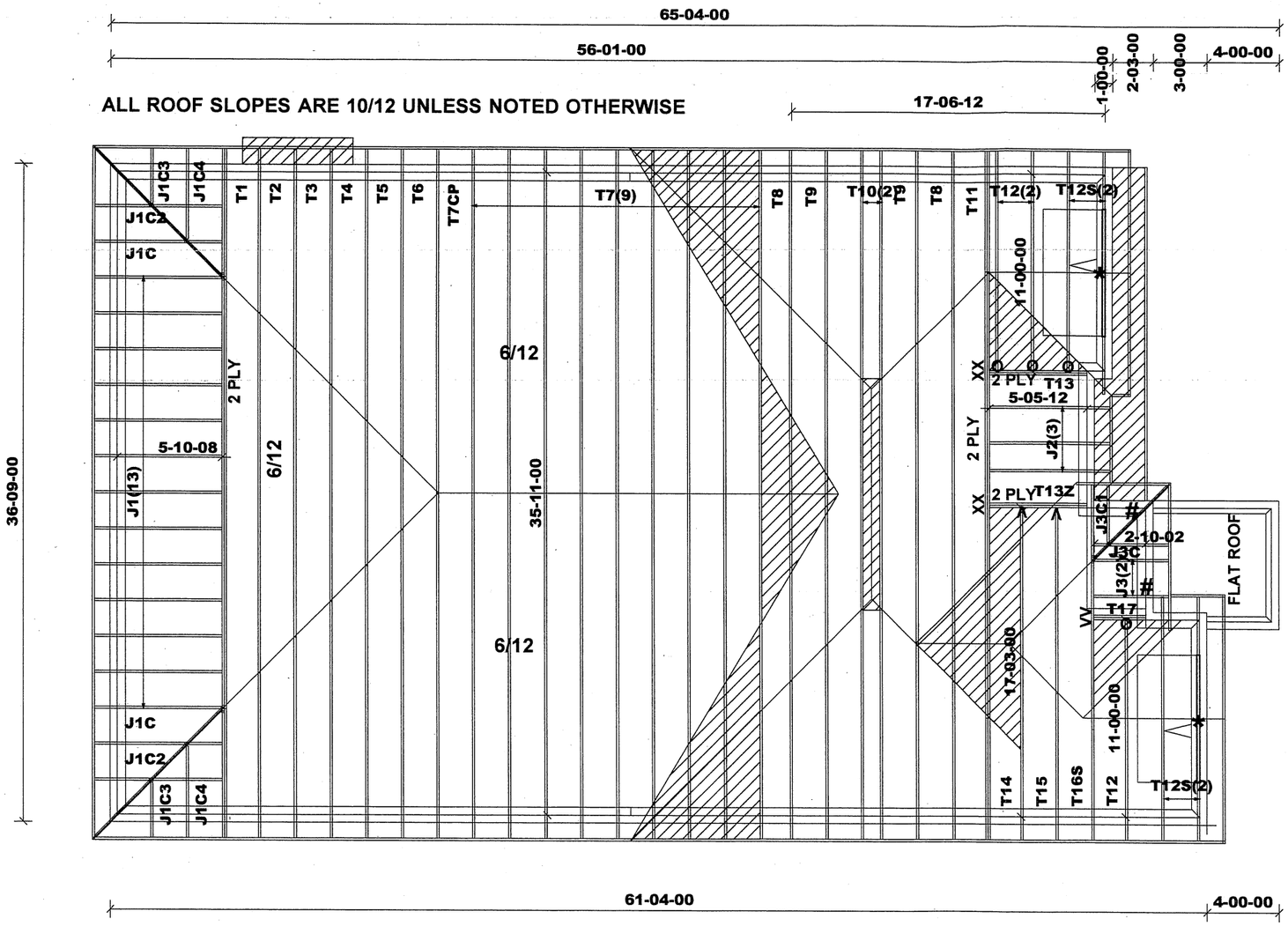
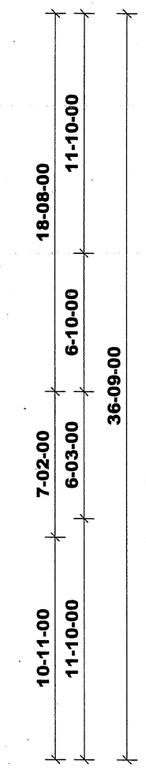
ALL ROOF SLOPES ARE 10/12 UNLESS NOTED OTHERWISE





ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)

DENOTES:

***: 12" RAISED CEILING**
#: 20" RAISED CEILING

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127208
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

DWG# T-2126452 to T-2126480, T1800218

M14217

TAMARACK ROOF TRUSSES INC.

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4505 / A STD (4 OR 5 BED)	Mitek ver 8.4.2.286
Layout ID: 420375	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 205032	Date: 2021-08-05 Designer: ND		

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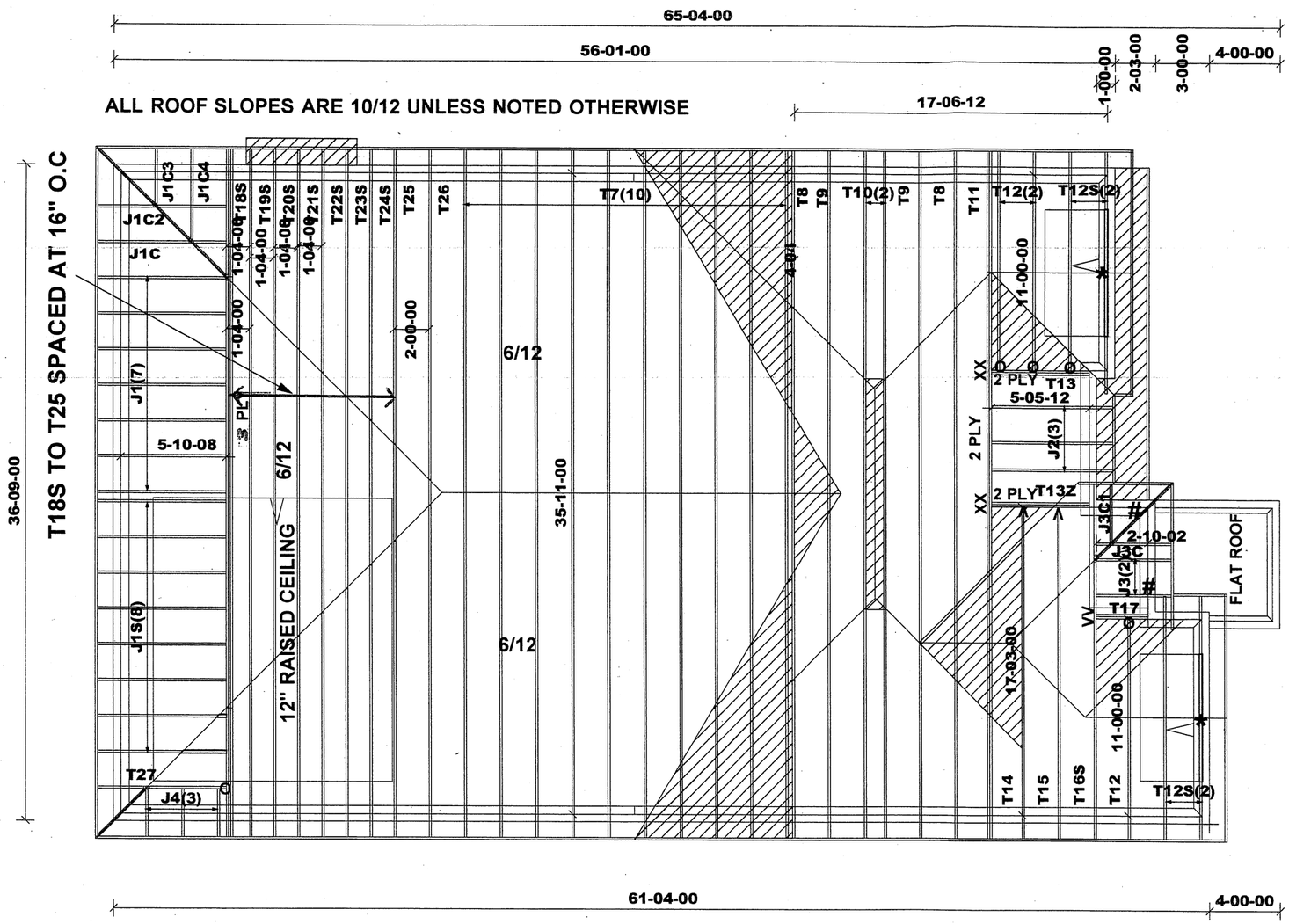
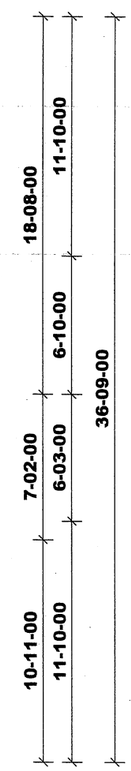
ALL ROOF SLOPES ARE 10/12 UNLESS NOTED OTHERWISE





ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)

DENOTES:

***: 12" RAISED CEILING**
#: 20" RAISED CEILING

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

LICENSED PROFESSIONAL ENGINEER
08/01/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127209
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

DWG# T-2126452 to T-2126460, T-2126467, T-2126469 to T-2126492, T1800218

M14217

TAMARACK ROOF TRUSSES INC.

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4505 / A OPT. COFF (4 BED)	Mitek ver 8.4.2.286
Layout ID: 420376	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 205032	Date: 2021-08-05 Designer: ND		

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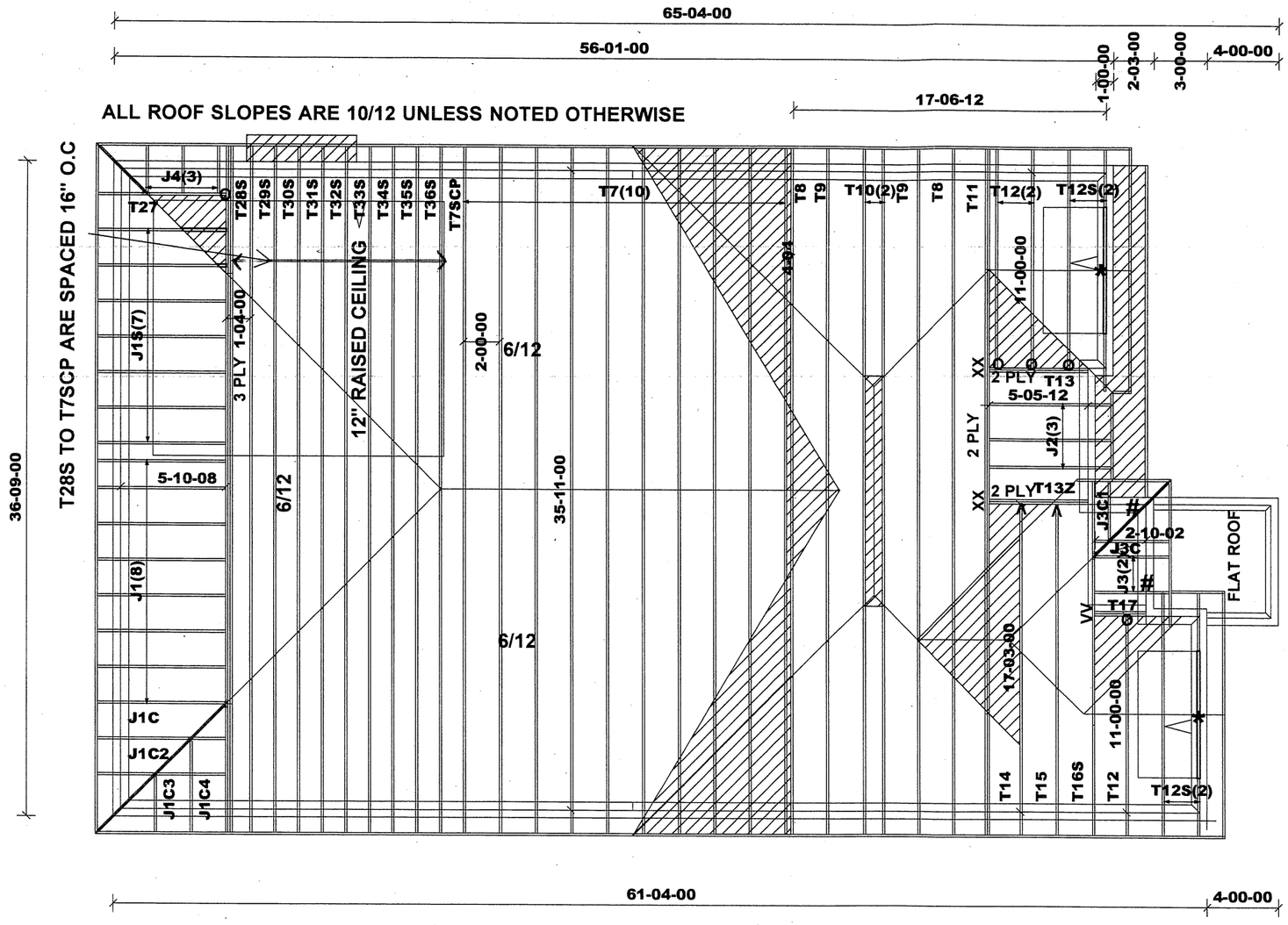

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)

DENOTES:

***: 12" RAISED CEILING**
#: 20" RAISED CEILING

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127210
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

DWG# T-2126452 to T-2126460, T-2126467, T-2126469 to T-2126482, T-2126492 to T-2126502, T1800218

M14217

TAMARACK ROOF TRUSSES INC.

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4505 / A OPT. COFF (5 BED)	Mitek ver 8.4.2.286
Layout ID: 420377	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 205032	Date: 2021-08-05 Designer: ND		

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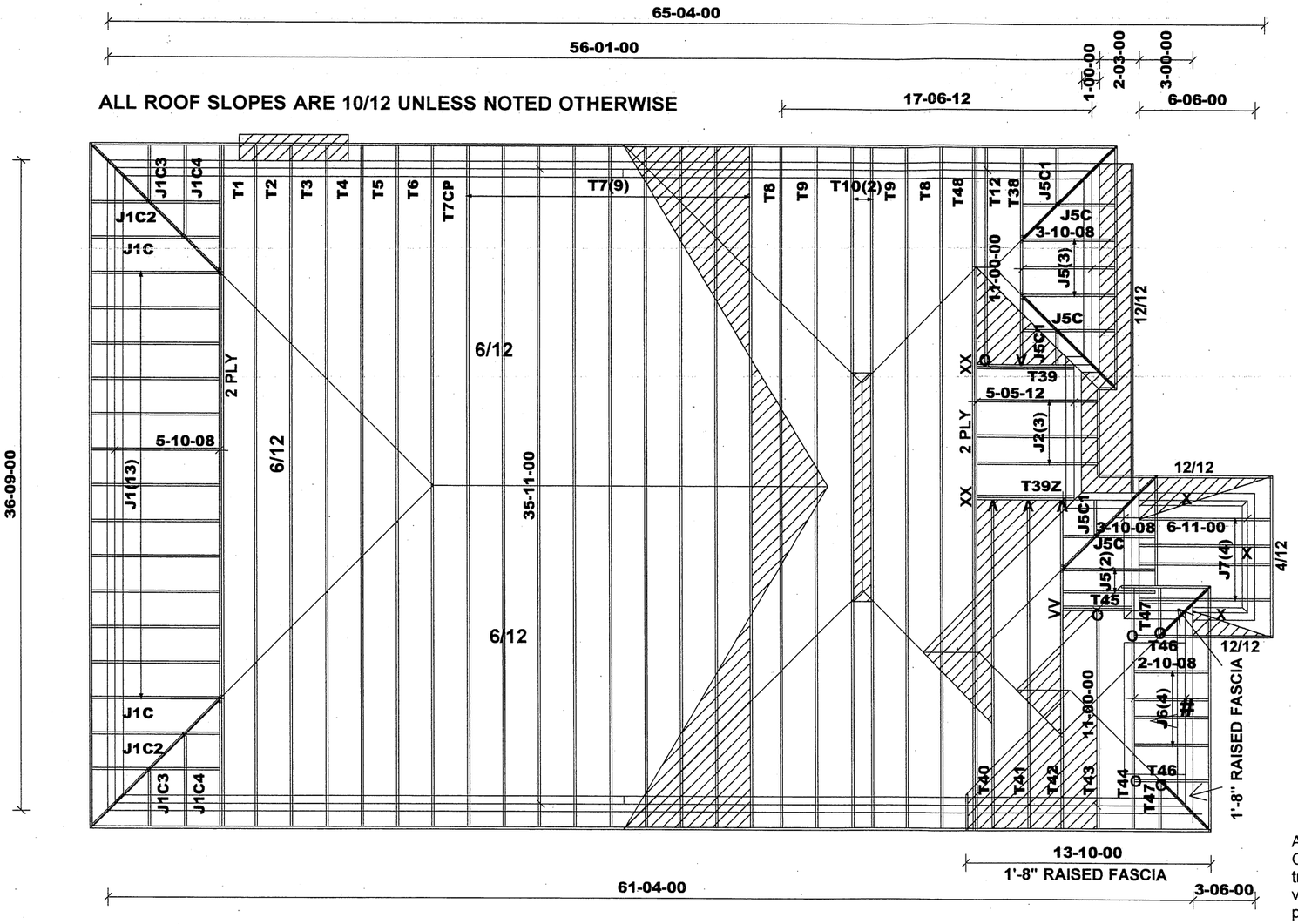
ALL ROOF SLOPES ARE 10/12 UNLESS NOTED OTHERWISE



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)

DENOTES:

#: 20" RAISED CEILING

X: DROPPED BEAM BY OTHERS

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

LICENSED PROFESSIONAL ENGINEER
09/10/21
H. J. G/ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127211
STRUCTURAL
COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

DWG# T-2126452 to T-2126457, T-2126461 to T-2126471, T-2126473, T-2126503 to T-2126519, T1800218

M1427

TAMARACK ROOF TRUSSES INC.

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4505 / B STD (4 OR 5 BED)	Mitek ver 8.4.2.286
Layout ID: 420378	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 205032	Date: 2021-08-05 Designer: ND		

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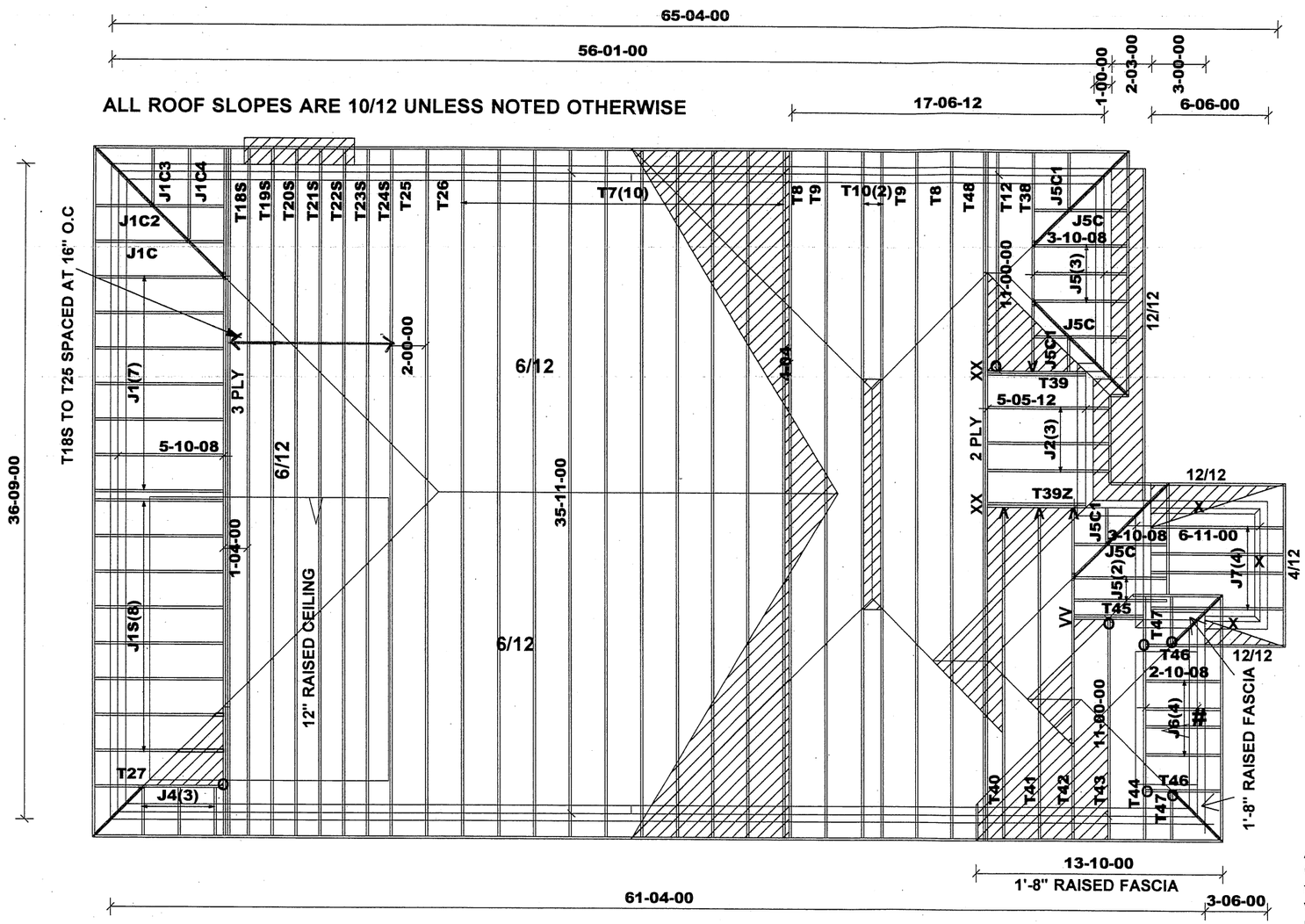
ALL ROOF SLOPES ARE 10/12 UNLESS NOTED OTHERWISE



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)

DENOTES:

#: 20" RAISED CEILING

X: DROPPED BEAM BY OTHERS

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. J. G. ALVES
100009021
PROVINCE OF ONTARIO

DWG NO. TAM T-2127212
STRUCTURAL
COMPONENT ONLY

DWG# T-2126452 to T-2126457, T-2126467 , T-2126469 to T-2126471, T-2126473, T-2126481 to T-2126492, T-2126503 to T-2126519, T1800218

TRUSS PLACEMENT PLAN.

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M14217

TAMARACK ROOF TRUSSES INC.

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4505 / B OPT. COFF (4 BED)	Mitek ver 8.4.2.286
Layout ID: 420379	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 205032	Date: 2021-08-05 Designer: ND		

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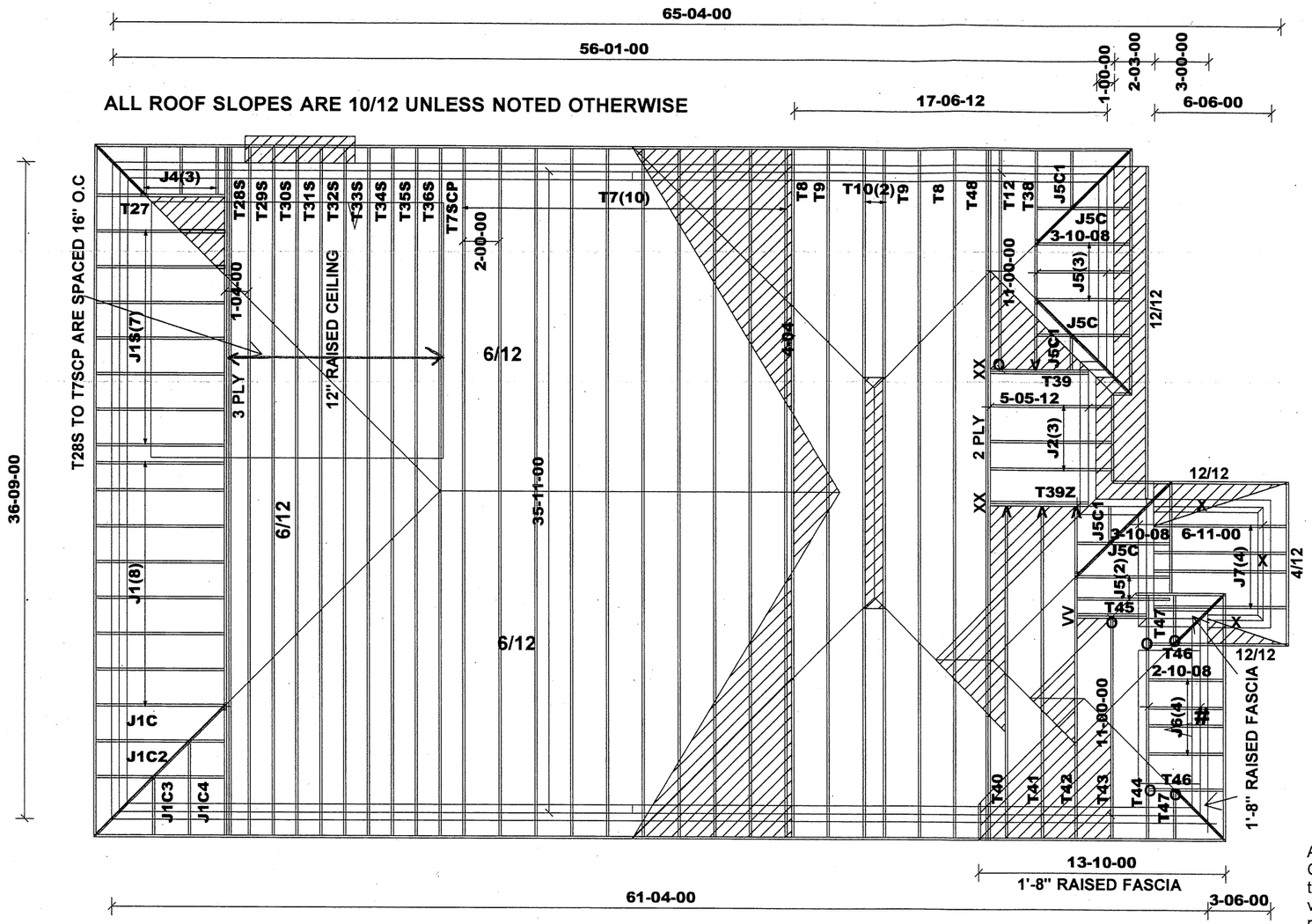

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)

DENOTES:

#: 20" RAISED CEILING

X: DROPPED BEAM BY OTHERS

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. J. GALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127213
STRUCTURAL
COMPONENT ONLY

DWG# T-2126452 to T-2126457, T-2126467 , T-2126469 to T-2126471, T-2126473, T-2126481 to T-2126482, T-2126492 to T-2126519, T1800218

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

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M14217

TAMARACK ROOF TRUSSES INC.
ALBA-LOWER GROUP

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4505 / B OPT. COFF (5 BED)	Mitek ver 8.4.2.286
Layout ID: 420380	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 205032	Date: 2021-08-05 Designer: ND		

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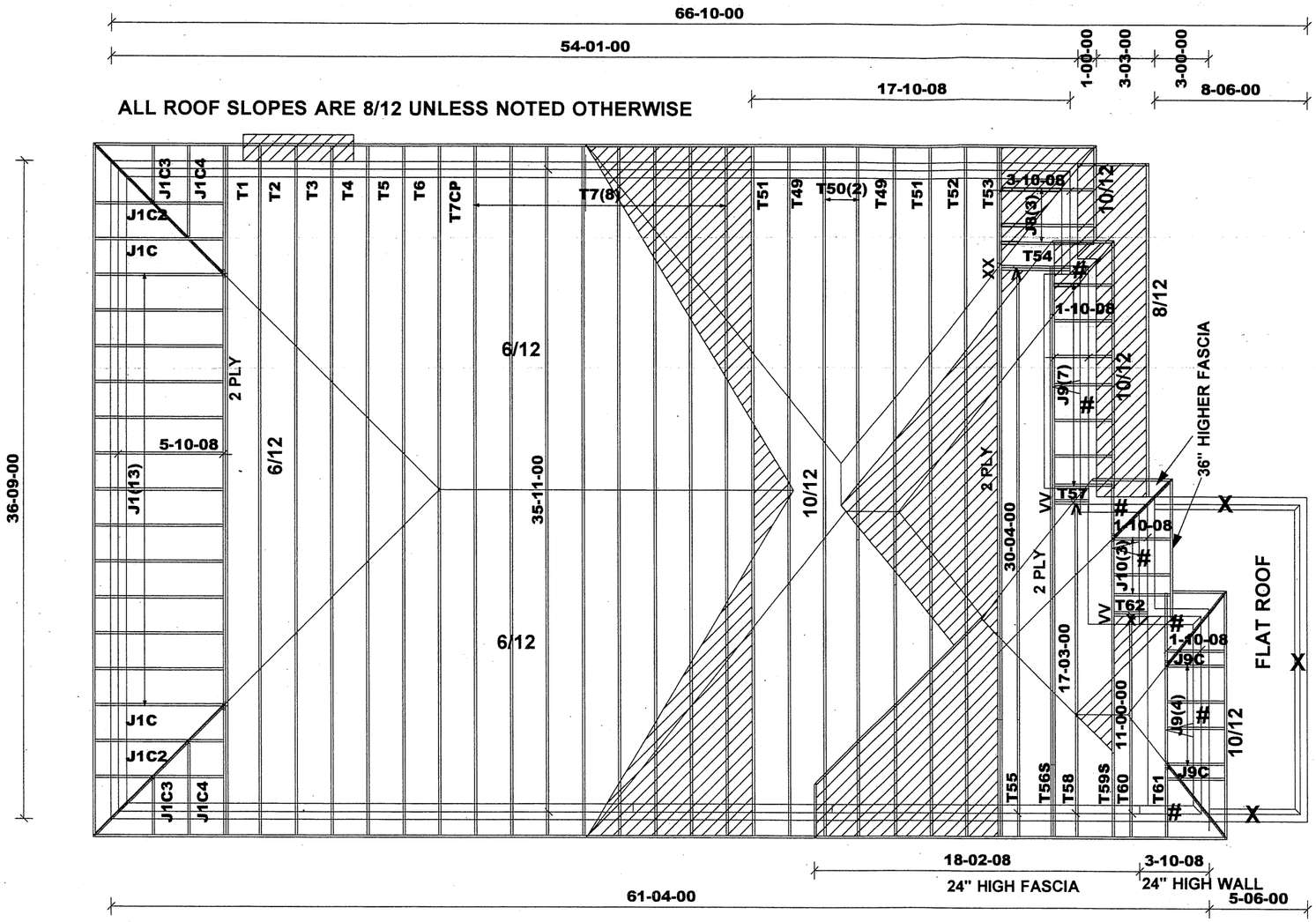
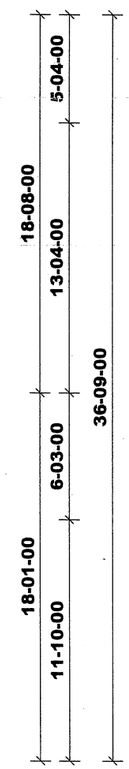
ALL ROOF SLOPES ARE 8/12 UNLESS NOTED OTHERWISE





ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
HGUS26 - (**X**)
LJS26DS - (**V**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)

DENOTES:

#: 24" RAISED CEILING
X: DROPPED BEAM BY OTHERS

DENOTES:
CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127214
STRUCTURAL COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

DWG# T-2126452 to T-2126456, T-2126461 to T-2126468, T-2126520 to T-2126537, T1800218

M14217

TAMARACK ROOF TRUSSES INC.

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4505 / C STD (4 OR 5 BED)	Mitek ver 8.4.2.286
Layout ID: 420381	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 205032	Date: 2021-08-05 Designer: ND		

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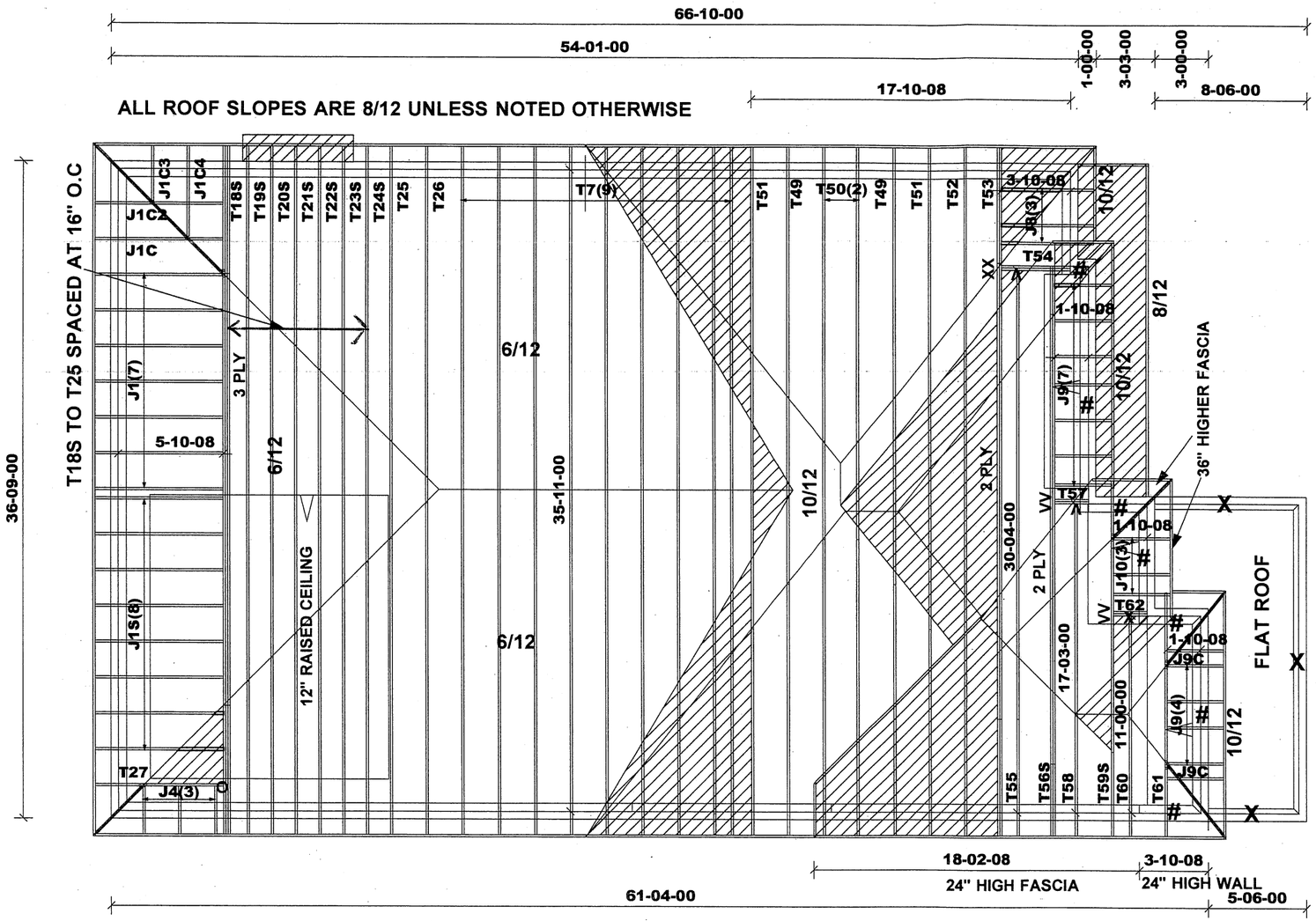


ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26 - (**X**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)

DENOTES:

#: 24" RAISED CEILING
X: DROPPED BEAM BY OTHERS

DENOTES:
CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127215
STRUCTURAL
COMPONENT ONLY

DWG# T-2126452 to T-2126456, T-2126467, T-2126481 to T-2126492, T-2126520 to T-2126537, T1800218

TRUSS PLACEMENT PLAN.

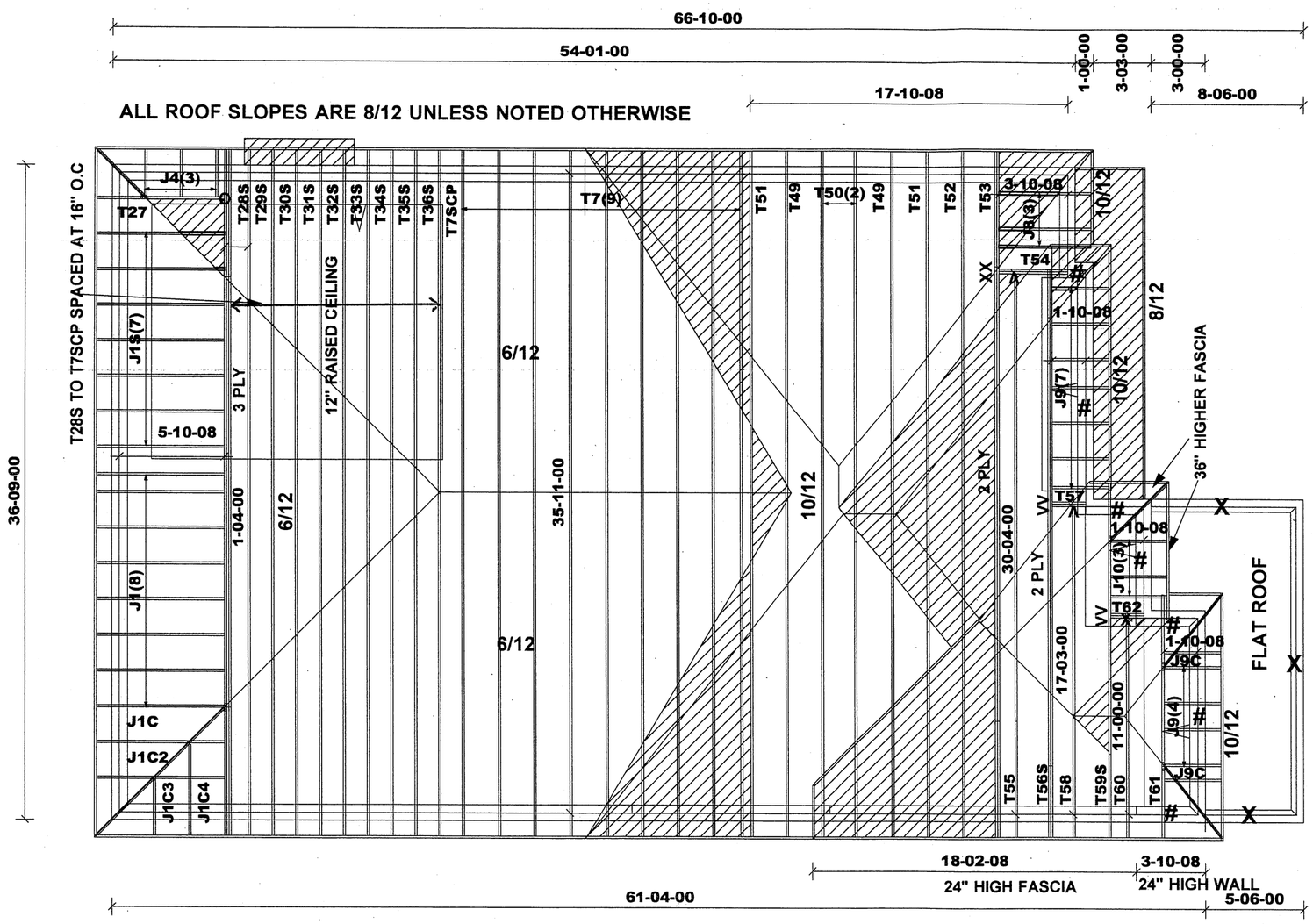
This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

M14217

TAMARACK ROOF TRUSSES INC.	Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4505 / C OPT. COFF (4 BED) — Mitek ver 8.4.2.286
	Layout ID: 420382	Project: VALES OF CASTLEMORE NORTH	
	Plan Log: 205032	Date: 2021-08-05 Designer: ND	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
(2019 AMENDMENT)
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26 - (**X**)
LUS26-2 - (**VV**)
HGUS26-2 - (**XX**)

DENOTES:

#: 24" RAISED CEILING
X: DROPPED BEAM BY OTHERS

DENOTES:
CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

LICENSED PROFESSIONAL ENGINEER
oct/0/11
H. J. G. ALVES
100009047
PROVINCE OF ONTARIO

DWG NO. TAM T-212 7216
STRUCTURAL COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

DWG# T-2126452 to T-2126456, T-2126467, T-2126481 to T-2126482, T-2126492 to T-2126502, T-2126520 to T-2126537, T1800218

M14217

TAMARACK ROOF TRUSSES INC.

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 4505 / C OPT. COFF (5 BED)	Mitek ver 8.4.2.286
Layout ID: 420383	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 205032	Date: 2021-08-05 Designer: ND		

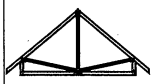
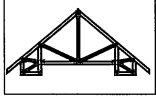
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420375
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	1 of 3
Lot #:		Date:	08-05-2021
Elevation:	A STD (4 OR 5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

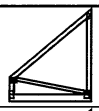
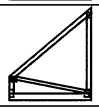
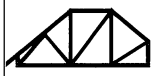
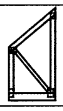
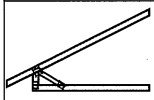
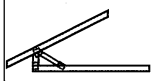
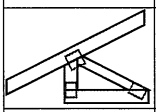
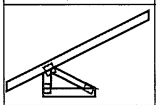
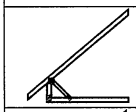
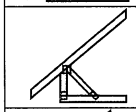
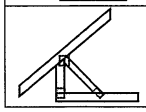
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	**35-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	333.31 207.33		
	1	T2 Hip	6 /12	**35-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.28 90.50		
	1	T3 Hip	6 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	148.12 93.00		
	1	T4 Hip	6 /12	**35-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	146.64 90.50		
	1	T5 Hip	6 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	163.39 103.33		
	1	T6 Hip	6 /12	**35-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	158.74 100.33		
	9	T7 Common	6 /12	**35-11-00**	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1388 849.00		
	1	T7CP Hip	6 /12	**35-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	154.04 94.33		
	2	T8 Hip	10 /12	**35-11-00**	7-10-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	317.07 197.33		
	2	T9 Hip	10 /12	**35-11-00**	9-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	374.82 232.00		
	2	T10 Hip	10 /12	**35-11-00**	11-02-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	386.25 236.67		
	1 2-ply	T11 Hip Girder	10 /12	**35-11-00**	6-02-08	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	379.01 234.00		
	3	T12 Common	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	147.28 97.00		
	4	T12S Roof Special	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	227.43 154.00		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420375
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	2 of 3
Lot #:		Date:	08-05-2021
Elevation:	A STD (4 OR 5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

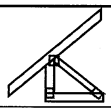
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T13 Jack-Closed Girder	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	59.24 39.33		
	1 2-ply	T13Z Jack-Closed Girder	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	59.24 39.33		
	1	T14 Hip	10 /12	**17-03-00**	9-00-07	2 x 4	1-03-08	1-07-11 3-03-11	97.41 62.67		
	1	T15 Hip	10 /12	**17-03-00**	7-04-07	2 x 4	1-03-08	1-07-11 3-03-11	81.15 51.33		
	1	T16S Hip Girder	10 /12	**17-03-00**	5-08-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	100.31 63.67		
	1 2-ply	T17 Jack-Closed Girder	10 /12	**2-10-08**	5-08-07	2 x 4 2 x 6		3-03-11 5-08-07	42.46 26.00		
	13	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	234.21 151.67		
	2	J1C Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	30.71 19.33		
	2	J1C2 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	25.61 16.67		
	2	J1C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	16.85 11.33		
	2	J1C4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	21.95 14.00		
	3	J2 Jack-Open	10 /12	**5-05-12**	6-02-08	2 x 4	1-03-08	1-07-11 6-02-08	58.27 39.00		
	2	J3 Jack-Open	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	25.18 16.67		
	1	J3C Jack-Open	10 /12	**1-10-15**	3-02-13	2 x 4	1-03-08 11-09	1-07-11 3-02-13	11.16 7.67		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420375
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	3 of 3
Lot #:		Date:	08-05-2021
Elevation:	A STD (4 OR 5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J3C1 Jack-Open	10 /12	1-10-15	3-02-13	2 x 4	1-03-08 1-01	1-07-11 3-02-13	10.4 7.50		

TOTAL # TRUSS= **69** TOTAL BFT OF ALL TRUSSES= **3345.49** BFT. TOTAL WEIGHT OF ALL TRSSES **5341.52** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
4	Hardware	LUS24	
1	Hardware	LUS26-2	
2	Hardware	HGUS26-2	

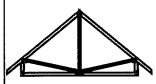
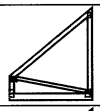
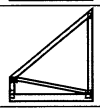
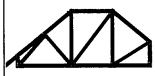
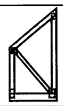
TOTAL NUMBER OF ITEMS= **9**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420376
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	1 of 3
Lot #:		Date:	08-05-2021
Elevation:	A OPT. COFF (4 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

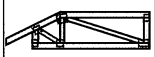
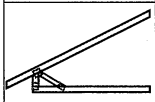
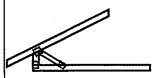
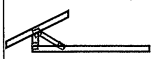
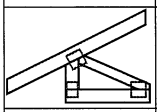
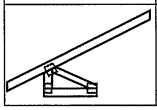
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	10	**T7 Common**	6 /12	**35-11-00**	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1511.62 926.67		
	2	**T8 Hip**	10 /12	**35-11-00**	7-10-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	317.07 197.33		
	2	**T9 Hip**	10 /12	**35-11-00**	9-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	374.82 232.00		
	2	**T10 Hip**	10 /12	**35-11-00**	11-02-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	386.25 236.67		
	1 2-ply	**T11 Hip Girder**	10 /12	**35-11-00**	6-02-08	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	369.01 227.33		
	**3**	**T12 Common**	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	147.28 97.00		
	4	**T12S Roof Special**	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	227.43 154.00		
	**1 2-ply**	**T13 Jack-Closed Girder**	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	59.24 39.33		
	**1 2-ply**	**T13Z Jack-Closed Girder**	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	59.24 39.33		
	1	**T14 Hip**	10 /12	**17-03-00**	9-00-07	2 x 4	1-03-08	1-07-11 3-03-11	97.41 62.67		
	**1**	**T15 Hip**	10 /12	**17-03-00**	7-04-07	2 x 4	1-03-08	1-07-11 3-03-11	81.15 51.33		
	1	**T16S Hip Girder**	10 /12	**17-03-00**	5-08-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	100.31 63.67		
	**1 2-ply**	**T17 Jack-Closed Girder**	10 /12	**2-10-08**	5-08-07	2 x 4 2 x 6		3-03-11 5-08-07	42.46 26.00		
	1 3-ply	**T18S Hip Girder**	6 /12	**35-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	513.74 329.00		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420376
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	2 of 3
Lot #:		Date:	08-05-2021
Elevation:	A OPT. COFF (4 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

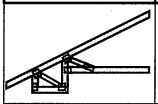
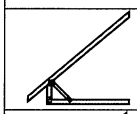
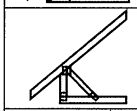
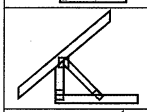
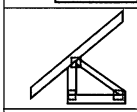
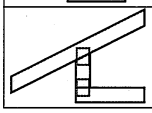
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T19S Hip	6 /12	**35-11-00**	4-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	147.17 94.67		
	1	T20S Hip	6 /12	**35-11-00**	5-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	153.43 96.67		
	1	T21S Hip	6 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	160.79 101.67		
	1	T22S Hip	6 /12	**35-11-00**	6-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	154.36 99.00		
	1	T23S Hip	6 /12	**35-11-00**	7-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	157.09 99.17		
	1	T24S Hip	6 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	160.27 102.00		
	1	T25 Hip	6 /12	**35-11-00**	8-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	145.69 90.67		
	1	T26 Hip	6 /12	**35-11-00**	9-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	163.77 102.67		
	1	T27 Half Hip Girder	6 /12	**5-10-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	23.14 16.50		
	7	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	126.11 81.67		
	1	J1C Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	15.35 9.67		
	1	J1C2 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	12.8 8.33		
	1	J1C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	8.43 5.67		
	1	J1C4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	10.98 7.00		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420376
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	3 of 3
Lot #:		Date:	08-05-2021
Elevation:	A OPT. COFF (4 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	**8**	**J1S** **Jack-Open**	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	165.75 117.33		
	**3**	**J2** **Jack-Open**	10 /12	**5-05-12**	6-02-08	2 x 4	1-03-08	1-07-11 6-02-08	58.27 39.00		
	**2**	**J3** **Jack-Open**	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	25.18 16.67		
	**1**	**J3C** **Jack-Open**	10 /12	**1-10-15**	3-02-13	2 x 4	1-03-08 11-09	1-07-11 3-02-13	11.16 7.67		
	**1**	**J3C1** **Jack-Open**	10 /12	**1-10-15**	3-02-13	2 x 4	1-03-08 1-01	1-07-11 3-02-13	10.4 7.50		
	**3**	**J4** **Jack-Open**	6 /12	**1-04-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	17.78 14.00		

TOTAL # TRUSS= **75** TOTAL BFT OF ALL TRUSSES= **3799.86** BFT. TOTAL WEIGHT OF ALL TRSSES **6014.94** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
5	Hardware	LUS24	
1	Hardware	LUS26-2	
2	Hardware	HGUS26-2	

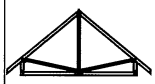
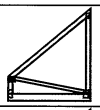
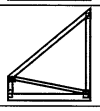
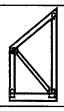
TOTAL NUMBER OF ITEMS= **10**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420377
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	1 of 3
Lot #:		Date:	08-05-2021
Elevation:	A OPT. COFF (5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

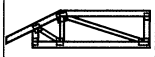
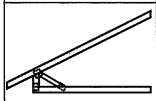
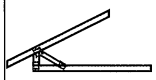
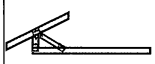
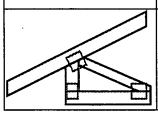
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	10	**T7 Common**	6 /12	**35-11-00**	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1511.62 926.67		
	1	**T7SCP Hip**	6 /12	**35-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	168.94 106.00		
	2	**T8 Hip**	10 /12	**35-11-00**	7-10-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	317.07 197.33		
	2	**T9 Hip**	10 /12	**35-11-00**	9-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	374.82 232.00		
	2	**T10 Hip**	10 /12	**35-11-00**	11-02-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	386.25 236.67		
	1 2-ply	**T11 Hip Girder**	10 /12	**35-11-00**	6-02-08	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	369.01 227.33		
	**3**	**T12 Common**	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	147.28 97.00		
	4	**T12S Roof Special**	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	227.43 154.00		
	**1 2-ply**	**T13 Jack-Closed Girder**	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	59.24 39.33		
	**1 2-ply**	**T13Z Jack-Closed Girder**	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	59.24 39.33		
	1	**T14 Hip**	10 /12	**17-03-00**	9-00-07	2 x 4	1-03-08	1-07-11 3-03-11	97.41 62.67		
	1	**T15 Hip**	10 /12	**17-03-00**	7-04-07	2 x 4	1-03-08	1-07-11 3-03-11	81.15 51.33		
	1	**T16S Hip Girder**	10 /12	**17-03-00**	5-08-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	100.31 63.67		
	**1 2-ply**	**T17 Jack-Closed Girder**	10 /12	**2-10-08**	5-08-07	2 x 4 2 x 6		3-03-11 5-08-07	42.46 26.00		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420377
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	2 of 3
Lot #:		Date:	08-05-2021
Elevation:	A OPT. COFF (5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

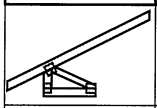
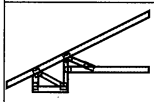
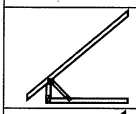
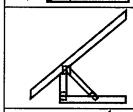
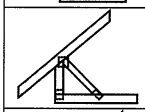
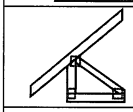
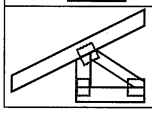
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T27 Half Hip Girder	6 /12	**5-10-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	23.14 16.50		
	1 3-ply	T28S Hip Girder	6 /12	**35-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	508.39 326.00		
	1	T29S Hip	6 /12	**35-11-00**	4-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	148.03 96.50		
	1	T30S Hip	6 /12	**35-11-00**	5-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	154.69 98.83		
	1	T31S Hip	6 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	155.39 98.83		
	1	T32S Hip	6 /12	**35-11-00**	6-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	159.96 103.67		
	1	T33S Hip	6 /12	**35-11-00**	7-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	164.97 104.33		
	1	T34S Hip	6 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	162.18 102.67		
	1	T35S Hip	6 /12	**35-11-00**	8-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	166.93 107.00		
	1	T36S Hip	6 /12	**35-11-00**	9-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	167.6 106.83		
	8	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	144.13 93.33		
	1	J1C Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	15.35 9.67		
	1	J1C2 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	12.8 8.33		
	1	J1C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	8.43 5.67		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420377
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	3 of 3
Lot #:		Date:	08-05-2021
Elevation:	A OPT. COFF (5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J1C4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	10.98 7.00		
	7	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	142.15 100.33		
	3	J2 Jack-Open	10 /12	**5-05-12**	6-02-08	2 x 4	1-03-08	1-07-11 6-02-08	58.27 39.00		
	2	J3 Jack-Open	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	25.18 16.67		
	1	J3C Jack-Open	10 /12	**1-10-15**	3-02-13	2 x 4	1-03-08 11-09	1-07-11 3-02-13	11.16 7.67		
	1	J3C1 Jack-Open	10 /12	**1-10-15**	3-02-13	2 x 4	1-03-08 1-01	1-07-11 3-02-13	10.4 7.50		
	3	J4 Jack-Open	6 /12	**1-04-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	20.92 17.00		

TOTAL # TRUSS= **76** TOTAL BFT OF ALL TRUSSES= **3932.66** BFT. TOTAL WEIGHT OF ALL TRSSES **6213.28** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
5	Hardware	LUS24	
1	Hardware	LUS26-2	
2	Hardware	HGUS26-2	

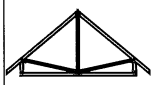
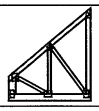
TOTAL NUMBER OF ITEMS= **10**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420378
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	1 of 3
Lot #:		Date:	08-05-2021
Elevation:	B STD (4 OR 5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

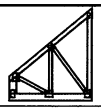
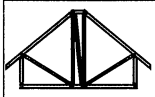
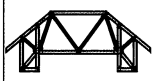
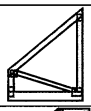
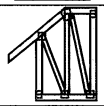
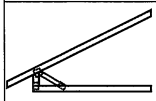
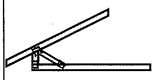
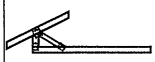
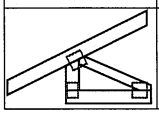
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	**35-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	326.21 203.33		
	1	T2 Hip	6 /12	**35-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.28 90.50		
	1	T3 Hip	6 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	148.12 93.00		
	1	T4 Hip	6 /12	**35-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	146.64 90.50		
	1	T5 Hip	6 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	163.39 103.33		
	1	T6 Hip	6 /12	**35-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	158.74 100.33		
	9	T7 Common	6 /12	**35-11-00**	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1360.46 834.00		
	1	T7CP Hip	6 /12	**35-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	150.98 92.67		
	2	T8 Hip	10 /12	**35-11-00**	7-10-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	317.07 197.33		
	2	T9 Hip	10 /12	**35-11-00**	9-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	374.82 232.00		
	2	T10 Hip	10 /12	**35-11-00**	11-02-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	386.25 236.67		
	1	T12 Common	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	49.09 32.33		
	1	T38 Hip Girder	10 /12	**11-00-00**	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	52.24 34.00		
	1 2-ply	T39 Jack-Closed Girder	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	68.59 45.67		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420378
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	2 of 3
Lot #:		Date:	08-05-2021
Elevation:	B STD (4 OR 5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

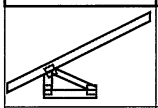
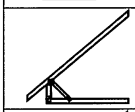
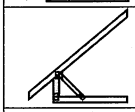
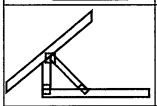
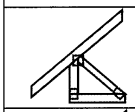
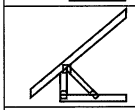
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T39Z Jack-Closed Girder	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	68.59 45.67		
	1	T40 Hip	10 /12	**17-03-00**	8-01-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	88.63 57.33		
	1	T41 Hip	10 /12	**17-03-00**	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	75.53 48.00		
	1	T42 Hip Girder	10 /12	**17-03-00**	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	86.03 55.17		
	1	T43 Hip	10 /12	**11-00-00**	7-04-07	2 x 4	1-03-08 1-03-08	3-03-11 3-03-11	64.83 41.17		
	1	T44 Hip Girder	10 /12	**11-00-00**	5-08-07	2 x 4	1-03-08 1-03-08	3-03-11 3-03-11	67.75 45.67		
	1 2-ply	T45 Jack-Closed Girder	10 /12	**3-10-08**	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	2	T46 Half Hip Girder	10 /12	**2-10-08**	4-05-07	2 x 4	1-03-08	3-03-11 4-05-07	51.72 33.33		
	2	T47 Monopitch	10 /12	**1-04-08**	4-05-07	2 x 4	1-03-08	3-03-11 4-05-07	31.83 21.33		
	1 2-ply	T48 Hip Girder	10 /12	**35-11-00**	6-02-08	2 x 4 2 x 6	1-03-08 1-05-00	1-07-11 1-07-11	394.14 242.00		
	13	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	234.21 151.67		
	2	J1C Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	30.71 19.33		
	2	J1C2 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	25.61 16.67		
	2	J1C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	16.86 11.33		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420378
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	3 of 3
Lot #:		Date:	08-05-2021
Elevation:	B STD (4 OR 5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	**2**	**J1C4** **Jack-Open**	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	21.96 14.00		
	**3**	**J2** **Jack-Open**	10 /12	**5-05-12**	6-02-08	2 x 4	1-03-08	1-07-11 6-02-08	58.27 39.00		
	**5**	**J5** **Jack-Open**	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	76.06 48.33		
	**3**	**J5C** **Jack-Open**	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	36.34 25.00		
	**3**	**J5C1** **Jack-Open**	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	29.97 21.00		
	**4**	**J6** **Jack-Open**	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	50.35 33.33		
	4	**J7** **Jack-Open**	4 /12	**6-11-00**	3-00-08	2 x 4	1-03-08	3-15 2-04-04	82.23 50.67		

TOTAL # TRUSS= **85** TOTAL BFT OF ALL TRUSSES= **3434.99** BFT. TOTAL WEIGHT OF ALL TRSSES **5480.65** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
6	Hardware	LUS24	
1	Hardware	LUS26-2	
2	Hardware	HGUS26-2	

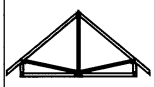
TOTAL NUMBER OF ITEMS= **13**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420379
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	1 of 4
Lot #:		Date:	08-05-2021
Elevation:	B OPT. COFF (4 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	10	**T7** **Common**	6 /12	**35-11-00**	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1511.62 926.67		
	2	**T8** **Hip**	10 /12	**35-11-00**	7-10-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	317.07 197.33		
	2	**T9** **Hip**	10 /12	**35-11-00**	9-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	374.82 232.00		
	2	**T10** **Hip**	10 /12	**35-11-00**	11-02-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	386.25 236.67		
	**1**	**T12** **Common**	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	49.09 32.33		
	1 **3-ply**	**T18S** **Hip Girder**	6 /12	**35-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	513.74 329.00		
	1	**T19S** **Hip**	6 /12	**35-11-00**	4-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	147.17 94.67		
	1	**T20S** **Hip**	6 /12	**35-11-00**	5-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	152.3 95.67		
	1	**T21S** **Hip**	6 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	159.46 100.67		
	1	**T22S** **Hip**	6 /12	**35-11-00**	6-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	154.36 99.00		
	1	**T23S** **Hip**	6 /12	**35-11-00**	7-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	157.09 99.17		
	1	**T24S** **Hip**	6 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	160.27 102.00		
	1	**T25** **Hip**	6 /12	**35-11-00**	8-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	145.69 90.67		
	1	**T26** **Hip**	6 /12	**35-11-00**	9-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	163.77 102.67		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
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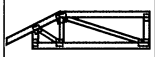
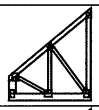
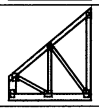
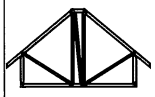
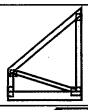
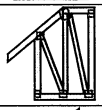
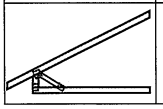
		Sales Rep:	Rick DiCiano

Roof Trusses

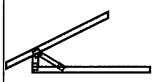
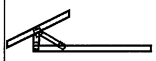
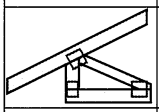
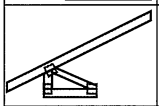
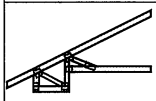
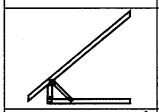
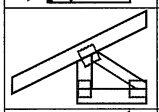
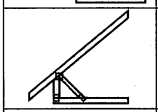
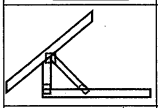
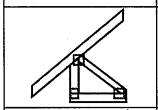
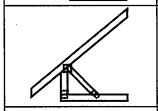
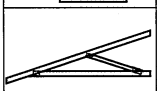
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T27 Half Hip Girder	6 /12	**5-10-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	23.14 16.50		
	1	T38 Hip Girder	10 /12	**11-00-00**	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	52.24 34.00		
	1 2-ply	T39 Jack-Closed Girder	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	68.59 45.67		
	1 2-ply	T39Z Jack-Closed Girder	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	68.59 45.67		
	1	T40 Hip	10 /12	**17-03-00**	8-01-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	88.63 57.33		
	1	T41 Hip	10 /12	**17-03-00**	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	75.53 48.00		
	1	T42 Hip Girder	10 /12	**17-03-00**	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	86.03 55.17		
	1	T43 Hip	10 /12	**11-00-00**	7-04-07	2 x 4	1-03-08 1-03-08	3-03-11 3-03-11	64.83 41.17		
	1	T44 Hip Girder	10 /12	**11-00-00**	5-08-07	2 x 4	1-03-08 1-03-08	3-03-11 3-03-11	67.75 45.67		
	1 2-ply	T45 Jack-Closed Girder	10 /12	**3-10-08**	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	2	T46 Half Hip Girder	10 /12	**2-10-08**	4-05-07	2 x 4	1-03-08	3-03-11 4-05-07	51.72 33.33		
	2	T47 Monopitch	10 /12	**1-04-08**	4-05-07	2 x 4	1-03-08	3-03-11 4-05-07	31.83 21.33		
	1 2-ply	T48 Hip Girder	10 /12	**35-11-00**	6-02-08	2 x 4 2 x 6	1-03-08 1-05-00	1-07-11 1-07-11	387.69 238.67		
	7	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	126.11 81.67		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420379
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	3 of 4
Lot #:		Date:	08-05-2021
Elevation:	B OPT. COFF (4 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J1C Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	15.35 9.67		
	1	J1C2 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	12.8 8.33		
	1	J1C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	8.43 5.67		
	1	J1C4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	10.98 7.00		
	8	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	162.46 114.67		
	3	J2 Jack-Open	10 /12	**5-05-12**	6-02-08	2 x 4	1-03-08	1-07-11 6-02-08	58.27 39.00		
	3	J4 Jack-Open	6 /12	**1-04-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	20.92 17.00		
	5	J5 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	76.06 48.33		
	3	J5C Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	36.34 25.00		
	3	J5C1 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	29.97 21.00		
	4	J6 Jack-Open	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	50.35 33.33		
	4	J7 Jack-Open	4 /12	**6-11-00**	3-00-08	2 x 4	1-03-08	3-15 2-04-04	82.23 50.67		

TOTAL # TRUSS= **91** TOTAL BFT OF ALL TRUSSES= **3911.7** BFT. TOTAL WEIGHT OF ALL TRSSES **6192.72** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
7	Hardware	LUS24	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420379
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	4 of 4
Lot #:		Date:	08-05-2021
Elevation:	B OPT. COFF (4 BED)	Designer:	
		Sales Rep:	Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LUS26-2	
2	Hardware	HGUS26-2	

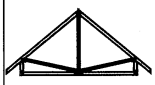
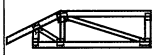
TOTAL NUMBER OF ITEMS= **14**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420380
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	1 of 4
Lot #:		Date:	08-05-2021
Elevation:	B OPT. COFF (5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

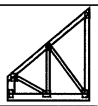
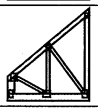
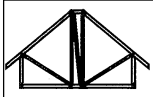
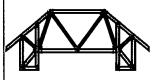
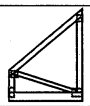
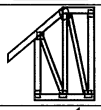
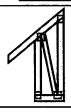
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	10	T7 Common	6 /12	35-11-00	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1511.62 926.67		
	1	T7SCP Hip	6 /12	35-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	168.94 106.00		
	2	T8 Hip	10 /12	35-11-00	7-10-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	317.07 197.33		
	2	T9 Hip	10 /12	35-11-00	9-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	374.82 232.00		
	2	T10 Hip	10 /12	35-11-00	11-02-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	386.25 236.67		
	1	T12 Common	10 /12	11-00-00	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	49.09 32.33		
	1	T27 Half Hip Girder	6 /12	5-10-08	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	23.14 16.50		
	1 3-ply	T28S Hip Girder	6 /12	35-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	508.39 326.00		
	1	T29S Hip	6 /12	35-11-00	4-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	148.03 95.83		
	1	T30S Hip	6 /12	35-11-00	5-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	153.56 97.83		
	1	T31S Hip	6 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	154.07 97.83		
	1	T32S Hip	6 /12	35-11-00	6-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	159.96 103.67		
	1	T33S Hip	6 /12	35-11-00	7-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	164.97 104.33		
	1	T34S Hip	6 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	162.18 102.67		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420380
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	2 of 4
Lot #:		Date:	08-05-2021
Elevation:	B OPT. COFF (5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

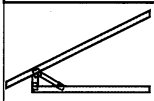
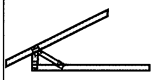
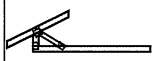
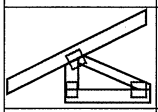
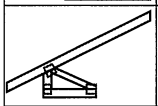
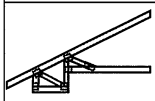
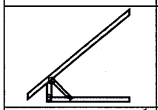
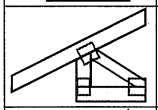
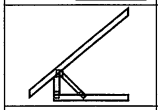
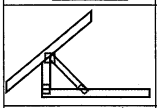
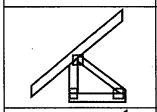
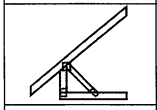
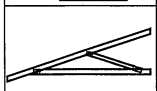
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T35S Hip	6 /12	**35-11-00**	8-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	164.55 105.50		
	1	T36S Hip	6 /12	**35-11-00**	9-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	167.6 106.83		
	1	T38 Hip Girder	10 /12	**11-00-00**	4-10-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	52.24 34.00		
	1 2-ply	T39 Jack-Closed Girder	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	68.59 45.67		
	1 2-ply	T39Z Jack-Closed Girder	10 /12	**5-05-12**	6-02-08	2 x 4 2 x 6		1-07-11 6-02-08	68.59 45.67		
	1	T40 Hip	10 /12	**17-03-00**	8-01-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	88.63 57.33		
	1	T41 Hip	10 /12	**17-03-00**	6-05-03	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	75.53 48.00		
	1	T42 Hip Girder	10 /12	**17-03-00**	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	86.03 55.17		
	1	T43 Hip	10 /12	**11-00-00**	7-04-07	2 x 4	1-03-08 1-03-08	3-03-11 3-03-11	64.83 41.17		
	1	T44 Hip Girder	10 /12	**11-00-00**	5-08-07	2 x 4	1-03-08 1-03-08	3-03-11 3-03-11	67.75 45.67		
	1 2-ply	T45 Jack-Closed Girder	10 /12	**3-10-08**	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	43.15 29.33		
	2	T46 Half Hip Girder	10 /12	**2-10-08**	4-05-07	2 x 4	1-03-08	3-03-11 4-05-07	51.72 33.33		
	2	T47 Monopitch	10 /12	**1-04-08**	4-05-07	2 x 4	1-03-08	3-03-11 4-05-07	31.83 21.33		
	1 2-ply	T48 Hip Girder	10 /12	**35-11-00**	6-02-08	2 x 4 2 x 6	1-03-08 1-05-00	1-07-11 1-07-11	387.69 238.67		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420380
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	3 of 4
Lot #:		Date:	08-05-2021
Elevation:	B OPT. COFF (5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	8	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	144.13 93.33		
	1	J1C Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	15.35 9.67		
	1	J1C2 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	12.8 8.33		
	1	J1C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	8.43 5.67		
	1	J1C4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	10.98 7.00		
	7	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	142.15 100.33		
	3	J2 Jack-Open	10 /12	**5-05-12**	6-02-08	2 x 4	1-03-08	1-07-11 6-02-08	58.27 39.00		
	3	J4 Jack-Open	6 /12	**1-04-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	20.92 17.00		
	5	J5 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	76.06 48.33		
	3	J5C Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	36.34 25.00		
	3	J5C1 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	29.97 21.00		
	4	J6 Jack-Open	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	50.35 33.33		
	4	J7 Jack-Open	4 /12	**6-11-00**	3-00-08	2 x 4	1-03-08	3-15 2-04-04	82.23 50.67		

TOTAL # TRUSS= **92** TOTAL BFT OF ALL TRUSSES= **4041.99** BFT. TOTAL WEIGHT OF ALL TRSSES **6388.82** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420380
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	4 of 4
Lot #:		Date:	08-05-2021
Elevation:	B OPT. COFF (5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
7	Hardware	LUS24	
1	Hardware	LUS26-2	
2	Hardware	HGUS26-2	

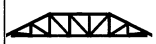
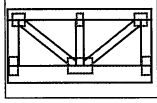
TOTAL NUMBER OF ITEMS= **14**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420381
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	1 of 3
Lot #:		Date:	08-05-2021
Elevation:	C STD (4 OR 5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

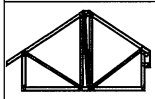
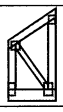
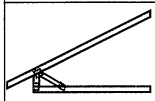
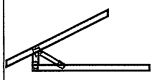
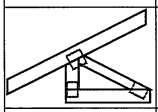
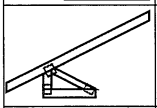
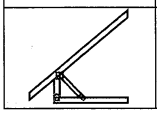
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	35-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	333.31 207.33		
	1	T2 Hip	6 /12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	143.28 90.50		
	1	T3 Hip	6 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	148.12 93.00		
	1	T4 Hip	6 /12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	146.64 90.50		
	1	T5 Hip	6 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	163.39 103.33		
	1	T6 Hip	6 /12	35-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	158.74 100.33		
	8	T7 Common	6 /12	35-11-00	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1209.3 741.33		
	1	T7CP Hip	6 /12	35-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	150.98 92.67		
	2	T49 Hip	8 /12	35-11-00	9-10-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	343.98 216.67		
	2	T50 Hip	8 /12	35-11-00	11-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	359.47 222.67		
	2	T51 Hip	8 /12	35-11-00	8-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	322.55 199.33		
	1	T52 Hip	8 /12	35-11-00	6-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	147.86 90.83		
	1 2-ply	T53 Hip Girder	8 /12	35-11-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	343.42 210.00		
	1 2-ply	T54 Flat Girder	0 /12	3-10-08	2-00-00	2 x 4 2 x 6		2-00-00 2-00-00	36.93 24.67		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420381
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	2 of 3
Lot #:		Date:	08-05-2021
Elevation:	C STD (4 OR 5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

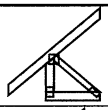
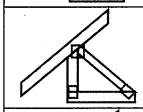
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T55 Hip	8 /12	**30-04-00**	6-08-07	2 x 4	1-03-08 1-03-08	3-02-13 3-04-13	132.8 82.83		
	1 2-ply	T56S Hip Girder	8 /12	**30-07-00**	5-02-07	2 x 4 2 x 6	1-03-08	3-04-13 1-04-13	311.48 199.33		
	1 2-ply	T57 Jack-Closed Girder	10 /12	**1-10-08**	5-02-07	2 x 4 2 x 6		3-07-11 5-02-07	35.18 24.00		
	1	T58 Hip	8 /12	**17-03-00**	5-11-13	2 x 4	1-03-08 1-03-08	2-04-13 3-04-13	83.12 52.83		
	1	T59S Hip Girder	8 /12	**17-03-00**	5-07-13	2 x 4 2 x 6	1-03-08 1-03-08	3-04-13 2-04-13	101.7 63.33		
	1	T60 Hip	8 /12	**11-00-00**	6-10-07	2 x 4	1-03-08	3-04-13 1-04-13	63.11 41.50		
	1	T61 Hip Girder	8 /12	**11-00-00**	3-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	49 30.83		
	1 2-ply	T62 Jack-Closed Girder	8 /12	**1-10-08**	3-07-13	2 x 4 2 x 6		2-04-13 3-07-13	26.45 17.67		
	13	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	234.21 151.67		
	2	J1C Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	30.71 19.33		
	2	J1C2 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	25.61 16.67		
	2	J1C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	16.85 11.33		
	2	J1C4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	21.95 14.00		
	3	J8 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.64 29.00		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420381
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	3 of 3
Lot #:		Date:	08-05-2021
Elevation:	C STD (4 OR 5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	**11**	**J9** **Jack-Open**	10 /12	**1-10-08**	3-02-07	2 x 4	1-03-08	1-07-11 3-02-07	111.32 77.00		
	**2**	**J9C** **Jack-Open**	10 /12	**1-01-09**	2-07-00	2 x 4	1-03-08 8-15	1-07-11 2-07-00	18.04 12.67		
	3	**J10** **Jack-Open**	8 /12	**1-10-08**	3-07-13	2 x 4	1-03-08	2-04-13 3-07-13	32.92 20.50		

TOTAL # TRUSS= **78** TOTAL BFT OF ALL TRUSSES= **3347.65** BFT. TOTAL WEIGHT OF ALL TRSSES **5348.05** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
3	Hardware	LUS26-2	
1	Hardware	HGUS26-2	
1	Hardware	HGUS26	

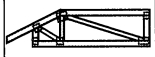
TOTAL NUMBER OF ITEMS= **7**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420382
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	1 of 3
Lot #:		Date:	08-05-2021
Elevation:	C OPT. COFF (4 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

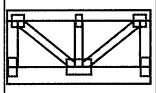
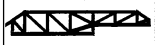
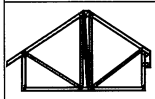
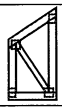
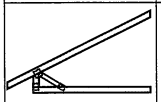
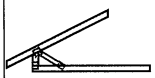
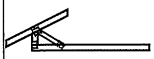
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	T7 Common	6 /12	**35-11-00**	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1360.46 834.00		
	1 3-ply	T18S Hip Girder	6 /12	**35-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	513.74 329.00		
	1	T19S Hip	6 /12	**35-11-00**	4-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	147.17 94.67		
	1	T20S Hip	6 /12	**35-11-00**	5-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	152.3 95.67		
	1	T21S Hip	6 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	159.46 100.67		
	1	T22S Hip	6 /12	**35-11-00**	6-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	154.36 99.00		
	1	T23S Hip	6 /12	**35-11-00**	7-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	157.09 99.17		
	1	T24S Hip	6 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	160.27 102.00		
	1	T25 Hip	6 /12	**35-11-00**	8-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	145.69 90.67		
	1	T26 Hip	6 /12	**35-11-00**	9-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	163.77 102.67		
	1	T27 Half Hip Girder	6 /12	**5-10-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	23.14 16.50		
	2	T49 Hip	8 /12	**35-11-00**	9-10-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	343.98 216.67		
	2	T50 Hip	8 /12	**35-11-00**	11-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	353.78 219.33		
	2	T51 Hip	8 /12	**35-11-00**	8-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	322.55 199.33		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420382
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	2 of 3
Lot #:		Date:	08-05-2021
Elevation:	C OPT. COFF (4 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

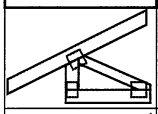
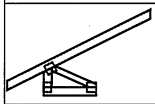
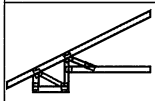
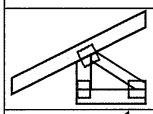
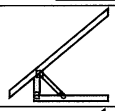
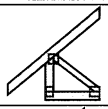
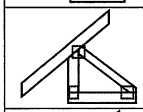
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T52 Hip	8 /12	**35-11-00**	6-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	147.86 90.83		
	1 2-ply	T53 Hip Girder	8 /12	**35-11-00**	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	341.05 210.67		
	1 2-ply	T54 Flat Girder	0 /12	**3-10-08**	2-00-00	2 x 4 2 x 6		2-00-00 2-00-00	36.93 24.67		
	1	T55 Hip	8 /12	**30-04-00**	6-08-07	2 x 4	1-03-08 1-03-08	3-02-13 3-04-13	132.8 82.83		
	1 2-ply	T56S Hip Girder	8 /12	**30-07-00**	5-02-07	2 x 4 2 x 6	1-03-08	3-04-13 1-04-13	311.48 199.33		
	1 2-ply	T57 Jack-Closed Girder	10 /12	**1-10-08**	5-02-07	2 x 4		3-07-11 5-02-07	33.75 22.67		
	1	T58 Hip	8 /12	**17-03-00**	5-11-13	2 x 4	1-03-08 1-03-08	2-04-13 3-04-13	83.12 52.83		
	1	T59S Hip Girder	8 /12	**17-03-00**	5-07-13	2 x 4 2 x 6	1-03-08 1-03-08	3-04-13 2-04-13	100.28 62.50		
	1	T60 Hip	8 /12	**11-00-00**	6-10-07	2 x 4	1-03-08	3-04-13 1-04-13	63.11 41.50		
	1	T61 Hip Girder	8 /12	**11-00-00**	3-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	49 30.83		
	1 2-ply	T62 Jack-Closed Girder	8 /12	**1-10-08**	3-07-13	2 x 4		2-04-13 3-07-13	24.99 17.67		
	7	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	126.11 81.67		
	1	J1C Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	15.35 9.67		
	1	J1C2 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	12.8 8.33		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420382
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	3 of 3
Lot #:		Date:	08-05-2021
Elevation:	C OPT. COFF (4 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J1C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	8.43 5.67		
	1	J1C4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	10.98 7.00		
	8	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	162.46 114.67		
	3	J4 Jack-Open	6 /12	**1-04-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	20.92 17.00		
	3	J8 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.64 29.00		
	11	J9 Jack-Open	10 /12	**1-10-08**	3-02-07	2 x 4	1-03-08	1-07-11 3-02-07	111.35 77.00		
	2	J9C Jack-Open	10 /12	**1-01-09**	2-07-00	2 x 4	1-03-08 8-15	1-07-11 2-07-00	18.04 12.67		
	3	J10 Jack-Open	8 /12	**1-10-08**	3-07-13	2 x 4	1-03-08	2-04-13 3-07-13	32.92 20.50		

TOTAL # TRUSS= **84** TOTAL BFT OF ALL TRUSSES= **3818.86** BFT. TOTAL WEIGHT OF ALL TRSSES **6047.14** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
1	Hardware	LUS24	
3	Hardware	LUS26-2	
1	Hardware	HGUS26-2	
1	Hardware	HGUS26	

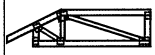
TOTAL NUMBER OF ITEMS= **8**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420383
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	1 of 3
Lot #:		Date:	08-05-2021
Elevation:	C OPT. COFF (5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

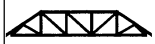
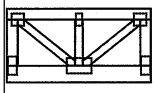
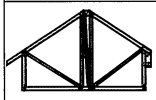
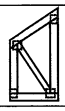
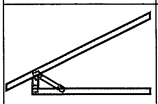
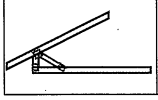
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	T7 Common	6 /12	**35-11-00**	10-01-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1360.46 834.00		
	1	T7SCP Hip	6 /12	**35-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	168.94 106.00		
	1	T27 Half Hip Girder	6 /12	**5-10-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	23.14 16.50		
	1 3-ply	T28S Hip Girder	6 /12	**35-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	508.39 326.00		
	1	T29S Hip	6 /12	**35-11-00**	4-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	148.03 95.83		
	1	T30S Hip	6 /12	**35-11-00**	5-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	153.56 97.83		
	1	T31S Hip	6 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	154.07 97.83		
	1	T32S Hip	6 /12	**35-11-00**	6-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	159.96 103.67		
	1	T33S Hip	6 /12	**35-11-00**	7-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	164.97 104.33		
	1	T34S Hip	6 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	162.18 102.67		
	1	T35S Hip	6 /12	**35-11-00**	8-09-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	164.55 105.50		
	1	T36S Hip	6 /12	**35-11-00**	9-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	167.6 106.83		
	2	T49 Hip	8 /12	**35-11-00**	9-10-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	343.98 216.67		
	2	T50 Hip	8 /12	**35-11-00**	11-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	353.78 219.33		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420383
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	2 of 3
Lot #:		Date:	08-05-2021
Elevation:	C OPT. COFF (5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

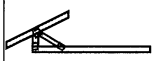
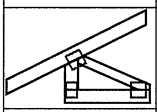
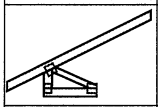
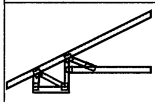
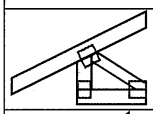
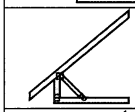
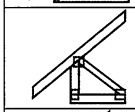
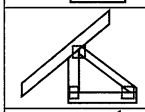
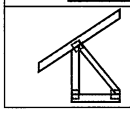
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T51 Hip	8 /12	**35-11-00**	8-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	322.55 199.33		
	1	T52 Hip	8 /12	**35-11-00**	6-06-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	147.86 90.83		
	1 2-ply	T53 Hip Girder	8 /12	**35-11-00**	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	341.05 210.67		
	1 2-ply	T54 Flat Girder	0 /12	**3-10-08**	2-00-00	2 x 4 2 x 6		2-00-00 2-00-00	36.93 24.67		
	1	T55 Hip	8 /12	**30-04-00**	6-08-07	2 x 4	1-03-08 1-03-08	3-02-13 3-04-13	132.8 82.83		
	1 2-ply	T56S Hip Girder	8 /12	**30-07-00**	5-02-07	2 x 4 2 x 6	1-03-08	3-04-13 1-04-13	311.48 199.33		
	1 2-ply	T57 Jack-Closed Girder	10 /12	**1-10-08**	5-02-07	2 x 4		3-07-11 5-02-07	33.75 22.67		
	1	T58 Hip	8 /12	**17-03-00**	5-11-13	2 x 4	1-03-08 1-03-08	2-04-13 3-04-13	83.12 52.83		
	1	T59S Hip Girder	8 /12	**17-03-00**	5-07-13	2 x 4 2 x 6	1-03-08 1-03-08	3-04-13 2-04-13	100.28 62.50		
	1	T60 Hip	8 /12	**11-00-00**	6-10-07	2 x 4	1-03-08	3-04-13 1-04-13	63.11 41.50		
	1	T61 Hip Girder	8 /12	**11-00-00**	3-02-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	49 30.83		
	1 2-ply	T62 Jack-Closed Girder	8 /12	**1-10-08**	3-07-13	2 x 4		2-04-13 3-07-13	24.99 17.67		
	8	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	144.13 93.33		
	1	J1C Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	15.35 9.67		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	205032
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420383
Location:	BRAMPTON	Ref #	
Model:	UNIT 4505	Page:	3 of 3
Lot #:		Date:	08-05-2021
Elevation:	C OPT. COFF (5 BED)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J1C2 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	12.8 8.33		
	1	J1C3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	8.43 5.67		
	1	J1C4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	10.98 7.00		
	7	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	142.15 100.33		
	3	J4 Jack-Open	6 /12	**1-04-08**	1-10-04	2 x 4	1-03-08	1-02-00 1-10-04	20.92 17.00		
	3	J8 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	45.64 29.00		
	11	J9 Jack-Open	10 /12	**1-10-08**	3-02-07	2 x 4	1-03-08	1-07-11 3-02-07	111.35 77.00		
	2	J9C Jack-Open	10 /12	**1-01-09**	2-07-00	2 x 4	1-03-08 8-15	1-07-11 2-07-00	18.04 12.67		
	3	J10 Jack-Open	8 /12	**1-10-08**	3-07-13	2 x 4	1-03-08	2-04-13 3-07-13	32.92 20.50		

TOTAL # TRUSS= **85** TOTAL BFT OF ALL TRUSSES= **3949.15** BFT. TOTAL WEIGHT OF ALL TRSSES **6243.25** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
1	Hardware	LUS24	
3	Hardware	LUS26-2	
1	Hardware	HGUS26-2	
1	Hardware	HGUS26	

TOTAL NUMBER OF ITEMS= **8**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T1	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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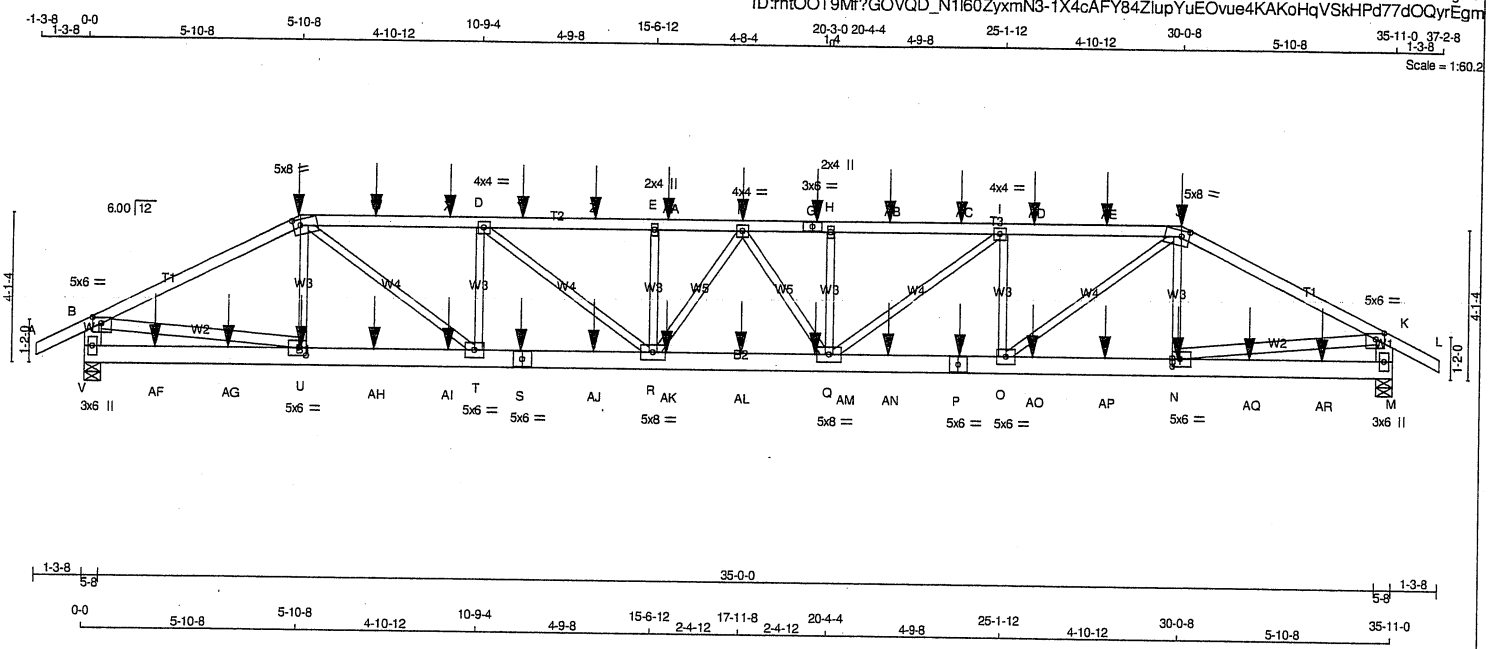

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - U	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- G	1	12	SIDE(61.0)
G- J	1	12	SIDE(61.0)
J- L	1	12	SIDE(61.0)
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(183.1)
S- P	2	12	SIDE(0.0)
P- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
U- C	1	6	SIDE(19.2)
N- J	1	6	SIDE(19.2)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

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100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126461

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3722	0	3722	0	0	5-8	5-8
M	3721	0	3721	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2640	1689 / 0	0 / 0	0 / 0	0 / 0	950 / 0	0 / 0
M	2639	1689 / 0	0 / 0	0 / 0	0 / 0	950 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.06 (1)		10.00
B- C	-5801 / 0	-84.9	-84.9	0.55 (1)		3.64
C- W	-7799 / 0	-84.9	-84.9	0.65 (1)		3.00
W- X	-7799 / 0	-84.9	-84.9	0.65 (1)		3.00
X- D	-7799 / 0	-84.9	-84.9	0.65 (1)		3.00
D- Y	-8979 / 0	-84.9	-84.9	0.74 (1)		2.70
Y- Z	-8979 / 0	-84.9	-84.9	0.74 (1)		2.70
Z- E	-8979 / 0	-84.9	-84.9	0.74 (1)		2.70
E-AA	-8979 / 0	-84.9	-84.9	0.41 (1)		2.99
AA- F	-8979 / 0	-84.9	-84.9	0.41 (1)		2.99
F- G	-8975 / 0	-84.9	-84.9	0.42 (1)		2.98
G- H	-8975 / 0	-84.9	-84.9	0.42 (1)		2.98
H-AB	-8975 / 0	-84.9	-84.9	0.74 (1)		2.70
AB-AC	-8975 / 0	-84.9	-84.9	0.74 (1)		2.70
AC- I	-8975 / 0	-84.9	-84.9	0.74 (1)		2.70
I-AD	-7796 / 0	-84.9	-84.9	0.64 (1)		3.00
AD-AE	-7796 / 0	-84.9	-84.9	0.64 (1)		3.00
AE- J	-7796 / 0	-84.9	-84.9	0.64 (1)		3.00
J- K	-5799 / 0	-84.9	-84.9	0.55 (1)		3.64
K- L	0 / 26	-84.9	-84.9	0.06 (1)		10.00
V- B	-3637 / 0	0.0	0.0	0.13 (1)		7.37
M- K	-3635 / 0	0.0	0.0	0.13 (1)		7.37
V-AF	0 / 0	-18.5	-18.5	0.07 (4)		10.00
AF-AG	0 / 0	-18.5	-18.5	0.07 (4)		10.00
AG- U	0 / 0	-18.5	-18.5	0.07 (4)		10.00
U-AH	0 / 5173	-18.5	-18.5	0.38 (1)		10.00
AH-AI	0 / 5173	-18.5	-18.5	0.38 (1)		10.00
AI- T	0 / 5173	-18.5	-18.5	0.38 (1)		10.00
T- S	0 / 7800	-18.5	-18.5	0.57 (1)		10.00
S-AJ	0 / 7800	-18.5	-18.5	0.57 (1)		10.00
AJ- R	0 / 7800	-18.5	-18.5	0.57 (1)		10.00
R-AK	0 / 9070	-18.5	-18.5	0.66 (1)		10.00
AK-AL	0 / 9070	-18.5	-18.5	0.66 (1)		10.00
AL-AM	0 / 9070	-18.5	-18.5	0.66 (1)		10.00
AM- Q	0 / 9070	-18.5	-18.5	0.66 (1)		10.00
Q-AN	0 / 7796	-18.5	-18.5	0.57 (1)		10.00
AN- P	0 / 7796	-18.5	-18.5	0.57 (1)		10.00
P- O	0 / 7796	-18.5	-18.5	0.57 (1)		10.00
O-AO	0 / 5171	-18.5	-18.5	0.38 (1)		10.00
AO-AP	0 / 5171	-18.5	-18.5	0.38 (1)		10.00
AP- N	0 / 5171	-18.5	-18.5	0.38 (1)		10.00
N-AQ	0 / 0	-18.5	-18.5	0.07 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-448 / 13	0.06 (1)
C- T	0 / 3299	0.41 (1)
T- D	-1836 / 0	0.23 (1)
D- R	0 / 1494	0.18 (1)
R- E	-609 / 0	0.08 (1)
Q- H	-603 / 0	0.08 (1)
Q- I	0 / 1494	0.18 (1)
O- I	-1834 / 0	0.23 (1)
O- J	0 / 3296	0.41 (1)
N- J	-448 / 13	0.06 (1)
B- U	0 / 5229	0.46 (1)
N- K	0 / 5227	0.46 (1)
R- F	-169 / 0	0.03 (1)
F- Q	-175 / 0	0.03 (1)

TOTAL WEIGHT = 2 X 167 = 333 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 809 (0.53")

CSI: TC=0.74/1.00 (D-E:1) , BC=0.66/1.00 (Q-R:1) , WB=0.46/1.00 (B-U:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (U) (INPUT = 0.90)
JSI METAL= 0.73 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TTWW-m	MT20	5.0	8.0	2.00	2.50
D, F, I						
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
J	TTWW-m	MT20	5.0	8.0	2.00	2.50
K	TMVW-p	MT20	5.0	6.0	2.00	2.75
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)
AQ-AR	0 / 0	-18.5	-18.5	0.07 (4)
AR- M	0 / 0	-18.5	-18.5	0.07 (4)

WEBS

MEMB. FR-TO	MAX. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AQ-AR	10.00		
AR- M	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-374	-374	---	FRONT	VERT	TOTAL	---	C1
F	17-11-8	-113	-113	---	FRONT	VERT	TOTAL	---	C1
G	19-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
J	30-0-8	-374	-374	---	FRONT	VERT	TOTAL	---	C1
N	29-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
P	23-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
S	11-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
U	5-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
W	7-11-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
X	9-11-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
Y	11-11-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
Z	13-11-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AA	15-11-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AB	21-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AC	23-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AD	25-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AE	27-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AF	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AG	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AH	7-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AI	9-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AJ	13-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AK	15-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AL	17-11-8	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AM	19-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AN	21-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AO	25-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AP	27-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AQ	31-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AR	33-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

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PROVINCE OF ONTARIO

Structural component only
DWG# T-2126461

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T2	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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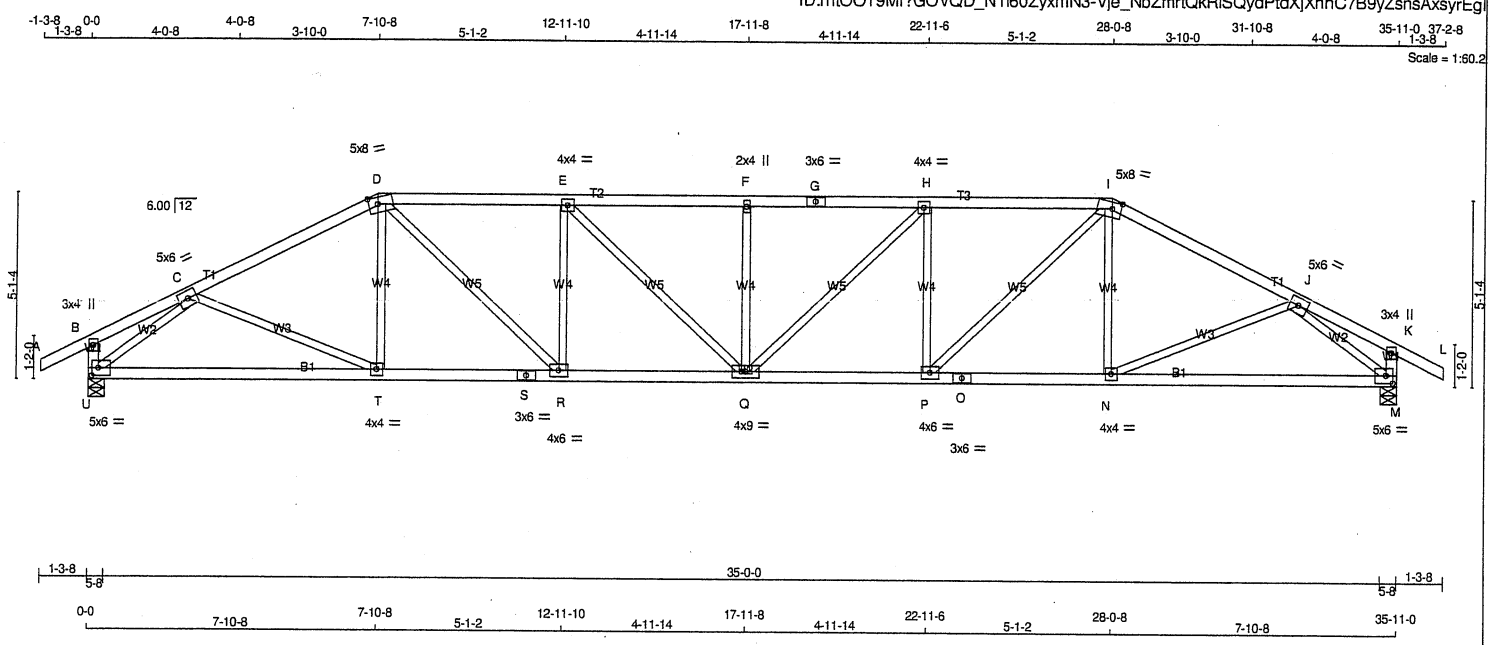

TOTAL WEIGHT = 143 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	2.25	3.00
J	TMWW-t	MT20	5.0	6.0		
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.50	2.25
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BMWW-t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.50	2.25

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126462

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1971	0	1971	0	0	5-8	5-8
M	1971	0	1971	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 28	-84.9	-84.9	0.24 (1)	10.00
C- D	-2696 / 0	-84.9	-84.9	0.44 (1)	3.85
D- E	-3243 / 0	-84.9	-84.9	0.51 (1)	3.46
E- F	-3497 / 0	-84.9	-84.9	0.54 (1)	3.32
F- G	-3497 / 0	-84.9	-84.9	0.54 (1)	3.32
G- H	-3497 / 0	-84.9	-84.9	0.54 (1)	3.32
H- I	-3243 / 0	-84.9	-84.9	0.51 (1)	3.46
I- J	-2696 / 0	-84.9	-84.9	0.44 (1)	3.85
J- K	0 / 28	-84.9	-84.9	0.24 (1)	10.00
K- L	0 / 26	-84.9	-84.9	0.11 (1)	10.00
U- B	-170 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-170 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 / 2144	-18.5	-18.5	0.48 (1)	10.00
T- S	0 / 2397	-18.5	-18.5	0.52 (1)	10.00
S- R	0 / 2397	-18.5	-18.5	0.52 (1)	10.00
R- Q	0 / 3243	-18.5	-18.5	0.57 (1)	10.00
Q- P	0 / 3243	-18.5	-18.5	0.57 (1)	10.00
P- O	0 / 2397	-18.5	-18.5	0.52 (1)	10.00
O- N	0 / 2397	-18.5	-18.5	0.52 (1)	10.00
N- M	0 / 2144	-18.5	-18.5	0.48 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 275	0.06 (1)
T- D	0 / 103	0.04 (4)
D- R	0 / 1162	0.26 (1)
R- E	-727 / 0	0.28 (1)
E- Q	0 / 353	0.08 (1)
Q- F	-390 / 0	0.15 (1)
Q- H	0 / 353	0.08 (1)
P- H	-727 / 0	0.28 (1)
P- I	0 / 1162	0.26 (1)
N- I	0 / 103	0.04 (4)
N- J	0 / 275	0.06 (1)
U- C	-2761 / 0	0.63 (1)
J- M	-2761 / 0	0.63 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.57/1.00 (P-Q:1) , WB=0.63/1.00 (J-M:1) , SSI=0.20/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.70 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T3	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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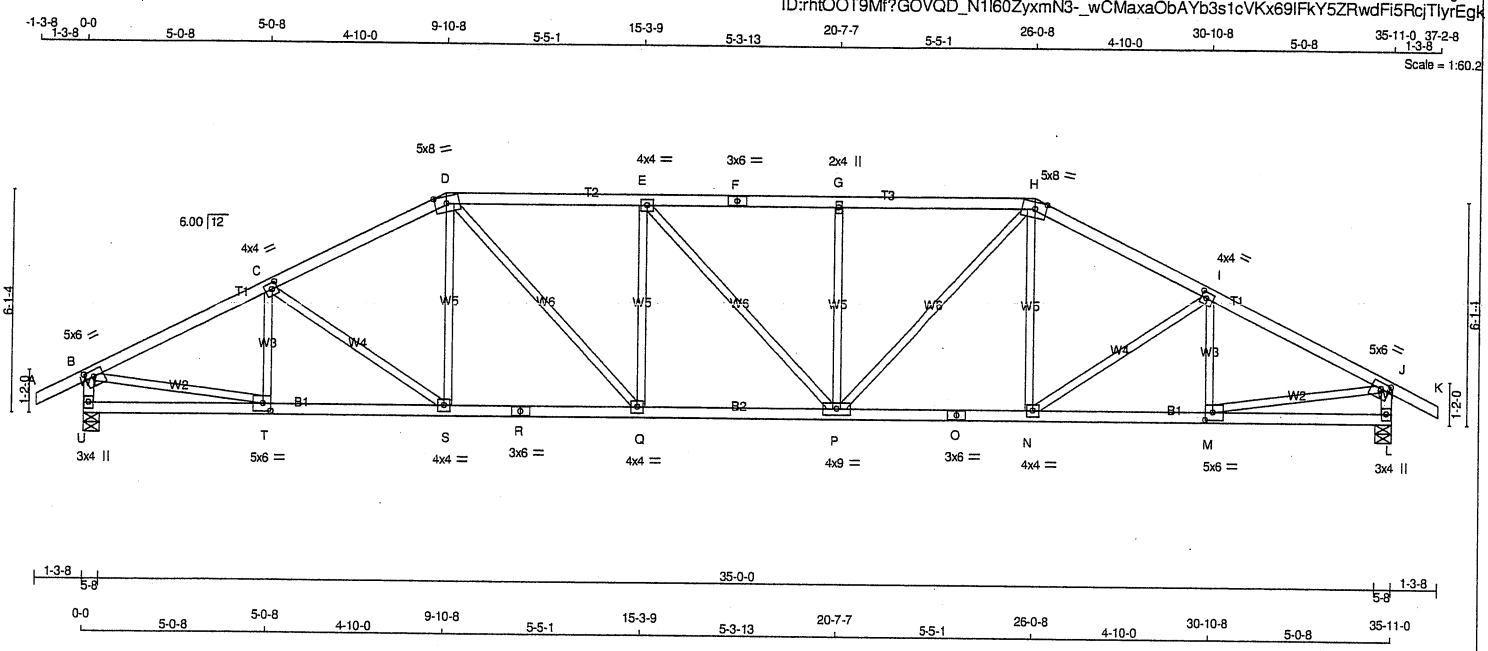

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.75
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	5.0	6.0	2.00	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N, Q, S						
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1971	0	1971	0	0	5-8	5-8
L	1971	0	1971	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2723 / 0	-84.9	-84.9	0.36 (1)	3.91
C- D	-2588 / 0	-84.9	-84.9	0.34 (1)	4.01
D- E	-2851 / 0	-84.9	-84.9	0.40 (1)	3.78
E- F	-2849 / 0	-84.9	-84.9	0.37 (1)	3.82
F- G	-2849 / 0	-84.9	-84.9	0.37 (1)	3.82
G- H	-2849 / 0	-84.9	-84.9	0.40 (1)	3.78
H- I	-2588 / 0	-84.9	-84.9	0.34 (1)	4.01
I- J	-2723 / 0	-84.9	-84.9	0.36 (1)	3.91
J- K	0 / 26	-84.9	-84.9	0.11 (1)	10.00
U- B	-1928 / 0	0.0	0.0	0.19 (1)	6.06
L- J	-1928 / 0	0.0	0.0	0.19 (1)	6.06
U- T	0 / 0	-18.5	-18.5	0.10 (4)	10.00
T- S	0 / 2454	-18.5	-18.5	0.45 (1)	10.00
S- R	0 / 2298	-18.5	-18.5	0.43 (1)	10.00
R- Q	0 / 2298	-18.5	-18.5	0.43 (1)	10.00
Q- P	0 / 2851	-18.5	-18.5	0.51 (1)	10.00
P- O	0 / 2298	-18.5	-18.5	0.43 (1)	10.00
O- N	0 / 2298	-18.5	-18.5	0.43 (1)	10.00
N- M	0 / 2454	-18.5	-18.5	0.45 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-327 / 0	0.07 (1)
S- D	-194 / 0	0.12 (1)
D- Q	0 / 211	0.05 (4)
Q- E	0 / 810	0.18 (1)
E- P	-494 / 0	0.29 (1)
P- G	-2 / 0	0.00 (1)
P- H	-493 / 0	0.29 (1)
N- H	0 / 807	0.18 (1)
N- I	0 / 212	0.05 (4)
M- I	-193 / 0	0.12 (1)
B- T	-327 / 0	0.07 (1)
M- J	0 / 2489	0.56 (1)
	0 / 2489	0.56 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.40/1.00 (G-H:1) , BC=0.51/1.00 (P-Q:1) , WB=0.56/1.00 (B-T:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)
JSI METAL= 0.70 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126463

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T4	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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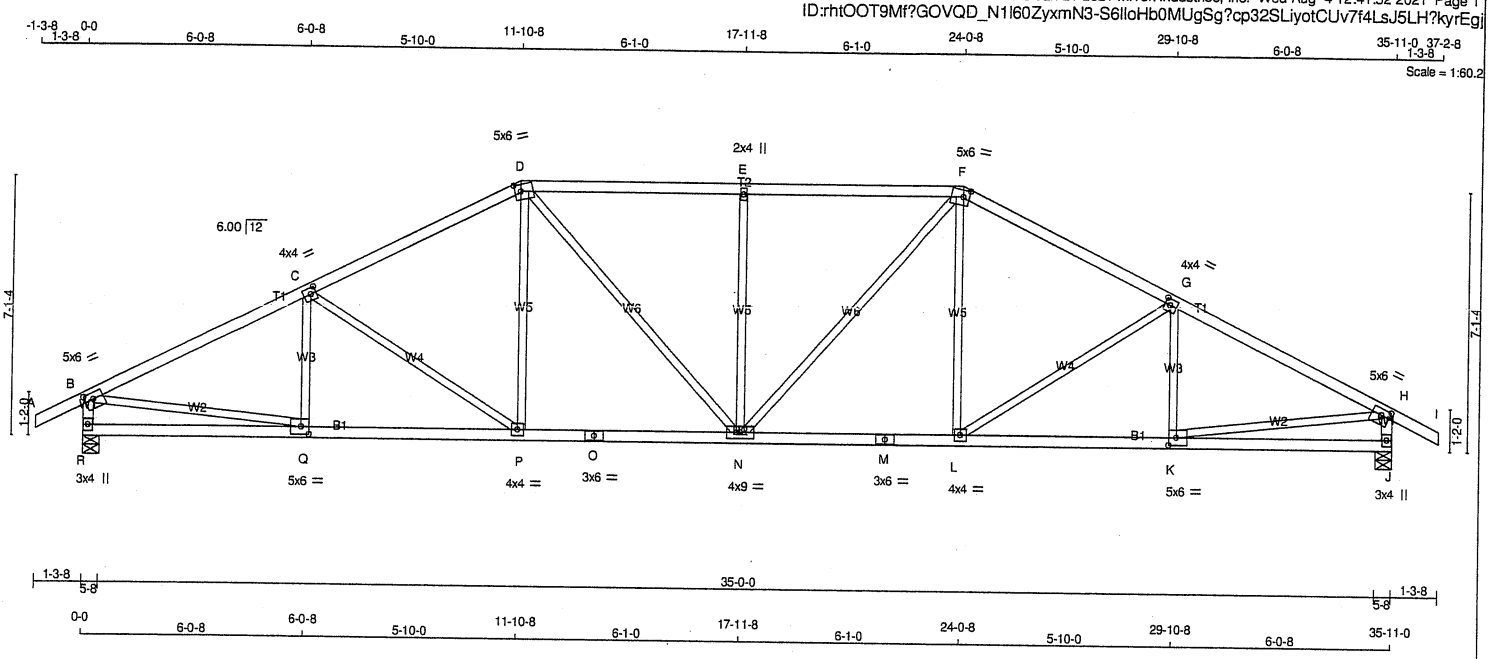

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.00	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1971	0	1971	0	0	5-8	5-8
J	1971	0	1971	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
J	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2775 / 0	-84.9	-84.9	0.54 (1)	3.70
C- D	-2457 / 0	-84.9	-84.9	0.48 (1)	3.94
D- E	-2508 / 0	-84.9	-84.9	0.51 (1)	3.86
E- F	-2508 / 0	-84.9	-84.9	0.51 (1)	3.86
F- G	-2457 / 0	-84.9	-84.9	0.48 (1)	3.94
G- H	-2775 / 0	-84.9	-84.9	0.54 (1)	3.70
H- I	0 / 26	-84.9	-84.9	0.11 (1)	10.00
R- B	-1922 / 0	0.0	0.0	0.19 (1)	6.07
J- H	-1922 / 0	0.0	0.0	0.19 (1)	6.07
R- Q	0 / 0	-18.5	-18.5	0.14 (4)	10.00
Q- P	0 / 2505	-18.5	-18.5	0.48 (1)	10.00
P- O	0 / 2177	-18.5	-18.5	0.42 (1)	10.00
O- N	0 / 2177	-18.5	-18.5	0.42 (1)	10.00
N- M	0 / 2177	-18.5	-18.5	0.42 (1)	10.00
M- L	0 / 2177	-18.5	-18.5	0.42 (1)	10.00
L- K	0 / 2505	-18.5	-18.5	0.48 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-240 / 8	0.06 (1)
C- P	-398 / 0	0.38 (1)
P- D	0 / 332	0.07 (1)
D- N	0 / 496	0.11 (1)
N- E	-633 / 0	0.55 (1)
N- F	0 / 496	0.11 (1)
L- F	0 / 332	0.07 (1)
L- G	-398 / 0	0.38 (1)
K- G	-240 / 8	0.06 (1)
B- Q	0 / 2530	0.57 (1)
K- H	0 / 2530	0.57 (1)

TOTAL WEIGHT = 147 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.54/1.00 (G-H:1) , BC=0.48/1.00 (K-L:1) , WB=0.57/1.00 (H-K:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.67 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126464

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T5	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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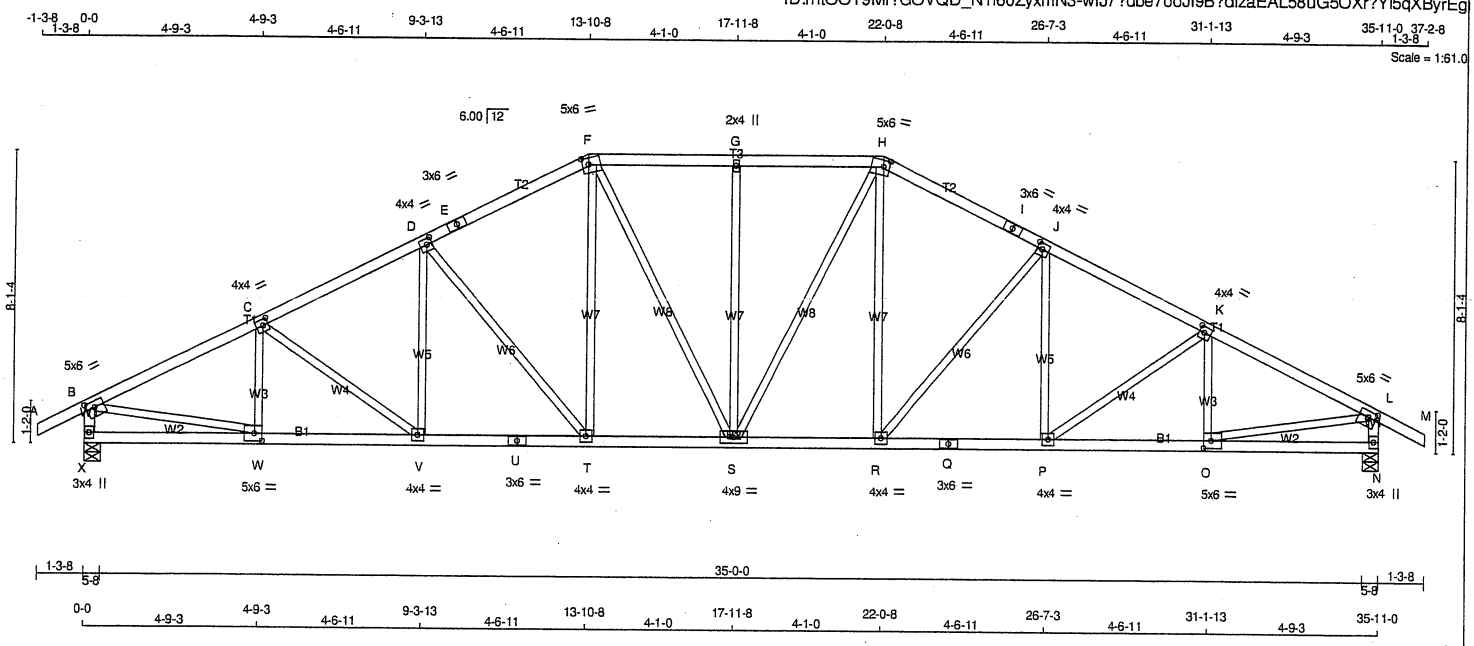

TOTAL WEIGHT = 163 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
X - U	2x4	DRY	No.2	SPF
U - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C, D, J, K						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.25	2.00
I	TS-t	MT20	3.0	6.0		
L	TMVW-t	MT20	5.0	6.0	2.00	2.75
N	BMV1+p	MT20	3.0	4.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P, R, T, V						
P	BMWW-t	MT20	4.0	4.0		
Q	BS-t	MT20	3.0	6.0		
S	BMWWW-t	MT20	4.0	9.0		
U	BS-t	MT20	3.0	6.0		
W	BMWW-t	MT20	5.0	6.0	2.50	2.50
X	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
08/15/14
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126465

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	1971	0	1971	0	0	5-8	5-8
N	1971	0	1971	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
N	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2692 / 0	-84.9	-84.9	0.33 (1)	3.96
C- D	-2634 / 0	-84.9	-84.9	0.28 (1)	4.05
D- E	-2280 / 0	-84.9	-84.9	0.27 (1)	4.31
E- F	-2280 / 0	-84.9	-84.9	0.27 (1)	4.31
F- G	-2156 / 0	-84.9	-84.9	0.22 (1)	4.45
G- H	-2156 / 0	-84.9	-84.9	0.22 (1)	4.45
H- I	-2280 / 0	-84.9	-84.9	0.27 (1)	4.31
I- J	-2280 / 0	-84.9	-84.9	0.27 (1)	4.31
J- K	-2634 / 0	-84.9	-84.9	0.28 (1)	4.05
K- L	-2692 / 0	-84.9	-84.9	0.33 (1)	3.96
L- M	0 / 26	-84.9	-84.9	0.11 (1)	10.00
X- B	-1930 / 0	0.0	0.0	0.20 (1)	6.06
N- L	-1930 / 0	0.0	0.0	0.20 (1)	6.06
X- W	0 / 0	-18.5	-18.5	0.09 (4)	10.00
W- V	0 / 2421	-18.5	-18.5	0.43 (1)	10.00
V- U	0 / 2356	-18.5	-18.5	0.42 (1)	10.00
U- T	0 / 2356	-18.5	-18.5	0.42 (1)	10.00
T- S	0 / 2029	-18.5	-18.5	0.36 (1)	10.00
S- R	0 / 2029	-18.5	-18.5	0.36 (1)	10.00
R- Q	0 / 2356	-18.5	-18.5	0.42 (1)	10.00
Q- P	0 / 2356	-18.5	-18.5	0.42 (1)	10.00
P- O	0 / 2421	-18.5	-18.5	0.43 (1)	10.00
O- N	0 / 0	-18.5	-18.5	0.09 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-349 / 0	0.07 (1)
C- V	-80 / 0	0.04 (1)
V- D	0 / 133	0.04 (4)
D- T	-516 / 0	0.52 (1)
T- F	0 / 478	0.11 (1)
F- S	0 / 274	0.06 (1)
S- G	-416 / 0	0.53 (1)
S- H	0 / 274	0.06 (1)
R- H	0 / 478	0.11 (1)
R- J	-516 / 0	0.52 (1)
P- J	0 / 133	0.04 (4)
P- K	-80 / 0	0.04 (1)
O- K	-349 / 0	0.07 (1)
B- W	0 / 2460	0.55 (1)
O- L	0 / 2460	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.33/1.00 (K-L:1) , BC=0.43/1.00 (O-P:1) , WB=0.55/1.00 (L-O:1) , SSI=0.17/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.72 (U) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T6	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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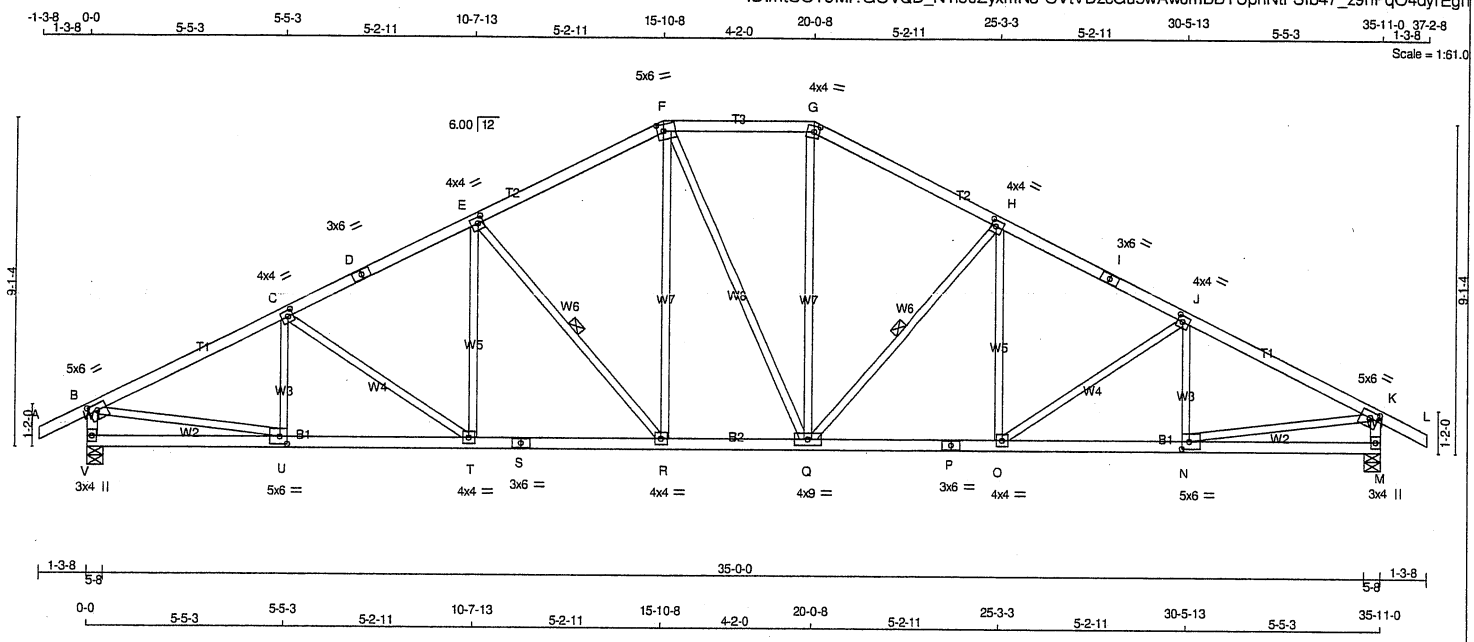

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C, E, H, J						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TTW-m	MT20	4.0	4.0	2.00	1.75
I	TS-t	MT20	3.0	6.0		
K	TMVW-t	MT20	5.0	6.0	2.00	2.75
M	BMV1+p	MT20	3.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O, R, T						
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
S	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1971	0	1971	0	0	5-8	5-8
M	1971	0	1971	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-R, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2736 / 0	-84.9	-84.9	0.42 (1)	3.85
C- D	-2561 / 0	-84.9	-84.9	0.35 (1)	4.04
D- E	-2561 / 0	-84.9	-84.9	0.35 (1)	4.04
E- F	-2105 / 0	-84.9	-84.9	0.34 (1)	4.38
F- G	-1873 / 0	-84.9	-84.9	0.23 (1)	4.70
G- H	-2106 / 0	-84.9	-84.9	0.34 (1)	4.38
H- I	-2561 / 0	-84.9	-84.9	0.35 (1)	4.04
I- J	-2561 / 0	-84.9	-84.9	0.35 (1)	4.04
J- K	-2737 / 0	-84.9	-84.9	0.42 (1)	3.85
K- L	0 / 26	-84.9	-84.9	0.11 (1)	10.00
V- B	-1926 / 0	0.0	0.0	0.19 (1)	6.06
M- K	-1926 / 0	0.0	0.0	0.19 (1)	6.06
V- U	0 / 0	-18.5	-18.5	0.11 (4)	10.00
U- T	0 / 2464	-18.5	-18.5	0.45 (1)	10.00
T- S	0 / 2291	-18.5	-18.5	0.42 (1)	10.00
S- R	0 / 2291	-18.5	-18.5	0.42 (1)	10.00
R- Q	0 / 1871	-18.5	-18.5	0.35 (1)	10.00
Q- P	0 / 2290	-18.5	-18.5	0.42 (1)	10.00
P- O	0 / 2290	-18.5	-18.5	0.42 (1)	10.00
O- N	0 / 2464	-18.5	-18.5	0.44 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-287 / 0	0.07 (1)
C- T	-210 / 0	0.15 (1)
T- E	0 / 219	0.05 (4)
E- R	-653 / 0	0.29 (1)
R- F	0 / 576	0.13 (1)
F- Q	0 / 3	0.00 (1)
Q- G	0 / 580	0.13 (1)
Q- H	-651 / 0	0.29 (1)
O- H	0 / 216	0.05 (4)
O- J	-210 / 0	0.15 (1)
N- J	-287 / 0	0.07 (1)
B- U	0 / 2494	0.56 (1)
N- K	0 / 2495	0.56 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.42/1.00 (J-K:1) , BC=0.45/1.00 (T-U:1) , WB=0.56/1.00 (K-N:1) , SSI=0.19/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.68 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126466

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T7	9	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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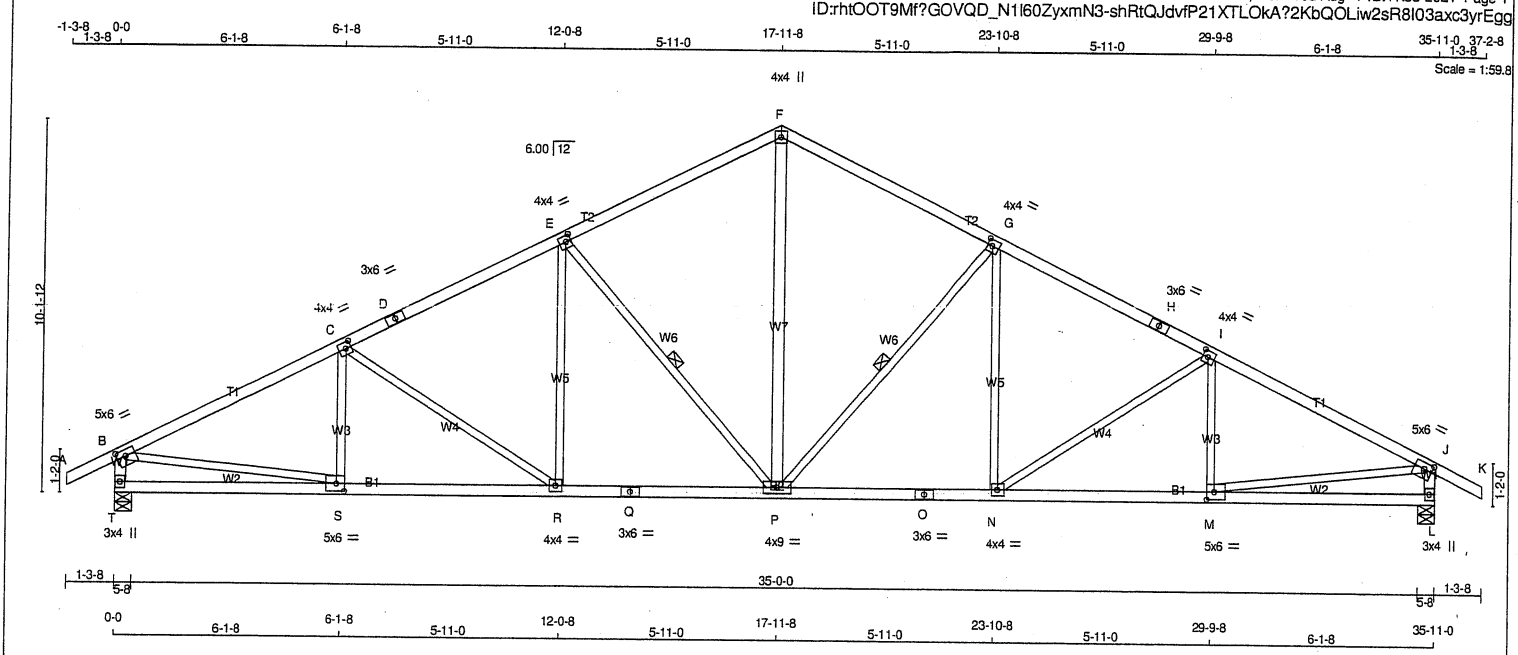

TOTAL WEIGHT = 9 X 154 = 1388 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C, E, G, I						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.00	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1971	0	1971	0	0	5-8	5-8
L	1971	0	1971	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-P, E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2764 / 0	-84.9	-84.9	0.54 (1)	3.72
C- D	-2469 / 0	-84.9	-84.9	0.44 (1)	4.02
D- E	-2469 / 0	-84.9	-84.9	0.44 (1)	4.02
E- F	-1923 / 0	-84.9	-84.9	0.42 (1)	4.45
F- G	-1923 / 0	-84.9	-84.9	0.42 (1)	4.45
G- H	-2469 / 0	-84.9	-84.9	0.44 (1)	4.02
H- I	-2469 / 0	-84.9	-84.9	0.44 (1)	4.02
I- J	-2764 / 0	-84.9	-84.9	0.54 (1)	3.72
J- K	0 / 26	-84.9	-84.9	0.11 (1)	10.00
T- B	-1922 / 0	0.0	0.0	0.19 (1)	6.07
L- J	-1922 / 0	0.0	0.0	0.19 (1)	6.07
T- S	0 / 0	-18.5	-18.5	0.15 (4)	10.00
S- R	0 / 2491	-18.5	-18.5	0.46 (1)	10.00
R- Q	0 / 2208	-18.5	-18.5	0.44 (1)	10.00
Q- P	0 / 2208	-18.5	-18.5	0.44 (1)	10.00
P- O	0 / 2208	-18.5	-18.5	0.44 (1)	10.00
O- N	0 / 2208	-18.5	-18.5	0.44 (1)	10.00
N- M	0 / 2491	-18.5	-18.5	0.46 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	0 / 1295	0.21 (1)
N- G	-778 / 0	0.44 (1)
N- I	0 / 293	0.07 (1)
M- I	-340 / 0	0.33 (1)
E- P	-229 / 13	0.06 (1)
R- E	-778 / 0	0.44 (1)
C- R	0 / 293	0.07 (1)
S- C	-340 / 0	0.33 (1)
B- S	-229 / 13	0.06 (1)
M- J	0 / 2515	0.57 (1)
	0 / 2515	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.54/1.00 (I-J:1) , BC=0.46/1.00 (R-S:1) , WB=0.57/1.00 (B-S:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.68 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126467

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T7CP	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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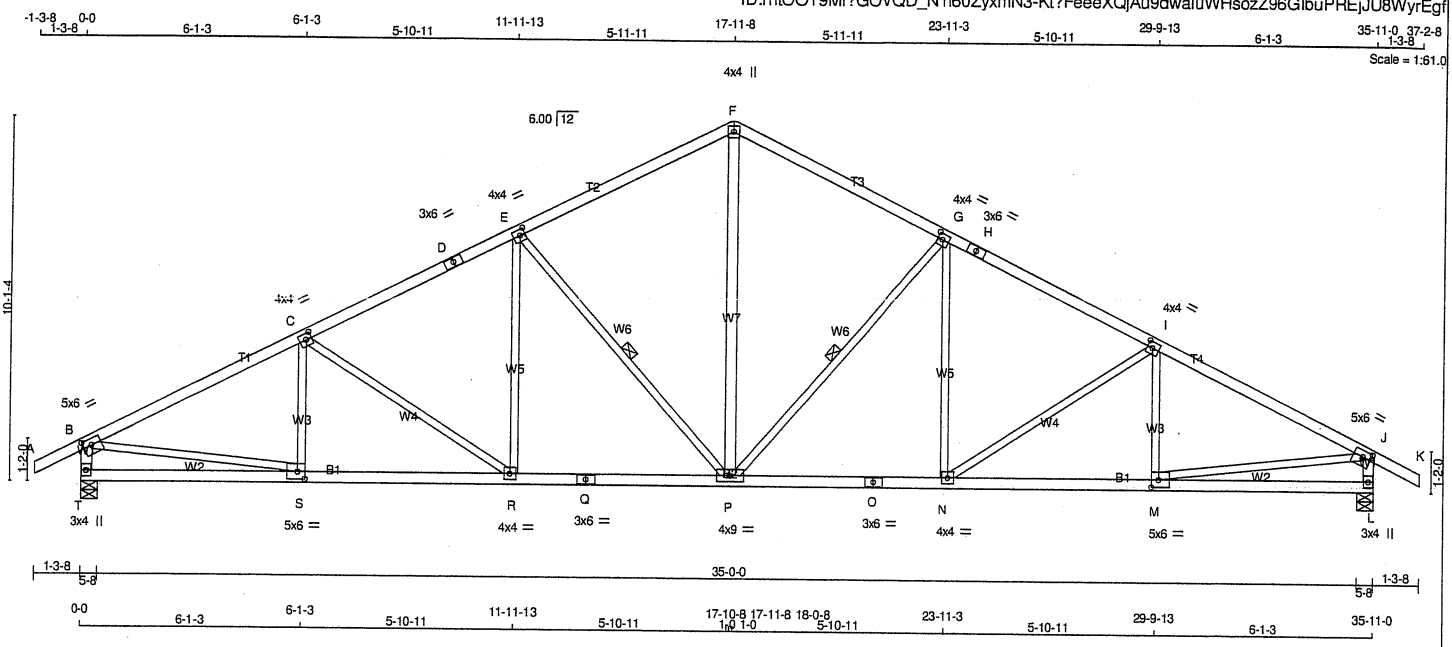

TOTAL WEIGHT = 154 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C, E, G, I						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.00	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1971	0	1971	0	0	5-8	5-8
L	1971	0	1971	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2763 / 0	-84.9	-84.9	0.54 (1)	3.72
C- D	-2473 / 0	-84.9	-84.9	0.44 (1)	4.02
D- E	-2473 / 0	-84.9	-84.9	0.44 (1)	4.02
E- F	-1923 / 0	-84.9	-84.9	0.43 (1)	4.44
F- G	-1923 / 0	-84.9	-84.9	0.43 (1)	4.44
G- H	-2473 / 0	-84.9	-84.9	0.44 (1)	4.02
H- I	-2473 / 0	-84.9	-84.9	0.44 (1)	4.02
I- J	-2763 / 0	-84.9	-84.9	0.54 (1)	3.72
J- K	0 / 26	-84.9	-84.9	0.11 (1)	10.00
T- B	-1922 / 0	0.0	0.0	0.19 (1)	6.07
L- J	-1922 / 0	0.0	0.0	0.19 (1)	6.07
T- S	0 / 0	-18.5	-18.5	0.15 (4)	10.00
S- R	0 / 2490	-18.5	-18.5	0.46 (1)	10.00
R- Q	0 / 2212	-18.5	-18.5	0.44 (1)	10.00
Q- P	0 / 2212	-18.5	-18.5	0.44 (1)	10.00
P- O	0 / 2212	-18.5	-18.5	0.44 (1)	10.00
O- N	0 / 2212	-18.5	-18.5	0.44 (1)	10.00
N- M	0 / 2490	-18.5	-18.5	0.46 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-232 / 11	0.06 (1)
C- R	-333 / 0	0.32 (1)
R- E	0 / 290	0.07 (1)
N- G	0 / 290	0.07 (1)
N- I	-333 / 0	0.32 (1)
M- I	-232 / 11	0.06 (1)
B- S	0 / 2514	0.57 (1)
M- J	0 / 2514	0.57 (1)
P- F	0 / 1291	0.21 (1)
E- P	-779 / 0	0.44 (1)
P- G	-779 / 0	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.54/1.00 (I-J:1) , BC=0.46/1.00 (M-N:1) , WB=0.57/1.00 (J-M:1) , SSI=0.21/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.68 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126468

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T7SCP	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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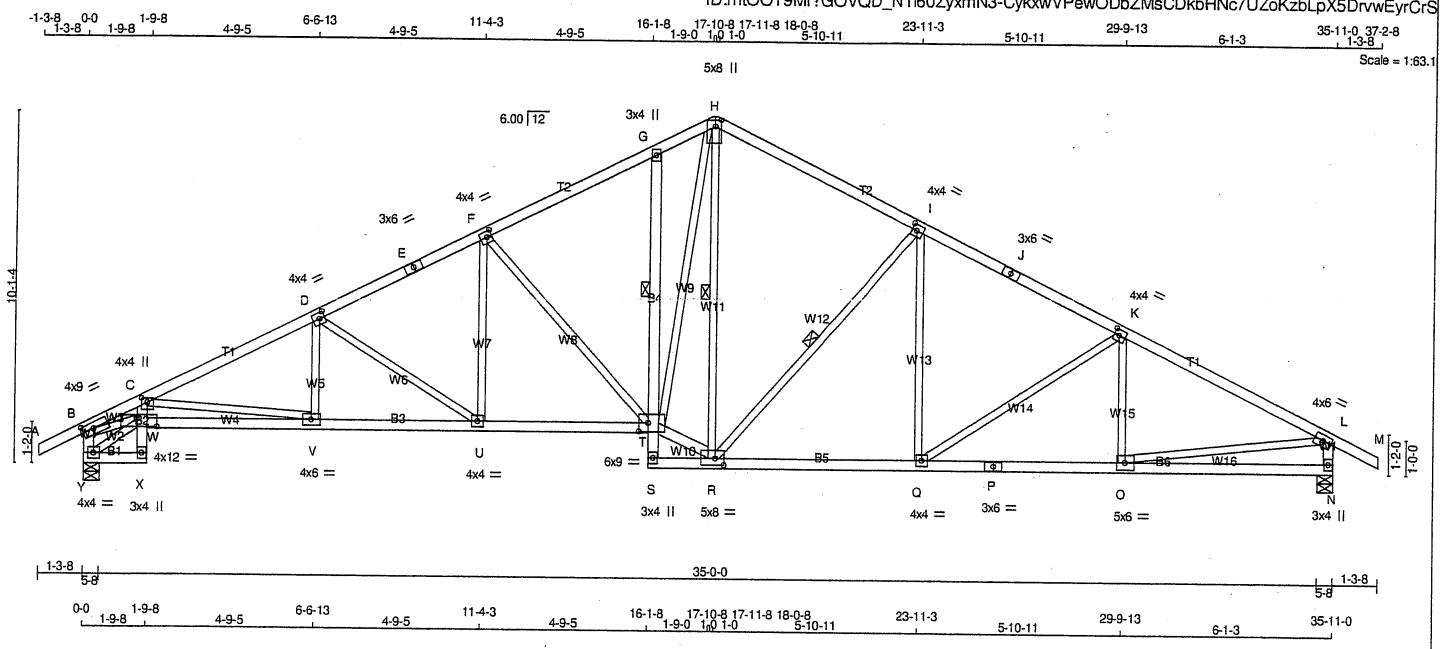

TOTAL WEIGHT = 169 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - X	2x4	DRY	No.2	SPF
X - C	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D, F, I, K						
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	TTWW+p	MT20	5.0	8.0	2.25	2.00
J	TS-t	MT20	3.0	6.0		
L	TMVW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	8.0	2.25	3.00
S	BMV+p	MT20	3.0	4.0		
T	BVMWWW-l	MT20	6.0	9.0	3.00	3.25
U	BMWW-t	MT20	4.0	4.0		
V	BMWW-t	MT20	4.0	6.0		
W	BVMWW-l	MT20	4.0	12.0	2.75	6.75
X	BMV+p	MT20	3.0	4.0		
Y	BMVW1-t	MT20	4.0	4.0		

LICENSED PROFESSIONAL ENGINEER
08/15/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126493

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1313	0	1313	0	0	5-8	5-8
Y	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
Y	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, Y

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-T, I-R, H-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-2993 / 0	-56.6	-56.6	0.21 (1)	3.89
C- D	-2384 / 0	-56.6	-56.6	0.32 (1)	4.19
D- E	-1964 / 0	-56.6	-56.6	0.22 (1)	4.64
E- F	-1964 / 0	-56.6	-56.6	0.22 (1)	4.64
F- G	-1550 / 0	-56.6	-56.6	0.16 (1)	5.16
G- H	-1552 / 0	-56.6	-56.6	0.08 (1)	5.22
H- I	-1284 / 0	-56.6	-56.6	0.27 (1)	5.41
I- J	-1645 / 0	-56.6	-56.6	0.26 (1)	4.94
J- K	-1645 / 0	-56.6	-56.6	0.26 (1)	4.94
K- L	-1840 / 0	-56.6	-56.6	0.32 (1)	4.67
L- M	0 / 17	-56.6	-56.6	0.07 (1)	10.00
Y- B	-1144 / 0	0.0	0.0	0.12 (1)	7.43
N- L	-1280 / 0	0.0	0.0	0.13 (1)	7.13
Y- X	0 / 242	-12.3	-12.3	0.04 (1)	10.00
X- W	0 / 11	0.0	0.0	0.36 (1)	10.00
W- C	0 / 334	0.0	0.0	0.42 (1)	10.00
W- V	0 / 3102	-12.3	-12.3	0.54 (1)	10.00
V- U	0 / 2153	-12.3	-12.3	0.38 (1)	10.00
U- T	0 / 1753	-12.3	-12.3	0.32 (1)	10.00
S- T	-24 / 0	0.0	0.0	0.04 (1)	7.81
T- G	-205 / 0	0.0	0.0	0.05 (1)	6.25
S- R	0 / 36	-12.3	-12.3	0.09 (4)	10.00
R- Q	0 / 1472	-12.3	-12.3	0.32 (1)	10.00
Q- P	0 / 1658	-12.3	-12.3	0.31 (1)	10.00
P- O	0 / 1658	-12.3	-12.3	0.31 (1)	10.00
O- N	0 / 0	-12.3	-12.3	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	-957 / 0	0.40 (1)
V- D	0 / 128	0.03 (4)
D- U	-479 / 0	0.27 (1)
U- F	0 / 329	0.07 (1)
F- T	-558 / 0	0.60 (1)
T- R	0 / 1269	0.20 (1)
R- I	-515 / 0	0.30 (1)
Q- I	0 / 188	0.04 (4)
Q- K	-223 / 0	0.21 (1)
O- K	-153 / 8	0.04 (1)
O- L	0 / 1674	0.38 (1)
R- H	-152 / 0	0.11 (1)
Y- W	-291 / 0	0.04 (1)
B- W	0 / 2669	0.60 (1)
T- H	0 / 1276	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.32/1.00 (C-D:1) , BC=0.54/1.00 (V-W:1) , WB=0.60/1.00 (B-W:1) , SSI=0.36/1.00 (C-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (R) (INPUT = 0.90)
JSI METAL= 0.65 (W) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T8	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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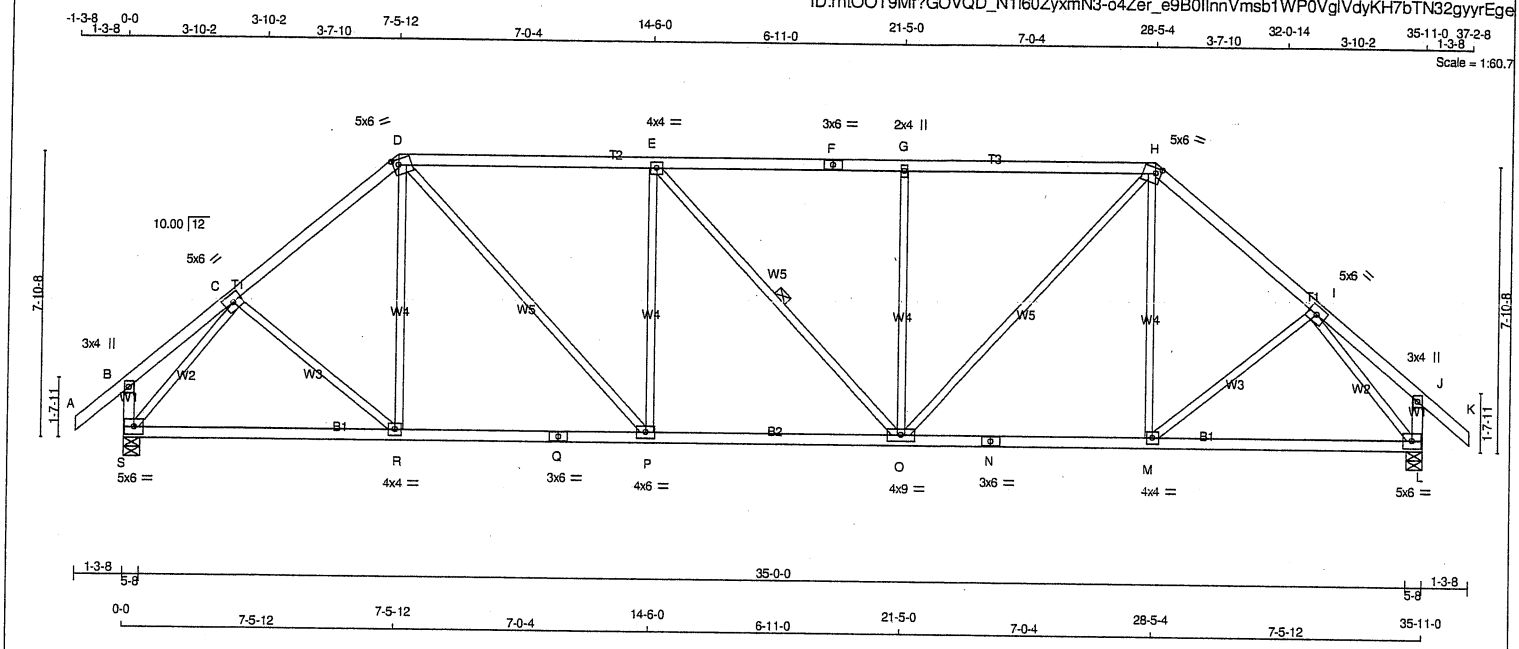

TOTAL WEIGHT = 2 X 159 = 317 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	1.75	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	1.75
I	TMWW-t	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1974	0	1974	0	0	5-8	5-8
L	1974	0	1974	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1399	901 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1399	901 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 32	-84.9	-84.9	0.20 (1)	10.00
C- D	-1936 / 0	-84.9	-84.9	0.30 (1)	4.58
D- E	-2178 / 0	-84.9	-84.9	0.80 (1)	3.73
E- F	-2177 / 0	-84.9	-84.9	0.80 (1)	3.73
F- G	-2177 / 0	-84.9	-84.9	0.80 (1)	3.73
G- H	-2177 / 0	-84.9	-84.9	0.80 (1)	3.74
H- I	-1936 / 0	-84.9	-84.9	0.30 (1)	4.58
I- J	0 / 32	-84.9	-84.9	0.20 (1)	10.00
J- K	0 / 38	-84.9	-84.9	0.12 (1)	10.00
S- B	-192 / 0	0.0	0.0	0.02 (1)	7.81
L- J	-192 / 0	0.0	0.0	0.02 (1)	7.81
S- R	0 / 1350	-18.5	-18.5	0.36 (4)	10.00
R- Q	0 / 1468	-18.5	-18.5	0.38 (4)	10.00
Q- P	0 / 1468	-18.5	-18.5	0.38 (4)	10.00
P- O	0 / 2178	-18.5	-18.5	0.43 (1)	10.00
O- N	0 / 1469	-18.5	-18.5	0.38 (4)	10.00
N- M	0 / 1469	-18.5	-18.5	0.38 (4)	10.00
M- L	0 / 1350	-18.5	-18.5	0.36 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 152	0.03 (4)
R- D	0 / 109	0.04 (4)
D- P	0 / 1043	0.23 (1)
P- E	-642 / 0	0.75 (1)
E- O	-2 / 0	0.00 (1)
O- G	-641 / 0	0.75 (1)
O- H	0 / 1041	0.23 (1)
M- H	0 / 110	0.04 (4)
M- I	0 / 152	0.03 (4)
S- C	-2190 / 0	0.79 (1)
I- L	-2190 / 0	0.79 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.43/1.00 (O-P:1) , WB=0.79/1.00 (I-L:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.49 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126469

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T9	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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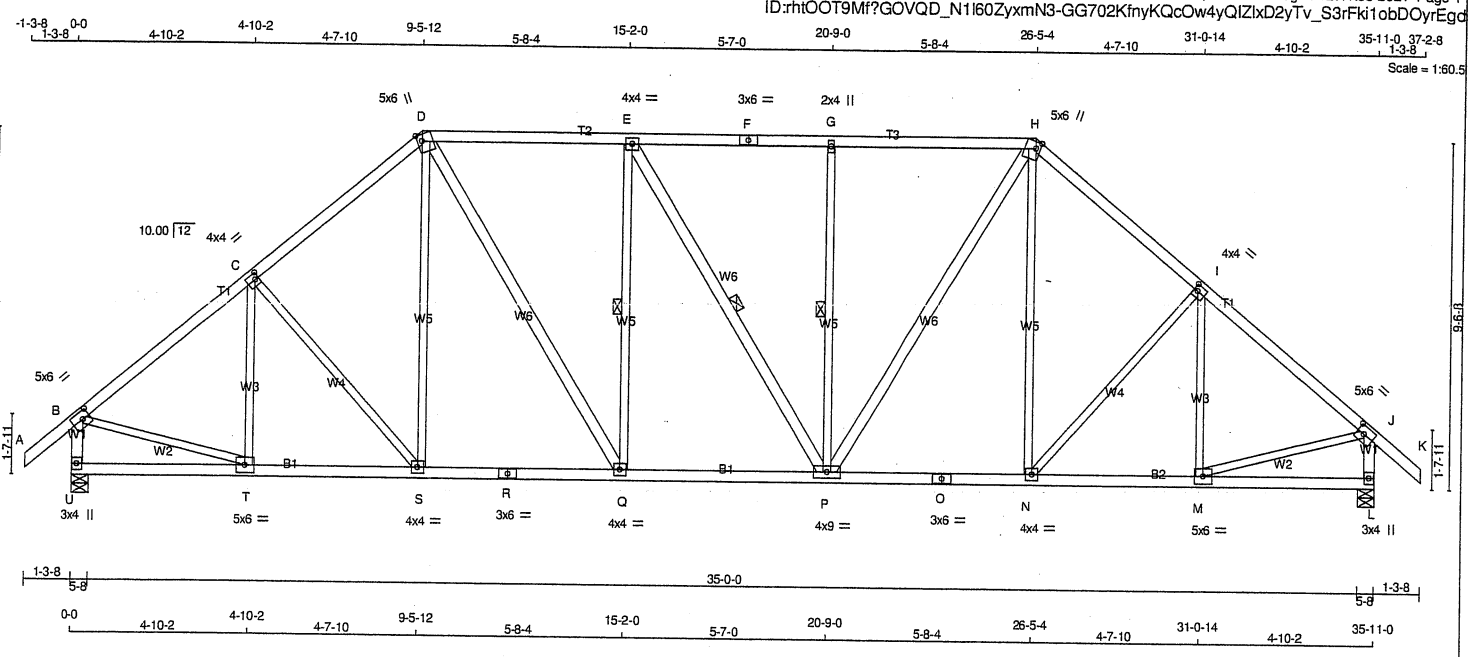

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-t	MT20	5.0	6.0	2.50	2.50
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N, Q, S						
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126470

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1974	0	1974	0	0	5-8	5-8
L	1974	0	1974	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1399	901 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1399	901 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1928 / 0	-84.9	-84.9	0.31 (1)	4.58
C- D	-1866 / 0	-84.9	-84.9	0.30 (1)	4.64
D- E	-1792 / 0	-84.9	-84.9	0.36 (1)	4.63
E- F	-1791 / 0	-84.9	-84.9	0.33 (1)	4.67
F- G	-1791 / 0	-84.9	-84.9	0.33 (1)	4.67
G- H	-1791 / 0	-84.9	-84.9	0.36 (1)	4.64
H- I	-1866 / 0	-84.9	-84.9	0.30 (1)	4.64
I- J	-1928 / 0	-84.9	-84.9	0.31 (1)	4.58
J- K	0 / 38	-84.9	-84.9	0.12 (1)	10.00
U- B	-1935 / 0	0.0	0.0	0.21 (1)	6.05
L- J	-1935 / 0	0.0	0.0	0.21 (1)	6.05
U- T	0 / 0	-18.5	-18.5	0.09 (4)	10.00
T- S	0 / 1504	-18.5	-18.5	0.30 (1)	10.00
S- R	0 / 1409	-18.5	-18.5	0.29 (1)	10.00
R- Q	0 / 1409	-18.5	-18.5	0.29 (1)	10.00
Q- P	0 / 1793	-18.5	-18.5	0.35 (1)	10.00
P- O	0 / 1409	-18.5	-18.5	0.28 (1)	10.00
O- N	0 / 1409	-18.5	-18.5	0.28 (1)	10.00
N- M	0 / 1504	-18.5	-18.5	0.30 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.09 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-313 / 0	0.15 (1)
C- S	-147 / 0	0.14 (1)
S- D	0 / 206	0.05 (4)
D- Q	0 / 732	0.12 (1)
Q- E	-517 / 0	0.31 (1)
E- P	-3 / 0	0.00 (1)
P- G	-516 / 0	0.31 (1)
P- H	0 / 729	0.12 (1)
N- H	0 / 208	0.05 (4)
N- I	-147 / 0	0.14 (1)
M- I	-313 / 0	0.15 (1)
B- T	0 / 1558	0.35 (1)
M- J	0 / 1557	0.35 (1)

TOTAL WEIGHT = 2 X 187 = 375 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.35/1.00 (P-Q:1) , WB=0.35/1.00 (B-T:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T10	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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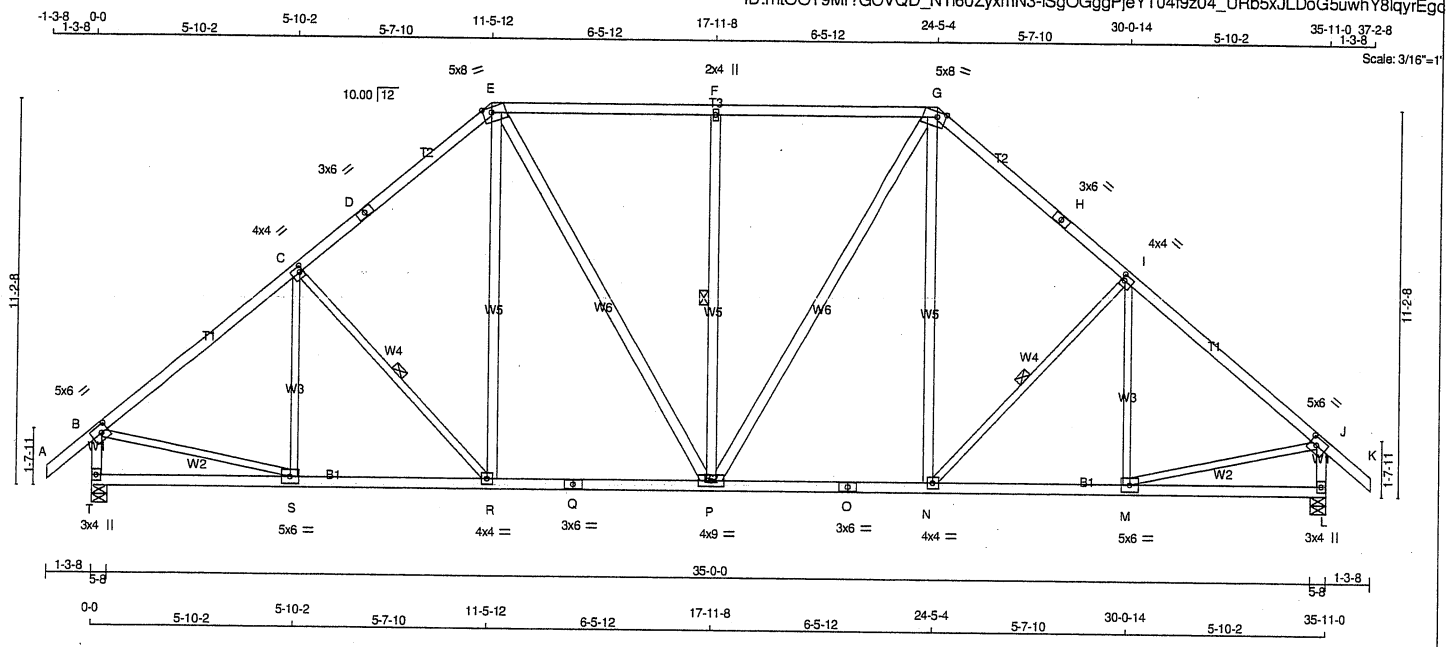

TOTAL WEIGHT = 2 X 193 = 386 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
C - R	2x3	DRY	No.2	SPF
N - I	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-t	MT20	5.0	6.0	2.50	2.50
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1974	0	1974	0	0	5-8	5-8
L	1974	0	1974	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1399	901 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1399	901 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	
B- C	-1960 / 0	-84.9	-84.9	0.47 (1)	4.37	
C- D	-1776 / 0	-84.9	-84.9	0.44 (1)	4.57	
D- E	-1776 / 0	-84.9	-84.9	0.44 (1)	4.57	
E- F	-1572 / 0	-84.9	-84.9	0.50 (1)	4.65	
F- G	-1572 / 0	-84.9	-84.9	0.50 (1)	4.65	
G- H	-1776 / 0	-84.9	-84.9	0.44 (1)	4.57	
H- I	-1776 / 0	-84.9	-84.9	0.44 (1)	4.57	
I- J	-1960 / 0	-84.9	-84.9	0.47 (1)	4.37	
J- K	0 / 38	-84.9	-84.9	0.12 (1)	10.00	
T- B	-1928 / 0	0.0	0.0	0.21 (1)	6.06	
L- J	-1928 / 0	0.0	0.0	0.21 (1)	6.06	
T- S	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
S- R	0 / 1535	-18.5	-18.5	0.32 (1)	10.00	
R- Q	0 / 1337	-18.5	-18.5	0.30 (1)	10.00	
Q- P	0 / 1337	-18.5	-18.5	0.30 (1)	10.00	
P- O	0 / 1337	-18.5	-18.5	0.30 (1)	10.00	
O- N	0 / 1337	-18.5	-18.5	0.30 (1)	10.00	
N- M	0 / 1535	-18.5	-18.5	0.32 (1)	10.00	
M- L	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-227 / 9	0.16 (1)
C- R	-296 / 0	0.14 (1)
R- E	0 / 327	0.05 (1)
E- P	0 / 461	0.07 (1)
P- F	-673 / 0	0.44 (1)
P- G	0 / 461	0.07 (1)
N- G	0 / 328	0.05 (1)
N- I	-296 / 0	0.14 (1)
M- I	-227 / 9	0.16 (1)
B- S	0 / 1572	0.35 (1)
M- J	0 / 1572	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.50/1.00 (E-F:1) , BC=0.32/1.00 (M-N:1) , WB=0.44/1.00 (F-P:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (J) (INPUT = 0.90)
JSI METAL= 0.50 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126471

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T11	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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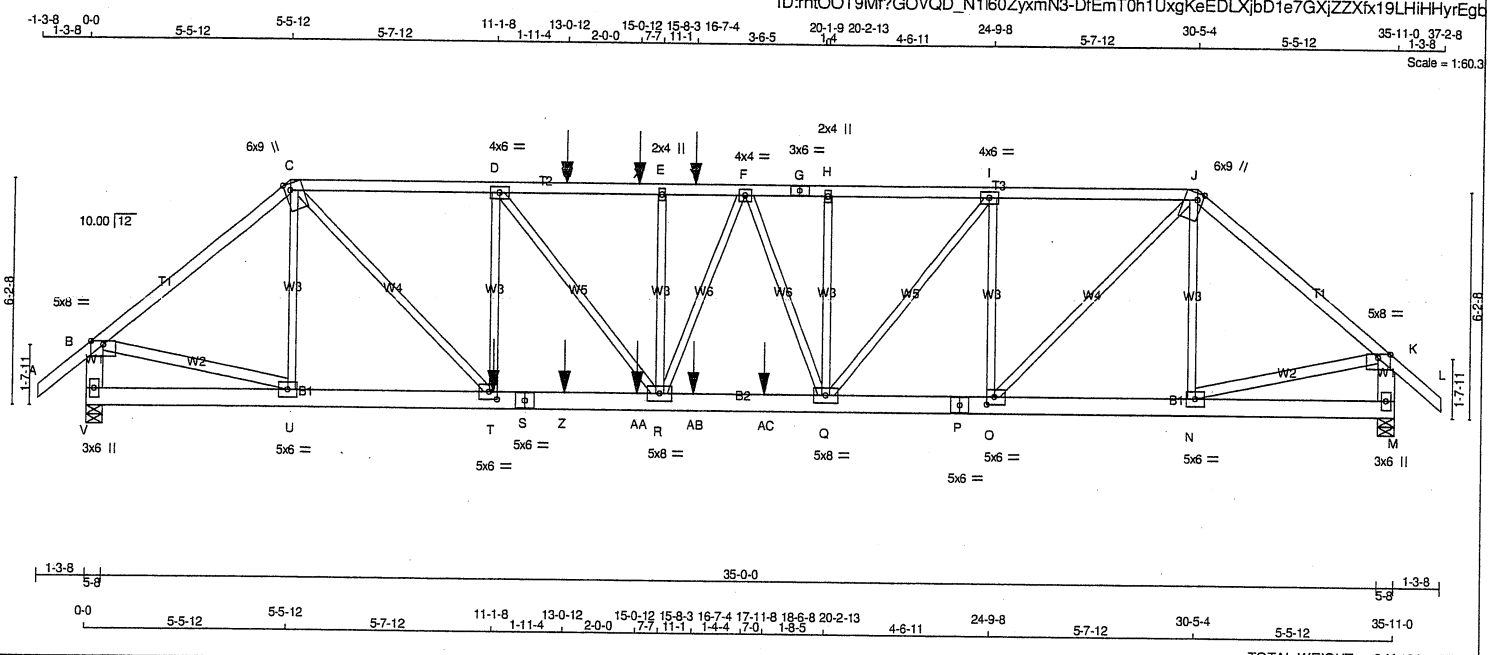

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - U	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- G	1	12	SIDE(0.0)
G- J	1	12	TOP
J- L	1	12	TOP
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(183.1)
S- P	2	12	SIDE(0.1)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
T- D	1	6	SIDE(179.4)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126472

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3790	0	3790	0	0	5-8	5-8
M	3207	0	3207	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2681	1755 / 0	0 / 0	0 / 0	0 / 0	927 / 0	0 / 0
M	2270	1477 / 0	0 / 0	0 / 0	0 / 0	793 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.06 (1)	10.00
B- C	-4209 / 0	-84.9	-84.9	0.42 (1)	4.32
C- D	-5936 / 0	-84.9	-84.9	0.45 (1)	3.68
D- W	-6589 / 0	-84.9	-84.9	0.44 (1)	3.47
W- X	-6589 / 0	-84.9	-84.9	0.44 (1)	3.47
X- E	-6589 / 0	-84.9	-84.9	0.44 (1)	3.47
E- Y	-6589 / 0	-84.9	-84.9	0.28 (1)	3.60
Y- F	-6589 / 0	-84.9	-84.9	0.28 (1)	3.60
F- G	-6192 / 0	-84.9	-84.9	0.17 (1)	3.84
G- H	-6192 / 0	-84.9	-84.9	0.17 (1)	3.84
H- I	-6192 / 0	-84.9	-84.9	0.38 (1)	3.64
I- J	-4824 / 0	-84.9	-84.9	0.34 (1)	4.13
J- K	-3484 / 0	-84.9	-84.9	0.37 (1)	4.70
K- L	0 / 38	-84.9	-84.9	0.06 (1)	10.00
V- B	-3758 / 0	0.0	0.0	0.14 (1)	7.28
M- K	-3172 / 0	0.0	0.0	0.12 (1)	7.76
V- U	0 / 0	-18.5	-18.5	0.05 (4)	10.00
U- T	0 / 3217	-18.5	-18.5	0.26 (1)	10.00
T- S	0 / 5936	-18.5	-18.5	0.49 (1)	10.00
S- Z	0 / 5936	-18.5	-18.5	0.49 (1)	10.00
Z-AA	0 / 5936	-18.5	-18.5	0.49 (1)	10.00
AA- R	0 / 5936	-18.5	-18.5	0.49 (1)	10.00
R-AB	0 / 6427	-18.5	-18.5	0.76 (1)	10.00
AB-AC	0 / 6427	-18.5	-18.5	0.76 (1)	10.00
AC- Q	0 / 6427	-18.5	-18.5	0.76 (1)	10.00
Q- P	0 / 4824	-18.5	-18.5	0.44 (1)	10.00
P- O	0 / 4824	-18.5	-18.5	0.44 (1)	10.00
O- N	0 / 2664	-18.5	-18.5	0.21 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-540 / 0	0.16 (1)
N- J	-433 / 0	0.13 (1)
B- U	0 / 3303	0.29 (1)
N- K	0 / 2735	0.24 (1)
T- D	-1429 / 0	0.42 (1)
O- I	-2256 / 0	0.66 (1)
C- T	0 / 3902	0.48 (1)
O- J	0 / 3100	0.38 (1)
Q- I	0 / 2222	0.27 (1)
D- R	0 / 1060	0.13 (1)
Q- H	-247 / 0	0.07 (1)
R- E	-599 / 0	0.17 (1)
R- F	0 / 443	0.05 (1)
F- Q	-647 / 0	0.22 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	11-1-8	-978	-978	---	BACK	VERT	TOTAL	---	C1
W	13-0-12	-102	-102	---	BACK	VERT	TOTAL	---	C1
X	15-0-12	-102	-102	---	BACK	VERT	TOTAL	---	C1
Y	16-7-4	-102	-102	---	BACK	VERT	TOTAL	---	C1
Z	13-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AA	15-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
AB	16-7-4	-26	-26	---	BACK	VERT	TOTAL	---	C1
AC	18-6-8	-793	-793	---	BACK	VERT	TOTAL	---	C1

TOTAL WEIGHT = 2 X 190 = 379 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.45/1.00 (C-D:1) , BC=0.76/1.00 (Q-R:1) , WB=0.66/1.00 (I-O:1) , SSI=0.25/1.00 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (T) (INPUT = 0.90)
JSI METAL= 0.55 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420375	T11	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	6.0	9.0	Edge	1.75
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMWW-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
22/05/11
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126472

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T12	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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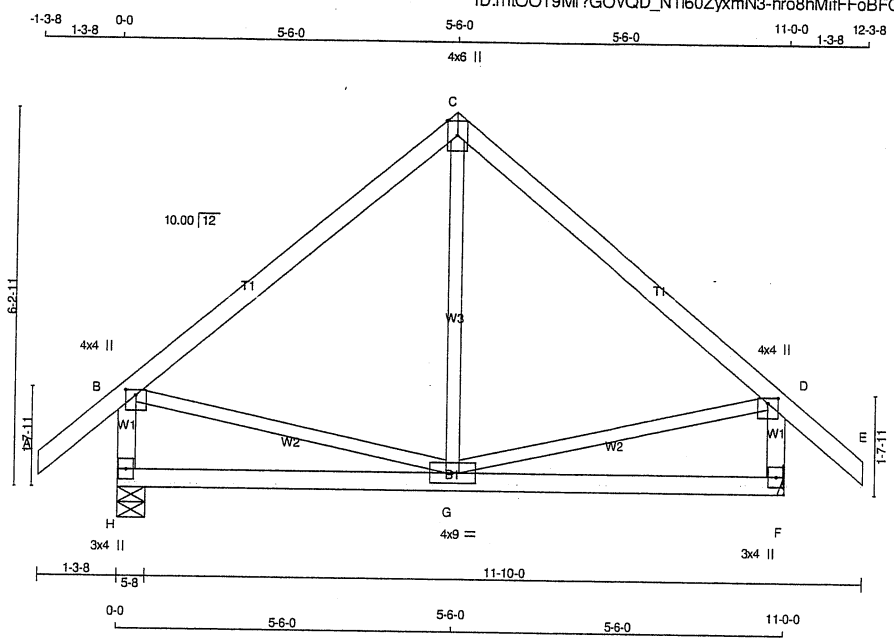

TOTAL WEIGHT = 3 X 49 = 147 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	686	0	686	0	0	5-8	5-8
F	686	0	686	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	485	321 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
F	485	321 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-360 / 0	-84.9	-84.9	0.33 (1)	6.25
C- D	-360 / 0	-84.9	-84.9	0.33 (1)	6.25
D- E	0 / 38	-84.9	-84.9	0.12 (1)	10.00
H- B	-647 / 0	0.0	0.0	0.07 (1)	7.81
F- D	-647 / 0	0.0	0.0	0.07 (1)	7.81
H- G	0 / 0	-18.5	-18.5	0.16 (4)	10.00
G- F	0 / 0	-18.5	-18.5	0.16 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-6 / 90	0.03 (4)
B- G	0 / 284	0.06 (1)
G- D	0 / 284	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.33/1.00 (C-D:1) , BC=0.16/1.00 (G-H:4) , WB=0.06/1.00 (D-G:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (D) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126473

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T12S	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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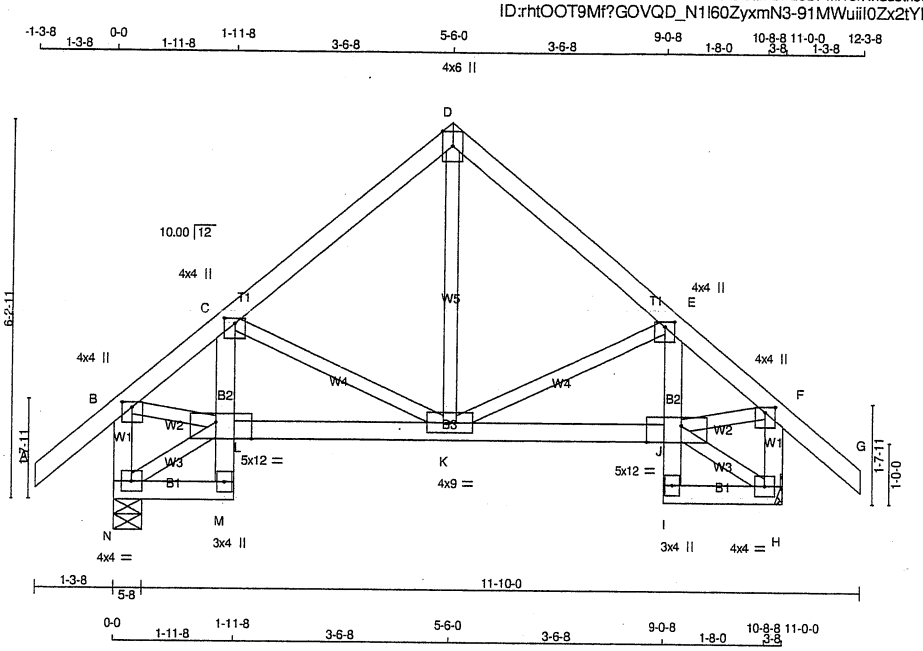

TOTAL WEIGHT = 4 X 57 = 227 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, C, E, F						
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	5.0	12.0	Edge	7.00
K	BMWWW-t	MT20	4.0	9.0		
L	BVMWW-l	MT20	5.0	12.0	Edge	7.00
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	686	0	686	0	0	5-8	5-8
H	686	0	686	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	485	321 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
H	485	321 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-637 / 0	-84.9	-84.9	0.09 (1)	6.25
C- D	-417 / 0	-84.9	-84.9	0.14 (1)	6.25
D- E	-417 / 0	-84.9	-84.9	0.14 (1)	6.25
E- F	-637 / 0	-84.9	-84.9	0.09 (1)	6.25
F- G	0 / 38	-84.9	-84.9	0.12 (1)	10.00
N- B	-660 / 0	0.0	0.0	0.07 (1)	7.81
H- F	-660 / 0	0.0	0.0	0.07 (1)	7.81
N- M	0 / 16	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 19	0.0	0.0	0.03 (1)	10.00
L- C	-12 / 28	0.0	0.0	0.02 (4)	7.81
L- K	0 / 500	-18.5	-18.5	0.12 (1)	10.00
K- J	0 / 500	-18.5	-18.5	0.12 (1)	10.00
I- J	0 / 19	0.0	0.0	0.03 (1)	10.00
J- E	-12 / 28	0.0	0.0	0.02 (4)	7.81
I- H	0 / 16	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 247	0.06 (1)
K- E	-213 / 0	0.06 (1)
C- K	-213 / 0	0.06 (1)
N- L	-18 / 0	0.00 (1)
B- L	0 / 493	0.11 (1)
J- H	-18 / 0	0.00 (1)
J- F	0 / 493	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.14/1.00 (C-D:1) , BC=0.12/1.00 (K-L:1) , WB=0.11/1.00 (B-L:1) , SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (F) (INPUT = 0.90)
JSI METAL= 0.19 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126474

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T13	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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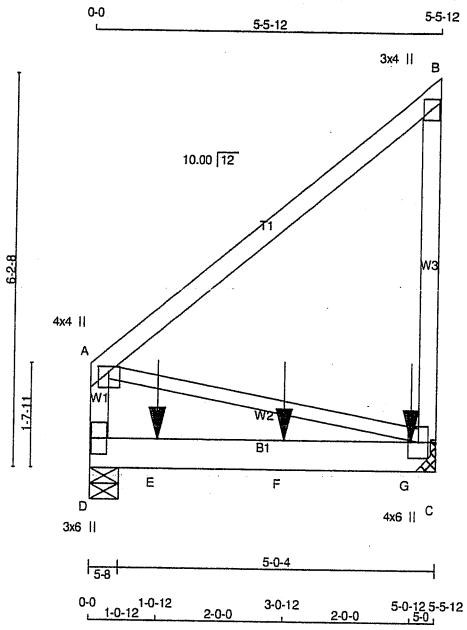

TOTAL WEIGHT = 2 X 30 = 59 lb
[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	SIDE(53.5)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(122.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126475

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1167	0	1167	0	0	5-8	5-8
C	1408	0	1408	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	823	552 / 0	0 / 0	0 / 0	0 / 0	271 / 0	0 / 0
C	993	665 / 0	0 / 0	0 / 0	0 / 0	328 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
D- A	-232 / 0	0.0	0.0	0.01 (1)	7.81	
A- B	0 / 0	-84.9	-84.9	0.24 (1)	10.00	
C- B	-232 / 0	0.0	0.0	0.08 (1)	7.81	
D- E	0 / 0	-18.5	-18.5	0.53 (1)	10.00	
E- F	0 / 0	-18.5	-18.5	0.53 (1)	10.00	
F- G	0 / 0	-18.5	-18.5	0.53 (1)	10.00	
G- C	0 / 0	-18.5	-18.5	0.53 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- C	0 / 0	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-0-12	-470	-470	---	FRONT	VERT	TOTAL	---	C1
F	3-0-12	-470	-470	---	FRONT	VERT	TOTAL	---	C1
G	5-0-12	-474	-474	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 597 (0.11")

CSI: TC=0.24/1.00 (A-B:1) , BC=0.53/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.39/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (A) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T13Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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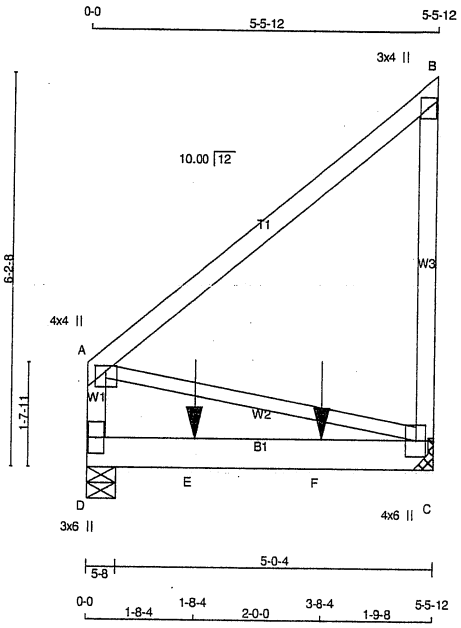

TOTAL WEIGHT = 2 X 30 = 59 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2126476

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1173	0	1173	0	0	5-8	5-8
C	1140	0	1140	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	831	537 / 0	0 / 0	0 / 0	0 / 0	294 / 0	0 / 0
C	807	522 / 0	0 / 0	0 / 0	0 / 0	286 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
D- A	-232 / 0	0.0	0.0	0.01 (1)	7.81	
A- B	0 / 0	-84.9	-84.9	0.24 (1)	10.00	
C- B	-232 / 0	0.0	0.0	0.08 (1)	7.81	
D- E	0 / 0	-18.5	-18.5	0.59 (1)	10.00	
E- F	0 / 0	-18.5	-18.5	0.59 (1)	10.00	
F- C	0 / 0	-18.5	-18.5	0.59 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- C	0 / 0	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-8-4	-618	-618	---	BACK	VERT	TOTAL	---	C1
F	3-8-4	-618	-618	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 961 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 493 (0.13")

CSI: TC=0.24/1.00 (A-B:1) , BC=0.59/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (A) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T14	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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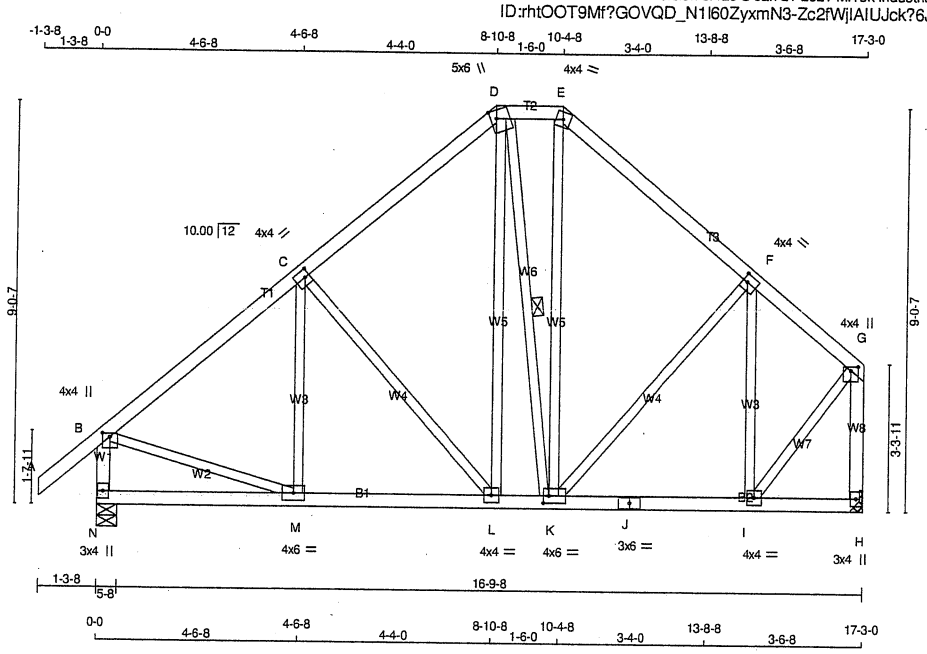

TOTAL WEIGHT = 97 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0	2.00	1.50
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	891	0	891	0	0	MECHANICAL	
N	1009	0	1009	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	633	402 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0
N	714	466 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-796 / 0	-84.9	-84.9	0.22 (1)	6.25
C- D	-601 / 0	-84.9	-84.9	0.22 (1)	6.25
D- E	-428 / 0	-84.9	-84.9	0.03 (1)	6.25
E- F	-578 / 0	-84.9	-84.9	0.18 (1)	6.25
F- G	-499 / 0	-84.9	-84.9	0.15 (1)	6.25
N- B	-975 / 0	0.0	0.0	0.10 (1)	7.81
H- G	-875 / 0	0.0	0.0	0.16 (1)	7.81
N- M	0 / 0	-18.5	-18.5	0.09 (4)	10.00
M- L	0 / 633	-18.5	-18.5	0.15 (1)	10.00
L- K	0 / 441	-18.5	-18.5	0.12 (1)	10.00
K- J	0 / 412	-18.5	-18.5	0.11 (1)	10.00
J- I	0 / 412	-18.5	-18.5	0.11 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.05 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-83 / 44	0.04 (1)
C- L	-297 / 0	0.25 (1)
L- D	0 / 246	0.06 (1)
D- K	-81 / 0	0.04 (1)
K- E	0 / 143	0.03 (1)
K- F	0 / 22	0.01 (1)
I- F	-413 / 0	0.18 (1)
B- M	0 / 659	0.15 (1)
I- G	0 / 633	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.15/1.00 (L-M:1) , WB=0.25/1.00 (C-L:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126477

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T15	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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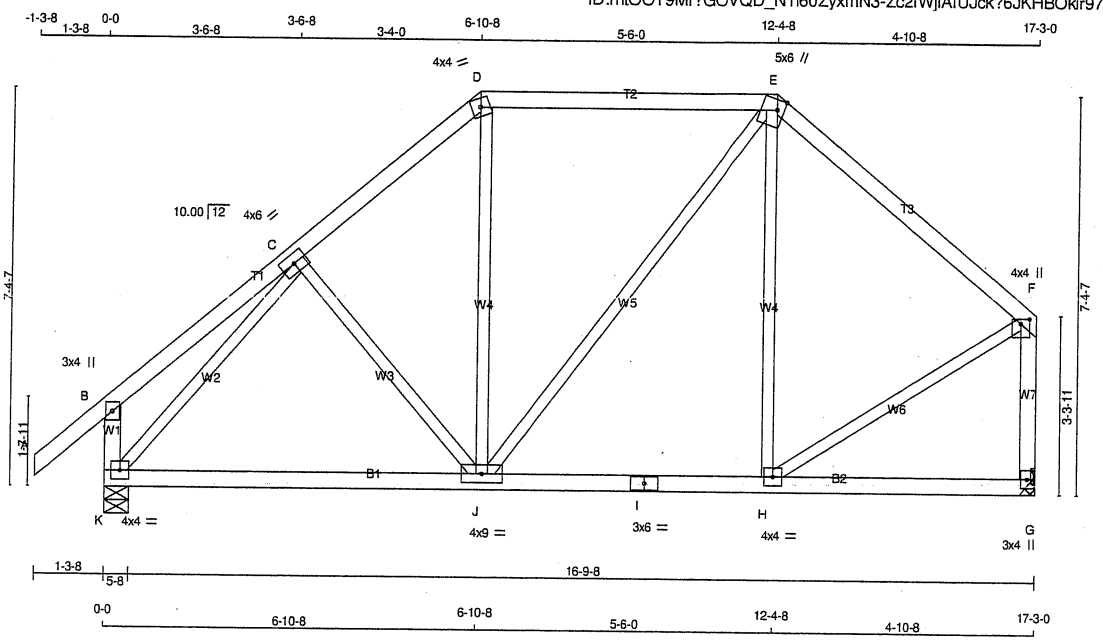

TOTAL WEIGHT = 81 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1009	0	1009	0	0	5-8	5-8
G	891	0	891	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	714	466 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
G	633	402 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 21	-84.9	-84.9	0.16 (1)	10.00
C- D	-715 / 0	-84.9	-84.9	0.13 (1)	6.25
D- E	-533 / 0	-84.9	-84.9	0.33 (1)	6.25
E- F	-588 / 0	-84.9	-84.9	0.26 (1)	6.25
K- B	-229 / 0	0.0	0.0	0.02 (1)	7.81
G- F	-853 / 0	0.0	0.0	0.16 (1)	7.81
K- J	0 / 598	-18.5	-18.5	0.24 (4)	10.00
J- I	0 / 448	-18.5	-18.5	0.23 (4)	10.00
I- H	0 / 448	-18.5	-18.5	0.23 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-102 / 4	0.05 (1)
J- D	0 / 100	0.03 (4)
J- E	0 / 139	0.03 (1)
H- E	-173 / 6	0.17 (1)
K- C	-941 / 0	0.44 (1)
H- F	0 / 525	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.33/1.00 (D-E:1) , BC=0.24/1.00 (J-K:4) , WB=0.44/1.00 (C-K:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (K) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126478

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T16S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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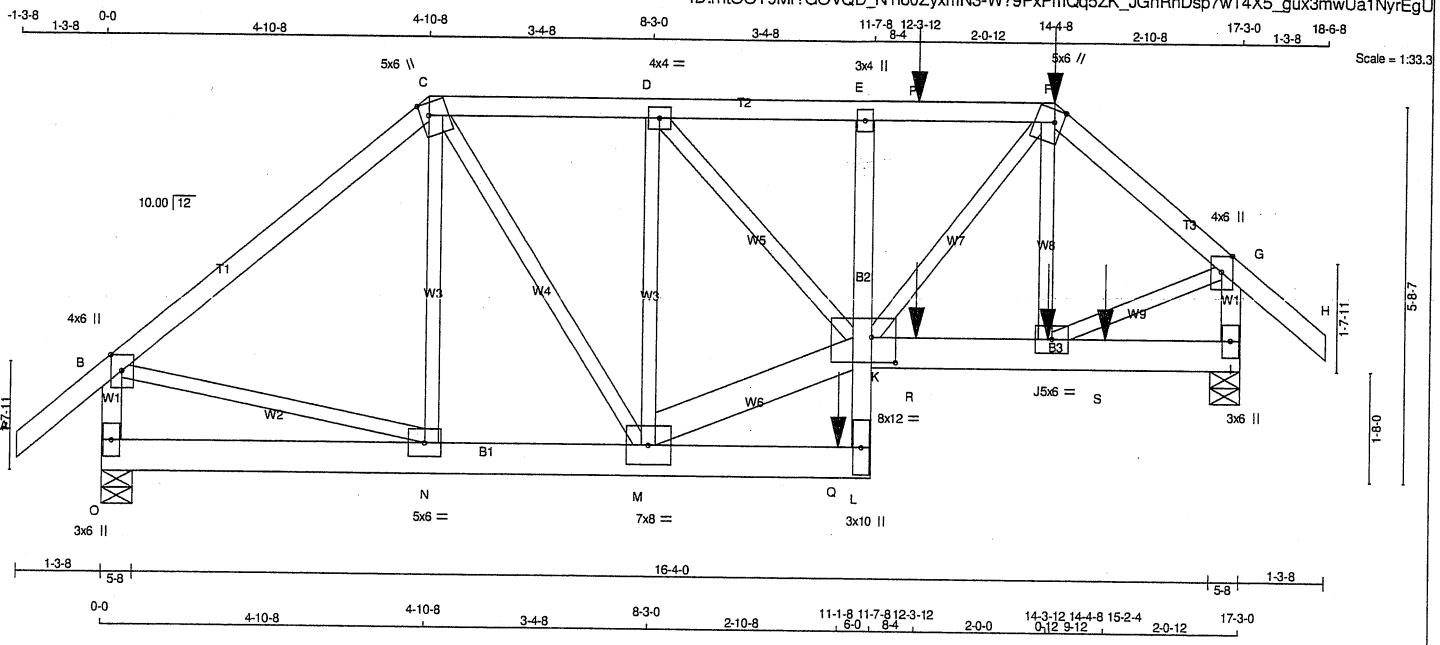

TOTAL WEIGHT = 100 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	4.0	6.0	Edge	
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BVMWWW-l	MT20	8.0	12.0	4.50	4.50
L	BMV+p	MT20	3.0	10.0		
M	BMWWW-t	MT20	7.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1189	0	1189	0	0	5-8	5-8
I	1444	0	1444	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	841	553 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0
I	1020	674 / 0	0 / 0	0 / 0	0 / 0	346 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.13 (1)	10.00
B- C	-991 / 0	-84.9	-84.9	0.43 (1)	5.65
C- D	-1020 / 0	-84.9	-84.9	0.15 (1)	6.00
D- E	-1471 / 0	-84.9	-84.9	0.17 (1)	5.19
E- P	-1478 / 0	-84.9	-84.9	0.14 (1)	5.22
P- F	-1478 / 0	-84.9	-84.9	0.14 (1)	5.22
F- G	-1232 / 0	-84.9	-84.9	0.16 (1)	5.58
G- H	0 / 38	-84.9	-84.9	0.13 (1)	10.00
O- B	-1147 / 0	0.0	0.0	0.13 (1)	7.39
I- G	-1419 / 0	0.0	0.0	0.16 (1)	6.81
O- N	0 / 0	-18.5	-18.5	0.05 (4)	10.00
N- M	0 / 758	-18.5	-18.5	0.13 (1)	10.00
M- Q	0 / 16	-18.5	-18.5	0.09 (1)	10.00
Q- L	0 / 16	-18.5	-18.5	0.09 (1)	10.00
L- K	0 / 357	0.0	0.0	0.10 (1)	10.00
K- E	-286 / 0	0.0	0.0	0.05 (1)	7.81
K- R	0 / 934	-18.5	-18.5	0.13 (1)	10.00
R- J	0 / 934	-18.5	-18.5	0.13 (1)	10.00
J- S	0 / 0	-18.5	-18.5	0.02 (4)	10.00
S- I	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-114 / 28	0.05 (1)
C- M	0 / 496	0.12 (1)
M- D	-810 / 0	0.39 (1)
M- K	0 / 1136	0.15 (1)
D- K	0 / 667	0.17 (1)
K- F	0 / 873	0.22 (1)
J- F	-306 / 0	0.08 (1)
B- N	0 / 782	0.19 (1)
J- G	0 / 1019	0.25 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	14-4-8	-117	-117	---	FRONT	VERT	TOTAL	---	C1
J	14-3-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
P	12-3-12	-26	-26	---	FRONT	VERT	TOTAL	---	C1
Q	11-1-8	-269	-269	---	FRONT	VERT	TOTAL	---	C1
R	12-3-12	-8	-8	---	FRONT	VERT	TOTAL	---	C1
S	15-2-4	-7	-7	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.43/1.00 (B-C:1) , BC=0.13/1.00 (M-N:1) , WB=0.39/1.00 (D-M:1) , SSI=0.24/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.58 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126479

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	T17	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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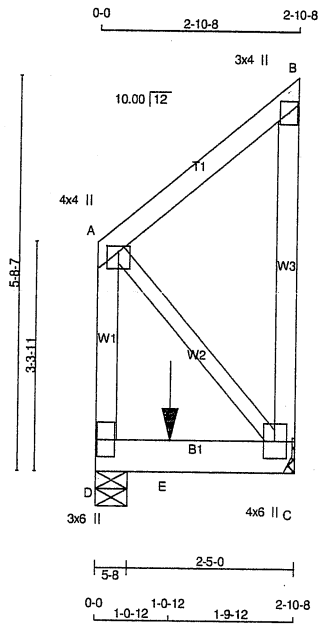


TOTAL WEIGHT = 2 X 21 = 42 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

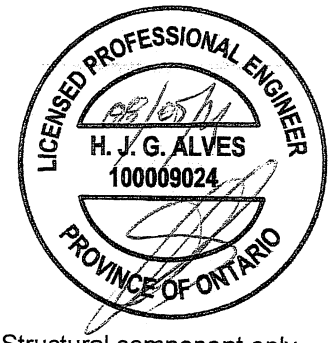
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		



Structural component only
DWG# T-2126480

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	569	0	569	0	0	5-8	5-8
C	395	0	395	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	402	269 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
C	279	186 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
D- A	-122 / 0	0.0	0.0	0.01 (1)	7.81
A- B	0 / 0	-84.9	-84.9	0.07 (1)	10.00
C- B	-122 / 0	0.0	0.0	0.03 (1)	7.81
D- E	0 / 0	-18.5	-18.5	0.17 (1)	10.00
E- C	0 / 0	-18.5	-18.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- C	0 / 0	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-0-12	-470	-470	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.07/1.00 (A-B:1) , BC=0.17/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T18S	1	3	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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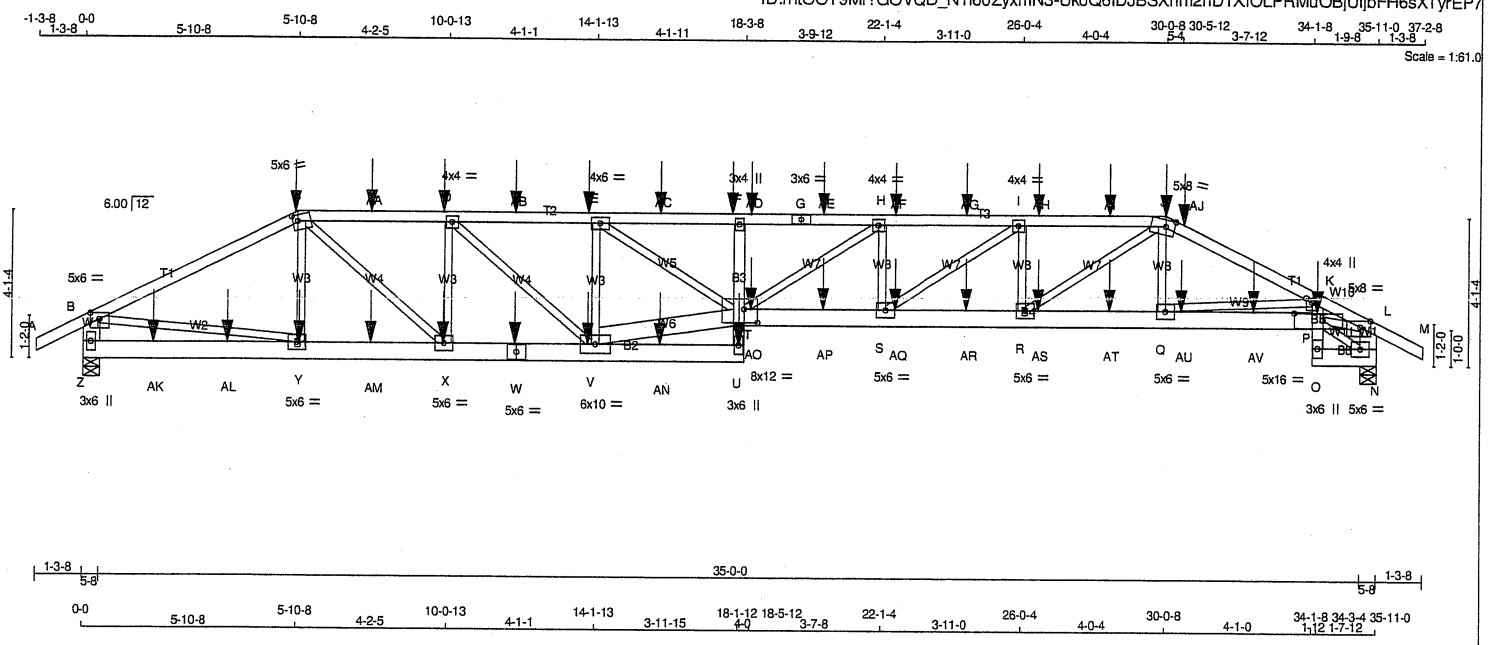

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Z - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Z - W	2x6	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
U - F	2x4	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(91.5)
C- G	1	12	SIDE(91.5)
G- J	1	12	SIDE(91.5)
J- M	1	12	SIDE(91.5)
Z- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Z- W	2	12	SIDE(274.6)
W- U	2	12	SIDE(274.6)
T- P	2	12	SIDE(91.5)
O- N	2	12	TOP
U- F	1	12	TOP
K- O	1	12	SIDE(50.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126483

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	3781	0	3781	0	0	5-8	5-8
N	4180	0	4180	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	2682	1712 / 0	0 / 0	0 / 0	0 / 0	970 / 0	0 / 0
N	2957	1938 / 0	0 / 0	0 / 0	0 / 0	1019 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.04 (1)	10.00
B- C	-5892 / 0	-84.9	-84.9	0.27 (1)	4.60
C-AA	-7627 / 0	-84.9	-84.9	0.21 (1)	4.17
AA- D	-7627 / 0	-84.9	-84.9	0.21 (1)	4.17
D-AB	-8984 / 0	-84.9	-84.9	0.24 (1)	3.88
AB- E	-8984 / 0	-84.9	-84.9	0.24 (1)	3.88
E-AC	-12783 / 0	-84.9	-84.9	0.35 (1)	3.22
AC- F	-12783 / 0	-84.9	-84.9	0.35 (1)	3.22
F-AD	-12908 / 0	-84.9	-84.9	0.34 (1)	3.21
AD- G	-12908 / 0	-84.9	-84.9	0.34 (1)	3.21
G-AE	-12908 / 0	-84.9	-84.9	0.34 (1)	3.21
AE- H	-12908 / 0	-84.9	-84.9	0.34 (1)	3.21
H-AF	-12099 / 0	-84.9	-84.9	0.30 (1)	3.34
AF-AG	-12099 / 0	-84.9	-84.9	0.30 (1)	3.34
AG- I	-12099 / 0	-84.9	-84.9	0.30 (1)	3.34
I-AH	-10265 / 0	-84.9	-84.9	0.27 (1)	3.63
AH-AI	-10265 / 0	-84.9	-84.9	0.27 (1)	3.63
AI- J	-10265 / 0	-84.9	-84.9	0.27 (1)	3.63
J-AJ	-8036 / 0	-84.9	-84.9	0.30 (1)	3.99
AJ- K	-8036 / 0	-84.9	-84.9	0.30 (1)	3.99
K- L	-9837 / 0	-84.9	-84.9	0.25 (1)	3.66
L- M	0 / 26	-84.9	-84.9	0.04 (1)	10.00
Z- B	-3688 / 0	0.0	0.0	0.08 (1)	7.81
N- L	-3644 / 0	0.0	0.0	0.08 (1)	7.81
Z-AK	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AK-AL	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AL- Y	0 / 0	-18.5	-18.5	0.05 (4)	10.00
Y-AM	0 / 5253	-18.5	-18.5	0.23 (1)	10.00
AM- X	0 / 5253	-18.5	-18.5	0.23 (1)	10.00
X- W	0 / 7628	-18.5	-18.5	0.33 (1)	10.00
W- V	0 / 7628	-18.5	-18.5	0.33 (1)	10.00
V-AN	0 / 343	-18.5	-18.5	0.03 (1)	10.00
AN- U	0 / 343	-18.5	-18.5	0.03 (1)	10.00
U- T	0 / 105	0.0	0.0	0.16 (1)	10.00
T- F	-657 / 0	0.0	0.0	0.16 (1)	7.81
T-AO	0 / 12099	-18.5	-18.5	0.53 (1)	10.00
AO-AP	0 / 12099	-18.5	-18.5	0.53 (1)	10.00
AP- S	0 / 12099	-18.5	-18.5	0.53 (1)	10.00
S-AQ	0 / 10265	-18.5	-18.5	0.47 (1)	10.00
AQ-AR	0 / 10265	-18.5	-18.5	0.47 (1)	10.00
AR- R	0 / 10265	-18.5	-18.5	0.47 (1)	10.00
R-AS	0 / 7259	-18.5	-18.5	0.32 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-481 / 0	0.04 (1)
V- E	-3247 / 0	0.26 (1)
V- T	0 / 8969	0.35 (1)
E- T	0 / 4518	0.34 (1)
Q- J	0 / 771	0.06 (1)
Q- K	-2909 / 0	0.31 (1)
B- Y	0 / 5311	0.40 (1)
P- N	-1013 / 0	0.05 (1)
P- L	0 / 8778	0.66 (1)
R- J	0 / 3629	0.27 (1)
T- H	0 / 983	0.07 (1)
R- I	-1837 / 0	0.11 (1)
S- H	-1094 / 0	0.06 (1)
S- I	0 / 2235	0.17 (1)
D- V	0 / 1853	0.14 (1)
C- X	0 / 3175	0.24 (1)
X- D	-1995 / 0	0.16 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 732 (0.59")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.53/1.00 (S-T:1) , WB=0.66/1.00 (L-P:1) , SSI=0.37/1.00 (K-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T18S	1	3	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D, H, I						
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTWW-m	MT20	5.0	8.0	2.25	3.00
K	TMVW+p	MT20	4.0	4.0	1.50	2.00
L	TMVW-p	MT20	5.0	8.0	2.00	3.50
N	BMVW1-t	MT20	5.0	6.0		
O	BMV+p	MT20	3.0	6.0		
P	BVMWW-l	MT20	5.0	16.0	2.25	9.50
Q, R, S, X, Y						
Q	BMWW-t	MT20	5.0	6.0		
T	BVMWWW-l	MT20	8.0	12.0	4.50	4.50
U	BMV+p	MT20	3.0	6.0		
V	BMWWW-t	MT20	6.0	10.0		
W	BS-t	MT20	5.0	6.0		
Z	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AS-AT	0 / 7259	-18.5	-18.5	0.32 (1)	10.00
AT- Q	0 / 7259	-18.5	-18.5	0.32 (1)	10.00
Q-AU	0 / 10112	-18.5	-18.5	0.42 (1)	10.00
AU-AV	0 / 10112	-18.5	-18.5	0.42 (1)	10.00
AV- P	0 / 10112	-18.5	-18.5	0.42 (1)	10.00
O- P	0 / 17	0.0	0.0	0.39 (1)	10.00
P- K	0 / 1164	0.0	0.0	0.45 (1)	10.00
O- N	0 / 869	-18.5	-18.5	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-374	-374	---	FRONT	VERT	TOTAL	---	C1
D	9-11-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
E	13-11-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
F	17-11-8	-113	-113	---	FRONT	VERT	TOTAL	---	C1
J	30-0-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
J	30-0-8	-134	-134	---	FRONT	VERT	SNOW	---	C1
P	34-3-4	-345	-345	---	FRONT	VERT	TOTAL	---	C1
U	18-1-12	-35	-35	---	FRONT	VERT	TOTAL	---	C1
V	13-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
W	11-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
X	9-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
Y	5-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AA	7-11-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AB	11-11-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AC	15-11-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AD	18-5-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
AE	20-5-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
AF	22-5-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
AG	24-5-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
AH	26-5-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
AI	28-5-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
AJ	30-5-12	-86	-86	---	FRONT	VERT	TOTAL	---	C1
AK	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AL	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AM	7-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AN	15-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AO	18-5-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
AP	20-5-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AQ	22-5-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AR	24-5-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AS	26-5-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AT	28-5-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AU	30-5-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AV	32-5-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.57 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126483

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T19S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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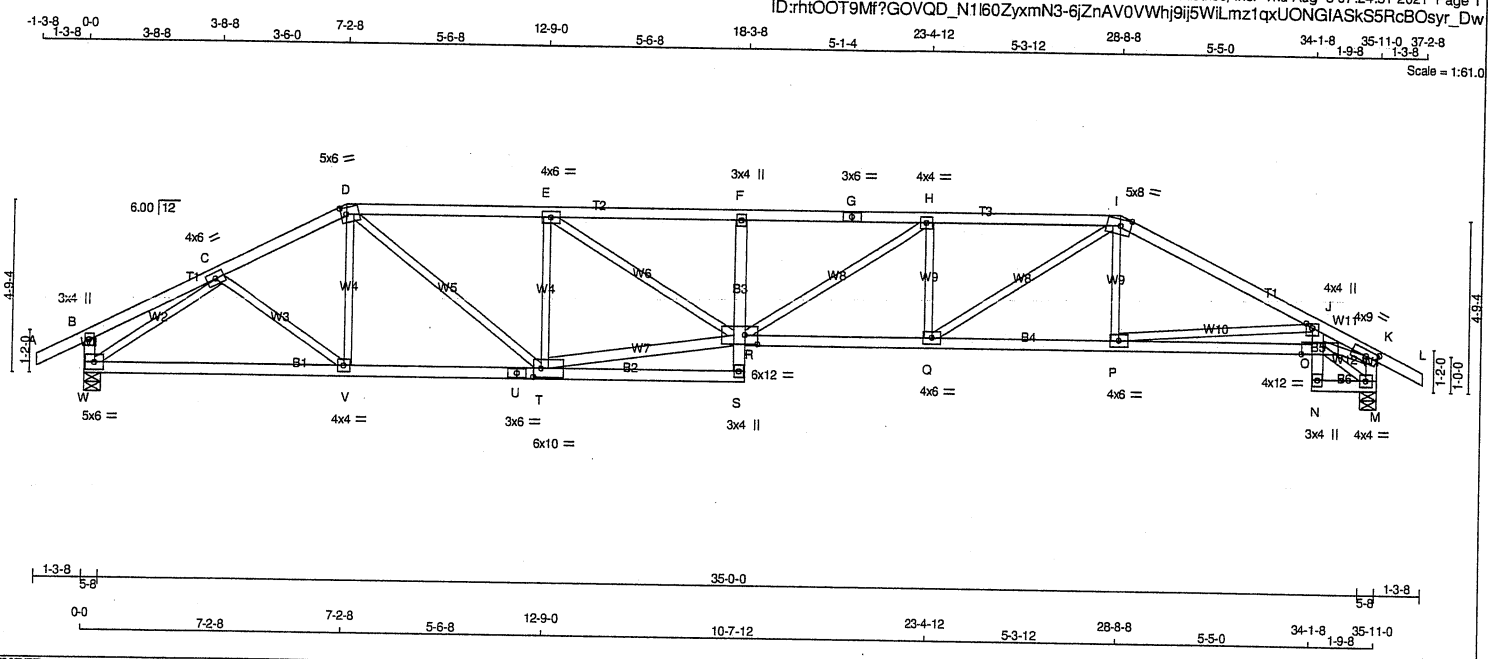

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x4	DRY	2100F 1.8E	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	2.25	3.50
J	TMVW+p	MT20	4.0	4.0	1.50	2.00
K	TMVW-t	MT20	4.0	9.0	2.00	4.00
M	BMVW1-t	MT20	4.0	4.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	4.0	12.0	3.25	7.00
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BVMWWW-l	MT20	6.0	12.0	3.00	4.25
S	BMV+p	MT20	3.0	4.0		
T	BMWWW-t	MT20	6.0	10.0	3.00	2.50
U	BS-t	MT20	3.0	6.0		
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1-t	MT20	5.0	6.0		

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126484

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	1313	0	1313	0	0	5-8	5-8
M	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
M	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	0 / 8	-56.6	-56.6	0.09 (1)	10.00
C- D	-1804 / 0	-56.6	-56.6	0.13 (1)	4.87
D- E	-2302 / 0	-56.6	-56.6	0.37 (1)	4.19
E- F	-3182 / 0	-56.6	-56.6	0.46 (1)	3.58
F- G	-3211 / 0	-56.6	-56.6	0.42 (1)	3.61
G- H	-3211 / 0	-56.6	-56.6	0.42 (1)	3.61
H- I	-2937 / 0	-56.6	-56.6	0.39 (1)	3.77
I- J	-2294 / 0	-56.6	-56.6	0.45 (1)	4.11
J- K	-3134 / 0	-56.6	-56.6	0.17 (1)	3.83
K- L	0 / 17	-56.6	-56.6	0.07 (1)	10.00
W- B	-161 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-1135 / 0	0.0	0.0	0.11 (1)	7.45
W- V	0 / 1511	-12.3	-12.3	0.32 (1)	10.00
V- U	0 / 1605	-12.3	-12.3	0.33 (1)	10.00
U- T	0 / 1605	-12.3	-12.3	0.33 (1)	10.00
T- S	0 / 101	-12.3	-12.3	0.09 (4)	10.00
S- R	0 / 32	0.0	0.0	0.14 (1)	10.00
R- F	-273 / 0	0.0	0.0	0.14 (1)	7.81
R- Q	0 / 2937	-12.3	-12.3	0.26 (1)	10.00
Q- P	0 / 2056	-12.3	-12.3	0.19 (1)	10.00
P- O	0 / 3275	-12.3	-12.3	0.28 (1)	10.00
N- O	0 / 11	0.0	0.0	0.38 (1)	10.00
O- J	0 / 343	0.0	0.0	0.44 (1)	10.00
N- M	0 / 254	-12.3	-12.3	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 115	0.03 (4)
V- D	0 / 56	0.02 (4)
D- T	0 / 901	0.20 (1)
T- E	-910 / 0	0.30 (1)
T- R	0 / 2254	0.36 (1)
E- R	0 / 1027	0.23 (1)
P- I	0 / 169	0.04 (4)
P- J	-1233 / 0	0.70 (1)
W- C	-1877 / 0	0.62 (1)
O- M	-306 / 0	0.05 (1)
O- K	0 / 2821	0.63 (1)
Q- I	0 / 1053	0.24 (1)
R- H	0 / 329	0.07 (1)
Q- H	-517 / 0	0.12 (1)

TOTAL WEIGHT = 147 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 966 (0.45")

CSI: TC=0.46/1.00 (E-F:1) , BC=0.44/1.00 (J-O:1) , WB=0.70/1.00 (J-P:1) , SSI=0.38/1.00 (J-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.69 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T20S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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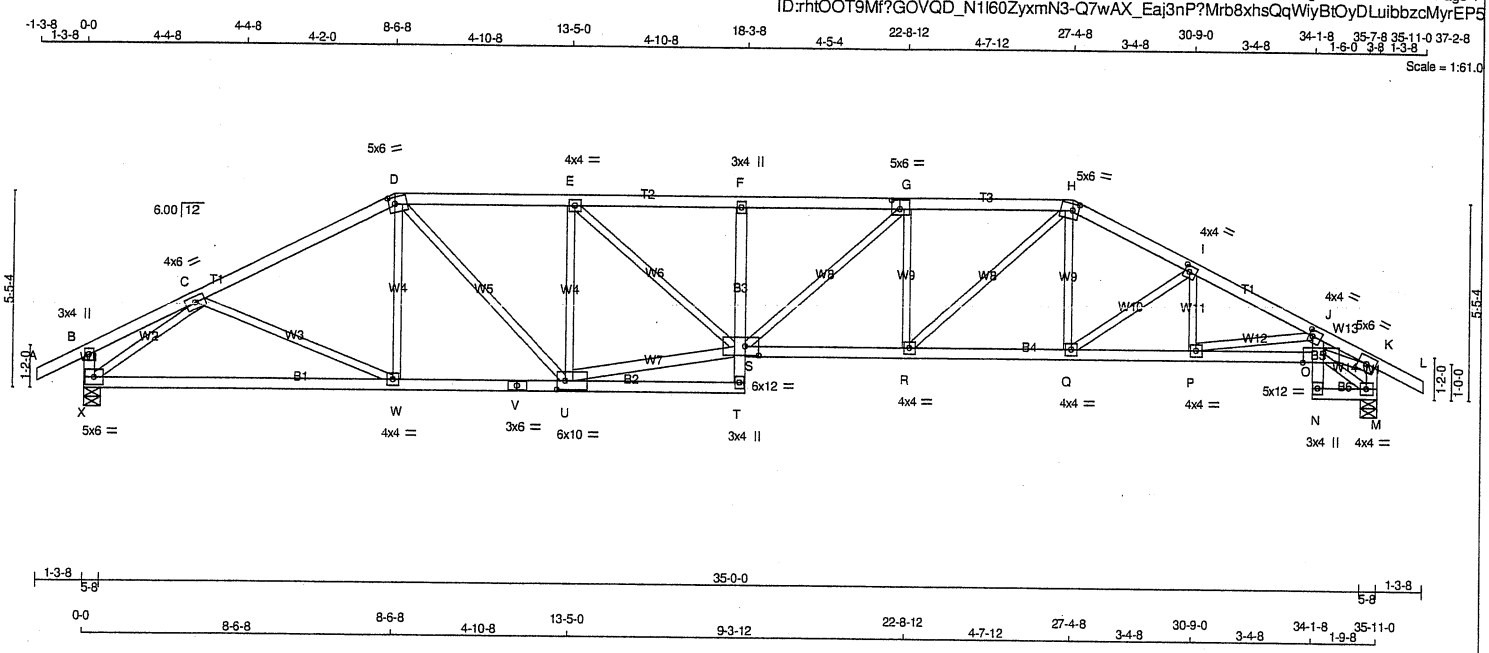

TOTAL WEIGHT = 153 lb
[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
X - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - F	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TSWW-l	MT20	5.0	6.0	3.00	2.75
H	TTWW-m	MT20	5.0	6.0	2.25	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	4.0	4.0	2.00	1.25
K	TMVW-t	MT20	5.0	6.0		
M	BMVW1-t	MT20	4.0	4.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	5.0	12.0	Edge	6.75
P, Q, R, W						
P	BMWW-t	MT20	4.0	4.0		
S	BVMWWW-l	MT20	6.0	12.0	3.00	4.50
T	BMV+p	MT20	3.0	4.0		
U	BMWWW-t	MT20	6.0	10.0	3.00	2.75
V	BS-t	MT20	3.0	6.0		
X	BMVW1-t	MT20	5.0	6.0		

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126485

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	1313	0	1313	0	0	5-8	5-8
M	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
M	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	0 / 19	-56.6	-56.6	0.18 (1)	10.00
C- D	-1779 / 0	-56.6	-56.6	0.29 (1)	4.75
D- E	-2038 / 0	-56.6	-56.6	0.25 (1)	4.52
E- F	-2681 / 0	-56.6	-56.6	0.30 (1)	3.99
F- G	-2697 / 0	-56.6	-56.6	0.26 (1)	4.03
G- H	-2486 / 0	-56.6	-56.6	0.32 (1)	4.09
H- I	-2180 / 0	-56.6	-56.6	0.20 (1)	4.44
I- J	-2498 / 0	-56.6	-56.6	0.30 (1)	4.08
J- K	-2930 / 0	-56.6	-56.6	0.29 (1)	3.79
K- L	0 / 17	-56.6	-56.6	0.07 (1)	10.00
X- B	-121 / 0	0.0	0.0	0.01 (1)	7.81
M- K	-1147 / 0	0.0	0.0	0.12 (1)	7.42
X- W	0 / 1476	-12.3	-12.3	0.35 (1)	10.00
W- V	0 / 1581	-12.3	-12.3	0.36 (1)	10.00
V- U	0 / 1581	-12.3	-12.3	0.36 (1)	10.00
U- T	0 / 68	-12.3	-12.3	0.07 (4)	10.00
T- S	0 / 30	0.0	0.0	0.10 (1)	10.00
S- F	-279 / 0	0.0	0.0	0.10 (1)	7.81
S- R	0 / 2486	-12.3	-12.3	0.43 (1)	10.00
R- Q	0 / 1943	-12.3	-12.3	0.34 (1)	10.00
Q- P	0 / 2262	-12.3	-12.3	0.39 (1)	10.00
P- O	0 / 3025	-12.3	-12.3	0.52 (1)	10.00
N- O	0 / 11	0.0	0.0	0.36 (1)	10.00
O- J	0 / 348	0.0	0.0	0.41 (1)	10.00
N- M	0 / 236	-12.3	-12.3	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- W	0 / 114	0.03 (4)
W- D	0 / 82	0.03 (4)
D- U	0 / 668	0.15 (1)
U- E	-854 / 0	0.38 (1)
U- S	0 / 2024	0.33 (1)
E- S	0 / 840	0.19 (1)
Q- H	0 / 287	0.06 (1)
Q- I	-399 / 0	0.12 (1)
X- C	-1869 / 0	0.49 (1)
R- H	0 / 728	0.16 (1)
S- G	0 / 284	0.06 (1)
R- G	-426 / 0	0.12 (1)
O- M	-284 / 0	0.04 (1)
O- K	0 / 2601	0.59 (1)
P- I	0 / 92	0.02 (4)
P- J	-775 / 0	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.32/1.00 (G-H:1) , BC=0.52/1.00 (O-P:1) , WB=0.59/1.00 (K-O:1) , SSI=0.35/1.00 (J-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.68 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T21S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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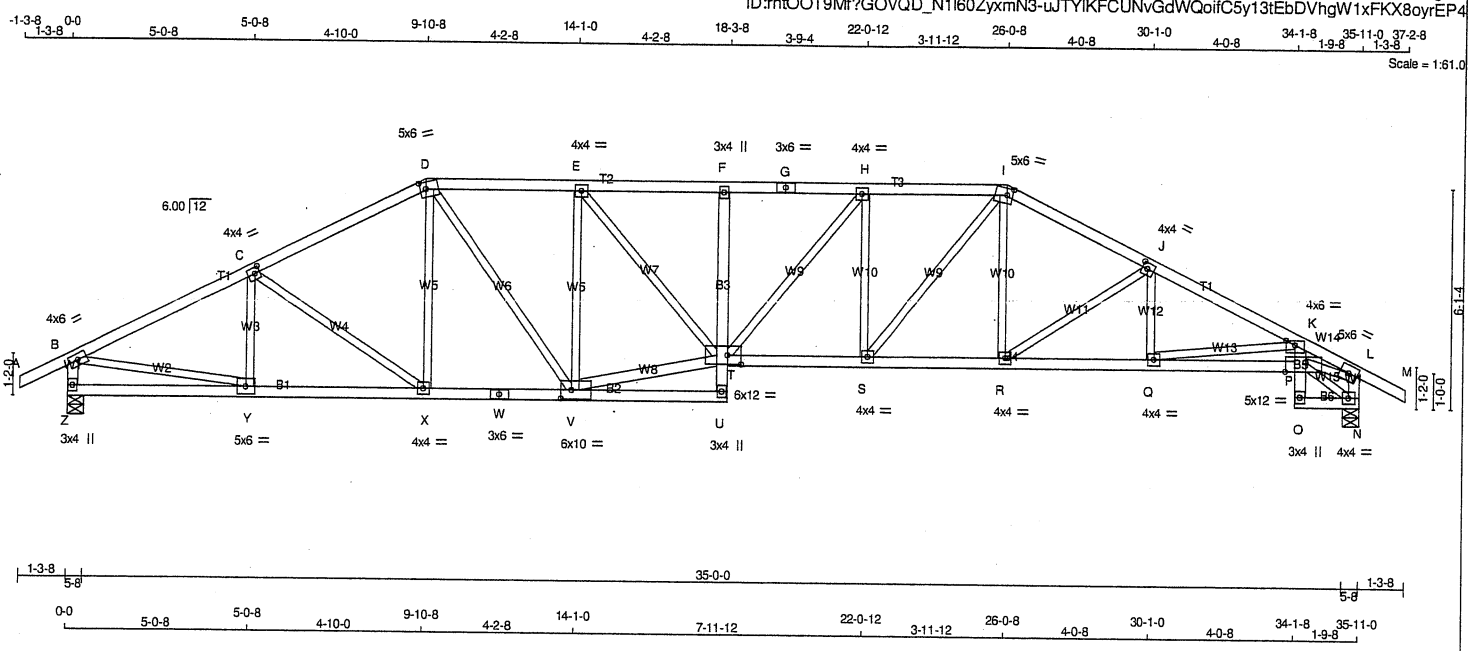

TOTAL WEIGHT = 161 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
Z - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Z - W	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
U - F	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
V - T	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	6.0	2.25	2.00
J	TMWW-t	MT20	4.0	4.0	2.00	1.75
K	TMVW-p	MT20	4.0	6.0	Edge	
L	TMVW-t	MT20	5.0	6.0		
N	BMVW1-t	MT20	4.0	4.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWW-l	MT20	5.0	12.0	Edge	6.75
Q, R, S, X						
Q	BMWW-t	MT20	4.0	4.0		
T	BVMWWW-l	MT20	6.0	12.0	3.00	4.50
U	BMV+p	MT20	3.0	4.0		
V	BMWWW-t	MT20	6.0	10.0	3.00	3.50
W	BS-t	MT20	3.0	6.0		
Y	BMWW-t	MT20	5.0	6.0		
Z	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126486

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	1313	0	1313	0	0	5-8	5-8
N	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
N	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-1814 / 0	-56.6	-56.6	0.22 (1)	4.79
C- D	-1721 / 0	-56.6	-56.6	0.21 (1)	4.90
D- E	-1835 / 0	-56.6	-56.6	0.15 (1)	4.83
E- F	-2303 / 0	-56.6	-56.6	0.17 (1)	4.39
F- G	-2313 / 0	-56.6	-56.6	0.15 (1)	4.40
G- H	-2313 / 0	-56.6	-56.6	0.15 (1)	4.40
H- I	-2201 / 0	-56.6	-56.6	0.15 (1)	4.49
I- J	-2067 / 0	-56.6	-56.6	0.20 (1)	4.56
J- K	-2446 / 0	-56.6	-56.6	0.28 (1)	4.16
K- L	-2952 / 0	-56.6	-56.6	0.23 (1)	3.88
L- M	0 / 17	-56.6	-56.6	0.07 (1)	10.00
Z- B	-1284 / 0	0.0	0.0	0.13 (1)	7.11
N- L	-1146 / 0	0.0	0.0	0.12 (1)	7.43
Z- Y	0 / 0	-12.3	-12.3	0.06 (4)	10.00
Y- X	0 / 1634	-12.3	-12.3	0.29 (1)	10.00
X- W	0 / 1528	-12.3	-12.3	0.27 (1)	10.00
W- V	0 / 1528	-12.3	-12.3	0.27 (1)	10.00
V- U	0 / 49	-12.3	-12.3	0.05 (4)	10.00
U- T	0 / 23	0.0	0.0	0.07 (1)	10.00
T- F	-204 / 0	0.0	0.0	0.07 (1)	7.81
T- S	0 / 2201	-12.3	-12.3	0.38 (1)	10.00
S- R	0 / 1841	-12.3	-12.3	0.32 (1)	10.00
R- Q	0 / 2214	-12.3	-12.3	0.38 (1)	10.00
Q- P	0 / 3052	-12.3	-12.3	0.53 (1)	10.00
O- P	0 / 11	0.0	0.0	0.36 (1)	10.00
P- K	0 / 338	0.0	0.0	0.41 (1)	10.00
O- N	0 / 238	-12.3	-12.3	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-216 / 0	0.05 (1)
C- X	-132 / 0	0.08 (1)
X- D	0 / 132	0.03 (1)
D- V	0 / 528	0.12 (1)
V- E	-810 / 0	0.47 (1)
V- T	0 / 1846	0.30 (1)
E- T	0 / 712	0.16 (1)
R- I	0 / 309	0.07 (1)
B- Y	0 / 1658	0.37 (1)
S- I	0 / 565	0.13 (1)
T- H	0 / 178	0.04 (1)
S- H	-387 / 0	0.15 (1)
P- N	-287 / 0	0.04 (1)
P- L	0 / 2625	0.59 (1)
R- J	-457 / 0	0.19 (1)
Q- J	0 / 110	0.03 (4)
Q- K	-848 / 0	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.28/1.00 (J-K:1) , BC=0.53/1.00 (P-Q:1) , WB=0.59/1.00 (L-P:1) , SSI=0.35/1.00 (K-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.68 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T22S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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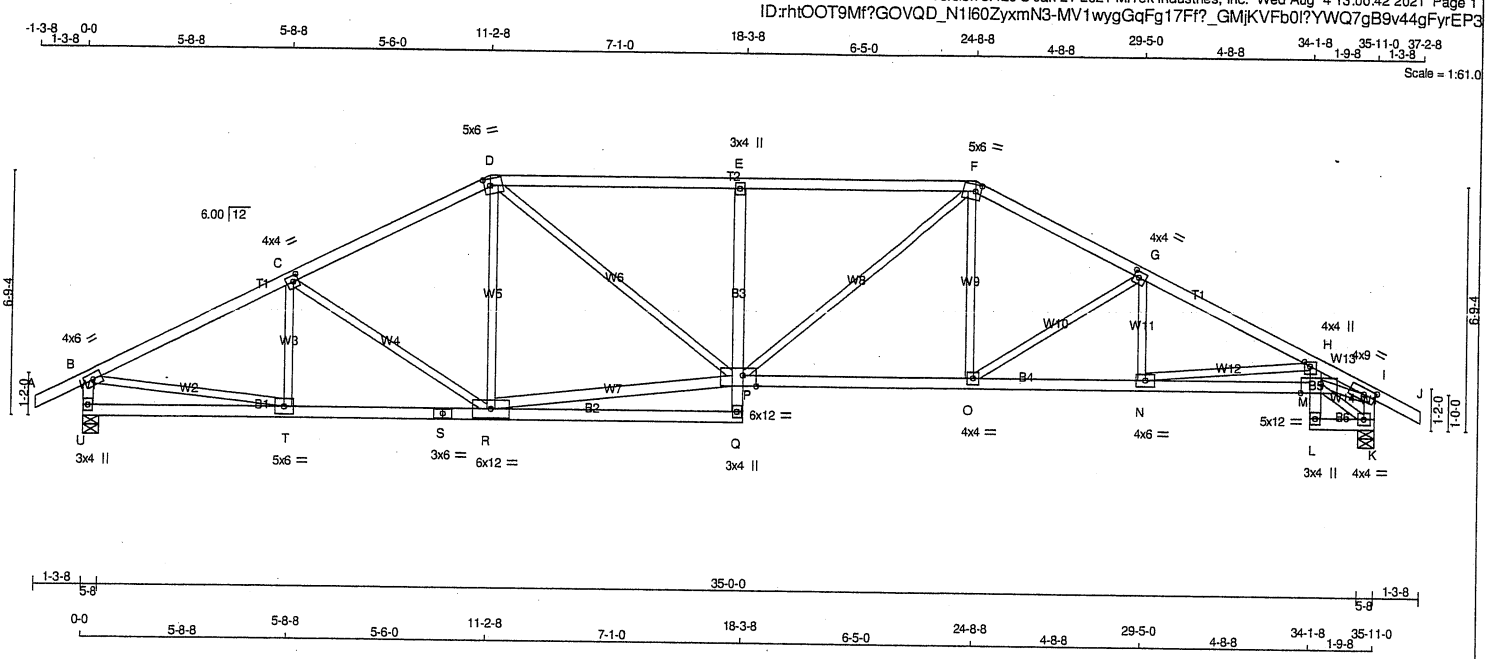

TOTAL WEIGHT = 154 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW+p	MT20	4.0	4.0	1.50	2.00
I	TMVW-t	MT20	4.0	9.0	2.00	4.00
K	BMVW1-t	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-l	MT20	5.0	12.0	Edge	6.75
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-l	MT20	6.0	12.0	Edge	4.50
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	6.0	12.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126487

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1313	0	1313	0	0	5-8	5-8
K	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
K	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-1837 / 0	-56.6	-56.6	0.28 (1)	4.70
C- D	-1673 / 0	-56.6	-56.6	0.27 (1)	4.89
D- E	-2070 / 0	-56.6	-56.6	0.43 (1)	4.32
E- F	-2079 / 0	-56.6	-56.6	0.41 (1)	4.32
F- G	-1963 / 0	-56.6	-56.6	0.24 (1)	4.61
G- H	-2394 / 0	-56.6	-56.6	0.31 (1)	4.19
H- I	-2978 / 0	-56.6	-56.6	0.21 (1)	3.88
I- J	0 / 17	-56.6	-56.6	0.07 (1)	10.00
U- B	-1280 / 0	0.0	0.0	0.13 (1)	7.13
K- I	-1144 / 0	0.0	0.0	0.12 (1)	7.43
U- T	0 / 0	-12.3	-12.3	0.09 (4)	10.00
T- S	0 / 1658	-12.3	-12.3	0.34 (1)	10.00
S- R	0 / 1658	-12.3	-12.3	0.34 (1)	10.00
R- Q	0 / 48	-12.3	-12.3	0.13 (4)	10.00
Q- P	0 / 39	0.0	0.0	0.07 (1)	10.00
P- E	-468 / 0	0.0	0.0	0.17 (1)	7.81
P- O	0 / 1747	-12.3	-12.3	0.34 (1)	10.00
O- N	0 / 2165	-12.3	-12.3	0.41 (1)	10.00
N- M	0 / 3083	-12.3	-12.3	0.54 (1)	10.00
L- M	0 / 11	0.0	0.0	0.36 (1)	10.00
M- H	0 / 334	0.0	0.0	0.41 (1)	10.00
L- K	0 / 240	-12.3	-12.3	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-185 / 0	0.05 (1)
C- R	-213 / 0	0.18 (1)
R- D	0 / 61	0.02 (4)
R- P	0 / 1447	0.23 (1)
D- P	0 / 751	0.17 (1)
P- F	0 / 433	0.10 (1)
O- F	0 / 359	0.08 (1)
O- G	-506 / 0	0.28 (1)
N- G	0 / 117	0.03 (4)
N- H	-926 / 0	0.37 (1)
B- T	0 / 1676	0.38 (1)
M- K	-289 / 0	0.04 (1)
M- I	0 / 2652	0.60 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.54/1.00 (M-N:1) , WB=0.60/1.00 (I-M:1) , SSI=0.36/1.00 (H-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.52 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T23S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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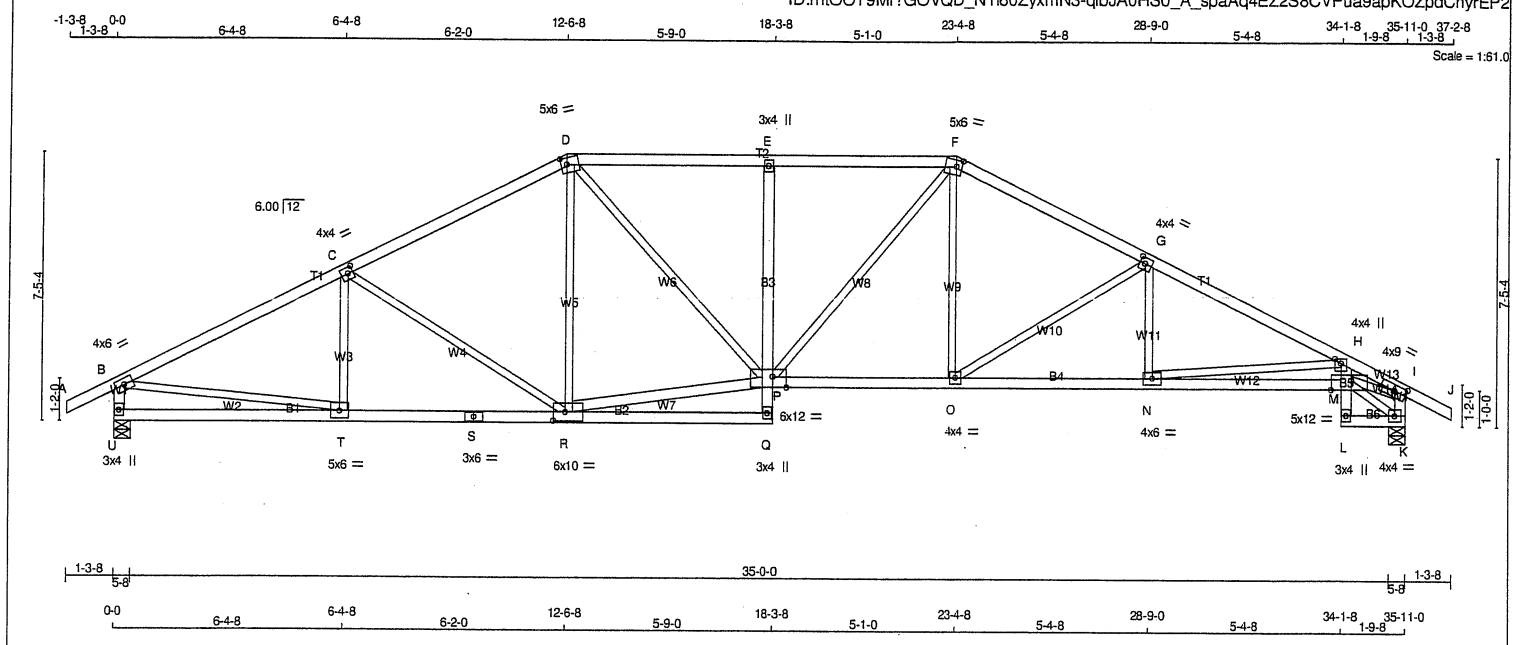

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
R - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW+p	MT20	4.0	4.0	1.50	2.00
I	TMVW-t	MT20	4.0	9.0	2.00	4.00
K	BMVW1-t	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-l	MT20	5.0	12.0	Edge	6.75
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-l	MT20	6.0	12.0	Edge	4.50
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	6.0	10.0	3.00	4.00
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126488

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1313	0	1313	0	0	5-8	5-8
K	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
K	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-1853 / 0	-56.6	-56.6	0.36 (1)	4.60
C- D	-1606 / 0	-56.6	-56.6	0.33 (1)	4.90
D- E	-1830 / 0	-56.6	-56.6	0.26 (1)	4.72
E- F	-1835 / 0	-56.6	-56.6	0.25 (1)	4.72
F- G	-1853 / 0	-56.6	-56.6	0.29 (1)	4.67
G- H	-2347 / 0	-56.6	-56.6	0.35 (1)	4.18
H- I	-3007 / 0	-56.6	-56.6	0.19 (1)	3.89
I- J	0 / 17	-56.6	-56.6	0.07 (1)	10.00
U- B	-1278 / 0	0.0	0.0	0.13 (1)	7.13
K- I	-1143 / 0	0.0	0.0	0.12 (1)	7.43
U- T	0 / 0	-12.3	-12.3	0.10 (4)	10.00
T- S	0 / 1674	-12.3	-12.3	0.33 (1)	10.00
S- R	0 / 1674	-12.3	-12.3	0.33 (1)	10.00
R- Q	0 / 34	-12.3	-12.3	0.10 (4)	10.00
Q- P	0 / 30	0.0	0.0	0.05 (1)	10.00
P- E	-373 / 0	0.0	0.0	0.17 (1)	7.81
P- O	0 / 1646	-12.3	-12.3	0.31 (1)	10.00
O- N	0 / 2122	-12.3	-12.3	0.39 (1)	10.00
N- M	0 / 3119	-12.3	-12.3	0.55 (1)	10.00
L- M	0 / 11	0.0	0.0	0.37 (1)	10.00
M- H	0 / 331	0.0	0.0	0.42 (1)	10.00
L- K	0 / 243	-12.3	-12.3	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-145 / 13	0.04 (1)
C- R	-306 / 0	0.34 (1)
R- D	0 / 55	0.02 (4)
R- P	0 / 1407	0.23 (1)
D- P	0 / 610	0.14 (1)
P- F	0 / 291	0.07 (1)
O- F	0 / 376	0.08 (1)
O- G	-569 / 0	0.43 (1)
N- G	0 / 135	0.04 (4)
N- H	-1004 / 0	0.53 (1)
B- T	0 / 1689	0.38 (1)
M- K	-292 / 0	0.04 (1)
M- I	0 / 2684	0.60 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.36/1.00 (B-C:1) , BC=0.55/1.00 (M-N:1) , WB=0.60/1.00 (I-M:1) , SSI=0.36/1.00 (H-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.52 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T24S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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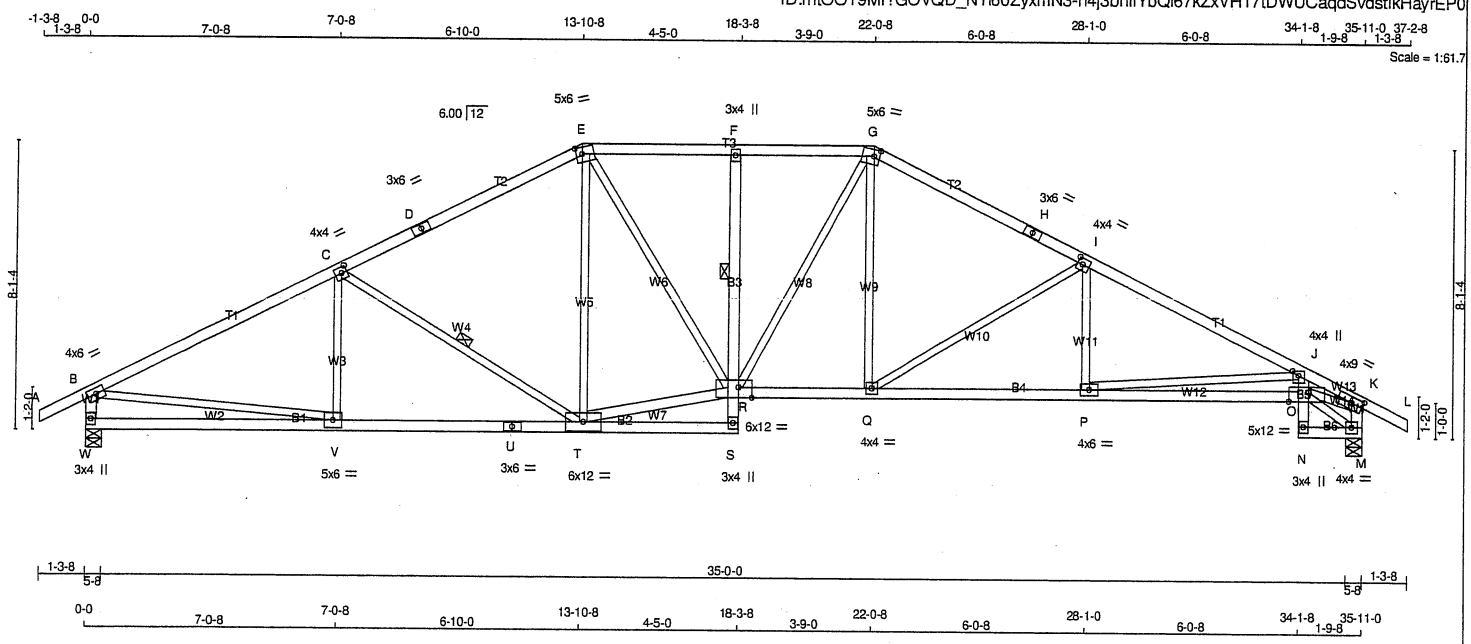

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW+p	MT20	4.0	4.0	1.50	2.00
K	TMVW-t	MT20	4.0	9.0	2.00	4.00
M	BMVW1-t	MT20	4.0	4.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	5.0	12.0	Edge	6.75
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BVMWWW-l	MT20	6.0	12.0	Edge	4.50
S	BMV+p	MT20	3.0	4.0		
T	BMWWW-t	MT20	6.0	12.0		
U	BS-t	MT20	3.0	6.0		
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126489

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	1313	0	1313	0	0	5-8	5-8
M	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
M	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-R, C-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-1861 / 0	-56.6	-56.6	0.46 (1)	4.49
C- D	-1534 / 0	-56.6	-56.6	0.41 (1)	4.88
D- E	-1534 / 0	-56.6	-56.6	0.41 (1)	4.88
E- F	-1639 / 0	-56.6	-56.6	0.14 (1)	5.06
F- G	-1642 / 0	-56.6	-56.6	0.14 (1)	5.05
G- H	-1746 / 0	-56.6	-56.6	0.35 (1)	4.71
H- I	-1746 / 0	-56.6	-56.6	0.35 (1)	4.71
I- J	-2300 / 0	-56.6	-56.6	0.42 (1)	4.16
J- K	-3039 / 0	-56.6	-56.6	0.17 (1)	3.89
K- L	0 / 17	-56.6	-56.6	0.07 (1)	10.00
W- B	-1276 / 0	0.0	0.0	0.13 (1)	7.13
M- K	-1141 / 0	0.0	0.0	0.12 (1)	7.44
W- V	0 / 0	-12.3	-12.3	0.14 (4)	10.00
V- U	0 / 1683	-12.3	-12.3	0.33 (1)	10.00
U- T	0 / 1683	-12.3	-12.3	0.33 (1)	10.00
T- S	0 / 24	-12.3	-12.3	0.09 (4)	10.00
S- R	0 / 20	0.0	0.0	0.04 (1)	10.00
R- F	-276 / 0	0.0	0.0	0.03 (1)	6.25
R- Q	0 / 1549	-12.3	-12.3	0.29 (1)	10.00
Q- P	0 / 2079	-12.3	-12.3	0.39 (1)	10.00
P- O	0 / 3158	-12.3	-12.3	0.56 (1)	10.00
N- O	0 / 11	0.0	0.0	0.37 (1)	10.00
O- J	0 / 331	0.0	0.0	0.42 (1)	10.00
N- M	0 / 246	-12.3	-12.3	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-112 / 35	0.04 (1)
C- T	-392 / 0	0.18 (1)
T- E	-23 / 42	0.03 (1)
T- R	0 / 1365	0.22 (1)
E- R	0 / 539	0.12 (1)
R- G	0 / 189	0.04 (1)
Q- G	0 / 397	0.09 (1)
Q- I	-630 / 0	0.64 (1)
P- I	0 / 150	0.04 (4)
P- J	-1085 / 0	0.76 (1)
B- V	0 / 1695	0.38 (1)
O- M	-296 / 0	0.04 (1)
O- K	0 / 2718	0.61 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 ; ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.56/1.00 (O-P:1) , WB=0.76/1.00 (J-P:1) , SSI=0.37/1.00 (J-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.53 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T25	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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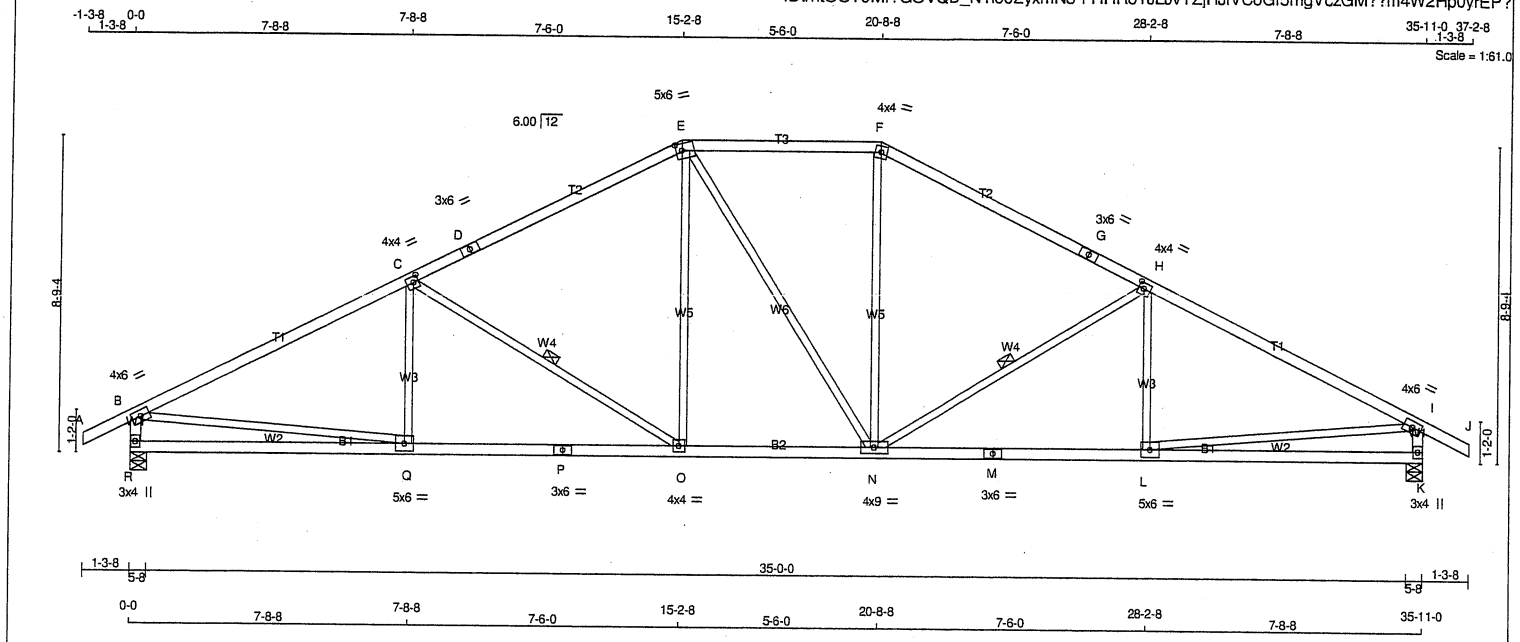

TOTAL WEIGHT = 146 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMVW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1314	0	1314	0	0	5-8	5-8
K	1314	0	1314	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	931	600 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
K	931	600 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-1864 / 0	-56.6	-56.6	0.57 (1)	4.36
C- D	-1457 / 0	-56.6	-56.6	0.50 (1)	4.86
D- E	-1457 / 0	-56.6	-56.6	0.50 (1)	4.86
E- F	-1286 / 0	-56.6	-56.6	0.24 (1)	5.43
F- G	-1458 / 0	-56.6	-56.6	0.50 (1)	4.86
G- H	-1458 / 0	-56.6	-56.6	0.50 (1)	4.86
H- I	-1863 / 0	-56.6	-56.6	0.57 (1)	4.36
I- J	0 / 17	-56.6	-56.6	0.07 (1)	10.00
R- B	-1275 / 0	0.0	0.0	0.13 (1)	7.13
K- I	-1275 / 0	0.0	0.0	0.13 (1)	7.13
R- Q	0 / 0	-12.3	-12.3	0.18 (4)	10.00
Q- P	0 / 1687	-12.3	-12.3	0.36 (1)	10.00
P- O	0 / 1687	-12.3	-12.3	0.36 (1)	10.00
O- N	0 / 1286	-12.3	-12.3	0.26 (1)	10.00
N- M	0 / 1687	-12.3	-12.3	0.36 (1)	10.00
M- L	0 / 1687	-12.3	-12.3	0.36 (1)	10.00
L- K	0 / 0	-12.3	-12.3	0.18 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-82 / 54	0.03 (1)
C- O	-477 / 0	0.27 (1)
O- E	0 / 327	0.07 (1)
E- N	0 / 0	0.00 (1)
N- F	0 / 328	0.07 (1)
N- H	-476 / 0	0.26 (1)
L- H	-82 / 54	0.03 (1)
B- Q	0 / 1698	0.38 (1)
L- I	0 / 1698	0.38 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.36/1.00 (O-Q:1) , WB=0.38/1.00 (B-Q:1) , SSI=0.19/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.57 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126490

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T26	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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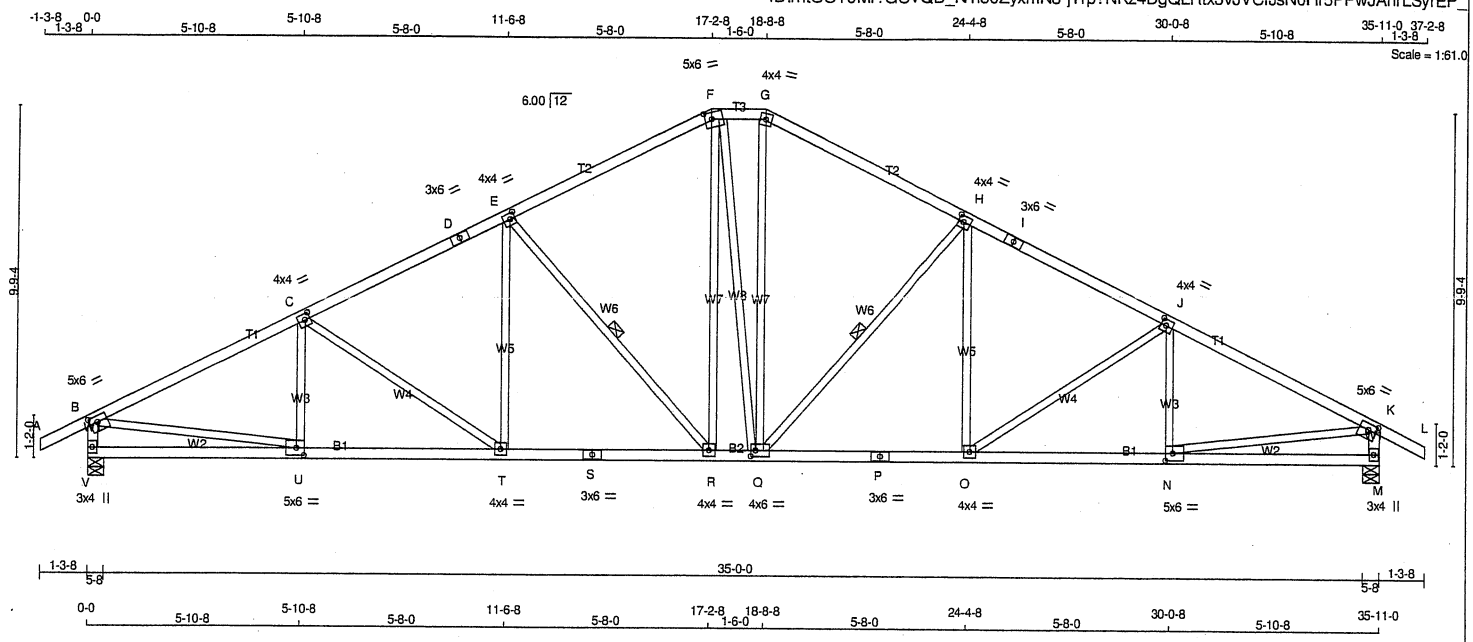

TOTAL WEIGHT = 164 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C, E, H, J						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.25
G	TTW-m	MT20	4.0	4.0		
I	TS-t	MT20	3.0	6.0		
K	TMVW-t	MT20	5.0	6.0	2.00	2.75
M	BMV1+p	MT20	3.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O, R, T						
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0	2.00	1.50
S	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2126491

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1971	0	1971	0	0	5-8	5-8
M	1971	0	1971	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1397	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-R, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2755 / 0	-84.9	-84.9	0.50 (1)	3.77
C- D	-2505 / 0	-84.9	-84.9	0.40 (1)	4.03
D- E	-2505 / 0	-84.9	-84.9	0.40 (1)	4.03
E- F	-1981 / 0	-84.9	-84.9	0.39 (1)	4.44
F- G	-1764 / 0	-84.9	-84.9	0.06 (1)	5.00
G- H	-1986 / 0	-84.9	-84.9	0.39 (1)	4.43
H- I	-2503 / 0	-84.9	-84.9	0.40 (1)	4.03
I- J	-2503 / 0	-84.9	-84.9	0.40 (1)	4.03
J- K	-2756 / 0	-84.9	-84.9	0.50 (1)	3.77
K- L	0 / 26	-84.9	-84.9	0.11 (1)	10.00
V- B	-1923 / 0	0.0	0.0	0.19 (1)	6.07
M- K	-1924 / 0	0.0	0.0	0.19 (1)	6.06
V- U	0 / 0	-18.5	-18.5	0.13 (4)	10.00
U- T	0 / 2482	-18.5	-18.5	0.46 (1)	10.00
T- S	0 / 2240	-18.5	-18.5	0.43 (1)	10.00
S- R	0 / 2240	-18.5	-18.5	0.43 (1)	10.00
R- Q	0 / 1759	-18.5	-18.5	0.37 (1)	10.00
Q- P	0 / 2239	-18.5	-18.5	0.45 (1)	10.00
P- O	0 / 2239	-18.5	-18.5	0.45 (1)	10.00
O- N	0 / 2483	-18.5	-18.5	0.45 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.13 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-251 / 0	0.06 (1)
C- T	-291 / 0	0.25 (1)
T- E	0 / 275	0.06 (4)
E- R	-743 / 0	0.39 (1)
R- F	0 / 588	0.13 (1)
F- Q	0 / 33	0.01 (1)
Q- G	0 / 623	0.14 (1)
Q- H	-734 / 0	0.39 (1)
O- H	0 / 265	0.06 (4)
O- J	-294 / 0	0.26 (1)
N- J	-249 / 1	0.06 (1)
B- U	0 / 2509	0.56 (1)
N- K	0 / 2509	0.56 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.50/1.00 (J-K:1) , BC=0.46/1.00 (T-U:1) , WB=0.56/1.00 (K-N:1) , SSI=0.21/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.73 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	T27	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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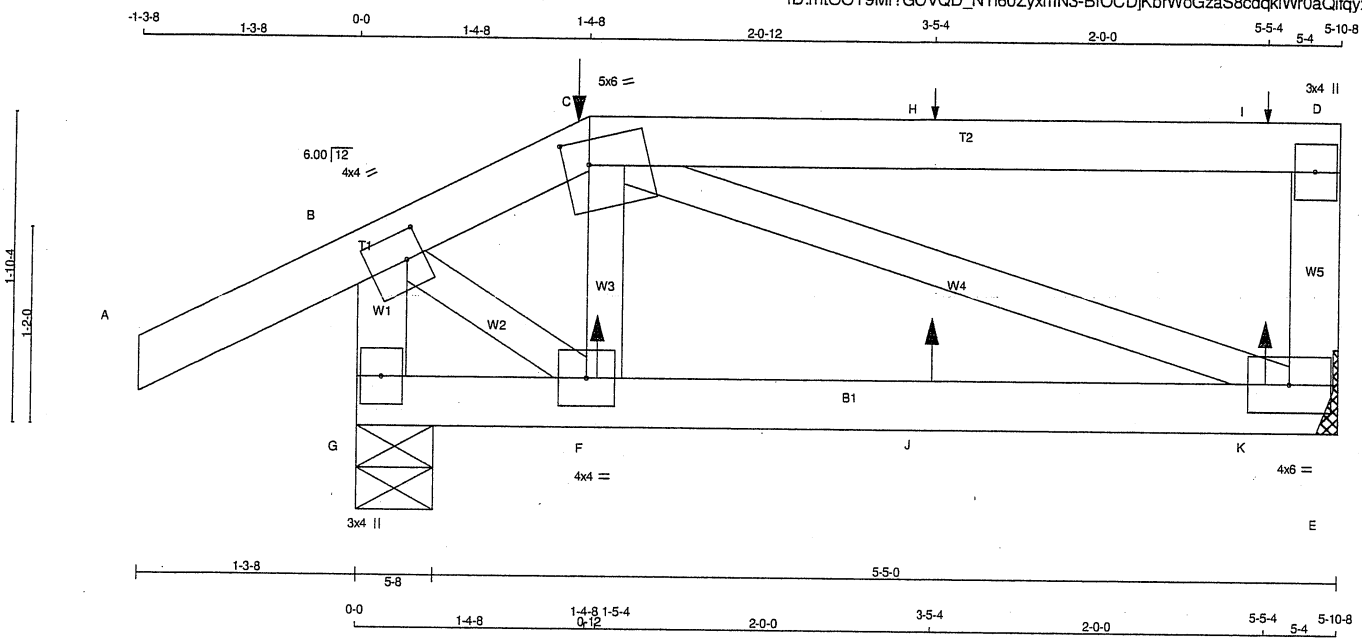

TOTAL WEIGHT = 23 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	6.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	483	0	483	0	0	MECHANICAL	
G	661	0	661	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	338	243 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
G	460	344 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.08 (1)	10.00
B- C	-529 / 0	-56.6	-56.6	0.08 (1)	6.25
C- H	0 / 0	-140.6	-140.6	0.53 (1)	10.00
H- I	0 / 0	-140.6	-140.6	0.53 (1)	10.00
I- D	0 / 0	-140.6	-140.6	0.53 (1)	10.00
E- D	-316 / 0	0.0	0.0	0.04 (1)	7.81
G- B	-650 / 0	0.0	0.0	0.07 (1)	7.81
G- F	0 / 0	-12.3	-12.3	0.05 (4)	10.00
F- J	0 / 440	-12.3	-12.3	0.11 (1)	10.00
J- K	0 / 440	-12.3	-12.3	0.11 (1)	10.00
K- E	0 / 440	-12.3	-12.3	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-249 / 0	0.04 (1)
C- E	-466 / 0	0.16 (1)
B- F	0 / 528	0.13 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-4-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	1-4-8	-52	-52	53	FRONT	VERT	TOTAL	---	C1
C	1-4-8	-134	-134	---	FRONT	VERT	SNOW	---	C1
F	1-5-4	7	1	9	FRONT	VERT	TOTAL	---	C1
H	3-5-4	1	1	53	FRONT	VERT	TOTAL	---	C1
I	5-5-4	1	1	45	FRONT	VERT	TOTAL	---	C1
J	3-5-4	7	1	9	FRONT	VERT	TOTAL	---	C1
K	5-5-4	6	1	8	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.53/1.00 (C-D:1) , BC=0.11/1.00 (E-F:1) , WB=0.16/1.00 (C-E:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126492

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T28S	1	3	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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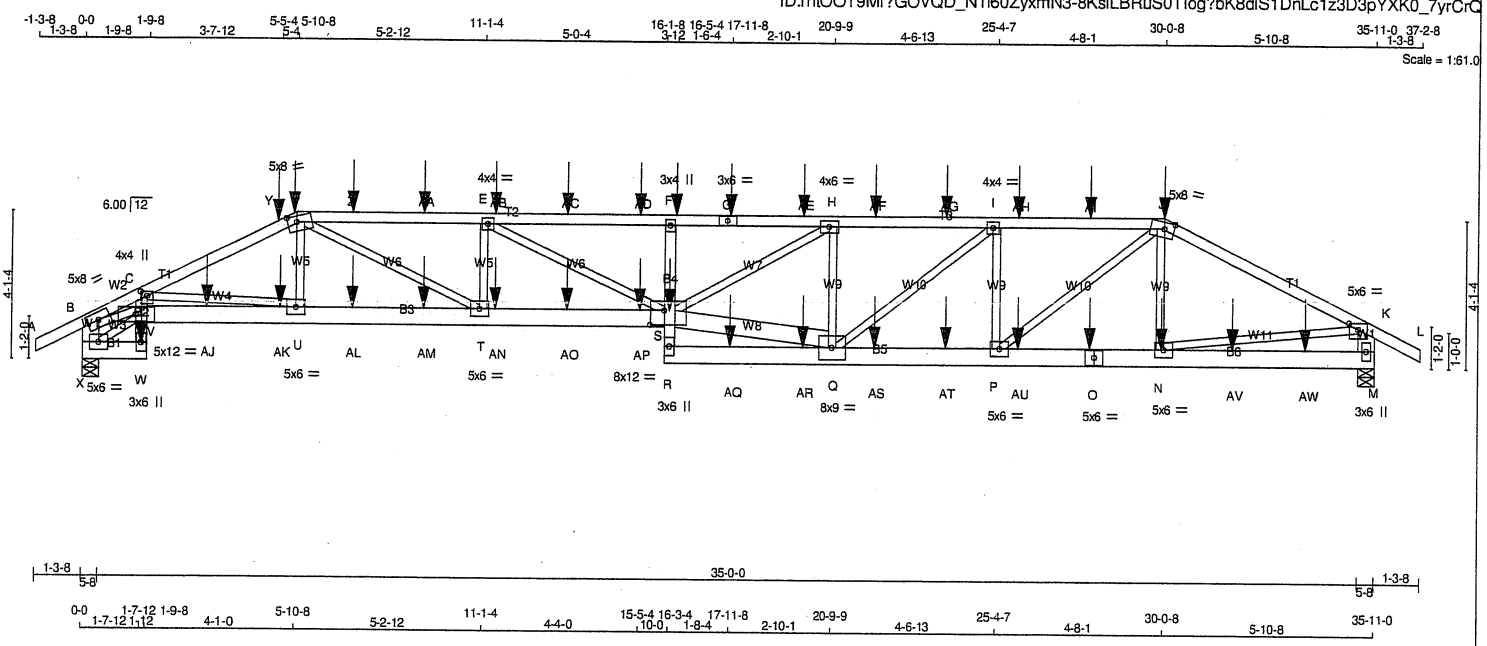

TOTAL WEIGHT = 3 X 169 = 508 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
X - W	2x6	DRY	No.2	SPF
W - C	2x4	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
R - F	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(91.5)
D- G	1	12	SIDE(91.5)
G- J	1	12	SIDE(91.5)
J- L	1	12	SIDE(91.5)
X- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X- W	2	12	SIDE(183.1)
V- S	2	12	SIDE(274.6)
R- O	2	12	SIDE(10.5)
O- M	2	12	SIDE(274.6)
W- C	1	12	TOP
F- R	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126494

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	3783	0	3783	0	0	5-8	5-8
X	4260	0	4260	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2684	1715 / 0	0 / 0	0 / 0	0 / 0	968 / 0	0 / 0
X	3013	1976 / 0	0 / 0	0 / 0	0 / 0	1037 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.04 (1)	10.00
B- C	-10146 / 0	-84.9	-84.9	0.27 (1)	3.58
C- Y	-8118 / 0	-84.9	-84.9	0.31 (1)	3.96
Y- D	-8118 / 0	-84.9	-84.9	0.31 (1)	3.96
D- Z	-11078 / 0	-84.9	-84.9	0.41 (1)	3.41
Z-AA	-11078 / 0	-84.9	-84.9	0.41 (1)	3.41
AA- E	-11078 / 0	-84.9	-84.9	0.41 (1)	3.41
E-AB	-12853 / 0	-84.9	-84.9	0.50 (1)	3.12
AB-AC	-12853 / 0	-84.9	-84.9	0.50 (1)	3.12
AC-AD	-12853 / 0	-84.9	-84.9	0.50 (1)	3.12
AD- F	-12853 / 0	-84.9	-84.9	0.50 (1)	3.12
F- G	-12720 / 0	-84.9	-84.9	0.42 (1)	3.18
G-AE	-12720 / 0	-84.9	-84.9	0.42 (1)	3.18
AE- H	-12720 / 0	-84.9	-84.9	0.42 (1)	3.18
H-AF	-9189 / 0	-84.9	-84.9	0.31 (1)	3.77
AF-AG	-9189 / 0	-84.9	-84.9	0.31 (1)	3.77
AG- I	-9189 / 0	-84.9	-84.9	0.31 (1)	3.77
I-AH	-7860 / 0	-84.9	-84.9	0.27 (1)	4.06
AH-AI	-7860 / 0	-84.9	-84.9	0.27 (1)	4.06
AI- J	-7860 / 0	-84.9	-84.9	0.27 (1)	4.06
J- K	-5901 / 0	-84.9	-84.9	0.28 (1)	4.60
K- L	0 / 26	-84.9	-84.9	0.04 (1)	10.00
X- B	-3695 / 0	0.0	0.0	0.08 (1)	7.81
M- K	-3693 / 0	0.0	0.0	0.08 (1)	7.81
X- W	0 / 906	-18.5	-18.5	0.04 (1)	10.00
W- V	0 / 592	0.0	0.0	0.44 (1)	10.00
V- C	0 / 1327	0.0	0.0	0.48 (1)	10.00
V-AJ	0 / 10490	-18.5	-18.5	0.45 (1)	10.00
AJ-AK	0 / 10490	-18.5	-18.5	0.45 (1)	10.00
AK- U	0 / 10490	-18.5	-18.5	0.45 (1)	10.00
U-AL	0 / 7339	-18.5	-18.5	0.34 (1)	10.00
AL-AM	0 / 7339	-18.5	-18.5	0.34 (1)	10.00
AM- T	0 / 7339	-18.5	-18.5	0.34 (1)	10.00
T-AN	0 / 11078	-18.5	-18.5	0.50 (1)	10.00
AN-AO	0 / 11078	-18.5	-18.5	0.50 (1)	10.00
AO-AP	0 / 11078	-18.5	-18.5	0.50 (1)	10.00
AP- S	0 / 11078	-18.5	-18.5	0.50 (1)	10.00
R- S	0 / 75	0.0	0.0	0.17 (1)	10.00
S- F	-787 / 0	0.0	0.0	0.17 (1)	7.81
R-AQ	0 / 365	-18.5	-18.5	0.04 (4)	10.00
AQ-AR	0 / 365	-18.5	-18.5	0.04 (4)	10.00
AR- Q	0 / 365	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- U	-3212 / 0	0.34 (1)
U- D	0 / 831	0.06 (1)
S- Q	0 / 9096	0.36 (1)
S- H	0 / 4045	0.30 (1)
Q- H	-2857 / 0	0.23 (1)
Q- I	0 / 1729	0.13 (1)
P- I	-1939 / 0	0.15 (1)
P- J	0 / 3322	0.25 (1)
N- J	-469 / 5	0.04 (1)
N- K	0 / 5319	0.40 (1)
X- V	-1060 / 0	0.05 (1)
B- V	0 / 9058	0.68 (1)
E- S	0 / 2008	0.15 (1)
D- T	0 / 4214	0.32 (1)
T- E	-1738 / 0	0.10 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 725 (0.59")

CSI: TC=0.50/1.00 (E-F:1) , BC=0.50/1.00 (S-T:1) , WB=0.68/1.00 (B-V:1) , SSI=0.39/1.00 (C-V:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T28S	1	3	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TTWW-m	MT20	5.0	8.0	2.00	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMWW-t	MT20	4.0	4.0		
J	TTWW-m	MT20	5.0	8.0	2.25	3.25
K	TMVW-p	MT20	5.0	6.0	2.00	3.00
M	BMV1+p	MT20	3.0	6.0		
N, P, T, U						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	8.0	9.0		
R	BMV+p	MT20	3.0	6.0		
S	BVMWWW-l	MT20	8.0	12.0	5.00	4.75
V	BVMWW-l	MT20	5.0	12.0		
W	BMV+p	MT20	3.0	6.0		
X	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
Q-AS	0 / 7860	-18.5	-18.5	0.34 (1)	10.00
AS-AT	0 / 7860	-18.5	-18.5	0.34 (1)	10.00
AT- P	0 / 7860	-18.5	-18.5	0.34 (1)	10.00
P-AU	0 / 5262	-18.5	-18.5	0.24 (1)	10.00
AU- O	0 / 5262	-18.5	-18.5	0.24 (1)	10.00
O- N	0 / 5262	-18.5	-18.5	0.24 (1)	10.00
N-AV	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AV-AW	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AW- M	0 / 0	-18.5	-18.5	0.05 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
D	5-10-8	-133	-133	---	FRONT	VERT	SNOW	---	C1
F	16-5-4	-113	-113	---	FRONT	VERT	TOTAL	---	C1
G	17-11-8	-113	-113	---	FRONT	VERT	TOTAL	---	C1
J	30-0-8	-374	-374	---	FRONT	VERT	TOTAL	---	C1
N	29-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
O	27-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
S	16-3-4	-35	-35	---	FRONT	VERT	TOTAL	---	C1
W	1-7-12	-402	-402	---	FRONT	VERT	TOTAL	---	C1
Y	5-5-4	-90	-90	---	FRONT	VERT	TOTAL	---	C1
Z	7-5-4	-73	-73	---	FRONT	VERT	TOTAL	---	C1
AA	9-5-4	-73	-73	---	FRONT	VERT	TOTAL	---	C1
AB	11-5-4	-73	-73	---	FRONT	VERT	TOTAL	---	C1
AC	13-5-4	-73	-73	---	FRONT	VERT	TOTAL	---	C1
AD	15-5-4	-73	-73	---	FRONT	VERT	TOTAL	---	C1
AE	19-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AF	21-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AG	23-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AH	25-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AI	27-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AJ	3-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AK	5-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AL	7-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AM	9-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AN	11-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AO	13-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AP	15-5-4	-61	-61	---	FRONT	VERT	TOTAL	---	C1
AQ	17-11-8	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AR	19-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AS	21-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AT	23-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AU	25-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AV	31-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AW	33-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.62 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126494

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T29S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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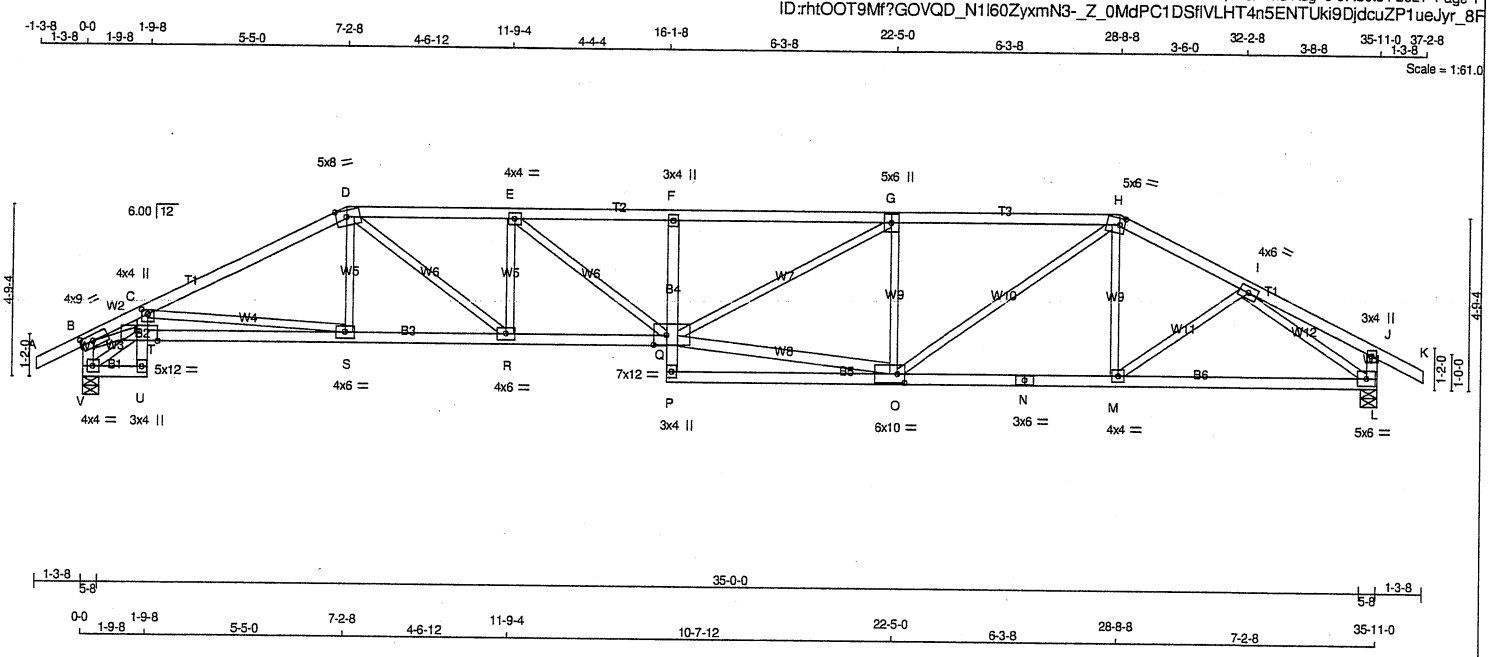

TOTAL WEIGHT = 148 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	2100F 1.8E	SPF
P - F	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TSWW+l	MT20	5.0	6.0		
H	TTWW-m	MT20	5.0	6.0	2.25	1.75
I	TMWW-t	MT20	4.0	6.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	6.0	10.0	3.00	2.50
P	BMV+p	MT20	3.0	4.0		
Q	BVMWWW-l	MT20	7.0	12.0	3.25	4.50
R	BMWW-t	MT20	4.0	6.0		
S	BMWW-t	MT20	4.0	6.0		
T	BVMWW-l	MT20	5.0	12.0	Edge	6.75
U	BMV+p	MT20	3.0	4.0		
V	BMVW1-t	MT20	4.0	4.0		

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126495

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1313	0	1313	0	0	5-8	5-8
V	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
V	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)		10.00
B- C	-3134 / 0	-56.6	-56.6	0.17 (1)		3.83
C- D	-2293 / 0	-56.6	-56.6	0.45 (1)		4.11
D- E	-2829 / 0	-56.6	-56.6	0.28 (1)		3.93
E- F	-3204 / 0	-56.6	-56.6	0.40 (1)		3.59
F- G	-3172 / 0	-56.6	-56.6	0.51 (1)		3.57
G- H	-2317 / 0	-56.6	-56.6	0.59 (1)		3.94
H- I	-1804 / 0	-56.6	-56.6	0.13 (1)		4.87
I- J	0 / 8	-56.6	-56.6	0.09 (1)		10.00
J- K	0 / 17	-56.6	-56.6	0.07 (1)		10.00
V- B	-1135 / 0	0.0	0.0	0.11 (1)		7.45
L- J	-161 / 0	0.0	0.0	0.02 (1)		7.81
V- U	0 / 254	-12.3	-12.3	0.05 (1)		10.00
U- T	0 / 11	0.0	0.0	0.38 (1)		10.00
T- C	0 / 344	0.0	0.0	0.44 (1)		10.00
T- S	0 / 3275	-12.3	-12.3	0.28 (1)		10.00
S- R	0 / 2055	-12.3	-12.3	0.19 (1)		10.00
R- Q	0 / 2829	-12.3	-12.3	0.24 (1)		10.00
P- Q	0 / 36	0.0	0.0	0.16 (1)		10.00
Q- F	-342 / 0	0.0	0.0	0.16 (1)		7.81
P- O	0 / 110	-12.3	-12.3	0.11 (4)		10.00
O- N	0 / 1605	-12.3	-12.3	0.33 (1)		10.00
N- M	0 / 1605	-12.3	-12.3	0.33 (1)		10.00
M- L	0 / 1511	-12.3	-12.3	0.32 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	-1234 / 0	0.70 (1)
S- D	0 / 166	0.04 (4)
Q- O	0 / 2270	0.36 (1)
Q- G	0 / 946	0.21 (1)
O- G	-795 / 0	0.27 (1)
O- H	0 / 878	0.20 (1)
M- H	0 / 58	0.02 (4)
M- I	0 / 116	0.03 (4)
I- L	-1877 / 0	0.62 (1)
V- T	-306 / 0	0.05 (1)
B- T	0 / 2821	0.63 (1)
E- Q	0 / 474	0.11 (1)
D- R	0 / 973	0.22 (1)
R- E	-543 / 0	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 982 (0.44")

CSI: TC=0.59/1.00 (G-H:1) , BC=0.44/1.00 (C-T:1) , WB=0.70/1.00 (C-S:1) , SSI=0.38/1.00 (C-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.55 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T30S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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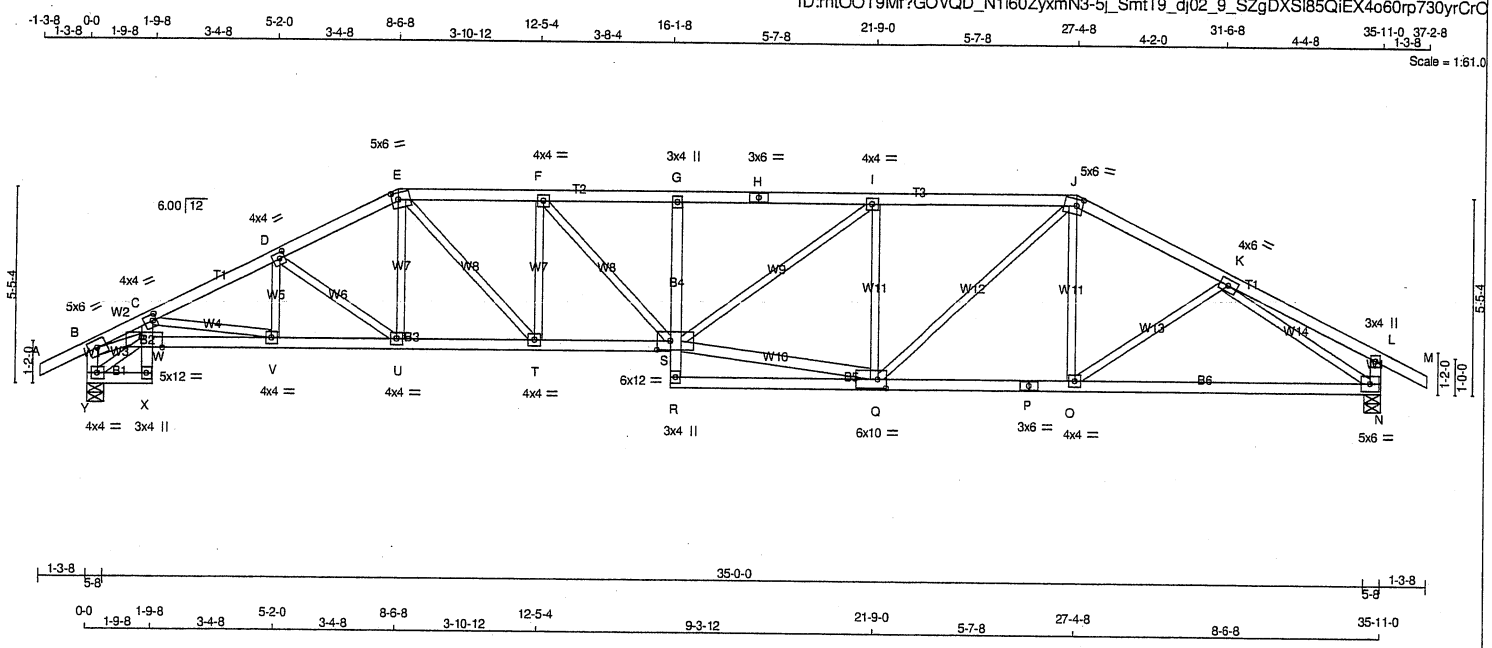

TOTAL WEIGHT = 155 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - X	2x4	DRY	No.2	SPF
X - C	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0		
J	TTWW-m	MT20	5.0	6.0	2.25	2.00
K	TMWW-t	MT20	4.0	6.0		
L	TMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		
O, T, U, V						
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	6.0	10.0	3.00	2.75
R	BMV+p	MT20	3.0	4.0		
S	BVMWWW-l	MT20	6.0	12.0	3.00	4.50
W	BVMWW-l	MT20	5.0	12.0	Edge	6.75
X	BMV+p	MT20	3.0	4.0		
Y	BMVW1-t	MT20	4.0	4.0		

LICENSED PROFESSIONAL ENGINEER
08/05/14
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126496

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1313	0	1313	0	0	5-8	5-8
Y	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
Y	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, Y

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-2930 / 0	-56.6	-56.6	0.29 (1)	3.79
C- D	-2498 / 0	-56.6	-56.6	0.30 (1)	4.08
D- E	-2179 / 0	-56.6	-56.6	0.20 (1)	4.44
E- F	-2437 / 0	-56.6	-56.6	0.19 (1)	4.27
F- G	-2668 / 0	-56.6	-56.6	0.20 (1)	4.09
G- H	-2650 / 0	-56.6	-56.6	0.42 (1)	3.91
H- I	-2650 / 0	-56.6	-56.6	0.42 (1)	3.91
I- J	-2093 / 0	-56.6	-56.6	0.37 (1)	4.35
J- K	-1778 / 0	-56.6	-56.6	0.20 (1)	4.83
K- L	0 / 11	-56.6	-56.6	0.14 (1)	10.00
L- M	0 / 17	-56.6	-56.6	0.07 (1)	10.00
Y- B	-1147 / 0	0.0	0.0	0.12 (1)	7.42
N- L	-174 / 0	0.0	0.0	0.02 (1)	7.81
Y- X	0 / 236	-12.3	-12.3	0.04 (1)	10.00
X- W	0 / 11	0.0	0.0	0.36 (1)	10.00
W- C	0 / 347	0.0	0.0	0.41 (1)	10.00
W- V	0 / 3025	-12.3	-12.3	0.52 (1)	10.00
V- U	0 / 2263	-12.3	-12.3	0.39 (1)	10.00
U- T	0 / 1942	-12.3	-12.3	0.33 (1)	10.00
T- S	0 / 2437	-12.3	-12.3	0.42 (1)	10.00
R- S	0 / 33	0.0	0.0	0.11 (1)	10.00
S- G	-245 / 0	0.0	0.0	0.11 (1)	7.81
R- Q	0 / 74	-12.3	-12.3	0.09 (4)	10.00
Q- P	0 / 1581	-12.3	-12.3	0.36 (1)	10.00
P- O	0 / 1581	-12.3	-12.3	0.36 (1)	10.00
O- N	0 / 1563	-12.3	-12.3	0.36 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- E	0 / 284	0.06 (1)
S- Q	0 / 2063	0.33 (1)
S- I	0 / 684	0.15 (1)
Q- I	-778 / 0	0.34 (1)
Q- J	0 / 697	0.16 (1)
O- J	0 / 94	0.03 (4)
O- K	0 / 53	0.02 (4)
K- N	-1908 / 0	0.86 (1)
Y- W	-284 / 0	0.04 (1)
B- W	0 / 2601	0.59 (1)
F- S	0 / 340	0.08 (1)
E- T	0 / 722	0.16 (1)
T- F	-479 / 0	0.14 (1)
D- U	-400 / 0	0.12 (1)
V- D	0 / 93	0.02 (4)
C- V	-775 / 0	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.42/1.00 (G-I:1) , BC=0.52/1.00 (V-W:1) , WB=0.86/1.00 (K-N:1) , SSI=0.35/1.00 (C-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.68 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T31S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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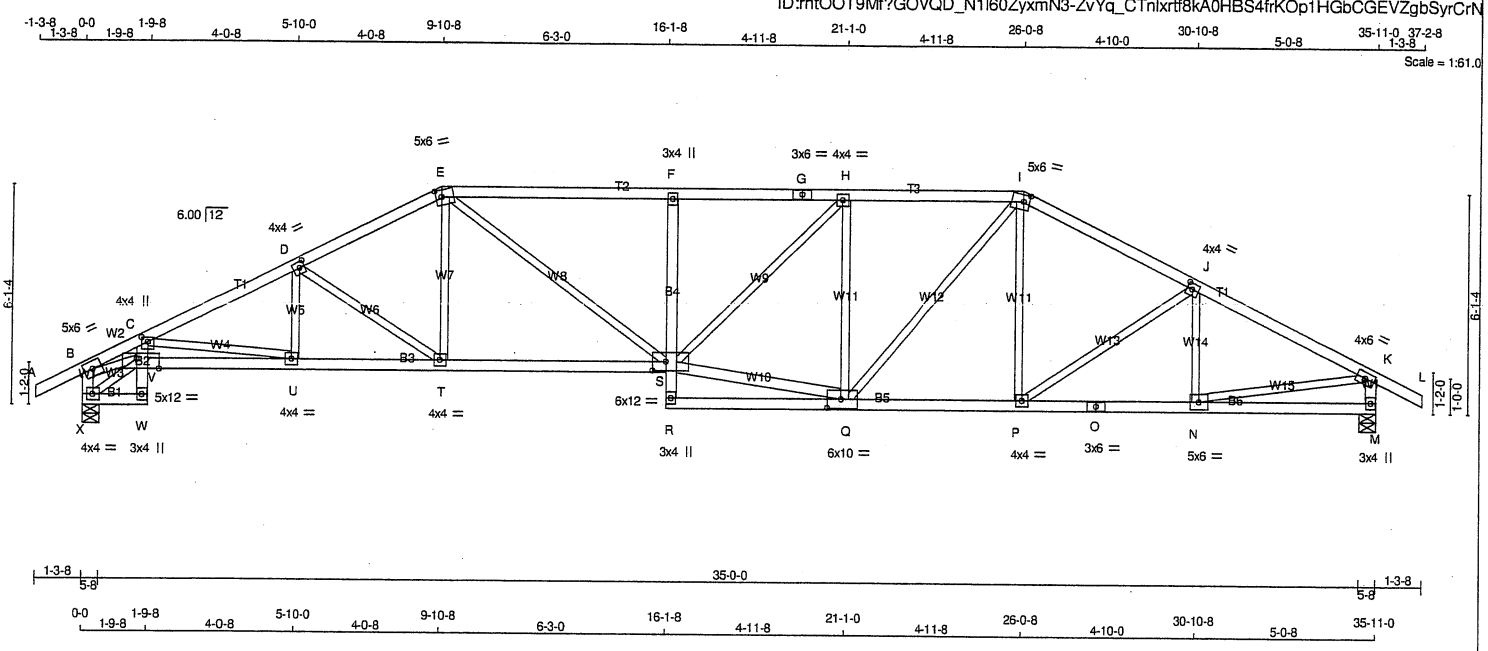

TOTAL WEIGHT = 155 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
X - W	2x4	DRY	No.2	SPF
W - C	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
R - F	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	6.0	2.25	2.00
J	TMWW-t	MT20	4.0	4.0	2.00	1.75
K	TMVW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P, T, U						
P	BMWW-t	MT20	4.0	4.0		
Q	BMWWW-t	MT20	6.0	10.0	3.00	4.50
R	BMV+p	MT20	3.0	4.0		
S	BVMWWW-l	MT20	6.0	12.0	3.00	4.50
V	BVMWW-l	MT20	5.0	12.0	Edge	7.25
W	BMV+p	MT20	3.0	4.0		
X	BMVW1-t	MT20	4.0	4.0		

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126497

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1313	0	1313	0	0	5-8	5-8
X	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
X	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-2952 / 0	-56.6	-56.6	0.23 (1)	3.88
C- D	-2444 / 0	-56.6	-56.6	0.28 (1)	4.16
D- E	-2073 / 0	-56.6	-56.6	0.20 (1)	4.55
E- F	-2323 / 0	-56.6	-56.6	0.38 (1)	4.17
F- G	-2312 / 0	-56.6	-56.6	0.28 (1)	4.27
G- H	-2312 / 0	-56.6	-56.6	0.28 (1)	4.27
H- I	-1869 / 0	-56.6	-56.6	0.21 (1)	4.74
I- J	-1722 / 0	-56.6	-56.6	0.21 (1)	4.90
J- K	-1814 / 0	-56.6	-56.6	0.22 (1)	4.79
K- L	0 / 17	-56.6	-56.6	0.07 (1)	10.00
X- B	-1146 / 0	0.0	0.0	0.12 (1)	7.43
M- K	-1284 / 0	0.0	0.0	0.13 (1)	7.11
X- W	0 / 238	-12.3	-12.3	0.04 (1)	10.00
W- V	0 / 11	0.0	0.0	0.36 (1)	10.00
V- C	0 / 339	0.0	0.0	0.41 (1)	10.00
V- U	0 / 3052	-12.3	-12.3	0.53 (1)	10.00
U- T	0 / 2211	-12.3	-12.3	0.41 (1)	10.00
T- S	0 / 1846	-12.3	-12.3	0.35 (1)	10.00
R- S	0 / 27	0.0	0.0	0.08 (1)	10.00
S- F	-361 / 0	0.0	0.0	0.10 (1)	7.81
R- Q	0 / 53	-12.3	-12.3	0.07 (4)	10.00
Q- P	0 / 1529	-12.3	-12.3	0.28 (1)	10.00
P- O	0 / 1634	-12.3	-12.3	0.29 (1)	10.00
O- N	0 / 1634	-12.3	-12.3	0.29 (1)	10.00
N- M	0 / 0	-12.3	-12.3	0.06 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- E	0 / 331	0.07 (1)
E- S	0 / 596	0.13 (1)
S- Q	0 / 1863	0.30 (1)
S- H	0 / 614	0.14 (1)
Q- H	-713 / 0	0.42 (1)
Q- I	0 / 529	0.12 (1)
P- I	0 / 136	0.03 (4)
P- J	-131 / 0	0.08 (1)
N- J	-216 / 0	0.05 (1)
N- K	0 / 1658	0.37 (1)
X- V	-287 / 0	0.04 (1)
B- V	0 / 2625	0.59 (1)
D- T	-448 / 0	0.18 (1)
U- D	0 / 101	0.03 (4)
C- U	-851 / 0	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.38/1.00 (E-F:1) , BC=0.53/1.00 (U-V:1) , WB=0.59/1.00 (B-V:1) , SSI=0.35/1.00 (C-V:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.68 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T32S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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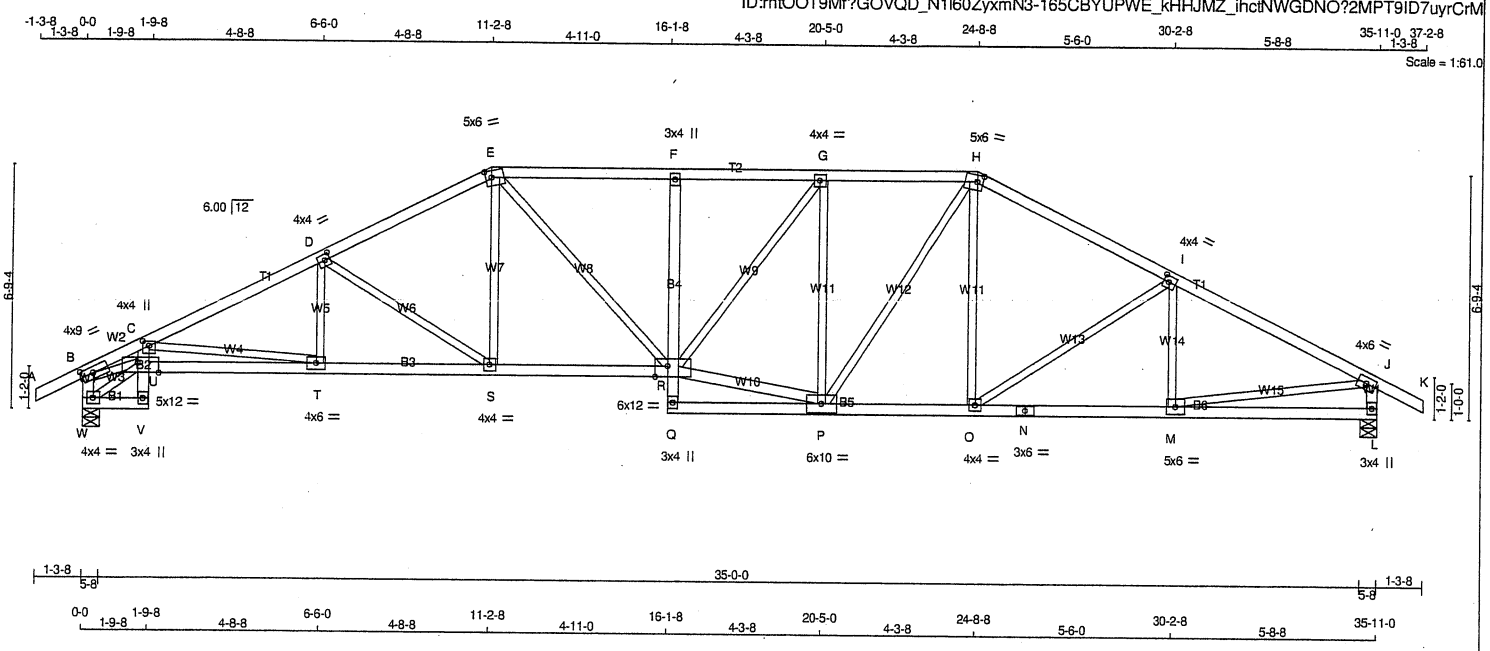

TOTAL WEIGHT = 160 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMV+p	MT20	3.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.25	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	6.0	10.0		
Q	BMV+p	MT20	3.0	4.0		
R	BVMWWW-l	MT20	6.0	12.0	Edge	4.25
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	4.0	6.0		
U	BVMWW-l	MT20	5.0	12.0	Edge	6.75
V	BMV+p	MT20	3.0	4.0		
W	BMVW1-t	MT20	4.0	4.0		

Structural component only
DWG# T-2126498

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1313	0	1313	0	0	5-8	5-8
W	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
W	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-2978 / 0	-56.6	-56.6	0.21 (1)	3.88
C- D	-2395 / 0	-56.6	-56.6	0.31 (1)	4.18
D- E	-1960 / 0	-56.6	-56.6	0.24 (1)	4.61
E- F	-2026 / 0	-56.6	-56.6	0.22 (1)	4.57
F- G	-2019 / 0	-56.6	-56.6	0.17 (1)	4.63
G- H	-1693 / 0	-56.6	-56.6	0.15 (1)	4.99
H- I	-1665 / 0	-56.6	-56.6	0.26 (1)	4.90
I- J	-1840 / 0	-56.6	-56.6	0.28 (1)	4.70
J- K	0 / 17	-56.6	-56.6	0.07 (1)	10.00
W- B	-1144 / 0	0.0	0.0	0.12 (1)	7.43
L- J	-1282 / 0	0.0	0.0	0.13 (1)	7.12
W- V	0 / 240	-12.3	-12.3	0.04 (1)	10.00
V- U	0 / 11	0.0	0.0	0.36 (1)	10.00
U- C	0 / 333	0.0	0.0	0.41 (1)	10.00
U- T	0 / 3083	-12.3	-12.3	0.54 (1)	10.00
T- S	0 / 2166	-12.3	-12.3	0.39 (1)	10.00
S- R	0 / 1744	-12.3	-12.3	0.32 (1)	10.00
Q- R	0 / 24	0.0	0.0	0.06 (1)	10.00
R- F	-288 / 0	0.0	0.0	0.09 (1)	7.81
Q- P	0 / 39	-12.3	-12.3	0.05 (4)	10.00
P- O	0 / 1476	-12.3	-12.3	0.27 (1)	10.00
O- N	0 / 1660	-12.3	-12.3	0.30 (1)	10.00
N- M	0 / 1660	-12.3	-12.3	0.30 (1)	10.00
M- L	0 / 0	-12.3	-12.3	0.09 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	-925 / 0	0.37 (1)
T- D	0 / 122	0.03 (4)
D- S	-511 / 0	0.29 (1)
S- E	0 / 347	0.08 (1)
E- R	0 / 417	0.09 (1)
R- P	0 / 1706	0.27 (1)
R- G	0 / 530	0.12 (1)
P- G	-671 / 0	0.51 (1)
P- H	0 / 396	0.09 (1)
O- H	0 / 185	0.04 (1)
O- I	-224 / 0	0.19 (1)
M- I	-175 / 0	0.04 (1)
M- J	0 / 1678	0.38 (1)
W- U	-289 / 0	0.04 (1)
B- U	0 / 2652	0.60 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.31/1.00 (C-D:1) , BC=0.54/1.00 (T-U:1) , WB=0.60/1.00 (B-U:1) , SSI=0.36/1.00 (C-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.52 (U) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T33S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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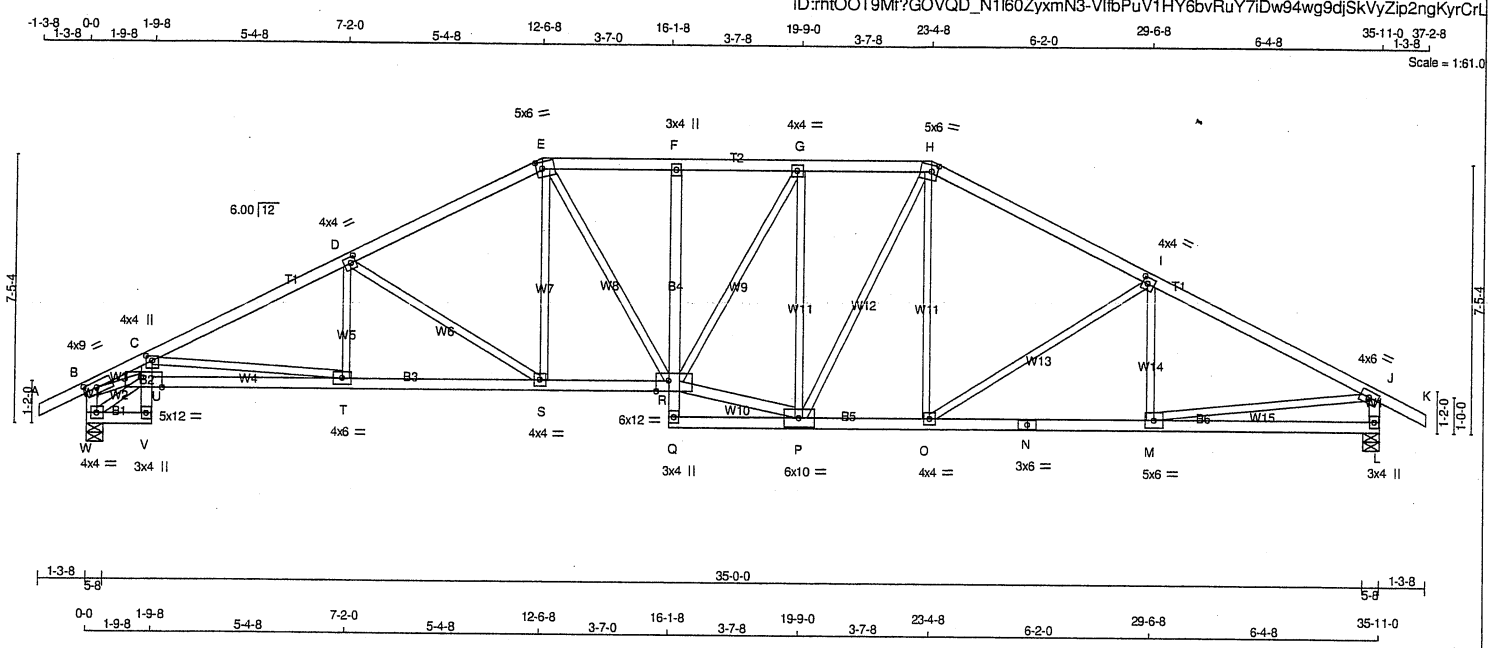

TOTAL WEIGHT = 165 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMV+p	MT20	3.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.25	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	6.0	10.0		
Q	BMV+p	MT20	3.0	4.0		
R	BVMWWW-l	MT20	6.0	12.0	Edge	4.25
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	4.0	6.0		
U	BVMWW-l	MT20	5.0	12.0	Edge	6.75
V	BMV+p	MT20	3.0	4.0		
W	BMVW1-t	MT20	4.0	4.0		

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126499

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1313	0	1313	0	0	5-8	5-8
W	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
W	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-3007 / 0	-56.6	-56.6	0.19 (1)	3.89
C- D	-2347 / 0	-56.6	-56.6	0.35 (1)	4.18
D- E	-1851 / 0	-56.6	-56.6	0.29 (1)	4.67
E- F	-1797 / 0	-56.6	-56.6	0.13 (1)	4.90
F- G	-1792 / 0	-56.6	-56.6	0.11 (1)	4.92
G- H	-1546 / 0	-56.6	-56.6	0.10 (1)	5.21
H- I	-1600 / 0	-56.6	-56.6	0.33 (1)	4.90
I- J	-1855 / 0	-56.6	-56.6	0.36 (1)	4.60
J- K	0 / 17	-56.6	-56.6	0.07 (1)	10.00
W- B	-1143 / 0	0.0	0.0	0.12 (1)	7.43
L- J	-1279 / 0	0.0	0.0	0.13 (1)	7.13
W- V	0 / 243	-12.3	-12.3	0.04 (1)	10.00
V- U	0 / 11	0.0	0.0	0.37 (1)	10.00
U- C	0 / 331	0.0	0.0	0.42 (1)	10.00
U- T	0 / 3119	-12.3	-12.3	0.55 (1)	10.00
T- S	0 / 2122	-12.3	-12.3	0.38 (1)	10.00
S- R	0 / 1645	-12.3	-12.3	0.30 (1)	10.00
Q- R	0 / 20	0.0	0.0	0.04 (1)	10.00
R- F	-214 / 0	0.0	0.0	0.08 (1)	7.81
Q- P	0 / 31	-12.3	-12.3	0.03 (4)	10.00
P- O	0 / 1417	-12.3	-12.3	0.26 (1)	10.00
O- N	0 / 1675	-12.3	-12.3	0.32 (1)	10.00
N- M	0 / 1675	-12.3	-12.3	0.32 (1)	10.00
M- L	0 / 0	-12.3	-12.3	0.12 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	-1003 / 0	0.53 (1)
T- D	0 / 138	0.04 (4)
D- S	-571 / 0	0.43 (1)
S- E	0 / 366	0.08 (1)
E- R	0 / 294	0.07 (1)
R- P	0 / 1580	0.25 (1)
R- G	0 / 490	0.11 (1)
P- G	-643 / 0	0.64 (1)
P- H	0 / 288	0.06 (1)
O- H	0 / 231	0.05 (1)
O- I	-312 / 0	0.35 (1)
M- I	-140 / 18	0.04 (1)
M- J	0 / 1690	0.38 (1)
W- U	-292 / 0	0.04 (1)
B- U	0 / 2684	0.60 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.36/1.00 (I-J:1) , BC=0.55/1.00 (T-U:1) , WB=0.64/1.00 (G-P:1) , SSI=0.36/1.00 (C-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.54 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T34S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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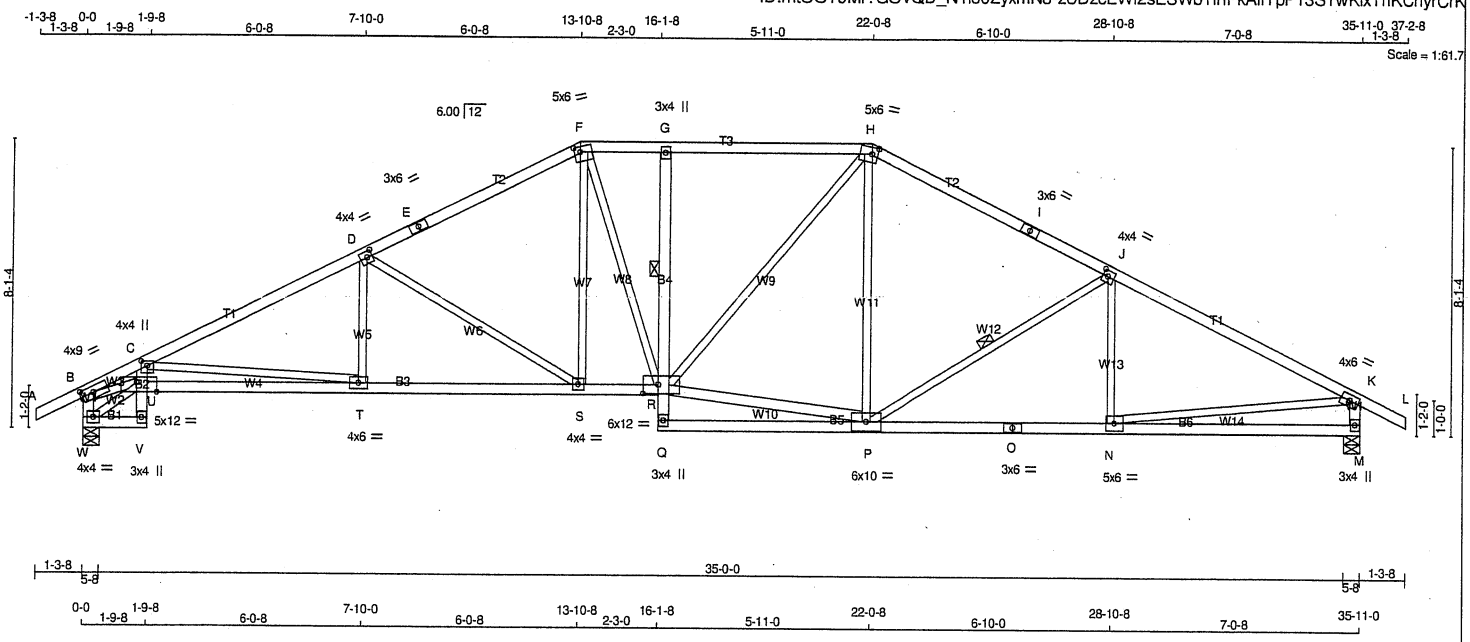

TOTAL WEIGHT = 162 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TTWW-m	MT20	5.0	6.0	1.75	2.00
G	TMV+p	MT20	3.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.25	2.00
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	4.0	4.0	2.00	1.75
K	TMVW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	6.0	10.0		
Q	BMV+p	MT20	3.0	4.0		
R	BVMWWW-l	MT20	6.0	12.0	3.00	5.00
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	4.0	6.0		
U	BVMWW-l	MT20	5.0	12.0	Edge	6.75
V	BMV+p	MT20	3.0	4.0		
W	BMVW1-t	MT20	4.0	4.0		

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126500

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1313	0	1313	0	0	5-8	5-8
W	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
W	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, W

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-R, J-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-3039 / 0	-56.6	-56.6	0.17 (1)	3.89
C- D	-2300 / 0	-56.6	-56.6	0.42 (1)	4.16
D- E	-1745 / 0	-56.6	-56.6	0.35 (1)	4.72
E- F	-1745 / 0	-56.6	-56.6	0.35 (1)	4.72
F- G	-1636 / 0	-56.6	-56.6	0.20 (1)	4.99
G- H	-1632 / 0	-56.6	-56.6	0.26 (1)	4.94
H- I	-1535 / 0	-56.6	-56.6	0.41 (1)	4.88
I- J	-1535 / 0	-56.6	-56.6	0.41 (1)	4.88
J- K	-1860 / 0	-56.6	-56.6	0.46 (1)	4.49
K- L	0 / 17	-56.6	-56.6	0.07 (1)	10.00
W- B	-1141 / 0	0.0	0.0	0.12 (1)	7.44
M- K	-1276 / 0	0.0	0.0	0.13 (1)	7.13
W- V	0 / 246	-12.3	-12.3	0.05 (1)	10.00
V- U	0 / 11	0.0	0.0	0.37 (1)	10.00
U- C	0 / 331	0.0	0.0	0.42 (1)	10.00
U- T	0 / 3158	-12.3	-12.3	0.56 (1)	10.00
T- S	0 / 2079	-12.3	-12.3	0.39 (1)	10.00
S- R	0 / 1548	-12.3	-12.3	0.28 (1)	10.00
Q- R	0 / 31	0.0	0.0	0.04 (1)	10.00
R- G	-319 / 0	0.0	0.0	0.04 (1)	6.25
Q- P	0 / 28	-12.3	-12.3	0.11 (4)	10.00
P- O	0 / 1682	-12.3	-12.3	0.34 (1)	10.00
O- N	0 / 1682	-12.3	-12.3	0.34 (1)	10.00
N- M	0 / 0	-12.3	-12.3	0.13 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	-1085 / 0	0.76 (1)
T- D	0 / 151	0.04 (4)
D- S	-631 / 0	0.64 (1)
S- F	0 / 391	0.09 (1)
F- R	0 / 266	0.06 (1)
R- P	0 / 1345	0.22 (1)
R- H	0 / 431	0.10 (1)
P- H	0 / 82	0.03 (4)
P- J	-390 / 0	0.18 (1)
N- J	-113 / 33	0.04 (1)
N- K	0 / 1695	0.38 (1)
W- U	-296 / 0	0.04 (1)
B- U	0 / 2718	0.61 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.46/1.00 (J-K:1) , BC=0.56/1.00 (T-U:1) , WB=0.76/1.00 (C-T:1) , SSI=0.37/1.00 (C-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.53 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T35S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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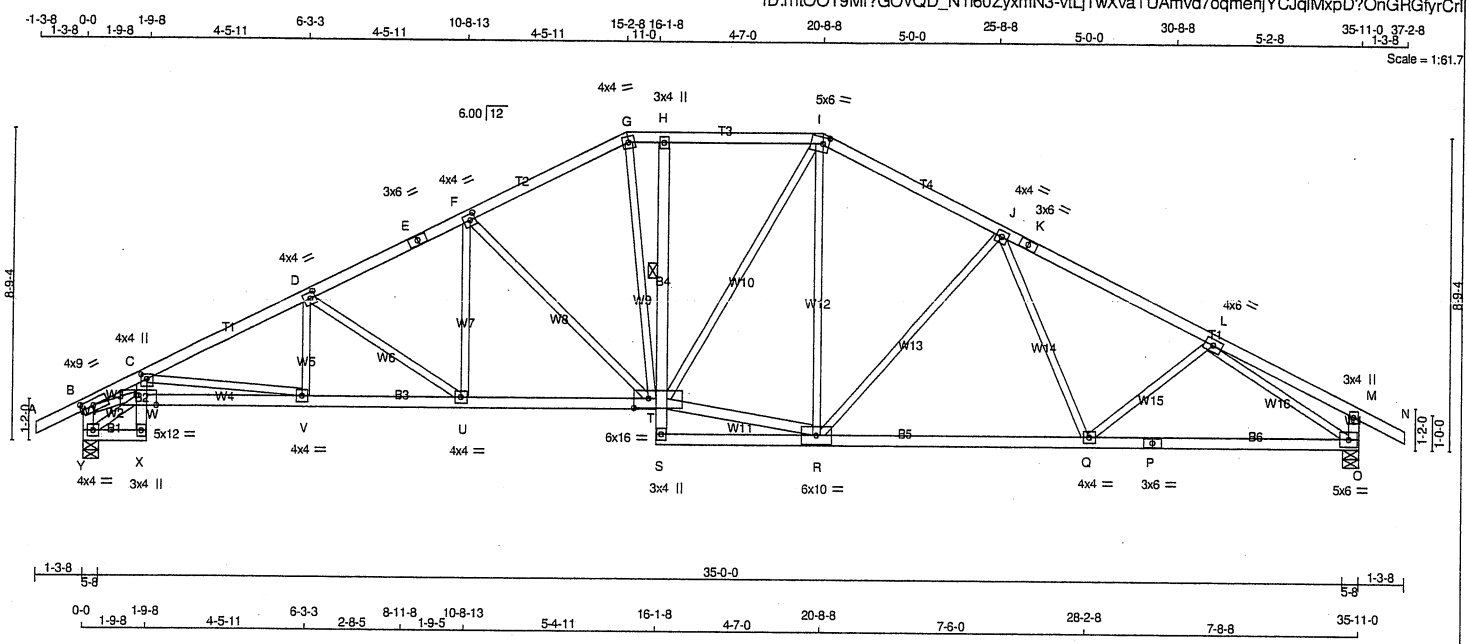

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
Y - X	2x4	DRY	No.2	SPF
X - C	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
S - H	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TTW-m	MT20	4.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW-m	MT20	5.0	6.0	2.25	2.00
J	TMWW-t	MT20	4.0	4.0		
K	TS-t	MT20	3.0	6.0		
L	TMWW-t	MT20	4.0	6.0		
M	TMV+p	MT20	3.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q, U, V						
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	6.0	10.0		
S	BMV+p	MT20	3.0	4.0		
T	BVMWWWW-t	MT20	6.0	16.0	3.25	5.00
W	BVMWW-l	MT20	5.0	12.0	Edge	6.75
X	BMV+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
08/05/21
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100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126501

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1313	0	1313	0	0	5-8	5-8
Y	1316	0	1316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	930	599 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0
Y	932	601 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, Y

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-2980 / 0	-56.6	-56.6	0.21 (1)	3.88
C- D	-2407 / 0	-56.6	-56.6	0.30 (1)	4.18
D- E	-2015 / 0	-56.6	-56.6	0.20 (1)	4.61
E- F	-2015 / 0	-56.6	-56.6	0.20 (1)	4.61
F- G	-1566 / 0	-56.6	-56.6	0.17 (1)	5.13
G- H	-1478 / 0	-56.6	-56.6	0.10 (1)	5.31
H- I	-1476 / 0	-56.6	-56.6	0.16 (1)	5.24
I- J	-1450 / 0	-56.6	-56.6	0.22 (1)	5.21
J- K	-1766 / 0	-56.6	-56.6	0.26 (1)	4.82
K- L	-1766 / 0	-56.6	-56.6	0.26 (1)	4.82
L- M	0 / 13	-56.6	-56.6	0.17 (1)	10.00
M- N	0 / 17	-56.6	-56.6	0.07 (1)	10.00
Y- B	-1144 / 0	0.0	0.0	0.12 (1)	7.43
O- M	-164 / 0	0.0	0.0	0.02 (1)	7.81
Y- X	0 / 240	-12.3	-12.3	0.04 (1)	10.00
X- W	0 / 11	0.0	0.0	0.36 (1)	10.00
W- C	0 / 336	0.0	0.0	0.41 (1)	10.00
W- V	0 / 3085	-12.3	-12.3	0.54 (1)	10.00
V- U	0 / 2174	-12.3	-12.3	0.39 (1)	10.00
U- T	0 / 1799	-12.3	-12.3	0.34 (1)	10.00
S- T	0 / 19	0.0	0.0	0.03 (1)	10.00
T- H	-238 / 0	0.0	0.0	0.03 (1)	6.25
S- R	0 / 19	-12.3	-12.3	0.10 (4)	10.00
R- Q	0 / 1545	-12.3	-12.3	0.33 (1)	10.00
Q- P	0 / 1569	-12.3	-12.3	0.34 (1)	10.00
P- O	0 / 1569	-12.3	-12.3	0.34 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- R	0 / 1295	0.21 (1)
T- I	0 / 378	0.09 (1)
R- I	0 / 97	0.03 (4)
R- J	-400 / 0	0.52 (1)
J- Q	0 / 100	0.03 (4)
Q- L	0 / 54	0.02 (4)
L- O	-1922 / 0	0.80 (1)
G- T	0 / 636	0.14 (1)
Y- W	-290 / 0	0.04 (1)
B- W	0 / 2654	0.60 (1)
C- V	-920 / 0	0.33 (1)
V- D	0 / 119	0.03 (4)
D- U	-451 / 0	0.22 (1)
U- F	0 / 321	0.07 (1)
F- T	-560 / 0	0.61 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.54/1.00 (V-W:1) , WB=0.80/1.00 (L-O:1) , SSI=0.36/1.00 (C-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (V) (INPUT = 0.90)
JSI METAL= 0.52 (W) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420377	T35S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 14:47:07 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Y	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126501

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420377	T36S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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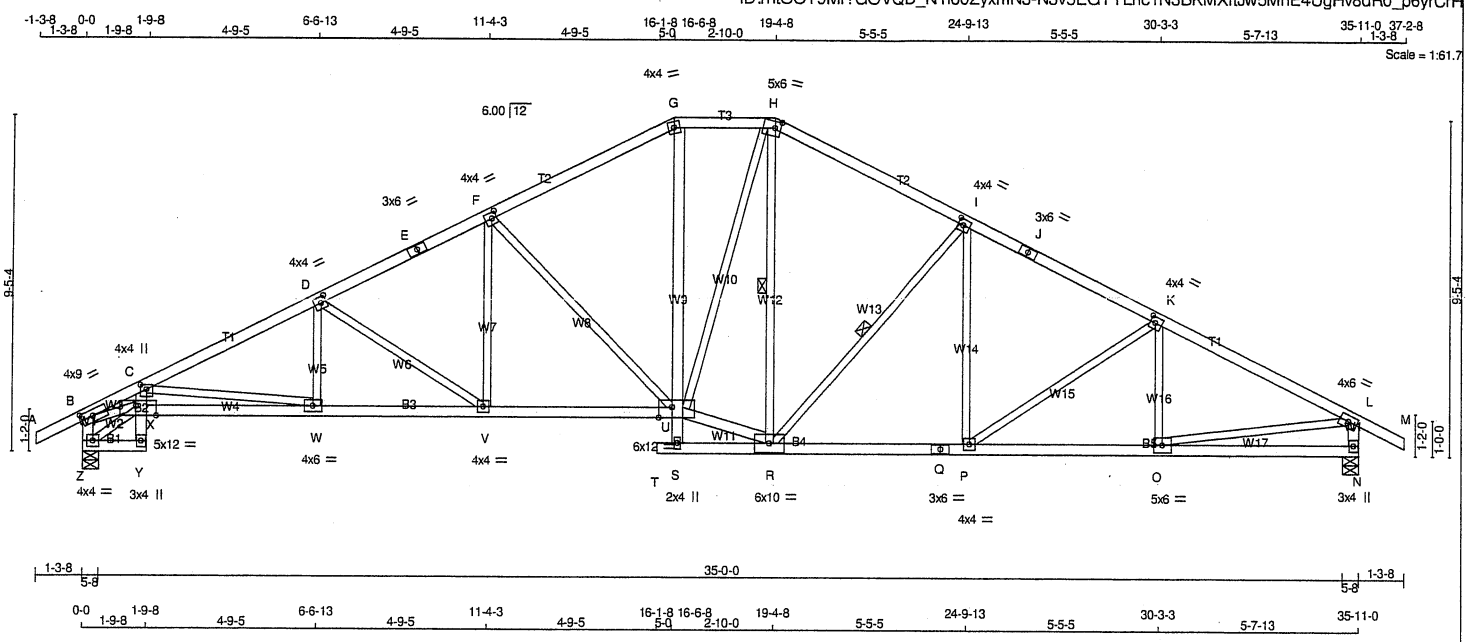

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Z - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Z - Y	2x4	DRY	No.2	SPF
Y - C	2x4	DRY	No.2	SPF
X - U	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - G	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	4.00
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D, F, I, K						
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.25	2.00
J	TS-t	MT20	3.0	6.0		
L	TMVW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWWW-t	MT20	6.0	10.0		
S	BMW+w	MT20	2.0	4.0		
U	BWMWWW*-l	MT20	6.0	12.0	Edge	4.50
V	BMWW-t	MT20	4.0	4.0		
W	BMWW-t	MT20	4.0	6.0		
X	BVMWW-l	MT20	5.0	12.0	Edge	6.75
Y	BMV+p	MT20	3.0	4.0		
Z	BMVW1-t	MT20	4.0	4.0		

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126502

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1316	0	1316	0	0	5-8	5-8
Z	1320	0	1320	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	933	599 / 0	0 / 0	0 / 0	0 / 0	334 / 0	0 / 0
Z	936	601 / 0	0 / 0	0 / 0	0 / 0	335 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, Z

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-R, I-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-56.6	-56.6	0.07 (1)	10.00
B- C	-3006 / 0	-56.6	-56.6	0.20 (1)	3.88
C- D	-2391 / 0	-56.6	-56.6	0.32 (1)	4.17
D- E	-1976 / 0	-56.6	-56.6	0.21 (1)	4.63
E- F	-1976 / 0	-56.6	-56.6	0.21 (1)	4.63
F- G	-1524 / 0	-56.6	-56.6	0.21 (1)	5.13
G- H	-1360 / 0	-56.6	-56.6	0.08 (1)	5.51
H- I	-1372 / 0	-56.6	-56.6	0.22 (1)	5.33
I- J	-1692 / 0	-56.6	-56.6	0.22 (1)	4.92
J- K	-1692 / 0	-56.6	-56.6	0.22 (1)	4.92
K- L	-1835 / 0	-56.6	-56.6	0.27 (1)	4.72
L- M	0 / 17	-56.6	-56.6	0.07 (1)	10.00
Z- B	-1147 / 0	0.0	0.0	0.12 (1)	7.43
N- L	-1285 / 0	0.0	0.0	0.13 (1)	7.11
Z- Y	0 / 243	-12.3	-12.3	0.04 (1)	10.00
Y- X	0 / 11	0.0	0.0	0.37 (1)	10.00
X- C	0 / 335	0.0	0.0	0.42 (1)	10.00
X- W	0 / 3116	-12.3	-12.3	0.54 (1)	10.00
W- V	0 / 2159	-12.3	-12.3	0.38 (1)	10.00
V- U	0 / 1767	-12.3	-12.3	0.33 (1)	10.00
T- S	0 / 0	-12.3	-12.3	0.00 (4)	10.00
S- R	0 / 15	-12.3	-12.3	0.06 (4)	10.00
R- Q	0 / 1514	-12.3	-12.3	0.29 (1)	10.00
Q- P	0 / 1514	-12.3	-12.3	0.29 (1)	10.00
P- O	0 / 1652	-12.3	-12.3	0.30 (1)	10.00
O- N	0 / 0	-12.3	-12.3	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- W	-966 / 0	0.40 (1)
W- D	0 / 127	0.03 (4)
D- V	-469 / 0	0.26 (1)
V- F	0 / 328	0.07 (1)
R- H	-42 / 22	0.02 (1)
R- I	-461 / 0	0.23 (1)
P- I	0 / 160	0.04 (4)
P- K	-168 / 0	0.13 (1)
O- K	-178 / 0	0.04 (1)
O- L	0 / 1671	0.38 (1)
S- U	0 / 15	0.02 (1)
U- G	0 / 477	0.10 (1)
F- U	-596 / 0	0.70 (1)
Z- X	-292 / 0	0.04 (1)
B- X	0 / 2681	0.60 (1)
U- R	0 / 1282	0.21 (1)
U- H	0 / 470	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 16.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.32/1.00 (C-D:1) , BC=0.54/1.00 (W-X:1) , WB=0.70/1.00 (F-U:1) , SSI=0.36/1.00 (C-X:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (L) (INPUT = 0.90)
JSI METAL= 0.52 (X) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T38	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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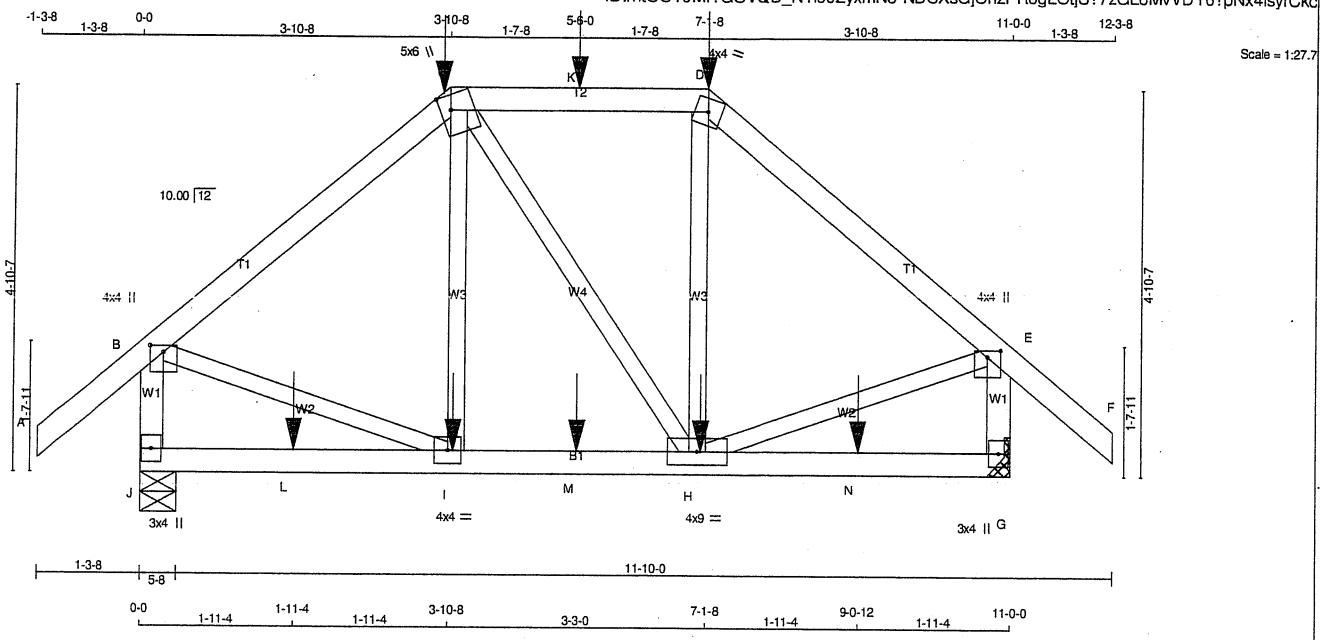

TOTAL WEIGHT = 52 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1040	0	1040	0	0	5-8	5-8
G	1040	0	1040	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	735	488 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0
G	735	488 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.13 (1)	10.00
B- C	-805 / 0	-84.9	-84.9	0.26 (1)	6.25
C- K	-614 / 0	-84.9	-84.9	0.27 (1)	6.25
K- D	-614 / 0	-84.9	-84.9	0.27 (1)	6.25
D- E	-805 / 0	-84.9	-84.9	0.26 (1)	6.25
E- F	0 / 38	-84.9	-84.9	0.13 (1)	10.00
J- B	-1002 / 0	0.0	0.0	0.12 (1)	7.78
G- E	-1001 / 0	0.0	0.0	0.12 (1)	7.78
J- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00
L- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00
I- M	0 / 615	-18.5	-18.5	0.15 (1)	10.00
M- H	0 / 615	-18.5	-18.5	0.15 (1)	10.00
H- N	0 / 0	-18.5	-18.5	0.10 (4)	10.00
N- G	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-95 / 50	0.03 (1)
C- H	0 / 0	0.00 (1)
H- D	-96 / 51	0.03 (1)
B- I	0 / 649	0.16 (1)
H- E	0 / 649	0.16 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-188	-188	---	BACK	VERT	TOTAL	---	C1
D	7-1-8	-188	-188	---	BACK	VERT	TOTAL	---	C1
H	7-0-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
I	3-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
K	5-6-0	-55	-55	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
M	5-6-0	-14	-14	---	BACK	VERT	TOTAL	---	C1
N	9-0-12	-14	-14	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.15/1.00 (H-I:1) , WB=0.16/1.00 (B-I:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126508

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T39	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Aug 4 14:54:17 2021 Page 1
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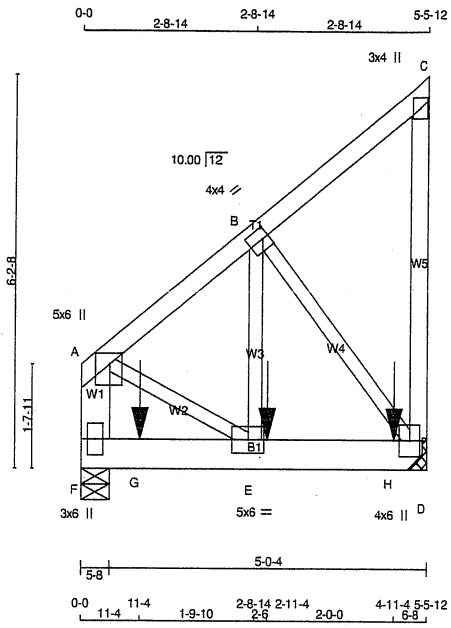

TOTAL WEIGHT = 2 X 34 = 69 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	SIDE(53.2)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(83.5)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126509

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1060	0	1060	0	0	IN-SX	IN-SX
D	1441	0	1441	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	746	512 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
D	1016	683 / 0	0 / 0	0 / 0	0 / 0	333 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-886 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	-793 / 0	-84.9	-84.9	0.05 (1)	6.25	
B- C	-16 / 0	-84.9	-84.9	0.05 (1)	6.25	
D- C	-91 / 0	0.0	0.0	0.03 (1)	7.81	
F- G	0 / 0	-18.5	-18.5	0.06 (1)	10.00	
G- E	0 / 0	-18.5	-18.5	0.06 (1)	10.00	
E- H	0 / 621	-18.5	-18.5	0.15 (1)	10.00	
H- D	0 / 621	-18.5	-18.5	0.15 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 680	0.08 (1)
E- B	0 / 1030	0.13 (1)
B- D	-1009 / 0	0.16 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-11-4	-720	-720	---	FRONT	VERT	TOTAL	---	C1
G	11-4	-166	-166	---	TOP	VERT	TOTAL	---	C1
H	4-11-4	-473	-473	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.05/1.00 (A-B:1) , BC=0.15/1.00 (D-E:1) , WB=0.16/1.00 (B-D:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420378	T39	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126509

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T39Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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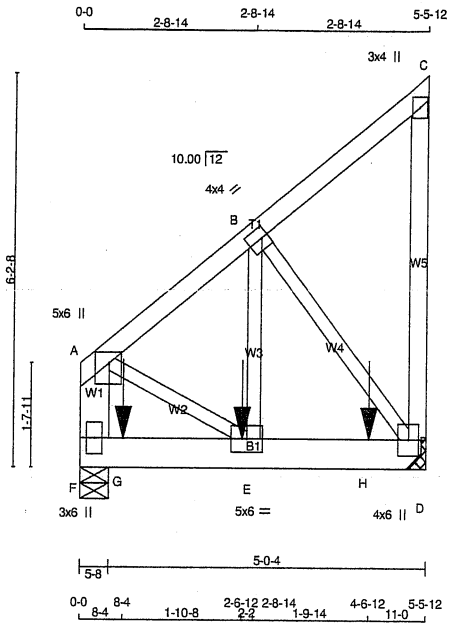

TOTAL WEIGHT = 2 X 34 = 69 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	3	2	SIDE(242.8)
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(75.3)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		

Structural component only
DWG# T-2126510

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	2372	0	2372	0	0	5-8	5-8
D	1809	0	1809	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1675	1113 / 0	0 / 0	0 / 0	0 / 0	562 / 0	0 / 0
D	1278	846 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-1231 / 0	0.0	0.0	0.05 (1)	7.81	
A- B	-1143 / 0	-84.9	-84.9	0.05 (1)	6.25	
B- C	-15 / 0	-84.9	-84.9	0.05 (1)	6.25	
D- C	-93 / 0	0.0	0.0	0.03 (1)	7.81	
F- G	0 / 0	-18.5	-18.5	0.29 (1)	10.00	
G- E	0 / 0	-18.5	-18.5	0.29 (1)	10.00	
E- H	0 / 890	-18.5	-18.5	0.25 (1)	10.00	
H- D	0 / 890	-18.5	-18.5	0.25 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
A- E	0 / 974	0.12 (1)
E- B	0 / 1601	0.20 (1)
B- D	-1445 / 0	0.23 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-6-12	-699	-699	---	BACK	VERT	TOTAL	---	C1
G	8-4	-1152	-1152	---	BACK	VERT	TOTAL	---	C1
H	4-6-12	-700	-700	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.05/1.00 (A-B:1) , BC=0.29/1.00 (E-F:1) , WB=0.23/1.00 (B-D:1) , SSI=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420378	T39Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126510

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T40	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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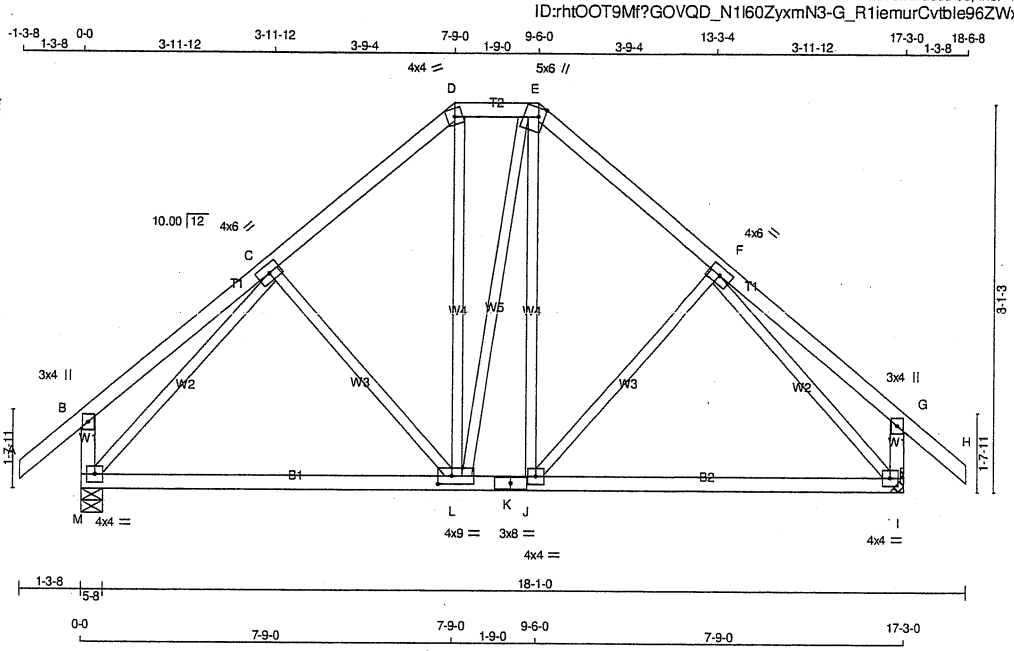

TOTAL WEIGHT = 89 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	9.0	2.00	3.50
M	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1009	0	1009	0	0	5-8	5-8
I	1009	0	1009	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	714	466 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
I	714	466 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)		10.00
B- C	0 / 25	-84.9	-84.9	0.21 (1)		10.00
C- D	-677 / 0	-84.9	-84.9	0.16 (1)		6.25
D- E	-503 / 0	-84.9	-84.9	0.04 (1)		6.25
E- F	-676 / 0	-84.9	-84.9	0.16 (1)		6.25
F- G	0 / 25	-84.9	-84.9	0.21 (1)		10.00
G- H	0 / 38	-84.9	-84.9	0.12 (1)		10.00
M- B	-242 / 0	0.0	0.0	0.03 (1)		7.81
I- G	-242 / 0	0.0	0.0	0.03 (1)		7.81
M- L	0 / 604	-18.5	-18.5	0.31 (4)		10.00
L- K	0 / 502	-18.5	-18.5	0.31 (4)		10.00
K- J	0 / 502	-18.5	-18.5	0.31 (4)		10.00
J- I	0 / 603	-18.5	-18.5	0.30 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
C- L	-156 / 0	0.10 (1)
L- D	0 / 218	0.05 (1)
L- E	0 / 5	0.00 (1)
J- E	0 / 212	0.05 (1)
J- F	-157 / 0	0.10 (1)
M- C	-931 / 0	0.55 (1)
F- I	-929 / 0	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.21/1.00 (F-G:1) , BC=0.31/1.00 (L-M:4) , WB=0.55/1.00 (C-M:1) , SSI=0.12/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (M) (INPUT = 0.90)
JSI METAL= 0.56 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126511

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T41	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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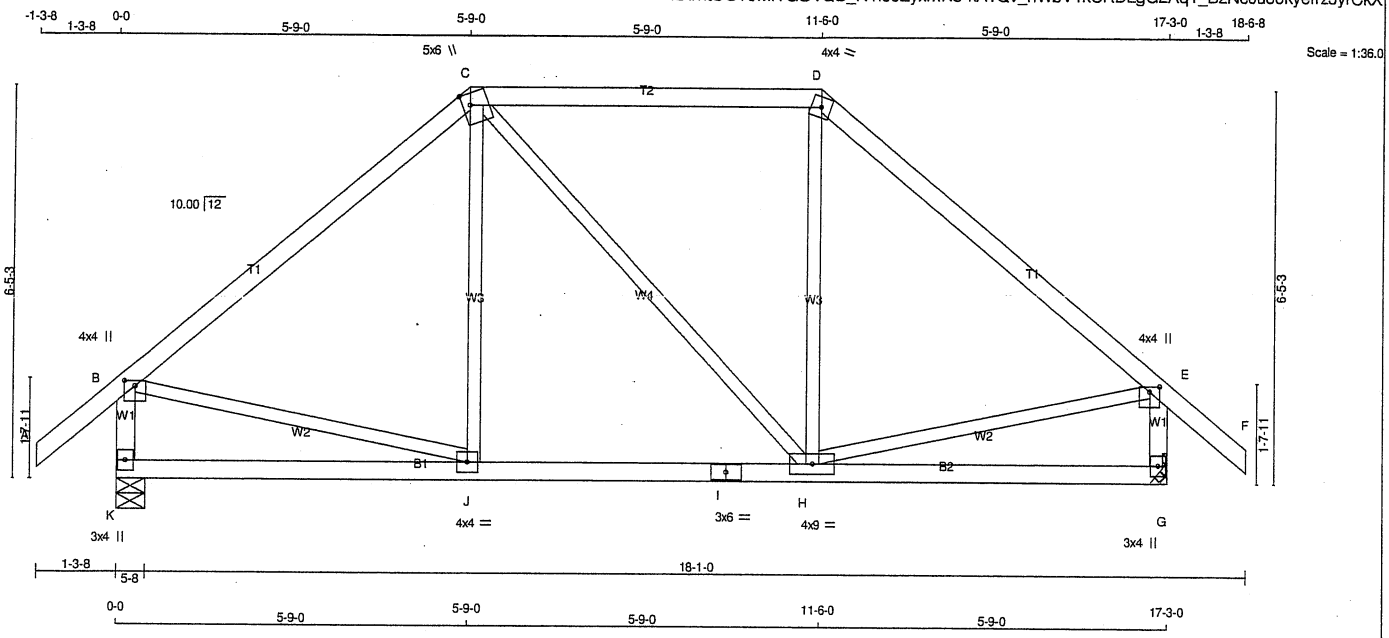

TOTAL WEIGHT = 76 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1009	0	1009	0	0	5-8	5-8
G	1009	0	1009	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	714	466 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
G	714	466 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-742 / 0	-84.9	-84.9	0.37 (1)	6.25
C- D	-569 / 0	-84.9	-84.9	0.36 (1)	6.25
D- E	-743 / 0	-84.9	-84.9	0.37 (1)	6.25
E- F	0 / 38	-84.9	-84.9	0.12 (1)	10.00
K- B	-964 / 0	0.0	0.0	0.10 (1)	7.81
G- E	-964 / 0	0.0	0.0	0.10 (1)	7.81
K- J	0 / 0	-18.5	-18.5	0.14 (4)	10.00
J- I	0 / 568	-18.5	-18.5	0.19 (4)	10.00
I- H	0 / 568	-18.5	-18.5	0.19 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-13 / 81	0.03 (4)
C- H	0 / 2	0.00 (4)
H- D	-11 / 82	0.03 (4)
B- J	0 / 583	0.13 (1)
H- E	0 / 583	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.37/1.00 (D-E:1) , BC=0.19/1.00 (H-J:4) , WB=0.13/1.00 (E-H:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.25 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126512

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T42	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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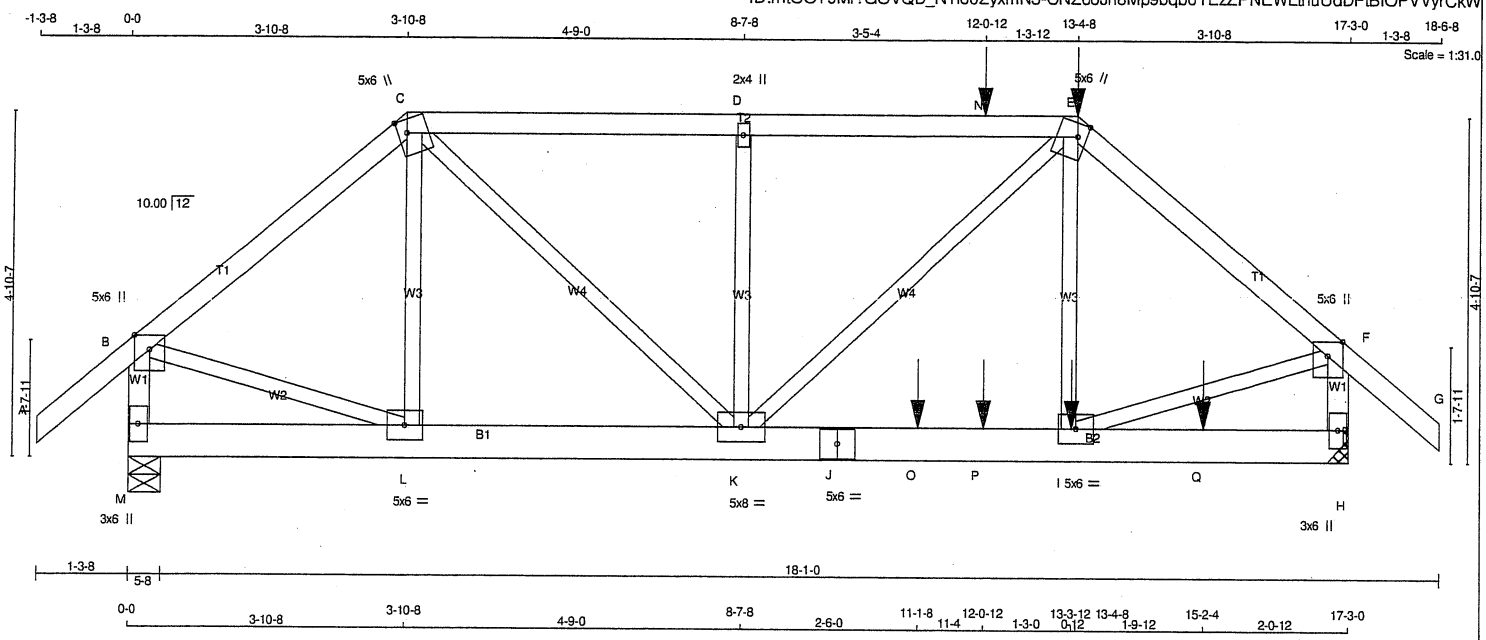

TOTAL WEIGHT = 86 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1288	0	1288	0	0	5-8	5-8
H	1648	0	1648	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	910	601 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
H	1164	771 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.13 (1)	10.00	
B- C	-1103 / 0	-84.9	-84.9	0.27 (1)	5.66	
C- D	-1481 / 0	-84.9	-84.9	0.43 (1)	4.82	
D- N	-1481 / 0	-84.9	-84.9	0.43 (1)	4.82	
N- E	-1481 / 0	-84.9	-84.9	0.43 (1)	4.82	
E- F	-1568 / 0	-84.9	-84.9	0.30 (1)	4.91	
F- G	0 / 38	-84.9	-84.9	0.13 (1)	10.00	
M- B	-1250 / 0	0.0	0.0	0.14 (1)	7.16	
H- F	-1658 / 0	0.0	0.0	0.19 (1)	6.39	
M- L	0 / 0	-18.5	-18.5	0.03 (4)	10.00	
L- K	0 / 840	-18.5	-18.5	0.23 (1)	10.00	
K- J	0 / 1203	-18.5	-18.5	0.50 (1)	10.00	
J- O	0 / 1203	-18.5	-18.5	0.50 (1)	10.00	
O- P	0 / 1203	-18.5	-18.5	0.50 (1)	10.00	
P- I	0 / 1203	-18.5	-18.5	0.50 (1)	10.00	
I- Q	0 / 0	-18.5	-18.5	0.16 (1)	10.00	
Q- H	0 / 0	-18.5	-18.5	0.16 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
L- C	-216 / 0	0.07 (1)
C- K	0 / 881	0.22 (1)
K- D	-515 / 0	0.17 (1)
K- E	0 / 382	0.09 (1)
I- E	0 / 122	0.05 (4)
B- L	0 / 884	0.22 (1)
I- F	0 / 1256	0.31 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	13-4-8	-188	-188	---	FRONT	VERT	TOTAL	---	C1
I	13-3-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
N	12-0-12	-55	-55	---	FRONT	VERT	TOTAL	---	C1
O	11-1-8	-362	-362	---	FRONT	VERT	TOTAL	---	C1
P	12-0-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
Q	15-2-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.50/1.00 (I-K:1) , WB=0.31/1.00 (F-I:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.58 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126513

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T43	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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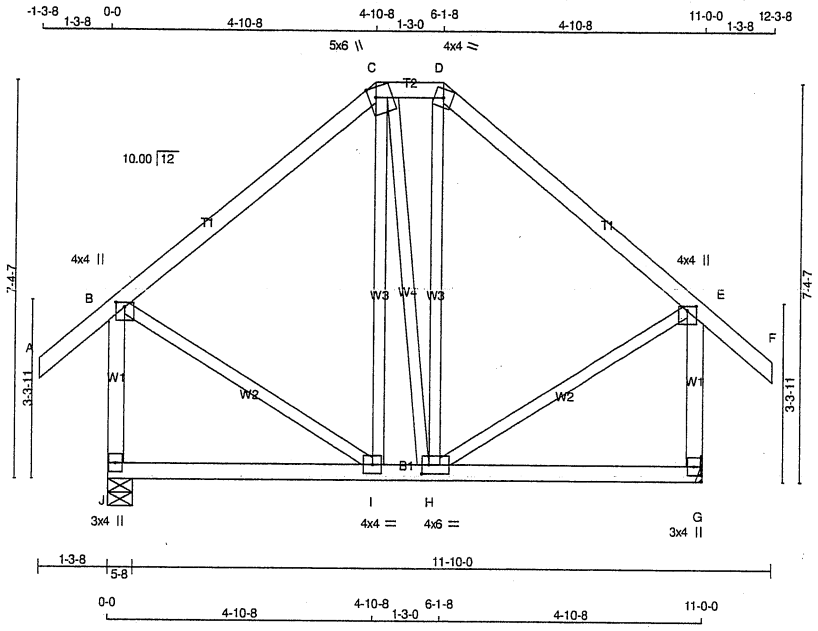

TOTAL WEIGHT = 65 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	686	0	686	0	0	IN-SX	IN-SX
G	686	0	686	0	0	5-8	5-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	485	321 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
G	485	321 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-295 / 0	-84.9	-84.9	0.26 (1)	6.25
C- D	-224 / 0	-84.9	-84.9	0.02 (1)	6.25
D- E	-294 / 0	-84.9	-84.9	0.26 (1)	6.25
E- F	0 / 38	-84.9	-84.9	0.12 (1)	10.00
J- B	-649 / 0	0.0	0.0	0.12 (1)	7.81
G- E	-648 / 0	0.0	0.0	0.12 (1)	7.81
J- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00
I- H	0 / 225	-18.5	-18.5	0.12 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-66 / 19	0.06 (1)
C- H	-4 / 1	0.00 (1)
H- D	-71 / 21	0.07 (1)
B- I	0 / 263	0.06 (1)
H- E	0 / 262	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.26/1.00 (B-C:1) , BC=0.12/1.00 (H-I:4) , WB=0.07/1.00 (D-H:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

Structural component only
DWG# T-2126514

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T44	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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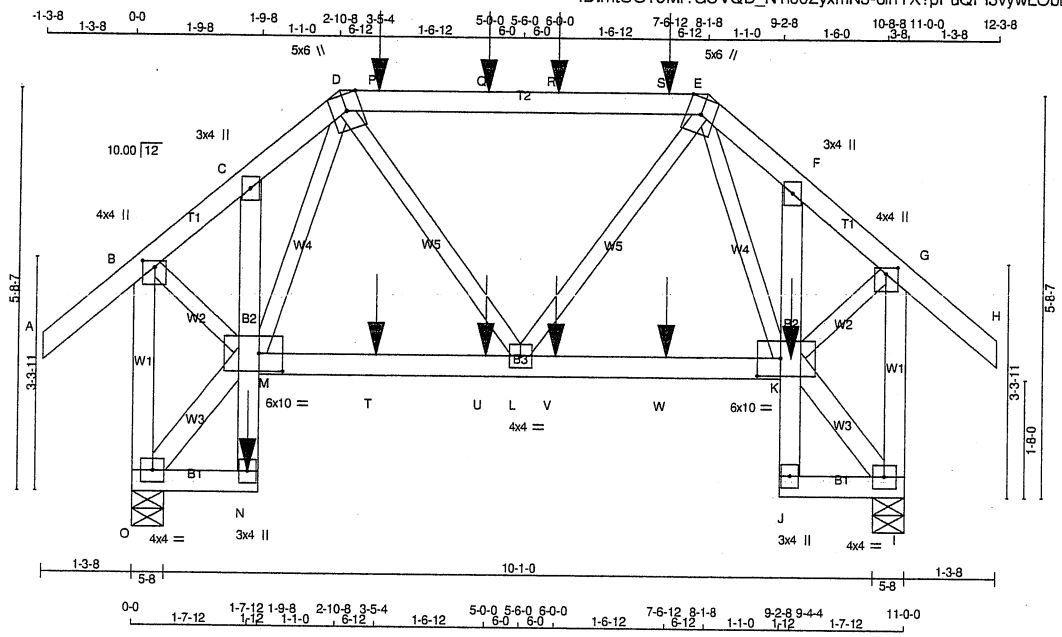

TOTAL WEIGHT = 68 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - M	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	5.0	6.0	Edge	
E	TTWW+m	MT20	5.0	6.0	Edge	
F	TMV+p	MT20	3.0	4.0		
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
I	BMVW1-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMWWW-I	MT20	6.0	10.0	3.00	4.00
L	BMWW-t	MT20	4.0	4.0		
M	BVMWWW-I	MT20	6.0	10.0	3.00	4.00
N	BMV+p	MT20	3.0	4.0		
O	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126515

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	925	0	925	0	0	5-8	5-8
I	925	0	925	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	653	434 / 0	0 / 0	0 / 0	0 / 0	219 / 0	0 / 0
I	653	434 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.13 (1)	10.00
B- C	-565 / 0	-84.9	-84.9	0.13 (1)	6.25
C- D	-530 / 0	-84.9	-84.9	0.04 (1)	6.25
D- P	-480 / 0	-84.9	-84.9	0.60 (1)	6.25
P- Q	-480 / 0	-84.9	-84.9	0.60 (1)	6.25
Q- R	-480 / 0	-84.9	-84.9	0.60 (1)	6.25
R- S	-480 / 0	-84.9	-84.9	0.60 (1)	6.25
S- E	-480 / 0	-84.9	-84.9	0.60 (1)	6.25
E- F	-530 / 0	-84.9	-84.9	0.04 (1)	6.25
F- G	-565 / 0	-84.9	-84.9	0.13 (1)	6.25
G- H	0 / 38	-84.9	-84.9	0.13 (1)	10.00
O- B	-901 / 0	0.0	0.0	0.18 (1)	7.81
I- G	-901 / 0	0.0	0.0	0.18 (1)	7.81
O- N	0 / 8	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 143	0.0	0.0	0.05 (1)	10.00
M- C	-65 / 0	0.0	0.0	0.02 (1)	7.81
M- T	0 / 441	-18.5	-18.5	0.15 (4)	10.00
T- U	0 / 441	-18.5	-18.5	0.15 (4)	10.00
U- L	0 / 441	-18.5	-18.5	0.15 (4)	10.00
L- V	0 / 441	-18.5	-18.5	0.15 (4)	10.00
V- W	0 / 441	-18.5	-18.5	0.15 (4)	10.00
W- K	0 / 441	-18.5	-18.5	0.15 (4)	10.00
J- K	0 / 17	0.0	0.0	0.02 (1)	10.00
K- F	-65 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 8	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- M	-12 / 0	0.00 (1)
K- I	-12 / 0	0.00 (1)
B- M	0 / 519	0.13 (1)
K- G	0 / 519	0.13 (1)
D- L	0 / 78	0.03 (4)
M- D	-97 / 0	0.03 (1)
L- E	0 / 78	0.03 (4)
E- K	-97 / 0	0.03 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	9-4-4	-90	-90	---	FRONT	VERT	TOTAL	---	C1
N	1-7-12	-90	-90	---	FRONT	VERT	TOTAL	---	C1
P	3-5-4	-40	-40	---	FRONT	VERT	TOTAL	---	C1
Q	5-0-0	-26	-26	---	FRONT	VERT	TOTAL	---	C1
R	6-0-0	-26	-26	---	FRONT	VERT	TOTAL	---	C1
S	7-6-12	-40	-40	---	FRONT	VERT	TOTAL	---	C1
T	3-5-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
U	5-0-0	-6	-6	---	FRONT	VERT	TOTAL	---	C1
V	6-0-0	-6	-6	---	FRONT	VERT	TOTAL	---	C1
W	7-6-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.60/1.00 (D-E:1) , BC=0.15/1.00 (K-L:4) , WB=0.13/1.00 (G-K:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (B) (INPUT = 0.90)
JSI METAL= 0.18 (G) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T45	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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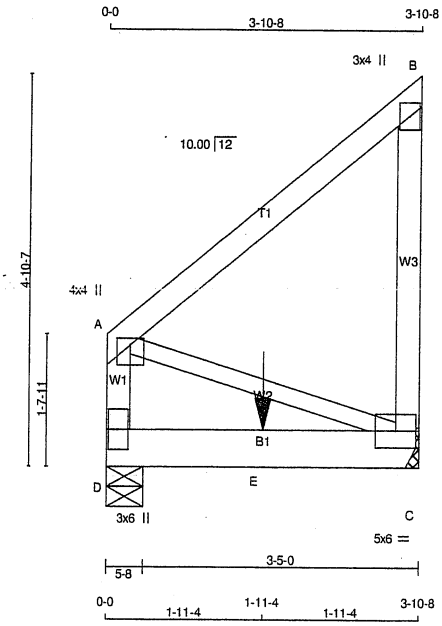

TOTAL WEIGHT = 2 X 22 = 43 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2126516

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	534	0	534	0	0	5-8	5-8
C	534	0	534	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	377	251 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0
C	377	251 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
D- A	-164 / 0	0.0	0.0	0.01 (1)	7.81
A- B	0 / 0	-84.9	-84.9	0.12 (1)	10.00
C- B	-164 / 0	0.0	0.0	0.03 (1)	7.81
D- E	0 / 0	-18.5	-18.5	0.25 (1)	10.00
E- C	0 / 0	-18.5	-18.5	0.25 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- C	0 / 0	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-11-4	-470	-470	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.25/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (A) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T46	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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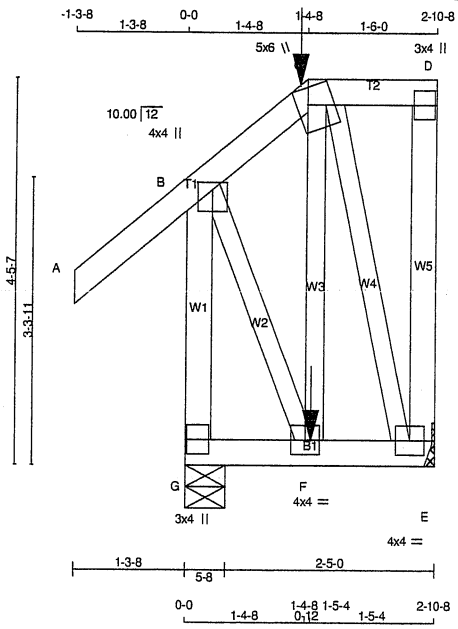

TOTAL WEIGHT = 2 X 26 = 52 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	138	0	138	0	0	MECHANICAL	
G	314	0	314	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	99	61 / -16	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
G	220	157 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (9)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.16 (5)	10.00	
B- C	-68 / 0	-84.9	-84.9	0.15 (5)	6.25	
C- D	0 / 0	-84.9	-84.9	0.04 (1)	10.00	
E- D	-64 / 0	0.0	0.0	0.02 (1)	7.81	
G- B	-299 / 0	0.0	0.0	0.06 (1)	7.81	
G- F	0 / 0	-18.5	-18.5	0.01 (4)	10.00	
F- E	-3 / 22	-18.5	-18.5	0.01 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-37 / 28	0.01 (6)
C- E	-64 / 8	0.02 (1)
B- F	-8 / 50	0.01 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-4-8	-25	-25	---	BACK	VERT	TOTAL	---	C1
F	1-5-4	-2	-2	30	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.16/1.00 (A-B:5) , BC=0.01/1.00 (E-F:1) , WB=0.02/1.00 (C-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126517

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T47	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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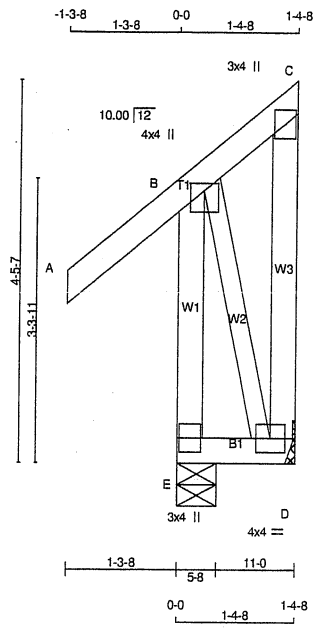

TOTAL WEIGHT = 2 X 16 = 32 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	12	0	14	0	-43	MECHANICAL	
E	248	0	248	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	10	0 / -32	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
E	172	129 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-38 / 0	-84.9	-84.9	0.11 (1)	6.25
D- C	0 / 52	0.0	0.0	0.01 (5)	10.00
E- B	-235 / 0	0.0	0.0	0.04 (1)	7.81
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- D	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126518

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	T48	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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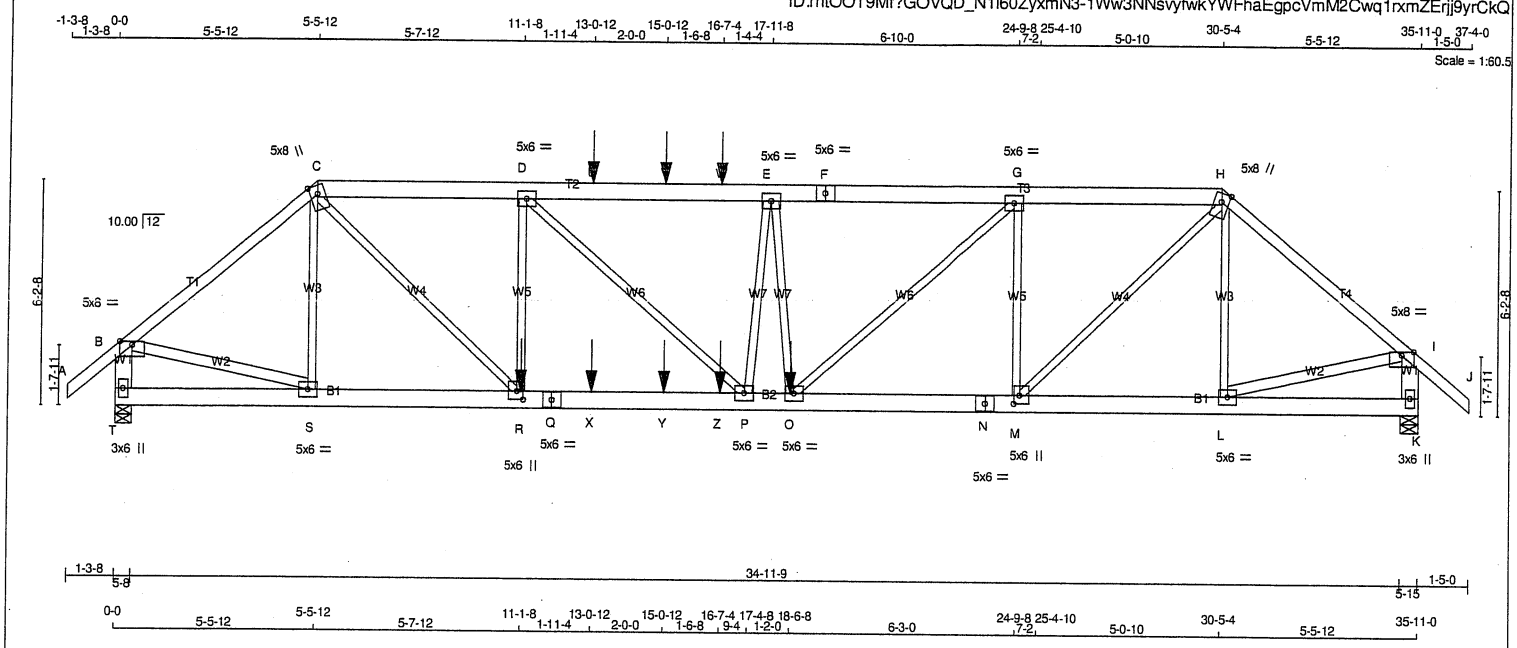

TOTAL WEIGHT = 2 X 197 = 394 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
B - S	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
H- J	1	12	TOP
C- F	2	12	SIDE(0.0)
F- H	2	12	TOP
T- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- Q	2	12	SIDE(183.1)
Q- N	2	12	SIDE(183.1)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
R- D	1	6	SIDE(188.0)
E- O	1	6	SIDE(284.0)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126519

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	4136	0	4136	0	0	5-8	5-8
K	3574	0	3574	0	0	5-15	5-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2924	1923 / 0	0 / 0	0 / 0	0 / 0	1001 / 0	0 / 0
K	2528	1656 / 0	0 / 0	0 / 0	0 / 0	872 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.06 (1)	10.00
B- C	-4611 / 0	-84.9	-84.9	0.45 (1)	4.14
C- D	-6731 / 0	-84.9	-84.9	0.24 (1)	4.43
D- U	-7691 / 0	-84.9	-84.9	0.32 (1)	4.11
U- V	-7691 / 0	-84.9	-84.9	0.32 (1)	4.11
V- W	-7691 / 0	-84.9	-84.9	0.32 (1)	4.11
W- E	-7691 / 0	-84.9	-84.9	0.32 (1)	4.11
E- F	-7705 / 0	-84.9	-84.9	0.29 (1)	4.15
F- G	-7705 / 0	-84.9	-84.9	0.29 (1)	4.15
G- H	-5606 / 0	-84.9	-84.9	0.15 (1)	4.87
H- I	-3902 / 0	-84.9	-84.9	0.39 (1)	4.47
I- J	0 / 41	-84.9	-84.9	0.08 (1)	10.00
T- B	-4101 / 0	0.0	0.0	0.15 (1)	7.03
K- I	-3534 / 0	0.0	0.0	0.13 (1)	7.45
T- S	0 / 0	-18.5	-18.5	0.04 (4)	10.00
S- R	0 / 3561	-18.5	-18.5	0.26 (1)	10.00
R- Q	0 / 6731	-18.5	-18.5	0.49 (1)	10.00
Q- X	0 / 6731	-18.5	-18.5	0.49 (1)	10.00
X- Y	0 / 6731	-18.5	-18.5	0.49 (1)	10.00
Y- Z	0 / 6731	-18.5	-18.5	0.49 (1)	10.00
Z- P	0 / 6731	-18.5	-18.5	0.49 (1)	10.00
P- O	0 / 7742	-18.5	-18.5	0.57 (1)	10.00
O- N	0 / 5606	-18.5	-18.5	0.43 (1)	10.00
N- M	0 / 5606	-18.5	-18.5	0.43 (1)	10.00
M- L	0 / 3013	-18.5	-18.5	0.22 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-671 / 0	0.19 (1)
L- H	-572 / 0	0.16 (1)
B- S	0 / 3639	0.32 (1)
L- I	0 / 3079	0.27 (1)
R- D	-1697 / 0	0.46 (1)
M- G	-2499 / 0	0.68 (1)
C- R	0 / 4532	0.56 (1)
M- H	0 / 3707	0.46 (1)
E- O	-304 / 0	0.08 (1)
P- E	-562 / 0	0.16 (1)
D- P	0 / 1306	0.16 (1)
O- G	0 / 2857	0.35 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	18-6-8	-1263	-1263	---	BACK	VERT	TOTAL	---	C1
R	11-1-8	-1001	-1001	---	BACK	VERT	TOTAL	---	C1
U	13-0-12	-102	-102	---	BACK	VERT	TOTAL	---	C1
V	15-0-12	-102	-102	---	BACK	VERT	TOTAL	---	C1
W	16-7-4	-102	-102	---	BACK	VERT	TOTAL	---	C1
X	13-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
Y	15-0-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
Z	16-7-4	-26	-26	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.57/1.00 (O-P:1) , WB=0.68/1.00 (G-M:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (L) (INPUT = 0.90)
JSI METAL= 0.61 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D, E, G						
D	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	8.0	Edge	
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L, O, P, S						
L	BMWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	5.0	6.0	2.75	2.00
N	BS-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	5.0	6.0	2.75	2.00
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126519

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T49	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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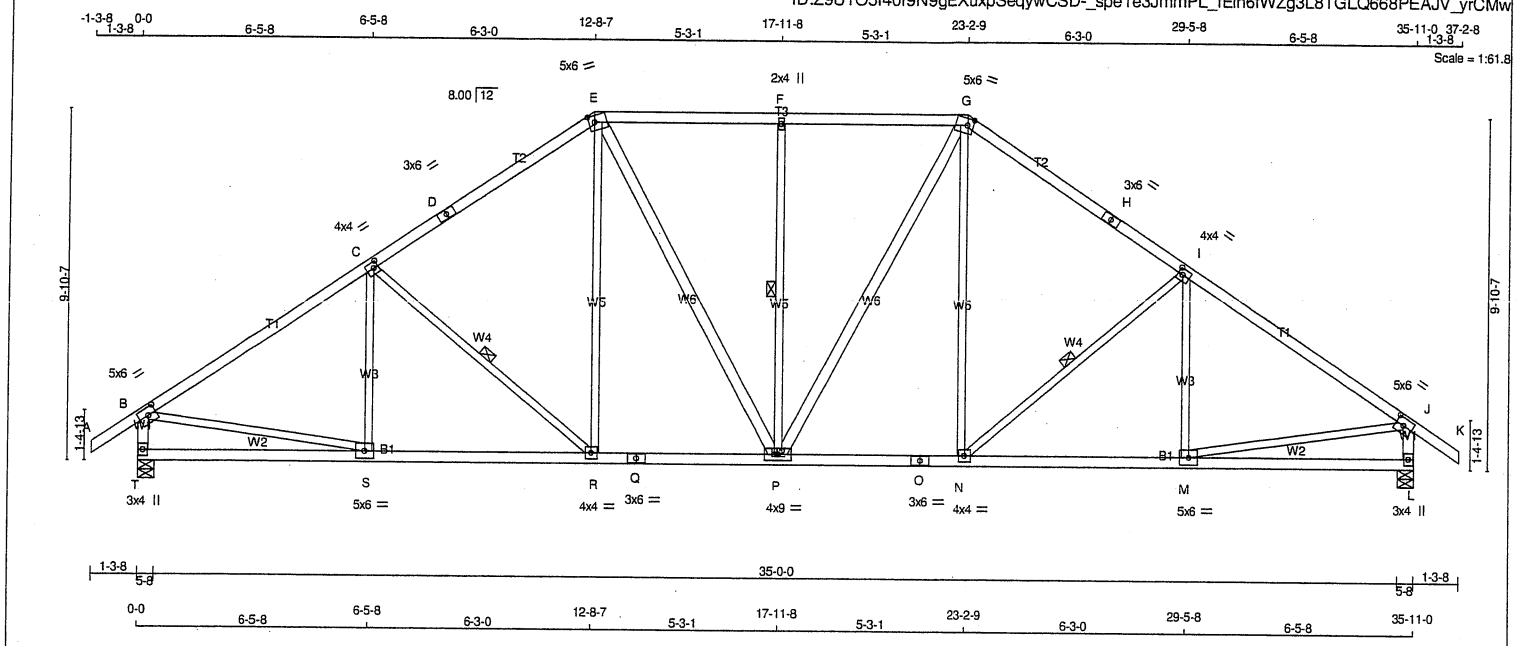

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1973	0	1973	0	0	5-8	5-8
L	1973	0	1973	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1398	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1398	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-2266 / 0	-84.9	-84.9	0.60 (1)	3.97
C- D	-1956 / 0	-84.9	-84.9	0.54 (1)	4.26
D- E	-1956 / 0	-84.9	-84.9	0.54 (1)	4.26
E- F	-1773 / 0	-84.9	-84.9	0.34 (1)	4.67
F- G	-1773 / 0	-84.9	-84.9	0.34 (1)	4.67
G- H	-1956 / 0	-84.9	-84.9	0.54 (1)	4.26
H- I	-1956 / 0	-84.9	-84.9	0.54 (1)	4.26
I- J	-2266 / 0	-84.9	-84.9	0.60 (1)	3.97
J- K	0 / 32	-84.9	-84.9	0.11 (1)	10.00
T- B	-1923 / 0	0.0	0.0	0.20 (1)	6.07
L- J	-1923 / 0	0.0	0.0	0.20 (1)	6.07
T- S	0 / 0	-18.5	-18.5	0.18 (4)	10.00
S- R	0 / 1915	-18.5	-18.5	0.40 (1)	10.00
R- Q	0 / 1599	-18.5	-18.5	0.32 (1)	10.00
Q- P	0 / 1599	-18.5	-18.5	0.32 (1)	10.00
P- O	0 / 1599	-18.5	-18.5	0.32 (1)	10.00
O- N	0 / 1599	-18.5	-18.5	0.32 (1)	10.00
N- M	0 / 1915	-18.5	-18.5	0.40 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.18 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-189 / 35	0.09 (1)
R- E	-420 / 0	0.20 (1)
E- P	0 / 378	0.09 (1)
P- F	0 / 361	0.06 (1)
P- G	-543 / 0	0.36 (1)
N- G	0 / 361	0.06 (1)
N- I	0 / 378	0.09 (1)
M- I	-420 / 0	0.20 (1)
B- S	-189 / 35	0.09 (1)
M- J	0 / 1942	0.44 (1)
	0 / 1942	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.60/1.00 (I-J:1) , BC=0.40/1.00 (M-N:1) , WB=0.44/1.00 (J-M:1) , SSI=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.56 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126524

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T50	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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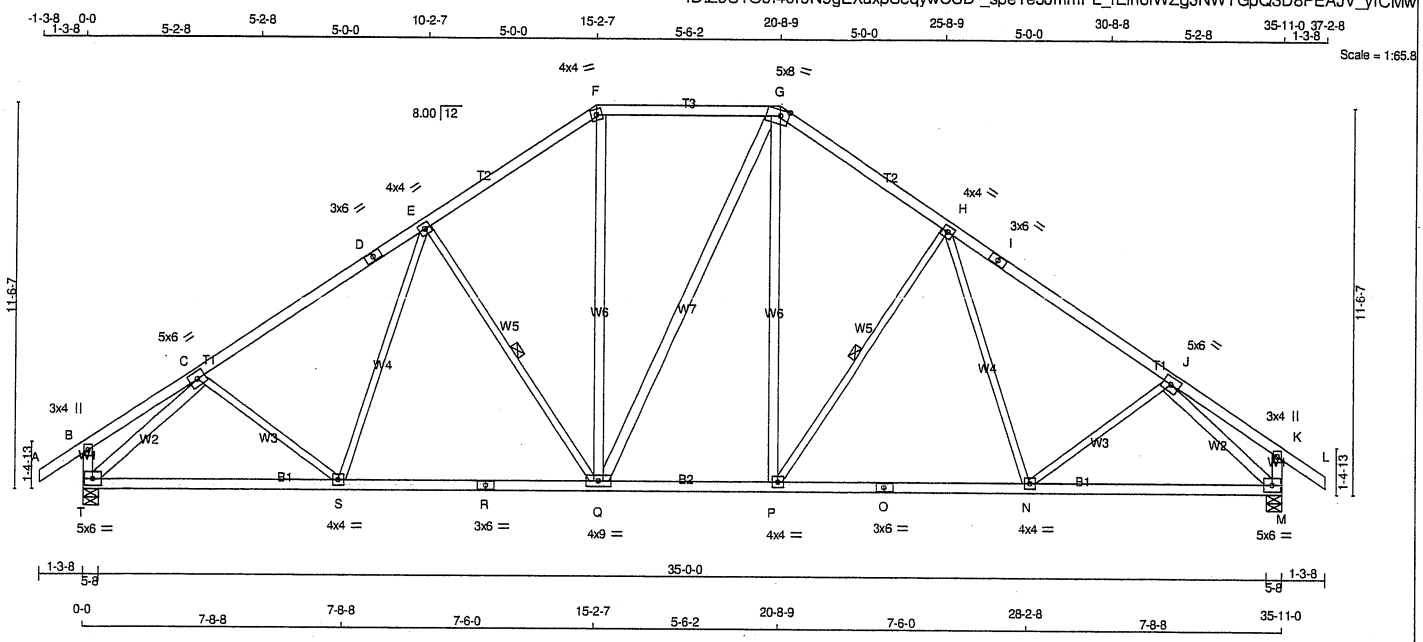


TOTAL WEIGHT = 2 X 180 = 359 lb
[M][F]

LUMBER
N. L. G. A. RULES

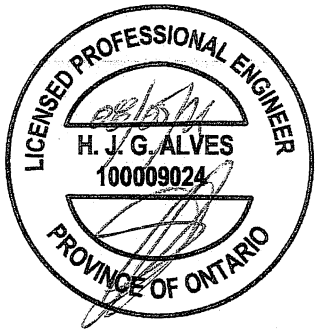
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - S	2x3	DRY	No.2	SPF
S - E	2x3	DRY	No.2	SPF
E - Q	2x3	DRY	No.2	SPF
P - H	2x3	DRY	No.2	SPF
H - N	2x3	DRY	No.2	SPF
N - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.00	3.00
H	TMWW-t	MT20	4.0	4.0		
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	5.0	6.0		
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		
N, P, S						
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
T	BMVW1-t	MT20	5.0	6.0		

~~NOTES~~ (1)
1)



Structural component only
DWG# T-2126525

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1973	0	1973	0	0	5-8	5-8
M	1973	0	1973	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1398	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1398	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, H-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 40	-84.9	-84.9	0.33 (1)	10.00
C- D	-2156 / 0	-84.9	-84.9	0.51 (1)	4.21
D- E	-2156 / 0	-84.9	-84.9	0.51 (1)	4.21
E- F	-1777 / 0	-84.9	-84.9	0.39 (1)	4.61
F- G	-1455 / 0	-84.9	-84.9	0.36 (1)	5.02
G- H	-1777 / 0	-84.9	-84.9	0.39 (1)	4.61
H- I	-2156 / 0	-84.9	-84.9	0.51 (1)	4.21
I- J	-2156 / 0	-84.9	-84.9	0.51 (1)	4.21
J- K	0 / 40	-84.9	-84.9	0.33 (1)	10.00
K- L	0 / 32	-84.9	-84.9	0.11 (1)	10.00
T- B	-186 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-186 / 0	0.0	0.0	0.02 (1)	7.81
T- S	0 / 1746	-18.5	-18.5	0.43 (1)	10.00
S- R	0 / 1761	-18.5	-18.5	0.43 (4)	10.00
R- Q	0 / 1761	-18.5	-18.5	0.43 (4)	10.00
Q- P	0 / 1455	-18.5	-18.5	0.32 (1)	10.00
P- O	0 / 1761	-18.5	-18.5	0.43 (1)	10.00
O- N	0 / 1761	-18.5	-18.5	0.43 (1)	10.00
N- M	0 / 1746	-18.5	-18.5	0.43 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	0 / 87	0.03 (4)
S- E	0 / 137	0.05 (4)
E- Q	-564 / 0	0.35 (1)
Q- F	0 / 583	0.09 (1)
Q- G	0 / 1	0.00 (1)
P- G	0 / 581	0.09 (1)
P- H	-565 / 0	0.35 (1)
H- N	0 / 138	0.05 (4)
N- J	0 / 88	0.03 (4)
T- C	-2458 / 0	0.62 (1)
J- M	-2458 / 0	0.62 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.51/1.00 (H-J:1) , BC=0.43/1.00 (N-P:1) , WB=0.62/1.00 (J-M:1) , SSI=0.19/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (J) (INPUT = 0.90)
JSI METAL= 0.64 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T51	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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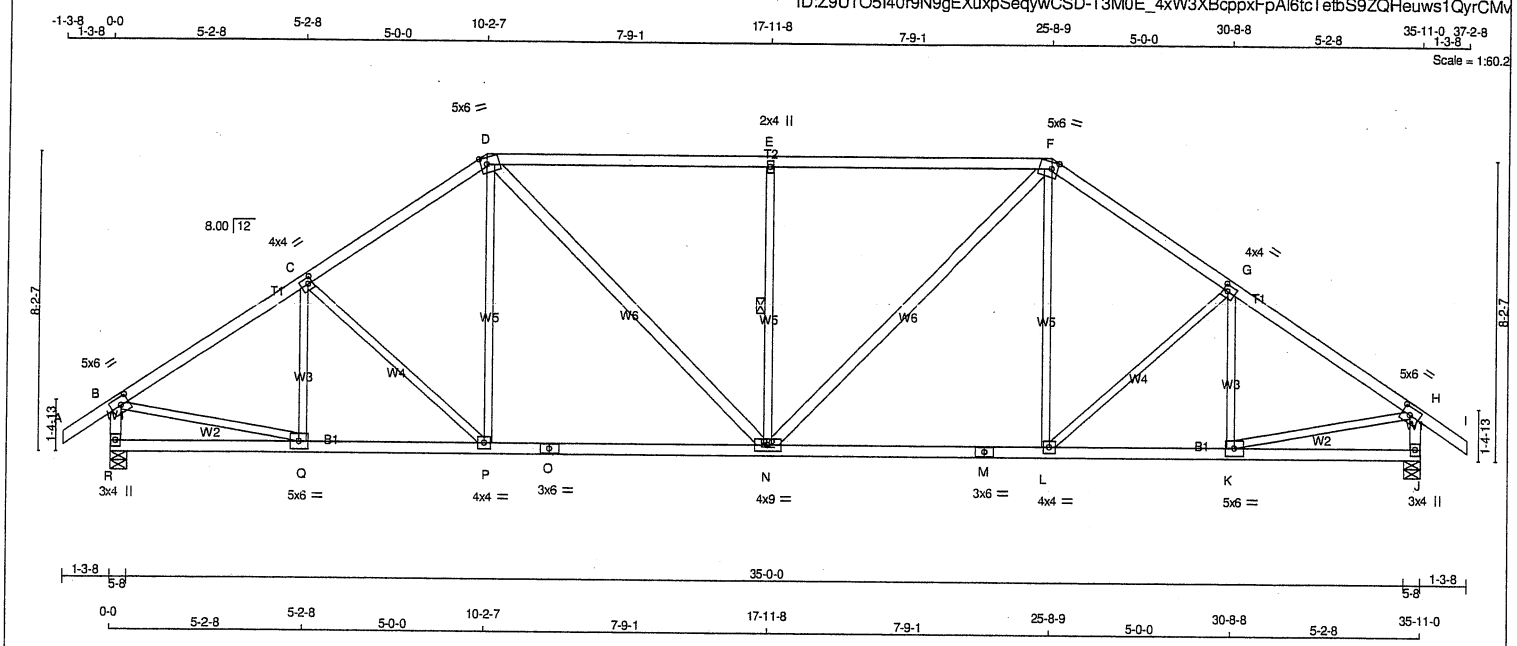

TOTAL WEIGHT = 2 X 161 = 323 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126526

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1973	0	1973	0	0	5-8	5-8
J	1973	0	1973	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1398	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
J	1398	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-2230 / 0	-84.9	-84.9	0.37 (1)	4.25
C- D	-2102 / 0	-84.9	-84.9	0.35 (1)	4.37
D- E	-2197 / 0	-84.9	-84.9	0.81 (1)	3.56
E- F	-2197 / 0	-84.9	-84.9	0.81 (1)	3.56
F- G	-2102 / 0	-84.9	-84.9	0.35 (1)	4.37
G- H	-2230 / 0	-84.9	-84.9	0.37 (1)	4.25
H- I	0 / 32	-84.9	-84.9	0.11 (1)	10.00
R- B	-1929 / 0	0.0	0.0	0.20 (1)	6.06
J- H	-1929 / 0	0.0	0.0	0.20 (1)	6.06
R- Q	0 / 0	-18.5	-18.5	0.11 (4)	10.00
Q- P	0 / 1878	-18.5	-18.5	0.39 (1)	10.00
P- O	0 / 1726	-18.5	-18.5	0.40 (1)	10.00
O- N	0 / 1726	-18.5	-18.5	0.40 (1)	10.00
N- M	0 / 1726	-18.5	-18.5	0.40 (1)	10.00
M- L	0 / 1726	-18.5	-18.5	0.40 (1)	10.00
L- K	0 / 1878	-18.5	-18.5	0.39 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-298 / 0	0.10 (1)
C- P	-208 / 0	0.18 (1)
P- D	0 / 261	0.06 (4)
D- N	0 / 672	0.11 (1)
N- E	-811 / 0	0.35 (1)
N- F	0 / 672	0.11 (1)
L- F	0 / 261	0.06 (4)
L- G	-208 / 0	0.18 (1)
K- G	-298 / 0	0.10 (1)
B- Q	0 / 1918	0.43 (1)
K- H	0 / 1918	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.81/1.00 (D-E:1) , BC=0.40/1.00 (N-P:1) , WB=0.43/1.00 (H-K:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.55 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T52	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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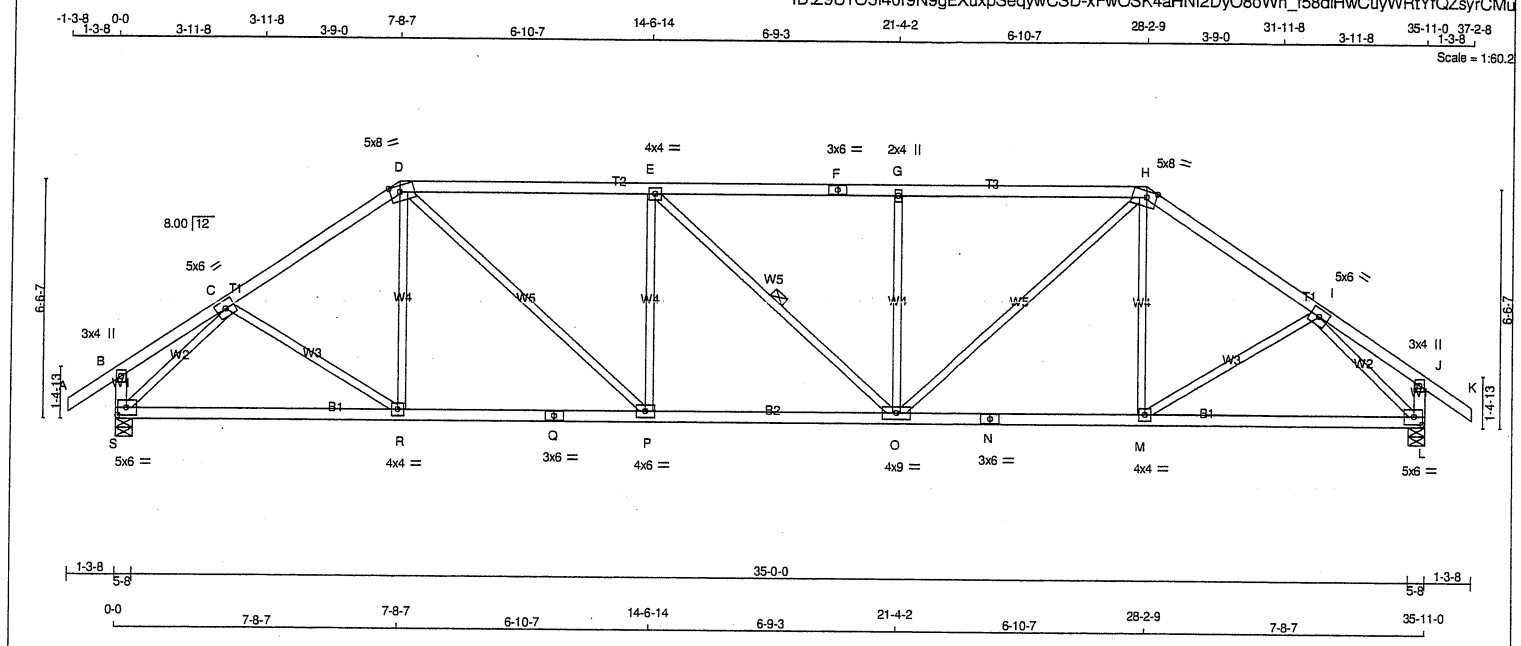

TOTAL WEIGHT = 148 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.25
I	TMWW-t	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0	2.50	2.75

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1973	0	1973	0	0	5-8	5-8
L	1973	0	1973	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1398	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1398	900 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 30	-84.9	-84.9	0.22 (1)	10.00
C- D	-2208 / 0	-84.9	-84.9	0.35 (1)	4.28
D- E	-2644 / 0	-84.9	-84.9	0.85 (1)	3.43
E- F	-2643 / 0	-84.9	-84.9	0.83 (1)	3.43
F- G	-2643 / 0	-84.9	-84.9	0.83 (1)	3.43
G- H	-2643 / 0	-84.9	-84.9	0.84 (1)	3.44
H- I	-2209 / 0	-84.9	-84.9	0.35 (1)	4.28
I- J	0 / 30	-84.9	-84.9	0.22 (1)	10.00
J- K	0 / 32	-84.9	-84.9	0.11 (1)	10.00
S- B	-187 / 0	0.0	0.0	0.02 (1)	7.81
L- J	-187 / 0	0.0	0.0	0.02 (1)	7.81
S- R	0 / 1667	-18.5	-18.5	0.41 (1)	10.00
R- Q	0 / 1821	-18.5	-18.5	0.43 (1)	10.00
Q- P	0 / 1821	-18.5	-18.5	0.43 (1)	10.00
P- O	0 / 2645	-18.5	-18.5	0.50 (1)	10.00
O- N	0 / 1821	-18.5	-18.5	0.44 (1)	10.00
N- M	0 / 1821	-18.5	-18.5	0.44 (1)	10.00
M- L	0 / 1668	-18.5	-18.5	0.41 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 179	0.04 (4)
R- D	0 / 112	0.04 (4)
D- P	0 / 1112	0.25 (1)
P- E	-629 / 0	0.44 (1)
E- O	-2 / 0	0.00 (1)
O- G	-628 / 0	0.44 (1)
O- H	0 / 1110	0.25 (1)
M- H	0 / 112	0.04 (4)
M- I	0 / 179	0.04 (4)
S- C	-2400 / 0	0.70 (1)
I- L	-2401 / 0	0.70 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.85/1.00 (D-E:1) , BC=0.50/1.00 (O-P:1) , WB=0.70/1.00 (I-L:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.59 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
05/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126527

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T53	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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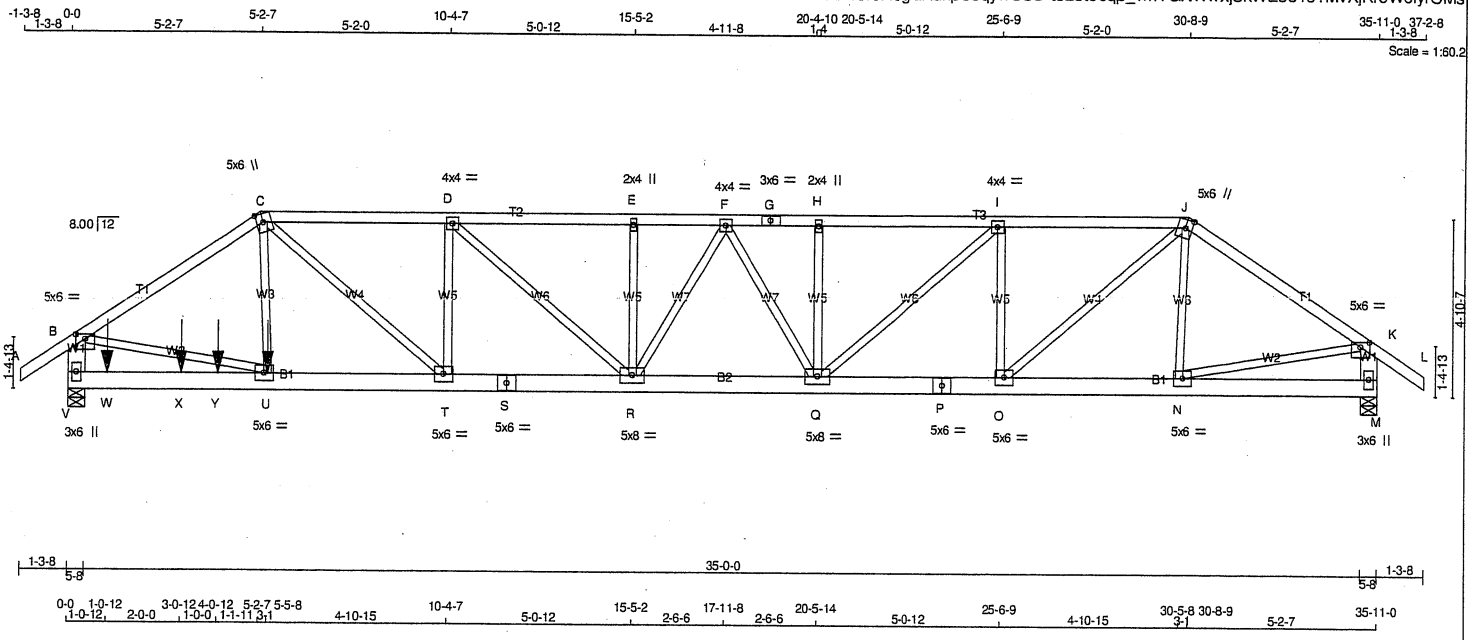

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- G	1	12	TOP
G- J	1	12	TOP
J- L	1	12	TOP
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(183.1)
S- P	2	12	TOP
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
C- U	1	6	SIDE(311.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126528

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3606	0	3606	0	0	5-8	5-8
M	2263	0	2263	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2556	1646 / 0	0 / 0	0 / 0	0 / 0	910 / 0	0 / 0
M	1604	1034 / 0	0 / 0	0 / 0	0 / 0	570 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B- C	-4518 / 0	-84.9	-84.9	0.37 (1)	4.21
C- D	-4747 / 0	-84.9	-84.9	0.30 (1)	4.18
D- E	-4970 / 0	-84.9	-84.9	0.31 (1)	4.10
E- F	-4970 / 0	-84.9	-84.9	0.16 (1)	4.22
F- G	-4643 / 0	-84.9	-84.9	0.15 (1)	4.35
G- H	-4643 / 0	-84.9	-84.9	0.15 (1)	4.35
H- I	-4643 / 0	-84.9	-84.9	0.30 (1)	4.22
I- J	-3763 / 0	-84.9	-84.9	0.26 (1)	4.64
J- K	-2661 / 0	-84.9	-84.9	0.28 (1)	5.29
K- L	0 / 32	-84.9	-84.9	0.06 (1)	10.00
V- B	-3532 / 0	0.0	0.0	0.13 (1)	7.45
M- K	-2220 / 0	0.0	0.0	0.08 (1)	7.81
V- W	0 / 0	-18.5	-18.5	0.05 (4)	10.00
W- X	0 / 0	-18.5	-18.5	0.05 (4)	10.00
X- Y	0 / 0	-18.5	-18.5	0.05 (4)	10.00
Y- U	0 / 0	-18.5	-18.5	0.05 (4)	10.00
U- T	0 / 3819	-18.5	-18.5	0.28 (1)	10.00
T- S	0 / 4747	-18.5	-18.5	0.34 (1)	10.00
S- R	0 / 4747	-18.5	-18.5	0.34 (1)	10.00
R- Q	0 / 4848	-18.5	-18.5	0.35 (1)	10.00
Q- P	0 / 3763	-18.5	-18.5	0.28 (1)	10.00
P- O	0 / 3763	-18.5	-18.5	0.28 (1)	10.00
O- N	0 / 2201	-18.5	-18.5	0.16 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 1235	0.15 (1)
T- D	-700 / 0	0.12 (1)
D- R	0 / 298	0.04 (1)
R- E	-329 / 0	0.06 (1)
Q- H	-329 / 0	0.06 (1)
Q- I	0 / 1177	0.15 (1)
O- I	-1285 / 0	0.22 (1)
O- J	0 / 2079	0.26 (1)
N- K	0 / 2251	0.28 (1)
R- F	0 / 248	0.03 (1)
F- Q	-418 / 0	0.09 (1)
C- U	0 / 1297	0.16 (1)
B- U	0 / 3822	0.47 (1)
N- J	-291 / 0	0.05 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	5-5-8	-1343	-1343	---	BACK	VERT	TOTAL	---	C1
W	1-0-12	-7	-7	---	BACK	VERT	TOTAL	---	C1
X	3-0-12	-7	-7	---	BACK	VERT	TOTAL	---	C1
Y	4-0-12	-7	-7	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.35/1.00 (Q-R:1) , WB=0.47/1.00 (B-U:1) , SSI=0.11/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.43 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420381	T53	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:Z9U1O5I40r9N9gEXuxpSeqywCSD-te28t06qp_vmTGXWwxjSkWE5e4e?MvXjKr8WelyrCMs

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	5.0	6.0	2.75	2.25
D, F, I						
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	5.0	6.0	2.75	2.25
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N, O, T, U						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126528

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T54	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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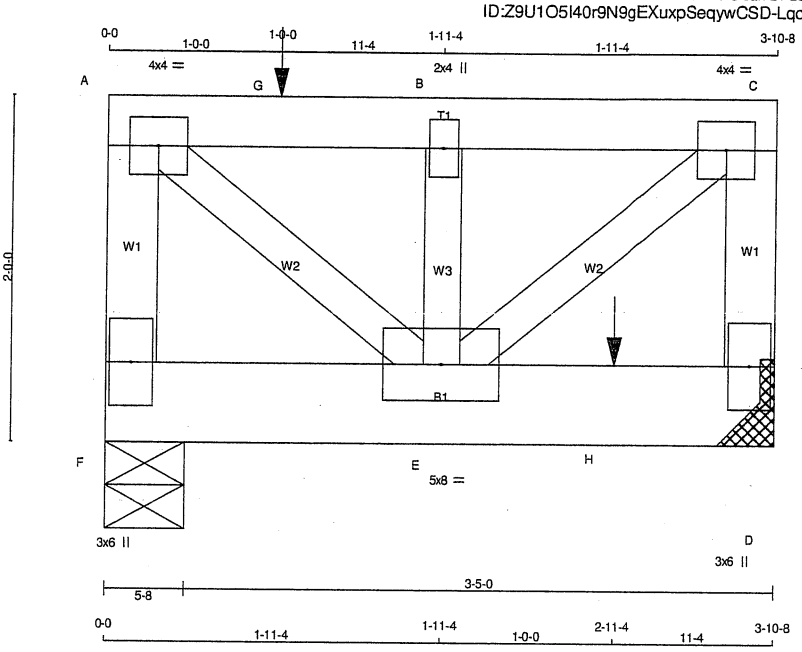

TOTAL WEIGHT = 2 X 18 = 37 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	5.0	8.0		

Structural component only
DWG# T-2126529

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1904	0	1904	0	0	5-8	5-8
D	1916	0	1916	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1351	861 / 0	0 / 0	0 / 0	0 / 0	490 / 0	0 / 0
D	1357	877 / 0	0 / 0	0 / 0	0 / 0	480 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-1878 / 0	0.0	0.0	0.11 (1)	7.81
A- G	-1298 / 0	-84.9	-84.9	0.57 (1)	6.18
G- B	-1298 / 0	-84.9	-84.9	0.57 (1)	6.18
B- C	-1298 / 0	-84.9	-84.9	0.21 (1)	6.25
D- C	-1030 / 0	0.0	0.0	0.06 (1)	7.81
F- E	0 / 0	-18.5	-18.5	0.01 (1)	10.00
E- H	0 / 0	-18.5	-18.5	0.30 (1)	10.00
H- D	0 / 0	-18.5	-18.5	0.30 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 1694	0.21 (1)
E- B	-1352 / 0	0.11 (1)
E- C	0 / 1694	0.21 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-0-0	-1245	-1245	---	TOP	VERT	TOTAL	---	C1
H	2-11-4	-1179	-1179	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.57/1.00 (A-B:1) , BC=0.30/1.00 (D-E:1) , WB=0.21/1.00 (A-E:1) , SSI=0.48/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.23 (A) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420381	T54	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126529

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T55	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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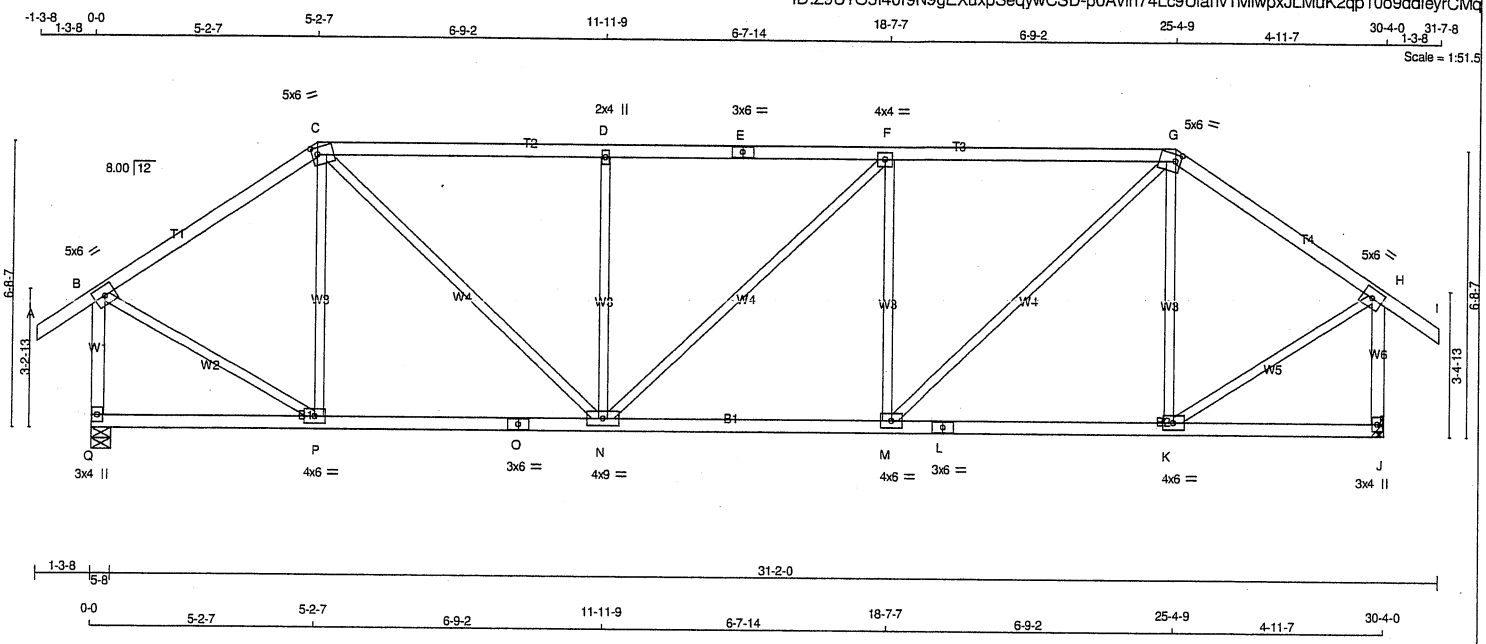

TOTAL WEIGHT = 133 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	1.50
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K, M, P						
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1684	0	1684	0	0	5-8	5-8
J	1684	0	1684	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1193	770 / 0	0 / 0	0 / 0	0 / 0	423 / 0	0 / 0
J	1193	770 / 0	0 / 0	0 / 0	0 / 0	423 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1294 / 0	-84.9	-84.9	0.46 (1)	5.08
C- D	-1838 / 0	-84.9	-84.9	0.67 (1)	4.16
D- E	-1838 / 0	-84.9	-84.9	0.67 (1)	4.15
E- F	-1838 / 0	-84.9	-84.9	0.67 (1)	4.15
F- G	-1826 / 0	-84.9	-84.9	0.68 (1)	4.16
G- H	-1246 / 0	-84.9	-84.9	0.41 (1)	5.23
H- I	0 / 32	-84.9	-84.9	0.11 (1)	10.00
Q- B	-1648 / 0	0.0	0.0	0.30 (1)	6.45
J- H	-1650 / 0	0.0	0.0	0.32 (1)	6.45
Q- P	0 / 0	-18.5	-18.5	0.16 (4)	10.00
P- O	0 / 1068	-18.5	-18.5	0.26 (1)	10.00
O- N	0 / 1068	-18.5	-18.5	0.26 (1)	10.00
N- M	0 / 1826	-18.5	-18.5	0.38 (1)	10.00
M- L	0 / 1027	-18.5	-18.5	0.25 (1)	10.00
L- K	0 / 1027	-18.5	-18.5	0.25 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-465 / 0	0.35 (1)
C- N	0 / 1060	0.24 (1)
N- D	-620 / 0	0.46 (1)
N- F	0 / 17	0.00 (1)
M- F	-633 / 0	0.47 (1)
M- G	0 / 1100	0.25 (1)
K- G	-509 / 0	0.38 (1)
B- P	0 / 1227	0.28 (1)
K- H	0 / 1212	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.68/1.00 (F-G:1) , BC=0.38/1.00 (M-N:1) , WB=0.47/1.00 (F-M:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.37 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126530

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T56S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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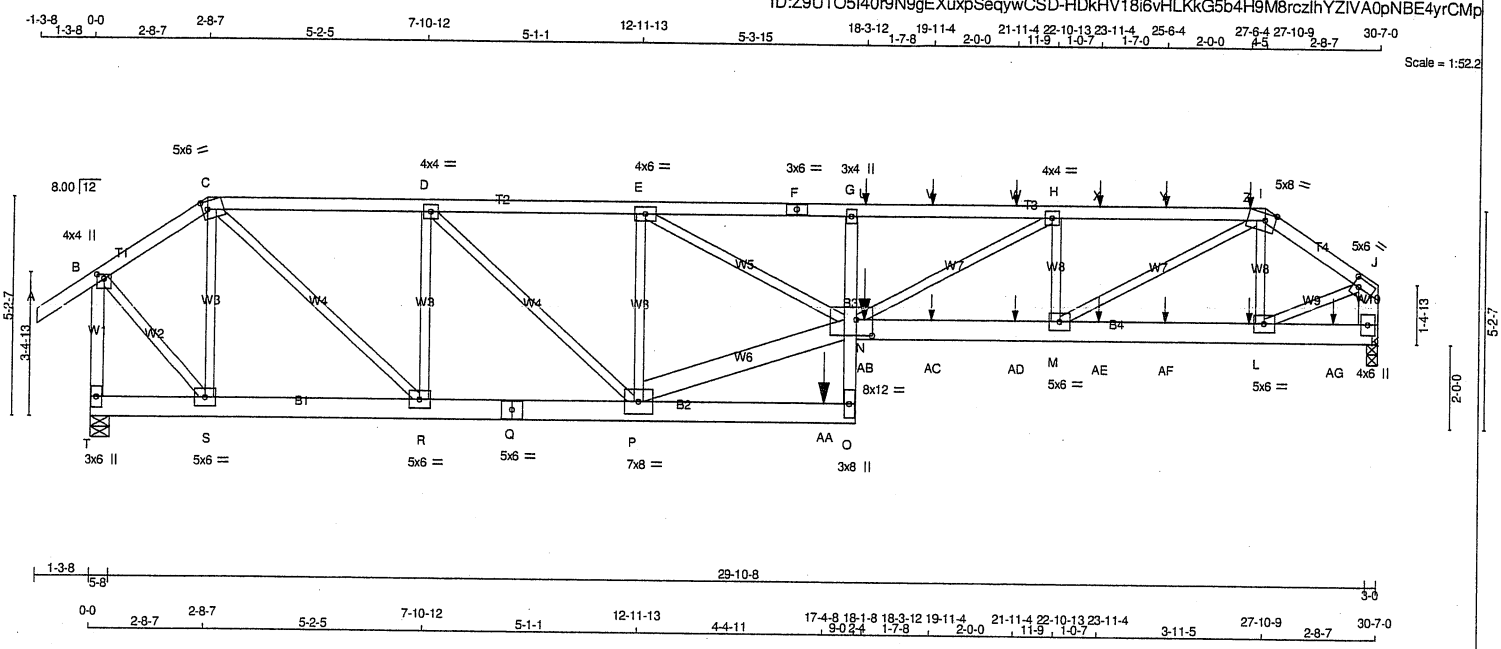

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- I	1	12	SIDE(61.0)
I- J	1	12	TOP
T- B	1	12	TOP
K- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- Q	2	12	TOP
Q- O	2	12	SIDE(0.0)
N- K	2	12	SIDE(0.0)
O- G	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126531

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1892	0	1892	0	0	5-8	5-8
K	1838	0	1838	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1340	869 / 0	0 / 0	0 / 0	0 / 0	471 / 0	0 / 0
K	1303	834 / 0	0 / 0	0 / 0	0 / 0	469 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B- C	-1150 / 0	-84.9	-84.9	0.06 (1)	6.25
C- D	-2277 / 0	-84.9	-84.9	0.20 (1)	5.70
D- E	-3019 / 0	-84.9	-84.9	0.23 (1)	5.10
E- F	-5281 / 0	-84.9	-84.9	0.28 (1)	4.02
F- G	-5281 / 0	-84.9	-84.9	0.28 (1)	4.02
G- U	-5321 / 0	-84.9	-84.9	0.30 (1)	3.98
U- V	-5321 / 0	-84.9	-84.9	0.30 (1)	3.98
V- W	-5321 / 0	-84.9	-84.9	0.30 (1)	3.98
W- H	-5321 / 0	-84.9	-84.9	0.30 (1)	3.98
H- X	-3956 / 0	-84.9	-84.9	0.24 (1)	4.57
X- Y	-3956 / 0	-84.9	-84.9	0.24 (1)	4.57
Y- Z	-3956 / 0	-84.9	-84.9	0.24 (1)	4.57
Z- I	-3956 / 0	-84.9	-84.9	0.24 (1)	4.57
I- J	-2028 / 0	-84.9	-84.9	0.07 (1)	6.12
T- B	-1891 / 0	0.0	0.0	0.18 (1)	7.81
K- J	-1843 / 0	0.0	0.0	0.07 (1)	7.81
T- S	0 / 0	-18.5	-18.5	0.03 (4)	10.00
S- R	0 / 935	-18.5	-18.5	0.09 (1)	10.00
R- Q	0 / 2277	-18.5	-18.5	0.19 (1)	10.00
Q- P	0 / 2277	-18.5	-18.5	0.19 (1)	10.00
P-AA	0 / 57	-18.5	-18.5	0.10 (1)	10.00
AA- O	0 / 57	-18.5	-18.5	0.10 (1)	10.00
O- N	0 / 422	0.0	0.0	0.12 (1)	10.00
N- G	-383 / 0	0.0	0.0	0.09 (1)	7.81
N-AB	0 / 3956	-18.5	-18.5	0.30 (1)	10.00
AB-AC	0 / 3956	-18.5	-18.5	0.30 (1)	10.00
AC-AD	0 / 3956	-18.5	-18.5	0.30 (1)	10.00
AD- M	0 / 3956	-18.5	-18.5	0.30 (1)	10.00
M-AE	0 / 1666	-18.5	-18.5	0.14 (1)	10.00
AE-AF	0 / 1666	-18.5	-18.5	0.14 (1)	10.00
AF- L	0 / 1666	-18.5	-18.5	0.14 (1)	10.00
L-AG	0 / 0	-18.5	-18.5	0.03 (1)	10.00
AG- K	0 / 0	-18.5	-18.5	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-901 / 0	0.17 (1)
P- E	-1699 / 0	0.33 (1)
P- N	0 / 3213	0.21 (1)
E- N	0 / 2569	0.32 (1)
L- I	-459 / 0	0.04 (1)
B- S	0 / 1396	0.17 (1)
L- J	0 / 1786	0.22 (1)
M- I	0 / 2630	0.33 (1)
N- H	0 / 1575	0.19 (1)
M- H	-1256 / 0	0.12 (1)
D- P	0 / 1030	0.13 (1)
C- R	0 / 1831	0.23 (1)
R- D	-1198 / 0	0.23 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	27-6-4	1	1	---	FRONT	VERT	TOTAL	---	C1
U	18-3-12	1	1	70	FRONT	VERT	TOTAL	---	C1
V	19-11-4	1	1	70	FRONT	VERT	TOTAL	---	C1
W	21-11-4	1	1	70	FRONT	VERT	TOTAL	---	C1
X	23-11-4	1	1	70	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.30/1.00 (G-H:1) , BC=0.30/1.00 (M-N:1) , WB=0.33/1.00 (E-P:1) , SSI=0.14/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420381	T56S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:Z9U1O5I40r9N9gEXuxpSeqywCSD-HDkHV18i6vHLKkG5b4H9M8rczlhYZIVA0pNBE4yrCMp

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	2.00	3.00
J	TMVW-t	MT20	5.0	6.0	2.50	1.75
K	BMV1+p	MT20	4.0	6.0		
L, M, R, S						
L	BMWW-t	MT20	5.0	6.0		
N	BVMWWW-l	MT20	8.0	12.0	4.50	4.50
O	BMV+p	MT20	3.0	8.0		
P	BMWWW-t	MT20	7.0	8.0		
Q	BS-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	25-6-4	1	1	70	FRONT	VERT	TOTAL	---	C1
Z	27-6-4	1	1	55	FRONT	VERT	TOTAL	---	C1
AA	17-4-8	-316	-316	---	FRONT	VERT	TOTAL	---	C1
AB	18-3-12	-3	-3	---	FRONT	VERT	TOTAL	---	C1
AC	19-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
AD	21-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
AE	23-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
AF	25-6-4	1	1	---	FRONT	VERT	TOTAL	---	C1
AG	29-6-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126531

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T57	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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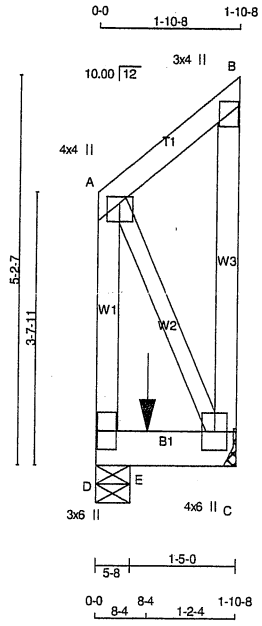

TOTAL WEIGHT = 2 X 18 = 35 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2126532

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	726	0	726	0	0	5-8	5-8
C	461	0	461	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	513	339 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0
C	326	214 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
D- A	-80 / 0	0.0	0.0	0.01 (1)	7.81
A- B	0 / 0	-84.9	-84.9	0.03 (1)	10.00
C- B	-80 / 0	0.0	0.0	0.02 (1)	7.81
D- E	0 / 0	-18.5	-18.5	0.16 (1)	10.00
E- C	0 / 0	-18.5	-18.5	0.16 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- C	0 / 0	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	8-4	-701	-701	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.03/1.00 (A-B:1) , BC=0.16/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.03 (A) (INPUT = 0.90)
JSI METAL= 0.01 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T58	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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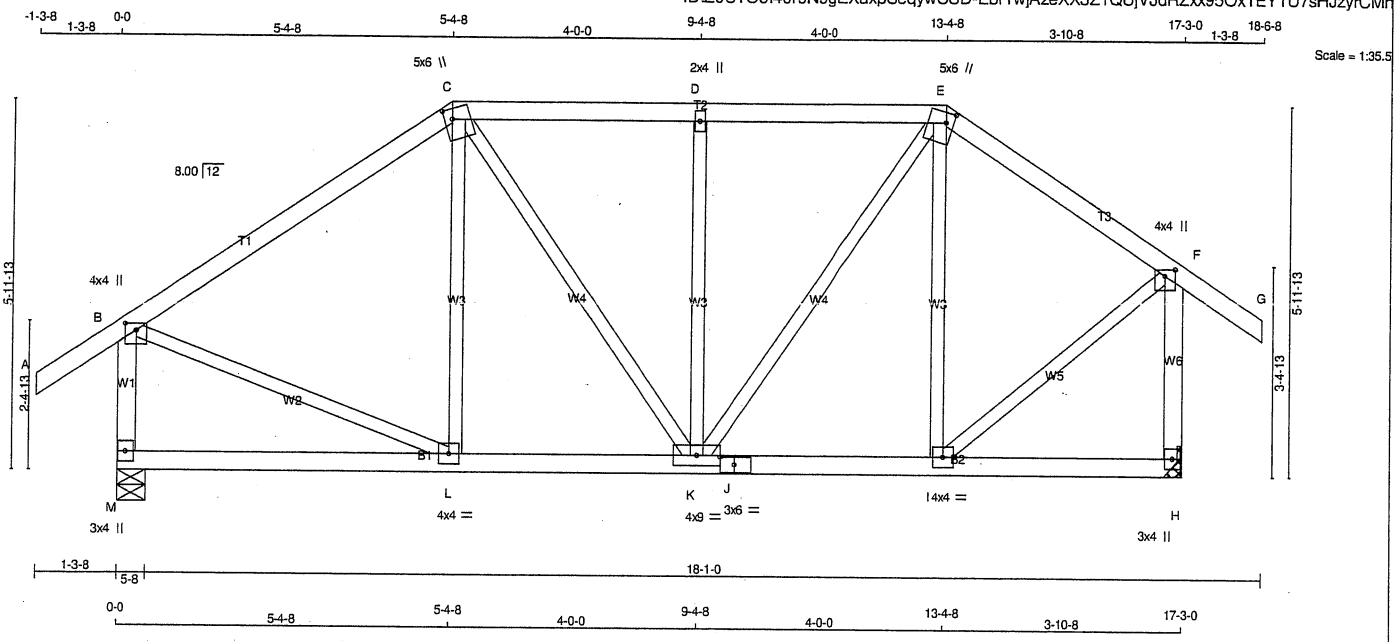

TOTAL WEIGHT = 83 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0	1.50	2.75
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1008	0	1008	0	0	5-8	5-8
H	1008	0	1008	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	713	466 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
H	713	466 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-713 / 0	-84.9	-84.9	0.32 (1)	6.25
C- D	-699 / 0	-84.9	-84.9	0.17 (1)	6.25
D- E	-699 / 0	-84.9	-84.9	0.17 (1)	6.25
E- F	-582 / 0	-84.9	-84.9	0.17 (1)	6.25
F- G	0 / 32	-84.9	-84.9	0.11 (1)	10.00
M- B	-966 / 0	0.0	0.0	0.12 (1)	7.81
H- F	-979 / 0	0.0	0.0	0.19 (1)	7.81
M- L	0 / 0	-18.5	-18.5	0.12 (4)	10.00
L- K	0 / 591	-18.5	-18.5	0.17 (4)	10.00
K- J	0 / 478	-18.5	-18.5	0.12 (1)	10.00
J- I	0 / 478	-18.5	-18.5	0.12 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-118 / 35	0.07 (1)
C- K	0 / 188	0.04 (1)
K- D	-413 / 0	0.23 (1)
K- E	0 / 383	0.09 (1)
I- E	-286 / 0	0.16 (1)
B- L	0 / 634	0.14 (1)
I- F	0 / 612	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.17/1.00 (K-L:4) , WB=0.23/1.00 (D-K:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (L) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/04/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126533

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420381	T59S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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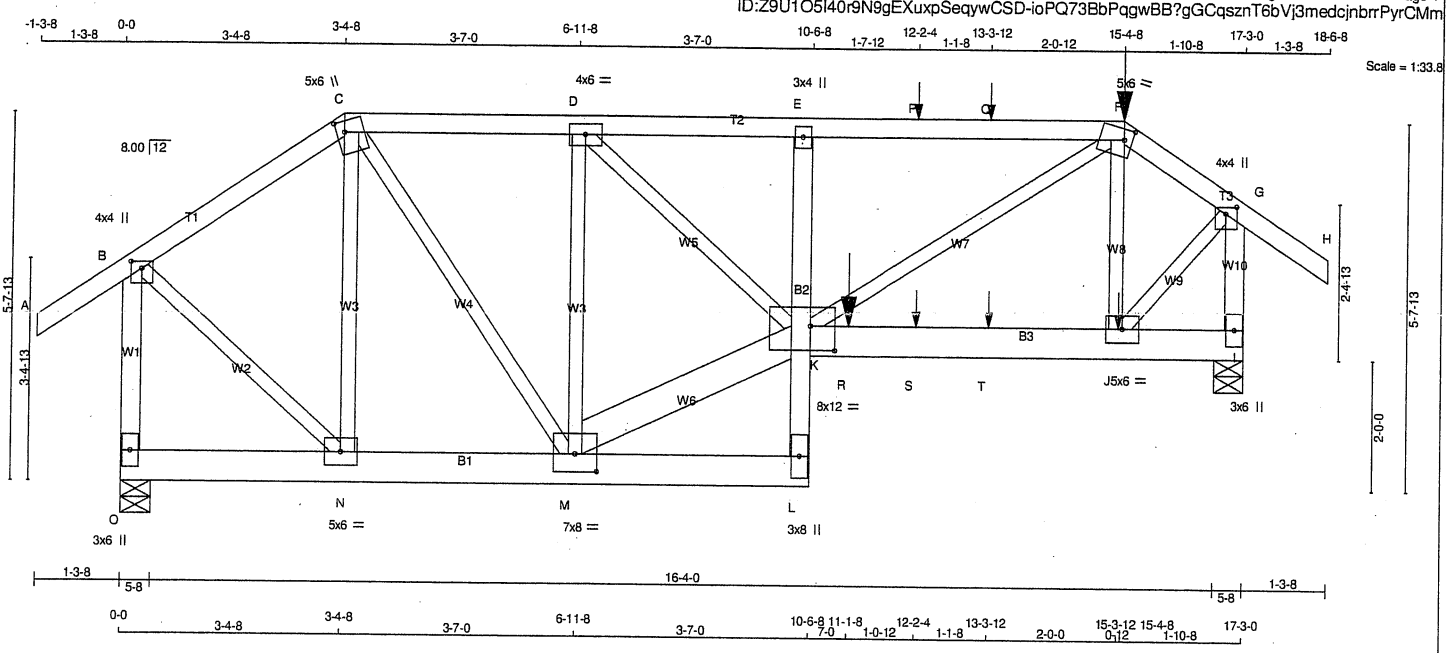

TOTAL WEIGHT = 102 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BVMWWW-l	MT20	8.0	12.0	4.50	4.50
L	BMV+p	MT20	3.0	8.0		
M	BMWWW-t	MT20	7.0	8.0	3.25	4.00
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2126534

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1131	0	1131	0	0	5-8	5-8
I	1290	0	1290	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	800	525 / 0	0 / 0	0 / 0	0 / 0	275 / 0	0 / 0
I	911	605 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.13 (1)	10.00
B- C	-666 / 0	-84.9	-84.9	0.19 (1)	6.25
C- D	-875 / 0	-84.9	-84.9	0.17 (5)	6.25
D- E	-1556 / 0	-84.9	-84.9	0.29 (1)	4.92
E- P	-1565 / 0	-84.9	-84.9	0.34 (1)	4.85
P- Q	-1565 / 0	-84.9	-84.9	0.34 (1)	4.85
Q- F	-1565 / 0	-84.9	-84.9	0.34 (1)	4.85
F- G	-811 / 0	-84.9	-84.9	0.13 (1)	6.25
G- H	0 / 32	-84.9	-84.9	0.13 (1)	10.00
O- B	-1107 / 0	0.0	0.0	0.21 (1)	7.49
I- G	-1337 / 0	0.0	0.0	0.17 (1)	6.97
O- N	0 / 0	-18.5	-18.5	0.03 (4)	10.00
N- M	0 / 545	-18.5	-18.5	0.09 (1)	10.00
M- L	0 / 15	-18.5	-18.5	0.03 (4)	10.00
L- K	0 / 30	0.0	0.0	0.05 (1)	10.00
K- E	-410 / 0	0.0	0.0	0.06 (1)	7.81
K- R	0 / 638	-18.5	-18.5	0.24 (1)	10.00
R- S	0 / 638	-18.5	-18.5	0.24 (1)	10.00
S- T	0 / 638	-18.5	-18.5	0.24 (1)	10.00
T- J	0 / 638	-18.5	-18.5	0.24 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-399 / 0	0.19 (1)
C- M	0 / 594	0.15 (1)
M- D	-930 / 0	0.43 (1)
M- K	0 / 1003	0.13 (1)
D- K	0 / 925	0.23 (1)
K- F	0 / 1108	0.27 (1)
J- F	-452 / 0	0.10 (1)
B- N	0 / 730	0.18 (1)
J- G	0 / 927	0.23 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	15-4-8	-45	-45	70	FRONT	VERT	TOTAL	---	C1
J	15-3-12	1	1	---	FRONT	VERT	TOTAL	---	C1
P	12-2-4	1	1	70	FRONT	VERT	TOTAL	---	C1
Q	13-3-12	1	1	70	FRONT	VERT	TOTAL	---	C1
R	11-1-8	-240	-240	---	FRONT	VERT	TOTAL	---	C1
S	12-2-4	1	1	---	FRONT	VERT	TOTAL	---	C1
T	13-3-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.34/1.00 (E-F:1) , BC=0.24/1.00 (J-K:1) , WB=0.43/1.00 (D-M:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.27 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T60	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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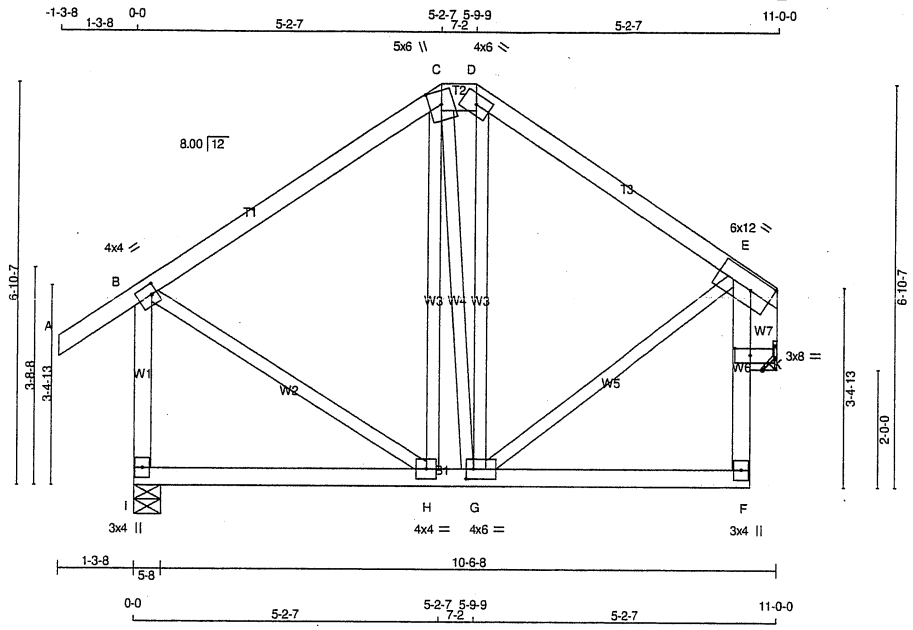

TOTAL WEIGHT = 63 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TTWW+m	MT20	5.0	6.0	2.75	2.50
D	TTW-h	MT20	4.0	6.0		
E	TMVWW1-t	MT20	6.0	12.0	3.00	4.50
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0	2.00	1.50
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		
J	KP-p	MT20	3.0	8.0	1.50	3.25

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K(E)	529	0	529	0	0	MECHANICAL	
I	677	0	677	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K(E). MINIMUM BEARING LENGTH AT JOINT K(E) = 5-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K(E)	376	238 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0
I	478	316 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-286 / 0	-84.9	-84.9	0.40 (1)	6.25
C- D	-237 / 0	-84.9	-84.9	0.00 (1)	6.25
D- E	-283 / 0	-84.9	-84.9	0.33 (1)	6.25
I- B	-639 / 0	0.0	0.0	0.12 (1)	7.81
F- J	0 / 38	0.0	0.0	0.13 (1)	10.00
J- E	0 / 38	0.0	0.0	0.13 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.11 (4)	10.00
H- G	0 / 240	-18.5	-18.5	0.13 (4)	10.00
G- F	0 / 49	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-65 / 25	0.05 (1)
G- D	-71 / 15	0.06 (1)
B- H	0 / 279	0.06 (1)
C- G	-24 / 0	0.02 (1)
G- E	0 / 232	0.05 (1)
E- K	-540 / 0	0.04 (1)
J- K	0 / 108	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.13/1.00 (G-H:4) , WB=0.06/1.00 (B-H:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126535

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T61	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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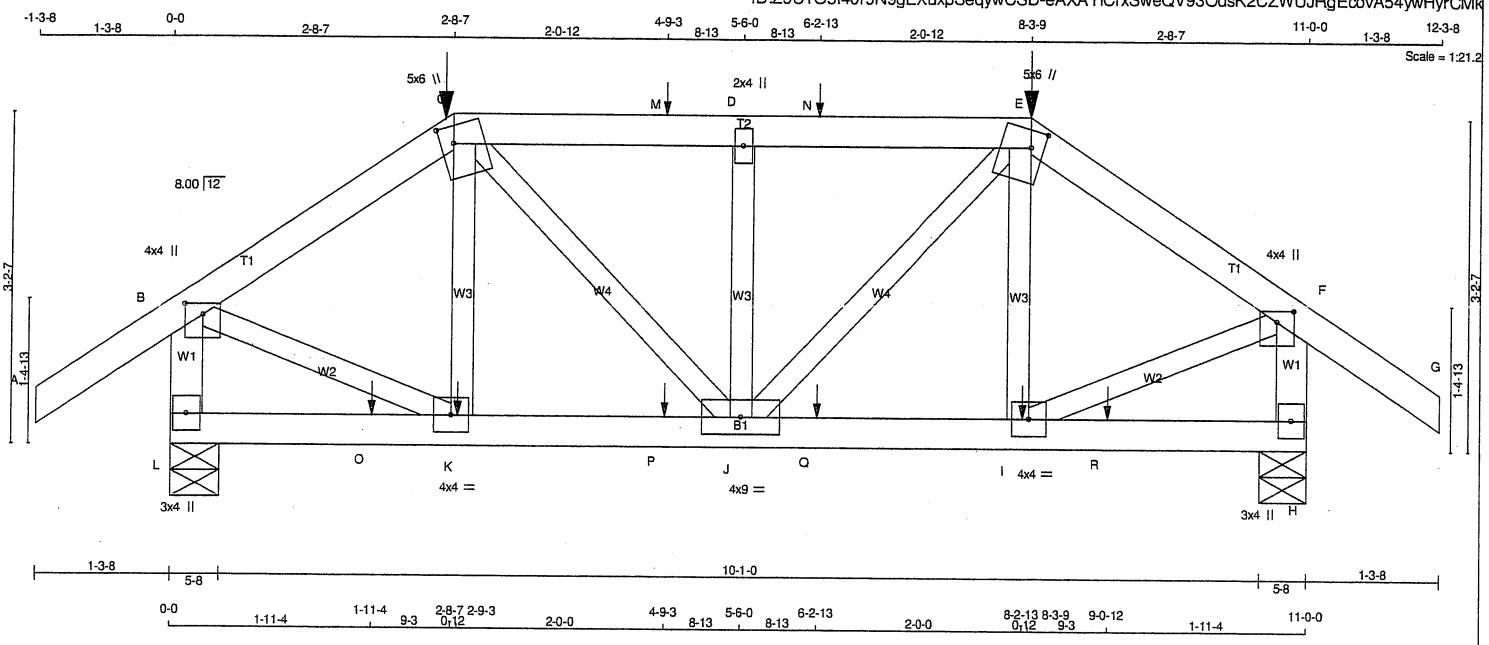

TOTAL WEIGHT = 49 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126536

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	790	0	790	0	0	5-8	5-8
H	761	0	761	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	554	390 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
H	535	371 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.13 (1)	10.00
B- C	-612 / 0	-84.9	-84.9	0.12 (1)	6.25
C- M	-645 / 0	-84.9	-84.9	0.11 (1)	6.25
M- D	-645 / 0	-84.9	-84.9	0.11 (1)	6.25
D- N	-645 / 0	-84.9	-84.9	0.11 (1)	6.25
N- E	-645 / 0	-84.9	-84.9	0.11 (1)	6.25
E- F	-580 / 0	-84.9	-84.9	0.12 (1)	6.25
F- G	0 / 32	-84.9	-84.9	0.13 (1)	10.00
L- B	-768 / 0	0.0	0.0	0.09 (1)	7.81
H- F	-740 / 0	0.0	0.0	0.08 (1)	7.81
L- O	0 / 0	-18.5	-18.5	0.04 (4)	10.00
O- K	0 / 0	-18.5	-18.5	0.04 (4)	10.00
K- P	0 / 503	-18.5	-18.5	0.11 (1)	10.00
P- J	0 / 503	-18.5	-18.5	0.11 (1)	10.00
J- Q	0 / 477	-18.5	-18.5	0.10 (1)	10.00
Q- I	0 / 477	-18.5	-18.5	0.10 (1)	10.00
I- R	0 / 0	-18.5	-18.5	0.04 (4)	10.00
R- H	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-136 / 7	0.03 (1)
C- J	0 / 204	0.05 (1)
J- D	-281 / 0	0.06 (1)
J- E	0 / 242	0.06 (1)
I- E	-125 / 7	0.03 (1)
B- K	0 / 544	0.13 (1)
I- F	0 / 516	0.13 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-8-7	-79	-79	49	FRONT	VERT	TOTAL	---	C1
E	8-3-9	-42	-42	70	FRONT	VERT	TOTAL	---	C1
I	8-2-13	1	1	---	FRONT	VERT	TOTAL	---	C1
K	2-9-3	1	1	---	FRONT	VERT	TOTAL	---	C1
M	4-9-3	1	1	70	FRONT	VERT	TOTAL	---	C1
N	6-2-13	1	1	70	FRONT	VERT	TOTAL	---	C1
O	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
P	4-9-3	1	1	---	FRONT	VERT	TOTAL	---	C1
Q	6-2-13	1	1	---	FRONT	VERT	TOTAL	---	C1
R	9-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.11/1.00 (J-K:1) , WB=0.13/1.00 (B-K:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (K) (INPUT = 0.90)
JSI METAL= 0.17 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	T62	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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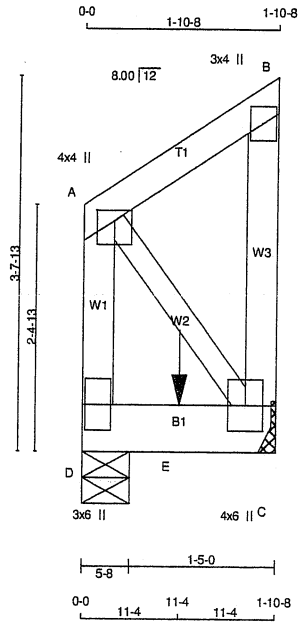

TOTAL WEIGHT = 2 X 13 = 26 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	1	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

Structural component only
DWG# T-2126537

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	353	0	353	0	0	5-8	5-8
C	353	0	353	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	250	163 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
C	250	163 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
D- A	-80 / 0	0.0	0.0	0.01 (1)	7.81
A- B	0 / 0	-84.9	-84.9	0.03 (1)	10.00
C- B	-80 / 0	0.0	0.0	0.01 (1)	7.81
D- E	0 / 0	-18.5	-18.5	0.09 (1)	10.00
E- C	0 / 0	-18.5	-18.5	0.09 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- C	0 / 0	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	11-4	-362	-362	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.03/1.00 (A-B:1) , BC=0.09/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.03 (A) (INPUT = 0.90)
JSI METAL= 0.01 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	J1	13	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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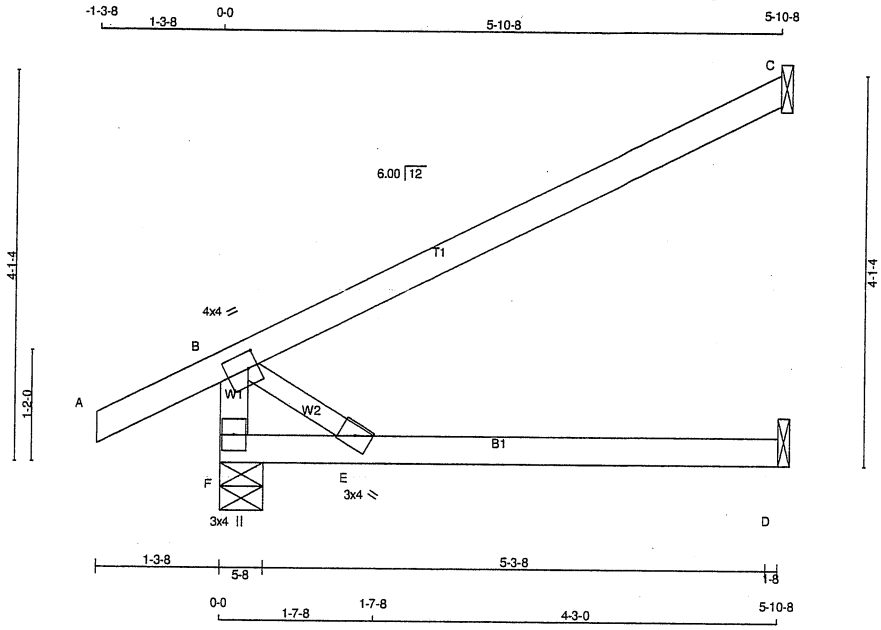

TOTAL WEIGHT = 13 X 18 = 234 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	419	0	419	0	0	5-8	5-8
C	249	0	249	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	295	200 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	172	137 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-364 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 0	-84.9	-84.9	0.50 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J./G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126452

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	J1S	8	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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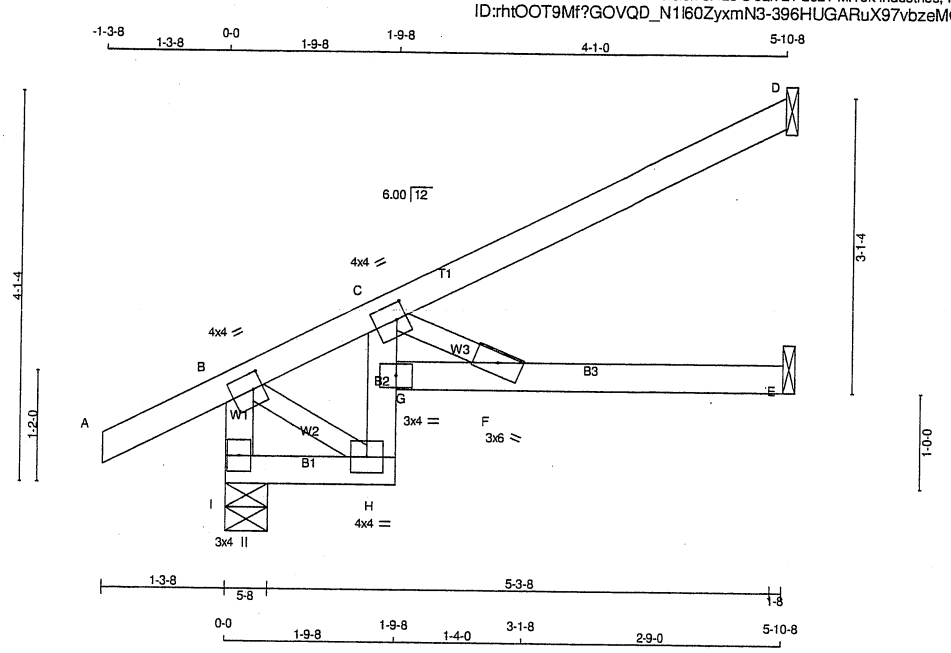

TOTAL WEIGHT = 8 X 21 = 166 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW+w	MT20	3.0	6.0		
G	BVM-I	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	432	0	432	0	0	5-8	5-8
D	184	0	184	0	0	1-8	1-8
E	107	0	107	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	304	207 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	128	96 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
E	79	33 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-417 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-216 / 0	-84.9	-84.9	0.11 (1)	6.25
C- D	0 / 4	-84.9	-84.9	0.27 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00
H- G	-73 / 0	0.0	0.0	0.23 (1)	7.81
G- C	0 / 134	0.0	0.0	0.25 (1)	10.00
G- F	0 / 419	-18.5	-18.5	0.39 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.32 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- H	0 / 192	0.04 (1)
C- F	-481 / 0	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 587 (0.12")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.39/1.00 (F-G:1) , WB=0.07/1.00 (C-F:1) , SSI=0.19/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (C) (INPUT = 0.90)
JSI METAL= 0.15 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126481

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	J1C	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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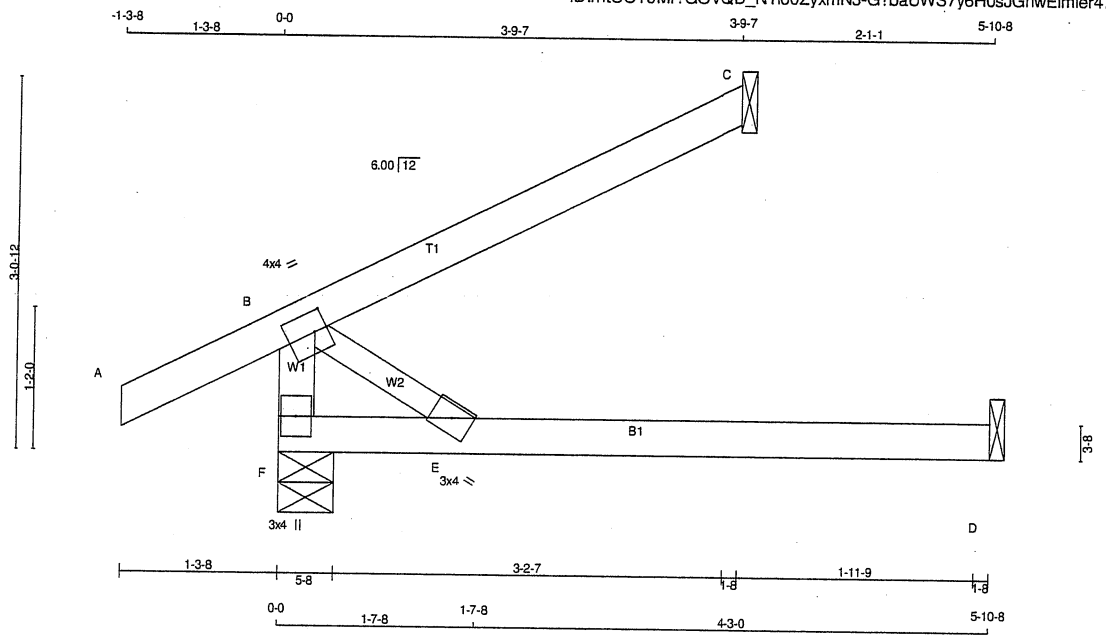

TOTAL WEIGHT = 2 X 15 = 31 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	330	0	330	0	0	5-8	5-8
C	161	0	161	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	234	151 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	111	88 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-276 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 0	-84.9	-84.9	0.21 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/04/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126453

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	J1C2	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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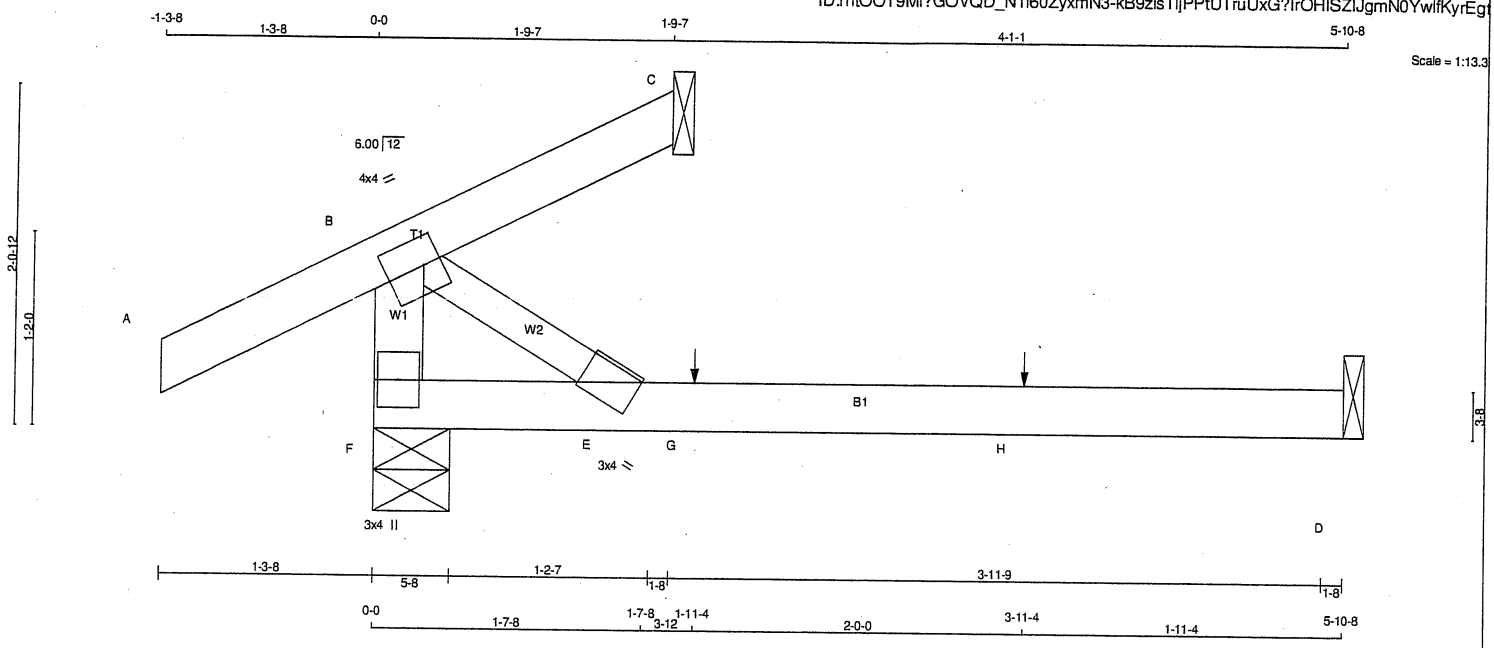

TOTAL WEIGHT = 2 X 13 = 26 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	289	0	289	0	0	5-8	5-8
C	32	0	32	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	205	129 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	22	18 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-235 / 0	0.0	0.0	0.02 (1)	7.81	
A- B	0 / 26	-84.9	-84.9	0.12 (1)	10.00	
B- C	-20 / 0	-84.9	-84.9	0.11 (1)	6.25	
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
E- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00	
G- H	0 / 0	-18.5	-18.5	0.19 (4)	10.00	
H- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.96 NAIL=0.96 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126454

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	J1C3	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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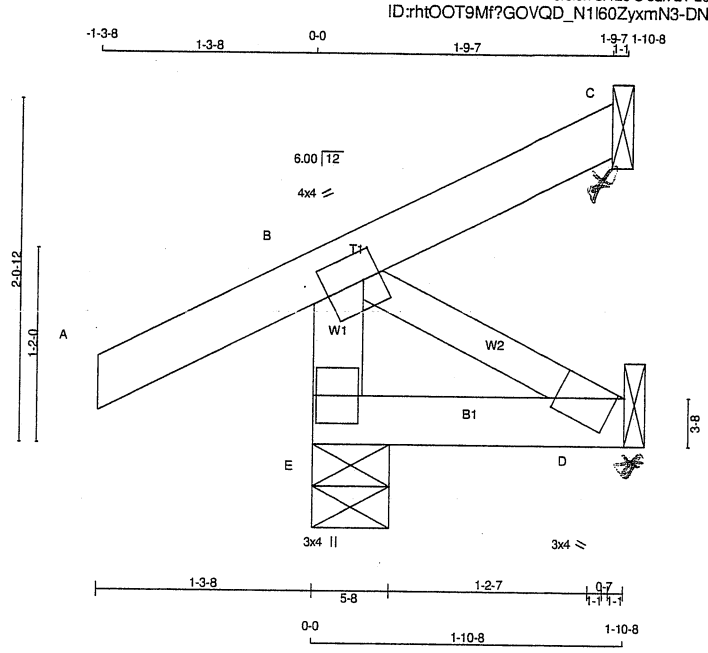

TOTAL WEIGHT = 2 X 8 = 17 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	BMW1+w	MT20	3.0	4.0	2.00	1.75
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	250	0	250	0	0	5-8	5-8
C	32	0	32	0	-34	1-8	1-8
D	15	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	174	129 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
C	22	18 / -24	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-235 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-20 / 0	-84.9	-84.9	0.11 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- D	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126455

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	J1C4	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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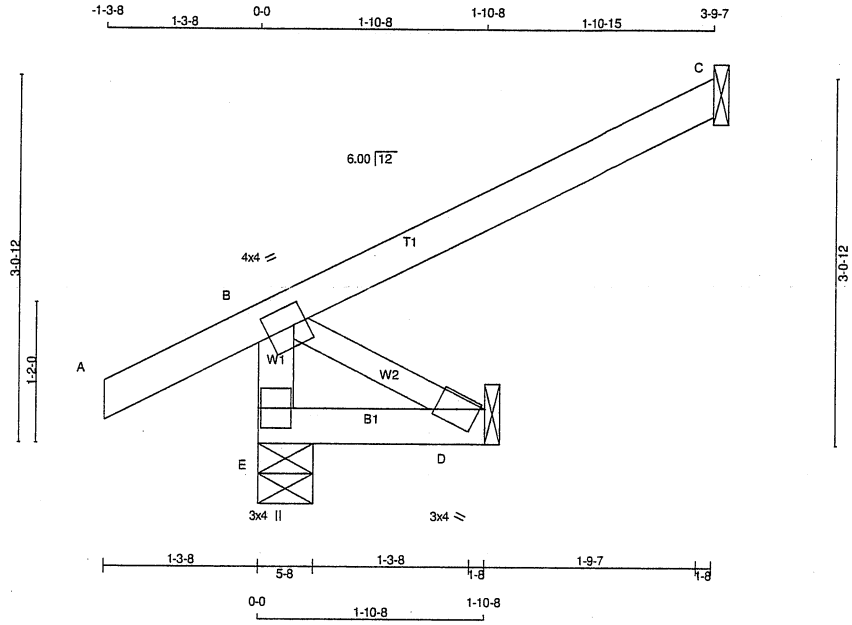

TOTAL WEIGHT = 2 X 11 = 22 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	BMW1+w	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	291	0	291	0	0	5-8	5-8
C	161	0	161	0	0	1-8	1-8
D	15	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	203	151 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
C	111	88 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-276 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 26	-84.9	-84.9	0.12 (5)	10.00
B- C	0 / 0	-84.9	-84.9	0.21 (1)	10.00
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- D	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126456

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	J2	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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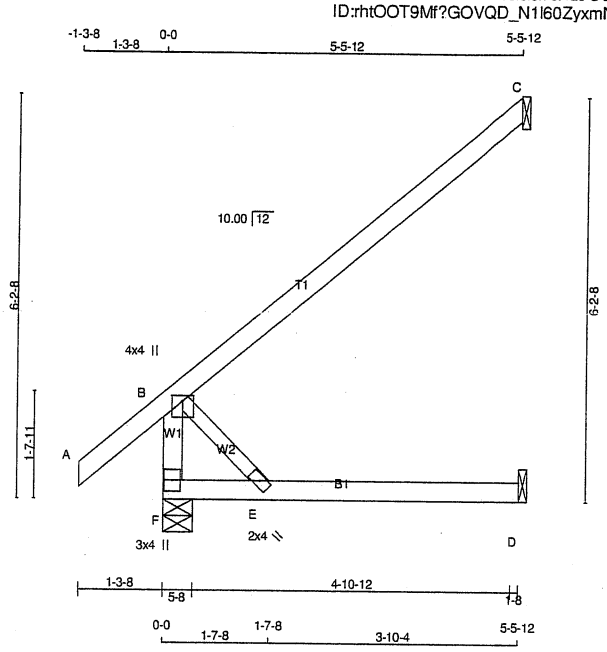

TOTAL WEIGHT = 3 X 19 = 58 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	401	0	401	0	0	5-8	5-8
C	232	0	232	0	0	1-8	1-8
D	51	0	57	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	282	192 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
C	160	128 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
D	41	0 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-350 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 0	-84.9	-84.9	0.43 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.13 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.16 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.43/1.00 (B-C:1) , BC=0.16/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126457

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	J3	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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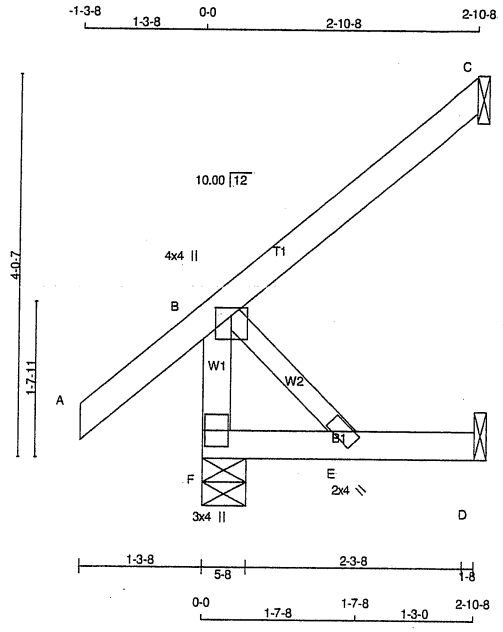

TOTAL WEIGHT = 2 X 13 = 25 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	266	0	266	0	0	5-8	5-8
C	122	0	122	0	0	1-8	1-8
D	27	0	30	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	187	131 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	84	67 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	21	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-240 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 38	-84.9	-84.9	0.13 (5)	10.00
B- C	0 / 0	-84.9	-84.9	0.12 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.04 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.04/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126458

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	J3C	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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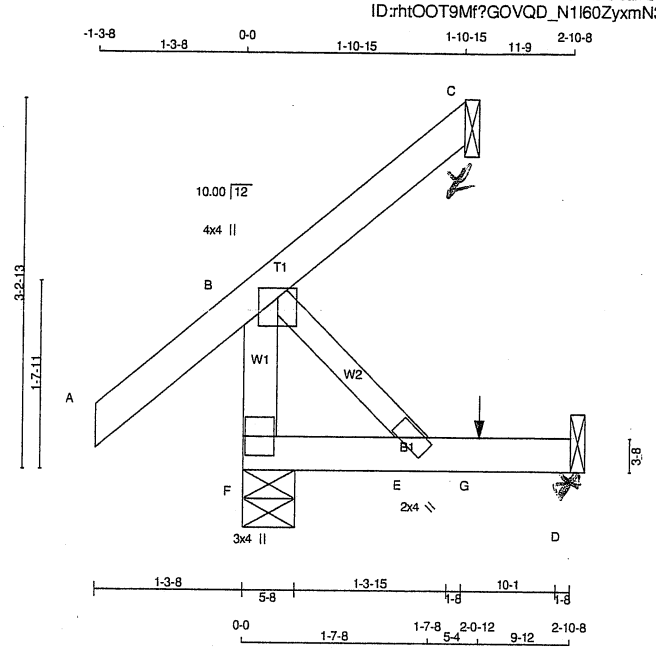

TOTAL WEIGHT = 11 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	268	0	268	0	0	5-8	5-8
C	39	0	39	0	-32	1-8	1-8
D	27	0	30	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	188	132 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	27	21 / -23	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	21	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-241 / 0	0.0	0.0		0.03 (1)	7.81
A- B	0 / 38	-84.9	-84.9		0.13 (5)	10.00
B- C	-27 / 0	-84.9	-84.9		0.13 (5)	6.25
F- E	0 / 0	-18.5	-18.5		0.04 (4)	10.00
E- G	0 / 0	-18.5	-18.5		0.04 (4)	10.00
G- D	0 / 0	-18.5	-18.5		0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-0	-0	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.04/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126459

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420375	J3C1	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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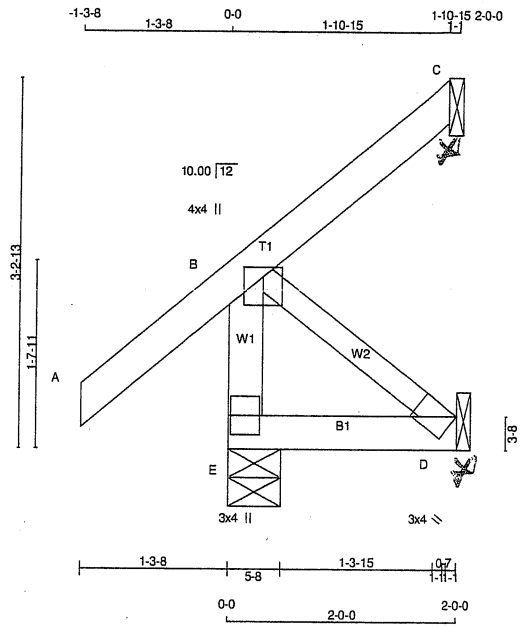

TOTAL WEIGHT = 10 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00.
D	BMW1+w	MT20	3.0	4.0	3.25	Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	258	0	258	0	0	5-8	5-8
C	39	0	39	0	-32	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	180	132 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	27	21 / -23	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-241 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 38	-84.9	-84.9	0.12 (5)	10.00
B- C	-27 / 0	-84.9	-84.9	0.11 (5)	6.25
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
B- D	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:5) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126460

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420376	J4	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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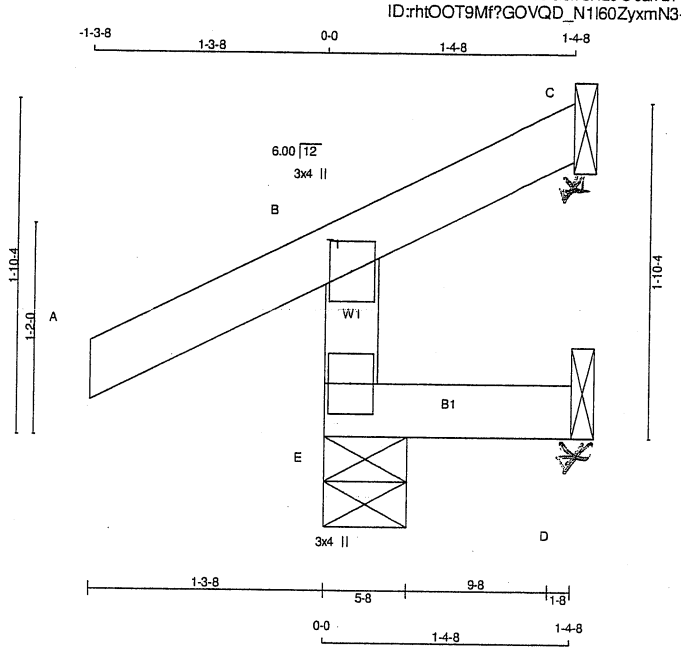

TOTAL WEIGHT = 3 X 6 = 18 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	243	0	243	0	0	5-8	5-8
C	15	0	15	0	-33	1-8	1-8
D	0	0	11	0	-7	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	169	126 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
C	10	8 / -22	0 / 0	0 / 0	0 / 0	3 / 0	0 / 0
D	0	0 / -9	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-217 / 0	0.0	0.0	0.03 (5)	7.81
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-20 / 0	-84.9	-84.9	0.08 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.03 (5)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126482

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	J5	5	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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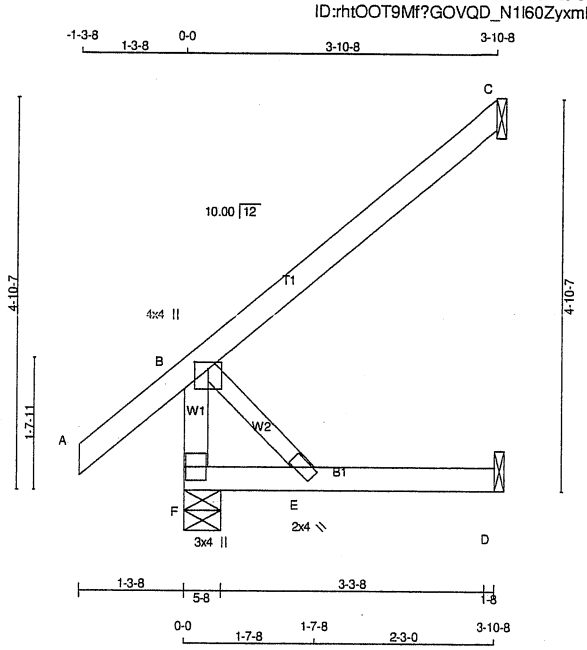

TOTAL WEIGHT = 5 X 15 = 76 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	318	0	318	0	0	5-8	5-8
C	164	0	164	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	223	155 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
C	113	90 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-282 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 38	-84.9	-84.9	0.14 (5)	10.00
B- C	0 / 0	-84.9	-84.9	0.22 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126503

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	J5C	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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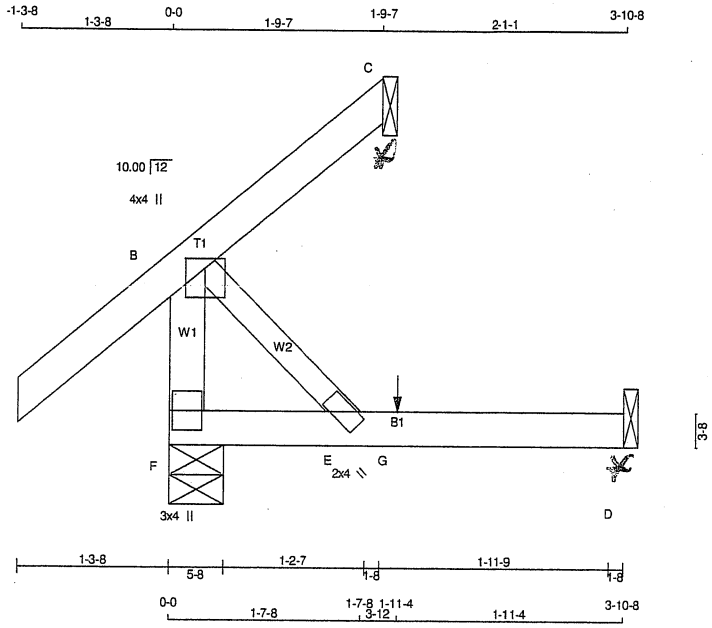

TOTAL WEIGHT = 3 X 12 = 36 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	275	0	275	0	0	5-8	5-8
C	30	0	30	0	-36	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	194	131 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-239 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 38	-84.9	-84.9	0.14 (5)	10.00
B- C	-29 / 0	-84.9	-84.9	0.13 (5)	6.25
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00
E- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00
G- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126504

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	J5C1	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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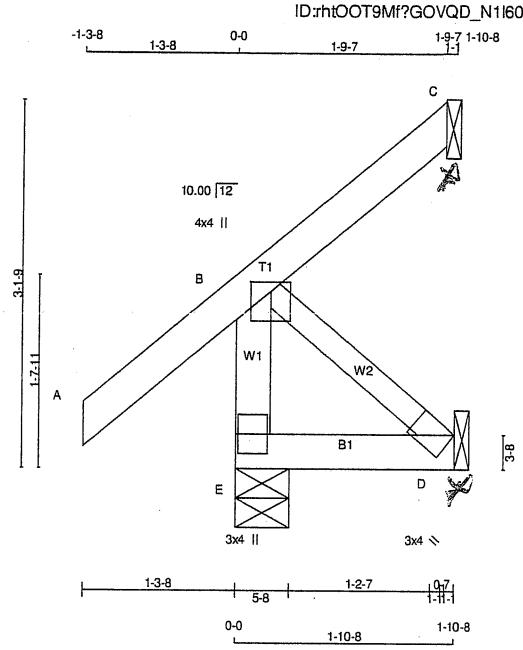

TOTAL WEIGHT = 3 X 10 = 30 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW1+w	MT20	3.0	4.0	3.00	Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	255	0	255	0	0	5-8	5-8
C	30	0	30	0	-36	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	178	131 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	21	17 / -25	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-239 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-29 / 0	-84.9	-84.9	0.11 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
B- D	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126505

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	J6	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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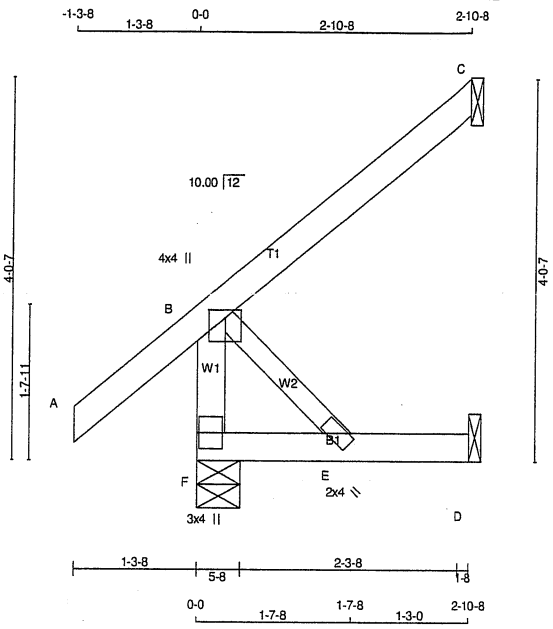

TOTAL WEIGHT = 4 X 13 = 50 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	266	0	266	0	0	5-8	5-8
C	122	0	122	0	0	1-8	1-8
D	27	0	30	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	187	131 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	84	67 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	21	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-240 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 38	-84.9	-84.9	0.13 (5)	10.00
B- C	0 / 0	-84.9	-84.9	0.12 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.04 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.04/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126506

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420378	J7	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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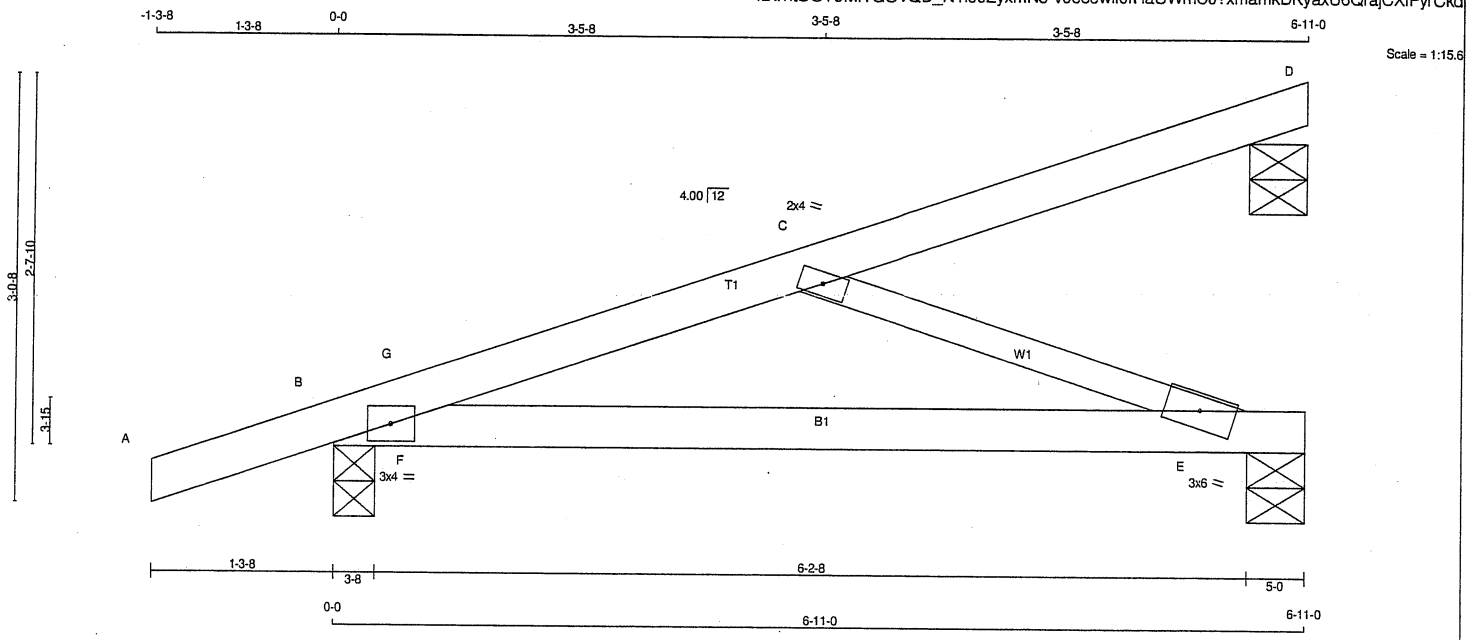

TOTAL WEIGHT = 4 X 21 = 82 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
E	BMW1+w	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	115	0	115	0	0	5-0 (4-15)	5-0
B	442	0	442	0	0	3-8	3-8
E	257	0	257	0	0	5-0	5-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	79	66 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
B	311	212 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0
E	184	107 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-84.9	-84.9	0.11 (1)	10.00
B- G	-564 / 0	-84.9	-84.9	0.11 (4)	6.25
G- C	-493 / 0	-84.9	-84.9	0.13 (1)	6.25
C- D	-11 / 0	-84.9	-84.9	0.12 (1)	6.25
B- F	0 / 488	-18.5	-18.5	0.11 (4)	10.00
F- E	0 / 488	-18.5	-18.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- E	-530 / 0	0.12 (1)
F- G	0 / 127	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.13/1.00 (C-G:1) , BC=0.17/1.00 (E-F:1) , WB=0.12/1.00 (C-E:1) , SSI=0.14/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (C) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126507

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	J8	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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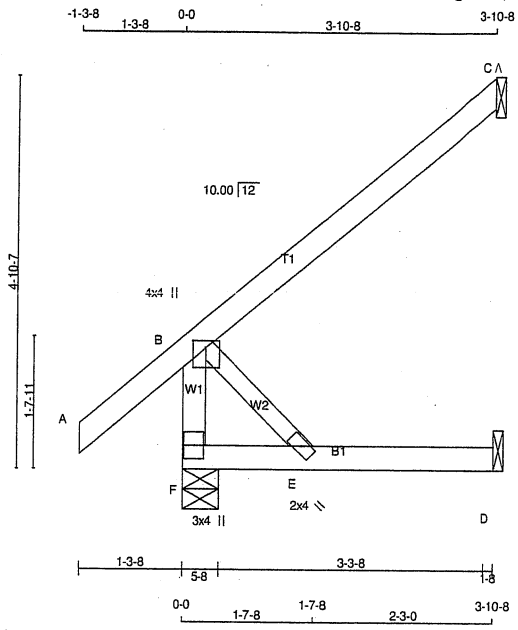

TOTAL WEIGHT = 3 X 15 = 46 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	367	0	367	0	0	5-8	5-8
G(C)	123	0	123	0	0	1-8	1-8
D	27	0	30	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	258	177 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
G(C)	85	68 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	22	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, G(C)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-344 / 0	0.0	0.0	0.08 (4)	7.81
A- B	0 / 38	-84.9	-84.9	0.14 (5)	10.00
B- C	-26 / 0	-84.9	-84.9	0.22 (1)	6.25
F- E	-32 / 0	-18.5	-18.5	0.03 (4)	6.25
E- D	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 42	0.01 (4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.04/1.00 (D-E:4) , WB=0.01/1.00 (B-E:4) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126520

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	J9	11	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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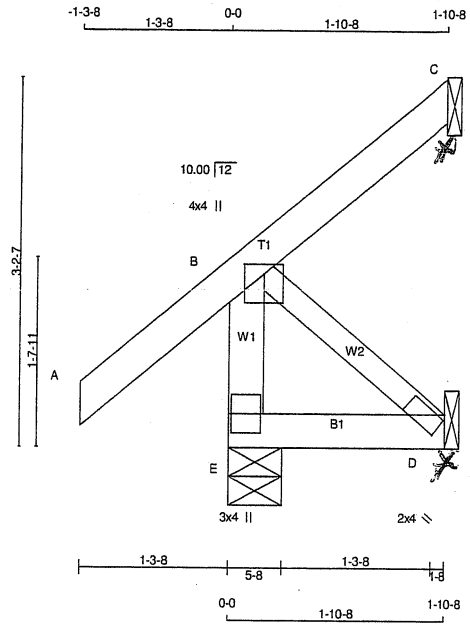

TOTAL WEIGHT = 11 X 10 = 111 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW1+w	MT20	2.0	4.0	3.25	
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	256	0	256	0	0	5-8	5-8
C	36	0	36	0	-33	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	179	132 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	25	20 / -24	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)
E- B	-241 / 0	0.0	0.0	0.03 (1)	
A- B	0 / 38	-84.9	-84.9	0.12 (1)	
B- C	-28 / 0	-84.9	-84.9	0.11 (1)	
E- D	0 / 0	-18.5	-18.5	0.02 (4)	

WEBS

MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
7.81	B- D	0 / 0	0.00 (1)
10.00			
6.25			
10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126521

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	J9C	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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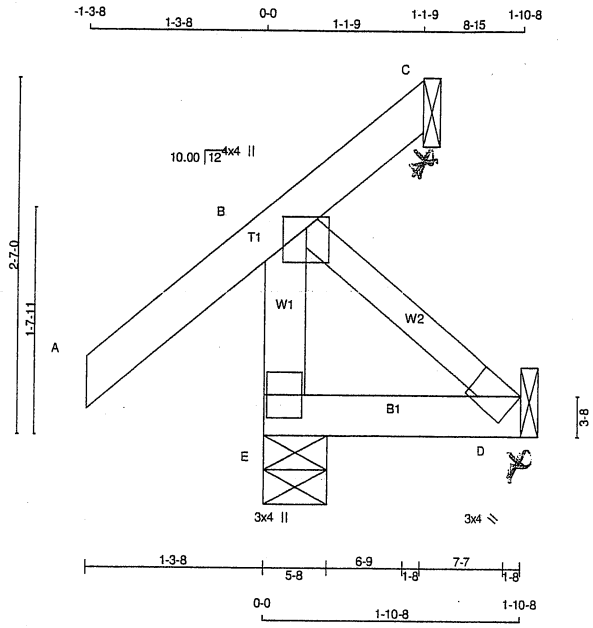

TOTAL WEIGHT = 2 X 9 = 18 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW1+w	MT20	3.0	4.0	3.00	Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	253	0	253	0	0	5-8	5-8
C	-24	0	0	0	-66	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	177	130 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	-16	0 / -39	0 / 0	0 / 0	0 / 0	0 / -3	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-237 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-46 / 0	-84.9	-84.9	0.11 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- D	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126522

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420381	J10	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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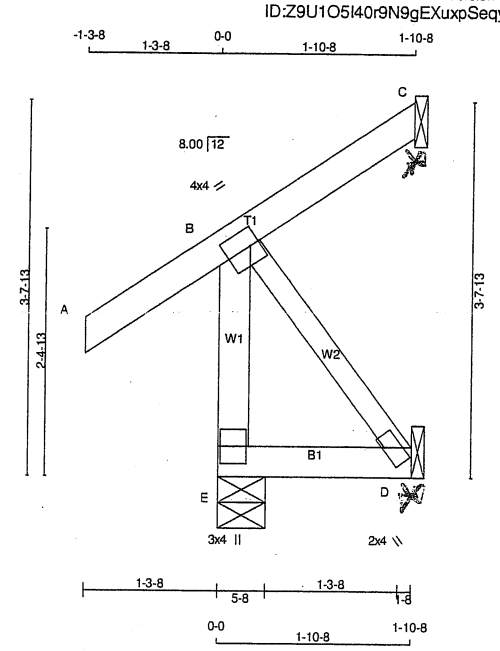

TOTAL WEIGHT = 3 X 11 = 33 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	BMW1+w	MT20	2.0	4.0	2.75	0.25
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	255	0	255	0	0	5-8	5-8
C	37	0	37	0	-33	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	178	131 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	25	20 / -23	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-239 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-24 / 0	-84.9	-84.9	0.11 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- D	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/05/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126523

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

LICENSED PROFESSIONAL ENGINEER
06/11/15
H.J.G. ALVES
PROVINCE OF ONTARIO

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes.

T-1800218 Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

[illegible]	[illegible]	[illegible]	LATERAL [illegible]		WITHDRAWAL [illegible]	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	**Common wire**	**Common spiral**	**Common wire**	**Common spiral**	**Gun Nail**
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
[illegible]	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

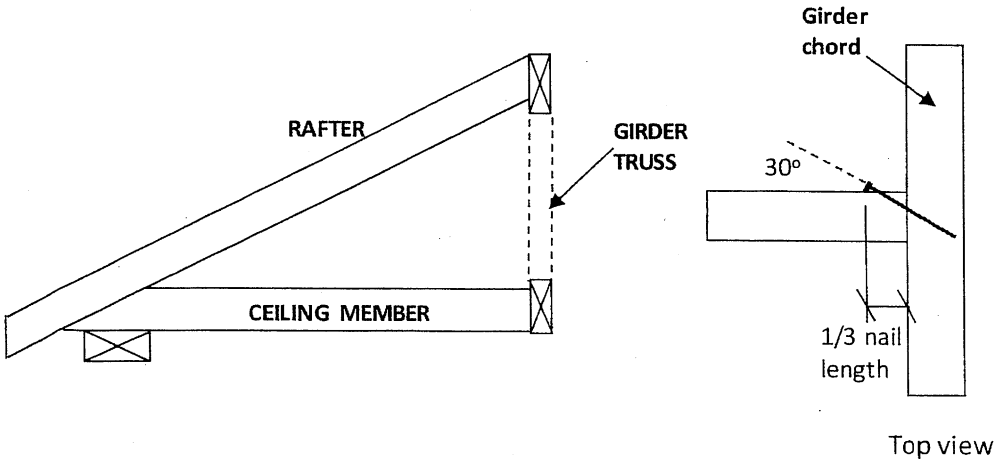

Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

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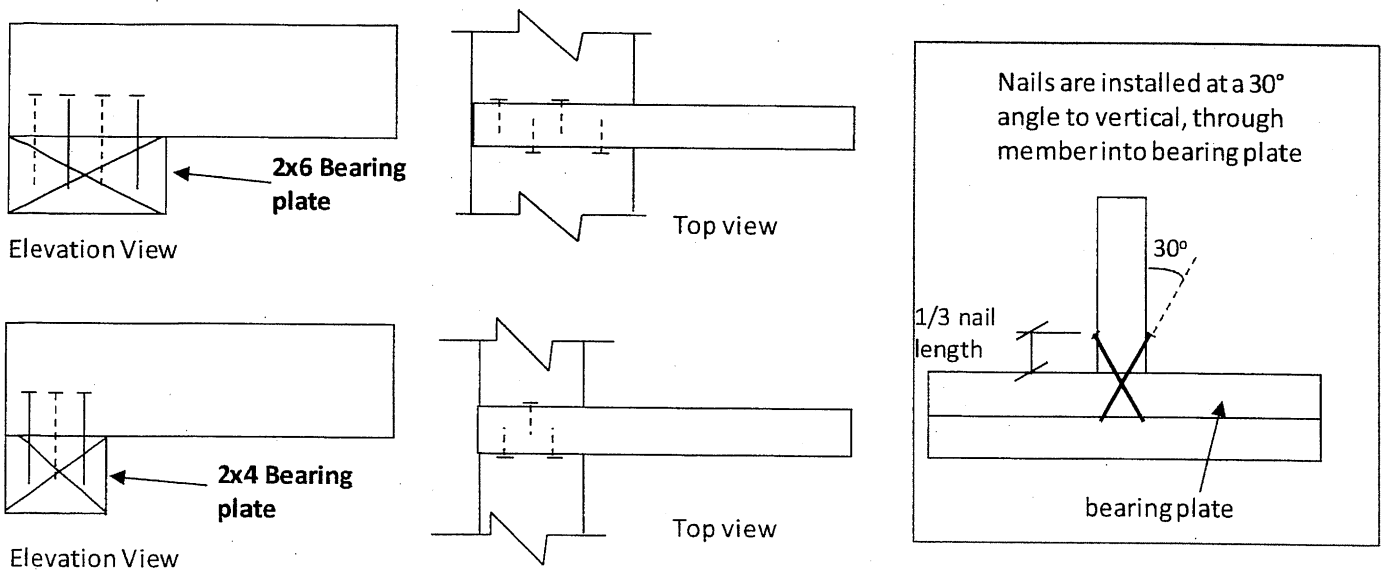
TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).

2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).

3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.

4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).

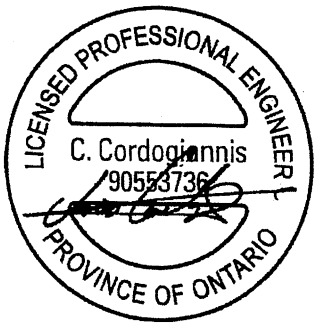
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.

6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.

7. Lumber must be dry (< 19% moisture content) at the time of nail installation.

8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



December 21, 2020

©2020 MiTek Canada Inc., 240 Stirling Crescent, Bradford, Ontario, L3Z 4L5 | (800) 268-3434, www.mitek.ca

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie®

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

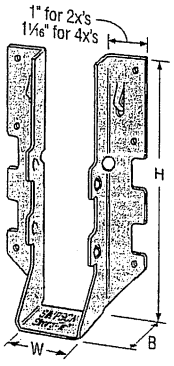
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

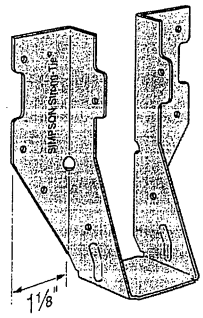
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

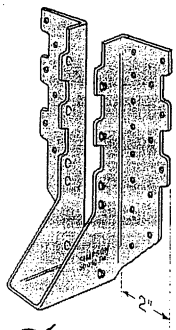
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



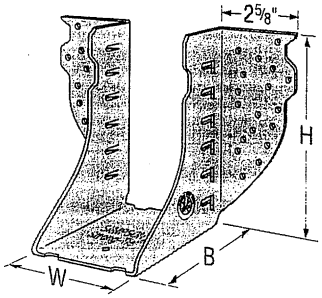
LUS28



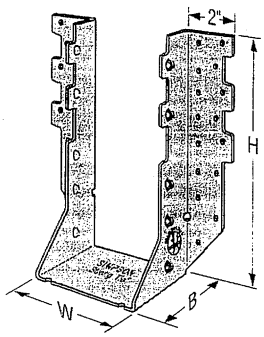
LU26L



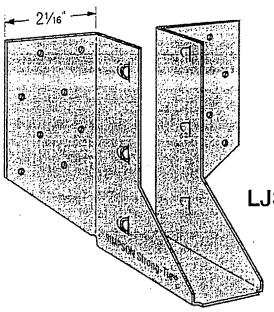
HUS210
(HUS26, HUS28, and HHUS similar)



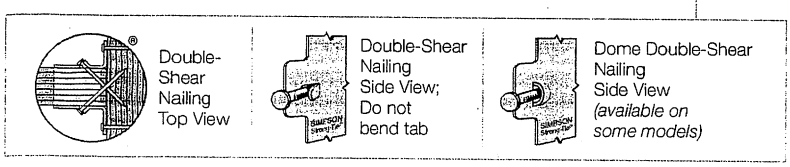
HGUS28-2

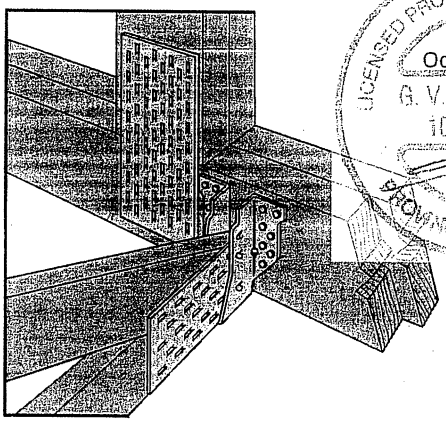


HHUS210-2



LJS26DS





Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)

LICENSED PROFESSIONAL ENGINEER
Oct. 14, 2020
G. V. LUBARSKY
100044463
PROVINCE OF ONTARIO

Plated Truss Connectors

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TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

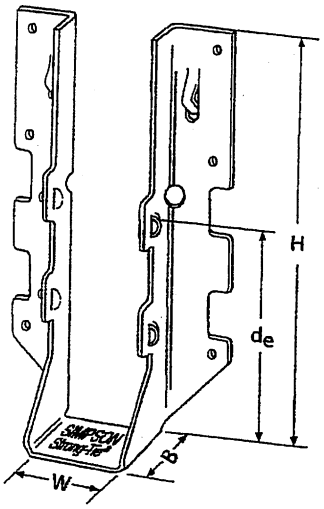
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

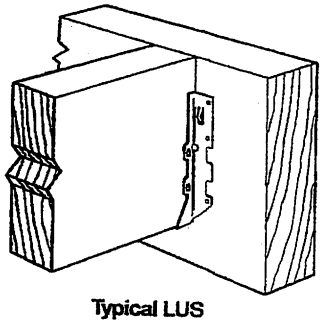
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



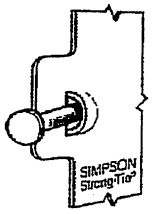
LUS28



Typical LUS Installation

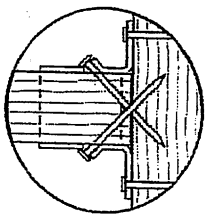
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3⅛	1¾	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4¾	1¾	3⅝	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4 3/16	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6⅝	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8 3/16	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

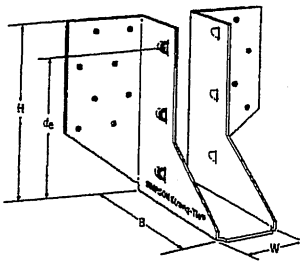
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

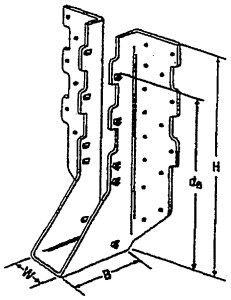
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

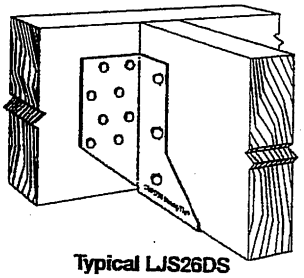
- See current catalogue for options



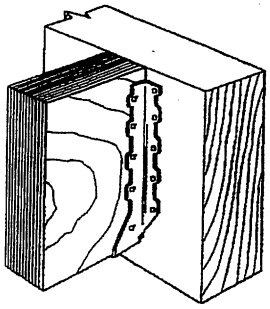
LJS26DS



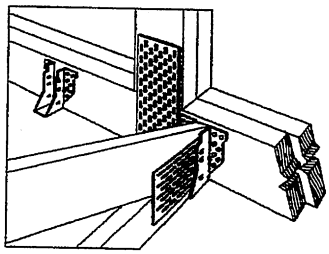
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



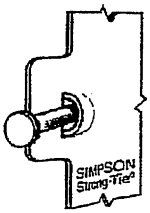
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

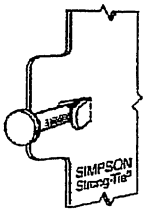
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 9/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 9/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

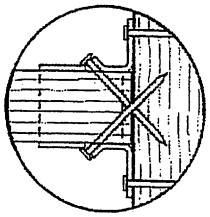


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

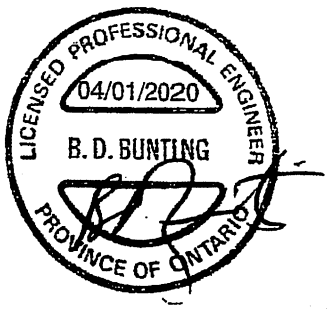
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

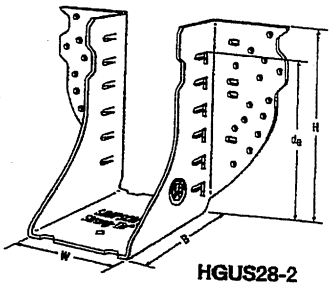
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

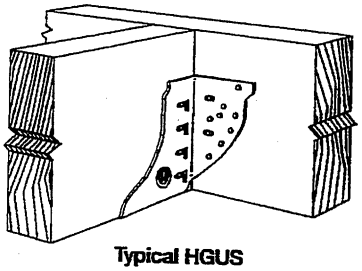
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

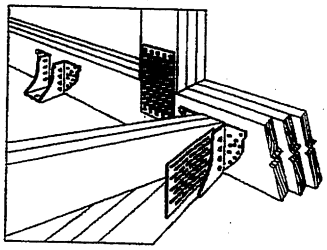
- See current catalogue for options



HGUS28-2



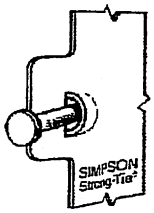
Typical HGUS Installation



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

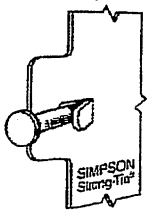
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

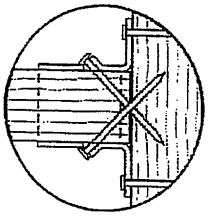


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

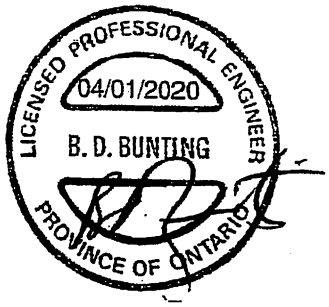
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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(800) 999-5099
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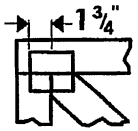
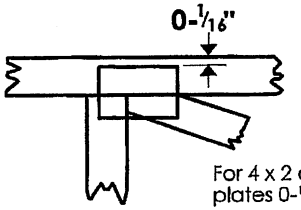
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

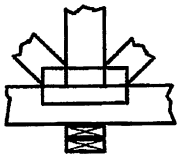
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

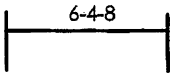


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

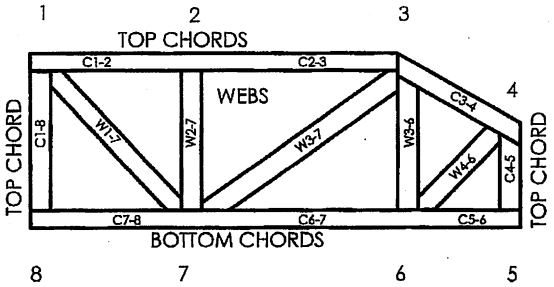
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.