

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

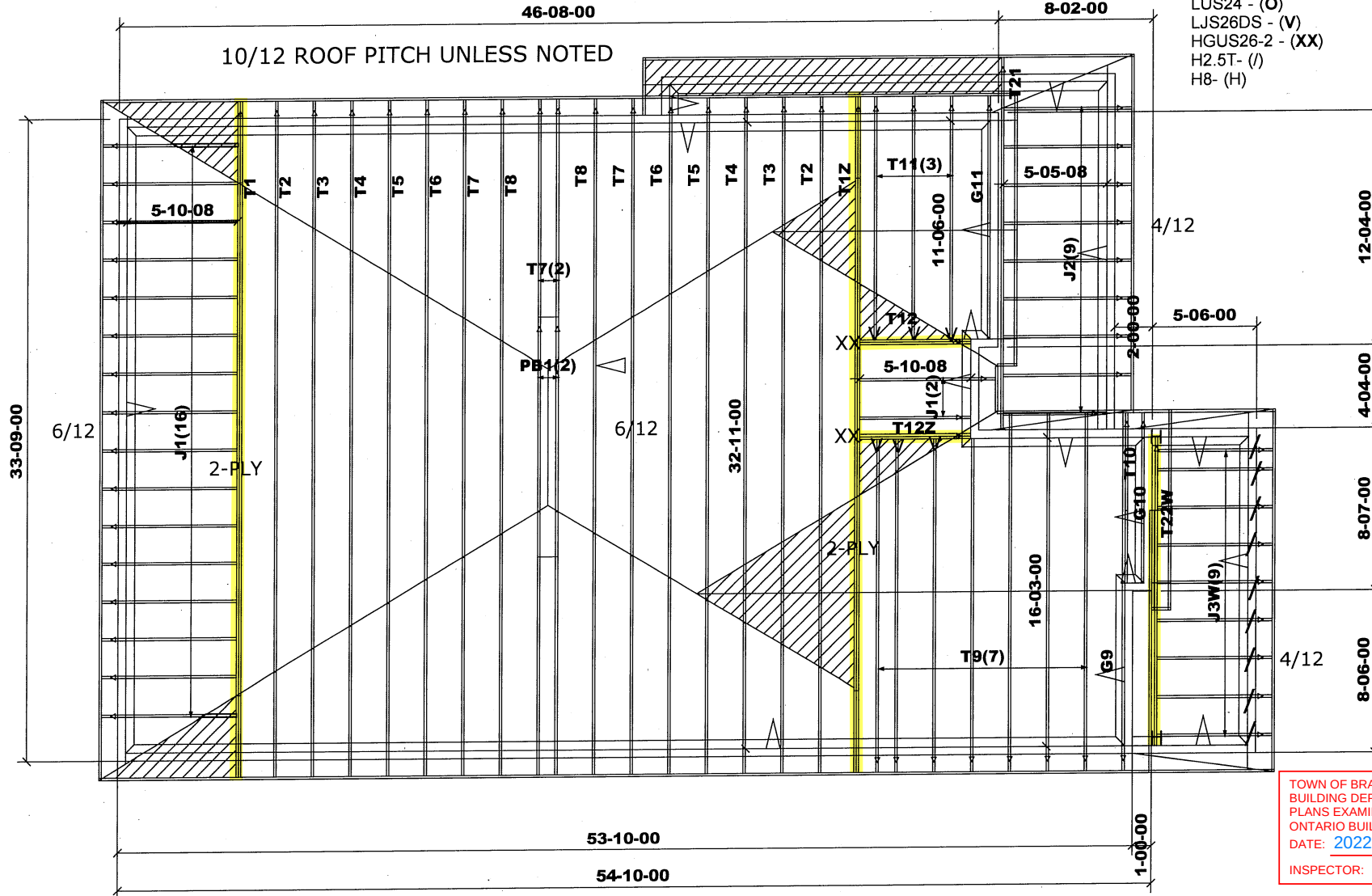
DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 43.8 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf



DENOTES:
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T- (I)
H8- (H)



TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2022-09-01
INSPECTOR: BG



Job Track: **50465**
Plan Log: **205571**
Layout ID: **423627**

Builder / Location:
BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: 2022-06-28 Sales: Rick DiCiano Designer: JG

Model / Elevation:
S42-19 / A

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.5.3.233

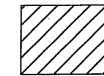
REVIEWED

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment).
Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

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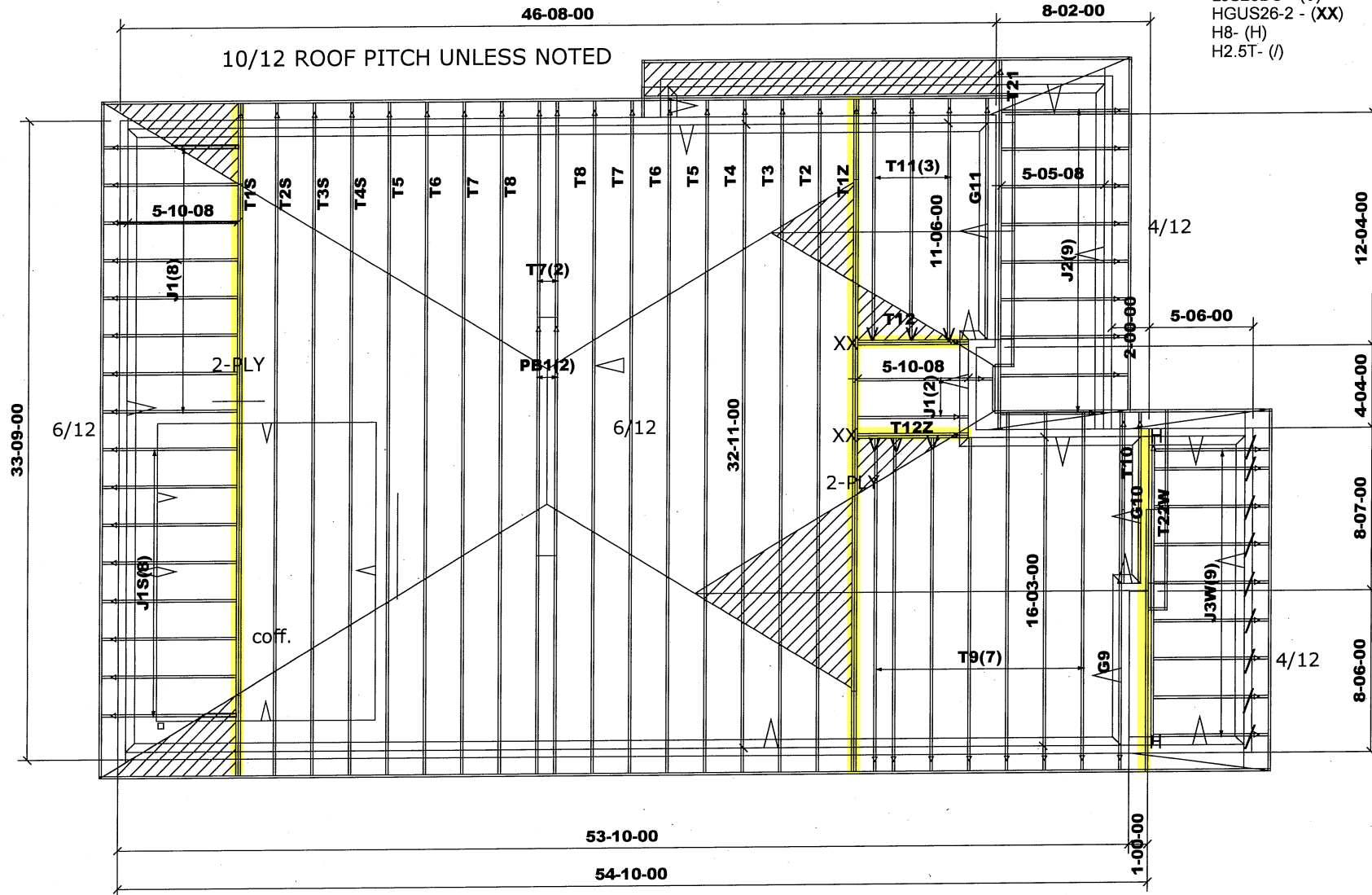
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DENOTES:
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H8- (H)
H2.5T- (I)



Job Track: **50465**
Plan Log: **205571**
Layout ID: **423626**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **2022-06-28** Sales: Rick DiCiano Designer: JG

Model / Elevation:

S42-19 / A-OPT. WITH COFF

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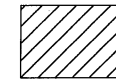
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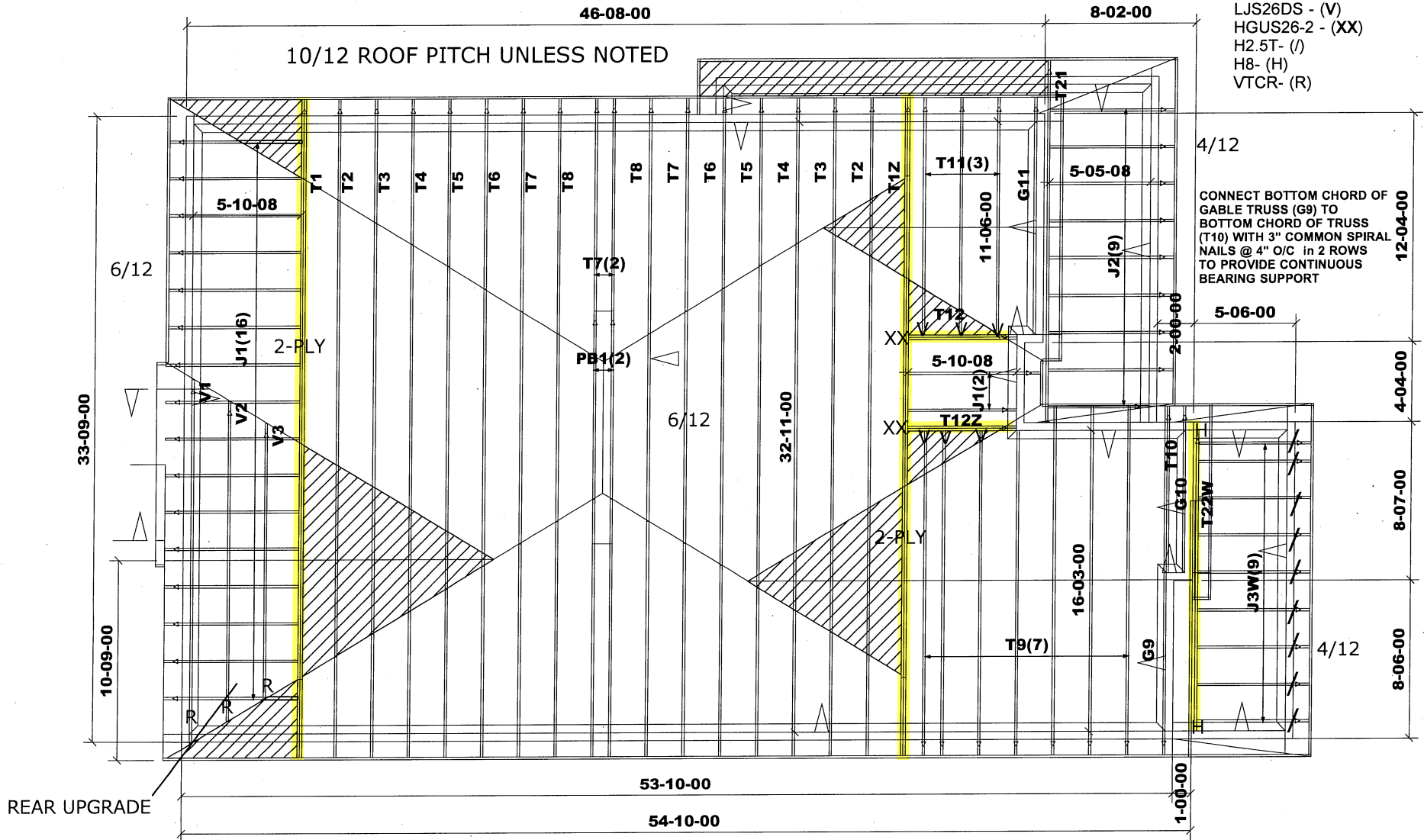
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DENOTES:
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T- (I)
H8- (H)
VTCR- (R)



Job Track: **50465**
Plan Log: **205571**
Layout ID: **423628**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: 2022-06-28 Sales: Rick DiCiano Designer: JG

Model / Elevation:

S42-19 / A-REAR UPGRADE

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REVIEWED

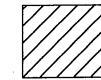
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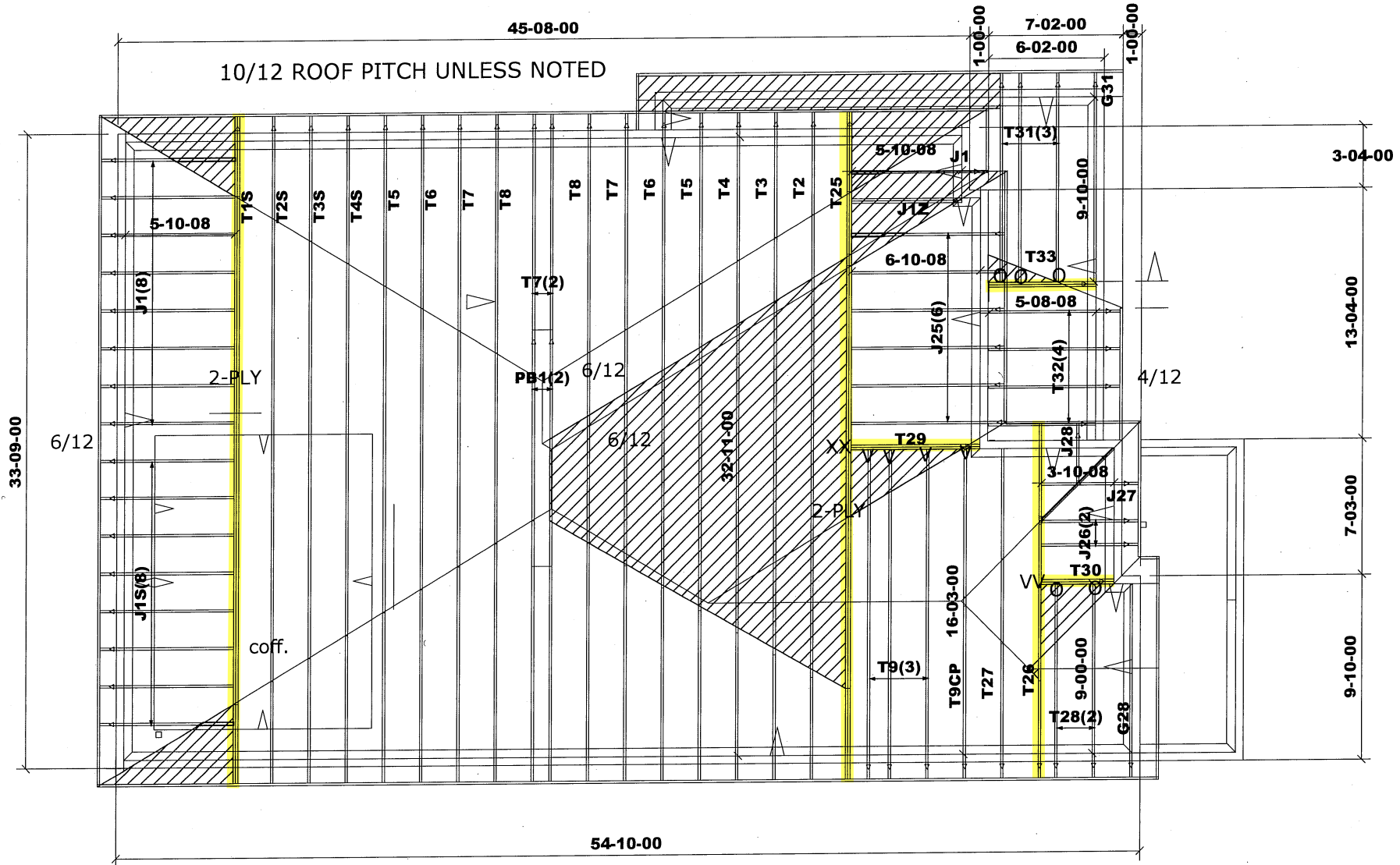
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HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
~~LUS26-2~~ (VV)



DENOTES:
CONVENTIONAL
FRAMING



Job Track: **50465**
Plan Log: **205571**
Layout ID: **423629**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: 2022-06-28 Sales: Rick DiCiano Designer: JG

Model / Elevation:

S42-19 / B-OPT. WITH COFF

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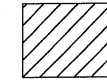
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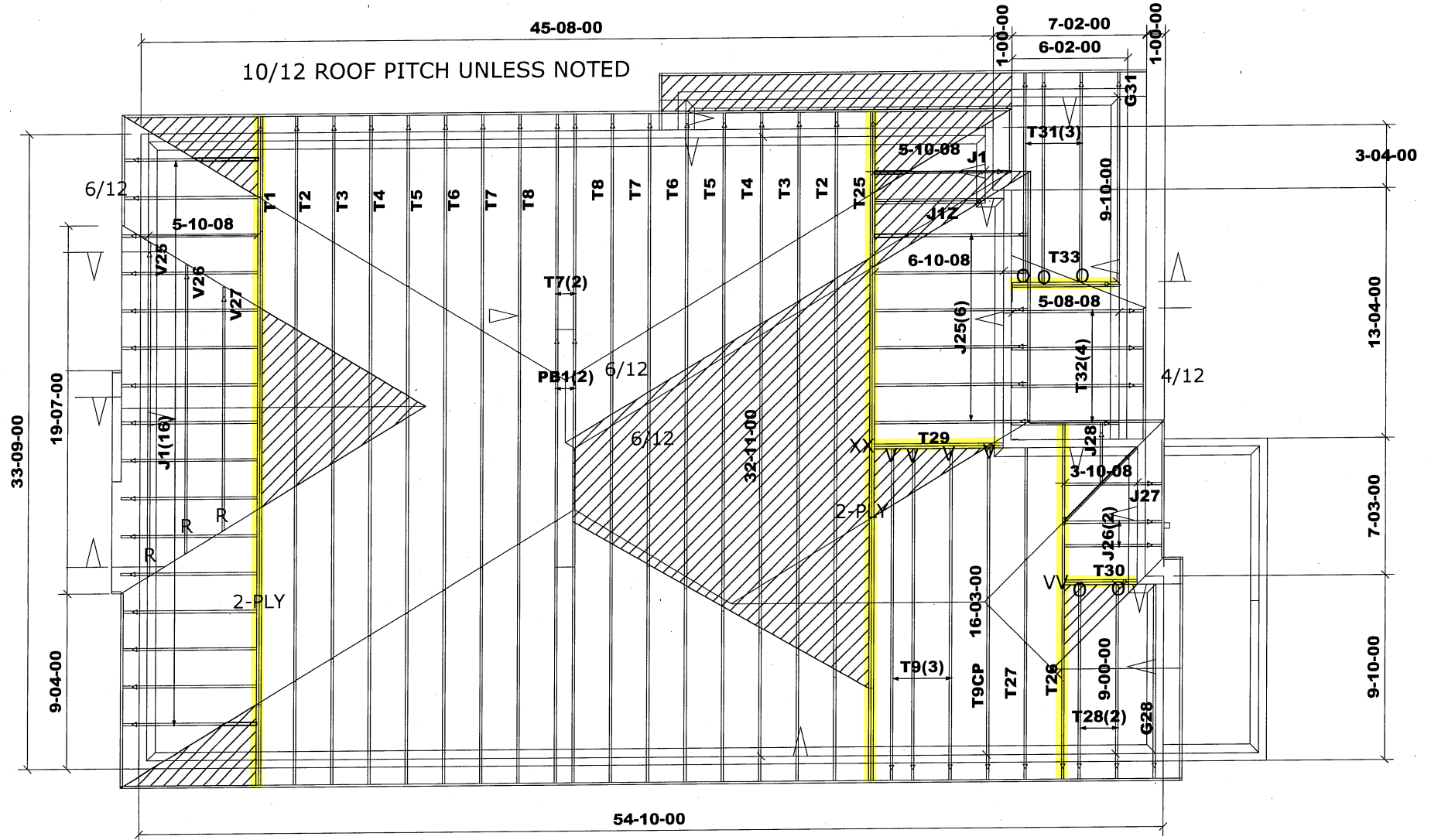
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LJS26DS - (V)
HGUS26-2 - (XX)
VTCR- (R)
LUS26-2- (VV)



DENOTES:
CONVENTIONAL
FRAMING



Job Track: **50465**
Plan Log: **205571**
Layout ID: **423631**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **2022-06-28**

Sales: Rick DiCiano

Designer: JG

Model / Elevation:

S42-19 / B-REAR UPGRADE

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Mitek ver 8.5.3.233

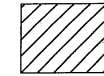
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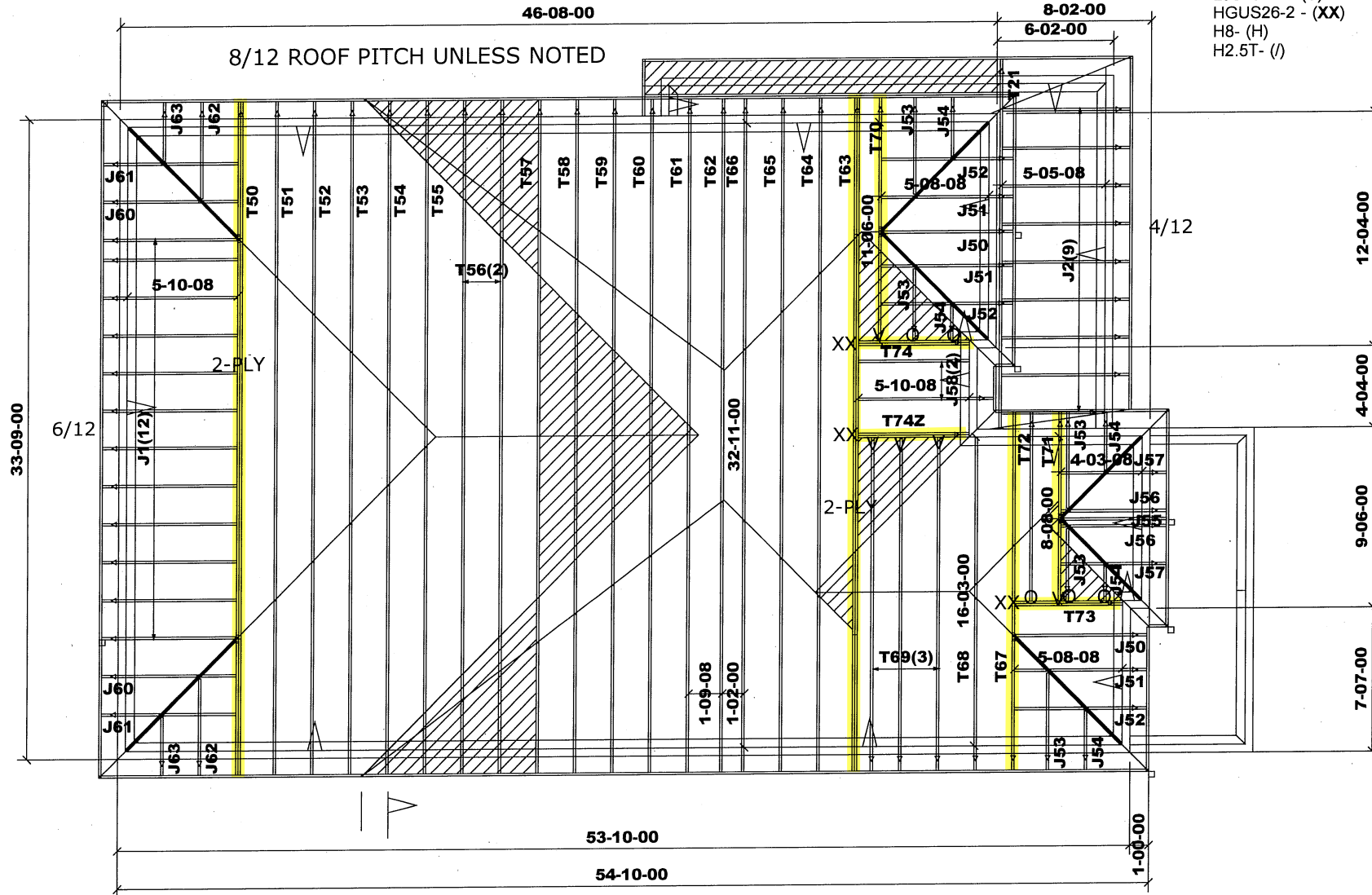
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FRAMING

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H8- (H)
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Job Track: **50465**
Plan Log: **205571**
Layout ID: **423633**

Builder / Location:
BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: 2022-06-28 Sales: Rick DiCiano Designer: JG

Model / Elevation:
S42-19 / C

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Mitek ver 8.5.8.233

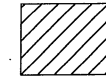
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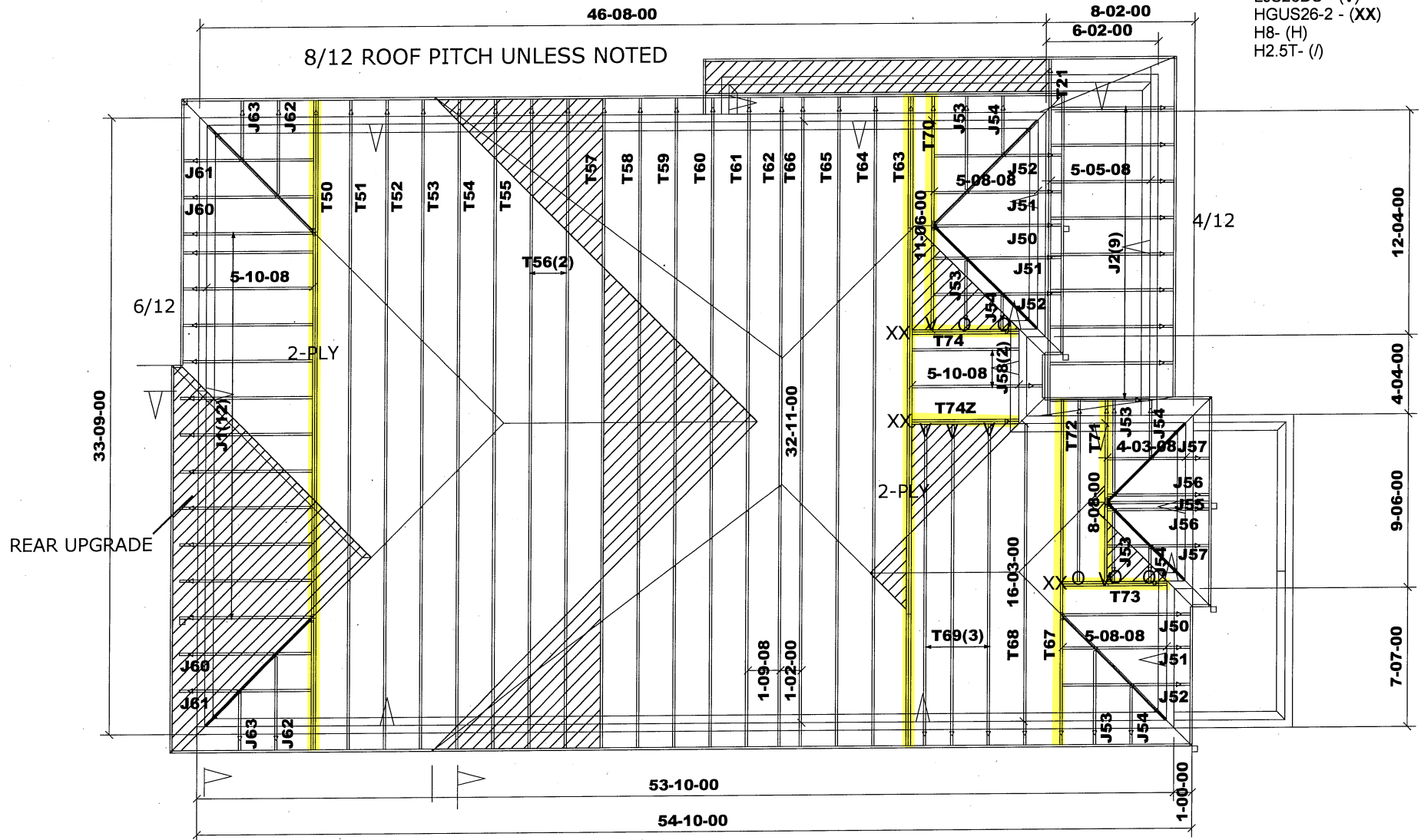
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FRAMING

HARDWARE:
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LJS26DS - (V)
HGUS26-2 - (XX)
H8- (H)
H2.5T- (I)



Job Track: **50465**
Plan Log: **205571**
Layout ID: **423633**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: 2022-06-28 Sales: Rick DiCiano Designer: JG

Model / Elevation:

S42-19 / C-REAR UPGRADE

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Miter ver 8.5.3.233

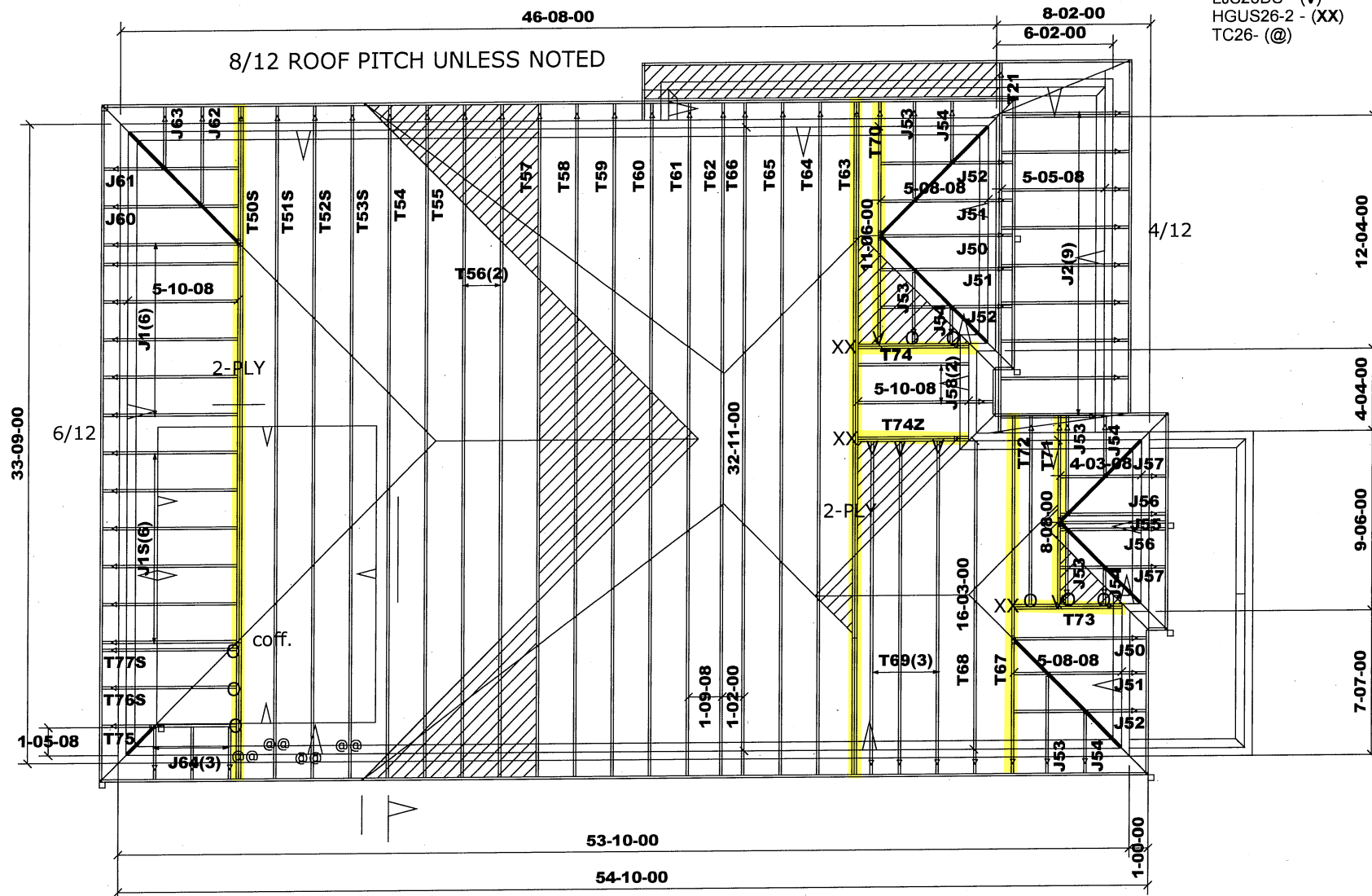
REVIEWED

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2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN LOADS:
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TCDL = 6.0 psf
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DENOTES:
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
TC26- (@)



Layout ID: 423632

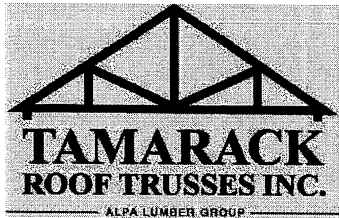
Designer: JG

S42-19 / C-OPT.WITH COFF

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Mitek ver 8.5.3.23

DELIVERY SHIPLIST



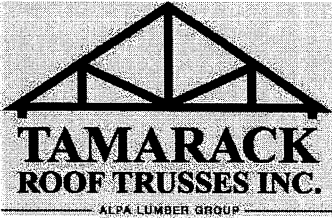
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19
 Lot #:
 Elevation: A

Job Track: 50465
 PlanLog: 205571
 Layout ID: 423627
 Ref #
 Page: 1 of 2
 Date: 06-28-2022
 Designer:
 Sales Rep: Rick DiCiano

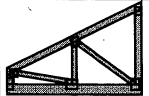
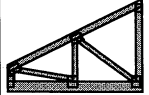
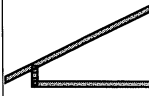
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10 / 12	32-11-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	377.72 229.33		
	1 2-ply	T1Z Hip Girder	10 / 12	32-11-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	377.72 229.33		
	2	T2 Hip	10 / 12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	284.28 179.67		
	2	T3 Hip	10 / 12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	287.31 180.67		
	2	T4 Hip	10 / 12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	320.52 202.33		
	2	T5 Hip	10 / 12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	310.24 196.67		
	2	T6 Hip	10 / 12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	324.22 202.33		
	4	T7 Hip	10 / 12	32-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	645.13 408.00		
	2	T8 Hip	10 / 12	32-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	371.55 230.00		
	7	T9 Common	10 / 12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	561.15 359.33		
	1	G9 GABLE	10 / 12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	81.51 53.33		
	1	T10 Common	10 / 12	7-09-00	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	36.92 23.83		
	1	G10 GABLE	10 / 12	7-09-00	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	38.3 24.83		
	3	T11 Common	10 / 12	11-06-00	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	168.28 111.00		

REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: A		Job Track: 50465 PlanLog: 205571 Layout ID: 423627 Ref # Page: 2 of 2 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano	
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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G11 GABLE	10 /12	11-06-00	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	55.89 37.00		
	1 2-ply	T12 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T12Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T21 Half Hip Girder	10 /12	18-02-08	2-06-13	2 x 6	1-03-08	1-00-07 2-01-12	96.93 63.33		
	1	T22W Flat Girder	0 /12	16-03-00	1-11-02	2 x 6		1-11-02 1-11-02	80.82 52.00		
	2	PB1 Piggyback	10 /12	12-07-04	2-00-00	2 x 4			76.56 51.33		
	18	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	302.3 192.00		
	9	J2 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	130.04 84.00		
	9	J3W Jack-Open	4 /12	4-09-08	2-04-00	2 x 4	1-03-08	3-15 1-11-02	115.98 72.00		

TOTAL # TRUSS= 78

TOTAL BFT OF ALL TRUSSES= 3256.31

BFT.

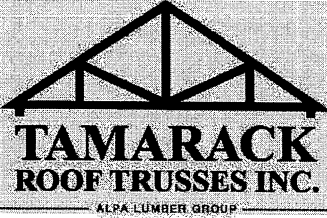
TOTAL WEIGHT OF ALL TRSSES 5158.67 LBS

HARDWARE

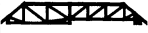
QTY	TYPE	MODEL	LENGTH
6	Hardware	LJS26DS	
9	Hardware	H2.5T	
2	Hardware	H8	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 19

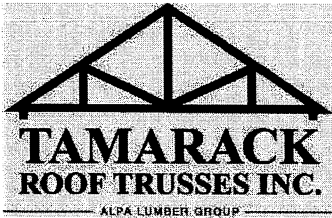
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 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST					
	Lumber Yard: TAMARACK LUMBER				Job Track: 50465	
	Builder: BAYVIEW WELLINGTON				PlanLog: 205571	
	Project: GREEN VALLEY EAST				Layout ID: 423626	
	Location: BRADFORD				Ref #	
	Model: S42-19				Page: 1 of 3	
Lot #:				Date: 06-28-2022		
Elevation: A-OPT.WITH COFF				Designer:		
				Sales Rep: Rick DiCiano		

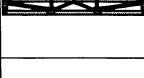
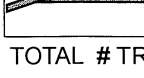
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	10 /12	32-11-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	370.37 227.00		
	1 2-ply	T1Z Hip Girder	10 /12	32-11-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	364.62 224.00		
	1	T2 Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	144.7 91.17		
	1	T2S Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	165.35 106.50		
	1	T3 Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.66 90.33		
	1	T3S Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	157.06 100.00		
	1	T4 Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	160.26 101.17		
	1	T4S Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	164.56 105.17		
	2	T5 Hip	10 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	317.34 200.67		
	2	T6 Hip	10 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	324.22 202.33		
	4	T7 Hip	10 /12	32-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	674.45 424.00		
	2	T8 Hip	10 /12	32-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	371.55 230.00		
	7	T9 Common	10 /12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	561.15 359.33		
	1	G9 GABLE	10 /12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	81.51 53.33		

REVIEWED

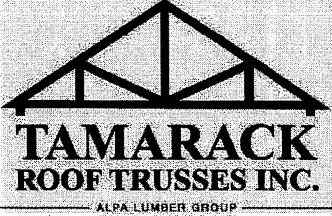
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER						Job Track: 50465	
	Builder: BAYVIEW WELLINGTON						PlanLog: 205571	
	Project: GREEN VALLEY EAST						Layout ID: 423626	
	Location: BRADFORD						Ref #	
	Model: S42-19						Page: 2 of 3	
Lot #:						Date: 06-28-2022		
Elevation: A-OPT.WITH COFF						Designer:		
						Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T10 Common	10 /12	7-09-00	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	36.92 23.83		
	1	G10 GABLE	10 /12	7-09-00	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	38.3 24.83		
	3	T11 Common	10 /12	11-06-00	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	168.28 111.00		
	1	G11 GABLE	10 /12	11-06-00	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	55.89 37.00		
	1 2-ply	T12 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T12Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T21 Half Hip Girder	10 /12	18-02-08	2-06-13	2 x 6	1-03-08	1-00-07 2-01-12	97.42 63.67		
	1	T22W Flat Girder	0 /12	16-03-00	1-11-02	2 x 6		1-11-02 1-11-02	79.42 51.00		
	2	PB1 Piggyback	10 /12	12-07-04	2-00-00	2 x 4			76.56 51.33		
	10	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	8	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	163.12 117.33		
	9	J2 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	130.04 84.00		
	9	J3W Jack-Open	4 /12	4-09-08	2-04-00	2 x 4	1-03-08	3-15 1-11-02	115.98 72.00		

TOTAL # TRUSS= 78 TOTAL BFT OF ALL TRUSSES= 3331.66 BFT. TOTAL WEIGHT OF ALL TRSSES 5245.95 LBS

REVIEWED

DELIVERY SHIPLIST		
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: A-OPT.WITH COFF	Job Track: 50465 PlanLog: 205571 Layout ID: 423626 Ref # Page: 3 of 3 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano

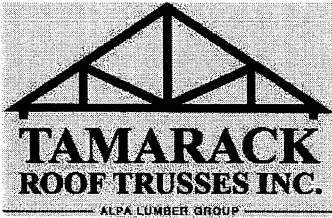
HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
6	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 8

9. H2.5T.
2. H8

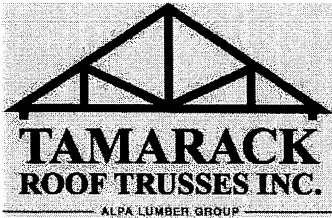
REVIEWED

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST						
	Lumber Yard: TAMARACK LUMBER						Job Track: 50465
	Builder: BAYVIEW WELLINGTON						PlanLog: 205571
	Project: GREEN VALLEY EAST						Layout ID: 423628
	Location: BRADFORD						Ref #
	Model: S42-19						Page: 1 of 3
Lot #:						Date: 06-28-2022	
Elevation: A-REAR UPGRADE						Designer:	
						Sales Rep: Rick DiCiano	

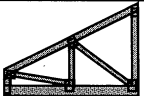
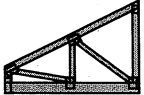
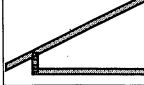
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10 / 12	32-11-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	398.9 245.33		
	1 2-ply	T12 Hip Girder	10 / 12	32-11-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	398.9 245.33		
	2	T2 Hip	10 / 12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	289.41 182.33		
	2	T3 Hip	10 / 12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	293.93 184.67		
	2	T4 Hip	10 / 12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	320.52 202.33		
	2	T5 Hip	10 / 12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	310.24 196.67		
	2	T6 Hip	10 / 12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	309.69 194.33		
	4	T7 Hip	10 / 12	32-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	645.13 408.00		
	2	T8 Hip	10 / 12	32-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	336.16 211.00		
	7	T9 Common	10 / 12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	561.15 359.33		
	1	G9 GABLE	10 / 12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	81.51 53.33		
	1	T10 Common	10 / 12	7-09-00	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	36.92 23.83		
	1	G10 GABLE	10 / 12	7-09-00	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	38.3 24.83		
	3	T11 Common	10 / 12	11-06-00	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	168.28 111.00		

REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: A-REAR UPGRADE		Job Track: 50465 PlanLog: 205571 Layout ID: 423628 Ref # Page: 2 of 3 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano	
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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G11 GABLE	10 / 12	11-06-00	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	55.89 37.00		
	1 2-ply	T12 Jack-Closed Girder	6 / 12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T12Z Jack-Closed Girder	6 / 12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T21 Half Hip Girder	10 / 12	18-02-08	2-06-13	2 x 6	1-03-08	1-00-07 2-01-12	96.93 63.33		
	1	T22W Flat Girder	0 / 12	16-03-00	1-11-02	2 x 6		1-11-02 1-11-02	80.82 52.00		
	1	V1 Valley	10 / 12	19-07-13	8-02-04	2 x 4			77.42 48.67		
	1	V2 Valley	10 / 12	17-03-00	7-02-04	2 x 4			54.76 34.17		
	1	V3 Valley	10 / 12	14-10-04	6-02-04	2 x 4			46.02 29.33		
	2	PB1 Piggyback	10 / 12	12-07-04	2-00-00	2 x 4			76.56 51.33		
	18	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	302.3 192.00		
	9	J2 Jack-Open	4 / 12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	130.04 84.00		
	9	J3W Jack-Open	4 / 12	4-09-08	2-04-00	2 x 4	1-03-08	3-15 1-11-02	115.98 72.00		

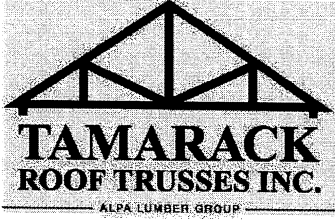
TOTAL # TRUSS= 81 TOTAL BFT OF ALL TRUSSES= 3380.14 BFT. TOTAL WEIGHT OF ALL TRSSES 5341.05 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
9	Hardware	H2.5T	
2	Hardware	H8	

REVIEWED

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
Builder: BAYVIEW WELLINGTON
Project: GREEN VALLEY EAST
Location: BRADFORD
Model: S42-19
Lot #:
Elevation: A-REAR UPGRADE

Job Track: 50465
PlanLog: 205571
Layout ID: 423628
Ref #
Page: 3 of 3
Date: 06-28-2022
Designer:
Sales Rep: Rick DiCiano

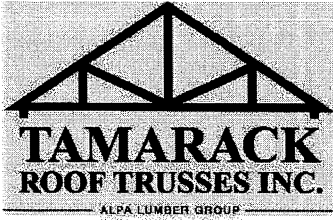
HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
6	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 19

REVIEWED

DELIVERY SHIPLIST



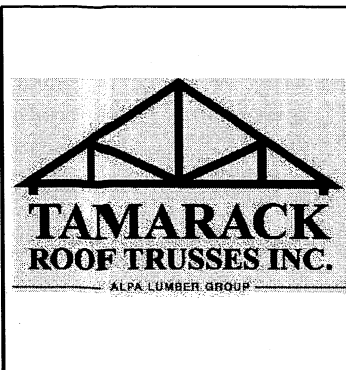
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19
 Lot #:
 Elevation: B

Job Track: 50465
 PlanLog: 205571
 Layout ID: 423630
 Ref #
 Page: 1 of 3
 Date: 06-28-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10 /12	32-11-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	345.85 210.00		
	2	T2 Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	284.28 179.67		
	2	T3 Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	287.31 180.67		
	2	T4 Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	320.52 202.33		
	2	T5 Hip	10 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	310.24 196.67		
	2	T6 Hip	10 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	324.22 202.33		
	4	T7 Hip	10 /12	32-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	645.13 408.00		
	2	T8 Hip	10 /12	32-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	371.55 230.00		
	3	T9 Common	10 /12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	240.49 154.00		
	1	T9CP Hip	10 /12	16-03-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	79.86 51.33		
	1 2-ply	T25 Roof Special Girder	10 /12	32-11-00	4-07-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	351.29 219.33		
	1	T26 Hip Girder	10 /12	16-03-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	82.52 52.50		
	1	T27 Hip	10 /12	16-03-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	78.26 50.67		
	2	T28 Common	10 /12	9-00-00	5-04-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	83.18 53.67		

REVIEWED



DELIVERY SHIPLIST

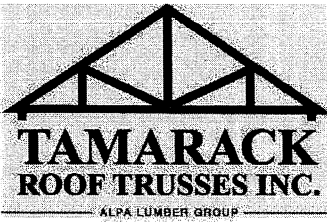
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19
 Lot #:
 Elevation: B

Job Track: 50465
 PlanLog: 205571
 Layout ID: 423630
 Ref #
 Page: 2 of 3
 Date: 06-28-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G28 GABLE	10 / 12	9-00-00	5-04-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	43.69 28.83		
	1 2-ply	T29 Jack-Closed Girder	6 / 12	6-10-08	4-01-04	2 x 4 2 x 6		8-00 4-01-04	59.73 37.33		
	1 2-ply	T30 Jack-Closed Girder	10 / 12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	3	T31 Common	10 / 12	9-10-00	5-06-10	2 x 4	1-03-08 1-03-08	1-00-07 1-00-07	127.95 82.50		
	1	G31 GABLE	10 / 12	9-10-00	5-06-10	2 x 4	1-03-08 1-03-08	1-00-07 1-00-07	41.9 27.50		
	4	T32 Monopitch	4 / 12	5-08-08	2-11-03	2 x 4	1-03-08	3-15 2-02-12	80.05 50.67		
	1 2-ply	T33 Monopitch Girder	4 / 12	5-08-08	2-06-04	2 x 4 2 x 6		3-15 2-02-12	49 32.00		
	2	PB1 Piggyback	10 / 12	12-07-04	2-00-00	2 x 4			76.56 51.33		
	17	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		
	1 2-ply	J1Z Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	33.59 21.33		
	6	J25 Jack-Open	6 / 12	6-10-08	4-07-04	2 x 4	1-03-08	1-02-00 4-07-04	115.26 72.00		
	2	J26 Jack-Open	10 / 12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	30.9 20.33		
	1	J27 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J28 Jack-Open	10 / 12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.99 7.00		

REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: B	Job Track: 50465 PlanLog: 205571 Layout ID: 423630 Ref # Page: 3 of 3 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
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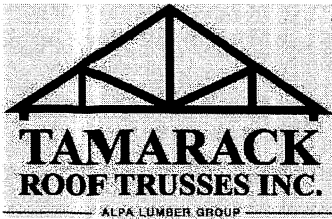
TOTAL # TRUSS= 74 TOTAL BFT OF ALL TRUSSES= 3040.98 BFT. TOTAL WEIGHT OF ALL TRSSES 4814.01 LBS

HARDWARE

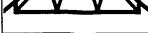
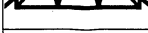
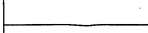
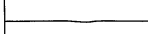
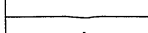
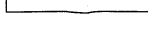
QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
5	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 11

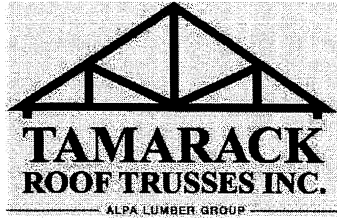
REVIEWED

		DELIVERY SHIPLIST			
		Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: B-REAR UPGRADE		Job Track: 50465 PlanLog: 205571 Layout ID: 423631 Ref # Page: 1 of 3 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10 /12	32-11-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	377.72 229.33		
	2	T2 Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	289.41 182.33		
	2	T3 Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	293.93 184.67		
	2	T4 Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	320.52 202.33		
	2	T5 Hip	10 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	317.34 200.67		
	2	T6 Hip	10 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	329.74 205.67		
	4	T7 Hip	10 /12	32-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	686.98 430.67		
	2	T8 Hip	10 /12	32-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	378.57 234.00		
	3	T9 Common	10 /12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	240.49 154.00		
	1	T9CP Hip	10 /12	16-03-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	79.86 51.33		
	1 2-ply	T25 Roof Special Girder	10 /12	32-11-00	4-07-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	390.32 248.00		
	1	T26 Hip Girder	10 /12	16-03-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	82.52 52.50		
	1	T27 Hip	10 /12	16-03-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	78.26 50.67		
	2	T28 Common	10 /12	9-00-00	5-04-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	83.18 53.67		

REVIEWED



DELIVERY SHIPLIST

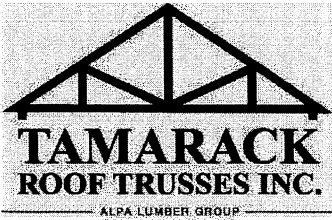
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19
 Lot #:
 Elevation: B-REAR UPGRADE

Job Track: 50465
 PlanLog: 205571
 Layout ID: 423631
 Ref #
 Page: 2 of 3
 Date: 06-28-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G28 GABLE	10 /12	9-00-00	5-04-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	43.69 28.83		
	1 2-ply	T29 Jack-Closed Girder	6 /12	6-10-08	4-01-04	2 x 4 2 x 6		8-00 4-01-04	59.73 37.33		
	1 2-ply	T30 Jack-Closed Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	3	T31 Common	10 /12	9-10-00	5-06-10	2 x 4	1-03-08 1-03-08	1-00-07 1-00-07	127.95 82.50		
	1	G31 GABLE	10 /12	9-10-00	5-06-10	2 x 4	1-03-08 1-03-08	1-00-07 1-00-07	41.9 27.50		
	4	T32 Monopitch	4 /12	5-08-08	2-11-03	2 x 4	1-03-08	3-15 2-02-12	80.05 50.67		
	1 2-ply	T33 Monopitch Girder	4 /12	5-08-08	2-06-04	2 x 4 2 x 6		3-15 2-02-12	49 32.00		
	1	V25 Valley	10 /12	17-08-14	7-04-11	2 x 4			67.85 43.50		
	1	V26 Valley	10 /12	15-04-01	6-04-11	2 x 4			47.79 30.00		
	1	V27 Valley	10 /12	12-11-04	5-04-11	2 x 4			39.05 23.83		
	2	PB1 Piggyback	10 /12	12-07-04	2-00-00	2 x 4			76.56 51.33		
	17	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		
	1 2-ply	J1Z Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	33.59 21.33		
	6	J25 Jack-Open	6 /12	6-10-08	4-07-04	2 x 4	1-03-08	1-02-00 4-07-04	154.81 99.00		

REVIEWED

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER		Job Track:	50465
	Builder:	BAYVIEW WELLINGTON		PlanLog:	205571
	Project:	GREEN VALLEY EAST		Layout ID:	423631
	Location:	BRADFORD		Ref #	
	Model:	S42-19		Page:	3 of 3
Lot #:			Date:	06-28-2022	
Elevation:	B-REAR UPGRADE		Designer:		
			Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J26 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	30.9 20.33		
	1	J27 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J28 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.99 7.00		

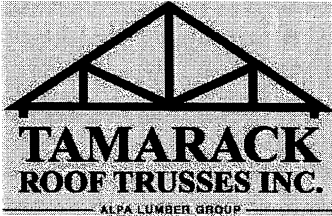
TOTAL # TRUSS= 77 TOTAL BFT OF ALL TRUSSES= 3253.98 BFT. TOTAL WEIGHT OF ALL TRSSES 5152.37 LBS

HARDWARE

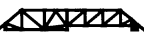
QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
5	Hardware	LUS24	
1	Hardware	LUS26-2	
1	Hardware	HGUS26-2	
3		VTOR	

TOTAL NUMBER OF ITEMS= 14

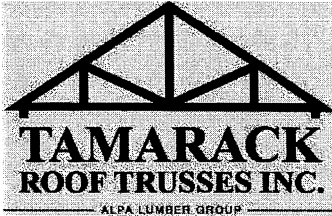
REVIEWED

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: B-OPT.WITH COFF		Job Track: 50465 PlanLog: 205571 Layout ID: 423629 Ref # Page: 1 of 3 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano	
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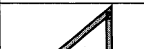
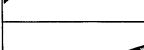
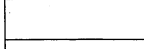
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1S Hip Girder	10 /12	32-11-00	4-01-11	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	370.37 227.00		
	1	T2 Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	142.14 89.83		
	1	T2S Hip	10 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.93 97.50		
	1	T3 Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	143.66 90.33		
	1	T3S Hip	10 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	157.06 100.00		
	1	T4 Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	160.26 101.17		
	1	T4S Hip	10 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	164.56 105.17		
	2	T5 Hip	10 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	310.24 196.67		
	2	T6 Hip	10 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	324.22 202.33		
	4	T7 Hip	10 /12	32-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	645.13 408.00		
	2	T8 Hip	10 /12	32-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	371.55 230.00		
	3	T9 Common	10 /12	16-03-00	8-04-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	240.49 154.00		
	1	T9CP Hip	10 /12	16-03-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	79.86 51.33		
	1 2-ply	T25 Roof Special Girder	10 /12	32-11-00	4-07-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	351.29 219.33		

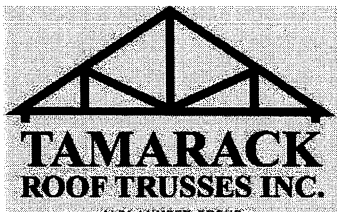
REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST						Job Track: 50465 PlanLog: 205571 Layout ID: 423629 Ref # Page: 2 of 3 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano	
	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: B-OPT.WITH COFF.							

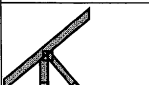
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T26 Hip Girder	10 / 12	16-03-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	82.52 52.50		
	1	T27 Hip	10 / 12	16-03-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	78.26 50.67		
	2	T28 Common	10 / 12	9-00-00	5-04-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	83.18 53.67		
	1	G28 GABLE	10 / 12	9-00-00	5-04-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	43.69 28.83		
	1 2-ply	T29 Jack-Closed Girder	6 / 12	6-10-08	4-01-04	2 x 4 2 x 6		8-00 4-01-04	59.73 37.33		
	1 2-ply	T30 Jack-Closed Girder	10 / 12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	3	T31 Common	10 / 12	9-10-00	5-06-10	2 x 4	1-03-08 1-03-08	1-00-07 1-00-07	127.95 82.50		
	1	G31 GABLE	10 / 12	9-10-00	5-06-10	2 x 4	1-03-08 1-03-08	1-00-07 1-00-07	41.9 27.50		
	4	T32 Monopitch	4 / 12	5-08-08	2-11-03	2 x 4	1-03-08	3-15 2-02-12	80.05 50.67		
	1 2-ply	T33 Monopitch Girder	4 / 12	5-08-08	2-06-04	2 x 4 2 x 6		3-15 2-02-12	49 32.00		
	2	PB1 Piggyback	10 / 12	12-07-04	2-00-00	2 x 4			76.56 51.33		
	9	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	8	J1S Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	163.12 117.33		
	1 2-ply	J1Z Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	33.59 21.33		

REVIEWED

DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	50465
	Builder:	BAYVIEW WELLINGTON	PlanLog:	205571
	Project:	GREEN VALLEY EAST	Layout ID:	423629
	Location:	BRADFORD	Ref #	
	Model:	S42-19	Page:	3 of 3
	Lot #:		Date:	06-28-2022
	Elevation:	B-OPT.WITH COFF	Designer:	
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	6	J25 Jack-Open	6 /12	6-10-08	4-07-04	2 x 4	1-03-08	1-02-00 4-07-04	115.26 72.00		
	2	J26 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	30.9 20.33		
	1	J27 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J28 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.74 7.00		

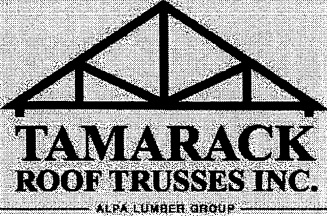
TOTAL # TRUSS= 74 TOTAL BFT OF ALL TRUSSES= 3111.31 BFT. TOTAL WEIGHT OF ALL TRSSES 4893.54 LBS

HARDWARE

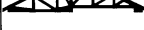
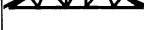
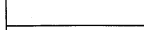
QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
5	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 11

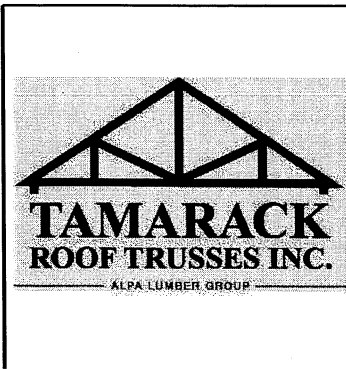
REVIEWED

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: C-OPT.WITH COFF		Job Track: 50465 PlanLog: 205571 Layout ID: 423632 Ref # Page: 1 of 4 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano	
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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T21 Half Hip Girder	10 /12	18-02-08	2-06-13	2 x 6	1-03-08	1-00-07 2-01-12	96.93 63.33		
	1 2-ply	T50S Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	309.65 200.33		
	1	T51S Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	146.78 94.50		
	1	T52S Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	144.41 92.50		
	1	T53S Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	145.79 93.83		
	1	T54 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.02 89.50		
	1	T55 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.6 90.00		
	2	T56 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	286.85 180.67		
	1	T57 Hip	8 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	138.13 87.17		
	1	T58 Hip	8 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	141.7 90.50		
	1	T59 Hip	8 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	149.69 95.50		
	1	T60 Hip	8 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	146.67 92.33		
	1	T61 Hip	8 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	156.8 98.50		
	1	T62 Hip	8 /12	32-11-00	10-00-00	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	162.52 103.00		

REVIEWED



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19
 Lot #:
 Elevation: C-OPT.WITH COFF

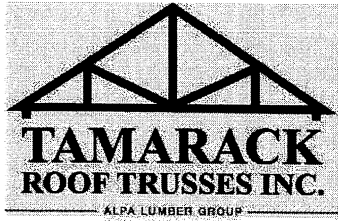
Job Track: 50465
 PlanLog: 205571
 Layout ID: 423632
 Ref #
 Page: 2 of 4
 Date: 06-28-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T63 Hip Girder	8 /12	32-11-00	5-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	312.25 192.33		
	1	T64 Hip	8 /12	32-11-00	6-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.8 94.17		
	1	T65 Hip	8 /12	32-11-00	7-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.91 93.00		
	1	T66 Hip	8 /12	32-11-00	9-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	158.39 99.83		
	1	T67 Hip Girder	8 /12	16-03-00	5-02-10	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	98.44 62.00		
	1	T68 Hip	8 /12	16-03-00	6-06-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	73.04 46.33		
	3	T69 Common	8 /12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	217.38 137.00		
	1	T70 Common Girder	8 /12	11-06-00	5-02-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	51.12 31.83		
	1	T71 Common Girder	8 /12	8-08-00	4-03-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	37.49 23.67		
	1	T72 Half Hip	8 /12	8-08-00	4-07-02	2 x 4	1-03-08	1-04-13 4-07-02	39.81 26.00		
	1 2-ply	T73 Jack-Closed Girder	8 /12	5-08-08	5-02-08	2 x 4 2 x 6		1-04-13 5-02-08	62.54 40.67		
	1 2-ply	T74 Jack-Closed Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1 2-ply	T74Z Jack-Closed Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1	T75 Half Hip Girder	6 /12	5-10-08	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	23.24 16.50		

REVIEWED

DELIVERY SHIPLIST



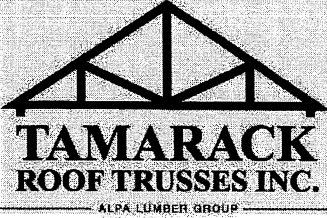
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST
 Location: BRADFORD
 Model: S42-19
 Lot #:
 Elevation: C-OPT.WITH COFF

Job Track: 50465
 PlanLog: 205571
 Layout ID: 423632
 Ref #
 Page: 3 of 4
 Date: 06-28-2022
 Designer:
 Sales Rep: Rick DiCiano

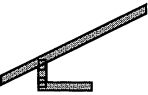
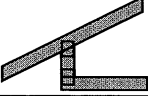
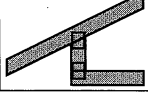
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T76S Half Hip	6 /12	5-10-08	2-10-12	2 x 4	1-03-08	1-02-00 1-10-12	26.1 19.50		
	1	T77S Half Hip	6 /12	5-10-08	3-10-12	2 x 4	1-03-08	1-02-00 2-10-12	26.41 18.67		
	6	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00		
	6	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	122.34 88.00		
	9	J2 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	130.04 84.00		
	2	J50 Jack-Open	8 /12	5-08-08	5-02-08	2 x 4	1-03-08	1-04-13 5-02-08	46.64 30.33		
	3	J51 Jack-Open	8 /12	3-09-07	3-11-02	2 x 4	1-03-08 1-11-01	1-04-13 3-11-02	49.11 31.00		
	3	J52 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 3-11-01	1-04-13 2-07-02	39.83 25.00		
	5	J53 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	59.55 38.33		
	5	J54 Jack-Open	8 /12	1-09-08	2-07-02	2 x 4	1-03-08 1-00	1-04-13 2-07-02	44.55 28.33		
	1	J55 Jack-Open	8 /12	4-03-08	4-03-03	2 x 4	1-03-08	1-04-13 4-03-03	15.45 9.67		
	2	J56 Jack-Open	8 /12	3-09-07	3-11-02	2 x 4	1-03-08 6-01	1-04-13 3-11-02	29.5 19.33		
	2	J57 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-06-01	1-04-13 2-07-02	23.13 15.33		
	2	J58 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	48.32 30.33		

REVIEWED

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST						
	Lumber Yard: TAMARACK LUMBER						Job Track: 50465
	Builder: BAYVIEW WELLINGTON						PlanLog: 205571
	Project: GREEN VALLEY EAST						Layout ID: 423632
	Location: BRADFORD						Ref #
	Model: S42-19						Page: 4 of 4
Lot #:						Date: 06-28-2022	
Elevation: C-OPT.WITH COFF						Designer:	
						Sales Rep: Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J60 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		
	1	J61 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	J62 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J63 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	3	J64 Jack-Open	6 /12	1-05-08	1-10-12	2 x 4	1-03-08	1-02-00 1-10-12	18.38 14.00		

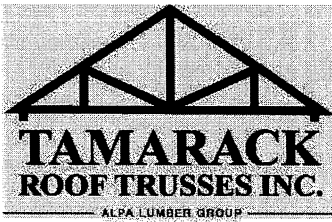
TOTAL # TRUSS= 91 TOTAL BFT OF ALL TRUSSES= 2958.82 BFT. TOTAL WEIGHT OF ALL TRSSES 4628.81 LBS

HARDWARE

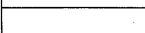
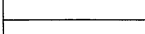
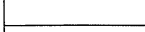
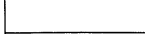
QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
8	Hardware	LUS24	
8	Hardware	TC26	

TOTAL NUMBER OF ITEMS= 21

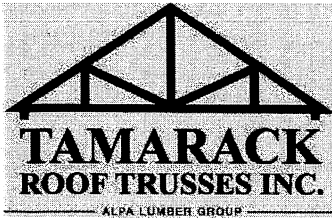
REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST						Job Track: 50465 PlanLog: 205571 Layout ID: 423633		
	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: C						Ref # Page: 1 of 4 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T21 Half Hip Girder	10 /12	18-02-08	2-06-13	2 x 6	1-03-08	1-00-07 2-01-12	96.93 63.33		
	1 2-ply	T50 Hip Girder	6 /12	32-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	300.17 191.33		
	1	T51 Hip	6 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	133.13 84.67		
	1	T52 Hip	6 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	132.96 83.67		
	1	T53 Hip	6 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.27 86.17		
	1	T54 Hip	6 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.67 87.50		
	1	T55 Hip	6 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	139.76 88.00		
	2	T56 Common	6 /12	32-11-00	9-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	286.85 180.67		
	1	T57 Hip	8 /12	32-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	134.74 85.17		
	1	T58 Hip	8 /12	32-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	138.83 88.83		
	1	T59 Hip	8 /12	32-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	149.69 95.50		
	1	T60 Hip	8 /12	32-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	146.67 92.33		
	1	T61 Hip	8 /12	32-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	153.27 96.50		
	1	T62 Hip	8 /12	32-11-00	10-00-00	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	152.04 97.33		

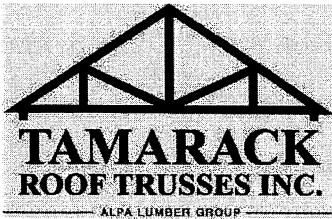
REVIEWED

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST						Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: C		Job Track: 50465 PlanLog: 205571 Layout ID: 423633 Ref # Page: 2 of 4 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano	
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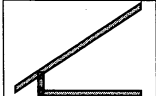
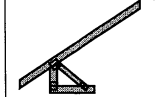
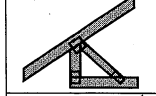
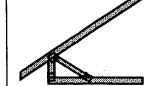
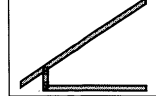
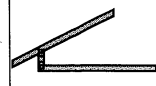
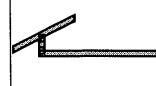
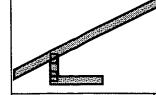
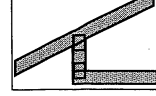
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T63 Hip Girder	8 /12	32-11-00	5-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	312.25 192.33		
	1	T64 Hip	8 /12	32-11-00	6-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.8 94.17		
	1	T65 Hip	8 /12	32-11-00	7-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	145.91 93.00		
	1	T66 Hip	8 /12	32-11-00	9-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	154.75 97.83		
	1	T67 Hip Girder	8 /12	16-03-00	5-02-10	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	95 60.00		
	1	T68 Hip	8 /12	16-03-00	6-06-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	73.04 46.33		
	3	T69 Common	8 /12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	217.38 137.00		
	1	T70 Common Girder	8 /12	11-06-00	5-02-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	51.12 31.83		
	1	T71 Common Girder	8 /12	8-08-00	4-03-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	37.49 23.67		
	1	T72 Half Hip	8 /12	8-08-00	4-07-02	2 x 4	1-03-08	1-04-13 4-07-02	39.81 26.00		
	1 2-ply	T73 Jack-Closed Girder	8 /12	5-08-08	5-02-08	2 x 4 2 x 6		1-04-13 5-02-08	62.54 40.67		
	1 2-ply	T74 Jack-Closed Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1 2-ply	T74Z Jack-Closed Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	12	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.53 128.00		

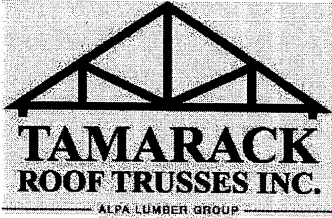
REVIEWED

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: BAYVIEW WELLINGTON Project: GREEN VALLEY EAST Location: BRADFORD Model: S42-19 Lot #: Elevation: C	Job Track: 50465 PlanLog: 205571 Layout ID: 423633 Ref # Page: 3 of 4 Date: 06-28-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	J2 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	130.04 84.00		
	2	J50 Jack-Open	8 /12	5-08-08	5-02-08	2 x 4	1-03-08	1-04-13 5-02-08	34.82 21.33		
	3	J51 Jack-Open	8 /12	3-09-07	3-11-02	2 x 4	1-03-08 1-11-01	1-04-13 3-11-02	49.11 31.00		
	3	J52 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 3-11-01	1-04-13 2-07-02	39.83 25.00		
	5	J53 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	58.54 38.33		
	5	J54 Jack-Open	8 /12	1-09-08	2-07-02	2 x 4	1-03-08 1-00	1-04-13 2-07-02	44.55 28.33		
	1	J55 Jack-Open	8 /12	4-03-08	4-03-03	2 x 4	1-03-08	1-04-13 4-03-03	15.45 9.67		
	2	J56 Jack-Open	8 /12	3-09-07	3-11-02	2 x 4	1-03-08 6-01	1-04-13 3-11-02	29.5 19.33		
	2	J57 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-06-01	1-04-13 2-07-02	23.13 15.33		
	2	J58 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	35.66 21.33		
	2	J60 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J61 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J62 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J63 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		

REVIEWED

		DELIVERY SHIPLIST	
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track: 50465
	Builder:	BAYVIEW WELLINGTON	PlanLog: 205571
	Project:	GREEN VALLEY EAST	Layout ID: 423633
	Location:	BRADFORD	Ref #
	Model:	S42-19	Page: 4 of 4
	Lot #:		Date: 06-28-2022
	Elevation:	C	Designer:
			Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
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TOTAL # TRUSS= 89
TOTAL BFT OF ALL TRUSSES= 2820.15 BFT.
TOTAL WEIGHT OF ALL TRSSES 4454.28 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
5	Hardware	LJS26DS	
5	Hardware	LUS24	

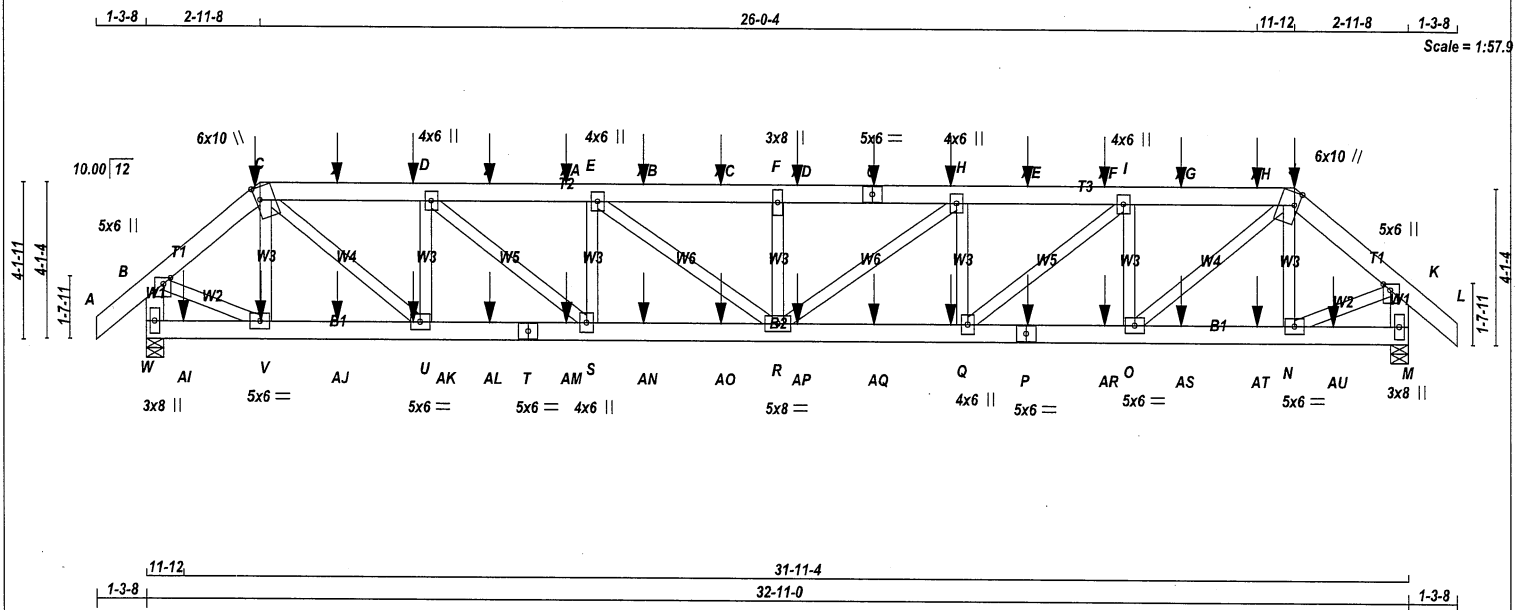
TOTAL NUMBER OF ITEMS= 13

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423628	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 M&T Industries, Inc.
Fri Jun 24 15:24:13 2022
Page 1
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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x6 DRY No.2 SPF C - G 2x6 DRY No.2 SPF G - J 2x6 DRY No.2 SPF J - L 2x6 DRY No.2 SPF W - B 2x6 DRY No.2 SPF M - K 2x6 DRY No.2 SPF W - T 2x6 DRY No.2 SPF T - P 2x6 DRY No.2 SPF P - M 2x6 DRY No.2 SPF ALL WEBS 2x4 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A - C 2 12 SIDE(122.0) C - G 2 12 SIDE(183.1) G - J 2 12 SIDE(183.1) J - L 2 12 SIDE(122.0) W - B 2 12 TOP M - K 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS W - T 2 12 SIDE(183.1) T - P 2 12 SIDE(183.1) P - M 2 12 SIDE(0.0) WEBS : (0.122"x3") SPIRAL NAILS 2x4 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQRD GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX W 3790 0 3790 0 0 5-8 5-8 M 3720 0 3720 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL W 2656 1884 / 0 0 / 0 0 / 0 771 / 0 0 / 0 M 2605 1854 / 0 0 / 0 0 / 0 751 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) C H O R D S W E B S MAX. FACTORED MAX. FACTORED MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX. UNBRAC. MEMB. FORCE MAX. (LBS) (PLF) (LC) LENGTH FR-TO (LBS) (LC) FR-TO A-B 0 / 52 -112.4 -112.4 0.05 (1) 10.00 V-C -859 / 0 0.07 (1) B-C -3740 / 0 -112.4 -112.4 0.06 (1) 5.78 C-U 0 / 3709 0.33 (1) C-X -5673 / 0 -112.4 -112.4 0.17 (1) 4.81 U-D -2262 / 0 0.19 (1) X-Y -5673 / 0 -112.4 -112.4 0.17 (1) 4.81 D-S 0 / 2279 0.20 (1) Y-D -5673 / 0 -112.4 -112.4 0.17 (1) 4.81 S-E -1315 / 0 0.11 (1) D-Z -7419 / 0 -112.4 -112.4 0.19 (1) 4.30 E-R 0 / 839 0.07 (1) Z-AA -7419 / 0 -112.4 -112.4 0.19 (1) 4.30 R-F -881 / 0 0.08 (1) AA-E -7419 / 0 -112.4 -112.4 0.19 (1) 4.30 R-H 0 / 857 0.08 (1) E-AB -8088 / 0 -112.4 -112.4 0.22 (1) 4.12 Q-H -1320 / 0 0.11 (1) AB-AC -8088 / 0 -112.4 -112.4 0.22 (1) 4.12 Q-I 0 / 2287 0.20 (1) AC-F -8088 / 0 -112.4 -112.4 0.22 (1) 4.12 O-I -2288 / 0 0.20 (1) F-AD -8088 / 0 -112.4 -112.4 0.23 (1) 4.11 O-J 0 / 3749 0.33 (1) AD-G -8088 / 0 -112.4 -112.4 0.23 (1) 4.11 N-J -845 / 0 0.07 (1) G-H -8088 / 0 -112.4 -112.4 0.23 (1) 4.11 B-V 0 / 3034 0.27 (1) H-AE -7405 / 0 -112.4 -112.4 0.20 (1) 4.30 N-K 0 / 2980 0.26 (1) AE-AF -7405 / 0 -112.4 -112.4 0.20 (1) 4.30 AF-I -7405 / 0 -112.4 -112.4 0.20 (1) 4.30 I-AG -5653 / 0 -112.4 -112.4 0.17 (1) 4.82 AG-AH -5653 / 0 -112.4 -112.4 0.17 (1) 4.82 AH-J -5653 / 0 -112.4 -112.4 0.17 (1) 4.82 J-K -3673 / 0 -112.4 -112.4 0.06 (1) 5.82 K-L 0 / 52 -112.4 -112.4 0.05 (1) 10.00 W-B -3765 / 0 0.0 0.0 14 (1) 7.27 M-K -3705 / 0 0.0 0.0 13 (1) 7.32 W-AI 0 / 0 -18.5 -18.5 0.03 (4) 10.00 AI-V 0 / 0 -18.5 -18.5 0.03 (4) 10.00 V-AJ 0 / 2922 -18.5 -18.5 0.21 (1) 10.00 AJ-AK 0 / 2922 -18.5 -18.5 0.21 (1) 10.00 AK-U 0 / 2922 -18.5 -18.5 0.21 (1) 10.00 U-AL 0 / 5674 -18.5 -18.5 0.41 (1) 10.00 AL-T 0 / 5674 -18.5 -18.5 0.41 (1) 10.00 T-AM 0 / 5674 -18.5 -18.5 0.41 (1) 10.00 AM-S 0 / 5674 -18.5 -18.5 0.41 (1) 10.00 S-AN 0 / 7419 -18.5 -18.5 0.53 (1) 10.00 AN-AO 0 / 7419 -18.5 -18.5 0.53 (1) 10.00 AO-R 0 / 7419 -18.5 -18.5 0.53 (1) 10.00 R-AP 0 / 7405 -18.5 -18.5 0.53 (1) 10.00 AP-AQ 0 / 7405 -18.5 -18.5 0.53 (1) 10.00 AQ-Q 0 / 7405 -18.5 -18.5 0.53 (1) 10.00 Q-P 0 / 5654 -18.5 -18.5 0.41 (1) 10.00 P-AR 0 / 5654 -18.5 -18.5 0.41 (1) 10.00				TOTAL WEIGHT = 2 X 199 = 399 lb DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.10") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.17") ALLOWABLE DEFL.(TL)= L/360 (1.10") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.30") CSI: TC=0.23/1.00 (F-H:1) , BC=0.53/1.00 (Q-R:1) , WB=0.33/1.00 (J-O:1) , SSI=0.16/1.00 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (Psi) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.80 (C) (INPUT = 0.90) JSI METAL= 0.53 (T) (INPUT = 1.00)	
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Structural component only
DWG# T-2215369

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423628	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	10.0	4.00	1.25
D, E, H, I						
D	TMWW+t	MT20	4.0	6.0		
F	TMW+w	MT20	3.0	8.0		
G	TS-t	MT20	5.0	6.0		
J	TTWW+m	MT20	6.0	10.0	4.00	1.25
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	8.0		
N, O, U, V						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	4.0	6.0		
T	BS-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
AR-O	0/5654	-18.5	-18.5	0.41 (1)	10.00		
O-AS	0/2771	-18.5	-18.5	0.21 (1)	10.00		
AS-AT	0/2771	-18.5	-18.5	0.21 (1)	10.00		
AT-N	0/2771	-18.5	-18.5	0.21 (1)	10.00		
N-AU	0/0	-18.5	-18.5	0.03 (4)	10.00		
AU-M	0/0	-18.5	-18.5	0.03 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-98	-98	---	FRONT	VERT	TOTAL	---	C1
C	2-11-8	-159	-159	---	FRONT	VERT	SNOW	---	C1
G	18-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
H	20-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
J	29-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
J	29-11-8	-159	-159	---	FRONT	VERT	SNOW	---	C1
P	22-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Q	20-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	2-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	4-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Y	6-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Z	8-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AA	10-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AB	12-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AC	14-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AD	16-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AE	22-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AF	24-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AG	26-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AH	28-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AI	11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	4-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	6-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	8-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	10-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	12-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	14-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	16-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	18-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	24-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	26-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	28-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	30-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

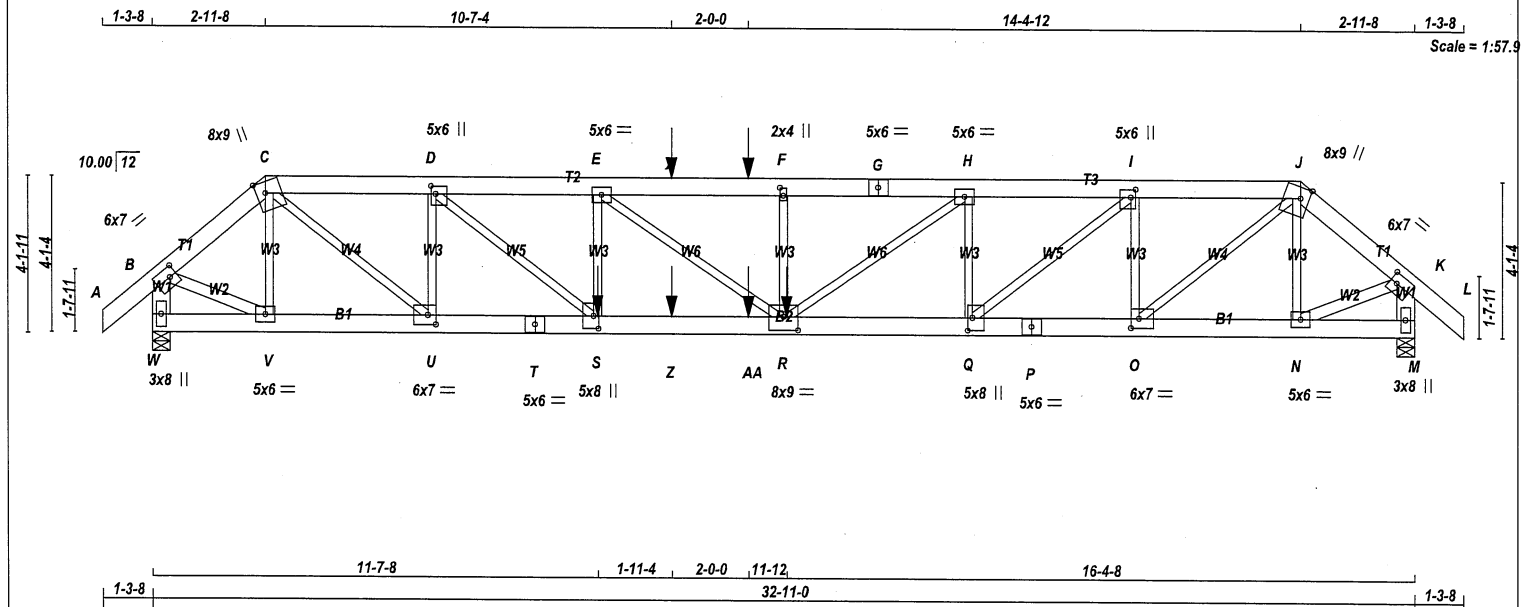
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
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REVIEWED



TOTAL WEIGHT = 2 X 182 = 365 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - V	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A - C	2	12	TOP
C - G	2	12	SIDE(0.0)
G - J	2	12	TOP
J - L	2	12	TOP
W - B	2	12	TOP
M - K	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
W - T	2	12	TOP
T - P	2	12	SIDE(197.8)
P - M	2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS			
E - S	1	6	SIDE(256.3)
F - R	1	4	SIDE(467.8)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.



Structural component only
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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	4898	0	4898	0	0	5-8	5-8
M	4350	0	4350	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3420	2493 / 0	0 / 0	0 / 0	0 / 0	927 / 0	0 / 0
M	3038	2209 / 0	0 / 0	0 / 0	0 / 0	830 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W. M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	UNBRAC	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)			(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A - B	0 / 52	-112.4 -112.4	0.05 (†)	10.00	V - C	-1184 / 0	0.14 (†)
B - C	-4993 / 0	-112.4 -112.4	0.08 (†)	5.16	U - D	0 / 6167	0.76 (†)
C - D	-8517 / 0	-112.4 -112.4	0.21 (†)	4.04	U - D	-3917 / 0	0.47 (†)
D - E	-12654 / 0	-112.4 -112.4	0.34 (†)	3.28	D - S	0 / 5406	0.67 (†)
E - X	-13955 / 0	-112.4 -112.4	0.52 (†)	2.96	S - E	-1619 / 0	0.19 (†)
X - Y	-13955 / 0	-112.4 -112.4	0.52 (†)	2.96	E - R	0 / 1629	0.20 (†)
Y - F	-13955 / 0	-112.4 -112.4	0.52 (†)	2.96	R - F	-686 / 0	0.08 (†)
F - G	-13955 / 0	-112.4 -112.4	0.45 (†)	3.04	R - H	0 / 3781	0.47 (†)
G - H	-13955 / 0	-112.4 -112.4	0.45 (†)	3.04	H - Q	-2791 / 0	0.33 (†)
H - I	-10936 / 0	-112.4 -112.4	0.27 (†)	3.58	Q - I	0 / 4589	0.57 (†)
I - J	-7425 / 0	-112.4 -112.4	0.17 (†)	4.32	O - I	-3421 / 0	0.41 (†)
J - K	-4390 / 0	-112.4 -112.4	0.07 (†)	5.43	O - J	0 / 5338	0.66 (†)
K - L	0 / 52	-112.4 -112.4	0.05 (†)	10.00	N - J	-1025 / 0	0.12 (†)
W - B	-4934 / 0	0.0 0.0	0.18 (†)	6.54	B - V	0 / 4057	0.36 (†)
M - K	-4381 / 0	0.0 0.0	0.16 (†)	6.85	N - K	0 / 3568	0.32 (†)
W - V	0 / 0	-18.5 -18.5	0.04 (†)	10.00			
V - U	0 / 3779	-18.5 -18.5	0.14 (†)	10.00			
U - T	0 / 8517	-18.5 -18.5	0.26 (†)	10.00			
T - S	0 / 8517	-18.5 -18.5	0.26 (†)	10.00			
S - Z	0 / 12654	-18.5 -18.5	0.41 (†)	10.00			
Z - AA	0 / 12654	-18.5 -18.5	0.41 (†)	10.00			
AA - R	0 / 12654	-18.5 -18.5	0.41 (†)	10.00			
R - Q	0 / 10936	-18.5 -18.5	0.36 (†)	10.00			
Q - P	0 / 7425	-18.5 -18.5	0.22 (†)	10.00			
P - O	0 / 7425	-18.5 -18.5	0.22 (†)	10.00			
O - N	0 / 3323	-18.5 -18.5	0.12 (†)	10.00			
N - M	0 / 0	-18.5 -18.5	0.04 (†)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	16-6-8	-1778	-1778	---	BACK	VERT	TOTAL	---	C1
S	11-7-8	-1211	-1211	---	BACK	VERT	TOTAL	---	C1
X	13-6-12	-93	-93	---	BACK	VERT	TOTAL	---	C1
Y	15-6-12	-93	-93	---	BACK	VERT	TOTAL	---	C1
Z	13-6-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AA	15-6-12	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.10")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.29")
 ALLOWABLE DEFL.(TL)= $L/360$ (1.10")
 CALCULATED VERT. DEFL.(TL) = $L/784$ (0.50")

CSI: TC=0.52/1.00 (E-F:1) , BC=0.41/1.00 (R-S:1) ,
WB=0.76/1.00 (C-U:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (U) (INPUT = 0.90)
JSI METAL= 0.81 (T) (INPUT = 1.00)

REVIEWED CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	T1Z	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	7.0	3.00	2.25
C	TTWW+m	MT20	8.0	9.0	Edge	2.75
D	TMWW+t	MT20	5.0	6.0	2.50	1.50
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMWW+t	MT20	5.0	6.0	2.50	1.50
J	TTWW+m	MT20	8.0	9.0	Edge	2.75
K	TMVW-t	MT20	6.0	7.0	3.00	2.25
M	BMV1+p	MT20	3.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	6.0	7.0	3.00	2.50
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	8.0	4.00	1.50
R	BMWWW-t	MT20	8.0	9.0	4.25	4.50
S	BMWW+t	MT20	5.0	8.0	4.00	1.50
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	6.0	7.0	3.00	2.50
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



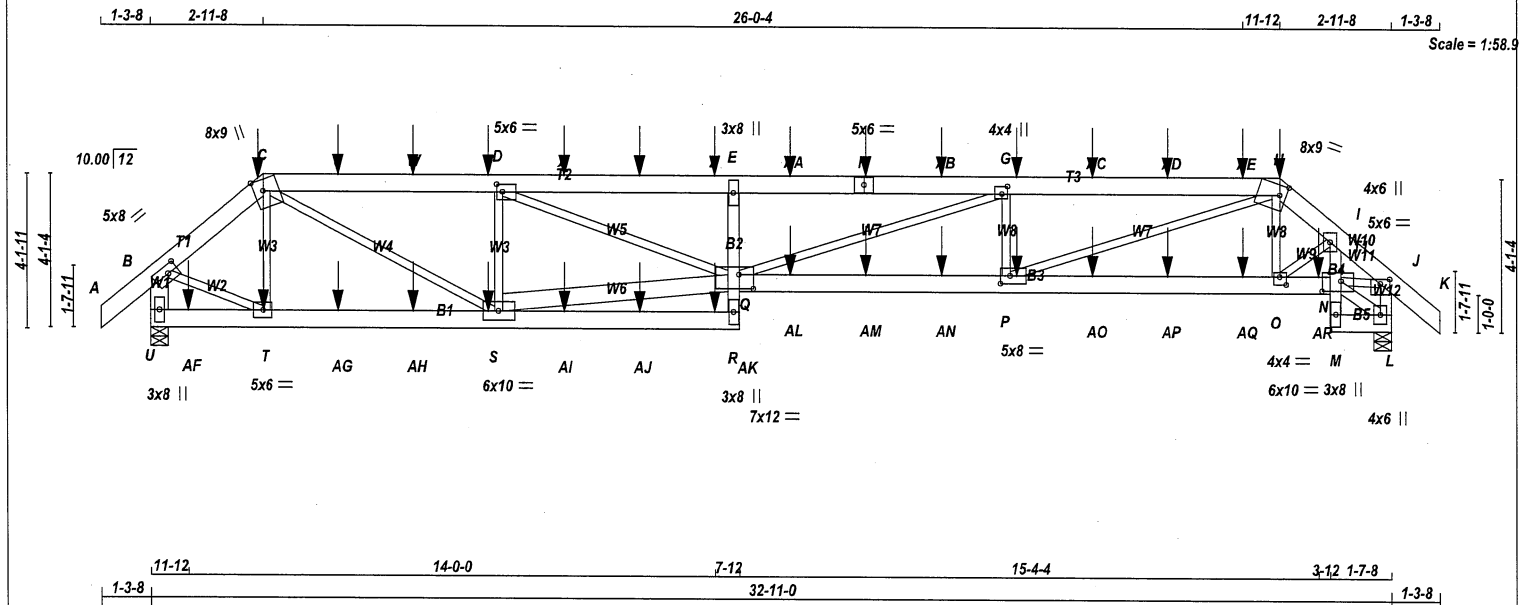
Structural component only
DWG# T-2215351

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	T1S	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 185 = 370 lb

LUMBER			LUMBER		
N. L. G. A. RULES	CHORDS	SIZE	DRY	No.2	DESCR.
A - C	2x6	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - K	2x6	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - E	2x4	DRY	No.2	SPF	
Q - N	2x6	DRY	No.2	SPF	
M - I	2x4	DRY	No.2	SPF	
M - L	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
N - L	2x4	DRY	No.2	SPF	
S - Q	2x6	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	SIDE(122.0)
C - F 2	12	SIDE(183.1)
F - H 2	12	SIDE(183.1)
H - K 2	12	SIDE(122.0)
U - B 2	12	TOP
L - J 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U - R 2	12	SIDE(183.1)
Q - N 2	12	SIDE(183.1)
M - L 2	12	TOP
R - E 1	12	TOP
I - M 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	
2x6 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG
U	3954	0	3954	0	5-8
L	4161	0	4161	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
U	2770	1965 / 0	0 / 0	0 / 0	0 / 0
L	2913	2078 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM TO	CSI (LC)	FR-TO	
A-B	0 / 52	-112.4 -112.4	0.05 (1)	10.00	T-C -872 / 0
B-C	-3932 / 0	-112.4 -112.4	0.07 (1)	5.67	O-H 0 / 317
C-V	-7079 / 0	-112.4 -112.4	0.36 (1)	4.21	O-I 0 / 433
V-W	-7079 / 0	-112.4 -112.4	0.36 (1)	4.21	B-T 0 / 3195
W-D	-7079 / 0	-112.4 -112.4	0.36 (1)	4.21	N-L -162 / 0
D-X	-11958 / 0	-112.4 -112.4	0.54 (1)	3.17	N-J 0 / 4001
X-Y	-11958 / 0	-112.4 -112.4	0.54 (1)	3.17	C-S 0 / 4750
Y-Z	-11958 / 0	-112.4 -112.4	0.54 (1)	3.17	S-D -3291 / 0
Z-E	-11958 / 0	-112.4 -112.4	0.54 (1)	3.17	S-Q 0 / 6870
E-AA	-12107 / 0	-112.4 -112.4	0.62 (1)	3.11	D-Q 0 / 5191
AA-F	-12107 / 0	-112.4 -112.4	0.62 (1)	3.11	P-H 0 / 6800
F-AB	-12107 / 0	-112.4 -112.4	0.62 (1)	3.11	Q-G 0 / 1343
AB-G	-12107 / 0	-112.4 -112.4	0.62 (1)	3.11	P-G -1891 / 0
G-AC	-10846 / 0	-112.4 -112.4	0.61 (1)	3.27	
AC-AD	-10846 / 0	-112.4 -112.4	0.61 (1)	3.27	
AD-AE	-10846 / 0	-112.4 -112.4	0.61 (1)	3.27	
AE-H	-10846 / 0	-112.4 -112.4	0.61 (1)	3.27	
H-I	-5692 / 0	-112.4 -112.4	0.16 (1)	4.80	
I-J	-5397 / 0	-112.4 -112.4	0.16 (1)	4.90	
J-K	0 / 52	-112.4 -112.4	0.05 (1)	10.00	
U-B	-3956 / 0	0.0	0.14 (1)	7.13	
L-J	-4053 / 0	0.0	0.23 (1)	5.88	
U-AF	0 / 0	-18.5 -18.5	0.06 (1)	10.00	
AF-T	0 / 0	-18.5 -18.5	0.06 (1)	10.00	
T-AG	0 / 2978	-18.5 -18.5	0.25 (1)	10.00	
AG-AH	0 / 2978	-18.5 -18.5	0.25 (1)	10.00	
AH-S	0 / 2978	-18.5 -18.5	0.25 (1)	10.00	
S-AI	0 / 394	-18.5 -18.5	0.09 (4)	10.00	
AI-AJ	0 / 394	-18.5 -18.5	0.09 (4)	10.00	
AJ-AK	0 / 394	-18.5 -18.5	0.09 (4)	10.00	
AK-R	0 / 394	-18.5 -18.5	0.09 (4)	10.00	
R-Q	0 / 109	0.0	0.30 (1)	10.00	
Q-E	-1084 / 0	0.0	0.31 (1)	7.81	
Q-AL	0 / 10846	-18.5 -18.5	0.88 (1)	10.00	
AL-AM	0 / 10846	-18.5 -18.5	0.88 (1)	10.00	
AM-AN	0 / 10846	-18.5 -18.5	0.88 (1)	10.00	
AN-P	0 / 10846	-18.5 -18.5	0.88 (1)	10.00	
P-AO	0 / 4446	-18.5 -18.5	0.49 (1)	10.00	
AO-AP	0 / 4446	-18.5 -18.5	0.49 (1)	10.00	
AP-AQ	0 / 4446	-18.5 -18.5	0.49 (1)	10.00	
AQ-O	0 / 4446	-18.5 -18.5	0.49 (1)	10.00	

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.38")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/584 (0.68")

CSI: TC=0.62/1.00 (E-G:1), BC=0.88/1.00 (P-Q:1), WB=0.84/1.00 (H-P:1), SSI=0.22/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.



Structural component only
DWG# T-2215350

REVIEWED

CONTINUED ON PAGE 2

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	Edge	2.75
D	TMVW-t	MT20	5.0	6.0	2.50	1.75
E	TMV+p	MT20	3.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW-t	MT20	4.0	4.0	2.50	1.75
H	TTWW-m	MT20	8.0	9.0	3.25	2.00
I	TMVW+p	MT20	4.0	6.0		
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMVW1+p	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	8.0		
N	BVMVW-I	MT20	6.0	10.0	3.25	6.00
O	BMVW-t	MT20	4.0	4.0	2.50	2.00
P	BMVW-t	MT20	5.0	8.0	2.50	3.00
Q	BVMVWW-I	MT20	7.0	12.0	4.50	4.50
R	BMV+p	MT20	3.0	8.0		
S	BMVWW-t	MT20	6.0	10.0		
T	BMVW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
O-AR	0 / 4096	-18.5	-18.5	0.47 (1)	10.00		
AR-N	0 / 4096	-18.5	-18.5	0.47 (1)	10.00		
M-N	0 / 15	0.0	0.0	0.11 (1)	10.00		
N-I	-386 / 0	0.0	0.0	0.10 (1)	7.81		
M-L	0 / 132	-18.5	-18.5	0.01 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-98	-98	---	FRONT	VERT	TOTAL	---	C1
C	2-11-8	-159	-159	---	FRONT	VERT	SNOW	---	C1
D	8-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
F	18-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
G	22-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
H	29-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
H	29-11-8	-159	-159	---	FRONT	VERT	SNOW	---	C1
P	22-11-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
S	8-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	2-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	4-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
W	6-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
X	10-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Y	12-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
Z	14-11-12	-93	-93	---	FRONT	VERT	TOTAL	---	C1
AA	16-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
AB	20-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
AC	24-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
AD	26-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	C1
AE	28-11-12	-95	-95	---	FRONT	VERT	TOTAL	---	C1
AF	11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	4-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	6-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AI	10-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	12-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	14-11-12	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AL	16-11-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
AM	18-11-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
AN	20-11-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
AO	24-11-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
AP	26-11-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
AQ	28-11-12	-72	-72	---	FRONT	VERT	TOTAL	---	C1
AR	30-11-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.73 (Q) (INPUT = 1.00)

Structural component only
DWG# T-2215350

REVIEWED

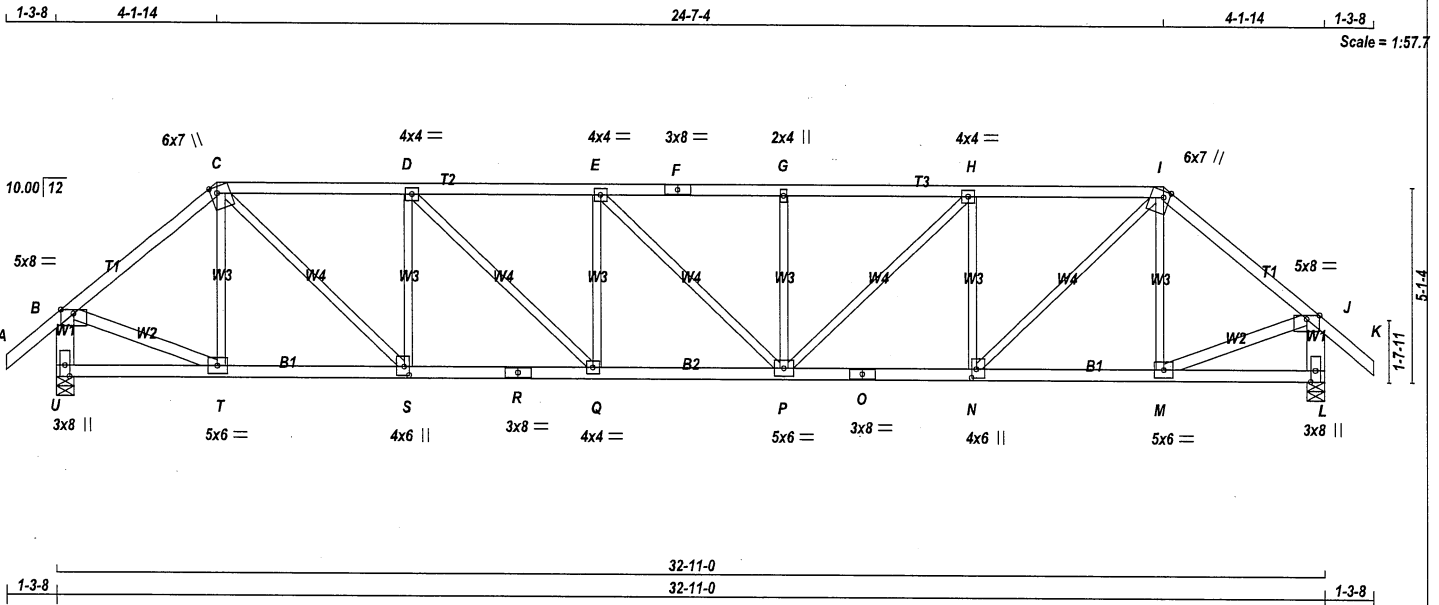


Structural component only
DWG# T-2215350

REVIEWED

JOB NAME 423626	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:c3jyij23uDijq_8pvRKbkZpy75XW-ETMMtiebhsTG9s5dgdhQ87FbwMwdUnH?qo8D6?z32Ow



TOTAL WEIGHT = 145 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
B - T	2x4	DRY	No.2
M - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	7.0	2.00	2.00
D, E, H						
D	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMVW+m	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	7.0	2.00	2.00
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	8.0	Edge	
M	BMVW-t	MT20	5.0	6.0		
N	BMVW+t	MT20	4.0	6.0	2.75	1.50
O	BS-t	MT20	3.0	8.0		
P	BMVWVW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	8.0		
S	BMVW+t	MT20	4.0	6.0	2.75	1.50
T	BMVW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
U	2311	0	2311	0	0	5-8	5-8	
L	2311	0	2311	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		PERM.LIVE		WIND		DEAD		SOIL	
	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE
U	1617	1159 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	0 / 0	0 / 0	0 / 0
L	1617	1159 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
A-B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	T-C	-407 / 0	0.16 (1)
B-C	-2157 / 0	-112.4 -112.4	0.45 (1)	4.18	C-S	0 / 1905	0.43 (1)
C-D	-3017 / 0	-112.4 -112.4	0.57 (1)	3.48	S-D	-1234 / 0	0.47 (1)
D-E	-3649 / 0	-112.4 -112.4	0.65 (1)	3.11	D-Q	0 / 888	0.20 (1)
E-F	-3647 / 0	-112.4 -112.4	0.53 (1)	3.28	Q-E	-532 / 0	0.20 (1)
F-G	-3647 / 0	-112.4 -112.4	0.53 (1)	3.28	E-P	-3 / 0	0.00 (1)
G-H	-3647 / 0	-112.4 -112.4	0.65 (1)	3.11	P-G	-531 / 0	0.20 (1)
H-I	-3017 / 0	-112.4 -112.4	0.58 (1)	3.48	P-H	0 / 885	0.20 (1)
I-J	-2156 / 0	-112.4 -112.4	0.45 (1)	4.18	N-H	-1233 / 0	0.47 (1)
J-K	0 / 50	-112.4 -112.4	0.15 (1)	10.00	N-I	0 / 1906	0.43 (1)
U-B	-2281 / 0	0.0 0.0	0.16 (1)	6.79	M-I	-408 / 0	0.16 (1)
L-J	-2281 / 0	0.0 0.0	0.16 (1)	6.79	B-T	0 / 1728	0.28 (1)
					M-J	0 / 1728	0.28 (1)
U-T	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
T-S	0 / 1644	-18.5 -18.5	0.32 (1)	10.00			
S-R	0 / 3017	-18.5 -18.5	0.53 (1)	10.00			
R-Q	0 / 3017	-18.5 -18.5	0.53 (1)	10.00			
Q-P	0 / 3649	-18.5 -18.5	0.64 (1)	10.00			
P-O	0 / 3017	-18.5 -18.5	0.53 (1)	10.00			
O-N	0 / 3017	-18.5 -18.5	0.53 (1)	10.00			
N-M	0 / 1643	-18.5 -18.5	0.32 (1)	10.00			
M-L	0 / 0	-18.5 -18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.36")

CSI: TC=0.65/1.00 (G-H:1), BC=0.64/1.00 (P-Q:1), WB=0.47/1.00 (D-S:1), SSI=0.26/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.93 (O) (INPUT = 1.00)



Structural component only
DWG# T-2215352

REVIEWED

REVIEWED CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	T2S	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:c3jy123uDiiq 8pvRKbkZpv75XW-jfwk42fDS9b7m0gpEKfhLnquiDVC93StmeRz32Ov

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2215353

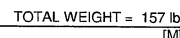
REVIEWED



JSI GRIP= 0.85 (L) (INPUT = 0.90)
JSI METAL= 0.80 (P) (INPUT = 1.00)



Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 15:13:10 2022 Page 1
ID:c3jyj23uDiq_8pvRkKbZpy75XW-f22UVkgT_nrr0KqBMIE7mmt3vZybh1_SXmMijz32Ot



REVIEWED CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	T3S	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 15:13:10 2022 Page 2
ID:c3jvj23uDijq 8pvRKbkZpv75XW-f22UVkgT nrr0KqBMIE7mmt3vZybh1 SXmMtjJz32Ot

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2215355

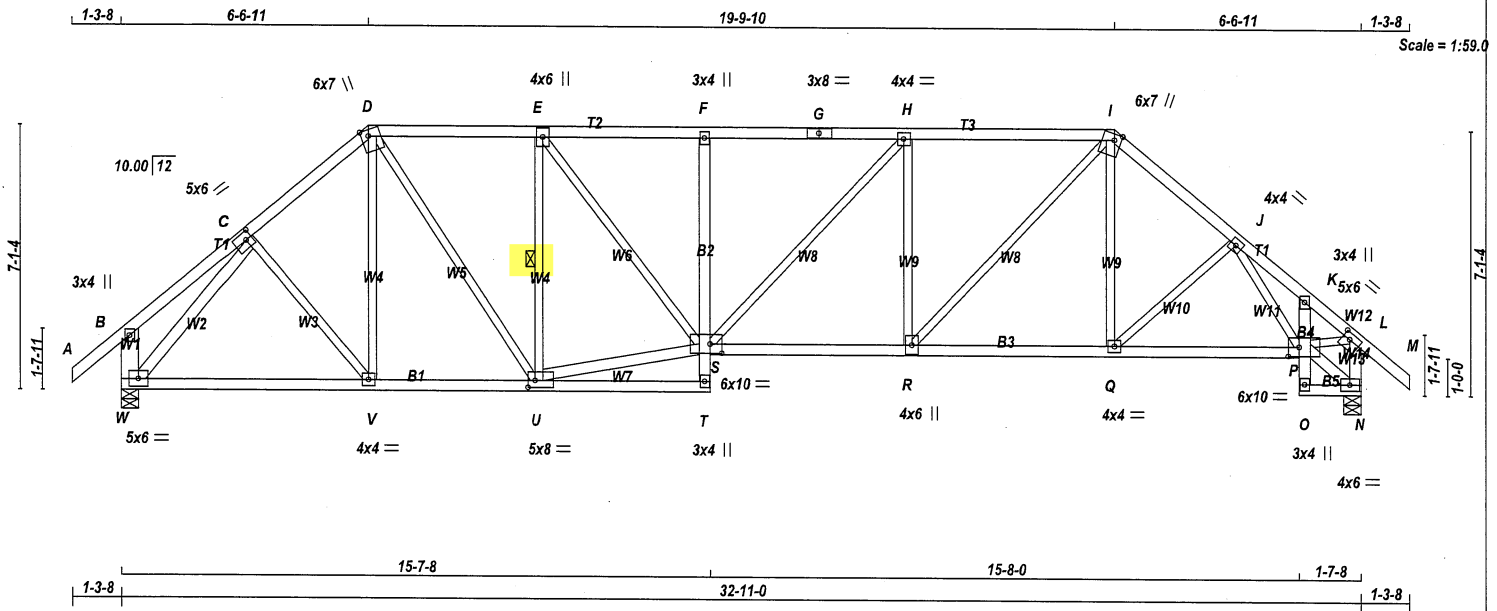
REVIEWED

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	T4S	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Fri Jun 24 15:13:11 2022 Page 1
ID:c3jy23uDijq_8pvRkKbZpy75XW-7Ectj4h6l4zidUOovSIMlZPFzIDQX6blQ6QFmz32Oos



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N - L, G, A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
W - C	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B, F, K						
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	7.0	2.00	2.25
E	TMVW+t	MT20	4.0	6.0		
G	TS-t	MT20	3.0	8.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	7.0	2.00	2.25
J	TMVW-t	MT20	4.0	4.0		
L	TMVW-t	MT20	5.0	6.0	2.00	2.50
N	BMVW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BMVWWW-t	MT20	6.0	10.0	3.00	3.50
Q	BMVW-t	MT20	4.0	4.0		
R	BMVW+t	MT20	4.0	6.0		
S	BMVWWW-t	MT20	6.0	10.0	3.00	3.75
T	BMV+p	MT20	3.0	4.0		
U	BMVWWW-t	MT20	5.0	8.0	2.25	2.25
V	BMVW-t	MT20	4.0	4.0		
W	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
W	2307	0	0	2307	0	0	5-8	5-8	5-8
N	2314	0	0	2314	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX / MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
W	1614	1157 / 0	0 / 0	0 / 0	457 / 0
N	1619	1161 / 0	0 / 0	0 / 0	458 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 50	C-V	0 / 137
B-C	0 / 25	D-V	0 / 76
C-D	-2211 / 0	D-U	0 / 1224
D-E	-2347 / 0	E-U	-1463 / 0
E-F	-3052 / 0	F-U	0 / 2357
F-G	-3062 / 0	G-U	0 / 1146
G-H	-3062 / 0	H-U	0 / 258
H-I	-2886 / 0	I-U	-886 / 0
I-J	-2560 / 0	J-U	0 / 1365
J-K	-2384 / 0	K-U	0 / 234
K-L	-2456 / 0	L-U	-188 / 0
L-M	0 / 50	M-U	-413 / 0
M-N	-298 / 0	N-U	-2519 / 0
N-L	-2253 / 0	P-N	-84 / 0
		P-L	0 / 1866

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/999 (0.28")

CSI: TC=0.73/1.00 (F-H:1), BC=0.53/1.00 (R-S:1), WB=0.74/1.00 (C-W:1), SSI=0.29/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.59 (L) (INPUT = 1.00)



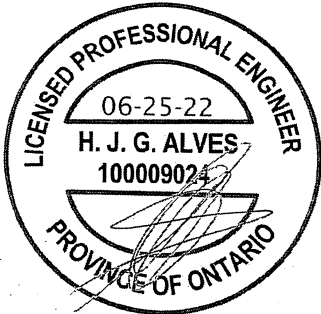
Structural component only
DWG# T-2215357

REVIEWED CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	T4S	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 15:13:11 2022 Page 2
ID:c3jvj23uDijq 8pvRKbkZpy75XW-7Ectj4h6l4zidUOOvSIMlzpFezIDQX6blQ6QFmz32Os

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



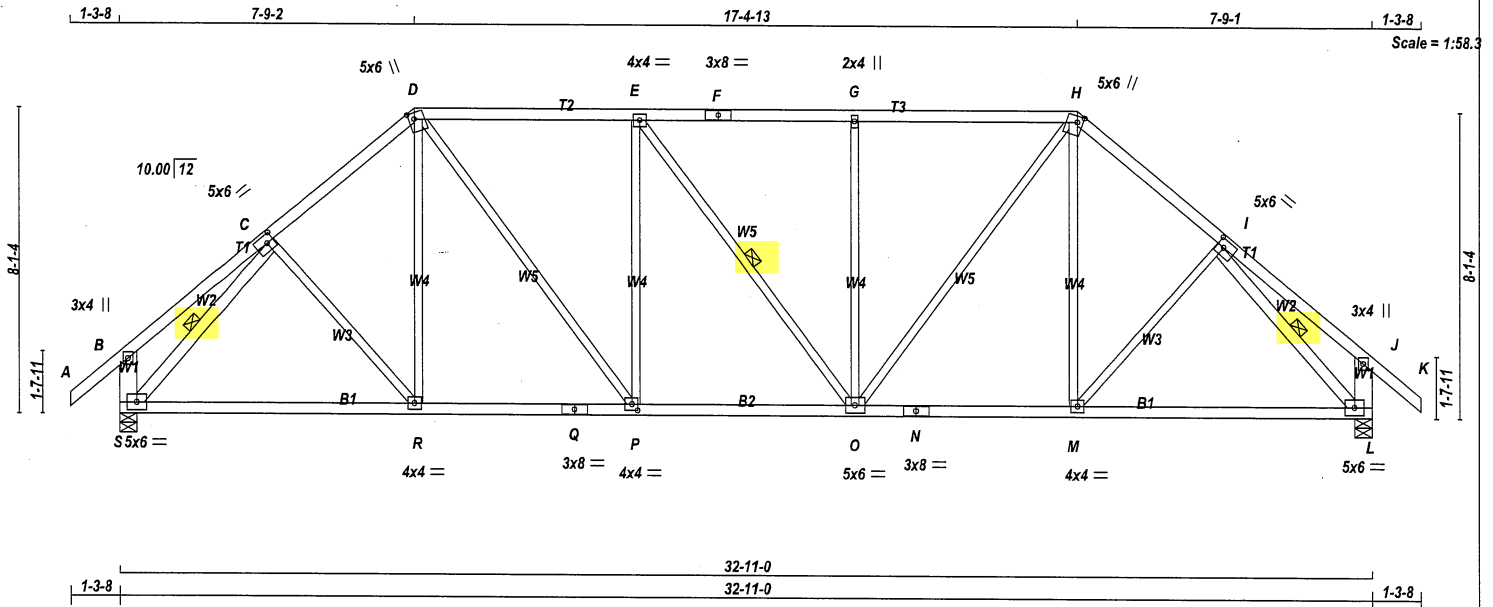
Structural component only
DWG# T-2215357

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423626	T5	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 15:13:12 2022 Page 1
ID:c3jy123uDi1q_8pvRKbkZpy75XW-bQAFwQikWO5ZFdaTAHBrByRHHNg79y0k_4r_nCz32Or



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x4 DRY No.2 SPF F - H 2x4 DRY No.2 SPF H - K 2x4 DRY No.2 SPF S - B 2x6 DRY No.2 SPF L - J 2x6 DRY No.2 SPF S - Q 2x4 DRY No.2 SPF Q - N 2x4 DRY No.2 SPF N - L 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT S - C 2x4 DRY No.2 SPF I - L 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				BEARINGS FACTORED MAXIMUM FACTORED INPUT REQRD GROSS REACTION GROSS REACTION BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX S 2311 0 2311 0 0 5-8 5-8 L 2311 0 2311 0 0 5-8 5-8				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.10") CALCULATED VERT. DEFL.(LL) = L/999 (0.10") ALLOWABLE DEFL.(TL)= L/360 (1.10") CALCULATED VERT. DEFL.(TL) = L/999 (0.19") CSI: TC=0.67/1.00 (D-E:1) , BC=0.42/1.00 (O-P:1) , WB=0.89/1.00 (E-P:1) , SSI=0.31/1.00 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (P.S.I) (P.L.I) (P.L.I) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (P) (INPUT = 0.90) JSI METAL= 0.57 (C) (INPUT = 1.00)				UNFACTORED REACTIONS 1ST LCASE MAX/MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL S 1617 1159 / 0 0 / 0 0 / 0 458 / 0 0 / 0 L 1617 1159 / 0 0 / 0 0 / 0 458 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O, C-S, I-L. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MAX. FACTORED MAX. FACTORED MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX MEMB. FORCE MAX (LBS) (PLF) (LC) UNBRAC (LBS) (LC) FR-TO FROM TO LENGTH FR-TO A-B 0 / 50 -112.4 -112.4 0.15 (1) 10.00 C-R 0 / 54 0.02 (4) B-C 0 / 32 -112.4 -112.4 0.26 (1) 10.00 R-D 0 / 142 0.04 (4) C-D -2188 / 0 -112.4 -112.4 0.36 (1) 4.27 D-P 0 / 999 0.22 (1) D-E -2253 / 0 -112.4 -112.4 0.67 (1) 3.78 P-E -704 / 0 0.89 (1) E-F -2251 / 0 -112.4 -112.4 0.67 (1) 3.78 E-O -3 / 0 0.00 (1) F-G -2251 / 0 -112.4 -112.4 0.67 (1) 3.78 O-G -703 / 0 0.89 (1) G-H -2251 / 0 -112.4 -112.4 0.66 (1) 3.80 O-H 0 / 996 0.22 (1) H-I -2188 / 0 -112.4 -112.4 0.36 (1) 4.27 M-H 0 / 143 0.04 (4) I-J 0 / 32 -112.4 -112.4 0.26 (1) 10.00 M-I 0 / 55 0.02 (4) J-K 0 / 50 -112.4 -112.4 0.15 (1) 10.00 S-C -2535 / 0 0.44 (1) S-B -322 / 0 0.0 0.0 0.02 (1) 7.81 I-L -2534 / 0 0.44 (1) L-J -322 / 0 0.0 0.0 0.02 (1) 7.81 S-R 0 / 1645 -18.5 -18.5 0.40 (1) 10.00 R-Q 0 / 1654 -18.5 -18.5 0.40 (1) 10.00 Q-P 0 / 1654 -18.5 -18.5 0.40 (1) 10.00 P-O 0 / 2253 -18.5 -18.5 0.42 (1) 10.00 O-N 0 / 1654 -18.5 -18.5 0.40 (1) 10.00 N-M 0 / 1654 -18.5 -18.5 0.40 (1) 10.00 M-L 0 / 1644 -18.5 -18.5 0.40 (1) 10.00			
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PLATES (table is in inches)	TYPE	SIZE	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.00	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.75
Q	BS-t	MT20	3.0	8.0		
R	BMVW-t	MT20	4.0	4.0		
S	BMVW-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



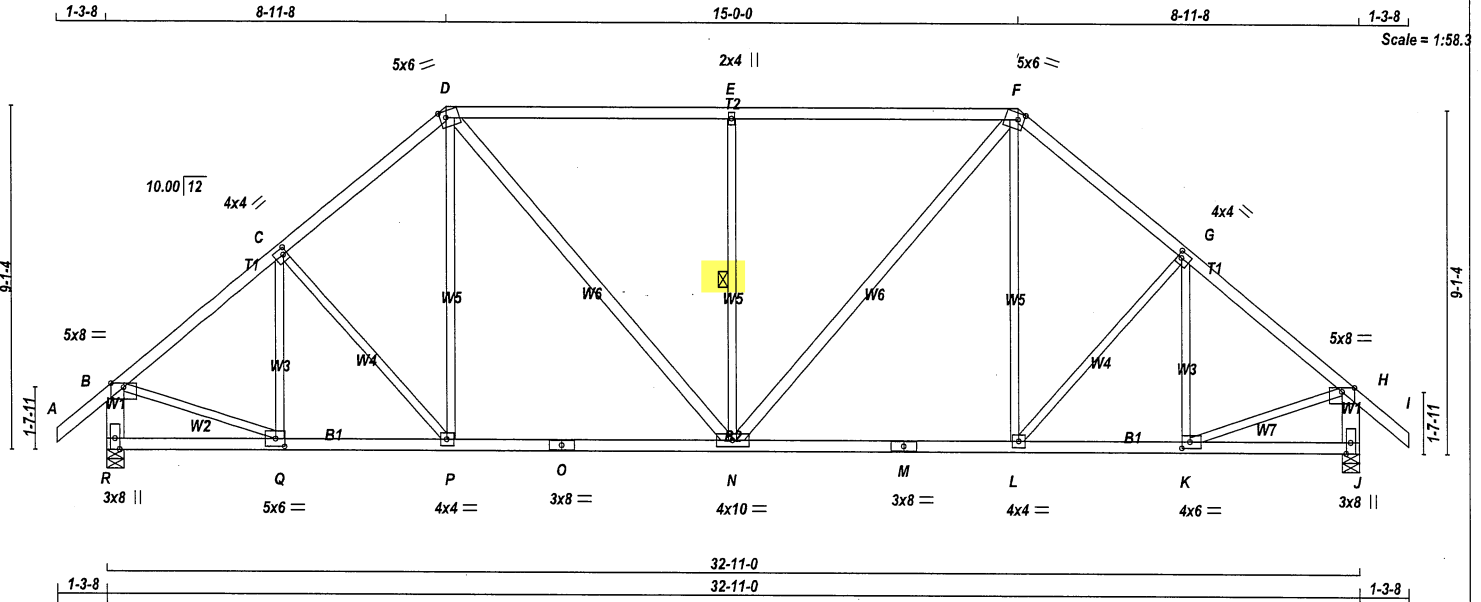
Structural component only
DWG# T-2215358

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	T6	2	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Fri Jun 24 15:13:13 2022 Page 1
ID:c3jyj23uDiq_8pvrKbkZpy75XW-3djd7mjMHiDPtnYm1toqOOUYtm14uUQuDkbXJez32Oq



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x4 DRY No.2 SPF F - I 2x4 DRY No.2 SPF R - B 2x6 DRY No.2 SPF J - H 2x6 DRY No.2 SPF R - O 2x4 DRY No.2 SPF O - M 2x4 DRY No.2 SPF M - J 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT D - N 2x4 DRY No.2 SPF N - F 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.10") CALCULATED VERT. DEFL.(LL) = L/999 (0.09") ALLOWABLE DEFL.(TL)= L/360 (1.10") CALCULATED VERT. DEFL.(TL) = L/999 (0.18") CSI: TC=0.94/1.00 (E-F:1), BC=0.38/1.00 (N-P:1), WB=0.56/1.00 (E-N:1), SSI=0.41/1.00 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.86 (Q) (INPUT = 0.90) JSI METAL= 0.58 (M) (INPUT = 1.00)			
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-p MT20 5.0 8.0 Edge C TMVW-t MT20 4.0 4.0 2.00 1.25 D TTWW-m MT20 5.0 6.0 2.00 2.00 E TMVW-w MT20 2.0 4.0 F TTWW-m MT20 5.0 6.0 2.00 2.00 G TMVW-t MT20 4.0 4.0 2.00 1.25 H TMVW-p MT20 5.0 8.0 Edge J BMV1+p MT20 3.0 8.0 Edge K BMVW-t MT20 4.0 6.0 2.00 2.50 L BMVW-t MT20 4.0 4.0 M BS-t MT20 3.0 8.0 N BMVW-w MT20 4.0 10.0 O BS-t MT20 3.0 8.0 P BMVW-t MT20 4.0 4.0 Q BMVW-t MT20 5.0 6.0 2.50 2.75 R BMV1+p MT20 3.0 8.0 Edge Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.				BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX R 2311 0 2311 0 0 5-8 5-8 J 2311 0 2311 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL R 1617 1159 / 0 0 / 0 0 / 0 458 / 0 0 / 0 J 1617 1159 / 0 0 / 0 0 / 0 458 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) CHORDS W E B S MEMB. MAX. FACTORED FORCE VERT. LOAD LC1 MAX. FACTORED FORCE MAX. FACTORED FORCE (LBS) (PLF) (LC) UNBRAC LENGTH FR-TO FR-TO A-B 0 / 50 -112.4 -112.4 0.15 (1) 10.00 Q-C -410 / 0 0.18 (1) B-C -2195 / 0 -112.4 -112.4 0.36 (1) 4.27 C-P -169 / 0 0.15 (1) C-D -2131 / 0 -112.4 -112.4 0.35 (1) 4.33 P-D 0 / 244 0.05 (1) D-E -2110 / 0 -112.4 -112.4 0.94 (1) 3.05 D-N 0 / 777 0.12 (1) E-F -2110 / 0 -112.4 -112.4 0.94 (1) 3.05 N-E -1038 / 0 0.56 (1) F-G -2131 / 0 -112.4 -112.4 0.35 (1) 4.33 N-F 0 / 777 0.12 (1) G-H -2194 / 0 -112.4 -112.4 0.36 (1) 4.28 L-F 0 / 244 0.05 (1) H-I 0 / 50 -112.4 -112.4 0.15 (1) 10.00 L-G -169 / 0 0.15 (1) R-B -2272 / 0 0.0 0.0 0.16 (1) 6.80 K-G -411 / 0 0.18 (1) J-H -2272 / 0 0.0 0.0 0.16 (1) 6.80 B-Q 0 / 1783 0.40 (1) K-H 0 / 1782 0.40 (1) R-Q 0 / 0 -18.5 -18.5 0.08 (4) 10.00 Q-P 0 / 1715 -18.5 -18.5 0.36 (1) 10.00 P-O 0 / 1606 -18.5 -18.5 0.38 (1) 10.00 O-N 0 / 1606 -18.5 -18.5 0.38 (1) 10.00 N-M 0 / 1606 -18.5 -18.5 0.38 (1) 10.00 M-L 0 / 1606 -18.5 -18.5 0.38 (1) 10.00 L-K 0 / 1714 -18.5 -18.5 0.36 (1) 10.00 K-J 0 / 0 -18.5 -18.5 0.08 (4) 10.00			



Structural component only
DWG# T-2215359

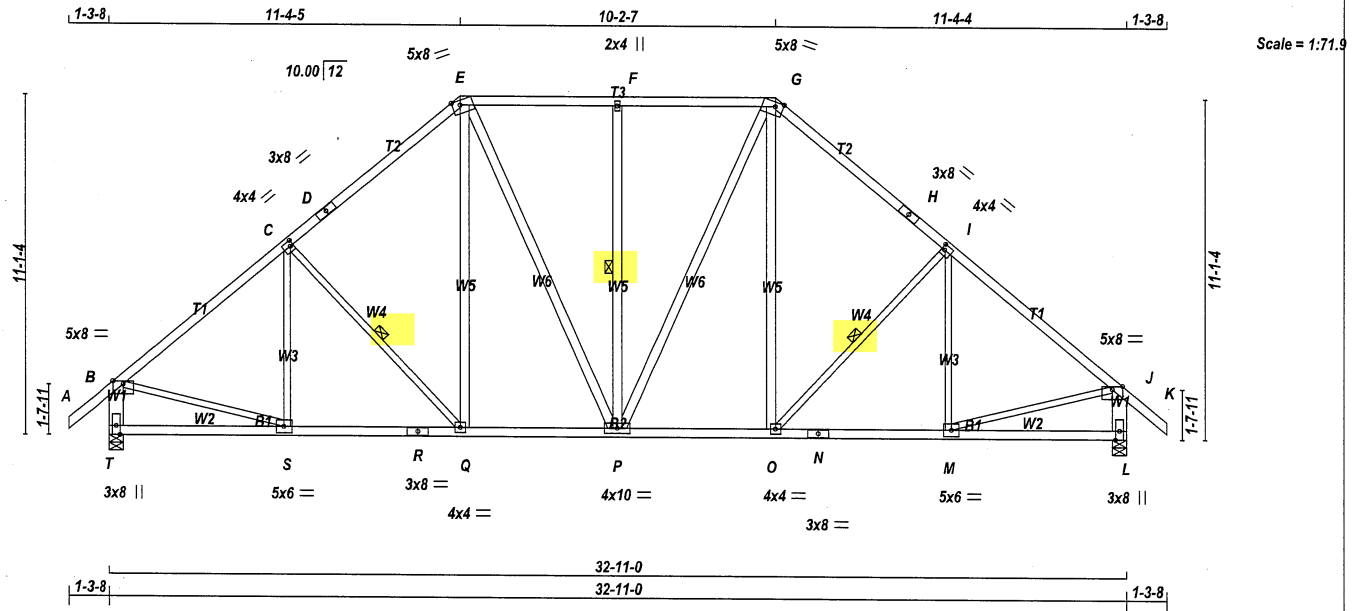
REVIEWED

REVIEWED

JOB NAME 423626	TRUSS NAME T8	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
T - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
T - R	2x4	DRY	No.2	SPF	
R - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
Q - E	2x4	DRY	No.2	SPF	
E - P	2x4	DRY	No.2	SPF	
P - F	2x4	DRY	No.2	SPF	
P - G	2x4	DRY	No.2	SPF	
O - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	8.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TS-t	MT20	3.0	8.0		
I	TMVW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	8.0	Edge	
M	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	4.0	10.0		
Q	BMVW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	8.0		
S	BMVW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	8.0	Edge	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
T	2311	0	2311	0	0	5-8	5-8		
L	2311	0	2311	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
L	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	MAX. UNBRACED	MEMB.	FORCE	MAX. UNBRACED	MAX. FACTORED
	(LBS)	(PLF)	CS1 (LC)		(LBS)	CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	S-C	-280 / 22	0.19 (1)
B-C	-2240 / 0	-112.4 -112.4	0.61 (1)	3.94	C-Q	-401 / 0	0.19 (1)
C-D	-1987 / 0	-112.4 -112.4	0.56 (1)	4.19	Q-E	0 / 392	0.06 (1)
D-E	-1987 / 0	-112.4 -112.4	0.56 (1)	4.19	E-P	0 / 434	0.07 (1)
E-F	-1675 / 0	-112.4 -112.4	0.41 (1)	4.68	P-F	-697 / 0	0.44 (1)
F-G	-1675 / 0	-112.4 -112.4	0.41 (1)	4.68	P-G	0 / 434	0.07 (1)
G-H	-1987 / 0	-112.4 -112.4	0.56 (1)	4.19	O-G	0 / 392	0.06 (1)
H-I	-1987 / 0	-112.4 -112.4	0.56 (1)	4.19	O-I	-400 / 0	0.19 (1)
I-J	-2239 / 0	-112.4 -112.4	0.61 (1)	3.94	M-I	-280 / 22	0.19 (1)
J-K	0 / 50	-112.4 -112.4	0.15 (1)	10.00	B-S	0 / 1803	0.41 (1)
T-B	-2266 / 0	0.0 0.0	0.16 (1)	6.81	M-J	0 / 1802	0.41 (1)
L-J	-2266 / 0	0.0 0.0	0.16 (1)	6.81			
T-S	0 / 0	-18.5 -18.5	0.15 (4)	10.00			
S-R	0 / 1759	-18.5 -18.5	0.35 (1)	10.00			
R-Q	0 / 1759	-18.5 -18.5	0.35 (1)	10.00			
Q-P	0 / 1490	-18.5 -18.5	0.30 (1)	10.00			
P-O	0 / 1490	-18.5 -18.5	0.30 (1)	10.00			
O-N	0 / 1758	-18.5 -18.5	0.35 (1)	10.00			
N-M	0 / 1758	-18.5 -18.5	0.35 (1)	10.00			
M-L	0 / 0	-18.5 -18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/999 (0.14")

CSI: TC=0.61/1.00 (B-C:1), BC=0.35/1.00 (Q-S:1), WB=0.44/1.00 (F-P:1), SS=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.53 (R) (INPUT = 1.00)



Structural component only
DWG# T-2215361

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	T8	2	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



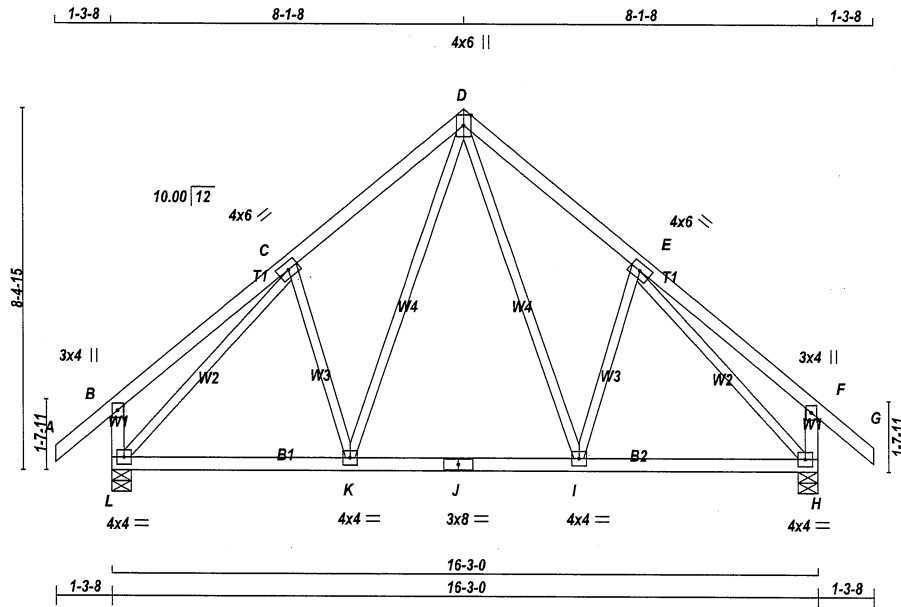
Structural component only
DWG# T-2215361

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423626	T9	7	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 7 X 80 = 561 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESCR.

SPF
SPF
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1220	0	1220	0
H	1220	0	1220	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
L	852
H	852

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
(LBS)	(PLF)	CSI (LC)	(LBS)	CSI (LC)
FR-TO			FR-TO	
A-B	0 / 50	-112.4 -112.4	D-I	0 / 384
B-C	0 / 35	-112.4 -112.4	I-E	-270 / 0
C-D	-883 / 0	-112.4 -112.4	K-D	0 / 384
D-E	-883 / 0	-112.4 -112.4	C-K	-270 / 0
E-F	0 / 35	-112.4 -112.4	L-C	-1123 / 0
F-G	0 / 50	-112.4 -112.4	E-H	-1123 / 0
L-B	-327 / 0	0.0 0.0		
H-F	-327 / 0	0.0 0.0		
L-K	0 / 734	-18.5 -18.5		
K-J	0 / 533	-18.5 -18.5		
J-I	0 / 533	-18.5 -18.5		
I-H	0 / 734	-18.5 -18.5		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 45.9 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL)= L/999 (0.05")

CSI: TC=0.30/1.00 (E-F:1), BC=0.18/1.00 (H-I:1), WB=0.73/1.00 (E-H:1), SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2215362

REVIEWED

 $\overline{M[F]}$

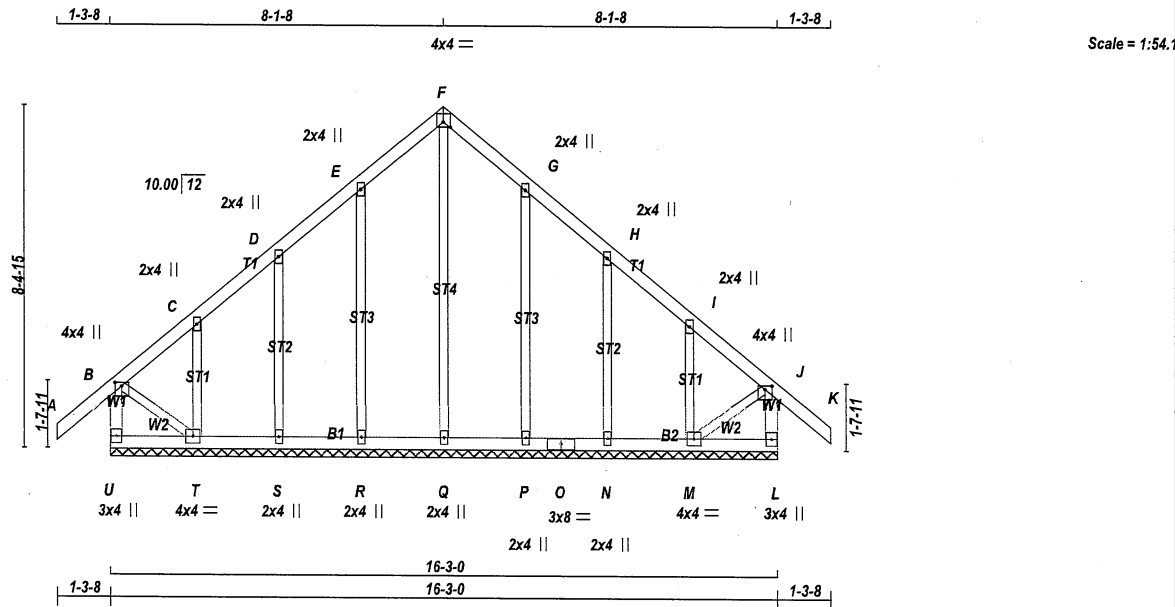
REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423626	G9	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 82 lb			
N. L. G. A. RULES				BEARINGS				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.				SPECIFIED LOADS:			
U - B	2x4	DRY	No.2	THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.				TOP CH. LL = 32.5 PSF			
A - F	2x4	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)				DL = 6.0 PSF			
F - K	2x4	DRY	No.2	BRACING				BOT CH. LL = 0.0 PSF			
L - J	2x4	DRY	No.2	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				DL = 7.4 PSF			
U - O	2x4	DRY	No.2	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				TOTAL LOAD = 45.9 PSF			
O - L	2x4	DRY	No.2	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				SPACING = 24.0 IN. C/C			
ALL WEBS	2x3	DRY	No.2	LOADING				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
ALL GABLE WEBS	2x3	DRY	No.2	TOTAL LOAD CASES: (4)				THIS DESIGN COMPLIES WITH:			
DRY: SEASONED LUMBER.				CHORDS				- PART 9 OF BCBC 2018, ABC 2019			
GABLE STUDS SPACED AT 2-0-0 OC.				MEMB. FORCE (LBS)				- PART 9 OF OBC 2012 (2019 AMENDMENT)			
				MAX. FACTORED				- CSA 086-14			
				VERT. LOAD LC1 MAX				- TPIC 2014			
				FROM TO				(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD			
				CSI (LC)				CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (M-N:4), WB=0.21/1.00 (F-Q:1), SSI=0.09/1.00 (A-B:1)			
				UNBRAC				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10			
				LENGTH FR-TO				COMPANION LIVE LOAD FACTOR = 1.00			
				MEMB. FORCE (LBS)				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
				MAX. FACTORED				NAIL VALUES			
				CSI (LC)				PLATE GRIP(DRY) SHEAR SECTION			
				FR-TO				(PSI) (PLI) (PLI)			
				U-B				MAX MIN MAX MIN MAX MIN			
				A-B				MT20 650 371 1747 788 1987 1873			
				B-C				PLATE PLACEMENT TOL. = 0.250 inches			
				C-D				PLATE ROTATION TOL. = 5.0 Deg.			
				D-E				JSI GRIP= 0.90 (F) (INPUT = 0.90)			
				E-F				JSI METAL= 0.14 (E) (INPUT = 1.00)			
				F-G							
				G-H							
				H-I							
				I-J							
				J-K							
				L-J							
				U-T							
				T-S							
				S-R							
				R-Q							
				Q-P							
				P-O							
				O-N							
				N-M							
				M-L							

06-25-22

H. J. G. ALVES

100009024

LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

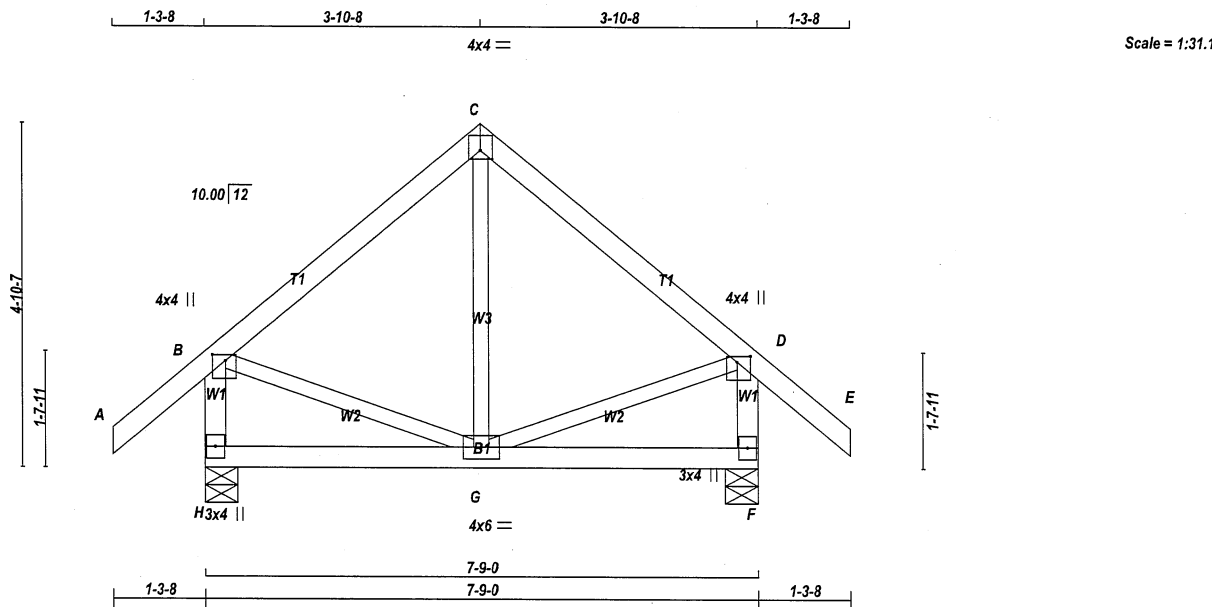
Structural component only

DWG# T-2215342

REVIEWED

JOB NAME 423626	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 37 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.25
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	1189	0	1189	0	5-8	5-8
F	1189	0	1189	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN SNOW		LIVE		PERM.LIVE		WIND		DEAD		SOIL	
	H	F	H	F	H	F	H	F	H	F	H	F	H	F
H	883	342 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	541 / 0	0 / 0	0 / 0	0 / 0	0 / 0
F	883	342 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	541 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)		MAX. CSI (LC)
	FR-TO	FROM TO	FR-TO	FROM TO				FR-TO	FROM TO	
A-B	0 / 94	-212.4 -212.4	0.36 (1)	10.00	G-C	-176 / 0	0.06 (1)			
B-C	-505 / 0	-212.4 -212.4	0.68 (1)	6.25	B-G	0 / 410	0.11 (1)			
C-D	-505 / 0	-212.4 -212.4	0.68 (1)	6.25	G-D	0 / 410	0.11 (1)			
D-E	0 / 94	-212.4 -212.4	0.36 (1)	10.00						
H-B	-1160 / 0	0.0	0.0	15 (1)	7.32					
F-D	-1160 / 0	0.0	0.0	15 (1)	7.32					
H-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00					
G-F	0 / 0	-18.5	-18.5	0.08 (4)	10.00					

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.68/1.00 (B-C:1), BC=0.08/1.00 (G-H:4), WB=0.11/1.00 (D-G:1), SSI=0.30/1.00 (B-C:1)

DOL LUMBER=0.90 NAIL=0.90 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)
JSI METAL= 0.25 (D) (INPUT = 1.00)

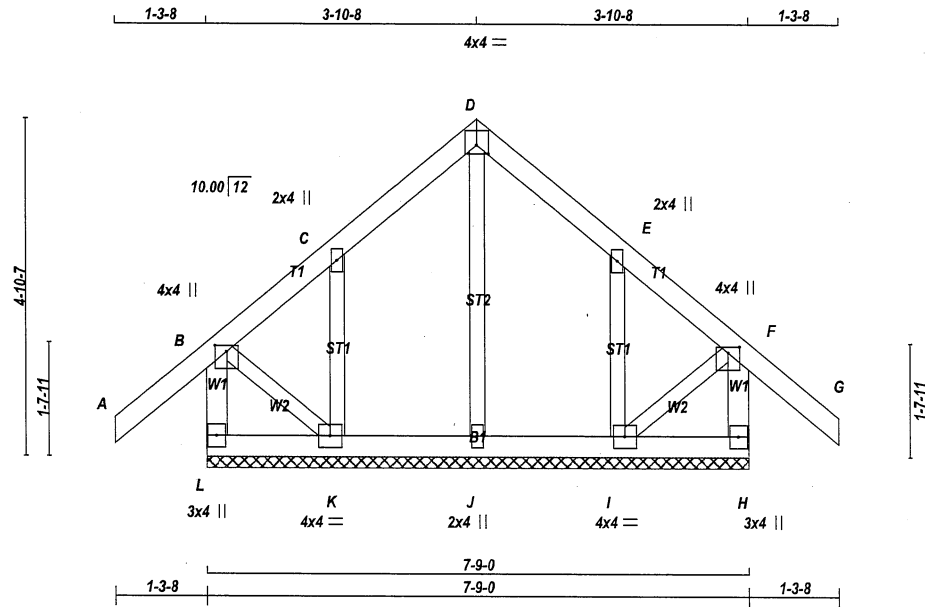
REVIEWED



Structural component only
DWG# T-2215363

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	G10	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale: 3/8"=1'

TOTAL WEIGHT = 38 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF		
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
L - H	2x4	DRY	No.2	SPF		

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMW1+t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMW1+t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED LENGTH (LC)	UNBRACED
FR-TO		FROM	TO	FR-TO			
L-B	-327 / 0	0.0	0.0	J-D	-188 / 0	0.06 (1)	7.81
A-B	0 / 50	-112.4	-112.4	K-C	-183 / 0	0.03 (1)	10.00
B-C	-51 / 0	-112.4	-112.4	I-E	-183 / 0	0.03 (1)	6.25
C-D	-24 / 0	-112.4	-112.4	B-K	0 / 21	0.00 (1)	6.25
D-E	-24 / 0	-112.4	-112.4	I-F	0 / 21	0.00 (1)	6.25
E-F	-51 / 0	-112.4	-112.4				6.25
F-G	0 / 50	-112.4	-112.4				10.00
H-F	-327 / 0	0.0	0.0				7.81
L-K	0 / 0	-18.5	-18.5				10.00
K-J	0 / 11	-18.5	-18.5				10.00
J-I	0 / 11	-18.5	-18.5				10.00
I-H	0 / 0	-18.5	-18.5				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (J-K:4) , WB=0.06/1.00 (D-J:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (D) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

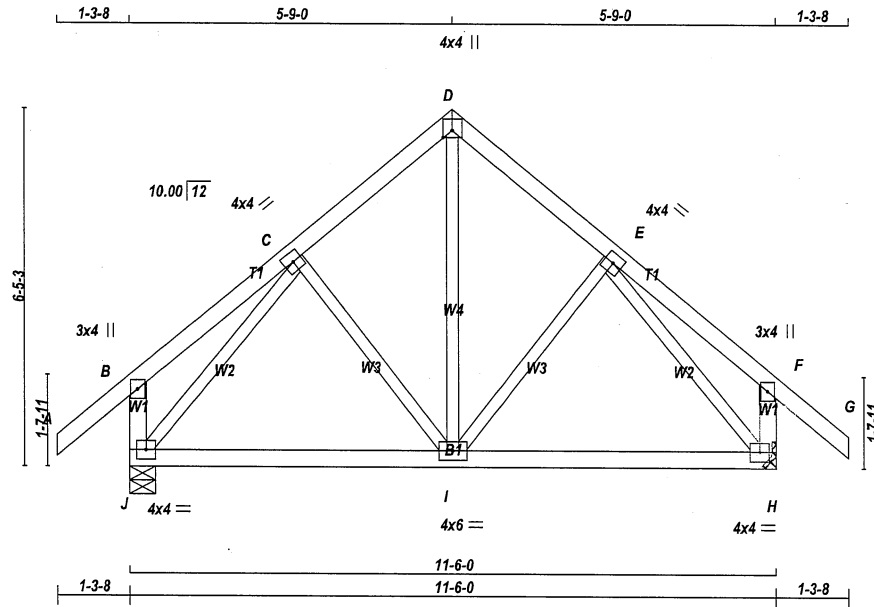


Structural component only
DWG# T-2215343

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	T11	3	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Fri Jun 24 15:13:18 2022 Page 1
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TOTAL WEIGHT = 3 X 56 = 168 lb
[M][F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2		
D - G	2x4	DRY	No.2		
J - B	2x4	DRY	No.2		
H - F	2x4	DRY	No.2		
J - H	2x4	DRY	No.2		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	4.0	1.50	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
J	909	0	909	0
H	909	0	909	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE
J	634	463 / 0	0 / 0	0 / 0
H	634	463 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX	MAX.	MEMB.	FORCE
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)
FR-TO		FROM TO		LENGTH	FR-TO	
A-B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	C-I	-143 / 0
B-C	0 / 23	-112.4 -112.4	0.14 (1)	10.00	I-D	0 / 355
C-D	-500 / 0	-112.4 -112.4	0.11 (1)	6.25	I-E	-143 / 0
D-E	-499 / 0	-112.4 -112.4	0.11 (1)	6.25	J-C	-743 / 0
E-F	0 / 23	-112.4 -112.4	0.14 (1)	10.00	E-H	-743 / 0
F-G	0 / 50	-112.4 -112.4	0.15 (1)	10.00		
J-B	-279 / 0	0.0	0.03 (1)	7.81		
H-F	-279 / 0	0.0	0.03 (1)	7.81		
J-I	0 / 455	-18.5 -18.5	0.20 (4)	10.00		
I-H	0 / 455	-18.5 -18.5	0.20 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.15/1.00 (A-B:1), BC=0.20/1.00 (I-J:4), WB=0.26/1.00 (C-J:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

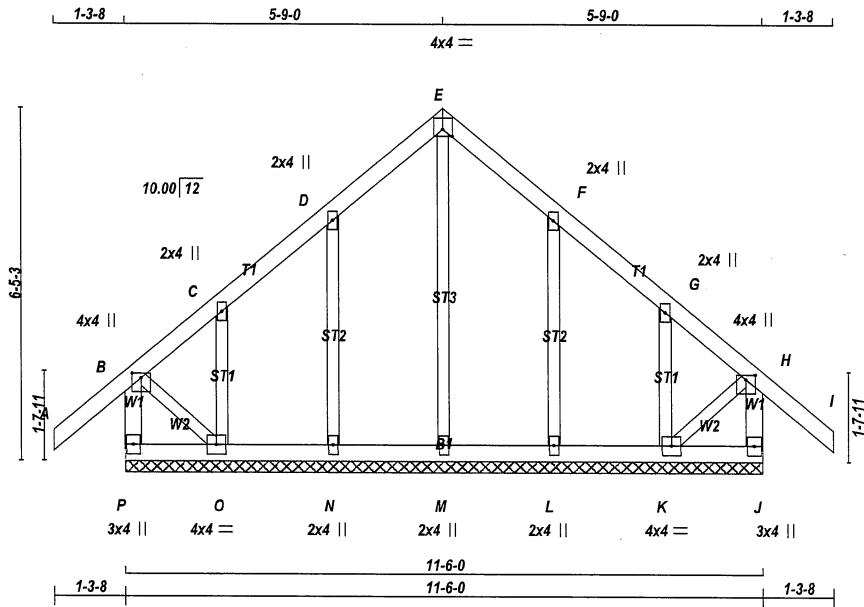


Structural component only
DWG# T-2215364

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	G11	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 56 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		
L, M, N						
L	BMV1+w	MT20	2.0	4.0		
O	BMVW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED LENGTH (LC)	UNBRACED
FR-TO		FROM	TO	FR-TO			
P-B	-340 / 0	0.0	0.0	0.04 (1)	7.81	M-E	-166 / 0
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	N-D	-271 / 0
B-C	-65 / 0	-112.4	-112.4	0.15 (1)	6.25	O-C	-124 / 0
C-D	-9 / 0	-112.4	-112.4	0.07 (1)	10.00	L-F	-271 / 0
D-E	-31 / 0	-112.4	-112.4	0.07 (1)	6.25	K-G	-123 / 0
E-F	-31 / 0	-112.4	-112.4	0.07 (1)	6.25	B-O	0 / 27
F-G	-9 / 0	-112.4	-112.4	0.07 (1)	10.00	K-H	0 / 27
G-H	-65 / 0	-112.4	-112.4	0.15 (1)	6.25		
H-I	0 / 50	-112.4	-112.4	0.15 (1)	10.00		
J-H	-340 / 0	0.0	0.0	0.04 (1)	7.81		
P-O	0 / 0	-18.5	-18.5	0.02 (4)	10.00		
O-N	0 / 17	-18.5	-18.5	0.02 (4)	10.00		
N-M	0 / 11	-18.5	-18.5	0.02 (4)	10.00		
M-L	0 / 11	-18.5	-18.5	0.02 (4)	10.00		
L-K	0 / 17	-18.5	-18.5	0.02 (4)	10.00		
K-J	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (N-O:4), WB=0.11/1.00 (E-M:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PLI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)



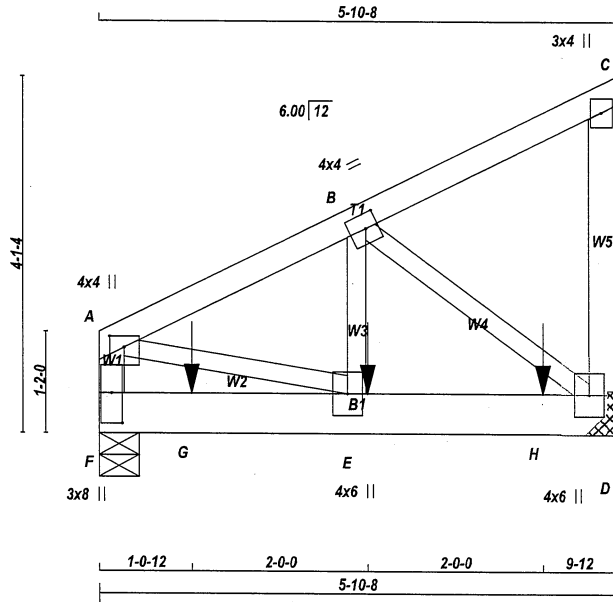
Structural component only
DWG# T-2215344

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423626	T12	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:25.4

TOTAL WEIGHT = 2 X 29 = 58 lb									
[M]									
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. F - A 2x4 DRY No.2 SPF A - C 2x4 DRY No.2 SPF D - C 2x4 DRY No.2 SPF F - D 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.									
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:									
CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)									
TOP CHORDS : (0.122"x3") SPIRAL NAILS									
F-A 1 12 TOP									
A-C 1 12 TOP									
C-D 1 12 TOP									
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS									
F-D 2 12 SIDE(183.1)									
WEBS : (0.122"x3") SPIRAL NAILS									
B-E 1 6 SIDE(49.1)									
2x3 1 6									
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.									
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.									
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.									
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.									
PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Y	X			
A	TMVW+p	MT20	4.0	4.0	1.50	2.00			
B	TMVW-t	MT20	4.0	4.0	2.00	1.75			
C	TMV+p	MT20	3.0	4.0					
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG					
F	VERT 1682 0	DOWN 1682 0	0	5-8	IN-SX	IN-SX			
D	HORZ 1759 0	UP/LIFT 1759 0	0	MECHANICAL					
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.									
UNFACTORED REACTIONS									
JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS							
F	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL							
F	1172	866 / 0 0 / 0 0 / 0 0 / 0 306 / 0 0 / 0							
D	1226	906 / 0 0 / 0 0 / 0 0 / 0 320 / 0 0 / 0							
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM TO	CSI (LC)		FR-TO				
F-A	-1155 / 0	0.0 0.0 0.06 (1)	7.81	A-E	0 / 1382	0.17 (1)			
A-B	-1480 / 0	-112.4 -112.4 0.08 (1)	6.25	E-B	0 / 1297	0.16 (1)			
B-C	-14 / 0	-112.4 -112.4 0.07 (1)	6.25	B-D	-1681 / 0	0.20 (1)			
D-C	-135 / 0	0.0 0.0 0.02 (1)	7.81						
F-G	0 / 0	-18.5 -18.5 0.20 (1)	10.00						
G-E	0 / 0	-18.5 -18.5 0.20 (1)	10.00						
E-H	0 / 1336	-18.5 -18.5 0.27 (1)	10.00						
H-D	0 / 1336	-18.5 -18.5 0.27 (1)	10.00						
SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	3-0-12	-619	-619		FRONT	VERT	TOTAL		C1
G	1-0-12	-619	-619		FRONT	VERT	TOTAL		C1
H	5-0-12	-621	-621		FRONT	VERT	TOTAL		C1
DESIGN CRITERIA									
SPECIFIED LOADS:									
TOP CH.	LL = 32.5 PSF	DL = 6.0 PSF							
BOT CH.	LL = 0.0 PSF	DL = 7.4 PSF							
TOTAL LOAD	= 45.9 PSF								
SPACING = 24.0 IN. C/C									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF CBC 2018, ABC 2019									
- PART 9 OF OBC 2012 (2019 AMENDMENT)									
- CSA 086-14									
- TPIC 2014									
(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD									
ALLOWABLE DEFL.(LL)= L/360 (0.20")									
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")									
ALLOWABLE DEFL.(TL)= L/360 (0.20")									
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")									
CSI: TC=0.08/1.00 (A-B:1), BC=0.27/1.00 (D-E:1), WB=0.20/1.00 (B-D:1), SSI=0.20/1.00 (D-E:1)									
DOL LUMBER-1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00									
COMPANION LIVE LOAD FACTOR = 1.00									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
NAIL VALUES									
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)						
	MAX MIN	MAX MIN	MAX MIN						
MT20	650 371	1747 788	1987 1873						
PLATE PLACEMENT TOL. = 0.250 inches									
PLATE ROTATION TOL. = 5.0 Deg.									
JSI GRIP= 0.84 (B) (INPUT = 0.90)									
JSI METAL= 0.30 (D) (INPUT = 1.00)									



Structural component only
DWG# T-2215365

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423626	T12	1	2	BAYVIEW WELLINGTON	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	8.0	4.25	1.50

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER

06-25-22

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only
DWG# T-2215365

REVIEWED

REVIEWED

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 15:13:19 2022 Page 1
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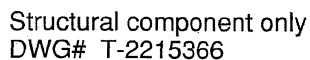


TOTAL WEIGHT = 2 X 29 = 58 lb

1) Lateral braces to be a minimum of 2X4 SPF #2.

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

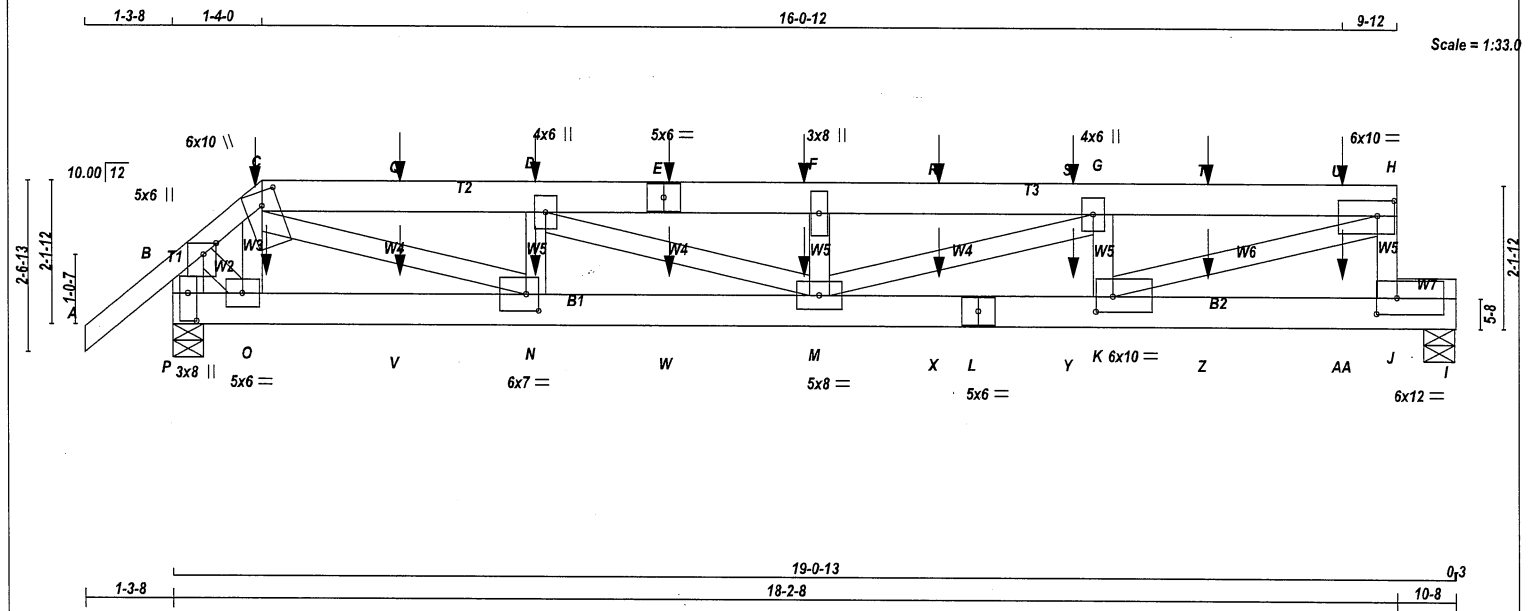
JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.42 (D) (INPUT = 1.00)



REVIEWED

JOB NAME 423626	TRUSS NAME T21	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 97 lb
(M)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x6	DRY	No.2
E - H	2x6	DRY	No.2
J - H	2x4	DRY	No.2
P - B	2x6	DRY	No.2
P - L	2x6	DRY	2100F 1.8E
L - I	2x6	DRY	2100F 1.8E
J - I	2x4	DRY	No.2
ALL WEBS 2x4 DRY No.2 SPF			
EXCEPT K - H 2x4 DRY 1650F 1.5E SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	10.0	2.50	3.00
D	TMVW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMVW+w	MT20	3.0	8.0		
G	TMVW+t	MT20	4.0	6.0		
H	TMVW-t	MT20	6.0	10.0	2.75	3.00
J	BMVW-t	MT20	6.0	12.0	3.00	3.75
K	BMVW-t	MT20	6.0	10.0	2.75	3.00
L	BS-t	MT20	5.0	6.0		
M	BMVW-w	MT20	5.0	8.0		
N	BMVW-t	MT20	6.0	7.0	3.00	2.25
O	BMVW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	8.0	5.00	1.50

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
P	2567 0	2567 0	5-8	5-8	
I	2172 0	2172 0	5-8	5-8	

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS			
P	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1795	1293 / 0	0 / 0	0 / 0	0 / 0	502 / 0	0 / 0
I	1524	1071 / 0	0 / 0	0 / 0	0 / 0	453 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 50	-112.4 -112.4	0.17 (1)	10.00	O-C	-538 / 0	0.06 (1)
B-C	-2373 / 0	-112.4 -112.4	0.23 (1)	4.19	B-O	0 / 2025	0.36 (1)
C-Q	-5356 / 0	-112.4 -112.4	0.42 (1)	3.40	K-H	0 / 5896	0.75 (1)
Q-D	-5356 / 0	-112.4 -112.4	0.42 (1)	3.40	K-G	-1254 / 0	0.14 (1)
D-E	-6454 / 0	-112.4 -112.4	0.50 (1)	3.03	M-G	0 / 1027	0.18 (1)
E-F	-6454 / 0	-112.4 -112.4	0.50 (1)	3.03	C-N	0 / 3811	0.67 (1)
F-R	-6454 / 0	-112.4 -112.4	0.52 (1)	3.01	M-F	-759 / 0	0.09 (1)
R-S	-6454 / 0	-112.4 -112.4	0.52 (1)	3.01	N-D	-1296 / 0	0.15 (1)
S-G	-6454 / 0	-112.4 -112.4	0.52 (1)	3.01	D-M	0 / 1183	0.21 (1)
G-T	-5500 / 0	-112.4 -112.4	0.48 (1)	3.29			
T-U	-5500 / 0	-112.4 -112.4	0.48 (1)	3.29			
U-H	-5500 / 0	-112.4 -112.4	0.48 (1)	3.29			
J-H	-2524 / 0	0.0	0.0	0.29 (1)	5.34		
P-B	-2716 / 0	0.0	0.0	0.19 (1)	6.29		
P-O	0 / 0	-18.5	-18.5	0.08 (1)	10.00		
O-V	0 / 1842	-18.5	-18.5	0.17 (1)	10.00		
V-N	0 / 1842	-18.5	-18.5	0.17 (1)	10.00		
N-W	0 / 5356	-18.5	-18.5	0.41 (1)	10.00		
W-M	0 / 5356	-18.5	-18.5	0.41 (1)	10.00		
M-X	0 / 5500	-18.5	-18.5	0.41 (1)	10.00		
X-L	0 / 5500	-18.5	-18.5	0.41 (1)	10.00		
L-Y	0 / 5500	-18.5	-18.5	0.41 (1)	10.00		
Y-K	0 / 5500	-18.5	-18.5	0.41 (1)	10.00		
K-Z	0 / 0	-18.5	-18.5	0.74 (1)	10.00		
Z-AA	0 / 0	-18.5	-18.5	0.74 (1)	10.00		
AA-J	0 / 0	-18.5	-18.5	0.74 (1)	10.00		
J-I	0 / 0	-18.5	-18.5	0.70 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	CONN.
C	1-4-0	-14	-14	---	FRONT	VERT	DEAD
C	1-4-0	-111	-111	---	BACK	VERT	TOTAL
C	1-4-0	-74	-74	---	FRONT	VERT	SNOW
D	5-4-12	-106	-106	---	BACK	VERT	TOTAL
E	7-4-12	-106	-106	---	BACK	VERT	TOTAL
F	9-4-12	-106	-106	---	BACK	VERT	TOTAL
N	9-4-12	-53	-53	---	BACK	VERT	TOTAL
N	5-4-12	-53	-53	---	BACK	VERT	TOTAL
O	1-4-12	-53	-53	---	BACK	VERT	TOTAL
Q	3-4-12	-106	-106	---	BACK	VERT	TOTAL
R	11-4-12	-106	-106	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/922 (0.25")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/527 (0.43")

CSI: TC=0.52/1.00 (F-G:1), BC=0.74/1.00 (J-K:1), WB=0.75/1.00 (H-K:1), SSI=0.88/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.



Structural component only
DWG# T-2215367

REVIEWED
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423626	T21	1	1	TRUSS DESC.		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	13-4-12	-106	-106	---	BACK	VERT	TOTAL	---	C1
T	15-4-12	-106	-106	---	BACK	VERT	TOTAL	---	C1
U	17-4-12	-113	-113	---	BACK	VERT	TOTAL	---	C1
V	3-4-12	-53	-53	---	BACK	VERT	TOTAL	---	C1
W	7-4-12	-53	-53	---	BACK	VERT	TOTAL	---	C1
X	11-4-12	-53	-53	---	BACK	VERT	TOTAL	---	C1
Y	13-4-12	-53	-53	---	BACK	VERT	TOTAL	---	C1
Z	15-4-12	-53	-53	---	BACK	VERT	TOTAL	---	C1
AA	17-4-12	-53	-53	---	BACK	VERT	TOTAL	---	C1

JSI GRIP= 0.88 (L) (INPUT = 0.90)

JSI METAL= 0.99 (L) (INPUT = 1.00)

- CONNECTION REQUIREMENTS
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only

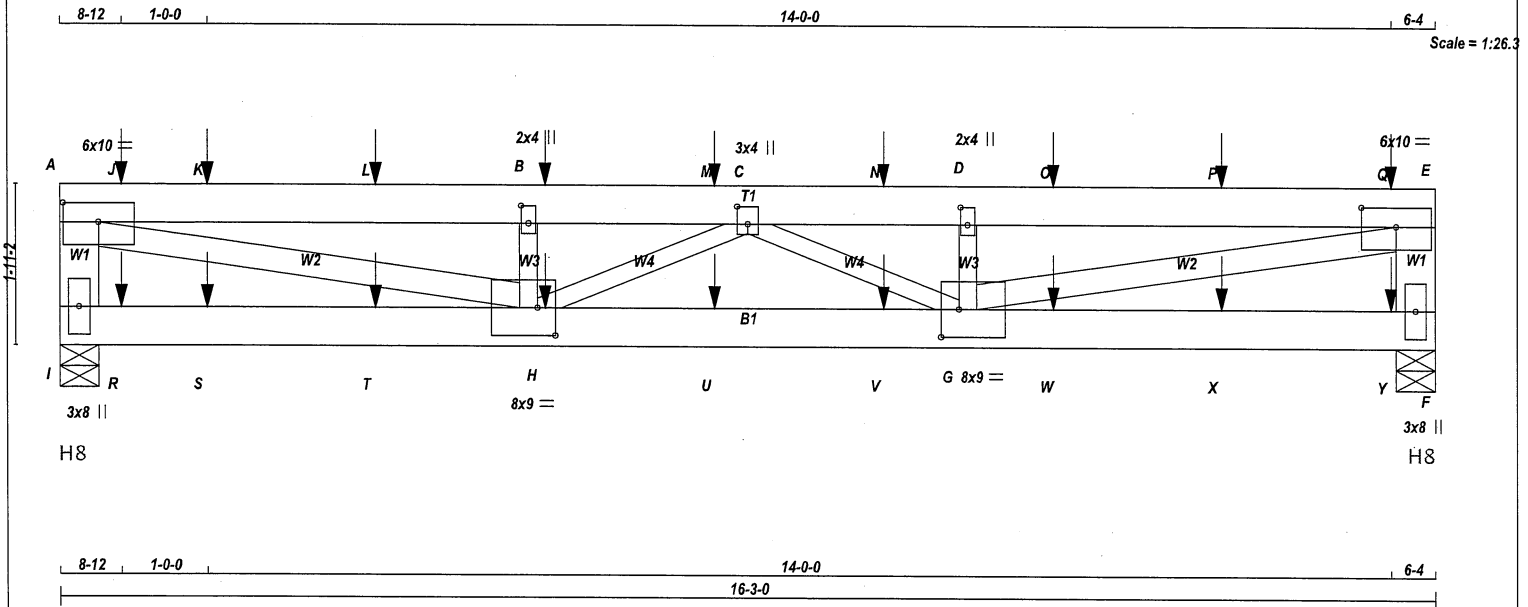
DWG# T-2215367

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423626	T22W	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 15:13:22 2022 Page 1
ID:c3jy23uDijq_8pvRKbkZpy75XW-IMm10rq?9SM8SAkV3GSxFINALP?FVT_DHDGW8dz32Oh



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. I - A 2x6 DRY No.2 SPF A - E 2x6 DRY No.2 SPF F - E 2x6 DRY No.2 SPF I - F 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT G - E 2x4 DRY No.2 SPF A - H 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TMVW-t MT20 6.0 10.0 2.75 5.00 B TMW+w MT20 2.0 4.0 2.50 1.00 C TMW+w MT20 3.0 4.0 2.50 1.50 D TMW+w MT20 2.0 4.0 2.50 1.00 E TMVW-t MT20 6.0 10.0 2.75 5.00 F BMV1+p MT20 3.0 8.0 G BMWVW-t MT20 8.0 9.0 4.00 2.50 H BMWVW-t MT20 8.0 9.0 4.00 2.50 I BMV1+p MT20 3.0 8.0				NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.54") CALCULATED VERT. DEFL.(LL) = L/897 (0.22") ALLOWABLE DEFL.(TL) = L/360 (0.54") CALCULATED VERT. DEFL.(TL) = L/517 (0.38") CSI: TC=0.64/1.00 (A-B:1), BC=0.82/1.00 (G-H:1), WB=0.86/1.00 (A-H:1), SSI=0.35/1.00 (A-B:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 798 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.86 (H) (INPUT = 0.90) JSI METAL= 0.73 (G) (INPUT = 1.00)				CHORDS MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX (PLF) CSI (LC) UNBRAC MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) CSI (LC) FR-TO FROM TO LENGTH FR-TO I-A -1739 / 840 0.0 0.0 0.13 (1) 7.49 G-E -2453 / 4801 0.85 (1) A-J -4700 / 2403 -112.4 -112.4 0.64 (1) 3.30 A-H -2482 / 4854 0.86 (1) J-K -4700 / 2403 -112.4 -112.4 0.64 (1) 3.30 G-D -832 / 313 0.13 (1) K-L -4700 / 2403 -112.4 -112.4 0.64 (1) 3.30 H-B -861 / 328 0.13 (1) L-B -4700 / 2403 -112.4 -112.4 0.64 (1) 3.30 H-C -202 / 103 0.04 (1) B-M -4619 / 2373 -112.4 -112.4 0.32 (7) 3.85 C-G -259 / 134 0.05 (1) M-C -4619 / 2373 -112.4 -112.4 0.32 (7) 3.85 C-N -4570 / 2346 -112.4 -112.4 0.31 (7) 3.87 N-D -4570 / 2346 -112.4 -112.4 0.31 (7) 3.87 D-O -4648 / 2374 -112.4 -112.4 0.63 (1) 3.34 O-P -4648 / 2374 -112.4 -112.4 0.63 (1) 3.34 P-Q -4648 / 2374 -112.4 -112.4 0.63 (1) 3.34 Q-E -4648 / 2374 -112.4 -112.4 0.63 (1) 3.34 F-E -1705 / 822 0.0 0.0 0.12 (1) 7.55 I-R 0/0 -18.5 -18.5 0.21 (1) 10.00 R-S 0/0 -18.5 -18.5 0.21 (1) 10.00 S-T 0/0 -18.5 -18.5 0.21 (1) 10.00 T-H 0/0 -18.5 -18.5 0.21 (1) 10.00 H-U -2461 / 4792 -18.5 -18.5 0.82 (1) 5.18 U-V -2461 / 4792 -18.5 -18.5 0.82 (1) 5.18 V-G -2461 / 4792 -18.5 -18.5 0.82 (1) 5.18 G-W 0/0 -18.5 -18.5 0.19 (1) 10.00 W-X 0/0 -18.5 -18.5 0.19 (1) 10.00 X-Y 0/0 -18.5 -18.5 0.19 (1) 10.00 Y-F 0/0 -18.5 -18.5 0.19 (1) 10.00 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. B 5-8-12 -83 -83 58 BACK VERT TOTAL --- C1 H 5-8-12 -45 -45 37 BACK VERT TOTAL --- C1 J 8-12 -93 -93 58 BACK VERT TOTAL --- C1 K 1-8-12 -83 -83 58 BACK VERT TOTAL --- C1 L 3-8-12 -83 -83 58 BACK VERT TOTAL --- C1 M 7-8-12 -83 -83 58 BACK VERT TOTAL --- C1 N 9-8-12 -83 -83 58 BACK VERT TOTAL --- C1 O 11-8-12 -83 -83 58 BACK VERT TOTAL --- C1 P 13-8-12 -83 -83 58 BACK VERT TOTAL --- C1 Q 15-8-12 -101 -101 58 BACK VERT TOTAL --- C1				WEBS MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) CSI (LC) FR-TO FROM TO LENGTH FR-TO G-E -2453 / 4801 0.85 (1) A-H -2482 / 4854 0.86 (1) G-D -832 / 313 0.13 (1) H-B -861 / 328 0.13 (1) H-C -202 / 103 0.04 (1) C-G -259 / 134 0.05 (1)				STRUCTURAL COMPONENT ONLY DWG# T-2215368			
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REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423626	T22W	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:c3jvj23uDiiq 8pvRKbkZpy75XW-IMm10rq?9SM8SAkV3GSxFIMALP?FVT DHdGW8dz32Oh

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	8-12	-47	-47	37	BACK	VERT	TOTAL	---	C1
S	1-8-12	-45	-45	37	BACK	VERT	TOTAL	---	C1
T	3-8-12	-45	-45	37	BACK	VERT	TOTAL	---	C1
U	7-8-12	-45	-45	37	BACK	VERT	TOTAL	---	C1
V	9-8-12	-45	-45	37	BACK	VERT	TOTAL	---	C1
W	11-8-12	-45	-45	37	BACK	VERT	TOTAL	---	C1
X	13-8-12	-45	-45	37	BACK	VERT	TOTAL	---	C1
Y	15-8-12	-49	-49	37	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

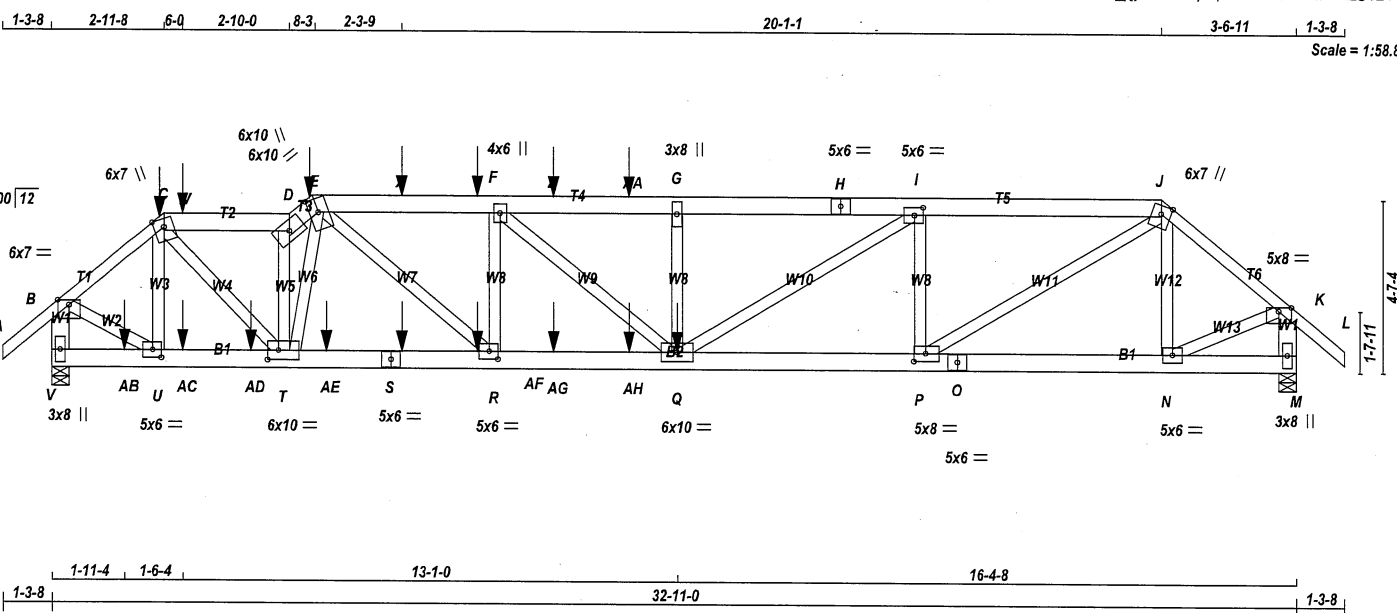
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (15-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.



Structural component only

DWG# T-2215368

REVIEWED



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - D 2x6 DRY No.2 SPF D - E 2x6 DRY No.2 SPF E - H 2x6 DRY No.2 SPF H - J 2x6 DRY No.2 SPF J - L 2x4 DRY No.2 SPF V - B 2x6 DRY No.2 SPF M - K 2x6 DRY No.2 SPF V - S 2x6 DRY No.2 SPF S - O 2x6 DRY No.2 SPF O - M 2x6 DRY No.2 SPF ALL WEBS 2x4 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 1 12 SIDE(61.0) J-L 1 12 TOP C-D 2 12 SIDE(61.0) D-E 2 12 SIDE(61.0) E-H 2 12 SIDE(122.0) H-J 2 12 TOP V-B 2 12 TOP M-K 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS V-S 2 12 SIDE(0.0) O-S 2 12 SIDE(183.1) O-M 2 12 TOP WEBS : (0.122"x3") SPIRAL NAILS 2x4 1 6 G-Q 1 3 SIDE(570.0) NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX V 5922 0 5922 0 0 5-8 5-8 M 4501 0 4501 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL V 4135 3010 / 0 0 / 0 0 / 0 1125 / 0 0 / 0 M 3145 2281 / 0 0 / 0 0 / 0 863 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED WEBS MEMB. FORCE VERT. LOAD LC1 MAX. MAX. MEMB. MAX. FORCE MAX. (LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC) FR-TO LENGTH FR-TO A-B 0 / 50 -112.4 -112.4 0.09 (1) 10.00 U-C -1627 / 0 0.14 (1) B-C -5761 / 0 -112.4 -112.4 0.22 (1) 3.90 C-T 0 / 6185 0.55 (1) C-W -8564 / 0 -112.4 -112.4 0.17 (1) 4.04 T-D -7330 / 0 0.63 (1) W-D -8564 / 0 -112.4 -112.4 0.17 (1) 4.04 N-J -1116 / 0 0.12 (1) D-E -11250 / 0 -112.4 -112.4 0.16 (1) 3.58 B-U 0 / 4840 0.43 (1) E-X -11516 / 0 -112.4 -112.4 0.33 (1) 3.42 N-K 0 / 3708 0.33 (1) X-Y -11516 / 0 -112.4 -112.4 0.33 (1) 3.42 F-Q 0 / 1850 0.16 (1) Y-F -11516 / 0 -112.4 -112.4 0.33 (1) 3.42 R-F -1864 / 0 0.19 (1) F-Z -12917 / 0 -112.4 -112.4 0.38 (1) 3.20 Q-G -595 / 0 0.06 (1) Z-AA -12917 / 0 -112.4 -112.4 0.38 (1) 3.20 T-E 0 / 3367 0.30 (1) AA-G -12917 / 0 -112.4 -112.4 0.38 (1) 3.20 E-R 0 / 4401 0.39 (1) G-H -12917 / 0 -112.4 -112.4 0.48 (1) 3.14 P-J 0 / 6483 0.57 (1) H-I -12917 / 0 -112.4 -112.4 0.48 (1) 3.14 Q-I 0 / 4707 0.42 (1) I-J -8963 / 0 -112.4 -112.4 0.31 (1) 3.86 P-I -3419 / 0 0.35 (1) J-K -4531 / 0 -112.4 -112.4 0.23 (1) 4.32 K-L 0 / 50 -112.4 -112.4 0.09 (1) 10.00 V-B -5919 / 0 0.0 0.0 0.22 (1) 6.07 M-K -4496 / 0 0.0 0.0 0.17 (1) 6.79 V-AB 0 / 0 -18.5 -18.5 0.03 (1) 10.00 AB-U 0 / 0 -18.5 -18.5 0.03 (1) 10.00 U-AC 0 / 4505 -18.5 -18.5 0.35 (1) 10.00 AC-AD 0 / 4505 -18.5 -18.5 0.35 (1) 10.00 AD-T 0 / 4505 -18.5 -18.5 0.35 (1) 10.00 T-AE 0 / 8199 -18.5 -18.5 0.70 (1) 10.00 AE-S 0 / 8199 -18.5 -18.5 0.70 (1) 10.00 S-AF 0 / 8199 -18.5 -18.5 0.70 (1) 10.00 AF-R 0 / 8199 -18.5 -18.5 0.70 (1) 10.00 R-AG 0 / 11516 -18.5 -18.5 0.91 (1) 10.00 AG-AH 0 / 11516 -18.5 -18.5 0.91 (1) 10.00 AH-Q 0 / 11516 -18.5 -18.5 0.91 (1) 10.00 Q-P 0 / 8963 -18.5 -18.5 0.65 (1) 10.00 P-O 0 / 3530 -18.5 -18.5 0.26 (1) 10.00 O-N 0 / 3530 -18.5 -18.5 0.26 (1) 10.00 N-M 0 / 0 -18.5 -18.5 0.04 (4) 10.00 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. C 2-11-8 -30 -30 --- FRONT VERT DEAD --- C1 C 2-11-8 -159 -159 --- FRONT VERT SNOW --- C1 E 6-11-11 -44 -44 --- FRONT VERT DEAD --- C1 E 6-11-11 -26 -26 --- BACK VERT TOTAL --- C1				DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.10") CALCULATED VERT. DEFL.(LL)= L/999 (0.27") ALLOWABLE DEFL.(TL)= L/360 (1.10") CALCULATED VERT. DEFL.(TL)= L/864 (0.46") CSI: TC=0.48/1.00 (G-I:1), BC=0.91/1.00 (Q-R:1), WB=0.63/1.00 (D-T:1), SSI=0.20/1.00 (R-T:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (T) (INPUT = 0.90) JSI METAL= 0.90 (S) (INPUT = 1.00)			
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Structural component only
DWG# T-2215380

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423631	T25	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 15:42:22 2022 Page 2
ID:c3jvi23uDijg_8pvRKbkZpy75XW-M5vpCotRO4aFCZlw_igzmDRYioqvBUNKOLmaDmz31zV

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TTWW+m	MT20	6.0	7.0	Edge	
D	TTW-h	MT20	6.0	10.0		
E	TTWW+m	MT20	6.0	10.0	4.50	2.75
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	8.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TTWW+m	MT20	6.0	7.0	Edge	
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	8.0	2.50	3.75
Q	BMWWW-t	MT20	6.0	10.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BS-t	MT20	5.0	6.0		
T	BMWWW-t	MT20	6.0	10.0	3.00	3.50
U	BMWW-t	MT20	5.0	6.0	2.50	2.75
V	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	6-11-11	-236	-236	---	FRONT	VERT	SNOW	---	C1
Q	16-6-8	-2012	-2012	---	BACK	VERT	TOTAL	---	C1
S	9-3-4	-204	-204	---	BACK	VERT	TOTAL	---	C1
W	3-5-8	-112	-112	---	BACK	VERT	TOTAL	---	C1
X	9-3-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	11-3-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Z	13-3-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AA	15-3-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AB	1-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	3-5-8	-98	-98	---	BACK	VERT	TOTAL	---	C1
AD	5-3-4	-204	-204	---	BACK	VERT	TOTAL	---	C1
AE	7-3-4	-204	-204	---	BACK	VERT	TOTAL	---	C1
AF	11-3-4	-204	-204	---	BACK	VERT	TOTAL	---	C1
AG	13-3-4	-204	-204	---	BACK	VERT	TOTAL	---	C1
AH	15-3-4	-204	-204	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

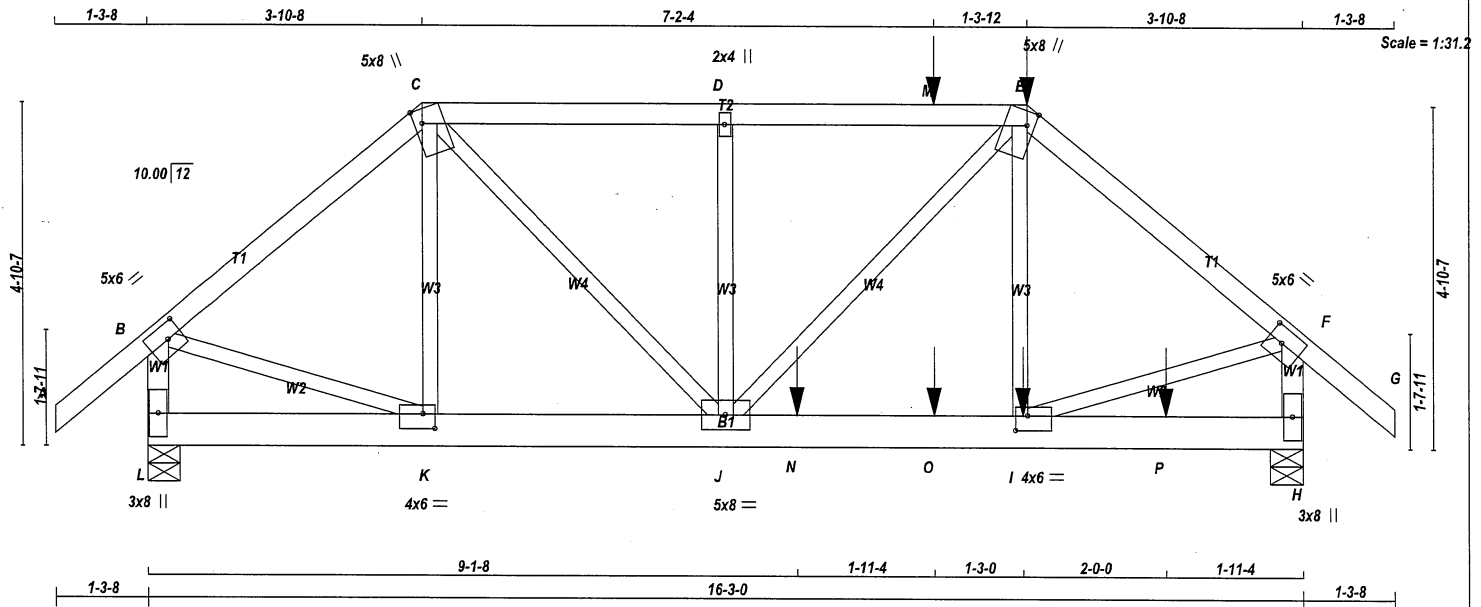


Structural component only
DWG# T-2215380

REVIEWED

JOB NAME 423631	TRUSS NAME T26	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 15:42:23 2022 Page 1
ID:c3jy23uDiq_8pvRKbkZpy75XW-qITCQ8u49OI6pjK6YPBCJRzjVCEjw_KTd?W7IDz31zU



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.		ALL WEBS 2x3 DRY No.2 EXCEPT DRY: SEASONED LUMBER.	
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-t MT20 5.0 6.0 2.50 2.50 C TTWW+m MT20 5.0 8.0 Edge 1.25 D TMW+w MT20 2.0 4.0 E TTWW+m MT20 5.0 8.0 Edge 1.25 F TMVW-t MT20 5.0 6.0 2.50 2.50 H BMV1+p MT20 3.0 8.0 I BMVW-t MT20 4.0 6.0 2.50 2.00 J BMVW-t MT20 5.0 8.0 K BMVW-t MT20 4.0 6.0 2.50 2.00 L BMV1+p MT20 3.0 8.0		Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.	
NOTES (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.54") CALCULATED VERT. DEFL.(LL) = L/999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (0.54") CALCULATED VERT. DEFL.(TL) = L/999 (0.10") CSI: TC=0.47/1.00 (D-E-1), BC=0.62/1.00 (I-J-1), WB=0.39/1.00 (F-I-1), SSI=0.54/1.00 (I-J-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (F) (INPUT = 0.90) JSI METAL= 0.50 (B) (INPUT = 1.00)	
BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX L 1768 0 1768 0 0 5-8 5-8 H 2092 0 2092 0 0 5-8 5-8		UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL L 1234 905 / 0 0 / 0 0 / 0 329 / 0 0 / 0 H 1460 1067 / 0 0 / 0 0 / 0 393 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H	
BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.		LOADING TOTAL LOAD CASES: (4) CHORDS W E B S MEMB. MAX. FACTORED FORCE VERT. LOAD LC1 MAX UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE MAX CSI (LC) FR-TO A-B 0 / 50 -112.4 -112.4 0.17 (1) 10.00 K-C -357 / 0 0.12 (1) B-C -1539 / 0 -112.4 -112.4 0.38 (1) 4.83 C-J 0 / 1295 0.32 (1) C-D -2063 / 0 -112.4 -112.4 0.47 (1) 4.13 J-D -610 / 0 0.21 (1) D-M -2063 / 0 -112.4 -112.4 0.47 (1) 4.13 J-E 0 / 816 0.20 (1) M-E -2063 / 0 -112.4 -112.4 0.47 (1) 4.13 I-E -66 / 79 0.03 (4) E-F -1961 / 0 -112.4 -112.4 0.41 (1) 4.34 B-K 0 / 1234 0.31 (1) F-G 0 / 50 -112.4 -112.4 0.17 (1) 10.00 I-F 0 / 1571 0.39 (1) L-B -1725 / 0 0.0 0.0 0.20 (1) 6.29 H-F -2095 / 0 0.0 0.0 0.24 (1) 5.80 L-K 0 / 0 -18.5 -18.5 0.04 (1) 10.00 K-J -18.5 -18.5 0.32 (1) 10.00 J-N -18.5 -18.5 0.62 (1) 10.00 N-O -18.5 -18.5 0.62 (1) 10.00 O-I -18.5 -18.5 0.62 (1) 10.00 I-P -18.5 -18.5 0.14 (1) 10.00 P-H 0 / 0 -18.5 -18.5 0.14 (1) 10.00	
SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. E 12-4-8 -175 -175 --- FRONT VERT TOTAL --- C1 I 12-3-12 -14 -14 --- FRONT VERT TOTAL --- C1 M 11-0-12 -72 -72 --- FRONT VERT TOTAL --- C1 N 9-1-8 -701 -701 --- FRONT VERT TOTAL --- C1 O 11-0-12 -14 -14 --- FRONT VERT TOTAL --- C1 P 14-3-12 -14 -14 --- FRONT VERT TOTAL --- C1		CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.	



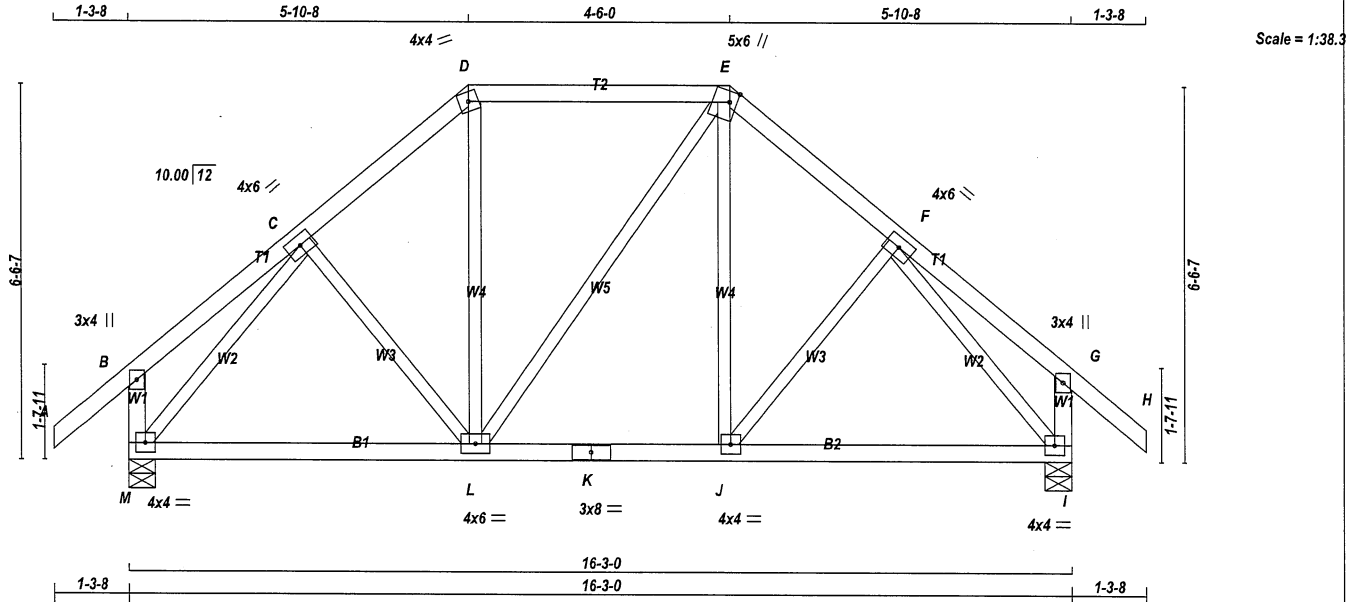
Structural component only
DWG# T-2215381

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423631	T27	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMVWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
M	1220	0	1220	0	0	5-8	5-8	5-8	5-8
I	1220	0	1220	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	852	618 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
I	852	618 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)			(LBS)	CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	C-L	-73 / 17	0.03 (1)
B-C	0 / 23	-112.4	-112.4	0.15 (1)	10.00	L-D	0 / 160	0.04 (1)
C-D	-868 / 0	-112.4	-112.4	0.12 (1)	6.25	L-E	0 / 0	0.00 (1)
D-E	-650 / 0	-112.4	-112.4	0.30 (1)	6.25	J-E	0 / 159	0.04 (1)
E-F	-868 / 0	-112.4	-112.4	0.12 (1)	6.25	J-F	-74 / 16	0.03 (1)
F-G	0 / 23	-112.4	-112.4	0.15 (1)	10.00	M-C	-1128 / 0	0.40 (1)
G-H	0 / 50	-112.4	-112.4	0.15 (1)	10.00	F-I	-1128 / 0	0.40 (1)
M-B	-283 / 0	0.0	0.0	0.03 (1)	7.81			
I-G	-283 / 0	0.0	0.0	0.03 (1)	7.81			
M-L	0 / 694	-18.5	-18.5	0.19 (1)	10.00			
L-K	0 / 650	-18.5	-18.5	0.17 (4)	10.00			
K-J	0 / 650	-18.5	-18.5	0.17 (4)	10.00			
J-I	0 / 694	-18.5	-18.5	0.19 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.30/1.00 (D-E:1), BC=0.19/1.00 (I-J:1), WB=0.40/1.00 (C-M:1), SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

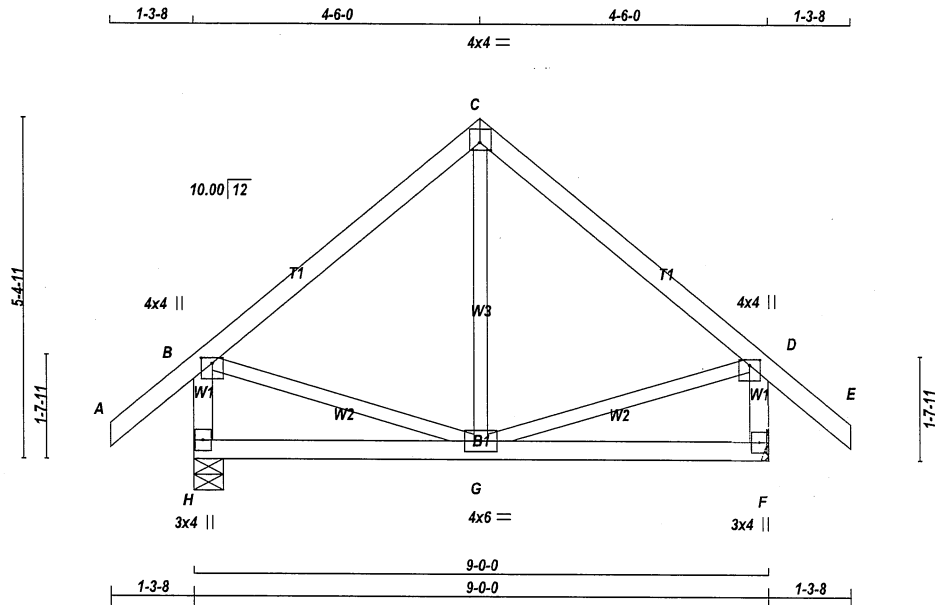


Structural component only
DWG# T-2215382

REVIEWED

JOB NAME 423631	TRUSS NAME T28	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:34.8

TOTAL WEIGHT = 2 X 42 = 83 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	745	0	745	0
F	745	0	745	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	
H	519	382 / 0	0 / 0	0 / 0	137 / 0
F	519	382 / 0	0 / 0	0 / 0	137 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	G-C	-55 / 66	0.02 (1)
B-C	-352 / 0	-112.4 -112.4	0.29 (1)	6.25	B-G	0 / 282	0.06 (1)
C-D	-352 / 0	-112.4 -112.4	0.29 (1)	6.25	G-D	0 / 282	0.06 (1)
D-E	0 / 50	-112.4 -112.4	0.15 (1)	10.00			
H-B	-713 / 0	0.0	0.0	0.08 (1)	7.81		
F-D	-713 / 0	0.0	0.0	0.08 (1)	7.81		
H-G	0 / 0	-18.5 -18.5	0.11 (4)	10.00			
G-F	0 / 0	-18.5 -18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.29/1.00 (C-D:1), BC=0.11/1.00 (G-H:4), WB=0.06/1.00 (B-G:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



Structural component only
DWG# T-2215383

REVIEWED

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TOTAL WEIGHT = 44 lb

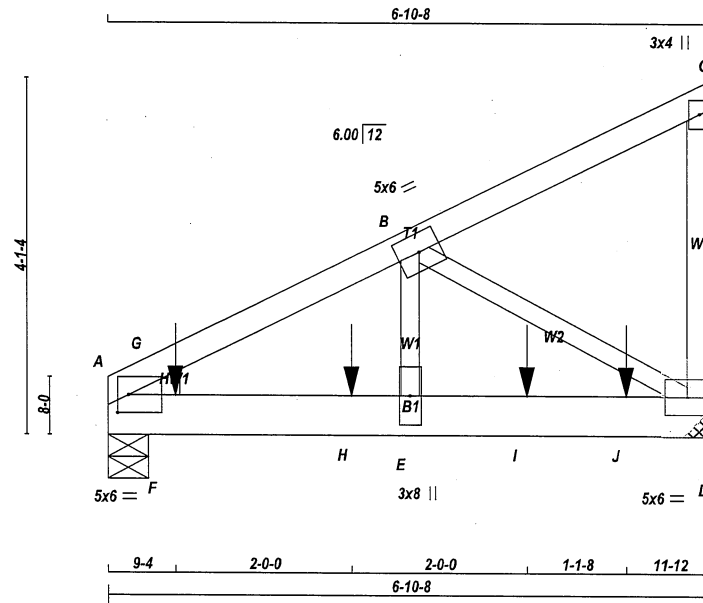
Structural component only
DWG# T-2215376

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423631	T29	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale = 1:25.3

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
A - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
A - D	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	5.0	6.0	2.50	1.50
B	TMWV-I	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMVW-w	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	HEEL
	VERT	HORZ	DOWN	HORZ	IN-SX
D	2905	0	2905	0	MECHANICAL
A	2801	0	2801	0	5-8
					5-8
					2x4 L

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0".

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2027	1486 / 0	0 / 0	0 / 0	0 / 0	541 / 0	0 / 0
A	1955	1432 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM TO		FR-TO			
A-G	-3717 / 0	-112.4 -112.4	0.06 (1)	E-B	0 / 2549	0.32 (1)	
G-B	-3036 / 0	-112.4 -112.4	0.09 (1)	B-D	-3147 / 0	0.41 (1)	
B-C	-10 / 0	-112.4 -112.4	0.10 (1)	F-G	0 / 993	0.00 (1)	
D-C	-171 / 0	0.0	0.02 (1)				
A-F	0 / 2722	-18.5 -18.5	0.23 (1)				
F-H	0 / 2722	-18.5 -18.5	0.43 (1)				
H-E	0 / 2722	-18.5 -18.5	0.43 (1)				
E-I	0 / 2722	-18.5 -18.5	0.59 (1)				
I-J	0 / 2722	-18.5 -18.5	0.59 (1)				
J-D	0 / 2722	-18.5 -18.5	0.59 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	9-4	-839	-839	---	TOP	VERT	TOTAL	---	C1
H	2-9-4	-837	-837	---	TOP	VERT	TOTAL	---	C1
I	4-9-4	-837	-837	---	TOP	VERT	TOTAL	---	C1
J	5-10-12	-837	-837	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 30 = 60 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.10/1.00 (B-C:1), BC=0.59/1.00 (D-E:1), WB=0.41/1.00 (B-D:1), SSI=0.43/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (A) (INPUT = 0.90)

JSI METAL= 0.36 (E) (INPUT = 1.00)



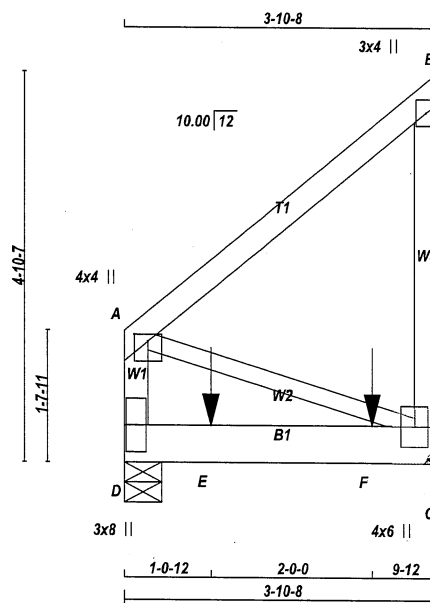
Structural component only
DWG# T-2215384

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423631	T30	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:c3jyj23uDiq_8pvRKbkZpy75XW-Et8K2AwysJ4hhB3hDYkvx3bJhQLn7R8wJzknMYz31zR



Scale = 1:27.5

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 1	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	934	0	934	0
C	1028	0	1028	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	650	484 / 0	0 / 0	0 / 0	0 / 0	167 / 0	0 / 0
C	716	533 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (K)	LC1	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)
FR-TO		FROM	TO	CS1 (K)	FR-TO		
D-A	-218 / 0	0.0	0.0	0.01 (1)	7.81	A-C	0 / 0
A-B	0 / 0	-112.4	-112.4	0.16 (1)	10.00		0.00 (1)
C-B	-218 / 0	0.0	0.0	0.04 (1)	7.81		
D-E	0 / 0	-18.5	-18.5	0.28 (1)	10.00		
E-F	0 / 0	-18.5	-18.5	0.28 (1)	10.00		
F-C	0 / 0	-18.5	-18.5	0.28 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-0-12	-505	-505	---	BACK	VERT	TOTAL	---	C1
F	3-0-12	-506	-506	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 22 = 43 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.16/1.00 (A-B:1), BC=0.28/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (A) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2215385

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423631	T30	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:c3iyi23uDijg 8pvRKbkZpy75XW-Et8K2AwvSJ4hhB3hDYkvx3bJhQLn7R8wjzknMYz31zR

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

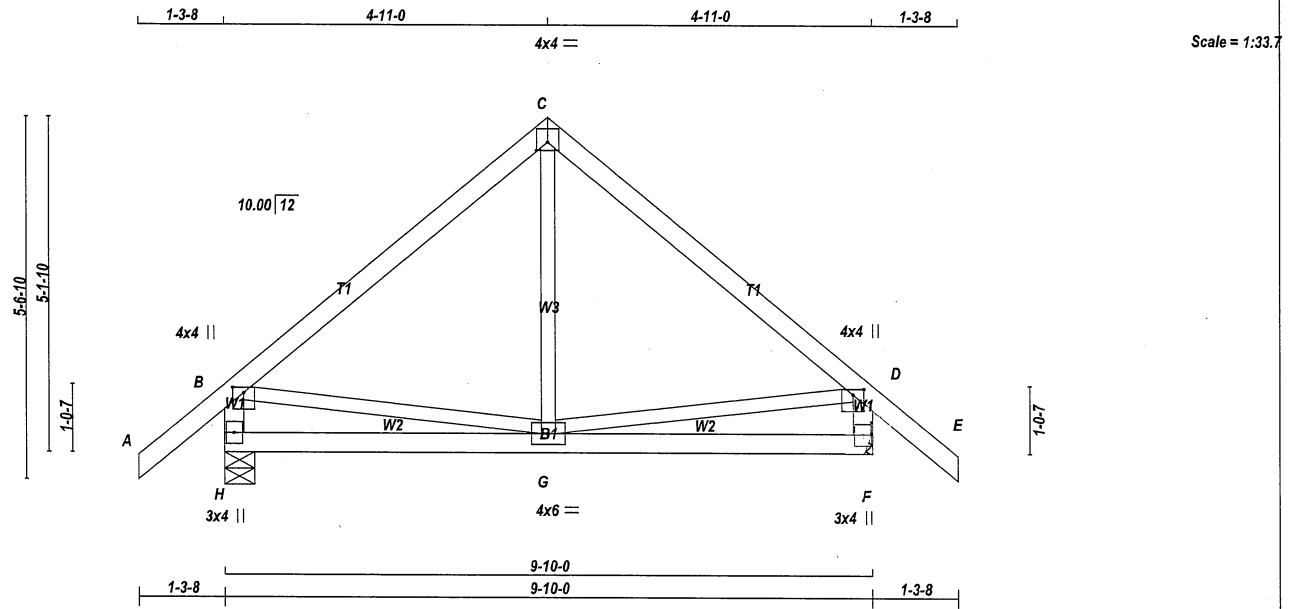


Structural component only
DWG# T-2215385

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423631	T31	3	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:c3jy23uDijq_8pvrKbkZpy75XW-i3iiGWxaDcCYILetnFG8TH8RQpkLstB3YdULu_z31zQ



TOTAL WEIGHT = 3 X 43 = 128 lb [M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
FACTORED	MAXIMUM FACTORED	INPUT	REQD		
GROSS REACTION	GROSS REACTION	BRG	BRG		
JT	VERT	HORZ	UPLIFT	IN-SX	IN-SX
H	799	0	799	0	5-8
F	799	0	799	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	558	409 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
F	558	409 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
A-B	0 / 50	-112.4 -112.4	0.15 (1)	10.00	G-C	0 / 96	0.03 (4)
B-C	-443 / 0	-112.4 -112.4	0.35 (1)	6.25	B-G	0 / 344	0.08 (1)
C-D	-443 / 0	-112.4 -112.4	0.35 (1)	6.25	G-D	0 / 344	0.08 (1)
D-E	0 / 50	-112.4 -112.4	0.15 (1)	10.00			
H-B	-764 / 0	0.0 0.0	0.08 (1)	7.81			
F-D	-764 / 0	0.0 0.0	0.08 (1)	7.81			
H-G	0 / 0	-18.5 -18.5	0.13 (4)	10.00			
G-F	0 / 0	-18.5 -18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.13/1.00 (F-G:4) , WB=0.08/1.00 (D-G:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (D) (INPUT = 0.90)
JSI METAL= 0.18 (D) (INPUT = 1.00)

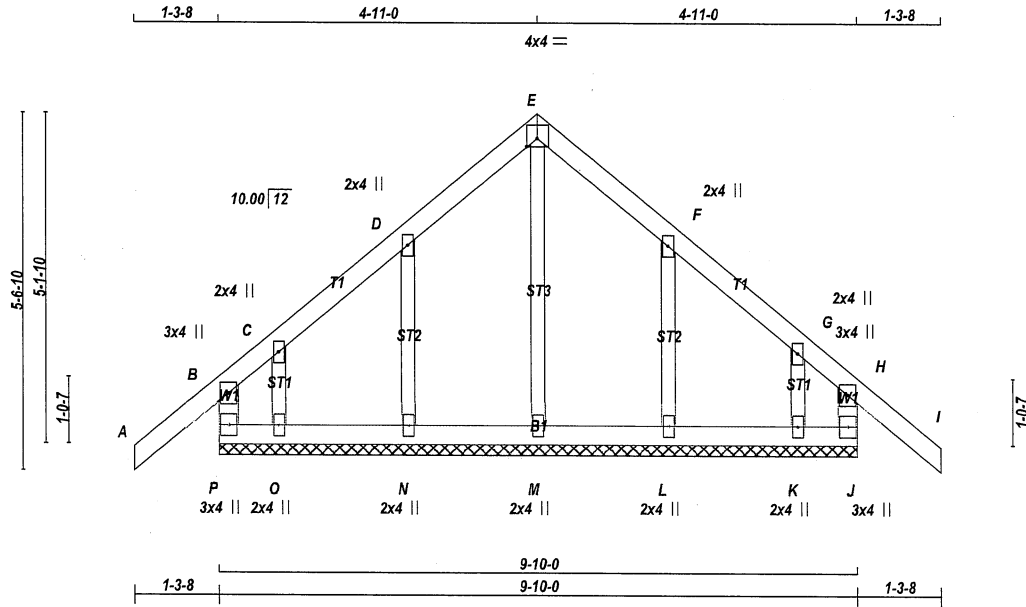


Structural component only
DWG# T-2215386

REVIEWED

JOB NAME 423631	TRUSS NAME G31	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:34.3

TOTAL WEIGHT = 42 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS	SIZE	DRY	No.2	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.				TOP CH.	LL	=	32.5 PSF
P - B	2x4	DRY	No.2	THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.				DL	=	6.0 PSF	
A - E	2x4	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)				BOT CH.	LL	=	0.0 PSF
E - I	2x4	DRY	No.2	BRACING				DL	=	7.4 PSF	
J - H	2x4	DRY	No.2	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				TOTAL LOAD	=	45.9 PSF	
P - J	2x4	DRY	No.2	MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.				SPACING = 24.0 IN. C/C			
ALL WEBS	2x3	DRY	No.2	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
ALL GABLE WEBS	2x3	DRY	No.2	LOADING				THIS DESIGN COMPLIES WITH:			
DRY: SEASONED LUMBER.				TOTAL LOAD CASES: (4)				- PART 9 OF BCBC 2018, ABC 2019			
GABLE STUDS SPACED AT 2-0-0 OC.				CHORDS				- PART 9 OF OBC 2012 (2019 AMENDMENT)			
				MEMB. MAX. FACTORED FORCE (LBS)				- CSA 086-14			
				VERT. LOAD LC1 MAX (PLF)				- TPIC 2014			
				FROM TO				DESIGN ASSUMPTIONS			
				MAX. FACTORED FORCE (LBS)				- OVERHANG NOT TO BE ALTERED OR CUT OFF.			
				MAX. FACTORED FORCE (LBS)				(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD			
				MAX. FACTORED FORCE (LBS)				CSI: TC=0.15/1.00 (A-B:1), BC=0.03/1.00 (O-P:1), WB=0.10/1.00 (E-M:1), SSI=0.09/1.00 (A-B:1)			
				MAX. FACTORED FORCE (LBS)				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10			
				MAX. FACTORED FORCE (LBS)				COMPANION LIVE LOAD FACTOR = 1.00			
				MAX. FACTORED FORCE (LBS)				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
				MAX. FACTORED FORCE (LBS)				NAIL VALUES			
				MAX. FACTORED FORCE (LBS)				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)			
				MAX. FACTORED FORCE (LBS)				MAX MIN MAX MIN MAX MIN			
				MAX. FACTORED FORCE (LBS)				MT20 650 371 1747 788 1987 1873			
				MAX. FACTORED FORCE (LBS)				PLATE PLACEMENT TOL. = 0.250 inches			
				MAX. FACTORED FORCE (LBS)				PLATE ROTATION TOL. = 5.0 Deg.			
				MAX. FACTORED FORCE (LBS)				JSI GRIP= 0.42 (E) (INPUT = 0.90)			
				MAX. FACTORED FORCE (LBS)				JSI METAL= 0.13 (D) (INPUT = 1.00)			

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

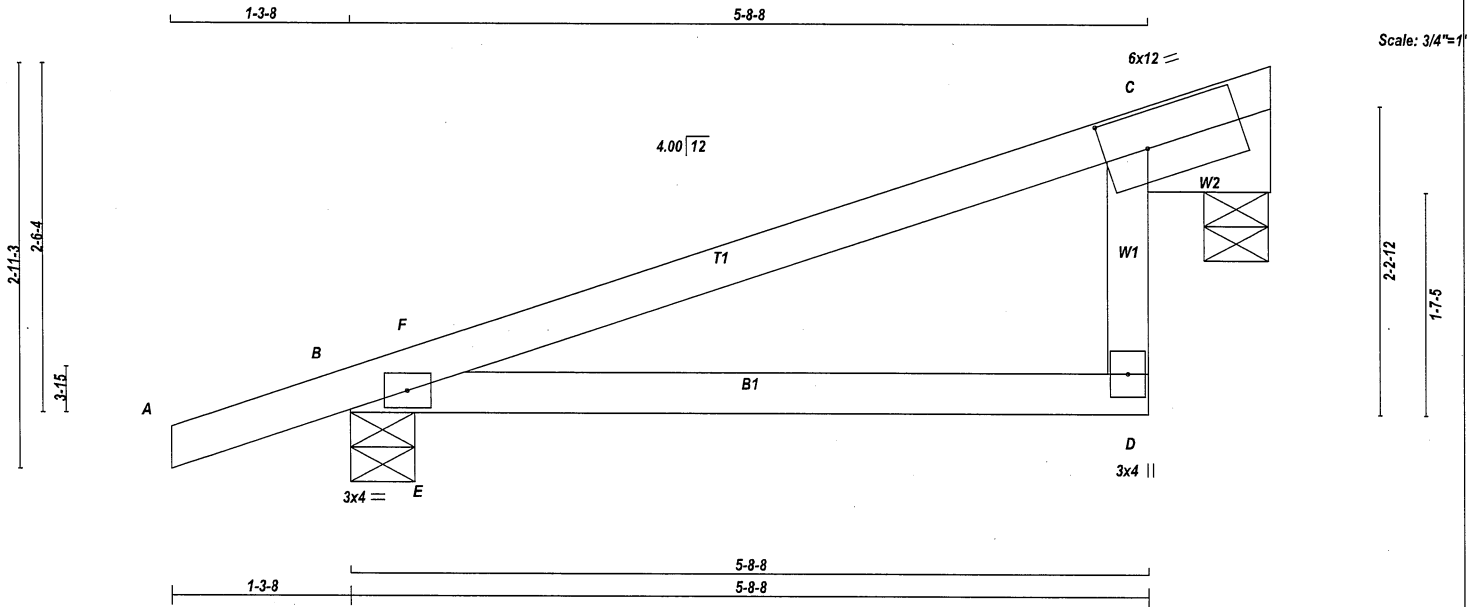


Structural component only
DWG# T-2215377

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423631	T32	4	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 4 X 20 = 80 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMVW1-t	MT20	6.0	12.0	3.25 3.75
D	BMV+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UPLIFT
C	374	0	374	0	0
B	524	0	524	0	0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	262	185 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
B	365	272 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 22	-112.4 -112.4	0.14 (1)	10.00	E-F	-398 / 12	0.00 (1)
B-F	-19 / 25	-112.4 -112.4	0.07 (4)	6.25			
F-C	-3 / 2	-112.4 -112.4	0.47 (1)	10.00			
D-C	0 / 95	0.0 0.0	0.02 (1)	10.00			
B-E	0 / 0	-18.5 -18.5	0.33 (1)	10.00			
E-D	0 / 0	-18.5 -18.5	0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/857 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/453 (0.15")

CSI: TC=0.47/1.00 (C-F:1), BC=0.33/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SSI=0.32/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

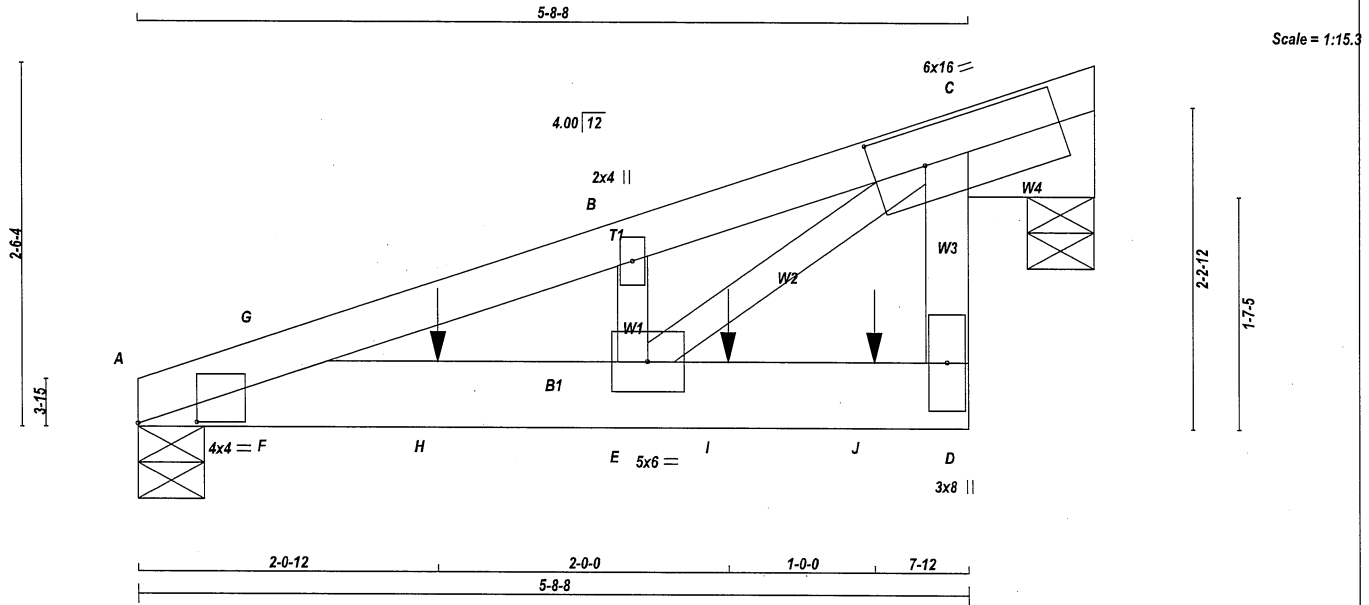


Structural component only
DWG# T-2215387

REVIEWED

JOB NAME 423631	TRUSS NAME T33	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:c3jyj23uDijq_8pvRKbkZpy75XW-fSqTgCzqkDSGYeoGvglcYiDrYdMqKiVM?xzRysz31zO



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		TOP
A - C 1	12	TOP
C - D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		SIDE(0.0)
A - D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	4.0	4.0	0.25	5.00
B	TMW+W	MT20	2.0	4.0		
C	TMVWW1-t	MT20	6.0	16.0	3.00	4.25
D	BMV+p	MT20	3.0	8.0		
E	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP	IN-SX
A	1186	0	1186	0	5-8
C	1907	0	1907	0	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	827	611 / 0	0 / 0	0 / 0	0 / 0	216 / 0	0 / 0
C	1328	988 / 0	0 / 0	0 / 0	0 / 0	341 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
MEMB.	FORCE	VERT.	LOAD	LC1	MAX	MAX	MAX	MAX	MAX
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH	FR-TO	FROM
A-G	-2338 / 0	-112.4	-112.4	0.09 (1)	5.78	E-B	-296 / 0	0.02 (1)	
G-B	-2297 / 0	-112.4	-112.4	0.12 (1)	5.78	E-C	0 / 2792	0.35 (1)	
B-C	-2294 / 0	-112.4	-112.4	0.08 (1)	5.83	F-G	-57 / 8	0.00 (1)	
D-C	0 / 751	0.0	0.0	0.07 (1)	10.00				
A-F	0 / 2170	-18.5	-18.5	0.29 (1)	10.00				
F-H	0 / 2190	-18.5	-18.5	0.38 (1)	10.00				
H-E	0 / 2190	-18.5	-18.5	0.38 (1)	10.00				
E-I	0 / 0	-18.5	-18.5	0.18 (1)	10.00				
I-J	0 / 0	-18.5	-18.5	0.18 (1)	10.00				
J-D	0 / 0	-18.5	-18.5	0.18 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	2-0-12	-543	-543	---	FRONT	VERT	TOTAL	---	C1
I	4-0-12	-543	-543	---	FRONT	VERT	TOTAL	---	C1
J	5-0-12	-546	-546	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 24 = 49 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.12/1.00 (B-G:1), BC=0.38/1.00 (E-F:1), WB=0.35/1.00 (C-E:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (E) (INPUT = 0.90)
JSI METAL= 0.42 (A) (INPUT = 1.00)



Structural component only
DWG# T-2215388

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423631	T33	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:c3jvi23uDijg 8pvRKbkZpy75XW-fSgTgCzqkDSGYeoGvglcYiDrYdMqKiVM?xzRysz31zO

NOTES- (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

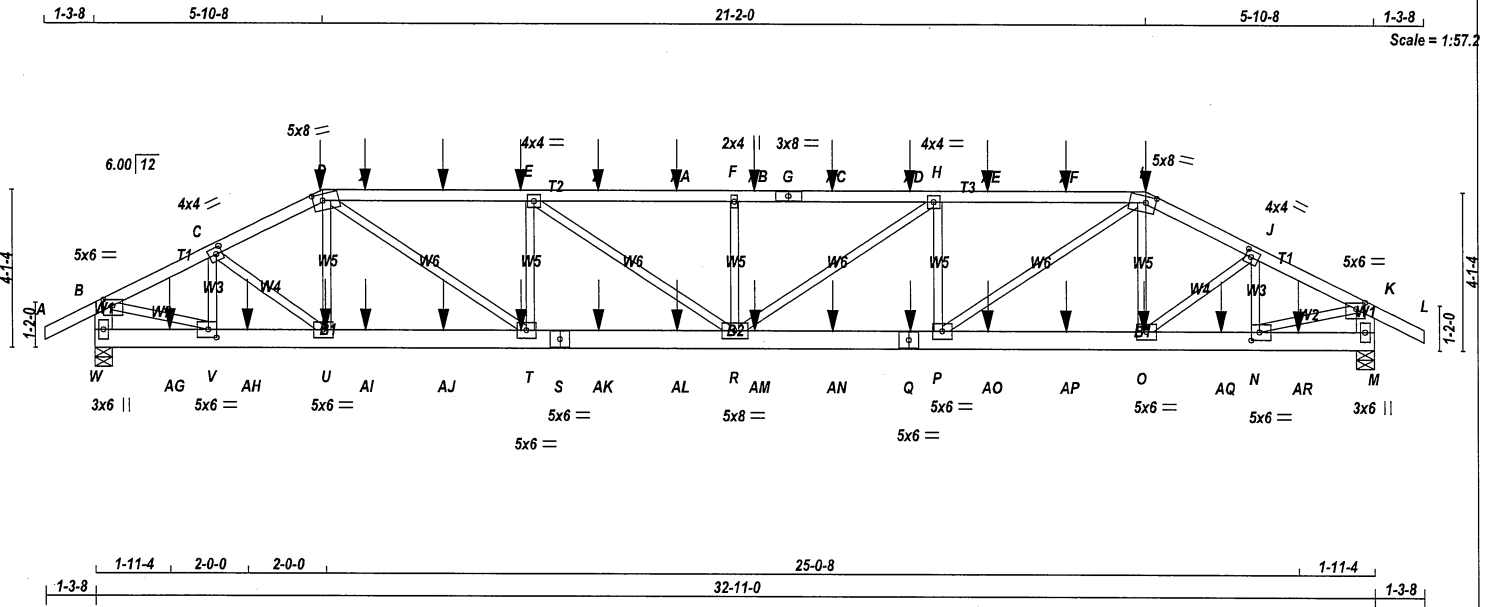


Structural component only
 DWG# T-2215388

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423633	T50	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:c3jyj23uDiJq_8pvrKbkZpy75XW-xT3WusYFr25GyfbxrndkjiZn8Mrla3ho?qNl10z31XX



TOTAL WEIGHT = 2 X 150 = 300 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	SIDE(61.0)
D-G 1	12	SIDE(61.0)
G-I 1	12	SIDE(61.0)
I-L 1	12	SIDE(61.0)
W-B 2	12	TOP
M-K 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W-S 2	12	SIDE(183.1)
S-Q 2	12	SIDE(0.1)
Q-M 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
W	3663	0	0	3663	0	0	5-8	5-8	5-8
M	3618	0	0	3618	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2569	1809 / 0	0 / 0	0 / 0	0 / 0	760 / 0	0 / 0
M	2537	1787 / 0	0 / 0	0 / 0	0 / 0	750 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 34	-112.4 -112.4	0.08 (1)	10.00	V-C	-1193 / 0	0.10 (1)
B-C	-4808 / 0	-112.4 -112.4	0.16 (1)	4.28	C-U	0 / 797	0.10 (1)
C-D	-5536 / 0	-112.4 -112.4	0.17 (1)	4.02	U-D	-200 / 71	0.03 (1)
D-X	-7391 / 0	-112.4 -112.4	0.71 (1)	3.01	D-T	0 / 2997	0.37 (1)
X-Y	-7391 / 0	-112.4 -112.4	0.71 (1)	3.01	T-E	-1574 / 0	0.20 (1)
Y-E	-7391 / 0	-112.4 -112.4	0.71 (1)	3.01	E-R	0 / 859	0.11 (1)
E-Z	-8090 / 0	-112.4 -112.4	0.77 (1)	2.82	R-F	-873 / 0	0.11 (1)
Z-AA	-8090 / 0	-112.4 -112.4	0.77 (1)	2.82	F-H	0 / 913	0.11 (1)
AA-F	-8090 / 0	-112.4 -112.4	0.77 (1)	2.82	H-P	-1595 / 0	0.20 (1)
F-AB	-8090 / 0	-112.4 -112.4	0.79 (1)	2.79	P-I	0 / 3030	0.38 (1)
AB-G	-8090 / 0	-112.4 -112.4	0.79 (1)	2.79	I-O	-201 / 62	0.03 (1)
G-AC	-8090 / 0	-112.4 -112.4	0.79 (1)	2.79	O-J	0 / 780	0.10 (1)
AC-AD	-8090 / 0	-112.4 -112.4	0.79 (1)	2.79	J-N	-1176 / 0	0.10 (1)
AD-H	-8090 / 0	-112.4 -112.4	0.79 (1)	2.79	N-B	0 / 4446	0.55 (1)
H-AE	-7347 / 0	-112.4 -112.4	0.73 (1)	2.99	B-K	0 / 4387	0.54 (1)
AE-AF	-7347 / 0	-112.4 -112.4	0.73 (1)	2.99			
AF-I	-7347 / 0	-112.4 -112.4	0.73 (1)	2.99			
I-J	-5457 / 0	-112.4 -112.4	0.16 (1)	4.05			
J-K	-4743 / 0	-112.4 -112.4	0.15 (1)	4.30			
K-L	0 / 34	-112.4 -112.4	0.08 (1)	10.00			
W-B	-3554 / 0	0.0	0.0	0.13 (1)	7.43		
M-K	-3510 / 0	0.0	0.0	0.12 (1)	7.47		

W-AG	0 / 0	-18.5	-18.5	0.08 (1)	10.00
AG-V	0 / 0	-18.5	-18.5	0.08 (1)	10.00
V-AH	0 / 4309	-18.5	-18.5	0.36 (1)	10.00
AH-U	0 / 4309	-18.5	-18.5	0.36 (1)	10.00
U-AI	0 / 4931	-18.5	-18.5	0.36 (1)	10.00
AI-AJ	0 / 4931	-18.5	-18.5	0.36 (1)	10.00
AJ-T	0 / 4931	-18.5	-18.5	0.36 (1)	10.00
T-S	0 / 7391	-18.5	-18.5	0.55 (1)	10.00
S-AK	0 / 7391	-18.5	-18.5	0.55 (1)	10.00
AK-AL	0 / 7391	-18.5	-18.5	0.55 (1)	10.00
AL-R	0 / 7391	-18.5	-18.5	0.55 (1)	10.00
R-AM	0 / 7347	-18.5	-18.5	0.55 (1)	10.00
AM-AN	0 / 7347	-18.5	-18.5	0.55 (1)	10.00
AN-Q	0 / 7347	-18.5	-18.5	0.55 (1)	10.00
Q-P	0 / 7347	-18.5	-18.5	0.55 (1)	10.00
P-AO	0 / 4860	-18.5	-18.5	0.36 (1)	10.00
AO-AP	0 / 4860	-18.5	-18.5	0.36 (1)	10.00
AP-O	0 / 4860	-18.5	-18.5	0.36 (1)	10.00
O-AQ	0 / 4251	-18.5	-18.5	0.36 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/962 (0.41")
CSI: TC=0.79/1.00 (F-H:1), BC=0.55/1.00 (R-T:1), WB=0.55/1.00 (B-V:1), SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.68 (S) (INPUT = 1.00)



Structural component only
DWG# T-2215435

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423633	T50	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:c3jvj23uDiq 8pvRKbkZoy75XW-xT3WusYFr25GvfbxrdnkJiZN8Mrla3ho?qNl10z31XX

PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-p MT20 5.0 6.0 2.00 2.75 C TMWW-t MT20 4.0 4.0 2.00 1.75 D TTWW-m MT20 5.0 8.0 2.00 3.00 E TMWW-t MT20 4.0 4.0 F TMW+w MT20 2.0 4.0 G TS-t MT20 3.0 8.0 H TMWW-t MT20 4.0 4.0 I TTWW-m MT20 5.0 8.0 2.00 3.00 J TMWW-t MT20 4.0 4.0 2.00 1.75 K TMVW-p MT20 5.0 6.0 2.00 2.75 M BMV1+p MT20 3.0 6.0 N BMWW-t MT20 5.0 6.0 2.50 2.50 O P, T, U Q BMWW-t MT20 5.0 6.0 R BS-t MT20 5.0 6.0 S BMWW-t MT20 5.0 8.0 T BS-t MT20 5.0 6.0 V BMWW-t MT20 5.0 6.0 2.50 2.50 W BMV1+p MT20 3.0 6.0				LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE VERT. LOAD LC1 MAX. MAX. FACTORED MEMB. (LBS) (PLF) CSI (LC) UNBRAC MEMB. FORCE MAX (LBS) CSI (LC) FR-TO FROM TO LENGTH FR-TO AQ-N 0 / 4251 -18.5 -18.5 0.36 (1) 10.00 N-AR 0 / 0 -18.5 -18.5 0.08 (1) 10.00 AR-M 0 / 0 -18.5 -18.5 0.08 (1) 10.00				WEBS MAX. FACTORED FORCE MAX. FACTORED MEMB. (LBS) CSI (LC) UNBRAC MEMB. FORCE MAX (LBS) CSI (LC) FR-TO FROM TO LENGTH FR-TO AQ-N 0 / 4251 -18.5 -18.5 0.36 (1) 10.00 N-AR 0 / 0 -18.5 -18.5 0.08 (1) 10.00 AR-M 0 / 0 -18.5 -18.5 0.08 (1) 10.00				SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. D 5-10-8 -308 -308 --- FRONT VERT TOTAL --- C1 E 10-11-12 -93 -93 --- FRONT VERT TOTAL --- C1 I 27-0-8 -308 -308 --- FRONT VERT TOTAL --- C1 O 26-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 Q 20-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 T 10-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 U 5-11-4 -21 -21 --- FRONT VERT TOTAL --- C1 X 6-11-12 -93 -93 --- FRONT VERT TOTAL --- C1 Y 8-11-12 -93 -93 --- FRONT VERT TOTAL --- C1 Z 12-11-12 -93 -93 --- FRONT VERT TOTAL --- C1 AA 14-11-12 -93 -93 --- FRONT VERT TOTAL --- C1 AB 16-11-12 -93 -93 --- FRONT VERT TOTAL --- C1 AC 18-11-12 -93 -93 --- FRONT VERT TOTAL --- C1 AD 20-11-12 -93 -93 --- FRONT VERT TOTAL --- C1 AE 22-11-12 -93 -93 --- FRONT VERT TOTAL --- C1 AF 24-11-12 -93 -93 --- FRONT VERT TOTAL --- C1 AG 1-11-4 -20 -20 --- FRONT VERT TOTAL --- C1 AH 3-11-4 -21 -21 --- FRONT VERT TOTAL --- C1 AI 6-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 AJ 8-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 AK 12-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 AL 14-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 AM 16-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 AN 18-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 AO 22-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 AP 24-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 AQ 28-11-12 -21 -21 --- FRONT VERT TOTAL --- C1 AR 30-11-12 -20 -20 --- FRONT VERT TOTAL --- C1				CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.			
--	--	--	--	--	--	--	--	---	--	--	--	--	--	--	--	---	--	--	--

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2215435

REVIEWED

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	T50S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:c3jivl23uDijg 8pvRKbkZpy75XW-8b8f779cn0p?MdB7Sam4zfzZLVCc KpadGYUCFz31bv

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.00	2.75
E	TMWW-t	MT20	5.0	6.0	2.50	2.25
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	6.0	10.0	2.00	4.50
J	TMWW-t	MT20	4.0	4.0	2.00	1.75
K						
K	TMBVWWW-t	MT18HS	10.0	16.0	Edge	6.00
L	TMVW-t	MT20	6.0	10.0	2.75	3.75
N	BMVW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P						
Q	BMWW-t	MT20	4.0	6.0		
R	BMWW-t	MT20	3.0	8.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BVMWWW-t	MT20	7.0	12.0	4.75	4.00
U	BMV+p	MT20	3.0	8.0		
V	BMWWW-t	MT20	6.0	10.0	3.00	3.25
W	BMWW-t	MT20	4.0	4.0	2.50	2.00
X	BMWW-t	MT20	5.0	6.0	2.50	2.25
Y	BMV1+p	MT20	3.0	8.0	4.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX
FR-TO		FROM	TO		FR-TO		FROM	TO	
AQ-AR	0 / 7887	-18.5	-18.5	0.63 (1)	10.00				
AR-R	0 / 7887	-18.5	-18.5	0.63 (1)	10.00				
R-AS	0 / 8714	-18.5	-18.5	0.71 (1)	10.00				
AS-Q	0 / 8714	-18.5	-18.5	0.71 (1)	10.00				
Q-P	0 / 10716	-18.5	-18.5	0.84 (1)	10.00				
O-P	0 / 16	0.0	0.0	0.45 (1)	10.00				
P-K	0 / 1830	0.0	0.0	0.68 (1)	10.00				
O-N	0 / 282	-18.5	-18.5	0.03 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-308	-308		FRONT	VERT	TOTAL		C1
E	10-11-12	-93	-93		FRONT	VERT	TOTAL		C1
H	20-11-12	-94	-94		FRONT	VERT	TOTAL		C1
I	27-0-8	-100	-100		FRONT	VERT	TOTAL		C1
P	31-4-12	-228	-228		FRONT	VERT	TOTAL		C1
Q	29-4-12	-243	-243		FRONT	VERT	TOTAL		C1
R	26-11-12	-72	-72		FRONT	VERT	TOTAL		C1
S	20-11-12	-72	-72		FRONT	VERT	TOTAL		C1
W	5-11-4	-21	-21		FRONT	VERT	TOTAL		C1
Z	6-11-12	-93	-93		FRONT	VERT	TOTAL		C1
AA	8-11-12	-93	-93		FRONT	VERT	TOTAL		C1
AB	12-11-12	-93	-93		FRONT	VERT	TOTAL		C1
AC	14-11-12	-93	-93		FRONT	VERT	TOTAL		C1
AD	16-11-12	-94	-94		FRONT	VERT	TOTAL		C1
AE	18-11-12	-94	-94		FRONT	VERT	TOTAL		C1
AF	22-11-12	-94	-94		FRONT	VERT	TOTAL		C1
AG	24-11-12	-94	-94		FRONT	VERT	TOTAL		C1
AH	1-11-4	-20	-20		FRONT	VERT	TOTAL		C1
AI	3-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AJ	6-11-12	-21	-21		FRONT	VERT	TOTAL		C1
AK	8-11-12	-21	-21		FRONT	VERT	TOTAL		C1
AL	10-11-12	-21	-21		FRONT	VERT	TOTAL		C1
AM	12-11-12	-21	-21		FRONT	VERT	TOTAL		C1
AN	14-11-12	-25	-25		FRONT	VERT	TOTAL		C1
AO	16-11-12	-72	-72		FRONT	VERT	TOTAL		C1
AP	18-11-12	-72	-72		FRONT	VERT	TOTAL		C1
AQ	22-11-12	-72	-72		FRONT	VERT	TOTAL		C1
AR	24-11-12	-72	-72		FRONT	VERT	TOTAL		C1
AS	27-4-12	-243	-243		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2215406

REVIEWED



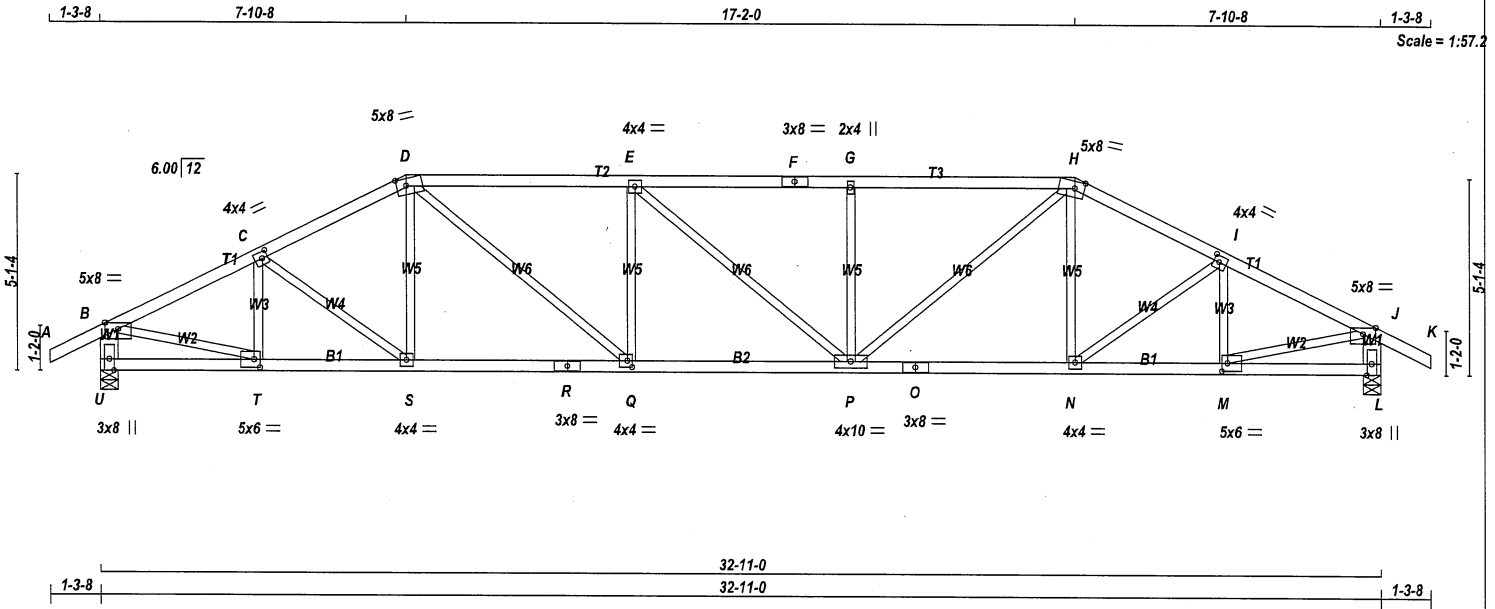
Structural component only
DWG# T-2215406

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423633	T51	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 16:12:13 2022 Page 1
ID:c3jy23uDiq_8pvrKbkZpy75XW-Qfdu5CZtcMD7ZpA7PLlzs6X1m94JVnyEU6saTz31XW



TOTAL WEIGHT = 133 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTVW-m	MT20	5.0	8.0	2.25	3.00
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTVW-m	MT20	5.0	8.0	2.25	3.00
I	TMVW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	8.0	Edge	
M	BMVW-t	MT20	5.0	6.0	2.50	1.75
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	10.0		
Q	BMVW-t	MT20	4.0	4.0	2.00	1.50
R	BS-t	MT20	3.0	8.0		
S	BMVW-t	MT20	4.0	4.0		
T	BMVW-t	MT20	5.0	6.0	2.50	1.75
U	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
U	2307	0	2307	0	5-8
L	2307	0	2307	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
L	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	MAX	CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	T-C	-519 / 0
B-C	-3013 / 0	-112.4	-112.4	0.40 (1)	3.67	C-S	-23 / 4
C-D	-3037 / 0	-112.4	-112.4	0.39 (1)	3.66	S-D	0 / 117
D-E	-3656 / 0	-112.4	-112.4	0.84 (1)	2.91	D-Q	0 / 1248
E-F	-3654 / 0	-112.4	-112.4	0.83 (1)	2.91	Q-E	-696 / 0
F-G	-3654 / 0	-112.4	-112.4	0.83 (1)	2.91	E-P	-2 / 0
G-H	-3654 / 0	-112.4	-112.4	0.83 (1)	2.92	P-G	-695 / 0
H-I	-3037 / 0	-112.4	-112.4	0.39 (1)	3.66	P-H	0 / 1245
I-J	-3013 / 0	-112.4	-112.4	0.40 (1)	3.67	N-H	0 / 119
J-K	0 / 34	-112.4	-112.4	0.15 (1)	10.00	N-I	-22 / 4
U-B	-2267 / 0	0.0	0.0	0.15 (1)	6.81	M-I	-519 / 0
L-J	-2267 / 0	0.0	0.0	0.15 (1)	6.81	B-T	0 / 2774
						M-J	0 / 2774
U-T	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
T-S	0 / 2713	-18.5	-18.5	0.49 (1)	10.00		
S-R	0 / 2698	-18.5	-18.5	0.49 (1)	10.00		
R-Q	0 / 2698	-18.5	-18.5	0.49 (1)	10.00		
Q-P	0 / 3656	-18.5	-18.5	0.64 (1)	10.00		
P-O	0 / 2698	-18.5	-18.5	0.49 (1)	10.00		
O-N	0 / 2698	-18.5	-18.5	0.49 (1)	10.00		
N-M	0 / 2713	-18.5	-18.5	0.50 (1)	10.00		
M-L	0 / 0	-18.5	-18.5	0.07 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.84/1.00 (D-E:1) , BC=0.64/1.00 (P-Q:1) ,
WB=0.62/1.00 (B-T:1) , SS=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.83 (O) (INPUT = 1.00)



Structural component only
DWG# T-2215436

REVIEWED



TOTAL WEIGHT = 147 lb

REVIEWED CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T51S	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 16:07:33 2022 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BMV+p	MT20	3.0	4.0		
V	BMVWWW*+t	MT20	8.0	9.0	3.25	1.75
W	BMVW-t	MT20	5.0	6.0	2.50	1.75
X	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



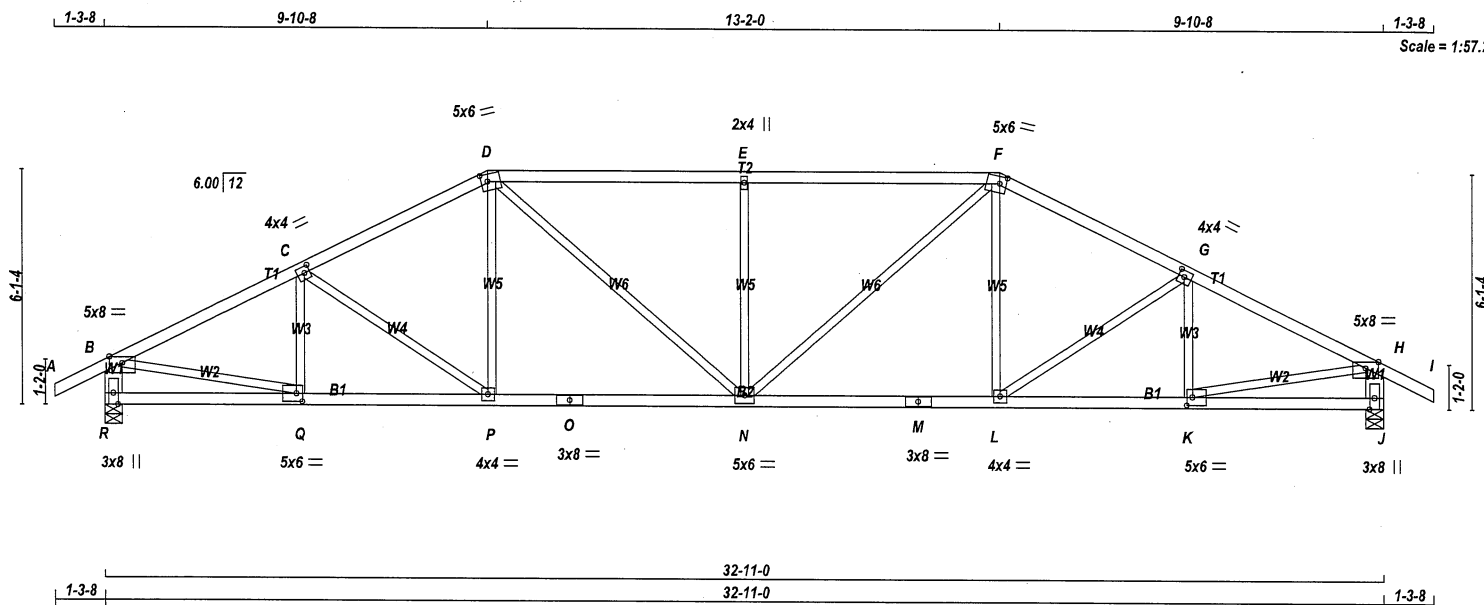
Structural component only
DWG# T-2215407

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423633	T52	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Fri Jun 24 16:12:14 2022 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
R - B	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75	
D	TTWW-m	MT20	5.0	6.0	2.25	2.00	
E	TMVW-w	MT20	2.0	4.0			
F	TTWW-m	MT20	5.0	6.0	2.25	2.00	
G	TMVW-t	MT20	4.0	4.0	2.00	1.75	
H	TMVW-p	MT20	5.0	8.0	Edge		
J	BMV1-p	MT20	3.0	8.0	Edge		
K	BMVW-t	MT20	5.0	6.0	2.50	1.75	
L	BMVW-t	MT20	4.0	4.0			
M	BS-t	MT20	3.0	8.0			
N	BMVW-t	MT20	5.0	6.0			
O	BS-t	MT20	3.0	8.0			
P	BMVW-t	MT20	4.0	4.0			
Q	BMVW-t	MT20	5.0	6.0	2.50	1.75	
R	BMV1-p	MT20	3.0	8.0	Edge		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
R	2307	0	2307	0	0	0	5-8	5-8	
J	2307	0	2307	0	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
J	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 34	-112.4 -112.4	0.15 (1)	10.00	Q-C	-395 / 0	0.09 (1)
B-C	-3122 / 0	-112.4 -112.4	0.48 (1)	3.54	C-P	-297 / 0	0.18 (1)
C-D	-2906 / 0	-112.4 -112.4	0.45 (1)	3.68	P-D	0 / 283	0.06 (1)
D-E	-3161 / 0	-112.4 -112.4	0.82 (1)	2.99	D-N	0 / 778	0.18 (1)
E-F	-3161 / 0	-112.4 -112.4	0.82 (1)	2.99	N-E	-910 / 0	0.53 (1)
F-G	-2906 / 0	-112.4 -112.4	0.45 (1)	3.68	N-F	0 / 778	0.18 (1)
G-H	-3122 / 0	-112.4 -112.4	0.48 (1)	3.54	L-F	0 / 283	0.06 (1)
H-I	0 / 34	-112.4 -112.4	0.15 (1)	10.00	L-G	-297 / 0	0.18 (1)
R-B	-2263 / 0	0.0 0.0	0.15 (1)	6.81	K-G	-395 / 0	0.09 (1)
J-H	-2263 / 0	0.0 0.0	0.15 (1)	6.81	B-Q	0 / 2858	0.64 (1)
					K-H	0 / 2858	0.64 (1)
R-Q	0 / 0	-18.5 -18.5	0.10 (4)	10.00			
Q-P	0 / 2817	-18.5 -18.5	0.53 (1)	10.00			
P-O	0 / 2577	-18.5 -18.5	0.49 (1)	10.00			
O-N	0 / 2577	-18.5 -18.5	0.49 (1)	10.00			
N-M	0 / 2577	-18.5 -18.5	0.49 (1)	10.00			
M-L	0 / 2577	-18.5 -18.5	0.49 (1)	10.00			
L-K	0 / 2817	-18.5 -18.5	0.53 (1)	10.00			
K-J	0 / 0	-18.5 -18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")
CSI: TC=0.82/1.00 (D-E:1), BC=0.53/1.00 (P-Q:1), WB=0.64/1.00 (B-Q:1), SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.80 (O) (INPUT = 1.00)

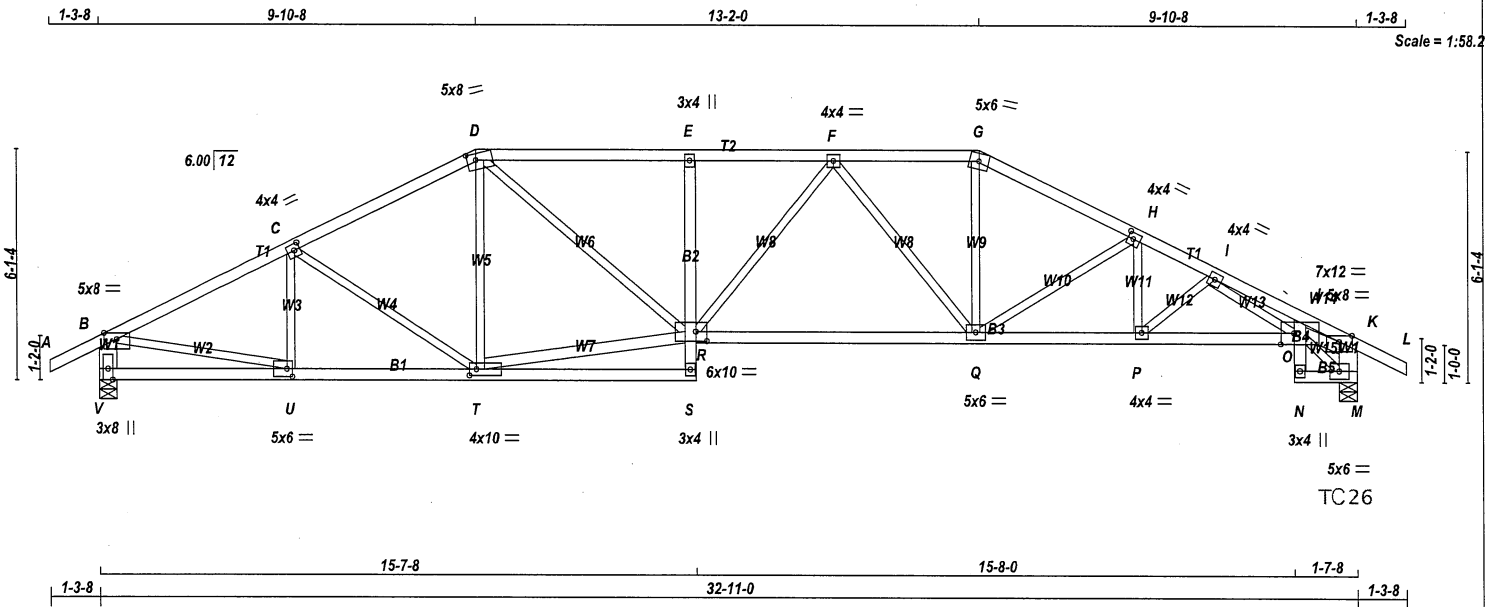


Structural component only
DWG# T-2215437

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T52S	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 16:07:34 2022 Page 1
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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - G 2x4 DRY No.2 SPF G - L 2x4 DRY No.2 SPF V - B 2x6 DRY No.2 SPF M - K 2x6 DRY No.2 SPF V - S 2x4 DRY No.2 SPF S - E 2x4 DRY No.2 SPF R - O 2x4 DRY No.2 SPF N - J 2x4 DRY No.2 SPF N - M 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT T - R 2x4 DRY No.2 SPF O - K 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX V 2304 0 2304 0 0 5-8 5-8 M 2311 0 2311 0 0 5-8 5-8 ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD. UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL V 1612 1155 / 0 0 / 0 0 / 0 457 / 0 0 / 0 M 1617 1159 / 0 0 / 0 0 / 0 458 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.66 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FACTORED WEBS MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX. MAX. MEMB. FORCE MAX. (LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC) FR-TO FROM TO LENGTH FR-TO A-B 0 / 34 -112.4 -112.4 0.15 (1) 10.00 U-C -397 / 0 0.09 (1) B-C -3117 / 0 -112.4 -112.4 0.48 (1) 3.55 C-T -294 / 0 0.18 (1) C-D -2905 / 0 -112.4 -112.4 0.45 (1) 3.68 T-D -158 / 36 0.09 (1) D-E -3705 / 0 -112.4 -112.4 0.63 (1) 3.09 T-R 0 / 2526 0.41 (1) E-F -3722 / 0 -112.4 -112.4 0.47 (1) 3.23 D-R 0 / 1494 0.34 (1) F-G -3119 / 0 -112.4 -112.4 0.31 (1) 3.71 Q-G 0 / 1160 0.26 (1) G-H -3476 / 0 -112.4 -112.4 0.39 (1) 3.47 Q-H -782 / 0 0.33 (1) H-I -4176 / 0 -112.4 -112.4 0.32 (1) 3.22 P-H 0 / 331 0.07 (1) I-J -4990 / 0 -112.4 -112.4 0.55 (1) 2.66 B-U 0 / 2853 0.64 (1) J-K -4563 / 0 -112.4 -112.4 0.47 (1) 2.89 R-F 0 / 244 0.05 (1) K-L 0 / 34 -112.4 -112.4 0.15 (1) 10.00 F-Q -743 / 0 0.48 (1) V-B -2259 / 0 0.0 0.0 0.15 (1) 6.81 O-M -371 / 0 0.05 (1) M-K -2089 / 0 0.0 0.0 0.14 (1) 7.02 O-K 0 / 4001 0.64 (1) V-U 0 / 0 -18.5 -18.5 0.10 (4) 10.00 I-O 0 / 543 0.12 (1) U-T 0 / 2812 -18.5 -18.5 0.53 (1) 10.00 T-S 0 / 84 -18.5 -18.5 0.13 (4) 10.00 S-R 0 / 47 0.0 0.0 0.12 (1) 10.00 R-E -622 / 0 0.0 0.0 0.20 (1) 7.81 R-Q 0 / 3572 -18.5 -18.5 0.70 (1) 10.00 Q-P 0 / 3751 -18.5 -18.5 0.72 (1) 10.00 P-O 0 / 4084 -18.5 -18.5 0.73 (1) 10.00 N-O 0 / 15 0.0 0.0 0.42 (1) 10.00 O-J -35 / 14 0.0 0.0 0.32 (1) 7.81 N-M 0 / 308 -18.5 -18.5 0.06 (1) 10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.10") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.25") ALLOWABLE DEFL.(TL)= L/360 (1.10") CALCULATED VERT. DEFL.(TL)= L/ 791 (0.50") CSI: TC=0.63/1.00 (D-E:1), BC=0.73/1.00 (O-P:1), WB=0.64/1.00 (K-O:1), SSI=0.42/1.00 (J-O:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (R) (INPUT = 0.90) JSI METAL= 0.69 (K) (INPUT = 1.00)			
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Structural component only
DWG# T-2215408

REVIEWED
CONTINUED ON PAGE 21

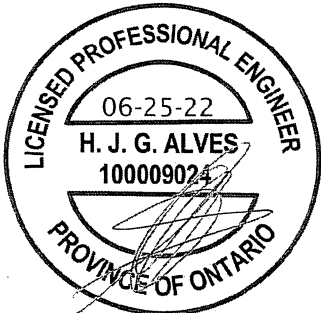
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	T52S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:c3iyi23uDijg 8ovRKbkZoy75XW-4 GQYpAtJe4jbXLWZFoY242vmlvTSIQ4a1bH8z31bt

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



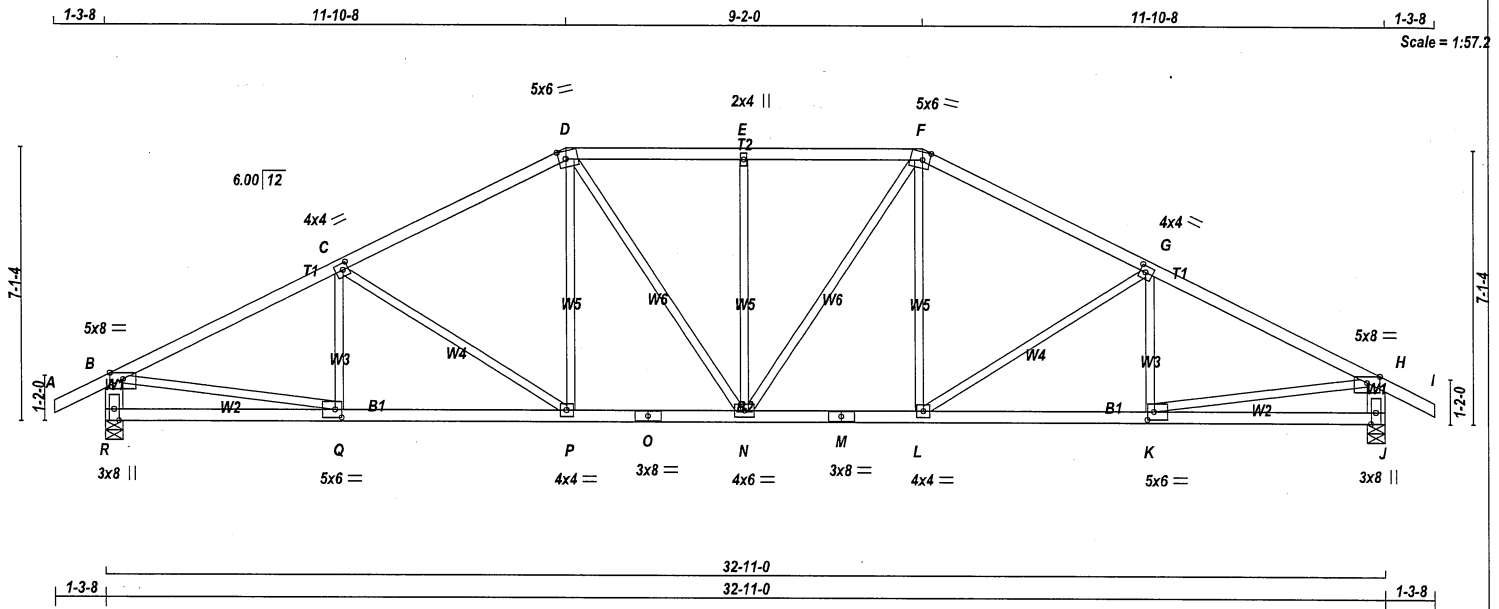
Structural component only
DWG# T-2215408

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423633	T53	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 139 lb
[M/F]

LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.50	2.25
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.25
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	8.0	Edge	
K	BMVW-t	MT20	5.0	6.0	2.50	2.00
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-w	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
R	2307	0	2307	0	0	5-8	5-8		
J	2307	0	2307	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
J	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 34	-112.4 -112.4	0.15 (1)	10.00	Q-C	-291 / 24 0.08 (1)	
B-C	-3175 / 0	-112.4 -112.4	0.71 (1)	3.23	C-P	-556 / 0 0.54 (1)	
C-D	-2726 / 0	-112.4 -112.4	0.64 (1)	3.55	P-D	0 / 408 0.09 (1)	
D-E	-2645 / 0	-112.4 -112.4	0.38 (1)	3.91	D-N	0 / 419 0.09 (1)	
E-F	-2645 / 0	-112.4 -112.4	0.38 (1)	3.91	N-E	-625 / 0 0.55 (1)	
F-G	-2726 / 0	-112.4 -112.4	0.64 (1)	3.55	N-F	0 / 419 0.09 (1)	
G-H	-3175 / 0	-112.4 -112.4	0.71 (1)	3.23	L-F	0 / 408 0.09 (1)	
H-I	0 / 34	-112.4 -112.4	0.15 (1)	10.00	L-G	-556 / 0 0.54 (1)	
R-B	-2258 / 0	0.0 0.0	0.15 (1)	6.81	K-G	-291 / 24 0.08 (1)	
J-H	-2258 / 0	0.0 0.0	0.15 (1)	6.81	B-Q	0 / 2899 0.65 (1)	
					K-H	0 / 2899 0.65 (1)	
R-Q	0 / 0	-18.5 -18.5	0.15 (4)	10.00			
Q-P	0 / 2871	-18.5 -18.5	0.52 (1)	10.00			
P-O	0 / 2411	-18.5 -18.5	0.45 (1)	10.00			
O-N	0 / 2411	-18.5 -18.5	0.45 (1)	10.00			
N-M	0 / 2411	-18.5 -18.5	0.45 (1)	10.00			
M-L	0 / 2411	-18.5 -18.5	0.45 (1)	10.00			
L-K	0 / 2871	-18.5 -18.5	0.52 (1)	10.00			
K-J	0 / 0	-18.5 -18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.71/1.00 (G-H:1), BC=0.52/1.00 (P-Q:1), WB=0.65/1.00 (B-Q:1), SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

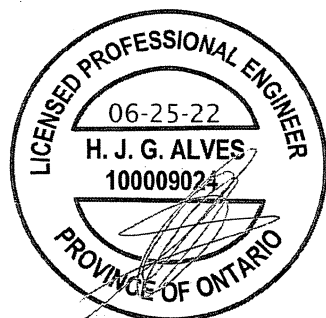
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.71 (O) (INPUT = 1.00)



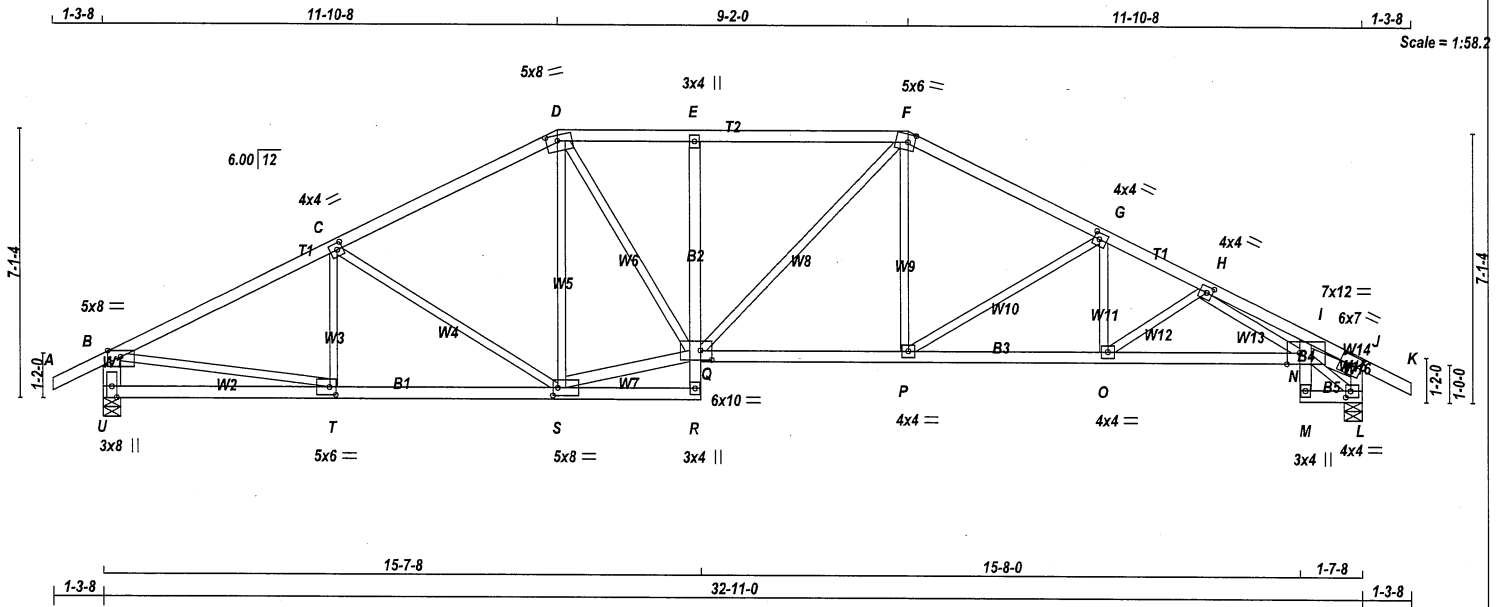
Structural component only
DWG# T-2215438

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	T53S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 146 lb

(M)(F)

LUMBER				DESCR.	
N. L. G. A. RULES				BUILDING DESIGNER	
CHORDS	SIZE	LUMBER		BEARINGS	
A - D	2x4	DRY	2100F 1.8E	SPF	FACTORED
D - F	2x4	DRY	2100F 1.8E	SPF	GROSS REACTION
F - K	2x4	DRY	2100F 1.8E	SPF	VERT HORZ
U - B	2x6	DRY	No.2	SPF	JT
L - J	2x4	DRY	No.2	SPF	U
U - R	2x4	DRY	No.2	SPF	L
R - E	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
M - I	2x4	DRY	No.2	SPF	
M - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT					
S - Q	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C, G, H					
C TMVW-t	MT20	4.0	4.0	2.00	1.75
D TTWW-m	MT20	5.0	8.0	1.75	3.50
E TMV+p	MT20	3.0	4.0		
F TTWW-m	MT20	5.0	6.0	2.50	2.25
I					
I TMBVWWVW-t	MT20	7.0	12.0	Edge	4.00
J TMVW-t	MT20	6.0	7.0		
L BMVW1-t	MT20	4.0	4.0	2.00	1.50
M BMV+p	MT20	3.0	4.0		
N					
O BMVW-t	MT20	4.0	4.0		
P BMVW-t	MT20	4.0	4.0		
Q BVMVWWVW-t	MT20	6.0	10.0	3.00	3.50
R BMV+p	MT20	3.0	4.0		
S BMVWW-t	MT20	5.0	8.0	2.50	1.50
T BMVW-t	MT20	5.0	6.0	2.50	2.00
U BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2215409

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION		GROSS REACTION			
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
JT	2304	0	2304	0	0	5-8
U	2311	0	2311	0	0	5-8
L						

UNFACTORED REACTIONS

	1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
JT	1612	1155 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0	
U	1617	1159 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	
L								

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 34	-112.4 -112.4	0.10 (1)	10.00	T-C	-292 / 23	0.08 (1)	
B-C	-3169 / 0	-112.4 -112.4	0.40 (1)	4.41	C-S	-551 / 0	0.53 (1)	
C-D	-2727 / 0	-112.4 -112.4	0.37 (1)	4.71	S-D	-247 / 0	0.22 (1)	
D-E	-3075 / 0	-112.4 -112.4	0.25 (1)	4.61	S-Q	0 / 2447	0.39 (1)	
E-F	-3084 / 0	-112.4 -112.4	0.31 (1)	4.55	D-Q	0 / 1270	0.29 (1)	
F-G	-3160 / 0	-112.4 -112.4	0.29 (1)	4.54	Q-F	0 / 385	0.09 (1)	
G-H	-4026 / 0	-112.4 -112.4	0.22 (1)	4.17	P-F	0 / 624	0.14 (1)	
H-I	-4985 / 0	-112.4 -112.4	0.33 (1)	3.69	P-G	-966 / 0	0.64 (1)	
I-J	-4615 / 0	-112.4 -112.4	0.25 (1)	3.90	O-G	0 / 362	0.08 (1)	
J-K	0 / 34	-112.4 -112.4	0.10 (1)	10.00	B-T	0 / 2893	0.65 (1)	
U-B	-2255 / 0	0.0	0.0	15 (1)	6.82	N-L	-333 / 0	0.05 (1)
L-J	-2110 / 0	0.0	0.0	0.21 (1)	5.83	N-J	0 / 4046	0.65 (1)
U-T	0 / 0	-18.5	-18.5	0.15 (4)	10.00	O-H	-467 / 0	0.10 (1)
T-S	0 / 2965	-18.5	-18.5	0.53 (1)	10.00	H-N	0 / 613	0.14 (1)
S-R	0 / 48	-18.5	-18.5	0.10 (4)	10.00			
R-Q	0 / 25	0.0	0.0	0.07 (1)	10.00			
Q-E	-642 / 0	0.0	0.0	0.34 (1)	7.81			
Q-P	0 / 2818	-18.5	-18.5	0.53 (1)	10.00			
P-O	0 / 3628	-18.5	-18.5	0.66 (1)	10.00			
O-N	0 / 4009	-18.5	-18.5	0.72 (1)	10.00			
M-N	0 / 15	0.0	0.0	0.38 (1)	10.00			
N-I	-46 / 16	0.0	0.0	0.29 (1)	7.81			
M-L	0 / 276	-18.5	-18.5	0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	32.5	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/100.12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
 ALLOWABLE DEFL.(TL)= L/360 (1.10")
 CALCULATED VERT. DEFL.(TL)= L/999 (0.35")

CSI: TC=0.40/1.00 (B-C-1), BC=0.72/1.00 (N-O-1), WB=0.65/1.00 (B-T-1), SSI=0.38/1.00 (I-N-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.92 (J) (INPUT = 1.00)

REVIEWED



Structural component only
DWG# T-2215409

REVIEWED
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T53S	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:c3iyj23uDijg_8ovRKbkZpy75XW-ZAqol9BV4xCaD4wi7yJnbHb73iFrBIX1JEm8paz31bs

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



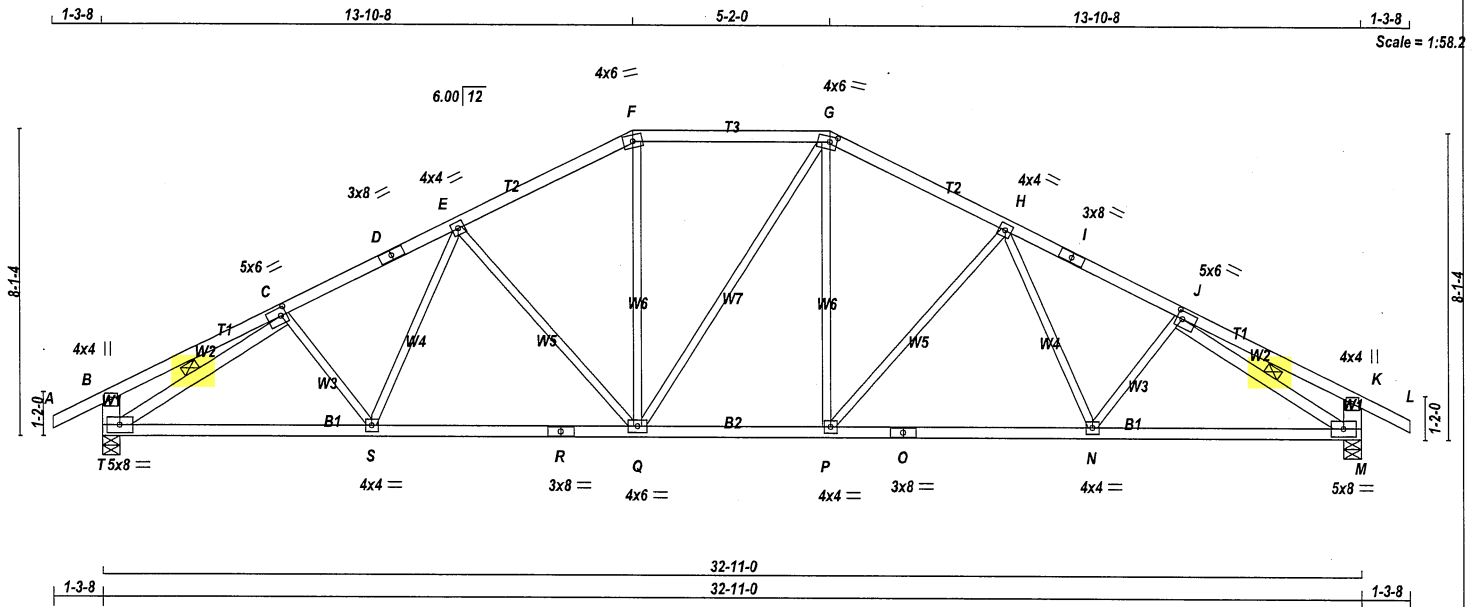
Structural component only
DWG# T-2215409

REVIEWED

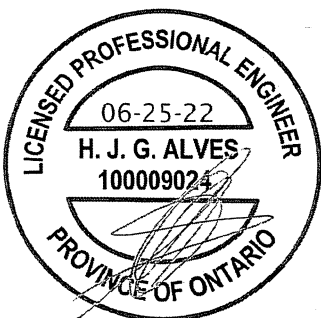
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	T54	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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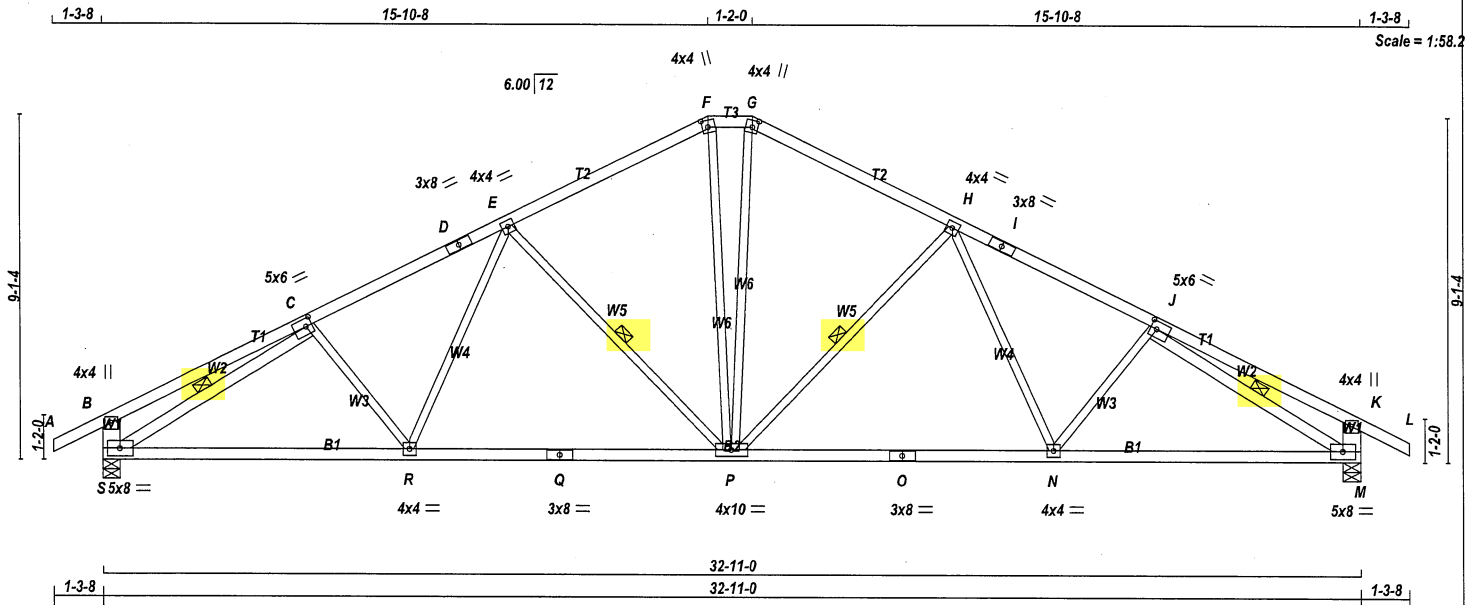


LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED				TOP CH. LL = 32.5 PSF			
A - D	2x4	DRY	No.2	GROSS REACTION				DL = 6.0 PSF			
D - F	2x4	DRY	No.2	MAXIMUM FACTORED				BOT CH. LL = 0.0 PSF			
F - G	2x4	DRY	No.2	DOWN				DL = 7.4 PSF			
G - I	2x4	DRY	No.2	UP				TOTAL LOAD = 45.9 PSF			
I - L	2x4	DRY	No.2	UNFACTORED REACTIONS				SPACING = 24.0 IN. C/C			
T - B	2x6	DRY	No.2	1ST LCASE				LOADING IN FLAT SECTION BASED ON A SLOPE			
M - K	2x6	DRY	No.2	MAX./MIN. COMPONENT REACTIONS				OF 6.00/12			
T - R	2x4	DRY	No.2	JT COMBINED				THIS TRUSS IS DESIGNED FOR RESIDENTIAL			
R - O	2x4	DRY	No.2	T				OR SMALL BUILDING REQUIREMENTS OF PART			
O - M	2x4	DRY	No.2	M				9, NBCC 2015			
ALL WEBS	2x3	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M				THIS DESIGN COMPLIES WITH:			
EXCEPT				BRACING				- PART 9 OF CBC 2018, ABC 2019			
T - C	2x4	DRY	No.2	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.				- PART 9 OF OBC 2012 (2019 AMENDMENT)			
J - M	2x4	DRY	No.2	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				- CSA 086-14			
DRY: SEASONED LUMBER.				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				- TPIC 2014			
PLATES (table is in inches)				1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-T, J-M.				(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD			
JT TYPE	PLATES	W	LEN Y X	END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				ALLOWABLE DEFL.(LL)= L/360 (1.10")			
B TMV+p	MT20	4.0	4.0	LOADING				CALCULATED VERT. DEFL.(LL)= L/ 999 (0.14")			
C TMWW-t	MT20	5.0	6.0	TOTAL LOAD CASES: (4)				ALLOWABLE DEFL.(TL)= L/360 (1.10")			
D TS-t	MT20	3.0	8.0	CHORDS				CALCULATED VERT. DEFL.(TL)= L/ 999 (0.27")			
E TMWW-t	MT20	4.0	4.0	MAX. FACTORED				CSI: TC=0.45/1.00 (F-G:1), BC=0.55/1.00 (S-T:1),			
F TTW-m	MT20	4.0	6.0	FORCE				WB=0.68/1.00 (H-P:1), SSI=0.23/1.00 (F-G:1)			
G TTWW-m	MT20	4.0	6.0	VERT. LOAD				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
H TMWW-t	MT20	4.0	4.0	LC1				COMP=1.10 SHEAR=1.10 TENS=1.10			
I TS-t	MT20	3.0	8.0	MAX				COMPANION LIVE LOAD FACTOR = 1.00			
J TMWW-t	MT20	5.0	6.0	UNBRAC				AUTOSOLVE HEELS OFF			
K TMV+p	MT20	4.0	4.0	LENGTH				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT			
M BMVW-t	MT20	5.0	8.0	FR-TO				NAIL VALUES			
N P-S				FROM				PLATE GRIP(DRY) SHEAR SECTION			
O BMVW-t	MT20	4.0	4.0	TO				(PSI) (PLI) (PLI)			
P BS-t	MT20	3.0	8.0	MAX. FACTORED				MAX MIN MAX MIN MAX MIN			
Q BMVW-t	MT20	4.0	6.0	FORCE				MT20 650 371 1747 798 1987 1873			
R BS-t	MT20	3.0	8.0	VERT. LOAD				PLATE PLACEMENT TOL. = 0.250 inches			
T BMVW-t	MT20	5.0	8.0	LC1				PLATE ROTATION TOL. = 5.0 Deg.			
NOTES- (1)				MAX				JSI GRIP= 0.85 (M) (INPUT = 0.90)			
1) Lateral braces to be a minimum of 2X4 SPF #2.				UNBRAC				JSI METAL= 0.84 (O) (INPUT = 1.00)			



Structural component only
DWG# T-2215410

REVIEWED



TOTAL WEIGHT = 144 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY			
N. L. G. A. RULES				BUILDING DESIGNER			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS			
A - D	2x4	DRY	No.2	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
D - F	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG
F - G	2x4	DRY	No.2	JT VERT	DOWN	HORZ	UPLIFT
G - I	2x4	DRY	No.2	S 2307	0	2307	0
I - L	2x4	DRY	No.2	M 2307	0	2307	0
S - B	2x6	DRY	No.2				
M - K	2x6	DRY	No.2				
S - Q	2x4	DRY	No.2				
Q - O	2x4	DRY	No.2				
O - M	2x4	DRY	No.2				
ALL WEBS	2x3	DRY	No.2				
EXCEPT							
S - C	2x4	DRY	No.2				
J - M	2x4	DRY	No.2				

DRY: SEASONED LUMBER.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

PLATES (table is in inches)				UNFACTORED REACTIONS			
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMV+p	MT20	4.0	4.0			
C	TMWV-t	MT20	5.0	6.0	2.50	2.00	
D	TS-t	MT20	3.0	8.0			
E	TMWV-t	MT20	4.0	4.0			
F	TTW+m	MT20	4.0	4.0	2.25	1.75	
G	TTW+m	MT20	4.0	4.0	2.25	1.75	
H	TMWV-t	MT20	4.0	4.0			
I	TS-t	MT20	3.0	8.0			
J	TMWV-t	MT20	5.0	6.0	2.50	2.00	
K	TMV+p	MT20	4.0	4.0			
M	BMWV-t	MT20	5.0	8.0			
N	BMWV-t	MT20	4.0	4.0			
O	BS-t	MT20	3.0	8.0			
P	BMWVWV-t	MT20	4.0	10.0			
Q	BS-t	MT20	3.0	8.0			
R	BMWV-t	MT20	4.0	4.0			
S	BMWV-t	MT20	5.0	8.0			

LOADING				CHORDS			
TOTAL LOAD CASES: (4)				MAX. FACTORED			
				MEMB. FORCE (LBS)			
				FR-TO			
				A-B	0 / 34	-112.4	-112.4
				B-C	0 / 24	-112.4	-112.4
				C-D	-3001 / 0	-112.4	-112.4
				D-E	-3001 / 0	-112.4	-112.4
				E-F	-2239 / 0	-112.4	-112.4
				F-G	-2024 / 0	-112.4	-112.4
				G-H	-2239 / 0	-112.4	-112.4
				H-I	-3001 / 0	-112.4	-112.4
				I-J	-3001 / 0	-112.4	-112.4
				J-K	0 / 24	-112.4	-112.4
				K-L	0 / 34	-112.4	-112.4
				S-B	-397 / 0	0.0	0.0
				M-K	-397 / 0	0.0	0.0

NOTES-				WEBS			
(1)				MAX. FACTORED			
1) Lateral braces to be a minimum of 2X4 SPF #2.				MEMB. FORCE (LBS)			
				FR-TO			
				C-R	-182 / 27	0.06	(1)
				R-E	0 / 336	0.08	(1)
				E-P	-842 / 0	0.41	(1)
				P-H	-842 / 0	0.41	(1)
				H-N	0 / 336	0.08	(1)
				N-J	-182 / 27	0.06	(1)
				S-C	-3353 / 0	0.65	(1)
				J-M	-3353 / 0	0.65	(1)
				F-P	0 / 693	0.16	(1)
				P-G	0 / 693	0.16	(1)



Structural component only

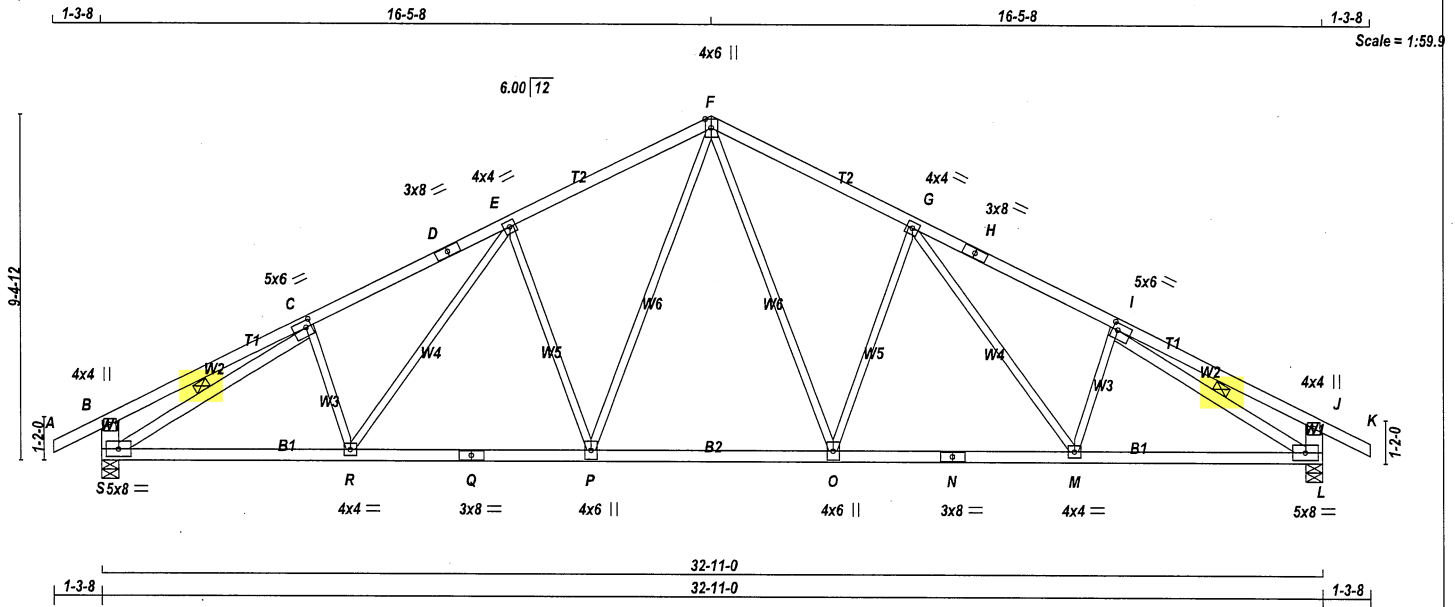
DWG# T-2215411

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T56	2	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 143 = 287 lb
(M/F)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
S - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	VERT	HORZ	BRG	BRG
S	2307	0	2307	0	5-8
L	2307	0	2307	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
L	1615	1157 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, I.L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 34	F-O	0 / 944
B-C	0 / 25	O-G	-811 / 0
C-D	-3074 / 0	G-M	0 / 400
D-E	-3074 / 0	M-I	-212 / 22
E-F	-2560 / 0	P-F	0 / 944
F-G	-2560 / 0	E-P	-811 / 0
G-H	-3074 / 0	R-E	0 / 400
H-I	-3074 / 0	C-R	-212 / 22
I-J	0 / 25	S-C	-3370 / 0
J-K	0 / 34	I-L	-3370 / 0
S-B	-405 / 0		
L-J	-405 / 0		
S-R	0 / 2818		
R-Q	0 / 2526		
Q-P	0 / 2526		
P-O	0 / 1947		
O-N	0 / 2526		
N-M	0 / 2526		
M-L	0 / 2818		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/999 (0.28")

CSI: TC=0.53/1.00 (C-E:1), BC=0.56/1.00 (R-S:1), WB=0.69/1.00 (G-O:1), SSI=0.26/1.00 (B-C:1)

DOL LUMBER-1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

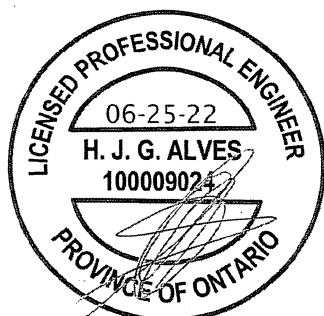
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.81 (Q) (INPUT = 1.00)

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	4.0	4.0	
C	TMVW-t	MT20	5.0	6.0	2.25 1.75
D	TS-t	MT20	3.0	8.0	
E	TMVW-t	MT20	4.0	4.0	
F	TTVW+p	MT20	4.0	6.0	Edge
G	TMVW-t	MT20	4.0	4.0	
H	TS-t	MT20	3.0	8.0	
I	TMVW-t	MT20	5.0	6.0	2.25 1.75
J	TMV+p	MT20	4.0	4.0	
L	BMVW-t	MT20	5.0	8.0	
M	BMVW-t	MT20	4.0	4.0	
N	BS-t	MT20	3.0	8.0	
O	BMVW-t	MT20	4.0	6.0	
P	BMVW-t	MT20	4.0	6.0	
Q	BS-t	MT20	3.0	8.0	
R	BMVW-t	MT20	4.0	4.0	
S	BMVW-t	MT20	5.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)



Structural component only
DWG# T-2215412

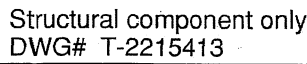
REVIEWED



1

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 43	-112.4 -112.4	0.15 (1)	10.00	R-C	-282 / 15	0.11 (1)
B-C	-2519 / 0	-112.4 -112.4	0.88 (1)	3.36	C-Q	0 / 1695	0.38 (1)
C-D	-3365 / 0	-112.4 -112.4	0.77 (1)	3.09	Q-D	-1020 / 0	0.39 (1)
D-E	-3735 / 0	-112.4 -112.4	0.82 (1)	2.88	D-O	0 / 496	0.11 (1)
E-F	-3735 / 0	-112.4 -112.4	0.82 (1)	2.88	O-E	-559 / 0	0.22 (1)
F-G	-3735 / 0	-112.4 -112.4	0.82 (1)	2.88	O-G	0 / 496	0.11 (1)
G-H	-3365 / 0	-112.4 -112.4	0.77 (1)	3.09	M-G	-1020 / 0	0.39 (1)
H-I	-2519 / 0	-112.4 -112.4	0.88 (1)	3.36	M-H	0 / 1695	0.38 (1)
I-J	0 / 43	-112.4 -112.4	0.15 (1)	10.00	L-H	-282 / 15	0.11 (1)
S-B	-2267 / 0	0.0 0.0	0.15 (1)	6.81	B-R	0 / 2131	0.34 (1)
K-I	-2267 / 0	0.0 0.0	0.15 (1)	6.81	L-I	0 / 2131	0.34 (1)
S-R	0 / 0	-18.5 -18.5	0.14 (4)	10.00			
R-Q	0 / 2087	-18.5 -18.5	0.41 (1)	10.00			
Q-P	0 / 3365	-18.5 -18.5	0.60 (1)	10.00			
P-O	0 / 3365	-18.5 -18.5	0.60 (1)	10.00			
O-N	0 / 3365	-18.5 -18.5	0.60 (1)	10.00			
N-M	0 / 3365	-18.5 -18.5	0.60 (1)	10.00			
M-L	0 / 2087	-18.5 -18.5	0.41 (1)	10.00			
L-K	0 / 0	-18.5 -18.5	0.14 (4)	10.00			

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.97 (P) (INPUT = 1.00)

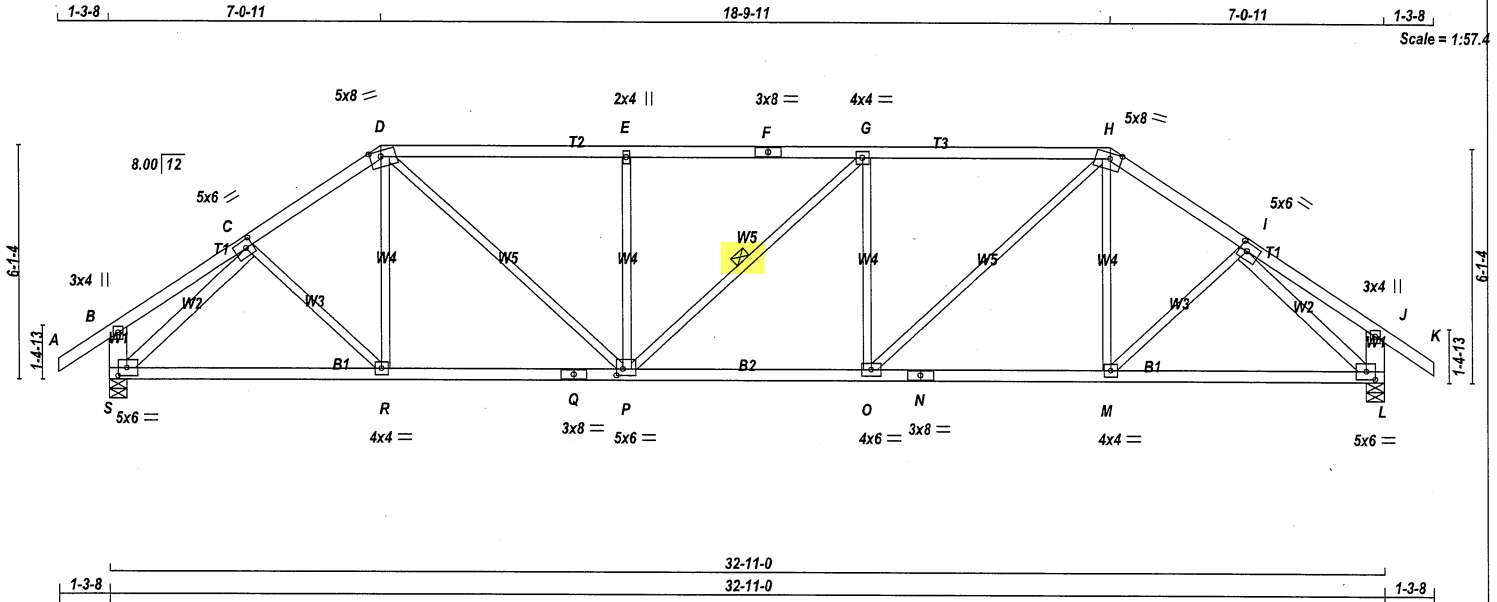


REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	T58	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

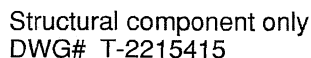
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TOTAL WEIGHT = 150 lb

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.63 (N) (INPUT = 1.00)

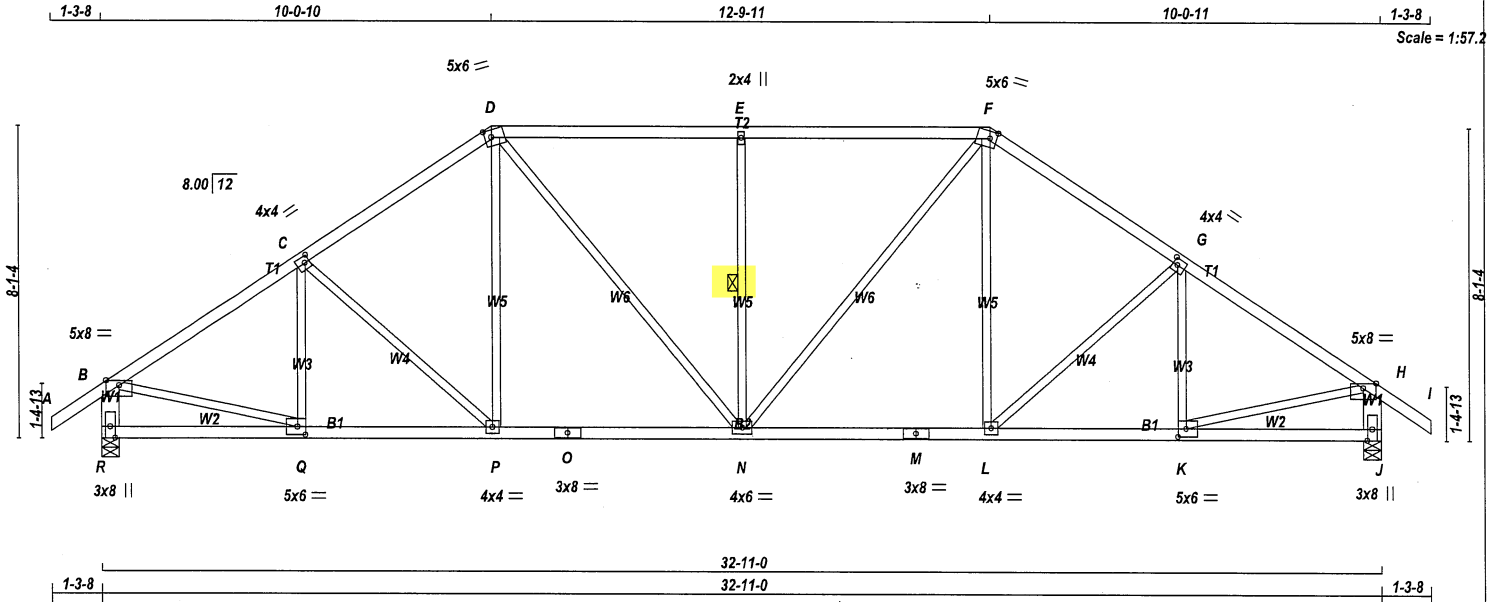


REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	T60	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 147 lb
[M][F]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
O - O	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	8.0	Edge	
K	BMVW-t	MT20	5.0	6.0	2.50	2.50
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-wt	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
R	2309	0	0	2309	0	0	0	5-8	5-8
J	2309	0	0	2309	0	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
J	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1 / 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	CS (LC)
FR-TO		FROM TO	10.00	FR-TO			
A-B	0 / 43	-112.4 -112.4	0.15 (1)	Q-C	-360 / 0	10.00	0.12 (1)
B-C	-2552 / 0	-112.4 -112.4	0.47 (1)	C-P	-296 / 0	10.00	0.25 (1)
C-D	-2364 / 0	-112.4 -112.4	0.45 (1)	P-D	0 / 307	10.00	0.07 (1)
D-E	-2348 / 0	-112.4 -112.4	0.69 (1)	D-N	0 / 648	10.00	0.15 (1)
E-F	-2348 / 0	-112.4 -112.4	0.69 (1)	N-E	-884 / 0	10.00	0.37 (1)
F-G	-2364 / 0	-112.4 -112.4	0.45 (1)	N-F	0 / 648	10.00	0.15 (1)
G-H	-2552 / 0	-112.4 -112.4	0.47 (1)	L-F	0 / 307	10.00	0.07 (1)
H-I	0 / 43	-112.4 -112.4	0.15 (1)	L-G	-296 / 0	10.00	0.25 (1)
R-B	-2267 / 0	0.0 0.0	0.15 (1)	K-G	-360 / 0	10.00	0.12 (1)
J-H	-2267 / 0	0.0 0.0	0.15 (1)	B-Q	0 / 2201	10.00	0.50 (1)
				K-H	0 / 2201	10.00	0.50 (1)
R-Q	0 / 0	-18.5 -18.5	0.10 (4)				
Q-P	0 / 2154	-18.5 -18.5	0.42 (1)				
P-O	0 / 1937	-18.5 -18.5	0.39 (1)				
O-N	0 / 1937	-18.5 -18.5	0.39 (1)				
N-M	0 / 1937	-18.5 -18.5	0.39 (1)				
M-L	0 / 1937	-18.5 -18.5	0.39 (1)				
L-K	0 / 2154	-18.5 -18.5	0.42 (1)				
K-J	0 / 0	-18.5 -18.5	0.10 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.69/1.00 (D-E-1), BC=0.42/1.00 (P-Q-1), WB=0.50/1.00 (H-K-1), SSI=0.35/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

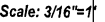
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (Q) (INPUT = 0.90)
JSI METAL= 0.60 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2215416

REVIEWED



REVIEWED

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 163 lb

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.96 (R) (INPUT = 1.00)

Structural component only
DWG# T-2215418

REVIEWED CONTINUED ON PAGE 2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T62	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

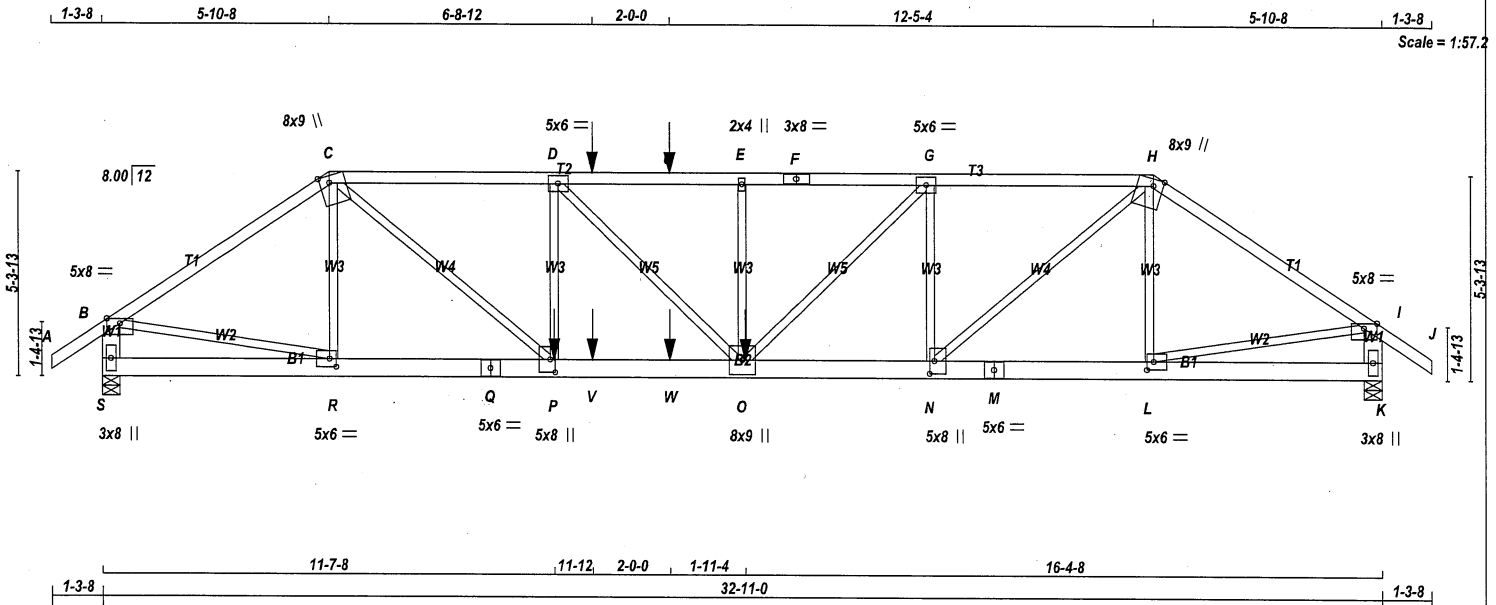


Structural component only
DWG# T-2215418

REVIEWED

JOB NAME 423632	TRUSS NAME T63	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2
M - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS

A-C 1 12 TOP SIDE(0.0)

C-F 1 12 TOP

F-H 1 12 TOP

H-J 1 12 TOP

S-B 2 12 TOP

K-I 2 12 TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS

S-Q 2 12 TOP

M-Q 2 12 SIDE(183.1)

M-K 2 12 TOP

WEBS : (0.122"x3") SPIRAL NAILS

D-P 1 6 SIDE(281.9)

E-O 1 6 SIDE(462.2)

2x3 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
S	4999	0	4999	0	0	5-8	5-8	5-8	5-8
K	4387	0	4387	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX /MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
S	3491	2540 / 0	0 / 0	0 / 0	0 / 0
K	3064	2225 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.43 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)
FR-TO	A-B	0 / 43	-112.4 -112.4 0.08 (1)
	B-C	-6426 / 0	-112.4 -112.4 0.79 (1)
	C-D	-9674 / 0	-112.4 -112.4 0.84 (1)
	D-T	-10503 / 0	-112.4 -112.4 0.80 (1)
	T-U	-10503 / 0	-112.4 -112.4 0.80 (1)
	U-E	-10503 / 0	-112.4 -112.4 0.81 (1)
	E-F	-10503 / 0	-112.4 -112.4 0.81 (1)
	F-G	-10503 / 0	-112.4 -112.4 0.81 (1)
	G-H	-8223 / 0	-112.4 -112.4 0.69 (1)
	H-I	-5549 / 0	-112.4 -112.4 0.69 (1)
	I-J	0 / 43	-112.4 -112.4 0.08 (1)
	S-B	-4948 / 0	0.0 0.0 0.18 (1)
	K-I	-4339 / 0	0.0 0.0 0.16 (1)
	S-R	0 / 0	-18.5 -18.5 0.04 (4)
	R-Q	0 / 5324	-18.5 -18.5 0.36 (1)
	Q-P	0 / 5324	-18.5 -18.5 0.36 (1)
	P-V	0 / 9674	-18.5 -18.5 0.80 (1)
	V-W	0 / 9674	-18.5 -18.5 0.80 (1)
	W-O	0 / 9674	-18.5 -18.5 0.80 (1)
	O-N	0 / 8223	-18.5 -18.5 0.58 (1)
	N-M	0 / 4598	-18.5 -18.5 0.33 (1)
	M-L	0 / 4598	-18.5 -18.5 0.33 (1)
	L-K	0 / 0	-18.5 -18.5 0.04 (4)

SPECIFIED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX.	MAX+	MAX-	TOTAL	TYPE	HEEL	CONN.	HEEL	CONN.
O	16-6-8	-1724	-1724	---	---	---	BACK	VERT	TOTAL	---	C1
P	11-7-8	-1242	-1242	---	---	---	BACK	VERT	TOTAL	---	C1
T	12-7-4	-12	-12	---	---	---	BACK	VERT	TOTAL	---	C1
U	14-7-4	-12	-12	---	---	---	BACK	VERT	TOTAL	---	C1
V	12-7-4	-167	-167	---	---	---	BACK	VERT	TOTAL	---	C1
W	14-7-4	-167	-167	---	---	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")

CALCULATED VERT. DEFL.(LL) = L/999 (0.27")

ALLOWABLE DEFL.(TL) = L/360 (1.10")

CALCULATED VERT. DEFL.(TL) = L/864 (0.46")

CSI: TC=0.84/1.00 (C-D:1), BC=0.80/1.00 (O-P:1), WB=0.71/1.00 (C-P:1), SSI=0.16/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (R) (INPUT = 0.90)

JSI METAL = 0.78 (P) (INPUT = 1.00)



Structural component only
DWG# T-2215419

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T63	1	2	BAYVIEW WELLINGTON	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	8.0	9.0	Edge 3.00	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge 3.00	
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	8.0		
L	BMWW-t	MT20	5.0	6.0	2.50 2.00	
M	BS-t	MT20	5.0	6.0		
N	BMWW+t	MT20	5.0	8.0	4.00 1.50	
O	BMWWW+t	MT20	8.0	9.0		
P	BMWW+t	MT20	5.0	8.0	4.00 1.50	
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50 2.00	
S	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

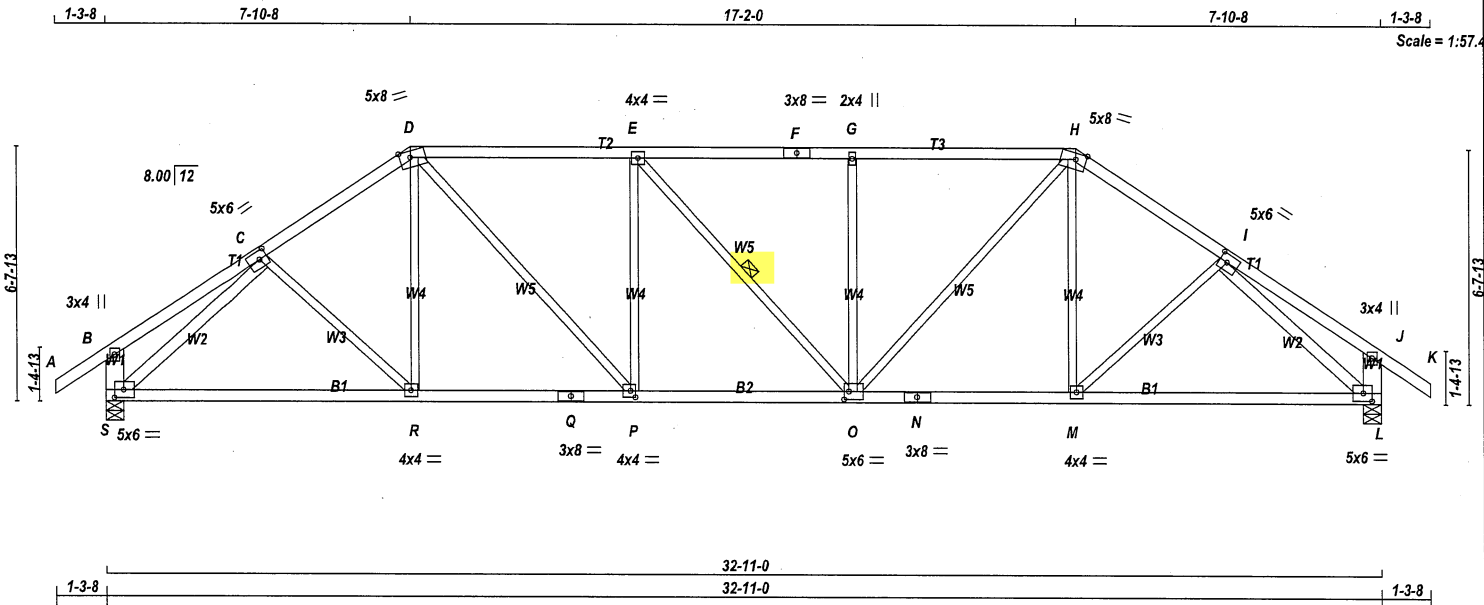


Structural component only
 DWG# T-2215419

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T64	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 146 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
S - B	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
S - Q	2x4	DRY No.2	SPF
Q - N	2x4	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
S - C	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.50
D	TTWV-m	MT20	5.0	8.0	2.00	3.25
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTWV-m	MT20	5.0	8.0	2.00	3.25
I	TMVW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.75
M	BMVW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMVWVW-t	MT20	5.0	6.0	2.50	1.50
P	BMVW-t	MT20	4.0	4.0	2.00	1.50
Q	BS-t	MT20	3.0	8.0		
R	BMVW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0	2.50	2.75

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
S	2309	0	2309	0	0	5-8	5-8	5-8
L	2309	0	2309	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX /MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	SNOW	LIVE	PERM.LIVE	WIND
S	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	458 / 0	0 / 0	0 / 0
L	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	458 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S				
MAX. FACTORED		FACTORED		MEMB.			MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A - B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	C - R	0 / 61	0.02 (4)
B - C	0 / 27	-112.4	-112.4	0.26 (1)	10.00	R - D	0 / 143	0.04 (4)
C - D	-2494 / 0	-112.4	-112.4	0.37 (1)	4.01	D - P	0 / 1064	0.24 (1)
D - E	-2767 / 0	-112.4	-112.4	0.71 (1)	3.43	P - E	-695 / 0	0.51 (1)
E - F	-2765 / 0	-112.4	-112.4	0.71 (1)	3.43	E - O	-3 / 0	0.00 (1)
F - G	-2765 / 0	-112.4	-112.4	0.71 (1)	3.43	O - G	-694 / 0	0.51 (1)
G - H	-2765 / 0	-112.4	-112.4	0.71 (1)	3.44	O - H	0 / 1061	0.24 (1)
H - I	-2494 / 0	-112.4	-112.4	0.37 (1)	4.01	M - H	0 / 145	0.04 (4)
I - J	0 / 27	-112.4	-112.4	0.26 (1)	10.00	M - I	0 / 61	0.02 (4)
J - K	0 / 43	-112.4	-112.4	0.15 (1)	10.00	S - C	-2803 / 0	0.90 (1)
S - B	-327 / 0	0.0	0.0	0.02 (1)	7.81	I - L	-2803 / 0	0.90 (1)
L - J	-327 / 0	0.0	0.0	0.02 (1)	7.81			
S - R	0 / 2041	-18.5	-18.5	0.47 (1)	10.00			
R - Q	0 / 2052	-18.5	-18.5	0.47 (1)	10.00			
Q - P	0 / 2052	-18.5	-18.5	0.47 (1)	10.00			
P - O	0 / 2767	-18.5	-18.5	0.51 (1)	10.00			
O - N	0 / 2052	-18.5	-18.5	0.47 (1)	10.00			
N - M	0 / 2052	-18.5	-18.5	0.47 (1)	10.00			
M - L	0 / 2041	-18.5	-18.5	0.47 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 45.9 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.71/1.00 (D-E:1), BC=0.51/1.00 (O-P:1), WB=0.90/1.00 (L:1), SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.63 (I) (INPUT = 1.00)



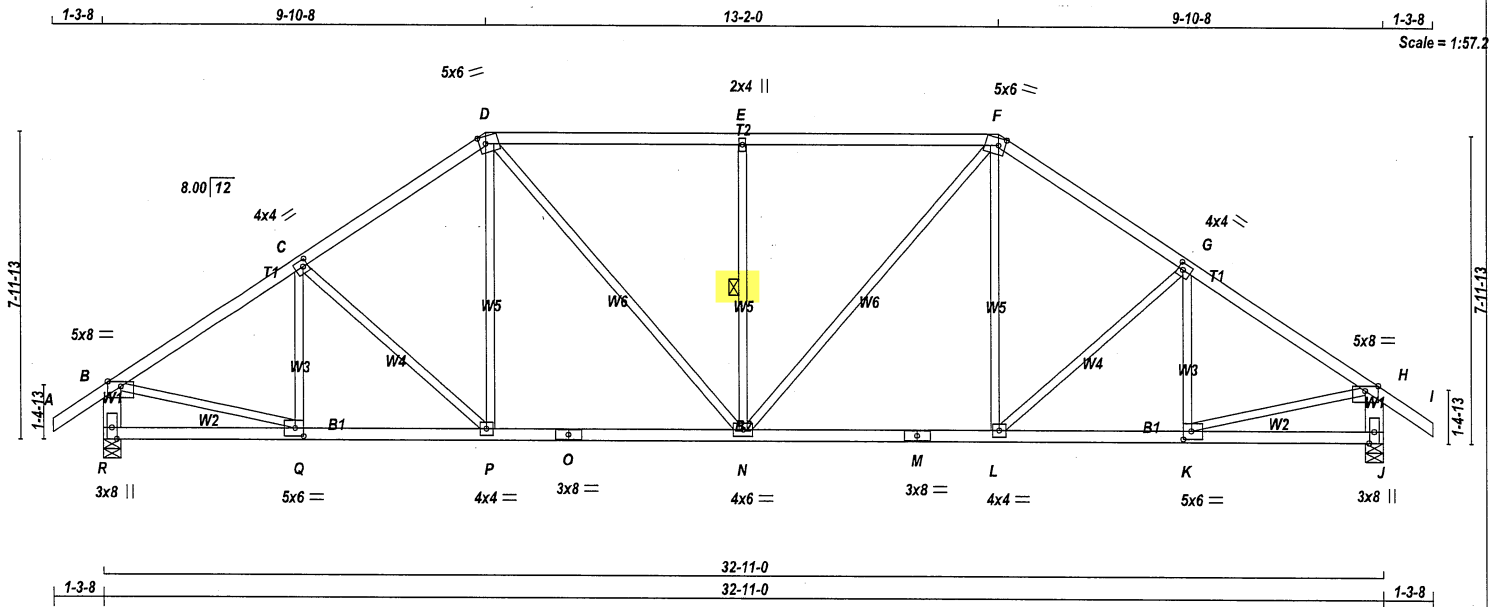
Structural component only
DWG# T-2215420

REVIEWED

JOB NAME 423632	TRUSS NAME T65	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:c3jyj23uDijq_8pvRKbkZpy75XW-kl?y3vKOUKa01nGpGm0MXbYw485AGmjerSxDiRz31bh



TOTAL WEIGHT = 146 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	8.0	Edge	
K	BMVW-t	MT20	5.0	6.0	2.50	2.50
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-w-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
R	2309	0	0	2309	0	0	5-8	5-8	
J	2309	0	0	2309	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
J	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH FR-TO
FR-TO				FR-TO			
A-B	0 / 43	-112.4	-112.4 0.15 (1)	Q-C	-370 / 0	0.12 (1)	10.00
B-C	-2548 / 0	-112.4	-112.4 0.45 (1)	C-P	-276 / 0	0.22 (1)	3.91
C-D	-2376 / 0	-112.4	-112.4 0.43 (1)	P-D	0 / 296	0.07 (1)	4.05
D-E	-2390 / 0	-112.4	-112.4 0.73 (1)	D-N	0 / 679	0.15 (1)	3.52
E-F	-2390 / 0	-112.4	-112.4 0.73 (1)	N-E	-909 / 0	0.37 (1)	3.52
F-G	-2376 / 0	-112.4	-112.4 0.43 (1)	N-F	0 / 679	0.15 (1)	4.05
G-H	-2548 / 0	-112.4	-112.4 0.45 (1)	L-F	0 / 296	0.07 (1)	3.91
H-I	0 / 43	-112.4	-112.4 0.15 (1)	L-G	-276 / 0	0.22 (1)	10.00
R-B	-2267 / 0	0.0	0.0 0.15 (1)	K-G	-370 / 0	0.12 (1)	6.81
J-H	-2267 / 0	0.0	0.0 0.15 (1)	B-Q	0 / 2198	0.49 (1)	6.81
R-Q	0 / 0	-18.5	-18.5 0.10 (4)	K-H	0 / 2198	0.49 (1)	10.00
Q-P	0 / 2149	-18.5	-18.5 0.42 (1)				10.00
P-O	0 / 1948	-18.5	-18.5 0.39 (1)				10.00
O-N	0 / 1948	-18.5	-18.5 0.39 (1)				10.00
N-M	0 / 1948	-18.5	-18.5 0.39 (1)				10.00
M-L	0 / 1948	-18.5	-18.5 0.39 (1)				10.00
L-K	0 / 2149	-18.5	-18.5 0.42 (1)				10.00
K-J	0 / 0	-18.5	-18.5 0.10 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.73/1.00 (D-E-1), BC=0.42/1.00 (P-Q-1), WB=0.49/1.00 (B-Q-1), SSI=0.36/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.62 (O) (INPUT = 1.00)

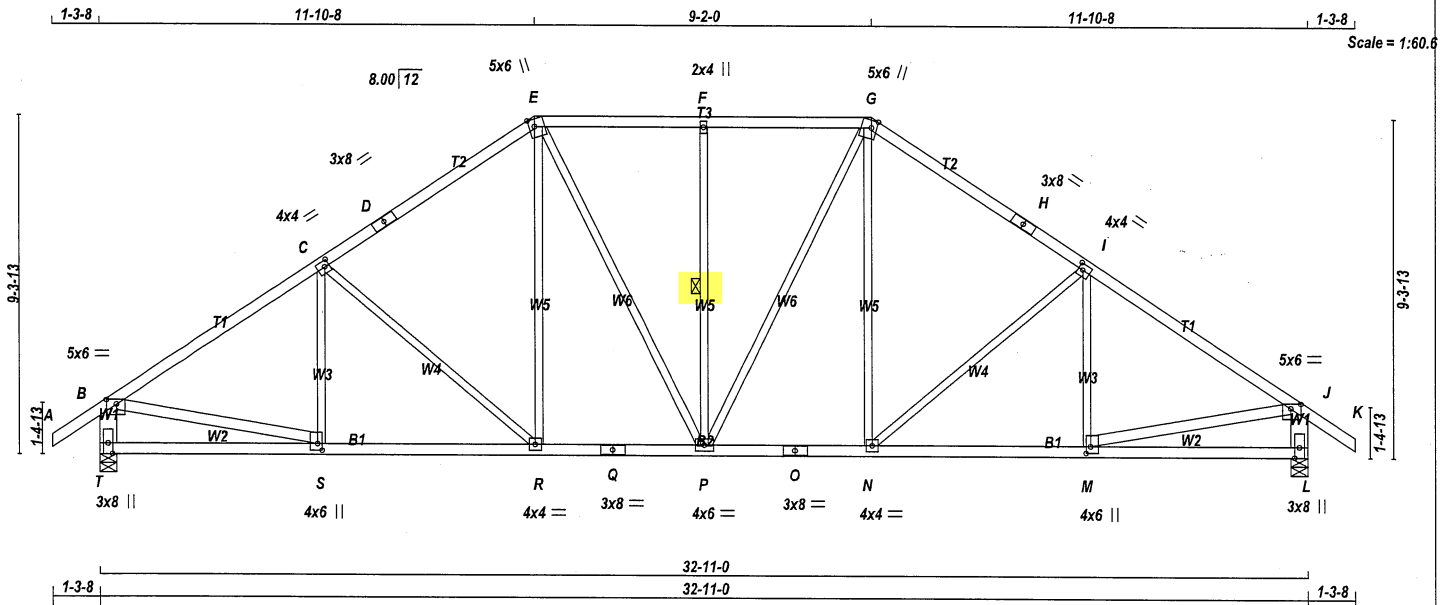


Structural component only
DWG# T-2215421

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T66	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Fri Jun 24 16:07:47 2022 Page 1
ID:c3jy23uDijq_8pvRKbkZpy75XW-CUYKGFL1Fdftwr0qUXb4p56iYQI?B3o36hnEzt31bg



TOTAL WEIGHT = 158 lb
[M/F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
T - Q	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
B - S	2x4	DRY	No.2
M - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.50	3.25
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	8.0		
E TTWW+m	MT20	5.0	6.0	Edge	1.75
F TMW+w	MT20	2.0	4.0		
G TTWW+m	MT20	5.0	6.0	Edge	1.75
H TS-t	MT20	3.0	8.0		
I TMVW-t	MT20	4.0	4.0	2.00	1.50
J TMVW-p	MT20	5.0	6.0	1.50	3.25
L BMV1+p	MT20	3.0	8.0	Edge	
M BMVW-t	MT20	4.0	6.0	2.25	1.50
N BMVW-t	MT20	4.0	4.0		
O BS-t	MT20	3.0	8.0		
P BMVW-t	MT20	4.0	6.0		
Q BS-t	MT20	3.0	8.0		
R BMVW-t	MT20	4.0	4.0		
S BMVW-t	MT20	4.0	6.0	2.25	1.50
T BMV1+p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
T	2309	0	2309	0	0	5-8	5-8	
L	2309	0	2309	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. SNOW		LIVE		PERM.LIVE		WIND		DEAD		SOIL	
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL							
T	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0							
L	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 43	-112.4	-112.4 0.15 (1)	10.00	S-C	-269 / 30	0.12 (1)
B-C	-2583 / 0	-112.4	-112.4 0.68 (1)	3.60	C-R	-499 / 0	0.62 (1)
C-D	-2230 / 0	-112.4	-112.4 0.62 (1)	3.89	R-E	0 / 418	0.09 (1)
D-E	-2230 / 0	-112.4	-112.4 0.62 (1)	3.89	E-P	0 / 391	0.09 (1)
E-F	-1997 / 0	-112.4	-112.4 0.34 (1)	4.44	P-F	-623 / 0	0.36 (1)
F-G	-1997 / 0	-112.4	-112.4 0.34 (1)	4.44	P-G	0 / 391	0.09 (1)
G-H	-2230 / 0	-112.4	-112.4 0.62 (1)	3.89	N-G	0 / 418	0.09 (1)
H-I	-2230 / 0	-112.4	-112.4 0.62 (1)	3.89	N-I	-499 / 0	0.62 (1)
I-J	-2583 / 0	-112.4	-112.4 0.68 (1)	3.60	M-I	-269 / 30	0.12 (1)
J-K	0 / 43	-112.4	-112.4 0.15 (1)	10.00	B-S	0 / 2221	0.36 (1)
T-B	-2262 / 0	0.0	0.0 0.15 (1)	6.81	M-J	0 / 2221	0.36 (1)
L-J	-2262 / 0	0.0	0.0 0.15 (1)	6.81			
T-S	0 / 0	-18.5	-18.5 0.16 (4)	10.00			
S-R	0 / 2186	-18.5	-18.5 0.42 (1)	10.00			
R-Q	0 / 1820	-18.5	-18.5 0.35 (1)	10.00			
Q-P	0 / 1820	-18.5	-18.5 0.35 (1)	10.00			
P-O	0 / 1820	-18.5	-18.5 0.35 (1)	10.00			
O-N	0 / 1820	-18.5	-18.5 0.35 (1)	10.00			
N-M	0 / 2186	-18.5	-18.5 0.42 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.68/1.00 (I-J:1), BC=0.42/1.00 (M-N:1), WB=0.62/1.00 (I-N:1), SSI=0.27/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	798	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (S) (INPUT = 0.90)
JSI METAL= 0.96 (S) (INPUT = 1.00)



Structural component only
DWG# T-2215422

REVIEWED

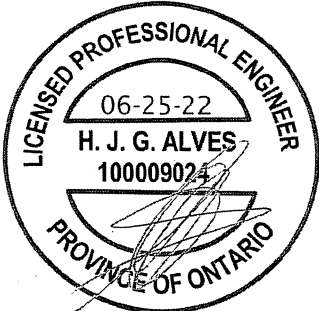
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T66	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2215422

REVIEWED

Tamarack Roof Truss, Burlington



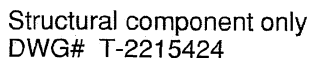
TOTAL WEIGHT = 73 lb

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

L-K	0 / 887	-18.5	-18.5	0.41 (4)	10.00
K-J	0 / 887	-18.5	-18.5	0.41 (4)	10.00
J-I	0 / 887	-18.5	-18.5	0.41 (4)	10.00

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL= 0.39 (F) (INPUT = 1.00)



REVIEWED

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Fri Jun 24 16:07:50 2022 Page 1
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TOTAL WEIGHT = 3 X 72 = 217 lb

DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 43	-112.4 -112.4	0.15 (1)	10.00	D-I	0 / 391	0.09 (1)
B-C	0 / 29	-112.4 -112.4	0.30 (1)	10.00	I-E	-273 / 0	0.08 (1)
C-D	-1002 / 0	-112.4 -112.4	0.24 (1)	5.96	K-D	0 / 391	0.09 (1)
D-E	-1002 / 0	-112.4 -112.4	0.24 (1)	5.96	C-K	-273 / 0	0.08 (1)
E-F	0 / 29	-112.4 -112.4	0.30 (1)	10.00	L-C	-1241 / 0	0.62 (1)
F-G	0 / 43	-112.4 -112.4	0.15 (1)	10.00	E-H	-1241 / 0	0.62 (1)
L-B	-328 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-328 / 0	0.0	0.0 0.03 (1)	7.81			
L-K	0 / 907	-18.5 -18.5	0.21 (1)	10.00			
K-J	0 / 662	-18.5 -18.5	0.17 (1)	10.00			
J-I	0 / 662	-18.5 -18.5	0.17 (1)	10.00			
I-H	0 / 907	-18.5 -18.5	0.21 (1)	10.00			

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.30/1.00 (B-C:1) , BC=0.21/1.00 (H-I:1) ,
WB=0.62/1.00 (C-L:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)



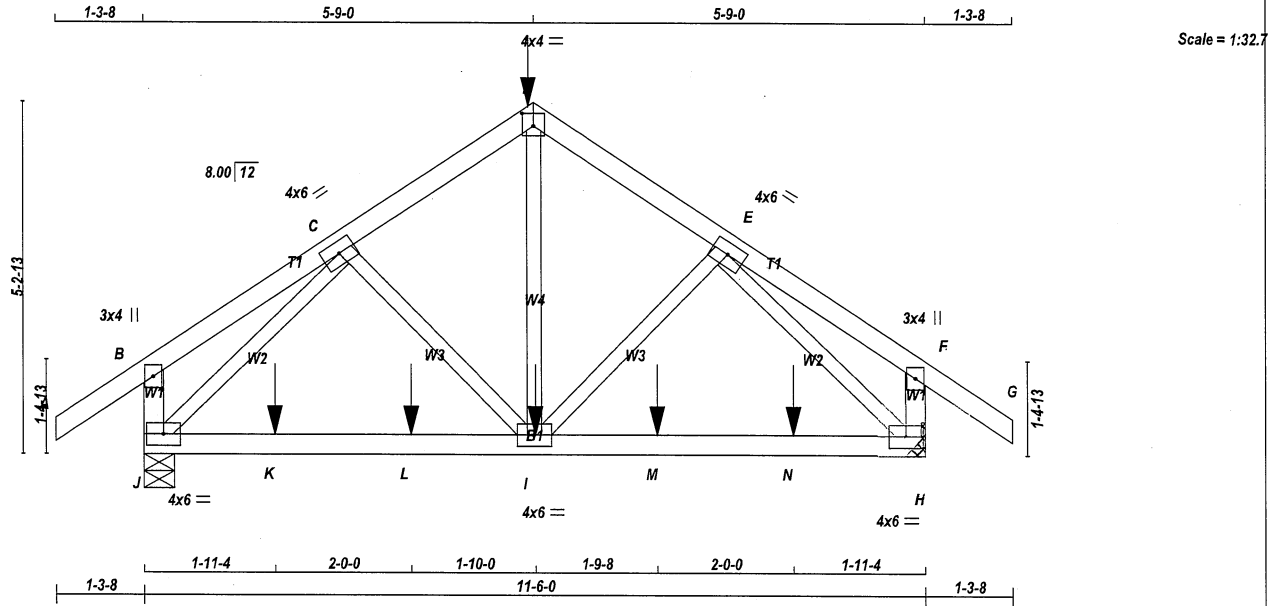
Structural component only
DWG# T-2215425

REVIEWED

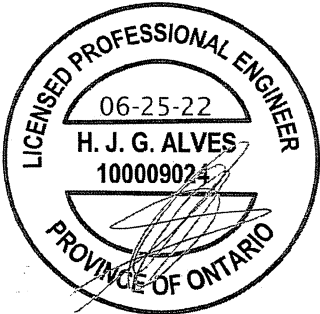
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	T70	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022
MiTek Industries, Inc.
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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - G 2x4 DRY No.2 SPF J - B 2x4 DRY No.2 SPF H - F 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX J 1474 0 1474 0 0 5-8 5-8 H 1474 0 1474 0 0 MECHANICAL A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.38") CALCULATED VERT. DEFL.(LL) = L/999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.38") CALCULATED VERT. DEFL.(TL) = L/999 (0.06") CSI: TC=0.17/1.00 (A-B:1) , BC=0.43/1.00 (H-I:4) , WB=0.45/1.00 (C-J:1) , SSI=0.17/1.00 (I-J:4) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (Psi) (Plf) (Plf) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (E) (INPUT = 0.90) JSI METAL= 0.35 (E) (INPUT = 1.00)			
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMV+p MT20 3.0 4.0 C TMVW-t MT20 4.0 6.0 D TTW-p MT20 4.0 4.0 2.25 2.00 E TMVW-t MT20 4.0 6.0 F TMV+p MT20 3.0 4.0 H BMVW1-t MT20 4.0 6.0 I BMVW1-t MT20 4.0 6.0 J BMVW1-t MT20 4.0 6.0 NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.				UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL J 1032 732 / 0 0 / 0 0 / 0 0 / 0 300 / 0 0 / 0 H 1032 732 / 0 0 / 0 0 / 0 0 / 0 300 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				LOADING TOTAL LOAD CASES: (4) C H O R D S W E B S MAX. FACTORED MAX. FACTORED MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX. MAX. MEMB. MAX. FORCE MAX. (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) FR-TO FROM TO A-B 0 / 43 -112.4 -112.4 0.17 (1) 10.00 I-D 0 / 401 0.10 (1) B-C 0 / 18 -112.4 -112.4 0.14 (1) 10.00 I-E 0 / 47 0.02 (4) C-D -1327 / 0 -112.4 -112.4 0.16 (1) 5.41 C-I 0 / 47 0.02 (4) D-E -1327 / 0 -112.4 -112.4 0.16 (1) 5.41 J-C -1558 / 0 0.45 (1) E-F 0 / 18 -112.4 -112.4 0.14 (1) 10.00 E-H -1558 / 0 0.45 (1) F-G 0 / 43 -112.4 -112.4 0.17 (1) 10.00 J-B -283 / 0 0.0 0.0 0.03 (1) 7.81 H-F -283 / 0 0.0 0.0 0.03 (1) 7.81 J-K 0 / 1078 -18.5 -18.5 0.43 (4) 10.00 K-L 0 / 1078 -18.5 -18.5 0.43 (4) 10.00 L-I 0 / 1078 -18.5 -18.5 0.43 (4) 10.00 I-M 0 / 1078 -18.5 -18.5 0.43 (4) 10.00 M-N 0 / 1078 -18.5 -18.5 0.43 (4) 10.00 N-H 0 / 1078 -18.5 -18.5 0.43 (4) 10.00 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. D 5-9-0 -544 -544 --- BACK VERT TOTAL --- C1 I 5-9-4 -145 -145 --- BACK VERT TOTAL --- C1 K 1-11-4 -27 -27 --- BACK VERT TOTAL --- C1 L 3-11-4 -27 -27 --- BACK VERT TOTAL --- C1 M 7-6-12 -27 -27 --- BACK VERT TOTAL --- C1 N 9-6-12 -27 -27 --- BACK VERT TOTAL --- C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.			



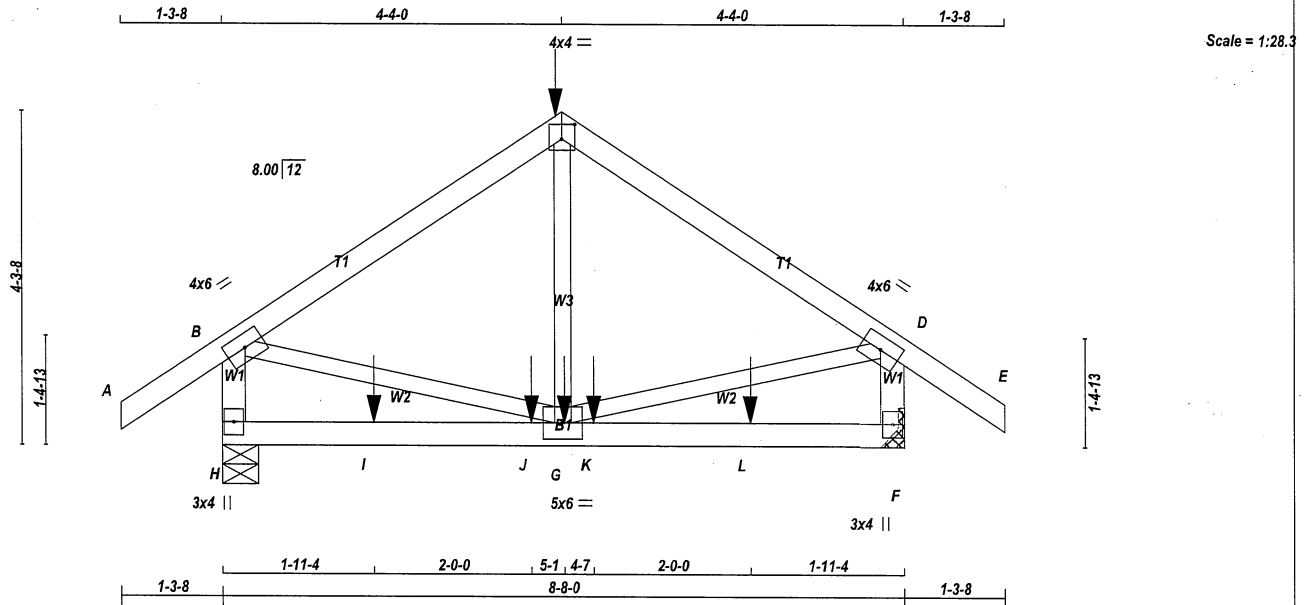
Structural component only
DWG# T-2215426

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	T71	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
H - B	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1223	0	1223	0	0	5-8	5-8
F	1223	0	1223	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	855	620 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0
F	855	620 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (UNBRAC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO						FR-TO		
A-B	0 / 43	-112.4	-112.4	0.17 (1)	10.00	G-C	-240 / 93	0.07 (1)
B-C	-1027 / 0	-112.4	-112.4	0.43 (1)	5.54	B-G	0 / 881	0.22 (1)
C-D	-1027 / 0	-112.4	-112.4	0.43 (1)	5.54	G-D	0 / 881	0.22 (1)
D-E	0 / 43	-112.4	-112.4	0.17 (1)	10.00			
H-B	-1180 / 0	0.0	0.0	0.13 (1)	7.31			
F-D	-1180 / 0	0.0	0.0	0.13 (1)	7.31			
H-I	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
I-J	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
J-G	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
G-K	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
K-L	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
L-F	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-4-0	-614	-614		BACK	VERT	TOTAL	---	C1
G	4-4-4	-17	-17		BACK	VERT	TOTAL	---	C1
I	1-11-4	-19	-19		BACK	VERT	TOTAL	---	C1
J	3-11-4	-17	-17		BACK	VERT	TOTAL	---	C1
K	4-8-12	-17	-17		BACK	VERT	TOTAL	---	C1
L	6-8-12	-19	-19		BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.43/1.00 (C-D:1), BC=0.17/1.00 (G-H:4), WB=0.22/1.00 (B-G:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)



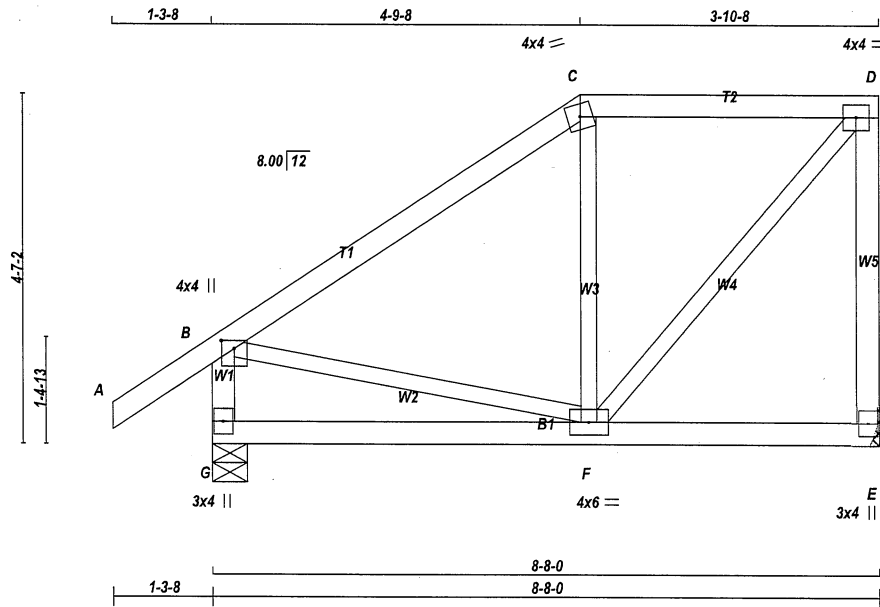
Structural component only
DWG# T-2215427

REVIEWED

JOB NAME 423632	TRUSS NAME T72	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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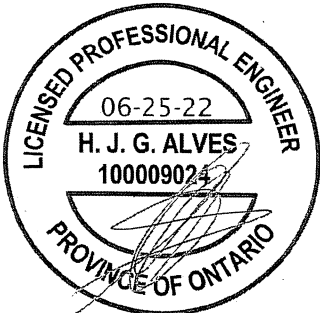


Scale = 1:28.9

LUMBER							TOTAL WEIGHT = 40 lb						
N. L. G. A. RULES							[M][F]						
CHORDS SIZE LUMBER													
A - C 2x4 DRY No.2 SPF													
C - D 2x4 DRY No.2 SPF													
E - D 2x4 DRY No.2 SPF													
G - B 2x4 DRY No.2 SPF													
G - E 2x4 DRY No.2 SPF													
ALL WEBS 2x3 DRY No.2 SPF													
EXCEPT													
DRY: SEASONED LUMBER.													
PLATES (table is in inches)													
JT TYPE PLATES W LEN Y X													
B TMVW+p MT20 4.0 4.0 1.25 2.00													
C TTW-m MT20 4.0 4.0													
D TMVW-t MT20 4.0 4.0													
E BMV1+p MT20 3.0 4.0													
F BMVWW-t MT20 4.0 6.0													
G BMV1+p MT20 3.0 4.0													
NOTES- (1)													
1) Lateral braces to be a minimum of 2X4 SPF #2.													

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED		MAXIMUM FACTORED		INPUT		REQRD			
GROSS REACTION		GROSS REACTION		BRG		BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
E	567	0	567	0	0	MECHANICAL			
G	722	0	722	0	0	5-8	5-8		
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.									
UNFACTORED REACTIONS									
1ST LCASE		MAX /MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
E	398	281 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0		
G	503	371 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS		FACTORED		WEBS		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	F-C	-292 / 0	0.09 (1)	
B-C	-353 / 0	-112.4	-112.4	0.33 (1)	6.25	F-D	0 / 431	0.10 (1)	
C-D	-281 / 0	-112.4	-112.4	0.22 (1)	6.25	B-F	0 / 300	0.07 (1)	
E-D	-544 / 0	0.0	0.0	0.18 (1)	7.81				
G-B	-684 / 0	0.0	0.0	0.07 (1)	7.81				
G-F	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
F-E	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	32.5 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD	=	45.9 PSF	
SPACING = 24.0 IN. C/C			
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12			
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
THIS DESIGN COMPLIES WITH:			
- PART 9 OF CBC 2018, ABC 2019			
- PART 9 OF OBC 2012 (2019 AMENDMENT)			
- CSA 086-14			
- TPIC 2014			
(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD			
ALLOWABLE DEFL.(LL)= L/360 (0.29")			
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")			
ALLOWABLE DEFL.(TL)= L/360 (0.29")			
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")			
CSI: TC=0.33/1.00 (B-C:1), BC=0.10/1.00 (E-F:4), WB=0.10/1.00 (D-F:1), SSI=0.17/1.00 (B-C:1)			
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
COMP=1.10 SHEAR=1.10 TENS= 1.10			
COMPANION LIVE LOAD FACTOR = 1.00			
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873
PLATE PLACEMENT TOL. = 0.250 inches			
PLATE ROTATION TOL. = 5.0 Deg.			
JSI GRIP= 0.51 (F) (INPUT = 0.90)			
JSI METAL= 0.14 (B) (INPUT = 1.00)			



Structural component only
DWG# T-2215428

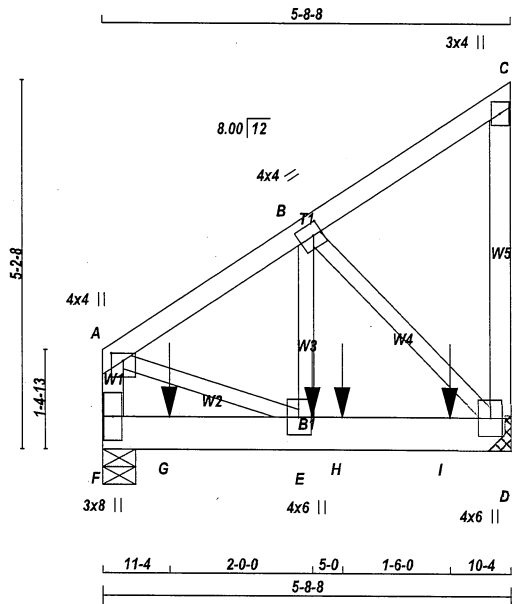
REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	T73	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:c3jyj23uDijq_8pvRKbkZpy75XW-1ewbXJPnrTT0Nr9Ake?K4LHbyWbO?rgS185SXz31ba



Scale = 1:31.7

TOTAL WEIGHT = 2 X 31 = 63 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:				
CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)		
TOP CHORDS : (0.122"x3") SPIRAL NAILS				
F-A	1	12	TOP	
A-C	1	12	TOP	
C-D	1	12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS				
F-D	2	12	SIDE (0.0)	
WEBS : (0.122"x3") SPIRAL NAILS				
2x3	1	6		
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.				
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.				
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.				
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.				
PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0 1.25 2.00
B	TMVW-t	MT20	4.0	4.0 2.00 1.50
C	TMV+p	MT20	3.0	4.0
D	BMVW1+p	MT20	4.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	MAXIMUM FACTORED	INPUT	REQORD					
	GROSS REACTION	GROSS REACTION	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
F	1402	0	1402	0	0	5-8	5-8		
D	1786	0	1786	0	0	MECHANICAL			
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.									
UNFACTORED REACTIONS									
	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
F	974	741 / 0	0 / 0	0 / 0	0 / 0	233 / 0	0 / 0		
D	1244	925 / 0	0 / 0	0 / 0	0 / 0	319 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.
		FORCE	VERT.	PLF				LENGTH	
		(LBS)							
FR-TO			FROM	TO					
F-A	-1245 / 0	0.0	0.0	0.07 (1)	7.81	A-E	0 / 1165	0.14 (1)	
A-B	-1304 / 0	-112.4	-112.4	0.08 (1)	6.25	E-B	0 / 1465	0.18 (1)	
B-C	-18 / 0	-112.4	-112.4	0.07 (1)	6.25	B-D	-1568 / 0	0.21 (1)	
D-C	-128 / 0	0.0	0.0	0.03 (1)	7.81				
F-G	0 / 0	-18.5	-18.5	0.09 (1)	10.00				
G-E	0 / 0	-18.5	-18.5	0.09 (1)	10.00				
E-H	0 / 1100	-18.5	-18.5	0.24 (1)	10.00				
H-I	0 / 1100	-18.5	-18.5	0.24 (1)	10.00				
I-D	0 / 1100	-18.5	-18.5	0.24 (1)	10.00				
SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-11-4	-252	-252	---	TOP	VERT	TOTAL	---	C1
G	11-4	-218	-218	---	TOP	VERT	TOTAL	---	C1
H	3-4-4	-840	-840	---	FRONT	VERT	TOTAL	---	C1
I	4-10-4	-384	-384	---	FRONT	VERT	TOTAL	---	C1
CONNECTION REQUIREMENTS									
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.									

DESIGN CRITERIA				
SPECIFIED LOADS:				
TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	
SPACING = 24.0 IN. C/C				
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015				
THIS DESIGN COMPLIES WITH:				
- PART 9 OF BCBC 2018, ABC 2019				
- PART 9 OF OBC 2012 (2019 AMENDMENT)				
- CSA 086-14				
- TPIC 2014				
(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD				
ALLOWABLE DEFL.(LL)= L/360 (0.19")				
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")				
ALLOWABLE DEFL.(TL)= L/360 (0.19")				
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")				
CSI: TC=0.08/1.00 (A-B:1) , BC=0.24/1.00 (D-E:1) , WB=0.21/1.00 (B-D:1) , SSI=0.42/1.00 (D-E:1)				
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00				
COMP=1.00 SHEAR=1.00 TENS=1.00				
COMPANION LIVE LOAD FACTOR = 1.00				
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .				
NAIL VALUES				
PLATE GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PLI)	(PLI)		
MAX MIN	MAX MIN	MAX MIN		
MT20	650 371	1747 788	1987 1873	
PLATE PLACEMENT TOL. = 0.250 inches				
PLATE ROTATION TOL. = 5.0 Deg.				
JSI GRIP= 0.87 (B) (INPUT = 0.90)				
JSI METAL= 0.25 (D) (INPUT = 1.00)				



Structural component only
DWG# T-2215429

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T73	1	2	BAYVIEW WELLINGTON	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
E	BMWw+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER

06-25-22

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only

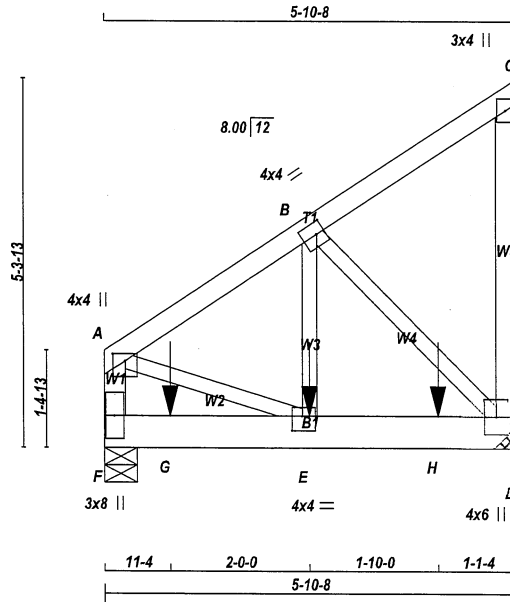
DWG# T-2215429

REVIEWED

JOB NAME 423632	TRUSS NAME T74	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale: 3/8"=1'

TOTAL WEIGHT = 2 X 32 = 64 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A 1	12	TOP
A - C 1	12	TOP
C - D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TMVW-i	MT20	4.0	4.0	2.00 1.50
C	TMV+p	MT20	3.0	4.0	
D	BMVW1+p	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	1109	0	1109	0	5-8
D	1801	0	1801	0	5-8 MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	770	587 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0
D	1257	921 / 0	0 / 0	0 / 0	0 / 0	336 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)	UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO			
F-A	-968 / 0	0.0	0.0 0.05 (1)	A-E	0 / 881	0.11 (1)	7.81
A-B	-982 / 0	-112.4	-112.4 0.08 (1)	E-B	0 / 993	0.12 (1)	6.25
B-C	-20 / 0	-112.4	-112.4 0.08 (1)	B-D	-1183 / 0	0.17 (1)	6.25
D-C	-129 / 0	0.0	0.0 0.03 (1)				7.81
F-G	0 / 0	-18.5	-18.5 0.11 (1)				10.00
G-E	0 / 0	-18.5	-18.5 0.11 (1)				10.00
E-H	0 / 834	-18.5	-18.5 0.39 (1)				10.00
H-D	0 / 834	-18.5	-18.5 0.39 (1)				10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-11-4	-252	-252	---	TOP	VERT	TOTAL	---	C1
G	11-4	-218	-218	---	TOP	VERT	TOTAL	---	C1
H	4-9-4	-1018	-1018	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.08/1.00 (A-B:1), BC=0.39/1.00 (D-E:1), WB=0.17/1.00 (B-D:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (E) (INPUT = 0.90)
JSI METAL= 0.19 (D) (INPUT = 1.00)



Structural component only
DWG# T-2215430

REVIEWED

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	T74	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW-t	MT20	4.0	4.0	2.50	1.75
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER

06-25-22

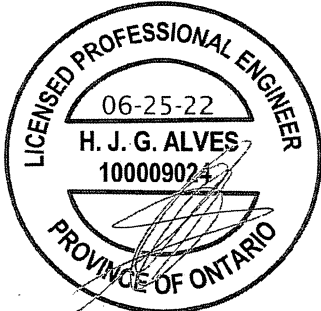
H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only
DWG# T-2215430

REVIEWED



Structural component only
DWG# T-2215430

REVIEWED

REVIEWED CONTINUED ON PAGE 2

JOB NAME 423632	TRUSS NAME T74Z	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWV+t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



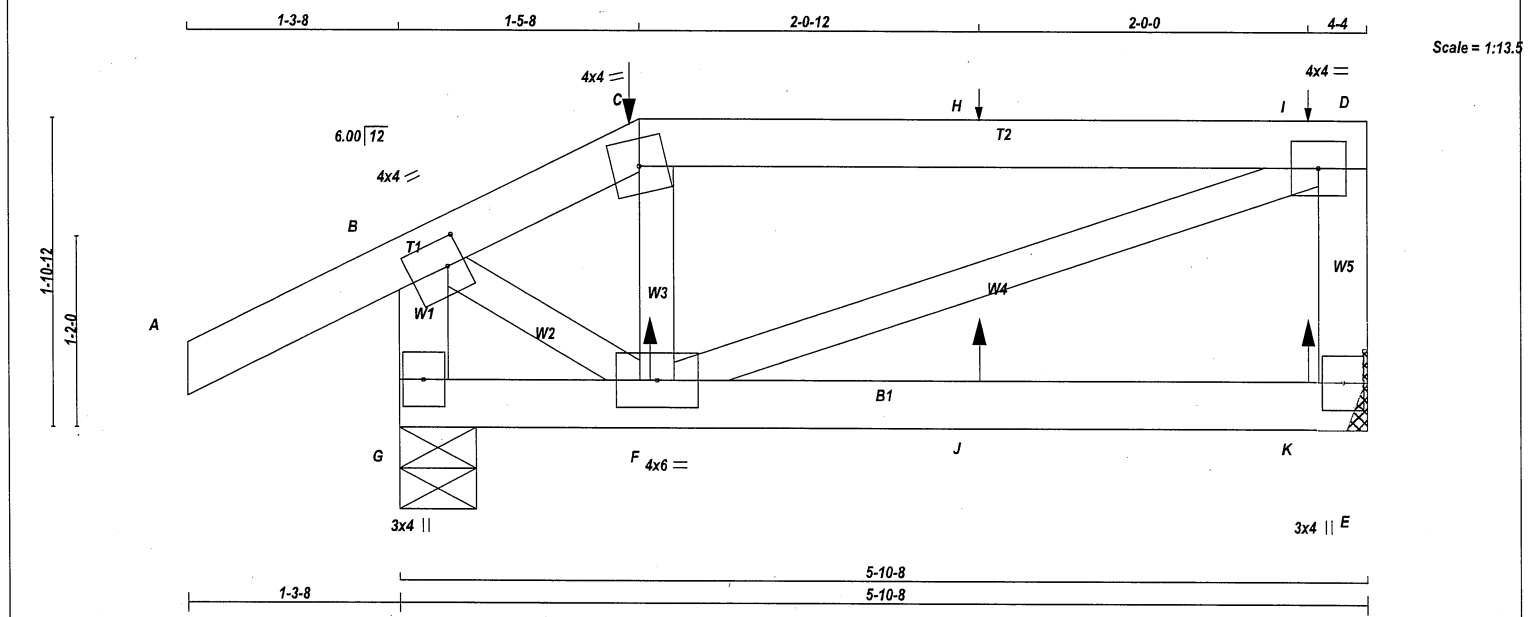
Structural component only
DWG# T-2215431

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	T75	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS 2x3				DRY	No.2
EXCEPT				SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG			
	VERT	HORZ	DOWN	HORZ			IN-SX	IN-SX	
E	344	0	344	0	0	MECHANICAL			
G	542	0	542	0	0	5-8	5-8		
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.									

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE	WIND			
E	242	165 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0	
G	377	280 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO	
A-B	0 / 34	-112.4	-112.4	0.16 (1)	10.00	F-C	-153 / 6	0.02 (1)
B-C	-277 / 0	-112.4	-112.4	0.16 (1)	6.25	F-D	0 / 223	0.06 (1)
C-H	-209 / 0	-112.4	-112.4	0.41 (1)	6.25	B-F	0 / 250	0.06 (1)
H-I	-209 / 0	-112.4	-112.4	0.41 (1)	6.25			
I-D	-209 / 0	-112.4	-112.4	0.41 (1)	6.25			
E-D	-326 / 0	0.0	0.0	0.04 (1)	7.81			
G-B	-536 / 0	0.0	0.0	0.06 (1)	7.81			
G-F	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
F-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
J-K	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
K-E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-5-8	-2	-2	94	FRONT	VERT	TOTAL	---	C1
F	1-6-4	9	1	12	FRONT	VERT	TOTAL	---	C1
H	3-6-4	1	1	94	FRONT	VERT	TOTAL	---	C1
I	5-6-4	1	1	73	FRONT	VERT	TOTAL	---	C1
J	3-6-4	9	1	12	FRONT	VERT	TOTAL	---	C1
K	5-6-4	9	1	12	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.41/1.00 (C-D:1), BC=0.08/1.00 (F-G:4), WB=0.06/1.00 (B-F:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215432

REVIEWED



Structural component only
DWG# T-2215432

REVIEWED

Tamarack Roof Truss, Burlington

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ID:c3iyj23uDiij_8pvRKbkZpy75XW-vP96NgSluizSsTcxPaixUwVvVawPKrIGNf6lzlz31bW



TOTAL WEIGHT = 26 lb

DRY: SEASONED LUMBER.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

BEARINGS

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	C-G	-364 / 0	0.06 (1)	
B-C	-627 / 0	-112.4	-112.4	0.15 (1)	6.25	G-D	-110 / 4	0.02 (1)	
C-D	-339 / 0	-112.4	-112.4	0.10 (1)	6.25	G-E	0 / 365	0.08 (1)	
D-E	-296 / 0	-112.4	-112.4	0.11 (1)	6.25	J-H	-73 / 0	0.01 (1)	
F-E	-348 / 0	0.0	0.0	0.04 (1)	7.81	B-H	0 / 532	0.12 (1)	
J-B	-497 / 0	0.0	0.0	0.05 (1)	7.81				
J-I	0 / 58	-18.5	-18.5	0.02 (1)	10.00				
I-H	0 / 15	0.0	0.0	0.09 (1)	10.00				
H-C	0 / 117	0.0	0.0	0.11 (1)	10.00				
H-G	0 / 661	-18.5	-18.5	0.12 (1)	10.00				
G-F	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

SPECIFIED LOADS:			
TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD		=	45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.12/1.00 (G-H:1) ,
WB=0.12/1.00 (B-H:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

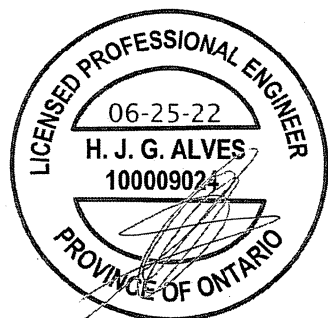
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)



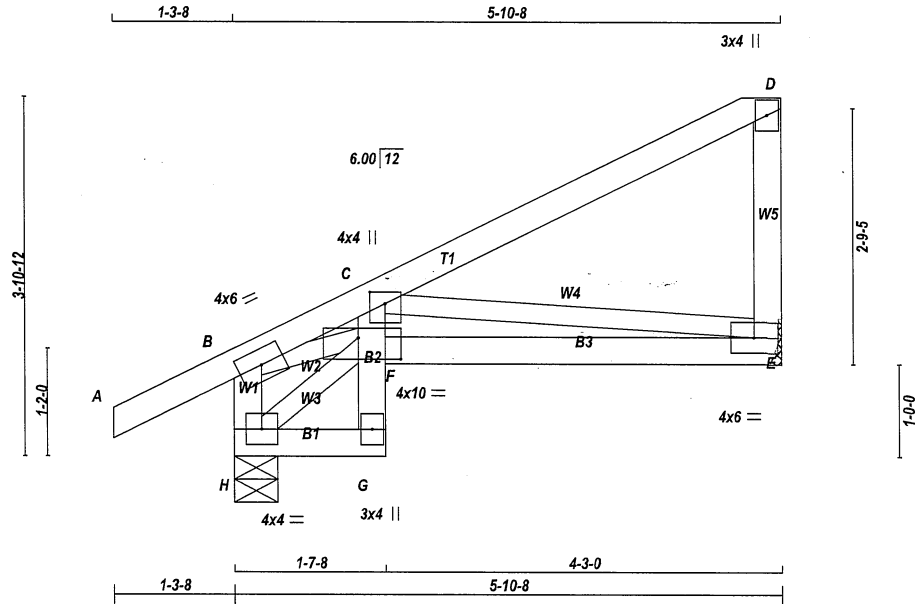
Structural component only
DWG# T-2215433

REVIEWED

JOB NAME 423632	TRUSS NAME T77S	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale: 1/2"=1'

TOTAL WEIGHT = 26 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
E - D	2x4 DRY	No.2	SPF		
H - B	2x4 DRY	No.2	SPF		
H - G	2x4 DRY	No.2	SPF		
G - C	2x4 DRY	No.2	SPF		
F - E	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	6.0		
F	BMVW1-t	MT20	4.0	10.0	2.75	5.50
G	BMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
E	367	0	367	0
H	555	0	555	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
E	257
H	386

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO						FR-TO		
A-B		0 / 34	-112.4	-112.4	0.15 (1)	C-E	-964 / 0	0.29 (1)
B-C		-856 / 0	-112.4	-112.4	0.16 (1)	H-F	-101 / 0	0.01 (1)
C-D		-11 / 0	-112.4	-112.4	0.30 (1)	B-F	0 / 779	0.18 (1)
E-D		-222 / 0	0.0	0.0	0.03 (1)			
H-B		-480 / 0	0.0	0.0	0.05 (1)			
H-G		0 / 80	-18.5	-18.5	0.02 (1)			
G-F		0 / 15	0.0	0.0	0.12 (1)			
F-C		0 / 155	0.0	0.0	0.15 (1)			
F-E		0 / 956	-18.5	-18.5	0.21 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.30/1.00 (C-D:1), BC=0.21/1.00 (E-F:1), WB=0.29/1.00 (C-E:1), SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)

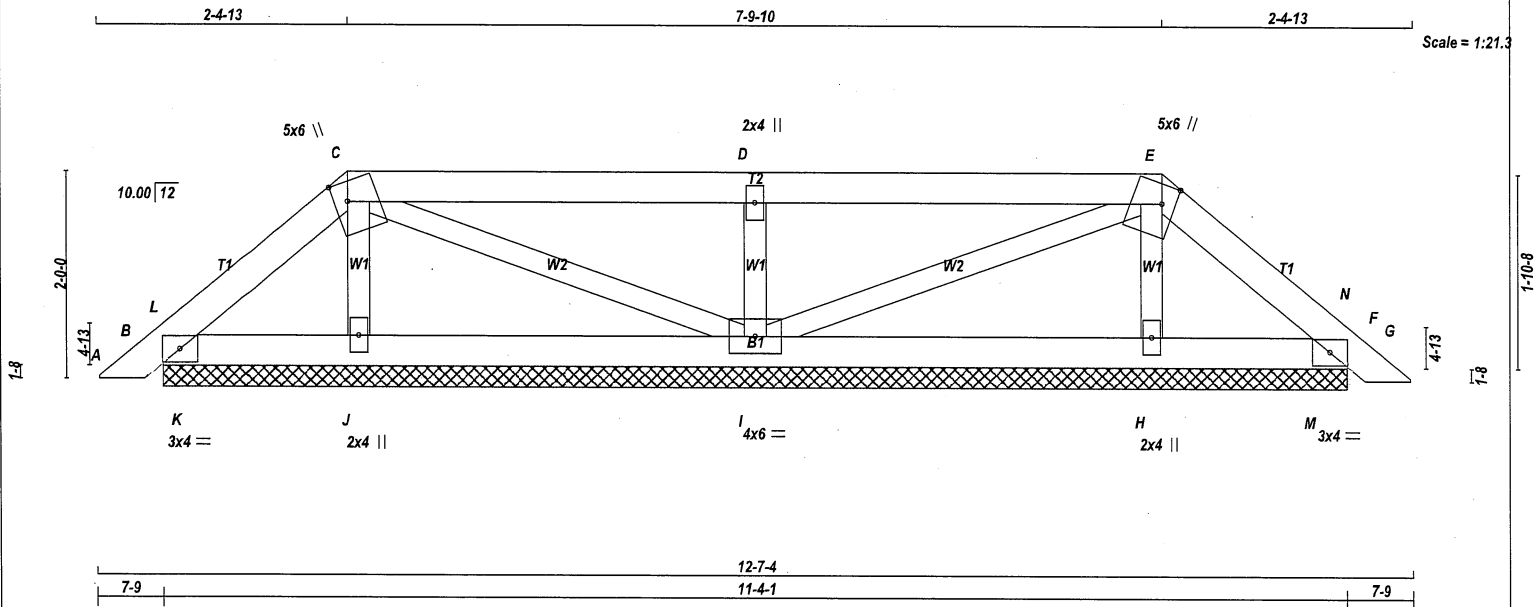


Structural component only
DWG# T-2215434

REVIEWED

JOB NAME 423626	TRUSS NAME PB1	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:c3jy23uDijq_8pvRKbkZpy75XW-Mi7r1Lb4edyqgFnRncU_H4?YkihY3SQwAA?_Dz32P



TOTAL WEIGHT = 2 X 38 = 77 lb

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT. TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		
C TTWW+m	MT20	5.0	6.0	2.25	1.50
D TMW+w	MT20	2.0	4.0		
E TTWW+m	MT20	5.0	6.0	2.25	1.50
F TMB1-I	MT20	3.0	4.0		
H BMW1+w	MT20	2.0	4.0		
I BMWWW1-t	MT20	4.0	6.0		
J BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
B	208	0	208	0	0	11-4-1	11-4-1		
F	208	0	208	0	0	11-4-1	11-4-1		
J	273	0	273	0	0	11-4-1	11-4-1		
I	640	0	640	0	0	11-4-1	11-4-1		
H	273	0	273	0	0	11-4-1	11-4-1		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE			
B	143	115 / 0	0 / 0	0 / 0	28 / 0	0 / 0
F	143	115 / 0	0 / 0	0 / 0	28 / 0	0 / 0
J	194	124 / 0	0 / 0	0 / 0	70 / 0	0 / 0
I	447	327 / 0	0 / 0	0 / 0	120 / 0	0 / 0
H	194	124 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)	MAX CS1 (LC)	MAX CS1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-112.4 -112.4	0.02 (1)	J-C	-191 / 0	0.03 (1)	
B-L	-45 / 0	-112.4 -112.4	0.01 (1)	C-I	-30 / 0	0.01 (1)	
L-C	-69 / 0	-112.4 -112.4	0.04 (1)	I-D	-548 / 0	0.08 (1)	
C-D	-9 / 0	-112.4 -112.4	0.29 (1)	D-E	-30 / 0	0.01 (1)	
D-E	-9 / 0	-112.4 -112.4	0.29 (1)	E-N	-191 / 0	0.03 (1)	
E-N	-69 / 0	-112.4 -112.4	0.04 (1)	N-F	-132 / 0	0.00 (1)	
N-F	-45 / 0	-112.4 -112.4	0.01 (1)	F-G	-132 / 0	0.00 (1)	
F-G	0 / 17	-112.4 -112.4	0.02 (1)				
B-K	0 / 50	-18.5 -18.5	0.05 (1)				
K-J	0 / 50	-18.5 -18.5	0.05 (1)				
J-I	0 / 37	-18.5 -18.5	0.06 (4)				
I-H	0 / 37	-18.5 -18.5	0.06 (4)				
H-M	0 / 50	-18.5 -18.5	0.05 (1)				
M-F	0 / 50	-18.5 -18.5	0.05 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1), BC=0.06/1.00 (H-I:4), WB=0.08/1.00 (D-I:1), SS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)



Structural component only
DWG# T-2215349

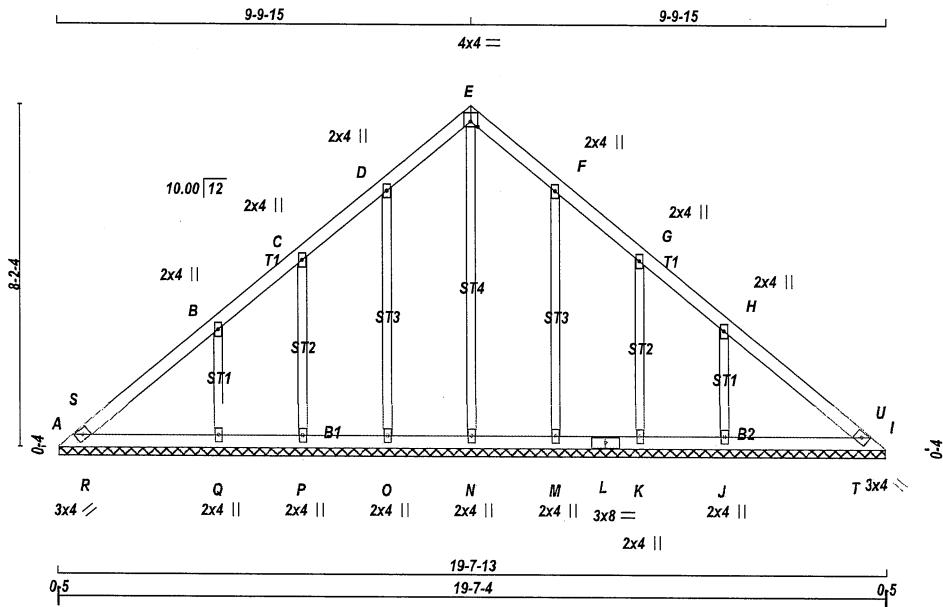
REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423628	V1	1	1	TRUSS DESC.		

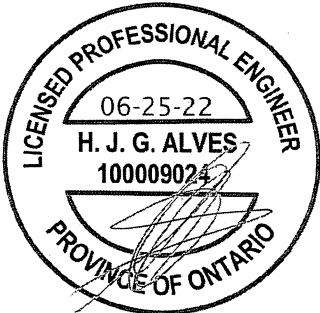
Tamarack Roof Truss, Burlington

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - E 2x4 DRY No.2 SPF E - I 2x4 DRY No.2 SPF A - L 2x4 DRY No.2 SPF L - I 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.				PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TBM1-h MT20 3.0 4.0 B, C, D, F, G, H E TMW+w MT20 2.0 4.0 B TTW-p MT20 4.0 4.0 1.50 2.00 I TBM1-h MT20 3.0 4.0 J, K, M, N, O, P, Q J BMW1+w MT20 2.0 4.0 L BS-t MT20 3.0 8.0				NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.			
DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.15/1.00 (H-U:1), BC=0.13/1.00 (J-T:1), WB=0.35/1.00 (E-N:1), SSI=0.12/1.00 (H-U:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.66 (E) (INPUT = 0.90) JSI METAL= 0.19 (B) (INPUT = 1.00)				BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX A 172 0 172 0 0 19-7-4 (6-7-8) 19-7-4 I 172 0 172 0 0 19-7-4 (6-7-8) 19-7-4 N 311 0 311 0 0 19-7-4 (6-7-8) 19-7-4 O 317 0 317 0 0 19-7-4 (6-7-8) 19-7-4 P 158 0 158 0 0 19-7-4 (6-7-8) 19-7-4 Q 480 0 480 0 0 19-7-4 (6-7-8) 19-7-4 M 317 0 317 0 0 19-7-4 (6-7-8) 19-7-4 K 158 0 158 0 0 19-7-4 (6-7-8) 19-7-4 J 480 0 480 0 0 19-7-4 (6-7-8) 19-7-4 VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH UNFACTORED REACTIONS 1ST LCASE MAX/MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL A 120 88 / 0 0 / 0 0 / 0 32 / 0 0 / 0 I 120 88 / 0 0 / 0 0 / 0 32 / 0 0 / 0 N 219 148 / 0 0 / 0 0 / 0 71 / 0 0 / 0 O 222 160 / 0 0 / 0 0 / 0 62 / 0 0 / 0 P 111 78 / 0 0 / 0 0 / 0 33 / 0 0 / 0 Q 336 237 / 0 0 / 0 0 / 0 99 / 0 0 / 0 M 222 160 / 0 0 / 0 0 / 0 62 / 0 0 / 0 K 111 78 / 0 0 / 0 0 / 0 33 / 0 0 / 0 J 336 237 / 0 0 / 0 0 / 0 99 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I, N, O, P, Q, M, K, J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE FACTORED VERT. LOAD LC1 MAX CSI (LC) WEBS MAX. FACTORED FORCE MAX CSI (LC) MEMB. (LBS) (PLF) (LC) UNBRAC LENGTH FR-TO FR-TO A-S 0 / 42 -112.4 -112.4 0.03 (4) 10.00 N-E -280 / 0 0.35 (1) S-B 0 / 72 -112.4 -112.4 0.15 (1) 10.00 O-D -268 / 0 0.19 (1) B-C 0 / 38 -112.4 -112.4 0.15 (1) 10.00 P-C -164 / 0 0.06 (1) C-D 0 / 80 -112.4 -112.4 0.08 (1) 10.00 Q-B -351 / 0 0.07 (1) D-E 0 / 58 -112.4 -112.4 0.08 (1) 10.00 M-F -268 / 0 0.19 (1) E-F 0 / 58 -112.4 -112.4 0.08 (1) 10.00 K-G -164 / 0 0.06 (1) F-G 0 / 80 -112.4 -112.4 0.08 (1) 10.00 J-H -351 / 0 0.07 (1) G-H 0 / 38 -112.4 -112.4 0.15 (1) 10.00 R-S -160 / 0 0.00 (1) H-U 0 / 72 -112.4 -112.4 0.15 (1) 10.00 T-U -160 / 0 0.00 (1) U-I 0 / 42 -112.4 -112.4 0.03 (4) 10.00 A-R -68 / 0 -18.5 -18.5 0.13 (1) 6.25 R-Q -38 / 0 -18.5 -18.5 0.13 (1) 6.25 Q-P -50 / 0 -18.5 -18.5 0.08 (1) 6.25 P-O -53 / 0 -18.5 -18.5 0.02 (4) 6.25 O-N -57 / 0 -18.5 -18.5 0.02 (4) 6.25 N-M -53 / 0 -18.5 -18.5 0.02 (4) 6.25 M-L -53 / 0 -18.5 -18.5 0.02 (4) 6.25 L-K -53 / 0 -18.5 -18.5 0.02 (4) 6.25 K-J -50 / 0 -18.5 -18.5 0.08 (1) 6.25 J-T -38 / 0 -18.5 -18.5 0.13 (1) 6.25 T-I -68 / 0 -18.5 -18.5 0.13 (1) 6.25				STRUCTURAL COMPONENT ONLY DWG# T-2215373			



Structural component only

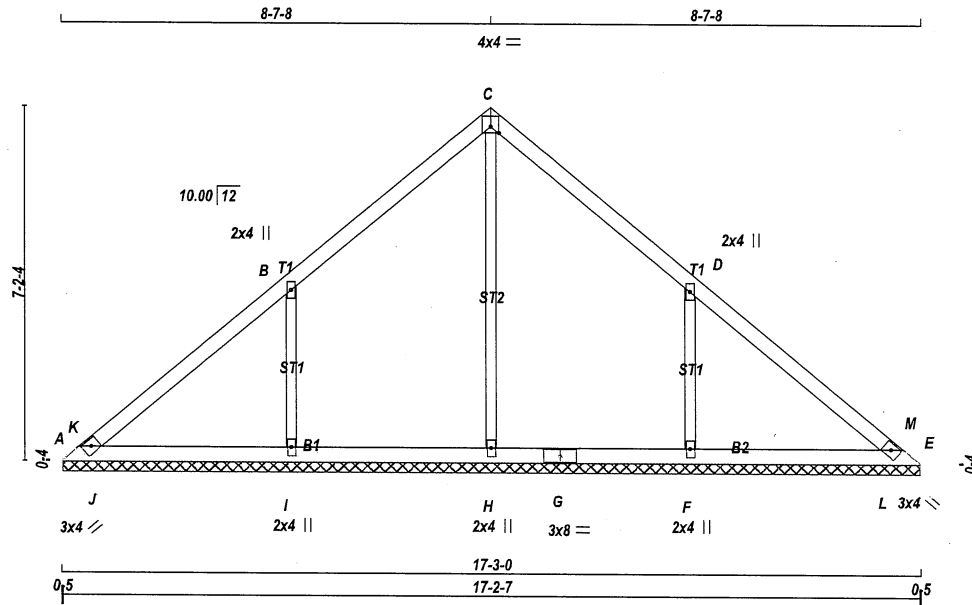
DWG# T-2215373

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423628	V2	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 55 lb

LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS	SPF
SIZE	
A - C	2x4 DRY No.2
C - E	2x4 DRY No.2
A - G	2x4 DRY No.2
G - E	2x4 DRY No.2
ALL WEBS	2x3 DRY No.2
DRY: SEASONED LUMBER.	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP/LIFT
A	175	0	175	0
E	175	0	175	0
H	534	0	534	0
I	684	0	684	0
F	684	0	684	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX /MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	122	91 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0	0 / 0
E	122	91 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0	0 / 0
H	378	249 / 0	0 / 0	0 / 0	0 / 0	129 / 0	0 / 0	0 / 0
I	479	343 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0	0 / 0
F	479	343 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO							FR-TO			
A-K	0 / 84	-112.4	-112.4	0.04 (4)	10.00	H-C	-495 / 0	0.43 (1)		
K-B	0 / 133	-112.4	-112.4	0.33 (1)	10.00	I-B	-549 / 0	0.13 (1)		
B-C	0 / 79	-112.4	-112.4	0.32 (1)	10.00	F-D	-549 / 0	0.13 (1)		
C-D	0 / 79	-112.4	-112.4	0.32 (1)	10.00	J-K	-175 / 5	0.00 (1)		
D-M	0 / 133	-112.4	-112.4	0.33 (1)	10.00	L-M	-175 / 5	0.00 (1)		
M-E	0 / 84	-112.4	-112.4	0.04 (4)	10.00					
A-J	-108 / 0	-18.5	-18.5	0.14 (1)	6.25					
J-I	-73 / 0	-18.5	-18.5	0.14 (1)	6.25					
I-H	-88 / 0	-18.5	-18.5	0.10 (1)	6.25					
H-G	-88 / 0	-18.5	-18.5	0.10 (1)	6.25					
G-F	-88 / 0	-18.5	-18.5	0.10 (1)	6.25					
F-L	-73 / 0	-18.5	-18.5	0.14 (1)	6.25					
L-E	-108 / 0	-18.5	-18.5	0.14 (1)	6.25					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33/1.00 (B-K:1), BC=0.14/1.00 (F-L:1), WB=0.43/1.00 (C-H:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.29 (D) (INPUT = 1.00)



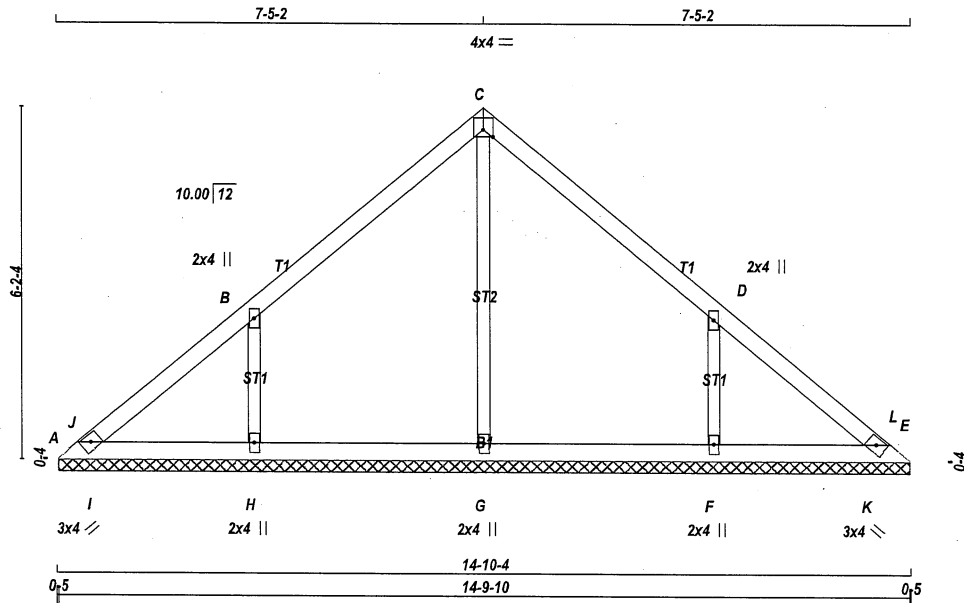
Structural component only
DWG# T-2215374

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423628	V3	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:38.7

TOTAL WEIGHT = 46 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
A	173	0	173	0
E	173	0	173	0
G	410	0	410	0
H	592	0	592	0
F	592	0	592	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
A	120	90 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	120	90 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
G	291	185 / 0	0 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
H	413	299 / 0	0 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	413	299 / 0	0 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-J	-60 / 0	-112.4 -112.4	0.04 (1)	G-C	-346 / 0	0.20 (1)	
J-B	0 / 15	-112.4 -112.4	0.26 (1)	H-B	-493 / 0	0.08 (1)	
B-C	-47 / 4	-112.4 -112.4	0.26 (1)	F-D	-493 / 0	0.08 (1)	
C-D	-47 / 4	-112.4 -112.4	0.26 (1)	I-J	-64 / 6	0.00 (1)	
D-L	0 / 15	-112.4 -112.4	0.26 (1)	K-L	-64 / 6	0.00 (1)	
L-E	-60 / 0	-112.4 -112.4	0.04 (1)				
A-I	-7 / 17	-18.5 -18.5	0.07 (1)				
I-H	-4 / 32	-18.5 -18.5	0.07 (1)				
H-G	-7 / 13	-18.5 -18.5	0.06 (4)				
G-F	-7 / 13	-18.5 -18.5	0.06 (4)				
F-K	-4 / 32	-18.5 -18.5	0.07 (1)				
K-E	-7 / 17	-18.5 -18.5	0.07 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (D-L:1), BC=0.07/1.00 (F-K:1), WB=0.20/1.00 (C-G:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches.

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (C) (INPUT = 0.90)

JSI METAL= 0.26 (D) (INPUT = 1.00)



Structural component only
DWG# T-2215375

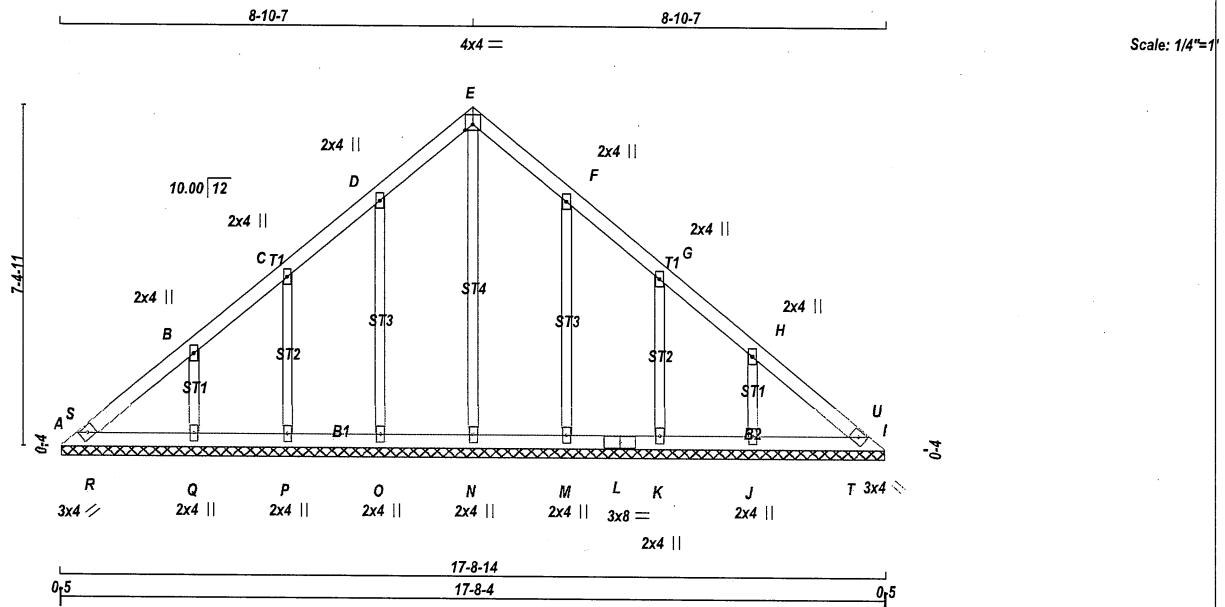
REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423631	V25	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - E 2x4 DRY No.2 SPF E - I 2x4 DRY No.2 SPF A - L 2x4 DRY No.2 SPF L - I 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.				PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TBM1-h MT20 3.0 4.0 B, C, D, F, G, H E TMW+w MT20 2.0 4.0 B TTW-p MT20 4.0 4.0 1.50 2.00 I TBM1-h MT20 3.0 4.0 J, K, M, N, O, P, Q J BMW1+w MT20 2.0 4.0 L BS-1 MT20 3.0 8.0				NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.			
DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.08/1.00 (H-U:1) , BC=0.08/1.00 (J-T:1) , WB=0.19/1.00 (E-N:1) , SSI=0.09/1.00 (H-U:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.80 (E) (INPUT = 0.90) JSI METAL= 0.14 (B) (INPUT = 1.00)				BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT BRG BRG A 157 0 157 0 0 17-8-4 (5-8-8) 17-8-4 I 157 0 157 0 0 17-8-4 (5-8-8) 17-8-4 N 238 0 238 0 0 17-8-4 (5-8-8) 17-8-4 O 303 0 303 0 0 17-8-4 (5-8-8) 17-8-4 P 210 0 210 0 0 17-8-4 (5-8-8) 17-8-4 Q 369 0 369 0 0 17-8-4 (5-8-8) 17-8-4 M 303 0 303 0 0 17-8-4 (5-8-8) 17-8-4 K 210 0 210 0 0 17-8-4 (5-8-8) 17-8-4 J 369 0 369 0 0 17-8-4 (5-8-8) 17-8-4 VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH UNFACTORED REACTIONS 1ST CASE MAX / MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL A 109 80 / 0 0 / 0 0 / 0 29 / 0 0 / 0 I 109 80 / 0 0 / 0 0 / 0 29 / 0 0 / 0 N 168 111 / 0 0 / 0 0 / 0 57 / 0 0 / 0 O 212 153 / 0 0 / 0 0 / 0 59 / 0 0 / 0 P 147 103 / 0 0 / 0 0 / 0 44 / 0 0 / 0 Q 259 182 / 0 0 / 0 0 / 0 76 / 0 0 / 0 M 212 153 / 0 0 / 0 0 / 0 59 / 0 0 / 0 K 147 103 / 0 0 / 0 0 / 0 44 / 0 0 / 0 J 259 182 / 0 0 / 0 0 / 0 76 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I, N, O, P, Q, M, K, J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) C H O R D S W E B S MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC) UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. CSI (LC) FR-TO A-S -26 / 0 -112.4 -112.4 0.02 (1) 6.25 N-E -204 / 0 0.19 (1) S-B 0 / 10 -112.4 -112.4 0.08 (1) 10.00 O-D -260 / 0 0.13 (1) B-C -10 / 4 -112.4 -112.4 0.08 (1) 10.00 P-C -195 / 0 0.05 (1) C-D 0 / 13 -112.4 -112.4 0.07 (1) 10.00 Q-B -272 / 0 0.04 (1) D-E -2 / 6 -112.4 -112.4 0.07 (1) 10.00 M-F -260 / 0 0.13 (1) E-F -2 / 6 -112.4 -112.4 0.07 (1) 10.00 K-G -195 / 0 0.05 (1) F-G 0 / 13 -112.4 -112.4 0.07 (1) 10.00 J-H -272 / 0 0.04 (1) G-H -10 / 4 -112.4 -112.4 0.08 (1) 10.00 R-S -97 / 0 0.00 (1) H-U 0 / 10 -112.4 -112.4 0.08 (1) 10.00 T-U -97 / 0 0.00 (1) U-I -26 / 0 -112.4 -112.4 0.02 (1) 6.25 A-R -7 / 0 -18.5 -18.5 0.08 (1) 10.00 R-Q -3 / 12 -18.5 -18.5 0.08 (1) 10.00 Q-P -5 / 0 -18.5 -18.5 0.05 (1) 10.00 P-O -6 / 0 -18.5 -18.5 0.02 (4) 10.00 O-N -10 / 0 -18.5 -18.5 0.02 (4) 10.00 N-M -10 / 0 -18.5 -18.5 0.02 (4) 10.00 M-L -6 / 0 -18.5 -18.5 0.02 (4) 10.00 L-K -6 / 0 -18.5 -18.5 0.02 (4) 10.00 K-J -5 / 0 -18.5 -18.5 0.05 (1) 10.00 J-T -3 / 12 -18.5 -18.5 0.08 (1) 10.00 T-I -7 / 0 -18.5 -18.5 0.08 (1) 10.00				REVIEWED			

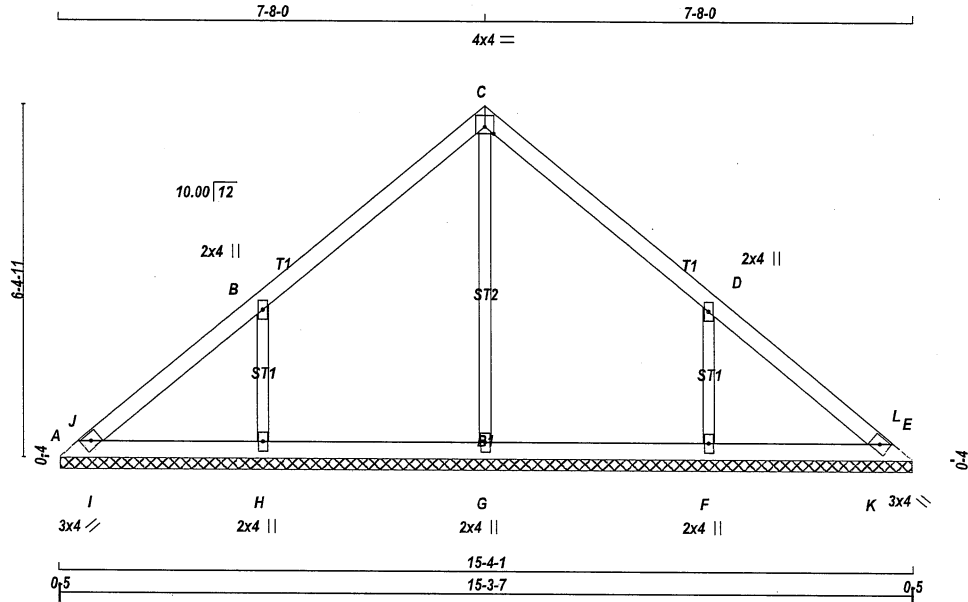


Structural component only
DWG# T-2215389

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423631	V26	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 48 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW-w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMW-w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1-w	MT20	2.0	4.0		

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP
A	177	0	177	0
E	177	0	177	0
G	432	0	432	0
H	608	0	608	0
F	608	0	608	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	123	92 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	123	92 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
G	306	196 / 0	0 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
H	425	307 / 0	0 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0
F	425	307 / 0	0 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED HORZ. LOAD (PLF)
FR-TO						FR-TO			
A-J	-36 / 1	-112.4	-112.4	0.04 (1)	6.25	G-C	-372 / 0	0.24 (1)	
J-B	0 / 32	-112.4	-112.4	0.27 (1)	10.00	H-B	-503 / 0	0.09 (1)	
B-C	-25 / 10	-112.4	-112.4	0.27 (1)	6.25	F-D	-503 / 0	0.09 (1)	
C-D	-25 / 10	-112.4	-112.4	0.27 (1)	6.25	I-J	-86 / 6	0.00 (1)	
D-L	0 / 32	-112.4	-112.4	0.27 (1)	10.00	K-L	-86 / 6	0.00 (1)	
L-E	-36 / 1	-112.4	-112.4	0.04 (1)	6.25				
A-I	-13 / 0	-18.5	-18.5	0.08 (1)	6.25				
I-H	-9 / 13	-18.5	-18.5	0.08 (1)	10.00				
H-G	-12 / 0	-18.5	-18.5	0.06 (4)	6.25				
G-F	-12 / 0	-18.5	-18.5	0.06 (4)	6.25				
F-K	-9 / 13	-18.5	-18.5	0.08 (1)	10.00				
K-E	-13 / 0	-18.5	-18.5	0.08 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (D-L:1), BC=0.08/1.00 (F-K:1), WB=0.24/1.00 (C-G:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (C) (INPUT = 0.90)
JSI METAL= 0.27 (D) (INPUT = 1.00)



Structural component only
DWG# T-2215390

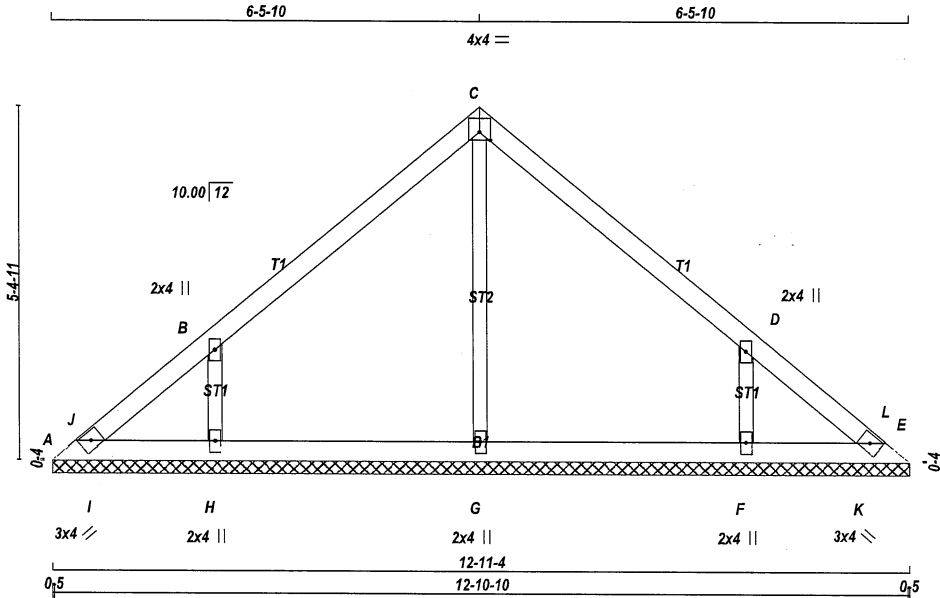
REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423631	V27	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 39 lb

[M]

LUMBER					DESCR.
N. L. G. A. RULES	SIZE	LUMBER			
A - C	2x4	DRY	No.2		SPF
C - E	2x4	DRY	No.2		SPF
A - E	2x4	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TMW-w	MT20	2.0	4.0	
C	TTW-p	MT20	4.0	4.0	1.50 2.00
D	TMW-w	MT20	2.0	4.0	
E	TBM1-h	MT20	3.0	4.0	
F, G, H					
F	BMW1-w	MT20	2.0	4.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
A	133	0	133	0	0	12-11-4	(12-10-1)-4		
E	133	0	133	0	0	12-11-4	(12-10-1)-4		
G	342	0	342	0	0	12-11-4	(12-10-1)-4		
H	540	0	540	0	0	12-11-4	(12-10-1)-4		
F	540	0	540	0	0	12-11-4	(12-10-1)-4		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	93	69 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
E	93	69 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
G	243	151 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
H	377	274 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
F	377	274 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-J	-131 / 0	G-C	-264 / 0
J-B	-37 / 0	H-B	-476 / 0
B-C	-112 / 0	F-D	-476 / 0
C-D	-112 / 0	I-J	0 / 23
D-L	-37 / 0	K-L	0 / 23
L-E	-131 / 0		

A-I	0 / 92	-18.5	-18.5	0.02 (1)	10.00
I-H	0 / 91	-18.5	-18.5	0.05 (4)	10.00
H-G	0 / 64	-18.5	-18.5	0.07 (4)	10.00
G-F	0 / 64	-18.5	-18.5	0.07 (4)	10.00
F-K	0 / 91	-18.5	-18.5	0.05 (4)	10.00
K-E	0 / 92	-18.5	-18.5	0.02 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (B-C:1) , BC=0.07/1.00 (F-G:4) , WB=0.11/1.00 (C-G:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (C) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)



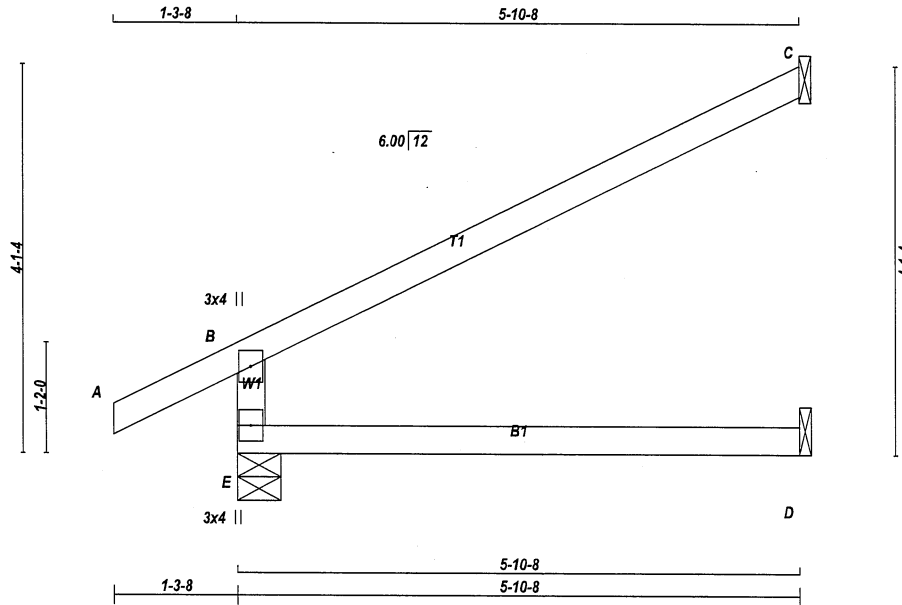
Structural component only

DWG# T-2215391

REVIEWED

JOB NAME 423626	TRUSS NAME J1	QUANTITY 10	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Fri Jun 24 15:13:00 2022 Page 1
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Scale = 1:23.3

TOTAL WEIGHT = 10 X 17 = 168 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS	2x4	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	DOWN	UP/LIFT
E		629	0	629	0
C		248	0	248	0
D		45	0	50	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	438	327 / 0	0 / 0	0 / 0	0 / 0	111 / 0
C	170	143 / 0	0 / 0	0 / 0	0 / 0	26 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE	VERT. LOAD	MAX. FACTORED	MEMB.	FORCE
	(LBS)	(PLF)	CSI (LC)		(LBS)
FR-TO		FROM TO		FR-TO	
E-B	-565 / 0	0.0 0.0	0.13 (4)	7.81	
A-B	0 / 34	-112.4 -112.4	0.15 (1)	10.00	
B-C	-37 / 0	-112.4 -112.4	0.66 (1)	6.25	
E-D	0 / 0	-18.5 -18.5	0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.66/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (E) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



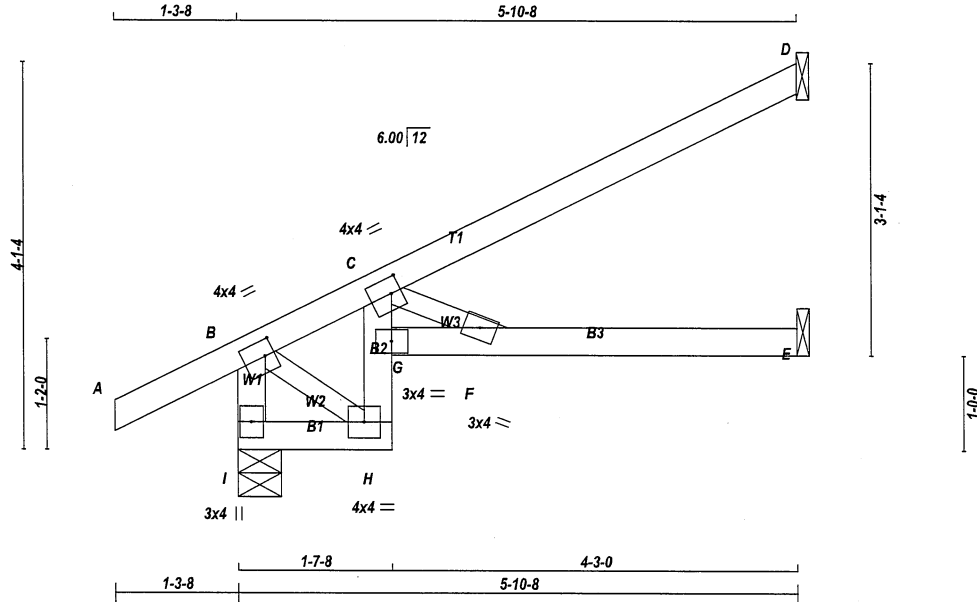
Structural component only
DWG# T-2215345

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423626	J1S	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:23.3

TOTAL WEIGHT = 8 X 20 = 163 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV-w	MT20	3.0	4.0		
G	BMV-h	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT
I	555	0	555	0	0
D	248	0	248	0	0
E	119	0	119	0	0

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	386	289 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	171	138 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
E	86	42 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	(LBS)	(PLF)	CS1 (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE	MAX
FR-TO										
I-B	-541 / 0	0.0	0.0	0.05 (1)	7.81			B-H	0 / 248	0.06 (1)
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00			C-F	-657 / 0	0.10 (1)
B-C	-273 / 0	-112.4	-112.4	0.14 (1)	6.25					
C-D	0 / 4	-112.4	-112.4	0.37 (1)	10.00					
I-H	0 / 0	-18.5	-18.5	0.01 (4)	10.00					
H-G	-111 / 0	0.0	0.0	0.29 (1)	7.81					
G-C	0 / 173	0.0	0.0	0.32 (1)	10.00					
G-F	0 / 574	-18.5	-18.5	0.49 (1)	10.00					
F-E	0 / 0	-18.5	-18.5	0.40 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/853 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/451 (0.16")

CSI: TC=0.37/1.00 (C-D:1), BC=0.49/1.00 (F-G:1), WB=0.10/1.00 (C-F:1), SSI=0.28/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (C) (INPUT = 0.90)
JSI METAL= 0.25 (F) (INPUT = 1.00)

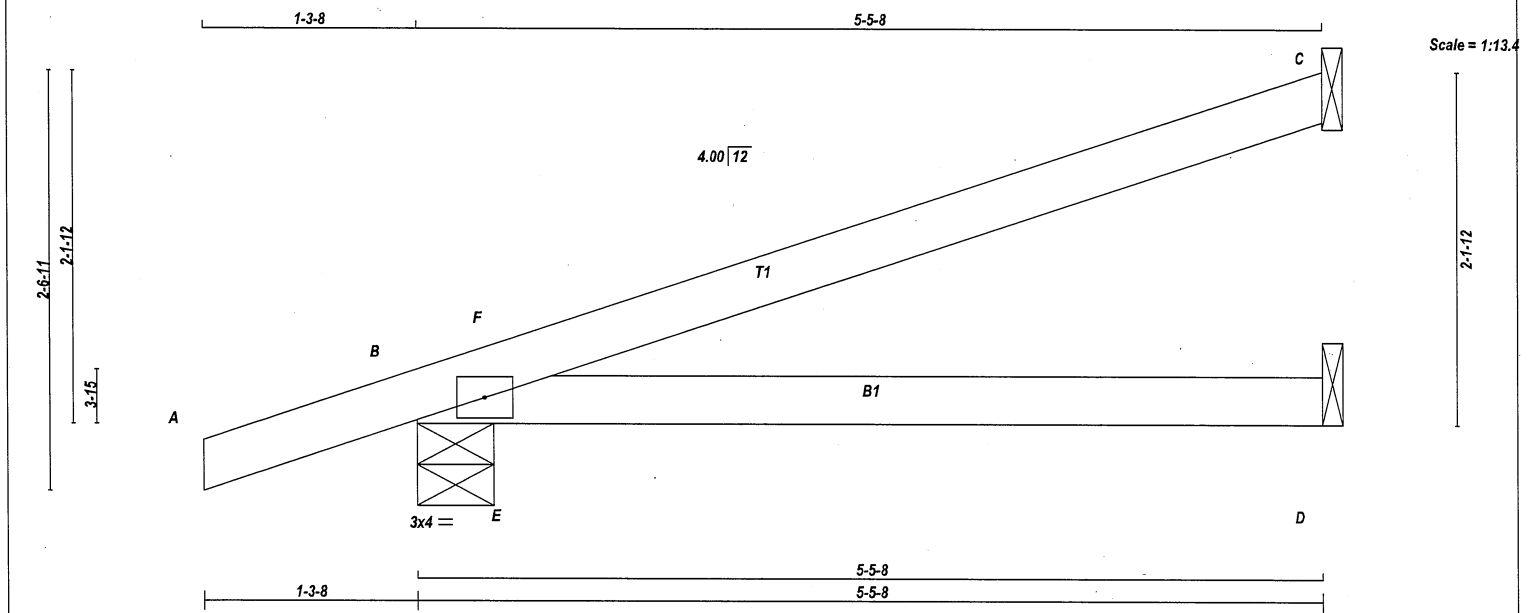


Structural component only
DWG# T-2215346

REVIEWED

JOB NAME 423626	TRUSS NAME J2	QUANTITY 9	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:13.4

TOTAL WEIGHT = 9 X 14 = 130 lb

LUMBER				DESCR.	SPF
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
C	266	0	266	0	1-8	1-8
B	508	0	508	0	5-8	5-8
D	92	0	92	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
C	183	149 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
B	353	264 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	68	28 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		VERT. LOAD	LC1	MAX	MAX.	MEMB.	WEBS		MAX	MAX.
	MAX. FACTORED	FORCE						MAX. FACTORED	FORCE		
	(LBS)	(LBS)						(LBS)			
FR-TO			FROM	TO	CS1 (LC)	UNBRAC	FR-TO			CS1 (LC)	
A-B	0 / 22		-112.4	-112.4	0.14 (1)	10.00	E-F	-368 / 11		0.00 (1)	
B-F	-18 / 19		-112.4	-112.4	0.06 (4)	6.25					
F-C	-2 / 2		-112.4	-112.4	0.43 (1)	10.00					
B-E	0 / 0		-18.5	-18.5	0.31 (1)	10.00					
E-D	0 / 0		-18.5	-18.5	0.31 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/973 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/516 (0.13")

CSI: TC=0.43/1.00 (C-F:1), BC=0.31/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SS=0.29/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00'

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



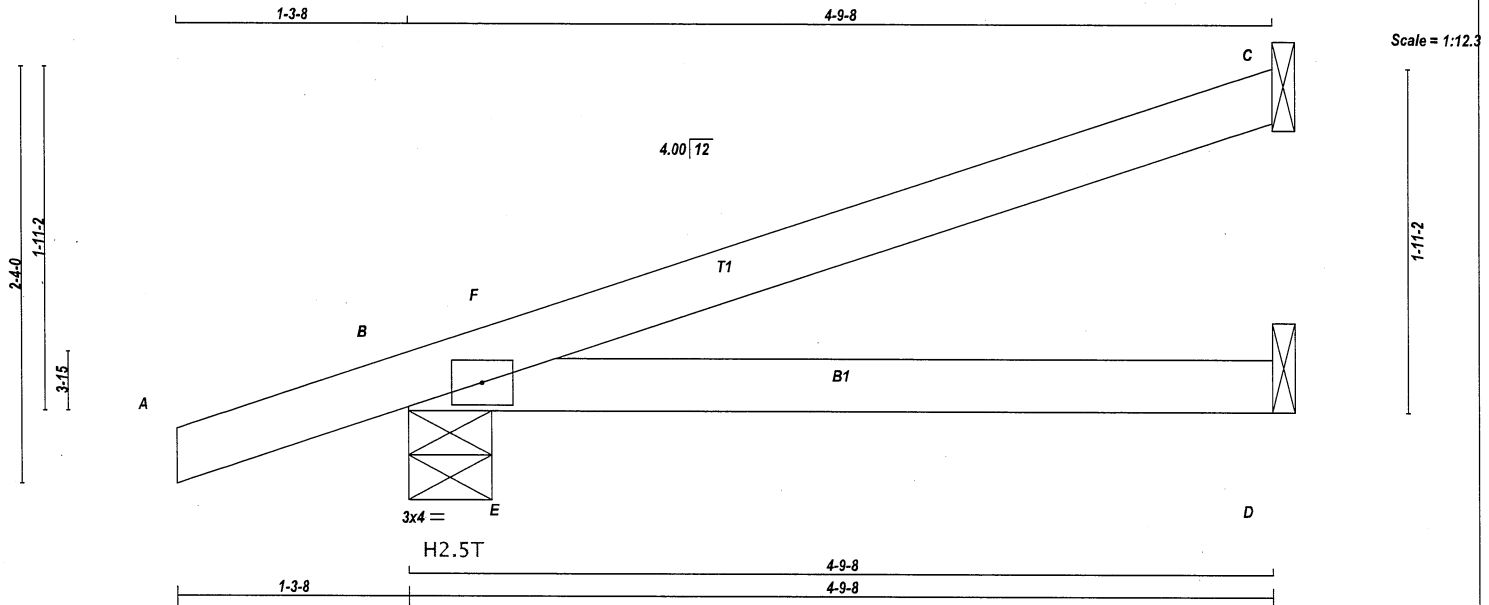
Structural component only
DWG# T-2215347

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423626	J3W	9	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2	SPF
	B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
C	232	0	232	0	-97	1-8
B	464	0	464	96	-174	5-8
D	81	0	81	0	-67	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 174 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

PROVIDE FOR 96 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	160	130 / 0	0 / 0	0 / 0	0 / -89	30 / 0	0 / 0
B	323	243 / 0	0 / 0	0 / 0	0 / -176	80 / 0	0 / 0
D	60	25 / 0	0 / 0	0 / 0	0 / -70	35 / 0	0 / 0

HORIZONTAL REACTIONS						
B	---	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO	UNBRAC LENGTH	FR-TO			
A- B	0 / 22	-112.4 -112.4	0.14 (1)	10.00	E- F	-302 / 73	
B- F	-66 / 8	-112.4 -112.4	0.05 (12)	6.25		0.00 (1)	
F- C	-38 / 2	-112.4 -112.4	0.33 (1)	6.25			
B- E	0 / 0	-18.5 -18.5	0.24 (1)	10.00			
E- D	0 / 0	-18.5 -18.5	0.24 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (15-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_pC_g, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/751 (0.08")

CSI: TC=0.33/1.00 (C-F:1), BC=0.24/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.23/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

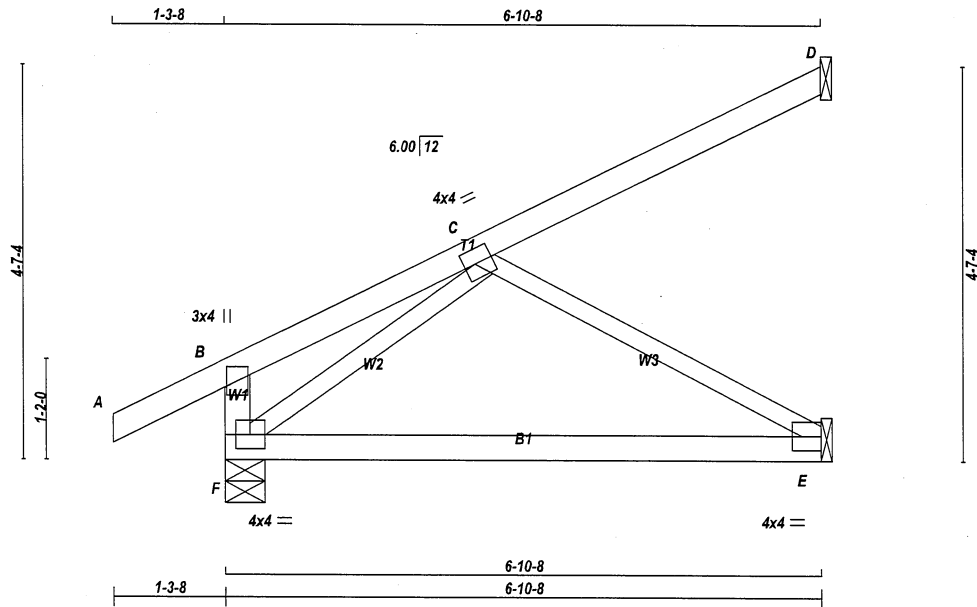


Structural component only
DWG# T-2215348

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423629	J25	6	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 6 X 26 = 154 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4 DRY	No.2	SPF
A - D	2x4 DRY	No.2	SPF
F - E	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	2.00 1.50
E	BMW1-t	MT20	4.0	4.0	2.00 Edge
F	BMVW1-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
F	598	0	598	0	0	5-8	5-8		
D	180	0	180	0	0	1-8	1-8		
E	272	0	272	0	0	1-8	1-8		

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	417	309 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0
D	123	104 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
E	193	121 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO	FR-TO			
F-B	-259 / 0	0.0	0.0	0.03 (1)	7.81	C-E	-407 / 0
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00	F-C	-445 / 0
B-C	0 / 26	-112.4	-112.4	0.23 (1)	10.00		
C-D	-19 / 0	-112.4	-112.4	0.19 (1)	6.25		
F-E	0 / 349	-18.5	-18.5	0.26 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 860 (0.09")

CSI: TC=0.23/1.00 (B-C:1), BC=0.26/1.00 (E-F:4), WB=0.15/1.00 (C-E:1), SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (F) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)



Structural component only
DWG# T-2216585

REVIEWED

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 15 = 31 lb

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) ,
WB=0.00/1.00 (B-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

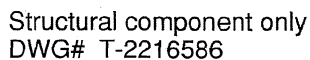
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

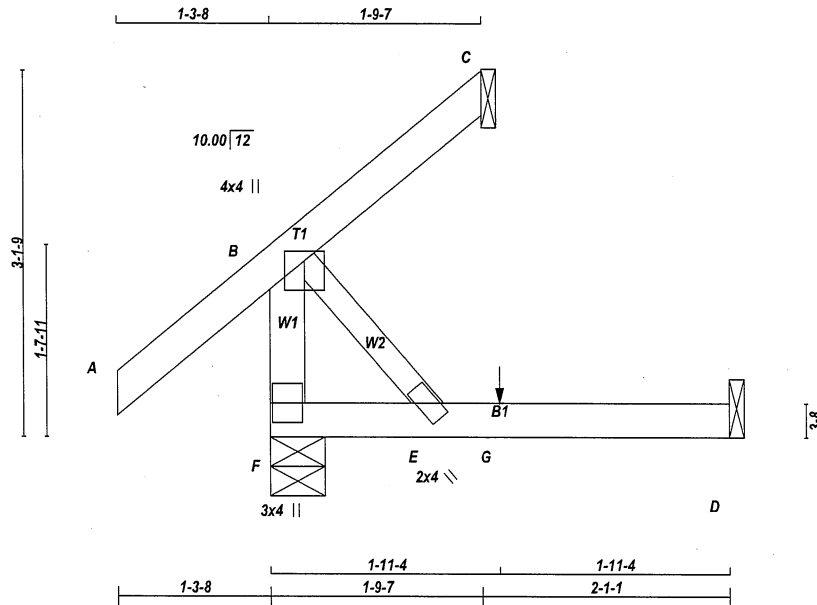
JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



REVIEWED

JOB NAME: 423629	TRUSS NAME: J27	QUANTITY: 1	PLY: 1	JOB DESC.: BAYVIEW WELLINGTON	DRWG NO.:
Tamarack Roof Truss, Burlington				TRUSS DESC.:	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jul 8 10:50:31 2022 Page 1
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TOTAL WEIGHT = 12 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
F	352	0	0	352	0	0	5-8	5-8	
C	40	0	0	40	0	-51	1-8	1-8	
D	36	0	0	40	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	245	183 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	27	23 / -35	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
F-B	-317 / 0	0.0	0.0 0.03 (1)	B-E	0 / 0
A-B	0 / 50	-112.4	-112.4 0.17 (5)		
B-C	-39 / 0	-112.4	-112.4 0.16 (5)		
F-E	0 / 0	-18.5	-18.5 0.07 (4)		
E-G	0 / 0	-18.5	-18.5 0.08 (4)		
G-D	0 / 0	-18.5	-18.5 0.08 (4)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.17/1.00 (A-B-5), BC=0.08/1.00 (D-E-4), WB=0.00/1.00 (B-E-1), SSI=0.10/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



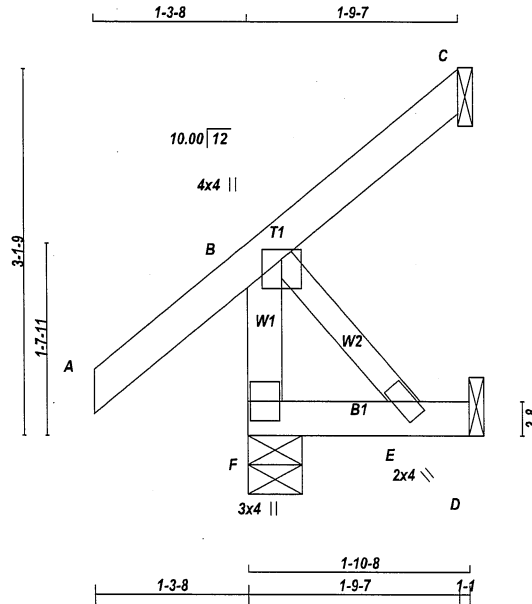
Structural component only
DWG# T-2216587

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423629	J28	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:18.8

TOTAL WEIGHT = 10 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
F	334	0	334	0
C	40	0	40	0
D	17	0	19	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	COMBINED						
F	231	183 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	27	23 / -35	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO						FR-TO			
F-B	-317 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00				
B-C	-39 / 0	-112.4	-112.4	0.15 (1)	6.25				
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(FSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

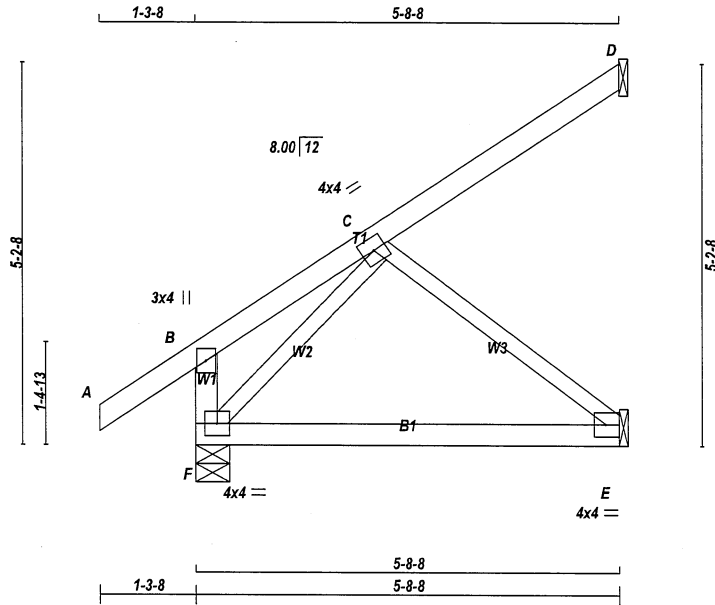


Structural component only
DWG# T-2216588

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	J50	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:30.0

TOTAL WEIGHT = 2 X 23 = 47 lb [M]

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
F - B	2x4	DRY No.2		SPF	
A - D	2x4	DRY No.2		SPF	
F - E	2x4	DRY No.2		SPF	
ALL WEBS	2x3	DRY No.2		SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
E	BMW1-t	MT20	4.0	4.0	2.00	Edge
F	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
F	525	0		525	0	0	5-8	5-8	
D	151	0		151	0	0	1-8	1-8	
E	224	0		224	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	365	273 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
D	103	87 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0
E	160	100 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM	TO		FR-TO		
F-B	-240 / 0	0.0	0.0	0.02 (1)	7.81	C-E	-270 / 0
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	F-C	-313 / 0
B-C	0 / 27	-112.4	-112.4	0.16 (1)	10.00		
C-D	-20 / 0	-112.4	-112.4	0.16 (1)	6.25		
F-E	0 / 208	-18.5	-18.5	0.18 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.18/1.00 (E-F:4) , WB=0.09/1.00 (C-E:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (F) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

Structural component only
DWG# T-2215392

REVIEWED



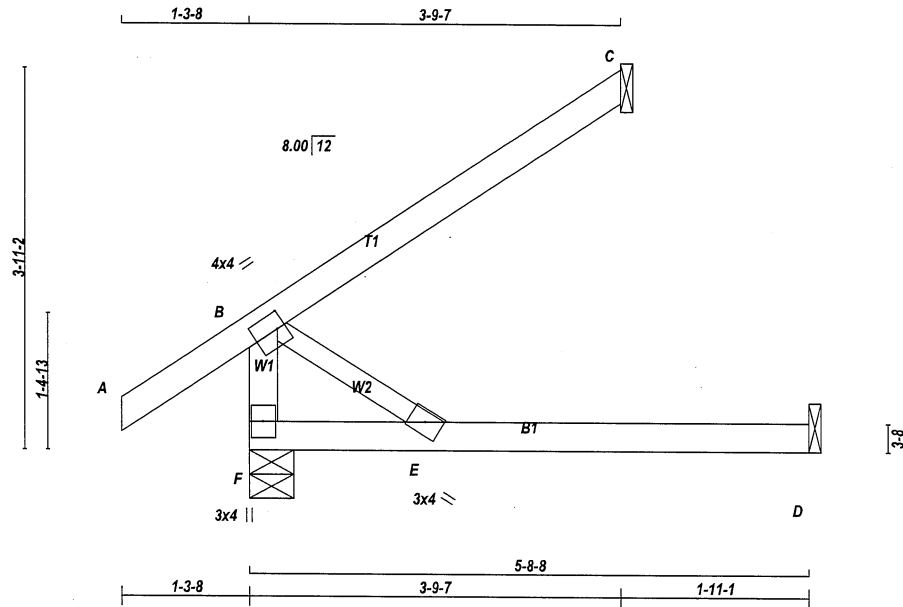
Structural component only
DWG# T-2215392

REVIEWED

JOB NAME 423632	TRUSS NAME J51	QUANTITY 3	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:22.7

TOTAL WEIGHT = 3 X 16 = 48 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.	BEARINGS				SPECIFIED LOADS:			
CHORDS	2x4	DRY	No.2	FACTORED				TOP CH. LL = 32.5 PSF			
F - B	2x4	DRY	No.2	GROSS REACTION				DL = 6.0 PSF			
A - C	2x4	DRY	No.2	MAXIMUM FACTORED				BOT CH. LL = 0.0 PSF			
F - D	2x4	DRY	No.2	INPUT				DL = 7.4 PSF			
ALL WEBS	2x3	DRY	No.2	REQRD				TOTAL LOAD = 45.9 PSF			
DRY: SEASONED LUMBER.				BRG				SPACING = 24.0 IN. C/C			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMV-w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	294	212 / 0	0 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	146	123 / 0	0 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	42	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO			
F-B	-367 / 0	0.0	0.0	0.04 (1)	7.81	0.00 (1)	
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00		
B-C	0 / 0	-112.4	-112.4	0.27 (1)	10.00		
F-E	0 / 0	-18.5	-18.5	0.15 (4)	10.00		
E-D	0 / 0	-18.5	-18.5	0.18 (4)	10.00		

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.27/1.00 (B-C:1), BC=0.18/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)

JSI METAL= 0.07 (B) (INPUT = 1.00)

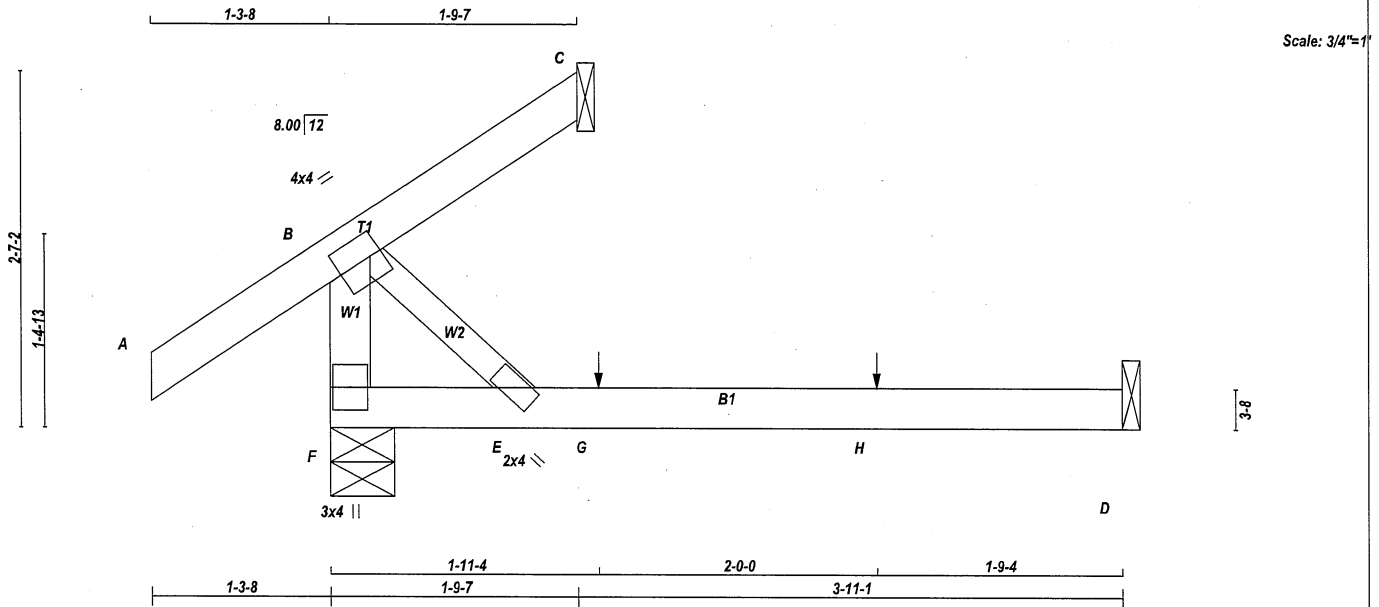


Structural component only
DWG# T-2215393

REVIEWED

JOB NAME 423632	TRUSS NAME J52	QUANTITY 3	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 3 X 13 = 40 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW-w	MT20	2.0	4.0		
F	BMW1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	367	0	367	0	5-8	5-8
C	41	0	41	0	1-8	1-8
D	53	0	59	0	1-8	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	IN-SX			
F	257	181 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
C	28	24 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	42	0 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE		FACTORED VERT. LOAD		MAX. FACTORED UNBRACED LENGTH		WEBS MAX. FACTORED FORCE	
	FR-TO	LC1	FROM	TO	FR-TO	LC1	FR-TO	LC1
F-B	-314 / 0	0.0	0.0	0.03 (1)	7.81	0.00 (1)	0 / 0	0.00 (1)
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00			
B-C	-33 / 0	-112.4	-112.4	0.14 (1)	6.25			
F-E	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
E-G	0 / 0	-18.5	-18.5	0.18 (4)	10.00			
G-H	0 / 0	-18.5	-18.5	0.18 (4)	10.00			
H-D	0 / 0	-18.5	-18.5	0.18 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.05")

CSI: TC=0.15/1.00 (A-B-1), BC=0.18/1.00 (D-E-4), WB=0.00/1.00 (B-E-1), SSI=0.10/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371 1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

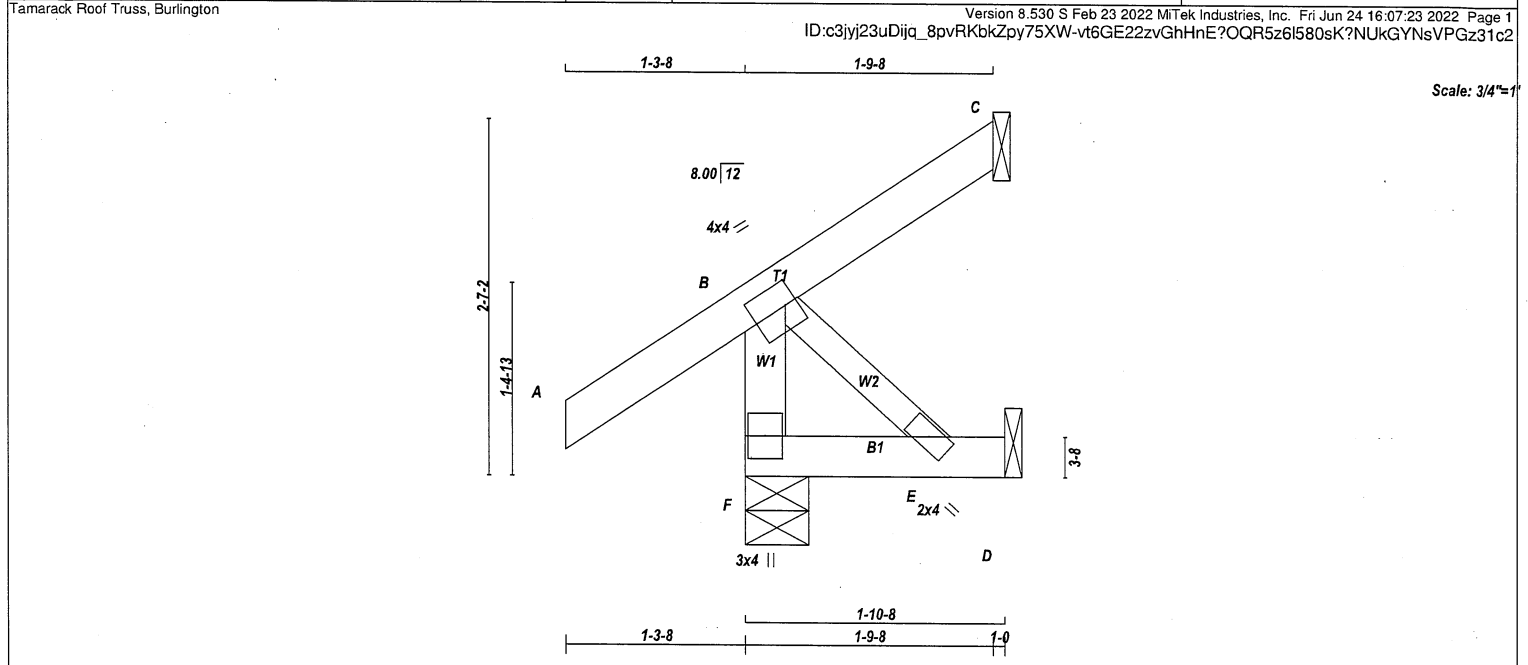
JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2215394

REVIEWED

JOB NAME 423632	TRUSS NAME J54	QUANTITY 5	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 5 X 9 = 45 lb											
N. L. G. A. RULES																				[M]											
CHORDS SIZE										LUMBER										DESCR.											
F - B 2x4 DRY										No.2										SPF											
A - C 2x4 DRY										No.2										SPF											
F - D 2x4 DRY										No.2										SPF											
ALL WEBS 2x3 DRY										No.2										SPF											
DRY: SEASONED LUMBER.																															
PLATES (table is in inches)																															
JT TYPE			PLATES			W			LEN			Y			X																
B TMVW-t			MT20			4.0			4.0			2.00			1.00																
E BMW+w			MT20			2.0			4.0																						
F BMV1+p			MT20			3.0			4.0																						
NOTES- (1)																															
1) Lateral braces to be a minimum of 2X4 SPF #2.																															
BEARINGS										FACTORED GROSS REACTION										MAXIMUM FACTORED GROSS REACTION											
JT										VERT										HORZ											
F										331										0											
C										41										0											
D										17										0											
										DOWN										HORZ											
										331										0											
										41										0											
										17										0											
										0										5-8											
										0										1-8											
										0										1-8											
SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D																															
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT																															
UNFACTORED REACTIONS																															
1ST LCASE										MAX /MIN. COMPONENT REACTIONS																					
JT COMBINED										SNOW										LIVE											
F										229										181 / 0											
C										28										24 / -34											
D										14										0 / 0											
										0 / 0										0 / 0											
										0 / 0										0 / 0											
										0 / 0										0 / 0											
										0 / 0										0 / 0											
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C																															
BRACING																															
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.																															
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.																															
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.																															
LOADING																															
TOTAL LOAD CASES: (5)																															
CHORDS										WEBS																					
MAX. FACTORED FORCE (LBS)										FACTORED VERT. LOAD (PLF)										MAX. FACTORED FORCE (LBS)											
MEMB.										CS1 (LC)										MEMB.											
FR-TO										FROM TO										FR-TO											
F-B										-314 / 0										0.0 0.0 0.03 (1)											
A-B										0 / 43										-112.4 -112.4 0.15 (1)											
B-C										-33 / 0										-112.4 -112.4 0.14 (1)											
																				6.25											



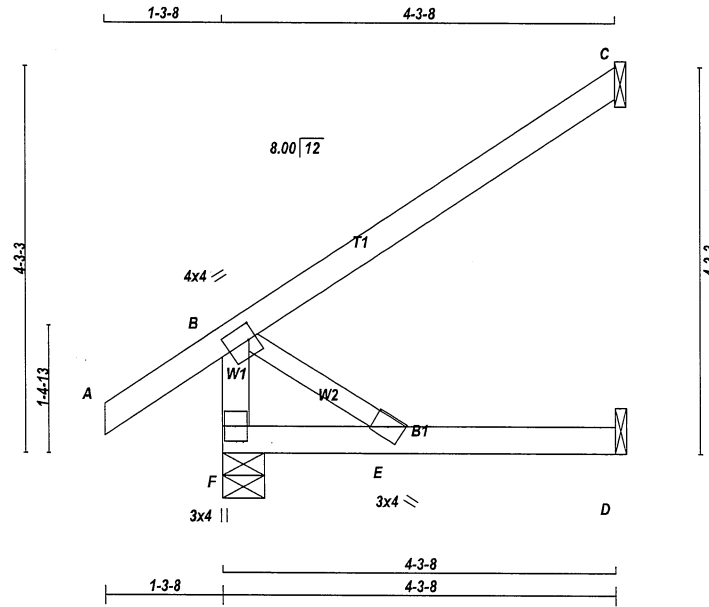
Structural component only
DWG# T-2215396

REVIEWED

JOB NAME 423632	TRUSS NAME J55	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:24.3

TOTAL WEIGHT = 15 lb

LUMBER

N. L. G. A. RULES					
CHORDS		SIZE		LUMBER	DESCR
F - B	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
F - D	2x4	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	DOWN	BRG	BRG
F	435	0	5-8	5-8
C	241	0	1-8	1-8
D	40	0	1-8	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	
F	303	229 / 0	0 / 0	0 / 0	74 / 0
C	165	139 / 0	0 / 0	0 / 0	26 / 0
D	32	0 / 0	0 / 0	0 / 0	32 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
F-B	-396 / 0	0.0 0.0 0.04 (1)	B-E	0 / 0
A-B	0 / 43	-112.4 -112.4 0.15 (1)		
B-C	0 / 0	-112.4 -112.4 0.35 (1)		
F-E	0 / 0	-18.5 -18.5 0.10 (4)		
E-D	0 / 0	-18.5 -18.5 0.10 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.35/1.00 (B-C:1), BC=0.10/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

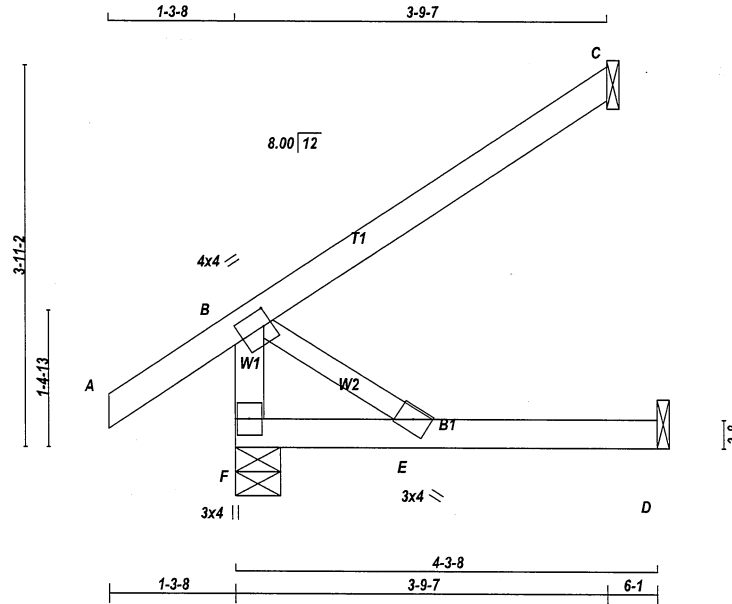


Structural component only
DWG# T-2215397

REVIEWED

JOB NAME 423632	TRUSS NAME J56	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:22.7

TOTAL WEIGHT = 2 X 15 = 30 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF	SPF
F - B	2x4	DRY	No.2	SPF			
A - C	2x4	DRY	No.2	SPF			
F - D	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
DRY: SEASONED LUMBER.							

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW-w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	283	212 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
C	146	123 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	32	0 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-367 / 0	0.0 0.0	0.04 (1)	B-E	0 / 0	0.00 (1)	
A-B	0 / 43	-112.4 -112.4	0.15 (1)				
B-C	0 / 0	-112.4 -112.4	0.27 (1)				
F-E	0 / 0	-18.5 -18.5	0.10 (4)				
E-D	0 / 0	-18.5 -18.5	0.10 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.10/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



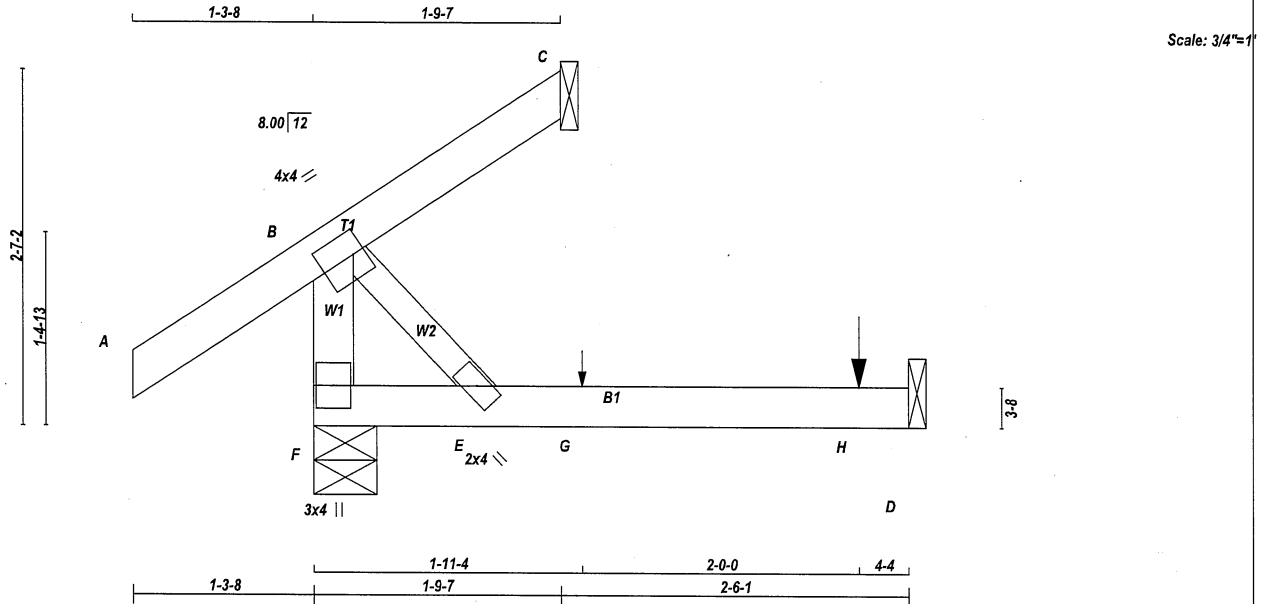
Structural component only
DWG# T-2215398

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
423632	J57	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 12 = 23 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	354	0	354	0	5-8	5-8
C	41	0	41	0	1-8	1-8
D	43	0	48	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	247	181 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
C	28	24 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
F-B	-314 / 0	0.0 0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 43	-112.4 -112.4	0.15 (1)	10.00			
B-C	-33 / 0	-112.4 -112.4	0.14 (1)	6.25			
F-E	0 / 0	-18.5 -18.5	0.08 (4)	10.00			
E-G	0 / 0	-18.5 -18.5	0.10 (4)	10.00			
G-H	0 / 0	-18.5 -18.5	0.10 (4)	10.00			
H-D	0 / 0	-18.5 -18.5	0.10 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	2	2	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (A-B:1), BC=0.10/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

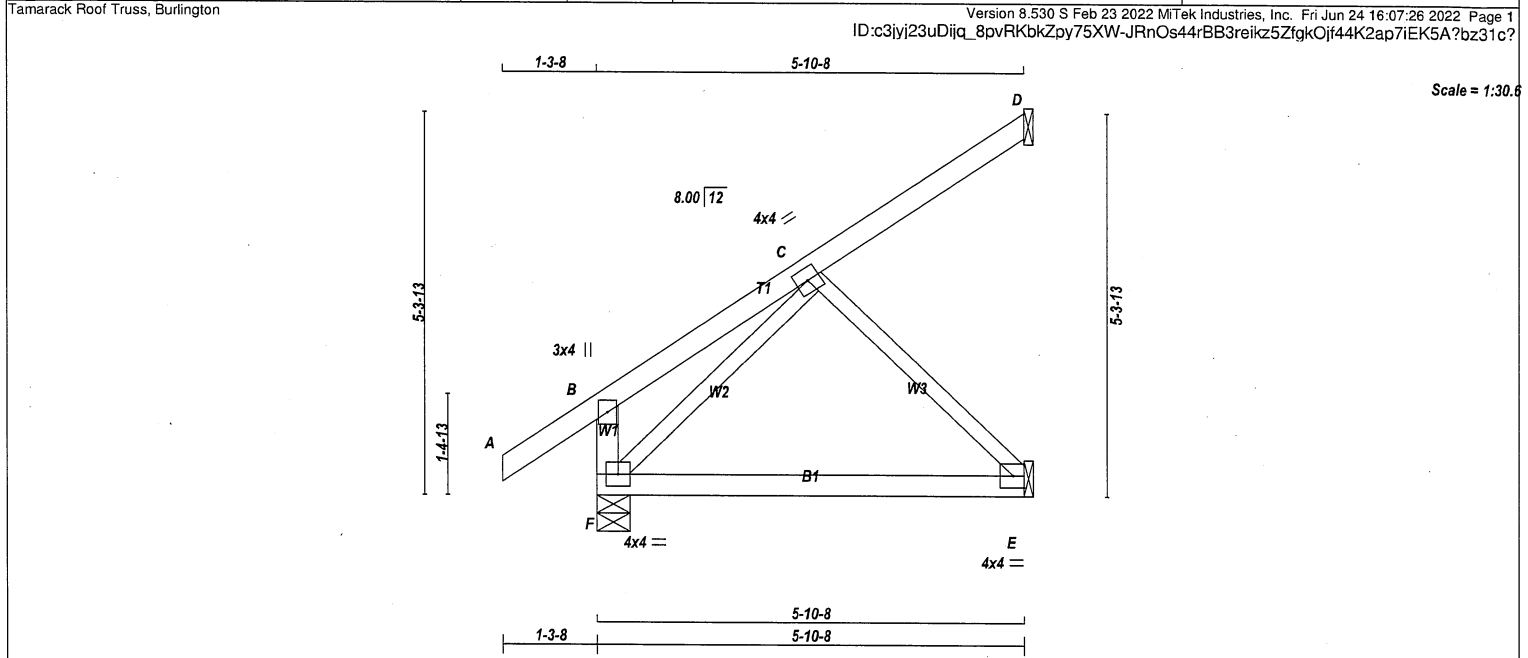
JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2215399

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	J58	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Jun 24 16:07:26 2022 Page 1					
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LUMBER

N. L. G. A. RULES

CHORDS

F - B

2x4

DRY

No.2

SPF

A - D

2x4

DRY

No.2

SPF

F - E

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
E	BMV1-t	MT20	4.0	4.0		Edge
F	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
F	536	0	536	0
D	129	0	129	0
E	257	0	257	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE	MAX /MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
F	373	278 / 0	0 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
D	89	75 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
E	182	118 / 0	0 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
F-B	-276 / 0	0.0	0.0	0.03 (1)	C-E	-284 / 0	0.09 (1)
A-B	0 / 43	-112.4	-112.4	0.15 (1)	F-C	-285 / 0	0.08 (1)
B-C	0 / 22	-112.4	-112.4	0.16 (1)			
C-D	-21 / 0	-112.4	-112.4	0.16 (1)			
F-E	0 / 197	-18.5	-18.5	0.19 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.16/1.00 (B-C:1), BC=0.19/1.00 (E-F:4), WB=0.09/1.00 (C-E:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

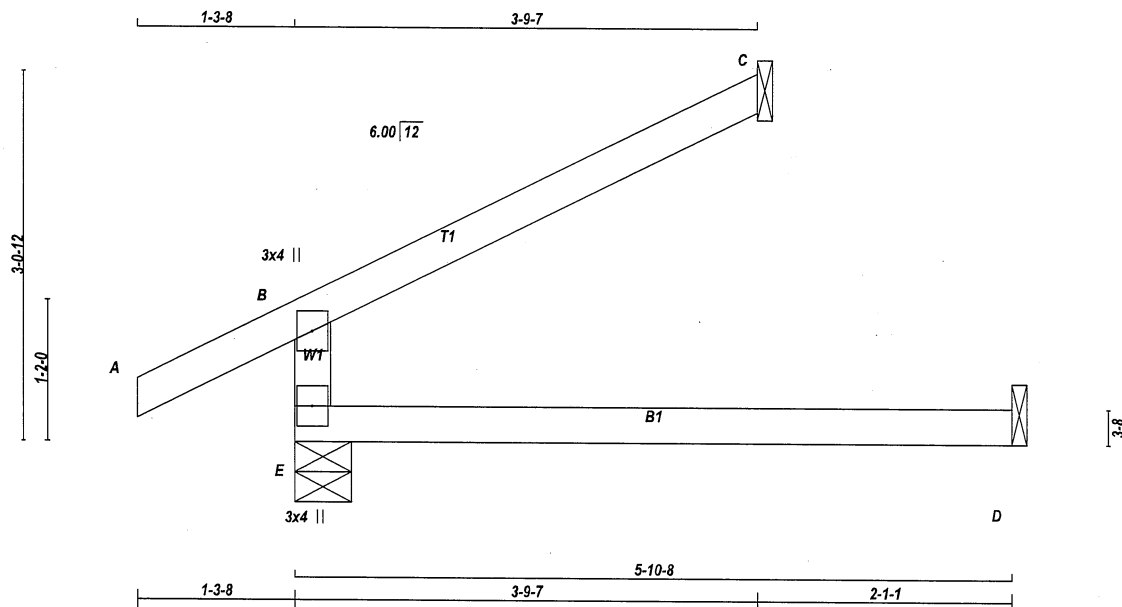


Structural component only
DWG# T-2215400

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
423632	J60	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 14 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			SPF
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP
E	482	0	482	0
C	160	0	160	0
D	45	0	45	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	337	242 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	109	92 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED
	MEMB.	FORCE	VERT. LOAD	LC1	MAX	MEMB.	FORCE	MAX	MEMB.
		(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
	FR-TO		FROM	TO	LENGTH	FR-TO			
E-B	-418 / 0	0.0	0.0	0.13 (4)	7.81				
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00				
B-C	-24 / 0	-112.4	-112.4	0.27 (1)	6.25				
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.27/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (E) (INPUT = 0.90)
JSI METAL = 0.12 (B) (INPUT = 1.00)

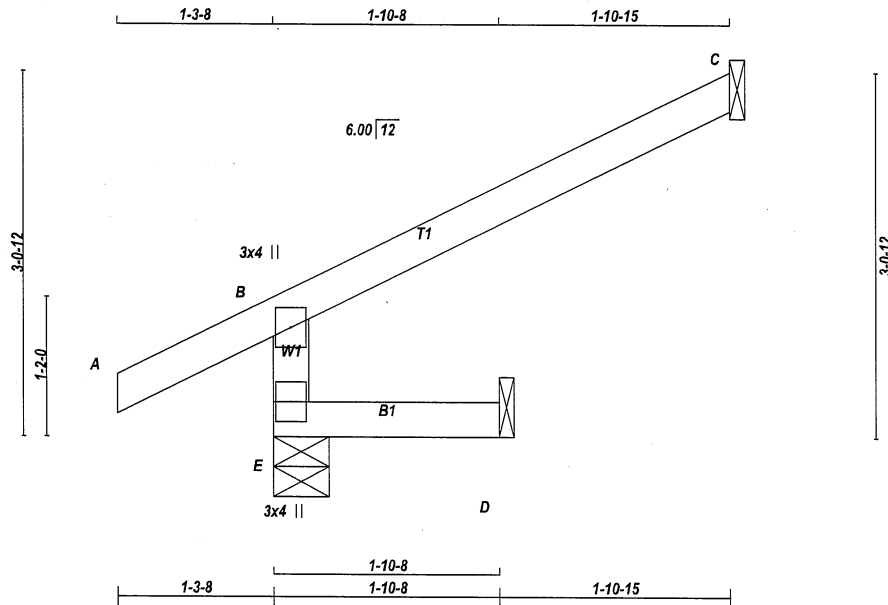


Structural component only
DWG# T-2215401

REVIEWED

JOB NAME 423632	TRUSS NAME J62	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:18.4

TOTAL WEIGHT = 10 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:			
CHORDS			SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL = 32.5 PSF			
E - B	2x4	DRY	No.2	VERT	DOWN	HORZ	UPLIFT	DL = 6.0 PSF			
A - C	2x4	DRY	No.2					BOT CH. LL = 0.0 PSF			
E - D	2x4	DRY	No.2					DL = 7.4 PSF			
DRY: SEASONED LUMBER.								TOTAL LOAD = 45.9 PSF			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	302	242 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	109	92 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-418 / 0	0.0	0.0 (4)	7.81			
A-B	0 / 34	-112.4	-112.4 (1)	10.00			
B-C	-24 / 0	-112.4	-112.4 (1)	6.25			
E-D	0 / 0	-18.5	-18.5 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")
CSI: TC=0.27/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.19/1.00 (B-C:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

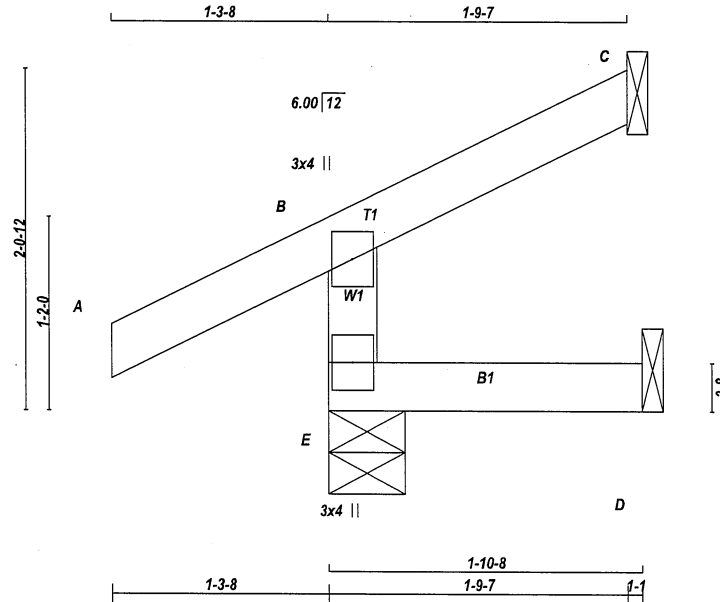


Structural component only
DWG# T-2215403

REVIEWED

JOB NAME 423632	TRUSS NAME J63	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:13.3

TOTAL WEIGHT = 7 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF	SPF	SPECIFIED LOADS:	TOP CH.	LL	PSF
E - B	2x4	DRY	No.2					DL = 6.0	PSF		
A - C	2x4	DRY	No.2					BOT CH.	LL	0.0	PSF
E - D	2x4	DRY	No.2					DL = 7.4	PSF		
DRY: SEASONED LUMBER.								TOTAL LOAD	=	45.9	PSF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
E	328	0	328	0	0	5-8	5-8		
C	54	0	54	0	-30	1-8	1-8		
D	6	0	17	0	-5	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	226	179 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0	0 / 0
C	37	31 / -23	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0	0 / 0
D	6	0 / -10	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (PLF)	MAX. CSI (LC)	UNBRAC LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. CSI (LC)
E-B	-299 / 0	0.0	0.0	0.05 (5)	7.81		
A-B	0 / 34	-112.4	-112.4	0.15 (1)	10.00		
B-C	-21 / 0	-112.4	-112.4	0.11 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

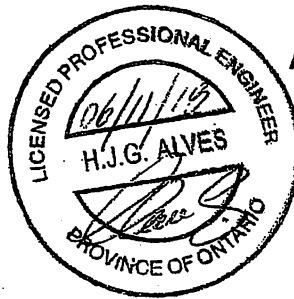
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2215404

REVIEWED



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes.

T-1300213

Feb 09, 2018

REVIEWED

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
MAXIMUM NUMBER OF TOE-NAILS					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

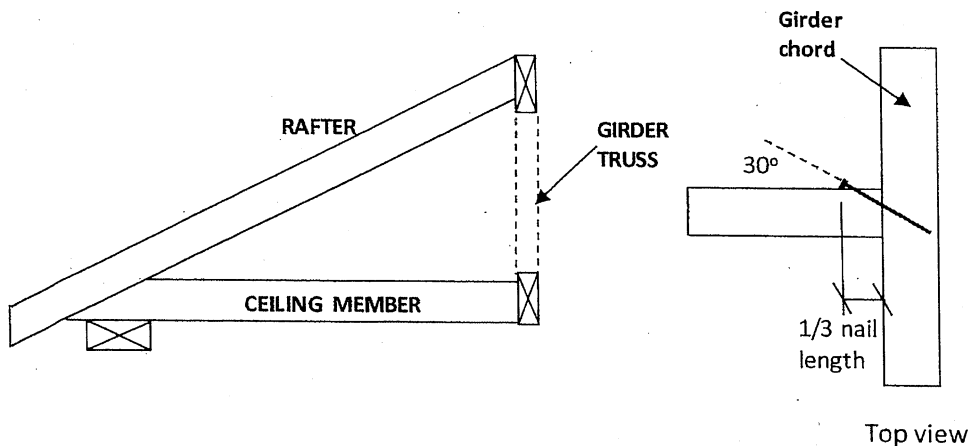


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

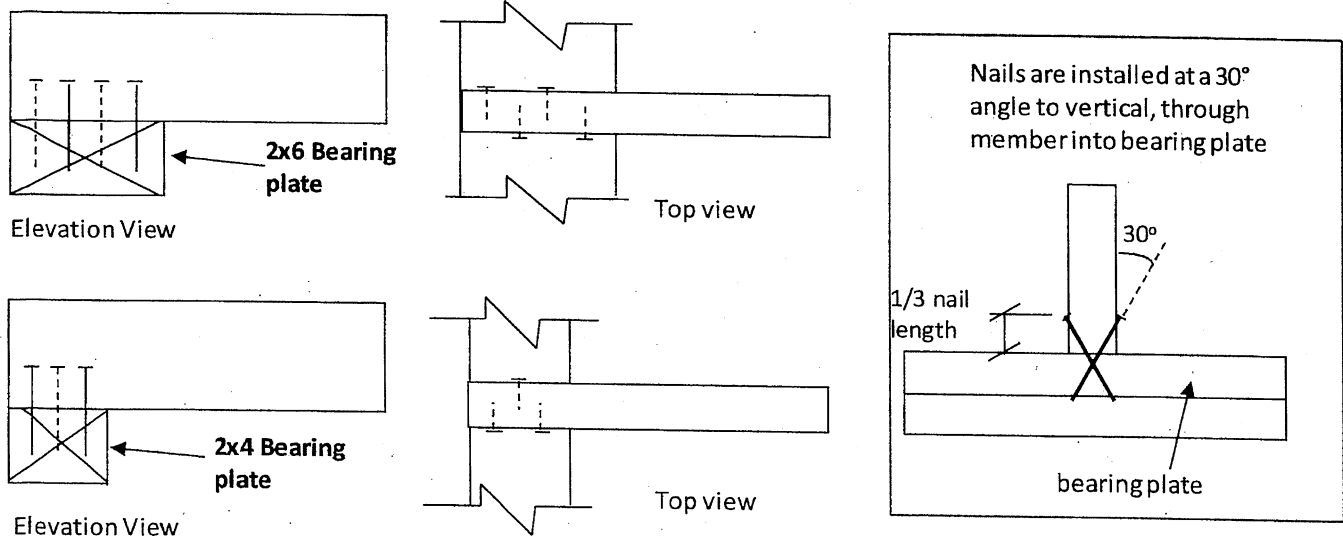
PEO
Certificate No. 10889485



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

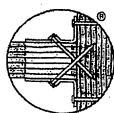
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear
Nailing
Top View

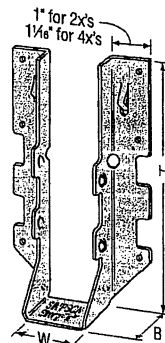
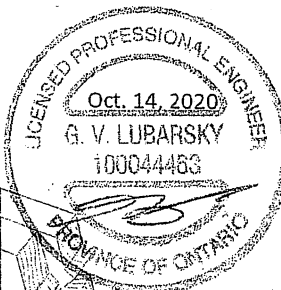
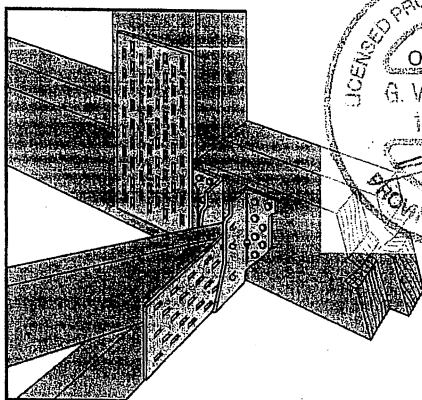


Double-Shear
Nailing
Side View;
Do not
bend tab

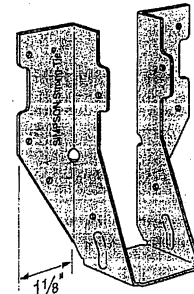


Dome Double-Shear
Nailing
Side View
(available on
some models)

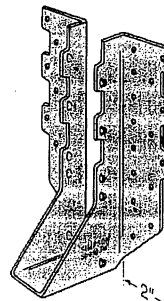
Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



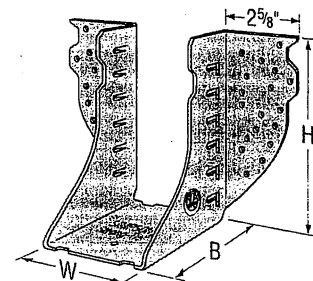
✓ LUS28



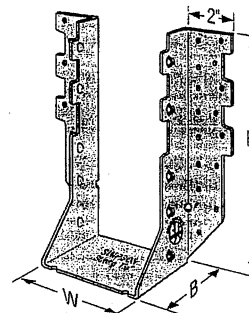
LU26L



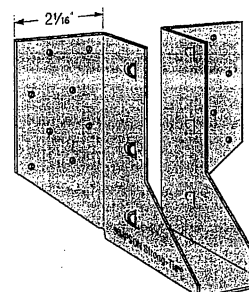
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



LJS26DS

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

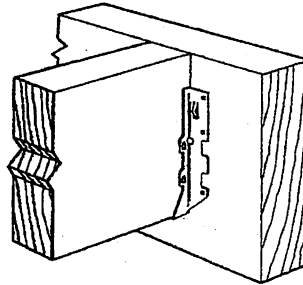
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

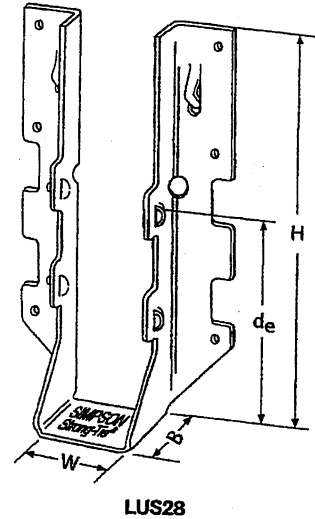
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

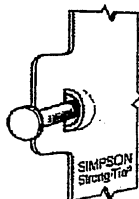


Typical LUS Installation

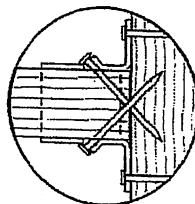


Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₀ ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1 1/8	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d		710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d		835	2020	590	1435
LUS26	18	1 1/8	4 1/8	1 3/4	3 5/8	(4) 10d	(4) 10d		1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d		1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/8	2	3 1/4	(4) 16d	(4) 16d		1720	2595	1545	2340
LUS28	18	1 1/8	6 1/8	1 3/4	3 3/4	(6) 10d	(6) 10d		1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d		1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/8	2	3 1/4	(6) 16d	(4) 16d		1720	3325	1545	2375
LUS210	18	1 1/8	7 13/16	1 3/4	3 3/8	(8) 10d	(4) 10d		1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d		2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/8	2	5 1/4	(8) 16d	(6) 16d		2580	3345	2320	2375

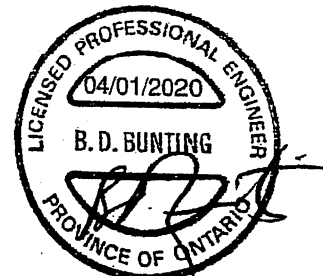
1. d₀ is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

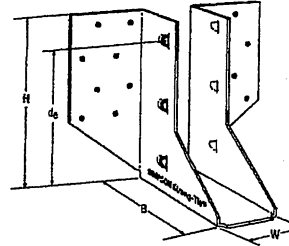
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

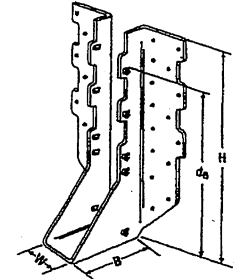
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

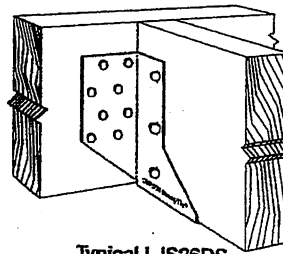
- See current catalogue for options



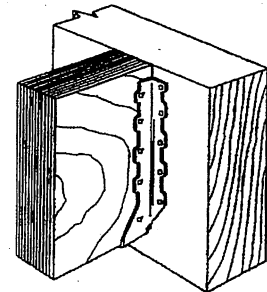
LJS26DS



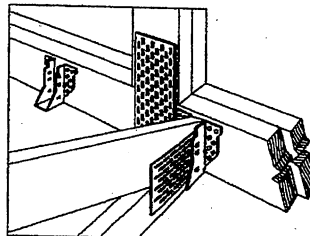
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



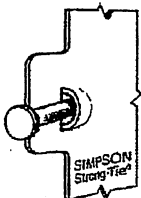
Typical HUS
Installation



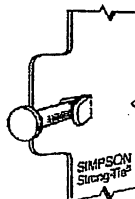
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 3/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 1/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 3/8	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

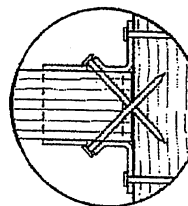
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



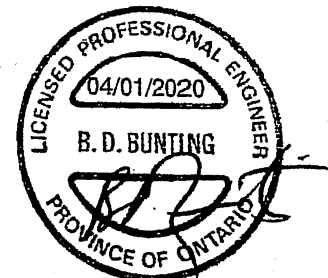
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

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HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

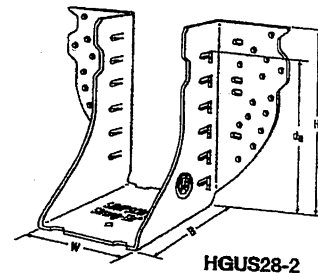
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

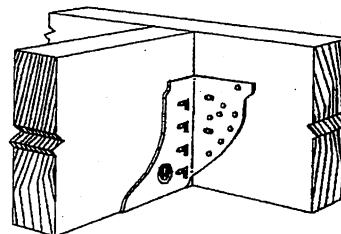
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

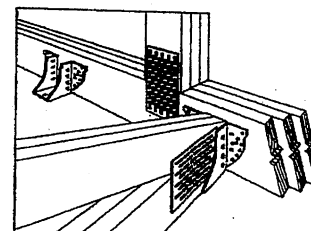
- See current catalogue for options



HGUS28-2



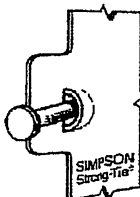
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

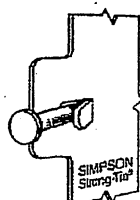
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _g =1.15)	(K _g =1.00)	(K _g =1.15)	(K _g =1.00)
HGUS26	12	1 1/4	5 3/4	5	4 5/8	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/4	7 1/4	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/16	7 1/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 1/4	9 1/4	5	7 7/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 1/16	9 1/16	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/16	10 3/4	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/16	12 3/4	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

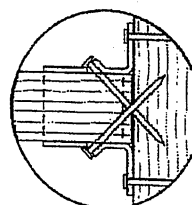


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

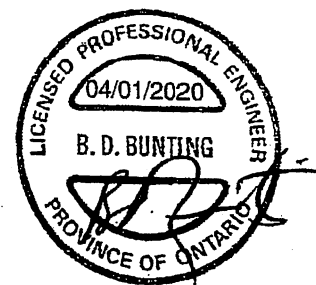
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

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REVIEWED

TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

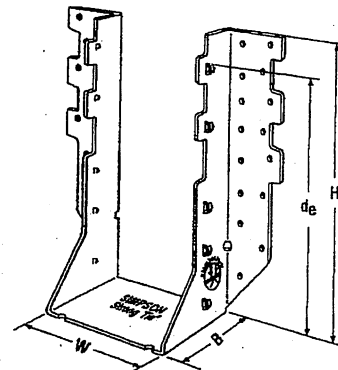
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

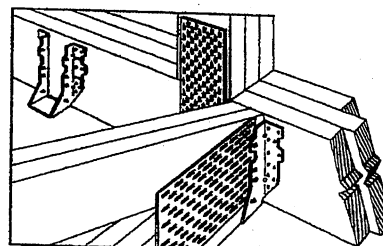
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

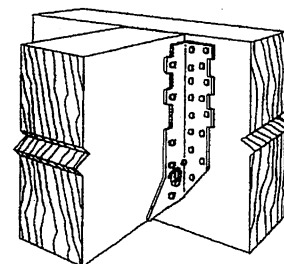
- See current catalogue for options



HHUS410



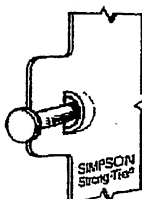
Typical HHUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)



Typical HHUS Installation

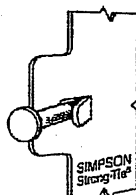
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
HHUS26-2	14	3 3/8	5 1/8	3	3 15/16	(14) 16d	(8) 16d	2850	7335	2065	5205
HHUS28-2	14	3 3/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 3/8	9 3/2	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 1/8	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 3/8	5 1/2	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 3/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 3/8	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 3/8	7 29/32	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_e is the distance from the seat of the hanger to the highest joist nail.

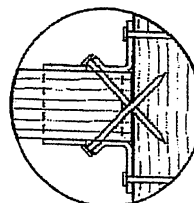


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

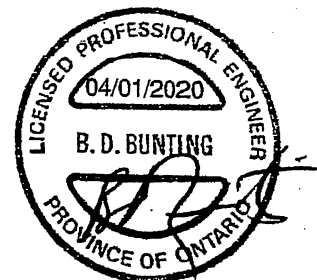
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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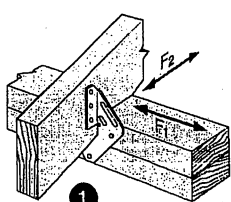
T-SPECHHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

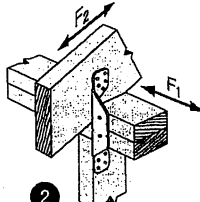
REVIEWED

H/TSP

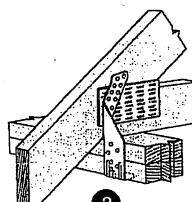
Seismic and Hurricane Ties (cont.)



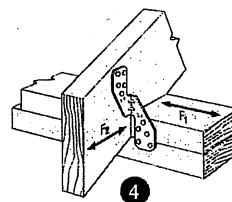
1 H1 Installation



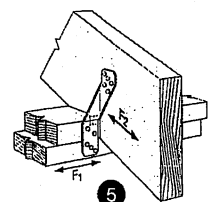
2 H2A Installation



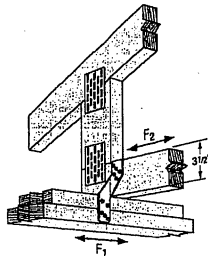
3 TSP Installation



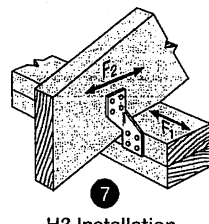
4 H2.5A Installation
(Nails into both top plates)



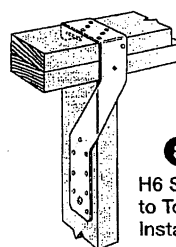
5 H2.5T Installation
(Nails into both top plates)



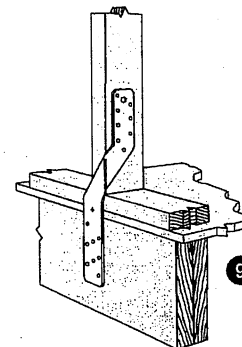
6 H2.5T Installation



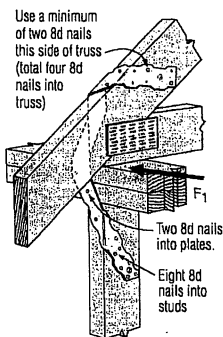
7 H3 Installation
(Nails into upper top plate)



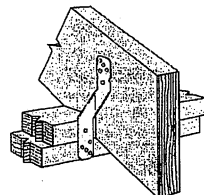
8 H6 Stud to Top Plate Installation



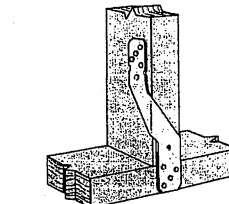
9 H6 Stud to Band Joist Installation



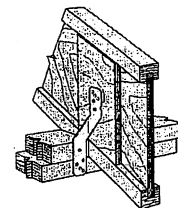
10 H7Z Installation



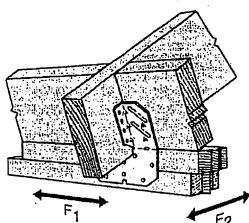
11 H8 Attaching Rafter to Double Top Plates



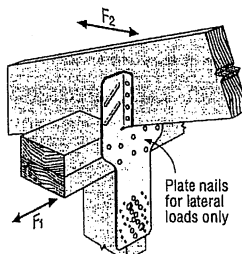
12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)



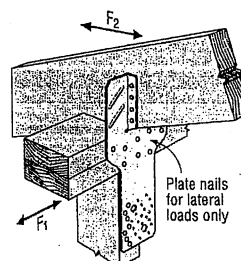
13 H8 attaching I-Joist to Double Top Plates



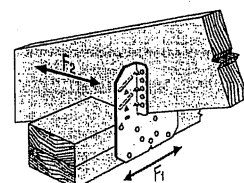
14 H10A Field-Bent Installation



15 H10S Installation

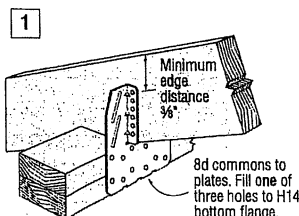


16 H10S Installation with Stud Offset

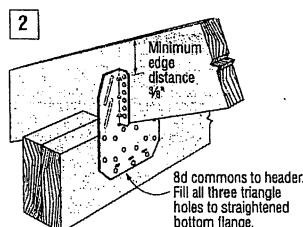


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

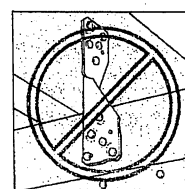


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

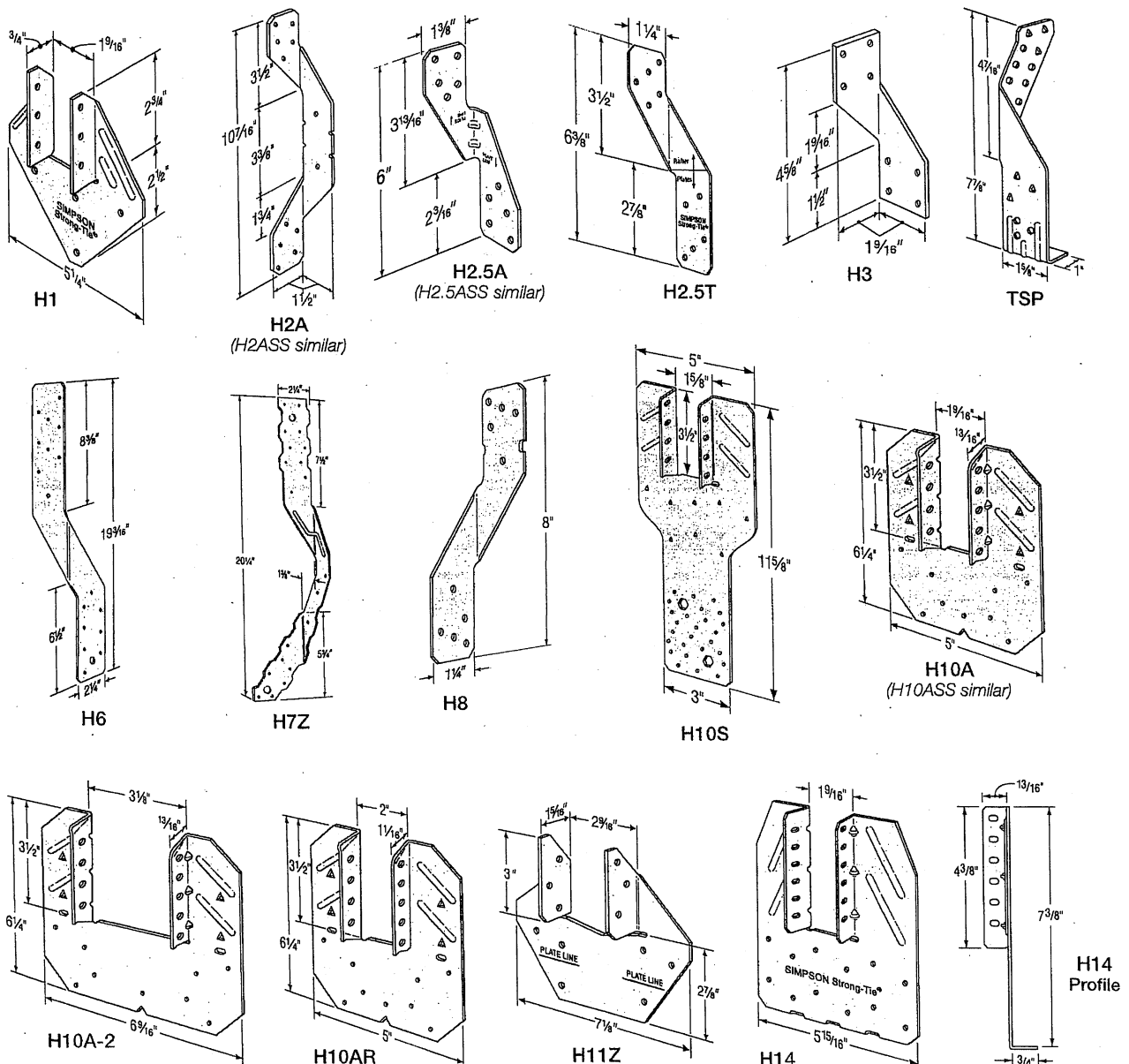
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp.20–24 or visit strongtie.com.

Installation:

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12; use H10A sloped loads for field bent installation.



TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

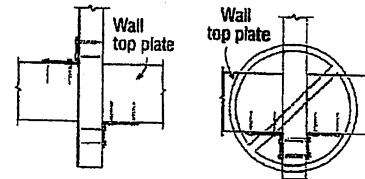
Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

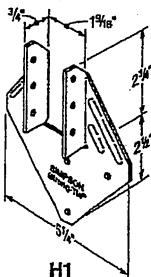
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

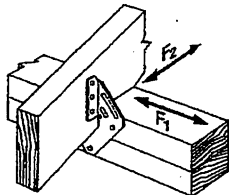


Install diagonally across from each other for minimum 2x truss.

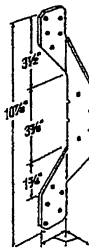
Nailing into both sides of a single ply 2x truss may cause the wood to split.



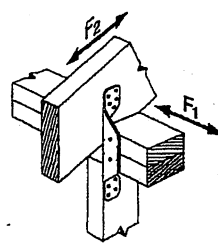
H1



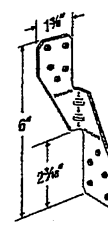
H1 Installation



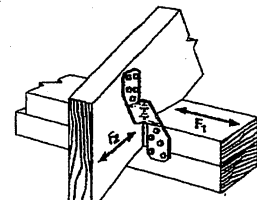
H2A



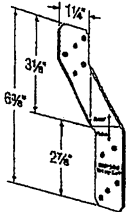
H2A Installation



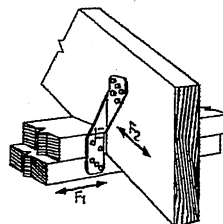
H2.5A



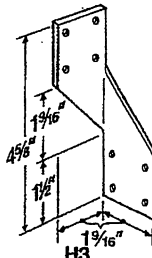
H2.5A Installation



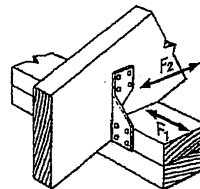
H2.5T



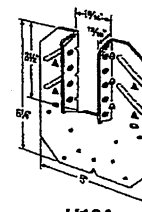
H2.5T Installation
(Nails into both top plates)



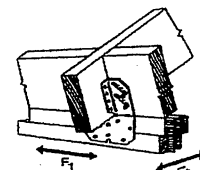
H3



H3 Installation



H10A



H10A Installation

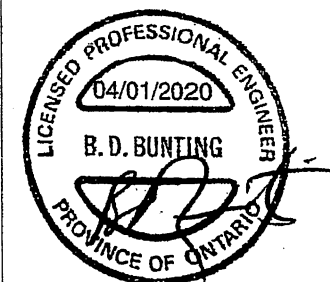
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
		To Rafter	To Plates	To Studs	D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
						(K _c =1.15)			(K _c =1.15)	
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

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T-SPECH20 3/20 exp. 6/22

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REVIEWED

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

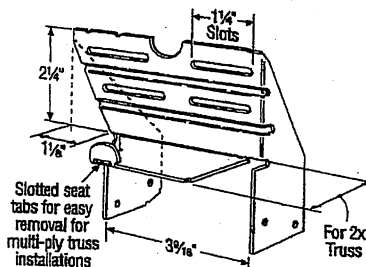
Design: Factored resistances are in accordance with CSA 086-14

Installation:

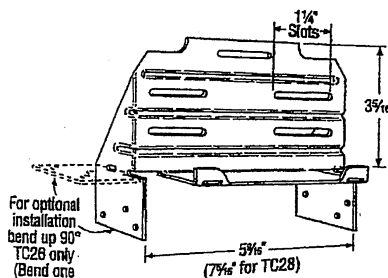
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

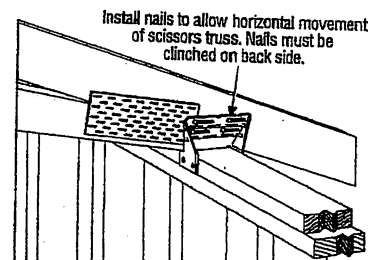
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



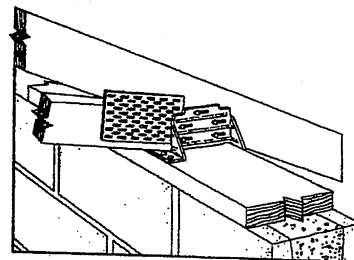
TC24
U.S. Patent 4,932,173



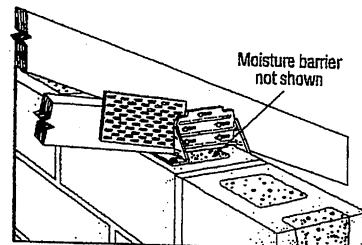
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



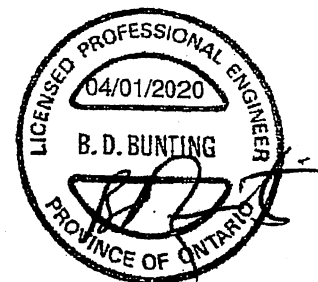
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L (K _o =1.15)	S-P-F (K _o =1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L (K _o =1.15)	S-P-F (K _o =1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1 1/2"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - 3/8" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



**LIMIT
STATES
DESIGN**

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T-SPECTC20 3/20 exp. 6/22

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HTU

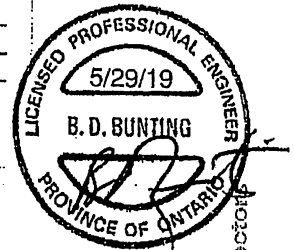
Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (In.)	Minimum Header Size	Fasteners		Factored Resistance			
			Header	Joist	D.Fir-L		S-P-F	
					Uplift	Normal	Uplift	Normal
					(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
HTU26 (Min.)	3½	(2) 2x4	(10) 16d	(14) 10d x 1½"	1740	3340	1235	2370
					7.74	14.86	5.49	10.54
HTU26 (Max.)	5½	(2) 2x4	(10) 16d	(20) 10d x 1½"	2470	4015	1755	2850
					10.99	17.86	7.81	12.68
HTU28 (Max.)	3½	(2) 2x6	(20) 16d	(26) 10d x 1½"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19
HTU210 (Max.)	7¼	(2) 2x6	(20) 16d	(32) 10d x 1½"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19

See table footnotes on p. 260.

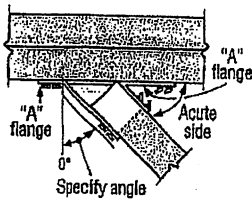


Hanger Options

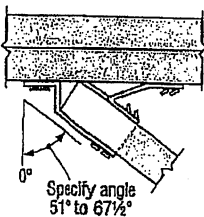
See Hanger Options Information on pp. 125-127.

Skewed Seat

- Skewable up to 67½°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
		Header	Joist	D.Fir-L		S-P-F	
				Uplift	Normal	Uplift	Normal
				(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
				lbs	lbs	lbs	lbs
				kN	kN	kN	kN
HTU26	< 51	(20) 16d	(14) 10d x 1½"	1835	4110	1300	2905
	51-67½	(20) 16d	(12) 10d x 1½"	8.16	18.28	5.78	12.92
HTU28	< 51	(26) 16d	(20) 10d x 1½"	1350	3620	965	2560
	51-67½	(26) 16d	(17) 10d x 1½"	6.01	18.10	4.25	11.39
HTU210	< 51	(32) 16d	(26) 10d x 1½"	2810	4270	1985	3030
	51-67½	(32) 16d	(22) 10d x 1½"	12.50	18.99	8.83	13.48
HTU26-2	< 51	(20) 16d	(14) 10d	2075	3930	1465	2780
	51-67½	(20) 16d	(12) 10d	9.23	17.48	6.52	12.37
HTU28-2	< 51	(26) 16d	(20) 10d	3785	4430	2675	3135
	51-67½	(26) 16d	(17) 10d	16.84	19.71	11.90	13.95
HTU210-2	< 51	(32) 16d	(26) 10d	2795	4240	1980	3000
	51-67½	(32) 16d	(22) 10d	12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
	51-67½	(20) 16d	(12) 10d	9.52	16.53	6.74	11.68
HTU28-2	< 51	(26) 16d	(20) 10d	1610	3920	1140	2785
	51-67½	(26) 16d	(17) 10d	7.16	17.44	5.07	12.39
HTU210-2	< 51	(32) 16d	(26) 10d	3960	5425	2815	3855
	51-67½	(32) 16d	(22) 10d	17.62	24.13	12.52	17.15
HTU26-2	< 51	(20) 16d	(14) 10d	2385	5425	1895	3855
	51-67½	(20) 16d	(12) 10d	10.61	24.13	7.54	17.15
HTU28-2	< 51	(26) 16d	(20) 10d	5025	6890	3570	4890
	51-67½	(26) 16d	(17) 10d	22.35	30.65	15.88	21.75
HTU210-2	< 51	(32) 16d	(26) 10d	3145	6680	2225	4745
	51-67½	(32) 16d	(22) 10d	13.99	29.72	9.90	21.10

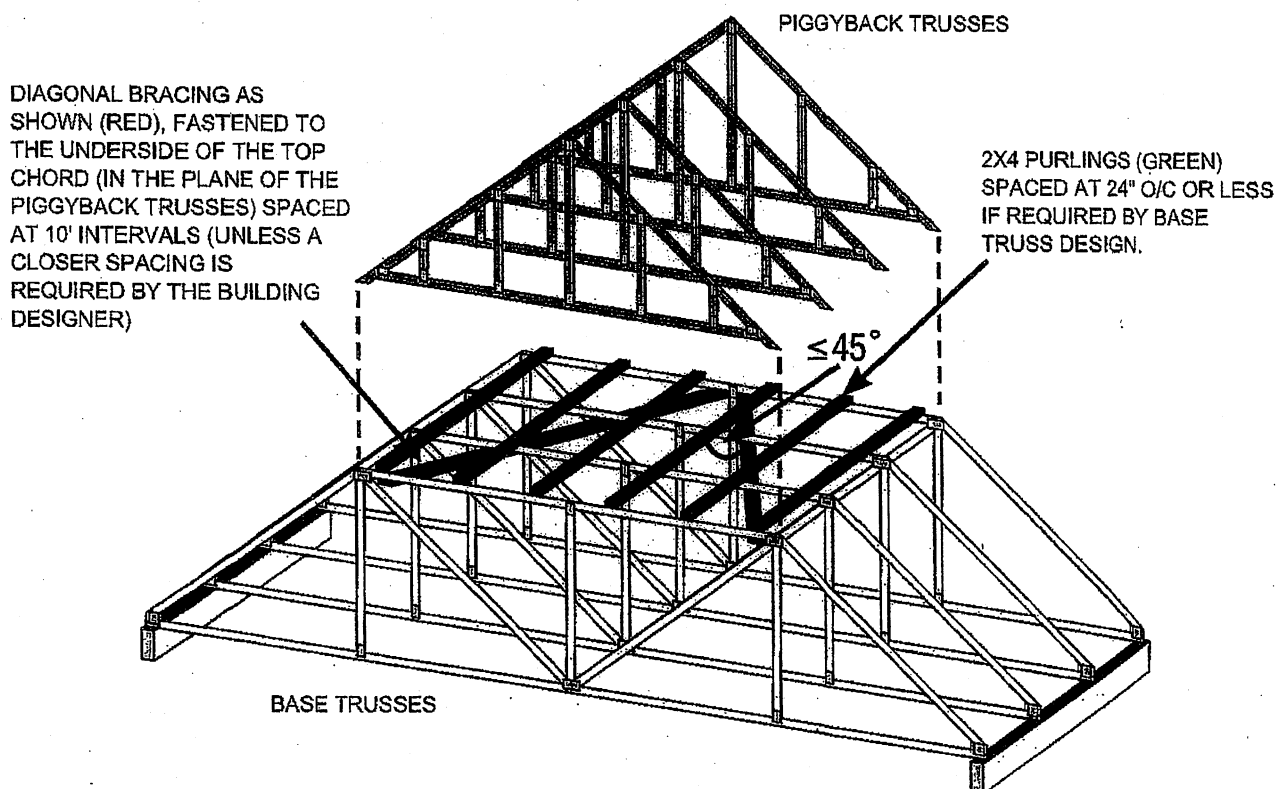
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3½" long, 10d x 1½" = 0.148" dia. x 1½" long, 10d = 0.148" dia. x 3" long. See pp. 27-28 for other nail sizes and information.

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

REVIEWED

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

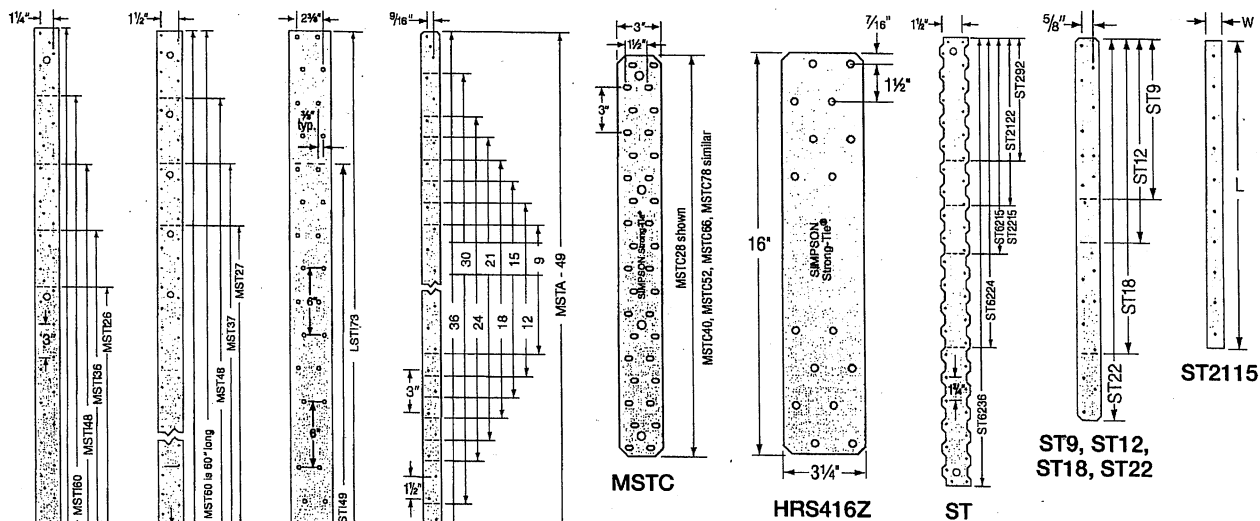
MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

Installation: Use all specified fasteners; see General Notes

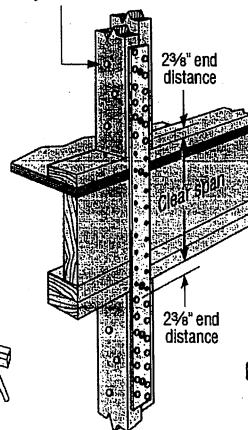
Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

Straps and Ties

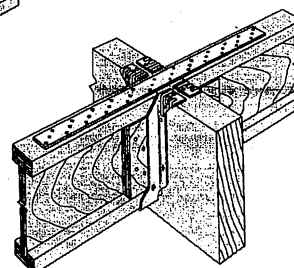


LSTA and MSTA
(Pilot holes not shown)

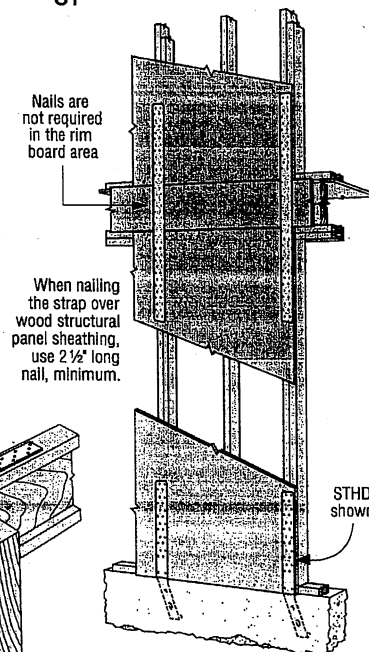
Stitch nailing
of double studs
by others



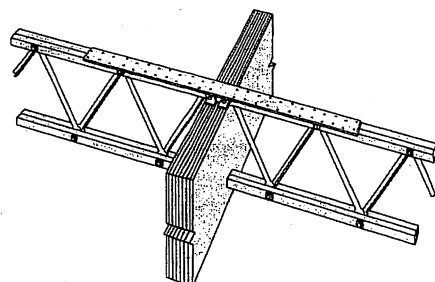
Floor-to-Floor Tie
Installation
Showing a
Clear Span



Typical MSTI Installation
(MIT hanger shown)
LSTI similar



Typical Detail with
Strap Installed over
Wood Structural Panel
Sheathing



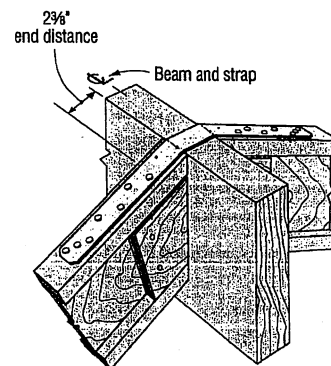
Typical LSTI Installation

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI
Strap Ties (cont.)

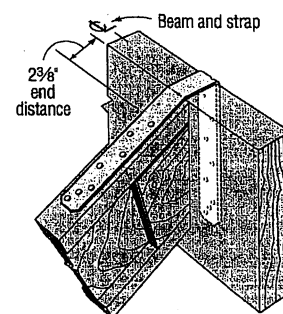
These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
		W	L		D.Fir-L		S-P-F	
					(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
LSTA9	20	1¼	9	(6) 10d	600	690	555	635
LSTA12		1¼	12	(8) 10d	2.67	3.07	2.47	2.82
LSTA15		1¼	15	(10) 10d	800	920	735	845
LSTA18		1¼	18	(12) 10d	3.56	4.09	3.27	3.76
LSTA21		1¼	21	(14) 10d	1000	1150	920	1060
LSTA24		1¼	24	(16) 10d	4.45	5.12	4.09	4.72
ST292		2½	9¾	(8) 8d	1200	1380	1105	1270
ST2122		2½	12¼	(12) 8d	5.34	6.14	4.92	5.65
ST2115		¾	16¾	(8) 8d	1400	1610	1290	1485
ST2215		2½	16¾	(16) 8d	6.23	7.16	5.74	6.61
LSTA30	18	1¼	30	(20) 10d	1600	1840	1475	1695
LSTA36		1¼	36	(24) 10d	7.12	8.19	6.56	7.54
LSTI49		3¾	49	(32) 10d x 1½"	585	675	535	615
LSTI73		3¾	73	(48) 10d x 1½"	2.60	3.00	2.38	2.74
MSTA9		1¼	9	(6) 10d	940	1085	865	995
MSTA12		1¼	12	(8) 10d	4.18	4.83	3.85	4.43
MSTA15		1¼	15	(10) 10d	670	770	615	710
MSTA18		1¼	18	(12) 10d	2.98	3.43	2.74	3.16
MSTA21		1¼	21	(14) 10d	1335	1540	1235	1420
MSTA24		1¼	24	(16) 10d	5.94	6.85	5.49	6.32
MSTA30	16	1¼	30	(20) 10d	2235	2465	2075	2385
MSTA36		1¼	36	(24) 10d	9.94	10.97	9.23	10.61
MSTA49		1¼	49	(28) 8d	2465	2465	2465	2465
ST6215		2½	16¾	(16) 8d	10.97	10.97	10.97	10.97
ST6224		2½	23¾	(24) 8d	3115	3580	2852	3280
ST9		1¼	9	(6) 8d	13.86	15.93	12.69	14.59
ST12		1¼	11¾	(8) 8d	4670	5370	4280	4920
ST18		1¼	17¾	(12) 8d	20.77	23.89	19.04	21.89
ST22		1¼	21¾	(18) 8d	670	770	625	715
					2.98	3.43	2.78	3.18



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.

REVIEWED

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

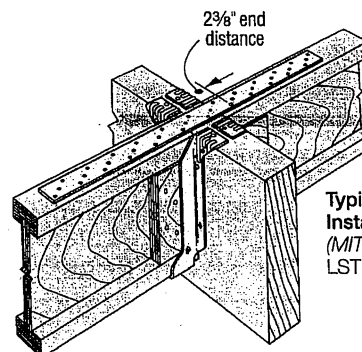
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

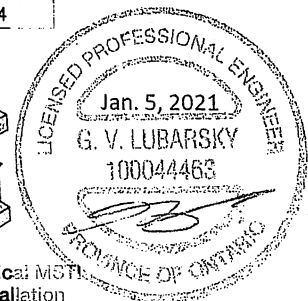
SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (In.)		Fasteners (Total)	Factored Tensile Resistance			
		W	L		D.Fir-L		S-P-F	
					(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
MSTC28	16	3	28¼	(32) 10d	3955	4545	3615	4155
					17.59	20.22	16.08	18.48
MSTC40		3	40¼	(48) 10d	5930	6820	5420	6235
					26.38	30.34	24.11	27.74
MSTC52		3	52¼	(54) 10d	6670	6940	6100	6940
					29.67	30.87	27.14	30.87
MSTC66	14	3	65¼	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
MSTC78		3	77¼	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
ST6236		2⅝	33⅞	(36) 8d	3735	4295	3270	3760
					16.61	19.11	14.55	16.73
MSTI26	12	2⅝	26	(22) 10d x 1 ½"	2825	3250	2475	2850
					12.57	14.46	11.01	12.68
MSTI36		2⅝	36	(32) 10d x 1 ½"	4110	4725	3600	4140
					18.28	21.02	16.01	18.42
MSTI48		2⅝	48	(44) 10d x 1 ½"	5650	6500	4955	5695
					25.13	28.91	22.04	25.33
MSTI60		2⅝	60	(56) 10d x 1 ½"	7195	7360	6305	7250
					32.01	32.74	28.05	32.25
MSTI72		2⅝	72	(68) 10d x 1 ½"	7360	7360	7240	7360
					32.74	32.74	32.21	32.74
MST27		2⅝	27	(26) 8d	2685	3090	2355	2710
					11.94	13.75	10.48	12.06
MST37		2⅝	37½	(38) 8d	3930	4515	3440	3960
					17.48	20.08	15.30	17.62
MST48		2⅝	48	(50) 8d	5170	5945	4530	5210
					23.00	26.45	20.15	23.18
HRS416Z		3¼	16	(16) ¼" x 1 ½" SDS	2400	2760	2120	2440
					10.68	12.28	9.43	10.85
MST60	10	2⅝	60	(64) 8d	6620	7610	5800	6670
					29.45	33.85	25.80	29.67
MST72		2⅝	72	(78) 8d	8065	9135	7065	8125
					35.88	40.64	31.43	36.14

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22–23 for other nail sizes and information.



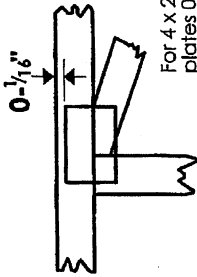
Typical MSTI Installation
(MIT hanger shown)
LSTI similar



Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

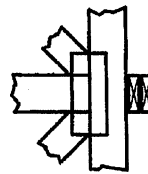
4 X 4
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, 1 or Eliminator bracing if indicated.

BEARING



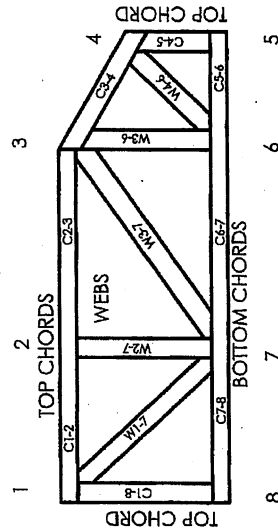
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

REVIEWED