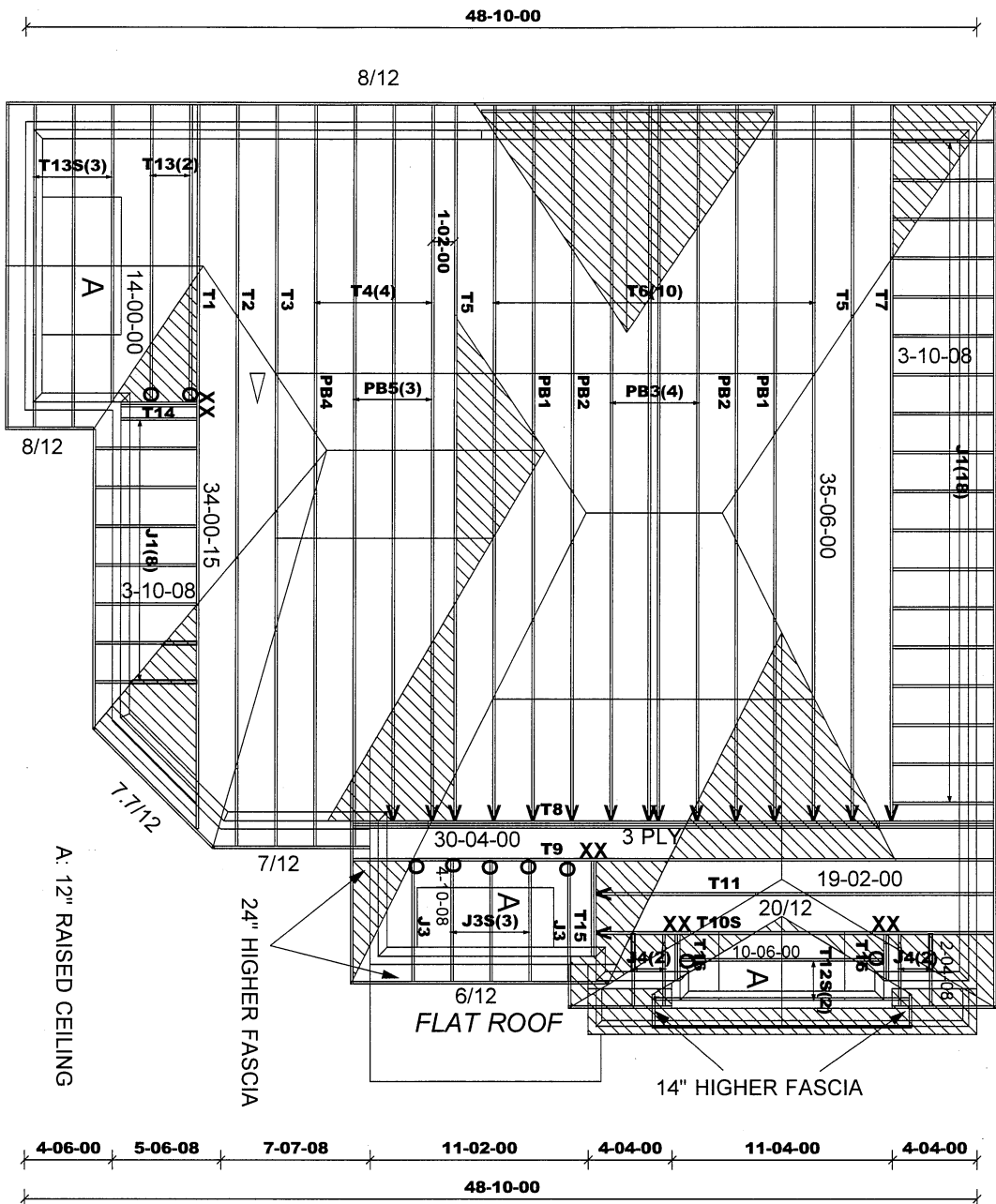


45-06-00
44-06-00

1-00-00
ALL ROOF PITCHES ARE 12/12
UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LUS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 27.1 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with
Part 9 of O.B.C. 2012. Roof rafters that
cross over or meet trusses to be min. 2x4
SPF #2 @ 24" o/c with a vertical post to the
truss at each cross point. Vertical posts
longer than 6' to have lateral bracing so that
the distance between the post end points
and lateral bracing does not exceed 6'.
[X]

M14234

Job Track: 52430

Plan Log: 205026

Layout ID: 420345

Builder / Location: ROYAL PINE HOMES / BRAMPTON

Project: VALES OF CASTLEMORE N.

Date: 2021-08-11

Sales: Rick DiCiano

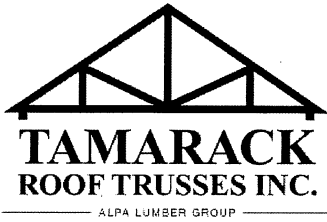
Designer: LC

Model / Elevation: UNIT 4100 / LOT 30

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Milek ver 8.4.2.286

DELIVERY SHIPLIST



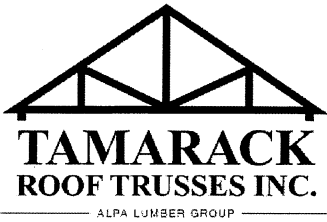
Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 4100 LOT 30
 Lot #:
 Elevation: ELEV. A

Job Track: 52430
 PlanLog: 205026
 Layout ID: 420345
 Ref #
 Page: 1 of 3
 Date: 08-11-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T1 Roof Special Girder	8 /12	34-00-15	5-09-00	2 x 6	1-03-08 1-09-15	1-04-13 1-04-07	181.92 111.50		
	1	T2 Hip	8 /12	35-06-00	7-09-00	2 x 4	1-03-08 1-03-08	1-04-13 1-03-07	153.76 97.67		
	1	T3 Hip	8 /12	35-06-00	9-09-00	2 x 4	1-03-08 1-03-08	1-04-13 1-03-07	162.59 101.33		
	4	T4 Piggyback Base	8 /12	35-06-00	9-09-00	2 x 4	1-03-08 1-03-08	1-04-13 1-03-07	653.34 409.33		
	2	T5 Roof Special	8 /12	35-06-00	7-09-00	2 x 4	1-03-08	1-04-13 6-07-08	339.93 217.33		
	10	T6 Roof Special	8 /12	35-06-00	9-09-00	2 x 4	1-03-08	1-04-13 6-07-08	1731.46 1088.33		
	1	T7 Half Hip Girder	8 /12	35-06-00	5-09-00	2 x 6	1-03-08	1-04-13 5-09-00	200.87 119.83		
	1 3-ply	T8 Hip Girder	12 /12	30-04-00	6-06-00	2 x 6	1-03-08 1-03-08	1-10-08 3-10-08	665.4 404.00		
	1	T9 Hip Girder	12 /12	30-04-00	5-07-04	2 x 4 2 x 6	1-03-08 1-03-08	1-10-08 3-10-08	185 112.00		
	1	T10S Hip Girder	12 /12	19-02-00	6-09-05	2 x 4 2 x 6	1-03-08	1-10-08 1-10-08	125.36 82.67		
	1	T11 Hip	12 /12	19-02-00	10-01-05	2 x 4	1-03-08	1-10-08 1-10-08	107.71 68.33		
	2	T12S Roof Special	12 /12	10-06-00	8-03-08	2 x 4	1-03-08 1-03-08	3-00-08 3-00-08	138.13 90.00		
	2	T13 Common	8 /12	14-00-00	6-00-13	2 x 4	1-03-08	1-04-13 1-04-13	117.47 74.00		
	3	T13S Roof Special	8 /12	14-00-00	6-00-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	207.55 139.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 4100 LOT 30
 Lot #:
 Elevation: ELEV. A

Job Track: 52430
 PlanLog: 205026
 Layout ID: 420345
 Ref #
 Page: 2 of 3
 Date: 08-11-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T14 Monopitch Girder	12 /12	3-10-08	5-09-00	2 x 4 2 x 6		1-10-08 5-09-00	49.04 30.67		
	1 2-ply	T15 Monopitch Girder	6 /12	4-10-08	5-07-04	2 x 4 2 x 6		3-02-00 5-07-04	58.54 35.67		
	2 2-ply	T16 Monopitch Girder	20 /12	2-04-08	5-11-08	2 x 4 2 x 6		2-09-13 5-11-08	104.27 67.33		
	2	PB1 Piggyback	8 /12	16-08-12	2-00-00	2 x 4			96.47 62.67		
	2	PB2 Piggyback	8 /12	16-08-12	4-00-00	2 x 4			100.7 65.00		
	4	PB3 Piggyback	8 /12	16-08-12	4-09-06	2 x 4			202.26 130.00		
	1	PB4 Piggyback	8 /12	8-05-10	2-00-00	2 x 4			24.2 16.50		
	3	PB5 Piggyback	8 /12	8-05-10	2-07-10	2 x 4			62.12 39.00		
	26	J1 Jack-Open	12 /12	3-10-08	5-09-00	2 x 4	1-03-08	1-10-08 5-09-00	425.73 268.67		
	2	J3 Jack-Open	6 /12	4-10-08	5-07-04	2 x 4	1-03-08	3-02-00 5-07-04	52.95 34.00		
	3	J3S Jack-Open	6 /12	4-10-08	5-07-04	2 x 4	1-03-08	2-02-00 5-07-04	102.09 68.00		
	4	J4 Jack-Open	20 /12	2-04-08	6-09-05	2 x 4	1-03-08	2-09-13 6-09-05	65.98 43.33		

TOTAL # TRUSS= 88

TOTAL BFT OF ALL TRUSSES= 3976.16

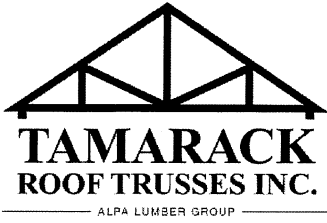
BFT.

TOTAL WEIGHT OF ALL TRSSES 6314.8 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	HGUS26-2	
17	Hardware	LJS26DS	

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: VALES OF CASTLEMORE NORTH
 Location: BRAMPTON
 Model: UNIT 4100 LOT 30
 Lot #:
 Elevation: ELEV. A

Job Track: 52430
 PlanLog: 205026
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 Page: 3 of 3
 Date: 08-11-2021
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HARDWARE

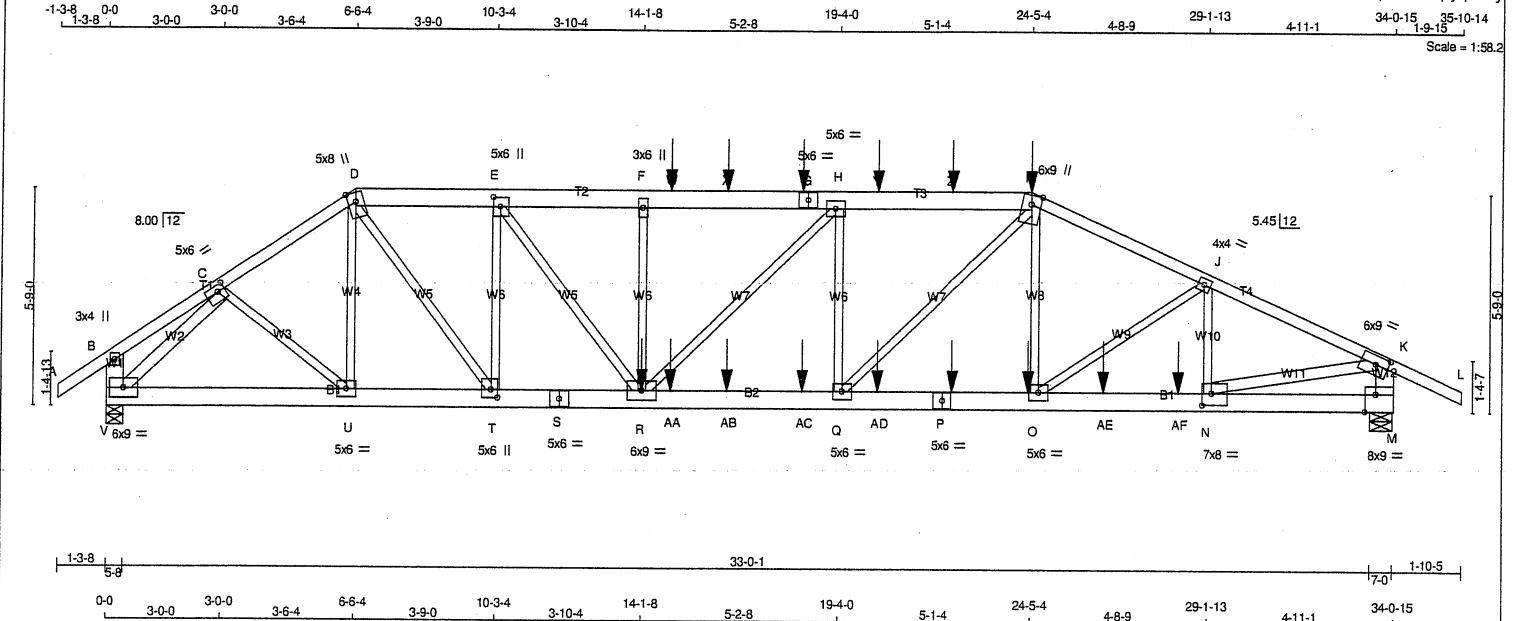
QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **25**

JOB NAME 420345	TRUSS NAME T1	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 11:37:37 2021 Page 1
ID:S8hvR8eJge955Hpp7HWlpPyrYaD-3Py9BHYbvrTHBPV9TsSi13ButlebVplWzNihpyqwWY



TOTAL WEIGHT = 182 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
I - L	2x4	DRY	2100F 1.8E
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
N - K	2x4	DRY	No.2
V - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	3.0	4.0		
C	TMWV-t	MT20	5.0	6.0	2.00	2.25
D	TTWW+m	MT20	5.0	8.0	Edge	
E	TMWV-t	MT20	5.0	6.0	3.00	2.25
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWV-t	MT20	5.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	
J	TMWV-t	MT20	4.0	4.0		
K	TMWV-t	MT20	6.0	9.0	2.75	4.00
M	BMV1-t	MT20	8.0	9.0	Edge	3.50
N	BMWV-t	MT20	7.0	8.0	3.75	3.00
Q, O, U						
O	BMWV-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
R	BMWV-t	MT20	6.0	9.0		
S	BS-t	MT20	5.0	6.0		
T	BMWV-t	MT20	5.0	6.0	2.50	2.00
V	BMV1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
M	3917	0	0	7-0
V	3053	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	
	COMBINED	SNOW	LIVE	PERM.LIVE
M	2773	1804 / 0	0 / 0	0 / 0
V	2161	1405 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE	FACTORED		MEMB.	FORCE	FACTORED	
	(LBS)	VERT. LOAD	MAX		(LBS)	MAX	
FR-TO		FROM TO	LENGTH	FR-TO		FROM TO	LENGTH
A-B	0 / 32	-84.9	-84.9 0.13 (1)	10.00	U-D	-249 / 0	0.12 (1)
B-C	0 / 16	-84.9	-84.9 0.13 (1)	10.00	O-I	0 / 283	0.09 (4)
C-D	-3835 / 0	-84.9	-84.9 0.39 (1)	3.27	O-J	-115 / 0	0.06 (1)
D-E	-4680 / 0	-84.9	-84.9 0.24 (1)	3.84	N-K	-858 / 0	0.19 (1)
E-F	-5890 / 0	-84.9	-84.9 0.29 (1)	3.42	N-K	0 / 5180	0.92 (1)
F-W	-5890 / 0	-84.9	-84.9 0.47 (1)	3.23	D-T	0 / 2575	0.64 (1)
W-X	-5890 / 0	-84.9	-84.9 0.47 (1)	3.23	R-F	-490 / 0	0.22 (1)
X-G	-5890 / 0	-84.9	-84.9 0.47 (1)	3.23	C-U	0 / 560	0.14 (1)
G-H	-5890 / 0	-84.9	-84.9 0.47 (1)	3.23	V-C	-3925 / 0	0.77 (1)
H-Y	-5888 / 0	-84.9	-84.9 0.46 (1)	3.23	T-E	-2008 / 0	0.91 (1)
Y-Z	-5888 / 0	-84.9	-84.9 0.46 (1)	3.23	E-R	0 / 2056	0.51 (1)
Z-I	-5888 / 0	-84.9	-84.9 0.46 (1)	3.23	R-H	0 / 11	0.00 (4)
I-J	-5500 / 0	-170.4	-170.4 0.70 (1)	3.08	Q-G	-788 / 0	0.36 (1)
J-K	-5543 / 0	-170.4	-170.4 0.72 (1)	3.06	Q-I	0 / 1311	0.32 (1)
K-L	0 / 33	-84.9	-84.9 0.15 (1)	10.00			
V-B	-210 / 0	0.0	0.0 0.02 (1)	7.81			
M-K	-3810 / 0	0.0	0.0 0.27 (1)	5.43			

V-U	0 / 2760	-18.5	-18.5 0.41 (1)	10.00
U-T	0 / 3196	-18.5	-18.5 0.45 (1)	10.00
T-S	0 / 4680	-18.5	-18.5 0.68 (1)	10.00
S-R	0 / 4680	-18.5	-18.5 0.68 (1)	10.00
R-AA	0 / 5887	-18.5	-18.5 0.87 (1)	10.00
AA-AB	0 / 5887	-18.5	-18.5 0.87 (1)	10.00
AB-AC	0 / 5887	-18.5	-18.5 0.87 (1)	10.00
AC-Q	0 / 5887	-18.5	-18.5 0.87 (1)	10.00
Q-AD	0 / 4980	-18.5	-18.5 0.70 (1)	10.00
AD-P	0 / 4980	-18.5	-18.5 0.70 (1)	10.00
P-O	0 / 4980	-18.5	-18.5 0.70 (1)	10.00
O-AE	0 / 5080	-37.3	-37.3 0.78 (1)	10.00
AE-AF	0 / 5080	-37.3	-37.3 0.78 (1)	10.00
AF-N	0 / 5080	-37.3	-37.3 0.78 (1)	10.00
N-M	0 / 0	-37.3	-37.3 0.12 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	18-4-8	-55	-55	---	FRONT	VERT	TOTAL	---	C1
I	24-5-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
I	24-5-4	-61	-61	---	FRONT	VERT	TOTAL	---	C1
I	24-5-4	-233	-233	---	FRONT	VERT	TOTAL	---	C1
O	24-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
P	22-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
R	14-1-8	-792	-792	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.14")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.14")
CALCULATED VERT. DEFL.(TL) = L/947 (0.43")

CSI: TO=0.72/1.00 (J-K:1), BC=0.87/1.00 (Q-R:1), WB=0.92/1.00 (K-N:1), SS=0.40/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.92 (K) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2126700 112

JOB NAME 420345	TRUSS NAME T1	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	14-10-8	-55	-55	---	FRONT	VERT	TOTAL	---	C1
X	16-4-8	-55	-55	---	FRONT	VERT	TOTAL	---	C1
Y	20-4-8	-55	-55	---	FRONT	VERT	TOTAL	---	C1
Z	22-4-8	-55	-55	---	FRONT	VERT	TOTAL	---	C1
AA	14-10-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AB	16-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AC	18-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AD	20-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AE	26-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1
AF	28-4-8	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

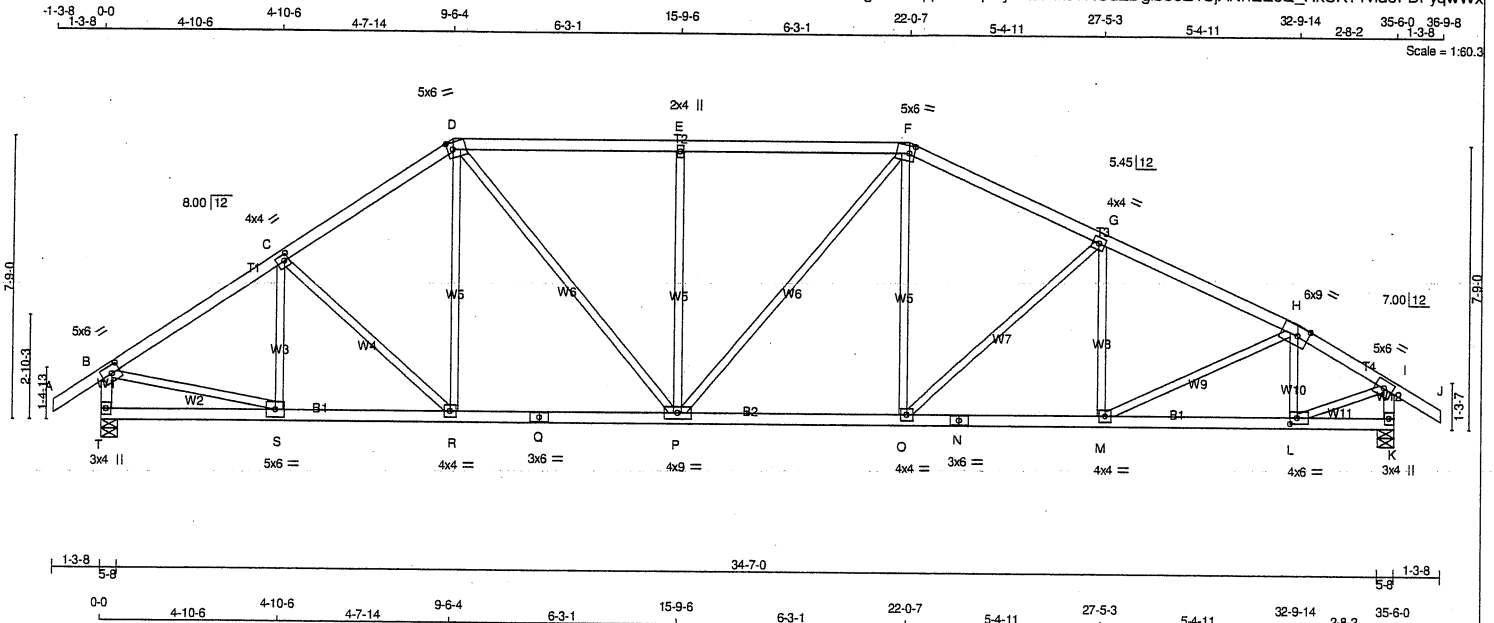


Structural component only
DWG# T-2126700 *M*

JOB NAME 420345	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 11:37:38 2021 Page 1
ID:S8hvR8eJge955Hpp7HWlpPyrYaD-XcVXOdZDgib8oZ4UjANhEEcQ_HkUK?1vld6FDfyqwWx



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
T - Q	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	1.75
G	TMVW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	3.00	3.50
I	TMVW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		
L	BMVW-t	MT20	4.0	6.0	2.00	2.25
M, O, R						
N	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
S	BMVW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
T	1951	0	1951	0	0	5-8	5-8		
K	1951	0	1951	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1383	891 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0
K	1382	890 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CSI) (LC)	MEMB.	FORCE (LBS)	MAX (CSI) (LC)	UNBRACED LENGTH (LC)
FR-TO				FR-TO			
A-B	0 / 32	-84.9	-84.9 0.11 (1)	10.00	S-C	-320 / 0	0.10 (1)
B-C	-2184 / 0	-84.9	-84.9 0.31 (1)	4.34	C-R	-157 / 0	0.12 (1)
C-D	-2099 / 0	-84.9	-84.9 0.30 (1)	4.42	R-D	0 / 212	0.05 (4)
D-E	-2217 / 0	-84.9	-84.9 0.51 (1)	4.05	D-P	0 / 765	0.17 (1)
E-F	-2217 / 0	-84.9	-84.9 0.51 (1)	4.05	P-E	-651 / 0	0.73 (1)
F-G	-2287 / 0	-84.9	-84.9 0.40 (1)	4.16	P-F	0 / 236	0.05 (1)
G-H	-2567 / 0	-84.9	-84.9 0.42 (1)	3.94	O-F	0 / 380	0.09 (1)
H-I	-2110 / 0	-84.9	-84.9 0.13 (1)	4.59	O-G	-400 / 0	0.44 (1)
I-J	0 / 29	-84.9	-84.9 0.11 (1)	10.00	M-G	-163 / 24	0.07 (1)
T-B	-1911 / 0	0.0	0.0 0.20 (1)	6.08	M-H	0 / 617	0.14 (1)
K-I	-1931 / 0	0.0	0.0 0.20 (1)	6.06	L-H	-559 / 0	0.10 (1)
					B-S	0 / 1883	0.42 (1)
T-S	0 / 0	-18.5	-18.5 0.09 (4)	10.00	L-I	0 / 1929	0.43 (1)
S-R	0 / 1838	-18.5	-18.5 0.36 (1)	10.00			
R-Q	0 / 1725	-18.5	-18.5 0.35 (1)	10.00			
Q-P	0 / 1725	-18.5	-18.5 0.35 (1)	10.00			
P-O	0 / 2065	-18.5	-18.5 0.40 (1)	10.00			
O-N	0 / 2357	-18.5	-18.5 0.45 (1)	10.00			
N-M	0 / 2357	-18.5	-18.5 0.45 (1)	10.00			
M-L	0 / 1798	-18.5	-18.5 0.34 (1)	10.00			
L-K	0 / 0	-18.5	-18.5 0.07 (4)	10.00			

TOTAL WEIGHT = 154 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.51/1.00 (E-F:1), BC=0.45/1.00 (M-O:1), WB=0.73/1.00 (E-P:1), SS=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.69 (N) (INPUT = 1.00)

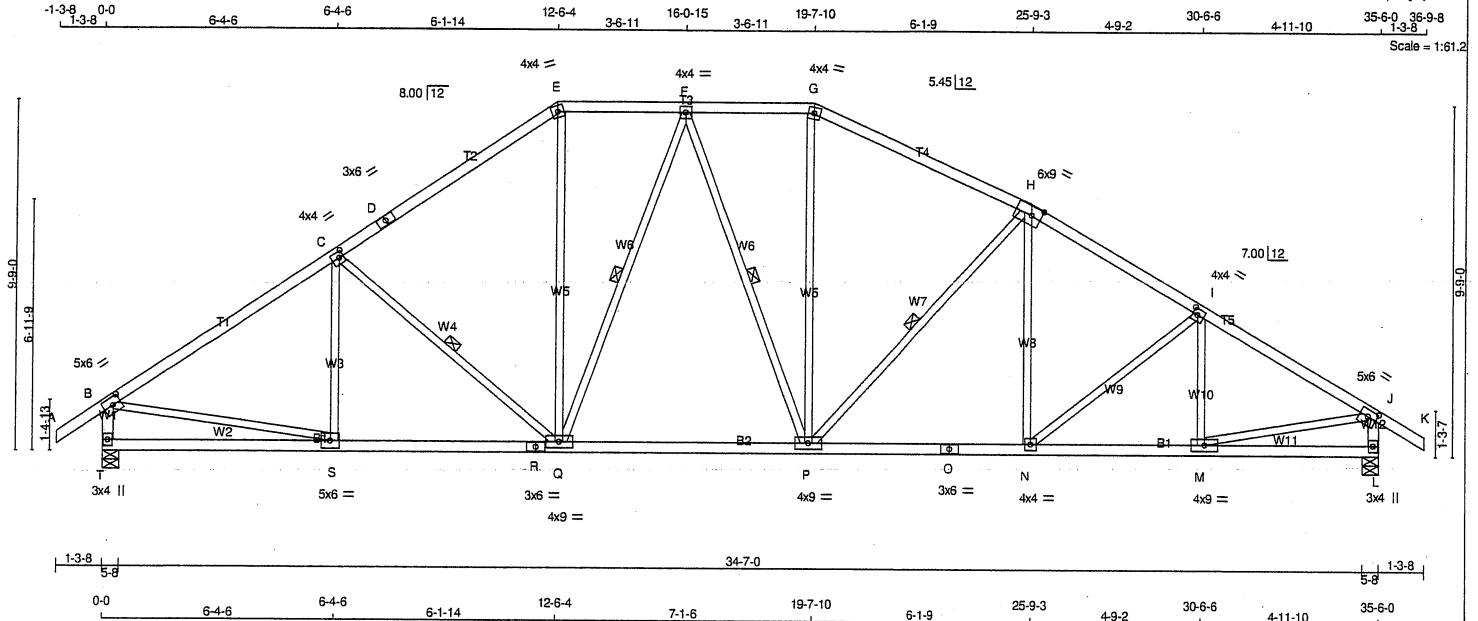


Structural component only
DWG# T-2126701

JOB NAME 420345	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:S8hvR8eJge955Hpp7HWlpPyrYaD-?o3wbzasR2?QjffGuuwnS8amh5M3W72zHsplhyqwwWw



TOTAL WEIGHT = 163 lb
[M][F]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-t	MT20	5.0	6.0	2.50	2.75
C	TMWV-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWV-m	MT20	4.0	4.0		
F	TMWV-t	MT20	4.0	4.0		
G	TTWV-m	MT20	4.0	4.0		
H	TTWV-m	MT20	6.0	9.0	3.00	3.25
I	TMWV-t	MT20	4.0	4.0	2.00	1.75
J	TMWV-t	MT20	5.0	6.0	2.25	3.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWVW-t	MT20	4.0	9.0		
N	BMWVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWVW-t	MT20	4.0	9.0		
Q	BMWVW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWVW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORIZ	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
T	1951	0	1951	0	0	5-8	5-8	5-8	5-8
L	1951	0	1951	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	COMBINED	SNOW	0/0	0/0	0/0	492/0	0/0
L	1382	891/0	0/0	0/0	0/0	492/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, H-F, P-Q, F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	UNBRAC
FR-TO		FROM TO		FR-TO			
A-B	0/32	-84.9	-84.9 0.11 (1)	S-C	-195/28	0.09 (1)	10.00
B-C	-2233/0	-84.9	-84.9 0.57 (1)	C-Q	-405/0	0.19 (1)	4.02
C-D	-1935/0	-84.9	-84.9 0.52 (1)	Q-E	0/734	0.17 (1)	4.31
D-E	-1935/0	-84.9	-84.9 0.52 (1)	P-G	0/396	0.09 (1)	4.31
E-F	-1586/0	-84.9	-84.9 0.15 (1)	P-H	-362/0	0.21 (1)	5.11
F-G	-1711/0	-84.9	-84.9 0.15 (1)	N-H	0/211	0.05 (4)	4.96
G-H	-1876/0	-84.9	-84.9 0.48 (1)	N-I	-179/0	0.12 (1)	4.39
H-I	-2284/0	-84.9	-84.9 0.32 (1)	M-I	-316/0	0.08 (1)	4.25
I-J	-2397/0	-84.9	-84.9 0.34 (1)	B-S	0/1914	0.43 (1)	4.16
J-K	0/29	-84.9	-84.9 0.11 (1)	M-J	0/2131	0.48 (1)	10.00
T-B	-1901/0	0.0	0.0 0.20 (1)	Q-F	-372/0	0.28 (1)	6.09
L-J	-1909/0	0.0	0.0 0.19 (1)	F-P	-6/32	0.01 (4)	6.08
T-S	0/0	-18.5	-18.5 0.16 (4)				10.00
S-R	0/1887	-18.5	-18.5 0.39 (1)				10.00
R-Q	0/1887	-18.5	-18.5 0.39 (1)				10.00
Q-P	0/1714	-18.5	-18.5 0.37 (1)				10.00
P-O	0/1954	-18.5	-18.5 0.41 (1)				10.00
O-N	0/1954	-18.5	-18.5 0.41 (1)				10.00
N-M	0/2091	-18.5	-18.5 0.39 (1)				10.00
M-L	0/0	-18.5	-18.5 0.09 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.18")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.18")
CALCULATED VERT. DEFL.(TL)= L/999 (0.20")

CSI: TC=0.57/1.00 (B-C:1), BC=0.41/1.00 (N-P:1), WB=0.48/1.00 (J-M:1), SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

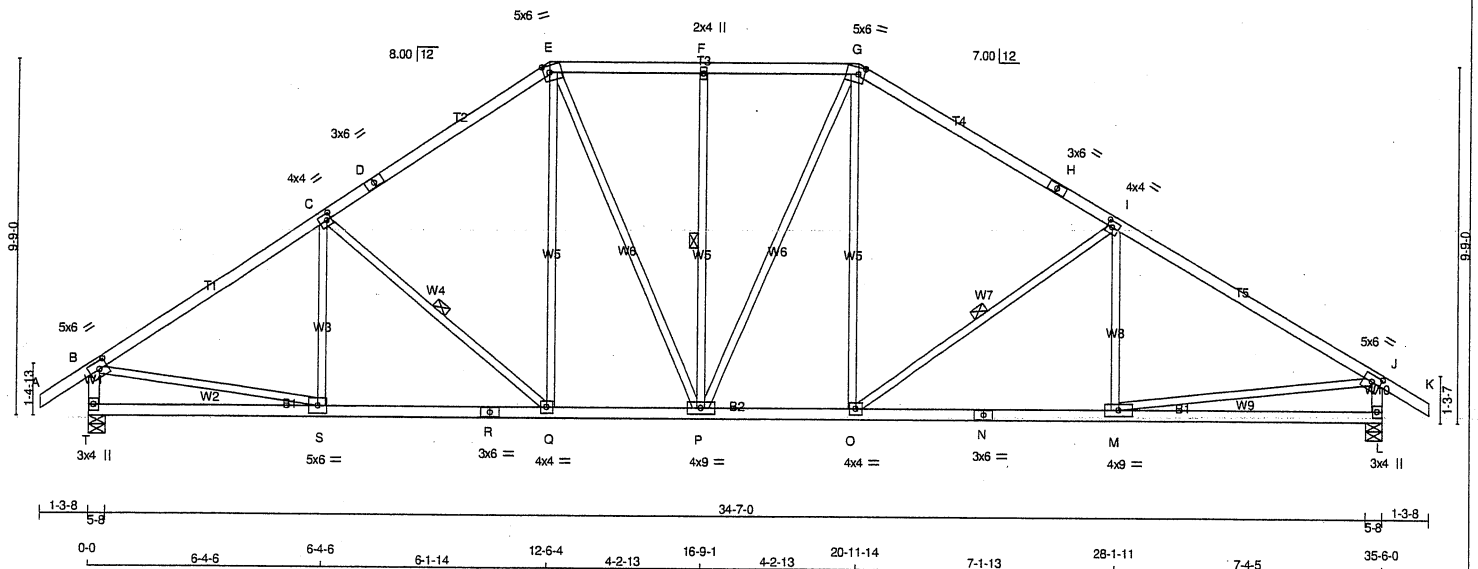
JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.63 (O) (INPUT = 1.00)



Structural component only
DWG# T-2126702

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420345	T4	4	1	ROYAL PINE HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 Scale = 1:80.3



LUMBER				DESCR.	
N.L.G.A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
T - R	2x4	DRY	No.2	SPF	
R - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	5.0	6.0	2.50 2.75
C	TMVW-t	MT20	4.0	4.0	2.00 1.50
D	TS-t	MT20	3.0	6.0	
E	TTWW-m	MT20	5.0	6.0	2.25 2.00
F	TMVW-w	MT20	2.0	4.0	
G	TTWW-m	MT20	5.0	6.0	2.25 2.00
H	TS-t	MT20	3.0	6.0	
I	TMVW-t	MT20	4.0	4.0	2.00 1.75
J	TMVW-t	MT20	5.0	6.0	2.25 3.00
L	BMV1+p	MT20	3.0	4.0	
M	BMVW-t	MT20	4.0	9.0	
N	BS-t	MT20	3.0	6.0	
O	BMVW-t	MT20	4.0	4.0	
P	BMVW-t	MT20	4.0	9.0	
Q	BMVW-t	MT20	4.0	4.0	
R	BS-t	MT20	3.0	6.0	
S	BMVW-t	MT20	5.0	6.0	
T	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORIZ	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
T	1951	0	1951	0	0	5-8	5-8		
L	1951	0	1951	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1383	891 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0
L	1382	890 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0 / 32	-84.9 -84.9	0.11 (1)	S-C	-190 / 34
B-C	-2234 / 0	-84.9 -84.9	0.57 (1)	C-Q	-410 / 0
C-D	-1932 / 0	-84.9 -84.9	0.52 (1)	Q-E	0 / 366
D-E	-1932 / 0	-84.9 -84.9	0.52 (1)	E-P	0 / 381
E-F	-1736 / 0	-84.9 -84.9	0.22 (1)	P-F	-431 / 0
F-G	-1736 / 0	-84.9 -84.9	0.22 (1)	P-G	0 / 155
G-H	-1970 / 0	-84.9 -84.9	0.72 (1)	O-G	0 / 463
H-I	-1970 / 0	-84.9 -84.9	0.72 (1)	O-I	-600 / 0
I-J	-2455 / 0	-84.9 -84.9	0.82 (1)	M-I	-132 / 72
J-K	0 / 29	-84.9 -84.9	0.11 (1)	B-S	0 / 1915
T-B	-1902 / 0	0.0 0.0	0.20 (1)	M-J	0 / 2172
L-J	-1895 / 0	0.0 0.0	0.19 (1)		
T-S	0 / 0	-18.5 -18.5	0.18 (4)		
S-R	0 / 1888	-18.5 -18.5	0.39 (1)		
R-Q	0 / 1888	-18.5 -18.5	0.39 (1)		
Q-P	0 / 1580	-18.5 -18.5	0.31 (1)		
P-O	0 / 1673	-18.5 -18.5	0.34 (1)		
O-N	0 / 2153	-18.5 -18.5	0.47 (1)		
N-M	0 / 2153	-18.5 -18.5	0.47 (1)		
M-L	0 / 0	-18.5 -18.5	0.24 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.18")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
 ALLOWABLE DEFL.(TL)= L/360 (1.18")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.82/1.00 (I-J:1), BC=0.47/1.00 (M-O:1), WB=0.49/1.00 (J-M:1), SSI=0.26/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

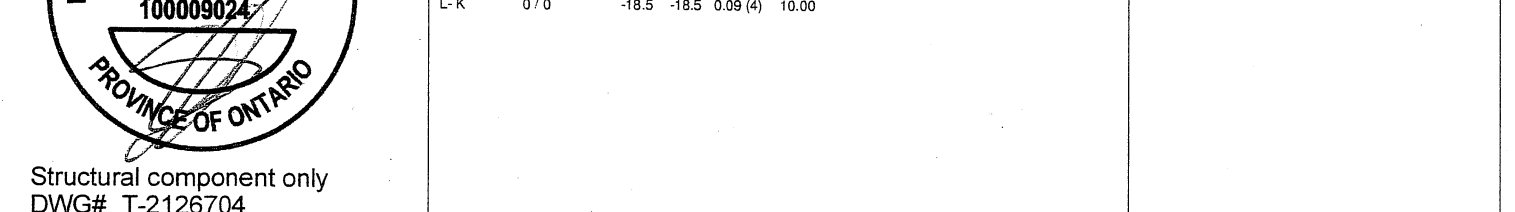
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
 JSI METAL= 0.73 (N) (INPUT = 1.00)



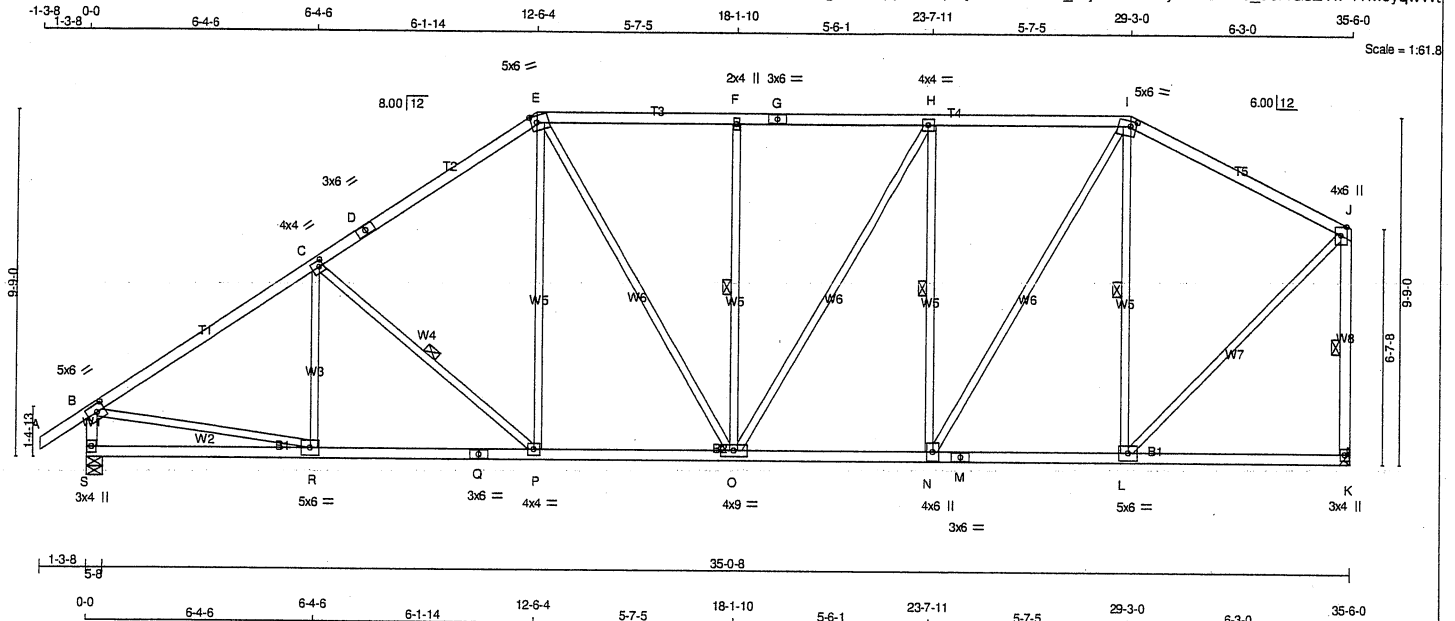
Structural component only
 DWG# T-2126703



JOB NAME 420345	TRUSS NAME T6	QUANTITY 10	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:S8hvrR8eJge955Hpp7HWlpPyrYaD-PNIE_cjz5aHANFy0SdO4m5_u6NGsZVfF4TM0yqWt



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
S - B	2x4	DRY	No.2
K - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	5.0	6.0 2.50 2.75
C	TMVW-t	MT20	4.0	4.0 2.00 1.50
D	TS-t	MT20	3.0	6.0
E	TTWW-m	MT20	5.0	6.0 2.25 2.00
F	TMVW-w	MT20	2.0	4.0
G	TS-t	MT20	3.0	6.0
H	TMVW-t	MT20	4.0	4.0
I	TTWW-m	MT20	5.0	6.0 1.75 2.00
J	TMVW-p	MT20	4.0	6.0 Edge
K	BMV1-p	MT20	3.0	4.0
L	BMVW-t	MT20	5.0	6.0
M	BS-t	MT20	3.0	6.0
N	BMVW-t	MT20	4.0	6.0
O	BMVW-t	MT20	4.0	9.0
P	BMVW-t	MT20	4.0	4.0
Q	BS-t	MT20	3.0	6.0
R	BMVW-t	MT20	5.0	6.0
S	BMV1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORIZ	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
S	1951	0	1951	0	0	5-8	5-8		
K	1835	0	1835	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1383	891 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0
K	1302	827 / 0	0 / 0	0 / 0	0 / 0	476 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-O, H-N, I-L, J-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)
A-B	0 / 32	-84.9	-84.9 0.11 (1)	R-C	-192 / 32	0.09 (1)	
B-C	-2234 / 0	-84.9	-84.9 0.57 (1)	C-P	-408 / 0	0.19 (1)	
C-D	-1933 / 0	-84.9	-84.9 0.52 (1)	P-E	0 / 374	0.08 (1)	
D-E	-1933 / 0	-84.9	-84.9 0.52 (1)	E-O	0 / 334	0.08 (1)	
E-F	-1751 / 0	-84.9	-84.9 0.35 (1)	O-F	-511 / 0	0.33 (1)	
F-G	-1751 / 0	-84.9	-84.9 0.32 (1)	O-H	0 / 375	0.08 (1)	
G-H	-1751 / 0	-84.9	-84.9 0.32 (1)	H-N	-840 / 0	0.54 (1)	
H-I	-1563 / 0	-84.9	-84.9 0.34 (1)	N-I	0 / 1086	0.24 (1)	
I-J	-1139 / 0	-84.9	-84.9 0.45 (1)	L-I	-883 / 0	0.56 (1)	
S-B	-1902 / 0	0.0	0.0 0.20 (1)	B-R	0 / 1915	0.43 (1)	
K-J	-1788 / 0	0.0	0.0 0.37 (1)	L-J	0 / 1436	0.32 (1)	
S-R	0 / 0	-18.5	-18.5 0.17 (4)				
R-Q	0 / 1888	-18.5	-18.5 0.39 (1)				
Q-P	0 / 1888	-18.5	-18.5 0.39 (1)				
P-O	0 / 1581	-18.5	-18.5 0.32 (1)				
O-N	0 / 1563	-18.5	-18.5 0.30 (1)				
N-M	0 / 1008	-18.5	-18.5 0.26 (1)				
M-L	0 / 1008	-18.5	-18.5 0.26 (1)				
L-K	0 / 0	-18.5	-18.5 0.17 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.57/1.00 (B-C:1), BC=0.39/1.00 (P-R:1), WB=0.56/1.00 (I-L:1), SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

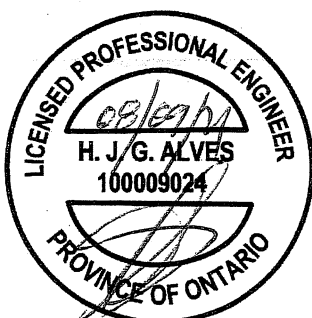
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.59 (J) (INPUT = 1.00)

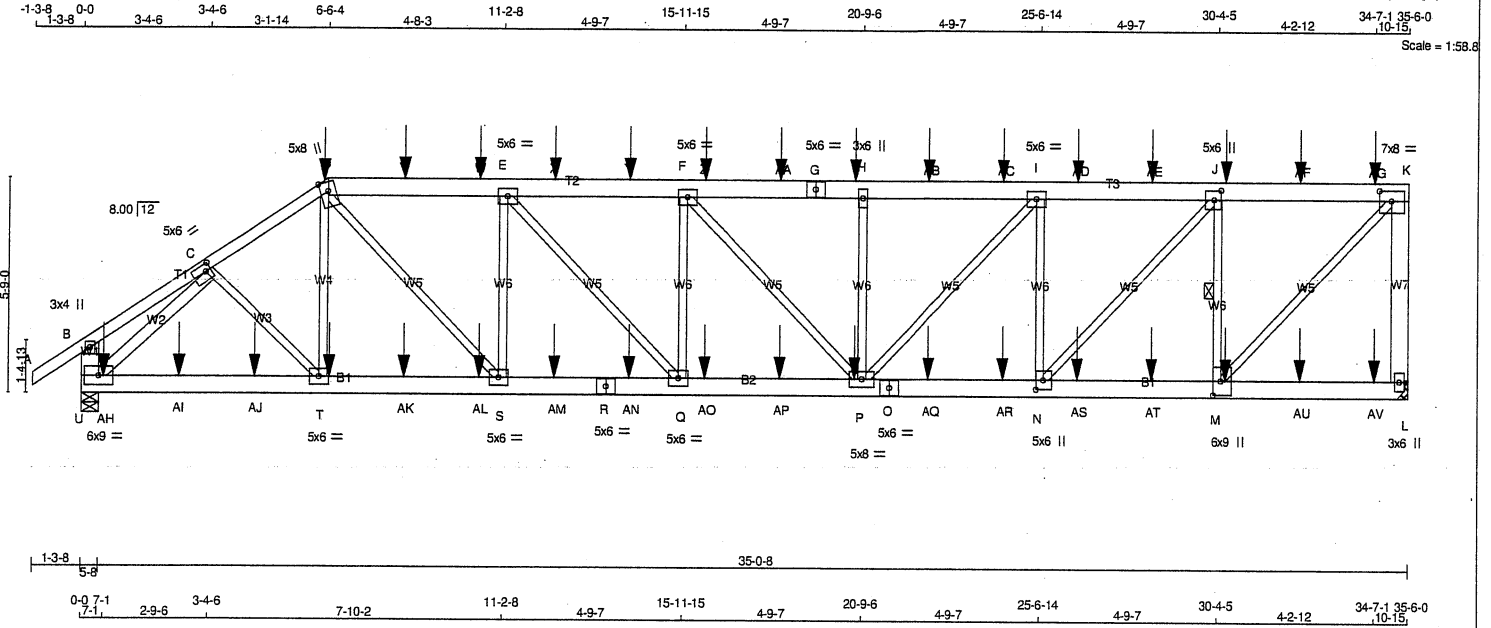


Structural component only
DWG# T-2126705

JOB NAME 420345	TRUSS NAME T7	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x6	DRY	No.2
G - K	2x6	DRY	No.2
L - K	2x6	DRY	No.2
U - B	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
U - C	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (Table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMW+p	MT20	3.0	4.0
C	TMW+W-t	MT20	5.0	6.0 2.25 1.75
D	TTWW+m	MT20	5.0	8.0 Edge
E, F, I				
E	TMW+W-t	MT20	5.0	6.0
G	TS-t	MT20	5.0	6.0
H	TMW+w	MT20	3.0	6.0
J	TMW+W-t	MT20	5.0	6.0 3.00 2.25
K	TMW+W-t	MT20	7.0	8.0 3.25 4.00
L	BMV1+p	MT20	3.0	6.0
M	BMV+W-t	MT20	6.0	9.0 4.50 2.50
N	BMV+W-t	MT20	5.0	6.0 3.00 2.25
O	BS-t	MT20	5.0	6.0
P	BMV+W-t	MT20	5.0	8.0
Q, S, T				
Q	BMV+W-t	MT20	5.0	6.0
R	BS-t	MT20	5.0	6.0
U	BMV+W-t	MT20	6.0	9.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT	
GROSS REACTION		GROSS REACTION		BRG	REQD
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
L	2739	0	2739	0	MECHANICAL
U	2854	0	2854	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
L	1944	1237 / 0	0 / 0	0 / 0	0 / 0	707 / 0
U	2024	1294 / 0	0 / 0	0 / 0	0 / 0	730 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO		FROM TO	FR-TO		
A-B	0 / 32	-84.9 -84.9 0.13 (1)	T-D	-120 / 67	0.06 (1)
B-C	0 / 14	-84.9 -84.9 0.12 (1)	M-K	0 / 3360	0.83 (1)
C-D	-3499 / 0	-84.9 -84.9 0.31 (1)	D-S	0 / 1821	0.45 (1)
D-V	-4118 / 0	-84.9 -84.9 0.31 (1)	M-J	-2255 / 0	0.51 (1)
V-W	-4118 / 0	-84.9 -84.9 0.31 (1)	S-E	-1249 / 0	0.57 (1)
W-E	-4118 / 0	-84.9 -84.9 0.31 (1)	N-J	0 / 2098	0.52 (1)
E-X	-4646 / 0	-84.9 -84.9 0.33 (1)	E-Q	0 / 788	0.20 (1)
X-Y	-4646 / 0	-84.9 -84.9 0.33 (1)	N-I	-1431 / 0	0.65 (1)
Y-F	-4646 / 0	-84.9 -84.9 0.33 (1)	Q-F	-454 / 0	0.21 (1)
F-Z	-4524 / 0	-84.9 -84.9 0.29 (1)	P-I	0 / 1156	0.29 (1)
Z-AA	-4524 / 0	-84.9 -84.9 0.29 (1)	F-P	-182 / 0	0.17 (1)
AA-G	-4524 / 0	-84.9 -84.9 0.29 (1)	P-H	-598 / 0	0.27 (1)
G-H	-4524 / 0	-84.9 -84.9 0.29 (1)	C-T	0 / 423	0.10 (1)
H-AB	-4524 / 0	-84.9 -84.9 0.29 (1)	U-C	-3645 / 0	0.83 (1)
AB-AC	-4524 / 0	-84.9 -84.9 0.29 (1)			
AC-I	-4524 / 0	-84.9 -84.9 0.29 (1)			
I-AD	-3749 / 0	-84.9 -84.9 0.31 (1)			
AD-AE	-3749 / 0	-84.9 -84.9 0.31 (1)			
AE-J	-3749 / 0	-84.9 -84.9 0.31 (1)			
J-AF	-2341 / 0	-84.9 -84.9 0.25 (1)			
AF-AG	-2341 / 0	-84.9 -84.9 0.25 (1)			
AG-K	-2341 / 0	-84.9 -84.9 0.25 (1)			
L-K	-2678 / 0	0.0 0.0 0.90 (1)			
U-B	-229 / 0	0.0 0.0 0.02 (1)			

U-AH	0 / 2612	-18.5 -18.5 0.42 (1)	10.00
AH-AI	0 / 2612	-18.5 -18.5 0.42 (1)	10.00
AI-AJ	0 / 2612	-18.5 -18.5 0.42 (1)	10.00
AJ-T	0 / 2612	-18.5 -18.5 0.42 (1)	10.00
T-AK	0 / 2913	-18.5 -18.5 0.43 (1)	10.00
AK-AL	0 / 2913	-18.5 -18.5 0.43 (1)	10.00
AL-S	0 / 2913	-18.5 -18.5 0.43 (1)	10.00
S-AM	0 / 4117	-18.5 -18.5 0.59 (1)	10.00
AM-R	0 / 4117	-18.5 -18.5 0.59 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.38")

CSI: TC=0.90/1.00 (K-L:1), BC=0.67/1.00 (P-Q:1), WB=0.83/1.00 (C-U:1), SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.83 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2126706

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420345	T7	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
R-AN	0 / 4117	-18.5	-18.5 0.59 (1)	10.00			
AN-Q	0 / 4117	-18.5	-18.5 0.59 (1)	10.00			
Q-AO	0 / 4646	-18.5	-18.5 0.67 (1)	10.00			
AO-AP	0 / 4646	-18.5	-18.5 0.67 (1)	10.00			
AP-P	0 / 4646	-18.5	-18.5 0.67 (1)	10.00			
P-O	0 / 3749	-18.5	-18.5 0.55 (1)	10.00			
O-AQ	0 / 3749	-18.5	-18.5 0.55 (1)	10.00			
AQ-AR	0 / 3749	-18.5	-18.5 0.55 (1)	10.00			
AR-N	0 / 3749	-18.5	-18.5 0.55 (1)	10.00			
N-AS	0 / 2341	-18.5	-18.5 0.35 (1)	10.00			
AS-AT	0 / 2341	-18.5	-18.5 0.35 (1)	10.00			
AT-M	0 / 2341	-18.5	-18.5 0.35 (1)	10.00			
M-AU	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
AU-AV	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
AV-L	0 / 0	-18.5	-18.5 0.08 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-6-4	-41	-41	---	FRONT	VERT	DEAD	---	C1
D	6-6-5	-61	-61	---	BACK	VERT	TOTAL	---	C1
D	6-6-4	-157	-157	---	FRONT	VERT	SNOW	---	C1
H	20-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
J	30-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
M	30-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
P	20-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
T	6-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
V	8-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
W	10-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
X	12-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
Y	14-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
Z	16-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
AA	18-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
AB	22-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
AC	24-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
AD	26-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
AE	28-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
AF	32-7-1	-55	-55	---	BACK	VERT	TOTAL	---	C1
AG	34-7-1	-57	-57	---	BACK	VERT	TOTAL	---	C1
AH	7-1	-17	-17	---	BACK	VERT	TOTAL	---	C1
AI	2-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AJ	4-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AK	8-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AL	10-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AM	12-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AN	14-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AO	16-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AP	18-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AQ	22-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AR	24-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AS	26-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AT	28-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AU	32-7-1	-14	-14	---	BACK	VERT	TOTAL	---	C1
AV	34-7-1	-15	-15	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

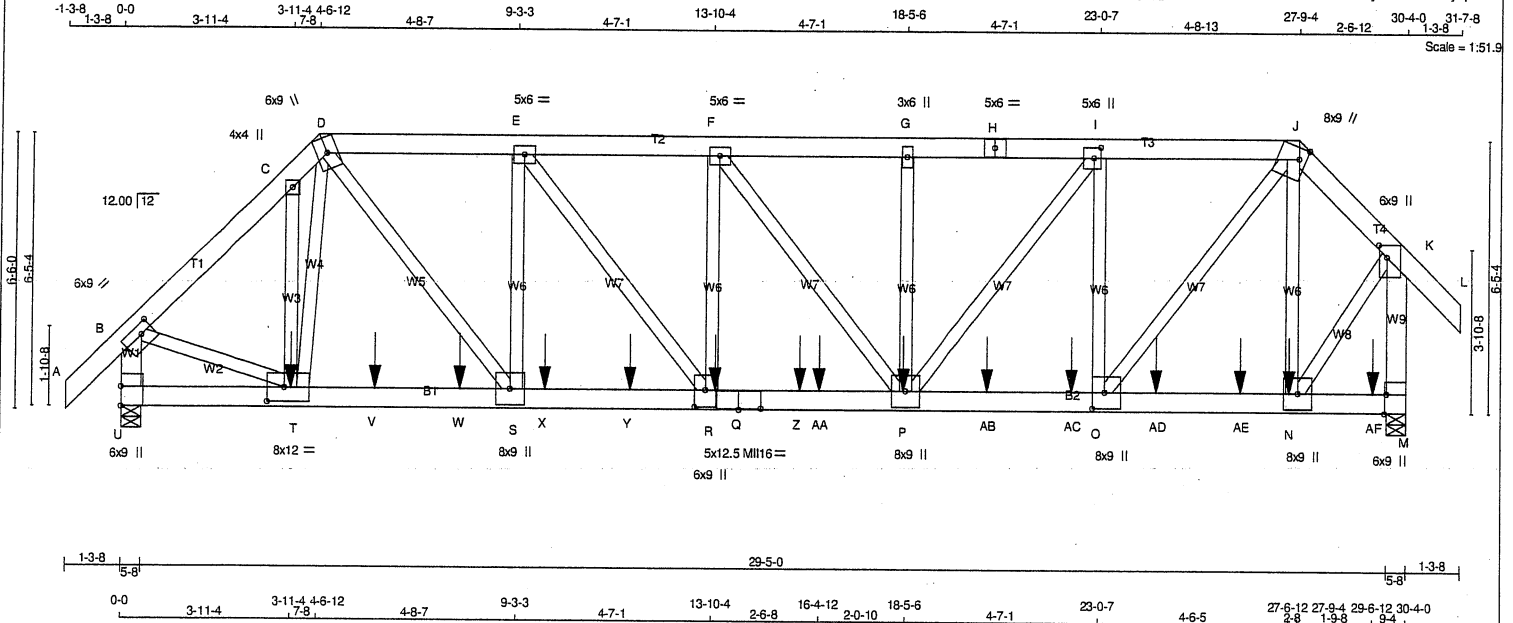


Structural component only
DWG# T-2126706 *me*

JOB NAME 420345	TRUSS NAME T8	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - D	2x6	DRY	No.2	SPF	SPF
D - H	2x6	DRY	No.2	SPF	SPF
H - J	2x6	DRY	No.2	SPF	SPF
J - L	2x6	DRY	No.2	SPF	SPF
U - B	2x6	DRY	No.2	SPF	SPF
M - K	2x6	DRY	No.2	SPF	SPF
U - Q	2x6	DRY	2100F 1.8E	SPF	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF	SPF
ALL WEBS 2x4 DRY No.2 SPF					
EXCEPT					
S - E	2x4	DRY	2100F 1.8E	SPF	SPF
N - J	2x4	DRY	2100F 1.8E	SPF	SPF
O - I	2x4	DRY	2100F 1.8E	SPF	SPF
R - F	2x4	DRY	2100F 1.8E	SPF	SPF
P - G	2x4	DRY	2100F 1.8E	SPF	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 2	12	TOP
D-H 2	12	TOP
H-J 2	12	TOP
J-L 2	12	TOP
U-B 2	12	TOP
M-K 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U-Q 2	9	SIDE(573.3)
Q-M 2	5	SIDE(947.6)
WEBS : (0.122"x3") SPIRAL NAILS		
C-T 1	6	SIDE(132.3)
D-T 1	6	SIDE(232.2)
R-F 1	6	SIDE(104.4)
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
U 14447	0	14447	0	5-8	5-8	5-8	
M 17316	0	17316	0	5-8	5-8	5-8	

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U 10242	6581 / 0	0 / 0	0 / 0	0 / 0	3661 / 0	0 / 0
M 12271	7906 / 0	0 / 0	0 / 0	0 / 0	4366 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M
BEARING SIZE FACTOR = 1.15 AT JNT(S) U, M (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 44	D-S	0 / 11913
B-C	-15320 / 0	S-E	-4710 / 0
C-D	-15063 / 0	N-J	-3910 / 0
D-E	-18003 / 0	N-K	0 / 11427
E-F	-21341 / 0	T-C	0 / 176
F-G	-20619 / 0	B-T	0 / 11348
G-H	-20619 / 0	T-D	0 / 814
H-I	-20619 / 0	O-J	0 / 13808
I-J	-15740 / 0	E-R	0 / 5483
J-K	-10307 / 0	O-I	-6802 / 0
K-L	0 / 44	R-F	0 / 525
U-B	-14647 / 0	P-I	0 / 8014
M-K	-16443 / 0	F-P	-1185 / 0
		P-G	-364 / 0
U-T	0 / 0		
T-V	0 / 10729		
V-W	0 / 10729		
W-S	0 / 10729		
S-X	0 / 18003		
X-Y	0 / 18003		
Y-R	0 / 18003		
R-Q	0 / 21341		
Q-Z	0 / 21341		
Z-AA	0 / 21341		
AA-P	0 / 21341		
P-AB	0 / 15739		
AB-AC	0 / 15739		
AC-O	0 / 15739		
O-AD	0 / 7125		
AD-AE	0 / 7125		
AE-N	0 / 7125		
N-AF	0 / 0		
AF-M	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	27-6-12	-1368	-1368	---	TOP	VERT	TOTAL	---	C1
P	18-4-12	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
R	13-11-4	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-1929	-1929	---	FRONT	VERT	TOTAL	---	C1
V	5-11-4	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
W	7-11-4	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1

TOTAL WEIGHT = 3 X 222 = 665 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/956 (0.38")

CSI: TC=0.78/1.00 (K-M:1), BC=0.63/1.00 (P-R:1),
WB=0.74/1.00 (J-O:1), SS=0.50/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
MI16	438	302	2547

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)

JSI METAL= 0.86 (K) (INPUT = 1.00)



Structural component only
DWG# T-2126707

CONTINUED ON PAGE 2

JOB NAME 420345	TRUSS NAME T8	QUANTITY 1	PLY 3	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.50	3.50
C	TMW+w	MT20	4.0	4.0		
D	TTWW+m	MT20	6.0	9.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	3.00	2.00
J	TTWW+m	MT20	8.0	9.0	3.25	2.00
K	TMVW+p	MT20	6.0	9.0	3.50	2.25
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BMWW-t	MT20	8.0	9.0		
O	BMWW-t	MT20	8.0	9.0	4.50	3.50
P	BMWWW+t	MT20	8.0	9.0		
Q	BS-t	MIH6	5.0	12.5	0.25	6.25
R	BMWW-t	MT20	6.0	9.0	4.75	3.00
S	BMWW-t	MT20	8.0	9.0		
T	BMWWW-t	MT20	8.0	12.0	4.00	5.00
U	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	9-11-4	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
Y	11-11-4	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
Z	15-11-4	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
AA	16-4-12	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
AB	20-4-12	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
AC	22-4-12	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
AD	24-4-12	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
AE	26-4-12	-1288	-1288	---	FRONT	VERT	TOTAL	---	C1
AF	29-6-12	-1369	-1369	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

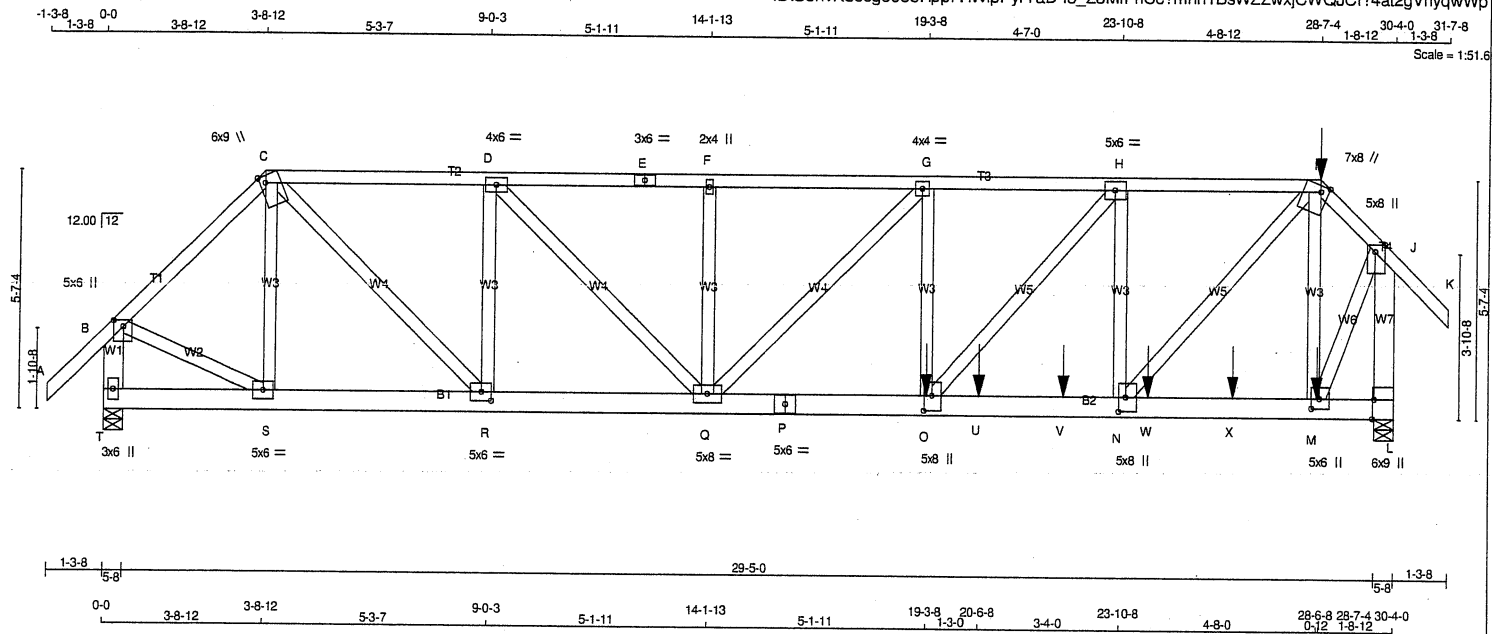


Structural component only
DWG# T-2126707 *m*

JOB NAME 420345	TRUSS NAME T9	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
T - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
T - P	2x6	DRY	No.2
P - L	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	6.0 1.75 2.75
C	TTWW+m	MT20	6.0	9.0 Edge 1.75
D	TMVW-t	MT20	4.0	6.0
E	TS-t	MT20	3.0	6.0
F	TMVW+w	MT20	2.0	4.0
G	TMVW-t	MT20	4.0	4.0
H	TMVW-t	MT20	5.0	6.0
I	TTWW+m	MT20	7.0	8.0 Edge 2.25
J	TMVW+p	MT20	5.0	8.0 2.00 2.75
L	BMV1+t	MT20	6.0	9.0 Edge 0.50
M	BMVW+t	MT20	5.0	6.0 2.75 2.25
N	BMVW+t	MT20	5.0	8.0 4.00 2.00
O	BMVW+t	MT20	5.0	8.0 4.25 2.50
P	BS-t	MT20	5.0	6.0
Q	BMVW-t	MT20	5.0	8.0
R	BMVW-t	MT20	5.0	6.0 2.50 2.75
S	BMVW-t	MT20	5.0	6.0
T	BMV1+p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS	REACTION	DOWN	UP	BRG	IN-SX	BRG	IN-SX
T	2344	0	2344	0	0	5-8	5-8	5-8	5-8
L	3442	0	3442	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
T	1658	1083 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	575 / 0	0 / 0	0 / 0
L	2430	1816 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	814 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L
BEARING SIZE FACTOR = 1.15 AT JNT(S) L (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-84.9	-84.9 0.13 (1)	S-C	-449 / 0	0.15 (1)	
B-C	-2102 / 0	-84.9	-84.9 0.32 (1)	M-I	-1607 / 0	0.52 (1)	
C-D	-3079 / 0	-84.9	-84.9 0.60 (1)	B-S	0 / 1579	0.28 (1)	
D-E	-4067 / 0	-84.9	-84.9 0.75 (1)	M-J	0 / 2390	0.42 (1)	
E-F	-4067 / 0	-84.9	-84.9 0.75 (1)	Q-G	-710 / 0	0.51 (1)	
F-G	-4067 / 0	-84.9	-84.9 0.68 (1)	C-R	0 / 2261	0.40 (1)	
G-H	-4565 / 0	-84.9	-84.9 0.63 (1)	O-G	0 / 104	0.02 (4)	
H-I	-3223 / 0	-84.9	-84.9 0.50 (1)	R-D	-1487 / 0	0.48 (1)	
I-J	-1666 / 0	-84.9	-84.9 0.16 (1)	Q-F	-427 / 0	0.14 (1)	
J-K	0 / 42	-84.9	-84.9 0.13 (1)	N-I	0 / 3305	0.58 (1)	
T-B	-2319 / 0	0.0	0.0 0.18 (1)	D-Q	0 / 1409	0.25 (1)	
L-J	-3502 / 0	0.0	0.0 0.59 (1)	N-H	-1834 / 0	0.60 (1)	
T-S	0 / 0	-18.5	-18.5 0.05 (4)				
S-R	0 / 1468	-18.5	-18.5 0.22 (1)				
R-Q	0 / 3080	-18.5	-18.5 0.46 (1)				
Q-P	0 / 4565	-18.5	-18.5 0.64 (1)				
P-O	0 / 4565	-18.5	-18.5 0.64 (1)				
O-U	0 / 3323	-18.5	-18.5 0.60 (1)				
U-V	0 / 3323	-18.5	-18.5 0.60 (1)				
V-N	0 / 3323	-18.5	-18.5 0.60 (1)				
N-W	0 / 1099	-18.5	-18.5 0.30 (1)				
W-X	0 / 1099	-18.5	-18.5 0.30 (1)				
X-M	0 / 1099	-18.5	-18.5 0.30 (1)				
M-L	0 / 0	-18.5	-18.5 0.11 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	28-7-4	-16	-16	---	FRONT	VERT	DEAD	---	C1
I	28-7-4	-63	-63	---	FRONT	VERT	SNOW	---	C1
M	28-6-8	-155	-155	---	BACK	VERT	TOTAL	---	C1
O	19-3-8	-874	-874	---	BACK	VERT	TOTAL	---	C1
U	20-6-8	-155	-155	---	BACK	VERT	TOTAL	---	C1
V	22-6-8	-145	-145	---	BACK	VERT	TOTAL	---	C1
W	24-6-8	-145	-145	---	BACK	VERT	TOTAL	---	C1
X	26-6-8	-145	-145	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.01")
CALCULATED VERT. DEFL(LL) = L/999 (0.16")
ALLOWABLE DEFL(TL)= L/360 (1.01")
CALCULATED VERT. DEFL(TL) = L/999 (0.30")

CSI: TC=0.75/1.00 (D-F:1), BC=0.64/1.00 (O-Q:1), WB=0.60/1.00 (H-N:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

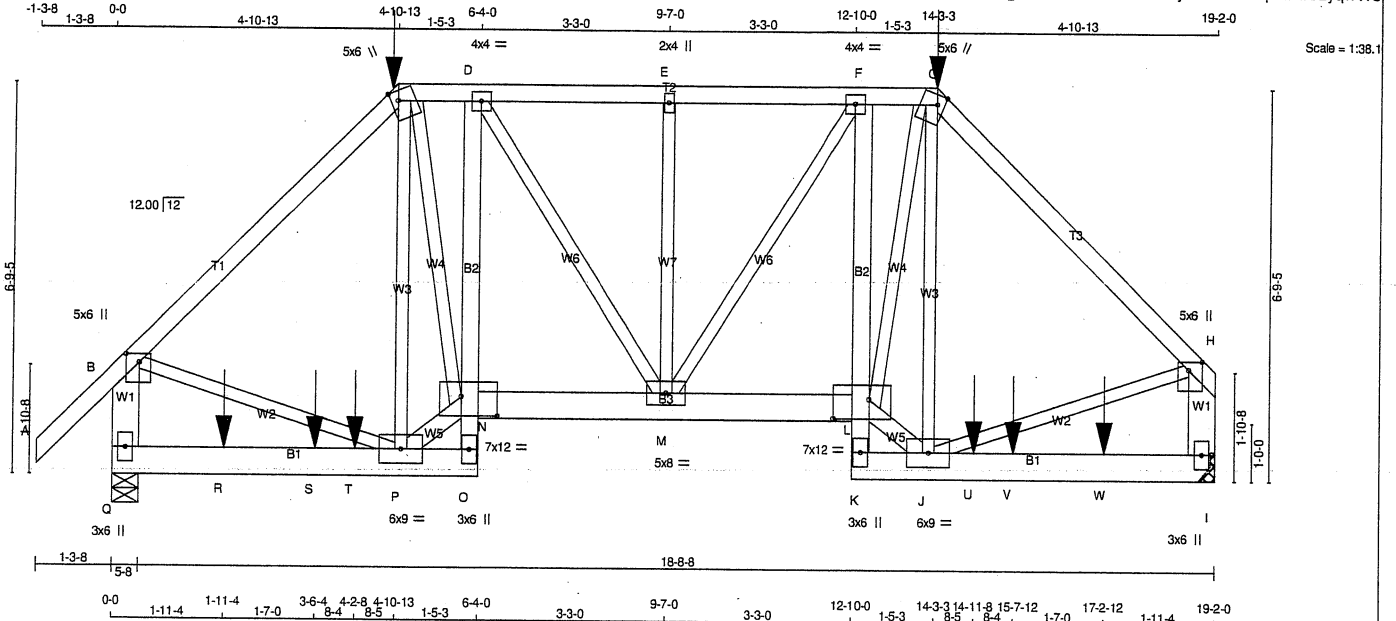


Structural component only
DWG# T-2126708

JOB NAME 420345	TRUSS NAME T10S	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:S8hvR8eJge955Hpp7HWpPyrYaD-mLYxHgtYWksOxGDkZ2o58Ty6vs2xBMepXoE2EyqwWo



TOTAL WEIGHT = 125 lb

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
Q - B	2x6	DRY	No.2
I - H	2x6	DRY	No.2
Q - O	2x6	DRY	No.2
O - D	2x6	DRY	No.2
N - L	2x6	DRY	No.2
K - F	2x4	DRY	No.2
K - I	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
P - N	2x4	DRY	No.2
L - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	1.75	2.75
I	BMV1+p	MT20	3.0	6.0		
J	BMVWW-t	MT20	6.0	9.0		
K	BMV+p	MT20	3.0	6.0		
L	BVMWW-t	MT20	7.0	12.0	4.00	7.50
M	BMVWW-t	MT20	5.0	8.0		
N	BVMWW-t	MT20	7.0	12.0	4.00	7.50
O	BMV+p	MT20	3.0	6.0		
P	BMVWW-t	MT20	6.0	9.0		
Q	BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
Q	1650	0	1650	0	0	5-8	5-8		
I	1531	0	1531	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1163	783 / 0	0 / 0	0 / 0	0 / 0	380 / 0	0 / 0
I	1082	718 / 0	0 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. (PLF)	LC1 (PLF)	MAX. (PLF)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	VERT. (PLF)	LC1 (PLF)	MAX. (PLF)	UNBRAC LENGTH
FR-TO						FR-TO					
A-B	0 / 42	-84.9	-84.9	0.13 (1)	10.00	P-C	-370 / 0	0.27 (1)			
B-C	-1395 / 0	-84.9	-84.9	0.49 (1)	4.91	D-M	0 / 222	0.05 (1)			
C-D	-1204 / 0	-84.9	-84.9	0.06 (1)	5.75	M-E	-327 / 0	0.16 (1)			
D-E	-1329 / 0	-84.9	-84.9	0.17 (1)	5.40	M-F	0 / 222	0.05 (1)			
E-F	-1329 / 0	-84.9	-84.9	0.17 (1)	5.40	J-G	-370 / 0	0.27 (1)			
F-G	-1204 / 0	-84.9	-84.9	0.06 (1)	5.75	B-P	0 / 1023	0.25 (1)			
G-H	-1395 / 0	-84.9	-84.9	0.49 (1)	4.91	J-H	0 / 1023	0.25 (1)			
Q-B	-1598 / 0	0.0	0.0	0.12 (1)	7.74	P-N	0 / 1228	0.22 (1)			
I-H	-1480 / 0	0.0	0.0	0.11 (1)	7.81	C-N	0 / 975	0.24 (1)			
Q-R	0 / 0	-18.5	-18.5	0.22 (1)	10.00	L-J	0 / 1228	0.22 (1)			
R-S	0 / 0	-18.5	-18.5	0.22 (1)	10.00	L-G	0 / 975	0.24 (1)			
S-T	0 / 0	-18.5	-18.5	0.22 (1)	10.00						
T-P	0 / 0	-18.5	-18.5	0.22 (1)	10.00						
P-O	0 / 40	-18.5	-18.5	0.23 (1)	10.00						
O-N	-244 / 0	0.0	0.0	0.06 (1)	7.81						
N-D	-361 / 0	0.0	0.0	0.12 (1)	7.81						
N-M	0 / 1212	-18.5	-18.5	0.19 (1)	10.00						
M-L	0 / 1212	-18.5	-18.5	0.19 (1)	10.00						
K-L	-244 / 0	0.0	0.0	0.06 (1)	7.81						
L-F	-361 / 0	0.0	0.0	0.12 (1)	7.81						
K-J	0 / 39	-18.5	-18.5	0.23 (1)	10.00						
J-U	0 / 0	-18.5	-18.5	0.22 (1)	10.00						
U-V	0 / 0	-18.5	-18.5	0.22 (1)	10.00						
V-W	0 / 0	-18.5	-18.5	0.22 (1)	10.00						
W-I	0 / 0	-18.5	-18.5	0.22 (1)	10.00						

SPECIFIED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX.	MAX+							
C	4-10-13	-20	-20		FRONT	VERT	DEAD			C1	
C	4-10-13	-78	-78		FRONT	VERT	SNOW			C1	
G	14-3-3	-20	-20		FRONT	VERT	DEAD			C1	
G	14-3-3	-78	-78		FRONT	VERT	SNOW			C1	
R	1-11-4	-3	-3		BACK	VERT	TOTAL			C1	
S	3-6-4	-3	-3		BACK	VERT	TOTAL			C1	
T	4-2-8	-275	-275		BACK	VERT	TOTAL			C1	
U	14-11-8	-275	-275		BACK	VERT	TOTAL			C1	
V	15-7-12	-3	-3		BACK	VERT	TOTAL			C1	
W	17-2-12	-3	-3		BACK	VERT	TOTAL			C1	

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.49/1.00 (G-H:1), BC=0.23/1.00 (J-K:1), WB=0.27/1.00 (C-P:1), SSI=0.29/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

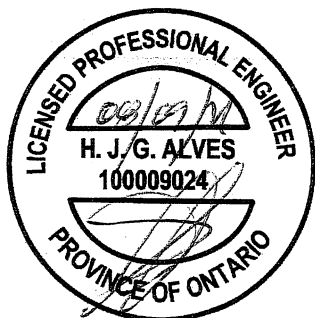
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (H) (INPUT = 0.90)
JSI METAL= 0.46 (H) (INPUT = 1.00)

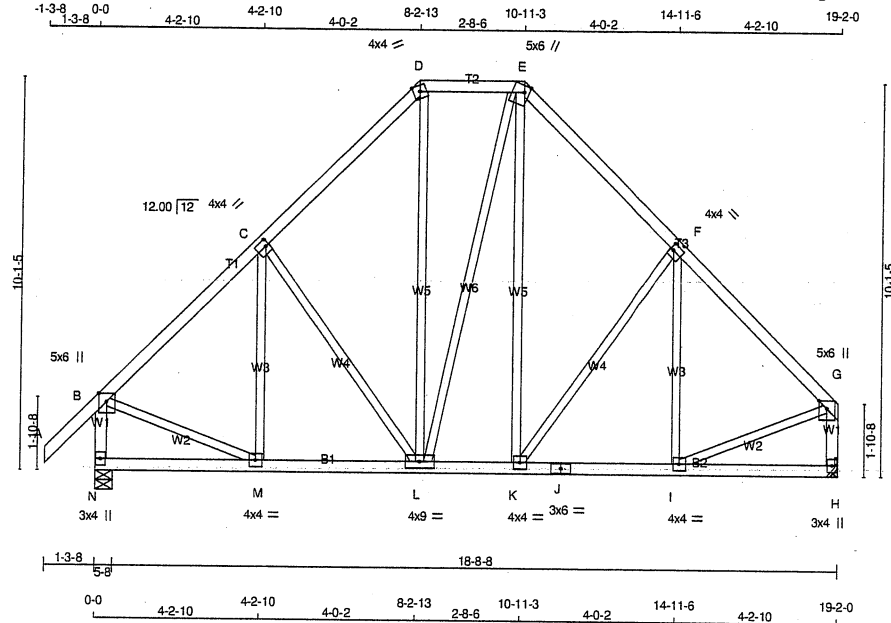


Structural component only
DWG# T-2126709

JOB NAME 420345	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:S8hvR8eJge955Hpp7HWlpPyrYaD-mLYxHgtYVWksOxGDKZ2o58T0gvtRxCHepXoE2EyqwWo



Scale = 1:56.7

TOTAL WEIGHT = 108 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-m	MT20	4.0	4.0	Edge	
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW-t	MT20	4.0	4.0	2.00	1.00
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I, K, M						
I	BMVW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	9.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS	REACTION	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
N	1109	0	1109	0	0	5-8	5-8		
H	991	0	991	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN.	COMPONENT REACTIONS						
N	785	511 / 0	0 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0	0 / 0
H	703	446 / 0	0 / 0	0 / 0	0 / 0	0 / 0	257 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	M-C	-132 / 20	0.08 (1)
B-C	-823 / 0	-84.9	-84.9	0.19 (1)	6.25	C-L	-219 / 0	0.21 (1)
C-D	-705 / 0	-84.9	-84.9	0.19 (1)	6.25	L-D	0 / 237	0.05 (1)
D-E	-478 / 0	-84.9	-84.9	0.08 (1)	6.25	L-E	0 / 5	0.00 (1)
E-F	-704 / 0	-84.9	-84.9	0.19 (1)	6.25	K-E	0 / 232	0.05 (1)
F-G	-824 / 0	-84.9	-84.9	0.19 (1)	6.25	K-F	-221 / 0	0.21 (1)
N-B	-1076 / 0	0.0	0.0	0.12 (1)	7.61	I-F	-128 / 22	0.07 (1)
H-G	-959 / 0	0.0	0.0	0.11 (1)	7.81	B-M	0 / 641	0.14 (1)
					I-G	0 / 641	0.14 (1)	
N-M	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
M-L	0 / 603	-18.5	-18.5	0.13 (1)	10.00			
L-K	0 / 477	-18.5	-18.5	0.10 (1)	10.00			
K-J	0 / 603	-18.5	-18.5	0.14 (1)	10.00			
J-I	0 / 603	-18.5	-18.5	0.14 (1)	10.00			
I-H	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.64")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.64")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.19/1.00 (F-G:1), BC=0.14/1.00 (I-K:1), WB=0.21/1.00 (F-K:1), SS=0.12/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

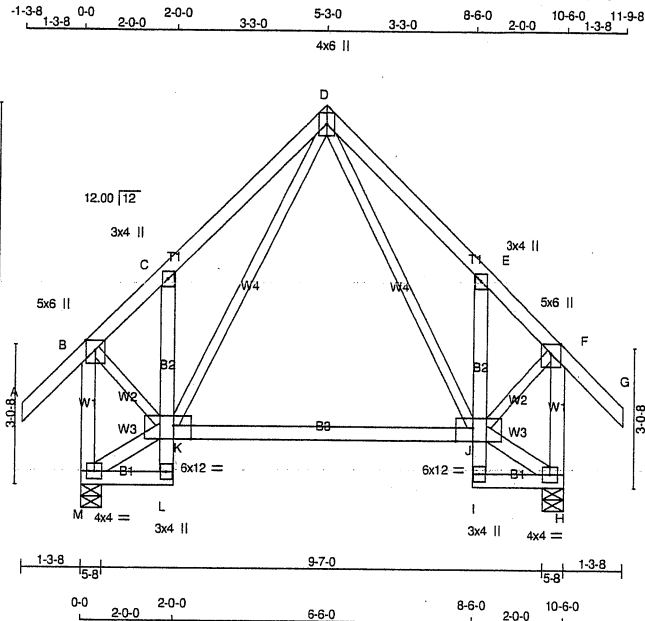
JSI GRIP = 0.72 (I) (INPUT = 0.90)
JSI METAL = 0.31 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126710

JOB NAME 420345	TRUSS NAME T12S	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale: 1/4"=1'

TOTAL WEIGHT = 2 X 69 = 138 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
M - L	2x4	DRY	No.2	SPF	
L - C	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
I - E	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
M - K	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	2.50
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	6.0	12.0	Edge	4.50
K	BMVW1-t	MT20	6.0	12.0	Edge	4.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
M	661	0	661	0	5-8	5-8
H	661	0	661	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOOSE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
M	467	309 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0
H	467	309 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED	VERT.	MAX	FACTORED	MEMB.	MAX	FACTORED
	FORCE	LOAD	LC1	MAX		FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	D-J	0 / 111
B-C	-331 / 0	-84.9	-84.9	0.09 (1)	6.25	K-D	0 / 111
C-D	-336 / 0	-84.9	-84.9	0.13 (1)	6.25	M-K	-8 / 0
D-E	-336 / 0	-84.9	-84.9	0.13 (1)	6.25	B-K	0 / 314
E-F	-331 / 0	-84.9	-84.9	0.09 (1)	6.25	J-H	-8 / 0
F-G	0 / 42	-84.9	-84.9	0.12 (1)	10.00	J-F	0 / 314
M-B	-640 / 0	0.0	0.0	0.11 (1)	7.81		
H-F	-640 / 0	0.0	0.0	0.11 (1)	7.81		
M-L	0 / 7	-18.5	-18.5	0.02 (4)	10.00		
L-K	0 / 19	0.0	0.0	0.01 (1)	10.00		
K-C	-235 / 0	0.0	0.0	0.01 (1)	7.81		
K-J	0 / 178	-18.5	-18.5	0.26 (4)	10.00		
I-J	0 / 19	0.0	0.0	0.01 (1)	10.00		
J-E	-235 / 0	0.0	0.0	0.01 (1)	7.81		
I-H	0 / 7	-18.5	-18.5	0.02 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.13/1.00 (D-E:1), BC=0.26/1.00 (J-K:4), WB=0.07/1.00 (F-J:1), SSI=0.09/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

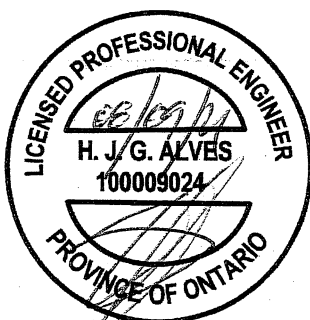
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

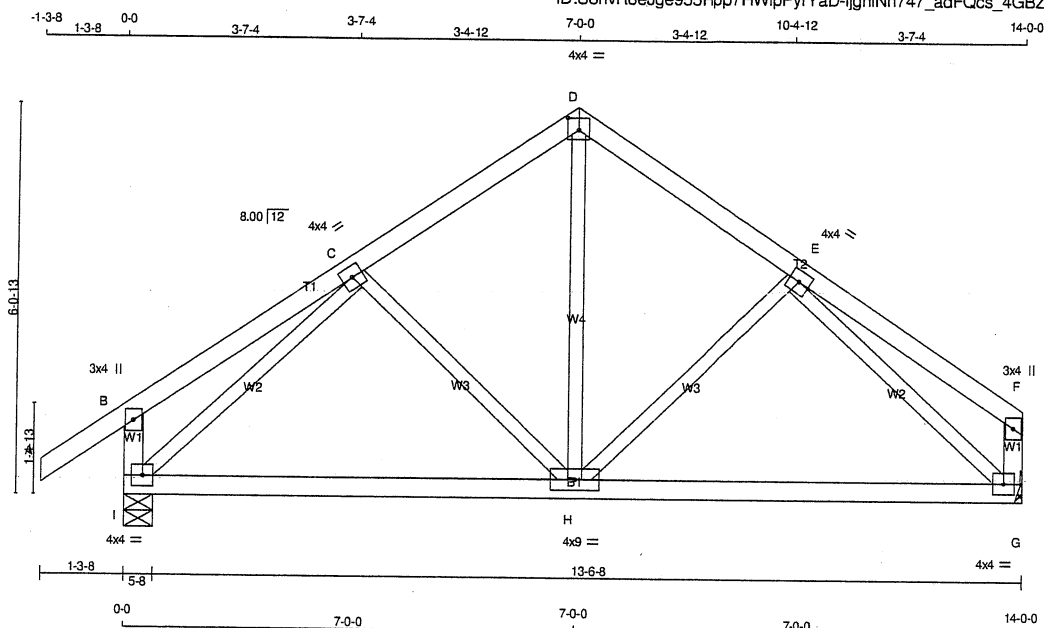
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (M) (INPUT = 0.90)
JSI METAL= 0.14 (F) (INPUT = 1.00)



Structural component only
DWG# T-2126711

JOB NAME 420345	TRUSS NAME T13	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 11:37:49 2021 Page 1 ID:S8hvR8eJge955Hpp7HWlpYrYaD-ijghiNh747_adFQcs_4GBZZNejXHP4LWGqHK66yqwWm	



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
I		840	0	840	0	0	5-8	5-8	
G		724	0	724	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	594	390 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
G	514	326 / 0	0 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.		FORCE (LBS)	VERT. LOAD	LC1	MAX (PLF)	MEMB.		FORCE (LBS)	MAX (LC)
FR-TO						FR-TO			
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	H-D	0 / 381	0.09 (1)	
B-C	0 / 19	-84.9	-84.9	0.16 (1)	10.00	H-E	-159 / 0	0.06 (1)	
C-D	-570 / 0	-84.9	-84.9	0.13 (1)	6.25	C-H	-159 / 0	0.06 (1)	
D-E	-570 / 0	-84.9	-84.9	0.13 (1)	6.25	I-C	-801 / 0	0.30 (1)	
E-F	0 / 19	-84.9	-84.9	0.16 (1)	10.00	E-G	-801 / 0	0.30 (1)	
I-B	-231 / 0	0.0	0.0	0.02 (1)	7.81				
G-F	-115 / 0	0.0	0.0	0.01 (1)	7.81				
I-H	0 / 573	-18.5	-18.5	0.31 (4)	10.00				
H-G	0 / 573	-18.5	-18.5	0.31 (4)	10.00				

TOTAL WEIGHT = 2 X 59 = 117 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.18/1.00 (B-C:1), BC=0.31/1.00 (H-I:4),
WB=0.30/1.00 (C-I:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

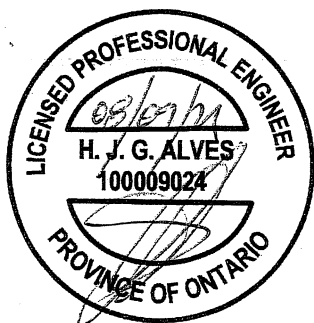
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (C) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)

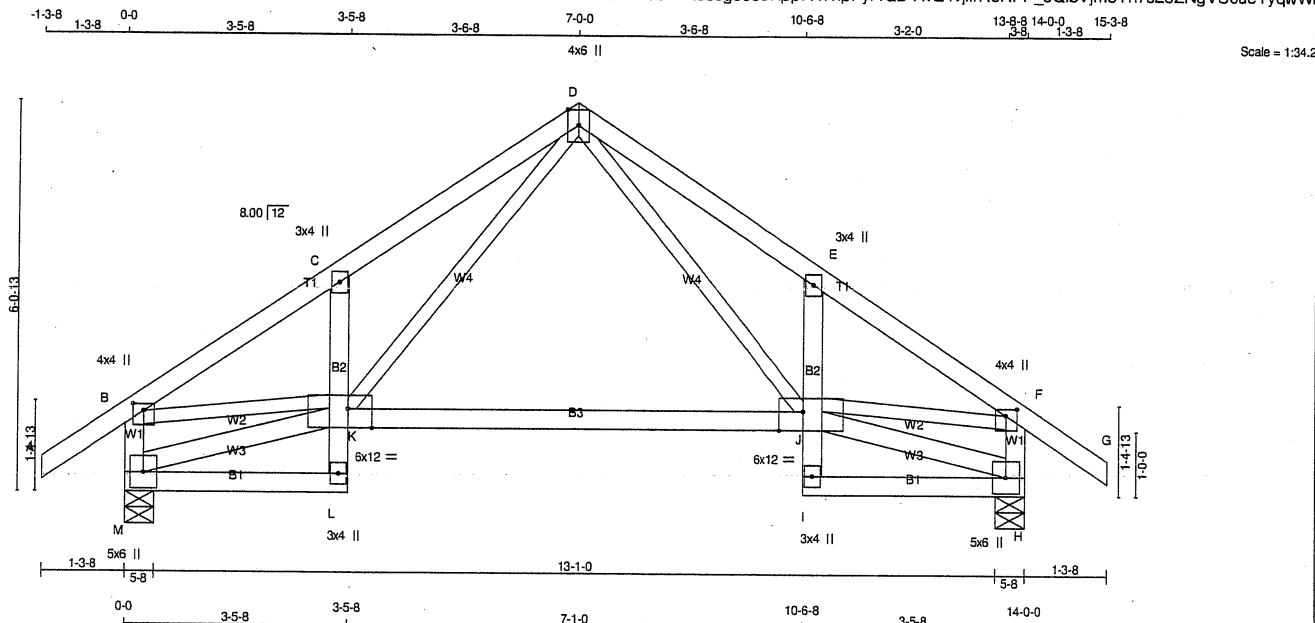


Structural component only
DWG# T-2126712

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420345	T13S	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 69 = 208 lb [M/F]

LUMBER				
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMVW1+p	MT20	5.0	6.0		
I	BMV+p	MT20	3.0	4.0		
J	BMVWWW-I	MT20	6.0	12.0	Edge	4.50
K	BMVWWW-I	MT20	6.0	12.0	Edge	4.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1+p	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
M	840	0	840	0	5-8	5-8
H	840	0	840	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
M	594	390 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
H	594	390 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 32	-84.9	-84.9 0.11 (1)	10.00	D-J	0 / 543	0.12 (1)
B-C	-994 / 0	-84.9	-84.9 0.12 (1)	6.13	K-D	0 / 543	0.12 (1)
C-D	-1025 / 0	-84.9	-84.9 0.14 (1)	6.04	M-K	-25 / 0	0.00 (1)
D-E	-1025 / 0	-84.9	-84.9 0.14 (1)	6.04	B-K	0 / 842	0.19 (1)
E-F	-994 / 0	-84.9	-84.9 0.12 (1)	6.13	J-H	-25 / 0	0.00 (1)
F-G	0 / 32	-84.9	-84.9 0.11 (1)	10.00	J-F	0 / 842	0.19 (1)
M-B	-802 / 0	0.0	0.0 0.08 (1)	7.81			
H-F	-802 / 0	0.0	0.0 0.08 (1)	7.81			
M-L	0 / 24	-18.5	-18.5 0.06 (4)	10.00			
L-K	0 / 34	0.0	0.0 0.04 (1)	10.00			
K-C	-357 / 0	0.0	0.0 0.03 (1)	7.81			
K-J	0 / 507	-18.5	-18.5 0.34 (4)	10.00			
I-J	0 / 34	0.0	0.0 0.04 (1)	10.00			
J-E	-357 / 0	0.0	0.0 0.03 (1)	7.81			
I-H	0 / 24	-18.5	-18.5 0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/967 (0.17")

CSI: TC=0.14/1.00 (D-E:1), BC=0.34/1.00 (J-K:4), WB=0.19/1.00 (F-J:1), SS=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

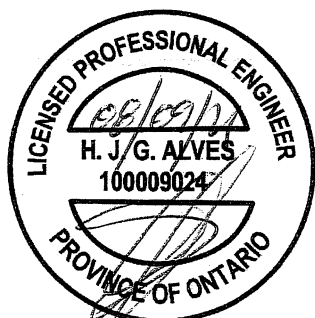
NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI)	(PLI)	(PLI)
MAX MIN MAX MIN MAX MIN		
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

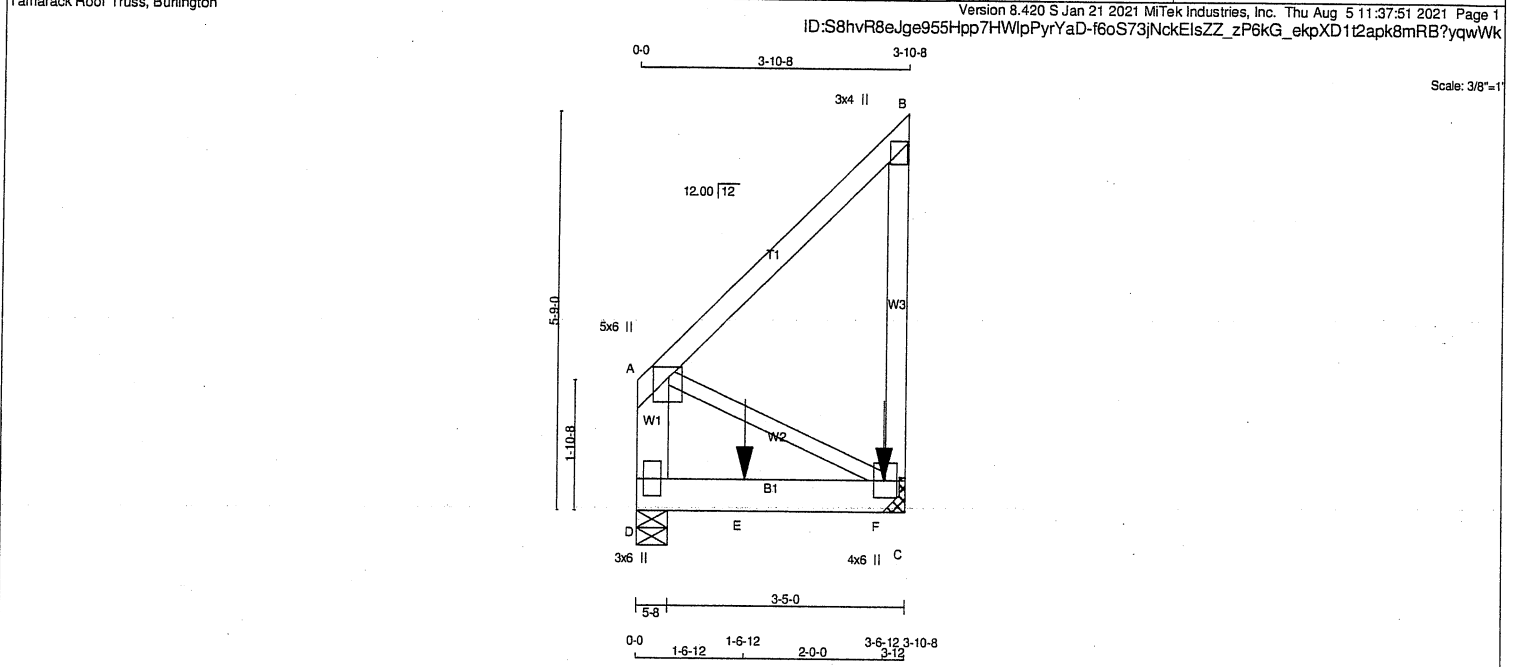
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.25 (F) (INPUT = 1.00)



Structural component only
DWG# T-2126713

JOB NAME 420345	TRUSS NAME T14	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					



LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N. L. G. A. RULES										BEARINGS										SPECIFIED LOADS:									
CHORDS SIZE LUMBER DESCR.										FACTORED GROSS REACTION										TOP CH. LL = 23.3 PSF									
A - B 2x4 DRY No.2 SPF										MAXIMUM FACTORED INPUT REQD										DL = 6.0 PSF									
C - B 2x4 DRY No.2 SPF										GROSS REACTION BRG BRG										BOT CH. LL = 0.0 PSF									
D - A 2x6 DRY No.2 SPF										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										DL = 7.4 PSF									
D - C 2x6 DRY No.2 SPF										C 1139 0 1139 0 0 5-8 MECHANICAL										TOTAL LOAD = 36.7 PSF									
ALL WEBS 2x3 DRY No.2 SPF										D 678 0 678 0 0 5-8 5-8																			
DRY: SEASONED LUMBER.										A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.																			
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:										UNFACTORED REACTIONS										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015									
CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)										1ST CASE MAX. MIN. COMPONENT REACTIONS										THIS DESIGN COMPLIES WITH:									
TOP CHORDS : (0.122"x3") SPIRAL NAILS										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										- PART 9 OF BCBC 2018 , ABC 2019									
A-B 1 12 TOP										C 807 522 / 0 0 / 0 0 / 0 0 / 0 285 / 0 0 / 0										- PART 9 OF OBC 2012 (2019 AMENDMENT)									
B-C 1 12 SIDE(63.5)										D 480 311 / 0 0 / 0 0 / 0 0 / 0 169 / 0 0 / 0										- CSA 086-14									
D-A 2 12 TOP										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D										- TPIC 2014									
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS										BRACING										(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD									
D-C 2 12 SIDE(122.0)										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.																			
WEBS : (0.122"x3") SPIRAL NAILS										MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.																			
2x3 1 6										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.																			
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.										LOADING										ALLOWABLE DEFL.(LL)= L/360 (0.19")									
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.										TOTAL LOAD CASES: (4)										CALCULATED VERT. DEFL.(LL) = L/999 (0.01")									
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.																				ALLOWABLE DEFL.(TL)= L/360 (0.19")									
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.																				CALCULATED VERT. DEFL.(TL) = L/999 (0.03")									
PLATES (table is in inches)																				CSI: TC=0.12/1.00 (A-B:1), BC=0.29/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.32/1.00 (C-D:1)									
JT TYPE PLATES W LEN Y X										CHORDS										WEBS									
A TMVW+p MT20 5.0 6.0 1.75 2.75										MAX. FACTORED FACTORED										MAX. FACTORED									
B TMV+p MT20 3.0 4.0										MEMB. FORCE VERT. LOAD LC1 MAX										MEMB. FORCE MAX									
C BMVWV1+p MT20 4.0 6.0										(LBS) (PLF) CSI (LC) UNBRAC										(LBS) CSI (LC)									
D BMV1+p MT20 3.0 6.0										FR-TO FROM TO LENGTH FR-TO										A-C 0 / 0 0.00 (1)									
										A-B 0 / 0 -84.9 -84.9 0.12 (1) 10.00																			
										C-B -164 / 0 0.0 0.0 0.04 (1) 7.81																			
										D-A -164 / 0 0.0 0.0 0.01 (1) 7.81																			
										D-E 0 / 0 -18.5 -18.5 0.29 (1) 10.00																			
										E-F 0 / 0 -18.5 -18.5 0.29 (1) 10.00																			
										F-C 0 / 0 -18.5 -18.5 0.29 (1) 10.00																			
										SPECIFIED CONCENTRATED LOADS (LBS)																			
										JT LOC. LC1 MAX- MAX+ FACE DIR.										TYPE HEEL CONN.									
										E 1-6-12 -499 -499 --- BACK VERT										TOTAL --- C1									
										F 3-6-12 -504 -504 --- BACK VERT										TOTAL --- C1									
																				NAIL VALUES									
																				PLATE GRIP(DRY) SHEAR SECTION									
																				(PSI) (PLI) (PLI)									
																				MAX. MIN. MAX. MIN. MAX. MIN.									

Structural component only
DWG# T-2126714

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(P.S.I) (P.LI) (P.LI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.04 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

Tamarack Roof Truss, Burlington

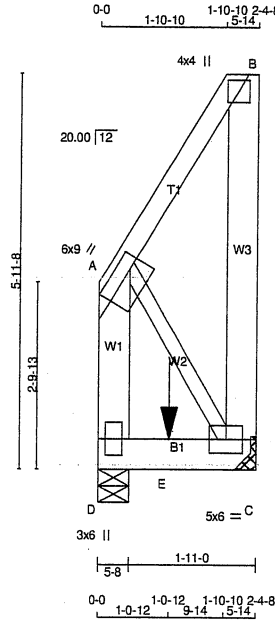
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 11:37:52 2021 Page 1
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Scale = 1:30.8

JOB NAME 420345	TRUSS NAME T16	QUANTITY 2	PLY 2	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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Scale = 1:33.2

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - B	2x4 DRY	No.2	SPF	
C - B	2x6 DRY	No.2	SPF	
D - A	2x6 DRY	No.2	SPF	
D - C	2x6 DRY	No.2	SPF	
ALL WEBS	2x3 DRY	No.2	SPF	
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B 1	12	TOP
B - C 2	12	TOP
D - A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D - C 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	2.25	2.25
B	TMV+p	MT20	4.0	4.0		
C	BMVW1-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
C	410	0	410	0	5-8	5-8
D	478	0	478	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	289	194 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
D	337	226 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO			FR-TO		
A-B	0 / 0	-84.9 -84.9 0.04 (1)	A-C	0 / 0	0.00 (1)
C-B	-101 / 0	0.0 0.0 0.02 (1)			
D-A	-101 / 0	0.0 0.0 0.01 (1)			
D-E	0 / 0	-18.5 -18.5 0.14 (1)			
E-C	0 / 0	-18.5 -18.5 0.14 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-0-12	452	452	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 26 = 104 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.04/1.00 (A-B:1), BC=0.14/1.00 (C-D:1),
WB=0.00/1.00 (A-C:1), SS=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

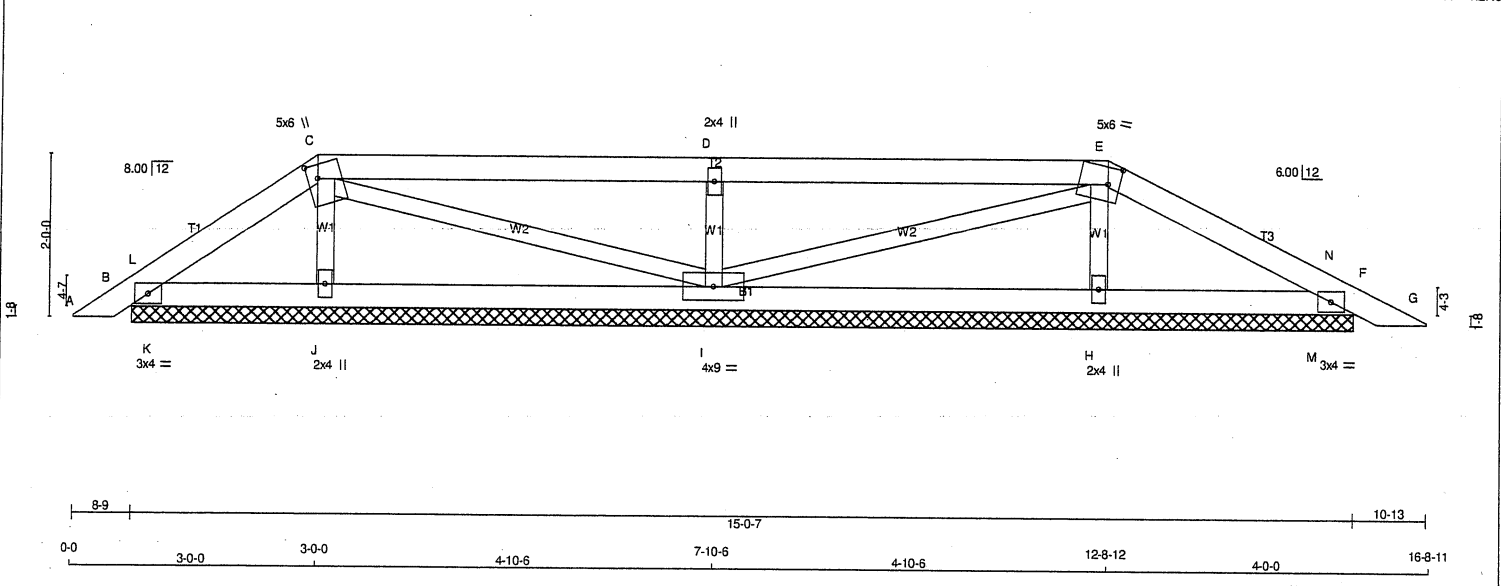
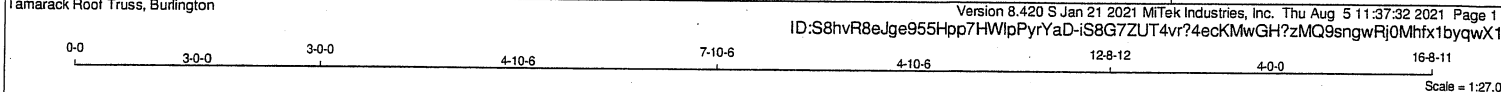
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.02 (B) (INPUT = 0.90)
JSI METAL= 0.01 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126716



TOTAL WEIGHT = 2 X 48 = 96 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	TOP CH. LL	LL	PSF	
A - C	2x4	DRY	No.2	JT VERT	DOWN	UP	IN-SX	DL	=	6.0	PSF
C - E	2x4	DRY	No.2	F	0	0	15-0-7	BOT CH. LL	=	0.0	PSF
E - G	2x4	DRY	No.2	J	0	0	15-0-7	DL	=	7.4	PSF
B - F	2x4	DRY	No.2	I	0	0	15-0-7	TOTAL LOAD	=	36.7	PSF
ALL WEBS 2x3 DRY No.2				H	0	0	15-0-7				
DRY: SEASONED LUMBER.											

PLATES (table is in inches)							UNFACTORED REACTIONS						
JT	TYPE	PLATES	W	LEN	Y	X	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
B	TMB1-I	MT20	3.0	4.0			JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	TTWW+m	MT20	5.0	6.0	2.00	1.50	B	130	97/0	0/0	0/0	33/0	0/0
D	TMW+w	MT20	2.0	4.0			F	169	121/0	0/0	0/0	48/0	0/0
E	TTWW-m	MT20	5.0	6.0	2.50	1.75	J	212	121/0	0/0	0/0	91/0	0/0
F	TMB1-I	MT20	3.0	4.0			I	435	287/0	0/0	0/0	148/0	0/0
H	BMW1+w	MT20	2.0	4.0			H	244	144/0	0/0	0/0	100/0	0/0
I	BMWWW1-t	MT20	4.0	9.0									
J	BMW1+w	MT20	2.0	4.0									

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO	
FR-TO		FROM	TO	CSI (LC)	FR-TO				
A-B	0/14	-84.9	-84.9	0.03 (1)	10.00	J-C	-198/0		0.03 (1)
B-L	-32/0	-84.9	-84.9	0.01 (1)	6.25	C-I	-22/0		0.01 (1)
L-C	-53/0	-84.9	-84.9	0.05 (1)	6.25	I-D	-516/0		0.07 (1)
C-D	-7/0	-84.9	-84.9	0.34 (1)	10.00	D-E	-26/0		0.01 (1)
D-E	-7/0	-84.9	-84.9	0.34 (1)	10.00	E-H	-232/0		0.03 (1)
E-N	-54/0	-84.9	-84.9	0.09 (1)	6.25	H-K	-129/0		0.00 (1)
N-F	-39/0	-84.9	-84.9	0.02 (1)	6.25	K-M	-159/0		0.00 (1)
F-G	0/15	-84.9	-84.9	0.04 (1)	10.00				
B-K	0/41	-18.5	-18.5	0.05 (1)	10.00				
K-J	0/41	-18.5	-18.5	0.07 (4)	10.00				
J-I	0/28	-18.5	-18.5	0.09 (4)	10.00				
I-H	0/31	-18.5	-18.5	0.09 (4)	10.00				
H-M	0/47	-18.5	-18.5	0.09 (1)	10.00				
M-F	0/47	-18.5	-18.5	0.09 (1)	10.00				

Structural component only
DWG# T-2126695

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

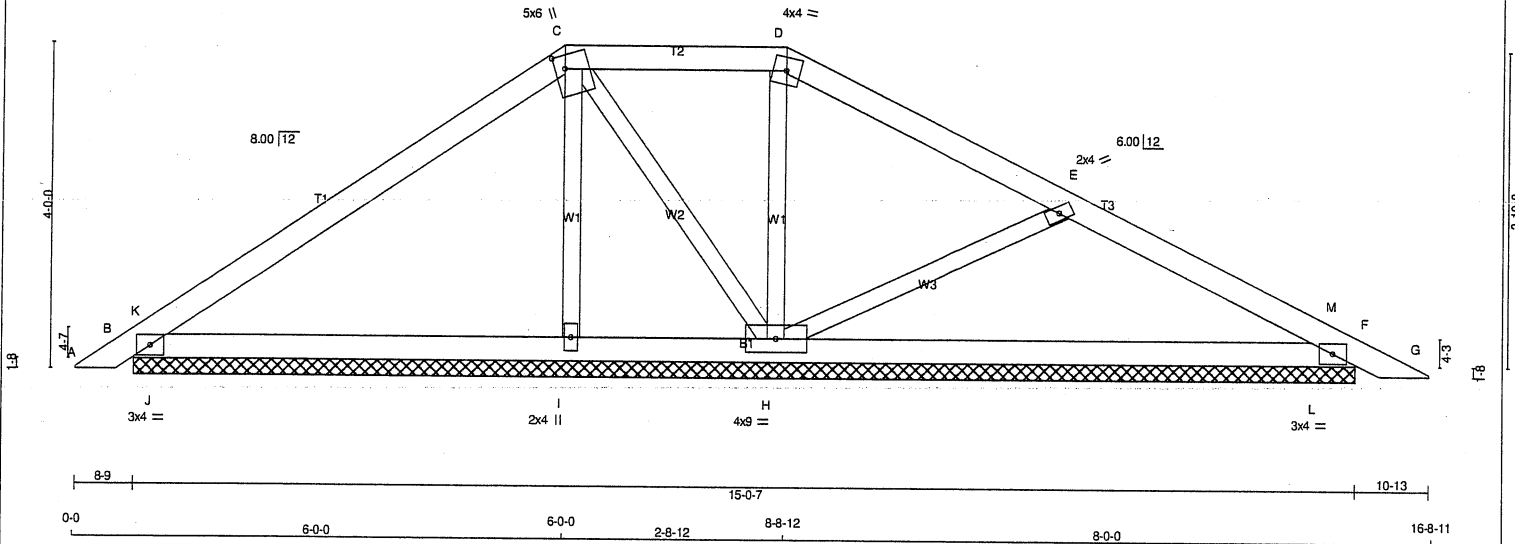
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (D) (INPUT = 0.90)
 JSI METAL= 0.11 (D) (INPUT = 1.00)

JOB NAME 420345	TRUSS NAME PB2	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Aug 5 11:37:33 2021 Page 1
ID:S8hvrR8eJge955Hpp7HWlpPyrYaD-AeielVv5rCzsiobXWdWXBuccG5ifuFAbLPUZ2yqwX0
0-0 6-0-0 6-0-0 2-8-12 8-8-12 3-4-9 12-1-5 4-7-7 16-8-11
Scale = 1:27.0



LUMBER				TOTAL WEIGHT = 2 X 50 = 101 lb			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF	SPF	SPF	SPF
CHORDS							
A - C	2x4 DRY	No.2	SPF				
C - D	2x4 DRY	No.2	SPF				
D - G	2x4 DRY	No.2	SPF				
B - F	2x4 DRY	No.2	SPF				
ALL WEBS	2x3 DRY	No.2	SPF				
DRY: SEASONED LUMBER.							

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TTW-m	MT20	4.0	4.0	
E	TMW+w	MT20	2.0	4.0	
F	TMB1-I	MT20	3.0	4.0	
H	BMWW1-t	MT20	4.0	9.0	
I	BMW1+w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	341	0	341	0	0	15-0-7	15-0-7
F	391	0	391	0	0	15-0-7	15-0-7
I	235	0	235	0	0	15-0-7	15-0-7
H	714	0	714	0	0	15-0-7	15-0-7

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	PERM. LIVE	WIND	DEAD	SOIL
B	241	159 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
F	277	182 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
I	168	99 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
H	505	329 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. MAX. (LC)	MEMB.	FORCE (LBS)	MAX. MAX. (LC)	MEMB.
FR-TO		FROM TO	CSI (LC)	FR-TO		CSI (LC)	
A-B	0 / 14	-84.9	-84.9 0.03 (1)	10.00	I-C	-102 / 0	0.02 (1)
B-K	-31 / 138	-84.9	-84.9 0.15 (1)	6.25	C-H	-216 / 0	0.07 (1)
K-C	-65 / 0	-84.9	-84.9 0.29 (1)	6.25	H-D	-263 / 0	0.06 (1)
C-D	0 / 86	-84.9	-84.9 0.12 (1)	10.00	H-E	-420 / 0	0.12 (1)
D-E	0 / 70	-84.9	-84.9 0.19 (1)	10.00	J-K	-542 / 0	0.00 (1)
E-M	-303 / 0	-84.9	-84.9 0.18 (1)	6.25	L-M	0 / 172	0.00 (1)
M-F	-419 / 0	-84.9	-84.9 0.14 (4)	6.25			
F-G	0 / 15	-84.9	-84.9 0.04 (1)	10.00			
B-J	0 / 45	-18.5	-18.5 0.23 (1)	10.00			
J-I	0 / 45	-18.5	-18.5 0.23 (1)	10.00			
I-H	0 / 42	-18.5	-18.5 0.15 (4)	10.00			
H-L	0 / 295	-18.5	-18.5 0.17 (4)	10.00			
L-F	0 / 295	-18.5	-18.5 0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS DESIGN IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-K:1), BC=0.23/1.00 (I-J:1), WB=0.12/1.00 (E-H:1), SS=0.42/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(Psi) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (E) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 1.00)

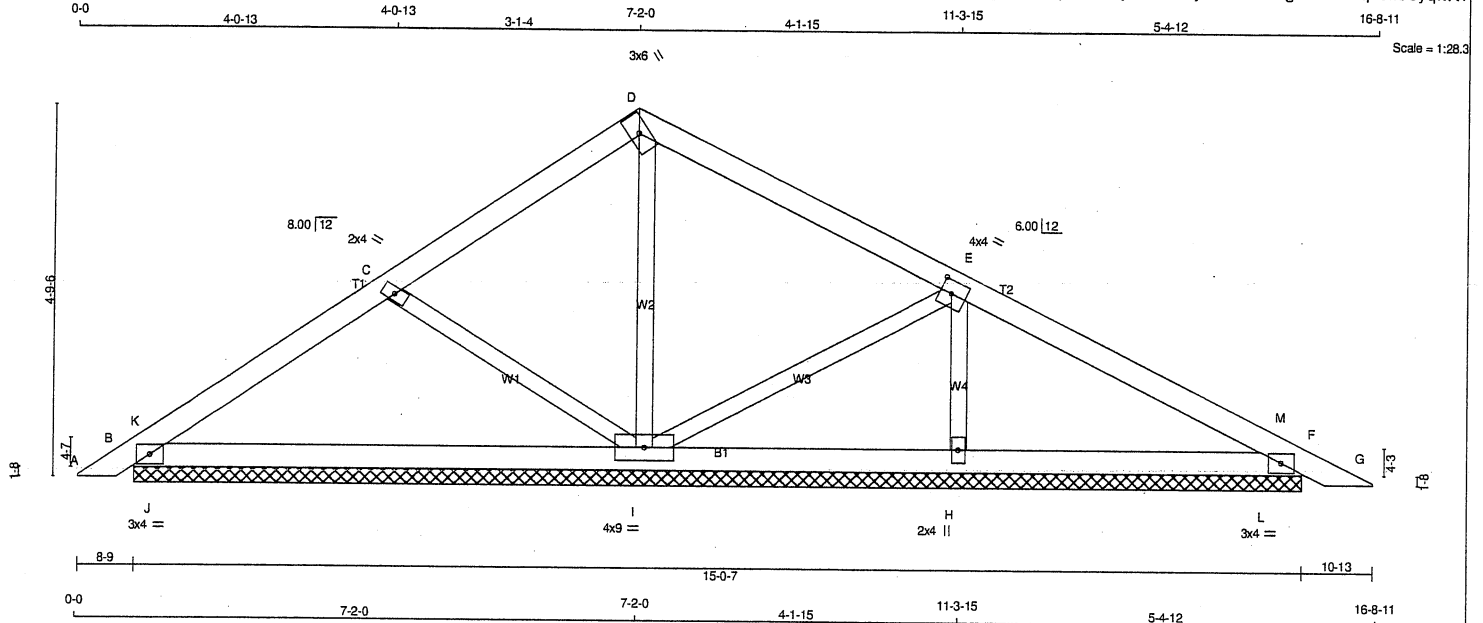


Structural component only
DWG# T-2126696

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420345	PB3	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:S8hvR8eJge955Hpp7HWlpPyrYaD-eqG1YFWjcW5iKxmjULJl4ORo3gRHOL1Jq7825UyqwX?



LUMBER					DESCR.
N. L. G. A. RULES	SIZE	LUMBER			
CHORDS					SPF
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
B - F	2x4	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMW1-w	MT20	2.0	4.0	
D	TTW1-h	MT20	3.0	6.0	
E	TMWW1-t	MT20	4.0	4.0	2.00 1.75
F	TMB1-I	MT20	3.0	4.0	
H	BMW1-w	MT20	2.0	4.0	
I	BMWW1-t	MT20	4.0	9.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
B		358	0	358	0	15-0-7	15-0-7	15-0-7	15-0-7
F		283	0	283	0	15-0-7	15-0-7	15-0-7	15-0-7
I		614	0	614	0	15-0-7	15-0-7	15-0-7	15-0-7
H		426	0	426	0	15-0-7	15-0-7	15-0-7	15-0-7

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
B	253	167 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0		
F	199	134 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0		
I	436	276 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0		
H	302	193 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. UNBRAC LENGTH	MAX. FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 14	-84.9 -84.9	0.03 (1)	10.00	C-I	-311 / 0	0.09 (1)
B-K	-379 / 0	-84.9 -84.9	0.11 (4)	6.25	I-D	-277 / 0	0.09 (1)
K-C	-241 / 0	-84.9 -84.9	0.15 (1)	6.25	I-E	-117 / 0	0.04 (1)
C-D	0 / 22	-84.9 -84.9	0.15 (1)	10.00	H-E	-327 / 0	0.05 (1)
D-E	0 / 26	-84.9 -84.9	0.25 (1)	10.00	J-K	0 / 178	0.00 (1)
E-M	-49 / 0	-84.9 -84.9	0.25 (1)	6.25	L-M	-149 / 17	0.00 (1)
M-F	-67 / 0	-84.9 -84.9	0.03 (4)	6.25			
F-G	0 / 15	-84.9 -84.9	0.04 (1)	10.00			
B-J	0 / 224	-18.5 -18.5	0.09 (4)	10.00			
J-I	0 / 224	-18.5 -18.5	0.15 (4)	10.00			
I-H	0 / 62	-18.5 -18.5	0.13 (4)	10.00			
H-L	0 / 62	-18.5 -18.5	0.10 (1)	10.00			
L-F	0 / 62	-18.5 -18.5	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (D-E:1), BC=0.15/1.00 (I-J:4), WB=0.09/1.00 (D-I:1), SSI=0.16/1.00 (B-K:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (E) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 1.00)

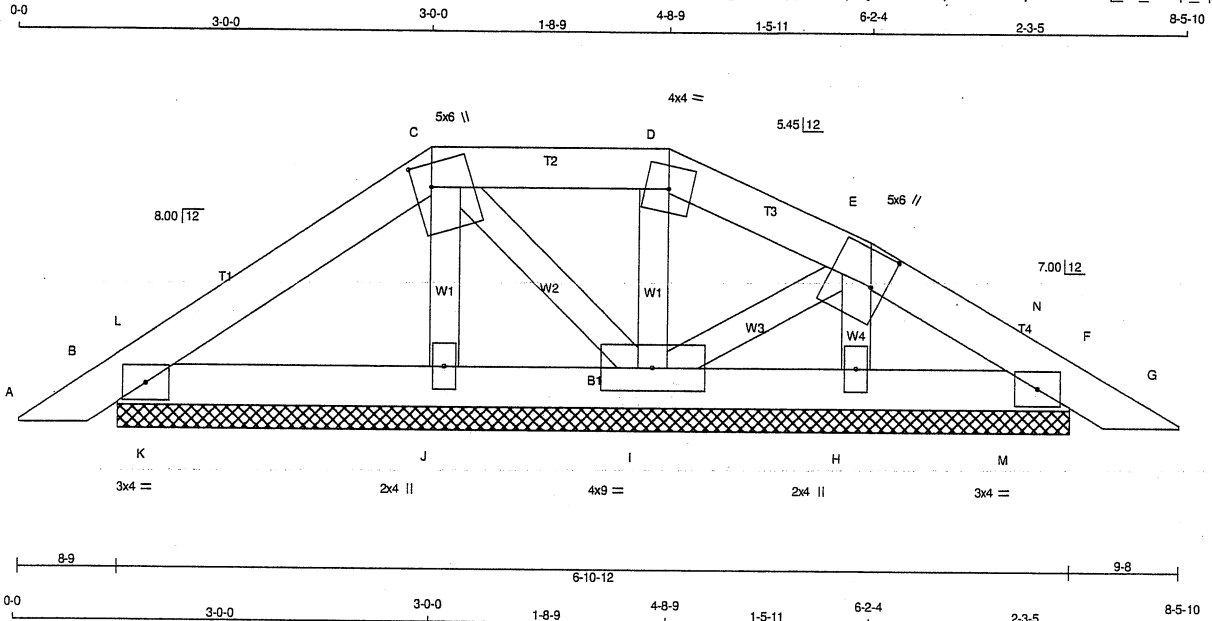


Structural component only
DWG# T-2126697

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
420345	PB4	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:S8hvR8eJge955Hpp7HWlpPyrYaD-71qPmbWLNqDZx5Lv12q_cc_0w4p_7pLT2fubdwyqwX



Scale: 3/4"=1'

LUMBER					DESCR.	
N. L. & A. RULES						
CHORDS	SIZE	LUMBER				
A - C	2x4	DRY	No.2			SPF
C - D	2x4	DRY	No.2			SPF
D - E	2x4	DRY	No.2			SPF
E - G	2x4	DRY	No.2			SPF
B - F	2x4	DRY	No.2			SPF
ALL WEBS 2x3 DRY					No.2	SPF
DRY: SEASONED LUMBER.						

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TTW-m	MT20	4.0	4.0	
E	TTWW+m	MT20	5.0	6.0	3.00 1.25
F	TMB1-I	MT20	3.0	4.0	
H	BMW1+w	MT20	2.0	4.0	
I	BMW1+w	MT20	4.0	9.0	
J	BMW1+w	MT20	2.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	192	0	192	0	0	6-10-12	6-10-12	6-10-12	6-10-12
F	151	0	151	0	0	6-10-12	6-10-12	6-10-12	6-10-12
J	170	0	170	0	0	6-10-12	6-10-12	6-10-12	6-10-12
I	169	0	169	0	0	6-10-12	6-10-12	6-10-12	6-10-12
H	148	0	148	0	0	6-10-12	6-10-12	6-10-12	6-10-12

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
B	134	94 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0	
F	105	76 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0	
J	122	71 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0	
I	119	80 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0	
H	106	64 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	FORCE (LBS)	LC1 MAX. (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 14	-84.9	-84.9 0.03 (1)	10.00	J-C	-95 / 0	0.01 (1)
B-L	-39 / 0	-84.9	-84.9 0.01 (1)	6.25	C-I	-44 / 0	0.01 (1)
L-C	-50 / 0	-84.9	-84.9 0.05 (1)	6.25	I-D	-130 / 0	0.02 (1)
C-D	-2 / 0	-84.9	-84.9 0.04 (1)	10.00	I-E	-9 / 0	0.00 (1)
D-E	-12 / 0	-84.9	-84.9 0.03 (1)	6.25	H-E	-95 / 0	0.01 (1)
E-N	-34 / 0	-84.9	-84.9 0.02 (1)	6.25	K-L	-114 / 0	0.00 (1)
N-F	-40 / 0	-84.9	-84.9 0.00 (4)	6.25	M-N	-50 / 0	0.00 (1)
F-G	0 / 14	-84.9	-84.9 0.03 (1)	10.00			
B-K	0 / 40	-18.5	-18.5 0.05 (1)	10.00			
K-J	0 / 40	-18.5	-18.5 0.05 (1)	10.00			
J-I	0 / 34	-18.5	-18.5 0.05 (1)	10.00			
I-H	0 / 19	-18.5	-18.5 0.02 (1)	10.00			
H-M	0 / 30	-18.5	-18.5 0.03 (1)	10.00			
M-F	0 / 30	-18.5	-18.5 0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD.

CSI: TC=0.05/1.00 (C-L:1), BC=0.05/1.00 (B-K:1),
WB=0.02/1.00 (D-I:1), SSI=0.09/1.00 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (D) (INPUT = 1.00)



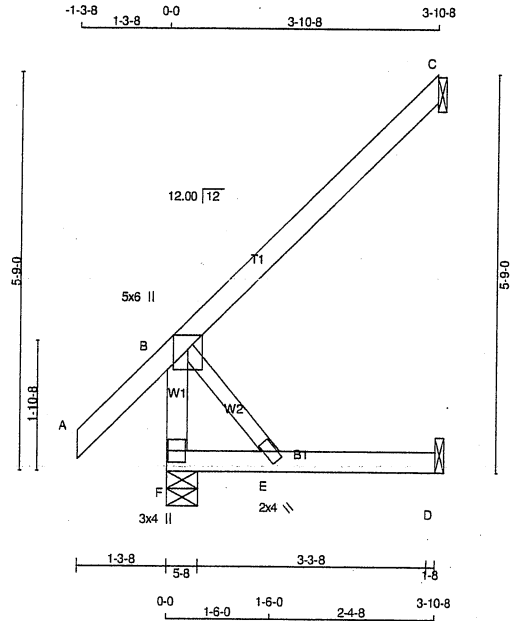
Structural component only
DWG# T-2126698

Structural component only
DWG# T-2126699

JOB NAME 420345	TRUSS NAME J1	QUANTITY 26	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 26 X 16 = 426 lb

LUMBER					DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			SPF	
CHORDS					SPF	
F - B	2x4	DRY	No.2		SPF	
A - C	2x4	DRY	No.2		SPF	
F - D	2x4	DRY	No.2		SPF	
ALL WEBS	2x3	DRY	No.2		SPF	
DRY: SEASONED LUMBER.						

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
F	319	0	319	0	5-8	5-8
C	164	0	164	0	1-8	1-8
D	36	0	40	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
F	224	155 / 0	0 / 0	0 / 0	69 / 0	0 / 0	0 / 0
C	113	90 / 0	0 / 0	0 / 0	23 / 0	0 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	MEMB.	MAX. FORCE (LBS)
FR-TO				FR-TO	
F-B	-283 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 42	-84.9	-84.9	0.14 (5)	10.00
B-C	0 / 0	-84.9	-84.9	0.22 (1)	10.00
F-E	0 / 0	-18.5	-18.5	0.08 (4)	10.00
E-D	0 / 0	-18.5	-18.5	0.08 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	36.7	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

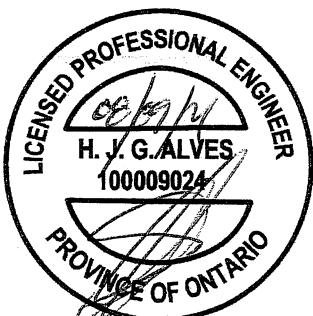
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

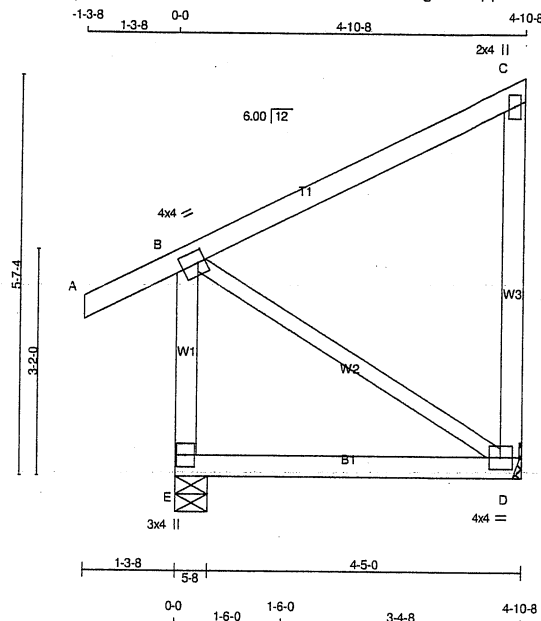
JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126691

JOB NAME 420345	TRUSS NAME J3	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER					DESCR.
N.L.G.A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
E - D	2x4	DRY	No.2		SPF
ALL WEBS EXCEPT D - C	2x3	DRY	No.2		SPF
	2x4	DRY	No.2		SPF

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW+w	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	360	0	360	0	5-8	5-8
D	239	0	239	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	253	173 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
D	170	107 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED UNBRAC LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO	
E-B	-316 / 0	0.0 0.0 0.05 (1)	7.81	B-D	0 / 3
A-B	0 / 26	-84.9 -84.9 0.11 (1)	10.00	D-C	-197 / 0
B-C	-3 / 0	-84.9 -84.9 0.31 (1)	10.00		0.07 (1)
E-D	0 / 0	-18.5 -18.5 0.12 (4)	10.00		

TOTAL WEIGHT = 2 X 26 = 53 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.31/1.00 (B-C:1), BC=0.12/1.00 (D-E:4),
WB=0.07/1.00 (C-D:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

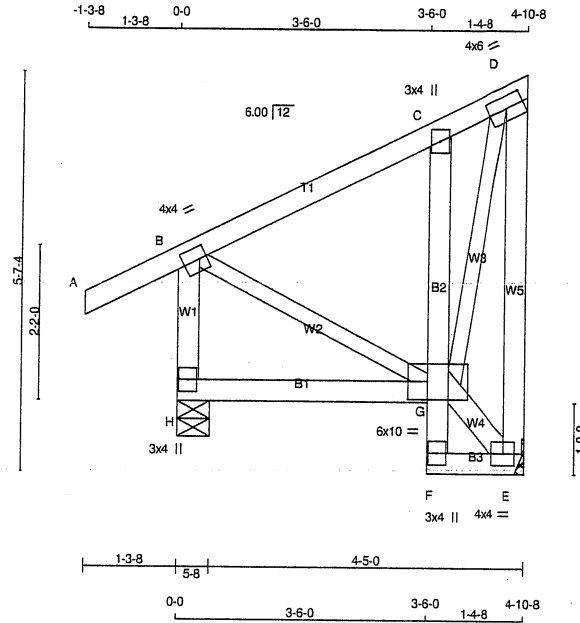


Structural component only
DWG# T-2126692

JOB NAME 420345	TRUSS NAME J3S	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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Scale = 1:30.8

LUMBER				
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	TMVW-t	MT20	4.0	6.0	2.00	3.00
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWW-I	MT20	6.0	10.0	3.00	3.25
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	359	0	359	0	5-8	5-8
E	225	0	225	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
H	253	173 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
E	160	100 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
H-B	-325 / 0	B-G	0 / 93
A-B	0 / 26	G-E	-7 / 0
B-C	-82 / 0	G-D	0 / 316
C-D	-127 / 0	E-D	-210 / 0
H-G	0 / 0		
F-G	0 / 11		
G-C	-313 / 0		
F-E	0 / 15		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.07/1.00 (G-H:4) , WB=0.07/1.00 (D-G:1) , SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

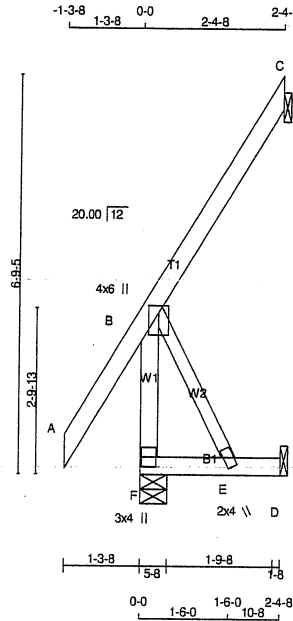
JSI GRIP= 0.26 (D) (INPUT = 0.90)
JSI METAL= 0.10 (G) (INPUT = 1.00)



Structural component only
DWG# T-2126693

JOB NAME 420345	TRUSS NAME J4	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:37.4

LUMBER				
N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
CHORDS				
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	1.50 2.00
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	243	0	243	0	0	5-8	5-8		
C	101	0	101	0	0	1-8	1-8		
D	22	0	25	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	170	121 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
C	70	55 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	18	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX (LC)	MAX (LC)
FR-TO		FROM	TO	FR-TO			
F-B	-221 / 0	0.0	0.0 0.03 (1)	B-E	0 / 0	0.00 (1)	
A-B	0 / 52	-84.9	-84.9 0.13 (5)				
B-C	0 / 0	-84.9	-84.9 0.08 (1)				
F-E	0 / 0	-18.5	-18.5 0.03 (4)				
E-D	0 / 0	-18.5	-18.5 0.03 (4)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 16 = 66 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 36.7 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.13/1.00 (A-B:5), BC=0.03/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.05/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2126694



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300213

Feb 09, 2018

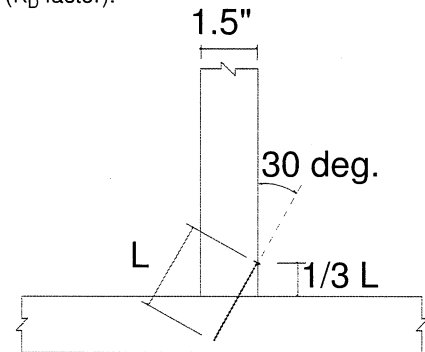
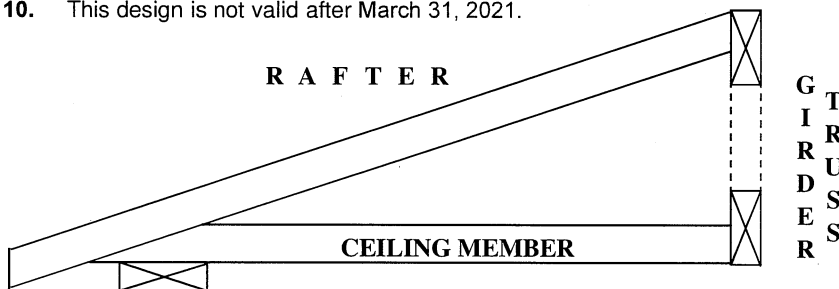
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

PEO
Certificate No. 10889485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

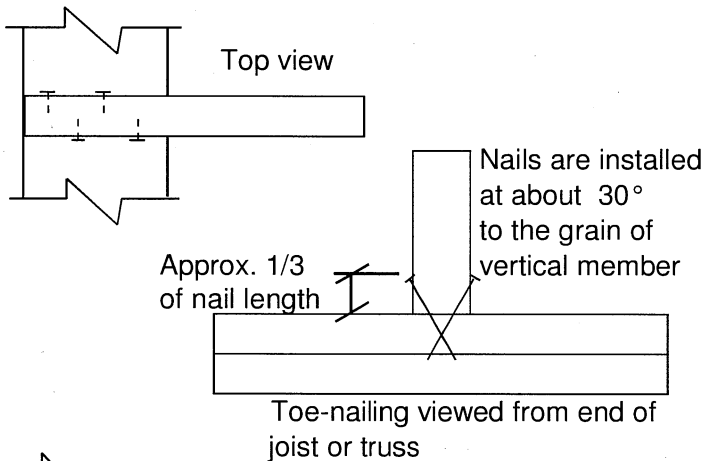
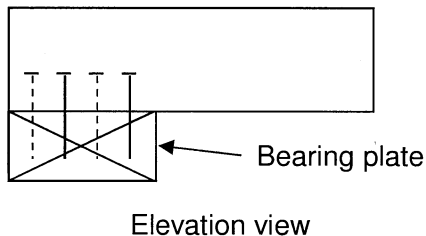
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

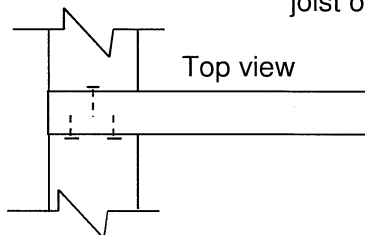
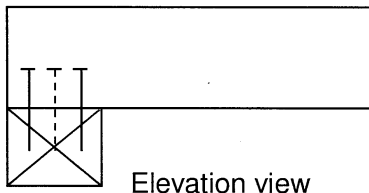
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek®

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



December 2, 2019

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

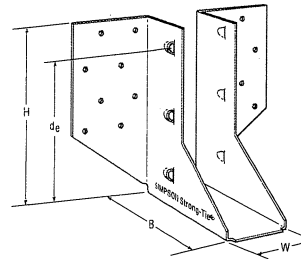
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

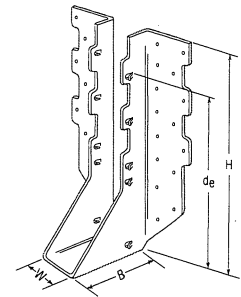
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

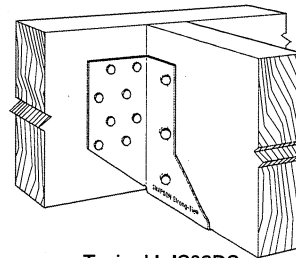
- See current catalogue for options



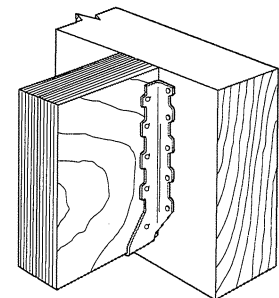
LJS26DS



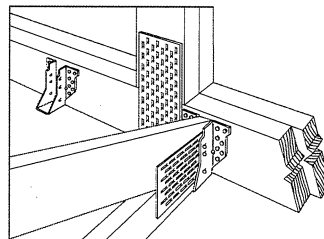
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



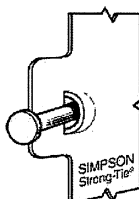
Typical HUS
Installation



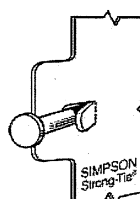
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15) lb.	Normal (K _o =1.00) lb.	Uplift (K _o =1.15) lb.	Normal (K _o =1.00) lb.
LJS26DS	18	1⅞	5	3½	4⅞	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅞	5⅞	3	3⅞	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅞	9⅞	3	7⅞	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1⅞	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

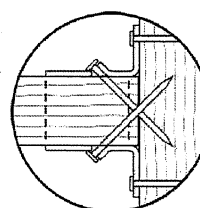
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

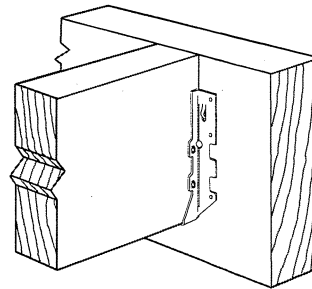
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

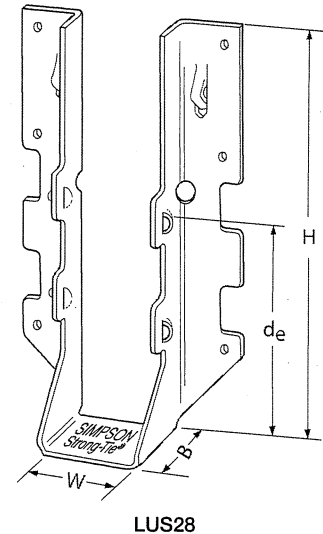
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

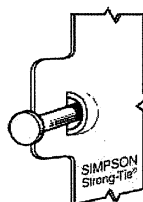


Typical LUS Installation



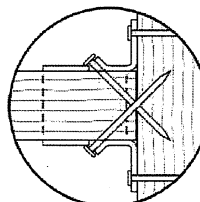
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LUS24	18	1½	3½	1¾	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4½	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6½	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4½	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 15/16	1¾	3½	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4½	8 3/16	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

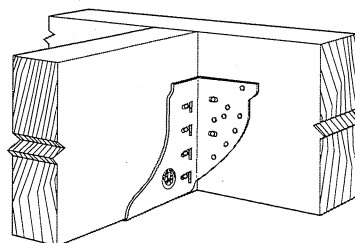
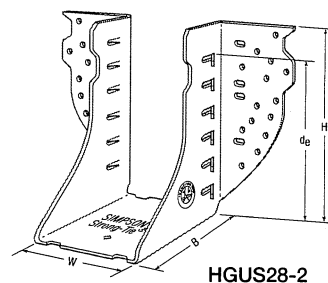
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

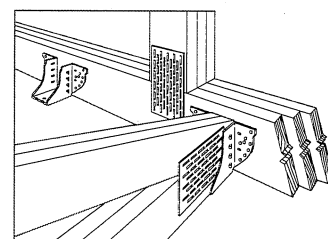
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



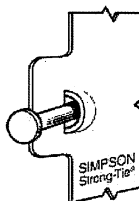
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

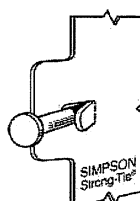
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
HGUS26	12	1⅞	5⅞	5	4⅝	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅞	5⅞	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4⅞	5⅞	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅞	7⅞	5	6⅞	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅞	7⅞	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4⅞	7⅞	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅞	9⅞	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⅞	9⅞	4	8⅞	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4⅞	9⅞	4	8⅞	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅞	9⅞	4	8⅞	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅞	10⅞	4	10⅞	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅞	12⅞	4	11⅞	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

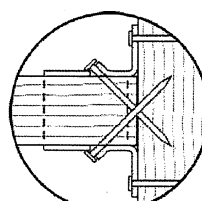


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

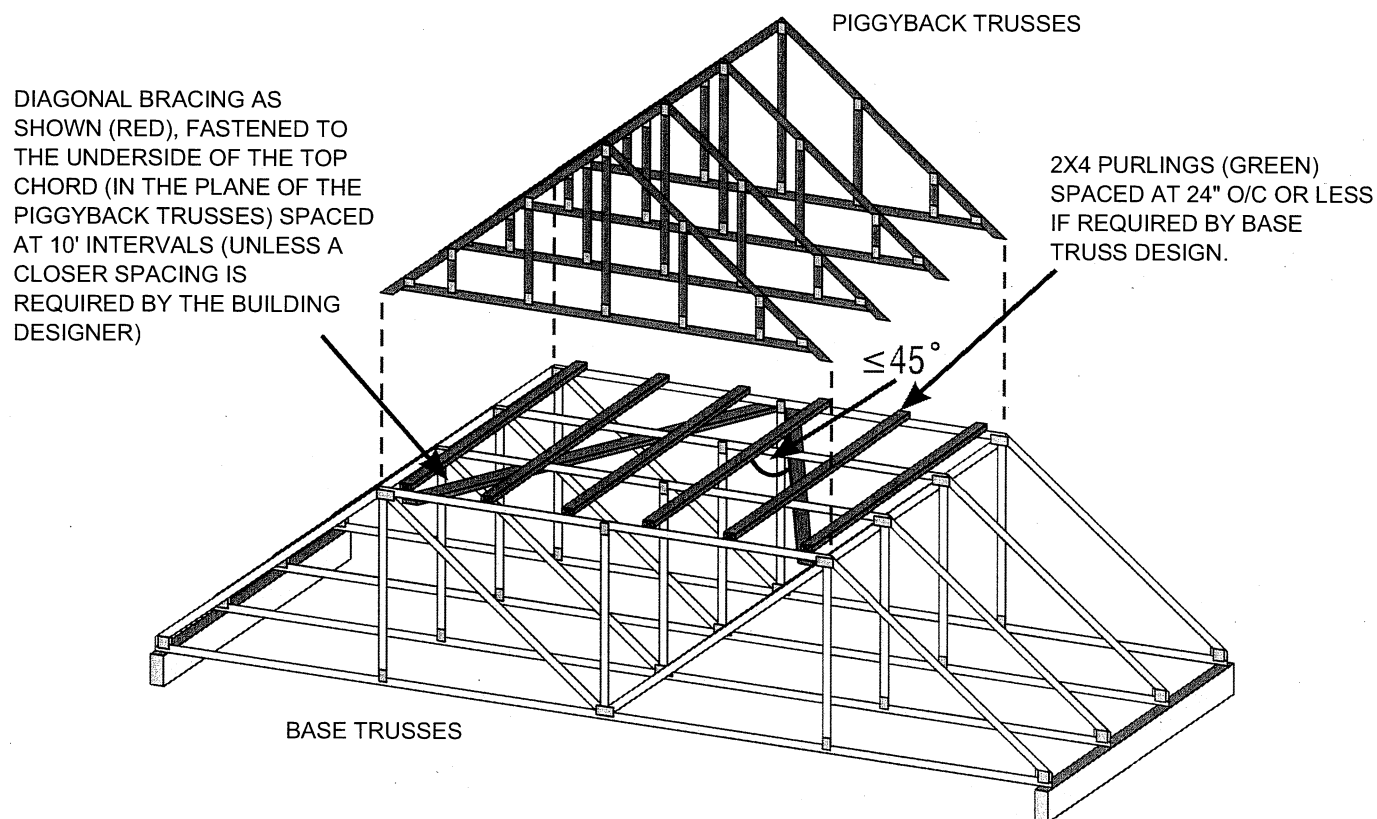
(800) 999-5099
strongtie.com

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

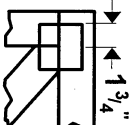
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

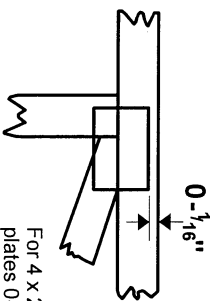
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **MITek 20/20** software or upon request.

PLATE SIZE

4 X 4

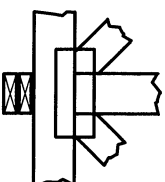
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



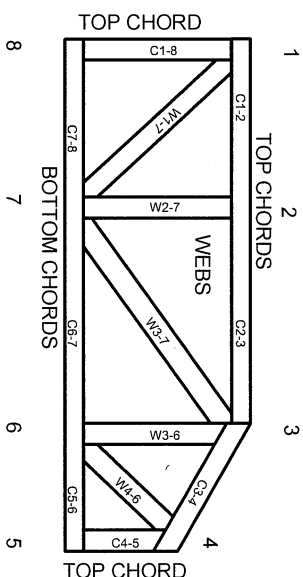
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:
ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3 These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T or I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.