.16	H-H	0 / 1457	0.18 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
G-H	-4741 / 0	-112.4 -112.4	0.29 (1)	4.16	Q-H	0 / 1643	0.20 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
H-I	-5126 / 0	-112.4 -112.4	0.23 (1)	4.10	Q-I	-2130 / 0	0.52 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
I-J	-5270 / 0	-112.4 -112.4	0.15 (1)	4.11	P-I	-2040 / 0	0.30 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
J-K	-5686 / 0	-112.4 -112.4	0.15 (1)	3.97	B-V	0 / 4441	0.55 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
K-L	-5746 / 0	-112.4 -112.4	0.19 (1)	3.92	J-O	0 / 1327	0.16 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																												
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AB-U	0 / 4087	-18.5 -18.5	0.38 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																															
U-T	0 / 9137	-18.5 -18.5	0.71 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																															
T-S	0 / 9506	-18.5 -18.5	0.70 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																															
S-R	0 / 5049	-18.5 -18.5	0.38 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																															
R-Q	0 / 3962	-18.5 -18.5	0.31 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																															
Q-P	0 / 5321	-18.5 -18.5	0.38 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																															
P-O	0 / 4045	-18.5 -18.5	0.35 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																															
O-N	0 / 0	-18.5 -18.5	0.09 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																															
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.																																																																																																																																																																																																																																																																																																																																																																																																																																										
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1																																																																																																																																																																																																																																																																																																																																																																																																																																										
C	2-11-8	-159	-159	---	FRONT	VERT	SNOW	---	C1																																																																																																																																																																																																																																																																																																																																																																																																																																										
O	30-6-8	-2471	-2471	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																																																										
U	7-8-8	-2080	-2080	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																																																										
X	3-9-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1																																																																																																																																																																																																																																																																																																																																																																																																																																										

| DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS | | | | | |-----|---|----|------------| | A-C | 1 | 12 | SIDE(61.0) | | C-E | 1 | 12 | SIDE(61.0) | | E-F | 1 | 12 | TOP | | F-H | 1 | 12 | TOP | | H-I | 1 | 12 | TOP | | I-J | 1 | 12 | TOP | | J-M | 1 | 12 | TOP | | W-B | 2 | 12 | TOP | | N-L | 2 | 12 | TOP | BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS | | | | | |-----|---|----|-------------| | W-N | 2 | 12 | SIDE(183.1) | |-----|---|----|-------------| WEBS : (0.122"x3") SPIRAL NAILS | | | | | |-----|---|---|-------------| | 2x3 | 1 | 6 | SIDE(593.4) | | D-U | 1 | 3 | SIDE(593.4) | | K-O | 1 | 2 | SIDE(739.7) | | 2x4 | 1 | 6 | SIDE(739.7) | NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. | | | | | | | | | | DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: | | | | |------------|---------------|--------------| | TOP CH. | LL = 32.5 PSF | DL = 6.0 PSF | | BOT CH. | LL = 0.0 PSF | DL = 7.4 PSF | | TOTAL LOAD | = 45.9 PSF | | SPACING = 24.0 IN. C/C LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.10") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22") ALLOWABLE DEFL.(TL)= L/360 (1.10") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39") CSI: TC=0.68/1.00 (C-D-1) , BC=0.71/1.00 (T-U-1) , WB=0.79/1.00 (C-U-1) , SSI=0.18/1.00 (C-D-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES | | | | |-----------------|----------|-----------| | PLATE GRIP(DRY) | SHEAR | SECTION | | (PSI) | (PLI) | (PLI) | | MAX MIN | MAX MIN | MAX MIN | | MT20 650 371 | 1747 798 | 1987 1873 | PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (U) (INPUT = 0.90) JSI METAL= 0.78 (S) (INPUT = 1.00) | | | | | | | | | |

LICENSED PROFESSIONAL ENGINEER

07-08-22

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only

DWG# T-2216610

REVIEWED

CONTINUED ON PAGE



Structural component only
DWG# T-2216610

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T8	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 11:48:06 2022 Page 2
ID:c3iyj23uDiiq 8pvRKbkZpv75XVW-U4yX9x vnQ0KltZlqrbb6lqdiDWBqaLGzXGikMz 9?7

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TTWW+m	MT20	8.0	9.0	Edge	2.75
D	TMWW-t	MT20	4.0	4.0		
E	TTWW-m	MT20	6.0	7.0		
F	TTWW+m	MT20	6.0	7.0	1.75	2.75
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.00
I	TTWW-m	MT20	5.0	6.0		
J	TTWW+m	MT20	5.0	6.0	Edge	
K	TMW+w	MT20	2.0	4.0		
L	TMVW-p	MT20	6.0	7.0	Edge	
N	BMV1+p	MT20	3.0	8.0		
O	BMWWW-t	MT20	5.0	8.0	2.50	1.75
P	BMWW+t	MT20	3.0	8.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BMWW+t	MT20	5.0	6.0	2.50	2.00
T	BMWW-t	MT20	4.0	4.0	2.50	2.00
U	BMWW-t	MT20	6.0	7.0	3.00	2.50
V	BMWW-t	MT20	5.0	6.0	2.50	2.50
W	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	5-9-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Z	1-9-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AA	3-9-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AB	5-9-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

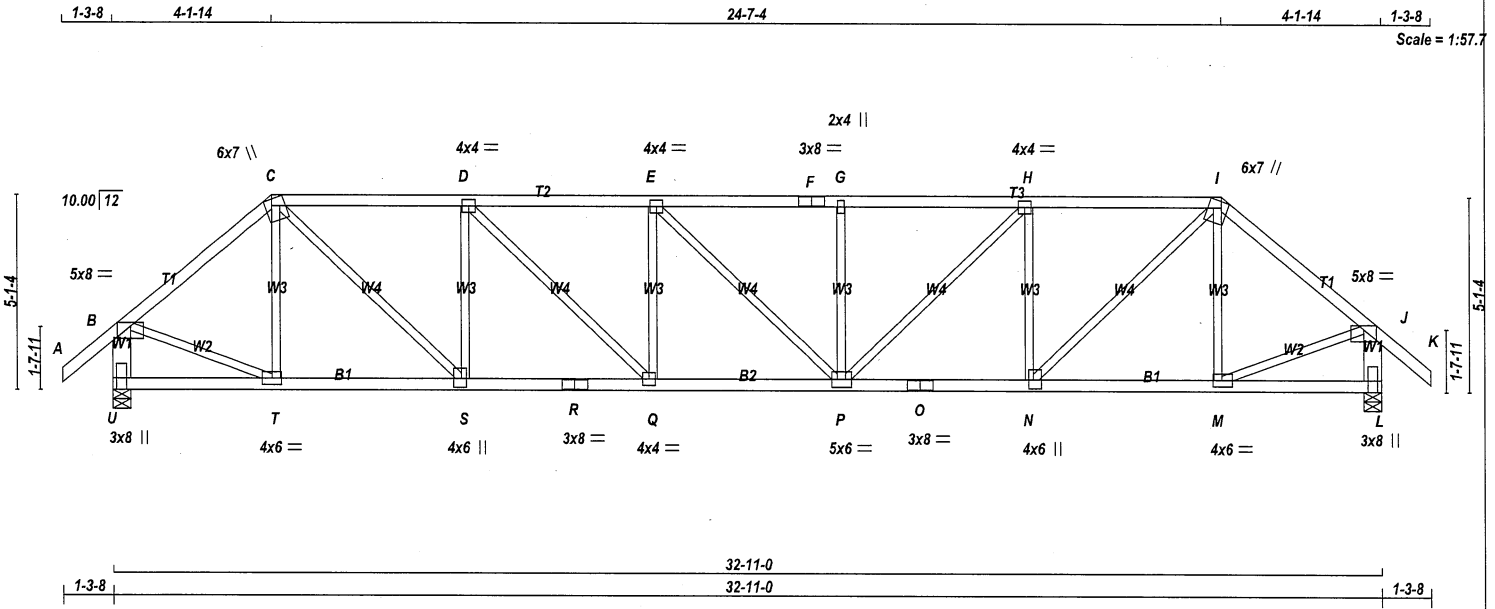


Structural component only
DWG# T-2216610

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T9	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
L - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	7.0	2.00	2.00
D, E, H						
F	TMVW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	8.0		
G	TMW+m	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	7.0	2.00	2.00
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	8.0	Edge	
M	BMVW-t	MT20	4.0	6.0	2.00	2.75
N	BMVW-t	MT20	4.0	6.0	2.75	1.50
O	BS-t	MT20	3.0	8.0		
P	BMVWW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	8.0		
S	BMVW+t	MT20	4.0	6.0	2.75	1.50
T	BMVW-t	MT20	4.0	6.0	2.00	2.75
U	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
U	2311	0	2311	0	5-8
L	2311	0	2311	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
L	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE	VERT.	MAX	MEMB.	FORCE	MAX	MAX
(LBS)	(PLF)	LC1	CS1 (LC)	(LBS)	(LBS)	CS1 (LC)	(LC)
FR-TO		FROM	TO	FR-TO		LENGTH	FR-TO
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	T-C	-408 / 0
B-C	-2156 / 0	-112.4	-112.4	0.45 (1)	4.18	C-S	0 / 1906
C-D	-3017 / 0	-112.4	-112.4	0.57 (1)	3.48	S-D	-1234 / 0
D-E	-3649 / 0	-112.4	-112.4	0.65 (1)	3.11	D-Q	0 / 888
E-F	-3647 / 0	-112.4	-112.4	0.53 (1)	3.28	Q-E	-532 / 0
F-G	-3647 / 0	-112.4	-112.4	0.53 (1)	3.28	E-P	-2 / 0
G-H	-3647 / 0	-112.4	-112.4	0.65 (1)	3.11	P-G	-531 / 0
H-I	-3018 / 0	-112.4	-112.4	0.58 (1)	3.48	P-H	0 / 884
I-J	-2157 / 0	-112.4	-112.4	0.45 (1)	4.18	H-N	-1233 / 0
J-K	0 / 50	-112.4	-112.4	0.15 (1)	10.00	N-I	0 / 1906
U-B	-2281 / 0	0.0	0.0	0.16 (1)	6.79	M-I	-408 / 0
L-J	-2281 / 0	0.0	0.0	0.16 (1)	6.79	B-T	0 / 1728
						M-J	0 / 1728
U-T	0 / 0	-18.5	-18.5	0.09 (4)	10.00		
T-S	0 / 1643	-18.5	-18.5	0.32 (1)	10.00		
S-R	0 / 3017	-18.5	-18.5	0.53 (1)	10.00		
R-Q	0 / 3017	-18.5	-18.5	0.53 (1)	10.00		
Q-P	0 / 3649	-18.5	-18.5	0.64 (1)	10.00		
P-O	0 / 3018	-18.5	-18.5	0.53 (1)	10.00		
O-N	0 / 3018	-18.5	-18.5	0.53 (1)	10.00		
N-M	0 / 1644	-18.5	-18.5	0.32 (1)	10.00		
M-L	0 / 0	-18.5	-18.5	0.09 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.36")

CSI: TC=0.65/1.00 (G-H:1), BC=0.64/1.00 (P-Q:1), WB=0.47/1.00 (D-S:1), SSI=0.26/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.93 (O) (INPUT = 1.00)



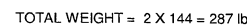
Structural component only
DWG# T-2216611

REVIEWED

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Fri Jul 8 11:48:08 2022 Page 1

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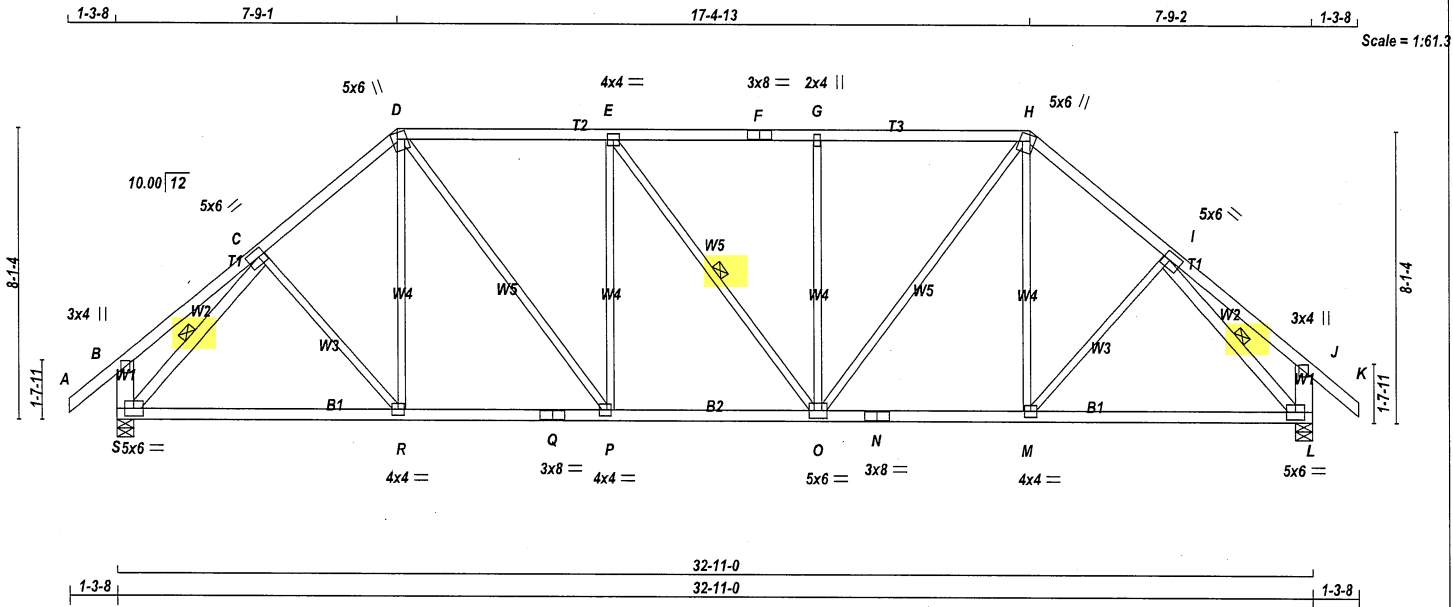


JSI GRIP= 0.85 (R) (INPUT = 0.90)
JSI METAL= 0.80 (P) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T12	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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ID:c3iyj23uDiIq_8pvRKbkZpy75XW-MrC2_11QqeWmnVs33hgXH8r1OqyfmNnsu9EyT7z_9?3



TOTAL WEIGHT = 2 X 159 = 317 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.00	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMVW-t	MT20	4.0	4.0	2.00	1.50
Q	BS-t	MT20	3.0	8.0		
R	BMVW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
S	2311	0	2311	0	0	0	5-8	5-8	5-8
L	2311	0	2311	0	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SO
S	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
L	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O, C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					W E B S				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)		(LC)	UNBRAC		(LBS)	(LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	C-R	0 / 55	0.02 (4)	
B-C	0 / 32	-112.4	-112.4	0.26 (1)	10.00	R-D	0 / 141	0.04 (4)	
C-D	-2187 / 0	-112.4	-112.4	0.36 (1)	4.27	D-P	0 / 999	0.22 (1)	
D-E	-2252 / 0	-112.4	-112.4	0.67 (1)	3.78	P-E	-704 / 0	0.89 (1)	
E-F	-2251 / 0	-112.4	-112.4	0.67 (1)	3.78	E-O	-3 / 0	0.00 (1)	
F-G	-2251 / 0	-112.4	-112.4	0.67 (1)	3.78	O-G	-703 / 0	0.89 (1)	
G-H	-2251 / 0	-112.4	-112.4	0.66 (1)	3.80	O-H	0 / 996	0.22 (1)	
H-I	-2188 / 0	-112.4	-112.4	0.36 (1)	4.27	M-H	0 / 144	0.04 (4)	
I-J	0 / 32	-112.4	-112.4	0.26 (1)	10.00	M-I	0 / 54	0.02 (4)	
J-K	0 / 50	-112.4	-112.4	0.15 (1)	10.00	S-C	-2534 / 0	0.44 (1)	
S-B	-322 / 0	0.0	0.0	0.02 (1)	7.81	I-L	-2535 / 0	0.44 (1)	
L-J	-322 / 0	0.0	0.0	0.02 (1)	7.81				
S-R	0 / 1644	-18.5	-18.5	0.40 (1)	10.00				
R-Q	0 / 1654	-18.5	-18.5	0.40 (1)	10.00				
Q-P	0 / 1654	-18.5	-18.5	0.40 (1)	10.00				
P-O	0 / 2253	-18.5	-18.5	0.42 (1)	10.00				
O-N	0 / 1654	-18.5	-18.5	0.40 (1)	10.00				
N-M	0 / 1654	-18.5	-18.5	0.40 (1)	10.00				
M-L	0 / 1645	-18.5	-18.5	0.40 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")
CSI: TC=0.67/1.00 (D-E-1), BC=0.42/1.00 (O-P-1), WB=0.89/1.00 (E-P-1), SSI=0.31/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)				
		MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	798	1987	1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.57 (I) (INPUT = 1.00)

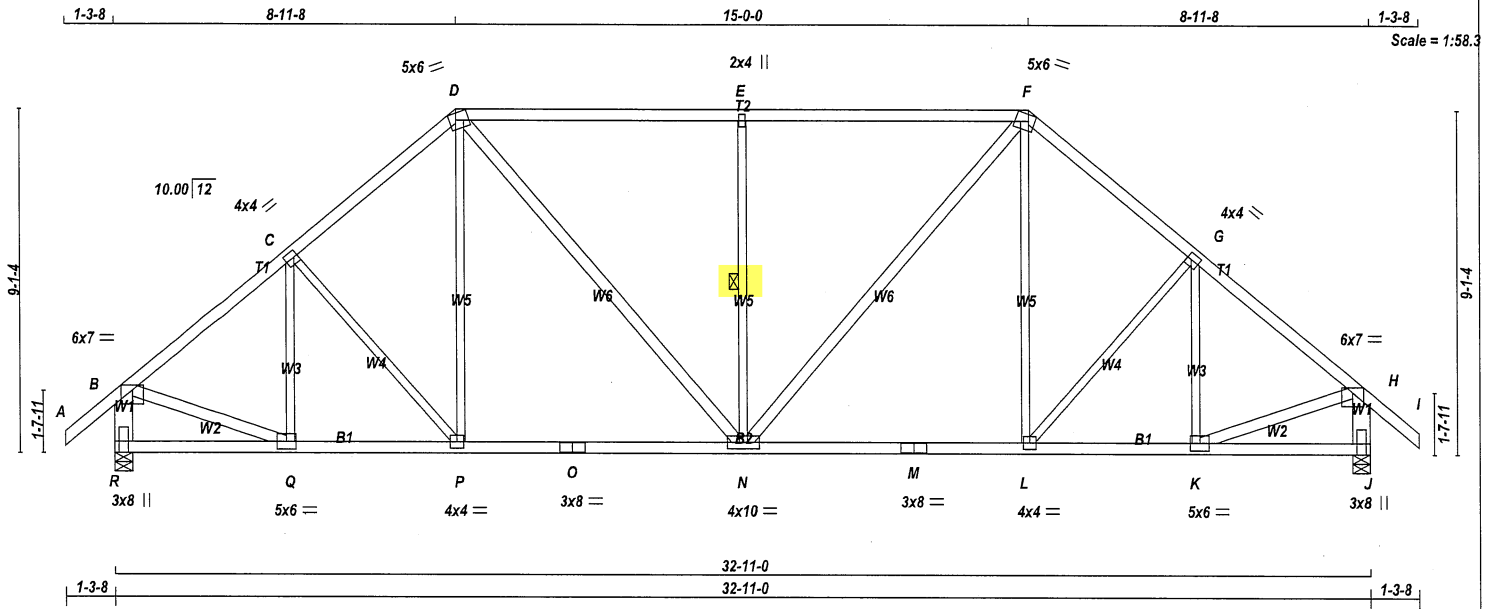


Structural component only
DWG# T-2216614

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T13	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Fri Jul 8 11:48:10 2022 Page 1
ID:c3iyj23uDijq_8pvRKbkZpy75XW-MrC2_11QqeWmnVs33hgXH8rEEqzMmSsyu9EyT7z_9?3



TOTAL WEIGHT = 2 X 165 = 330 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2
B - Q	2x4	DRY	No.2
K - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	6.0	7.0	Edge	
J	BMV1-p	MT20	3.0	8.0	Edge	
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-wt	MT20	4.0	10.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0		
R	BMV1-p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORZ	UPLIFT
R 2311 0	2311 0	0 5-8	5-8
J 2311 0	2311 0	0 5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R 1617 1159 / 0	0 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
J 1617 1159 / 0	0 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 50	-112.4 -112.4 0.15 (1)	10.00
B-C	-2195 / 0	-112.4 -112.4 0.36 (1)	4.27
C-D	-2131 / 0	-112.4 -112.4 0.35 (1)	4.33
D-E	-2110 / 0	-112.4 -112.4 0.94 (1)	3.05
E-F	-2110 / 0	-112.4 -112.4 0.94 (1)	3.05
F-G	-2131 / 0	-112.4 -112.4 0.35 (1)	4.33
G-H	-2195 / 0	-112.4 -112.4 0.36 (1)	4.27
H-I	0 / 50	-112.4 -112.4 0.15 (1)	10.00
R-B	-2272 / 0	0.0 0.0 0.16 (1)	6.80
J-H	-2272 / 0	0.0 0.0 0.16 (1)	6.80
R-Q	0 / 0	-18.5 -18.5 0.08 (4)	10.00
Q-P	0 / 1715	-18.5 -18.5 0.36 (1)	10.00
P-O	0 / 1606	-18.5 -18.5 0.38 (1)	10.00
O-N	0 / 1606	-18.5 -18.5 0.38 (1)	10.00
N-M	0 / 1606	-18.5 -18.5 0.38 (1)	10.00
M-L	0 / 1606	-18.5 -18.5 0.38 (1)	10.00
L-K	0 / 1715	-18.5 -18.5 0.36 (1)	10.00
K-J	0 / 0	-18.5 -18.5 0.08 (4)	10.00

WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	CS (LC)
Q-C	-410 / 0	0.18 (1)	
C-P	-170 / 0	0.15 (1)	
P-D	0 / 244	0.05 (1)	
D-N	0 / 777	0.12 (1)	
N-E	-1038 / 0	0.56 (1)	
N-F	0 / 777	0.12 (1)	
L-F	0 / 244	0.05 (1)	
L-G	-170 / 0	0.15 (1)	
K-G	-410 / 0	0.18 (1)	
B-Q	0 / 1783	0.29 (1)	
K-H	0 / 1783	0.29 (1)	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/999 (0.17")

CSI: TC=0.94/1.00 (D-E:1), BC=0.38/1.00 (L-N:1), WB=0.56/1.00 (E-N:1), SSI=0.41/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.58 (O) (INPUT = 1.00)



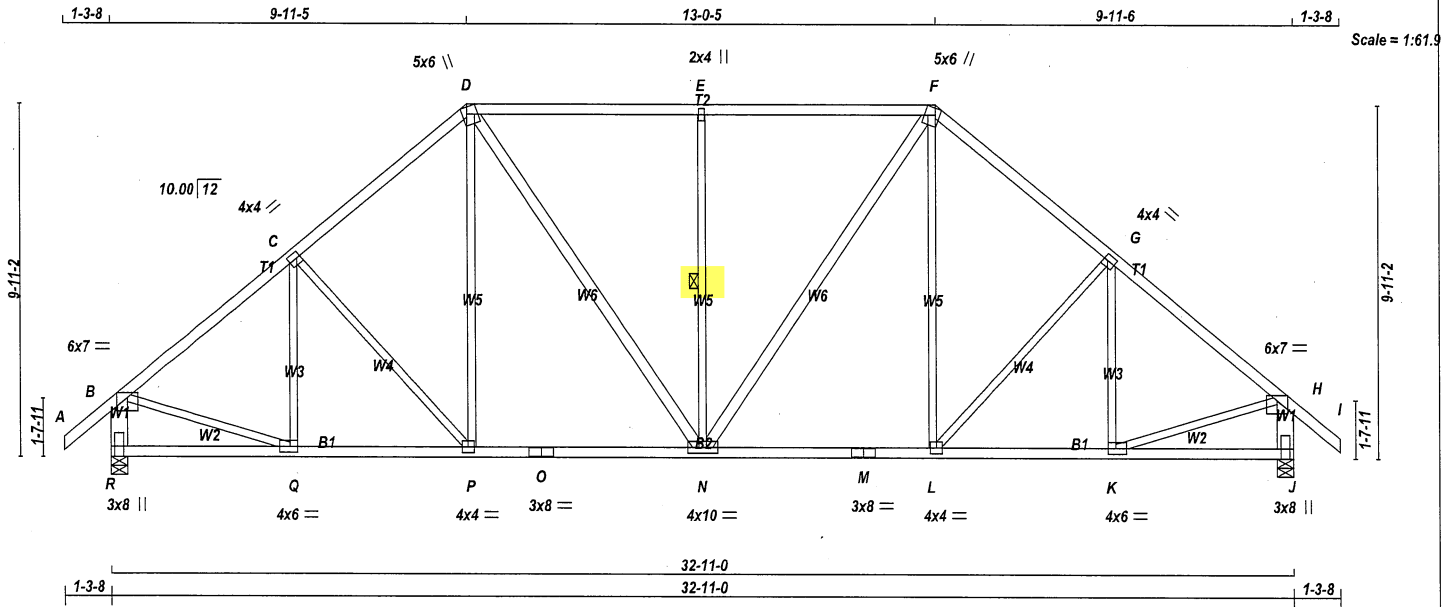
Structural component only
DWG# T-2216615

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	T14	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 167 lb
[M][F]

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	6.0	7.0	Edge	
J	BMV1+p	MT20	3.0	8.0	Edge	
K	BMVW-t	MT20	4.0	6.0	2.00	2.50
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-w	MT20	4.0	10.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	4.0	6.0	2.00	2.50
R	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
R	2311	0	2311	0	0	5-8	5-8		
J	2311	0	2311	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX / MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
J	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 50	-112.4 -112.4	0.15 (1)	Q-C	-352 / 0
B-C	-2219 / 0	-112.4 -112.4	0.45 (1)	C-P	-268 / 0
C-D	-2077 / 0	-112.4 -112.4	0.43 (1)	P-D	0 / 306
D-E	-1907 / 0	-112.4 -112.4	0.68 (1)	D-N	0 / 617
E-F	-1907 / 0	-112.4 -112.4	0.68 (1)	N-E	-898 / 0
F-G	-2077 / 0	-112.4 -112.4	0.43 (1)	N-F	0 / 617
G-H	-2220 / 0	-112.4 -112.4	0.45 (1)	L-F	0 / 306
H-I	0 / 50	-112.4 -112.4	0.15 (1)	L-G	-268 / 0
R-B	-2270 / 0	0.0	0.0	K-G	-351 / 0
J-H	-2270 / 0	0.0	0.0	B-Q	0 / 1794
				K-H	0 / 1794
R-Q	0 / 0	-18.5 -18.5	0.10 (4)		
Q-P	0 / 1737	-18.5 -18.5	0.35 (1)		
P-O	0 / 1561	-18.5 -18.5	0.34 (1)		
O-N	0 / 1561	-18.5 -18.5	0.34 (1)		
N-M	0 / 1562	-18.5 -18.5	0.34 (1)		
M-L	0 / 1562	-18.5 -18.5	0.34 (1)		
L-K	0 / 1738	-18.5 -18.5	0.35 (1)		
K-J	0 / 0	-18.5 -18.5	0.10 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.68/1.00 (D-E:1), BC=0.35/1.00 (K-L:1), WB=0.60/1.00 (E-N:1), SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.51 (O) (INPUT = 1.00)



Structural component only
DWG# T-2216616

REVIEWED

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 11:48:13 2022 Page 1
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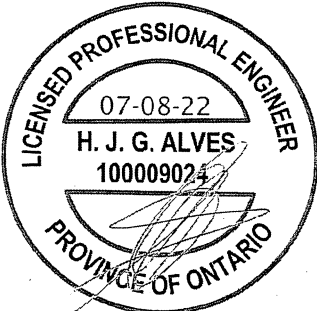
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T15	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 11:48:13 2022 Page 2
ID:c3iyi23uDijg 8pvRKbkZpy75XW-nQtBdK3l7ZuLeybekpDEumSvp20wzt2la7Tc4Sz 9?0

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.00	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	8.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0	2.50	1.50
L	BMWWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



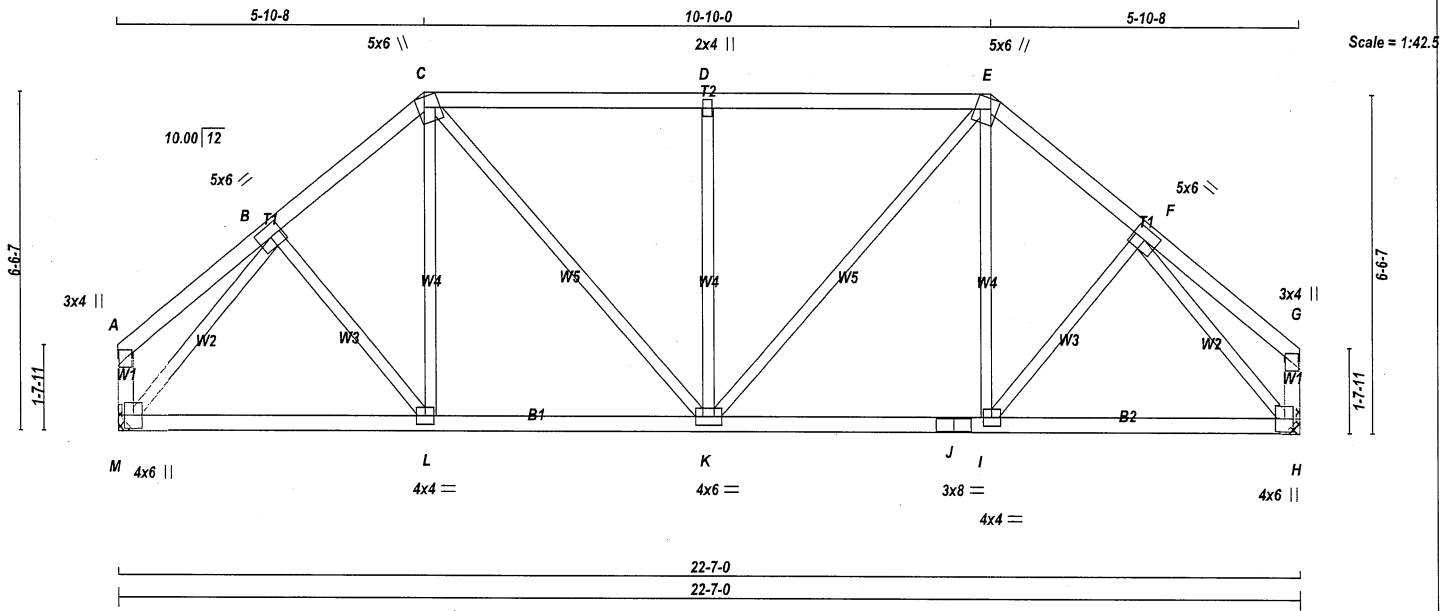
Structural component only
DWG# T-2216617

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	T16	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Fri Jul 8 11:48:13 2022 Page 1
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TOTAL WEIGHT = 101 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - A	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWw-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWw-t	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWw-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	6.0		
I	BMVW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	8.0		
K	BMVWw-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMVW1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT
M	1478	0	1478	0	0
H	1478	0	1478	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M = 3-8, JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1036	733 / 0	0 / 0	0 / 0	0 / 0	303 / 0	0 / 0
H	1036	733 / 0	0 / 0	0 / 0	0 / 0	303 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 22	-112.4 -112.4	0.14 (1)	10.00	B-L	0 / 49	0.02 (4)
B-C	-1383 / 0	-112.4 -112.4	0.13 (1)	5.41	L-C	0 / 98	0.03 (4)
C-D	-1404 / 0	-112.4 -112.4	0.45 (1)	4.95	C-K	0 / 549	0.12 (1)
D-E	-1404 / 0	-112.4 -112.4	0.45 (1)	4.95	K-D	-744 / 0	0.52 (1)
E-F	-1383 / 0	-112.4 -112.4	0.13 (1)	5.41	K-E	0 / 549	0.12 (1)
F-G	0 / 22	-112.4 -112.4	0.14 (1)	10.00	I-E	0 / 98	0.03 (4)
M-A	-129 / 0	0.0 0.0	0.01 (1)	7.81	I-F	0 / 49	0.02 (4)
H-G	-129 / 0	0.0 0.0	0.01 (1)	7.81	M-B	-1654 / 0	0.59 (1)
					F-H	-1654 / 0	0.59 (1)
M-L	0 / 1017	-18.5 -18.5	0.25 (1)	10.00			
L-K	0 / 1044	-18.5 -18.5	0.25 (1)	10.00			
K-J	0 / 1044	-18.5 -18.5	0.25 (1)	10.00			
J-I	0 / 1044	-18.5 -18.5	0.25 (1)	10.00			
I-H	0 / 1017	-18.5 -18.5	0.25 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.45/1.00 (C-D:1), BC=0.25/1.00 (I-K:1), WB=0.59/1.00 (B-M:1), SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (H) (INPUT = 0.90)
JSI METAL= 0.46 (H) (INPUT = 1.00)



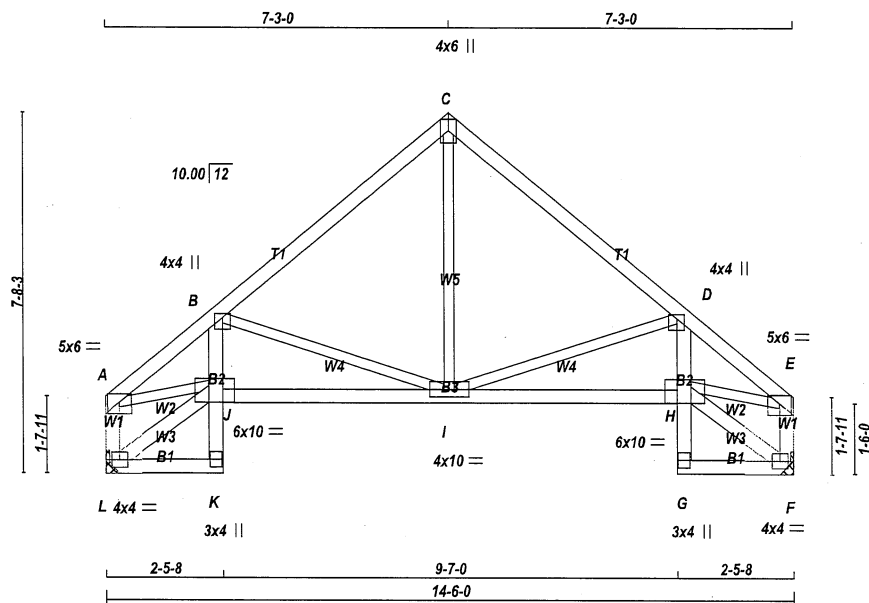
Structural component only
DWG# T-2216618

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T17S	3	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale = 1:46.9

TOTAL WEIGHT = 3 X 70 = 209 lb
[M][F]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
L - A	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
L - K	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
G - D	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
L - J	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	6.0	Edge	
D	TMVW-p	MT20	4.0	4.0	1.00	2.00
E	TMVW-p	MT20	5.0	6.0	Edge	
F	BMVW1-t	MT20	4.0	4.0		
G	BMV-p	MT20	3.0	4.0		
H	BVMWV-I	MT20	6.0	10.0	4.25	6.50
I	BVMWV-I	MT20	4.0	10.0		
J	BVMWV-I	MT20	6.0	10.0	4.25	6.50
K	BMV-p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
L	949	0	949	0	0
F	949	0	949	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L, F. MINIMUM BEARING LENGTH AT JOINT L = 3-8, JOINT F = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	665	471 / 0	0 / 0	0 / 0	0 / 0	194 / 0	0 / 0
F	665	471 / 0	0 / 0	0 / 0	0 / 0	194 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	(LBS)	(PLF)	CSI	(LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CSI (LC)
FR-TO										
A-B	-1595 / 0	-112.4	-112.4	0.26	(1)	4.96	I-C	0 / 532	0.12	(1)
B-C	-807 / 0	-112.4	-112.4	0.34	(1)	6.25	I-D	-747 / 0	0.34	(1)
C-D	-807 / 0	-112.4	-112.4	0.34	(1)	6.25	B-I	-747 / 0	0.34	(1)
D-E	-1595 / 0	-112.4	-112.4	0.26	(1)	4.96	L-J	-35 / 0	0.00	(1)
L-A	-907 / 0	0.0	0.0	0.10	(1)	7.81	A-J	0 / 1287	0.29	(1)
F-E	-907 / 0	0.0	0.0	0.10	(1)	7.81	H-F	-35 / 0	0.00	(1)
							H-E	0 / 1287	0.29	(1)
L-K	0 / 29	-18.5	-18.5	0.03	(4)	10.00				
K-J	0 / 24	0.0	0.0	0.07	(1)	10.00				
J-B	0 / 260	0.0	0.0	0.11	(1)	10.00				
J-I	0 / 1303	-18.5	-18.5	0.28	(1)	10.00				
I-H	0 / 1303	-18.5	-18.5	0.28	(1)	10.00				
G-H	0 / 24	0.0	0.0	0.07	(1)	10.00				
H-D	0 / 260	0.0	0.0	0.11	(1)	10.00				
G-F	0 / 29	-18.5	-18.5	0.03	(4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.34/1.00 (B-C:1), BC=0.28/1.00 (I-J:1), WB=0.34/1.00 (B-I:1), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

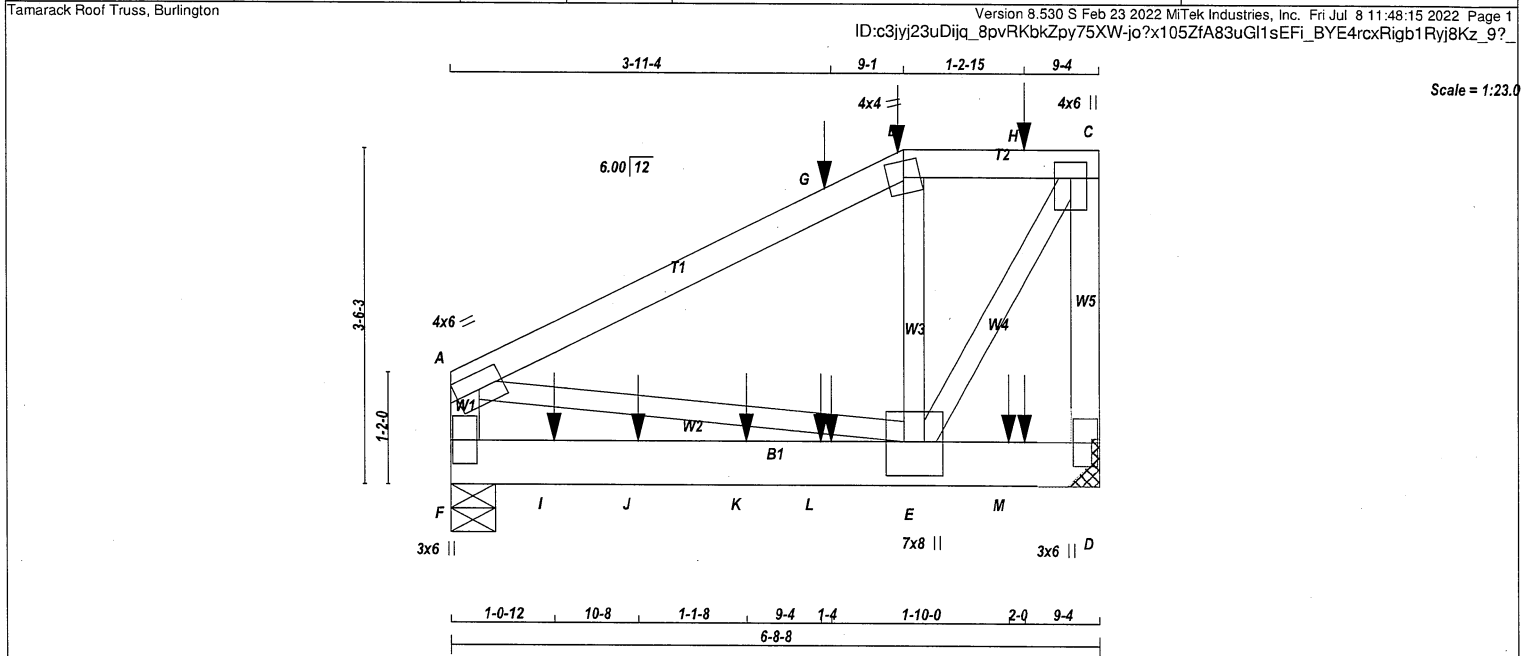
JSI GRIP= 0.61 (A) (INPUT = 0.90)
JSI METAL= 0.28 (J) (INPUT = 1.00)



Structural component only
DWG# T-2216619

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	T18	1	2	TRUSS DESC.		



TOTAL WEIGHT = 2 X 33 = 65 [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B	12	SIDE(61.0)
B - C	12	SIDE(61.0)
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	3556	0	3556	0	MECHANICAL	
F	2856	0	2856	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
D	2486	1794 / 0	0 / 0	0 / 0	0 / 0	692 / 0
F	1997	1440 / 0	0 / 0	0 / 0	0 / 0	557 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			W E B S		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A - G	-2393 / 0	-112.4 -112.4	0.28 (1)	E - B	0 / 538
G - B	-2393 / 0	-112.4 -112.4	0.28 (1)	E - C	0 / 4274
B - H	-2171 / 0	-112.4 -112.4	0.07 (1)	A - E	0 / 2170
H - C	-2171 / 0	-112.4 -112.4	0.07 (1)		
D - C	-3803 / 0	0.0 0.0	0.36 (1)		
F - A	-1685 / 0	0.0 0.0	0.09 (1)		
F - I	0 / 0	-18.5 -18.5	0.67 (1)		
I - J	0 / 0	-18.5 -18.5	0.67 (1)		
J - K	0 / 0	-18.5 -18.5	0.67 (1)		
K - L	0 / 0	-18.5 -18.5	0.67 (1)		
L - E	0 / 0	-18.5 -18.5	0.67 (1)		
E - M	0 / 0	-18.5 -18.5	0.67 (1)		
M - D	0 / 0	-18.5 -18.5	0.67 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	4-8-5	-13	-13	---	FRONT	VERT	DEAD	---	C1
B	4-8-5	-68	-68	---	FRONT	VERT	SNOW	---	C1
G	3-11-4	-19	-19	---	FRONT	VERT	TOTAL	---	C1
H	5-11-4	-18	-18	---	FRONT	VERT	TOTAL	---	C1
I	1-0-12	-650	-650	---	BACK	VERT	TOTAL	---	C1
J	1-11-4	-2	-2	---	FRONT	VERT	TOTAL	---	C1
K	3-0-12	-650	-650	---	BACK	VERT	TOTAL	---	C1
L	3-10-0	-1419	-1419	---	BACK	VERT	TOTAL	---	C1
L	3-11-4	-2	-2	---	FRONT	VERT	TOTAL	---	C1
M	5-9-4	-1022	-1022	---	BACK	VERT	TOTAL	---	C1
M	5-11-4	-4	-4	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5 PSF
	DL =	6.0 PSF
BOT CH.	LL =	0.0 PSF
	DL =	7.4 PSF
TOTAL LOAD	=	45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADD'TL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 716 (0.11")

CSI: TC=0.36/1.00 (C-D-1) , BC=0.67/1.00 (D-E-1) , WB=0.53/1.00 (C-E-1) , SSI=0.93/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.48 (C) (INPUT = 1.00)

Structural component only
DWG# T-2216620

REVIEWED

CONTINUED ON PAGE



Structural component only
DWG# T-2216620

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T18	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B	TTW-m	MT20	4.0	4.0		
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW+t	MT20	7.0	8.0	4.25	3.50
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



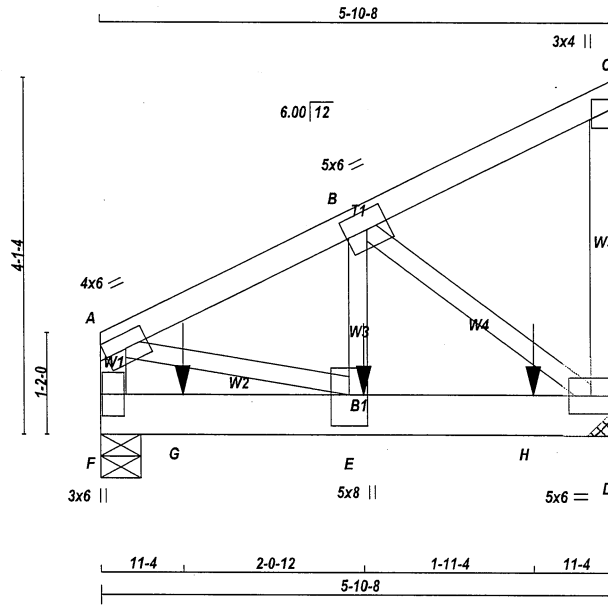
Structural component only
DWG# T-2216620

REVIEWED

JOB NAME 423665	TRUSS NAME T19	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 29 = 58 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
		DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	9	SIDE(305.2)
WEBS : (0.122"x3") SPIRAL NAILS		
B-E 1	2	SIDE(408.0)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B	TMVW-t	MT20	5.0	6.0		
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	2071	0	2071	0
D	2995	0	2995	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1447	1045 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0
D	2095	1504 / 0	0 / 0	0 / 0	0 / 0	591 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX. UNBRAC	MEMB. FORCE
	(LBS)	(PLF)	CS1 (LC)	(LBS)
FR-TO		FROM TO	LENGTH	FR-TO
F-A	-1972 / 0	0.0	0.0 0.11 (1)	7.81
A-B	-2668 / 0	-112.4	-112.4 0.09 (1)	5.49
B-C	-10 / 0	-112.4	-112.4 0.07 (1)	10.00
D-C	-143 / 0	0.0	0.0 0.02 (1)	7.81
F-G	0 / 0	-18.5	-18.5 0.03 (1)	10.00
G-E	0 / 0	-18.5	-18.5 0.03 (1)	10.00
E-H	0 / 2395	-18.5	-18.5 0.51 (1)	10.00
H-D	0 / 2395	-18.5	-18.5 0.51 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-0	-1912	-1912	---	FRONT	VERT	TOTAL	---	C1
G	11-4	-70	-70	---	FRONT	VERT	TOTAL	---	C1
H	4-11-4	-1022	-1022	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.11/1.00 (A-F:1), BC=0.51/1.00 (D-E:1), WB=0.36/1.00 (B-D:1), SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.45 (E) (INPUT = 1.00)



Structural component only
DWG# T-2216621

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T19	1	2	BAYVIEW WELLINGTON	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	5.0	8.0	4.25	2.50
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



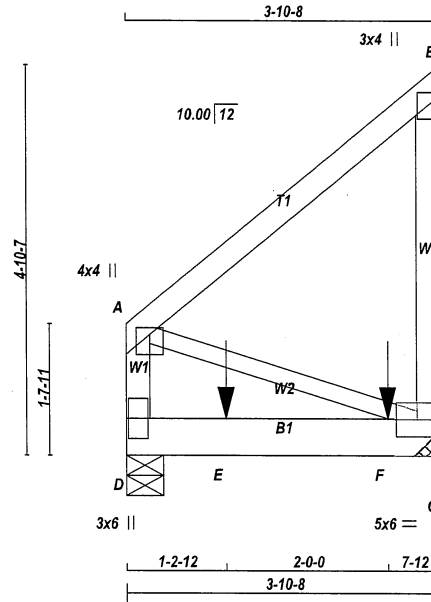
Structural component only
DWG# T-2216621

REVIEWED

REVIEWED

JOB NAME 423665	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:27.5

TOTAL WEIGHT = 2 X 22 = 43 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 1	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TMV+p	MT20	3.0	4.0	
C	BMVW1-t	MT20	5.0	6.0	
D	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	IN-SX	IN-SX
D	1045	0	1045	0	5-8
C	1327	0	1327	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	731	526 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0
C	928	668 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
D-A	-218 / 0	A-C	0 / 0
A-B	0 / 0		
C-B	-218 / 0		
D-E	0 / 0		
E-F	0 / 0		
F-C	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-2-12	-650	-650	---	FRONT	VERT	TOTAL	---	C1
F	3-2-12	-653	-653	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.16/1.00 (A-B:1), BC=0.37/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.37/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (A) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2216622

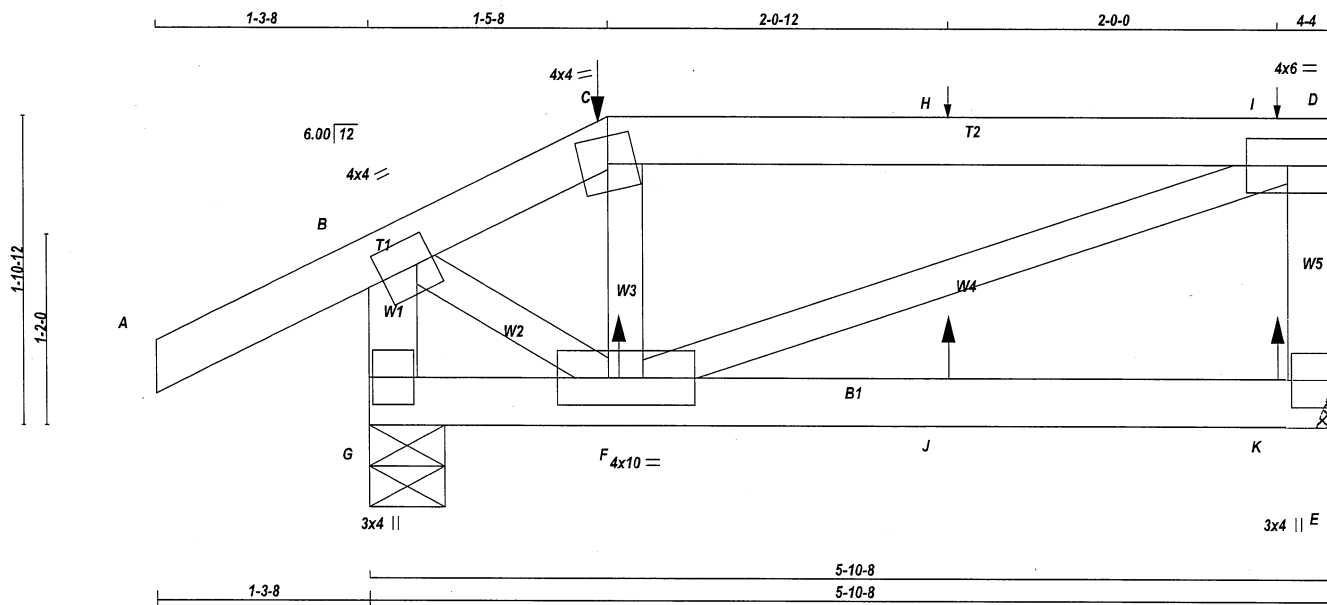
REVIEWED

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JOB NAME 423665	TRUSS NAME T21	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 23 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2		
C - D	2x4 DRY	No.2		
E - D	2x4 DRY	No.2		
G - B	2x4 DRY	No.2		
G - E	2x4 DRY	No.2		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	4.0	10.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

LOADING DESIGNER								
BEARINGS								
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD	
	GROSS REACTION		GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	344	0	344	0	0	MECHANICAL		
G	542	0	542	0	0	5-8	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	242	165 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
G	377	280 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		MAX. FACTORED		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE	VERT.	LC1	MAX	MAX	MEMB.	FORCE	MAX	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH		(LBS)	CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 34	-112.4	-112.4	0.16 (1)	10.00	F-C	-153 / 6	0.02 (1)	
B-C	-277 / 0	-112.4	-112.4	0.16 (1)	6.25	F-D	0 / 223	0.06 (1)	
C-H	-209 / 0	-112.4	-112.4	0.41 (1)	6.25	B-F	0 / 250	0.06 (1)	
H-I	-209 / 0	-112.4	-112.4	0.41 (1)	6.25				
I-D	-209 / 0	-112.4	-112.4	0.41 (1)	6.25				
E-D	-326 / 0	0.0	0.0	0.04 (1)	7.81				
G-B	-536 / 0	0.0	0.0	0.06 (1)	7.81				
G-F	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
F-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
J-K	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
K-E	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-5-8	-2	-94		FRONT	VERT	TOTAL		C1
F	1-6-4	9	12		FRONT	VERT	TOTAL		C1
H	3-6-4	1	94		FRONT	VERT	TOTAL		C1
I	5-6-4	1	73		FRONT	VERT	TOTAL		C1
J	3-6-4	9	12		FRONT	VERT	TOTAL		C1
K	5-6-4	9	12		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.41/1.00 (C-D-1), BC=0.08/1.00 (F-G-4), WB=0.06/1.00 (B-F-1), SSI=0.21/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



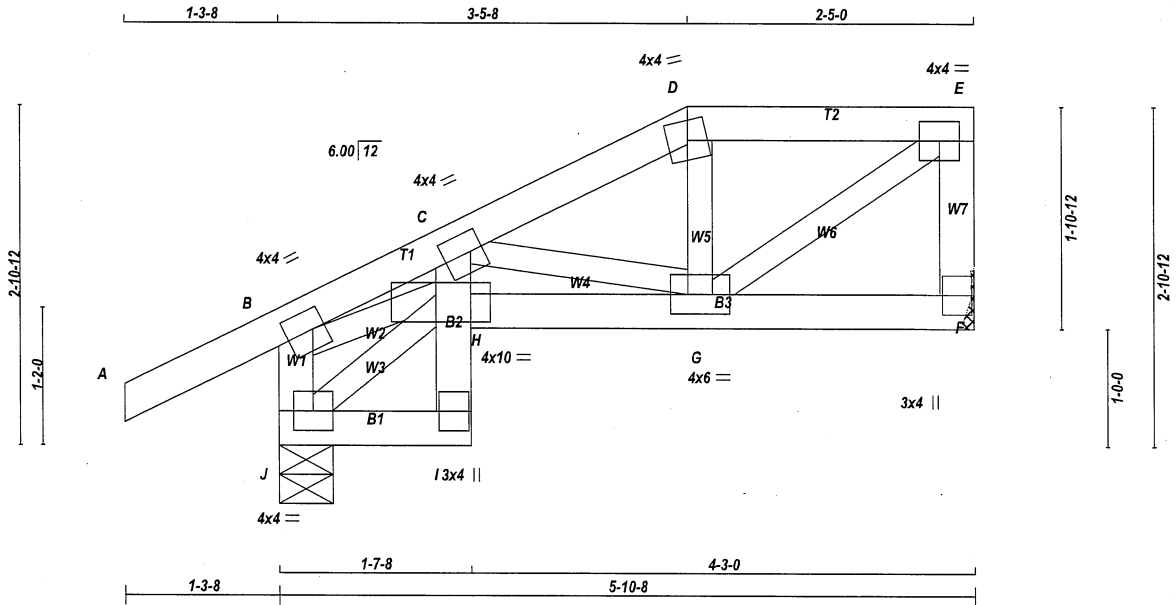
Structural component only
DWG# T-2216623

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	T22S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 26 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - E	2x4 DRY	No.2	SPF		
F - E	2x4 DRY	No.2	SPF		
J - B	2x4 DRY	No.2	SPF		
J - I	2x4 DRY	No.2	SPF		
I - C	2x4 DRY	No.2	SPF		
H - F	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMVWW-t	MT20	4.0	10.0	2.75	5.50
I	BMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
F	367	0	367	0
J	555	0	555	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	257	181 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
J	386	289 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	(PLF)	(LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE	MAX
	(LBS)							(LBS)	CSI (LC)
FR-TO		FROM	TO				FR-TO		
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	C-G	-364 / 0	0.06 (1)	
B-C	-627 / 0	-112.4	-112.4	0.15 (1)	6.25	G-D	-110 / 4	0.02 (1)	
C-D	-339 / 0	-112.4	-112.4	0.10 (1)	6.25	Q-E	0 / 365	0.08 (1)	
D-E	-296 / 0	-112.4	-112.4	0.11 (1)	6.25	J-H	-73 / 0	0.01 (1)	
F-E	-348 / 0	0.0	0.0	0.04 (1)	7.81	B-H	0 / 532	0.12 (1)	
J-B	-497 / 0	0.0	0.0	0.05 (1)	7.81				
J-I	0 / 58	-18.5	-18.5	0.02 (1)	10.00				
I-H	0 / 15	0.0	0.0	0.09 (1)	10.00				
H-C	0 / 117	0.0	0.0	0.11 (1)	10.00				
H-G	0 / 661	-18.5	-18.5	0.12 (1)	10.00				
G-F	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.15/1.00 (A-B:1), BC=0.12/1.00 (G-H:1), WB=0.12/1.00 (B-H:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

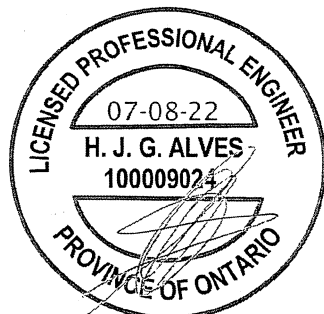
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)

JSI METAL= 0.25 (B) (INPUT = 1.00)



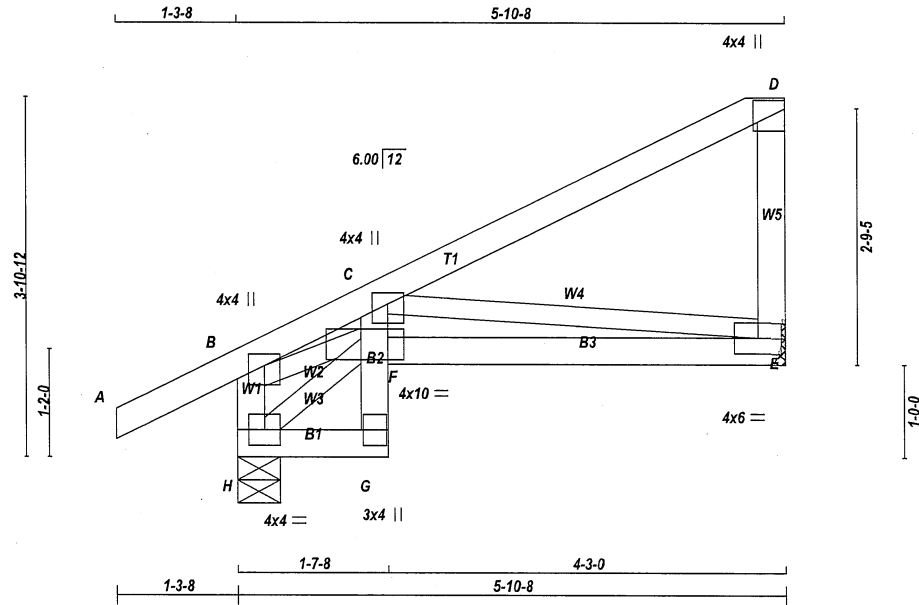
Structural component only
DWG# T-2216624

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	T23S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale: 1/2"=1'

TOTAL WEIGHT = 26 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
G - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TMV+p	MT20	4.0	4.0		Edge
E	BMVW1-t	MT20	4.0	4.0	6.0	
F	BMVW1-t	MT20	4.0	10.0	2.75	5.50
G	BMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	367	0	367	0
H	555	0	555	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	257	181 / 0	0 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
H	386	289 / 0	0 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 34	-112.4 -112.4	0.15 (1)	10.00	C-E	-964 / 0	0.29 (1)
B-C	-856 / 0	-112.4 -112.4	0.16 (1)	6.25	H-F	-101 / 0	0.01 (1)
C-D	-11 / 0	-112.4 -112.4	0.30 (1)	6.25	B-F	0 / 779	0.18 (1)
E-D	-222 / 0	0.0 0.0	0.03 (1)	7.81			
H-B	-480 / 0	0.0 0.0	0.05 (1)	7.81			
H-G	0 / 80	-18.5 -18.5	0.02 (1)	10.00			
G-F	0 / 15	0.0 0.0	0.12 (1)	10.00			
F-C	0 / 155	0.0 0.0	0.15 (1)	10.00			
F-E	0 / 956	-18.5 -18.5	0.21 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.30/1.00 (C-D:1), BC=0.21/1.00 (E-F:1),
WB=0.29/1.00 (C-E:1), SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)



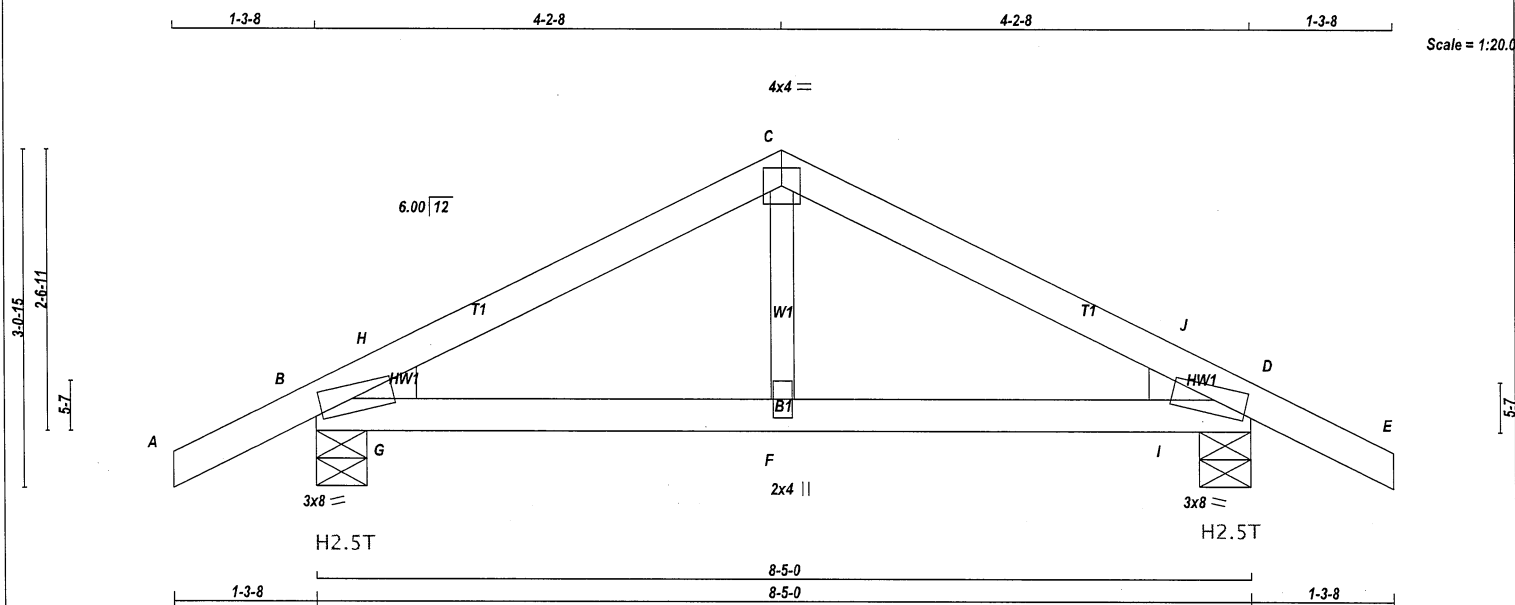
Structural component only
DWG# T-2216625

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	T24W	2	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 27 = 54 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0	1.50	3.50
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-m	MT20	3.0	8.0	1.50	3.50
F	BMW-w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	HEEL
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	UPLIFT	IN-SX
B	704	0	704	53	-276
D	704	0	704	53	-222

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 276 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 222 LBS. FACTORED UPLIFT

PROVIDE FOR 53 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	491	361 / 0	0 / 0	0 / 0	0 / -280	129 / 0	0 / 0
D	491	361 / 0	0 / 0	0 / 0	0 / -242	129 / 0	0 / 0

HORIZONTAL REACTIONS	B	D
---	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	TO	CS1 (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)
FR-TO								
A-B	0 / 28	-112.4	-112.4	0.15 (1)	10.00	F-C	-151 / 193	0.04 (1)
B-H	-645 / 225	-112.4	-112.4	0.06 (1)	6.25	G-H	-236 / 78	0.00 (1)
H-C	-633 / 271	-112.4	-112.4	0.19 (1)	6.25	I-J	-226 / 114	0.00 (1)
C-J	-633 / 290	-112.4	-112.4	0.19 (1)	6.25			
J-D	-645 / 272	-112.4	-112.4	0.06 (1)	6.25			
D-E	0 / 28	-112.4	-112.4	0.15 (1)	10.00			
B-G	-207 / 560	-18.5	-18.5	0.26 (1)	6.25			
G-F	-207 / 560	-18.5	-18.5	0.26 (1)	6.25			
F-I	-207 / 560	-18.5	-18.5	0.26 (1)	6.25			
I-D	-207 / 560	-18.5	-18.5	0.26 (1)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (15'-0") FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_pC_q BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0'-0") FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.19/1.00 (C-H:1), BC=0.26/1.00 (B-G:1), WB=0.04/1.00 (C-F:1), SSI=0.15/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

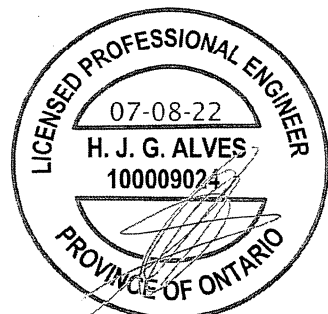
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

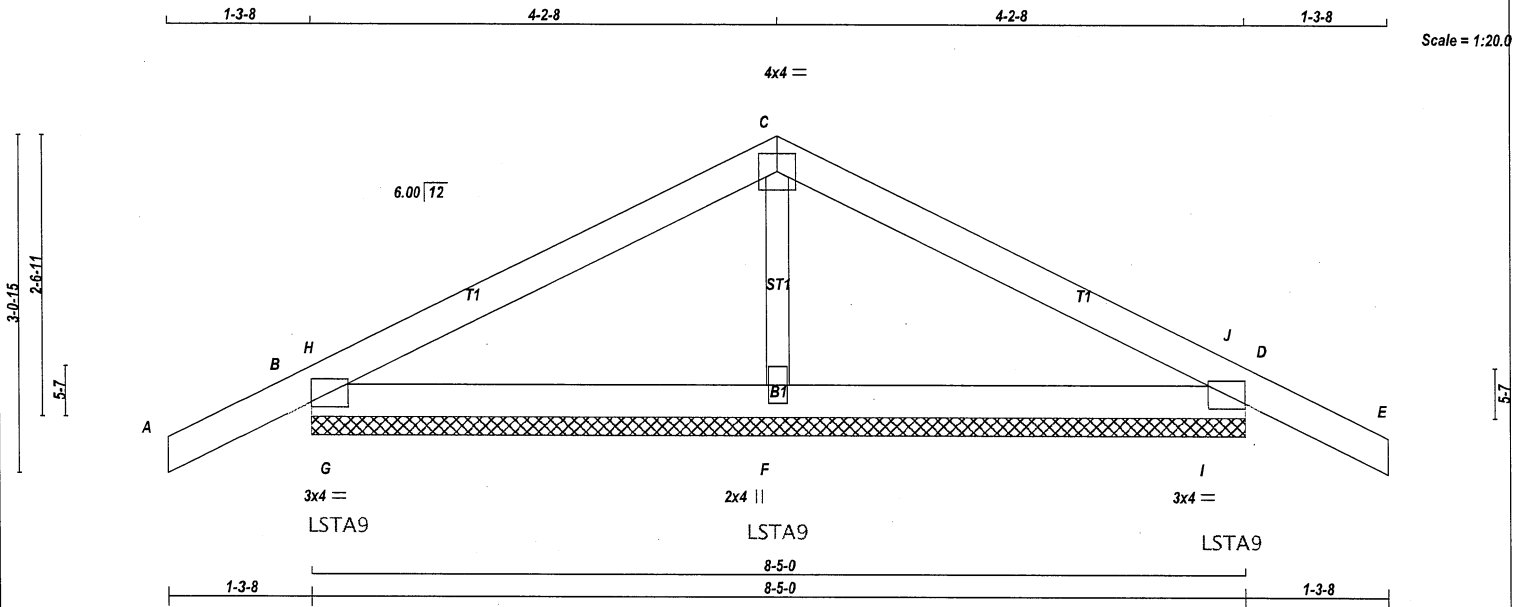


Structural component only
DWG# T-2216626

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	G24W	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 26 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS			SPF	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-i	MT20	3.0	4.0		Edge
C	TTW-p	MT20	4.0	4.0		
D	TMB1-i	MT20	3.0	4.0		Edge
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT.

PROVIDE FOR 53 LBS FACTORED HORIZONTAL REACTION AT JOINT B

HORIZONTAL REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
B ---	0/0	0/0	0/0	38/-38	0/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/28	-112.4 -112.4	0.15 (1)	10.00	F-C	-85/8	0.01 (1)
B-H	-145/8	-112.4 -112.4	0.15 (1)	6.25	G-H	-540/321	0.00 (1)
H-C	-364/139	-112.4 -112.4	0.24 (1)	6.25	I-J	-539/315	0.00 (1)
C-J	-360/121	-112.4 -112.4	0.24 (1)	6.25			
J-D	-141/0	-112.4 -112.4	0.15 (1)	6.25			
D-E	0/28	-112.4 -112.4	0.15 (1)	10.00			
B-G	-56/316	-18.5 -18.5	0.22 (1)	6.25			
G-F	-56/316	-18.5 -18.5	0.22 (1)	6.25			
F-I	-56/316	-18.5 -18.5	0.22 (1)	6.25			
I-D	-56/316	-18.5 -18.5	0.22 (1)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (15-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-H:1), BC=0.22/1.00 (B-G:1), WB=0.01/1.00 (C-F:1), SSI=0.39/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)

JSI METAL= 0.15 (B) (INPUT = 1.00)

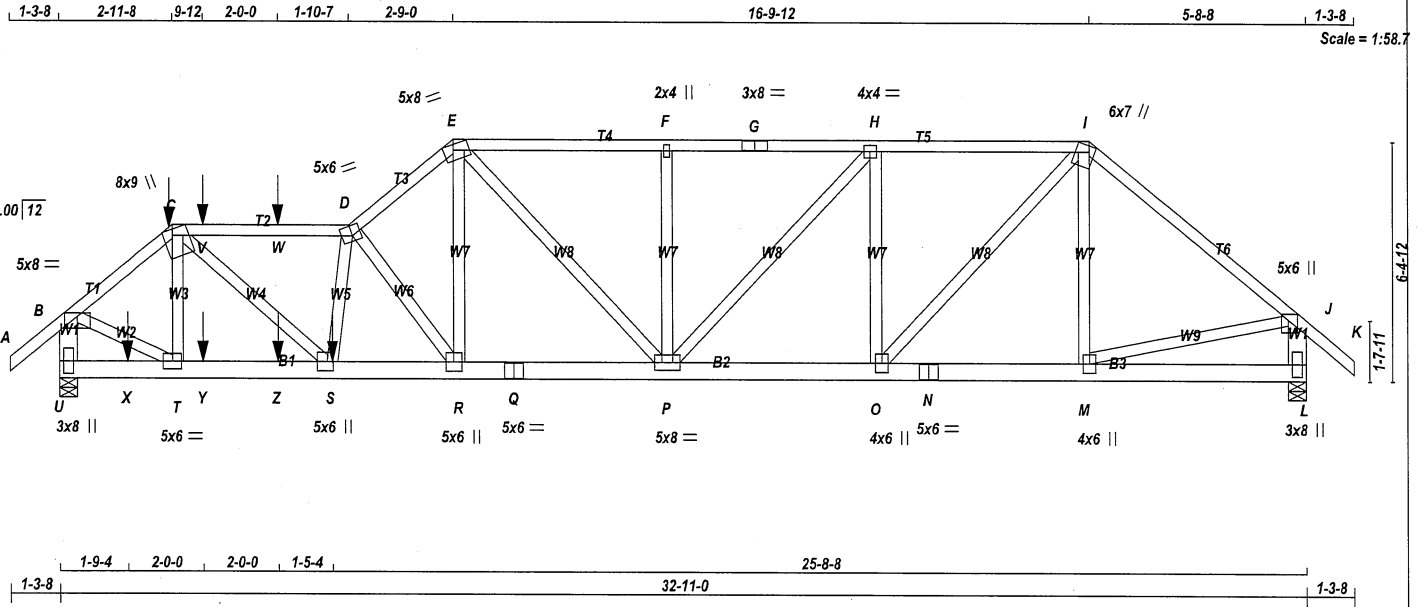


Structural component only
DWG# T-2216589

REVIEWED

JOB NAME 423670	TRUSS NAME T30	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Fri Jul 8 12:10:28 2022 Page 1
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TOTAL WEIGHT = 2 X 189 = 378 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - Q	2x6	DRY	No.2
Q - N	2x6	DRY	No.2
N - L	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-D	12	SIDE(61.0)
D-E	12	TOP
E-G	12	TOP
G-I	12	TOP
I-K	12	TOP
U-B	2	TOP
L-J	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U-Q	2	SIDE(183.1)
N-O	2	TOP
N-L	2	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	
D-S	1	SIDE(544.6)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
U 5053 0	5053 0 0	5-8	5-8
L 2993 0	2993 0 0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS				
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U 3539	2517 / 0	0 / 0	0 / 0	0 / 0	1022 / 0	0 / 0
L 2095	1497 / 0	0 / 0	0 / 0	0 / 0	599 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 50	-112.4 -112.4 0.09 (1)	10.00	T-C	-1229 / 0	0.11 (1)	
B-C	-5014 / 0	-112.4 -112.4 0.19 (1)	4.17	R-E	0 / 4014	0.35 (1)	
C-V	-7819 / 0	-112.4 -112.4 0.70 (1)	2.86	M-I	-371 / 0	0.08 (1)	
V-W	-7819 / 0	-112.4 -112.4 0.70 (1)	2.86	B-T	0 / 4125	0.36 (1)	
W-D	-7819 / 0	-112.4 -112.4 0.70 (1)	2.86	M-J	0 / 2414	0.21 (1)	
D-E	-6561 / 0	-112.4 -112.4 0.23 (1)	3.67	O-I	0 / 2411	0.21 (1)	
E-F	-4911 / 0	-112.4 -112.4 0.41 (1)	4.00	E-P	-300 / 0	0.15 (1)	
F-G	-4911 / 0	-112.4 -112.4 0.41 (1)	4.00	O-H	-1661 / 0	0.37 (1)	
G-H	-4911 / 0	-112.4 -112.4 0.41 (1)	4.00	P-F	-665 / 0	0.15 (1)	
H-I	-4008 / 0	-112.4 -112.4 0.37 (1)	4.39	P-H	0 / 1339	0.12 (1)	
I-J	-3082 / 0	-112.4 -112.4 0.48 (1)	4.78	S-D	-893 / 0	0.08 (1)	
J-K	0 / 50	-112.4 -112.4 0.09 (1)	10.00	C-S	0 / 5360	0.47 (1)	
U-B	-5060 / 0	0.0 0.0 0.19 (1)	6.47	D-R	-4813 / 0	0.62 (1)	
L-J	-2948 / 0	0.0 0.0 0.11 (1)	7.81				
U-X	0 / 0	-18.5 -18.5 0.05 (1)	10.00				
X-T	0 / 0	-18.5 -18.5 0.05 (1)	10.00				
T-Y	0 / 3789	-18.5 -18.5 0.38 (1)	10.00				
Y-Z	0 / 3789	-18.5 -18.5 0.38 (1)	10.00				
Z-S	0 / 3789	-18.5 -18.5 0.38 (1)	10.00				
S-R	0 / 7939	-18.5 -18.5 0.65 (1)	10.00				
R-Q	0 / 5117	-18.5 -18.5 0.36 (1)	10.00				
Q-P	0 / 5117	-18.5 -18.5 0.36 (1)	10.00				
P-O	0 / 4008	-18.5 -18.5 0.28 (1)	10.00				
O-N	0 / 2354	-18.5 -18.5 0.17 (1)	10.00				
N-M	0 / 2354	-18.5 -18.5 0.17 (1)	10.00				
M-L	0 / 0	-18.5 -18.5 0.04 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.	
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-159	-159	---	FRONT	VERT	SNOW	---	C1
S	7-2-8	-1957	-1957	---	FRONT	VERT	TOTAL	---	C1
V	3-9-4	-100	-100	---	FRONT	VERT	TOTAL	---	C1
W	5-3-4	-93	-93	---	FRONT	VERT	TOTAL	---	C1
X	1-9-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	3-9-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Z	5-9-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	=	32.5	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.70/1.00 (C-D-1), BC=0.65/1.00 (R-S-1), WB=0.62/1.00 (D-R-1), SSI=0.19/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (S) (INPUT = 0.90)
JSI METAL=0.75 (S) (INPUT = 1.00)



Structural component only
DWG# T-2216638

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423670	T30	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	8.0	9.0	Edge	2.75
D	TTWW-m	MT20	5.0	6.0	3.25	2.75
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	2.00	1.75
J	TMVW+p	MT20	5.0	6.0	2.00	2.25
L	BMV1+p	MT20	3.0	8.0		
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	5.0	6.0		
S	BMWW+t	MT20	5.0	6.0	3.00	2.00
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

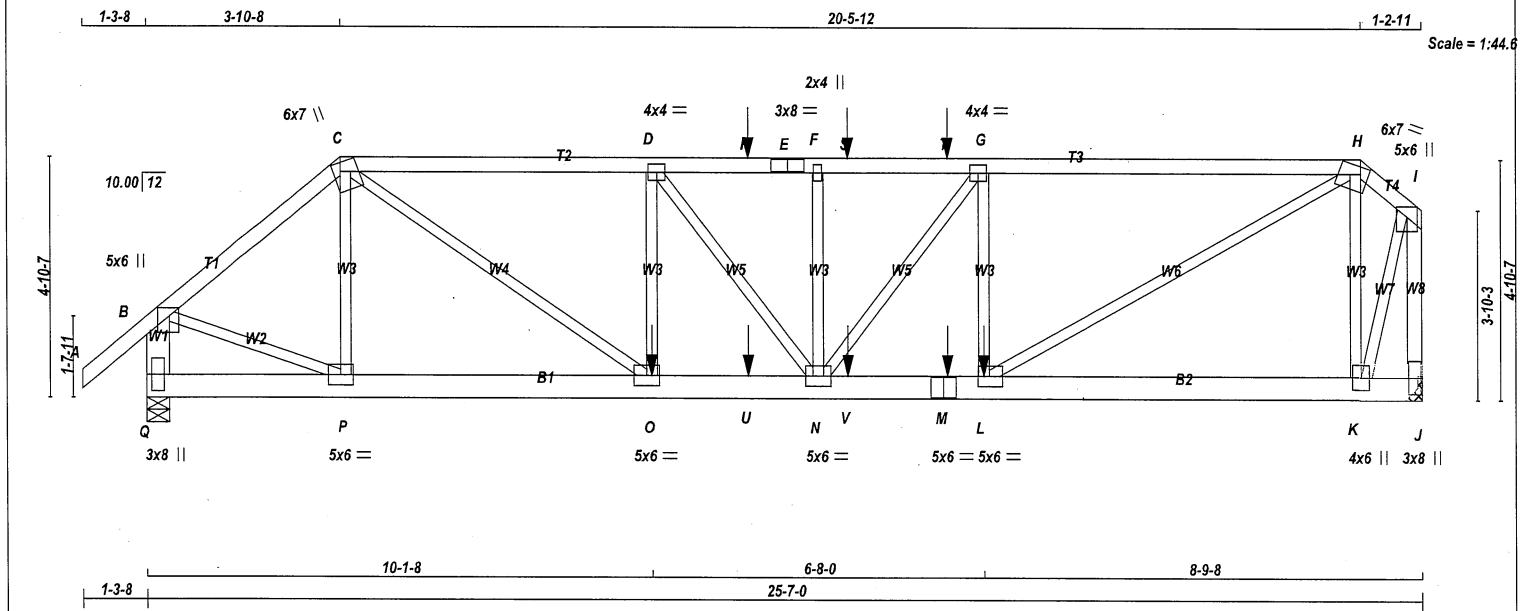


Structural component only
DWG# T-2216638

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423670	T31	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 129 = 257 lb
[M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
Q - B	2x6	DRY	No.2
J - I	2x4	DRY	No.2
Q - M	2x6	DRY	No.2
M - J	2x6	DRY	No.2

ALL WEBS	EXCEPT	2x3	DRY	No.2	SPF
SPF					

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	SIDE(0.0)
E - H	12	SIDE(0.0)
H - I	12	TOP
J - I	12	TOP
Q - B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q - M	12	SIDE(183.1)
M - J	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
D - O	6	SIDE(27.2)
G - L	6	SIDE(49.0)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED		MAXIMUM FACTORED		INPUT		REQRD			
GROSS REACTION		GROSS REACTION		BRG		BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
Q	2789	0	2789	0	0	5-8	5-8		
J	2780	0	2780	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 4-0.

UNFACTORED REACTIONS						
1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
Q	1951	1402 / 0	0 / 0	0 / 0	0 / 0	549 / 0
J	1949	1374 / 0	0 / 0	0 / 0	0 / 0	575 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	MAX. FACTORED	MEMB.	MAX. FACTORED	MAX. FACTORED	MAX. FACTORED	MAX. FACTORED
	FORCE	(PLF)	LC1	CS1 (LC)		FORCE	CS1 (LC)	FORCE	CS1 (LC)
FR-TO	(LBS)	FROM TO	UNBRAC	LENGTH	FR-TO	(LBS)	CS1 (LC)	FR-TO	(LBS)
A-B	0 / 50	-112.4 -112.4	0.09 (1)	10.00	P-C	-493 / 0	0.08 (1)		
B-C	-2739 / 0	-112.4 -112.4	0.20 (1)	5.32	C-O	0 / 2978	0.37 (1)		
C-D	-4508 / 0	-112.4 -112.4	0.48 (1)	4.09	O-D	-873 / 0	0.15 (1)		
D-R	-4588 / 0	-112.4 -112.4	0.39 (1)	4.06	D-N	0 / 134	0.02 (1)		
R-E	-4588 / 0	-112.4 -112.4	0.39 (1)	4.06	N-F	-219 / 0	0.04 (1)		
E-F	-4588 / 0	-112.4 -112.4	0.39 (1)	4.06	N-G	0 / 284	0.04 (1)		
F-S	-4588 / 0	-112.4 -112.4	0.54 (1)	3.80	L-G	-1200 / 0	0.20 (1)		
S-T	-4588 / 0	-112.4 -112.4	0.54 (1)	3.80	L-H	0 / 4226	0.52 (1)		
T-G	-4588 / 0	-112.4 -112.4	0.54 (1)	3.80	K-H	-1878 / 0	0.32 (1)		
G-H	-4418 / 0	-112.4 -112.4	0.74 (1)	3.88	B-P	0 / 2195	0.27 (1)		
H-I	-1083 / 0	-112.4 -112.4	0.02 (1)	6.25	K-I	0 / 2281	0.28 (1)		
Q-B	-2778 / 0	0.0	0.0	0.10 (1)					
J-I	-2898 / 0	0.0	0.0	0.34 (1)					

Q-P	0 / 0	-18.5	-18.5	0.04 (4)	10.00
P-O	0 / 2087	-18.5	-18.5	0.18 (1)	10.00
O-U	0 / 4508	-18.5	-18.5	0.33 (1)	10.00
U-N	0 / 4508	-18.5	-18.5	0.33 (1)	10.00
N-V	0 / 4419	-18.5	-18.5	0.34 (1)	10.00
V-M	0 / 4419	-18.5	-18.5	0.34 (1)	10.00
M-L	0 / 4419	-18.5	-18.5	0.34 (1)	10.00
L-K	0 / 781	-18.5	-18.5	0.11 (1)	10.00
K-J	0 / 0	-18.5	-18.5	0.06 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.	
L	16-9-8	-627	-627	---	BACK	VERT	TOTAL	---	C1
M	16-0-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
O	10-1-8	-561	-561	---	BACK	VERT	TOTAL	---	C1
R	12-0-12	-72	-72	---	BACK	VERT	TOTAL	---	C1
S	14-0-12	-72	-72	---	BACK	VERT	TOTAL	---	C1
T	16-0-12	-72	-72	---	BACK	VERT	TOTAL	---	C1
U	12-0-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
V	14-0-12	-14	-14	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CSI: TC=0.74/1.00 (G-H:1), BC=0.34/1.00 (L-N:1), WB=0.52/1.00 (H-L:1), SSI=0.22/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

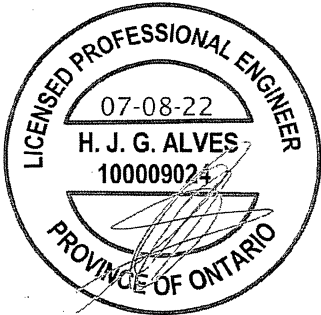
AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.43 (M) (INPUT = 1.00)



Structural component only
DWG# T-2216639

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423670	T31	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	7.0	2.00	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	7.0	1.75	2.00
I	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	8.0		
K	BMWV+t	MT20	4.0	6.0		
L	BMWV-t	MT20	5.0	6.0	2.50	2.75
M	BS-t	MT20	5.0	6.0		
N	BMWVW-t	MT20	5.0	6.0		
O	BMWV-t	MT20	5.0	6.0		
P	BMWV-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



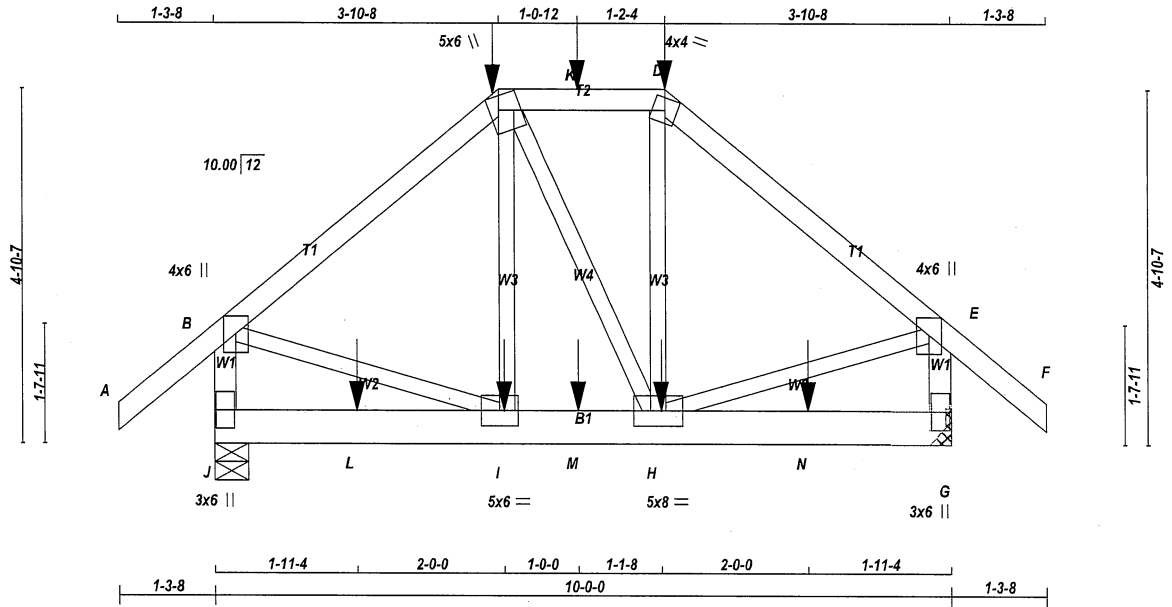
Structural component only
DWG# T-2216639

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423670	T32	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Fri Jul 8 12:10:31 2022 Page 1

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Scale = 1:30.2

TOTAL WEIGHT = 55 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2	SPF
	C - D	2x4	DRY	No.2	SPF
	D - F	2x4	DRY	No.2	SPF
	J - B	2x4	DRY	No.2	SPF
	G - E	2x4	DRY	No.2	SPF
	J - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	6.0		
H	BMVWW-t	MT20	5.0	8.0		
I	BMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP
J	1162	0	1162	0
G	1160	0	1160	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX /MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	COMBINED	812	588 / 0	0 / 0	0 / 0	0 / 0	223 / 0	0 / 0
G	COMBINED	811	588 / 0	0 / 0	0 / 0	0 / 0	223 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1	MAX. FACTORED VERT. LOAD (LBS)	LC1	MAX. FACTORED VERT. LOAD (LBS)	LC1	MAX. FACTORED VERT. LOAD (LBS)	LC1
FR-TO									
A-B	0 / 50	-112.4	-112.4	0.17	(1)	10.00	I-C	-101 / 49	0.03 (1)
B-C	-848 / 0	-112.4	-112.4	0.34	(1)	6.12	C-H	-9 / 1	0.00 (1)
C-K	-643 / 0	-112.4	-112.4	0.20	(1)	6.25	H-D	-107 / 52	0.04 (1)
K-D	-643 / 0	-112.4	-112.4	0.20	(1)	6.25	B-I	0 / 679	0.17 (1)
D-E	-843 / 0	-112.4	-112.4	0.34	(1)	6.13	H-E	0 / 676	0.17 (1)
E-F	0 / 50	-112.4	-112.4	0.17	(1)	10.00			
J-B	-1118 / 0	0.0	0.0	0.13	(1)	7.46			
G-E	-1114 / 0	0.0	0.0	0.13	(1)	7.48			
J-L	0 / 0	-18.5	-18.5	0.05	(4)	10.00			
L-I	0 / 0	-18.5	-18.5	0.05	(4)	10.00			
I-M	0 / 647	-18.5	-18.5	0.10	(1)	10.00			
M-H	0 / 647	-18.5	-18.5	0.10	(1)	10.00			
H-N	0 / 0	-18.5	-18.5	0.05	(4)	10.00			
N-G	0 / 0	-18.5	-18.5	0.05	(4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-175	-175	---	BACK	VERT	TOTAL	C1
D	6-1-8	-175	-175	---	BACK	VERT	TOTAL	C1
H	6-0-12	-14	-14	---	BACK	VERT	TOTAL	C1
I	3-11-4	-14	-14	---	BACK	VERT	TOTAL	C1
K	4-11-4	-72	-72	---	BACK	VERT	TOTAL	C1
L	1-11-4	-14	-14	---	BACK	VERT	TOTAL	C1
M	4-11-4	-14	-14	---	BACK	VERT	TOTAL	C1
N	8-0-12	-14	-14	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.34/1.00 (B-C:1), BC=0.10/1.00 (H-I:1), WB=0.17/1.00 (B-I:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (B) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

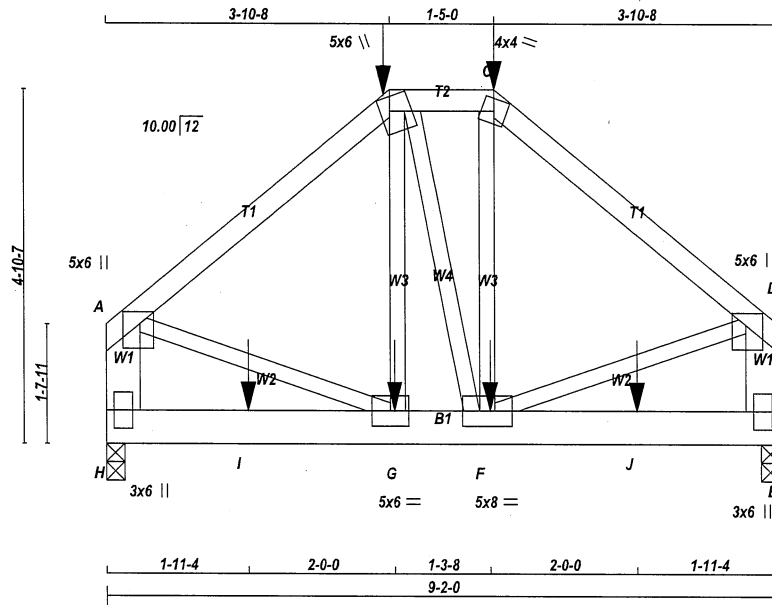


Structural component only
DWG# T-2216640

REVIEWED

JOB NAME 423670	TRUSS NAME T33	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:30.2

TOTAL WEIGHT = 50 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x6	DRY	No.2
H - A	2x6	DRY	No.2
H - E	2x6	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	5.0	6.0	2.00	2.25
E	BMV1+p	MT20	3.0	6.0		
F	BMVWW-t	MT20	5.0	8.0		
G	BMVWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	UPLIFT	IN-SX
H	889	0	0	3-0
E	889	0	0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE
H	623	441 / 0	0 / 0	0 / 0
E	623	441 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO		FR-TO		
A-B	-716 / 0	-112.4	-112.4	0.34 (1)	G-B	-80 / 37	0.03 (1)
B-C	-544 / 0	-112.4	-112.4	0.05 (1)	B-F	-12 / 5	0.00 (1)
C-D	-713 / 0	-112.4	-112.4	0.34 (1)	F-C	-94 / 43	0.03 (1)
H-A	-847 / 0	0.0	0.0	0.06 (1)	A-G	0 / 574	0.14 (1)
E-D	-843 / 0	0.0	0.0	0.06 (1)	F-D	0 / 571	0.14 (1)
H-I	0 / 0	-18.5	-18.5	0.05 (4)			
I-G	0 / 0	-18.5	-18.5	0.05 (4)			
G-F	0 / 547	-18.5	-18.5	0.08 (1)			
F-J	0 / 0	-18.5	-18.5	0.05 (4)			
J-E	0 / 0	-18.5	-18.5	0.05 (4)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-10-8	-175	-175	---	FRONT	VERT	TOTAL	---	C1
C	5-3-8	-175	-175	---	FRONT	VERT	TOTAL	---	C1
F	5-2-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
G	3-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
I	1-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
J	7-2-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.34/1.00 (A-B:1), BC=0.08/1.00 (F-G:1), WB=0.14/1.00 (A-G:1), SSI=0.14/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (A) (INPUT = 0.90)
JSI METAL= 0.14 (A) (INPUT = 1.00)

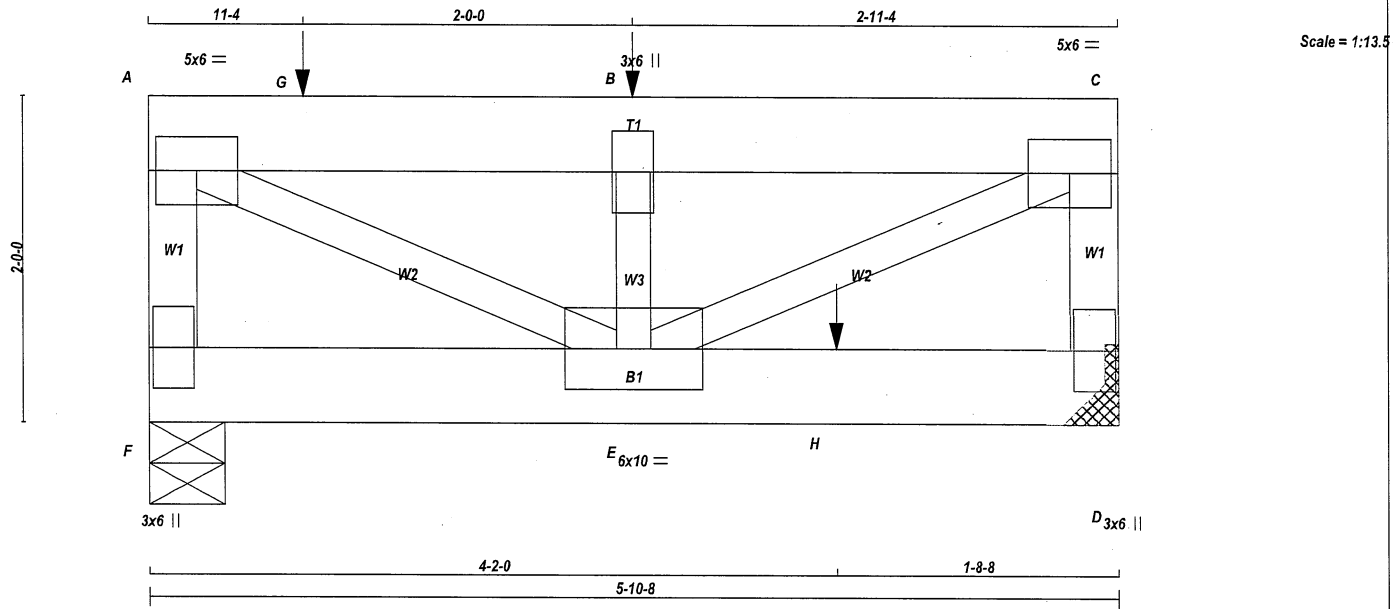


Structural component only
DWG# T-2216641

REVIEWED

JOB NAME 423670	TRUSS NAME T34	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - A	2x4	DRY	No.2
A - C	2x6	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A 1	12	TOP
C - D 1	12	TOP
A - C 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE (14.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	5.0	6.0	
B	TMW+w	MT20	3.0	6.0	
C	TMVW-t	MT20	5.0	6.0	
D	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
F	1850	0	1850	0	5-8
D	2808	0	2808	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
F	1302
D	1971

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
F-A	-1918 / 0	A-E	0 / 3225
A-G	-2856 / 0	E-B	-1013 / 0
G-B	-2856 / 0	B-C	0 / 3225
B-C	-2856 / 0	C-D	0 / 3225
D-C	-1750 / 0		
F-E	0 / 0		
E-H	0 / 0		
H-D	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-11-4	-546	-546	---	TOP	VERT	TOTAL	---	C1
G	11-4	-170	-170	---	TOP	VERT	TOTAL	---	C1
H	4-2-0	-1934	-1934	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.14/1.00 (A-B:1), BC=0.65/1.00 (D-E:1), WB=0.40/1.00 (A-E:1), SSI=0.59/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)



Structural component only
DWG# T-2216642

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423670	T34	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW-t	MT20	6.0	10.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER

07-08-22

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only
DWG# T-2216642

REVIEWED

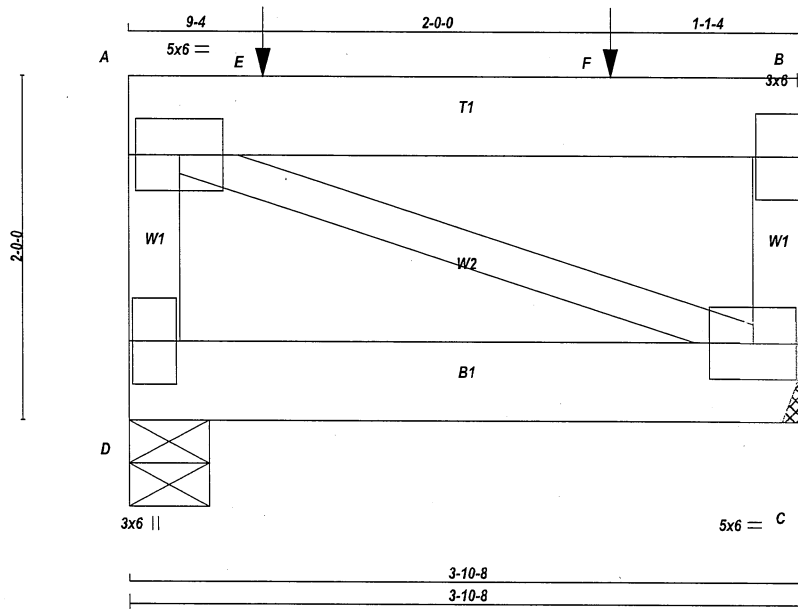


Structural component only

DWG# T-2216642

REVIEWED

JOB NAME 423670	TRUSS NAME T35	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 12:10:34 2022 Page 1 ID:c3jyj23uDijq_8pvRKbkZpy75XW-0PP83sITjbr312cNoyntZV4vcmtfQqhxP4eDz_8g3			



Scale = 1:12.8

TOTAL WEIGHT = 2 X 19 = 38 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	SPF
D - A	2x4	No.2	SPF
A - B	2x6	No.2	SPF
C - B	2x4	No.2	SPF
D - C	2x6	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 1 12	TOP	
B-C 1 12	TOP	
A-B 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2 12	SIDE(14.0)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMV+p	MT20	3.0	6.0		
C	BMVW1-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	727	0	727	0	5-8
C	908	0	908	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
D	513
C	642

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE	MEMB.	FORCE
FR-TO	MAX. FACTORED	FR-TO	MAX. FACTORED
	(LBS)		(LBS)
D-A	-642 / 0	A-C	0 / 0
A-E	0 / 0		
E-F	0 / 0		
F-B	0 / 0		
C-B	-824 / 0		
D-C	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	9-4	-175	-175	---	TOP	VERT	TOTAL	---	C1
F	2-9-4	-546	-546	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = **24.0** IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.31/1.00 (A-B:1), BC=0.05/1.00 (C-D:4), WB=0.00/1.00 (A-C:1), SSI=0.27/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (C) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

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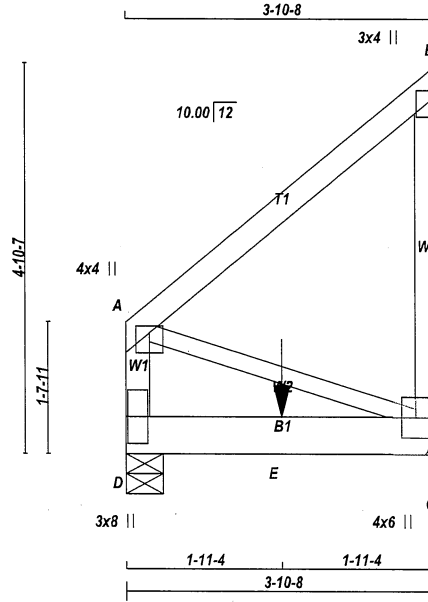
Structural component only
DWG# T-2216643



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423670	T36	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 12:10:35 2022 Page 1
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Scale = 1:27.5

TOTAL WEIGHT = 2 X 22 = 43 lb

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. D - A 2x4 DRY No.2 SPF A - B 2x4 DRY No.2 SPF C - B 2x4 DRY No.2 SPF D - C 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS D-A 1 12 TOP A-B 1 12 TOP B-C 1 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS D-C 2 12 SIDE(0.0) WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP. PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TMVW+p MT20 4.0 4.0 1.00 2.00 B TMV+p MT20 3.0 4.0 C BMVW1+p MT20 4.0 6.0 D BMV1+p MT20 3.0 8.0				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX D 825 0 825 0 0 5-8 5-8 C 824 0 824 0 0 MECHANICAL A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0. UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL D 576 420 / 0 0 / 0 0 / 0 156 / 0 0 / 0 C 576 420 / 0 0 / 0 0 / 0 156 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FACTORED WEBS MEMB. FORCE VERT. LOAD LC1 MAX. MAX. MEMB. MAX. FORCE MAX. (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) FR-TO D-A -218 / 0 0.0 0.0 0.01 (1) 7.81 A-C 0 / 0 0.00 (1) A-B 0 / 0 -112.4 -112.4 0.16 (1) 10.00 C-B -218 / 0 0.0 0.0 0.04 (1) 7.81 D-E 0 / 0 -18.5 -18.5 0.42 (1) 10.00 E-C 0 / 0 -18.5 -18.5 0.42 (1) 10.00 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. E 1-11-4 -796 -796 --- FRONT VERT TOTAL --- C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.02") ALLOWABLE DEFL.(TL) = L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/999 (0.04") CSI: TC=0.16/1.00 (A-B:1) , BC=0.42/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.20/1.00 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (Psi) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.08 (A) (INPUT = 0.90) JSI METAL= 0.04 (B) (INPUT = 1.00)			
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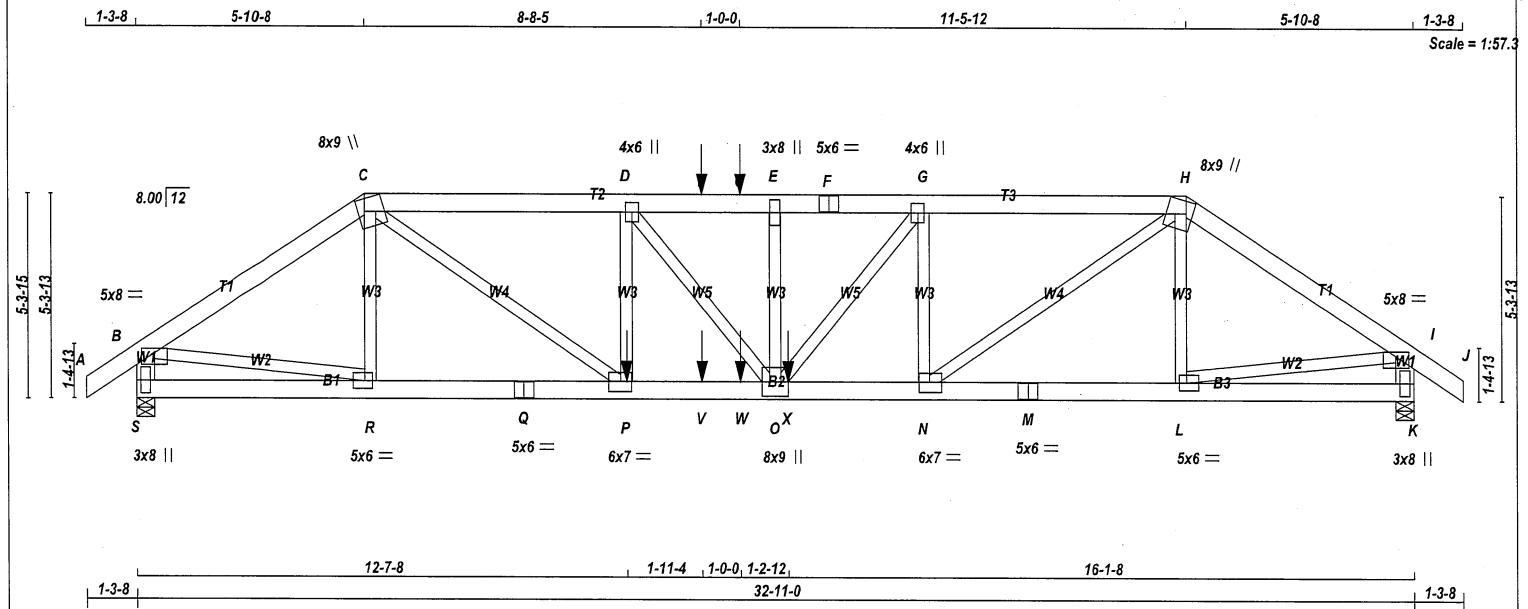
Structural component only
DWG# T-2216644

REVIEWED
CONTINUED ON PAGE 2

JOB NAME 423673	TRUSS NAME T40	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 199 = 398 lb

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY No.2	SPF
C - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - J	2x6	DRY No.2	SPF
S - B	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF
S - Q	2x6	DRY No.2	SPF
Q - M	2x6	DRY No.2	SPF
M - K	2x6	DRY No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2	12	TOP
C-F 2	12	SIDE(0.0)
F-H 2	12	TOP
H-J 2	12	TOP
S-B 2	12	TOP
K-I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-Q 2	12	TOP
Q-M 2	12	SIDE(183.1)
M-K 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
D-P 1	6	SIDE(365.5)
E-O 1	5	SIDE(427.6)
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
S	5033	0	5033	0	0	5-8	5-8		
K	4547	0	4547	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	3518	2539 / 0	0 / 0	0 / 0	0 / 0	979 / 0	0 / 0
K	3180	2288 / 0	0 / 0	0 / 0	0 / 0	892 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 44	-112.4 -112.4	0.04 (1)	10.00	R-C	-645 / 0	0.09 (1)
B-C	-6615 / 0	-112.4 -112.4	0.27 (1)	4.43	C-P	0 / 6250	0.55 (1)
C-D	-10556 / 0	-112.4 -112.4	0.40 (1)	3.52	P-D	-1542 / 0	0.21 (1)
D-T	-11077 / 0	-112.4 -112.4	0.34 (1)	3.46	D-O	0 / 842	0.07 (1)
T-U	-11077 / 0	-112.4 -112.4	0.34 (1)	3.46	O-E	-502 / 0	0.07 (1)
U-E	-11077 / 0	-112.4 -112.4	0.34 (1)	3.46	O-G	0 / 2818	0.25 (1)
E-F	-11077 / 0	-112.4 -112.4	0.32 (1)	3.48	N-G	-2923 / 0	0.40 (1)
F-G	-11077 / 0	-112.4 -112.4	0.32 (1)	3.48	N-H	0 / 5482	0.48 (1)
G-H	-9331 / 0	-112.4 -112.4	0.36 (1)	3.75	L-H	-604 / 0	0.08 (1)
H-I	-5892 / 0	-112.4 -112.4	0.25 (1)	4.66	L-I	0 / 4947	0.44 (1)
I-J	0 / 44	-112.4 -112.4	0.04 (1)	10.00	B-R	0 / 5555	0.49 (1)
S-B	-4994 / 0	0.0	0.0	18 (1)	6.51		
K-I	-4500 / 0	0.0	0.0	0.16 (1)	6.78		
S-R	0 / 0	-18.5	-18.5	0.05 (4)	10.00		
R-Q	0 / 5475	-18.5	-18.5	0.42 (1)	10.00		
Q-P	0 / 5475	-18.5	-18.5	0.42 (1)	10.00		
P-V	0 / 10556	-18.5	-18.5	0.75 (1)	10.00		
V-W	0 / 10556	-18.5	-18.5	0.75 (1)	10.00		
W-O	0 / 10556	-18.5	-18.5	0.75 (1)	10.00		
O-X	0 / 9332	-18.5	-18.5	0.82 (1)	10.00		
X-N	0 / 9332	-18.5	-18.5	0.82 (1)	10.00		
N-M	0 / 4875	-18.5	-18.5	0.34 (1)	10.00		
M-L	0 / 4875	-18.5	-18.5	0.34 (1)	10.00		
L-K	0 / 0	-18.5	-18.5	0.05 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	12-7-8	-1465	-1465		---	BACK	VERT	TOTAL	---
T	14-6-12	-149	-149		---	BACK	VERT	TOTAL	---
U	15-6-12	-149	-149		---	BACK	VERT	TOTAL	---
V	14-6-12	-29	-29		---	BACK	VERT	TOTAL	---
W	15-6-12	-29	-29		---	BACK	VERT	TOTAL	---
X	16-9-8	-1640	-1640		---	BACK	VERT	TOTAL	---

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/999 (0.35")
CSI: TC=0.40/1.00 (C-D:1), BC=0.82/1.00 (N-O:1), WB=0.55/1.00 (C-P:1), SSI=0.73/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)

JSI METAL= 0.62 (R) (INPUT = 1.00)



Structural component only
DWG# T-2216656

REVIEWED

CONTINUED ON PAGE 2

JOB NAME 423673	TRUSS NAME T40	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	4.00
C	TTWW+m	MT20	8.0	9.0	3.75	2.00
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.75	2.00
I	TMVW-p	MT20	5.0	8.0	1.50	4.00
K	BMV1+p	MT20	3.0	8.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	6.0	7.0	3.25	3.25
O	BMWWW+t	MT20	8.0	9.0		
P	BMWW-t	MT20	6.0	7.0	3.25	3.25
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.25
S	BMV1+p	MT20	3.0	8.0		

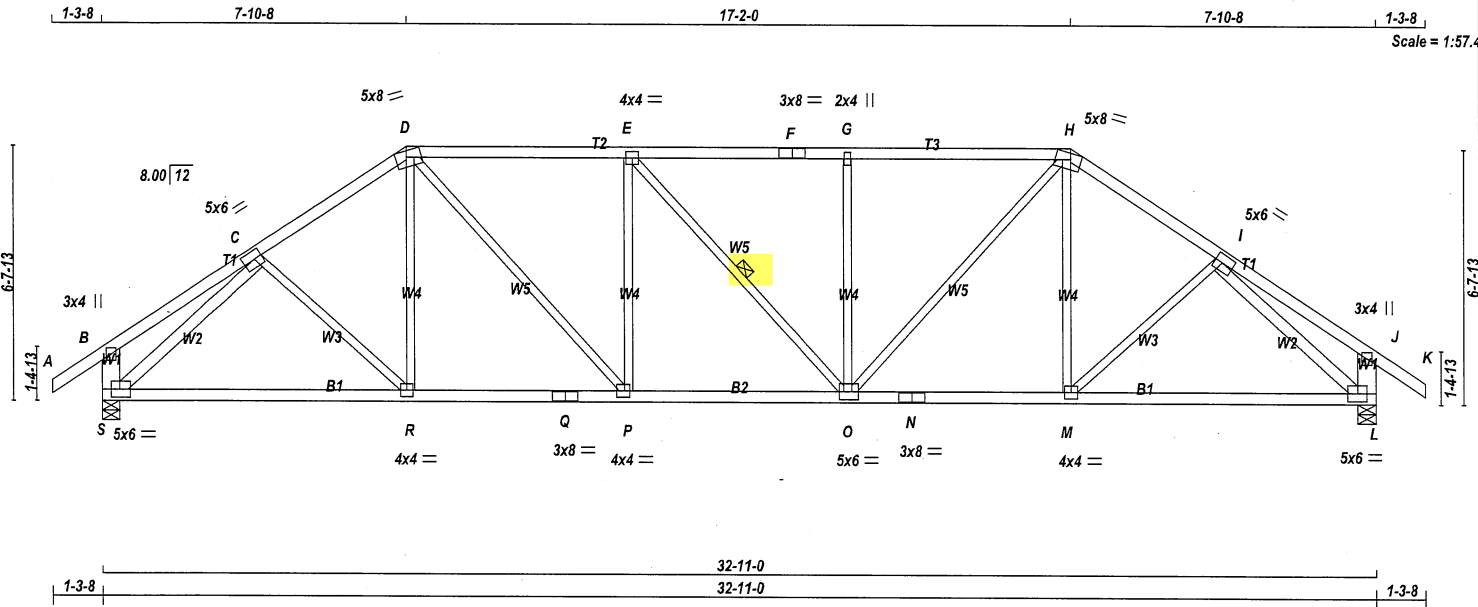
NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2216656

REVIEWED



TOTAL WEIGHT = 146 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2

ALL WEBS EXCEPT	SIZE	LUMBER	DESCR.
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
S	2309	0	2309	0	5-8	5-8
L	2309	0	2309	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	MAX / MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
		1ST LCASE	SNOW	LIVE	PERM. LIVE			
S	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	0 / 0
L	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.25
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMVWW-t	MT20	5.0	6.0	2.50	1.50
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0	2.50	2.75

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	C H O R D S		MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	W E B S	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
	FR-TO	FROM TO						
A-B	0 / 43	-112.4 -112.4 0.15 (1)	10.00	C-R	0 / 62	0.02 (4)		
B-C	0 / 27	-112.4 -112.4 0.26 (1)	10.00	R-D	0 / 143	0.04 (4)		
C-D	-2494 / 0	-112.4 -112.4 0.37 (1)	4.01	D-P	0 / 1065	0.24 (1)		
D-E	-2767 / 0	-112.4 -112.4 0.71 (1)	3.42	P-E	-695 / 0	0.51 (1)		
E-F	-2765 / 0	-112.4 -112.4 0.71 (1)	3.43	E-O	-2 / 0	0.00 (1)		
F-G	-2765 / 0	-112.4 -112.4 0.71 (1)	3.43	O-G	-694 / 0	0.51 (1)		
G-H	-2765 / 0	-112.4 -112.4 0.71 (1)	3.44	O-H	0 / 1061	0.24 (1)		
H-I	-2494 / 0	-112.4 -112.4 0.37 (1)	4.01	M-H	0 / 145	0.04 (4)		
I-J	0 / 27	-112.4 -112.4 0.26 (1)	10.00	M-I	0 / 61	0.02 (4)		
J-K	0 / 43	-112.4 -112.4 0.15 (1)	10.00	S-C	-2802 / 0	0.90 (1)		
S-B	-326 / 0	0.0 0.0 0.02 (1)	7.81	I-L	-2803 / 0	0.90 (1)		
L-J	-327 / 0	0.0 0.0 0.02 (1)	7.81					
S-R	0 / 2040	-18.5 -18.5 0.47 (1)	10.00					
R-Q	0 / 2051	-18.5 -18.5 0.47 (1)	10.00					
Q-P	0 / 2051	-18.5 -18.5 0.47 (1)	10.00					
P-O	0 / 2767	-18.5 -18.5 0.51 (1)	10.00					
O-N	0 / 2052	-18.5 -18.5 0.47 (1)	10.00					
N-M	0 / 2052	-18.5 -18.5 0.47 (1)	10.00					
M-L	0 / 2041	-18.5 -18.5 0.47 (1)	10.00					

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.71/1.00 (D-E:1), BC=0.51/1.00 (O-P:1), WB=0.90/1.00 (I-L:1), SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 798

1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

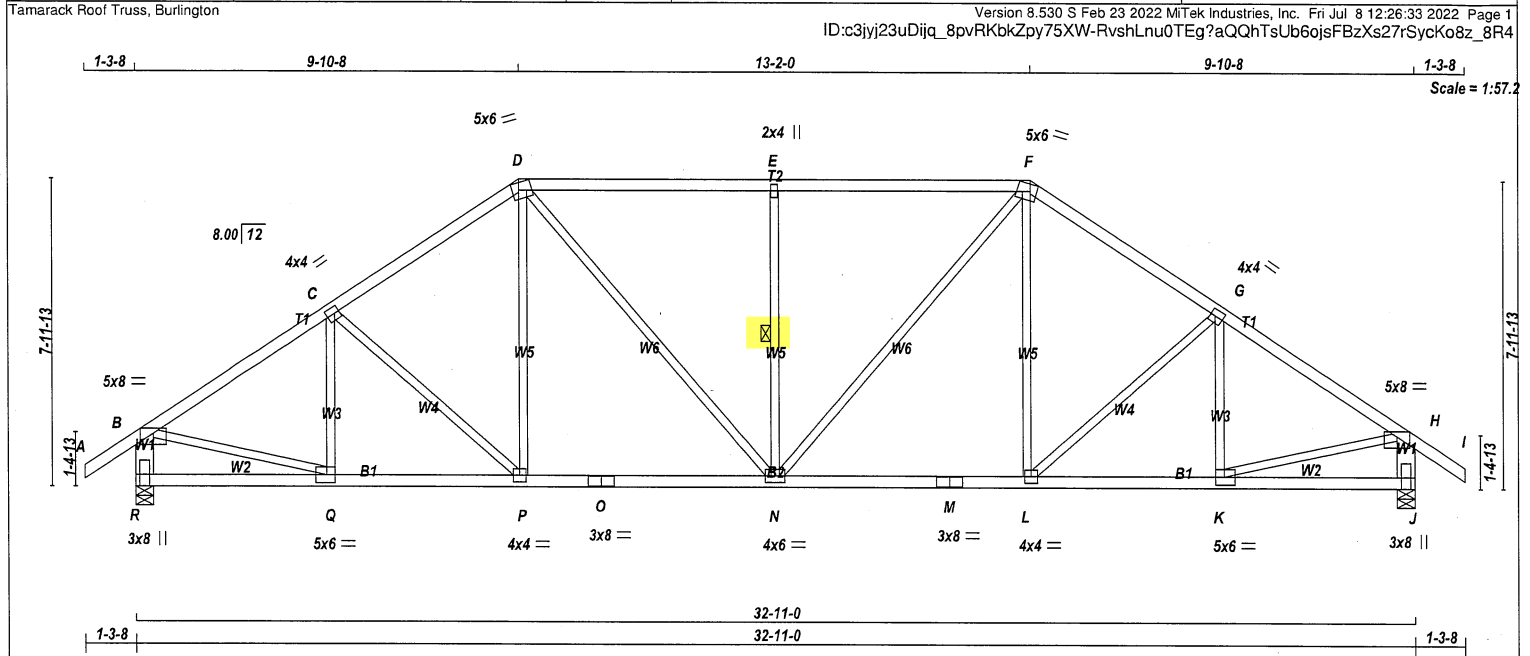
JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.63 (I) (INPUT = 1.00)



Structural component only
DWG# T-2216657

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423673	T42	1	1	TRUSS DESC.		



TOTAL WEIGHT = 146 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
R - B	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	8.0	Edge	
K	BMVW-t	MT20	5.0	6.0	2.50	2.50
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-w	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2216658

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
R	2309	0	2309	0
J	2309	0	2309	0

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN.	COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
R	1616	1158 / 0	0 / 0	0 / 0	458 / 0
J	1616	1158 / 0	0 / 0	0 / 0	458 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1	MAX LC2	MAX UNBRAC LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX LC3
MEMB.						MEMB.		
FR-TO						FR-TO		
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	Q-C	-371 / 0	0.12 (1)
B-C	-2546 / 0	-112.4	-112.4	0.45 (1)	3.91	C-P	-276 / 0	0.22 (1)
C-D	-2376 / 0	-112.4	-112.4	0.43 (1)	4.05	P-D	0 / 295	0.07 (1)
D-E	-2390 / 0	-112.4	-112.4	0.73 (1)	3.52	D-N	0 / 679	0.15 (1)
E-F	-2390 / 0	-112.4	-112.4	0.73 (1)	3.52	N-E	-909 / 0	0.37 (1)
F-G	-2376 / 0	-112.4	-112.4	0.43 (1)	4.05	N-F	0 / 679	0.15 (1)
G-H	-2548 / 0	-112.4	-112.4	0.45 (1)	3.91	L-F	0 / 296	0.07 (1)
H-I	0 / 43	-112.4	-112.4	0.15 (1)	10.00	L-G	-276 / 0	0.22 (1)
R-B	-2267 / 0	0.0	0.0	0.15 (1)	6.81	K-G	-370 / 0	0.12 (1)
J-H	-2267 / 0	0.0	0.0	0.15 (1)	6.81	B-Q	0 / 2197	0.49 (1)
						K-H	0 / 2198	0.49 (1)
R-Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
Q-P	0 / 2148	-18.5	-18.5	0.42 (1)	10.00			
P-O	0 / 1947	-18.5	-18.5	0.39 (1)	10.00			
O-N	0 / 1947	-18.5	-18.5	0.39 (1)	10.00			
N-M	0 / 1948	-18.5	-18.5	0.39 (1)	10.00			
M-L	0 / 1948	-18.5	-18.5	0.39 (1)	10.00			
L-K	0 / 2149	-18.5	-18.5	0.42 (1)	10.00			
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 32.5 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.73/1.00 (D-E:1) , BC=0.42/1.00 (K-L:1) , WB=0.49/1.00 (H-K:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Q) (INPUT = 0.90)
JSI METAL= 0.62 (Q) (INPUT = 1.00)

REVIEWED

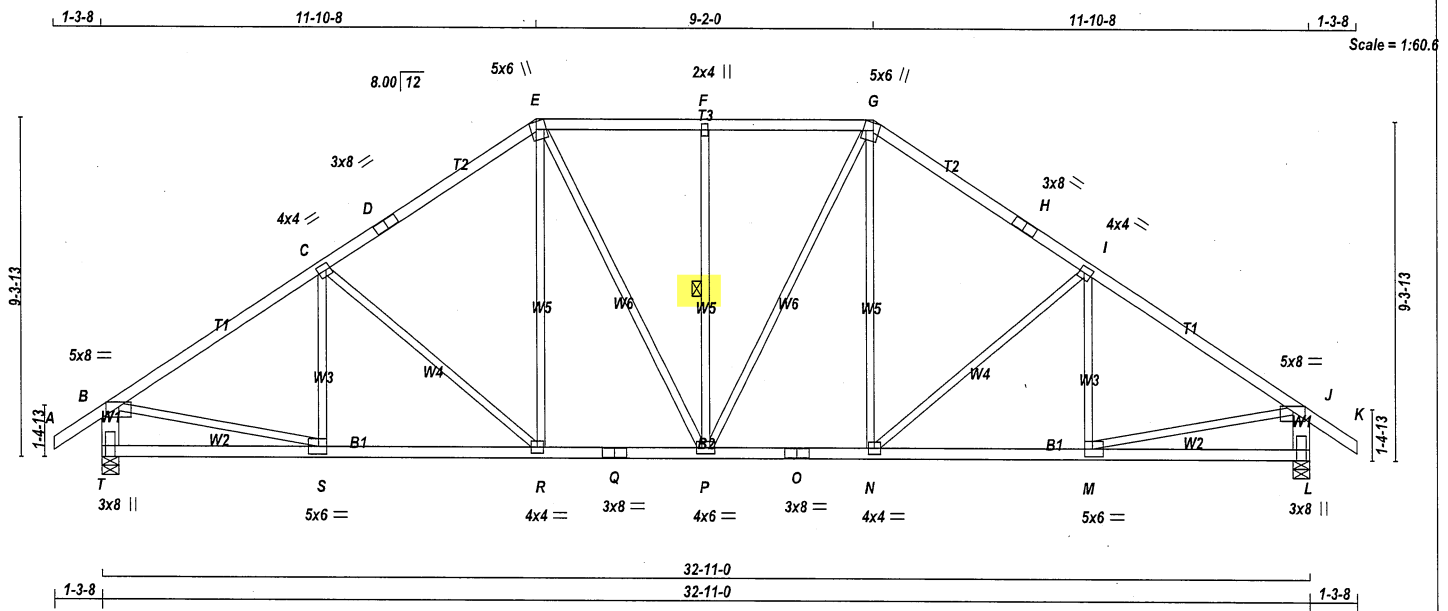


REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423673	T43	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 155 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
T - Q	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DESCR.	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	Edge	1.75
H	TS-t	MT20	3.0	8.0		
I	TMVW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	8.0	Edge	
M	BMVW-t	MT20	5.0	6.0	2.50	2.50
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-wt	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMVW-t	MT20	4.0	4.0		
S	BMVW-t	MT20	5.0	6.0	2.50	2.50
T	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
T	2309	0	2309	0	5-8
L	2309	0	2309	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
T	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	
L	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B-C	-2581 / 0	-112.4	-112.4	0.68 (1)	3.60
C-D	-2230 / 0	-112.4	-112.4	0.62 (1)	3.89
D-E	-2230 / 0	-112.4	-112.4	0.62 (1)	3.89
E-F	-1997 / 0	-112.4	-112.4	0.34 (1)	4.44
F-G	-1997 / 0	-112.4	-112.4	0.34 (1)	4.44
G-H	-2230 / 0	-112.4	-112.4	0.62 (1)	3.89
H-I	-2230 / 0	-112.4	-112.4	0.62 (1)	3.89
I-J	-2582 / 0	-112.4	-112.4	0.68 (1)	3.60
J-K	0 / 43	-112.4	-112.4	0.15 (1)	10.00
T-B	-2262 / 0	0.0	0.0	0.15 (1)	6.81
L-J	-2262 / 0	0.0	0.0	0.15 (1)	6.81
T-S	0 / 0	-18.5	-18.5	0.16 (4)	10.00
S-R	0 / 2184	-18.5	-18.5	0.42 (1)	10.00
R-Q	0 / 1820	-18.5	-18.5	0.35 (1)	10.00
Q-P	0 / 1820	-18.5	-18.5	0.35 (1)	10.00
P-O	0 / 1820	-18.5	-18.5	0.35 (1)	10.00
O-N	0 / 1820	-18.5	-18.5	0.35 (1)	10.00
N-M	0 / 2185	-18.5	-18.5	0.42 (1)	10.00
M-L	0 / 0	-18.5	-18.5	0.16 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.68/1.00 (I-J:1), BC=0.42/1.00 (M-N:1), WB=0.62/1.00 (I-N:1), SSI=0.27/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (S) (INPUT = 0.90)
JSI METAL= 0.55 (Q) (INPUT = 1.00)

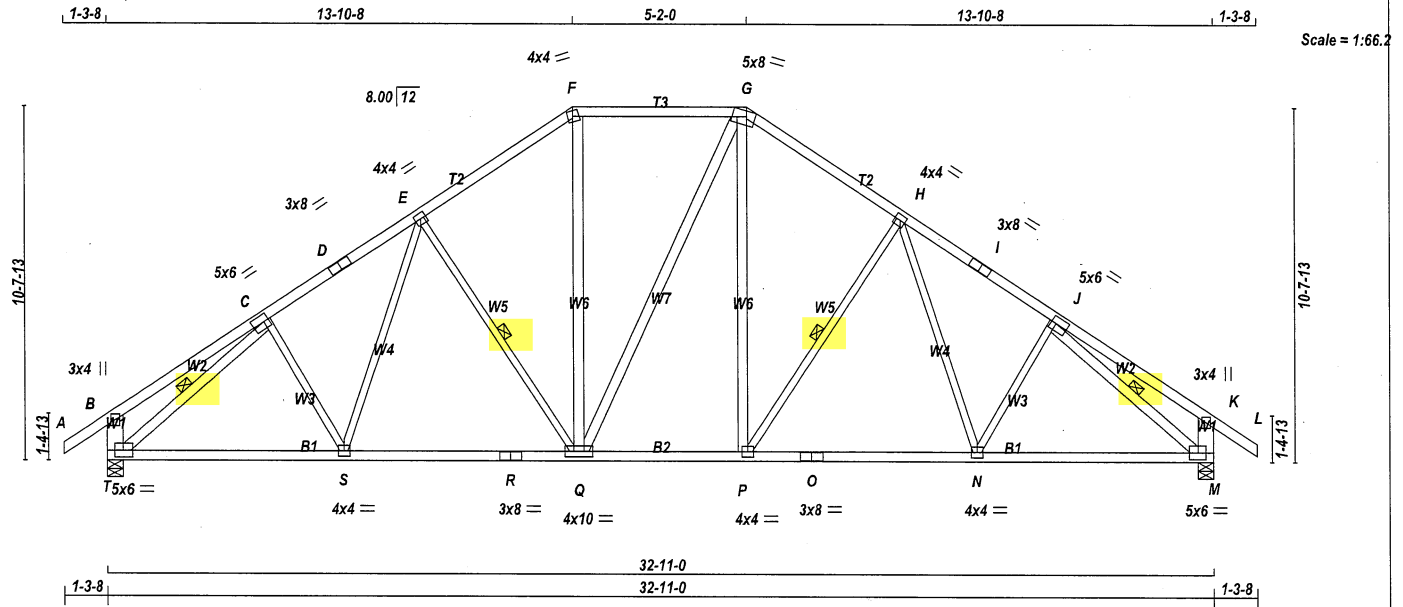


Structural component only
DWG# T-2216659

REVIEWED

JOB NAME 423673	TRUSS NAME T44	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 171 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
T - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
T - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
Q - F	2x4	DRY	No.2
Q - G	2x4	DRY	No.2
P - G	2x4	DRY	No.2
T - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TTWW-m	MT20	5.0	8.0	2.00	3.00
H	TMWW-t	MT20	4.0	4.0		
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.50	2.75
N, P, S						
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
Q	BMVWW-t	MT20	4.0	10.0		
R	BS-t	MT20	3.0	8.0		
T	BMVW1-t	MT20	5.0	6.0	2.50	2.75

NOTES- (1)
1)



Structural component only
DWG# T-2216660

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT
T	2309	0	5-8
M	2309	0	5-8
UNFACTORED REACTIONS			
1ST LCASE	MAX./MIN. COMPONENT REACTIONS		
JT	COMBINED	SNOW	LIVE
T	1616	1158 / 0	0 / 0
M	1616	1158 / 0	0 / 0
BRACING			
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.			
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.			
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.			
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M.			
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

LOADING

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 43	-112.4 -112.4	0.15 (1)
B-C	0 / 27	-112.4 -112.4	0.31 (1)
C-D	-2481 / 0	-112.4 -112.4	0.35 (1)
D-E	-2481 / 0	-112.4 -112.4	0.35 (1)
E-F	-2037 / 0	-112.4 -112.4	0.32 (1)
F-G	-1678 / 0	-112.4 -112.4	0.42 (1)
G-H	-2037 / 0	-112.4 -112.4	0.32 (1)
H-I	-2482 / 0	-112.4 -112.4	0.35 (1)
I-J	-2482 / 0	-112.4 -112.4	0.35 (1)
J-K	0 / 27	-112.4 -112.4	0.31 (1)
K-L	0 / 43	-112.4 -112.4	0.15 (1)
T-B	-366 / 0	0.0	0.02 (1)
M-K	-366 / 0	0.0	0.02 (1)
T-S	0 / 2098	-18.5 -18.5	0.45 (1)
S-R	0 / 2002	-18.5 -18.5	0.43 (1)
R-Q	0 / 2002	-18.5 -18.5	0.43 (1)
Q-P	0 / 1677	-18.5 -18.5	0.34 (1)
P-O	0 / 2002	-18.5 -18.5	0.44 (1)
O-N	0 / 2002	-18.5 -18.5	0.44 (1)
N-M	0 / 2099	-18.5 -18.5	0.45 (1)
WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	CS (LC)
FR-TO			
C-S	-65 / 44	10.00	0.03 (1)
S-E	0 / 211	10.00	0.05 (1)
E-Q	-615 / 0	4.09	0.32 (1)
Q-F	0 / 623	4.09	0.10 (1)
Q-G	0 / 1	4.45	0.00 (1)
P-G	0 / 621	4.64	0.10 (1)
P-H	-616 / 0	4.45	0.32 (1)
H-N	0 / 213	4.08	0.05 (1)
N-J	-66 / 44	4.08	0.03 (1)
T-C	-2823 / 0	10.00	0.53 (1)
J-M	-2824 / 0	10.00	0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/999 (0.19")

CSI: TC=0.42/1.00 (F-G:1), BC=0.45/1.00 (M-N:1), WB=0.53/1.00 (J-M:1), SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

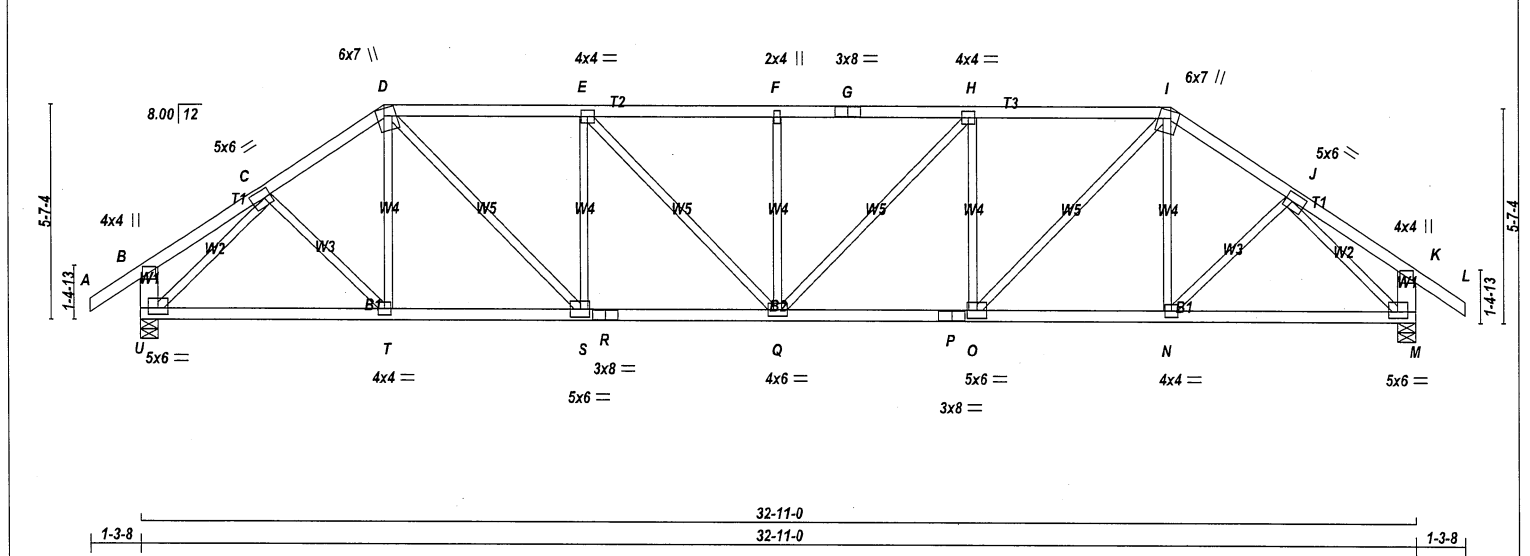
JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.65 (O) (INPUT = 1.00)

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423673	T45	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 142 lb									
[M]									
LUMBER									
N. L. G. A. RULES									
CHORDS SIZE LUMBER DESCR.									
A - D 2x4 DRY No.2 SPF									
D - G 2x4 DRY No.2 SPF									
G - I 2x4 DRY No.2 SPF									
I - L 2x4 DRY No.2 SPF									
U - B 2x6 DRY No.2 SPF									
M - K 2x6 DRY No.2 SPF									
U - R 2x4 DRY No.2 SPF									
R - P 2x4 DRY No.2 SPF									
P - M 2x4 DRY No.2 SPF									
ALL WEBS 2x3 DRY No.2 SPF									
EXCEPT									
DRY: SEASONED LUMBER.									
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED MAXIMUM FACTORED INPUT REQ'D									
GROSS REACTION GROSS REACTION BRG BRG									
JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX									
U 2309 0 2309 0 0 5-8 5-8									
M 2309 0 2309 0 0 5-8 5-8									
UNFACTORED REACTIONS									
1ST LCASE MAX /MIN. COMPONENT REACTIONS									
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL									
U 1616 1158 / 0 0 / 0 0 / 0 458 / 0 0 / 0									
M 1616 1158 / 0 0 / 0 0 / 0 458 / 0 0 / 0									
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS WEBS									
MAX. FACTORED FACTORED MAX. FACTORED									
MEMB. FORCE VERT. LOAD LC1 MAX MAX. MEMB. FORCE MAX									
(LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC)									
FR-TO FROM TO									
A-B 0 / 43 -112.4 -112.4 0.15 (1) 10.00 C-T 0 / 203 0.05 (1)									
B-C 0 / 18 -112.4 -112.4 0.14 (1) 10.00 T-D -17 / 68 0.02 (4)									
C-D -2531 / 0 -112.4 -112.4 0.23 (1) 4.14 D-S 0 / 1439 0.32 (1)									
D-E -3086 / 0 -112.4 -112.4 0.62 (1) 3.38 S-E -950 / 0 0.45 (1)									
E-F -3377 / 0 -112.4 -112.4 0.66 (1) 3.20 E-Q 0 / 423 0.10 (1)									
F-G -3377 / 0 -112.4 -112.4 0.66 (1) 3.20 Q-F -520 / 0 0.25 (1)									
G-H -3377 / 0 -112.4 -112.4 0.66 (1) 3.20 Q-H 0 / 422 0.10 (1)									
H-I -3087 / 0 -112.4 -112.4 0.62 (1) 3.38 O-H -949 / 0 0.45 (1)									
I-J -2532 / 0 -112.4 -112.4 0.23 (1) 4.14 O-I 0 / 1438 0.32 (1)									
J-K 0 / 18 -112.4 -112.4 0.14 (1) 10.00 N-I -16 / 68 0.02 (4)									
K-L 0 / 43 -112.4 -112.4 0.15 (1) 10.00 N-J 0 / 202 0.05 (1)									
U-B -298 / 0 0.0 0.0 0.02 (1) 7.81 U-C -2760 / 0 0.84 (1)									
M-K -298 / 0 0.0 0.0 0.02 (1) 7.81 J-M -2761 / 0 0.84 (1)									
U-T 0 / 1941 -18.5 -18.5 0.40 (1) 10.00									
T-S 0 / 2086 -18.5 -18.5 0.42 (1) 10.00									
S-R 0 / 3086 -18.5 -18.5 0.55 (1) 10.00									
R-Q 0 / 3086 -18.5 -18.5 0.55 (1) 10.00									
Q-P 0 / 3087 -18.5 -18.5 0.55 (1) 10.00									
P-O 0 / 3087 -18.5 -18.5 0.55 (1) 10.00									
O-N 0 / 2086 -18.5 -18.5 0.42 (1) 10.00									
N-M 0 / 1942 -18.5 -18.5 0.40 (1) 10.00									
DESIGN CRITERIA									
SPECIFIED LOADS:									
TOP CH. LL = 32.5 PSF									
DL = 6.0 PSF									
BOT CH. LL = 0.0 PSF									
DL = 7.4 PSF									
TOTAL LOAD = 45.9 PSF									
SPACING = 24.0 IN. C/C									
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF CBC 2018, ABC 2019									
- PART 9 OF OBC 2012 (2019 AMENDMENT)									
- CSA 086-14									
- TPIC 2014									
(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD									
ALLOWABLE DEFL.(LL)= L/360 (1.10")									
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.18")									
ALLOWABLE DEFL.(TL)= L/360 (1.10")									
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.31")									
CSI: TC=0.66/1.00 (E-F-1), BC=0.55/1.00 (O-Q-1), WB=0.84/1.00 (J-M-1), SSI=0.27/1.00 (D-E-1)									
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									
COMP=1.10 SHEAR=1.10 TENS=1.10									
COMPANION LIVE LOAD FACTOR = 1.00									
AUTOSOLVE HEELS OFF									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
NAIL VALUES									
PLATE GRIP(DRY) SHEAR SECTION									
(PSI) (PLI) (PLI)									
MAX MIN MAX MIN MAX MIN									
MT20 650 371 1747 788 1997 1873									
PLATE PLACEMENT TOL. = 0.250 inches									
PLATE ROTATION TOL. = 5.0 Deg.									
JSI GRIP= 0.89 (C) (INPUT = 0.90)									
JSI METAL= 0.87 (P) (INPUT = 1.00)									

07-08-22

H. J. G. ALVES

100009024

LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

Structural component only

DWG# T-2216661

REVIEWED

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	6.0	7.0	2.25	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	2.25	2.25
J	TMWW-t	MT20	5.0	6.0	2.50	2.25
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.00	3.00
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	8.0		
Q	BMVWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	8.0		
S	BMVW-t	MT20	5.0	6.0		
T	BMVW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.00	3.00

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

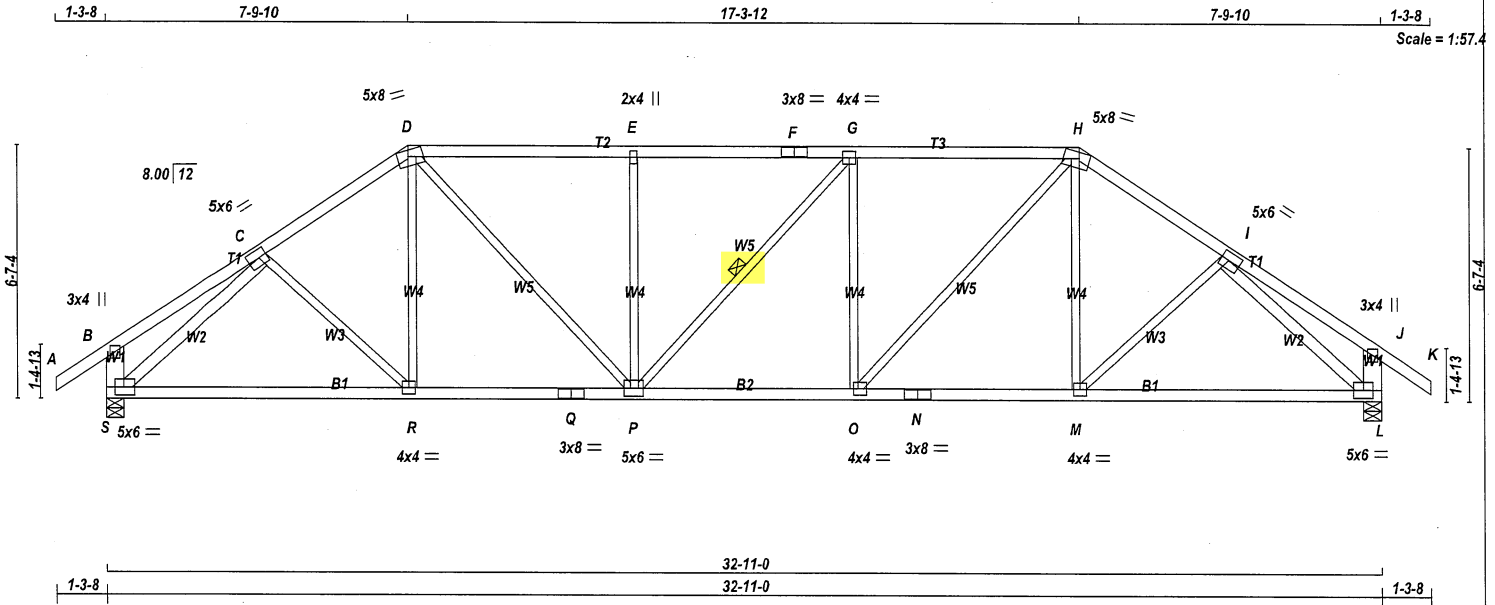


Structural component only
DWG# T-2216661

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423673	T46	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	5.0	6.0	2.50 2.50
D	TTWW-m	MT20	5.0	8.0	2.00 3.25
E	TMV+w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	8.0	
G	TMVW-t	MT20	4.0	4.0	
H	TTWW-m	MT20	5.0	8.0	2.00 3.25
I	TMVW-t	MT20	5.0	6.0	2.50 2.50
J	TMV+p	MT20	3.0	4.0	
L	BMVW1-t	MT20	5.0	6.0	2.50 2.75
M	BMVW-t	MT20	4.0	4.0	
N	BS-t	MT20	3.0	8.0	
O	BMVW-t	MT20	4.0	4.0	2.00 1.50
P	BMVW-t	MT20	5.0	6.0	2.50 2.00
Q	BS-t	MT20	3.0	8.0	
R	BMVW-t	MT20	4.0	4.0	
S	BMVW1-t	MT20	5.0	6.0	2.50 2.75

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
S	2309	0	2309	0	0	5-8	5-8	5-8	5-8
L	2309	0	2309	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
S	1616	1158 / 0	0 / 0
L	1616	1158 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S		FACTORED		W E B S	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 43	-112.4 -112.4	0.15 (1)	C-R	0 / 63
B-C	0 / 27	-112.4 -112.4	0.26 (1)	R-D	0 / 138
C-D	-2497 / 0	-112.4 -112.4	0.37 (1)	D-P	0 / 1079
D-E	-2785 / 0	-112.4 -112.4	0.72 (1)	P-E	-700 / 0
E-F	-2785 / 0	-112.4 -112.4	0.72 (1)	P-G	-3 / 0
F-G	-2785 / 0	-112.4 -112.4	0.72 (1)	G-O	-701 / 0
G-H	-2787 / 0	-112.4 -112.4	0.73 (1)	O-H	0 / 1082
H-I	-2497 / 0	-112.4 -112.4	0.37 (1)	M-H	0 / 137
I-J	0 / 27	-112.4 -112.4	0.26 (1)	M-I	0 / 63
J-K	0 / 43	-112.4 -112.4	0.15 (1)	S-C	-2801 / 0
S-B	-325 / 0	0.0	0.0	I-L	-2802 / 0
L-J	-325 / 0	0.0	0.0		

S-R	0 / 2037	-18.5	-18.5	0.46 (1)	10.00
R-Q	0 / 2054	-18.5	-18.5	0.47 (1)	10.00
Q-P	0 / 2054	-18.5	-18.5	0.47 (1)	10.00
P-O	0 / 2787	-18.5	-18.5	0.51 (1)	10.00
O-N	0 / 2054	-18.5	-18.5	0.47 (1)	10.00
N-M	0 / 2054	-18.5	-18.5	0.47 (1)	10.00
M-L	0 / 2038	-18.5	-18.5	0.46 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.73/1.00 (G-H:1), BC=0.51/1.00 (O-P:1), WB=0.89/1.00 (I-L:1), SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.63 (I) (INPUT = 1.00)



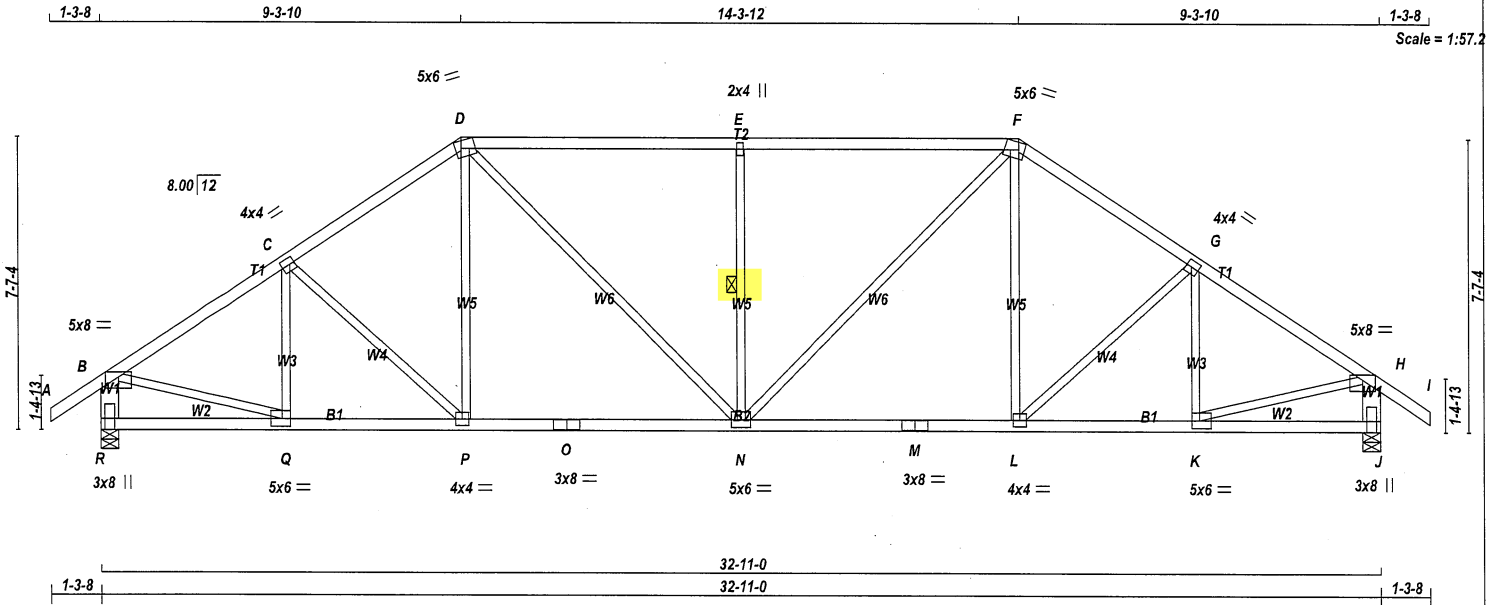
Structural component only
DWG# T-2216662

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423673	T47	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 144 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	1.75
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	8.0	Edge	
K	BMVW-t	MT20	5.0	6.0	2.50	2.50
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-wt	MT20	5.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
R	2309	0	2309	0	0	5-8	5-8		
J	2309	0	2309	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
R	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0		
J	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	Q-C	-405 / 0	0.13 (1)	
B-C	-2529 / 0	-112.4	-112.4	0.40 (1)	3.99	C-P	-211 / 0	0.15 (1)	
C-D	-2413 / 0	-112.4	-112.4	0.39 (1)	4.07	P-D	0 / 259	0.06 (1)	
D-E	-2531 / 0	-112.4	-112.4	0.89 (1)	3.05	D-N	0 / 787	0.18 (1)	
E-F	-2531 / 0	-112.4	-112.4	0.89 (1)	3.05	N-E	-990 / 0	0.36 (1)	
F-G	-2413 / 0	-112.4	-112.4	0.39 (1)	4.07	N-F	0 / 787	0.18 (1)	
G-H	-2530 / 0	-112.4	-112.4	0.40 (1)	3.98	L-F	0 / 259	0.06 (1)	
H-I	0 / 43	-112.4	-112.4	0.15 (1)	10.00	L-G	-212 / 0	0.15 (1)	
R-B	-2268 / 0	0.0	0.0	0.15 (1)	6.81	K-G	-404 / 0	0.12 (1)	
J-H	-2268 / 0	0.0	0.0	0.15 (1)	6.81	B-Q	0 / 2186	0.49 (1)	
						K-H	0 / 2187	0.49 (1)	
R-Q	0 / 0	-18.5	-18.5	0.09 (4)	10.00				
Q-P	0 / 2132	-18.5	-18.5	0.42 (1)	10.00				
P-O	0 / 1980	-18.5	-18.5	0.42 (1)	10.00				
O-N	0 / 1980	-18.5	-18.5	0.42 (1)	10.00				
N-M	0 / 1980	-18.5	-18.5	0.42 (1)	10.00				
M-L	0 / 1980	-18.5	-18.5	0.42 (1)	10.00				
L-K	0 / 2133	-18.5	-18.5	0.42 (1)	10.00				
K-J	0 / 0	-18.5	-18.5	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.89/1.00 (E-F:1), BC=0.42/1.00 (K-L:1), WB=0.49/1.00 (H-K:1), SSI=0.39/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

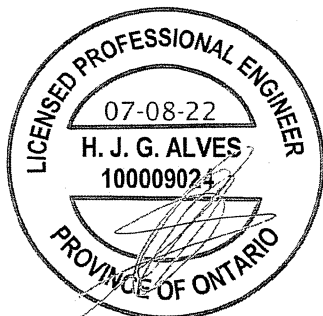
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.67 (O) (INPUT = 1.00)



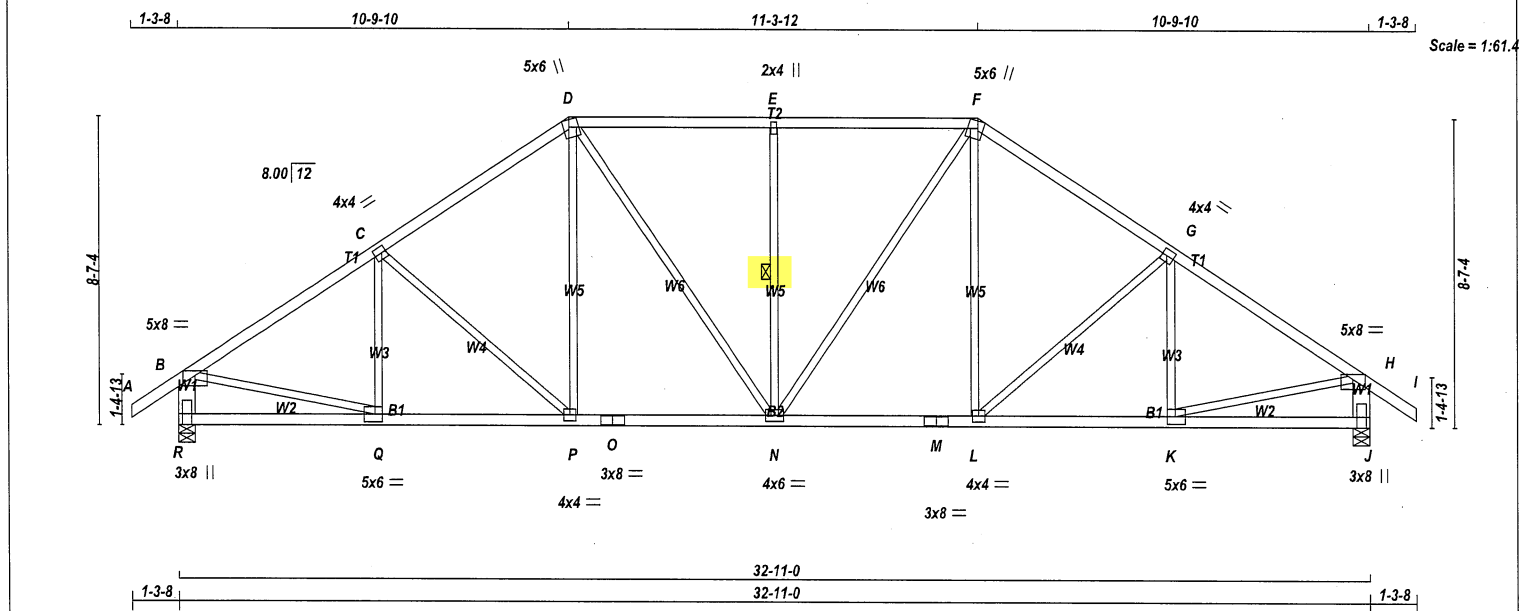
Structural component only
DWG# T-2216663

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423673	T48	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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MITek Industries, Inc.
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LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA				
N. L. G. A. RULES															TOTAL WEIGHT = 150 lb				
CHORDS SIZE LUMBER DESCR.															[M][F]				
A - D 2x4 DRY No.2 SPF															SPECIFIED LOADS:				
D - F 2x4 DRY No.2 SPF															TOP CH. LL = 32.5 PSF				
F - I 2x4 DRY No.2 SPF															DL = 6.0 PSF				
R - B 2x6 DRY No.2 SPF															BOT CH. LL = 0.0 PSF				
J - H 2x6 DRY No.2 SPF															DL = 7.4 PSF				
R - O 2x4 DRY No.2 SPF															TOTAL LOAD = 45.9 PSF				
O - M 2x4 DRY No.2 SPF															SPACING = 24.0 IN. C/C				
M - J 2x4 DRY No.2 SPF																			
ALL WEBS 2x3 DRY No.2 SPF															LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12				
EXCEPT															THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015				
DRY: SEASONED LUMBER.															THIS DESIGN COMPLIES WITH:				
															- PART 9 OF BCBC 2018, ABC 2019				
															- PART 9 OF OBC 2012 (2019 AMENDMENT)				
															- CSA 086-14				
															- TPIC 2014				
															(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD				
															ALLOWABLE DEFL.(LL)= L/360 (1.10")				
															CALCULATED VERT. DEFL.(LL)= L/ 999 (0.11")				
															ALLOWABLE DEFL.(TL)= L/360 (1.10")				
															CALCULATED VERT. DEFL.(TL)= L/ 999 (0.19")				
															CSI: TC=0.55/1.00 (G-H:1), BC=0.41/1.00 (K-L:1), WB=0.50/1.00 (H-K:1), SSI=0.31/1.00 (D-E:1)				
															DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10				
															COMP=1.10 SHEAR=1.10 TENS=1.10				
															COMPANION LIVE LOAD FACTOR = 1.00				
															AUTOSOLVE HEELS OFF				
															TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.				
															NAIL VALUES				
															PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)				
															MAX MIN MAX MIN MAX MIN				
															MT20 650 371 1747 788 1987 1873				
															PLATE PLACEMENT TOL. = 0.250 inches				
															PLATE ROTATION TOL. = 5.0 Deg.				
															JSI GRIP= 0.86 (Q) (INPUT = 0.90)				
															JSI METAL= 0.53 (M) (INPUT = 1.00)				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	8.0	Edge	
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CHORDS					WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 43	-112.4 -112.4	0.15 (1)	10.00	Q-C	-320 / 4	0.12 (1)		
B-C	-2567 / 0	-112.4 -112.4	0.55 (1)	3.78	C-P	-376 / 0	0.38 (1)		
C-D	-2311 / 0	-112.4 -112.4	0.51 (1)	3.99	P-D	0 / 353	0.08 (1)		
D-E	-2190 / 0	-112.4 -112.4	0.53 (1)	4.02	D-N	0 / 531	0.12 (1)		
E-F	-2190 / 0	-112.4 -112.4	0.53 (1)	4.02	N-E	-777 / 0	0.37 (1)		
F-G	-2311 / 0	-112.4 -112.4	0.51 (1)	3.99	N-F	0 / 531	0.12 (1)		
G-H	-2568 / 0	-112.4 -112.4	0.55 (1)	3.78	L-F	0 / 353	0.08 (1)		
H-I	0 / 43	-112.4 -112.4	0.15 (1)	10.00	L-G	-377 / 0	0.38 (1)		
R-B	-2265 / 0	0.0 0.0	0.15 (1)	6.81	K-G	-320 / 4	0.12 (1)		
J-H	-2265 / 0	0.0 0.0	0.15 (1)	6.81	B-Q	0 / 2210	0.50 (1)		
					K-H	0 / 2211	0.50 (1)		
R-Q	0 / 0	-18.5 -18.5	0.12 (4)	10.00					
Q-P	0 / 2169	-18.5 -18.5	0.41 (1)	10.00					
P-O	0 / 1891	-18.5 -18.5	0.37 (1)	10.00					
O-N	0 / 1891	-18.5 -18.5	0.37 (1)	10.00					
N-M	0 / 1891	-18.5 -18.5	0.37 (1)	10.00					
M-L	0 / 1891	-18.5 -18.5	0.37 (1)	10.00					
L-K	0 / 2170	-18.5 -18.5	0.41 (1)	10.00					
K-J	0 / 0	-18.5 -18.5	0.12 (4)	10.00					

LICENSED PROFESSIONAL ENGINEER

07-08-22

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only

DWG# T-2216664

REVIEWED

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	Edge
C	TMVW-t	MT20	4.0	4.0	2.00 1.50
D	TTWW+m	MT20	5.0	6.0	Edge 1.75
E	TMVW-w	MT20	2.0	4.0	
F	TTWW+m	MT20	5.0	6.0	Edge 1.75
G	TMVW-t	MT20	4.0	4.0	2.00 1.50
H	TMVW-p	MT20	5.0	8.0	Edge
J	BMV1+p	MT20	3.0	8.0	Edge
K	BMVW-t	MT20	5.0	6.0	2.50 2.50
L	BMVW-t	MT20	4.0	4.0	
M	BS-t	MT20	3.0	8.0	
N	BMVW-w	MT20	4.0	6.0	
O	BS-t	MT20	3.0	8.0	
P	BMVW-t	MT20	4.0	4.0	
Q	BMVW-t	MT20	5.0	6.0	2.50 2.50
R	BMV1+p	MT20	3.0	8.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

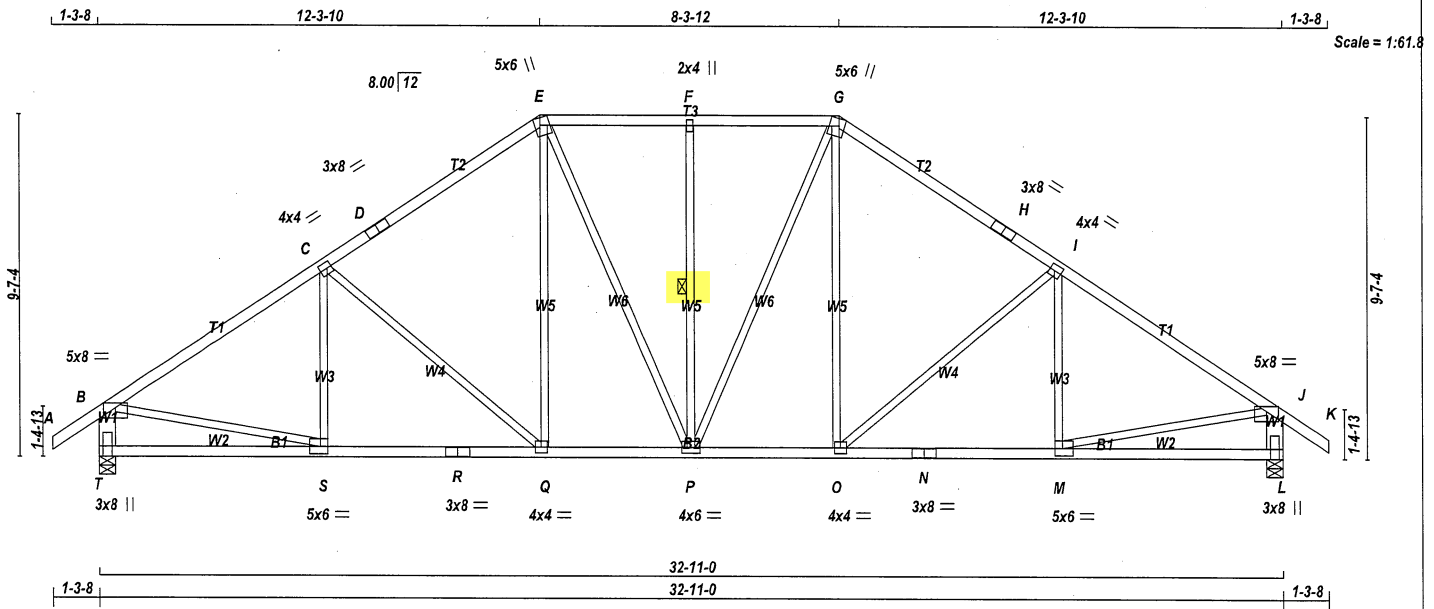
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2216664

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423673	T49	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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LUMBER	SIZE	DESCR.
N. L. G. A. RULES		
CHORDS		
A - D	2x4	No.2
D - E	2x4	DRY
E - G	2x4	DRY
G - H	2x4	DRY
H - K	2x4	DRY
T - B	2x6	DRY
L - J	2x6	DRY
T - R	2x4	DRY
R - N	2x4	DRY
N - L	2x4	DRY
ALL WEBS	2x3	DRY
EXCEPT		No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
F	TMW-w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	Edge	1.75
H	TS-t	MT20	3.0	8.0		
I	TMVW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	8.0	Edge	
M	BMVW-t	MT20	5.0	6.0	2.50	2.50
N	BS-t	MT20	3.0	8.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	4.0	6.0		
Q	BMVW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	8.0		
S	BMVW-t	MT20	5.0	6.0	2.50	2.50
T	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
FACTORED			MAXIMUM FACTORED			INPUT	REQRD	
GROSS REACTION			GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
T	2309	0	2309	0	0	5-8	5-8	
L	2309	0	2309	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
L	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	S-C	-252 / 40	0.12 (1)
B-C	-2584 / 0	-112.4	-112.4	0.73 (1)	3.52	C-Q	-529 / 0	0.73 (1)
C-D	-2196 / 0	-112.4	-112.4	0.66 (1)	3.85	Q-E	0 / 443	0.10 (1)
D-E	-2196 / 0	-112.4	-112.4	0.66 (1)	3.85	E-P	0 / 341	0.08 (1)
E-F	-1929 / 0	-112.4	-112.4	0.28 (1)	4.58	P-F	-561 / 0	0.34 (1)
F-G	-1929 / 0	-112.4	-112.4	0.28 (1)	4.58	P-G	0 / 340	0.08 (1)
G-H	-2196 / 0	-112.4	-112.4	0.66 (1)	3.85	O-G	0 / 443	0.10 (1)
H-I	-2196 / 0	-112.4	-112.4	0.66 (1)	3.85	O-I	-530 / 0	0.73 (1)
I-J	-2585 / 0	-112.4	-112.4	0.73 (1)	3.52	M-I	-251 / 40	0.12 (1)
J-K	0 / 43	-112.4	-112.4	0.15 (1)	10.00	B-S	0 / 2221	0.50 (1)
T-B	-2261 / 0	0.0	0.0	0.15 (1)	6.81	M-J	0 / 2221	0.50 (1)
L-J	-2261 / 0	0.0	0.0	0.15 (1)	6.81			
T-S	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
S-R	0 / 2188	-18.5	-18.5	0.43 (1)	10.00			
R-Q	0 / 2188	-18.5	-18.5	0.43 (1)	10.00			
Q-P	0 / 1791	-18.5	-18.5	0.34 (1)	10.00			
P-O	0 / 1791	-18.5	-18.5	0.34 (1)	10.00			
O-N	0 / 2189	-18.5	-18.5	0.43 (1)	10.00			
N-M	0 / 2189	-18.5	-18.5	0.43 (1)	10.00			
M-L	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.73/1.00 (I-J:1), BC=0.43/1.00 (M-O:1), WB=0.73/1.00 (I-O:1), SS=0.28/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

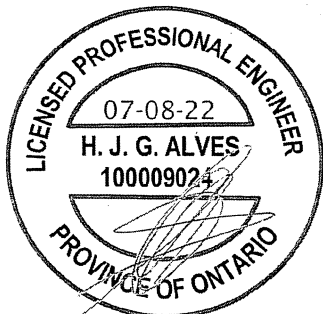
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (S) (INPUT = 0.90)
JSI METAL= 0.71 (R) (INPUT = 1.00)



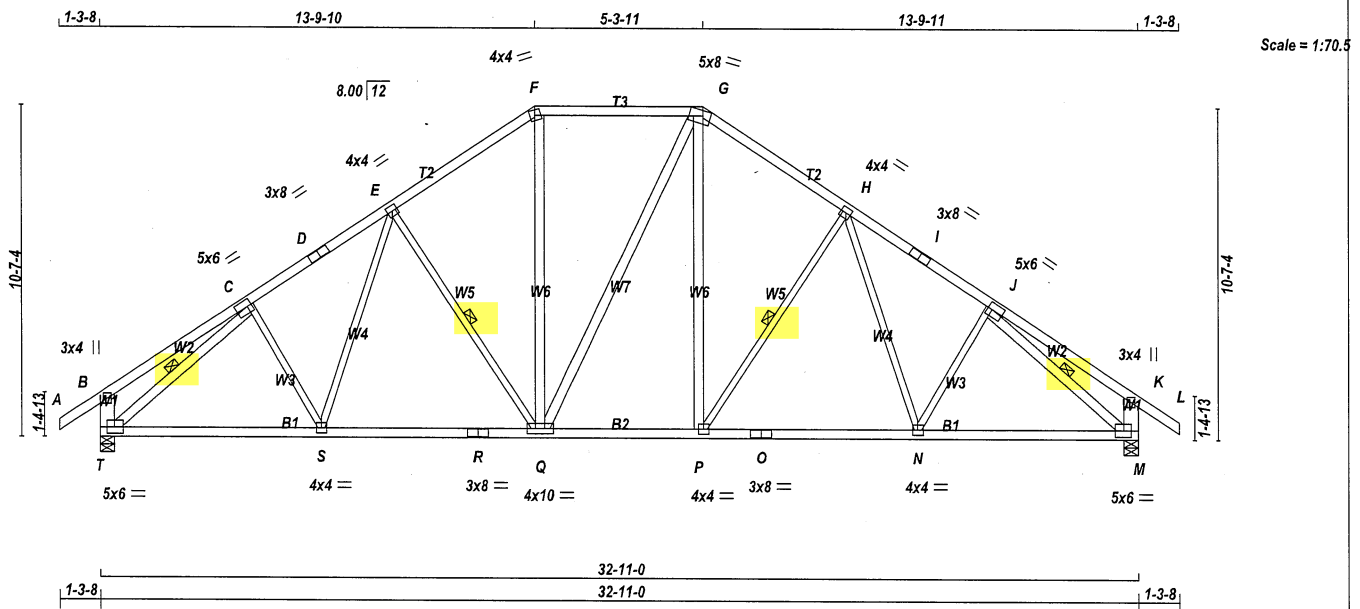
Structural component only
DWG# T-2216665

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423673	T50	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x4 DRY No.2 SPF F - G 2x4 DRY No.2 SPF G - I 2x4 DRY No.2 SPF I - L 2x4 DRY No.2 SPF T - B 2x6 DRY No.2 SPF M - K 2x6 DRY No.2 SPF T - R 2x4 DRY No.2 SPF R - O 2x4 DRY No.2 SPF O - M 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT Q - F 2x4 DRY No.2 SPF Q - G 2x4 DRY No.2 SPF P - G 2x4 DRY No.2 SPF T - C 2x4 DRY No.2 SPF J - M 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><td></td><td colspan="2">FACTORED</td><td colspan="2">MAXIMUM FACTORED</td><td colspan="2">INPUT</td><td colspan="2">REQRD</td></tr><tr><td></td><td colspan="2">GROSS REACTION</td><td colspan="2">GROSS REACTION</td><td colspan="2">BRG</td><td colspan="2">BRG</td></tr><tr><td>JT</td><td>VERT</td><td>HORZ</td><td>DOWN</td><td>HORZ</td><td>UPLIFT</td><td>IN-SX</td><td colspan="2">IN-SX</td></tr><tr><td>T</td><td>2309</td><td>0</td><td>2309</td><td>0</td><td>0</td><td>5-8</td><td colspan="2">5-8</td></tr><tr><td>M</td><td>2309</td><td>0</td><td>2309</td><td>0</td><td>0</td><td>5-8</td><td colspan="2">5-8</td></tr></table> UNFACTORED REACTIONS <table><tr><td></td><td colspan="2">1ST LCASE</td><td colspan="2">MAX/MIN. COMPONENT REACTIONS</td><td></td><td></td><td></td><td></td></tr><tr><td>JT</td><td colspan="2">COMBINED</td><td>SNOW</td><td>LIVE</td><td>PERM.LIVE</td><td>WIND</td><td>DEAD</td><td>SOIL</td></tr><tr><td>T</td><td>1616</td><td>1158 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>458 / 0</td><td>0 / 0</td></tr><tr><td>M</td><td>1616</td><td>1158 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>458 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) <table><tr><td colspan="2">CHORDS</td><td colspan="2">FACTORED</td><td colspan="2">WEBS</td></tr><tr><td>MEMB.</td><td>MAX. FORCE (LBS)</td><td>VERT. LOAD (PLF)</td><td>MAX. UNBRACED LENGTH (FT)</td><td>MEMB.</td><td>MAX. FORCE (LBS)</td></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td></tr><tr><td>A-B</td><td>0 / 43</td><td>-112.4 -112.4</td><td>0.15 (1)</td><td>10.00</td><td>C-S</td><td>-61 / 45</td></tr><tr><td>B-C</td><td>0 / 26</td><td>-112.4 -112.4</td><td>0.30 (1)</td><td>10.00</td><td>S-E</td><td>0 / 207</td></tr><tr><td>C-D</td><td>-2481 / 0</td><td>-112.4 -112.4</td><td>0.34 (1)</td><td>4.09</td><td>E-Q</td><td>-609 / 0</td></tr><tr><td>D-E</td><td>-2481 / 0</td><td>-112.4 -112.4</td><td>0.34 (1)</td><td>4.09</td><td>Q-F</td><td>0 / 619</td></tr><tr><td>E-F</td><td>-2044 / 0</td><td>-112.4 -112.4</td><td>0.32 (1)</td><td>4.44</td><td>Q-G</td><td>0 / 1</td></tr><tr><td>F-G</td><td>-1683 / 0</td><td>-112.4 -112.4</td><td>0.45 (1)</td><td>4.61</td><td>P-H</td><td>0 / 618</td></tr><tr><td>G-H</td><td>-2043 / 0</td><td>-112.4 -112.4</td><td>0.32 (1)</td><td>4.44</td><td>P-H</td><td>-611 / 0</td></tr><tr><td>H-I</td><td>-2482 / 0</td><td>-112.4 -112.4</td><td>0.34 (1)</td><td>4.09</td><td>H-N</td><td>0 / 209</td></tr><tr><td>I-J</td><td>-2482 / 0</td><td>-112.4 -112.4</td><td>0.34 (1)</td><td>4.09</td><td>N-J</td><td>-62 / 45</td></tr><tr><td>J-K</td><td>0 / 26</td><td>-112.4 -112.4</td><td>0.30 (1)</td><td>10.00</td><td>T-C</td><td>-2823 / 0</td></tr><tr><td>K-L</td><td>0 / 43</td><td>-112.4 -112.4</td><td>0.15 (1)</td><td>10.00</td><td>J-M</td><td>-2824 / 0</td></tr><tr><td>T-B</td><td>-365 / 0</td><td>0.0</td><td>0.0</td><td>0.02 (1)</td><td>7.81</td><td></td><td></td></tr><tr><td>M-K</td><td>-365 / 0</td><td>0.0</td><td>0.0</td><td>0.02 (1)</td><td>7.81</td><td></td><td></td></tr><tr><td>T-S</td><td>0 / 2096</td><td>-18.5</td><td>-18.5</td><td>0.45 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>S-R</td><td>0 / 2004</td><td>-18.5</td><td>-18.5</td><td>0.43 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>R-Q</td><td>0 / 2004</td><td>-18.5</td><td>-18.5</td><td>0.43 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>Q-P</td><td>0 / 1683</td><td>-18.5</td><td>-18.5</td><td>0.34 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>P-O</td><td>0 / 2004</td><td>-18.5</td><td>-18.5</td><td>0.43 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>O-N</td><td>0 / 2004</td><td>-18.5</td><td>-18.5</td><td>0.43 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>N-M</td><td>0 / 2098</td><td>-18.5</td><td>-18.5</td><td>0.45 (1)</td><td>10.00</td><td></td><td></td></tr></table>					FACTORED		MAXIMUM FACTORED		INPUT		REQRD			GROSS REACTION		GROSS REACTION		BRG		BRG		JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		T	2309	0	2309	0	0	5-8	5-8		M	2309	0	2309	0	0	5-8	5-8			1ST LCASE		MAX/MIN. COMPONENT REACTIONS						JT	COMBINED		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	T	1616	1158 / 0	0 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	M	1616	1158 / 0	0 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	CHORDS		FACTORED		WEBS		MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FORCE (LBS)	FR-TO		FROM TO		FR-TO		A-B	0 / 43	-112.4 -112.4	0.15 (1)	10.00	C-S	-61 / 45	B-C	0 / 26	-112.4 -112.4	0.30 (1)	10.00	S-E	0 / 207	C-D	-2481 / 0	-112.4 -112.4	0.34 (1)	4.09	E-Q	-609 / 0	D-E	-2481 / 0	-112.4 -112.4	0.34 (1)	4.09	Q-F	0 / 619	E-F	-2044 / 0	-112.4 -112.4	0.32 (1)	4.44	Q-G	0 / 1	F-G	-1683 / 0	-112.4 -112.4	0.45 (1)	4.61	P-H	0 / 618	G-H	-2043 / 0	-112.4 -112.4	0.32 (1)	4.44	P-H	-611 / 0	H-I	-2482 / 0	-112.4 -112.4	0.34 (1)	4.09	H-N	0 / 209	I-J	-2482 / 0	-112.4 -112.4	0.34 (1)	4.09	N-J	-62 / 45	J-K	0 / 26	-112.4 -112.4	0.30 (1)	10.00	T-C	-2823 / 0	K-L	0 / 43	-112.4 -112.4	0.15 (1)	10.00	J-M	-2824 / 0	T-B	-365 / 0	0.0	0.0	0.02 (1)	7.81			M-K	-365 / 0	0.0	0.0	0.02 (1)	7.81			T-S	0 / 2096	-18.5	-18.5	0.45 (1)	10.00			S-R	0 / 2004	-18.5	-18.5	0.43 (1)	10.00			R-Q	0 / 2004	-18.5	-18.5	0.43 (1)	10.00			Q-P	0 / 1683	-18.5	-18.5	0.34 (1)	10.00			P-O	0 / 2004	-18.5	-18.5	0.43 (1)	10.00			O-N	0 / 2004	-18.5	-18.5	0.43 (1)	10.00			N-M	0 / 2098	-18.5	-18.5	0.45 (1)	10.00			DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.10") CALCULATED VERT. DEFL.(LL)= L/999 (0.10") ALLOWABLE DEFL.(TL)= L/360 (1.10") CALCULATED VERT. DEFL.(TL)= L/999 (0.19") CSI: TC=0.45/1.00 (F-G-1), BC=0.45/1.00 (M-N-1), WB=0.52/1.00 (J-M-1), SSI=0.23/1.00 (F-G-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 798 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (M) (INPUT = 0.90) JSI METAL= 0.65 (O) (INPUT = 1.00)			
	FACTORED		MAXIMUM FACTORED		INPUT		REQRD																																																																																																																																																																																																																																																												
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T	2309	0	2309	0	0	5-8	5-8																																																																																																																																																																																																																																																												
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TTWW-m	MT20	5.0	8.0	2.00	3.00
H	TMWW-t	MT20	4.0	4.0		
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.50	2.75
N, P, S						
O	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
Q	BMWWW-t	MT20	4.0	10.0		
R	BS-t	MT20	3.0	8.0		
T	BMVW1-t	MT20	5.0	6.0	2.50	2.75

NOTES- (1)

1)

LICENSED PROFESSIONAL ENGINEER

07-08-22

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only
DWG# T-2216666

REVIEWED

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TTWW-m	MT20	5.0	8.0	2.00	3.00
H	TMWW-t	MT20	4.0	4.0		
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.50	2.75
N, P, S						
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
Q	BMVWW-t	MT20	4.0	10.0		
R	BS-t	MT20	3.0	8.0		
T	BMVW1-t	MT20	5.0	6.0	2.50	2.75

NOTES- (1)
1)

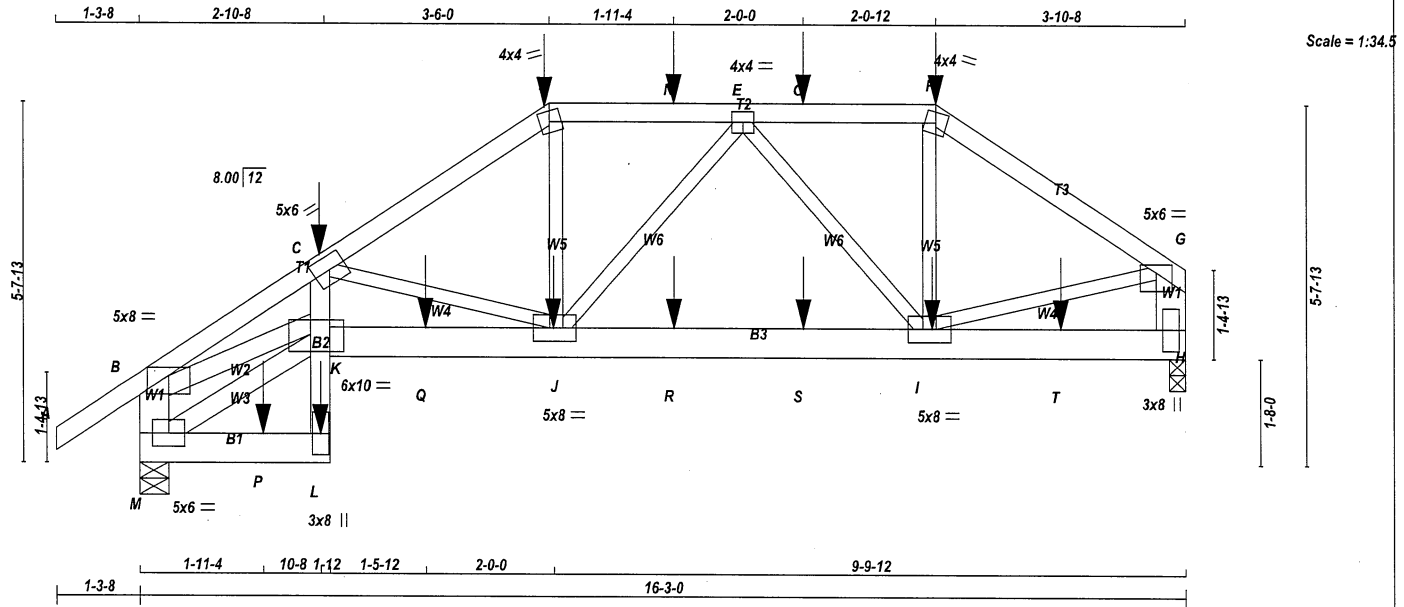
Structural component only
DWG# T-2216666

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423673	T51S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 12:26:42 2022 Page 1
ID:c3jy23uDiJq_8pvRkKbZpy75XW-gdv4Es?fm?oj9ocPVF8i_hbTop1OT4F9XslJb6z_8Qx



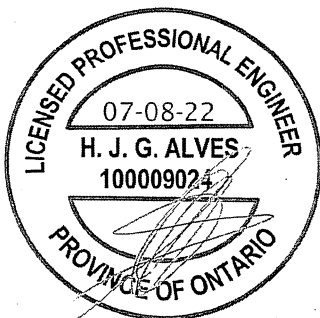
TOTAL WEIGHT = 82 lb

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x4 DRY No.2 SPF F - G 2x4 DRY No.2 SPF M - B 2x6 DRY No.2 SPF H - G 2x6 DRY No.2 SPF M - L 2x6 DRY No.2 SPF L - C 2x4 DRY No.2 SPF K - H 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT M - K 2x4 DRY No.2 SPF B - K 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQRD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX H 1601 0 1601 0 0 3-0 3-0 M 1870 0 1870 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL H 1122 792 / 0 0 / 0 0 / 0 330 / 0 0 / 0 M 1309 932 / 0 0 / 0 0 / 0 377 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (7) CHORDS MAX. FACTORED WEBS MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX. MAX. MEMB. FORCE MAX. (LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC) FR-TO LENGTH FR-TO A-B 0 / 43 -112.4 -112.4 0.17 (1) 10.00 C-J -1778 / 0 0.48 (1) B-C -4148 / 0 -112.4 -112.4 0.38 (1) 3.13 J-D 0 / 679 0.17 (1) C-D -2308 / 0 -112.4 -112.4 0.33 (1) 4.14 I-F 0 / 354 0.09 (1) D-N -1943 / 0 -112.4 -112.4 0.30 (1) 4.47 I-G 0 / 1505 0.37 (1) E-O -1478 / 0 -112.4 -112.4 0.30 (1) 4.47 M-K -121 / 0 0.02 (1) F-G -1478 / 0 -112.4 -112.4 0.28 (1) 5.02 B-K 0 / 3555 0.63 (1) O-F -1478 / 0 -112.4 -112.4 0.28 (1) 5.02 J-E -56 / 59 0.02 (4) F-G -1760 / 0 -112.4 -112.4 0.39 (1) 4.56 E-I -739 / 0 0.27 (1) M-B -1779 / 0 0.0 0.0 0.13 (1) 7.43 H-G -1558 / 0 0.0 0.0 0.11 (1) 7.81 M-P 0 / 104 -18.5 -18.5 0.03 (4) 10.00 P-L 0 / 104 -18.5 -18.5 0.03 (4) 10.00 L-K 0 / 325 0.0 0.0 0.32 (1) 10.00 K-C 0 / 1121 0.0 0.0 0.46 (1) 10.00 K-Q 0 / 3603 -18.5 -18.5 0.50 (1) 10.00 Q-J 0 / 3603 -18.5 -18.5 0.50 (1) 10.00 J-R 0 / 1935 -18.5 -18.5 0.31 (1) 10.00 R-S 0 / 1935 -18.5 -18.5 0.31 (1) 10.00 S-I 0 / 1935 -18.5 -18.5 0.31 (1) 10.00 I-T 0 / 0 -18.5 -18.5 0.07 (4) 10.00 T-H 0 / 0 -18.5 -18.5 0.07 (4) 10.00 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN. C 2-10-8 -43 -43 121 FRONT VERT TOTAL --- C1 D 6-4-8 -175 -175 --- FRONT VERT TOTAL --- C1 F 12-4-8 -175 -175 --- FRONT VERT TOTAL --- C1 I 12-3-12 -14 -14 --- FRONT VERT TOTAL --- C1 J 6-5-4 -14 -14 --- FRONT VERT TOTAL --- C1 L 2-9-12 -207 -207 --- FRONT VERT TOTAL --- C1 N 8-3-12 -72 -72 --- FRONT VERT TOTAL --- C1 O 10-3-12 -72 -72 --- FRONT VERT TOTAL --- C1 P 1-11-4 -7 -7 --- FRONT VERT TOTAL --- C1 Q 4-5-4 -14 -14 --- FRONT VERT TOTAL --- C1 R 8-3-12 -14 -14 --- FRONT VERT TOTAL --- C1 S 10-3-12 -14 -14 --- FRONT VERT TOTAL --- C1 T 14-3-12 -14 -14 --- FRONT VERT TOTAL --- C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.54") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.09") ALLOWABLE DEFL.(TL)= L/360 (0.54") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.15") CSI: TC=0.39/1.00 (F-G-1), BC=0.50/1.00 (J-K-1), WB=0.63/1.00 (B-K-1), SSI=0.25/1.00 (E-F-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 798 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.98 (B) (INPUT = 0.90) JSI METAL= 0.68 (B) (INPUT = 1.00)			
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JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0	2.00	1.75
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-p	MT20	5.0	6.0	1.50	3.00
H	BMV1+p	MT20	3.0	8.0		
I	BMVWW-t	MT20	5.0	8.0		
J	BMVWW-t	MT20	5.0	8.0	2.50	3.00
K	BMVWW-t	MT20	6.0	10.0	3.25	6.00
L	BMV+p	MT20	3.0	8.0		
M	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



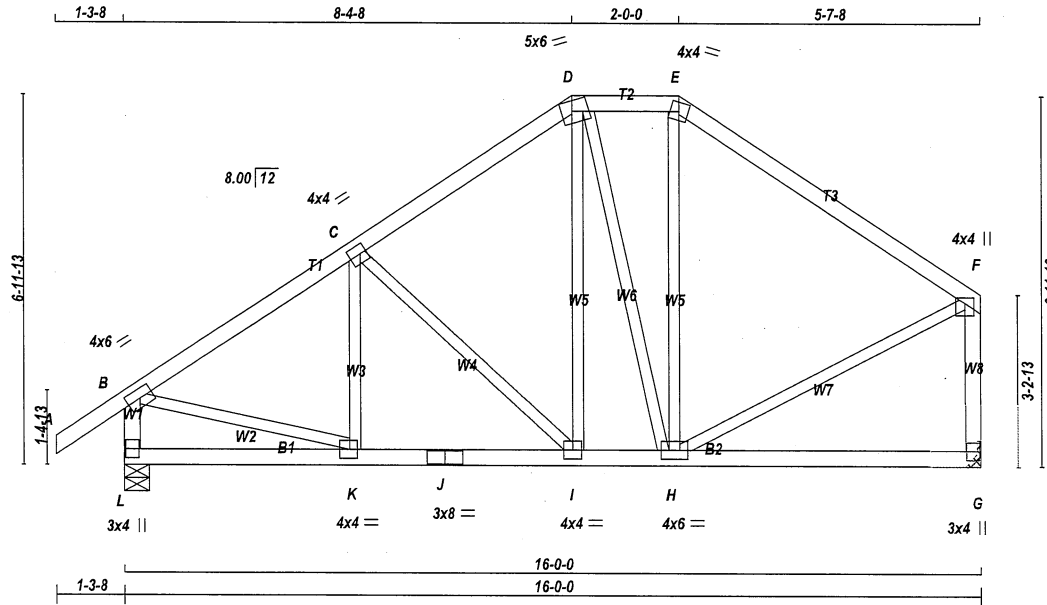
Structural component only
DWG# T-2216667

REVIEWED
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423673	T52	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:41.6

TOTAL WEIGHT = 78 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.25
E	TTWW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	6.0		
I	BMVWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	8.0		
K	BMVWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1202	0	1202	0
G	1047	0	1047	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT							
L	840	609 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0
G	734	520 / 0	0 / 0	0 / 0	0 / 0	214 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	W E B S
MEMB.	FORCE	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE	MAX
FR-TO			FROM	TO					CSI (LC)
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	K-C	-135 / 40	0.04 (1)	
B-C	-1053 / 0	-112.4	-112.4	0.26 (1)	5.82	C-I	-384 / 0	0.22 (1)	
C-D	-780 / 0	-112.4	-112.4	0.25 (1)	6.25	I-D	0 / 290	0.06 (1)	
D-E	-584 / 0	-112.4	-112.4	0.06 (1)	6.25	D-H	-154 / 0	0.14 (1)	
E-F	-703 / 0	-112.4	-112.4	0.46 (1)	6.25	H-E	-37 / 52	0.03 (1)	
L-B	-1169 / 0	0.0	0.0	0.12 (1)	7.37	B-K	0 / 930	0.21 (1)	
G-F	-1005 / 0	0.0	0.0	0.18 (1)	7.80	H-F	0 / 656	0.15 (1)	
L-K	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
K-J	0 / 901	-18.5	-18.5	0.18 (1)	10.00				
J-I	0 / 901	-18.5	-18.5	0.18 (1)	10.00				
I-H	0 / 626	-18.5	-18.5	0.18 (4)	10.00				
H-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.46/1.00 (E-F:1), BC=0.18/1.00 (I-K:1), WB=0.22/1.00 (C-I:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.31 (J) (INPUT = 1.00)



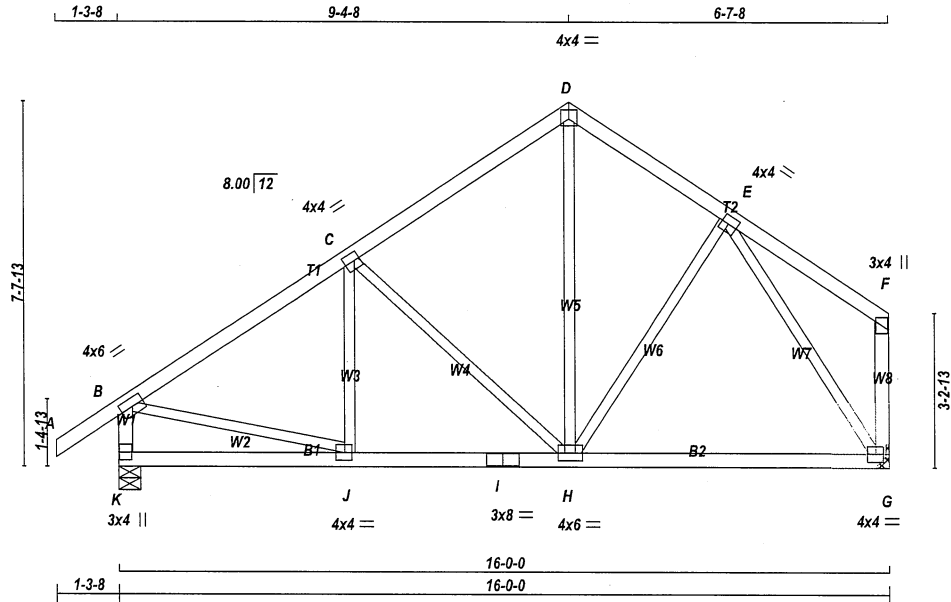
Structural component only
DWG# T-2216668

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423673	T53	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 73 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.75
F	TMV-p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMVW-t	MT20	4.0	4.0	2.00	1.75
K	BMV1-p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
K	1202	0	1202	0
G	1047	0	1047	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	COMBINED	609 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0
G		520 / 0	0 / 0	0 / 0	0 / 0	214 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT.	LOAD	LC1	MAX	CS1 (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	MAX. FACTORED	FORCE	MAX	CS1 (LC)
			(LBS)		(PLF)									(LBS)		
FR-TO																
A-B		0 / 43		-112.4	-112.4	0.15 (1)	10.00	J-C	-121 / 37		0.04 (1)					
B-C		-1044 / 0		-112.4	-112.4	0.33 (1)	5.74	C-H	-455 / 0		0.32 (1)					
C-D		-714 / 0		-112.4	-112.4	0.32 (1)	6.25	H-D	0 / 417		0.09 (1)					
D-E		-702 / 0		-112.4	-112.4	0.15 (1)	6.25	H-E	0 / 49		0.02 (4)					
E-F		0 / 23		-112.4	-112.4	0.19 (1)	10.00	B-J	0 / 920		0.21 (1)					
K-B		-1162 / 0		0.0	0.0	0.12 (1)	7.39	E-G	-1015 / 0		0.64 (1)					
G-F		-146 / 0		0.0	0.0	0.03 (1)	7.81									
K-J		0 / 0		-18.5	-18.5	0.09 (4)	10.00									
J-I		0 / 897		-18.5	-18.5	0.24 (1)	10.00									
I-H		0 / 897		-18.5	-18.5	0.24 (1)	10.00									
H-G		0 / 553		-18.5	-18.5	0.20 (4)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.33/1.00 (B-C:1), BC=0.24/1.00 (H-J:1), WB=0.64/1.00 (E-G:1), SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(FSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)



Structural component only
DWG# T-2216669

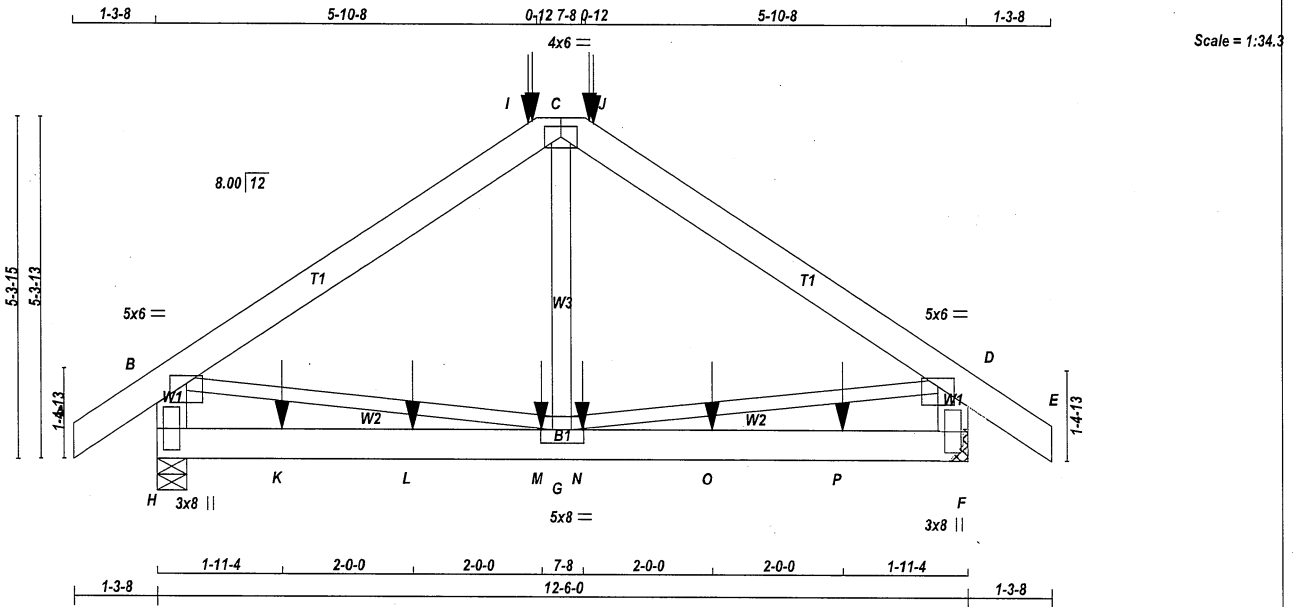
REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423673	T54	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:c3jyj23uDijq_8pvRKbkZpy75XW-4CaDsu2YewB10GL_ANiPbKdYc06jgVlcDqWzCRz_8Qu



TOTAL WEIGHT = 72 lb [M]

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - C	2x6	DRY	No.2	SPF	
C - E	2x6	DRY	No.2	SPF	
H - B	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
H - F	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
G - C	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTW-p	MT20	4.0	6.0		
D	TMVW-p	MT20	5.0	6.0	1.50	3.00
F	BMV1+p	MT20	3.0	8.0		
G	BMVWW-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	1776	0	1776	0
F	1769	0	1769	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1242	897 / 0	0 / 0	0 / 0	0 / 0	344 / 0	0 / 0
F	1237	893 / 0	0 / 0	0 / 0	0 / 0	344 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 44	-112.4 -112.4	0.09 (1)	10.00	B-G	0 / 1427	0.35 (1)
B-I	-1530 / 0	-112.4 -112.4	0.54 (1)	5.47	G-D	0 / 1427	0.35 (1)
I-C	-1530 / 0	-112.4 -112.4	0.54 (1)	5.47	G-C	-118 / 225	0.06 (4)
C-J	-1526 / 0	-112.4 -112.4	0.53 (1)	5.52			
J-D	-1526 / 0	-112.4 -112.4	0.53 (1)	5.52			
D-E	0 / 44	-112.4 -112.4	0.09 (1)	10.00			
H-B	-1697 / 0	0.0 0.0	0.12 (1)	7.57			
F-D	-1690 / 0	0.0 0.0	0.12 (1)	7.58			
H-K	0 / 0	-18.5 -18.5	0.20 (4)	10.00			
K-L	0 / 0	-18.5 -18.5	0.20 (4)	10.00			
L-M	0 / 0	-18.5 -18.5	0.20 (4)	10.00			
M-G	0 / 0	-18.5 -18.5	0.20 (4)	10.00			
G-N	0 / 0	-18.5 -18.5	0.20 (4)	10.00			
N-O	0 / 0	-18.5 -18.5	0.20 (4)	10.00			
O-P	0 / 0	-18.5 -18.5	0.20 (4)	10.00			
P-F	0 / 0	-18.5 -18.5	0.20 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
I	5-10-8	-295	-295	---	BACK	VERT	TOTAL	---	C1
I	5-11-4	-175	-175	---	BACK	VERT	TOTAL	---	C1
J	6-6-12	-176	-176	---	BACK	VERT	TOTAL	---	C1
J	6-7-8	-295	-295	---	BACK	VERT	TOTAL	---	C1
K	1-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
L	3-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
M	5-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
N	6-6-12	-29	-29	---	BACK	VERT	TOTAL	---	C1
O	8-6-12	-29	-29	---	BACK	VERT	TOTAL	---	C1
P	10-6-12	-29	-29	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.20/1.00 (G-H:4) , WB=0.35/1.00 (B-G:1) , SSI=0.55/1.00 (G-D:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.36 (B) (INPUT = 1.00)

Structural component only
DWG# T-2216670

REVIEWED

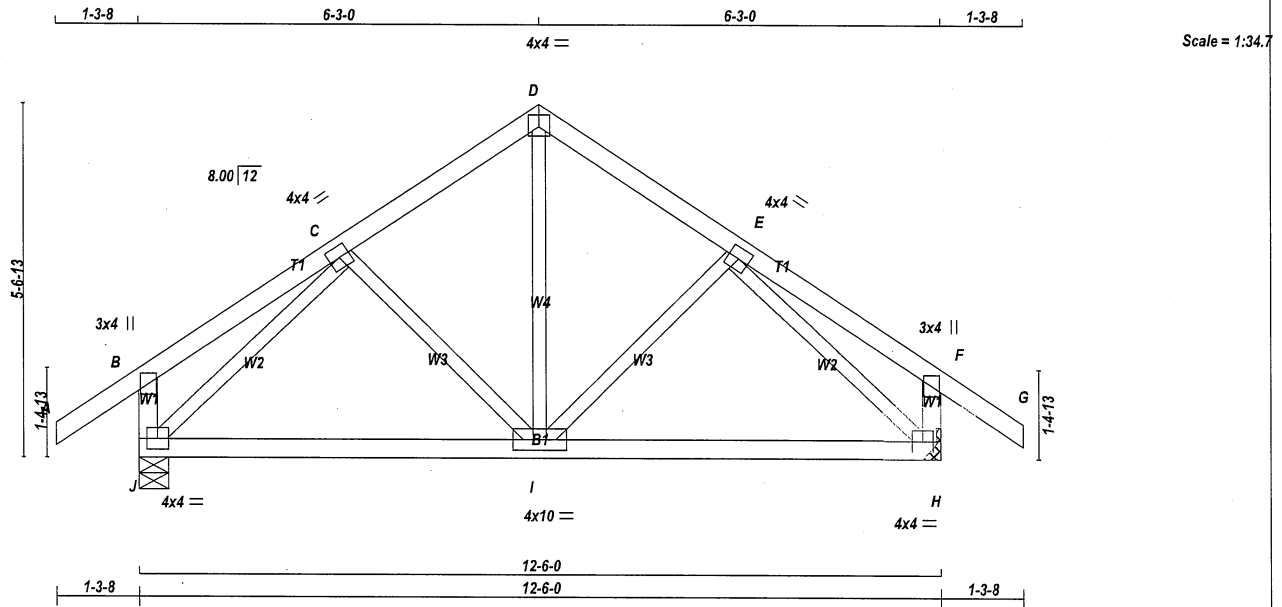


Structural component only
DWG# T-2216670

REVIEWED

JOB NAME 423673	TRUSS NAME T56	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 55 lb
[M/F]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	10.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP
J	973	0	973	0
H	973	0	973	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	679	495 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0
H	679	495 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 43	-112.4	-112.4	0.15 (1)	C-I	-179 / 0	0.06 (1)
B-C	0 / 22	-112.4	-112.4	0.17 (1)	I-D	0 / 395	0.09 (1)
C-D	-626 / 0	-112.4	-112.4	0.13 (1)	D-E	-179 / 0	0.06 (1)
D-E	-626 / 0	-112.4	-112.4	0.13 (1)	E-F	-895 / 0	0.28 (1)
E-F	0 / 22	-112.4	-112.4	0.17 (1)	F-G	-896 / 0	0.28 (1)
F-G	0 / 43	-112.4	-112.4	0.15 (1)	G-H	-896 / 0	0.28 (1)
H-I	-290 / 0	0.0	0.0	0.03 (1)	I-J	-290 / 0	0.03 (1)
I-J	-290 / 0	0.0	0.0	0.03 (1)	J-K	-290 / 0	0.03 (1)
J-K	0 / 629	-18.5	-18.5	0.25 (4)	K-L	0 / 629	-18.5
K-L	0 / 629	-18.5	-18.5	0.25 (4)	L-M	0 / 629	-18.5

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.17/1.00 (E-F:1), BC=0.25/1.00 (H-I:4), WB=0.28/1.00 (E-H:1), SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (E) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)



Structural component only
DWG# T-2216671

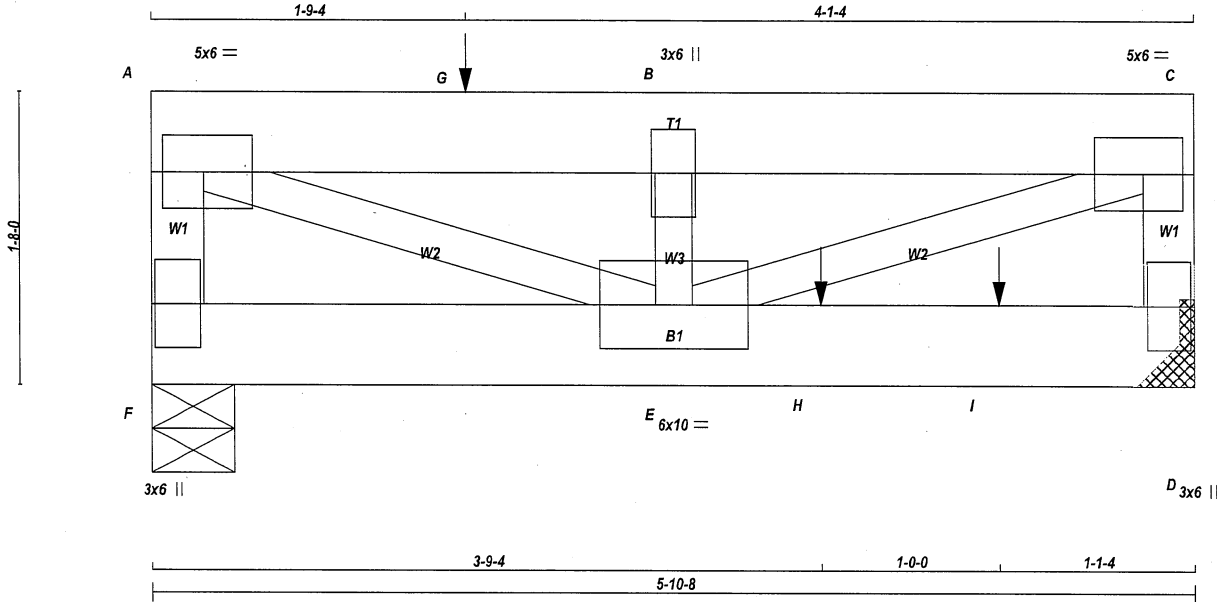
REVIEWED

JOB NAME 423673	TRUSS NAME T57	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:12.5



TOTAL WEIGHT = 2 X 28 = 55 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
				DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
C-D 1	12	TOP
A-C 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE (14.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.75
B	TMW+w	MT20	3.0	6.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.75
D	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
F	2050	0	2050	0
D	2360	0	2360	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
F	1444 983 / 0 0 / 0 0 / 0 461 / 0 0 / 0
D	1655 1166 / 0 0 / 0 0 / 0 489 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE	FACTORED VERT. LOAD	LC1	MAX. FACTORED VERT. LOAD	LC1	MAX. FACTORED VERT. LOAD	LC1	MAX. FACTORED VERT. LOAD	LC1
FR-TO									
F-A	-1995 / 0	0.0	0.0	0.11 (1)	7.80	A-E	0 / 3277	0.41 (1)	
A-G	-3031 / 0	-112.4	-112.4	0.44 (1)	5.66	E-B	-1243 / 0	0.10 (1)	
G-B	-3031 / 0	-112.4	-112.4	0.44 (1)	5.66	E-C	0 / 3277	0.41 (1)	
B-C	-3031 / 0	-112.4	-112.4	0.05 (1)	6.25				
D-C	-1404 / 0	0.0	0.0	0.08 (1)	7.81				
F-E	0 / 0	-43.5	-43.5	0.02 (4)	10.00				
E-H	0 / 0	-43.5	-43.5	0.38 (1)	10.00				
H-I	0 / 0	-18.5	-18.5	0.38 (1)	10.00				
I-D	0 / 0	-18.5	-18.5	0.38 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-9-4	-1045	-1045	---	TOP	VERT	TOTAL	---	C1
H	3-9-4	-719	-719	---	BACK	VERT	TOTAL	---	C1
I	4-9-4	-719	-719	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.44/1.00 (A-B:1), BC=0.38/1.00 (D-E:1), WB=0.41/1.00 (A-E:1), SSI=0.39/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

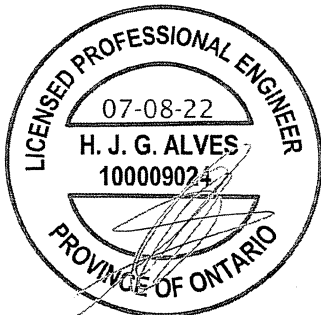
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)



Structural component only
DWG# T-2216672

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423673	T57	1	2	BAYVIEW WELLINGTON	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW-t	MT20	6.0	10.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



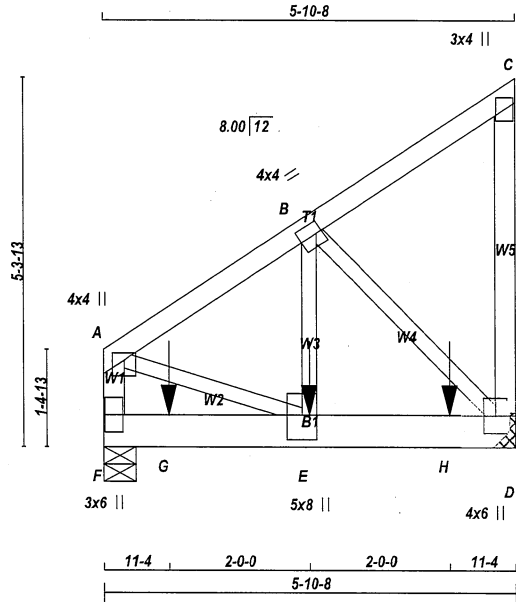
Structural component only
DWG# T-2216672

REVIEWED

JOB NAME 423673	TRUSS NAME T58	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 32 = 64 lb

[M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A 1	12	TOP
A - C 1	12	TOP
C - D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
B - E 1	4	SIDE(273.6)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMVW-i	MT20	4.0	4.0	2.00	1.00
C	TMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
F	1723	0	1723	0	5-8	5-8
D	2121	0	2121	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX /MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
F	1200	895 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
D	1480	1087 / 0	0 / 0	0 / 0	0 / 0	392 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM	TO		FR-TO		
F - A	-1472 / 0	0.0	0.0	0.08 (1)	7.81	A - E	0 / 1406
A - B	-1582 / 0	-112.4	-112.4	0.08 (1)	6.25	E - B	0 / 1832
B - C	-18 / 0	-112.4	-112.4	0.07 (1)	6.25	B - D	-1889 / 0
D - C	-133 / 0	0.0	0.0	0.03 (1)	7.81		
F - G	0 / 0	-18.5	-18.5	0.08 (1)	10.00		
G - E	0 / 0	-18.5	-18.5	0.08 (1)	10.00		
E - H	0 / 1332	-18.5	-18.5	0.31 (1)	10.00		
H - D	0 / 1332	-18.5	-18.5	0.31 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-11-4	-1222	-1222	---	FRONT	VERT	TOTAL	---	C1
G	11-4	-254	-254	---	TOP	VERT	TOTAL	---	C1
H	4-11-4	-665	-665	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.08/1.00 (A-F:1) , BC=0.31/1.00 (D-E:1) , WB=0.27/1.00 (B-D:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)

Structural component only
DWG# T-2216673

REVIEWED

CONTINUED ON PAGE



Structural component only
DWG# T-2216673

REVIEWED
CONTINUED ON PAGE 2

JOB NAME 423673	TRUSS NAME T58	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	5.0	8.0	4.25	2.50
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



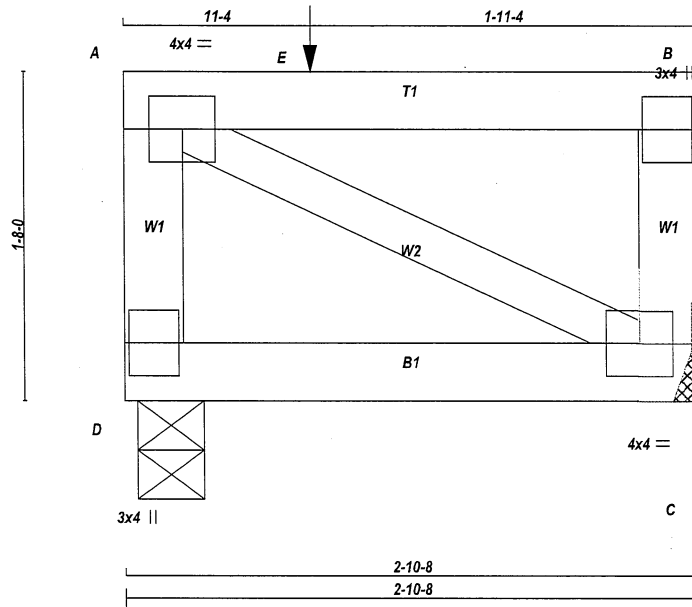
Structural component only
DWG# T-2216673

REVIEWED

JOB NAME 423673	TRUSS NAME T59	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:11.2

TOTAL WEIGHT = 2 X 11 = 22 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 1	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 1	12	SIDE (14.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	389	0	389	0	0
C	304	0	304	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	275	182 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	216	136 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED FORCE (LBS)	MEMB.	FACTORED FORCE (LBS)
FR-TO		FR-TO	
D-A	-327 / 0	A-C	0 / 0
A-E	0 / 0		
E-B	0 / 0		
C-B	-242 / 0		
D-C	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	11-4	-170	-170	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'TL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.19/1.00 (A-B:1), BC=0.06/1.00 (C-D:4),
WB=0.00/1.00 (A-C:1), SSI=0.14/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



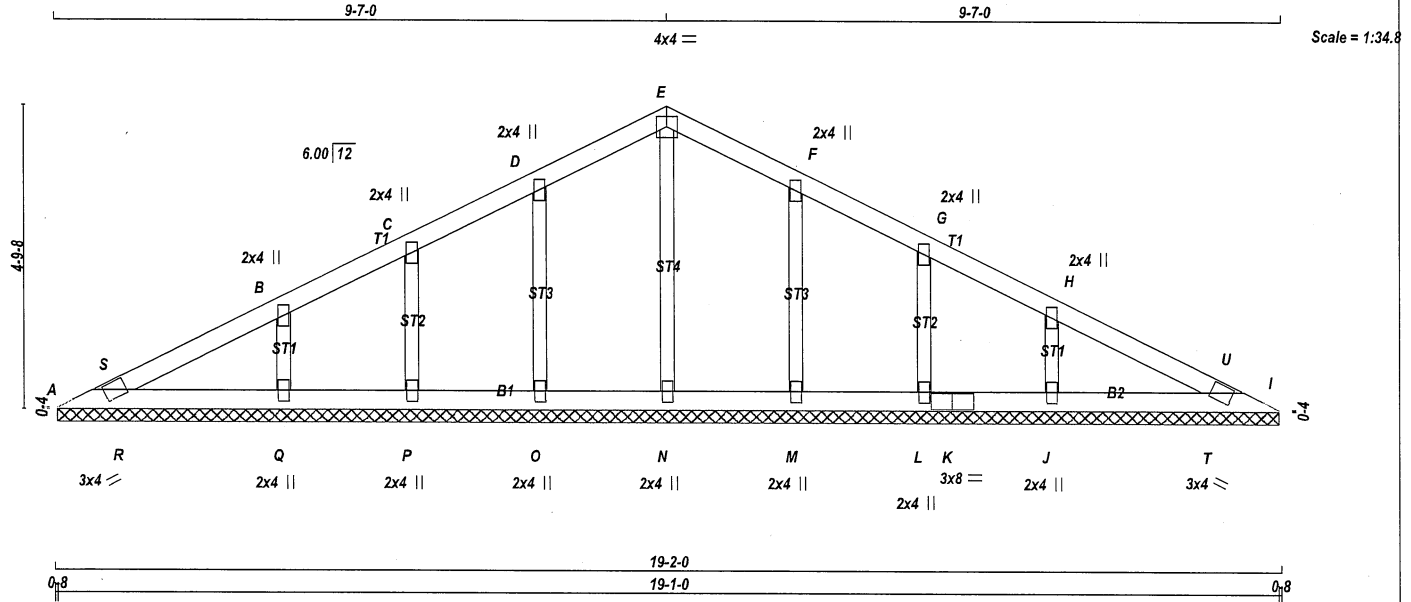
Structural component only
DWG# T-2216674

REVIEWED

JOB NAME 423667	TRUSS NAME V1	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 59 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
A - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B, C, D, F, G, H						
E	TMW+w	MT20	2.0	4.0		
B	TTW-p	MT20	4.0	4.0		
I	TBM1-h	MT20	3.0	4.0		
J, L, M, N, O, P, Q						
K	BMW1+w	MT20	2.0	4.0		
J	BS-t	MT20	3.0	8.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	164	0	164	0	19-1-0 (5-1-4)	19-1-0
I	164	0	164	0	19-1-0 (5-1-4)	19-1-0
N	297	0	297	0	19-1-0 (5-1-4)	19-1-0
O	313	0	313	0	19-1-0 (5-1-4)	19-1-0
P	175	0	175	0	19-1-0 (5-1-4)	19-1-0
Q	449	0	449	0	19-1-0 (5-1-4)	19-1-0
M	313	0	313	0	19-1-0 (5-1-4)	19-1-0
L	175	0	175	0	19-1-0 (5-1-4)	19-1-0
J	449	0	449	0	19-1-0 (5-1-4)	19-1-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	115	82 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
I	115	82 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
N	209	143 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
O	219	158 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
P	122	86 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
Q	315	222 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
M	219	158 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
L	122	86 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
J	315	222 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I, N, O, P, Q, M, L, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO		
A-S	0 / 51	-112.4 -112.4	0.06 (1)	10.00	N-E -265 / 0	0.09 (1)
S-B	0 / 88	-112.4 -112.4	0.14 (1)	10.00	O-D -266 / 0	0.06 (1)
B-C	0 / 67	-112.4 -112.4	0.13 (1)	10.00	P-C -175 / 0	0.03 (1)
C-D	0 / 93	-112.4 -112.4	0.09 (1)	10.00	Q-B -326 / 0	0.05 (1)
D-E	0 / 81	-112.4 -112.4	0.08 (1)	10.00	M-F -266 / 0	0.06 (1)
E-F	0 / 81	-112.4 -112.4	0.08 (1)	10.00	L-G -175 / 0	0.03 (1)
F-G	0 / 93	-112.4 -112.4	0.09 (1)	10.00	J-H -326 / 0	0.05 (1)
G-H	0 / 67	-112.4 -112.4	0.13 (1)	10.00	R-S -105 / 3	0.00 (1)
H-U	0 / 88	-112.4 -112.4	0.14 (1)	10.00	T-U -105 / 3	0.00 (1)
U-I	0 / 51	-112.4 -112.4	0.06 (1)	10.00		
A-R	-70 / 0	-18.5 -18.5	0.11 (1)	6.25		
R-Q	-59 / 0	-18.5 -18.5	0.11 (1)	6.25		
Q-P	-73 / 0	-18.5 -18.5	0.07 (1)	6.25		
P-O	-78 / 0	-18.5 -18.5	0.02 (4)	6.25		
O-N	-83 / 0	-18.5 -18.5	0.02 (4)	6.25		
N-M	-83 / 0	-18.5 -18.5	0.02 (4)	6.25		
M-L	-78 / 0	-18.5 -18.5	0.02 (4)	6.25		
L-K	-73 / 0	-18.5 -18.5	0.07 (1)	6.25		
K-J	-73 / 0	-18.5 -18.5	0.07 (1)	6.25		
J-T	-59 / 0	-18.5 -18.5	0.11 (1)	6.25		
T-I	-70 / 0	-18.5 -18.5	0.11 (1)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (B-S:1), BC=0.11/1.00 (O-R:1), WB=0.09/1.00 (E-N:1), SSI=0.13/1.00 (B-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (E) (INPUT = 0.90)

JSI METAL= 0.14 (B) (INPUT = 1.00)



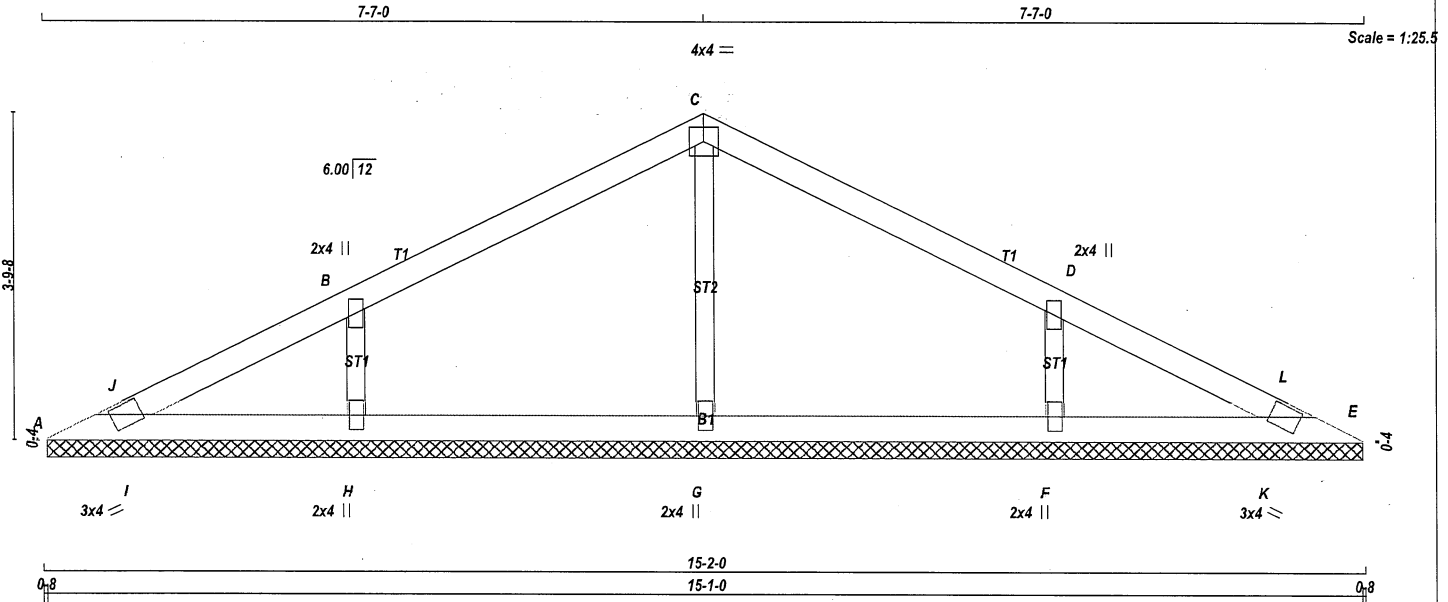
Structural component only
DWG# T-2216631

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423667	V2	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 40 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1+w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	154	0	154	0	15-1-0	15-1-0
E	154	0	154	0	15-1-0	15-1-0
G	468	0	468	0	15-1-0	15-1-0
H	600	0	600	0	15-1-0	15-1-0
F	600	0	600	0	15-1-0	15-1-0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
A	108	78 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	108	78 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
G	330	220 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
H	419	303 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0
F	419	303 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	UNBRAC LENGTH	FR-TO			
A-J	0 / 12	-112.4 -112.4	0.06 (1)	10.00	G-C	-405 / 0	0.09 (1)
J-B	0 / 65	-112.4 -112.4	0.27 (1)	10.00	H-B	-499 / 0	0.07 (1)
B-C	0 / 26	-112.4 -112.4	0.26 (1)	10.00	F-D	-499 / 0	0.07 (1)
C-D	0 / 26	-112.4 -112.4	0.26 (1)	10.00	I-J	-53 / 6	0.00 (1)
D-L	0 / 65	-112.4 -112.4	0.27 (1)	10.00	K-L	-53 / 6	0.00 (1)
L-E	0 / 12	-112.4 -112.4	0.06 (1)	10.00			
A-I	-27 / 0	-18.5 -18.5	0.07 (1)	6.25			
I-H	-20 / 0	-18.5 -18.5	0.07 (1)	6.25			
H-G	-42 / 0	-18.5 -18.5	0.06 (4)	6.25			
G-F	-42 / 0	-18.5 -18.5	0.06 (4)	6.25			
F-K	-20 / 0	-18.5 -18.5	0.07 (1)	6.25			
K-E	-27 / 0	-18.5 -18.5	0.07 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (B-J:1), BC=0.07/1.00 (H-I:1), WB=0.09/1.00 (C-G:1), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (D) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)



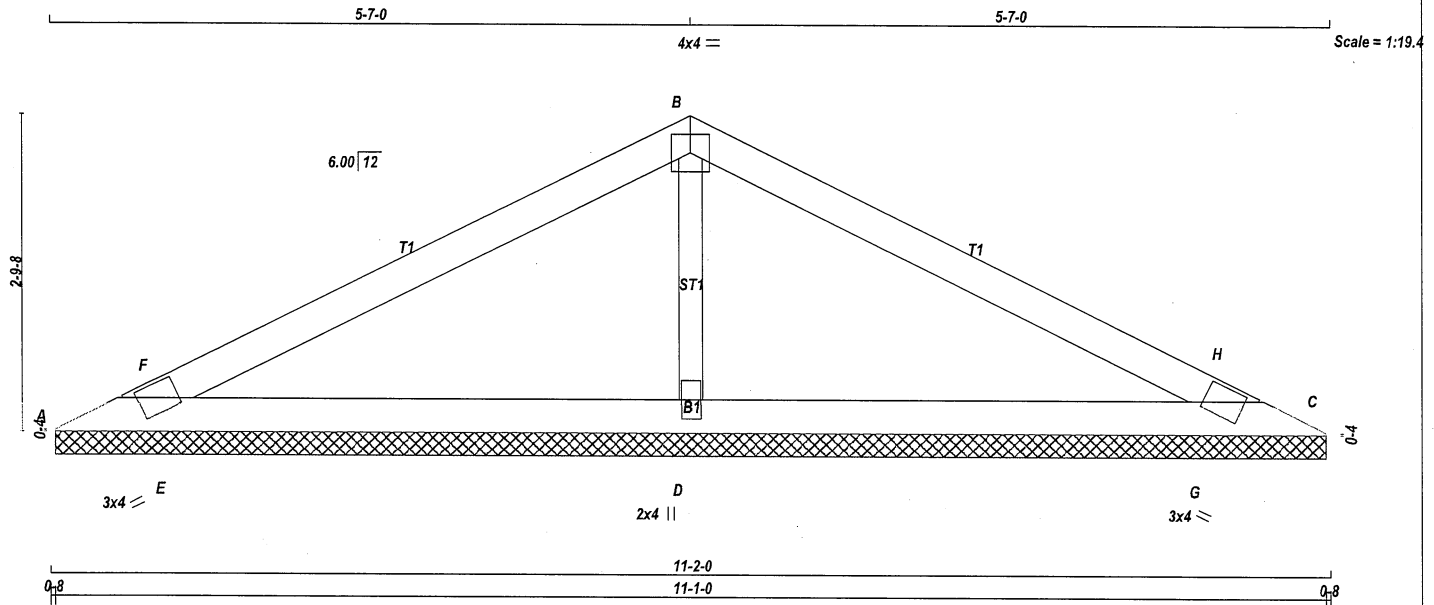
Structural component only
DWG# T-2216632

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423667	V3	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 11:54:22 2022 Page 1
ID:Wf1py42Pek5E0xRWzZrcVUzyU_V-TPgh3VXmDxN_PglZsplmMbF4nyiwnvcQ1MeWZz_8vF



TOTAL WEIGHT = 27 lb [M]

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF
CHORDS				
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TTW-p	MT20	4.0	4.0	
C	TBM1-h	MT20	3.0	4.0	
D	BMW1+w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG
A	41	0	41	0	0	11-1-0	11-1-0		
C	41	0	41	0	0	11-1-0	11-1-0		
D	1368	0	1368	0	0	11-1-0	11-1-0		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	29	19/0	0/0	0/0	0/0	11/0	0/0
C	29	19/0	0/0	0/0	0/0	11/0	0/0
D	958	683/0	0/0	0/0	0/0	276/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-F	0/711	-112.4 -112.4	0.23 (1)	D-B	-1109/0	0.19 (1)	
F-B	0/708	-112.4 -112.4	0.45 (1)	E-F	-328/0	0.00 (1)	
B-H	0/708	-112.4 -112.4	0.45 (1)	G-H	-328/0	0.00 (1)	
H-C	0/711	-112.4 -112.4	0.23 (1)				
A-E	-676/0	-18.5 -18.5	0.29 (1)				
E-D	-641/0	-18.5 -18.5	0.29 (1)				
D-G	-641/0	-18.5 -18.5	0.29 (1)				
G-C	-676/0	-18.5 -18.5	0.29 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.45/1.00 (B-F:1), BC=0.29/1.00 (D-E:1), WB=0.19/1.00 (B-D:1), SSI=0.19/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)



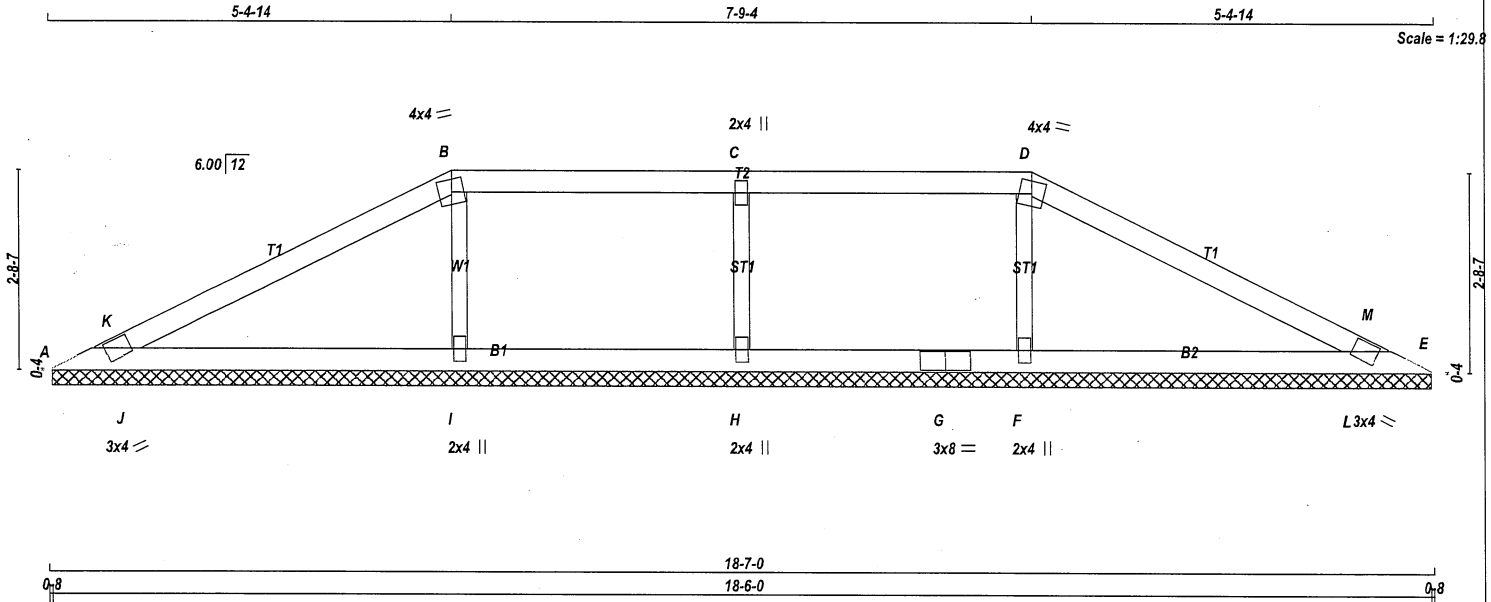
Structural component only
DWG# T-2216633

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423670	V30	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Fri Jul 8 12:10:35 2022 Page 1
ID:zoSDmrmKtN2Qs9fEkwL?SkzySAg-UbzWHCI54uzweBBZMfxHm2D_01VOs1qAvYeAfz_8g2



TOTAL WEIGHT = 47 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW-m	MT20	4.0	4.0		
C	TMW-w	MT20	2.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, H, I						
F	BMW1-w	MT20	2.0	4.0		
G	BS-t	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG
	VERT	HORZ	DOWN			IN-SX	IN-SX
A	111	0	111	0	0	18-6-0 (11-11)	18-6-0
E	111	0	111	0	0	18-6-0 (11-11)	18-6-0
F	832	0	832	0	0	18-6-0 (11-11)	18-6-0
H	537	0	537	0	0	18-6-0 (11-11)	18-6-0
I	832	0	832	0	0	18-6-0 (11-11)	18-6-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	78	55 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
E	78	55 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
F	584	407 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
H	374	278 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
I	584	407 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, F, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-K	0 / 536	-112.4	-112.4	0.15 (1)	10.00	F-D -642 / 0	0.11 (1)
K-B	0 / 544	-112.4	-112.4	0.42 (1)	10.00	H-C -549 / 0	0.09 (1)
B-C	0 / 518	-112.4	-112.4	0.37 (1)	10.00	I-B -642 / 0	0.11 (1)
C-D	0 / 518	-112.4	-112.4	0.37 (1)	10.00	J-K -288 / 0	0.00 (1)
D-M	0 / 544	-112.4	-112.4	0.42 (1)	10.00	L-M -288 / 0	0.00 (1)
M-E	0 / 536	-112.4	-112.4	0.15 (1)	10.00		
A-J	-522 / 0	-18.5	-18.5	0.28 (1)	6.25		
J-I	-490 / 0	-18.5	-18.5	0.28 (1)	6.25		
I-H	-518 / 0	-18.5	-18.5	0.18 (1)	6.25		
H-G	-518 / 0	-18.5	-18.5	0.18 (1)	6.25		
G-F	-518 / 0	-18.5	-18.5	0.18 (1)	6.25		
F-L	-490 / 0	-18.5	-18.5	0.28 (1)	6.25		
L-E	-522 / 0	-18.5	-18.5	0.28 (1)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.42/1.00 (D-M:1), BC=0.28/1.00 (F-L:1), WB=0.11/1.00 (D-F:1), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL = 0.29 (D) (INPUT = 1.00)



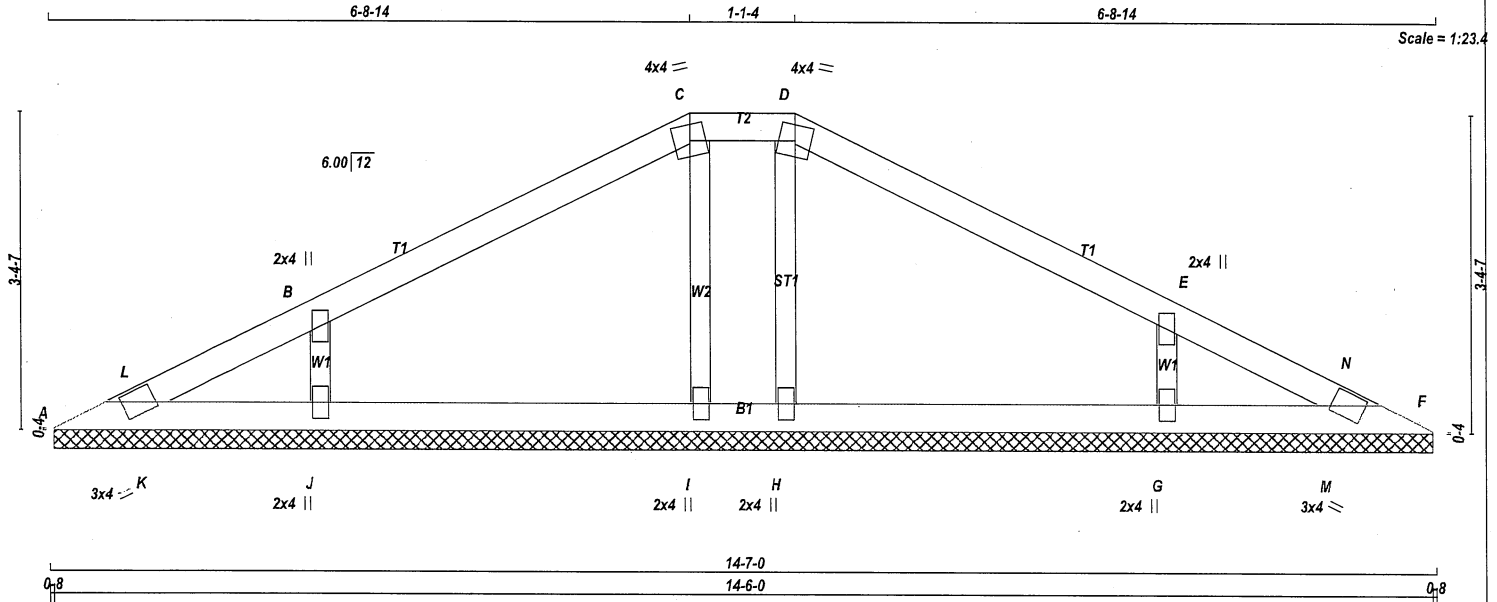
Structural component only
DWG# T-2216645

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423670	V31	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 39 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
A - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TBM1-h	MT20	3.0	4.0		
G, H, I, J						
G	BMW1+w	MT20	2.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
A	120	0	120	0	0	0	14-6-0	14-6-0	14-6-0
F	120	0	120	0	0	0	14-6-0	14-6-0	14-6-0
H	283	0	283	0	0	0	14-6-0	14-6-0	14-6-0
I	283	0	283	0	0	0	14-6-0	14-6-0	14-6-0
G	546	0	546	0	0	0	14-6-0	14-6-0	14-6-0
J	546	0	546	0	0	0	14-6-0	14-6-0	14-6-0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	84	60 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
F	84	60 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
H	199	137 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
I	199	137 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
G	382	274 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
J	382	274 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, H, I, G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-L	-44 / 0	-112.4 -112.4	0.05 (1)	H-D	-240 / 0	0.05 (1)	
L-B	0 / 19	-112.4 -112.4	0.24 (1)	I-C	-240 / 0	0.05 (1)	
B-C	-23 / 0	-112.4 -112.4	0.23 (1)	G-E	-468 / 0	0.07 (1)	
C-D	0 / 5	-112.4 -112.4	0.02 (1)	J-B	-468 / 0	0.07 (1)	
D-E	-23 / 0	-112.4 -112.4	0.23 (1)	K-L	-8 / 5	0.00 (1)	
E-N	0 / 19	-112.4 -112.4	0.24 (1)	M-N	-8 / 5	0.00 (1)	
N-F	-44 / 0	-112.4 -112.4	0.05 (1)				
A-K	0 / 30	-18.5 -18.5	0.03 (1)				
K-J	0 / 31	-18.5 -18.5	0.05 (4)				
J-I	-3 / 3	-18.5 -18.5	0.05 (4)				
I-H	-5 / 0	-18.5 -18.5	0.05 (4)				
H-G	-3 / 3	-18.5 -18.5	0.05 (4)				
G-M	0 / 31	-18.5 -18.5	0.05 (4)				
M-F	0 / 30	-18.5 -18.5	0.03 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (B-L:1), BC=0.05/1.00 (I-J:4), WB=0.07/1.00 (E-G:1), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (E) (INPUT = 0.90)
JSI METAL= 0.19 (E) (INPUT = 1.00)



Structural component only
DWG# T-2216646

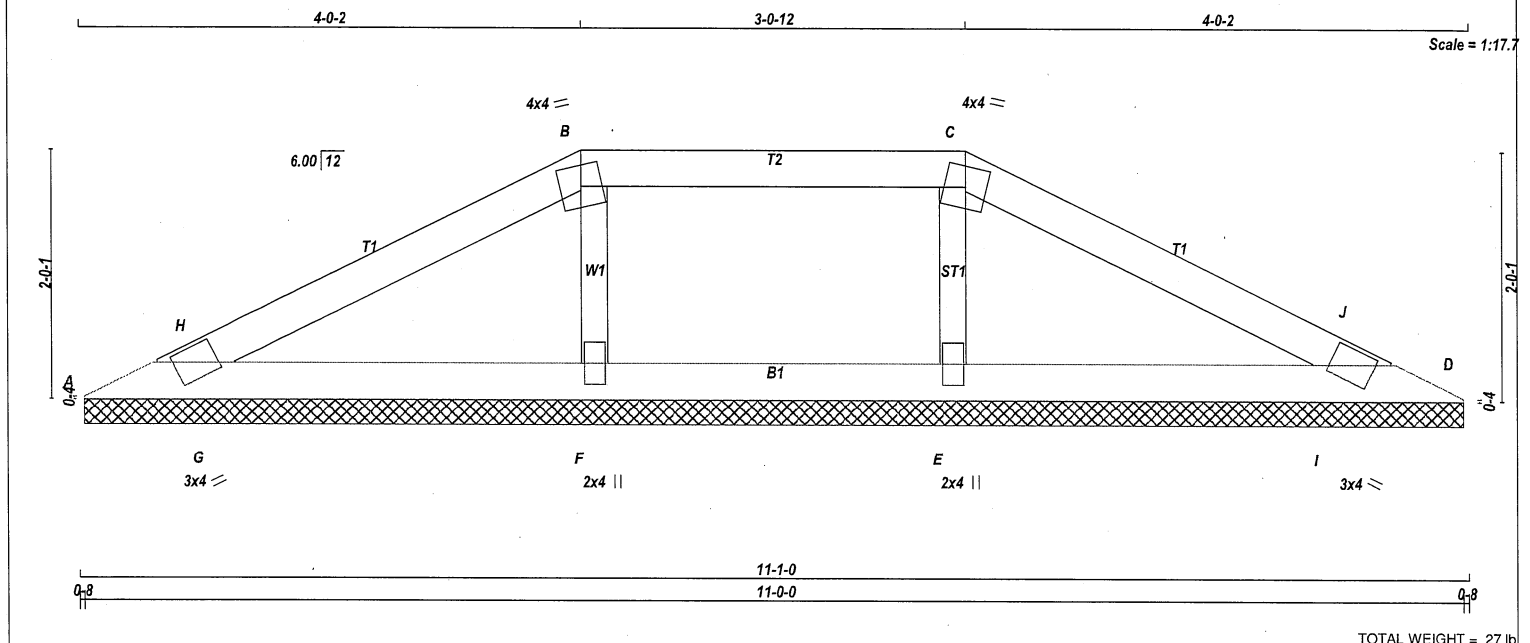
REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423673	V40	1	1	TRUSS DESC.		

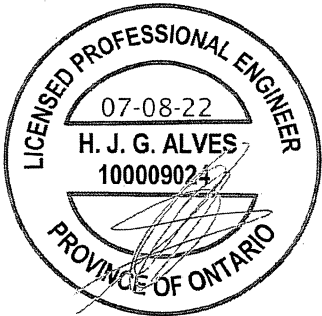
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Fri Jul 8 12:26:50 2022 Page 1

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - B 2x4 DRY No.2 SPF B - C 2x4 DRY No.2 SPF C - D 2x4 DRY No.2 SPF A - D 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.				PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TBM1-h MT20 3.0 4.0 B TTW-m MT20 4.0 4.0 C TTW-m MT20 4.0 4.0 D TBM1-h MT20 3.0 4.0 E BMW1+w MT20 2.0 4.0 F BMW1+w MT20 2.0 4.0				NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.			
DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.23/1.00 (B-C-1), BC=0.16/1.00 (E-I-1), WB=0.07/1.00 (B-F-1), SSI=0.13/1.00 (B-C-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 798 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.37 (B) (INPUT = 0.90) JSI METAL= 0.20 (B) (INPUT = 1.00)				BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX A 115 0 115 0 0 11-0-0 11-0-0 D 115 0 115 0 0 11-0-0 11-0-0 E 605 0 605 0 0 11-0-0 11-0-0 F 605 0 605 0 0 11-0-0 11-0-0 UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL A 81 58 / 0 0 / 0 0 / 0 23 / 0 0 / 0 D 81 58 / 0 0 / 0 0 / 0 23 / 0 0 / 0 E 424 300 / 0 0 / 0 0 / 0 124 / 0 0 / 0 F 424 300 / 0 0 / 0 0 / 0 124 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, D, E, F BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) C H O R D S W E B S MEMB. MAX. FACTORED FORCE FACTORED VERT. LOAD LC1 MAX. MEMB. MAX. FACTORED FORCE MAX. FACTORED FORCE MAX. FACTORED FORCE (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) (LBS) CSI (LC) FR-TO FROM TO A-H 0 / 327 -112.4 -112.4 0.08 (1) 10.00 E-C -496 / 0 0.07 (1) H-B 0 / 347 -112.4 -112.4 0.23 (1) 10.00 F-B -496 / 0 0.07 (1) B-C 0 / 340 -112.4 -112.4 0.23 (1) 10.00 G-H -171 / 0 0.00 (1) C-J 0 / 347 -112.4 -112.4 0.23 (1) 10.00 I-J -171 / 0 0.00 (1) J-D 0 / 327 -112.4 -112.4 0.08 (1) 10.00 A-G -328 / 0 -18.5 -18.5 0.16 (1) 6.25 G-F -309 / 0 -18.5 -18.5 0.16 (1) 6.25 F-E -340 / 0 -18.5 -18.5 0.08 (1) 6.25 E-I -309 / 0 -18.5 -18.5 0.16 (1) 6.25 I-D -328 / 0 -18.5 -18.5 0.16 (1) 6.25				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.23/1.00 (B-C-1), BC=0.16/1.00 (E-I-1), WB=0.07/1.00 (B-F-1), SSI=0.13/1.00 (B-C-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 798 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.37 (B) (INPUT = 0.90) JSI METAL= 0.20 (B) (INPUT = 1.00)			



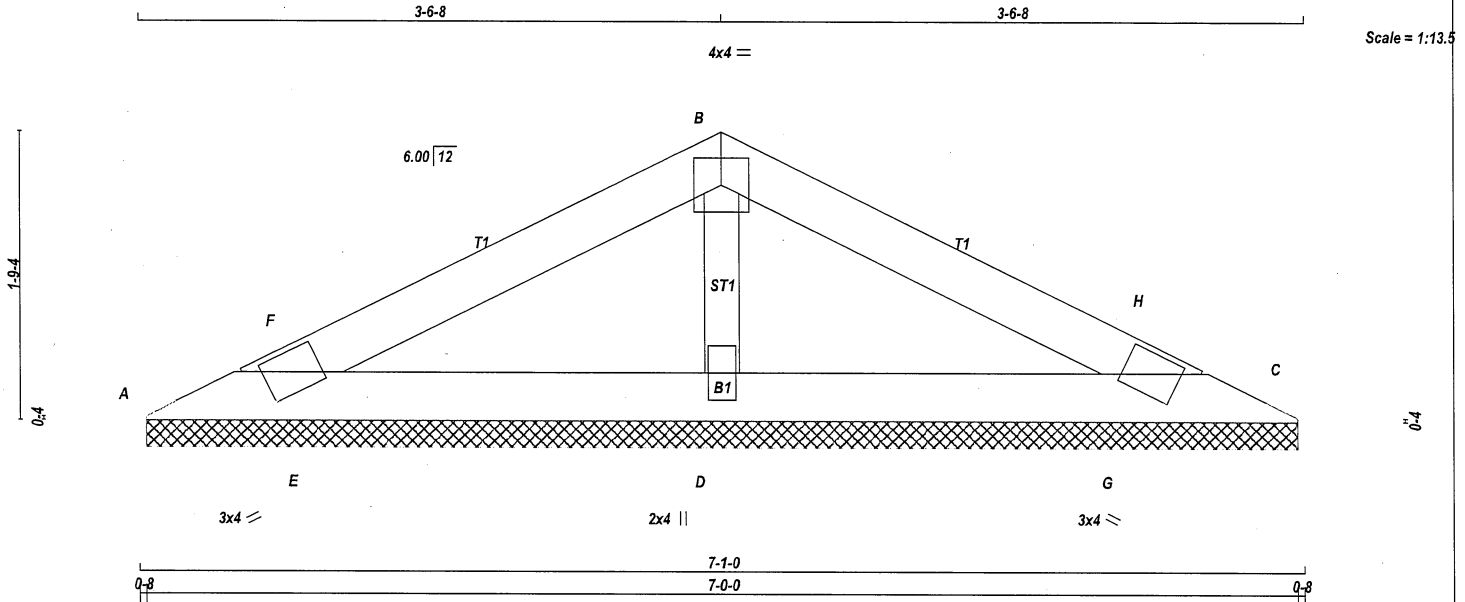
Structural component only
DWG# T-2216675

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423673	V41	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 16 lb

LUMBER					DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER			
A - B	2x4	DRY	No.2	SPF		
B - C	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW-p	MT20	4.0	4.0		
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
A	106	0	106	0	0	7-0-0	7-0-0	7-0-0	7-0-0
C	106	0	106	0	0	7-0-0	7-0-0	7-0-0	7-0-0
D	705	0	705	0	0	7-0-0	7-0-0	7-0-0	7-0-0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	74	53 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
C	74	53 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
D	494	349 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE	VERT.	LOAD	MAX.	MEMB.	FORCE	MAX.	MEMB.	FORCE
FR-TO	(LBS)	FROM	TO	CSI (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CSI (LC)
A-F	0 / 248	-112.4	-112.4	0.06 (1)	10.00	D-B	-509 / 0	0.07 (1)	
F-B	0 / 260	-112.4	-112.4	0.16 (1)	10.00	E-F	-167 / 0	0.00 (1)	
B-H	0 / 260	-112.4	-112.4	0.16 (1)	10.00	G-H	-167 / 0	0.00 (1)	
H-C	0 / 248	-112.4	-112.4	0.06 (1)	10.00				
A-E	-247 / 0	-18.5	-18.5	0.13 (1)	6.25				
E-D	-232 / 0	-18.5	-18.5	0.13 (1)	6.25				
D-G	-232 / 0	-18.5	-18.5	0.13 (1)	6.25				
G-C	-247 / 0	-18.5	-18.5	0.13 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (B-F:1), BC=0.13/1.00 (D-G:1), WB=0.07/1.00 (B-D:1), SSI=0.10/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)



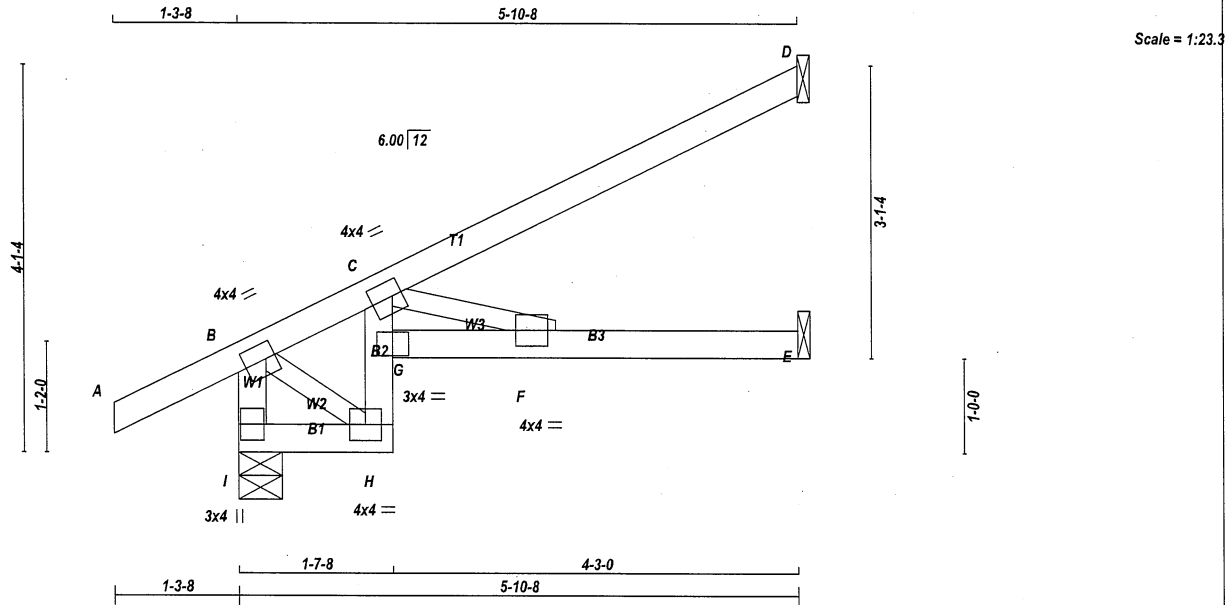
Structural component only
DWG# T-2216676

REVIEWED

REVIEWED

JOB NAME 423665	TRUSS NAME J1S	QUANTITY 5	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 5 X 21 = 104 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
I - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - C	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TMVW-t	MT20	4.0	4.0	2.00 1.25
F	BMV-t	MT20	4.0	4.0	
G	BMV-t	MT20	3.0	4.0	
H	BMVW-t	MT20	4.0	4.0	
I	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	COMBINED	GROSS REACTION	SNOW	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
I	555	0	555	0	0	5-8	5-8		
D	242	0	242	0	0	1-8	1-8		
E	125	0	125	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	386	289 / 0	0 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0	0 / 0
D	167	135 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0	0 / 0
E	91	45 / 0	0 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE	VERT.	LOAD LC1	MAX	MAX	MEMB.	FORCE	MAX	MAX
FR-TO	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CSI (LC)	(LC)
I-B	-541 / 0	0.0	0.0	0.05 (1)	7.81	B-H	0 / 268	0.06 (1)	
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	C-F	-662 / 0	0.10 (1)	
B-C	-284 / 0	-112.4	-112.4	0.14 (1)	6.25				
C-D	-2 / 3	-112.4	-112.4	0.35 (1)	10.00				
I-H	0 / 0	-18.5	-18.5	0.01 (4)	10.00				
H-G	-121 / 0	0.0	0.0	0.32 (1)	7.81				
G-C	0 / 67	0.0	0.0	0.33 (1)	10.00				
G-F	0 / 620	-18.5	-18.5	0.48 (1)	10.00				
F-E	0 / 0	-18.5	-18.5	0.38 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/954 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/496 (0.14")

CSI: TC=0.35/1.00 (C-D:1), BC=0.48/1.00 (F-G:1), WB=0.10/1.00 (C-F:1), SSI=0.30/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (C) (INPUT = 0.90)
JSI METAL= 0.23 (F) (INPUT = 1.00)

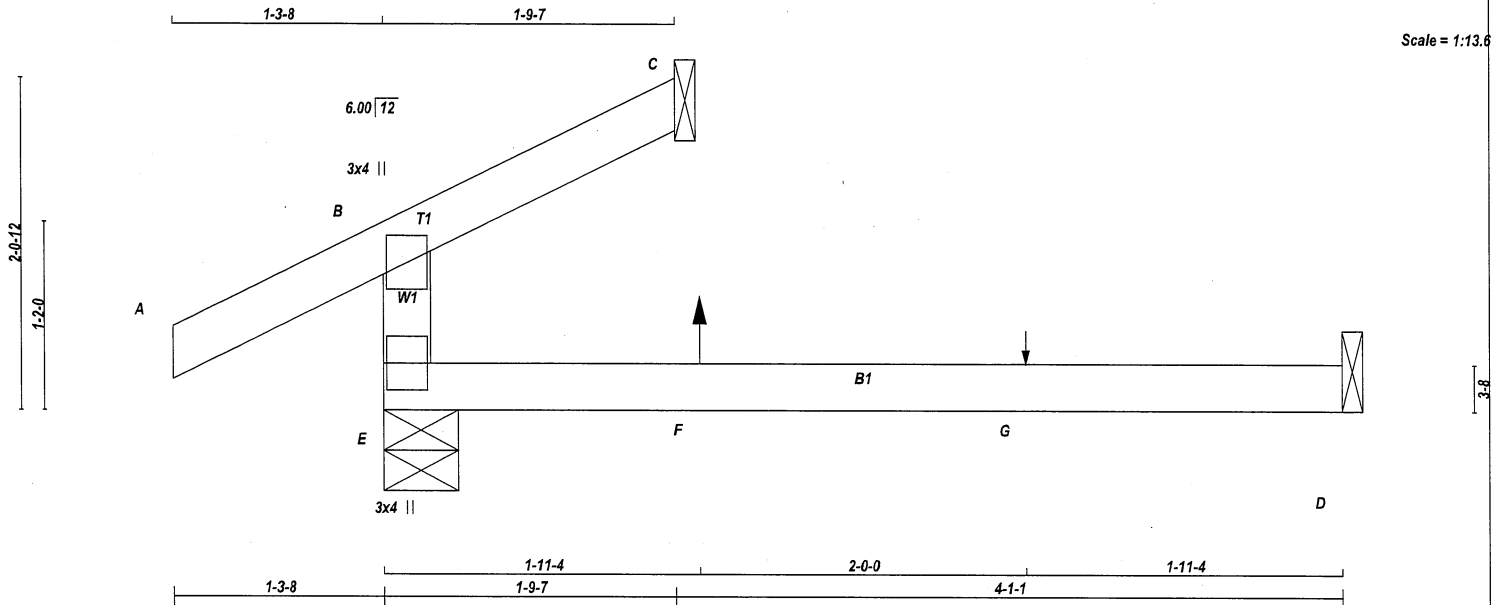


Structural component only
DWG# T-2216591

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	J3	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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Scale = 1:13.6

TOTAL WEIGHT = 12 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
E	BMV1+p	MT20	3.0	4.0			

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	340	0	340	0	5-8	5-8
C	71	0	71	0	1-8	1-8
D	43	0	52	0	1-8	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	237	175 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	52	26 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS		FACTORED		W E B S	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						
E-B	-284 / 0	0.0	0.0	0.11 (4)	7.81	
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	
B-C	-14 / 9	-112.4	-112.4	0.08 (4)	6.25	
E-F	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
F-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
G-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	6	1	10	BACK	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.15/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

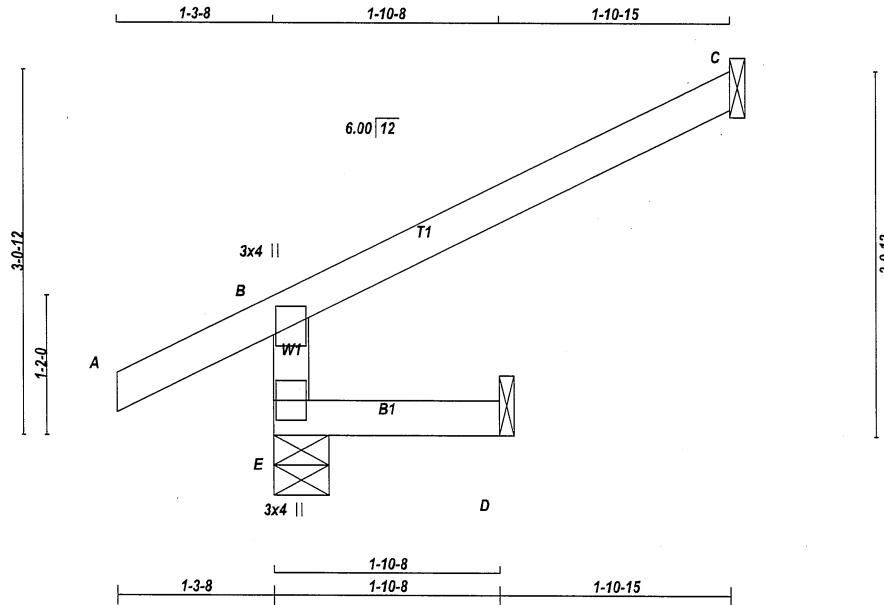


Structural component only
DWG# T-2216593

REVIEWED

JOB NAME 423665	TRUSS NAME J4	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:18.4

TOTAL WEIGHT = 10 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD. BRG
JT	VERT	HORZ	DOWN	HORZ
E	438	0	438	0
C	160	0	160	0
D	16	0	17	0

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	302	242 / 0	0 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	109	92 / 0	0 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE	FACTORED VERT. LOAD	FACTORED HORIZ. LOAD	MAX. FACTORED FORCE	MAX. FACTORED FORCE	MAX. FACTORED FORCE	MAX. FACTORED FORCE
MEMB.	(LBS)	(PLF)	(LC)	MEMB.	(LBS)	(LBS)	(LBS)
FR-TO		FROM	TO	FR-TO			
E-B	-418 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00		
B-C	-24 / 0	-112.4	-112.4	0.27 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.27/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



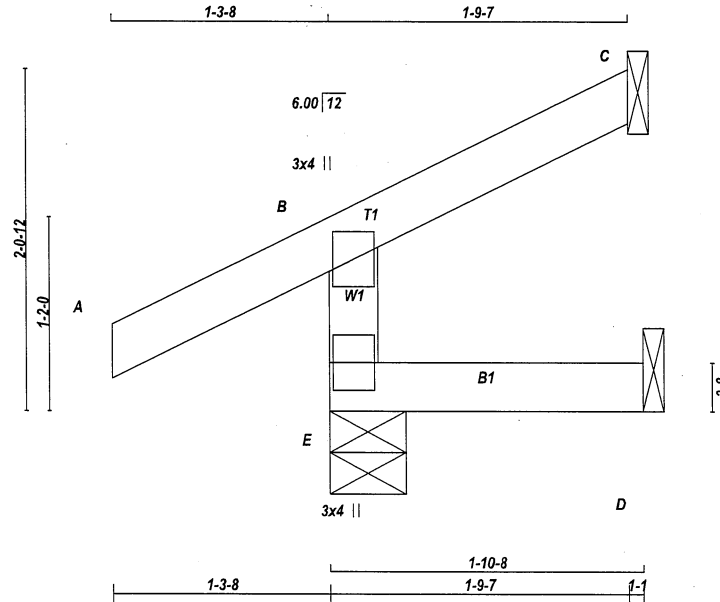
Structural component only
DWG# T-2216594

REVIEWED

JOB NAME 423665	TRUSS NAME J5	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:13.3

TOTAL WEIGHT = 7 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	328	0	328	0	5-8	5-8
C	54	0	54	0	1-8	1-8
D	6	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	226	179 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	37	31 / -23	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	6	0 / -10	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO				
E-B	-299 / 0	0.0	0.0	0.05 (5)	7.81	
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	
B-C	-21 / 0	-112.4	-112.4	0.11 (1)	6.25	
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

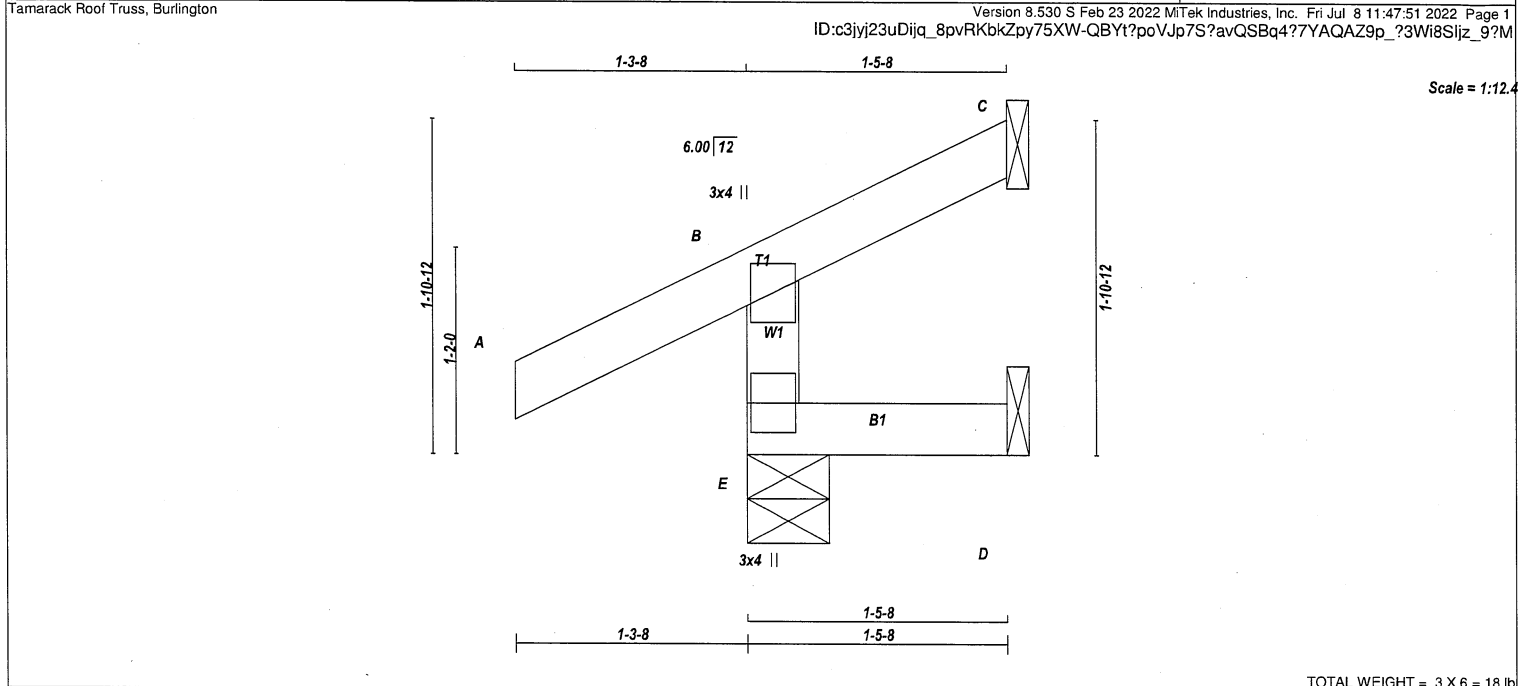
JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2216595

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	J6	3	1	TRUSS DESC.		



LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.				SPF	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2216596

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG		
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	
E	319	0	319	0	0	5-8	5-8		
C	27	0	27	0	-42	1-8	1-8		
D	-2	0	12	0	-11	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	220	176 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
C	19	15 / -29	0 / 0	0 / 0	0 / 0	3 / 0	0 / 0
D	-0	0 / -12	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	CS1 (LC)
FR-TO						FR-TO			
E-B	-290 / 0	0.0	0.0	0.04 (5)	7.81				
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00				
B-C	-25 / 0	-112.4	-112.4	0.11 (1)	6.25				
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SS=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

REVIEWED

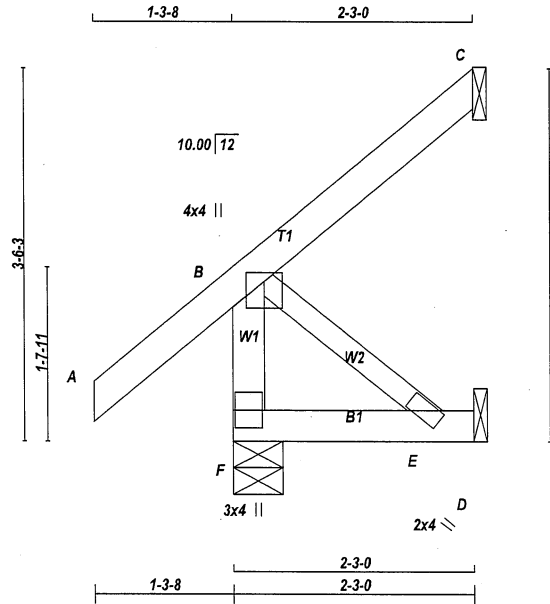


Structural component only
DWG# T-2216596

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423665	J7	3	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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Scale = 1:20.8

TOTAL WEIGHT = 3 X 11 = 33 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
F	303	0	0	303	0	5-8	5-8		
C	126	0	0	126	0	1-8	1-8		
D	21	0	0	23	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	210	163 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	87	73 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
D	17	0 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED				WEBS		FACTORED	
MEMB.	FORCE	VERT.	LOAD	LC1	MAX	MEMB.	FORCE	MAX	CS1 (LC)
FR-TO	(LBS)	FROM	TO	CS1 (LC)	UNBRAC	FR-TO	(LBS)	CS1 (LC)	
F-B	-282 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00				
B-C	0 / 0	-112.4	-112.4	0.10 (1)	10.00				
F-E	0 / 0	-18.5	-18.5	0.03 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.03/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

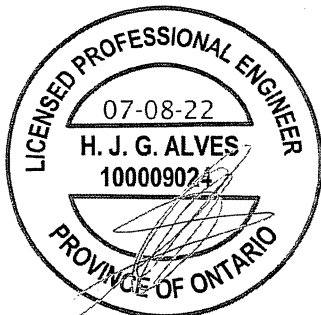
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2216597

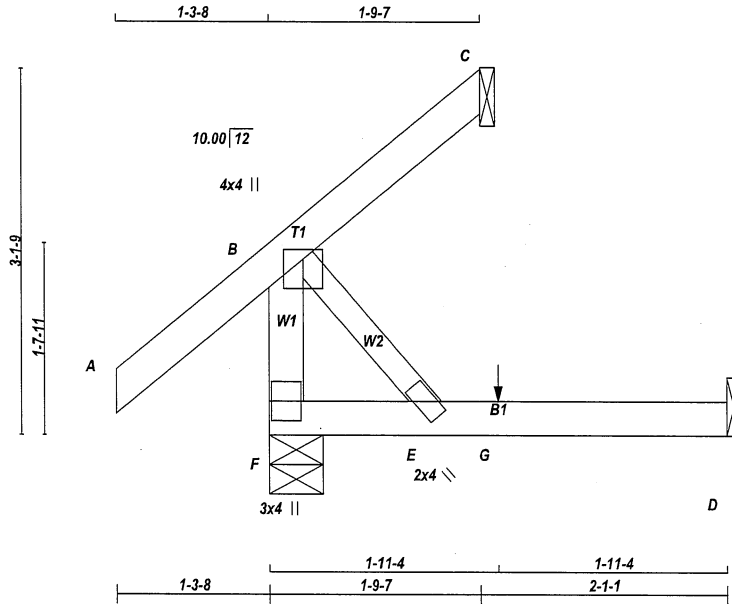
REVIEWED

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	J9	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:18.8

TOTAL WEIGHT = 12 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
F	352	0	352	0
C	40	0	40	0
D	36	0	40	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	COMBINED						
F	245	183 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	27	23 / -35	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (PLF)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
F-B	-317 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 50	-112.4	-112.4	0.17 (5)	10.00				
B-C	-39 / 0	-112.4	-112.4	0.16 (5)	6.25				
F-E	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
E-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
G-D	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.17/1.00 (A-B:5), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	371	1747	788
	1987	1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)

JSI METAL= 0.06 (B) (INPUT = 1.00)

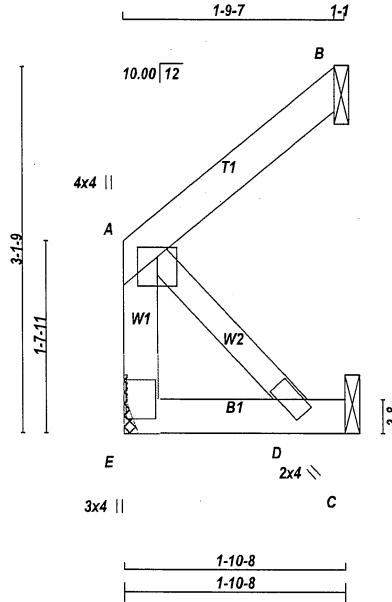


Structural component only
DWG# T-2216599

REVIEWED

JOB NAME 423665	TRUSS NAME J10	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:18.8

TOTAL WEIGHT = 8 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	118	0	118	0	MECHANICAL	
B	100	0	100	0	1-8	1-8
C	17	0	19	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	83	58 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	69	58 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
C	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		W E B S		FACTORED	
	MAX.	FORCE	VERT.	LOAD	MAX.	MEMB.	MAX.	FORCE
FR-TO		(LBS)	PLF	LC1	CS1 (LC)	UNBRAC		(LBS)
E-A	-100	/ 0	0.0	0.0	0.01 (1)	7.81	A-D	0 / 0
A-B	0 / 0		-112.4	-112.4	0.06 (1)	10.00		0.00 (1)
E-D	0 / 0		-18.5	-18.5	0.02 (4)	10.00		
D-C	0 / 0		-18.5	-18.5	0.01 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.06/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (A-D:1), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



Structural component only
DWG# T-2216600

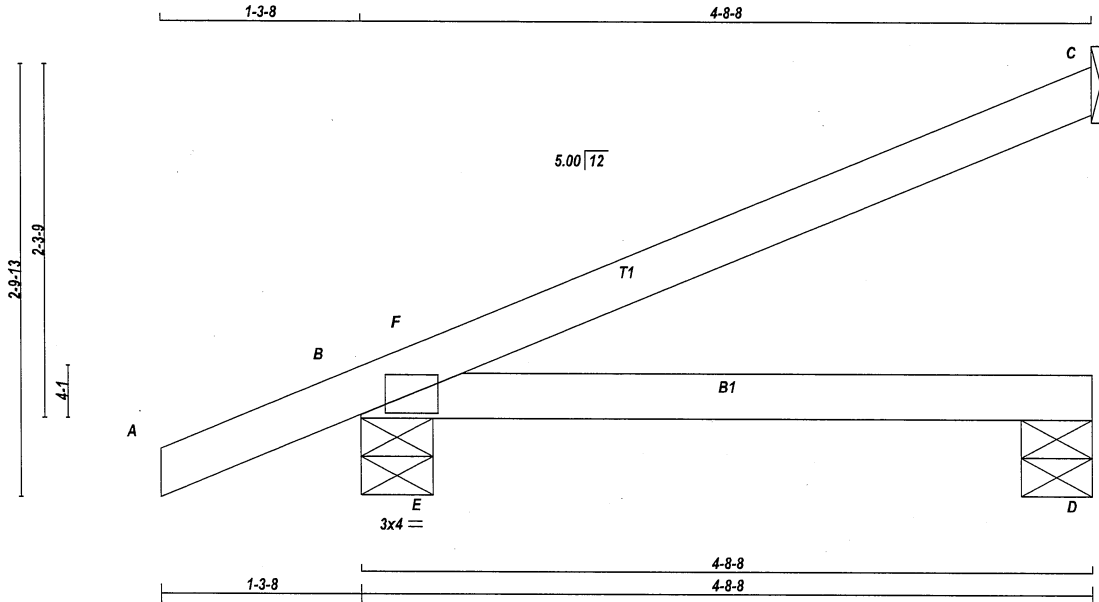
REVIEWED

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423665	J12	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:14.3

TOTAL WEIGHT = 2 X 13 = 26 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF
CHORDS				
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-1	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
C	229	0	229	0	0	1-8	1-8	
B	460	0	460	0	0	5-8	5-8	
D	79	0	79	0	0	5-8	5-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
C	158	129 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
B	320	240 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
D	58	24 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		W E B S		MAX. FACTORED	
	FORCE	VERT. LOAD	LC1	MAX	MEMB.	FORCE	MAX	
FR-TO	(LBS)	FROM TO	PLF	CSI (LC)	UNBRAC	(LBS)	CSI (LC)	
A-B	0 / 28	-112.4	-112.4	0.14 (1)	10.00	E-F	-342 / 8	0.00 (1)
B-F	-19 / 30	-112.4	-112.4	0.05 (1)	6.25			
F-C	-3 / 2	-112.4	-112.4	0.32 (1)	10.00			
B-E	0 / 0	-18.5	-18.5	0.23 (1)	10.00			
E-D	0 / 0	-18.5	-18.5	0.23 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/802 (0.07")

CSI: TC=0.32/1.00 (C-F:1), BC=0.23/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SSI=0.27/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.34 (B) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



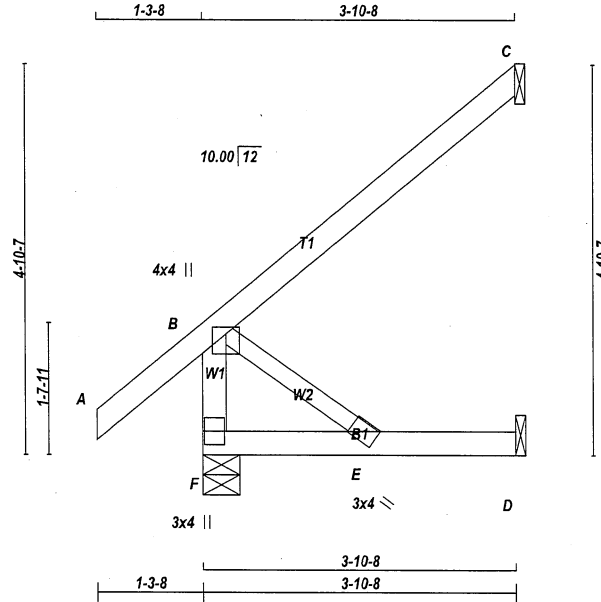
Structural component only
DWG# T-2216602

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423670	J30	8	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale = 1:27.5

TOTAL WEIGHT = 8 X 16 = 125 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF
CHORDS				
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
F	409	0	0	409	0	5-8	5-8		
C	218	0	0	218	0	1-8	1-8		
D	36	0	0	40	0	1-8	1-8		

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
F	284	216 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0		
C	149	126 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0		
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1	MEMB.	FORCE	MAX	MAX
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM	TO	FR-TO			
F-B	-374 / 0	0.0	0.0	0.04 (1)	7.81	0 / 0	0.00 (1)
A-B	0 / 50	-112.4	-112.4	0.16 (5)	10.00		
B-C	0 / 0	-112.4	-112.4	0.29 (1)	10.00		
F-E	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
E-D	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.29/1.00 (B-C:1), BC=0.08/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

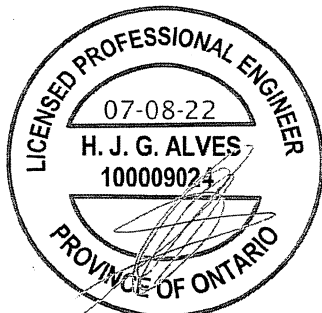
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

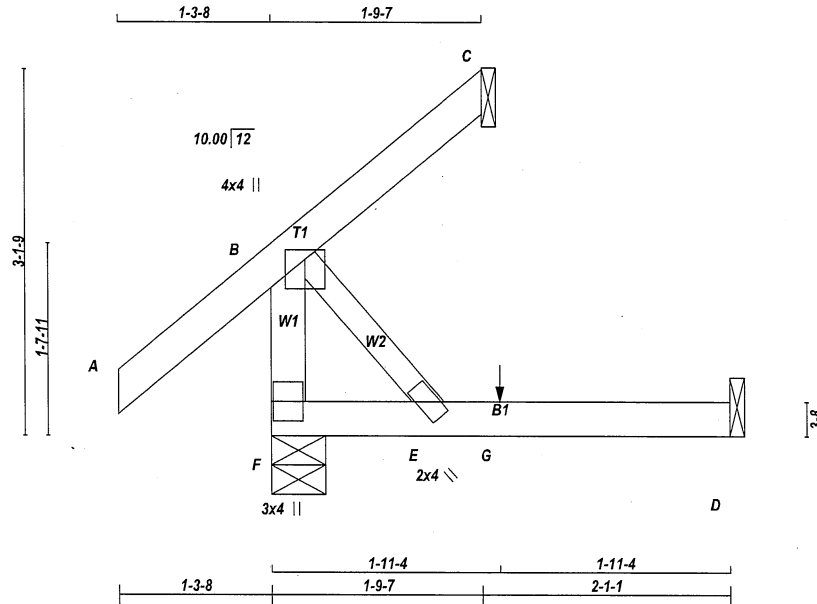


Structural component only
DWG# T-2216634

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423670	J31	4	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 4 X 12 = 48 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
F	352	0	352	0
C	40	0	40	0
D	36	0	40	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	COMBINED						
F	245	183 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	27	23 / -35	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO			
F-B	-316 / 0	0.0	0.0	0.03 (1)	B-E	0 / 0	0.00 (1)	
A-B	0 / 50	-112.4	-112.4	0.16 (5)				
B-C	-39 / 0	-112.4	-112.4	0.16 (5)				
F-E	0 / 0	-18.5	-18.5	0.07 (4)				
E-G	0 / 0	-18.5	-18.5	0.08 (4)				
G-D	0 / 0	-18.5	-18.5	0.08 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	6.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.4	PSF
TOTAL LOAD	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.16/1.00 (A-B:5), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	798
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

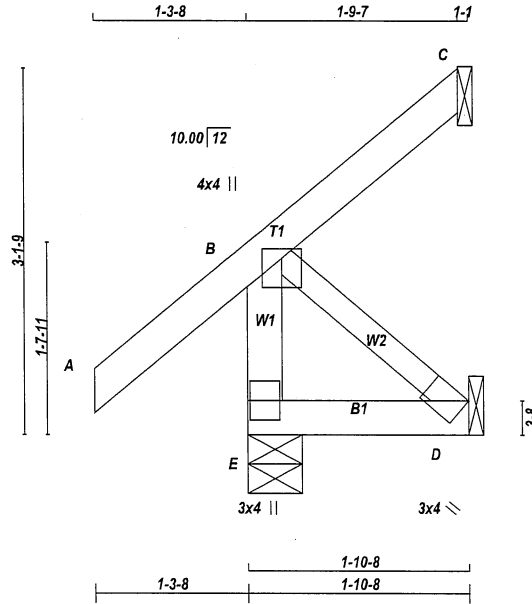


Structural component only
DWG# T-2216635

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423670	J32	4	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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Scale = 1:18.8

TOTAL WEIGHT = 4 X 10 = 40 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	BMW1+w	MT20	3.0	4.0	3.00 Edge
E	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
E	332	0	0	332	0	0	0	5-8	5-8
C	40	0	0	40	0	-51	1-8	1-8	1-8
D	16	0	0	18	0	0	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	229	183 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	27	23 / -35	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 (LC)
FR-TO		FROM	TO	FR-TO			
E-B	-317 / 0	0.0	0.0	0.03 (1)	7.81	B-D	0 / 0
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00		
B-C	-39 / 0	-112.4	-112.4	0.15 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (B-D:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.23 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)

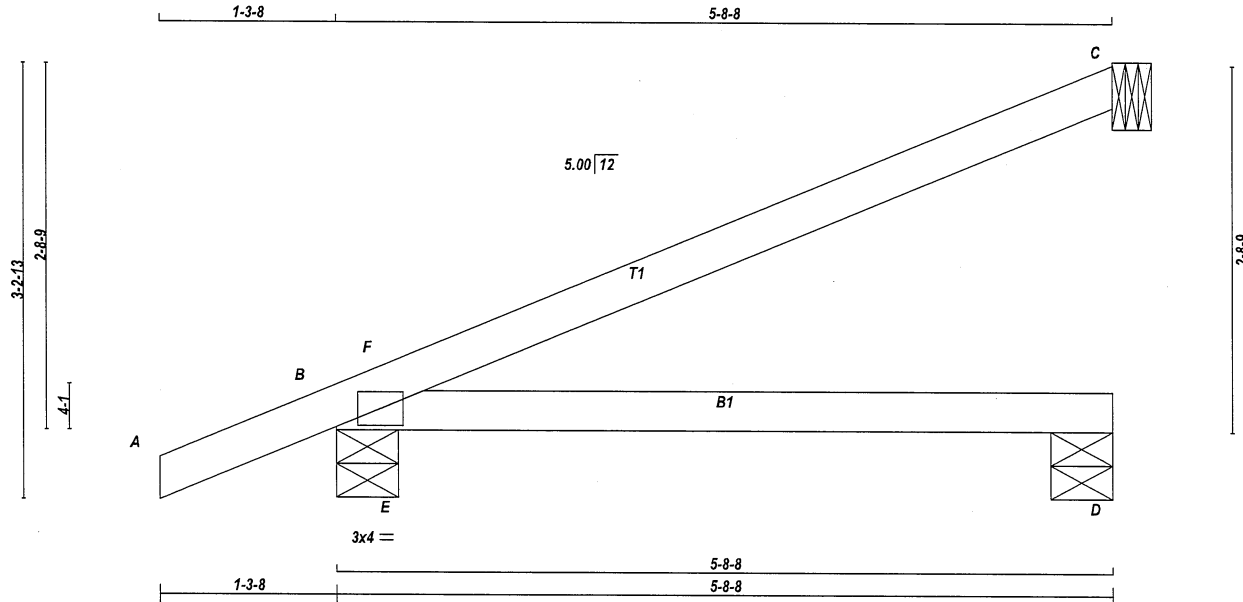


Structural component only
DWG# T-2216636

REVIEWED

JOB NAME 423670	TRUSS NAME J33	QUANTITY 4	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:16.3

TOTAL WEIGHT = 4 X 15 = 61 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP
C	279	0	279	0
B	525	0	525	0
D	94	0	94	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	192	157 / 0	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
B	366	273 / 0	0 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
D	70	28 / 0	0 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	FACTORED HORIZ. LOAD (PLF)
FR-TO						FR-TO			
A-B	0 / 28	-112.4	-112.4	0.14 (1)	10.00	E-F	-487 / 12	0.00 (1)	
B-F	-24 / 66	-112.4	-112.4	0.11 (1)	6.25				
F-C	-5 / 2	-112.4	-112.4	0.47 (1)	10.00				
B-E	0 / 0	-18.5	-18.5	0.33 (1)	10.00				
E-D	0 / 0	-18.5	-18.5	0.33 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/876 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/459 (0.15")

CSI: TC=0.47/1.00 (C-F:1), BC=0.33/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.39/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



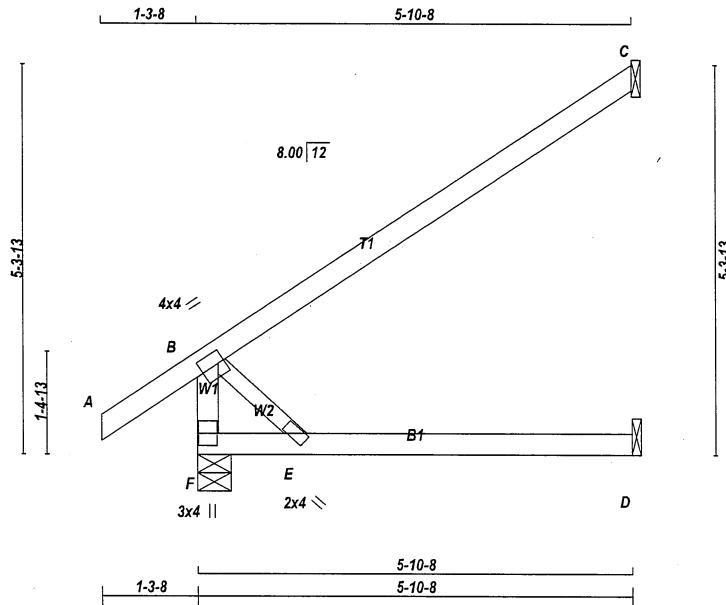
Structural component only
DWG# T-2216637

REVIEWED

JOB NAME 423673	TRUSS NAME J40	QUANTITY 4	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:30.0

TOTAL WEIGHT = 4 X 19 = 76 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	2100F 1.8E	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW-w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
F	539	0	539	0
C	330	0	330	0
D	54	0	61	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX /MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	375	280 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	226	191 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)
FR-TO						FR-TO				
F-B	-485 / 0	0.0	0.0	0.05 (1)	7.81	B-E	0 / 0	0.00 (1)		
A-B	0 / 43	-112.4	-112.4	0.10 (1)	10.00					
B-C	0 / 0	-112.4	-112.4	0.44 (1)	10.00					
F-E	0 / 0	-18.5	-18.5	0.13 (4)	10.00					
E-D	0 / 0	-18.5	-18.5	0.19 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.44/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

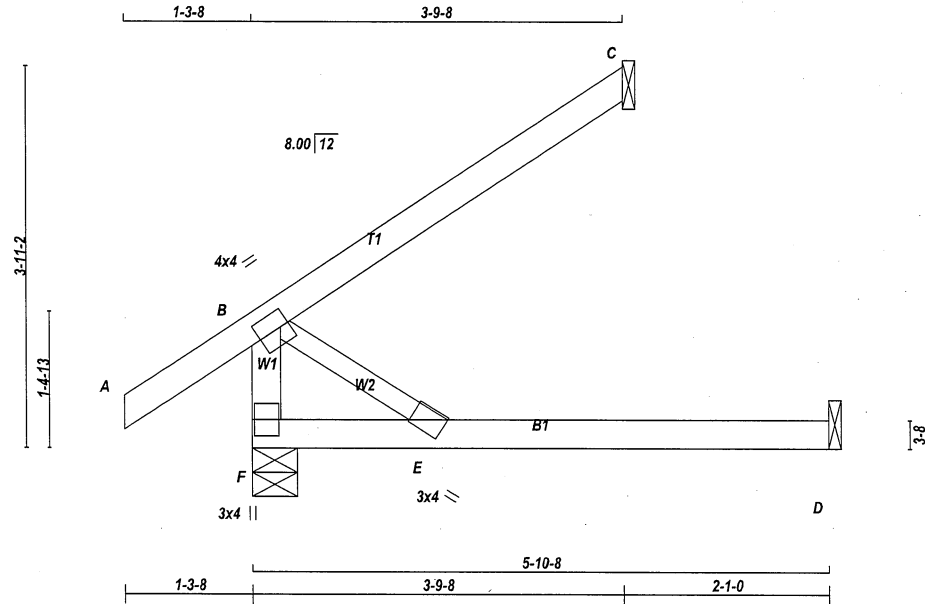


Structural component only
DWG# T-2216647

REVIEWED

JOB NAME 423673	TRUSS NAME J41	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 2 X 17 = 33 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS			SPF	
F - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
F - D	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	4.0	2.00 1.00
E	BMW-w	MT20	3.0	4.0	
F	BMV1-p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UP	BRG	IN-SX
F	422	0	0	422	0	0	5-8	5-8	
C	213	0	0	213	0	0	1-8	1-8	
D	54	0	0	61	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
F	295	212 / 0	0 / 0	0 / 0	0 / 0	83 / 0
C	146	123 / 0	0 / 0	0 / 0	0 / 0	23 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
F-B	-367 / 0	0.0	0.0	0.04 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 43	-112.4	-112.4	0.15 (1)			
B-C	0 / 0	-112.4	-112.4	0.27 (1)			
F-E	0 / 0	-18.5	-18.5	0.16 (4)			
E-D	0 / 0	-18.5	-18.5	0.19 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.27/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



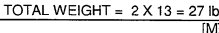
Structural component only
DWG# T-2216648

REVIEWED

Tamarack Roof Truss, Burlington

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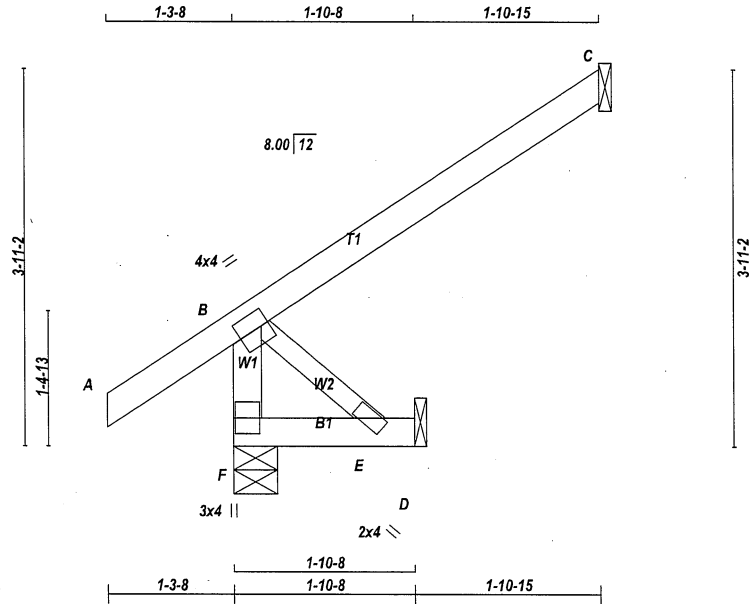
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REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423673	J43	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:22.9

TOTAL WEIGHT = 2 X 12 = 23 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	384	0	384	0
C	213	0	213	0
D	17	0	19	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	265	212 / 0	0 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	146	123 / 0	0 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)
FR-TO	MEMB.	FORCE (LBS)	FROM	TO	FROM	TO	CS1 (LC)	LENGTH	FR-TO	MEMB.	FORCE (LBS)	FROM	TO	FROM	TO
F-B		-367 / 0	0.0	0.0	0.04 (1)	7.81			B-E		0 / 0	0.00 (1)			
A-B		0 / 43	-112.4	-112.4	0.15 (1)	10.00									
B-C		0 / 0	-112.4	-112.4	0.27 (1)	10.00									
F-E		0 / 0	-18.5	-18.5	0.02 (4)	10.00									
E-D		0 / 0	-18.5	-18.5	0.01 (4)	10.00									

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.27/1.00 (B-C:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

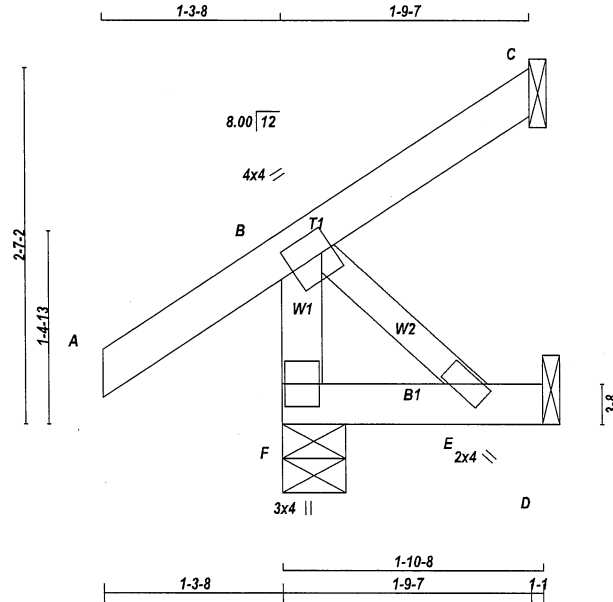
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2216650

REVIEWED



TOTAL WEIGHT = 5 X 9 = 45 lb

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS					
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW-w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
F	331	0	331	0	0	5-8	5-8	5-8	5-8
C	41	0	41	0	-50	1-8	1-8	1-8	1-8
D	17	0	19	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	229	181 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	28	24 / -34	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-314 / 0	0.0	0.0 0.03 (1)	B-E	0 / 0	0.00 (1)	
A-B	0 / 43	-112.4	-112.4 0.15 (1)				
B-C	-33 / 0	-112.4	-112.4 0.14 (1)				
F-E	0 / 0	-18.5	-18.5 0.02 (4)				
E-D	0 / 0	-18.5	-18.5 0.02 (4)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) ,
WB=0.00/1.00 (B-E:1) , SS=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(FSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



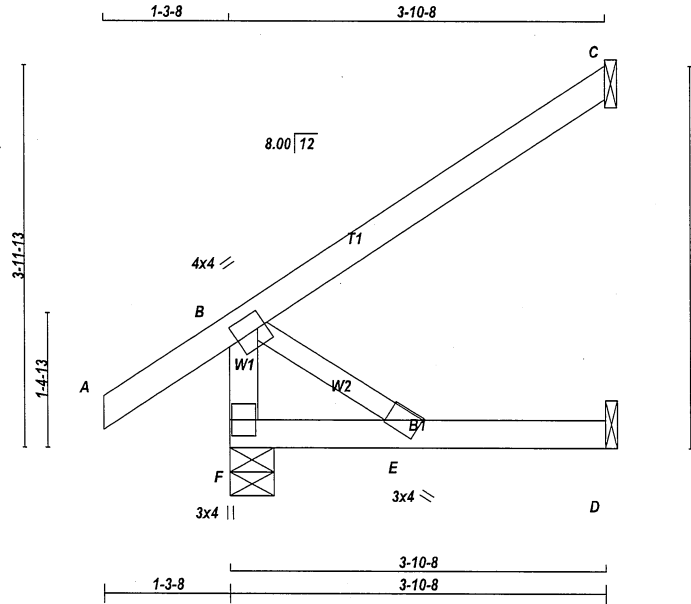
Structural component only
DWG# T-2216651

REVIEWED

JOB NAME 423673	TRUSS NAME J45	QUANTITY 4	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:22.9

TOTAL WEIGHT = 4 X 14 = 58 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW-w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
F	408	0	408	0
C	218	0	218	0
D	36	0	40	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM.LIVE
F	283	215 / 0	0 / 0	0 / 0
C	149	126 / 0	0 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	MEMB.	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX	MAX
	(LBS)	(PLF)	CS1 (LC)	UNBRAC
FR-TO		FROM TO	LENGTH	FR-TO
F-B	-372 / 0	0.0	0.0	0.04 (1)
A-B	0 / 43	-112.4	-112.4	0.16 (5)
B-C	0 / 0	-112.4	-112.4	0.29 (1)
F-E	0 / 0	-18.5	-18.5	0.08 (4)
E-D	0 / 0	-18.5	-18.5	0.08 (4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 32.5 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.29/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER-1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



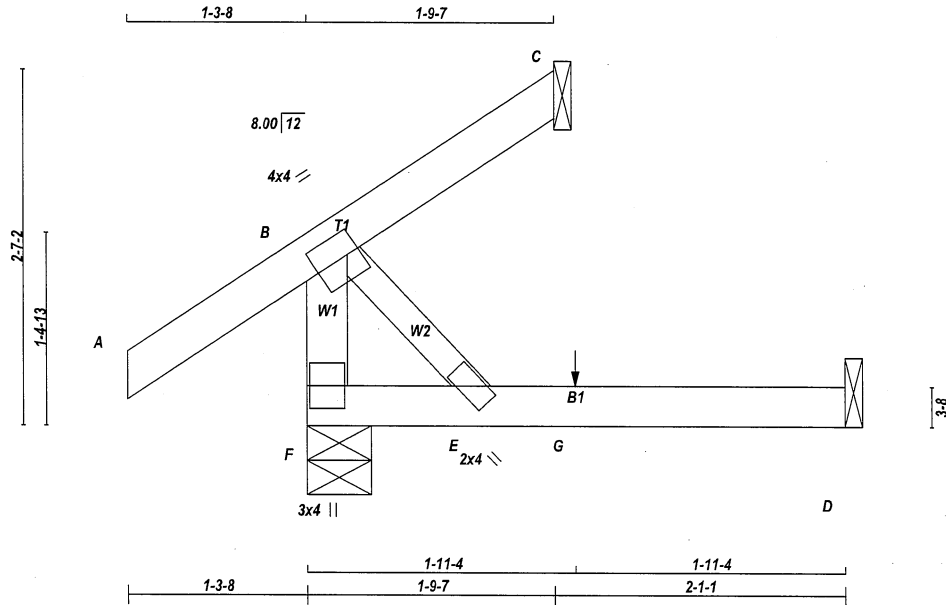
Structural component only
DWG# T-2216652

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423673	J46	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 11 = 22 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW-w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
F	350	0	350	0
C	41	0	41	0
D	36	0	40	0

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	244	181 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	28	24 / -34	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO							FR-TO			
F-B	-314 / 0	0.0	0.0	0.03	(1)	7.81	B-E	0 / 0	0.00	(1)
A-B	0 / 43	-112.4	-112.4	0.16	(5)	10.00				
B-C	-33 / 0	-112.4	-112.4	0.15	(5)	6.25				
F-E	0 / 0	-18.5	-18.5	0.07	(4)	10.00				
E-G	0 / 0	-18.5	-18.5	0.08	(4)	10.00				
G-D	0 / 0	-18.5	-18.5	0.08	(4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.16/1.00 (A-B-5), BC=0.08/1.00 (D-E-4), WB=0.00/1.00 (B-E-1), SSI=0.11/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

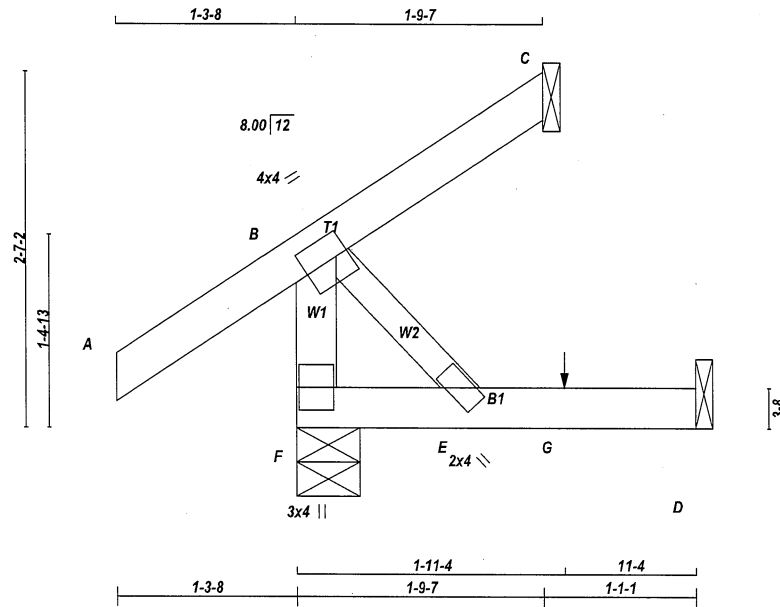


Structural component only
DWG# T-2216653

REVIEWED

JOB NAME 423673	TRUSS NAME J47	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 10 lb [M]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW-w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
F	341	0	341	0	0	5-8	5-8	5-8	5-8
C	41	0	41	0	-50	1-8	1-8	1-8	1-8
D	27	0	30	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
F	236	181 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0	
C	28	24 / -34	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0	
D	21	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
F-B	-314 / 0	0.0	0.0 0.03 (1)	B-E	0 / 0	0.00 (1)	
A-B	0 / 43	-112.4	-112.4 0.16 (5)				
B-C	-33 / 0	-112.4	-112.4 0.15 (5)				
F-E	0 / 0	-18.5	-18.5 0.04 (4)				
E-G	0 / 0	-18.5	-18.5 0.04 (4)				
G-D	0 / 0	-18.5	-18.5 0.04 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	---	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:5), BC=0.04/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

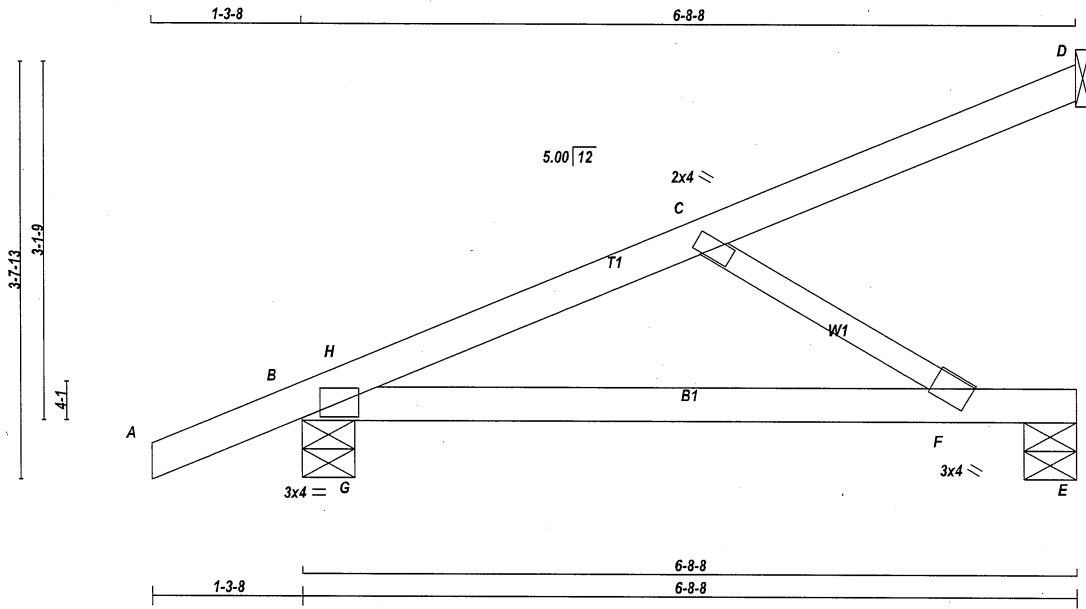


Structural component only
DWG# T-2216654

REVIEWED

JOB NAME 423673	TRUSS NAME J48	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Fri Jul 8 12:26:30 2022 Page 1
ID:c3jy23uDijq_8pvRKbkZpy75XW-0KAYils7AJIqjy6ojxuV95SNzx4fngOm_OgBpz_8R7



Scale = 1:19.3

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N.L.G.A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS	SIZE	DRY	LUMBER No.2	FACTORED	MAXIMUM FACTORED	INPUT	REQD	TOP CH.	LL	=	32.5 PSF
A - D	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG	DL	=	6.0 PSF	
B - E	2x4	DRY	No.2	DOWN	HORZ	UPLIFT	IN-SX	BOT CH.	LL	=	0.0 PSF
ALL WEBS 2x3 DRY				JT D	156	0	1-8	DL	=	7.4 PSF	
DRY: SEASONED LUMBER.				B	591	0	5-8	TOTAL LOAD	=	45.9 PSF	
				E	283	0	5-8				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
F	BMW+w	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	107	88 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
B	411	305 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
E	201	129 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-112.4 -112.4	0.14 (1)	C-F	-486 / 0	0.09 (1)	
B-H	-570 / 0	-112.4 -112.4	0.16 (1)	G-H	0 / 201	0.00 (1)	
H-C	-401 / 0	-112.4 -112.4	0.26 (1)				
C-D	-9 / 0	-112.4 -112.4	0.13 (1)				
B-G	0 / 396	-18.5 -18.5	0.12 (1)				
G-F	0 / 396	-18.5 -18.5	0.47 (1)				
F-E	0 / 0	-18.5 -18.5	0.40 (1)				

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL)= L/ 799 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL)= L/ 379 (0.21")

CSI: TC=0.26/1.00 (C-H:1) , BC=0.47/1.00 (F-G:1) , WB=0.09/1.00 (C-F:1) , SSI=0.23/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

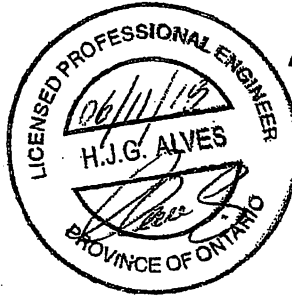
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.37 (B) (INPUT = 0.90)
JSI METAL= 0.22 (C) (INPUT = 1.00)



Structural component only
DWG# T-2216655

REVIEWED



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes.

T-1300213

Feb 09, 2018

REVIEWED

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
Minimum Number of Nails					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

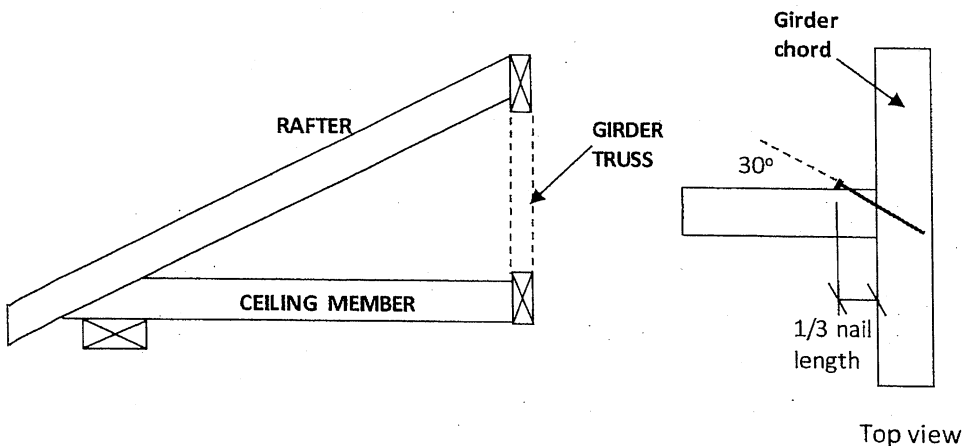
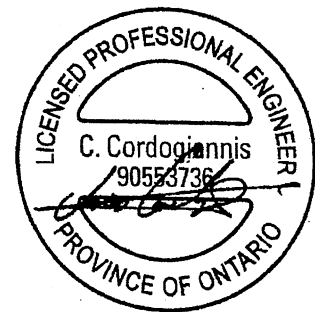


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

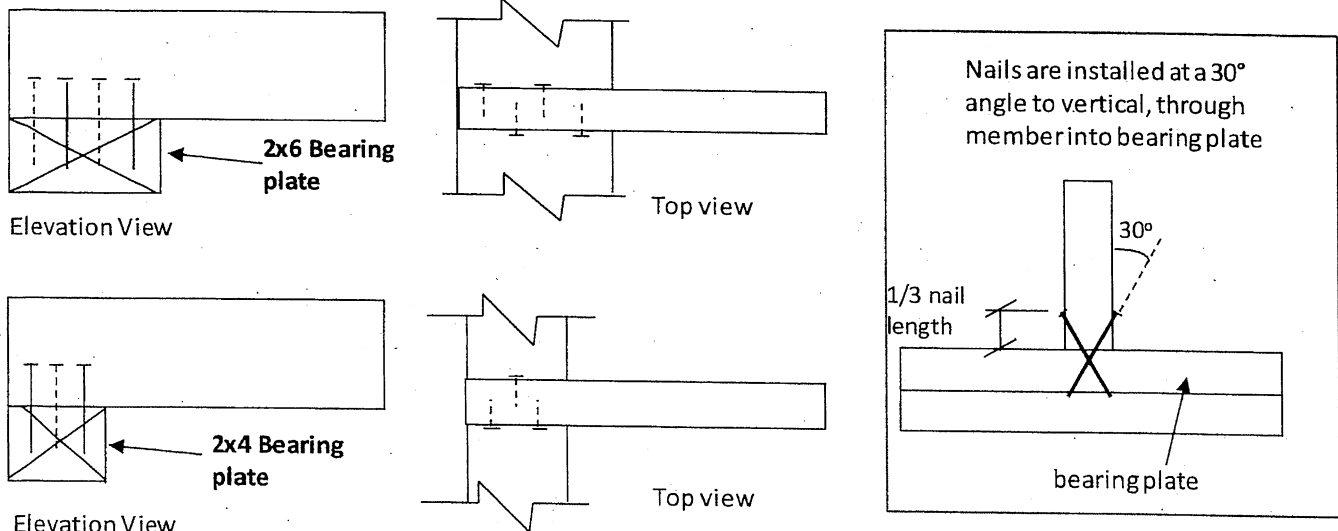
PEO
Certificate No. 10889485



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

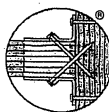
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

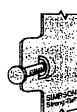
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear Nailing Top View

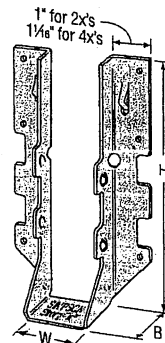
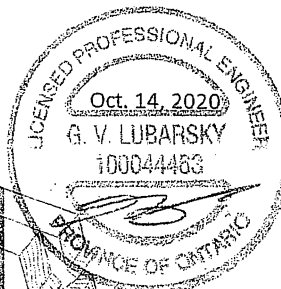
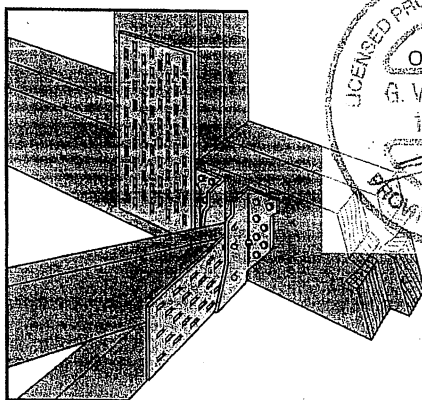


Double-Shear Nailing Side View; Do not bend tab

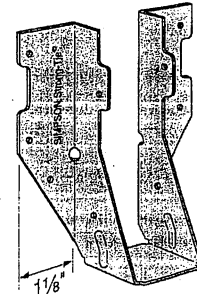


Dome Double-Shear Nailing Side View (available on some models)

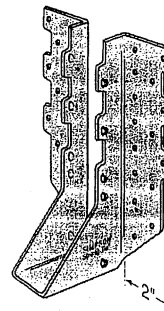
Typical HUS26 Installation with Reduced Heel Height (Truss Designer to provide fastener quantity for connecting multiple members together)



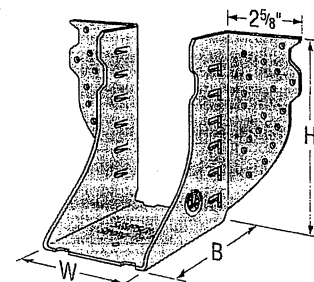
LUS28



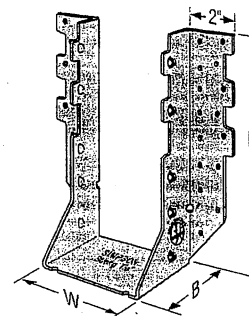
LU26L



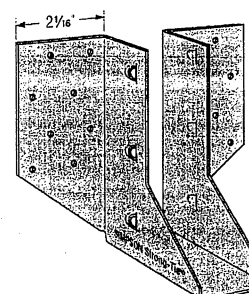
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2



LJS26DS

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

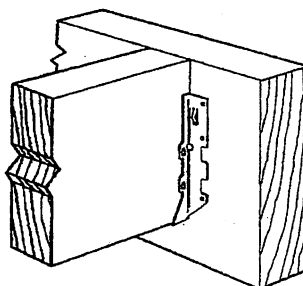
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

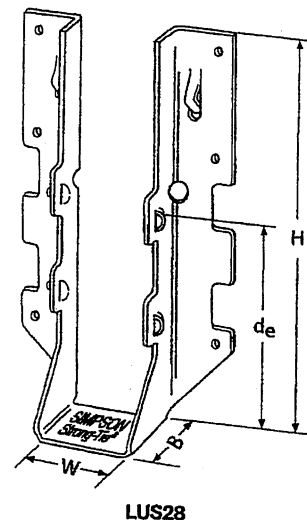
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

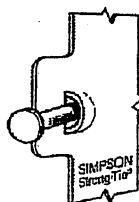


Typical LUS Installation



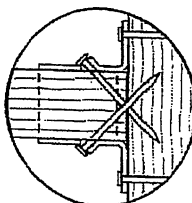
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1⅞	3⅞	1¾	1⅞	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅞	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⅞	4¾	1¾	3⅞	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¾	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¾	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅞	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8⅞	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

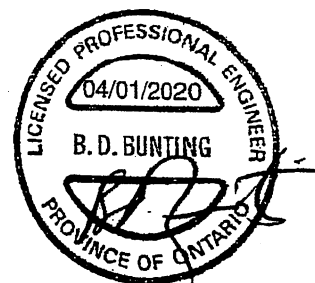


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

REVIEWED

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

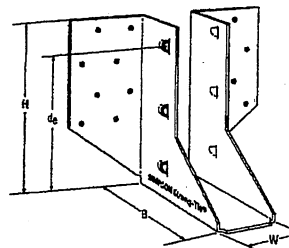
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

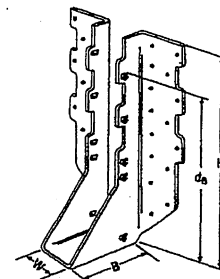
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

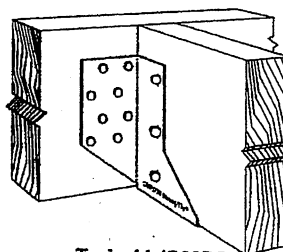
- See current catalogue for options



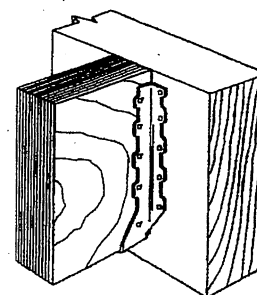
LJS26DS



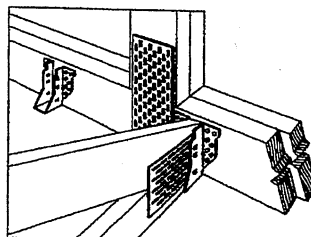
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



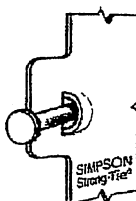
Typical HUS
Installation



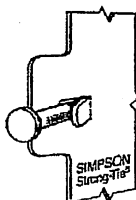
Typical HUS installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.8/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

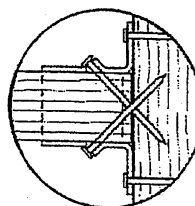
1. d_e is the distance from the seat of the hanger to the highest joist nail.



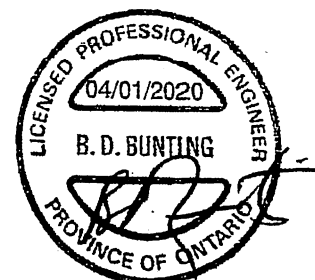
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

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REVIEWED

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

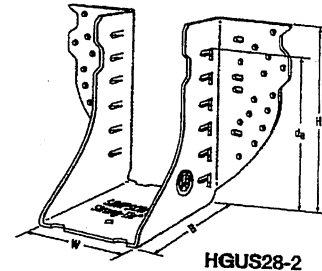
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

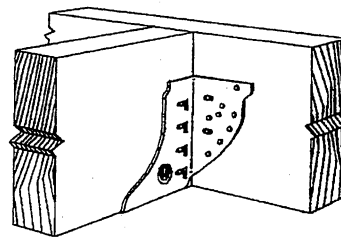
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

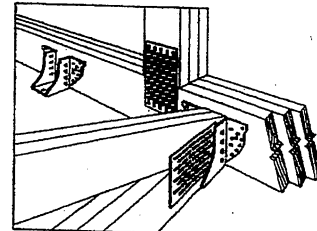
- See current catalogue for options



HGUS28-2



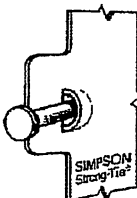
Typical HGUS
Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

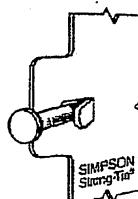
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1½	5½	5	4½ ₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½ ₁₆	5½ ₁₆	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½ ₁₆	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½ ₁₆	5½ ₁₆	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½ ₁₆	7½ ₁₆	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½ ₁₆	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½ ₁₆	7½ ₁₆	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½ ₁₆	9½ ₁₆	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½ ₁₆	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½ ₁₆	9½ ₁₆	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½ ₁₆	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½ ₁₆	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

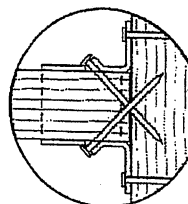


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

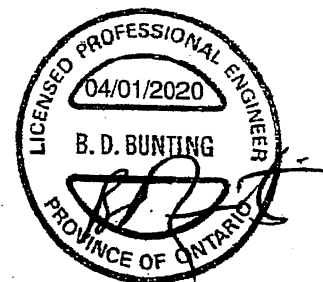
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

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REVIEWED

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

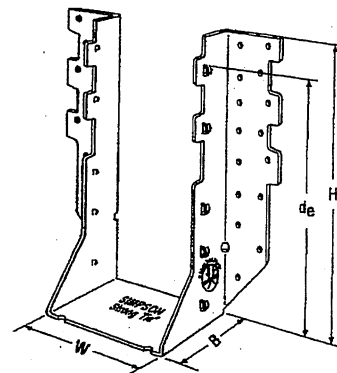
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

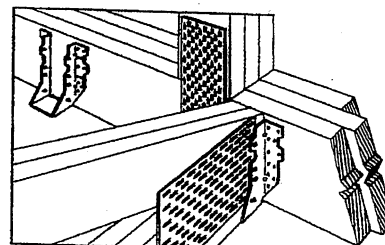
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



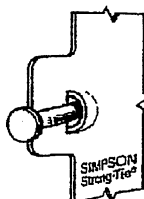
HHUS410



Typical HHUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

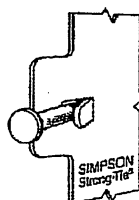
Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₆ ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K ₉ =1.15)	Normal (K ₉ =1.00)	Uplift (K ₉ =1.15)	Normal (K ₉ =1.00)
HHUS26-2	14	3 1/8	5 13/16	3	3 15/16	(14) 16d	(6) 16d		2850	7335	2065	5205
HHUS28-2	14	3 1/8	7 7/32	3	6 5/32	(22) 16d	(8) 16d		3765	8940	2675	6345
HHUS210-2	14	3 1/8	9 1/2	3	8	(30) 16d	(10) 16d		4670	9660	4235	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	(30) 16d	(10) 16d		4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d		4670	10155	4235	7210
HHUS46	14	3 3/8	5 13/16	3	3 15/16	(14) 16d	(6) 16d		2540	7335	2065	5205
HHUS48	14	3 3/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d		3765	8940	2675	6345
HHUS410	14	3 3/8	9	3	8	(30) 16d	(10) 16d		4670	9855	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d		4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 1/8	7 29/32	(30) 16d	(10) 16d		4670	10155	3370	7210

1. d₆ is the distance from the seat of the hanger to the highest joist nail.

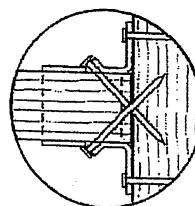


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

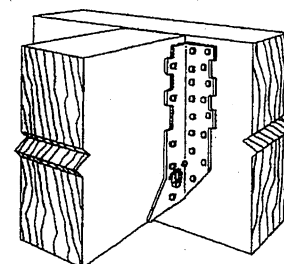
U.S. Patent 5,603,580



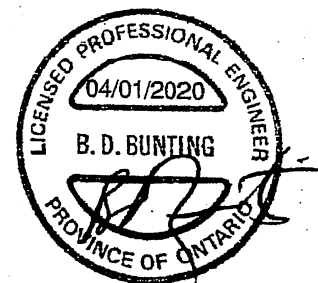
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



Typical HHUS Installation



LIMIT
STATES
DESIGN

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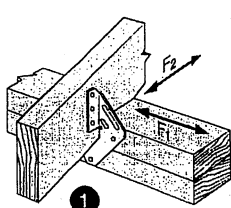
T-SPECHHUS20 3/20 exp. 6/22

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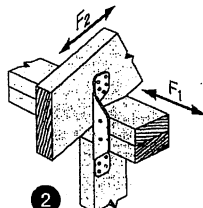
REVIEWED

H/TSP

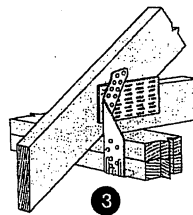
Seismic and Hurricane Ties (cont.)



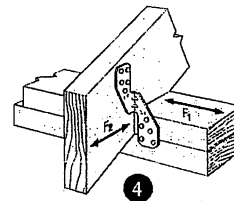
1 H1 Installation



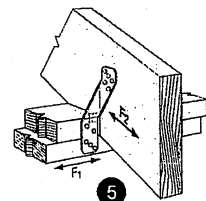
2 H2A Installation



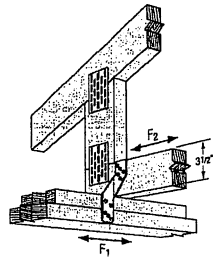
3 TSP Installation



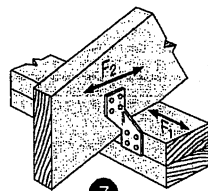
4 H2.5A Installation
(Nails into both top plates)



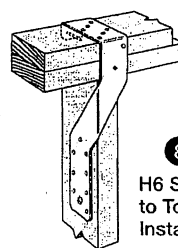
5 H2.5T Installation
(Nails into both top plates)



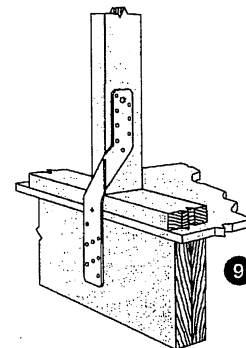
6 H2.5T Installation



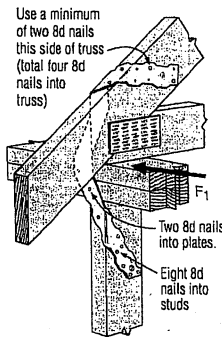
7 H3 Installation
(Nails into upper top plate)



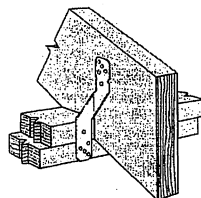
8 H6 Stud
to Top Plate
Installation



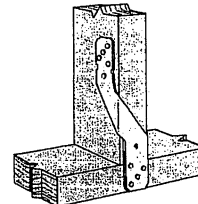
9 H6 Stud to
Band Joist
Installation



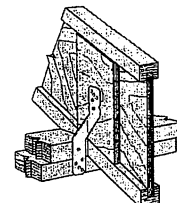
10 H7Z Installation



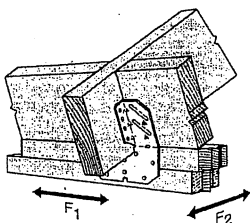
11 H8 Attaching
Rafter to Double
Top Plates



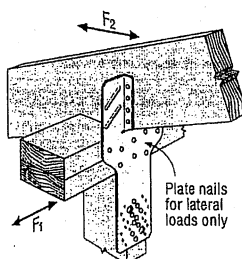
12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)



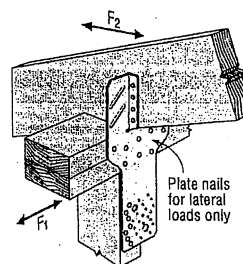
13 H8 attaching
I-Joist to Double
Top Plates



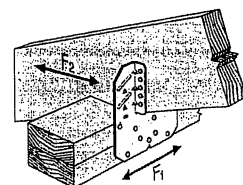
14 H10A Field-Bent
Installation



15 H10S Installation

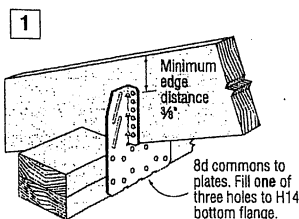


16 H10S Installation
with Stud Offset

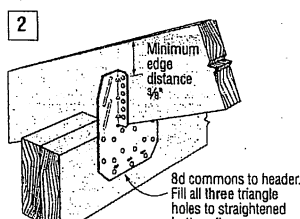


17 H10A
Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

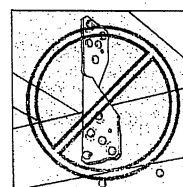


18 H14 Installation
to Double Top Plates



19 H14 Installation
to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

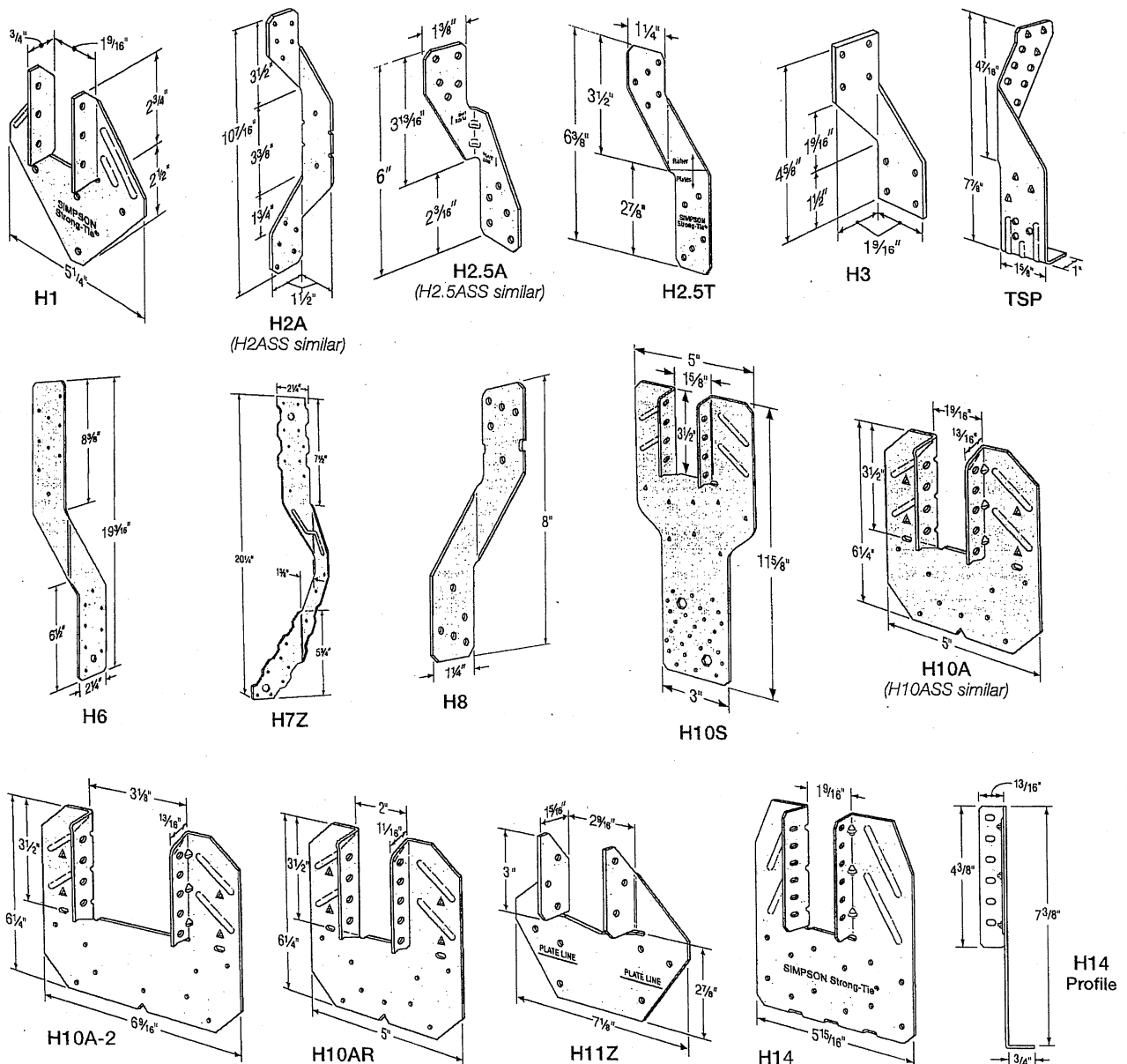
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA 086-14

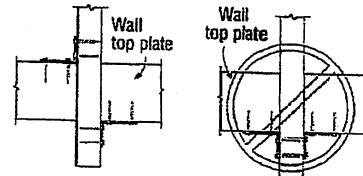
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

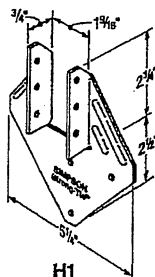
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

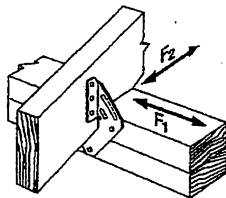


Install diagonally across from each other for minimum 2x truss.

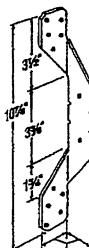
Nailing into both sides of a single ply 2x truss may cause the wood to split.



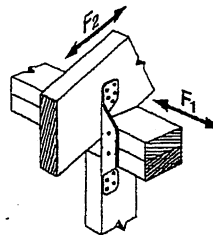
H1



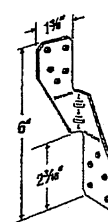
H1 Installation



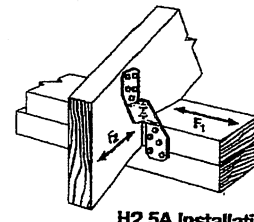
H2A



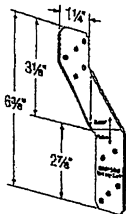
H2A Installation



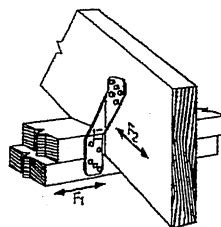
H2.5A



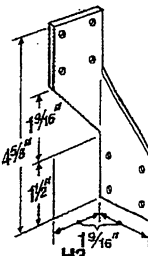
H2.5A Installation



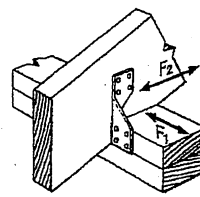
H2.5T



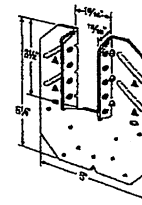
H2.5T Installation
(Nails into both top plates)



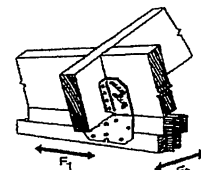
H3



H3 Installation



H10A

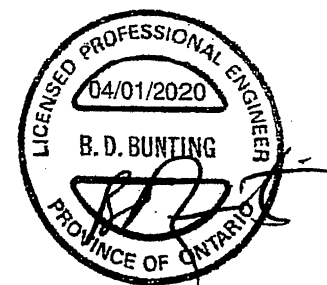


H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
					(K _p =1.15)					(K _p =1.15)
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

REVIEWED

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

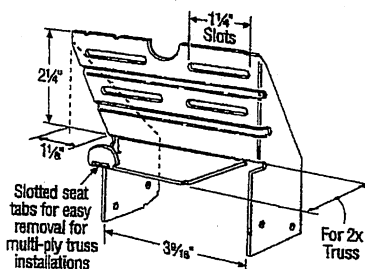
Design: Factored resistances are in accordance with CSA 086-14

Installation:

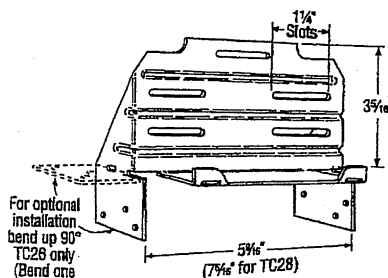
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

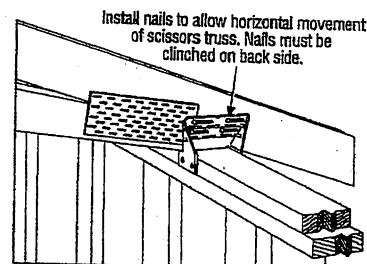
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



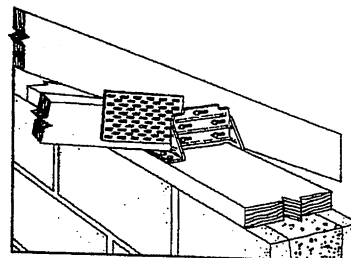
TC24
U.S. Patent 4,932,173



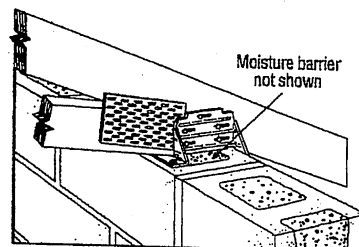
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



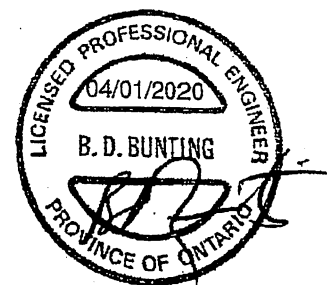
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
TC24	(4) 10d	(4) 10d	lb.	lb.
TC26	(5) 10d	(6) 10d	605	430
TC28	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECTC20 3/20 exp. 6/22

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REVIEWED

HTU



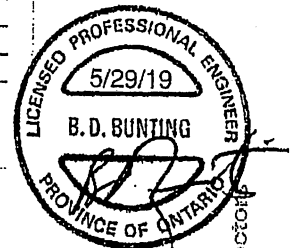
Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (in.)	Minimum Header Size	Fasteners		Factored Resistance			
					D.Fir-L		S-P-F	
			Header	Joist	Uplift ($K_D = 1.15$)	Normal ($K_D = 1.00$)	Uplift ($K_D = 1.15$)	Normal ($K_D = 1.00$)
					lb.	lb.	lb.	lb.
HTU26 (Min.)	3 3/4"	(2) 2x4	(10) 16d	(14) 10d x 1 1/2"	1740	3340	1235	2370
HTU26 (Max.)	5 1/2"	(2) 2x4	(10) 16d	(20) 10d x 1 1/2"	7.74	14.86	5.49	10.54
HTU28 (Max.)	3 3/4"	(2) 2x6	(20) 16d	(26) 10d x 1 1/2"	2470	4016	1755	2850
HTU210 (Max.)	7 1/4"	(2) 2x6	(20) 16d	(32) 10d x 1 1/2"	10.99	17.86	7.81	12.68
					4150	6395	2945	4540
					18.46	28.45	13.10	20.19
					4150	6395	2945	4540
					18.46	28.45	13.10	20.19

See table footnotes on p. 260.

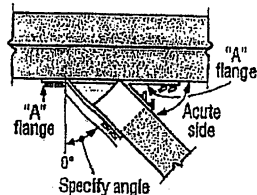


Hanger Options

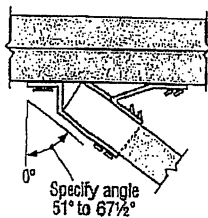
See Hanger Options information on pp. 125-127.

Skewed Seat

- Skewable up to 67 1/2°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
				D.Fir-L		S-P-F	
		Header	Joist	Uplift ($K_D = 1.15$)	Normal ($K_D = 1.00$)	Uplift ($K_D = 1.15$)	Normal ($K_D = 1.00$)
				lb.	lb.	lb.	lb.
HTU26	< 51	(20) 16d	(14) 10d x 1 1/2"	1835	4110	1300	2905
	51-67 1/2	(20) 16d	(12) 10d x 1 1/2"	8.16	18.28	5.78	12.92
HTU28	< 51	(26) 16d	(20) 10d x 1 1/2"	1350	3620	955	2560
	51-67 1/2	(26) 16d	(17) 10d x 1 1/2"	5.01	18.10	4.25	11.39
HTU210	< 51	(32) 16d	(26) 10d x 1 1/2"	2810	4270	1985	3030
	51-67 1/2	(32) 16d	(22) 10d x 1 1/2"	12.50	18.99	8.83	13.48
HTU26-2	< 51	(20) 16d	(14) 10d	2075	3930	1465	2780
	51-67 1/2	(20) 16d	(12) 10d	9.23	17.48	6.52	12.37
HTU28-2	< 51	(26) 16d	(20) 10d	3785	4430	2675	3135
	51-67 1/2	(26) 16d	(17) 10d	16.84	19.71	11.90	13.95
HTU210-2	< 51	(32) 16d	(26) 10d	2795	4240	1980	3000
	51-67 1/2	(32) 16d	(22) 10d	12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
	51-67 1/2	(20) 16d	(12) 10d	9.52	16.53	6.74	11.68
HTU28-2	< 51	(26) 16d	(20) 10d	1610	3920	1140	2785
	51-67 1/2	(26) 16d	(17) 10d	7.16	17.44	5.07	12.39
HTU210-2	< 51	(32) 16d	(26) 10d	3960	5425	2815	3855
	51-67 1/2	(32) 16d	(22) 10d	17.62	24.13	12.52	17.15
HTU26-2	< 51	(20) 16d	(14) 10d	2385	5425	1695	3855
	51-67 1/2	(20) 16d	(12) 10d	10.61	24.13	7.54	17.15
HTU28-2	< 51	(26) 16d	(20) 10d	5025	6890	3570	4890
	51-67 1/2	(26) 16d	(17) 10d	22.35	30.65	15.88	21.75
HTU210-2	< 51	(32) 16d	(26) 10d	3145	6680	2225	4745
	51-67 1/2	(32) 16d	(22) 10d	13.99	29.72	9.90	21.10

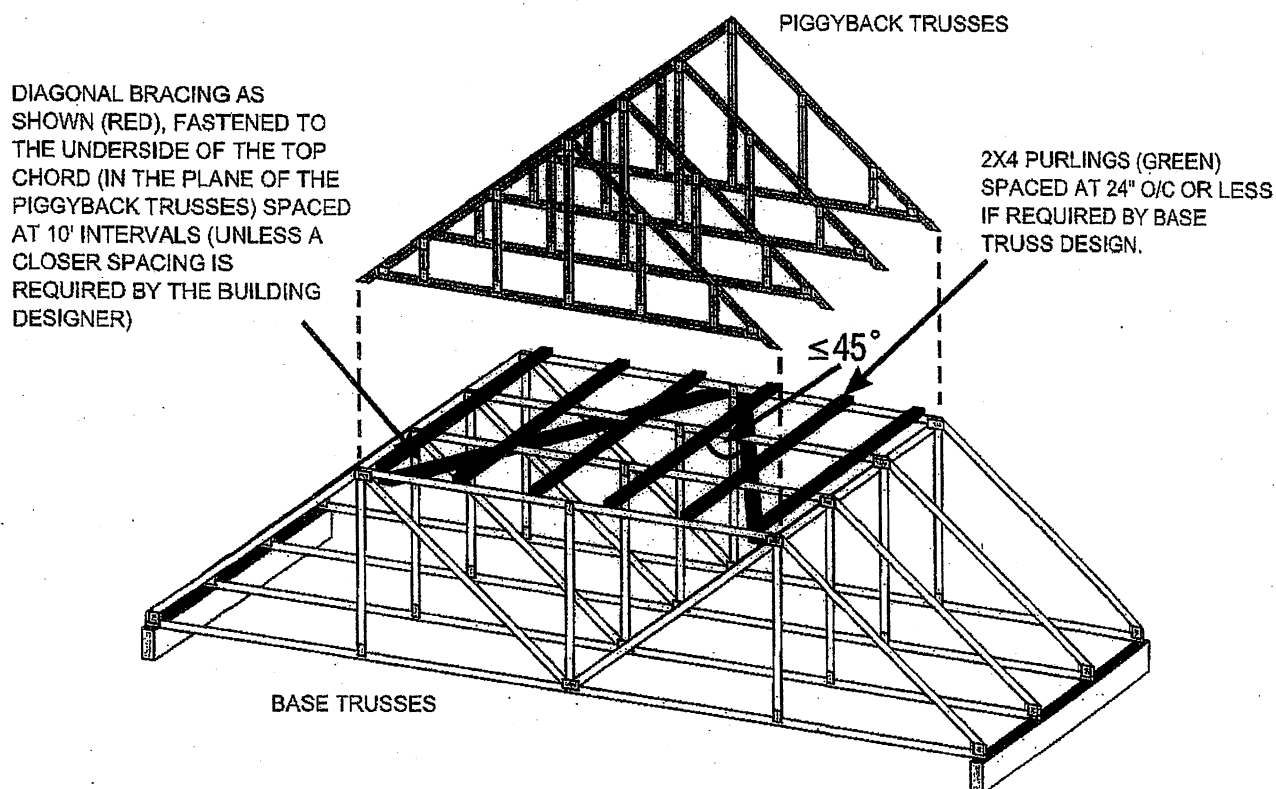
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3 1/2" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 10d = 0.148" dia. x 3" long. See pp. 27-28 for other nail sizes and information.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

REVIEWED

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

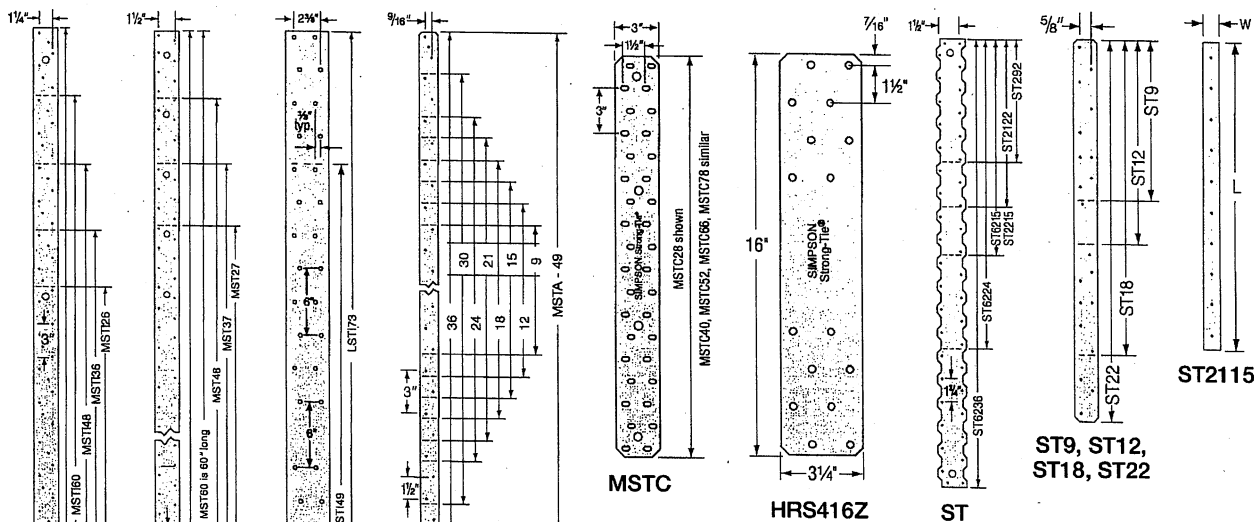
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

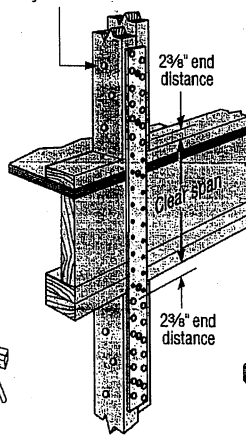
Installation: Use all specified fasteners; see General Notes

Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

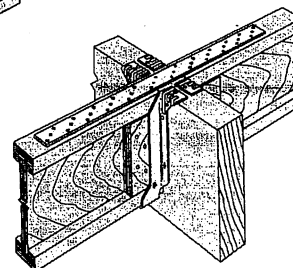


LSTA and MSTA
(Pilot holes not shown)

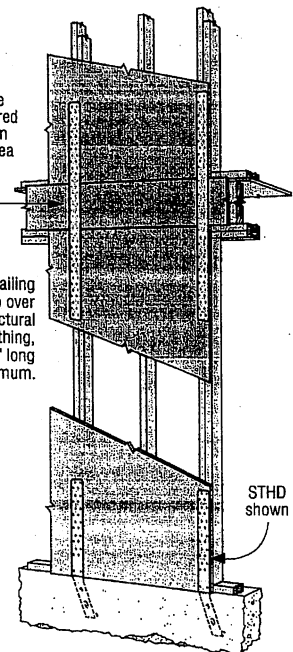
Stitch nailing
of double studs
by others



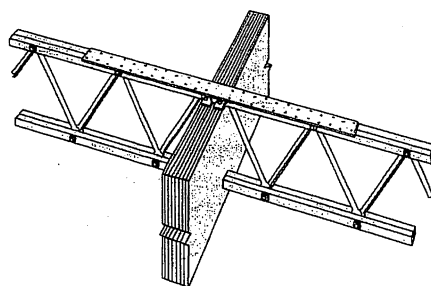
Floor-to-Floor Tie Installation
Showing a Clear Span



Typical MSTI Installation
(MIT hanger shown)
LSTI similar



Typical Detail with Strap Installed over Wood Structural Panel Sheathing



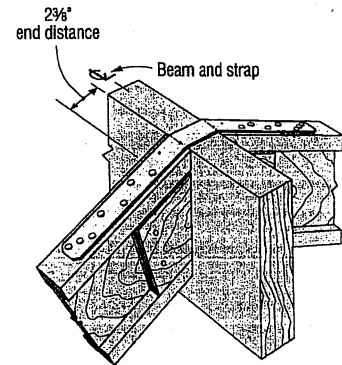
Typical LSTI Installation

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI
Strap Ties (cont.)

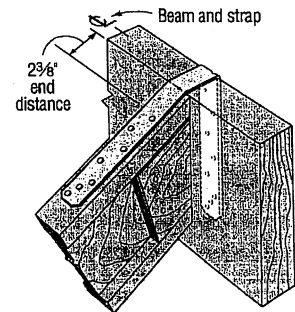
These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366-370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
					D.Fir-L		S-P-F	
		W	L		(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
LSTA9	20	1¼	9	(6) 10d	600	690	555	635
					2.67	3.07	2.47	2.82
LSTA12		1¼	12	(8) 10d	800	920	735	845
					3.56	4.09	3.27	3.76
LSTA15		1¼	15	(10) 10d	1000	1150	920	1060
					4.45	5.12	4.09	4.72
LSTA18		1¼	18	(12) 10d	1200	1380	1105	1270
					5.34	6.14	4.92	5.65
LSTA21		1¼	21	(14) 10d	1400	1610	1290	1485
					6.23	7.16	5.74	6.61
LSTA24		1¼	24	(16) 10d	1600	1840	1475	1695
					7.12	8.19	6.56	7.54
ST292		2⅞	9⅞	(8) 8d	585	675	535	615
					2.60	3.00	2.38	2.74
ST2122		2⅞	12⅞	(12) 8d	940	1085	865	995
					4.18	4.83	3.85	4.43
ST2115	¾	16⅞	(8) 8d	670	770	615	710	
				2.98	3.43	2.74	3.16	
ST2215	2⅞	16⅞	(16) 8d	1335	1540	1235	1420	
				5.94	6.85	5.49	6.32	
LSTA30	18	1¼	30	(20) 10d	2235	2465	2075	2385
					9.94	10.97	9.23	10.61
LSTA36		1¼	36	(24) 10d	2465	2465	2465	2465
					10.97	10.97	10.97	10.97
LSTI49		3¾	49	(32) 10d x 1½"	3115	3580	2852	3280
					13.86	15.93	12.69	14.59
LSTI73		3¾	73	(48) 10d x 1½"	4670	5370	4280	4920
					20.77	23.89	19.04	21.89
MSTA9		1¼	9	(6) 10d	670	770	625	715
					2.98	3.43	2.78	3.18
MSTA12		1¼	12	(8) 10d	895	1030	830	955
					3.98	4.58	3.69	4.25
MSTA15		1¼	15	(10) 10d	1120	1285	1040	1195
					4.98	5.72	4.63	5.32
MSTA18		1¼	18	(12) 10d	1340	1545	1245	1430
					5.96	6.87	5.54	6.36
MSTA21	1¼	21	(14) 10d	1565	1800	1455	1670	
				6.96	8.01	6.47	7.43	
MSTA24	1¼	24	(16) 10d	1790	2060	1660	1910	
				7.96	9.16	7.38	8.50	
MSTA30	1¼	30	(20) 10d	2470	2840	2260	2595	
				10.99	12.63	10.05	11.54	
MSTA36	1¼	36	(24) 10d	2965	3070	2710	3070	
				13.19	13.66	12.06	13.66	
MSTA49	1¼	49	(28) 8d	2725	2725	2545	2725	
				12.12	12.12	11.32	12.12	
ST6215	2⅞	16⅞	(16) 8d	1405	1615	1300	1500	
				6.25	7.18	5.78	6.67	
ST6224	2⅞	23⅞	(24) 8d	2305	2650	2155	2475	
				10.25	11.79	9.59	11.01	
ST9	1¼	9	(6) 8d	525	605	490	560	
				2.34	2.69	2.18	2.49	
ST12	1¼	11⅞	(8) 8d	700	805	650	750	
				3.11	3.58	2.89	3.34	
ST18	1¼	17¾	(12) 8d	1050	1210	975	1125	
				4.67	5.38	4.34	5.00	
ST22	1¼	21⅞	(18) 8d	1580	1790	1465	1685	
				7.03	7.96	6.52	7.50	



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Use half of the nails in each member being connected to achieve the listed resistances.
- Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22-23 for other nail sizes and information.

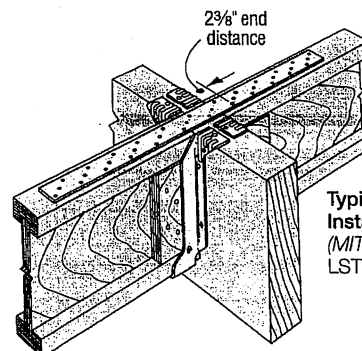
HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

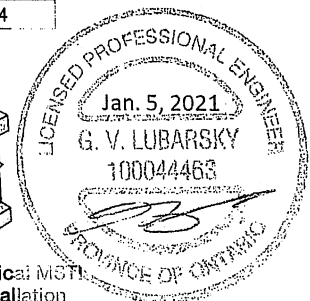
SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
		W	L		D.Fir-L		S-P-F	
					(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
MSTC28	16	3	28¼	(32) 10d	3955	4545	3615	4155
					17.59	20.22	16.08	18.48
MSTC40		3	40¼	(48) 10d	5930	6820	5420	6235
					26.38	30.34	24.11	27.74
MSTC52		3	52¼	(54) 10d	6670	6940	6100	6940
					29.67	30.87	27.14	30.87
MSTC66	14	3	65¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
MSTC78		3	77¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
ST6236		2⅝	33 13⁄₁₆	(36) 8d	3735	4295	3270	3760
					16.61	19.11	14.55	16.73
MSTI26	12	2⅝	26	(22) 10d x 1 ½"	2825	3250	2475	2850
					12.57	14.46	11.01	12.68
MSTI36		2⅝	36	(32) 10d x 1 ½"	4110	4725	3600	4140
					18.28	21.02	16.01	18.42
MSTI48		2⅝	48	(44) 10d x 1 ½"	5650	6500	4955	5695
					25.13	28.91	22.04	25.33
MSTI60		2⅝	60	(56) 10d x 1 ½"	7195	7360	6305	7250
					32.01	32.74	28.05	32.25
MSTI72		2⅝	72	(68) 10d x 1 ½"	7360	7360	7240	7360
					32.74	32.74	32.21	32.74
MST27		2⅝	27	(26) 8d	2685	3090	2355	2710
					11.94	13.75	10.48	12.06
MST37		2⅝	37½	(38) 8d	3930	4515	3440	3960
					17.48	20.08	15.30	17.62
MST48		2⅝	48	(50) 8d	5170	5945	4530	5210
					23.00	26.45	20.15	23.18
HRS416Z		3¼	16	(16) ¼" x 1 ½" SDS	2400	2760	2120	2440
					10.68	12.28	9.43	10.85
MST60	10	2⅝	60	(64) 8d	6620	7610	5800	6670
					29.45	33.85	25.80	29.67
MST72		2⅝	72	(78) 8d	8065	9135	7065	8125
					35.88	40.64	31.43	36.14

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. **Nails:** 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22–23 for other nail sizes and information.



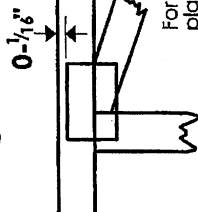
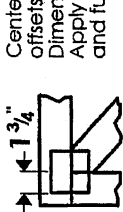
Typical MSTI
Installation
(MIT hanger shown)
LSTI similar



Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



* Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

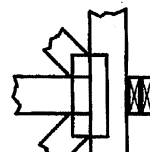
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



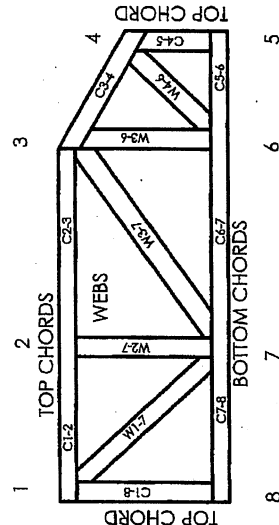
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
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MiTek Engineering Reference Sheet: Mill-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 17% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

REVIEWED