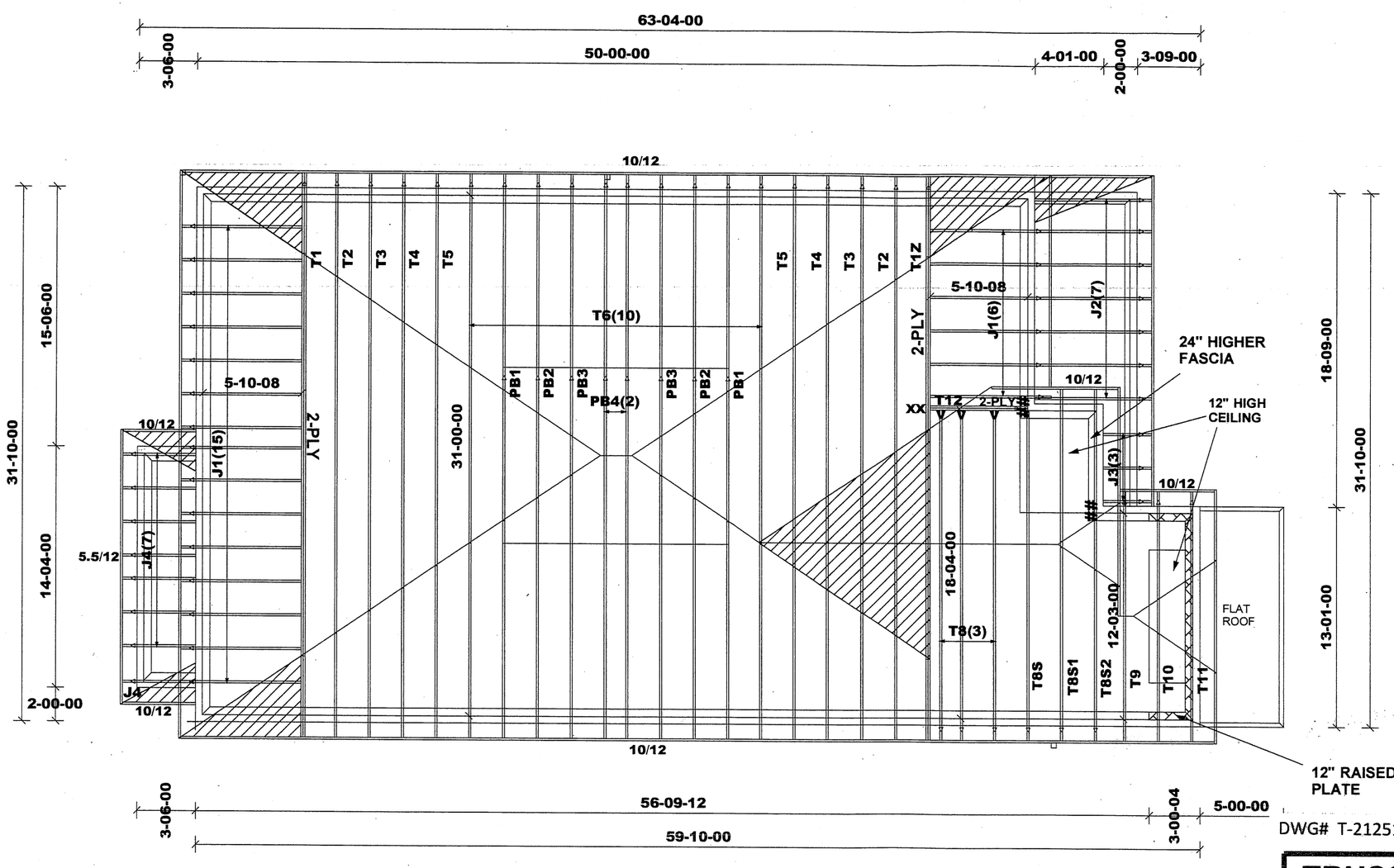
6.75/12 ROOF PITCHES UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DENOTES:
CONVENTIONAL FRAMING

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LJS26DS - (**V**)
HGUS26-2 - (**XX**)

- 12" PLATE DIFF.

LICENSED PROFESSIONAL ENGINEER
08/10/24
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127194
STRUCTURAL COMPONENT ONLY

DWG# T-2125139 to T-2125161, T1800218

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

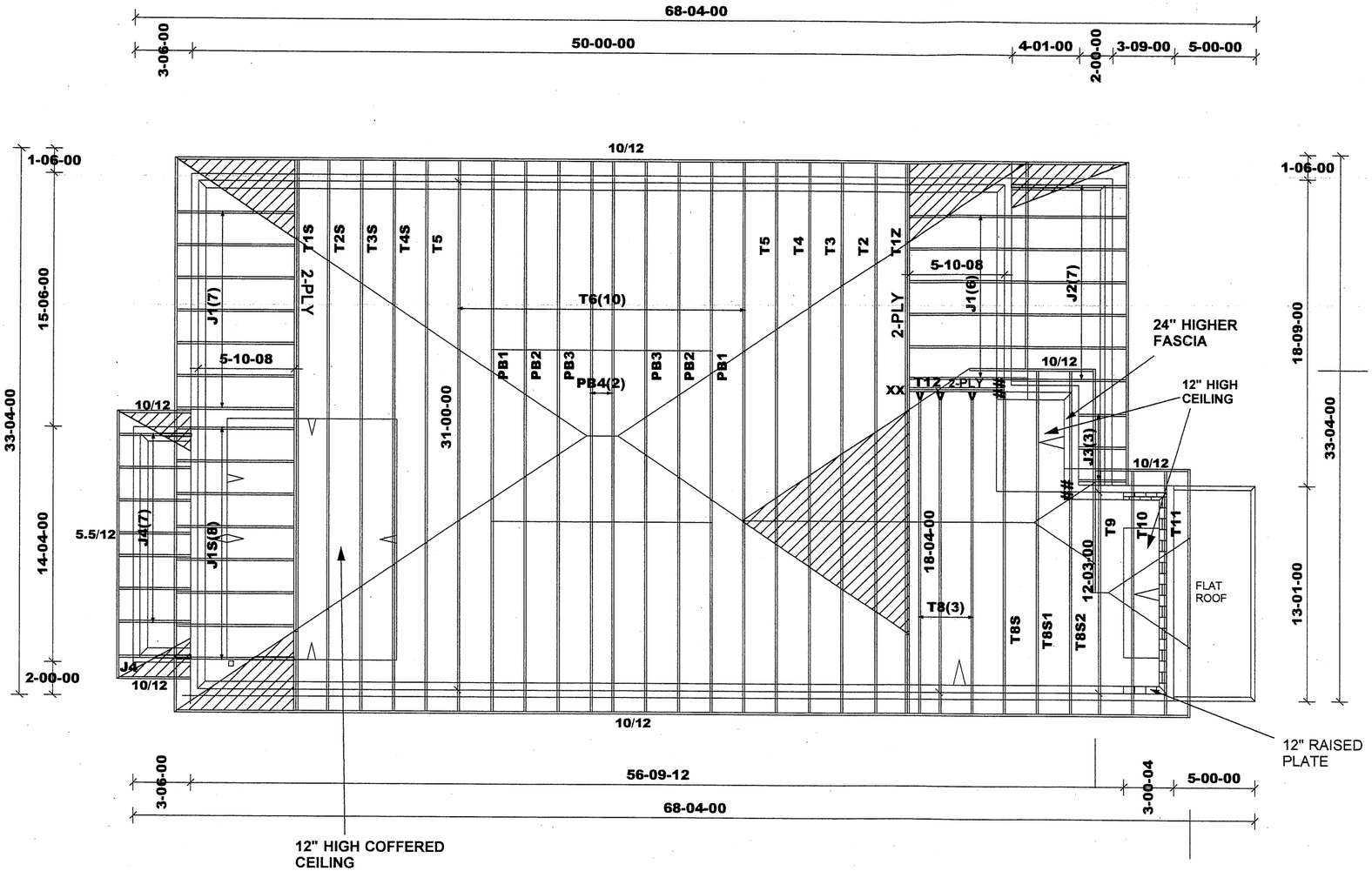
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

M14213

TAMARACK ROOF TRUSSES INC.

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: MODEL 40-3 / A STD 4 & 5 BEDROOM	Mitek ver 8.4.2.286
Layout ID: 420226	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 204995	Date: 2021-08-05	Designer: JC	

6.75/12 ROOF PITCHES UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DENOTES:
CONVENTIONAL FRAMING

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)

- 12" PLATE DIFF.

LICENSED PROFESSIONAL ENGINEER
09/10/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. IAM T-2127195
STRUCTURAL COMPONENT ONLY

DWG# T-2125139 to T-2125146, T-2125148 to T-2125165, T-2126618, T1800218

TRUSS PLACEMENT PLAN.

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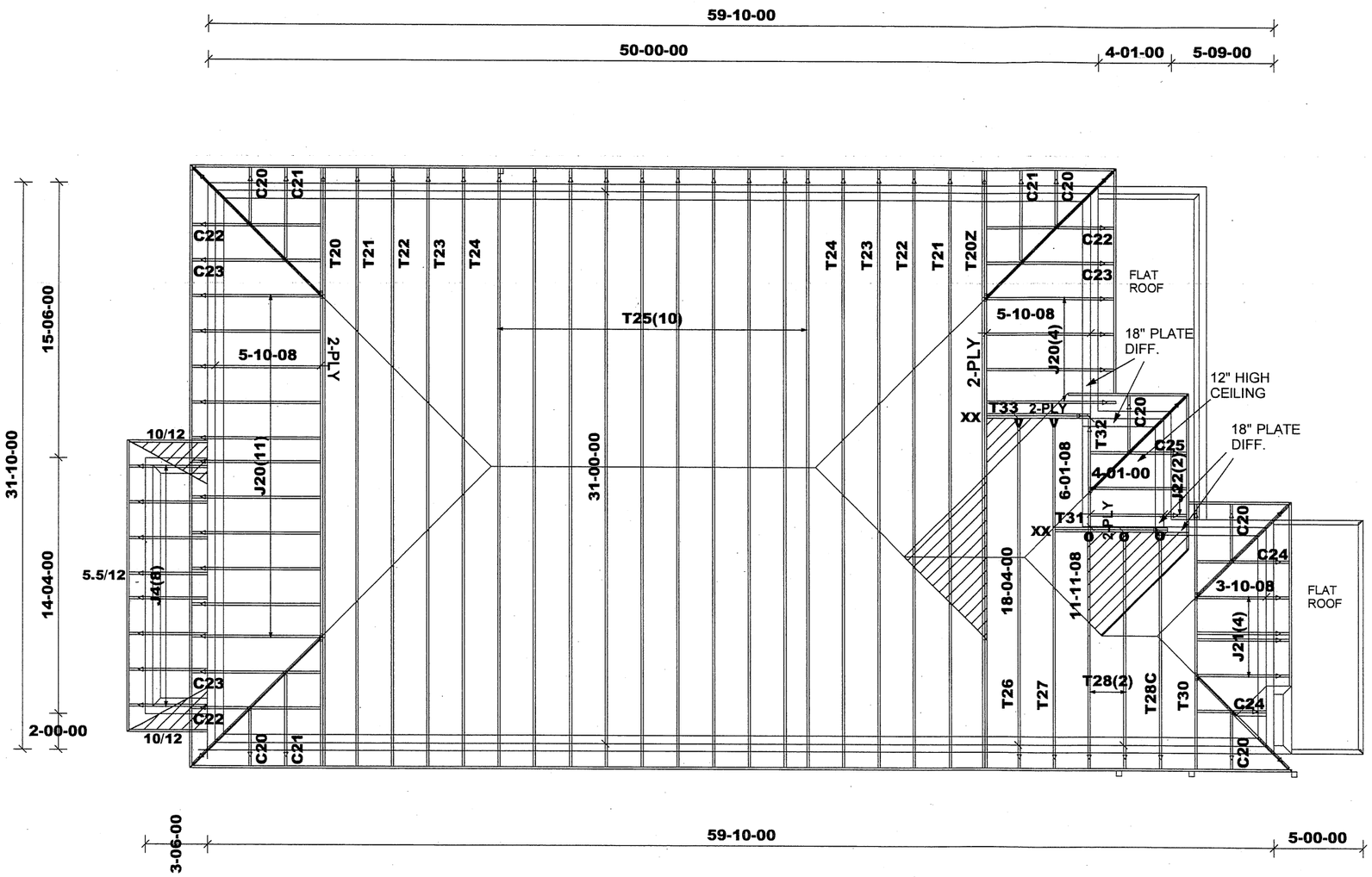
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

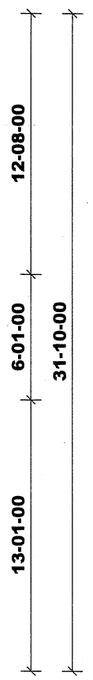
M14213

TAMARACK ROOF TRUSSES INC.
ALSO LUMBER GROUP

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: MODEL 40-3 / A OPT. 4 & 5 BEDROOM	Mitek ver 8.4.2.286
Layout ID: 420227	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 204995	Date: 2021-08-10 Designer: AC		

6.75/12 ROOF PITCHES UNLESS OTHERWISE NOTED





ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DENOTES:
CONVENTIONAL FRAMING

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. J. GALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127196
STRUCTURAL COMPONENT ONLY

DWG# T-2125142, T-2125167 to T-2125172, T-2125174 to T-2125191, T1800218

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

TRUSS PLACEMENT PLAN.

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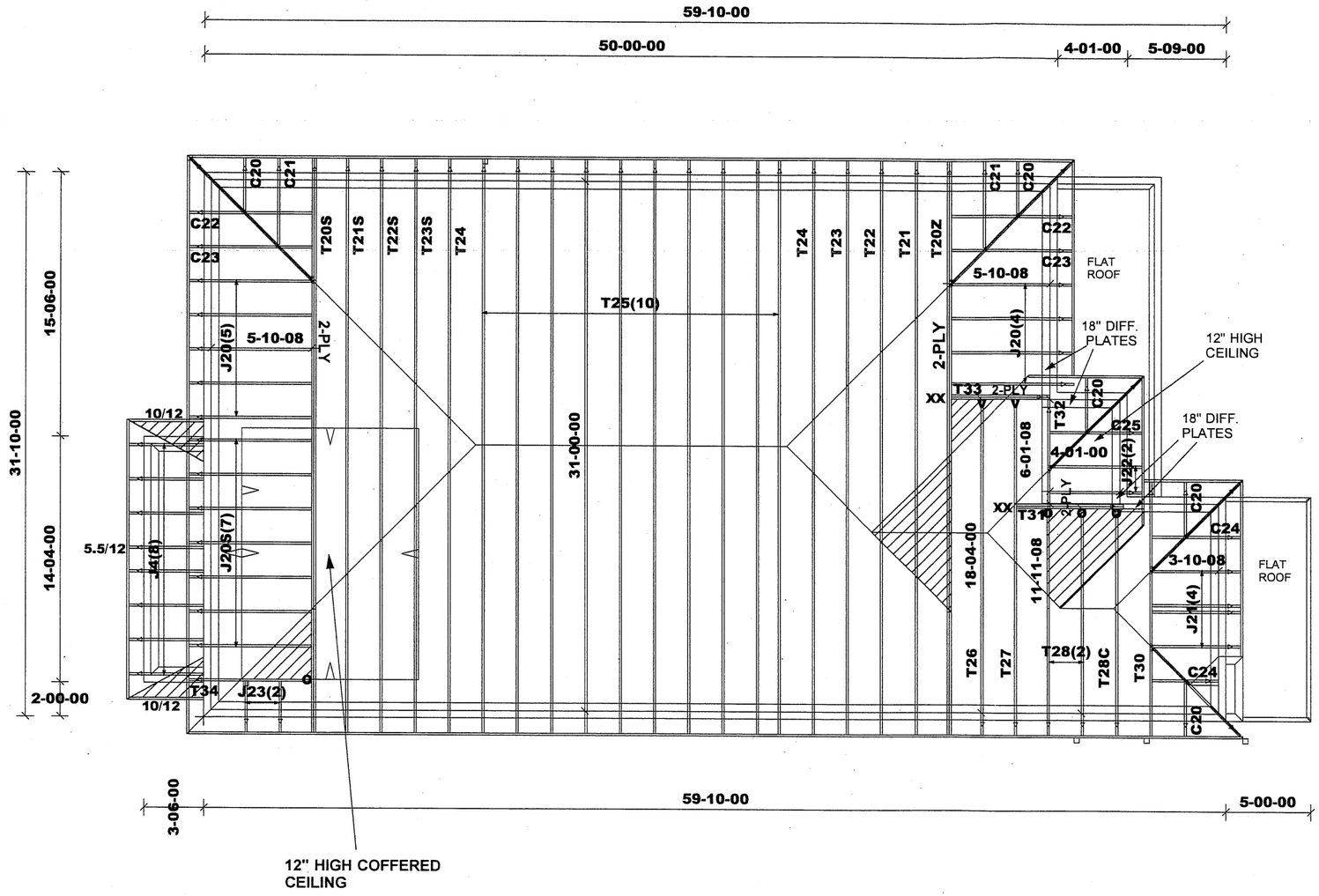
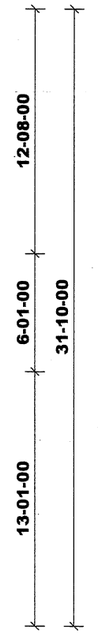
M14213

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: MODEL 40-3 / B STD. 4 & 5 BEDROOM	Mitek ver 8.4.2.286
Layout ID: 420228	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 204995	Date: 2021-08-05	Designer: JC	

6.75/12 ROOF PITCHES UNLESS OTHERWISE NOTED





ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DENOTES:
CONVENTIONAL FRAMING

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127197
STRUCTURAL COMPONENT ONLY

DWG# T-2125142, T-2125167 to T-2125172, T-2125174 to T-2125176, T-2125178 to T-2125196, T-2126373 to T-2126374, T1800218

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

TRUSS PLACEMENT PLAN.

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M14213

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

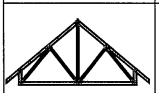
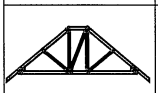
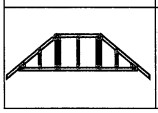
Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: MODEL 40-3 / B OPT. 4 & 5 BEDROOM	Mitek ver 8.4.2.286
Layout ID: 420229	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 204995	Date: 2021-08-05	Designer: JC	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204995
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420226
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-3	Page:	1 of 2
Lot #:		Date:	08-04-2021
Elevation:	A STD 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

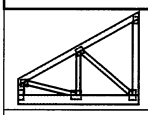
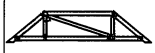
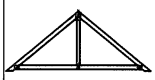
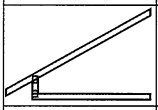
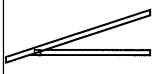
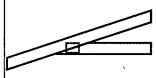
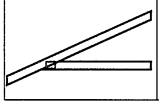
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	344.48 212.00		
	1 2-ply	T1Z Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	344.48 212.00		
	2	T2 Hip	10 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	266.14 173.33		
	2	T3 Hip	10 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	294.82 187.00		
	2	T4 Hip	10 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	300.43 188.33		
	2	T5 Hip	10 /12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	305.44 190.33		
	10	T6 Hip	10 /12	31-00-00	10-02-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	1587.19 983.33		
	3	T8 Common	10 /12	18-04-00	10-03-06	2 x 4	1-03-08	1-07-11 3-07-11	303.22 191.00		
	1	T8S Roof Special	10 /12	18-04-00	10-03-06	2 x 4	1-03-08 1-03-08	1-07-11 2-07-11	106.11 68.83		
	1	T8S1 Hip	10 /12	18-04-00	10-01-08	2 x 4	1-03-08 1-03-08	1-07-11 2-07-11	103.16 66.83		
	1	T8S2 Hip	10 /12	18-04-00	9-00-00	2 x 4	1-03-08 1-03-08	1-07-11 2-07-11	105.72 67.83		
	1	T9 Common	10 /12	12-03-00	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	59.1 38.17		
	1	T10 Hip	10 /12	12-03-00	5-08-05	2 x 4	1-03-08 1-03-08	7-11 7-11	53.92 36.17		
	1	T11 GABLE	10 /12	12-03-00	4-06-13	2 x 4	1-03-08 1-03-08	7-11 7-11	52.74 35.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204995
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420226
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-3	Page:	2 of 2
Lot #:		Date:	08-04-2021
Elevation:	A STD 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	**1** **2-ply**	**T12** **Monopitch Girder**	6.75 /1 2	**5-10-08**	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	**2**	**PB1** **Piggyback**	10 /12	**10-05-15**	1-01-08	2 x 4			60.53 40.33		
	**2**	**PB2** **Piggyback**	10 /12	**10-05-15**	2-03-00	2 x 4			61.66 41.00		
	**2**	**PB3** **Piggyback**	10 /12	**10-05-15**	3-04-08	2 x 4			64.57 43.33		
	**2**	**PB4** **Piggyback**	10 /12	**10-05-15**	4-04-08	2 x 4			57.95 36.00		
	**21**	**J1** **Jack-Open**	6.75 /1 2	**5-10-08**	4-06-12	2 x 4	1-03-08	1-03-01 4-06-12	360.5 224.00		
	**7**	**J2** **Jack-Open**	4 /12	**5-08-00**	2-10-14	2 x 4	1-03-08	3-15 2-02-10	118.18 74.67		
	**3**	**J3** **Jack-Open**	4 /12	**1-07-00**	1-06-08	2 x 4	1-03-08	3-15 10-04	21.97 16.00		
	**8**	**J4** **Jack-Open**	5.5 /12	**3-01-00**	2-08-08	2 x 4	1-03-08	4-02 1-09-01	89.21 53.33		

TOTAL # TRUSS= **80** TOTAL BFT OF ALL TRUSSES= **3217.14** BFT. TOTAL WEIGHT OF ALL TRSSES **5121.58** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

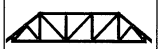
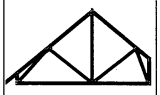
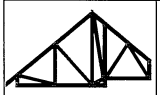
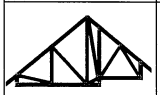
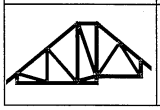
TOTAL NUMBER OF ITEMS= **4**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204995
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420227
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-3	Page:	1 of 3
Lot #:		Date:	08-04-2021
Elevation:	A OPT. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

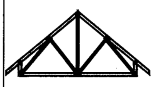
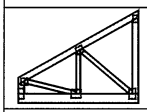
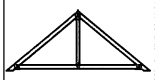
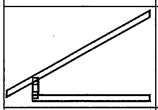
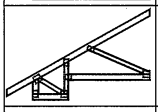
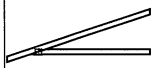
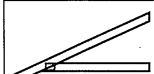
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	359.48 230.67		
	1 2-ply	T1Z Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	344.48 212.00		
	1	T2 Hip	10 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	126.31 81.17		
	1	T2S Hip	10 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	141.21 91.83		
	1	T3 Hip	10 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	138.51 87.50		
	1	T3S Hip	10 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	152.2 97.67		
	1	T4 Hip	10 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.22 94.17		
	1	T4S Hip	10 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	156.29 99.00		
	2	T5 Hip	10 /12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	305.44 190.33		
	10	T6 Hip	10 /12	31-00-00	10-02-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	1587.19 983.33		
	3	T8 Common	10 /12	18-04-00	10-03-06	2 x 4	1-03-08	1-07-11 3-07-11	253.84 161.00		
	1	T8S Roof Special	10 /12	18-04-00	10-03-06	2 x 4	1-03-08	1-07-11 2-07-11	104.19 67.50		
	1	T8S1 Hip	10 /12	18-04-00	10-01-08	2 x 4	1-03-08 1-03-08	1-07-11 2-07-11	103.16 66.83		
	1	T8S2 Hip	10 /12	18-04-00	9-00-00	2 x 4	1-03-08 1-03-08	1-07-11 2-07-11	105.72 67.83		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204995
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420227
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-3	Page:	2 of 3
Lot #:		Date:	08-04-2021
Elevation:	A OPT. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T9 Common	10 /12	12-03-00	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	59.1 38.17		
	1	T10 Hip	10 /12	12-03-00	5-08-05	2 x 4	1-03-08 1-03-08	7-11 7-11	53.92 36.17		
	1	T11 Hip	10 /12	12-03-00	4-06-13	2 x 4	1-03-08 1-03-08	7-11 7-11	47.04 31.33		
	1 2-ply	T12 Monopitch Girder	6.75 /12	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	2	PB1 Piggyback	10 /12	10-05-15	1-01-08	2 x 4			60.53 40.33		
	2	PB2 Piggyback	10 /12	10-05-15	2-03-00	2 x 4			61.66 41.00		
	2	PB3 Piggyback	10 /12	10-05-15	3-04-08	2 x 4			64.57 43.33		
	2	PB4 Piggyback	10 /12	10-05-15	4-04-08	2 x 4			57.95 36.00		
	13	J1 Jack-Open	6.75 /12	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 4-06-12	223.17 138.67		
	8	J1S Jack-Open	6.75 /12	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 3-06-12	179.27 122.67		
	7	J2 Jack-Open	4 /12	5-08-00	2-10-14	2 x 4	1-03-08	3-15 2-02-10	118.22 74.67		
	3	J3 Jack-Open	4 /12	1-07-00	1-06-08	2 x 4	1-03-08	3-15 10-04	21.97 16.00		
	8	J4 Jack-Open	5.5 /12	3-01-00	2-08-08	2 x 4	1-03-08	4-02 1-09-01	89.21 53.33		

TOTAL # TRUSS= **80** TOTAL BFT OF ALL TRUSSES= **3240.5** BFT. TOTAL WEIGHT OF ALL TRSSES **5124.93** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204995
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420227
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-3	Page:	3 of 3
Lot #:		Date:	08-04-2021
Elevation:	A OPT. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

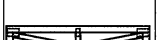
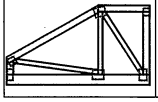
TOTAL NUMBER OF ITEMS= **4**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204995
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420228
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-3	Page:	1 of 2
Lot #:		Date:	08-04-2021
Elevation:	B STD. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T20 Hip Girder	6.75 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	320.2 193.33		
	1 2-ply	T20Z Hip Girder	6.75 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	320.2 193.33		
	2	T21 Hip	6.75 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	256.4 164.00		
	2	T22 Hip	6.75 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	262.3 165.00		
	2	T23 Hip	6.75 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	288.3 184.67		
	2	T24 Hip	6.75 /12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	284.85 179.00		
	10	T25 Common	6.75 /12	31-00-00	9-11-11	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	1348.68 833.33		
	1	T26 Common	6.75 /12	18-04-00	7-01-15	2 x 4	1-03-08	1-03-01 2-09-01	77.53 50.33		
	1	T27 Hip Girder	6.75 /12	18-04-00	6-02-02	2 x 4 2 x 6	1-03-08	1-03-01 2-09-01	90.9 59.00		
	2	T28 Common	6.75 /12	11-11-08	4-08-06	2 x 4	1-03-08	1-03-01 1-05-00	91.73 57.00		
	1	T28C Hip	6.75 /12	11-11-08	4-06-12	2 x 4	1-03-08	1-03-01 1-05-00	49.39 30.50		
	1	T30 Hip Girder	6.75 /12	12-03-00	3-05-04	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	56.68 37.50		
	1 2-ply	T31 Flat Girder	0 /12	6-03-08	1-06-00	2 x 4 2 x 6		1-06-00 1-06-00	50.8 33.00		
	1	T32 Half Hip Girder	6.75 /12	6-04-08	3-06-10	2 x 4 2 x 6		1-03-01 3-06-10	31.6 20.50		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
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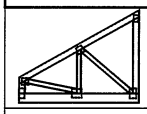
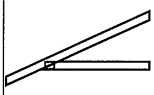
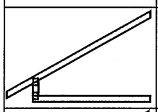
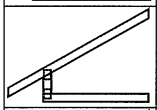
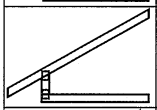
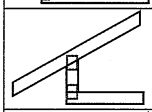
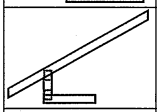
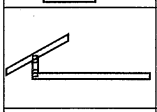
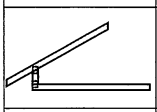
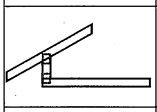
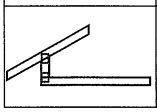
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420228
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-3	Page:	2 of 2
Lot #:		Date:	08-04-2021
Elevation:	B STD. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	**1 2-ply**	**T33 Monopitch Girder**	6.75 /12	**5-10-08**	4-06-12	2 x 4 2 x 6		1-03-01 4-06-11	60.07 38.00		
	**8**	**J4 Jack-Open**	5.5 /12	**3-01-00**	2-08-08	2 x 4	1-03-08	4-02 1-09-01	89.21 53.33		
	**15**	**J20 Jack-Open**	6.75 /12	**5-10-08**	4-06-12	2 x 4	1-03-08	1-03-01 4-06-12	257.5 160.00		
	**4**	**J21 Jack-Open**	6.75 /12	**3-10-08**	3-05-04	2 x 4	1-03-08	1-03-01 3-05-04	49.08 32.00		
	**2**	**J22 Jack-Open**	6.75 /12	**4-01-00**	3-06-10	2 x 4	1-03-08	1-03-01 3-06-10	25.56 17.33		
	**6**	**C20 Jack-Open**	6.75 /12	**1-09-07**	2-03-02	2 x 4	1-03-08 1-01	1-03-01 2-03-02	43.54 28.00		
	**3**	**C21 Jack-Open**	6.75 /12	**1-10-08**	3-04-10	2 x 4	1-03-08 1-10-15	1-03-01 2-03-11	29.62 18.00		
	**3**	**C22 Jack-Open**	6.75 /12	**1-09-07**	2-03-02	2 x 4	1-03-08 4-01-01	1-03-01 2-03-02	35.45 22.00		
	**3**	**C23 Jack-Open**	6.75 /12	**3-09-07**	3-04-10	2 x 4	1-03-08 2-01-01	1-03-01 3-04-10	43.3 26.00		
	**2**	**C24 Jack-Open**	6.75 /12	**1-09-07**	2-03-02	2 x 4	1-03-08 2-01-01	1-03-01 2-03-02	19.07 12.00		
	**1**	**C25 Jack-Open**	6.75 /12	**1-09-07**	2-03-02	2 x 4	1-03-08 2-03-09	1-03-01 2-03-02	9.77 6.67		

TOTAL # TRUSS= **80** TOTAL BFT OF ALL TRUSSES= **2613.82** BFT. TOTAL WEIGHT OF ALL TRSSES **4191.76** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
2	Hardware	LJS26DS	
2	Hardware	HGUS26-2	

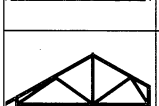
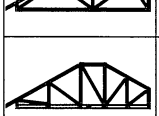
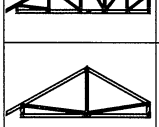
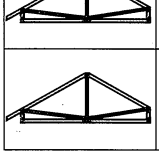
TOTAL NUMBER OF ITEMS= **7**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204995
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420229
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-3	Page:	1 of 3
Lot #:		Date:	08-10-2021
Elevation:	B OPT. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

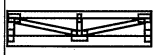
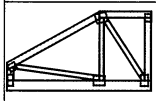
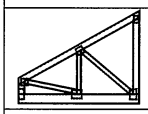
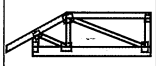
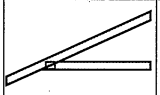
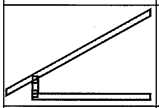
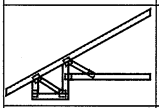
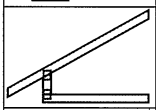
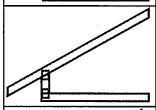
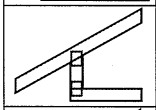
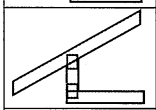
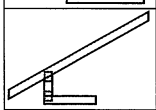
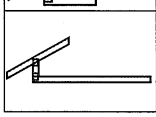
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T20S Hip Girder	6.75 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	331.98 208.00		
	1 2-ply	T20Z Hip Girder	6.75 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	320.2 193.33		
	1	T21 Hip	6.75 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	121.44 76.00		
	1	T21S Hip	6.75 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	136.91 87.33		
	1	T22 Hip	6.75 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	126.55 79.50		
	1	T22S Hip	6.75 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	138.98 87.67		
	1	T23 Hip	6.75 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	144.15 92.33		
	1	T23S Hip	6.75 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	144.58 92.83		
	2	T24 Hip	6.75 /12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	284.85 179.00		
	10	T25 Common	6.75 /12	31-00-00	9-11-11	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	1348.68 833.33		
	1	T26 Common	6.75 /12	18-04-00	7-01-15	2 x 4	1-03-08	1-03-01 2-09-01	74.52 47.83		
	1	T27 Hip Girder	6.75 /12	18-04-00	6-02-02	2 x 4 2 x 6	1-03-08	1-03-01 2-09-01	95.19 62.17		
	2	T28 Common	6.75 /12	11-11-08	4-08-06	2 x 4	1-03-08	1-03-01 1-05-00	91.73 57.00		
	1	T28C Hip	6.75 /12	11-11-08	4-06-12	2 x 4	1-03-08	1-03-01 1-05-00	45.71 28.50		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204995
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420229
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-3	Page:	2 of 3
Lot #:		Date:	08-10-2021
Elevation:	B OPT. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

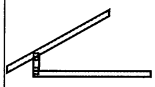
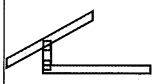
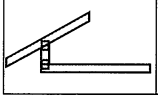
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T30 Hip Girder	6.75 /12	12-03-00	3-05-04	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	58.2 39.00		
	1 2-ply	T31 Flat Girder	0 /12	6-01-00	1-06-00	2 x 4 2 x 6		1-06-00 1-06-00	49.25 33.00		
	1	T32 Half Hip Girder	6.75 /12	6-04-08	3-06-10	2 x 4 2 x 6		1-03-01 3-06-10	31.6 20.50		
	1 2-ply	T33 Monopitch Girder	6.75 /12	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-11	60.07 38.00		
	1	T34 Half Hip Girder	6.75 /12	5-10-08	2-02-05	2 x 4 2 x 6	1-03-08	1-03-01 2-02-05	27.39 18.00		
	8	J4 Jack-Open	5.5 /12	3-01-00	2-08-08	2 x 4	1-03-08	4-02 1-09-01	89.21 53.33		
	9	J20 Jack-Open	6.75 /12	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 4-06-12	154.5 96.00		
	7	J20S Jack-Open	6.75 /12	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 3-06-12	150.02 102.67		
	4	J21 Jack-Open	6.75 /12	3-10-08	3-05-04	2 x 4	1-03-08	1-03-01 3-05-04	49.08 32.00		
	2	J22 Jack-Open	6.75 /12	4-01-00	3-06-10	2 x 4	1-03-08	1-03-01 3-06-10	25.56 17.33		
	2	J23 Jack-Open	6.75 /12	1-08-00	2-02-05	2 x 4	1-03-08	1-03-01 2-02-05	13.72 9.33		
	5	C20 Jack-Open	6.75 /12	1-09-07	2-03-02	2 x 4	1-03-08 1-01	1-03-01 2-03-02	36.28 23.33		
	2	C21 Jack-Open	6.75 /12	1-10-08	3-04-10	2 x 4	1-03-08 1-10-15	1-03-01 2-03-11	19.75 12.00		
	2	C22 Jack-Open	6.75 /12	1-09-07	2-03-02	2 x 4	1-03-08 4-01-01	1-03-01 2-03-02	23.64 14.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204995
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420229
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-3	Page:	3 of 3
Lot #:		Date:	08-10-2021
Elevation:	B OPT. 4 & 5 BEDROOM	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	C23 Jack-Open	6.75 /1 2	**3-09-07**	3-04-10	2 x 4	1-03-08 2-01-01	1-03-01 3-04-10	28.87 17.33		
	2	C24 Jack-Open	6.75 /1 2	**1-09-07**	2-03-02	2 x 4	1-03-08 2-01-01	1-03-01 2-03-02	19.07 12.00		
	1	C25 Jack-Open	6.75 /1 2	**1-09-07**	2-03-02	2 x 4	1-03-08 2-03-09	1-03-01 2-03-02	9.77 6.67		

TOTAL # TRUSS= **80** TOTAL BFT OF ALL TRUSSES= **2669.98** BFT. TOTAL WEIGHT OF ALL TRSSES **4251.47** LBS

HARDWARE

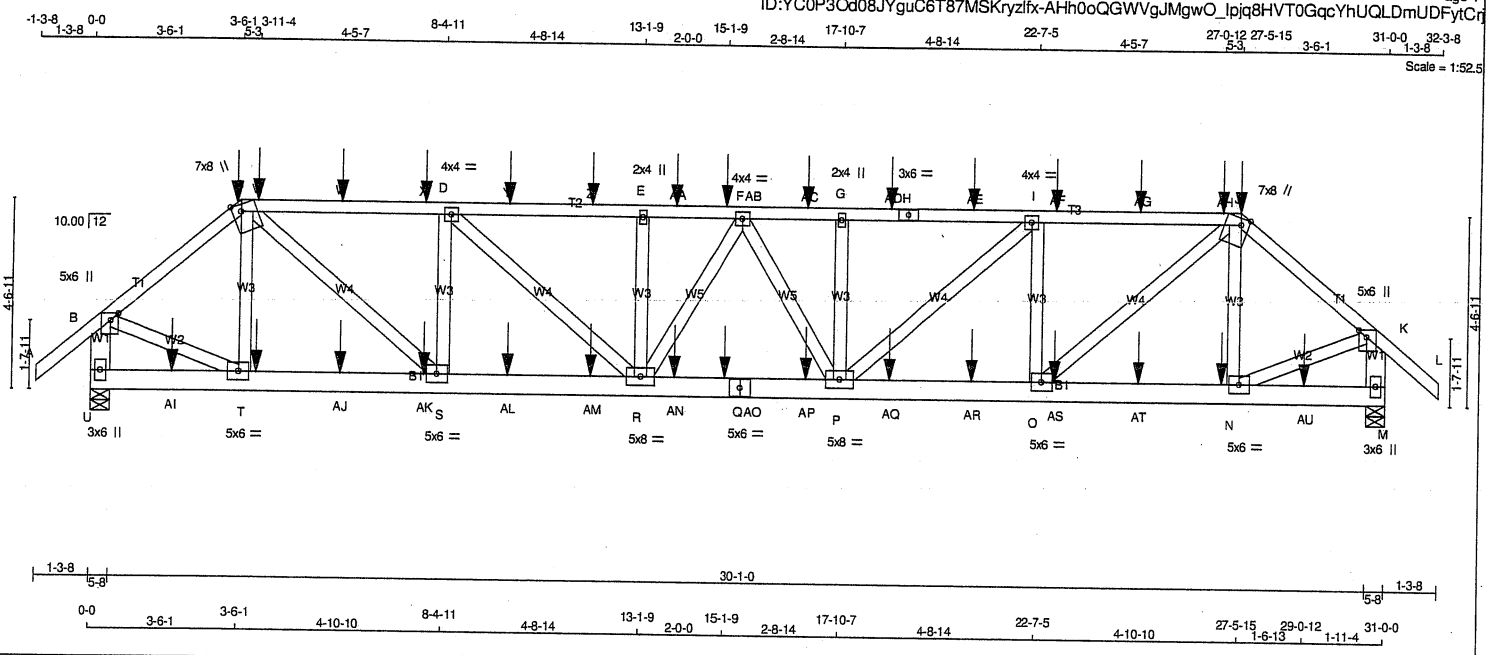
QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS24	
2	Hardware	LJS26DS	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **8**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T1	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 13:08:32 2021 Page 1
ID:YC0P3Od08JYguC6T87MSKryzlfx-AHh0oQGWVgJMgwO_lpjq8HVT0GqcYhUQLDmUDFytCn



TOTAL WEIGHT = 2 X 172 = 344 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
J- L	1	12	TOP
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(183.1)
Q- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2125147

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2852	0	2852	0	0	5-8	5-8
M	2849	0	2849	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2022	1298 / 0	0 / 0	0 / 0	0 / 0	725 / 0	0 / 0
M	2020	1296 / 0	0 / 0	0 / 0	0 / 0	724 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.06 (1)	10.00
B- C	-2839 / 0	-84.9	-84.9	0.13 (1)	5.32
C- V	-4114 / 0	-84.9	-84.9	0.36 (1)	4.35
V- W	-4114 / 0	-84.9	-84.9	0.36 (1)	4.35
W- X	-4114 / 0	-84.9	-84.9	0.36 (1)	4.35
X- D	-4114 / 0	-84.9	-84.9	0.36 (1)	4.35
D- Y	-4997 / 0	-84.9	-84.9	0.39 (1)	3.96
Y- Z	-4997 / 0	-84.9	-84.9	0.39 (1)	3.96
Z- E	-4997 / 0	-84.9	-84.9	0.39 (1)	3.96
E-AA	-4997 / 0	-84.9	-84.9	0.20 (1)	4.16
AA-AB	-4997 / 0	-84.9	-84.9	0.20 (1)	4.16
AB- F	-4997 / 0	-84.9	-84.9	0.20 (1)	4.16
F-AC	-4986 / 0	-84.9	-84.9	0.20 (1)	4.17
AC- G	-4986 / 0	-84.9	-84.9	0.20 (1)	4.17
G-AD	-4986 / 0	-84.9	-84.9	0.39 (1)	3.97
AD- H	-4986 / 0	-84.9	-84.9	0.39 (1)	3.97
H-AE	-4986 / 0	-84.9	-84.9	0.39 (1)	3.97
AE- I	-4986 / 0	-84.9	-84.9	0.39 (1)	3.97
I-AF	-4108 / 0	-84.9	-84.9	0.36 (1)	4.35
AF-AG	-4108 / 0	-84.9	-84.9	0.36 (1)	4.35
AG-AH	-4108 / 0	-84.9	-84.9	0.36 (1)	4.35
AH- J	-4108 / 0	-84.9	-84.9	0.36 (1)	4.35
J- K	-2836 / 0	-84.9	-84.9	0.13 (1)	5.32
K- L	0 / 38	-84.9	-84.9	0.06 (1)	10.00
U- B	-2820 / 0	0.0	0.0	0.10 (1)	7.81
M- K	-2817 / 0	0.0	0.0	0.10 (1)	7.81
U-AI	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AI- T	0 / 0	-18.5	-18.5	0.03 (4)	10.00
T-AJ	0 / 2155	-18.5	-18.5	0.16 (1)	10.00
AJ-AK	0 / 2155	-18.5	-18.5	0.16 (1)	10.00
AK- S	0 / 2155	-18.5	-18.5	0.16 (1)	10.00
S-AL	0 / 4114	-18.5	-18.5	0.30 (1)	10.00
AL-AM	0 / 4114	-18.5	-18.5	0.30 (1)	10.00
AM- R	0 / 4114	-18.5	-18.5	0.30 (1)	10.00
R-AN	0 / 5065	-18.5	-18.5	0.37 (1)	10.00
AN-AO	0 / 5065	-18.5	-18.5	0.37 (1)	10.00
AO- Q	0 / 5065	-18.5	-18.5	0.37 (1)	10.00
Q-AP	0 / 5065	-18.5	-18.5	0.37 (1)	10.00
AP- P	0 / 5065	-18.5	-18.5	0.37 (1)	10.00
P-AQ	0 / 4108	-18.5	-18.5	0.30 (1)	10.00
AQ-AR	0 / 4108	-18.5	-18.5	0.30 (1)	10.00
AR- O	0 / 4108	-18.5	-18.5	0.30 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-568 / 0	0.06 (1)
R- F	-137 / 0	0.02 (1)
F- P	-159 / 0	0.02 (1)
N- J	-568 / 0	0.06 (1)
B- T	0 / 2293	0.20 (1)
N- K	0 / 2290	0.20 (1)
O- J	0 / 2570	0.23 (1)
C- S	0 / 2574	0.23 (1)
O- I	-1526 / 0	0.16 (1)
S- D	-1528 / 0	0.16 (1)
P- I	0 / 1171	0.10 (1)
D- R	0 / 1179	0.10 (1)
P- G	-485 / 0	0.05 (1)
R- E	-506 / 0	0.05 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.39/1.00 (G-I:1) , BC=0.37/1.00 (P-R:1) , WB=0.23/1.00 (C-S:1) , SSI=0.19/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.48 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T1	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:YC0P3Od08JYguC6T87MSKryzlfx-AHh0oQGWVgJMgwO_Ipjq8HVT0GqcYhUQLDmUDFytCr

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D, F, I						
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

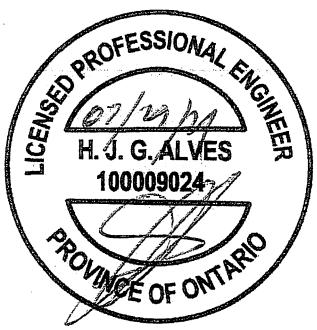
CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-AS	0 / 2152	-18.5	-18.5	0.16 (1)	10.00			
AS-AT	0 / 2152	-18.5	-18.5	0.16 (1)	10.00			
AT- N	0 / 2152	-18.5	-18.5	0.16 (1)	10.00			
N-AU	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
AU- M	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-6-1	-35	-35	---	TOP	VERT	DEAD	---	C1
C	3-6-1	-133	-133	---	TOP	VERT	SNOW	---	C1
J	27-5-15	-35	-35	---	TOP	VERT	DEAD	---	C1
J	27-5-15	-133	-133	---	TOP	VERT	SNOW	---	C1
N	27-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	3-11-4	-87	-87	---	FRONT	VERT	TOTAL	---	C1
W	5-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
X	7-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Y	9-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Z	11-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AA	13-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AB	15-1-9	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AC	17-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AD	19-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AE	21-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AF	23-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AG	25-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AH	27-0-12	-87	-87	---	FRONT	VERT	TOTAL	---	C1
AI	1-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	13-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	15-1-9	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	17-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	19-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	21-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	23-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	29-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only

DWG# T-2125147

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T1Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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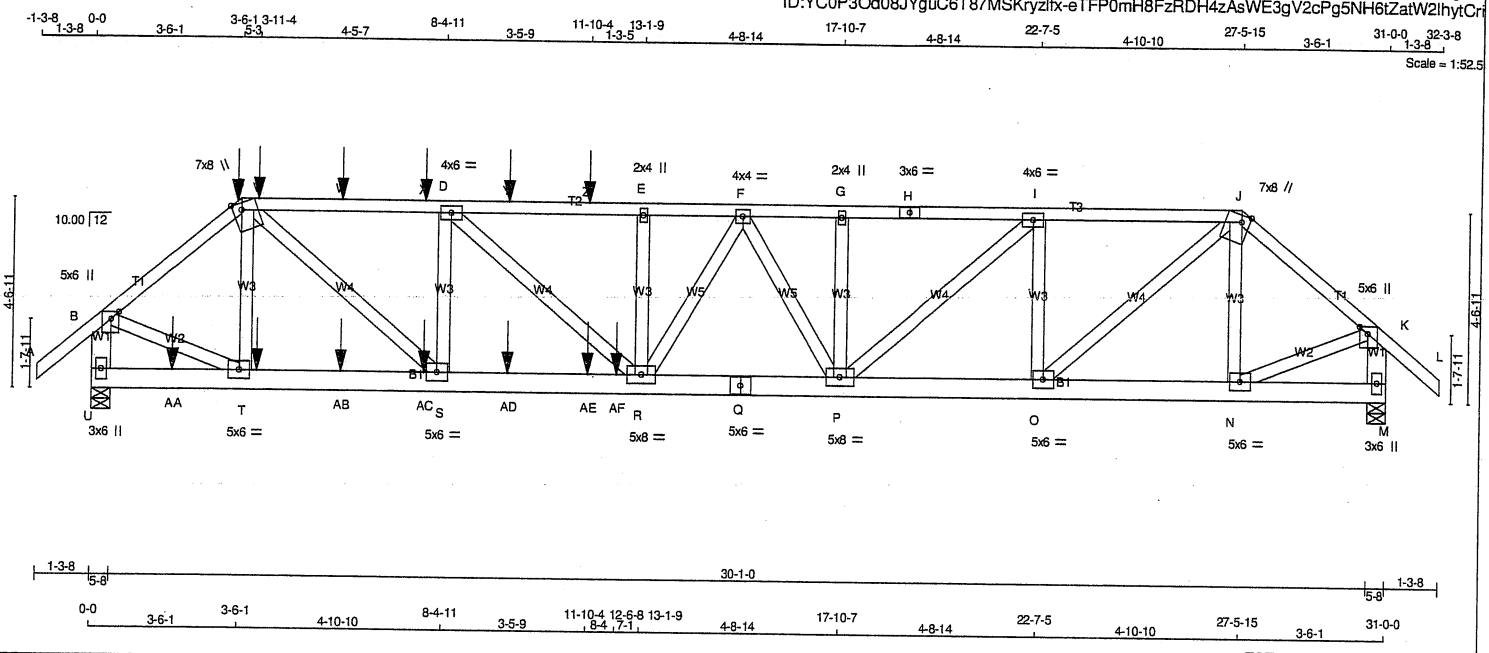

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- H	1	12	SIDE(61.0)
H- J	1	12	TOP
J- L	1	12	TOP
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(183.1)
Q- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2125148

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3686	0	3686	0	0	5-8	5-8
M	2748	0	2748	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2611	1690 / 0	0 / 0	0 / 0	0 / 0	921 / 0	0 / 0
M	1947	1259 / 0	0 / 0	0 / 0	0 / 0	688 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.06 (1)	10.00
B- C	-3752 / 0	-84.9	-84.9	0.16 (1)	4.74
C- V	-5828 / 0	-84.9	-84.9	0.45 (1)	3.66
V- W	-5828 / 0	-84.9	-84.9	0.45 (1)	3.66
W- X	-5828 / 0	-84.9	-84.9	0.45 (1)	3.66
X- D	-5828 / 0	-84.9	-84.9	0.45 (1)	3.66
D- Y	-7401 / 0	-84.9	-84.9	0.55 (1)	3.19
Y- Z	-7401 / 0	-84.9	-84.9	0.55 (1)	3.19
Z- E	-7401 / 0	-84.9	-84.9	0.55 (1)	3.19
E- F	-7401 / 0	-84.9	-84.9	0.28 (1)	3.43
F- G	-6101 / 0	-84.9	-84.9	0.19 (1)	3.83
G- H	-6101 / 0	-84.9	-84.9	0.34 (1)	3.72
H- I	-6101 / 0	-84.9	-84.9	0.34 (1)	3.72
I- J	-4454 / 0	-84.9	-84.9	0.26 (1)	4.33
J- K	-2733 / 0	-84.9	-84.9	0.13 (1)	5.40
K- L	0 / 38	-84.9	-84.9	0.06 (1)	10.00
U- B	-3641 / 0	0.0	0.0	0.13 (1)	7.37
M- K	-2725 / 0	0.0	0.0	0.10 (1)	7.81
U-AA	0 / 0	-18.5	-18.5	0.02 (4)	10.00
AA- T	0 / 0	-18.5	-18.5	0.02 (4)	10.00
T-AB	0 / 2843	-18.5	-18.5	0.25 (1)	10.00
AB-AC	0 / 2843	-18.5	-18.5	0.25 (1)	10.00
AC- S	0 / 2843	-18.5	-18.5	0.25 (1)	10.00
S-AD	0 / 5828	-18.5	-18.5	0.66 (1)	10.00
AD-AE	0 / 5828	-18.5	-18.5	0.66 (1)	10.00
AE-AF	0 / 5828	-18.5	-18.5	0.66 (1)	10.00
AF- R	0 / 5828	-18.5	-18.5	0.66 (1)	10.00
R- Q	0 / 6785	-18.5	-18.5	0.57 (1)	10.00
Q- P	0 / 6785	-18.5	-18.5	0.57 (1)	10.00
P- O	0 / 4454	-18.5	-18.5	0.35 (1)	10.00
O- N	0 / 2072	-18.5	-18.5	0.15 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-863 / 0	0.09 (1)
R- F	0 / 1250	0.11 (1)
F- P	-1389 / 0	0.19 (1)
N- J	-607 / 0	0.07 (1)
B- T	0 / 3030	0.27 (1)
N- K	0 / 2207	0.20 (1)
O- J	0 / 3130	0.28 (1)
C- S	0 / 3921	0.35 (1)
O- I	-1910 / 0	0.20 (1)
S- D	-2140 / 0	0.23 (1)
P- I	0 / 2196	0.19 (1)
D- R	0 / 2098	0.19 (1)
P- G	-308 / 0	0.03 (1)
R- E	-407 / 0	0.04 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-6-1	-35	-35	---	TOP	VERT	DEAD	---	C1
C	3-6-1	-133	-133	---	TOP	VERT	SNOW	---	C1
T	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
V	3-11-4	-87	-87	---	BACK	VERT	TOTAL	---	C1
W	5-11-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
X	7-11-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
Y	9-11-4	-71	-71	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.55/1.00 (D-E:1) , BC=0.66/1.00 (R-S:1) , WB=0.35/1.00 (C-S:1) , SSI=0.63/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.59 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420226	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Z	11-10-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
AA	1-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AB	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	7-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	9-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	11-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AF	12-6-8	-1456	-1456	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07/29/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125148

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420227	T1S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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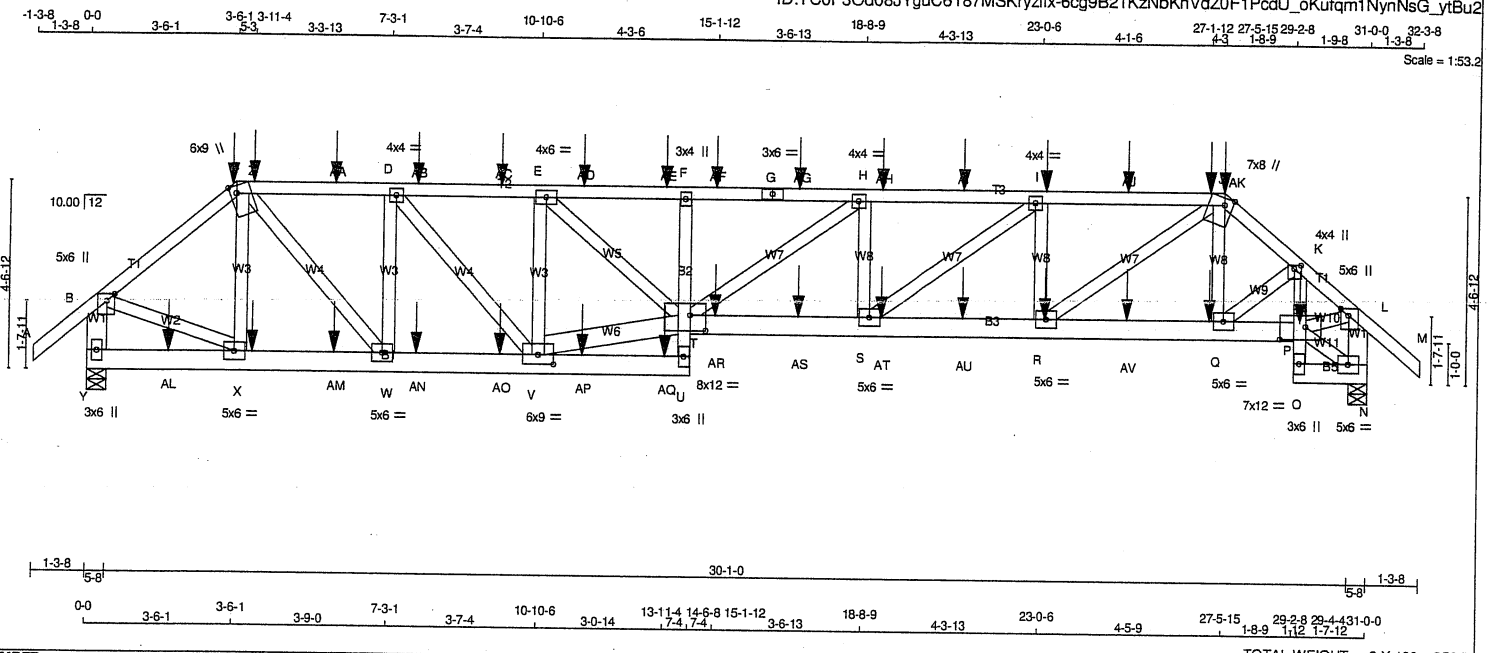

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - U	2x6	DRY	No.2	SPF
U - F	2x4	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- G	1	12	SIDE(61.0)
G- J	1	12	SIDE(61.0)
J- M	1	12	TOP
Y- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Y- U	2	12	SIDE(183.1)
T- P	2	12	SIDE(183.1)
O- N	2	12	TOP
U- F	1	12	TOP
K- O	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2125162

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	2985	0	2985	0	0	5-8	5-8
N	3205	0	3205	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2117	1357 / 0	0 / 0	0 / 0	0 / 0	759 / 0	0 / 0
N	2272	1461 / 0	0 / 0	0 / 0	0 / 0	811 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.06 (1)	10.00	
B- C	-2981 / 0	-84.9	-84.9	0.14 (1)	5.22	
C- Z	-3945 / 0	-84.9	-84.9	0.21 (1)	4.58	
Z-AA	-3945 / 0	-84.9	-84.9	0.21 (1)	4.58	
AA- D	-3945 / 0	-84.9	-84.9	0.21 (1)	4.58	
D-AB	-4975 / 0	-84.9	-84.9	0.25 (1)	4.13	
AB-AC	-4975 / 0	-84.9	-84.9	0.25 (1)	4.13	
AC- E	-4975 / 0	-84.9	-84.9	0.25 (1)	4.13	
E-AD	-7249 / 0	-84.9	-84.9	0.31 (1)	3.45	
AD-AE	-7249 / 0	-84.9	-84.9	0.31 (1)	3.45	
AE- F	-7249 / 0	-84.9	-84.9	0.31 (1)	3.45	
F-AF	-7295 / 0	-84.9	-84.9	0.37 (1)	3.39	
AF- G	-7295 / 0	-84.9	-84.9	0.37 (1)	3.39	
G-AG	-7295 / 0	-84.9	-84.9	0.37 (1)	3.39	
AG- H	-7295 / 0	-84.9	-84.9	0.37 (1)	3.39	
H-AH	-7111 / 0	-84.9	-84.9	0.40 (1)	3.39	
AH-AI	-7111 / 0	-84.9	-84.9	0.40 (1)	3.39	
AI- I	-7111 / 0	-84.9	-84.9	0.40 (1)	3.39	
I-AJ	-5785 / 0	-84.9	-84.9	0.33 (1)	3.80	
AJ-AK	-5785 / 0	-84.9	-84.9	0.33 (1)	3.80	
AK- J	-5785 / 0	-84.9	-84.9	0.33 (1)	3.80	
J- K	-4130 / 0	-84.9	-84.9	0.10 (1)	4.61	
K- L	-4026 / 0	-84.9	-84.9	0.11 (1)	4.64	
L- M	0 / 38	-84.9	-84.9	0.06 (1)	10.00	
Y- B	-2948 / 0	0.0	0.0	0.11 (1)	7.81	
N- L	-3132 / 0	0.0	0.0	0.12 (1)	7.80	
Y-AL	0 / 0	-18.5	-18.5	0.02 (4)	10.00	
AL- X	0 / 0	-18.5	-18.5	0.02 (4)	10.00	
X-AM	0 / 2262	-18.5	-18.5	0.17 (1)	10.00	
AM- W	0 / 2262	-18.5	-18.5	0.17 (1)	10.00	
W-AN	0 / 3945	-18.5	-18.5	0.28 (1)	10.00	
AN-AO	0 / 3945	-18.5	-18.5	0.28 (1)	10.00	
AO- V	0 / 3945	-18.5	-18.5	0.28 (1)	10.00	
V-AP	0 / 148	-18.5	-18.5	0.03 (1)	10.00	
AP-AQ	0 / 148	-18.5	-18.5	0.03 (1)	10.00	
AQ- U	0 / 148	-18.5	-18.5	0.03 (1)	10.00	
U- T	0 / 72	0.0	0.0	0.12 (1)	10.00	
T- F	-561 / 0	0.0	0.0	0.12 (1)	7.81	
T-AR	0 / 7111	-18.5	-18.5	0.53 (1)	10.00	
AR-AS	0 / 7111	-18.5	-18.5	0.53 (1)	10.00	
AS- S	0 / 7111	-18.5	-18.5	0.53 (1)	10.00	
S-AT	0 / 5785	-18.5	-18.5	0.44 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-621 / 0	0.07 (1)
C- W	0 / 2514	0.22 (1)
W- D	-1779 / 0	0.19 (1)
D- V	0 / 1604	0.14 (1)
V- E	-2458 / 0	0.26 (1)
V- T	0 / 5066	0.33 (1)
E- T	0 / 2984	0.26 (1)
Q- J	0 / 151	0.02 (4)
Q- K	0 / 65	0.01 (1)
B- X	0 / 2408	0.21 (1)
P- N	-107 / 0	0.01 (1)
P- L	0 / 3072	0.27 (1)
R- J	0 / 3201	0.28 (1)
T- H	0 / 229	0.02 (1)
R- I	-1604 / 0	0.12 (1)
S- H	-709 / 0	0.05 (1)
S- I	0 / 1647	0.15 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.40/1.00 (H-I:1) , BC=0.53/1.00 (S-T:1) , WB=0.33/1.00 (T-V:1) , SSI=0.13/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420227	T1S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D, H, I						
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	TMVW+p	MT20	5.0	6.0	2.00	2.25
N	BMVW1-t	MT20	5.0	6.0		
O	BMV+p	MT20	3.0	6.0		
P	BVMWW-l	MT20	7.0	12.0	3.75	7.50
Q, R, S, W, X						
Q	BMWW-t	MT20	5.0	6.0		
T	BVMWWW-l	MT20	8.0	12.0	4.50	4.50
U	BMV+p	MT20	3.0	6.0		
V	BMWWW-t	MT20	6.0	9.0	2.75	4.50
Y	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
AT-AU	0 / 5785	-18.5	-18.5	0.44 (1)	10.00
AU- R	0 / 5785	-18.5	-18.5	0.44 (1)	10.00
R-AV	0 / 3174	-18.5	-18.5	0.25 (1)	10.00
AV- Q	0 / 3174	-18.5	-18.5	0.25 (1)	10.00
Q- P	0 / 3116	-18.5	-18.5	0.24 (1)	10.00
O- P	0 / 17	0.0	0.0	0.07 (1)	10.00
P- K	-216 / 0	0.0	0.0	0.07 (1)	7.81
O- N	0 / 90	-18.5	-18.5	0.01 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-6-1	-35	-35	---	TOP	VERT	DEAD	---	C1
C	3-6-1	-133	-133	---	TOP	VERT	SNOW	---	C1
I	23-1-12	-67	-67	---	FRONT	VERT	TOTAL	---	C1
J	27-5-15	-35	-35	---	TOP	VERT	DEAD	---	C1
J	27-5-15	-133	-133	---	TOP	VERT	SNOW	---	C1
P	29-4-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Q	27-1-12	-65	-65	---	FRONT	VERT	TOTAL	---	C1
R	23-1-12	-65	-65	---	FRONT	VERT	TOTAL	---	C1
X	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Z	3-11-4	-87	-87	---	FRONT	VERT	TOTAL	---	C1
AA	5-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AB	7-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AC	9-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AD	11-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AE	13-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AF	15-1-12	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AG	17-1-12	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AH	19-1-12	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AI	21-1-12	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AJ	25-1-12	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AK	27-1-12	-87	-87	---	FRONT	VERT	TOTAL	---	C1
AL	1-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	13-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AR	15-1-12	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AS	17-1-12	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AT	19-1-12	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AU	21-1-12	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AV	25-1-12	-65	-65	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.88 (V) (INPUT = 0.90)
JSI METAL= 0.44 (V) (INPUT = 1.00)

Structural component only
DWG# T-2125162

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T2	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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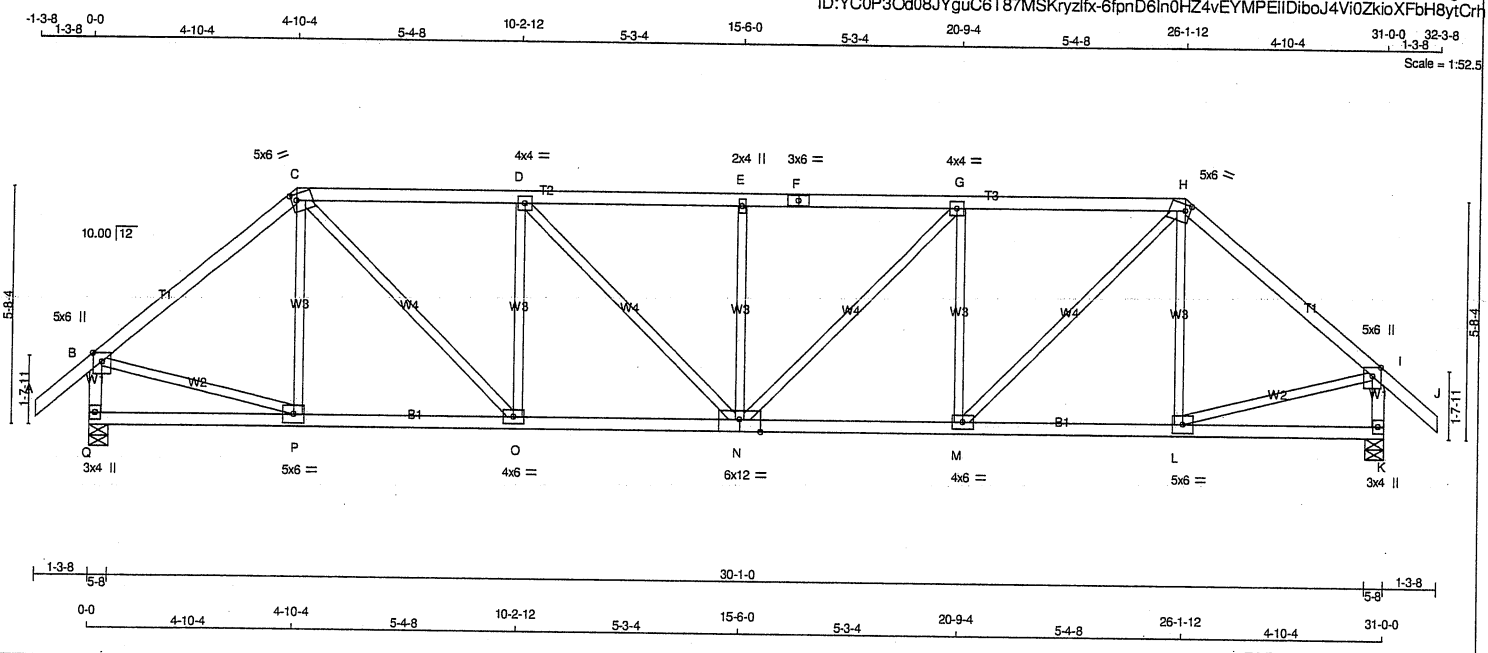

TOTAL WEIGHT = 2 X 133 = 266 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	1.50
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BSWWW-l	MT20	6.0	12.0	Edge	6.00
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125149

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1720	0	1720	0	0	5-8	5-8
K	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1611 / 0	-84.9	-84.9	0.44 (1)	4.74
C- D	-2089 / 0	-84.9	-84.9	0.45 (1)	4.23
D- E	-2331 / 0	-84.9	-84.9	0.47 (1)	4.02
E- F	-2331 / 0	-84.9	-84.9	0.47 (1)	4.02
F- G	-2331 / 0	-84.9	-84.9	0.47 (1)	4.02
G- H	-2089 / 0	-84.9	-84.9	0.45 (1)	4.23
H- I	-1611 / 0	-84.9	-84.9	0.44 (1)	4.74
I- J	0 / 38	-84.9	-84.9	0.12 (1)	10.00
Q- B	-1683 / 0	0.0	0.0	0.18 (1)	6.40
K- I	-1683 / 0	0.0	0.0	0.18 (1)	6.40
Q- P	0 / 0	-18.5	-18.5	0.11 (4)	10.00
P- O	0 / 1231	-18.5	-18.5	0.26 (1)	10.00
O- N	0 / 2090	-18.5	-18.5	0.40 (1)	10.00
N- M	0 / 2090	-18.5	-18.5	0.40 (1)	10.00
M- L	0 / 1231	-18.5	-18.5	0.26 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-220 / 0	0.11 (1)
C- O	0 / 1214	0.27 (1)
O- D	-754 / 0	0.37 (1)
D- N	0 / 345	0.08 (1)
N- E	-412 / 0	0.20 (1)
N- G	0 / 345	0.08 (1)
M- G	-754 / 0	0.37 (1)
M- H	0 / 1214	0.27 (1)
L- H	-220 / 0	0.11 (1)
B- P	0 / 1277	0.29 (1)
L- I	0 / 1277	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.47/1.00 (D-E:1) , BC=0.40/1.00 (M-N:1) , WB=0.37/1.00 (D-O:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.60 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420227	T2S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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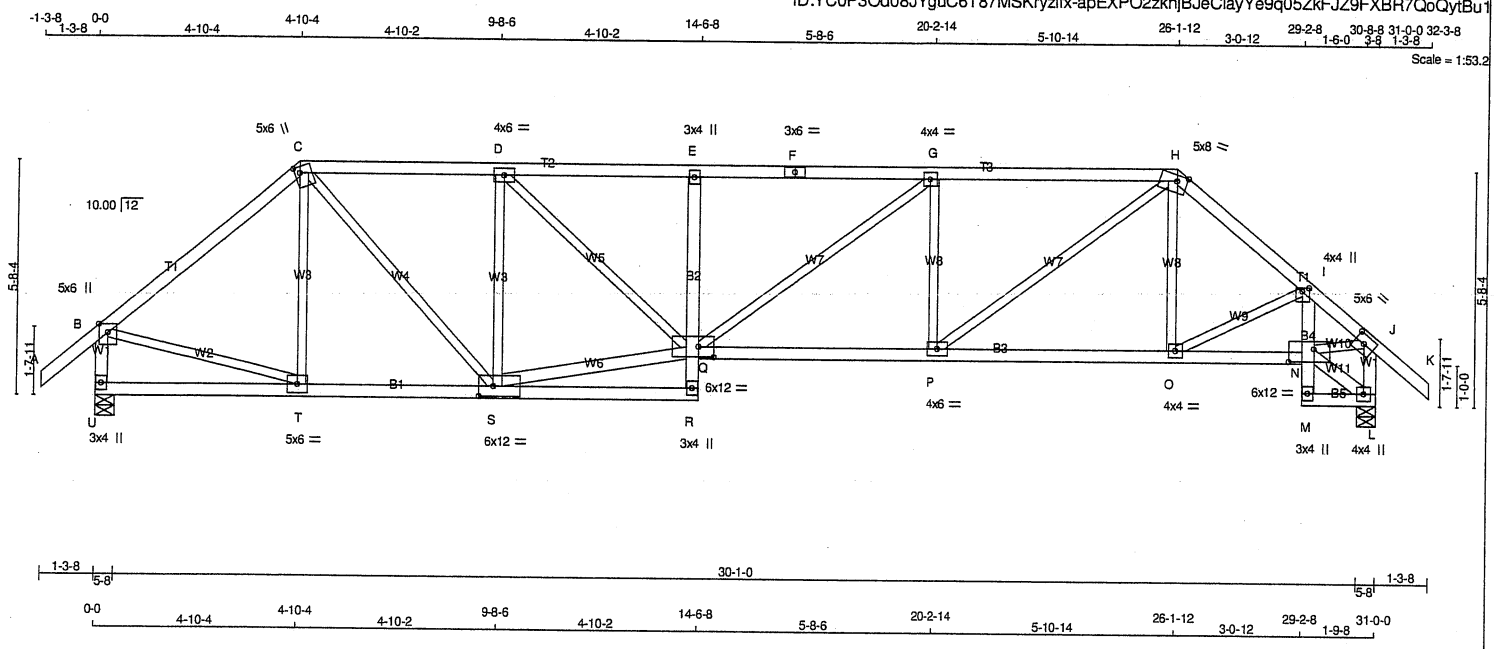

TOTAL WEIGHT = 141 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMVW1+p	MT20	4.0	4.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-l	MT20	6.0	12.0	3.75	7.25
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BVMWWW-l	MT20	6.0	12.0	3.00	4.50
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	12.0	3.00	4.25
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2125163

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1717	0	1717	0	0	5-8	5-8
L	1722	0	1722	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1220	788 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1607 / 0	-84.9	-84.9	0.43 (1)	4.74
C- D	-2005 / 0	-84.9	-84.9	0.34 (1)	4.43
D- E	-2823 / 0	-84.9	-84.9	0.41 (1)	3.78
E- F	-2838 / 0	-84.9	-84.9	0.66 (1)	3.50
F- G	-2838 / 0	-84.9	-84.9	0.66 (1)	3.50
G- H	-2631 / 0	-84.9	-84.9	0.63 (1)	3.65
H- I	-1958 / 0	-84.9	-84.9	0.18 (1)	4.66
I- J	-2053 / 0	-84.9	-84.9	0.16 (1)	4.59
J- K	0 / 38	-84.9	-84.9	0.12 (1)	10.00
U- B	-1680 / 0	0.0	0.0	0.18 (1)	6.41
L- J	-1672 / 0	0.0	0.0	0.18 (1)	6.42
U- T	0 / 0	-18.5	-18.5	0.10 (4)	10.00
T- S	0 / 1227	-18.5	-18.5	0.25 (1)	10.00
S- R	0 / 69	-18.5	-18.5	0.10 (4)	10.00
R- Q	0 / 40	0.0	0.0	0.10 (1)	10.00
Q- E	-414 / 0	0.0	0.0	0.12 (1)	7.81
Q- P	0 / 2631	-18.5	-18.5	0.50 (1)	10.00
P- O	0 / 1495	-18.5	-18.5	0.31 (1)	10.00
O- N	0 / 1601	-18.5	-18.5	0.31 (1)	10.00
M- N	0 / 17	0.0	0.0	0.08 (1)	10.00
N- I	-87 / 0	0.0	0.0	0.08 (1)	7.81
M- L	0 / 50	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
T- C	-225 / 0	0.11 (1)
C- S	0 / 1171	0.26 (1)
S- D	-1192 / 0	0.59 (1)
S- Q	0 / 1994	0.32 (1)
D- Q	0 / 1098	0.25 (1)
Q- G	0 / 259	0.06 (1)
P- G	-723 / 0	0.23 (1)
P- H	0 / 1413	0.32 (1)
O- H	0 / 93	0.03 (4)
O- I	-119 / 0	0.03 (1)
B- T	0 / 1274	0.29 (1)
N- L	-61 / 0	0.01 (1)
N- J	0 / 1570	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.66/1.00 (E-G:1) , BC=0.50/1.00 (P-Q:1) , WB=0.59/1.00 (D-S:1) , SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.60 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T3	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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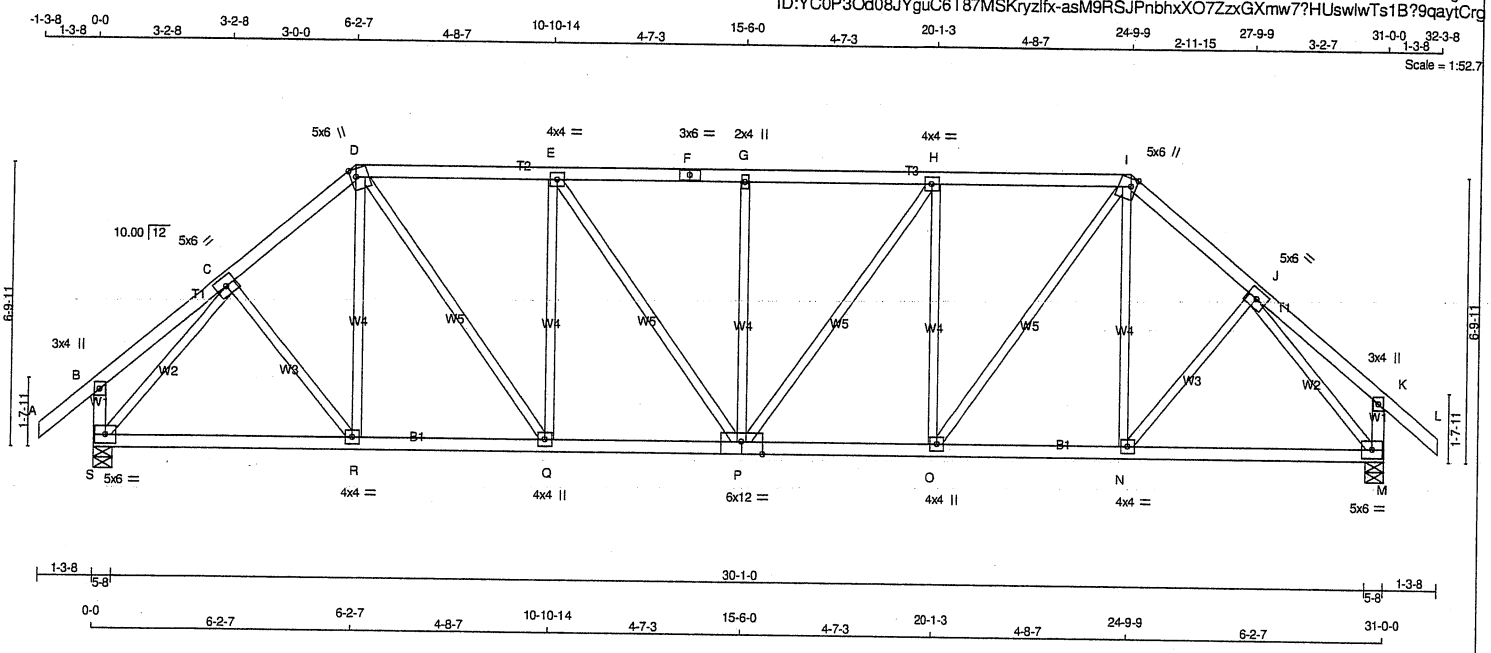

TOTAL WEIGHT = 2 X 147 = 295 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.25	1.50
J	TMWW-t	MT20	5.0	6.0		
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	4.0		
P	BSWWW-l	MT20	6.0	12.0	Edge	6.00
Q	BMWW+t	MT20	4.0	4.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1720	0	1720	0	0	5-8	5-8
M	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
M	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	C- R	0 / 127	0.03 (1)
B- C	0 / 17	-84.9	-84.9	0.11 (1)	10.00	R- D	0 / 70	0.02 (4)
C- D	-1628 / 0	-84.9	-84.9	0.15 (1)	5.05	D- Q	0 / 915	0.21 (1)
D- E	-1770 / 0	-84.9	-84.9	0.32 (1)	4.68	Q- E	-658 / 0	0.51 (1)
E- F	-1923 / 0	-84.9	-84.9	0.33 (1)	4.52	E- P	0 / 264	0.06 (1)
F- G	-1923 / 0	-84.9	-84.9	0.33 (1)	4.52	P- G	-358 / 0	0.28 (1)
G- H	-1923 / 0	-84.9	-84.9	0.33 (1)	4.52	P- H	0 / 264	0.06 (1)
H- I	-1770 / 0	-84.9	-84.9	0.32 (1)	4.68	O- H	-658 / 0	0.51 (1)
I- J	-1628 / 0	-84.9	-84.9	0.15 (1)	5.05	O- I	0 / 915	0.21 (1)
J- K	0 / 17	-84.9	-84.9	0.11 (1)	10.00	N- I	0 / 70	0.02 (4)
K- L	0 / 38	-84.9	-84.9	0.12 (1)	10.00	N- J	0 / 127	0.03 (1)
S- B	-222 / 0	0.0	0.0	0.02 (1)	7.81	S- C	-1852 / 0	0.72 (1)
M- K	-222 / 0	0.0	0.0	0.02 (1)	7.81	J- M	-1852 / 0	0.72 (1)
S- R	0 / 1152	-18.5	-18.5	0.27 (1)	10.00			
R- Q	0 / 1234	-18.5	-18.5	0.28 (1)	10.00			
Q- P	0 / 1770	-18.5	-18.5	0.33 (1)	10.00			
P- O	0 / 1770	-18.5	-18.5	0.33 (1)	10.00			
O- N	0 / 1234	-18.5	-18.5	0.28 (1)	10.00			
N- M	0 / 1152	-18.5	-18.5	0.27 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.33/1.00 (G-H:1) , BC=0.33/1.00 (O-P:1) , WB=0.72/1.00 (C-S:1) , SSI=0.19/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Q) (INPUT = 0.90)
JSI METAL= 0.41 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125150

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420227	T3S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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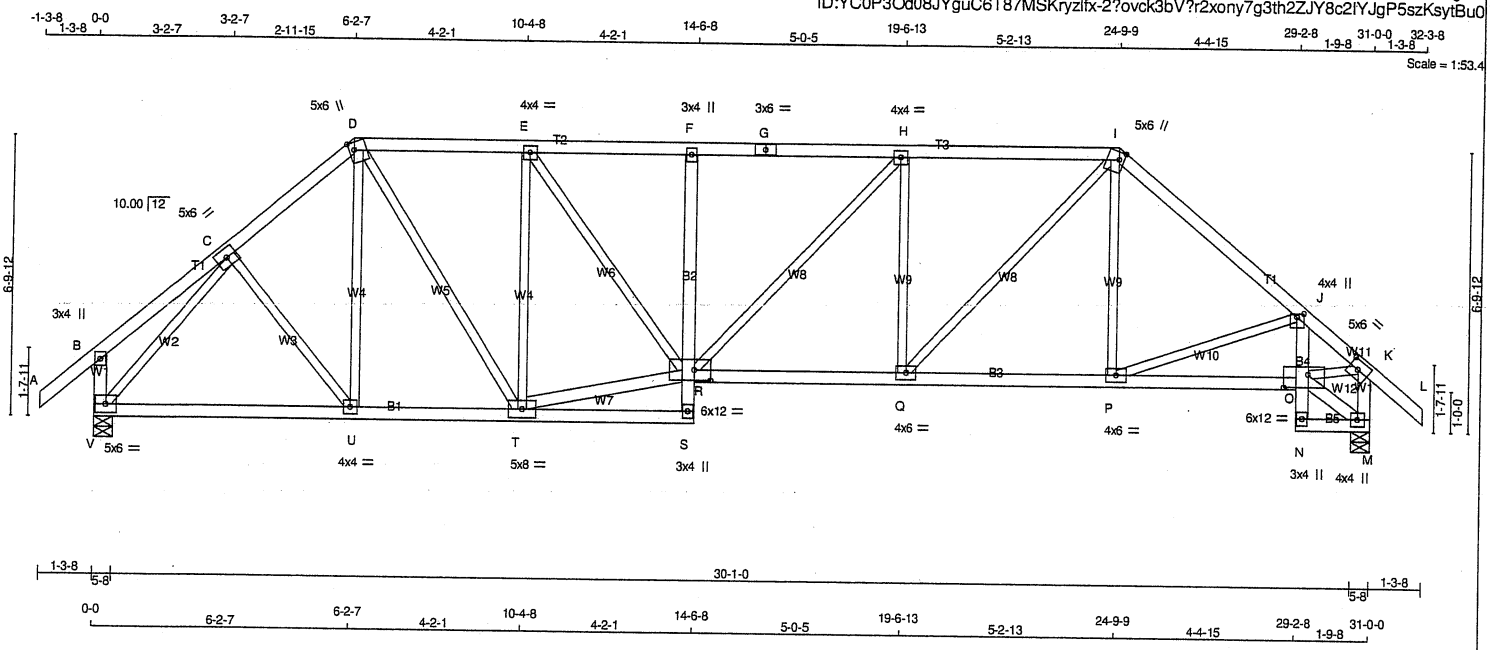

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.25	1.50
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW-t	MT20	5.0	6.0	2.50	2.75
M	BMVW1+p	MT20	4.0	4.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-I	MT20	6.0	12.0	3.75	7.25
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BVMWWW-I	MT20	6.0	12.0	3.00	4.75
S	BMV+p	MT20	3.0	4.0		
T	BMWWW-t	MT20	5.0	8.0		
U	BMWW-t	MT20	4.0	4.0		
V	BMVW1-t	MT20	5.0	6.0		

~~NOTES~~ (1)

1)

Structural component only
DWG# T-2125164

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1722	0	1722	0	0	5-8	5-8
V	1717	0	1717	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1220	788 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
V	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 17	-84.9	-84.9	0.11 (1)	10.00
C- D	-1625 / 0	-84.9	-84.9	0.15 (1)	5.06
D- E	-1709 / 0	-84.9	-84.9	0.23 (1)	4.85
E- F	-2245 / 0	-84.9	-84.9	0.26 (1)	4.32
F- G	-2253 / 0	-84.9	-84.9	0.45 (1)	4.10
G- H	-2253 / 0	-84.9	-84.9	0.45 (1)	4.10
H- I	-2137 / 0	-84.9	-84.9	0.44 (1)	4.20
I- J	-1898 / 0	-84.9	-84.9	0.31 (1)	4.60
J- K	-2094 / 0	-84.9	-84.9	0.22 (1)	4.47
K- L	0 / 38	-84.9	-84.9	0.12 (1)	10.00
V- B	-222 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-1672 / 0	0.0	0.0	0.18 (1)	6.42
V- U	0 / 1150	-18.5	-18.5	0.27 (1)	10.00
U- T	0 / 1231	-18.5	-18.5	0.28 (1)	10.00
T- S	0 / 46	-18.5	-18.5	0.07 (4)	10.00
S- R	0 / 36	0.0	0.0	0.07 (1)	10.00
R- F	-361 / 0	0.0	0.0	0.13 (1)	7.81
R- Q	0 / 2137	-18.5	-18.5	0.40 (1)	10.00
Q- P	0 / 1443	-18.5	-18.5	0.29 (1)	10.00
P- O	0 / 1660	-18.5	-18.5	0.33 (1)	10.00
N- O	0 / 17	0.0	0.0	0.08 (1)	10.00
O- J	-95 / 3	0.0	0.0	0.08 (1)	7.81
N- M	0 / 51	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- D	0 / 67	0.02 (4)
D- T	0 / 895	0.20 (1)
T- E	-1096 / 0	0.85 (1)
T- R	0 / 1724	0.28 (1)
E- R	0 / 891	0.20 (1)
R- H	0 / 170	0.04 (1)
Q- H	-627 / 0	0.32 (1)
Q- I	0 / 1007	0.23 (1)
P- I	0 / 128	0.04 (4)
P- J	-233 / 0	0.09 (1)
O- M	-62 / 0	0.01 (1)
O- K	0 / 1630	0.37 (1)
V- C	-1848 / 0	0.72 (1)
C- U	0 / 126	0.03 (4)

TOTAL WEIGHT = 152 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.45/1.00 (F-H:1) , BC=0.40/1.00 (Q-R:1) , WB=0.85/1.00 (E-T:1) , SSI=0.21/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.50 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T4	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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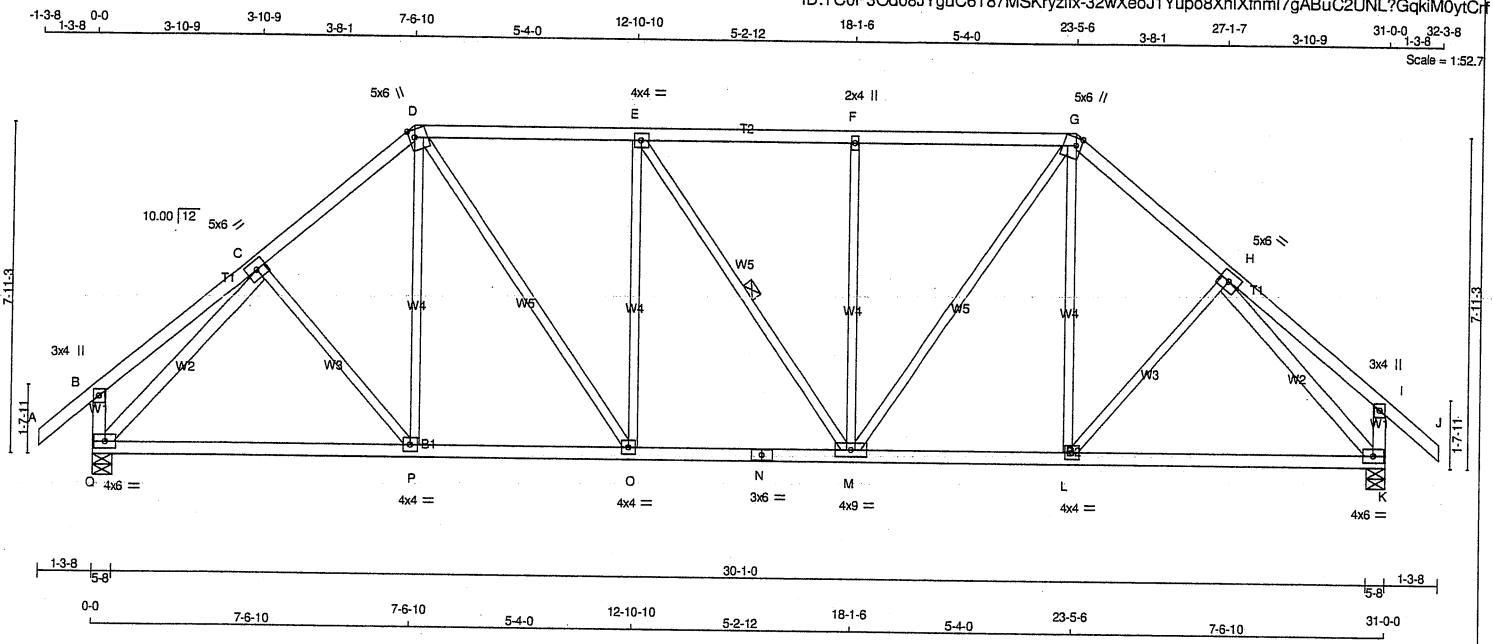

TOTAL WEIGHT = 2 X 150 = 300 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		
L, O, P						
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
Q	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1720	0	1720	0	0	5-8	5-8
K	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 23	-84.9	-84.9	0.19 (1)	10.00
C- D	-1606 / 0	-84.9	-84.9	0.23 (1)	4.98
D- E	-1612 / 0	-84.9	-84.9	0.38 (1)	4.77
E- F	-1611 / 0	-84.9	-84.9	0.38 (1)	4.78
F- G	-1611 / 0	-84.9	-84.9	0.38 (1)	4.79
G- H	-1606 / 0	-84.9	-84.9	0.23 (1)	4.98
H- I	0 / 23	-84.9	-84.9	0.19 (1)	10.00
I- J	0 / 38	-84.9	-84.9	0.12 (1)	10.00
Q- B	-240 / 0	0.0	0.0	0.03 (1)	7.81
K- I	-240 / 0	0.0	0.0	0.03 (1)	7.81
Q- P	0 / 1202	-18.5	-18.5	0.33 (4)	10.00
P- O	0 / 1214	-18.5	-18.5	0.34 (4)	10.00
O- N	0 / 1612	-18.5	-18.5	0.31 (1)	10.00
N- M	0 / 1612	-18.5	-18.5	0.31 (1)	10.00
M- L	0 / 1214	-18.5	-18.5	0.34 (4)	10.00
L- K	0 / 1202	-18.5	-18.5	0.33 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 51	0.02 (4)
P- D	0 / 133	0.04 (4)
D- O	0 / 694	0.16 (1)
O- E	-484 / 0	0.58 (1)
E- M	-3 / 0	0.00 (1)
M- F	-483 / 0	0.58 (1)
M- G	0 / 691	0.16 (1)
L- G	0 / 134	0.04 (4)
L- H	0 / 51	0.02 (4)
Q- C	-1861 / 0	0.75 (1)
H- K	-1862 / 0	0.75 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.34/1.00 (L-M:4) , WB=0.75/1.00 (H-K:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)

Structural component only
DWG# T-2125151

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420227	T4S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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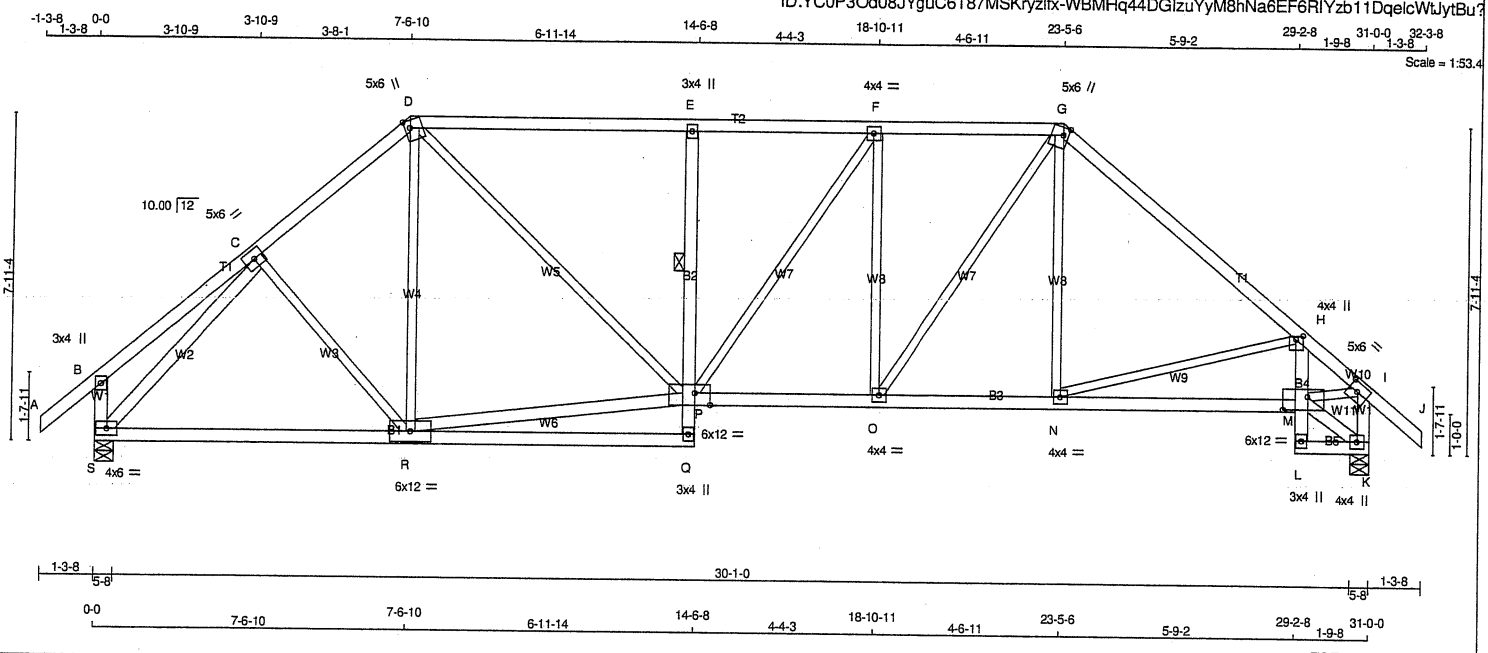

TOTAL WEIGHT = 156 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
I	TMVW-t	MT20	5.0	6.0	2.50	2.75
K	BMVW1+p	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-I	MT20	6.0	12.0	3.75	7.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-I	MT20	6.0	12.0	Edge	4.50
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	6.0	12.0		
S	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

~~NOTES~~ (1)
1)

Structural component only
DWG# T-2125165

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1717	0	1717	0	0	5-8	5-8
K	1722	0	1722	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1220	788 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 23	-84.9	-84.9	0.19 (1)	10.00
C- D	-1607 / 0	-84.9	-84.9	0.24 (1)	4.98
D- E	-1900 / 0	-84.9	-84.9	0.64 (1)	4.17
E- F	-1906 / 0	-84.9	-84.9	0.55 (1)	4.16
F- G	-1794 / 0	-84.9	-84.9	0.28 (1)	4.71
G- H	-1827 / 0	-84.9	-84.9	0.53 (1)	4.47
H- I	-2151 / 0	-84.9	-84.9	0.40 (1)	4.16
I- J	0 / 38	-84.9	-84.9	0.12 (1)	10.00
S- B	-240 / 0	0.0	0.0	0.03 (1)	7.81
K- I	-1671 / 0	0.0	0.0	0.18 (1)	6.42
S- R	0 / 1203	-18.5	-18.5	0.38 (4)	10.00
R- Q	0 / 35	-18.5	-18.5	0.28 (4)	10.00
Q- P	0 / 51	0.0	0.0	0.06 (1)	10.00
P- E	-575 / 0	0.0	0.0	0.07 (1)	6.25
P- O	0 / 1794	-18.5	-18.5	0.34 (1)	10.00
O- N	0 / 1383	-18.5	-18.5	0.30 (1)	10.00
N- M	0 / 1743	-18.5	-18.5	0.36 (1)	10.00
L- M	0 / 17	0.0	0.0	0.08 (1)	10.00
M- H	-102 / 11	0.0	0.0	0.08 (1)	7.81
L- K	0 / 52	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 52	0.02 (4)
R- D	-19 / 83	0.03 (4)
R- P	0 / 1190	0.19 (1)
D- P	0 / 957	0.22 (1)
P- F	0 / 199	0.04 (1)
O- F	-517 / 0	0.42 (1)
O- G	0 / 726	0.16 (1)
N- G	0 / 174	0.05 (4)
N- H	-377 / 0	0.26 (1)
S- C	-1863 / 0	0.75 (1)
M- K	-64 / 0	0.01 (1)
M- I	0 / 1712	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.64/1.00 (D-E:1) , BC=0.38/1.00 (R-S:4) , WB=0.75/1.00 (C-S:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.52 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T5	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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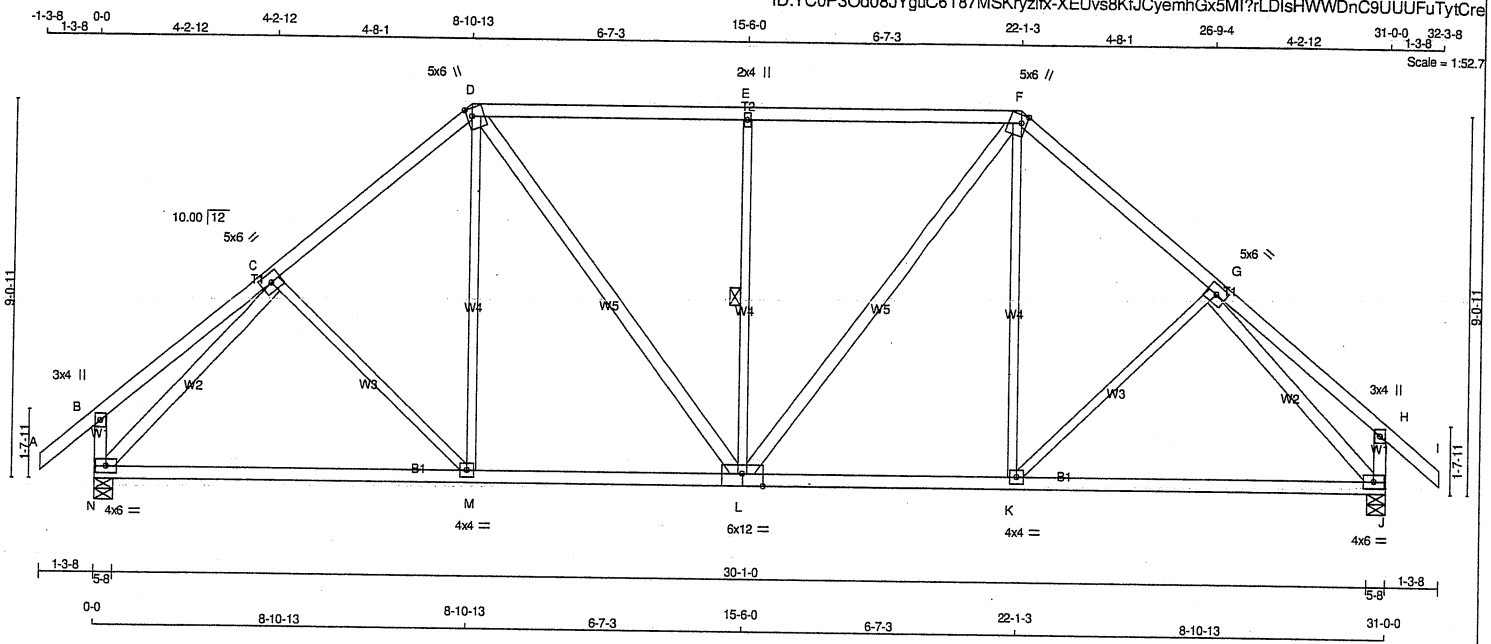

TOTAL WEIGHT = 2 X 153 = 305 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - M	2x3	DRY	No.2	SPF
M - D	2x3	DRY	No.2	SPF
L - E	2x3	DRY	No.2	SPF
K - F	2x3	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BSWWW-l	MT20	6.0	12.0	Edge	6.00
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125152

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1720	0	1720	0	0	5-8	5-8
J	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI	LC	MAX UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12	(1)	10.00
B- C	0 / 31	-84.9	-84.9	0.28	(1)	10.00
C- D	-1562 / 0	-84.9	-84.9	0.27	(1)	5.02
D- E	-1467 / 0	-84.9	-84.9	0.52	(1)	4.76
E- F	-1467 / 0	-84.9	-84.9	0.52	(1)	4.76
F- G	-1562 / 0	-84.9	-84.9	0.27	(1)	5.02
G- H	0 / 31	-84.9	-84.9	0.28	(1)	10.00
H- I	0 / 38	-84.9	-84.9	0.12	(1)	10.00
N- B	-243 / 0	0.0	0.0	0.03	(1)	7.81
J- H	-243 / 0	0.0	0.0	0.03	(1)	7.81
N- M	0 / 1224	-18.5	-18.5	0.45	(4)	10.00
M- L	0 / 1180	-18.5	-18.5	0.45	(4)	10.00
L- K	0 / 1180	-18.5	-18.5	0.45	(4)	10.00
K- J	0 / 1224	-18.5	-18.5	0.45	(4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
C- M	-65 / 30	0.06	(1)
M- D	0 / 226	0.06	(4)
D- L	0 / 478	0.08	(1)
L- E	-687 / 0	0.37	(1)
L- F	0 / 478	0.08	(1)
K- F	0 / 226	0.06	(4)
K- G	-65 / 30	0.06	(1)
N- C	-1867 / 0	0.90	(1)
G- J	-1867 / 0	0.90	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.52/1.00 (D-E:1) , BC=0.45/1.00 (L-M:4) , WB=0.90/1.00 (G-J:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.42 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T6	10	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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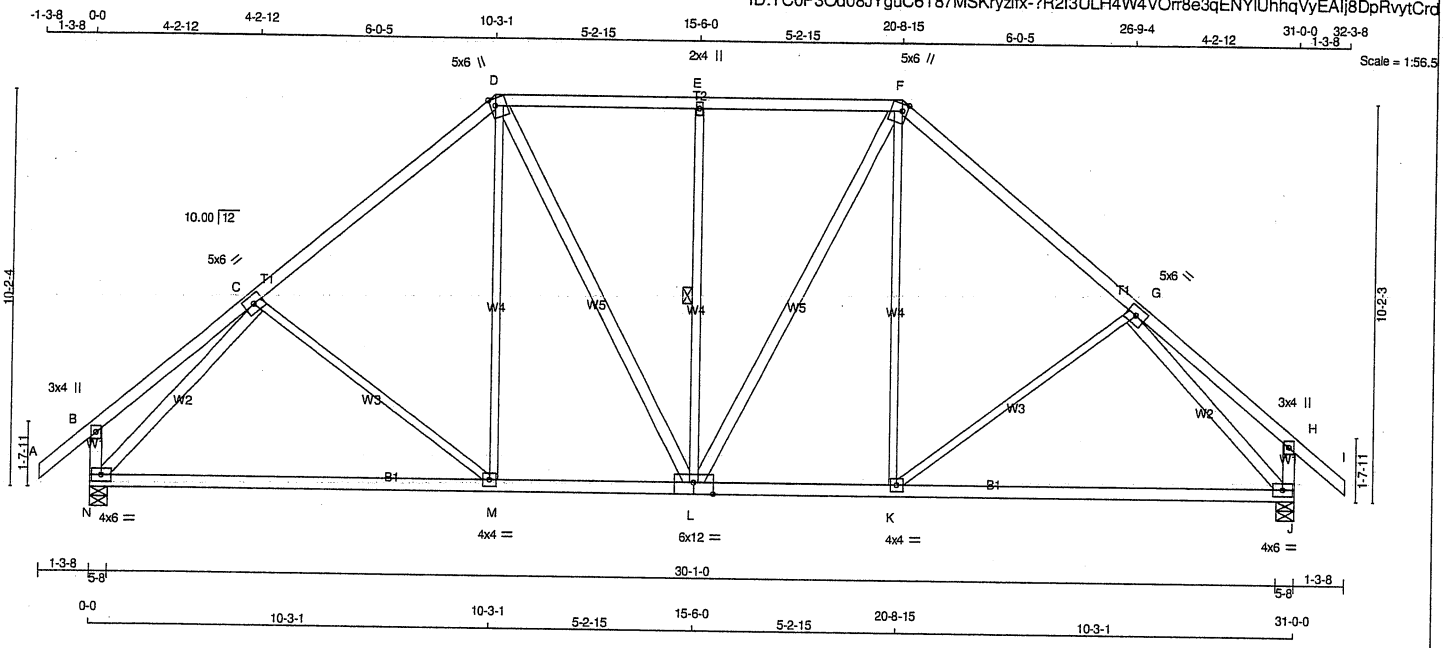

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - M	2x3	DRY	No.2	SPF
M - D	2x3	DRY	No.2	SPF
L - E	2x3	DRY	No.2	SPF
K - F	2x3	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BSWWW-I	MT20	6.0	12.0	Edge	6.00
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125153

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1720	0	1720	0	0	5-8	5-8
J	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 46	-84.9	-84.9	0.41 (1)	10.00
C- D	-1504 / 0	-84.9	-84.9	0.45 (1)	4.88
D- E	-1283 / 0	-84.9	-84.9	0.32 (1)	5.33
E- F	-1283 / 0	-84.9	-84.9	0.32 (1)	5.33
F- G	-1504 / 0	-84.9	-84.9	0.45 (1)	4.88
G- H	0 / 46	-84.9	-84.9	0.41 (1)	10.00
H- I	0 / 38	-84.9	-84.9	0.12 (1)	10.00
N- B	-220 / 0	0.0	0.0	0.02 (1)	7.81
J- H	-220 / 0	0.0	0.0	0.02 (1)	7.81
N- M	0 / 1235	-18.5	-18.5	0.53 (4)	10.00
M- L	0 / 1133	-18.5	-18.5	0.53 (4)	10.00
L- K	0 / 1133	-18.5	-18.5	0.53 (4)	10.00
K- J	0 / 1235	-18.5	-18.5	0.53 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-132 / 16	0.17 (1)
M- D	0 / 276	0.07 (4)
D- L	0 / 321	0.05 (1)
L- E	-541 / 0	0.39 (1)
L- F	0 / 321	0.05 (1)
K- F	0 / 276	0.07 (4)
K- G	-132 / 16	0.17 (1)
N- C	-1883 / 0	0.92 (1)
G- J	-1883 / 0	0.92 (1)

TOTAL WEIGHT = 10 X 159 = 1587 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.45/1.00 (C-D:1) , BC=0.53/1.00 (L-M:4) , WB=0.92/1.00 (G-J:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.42 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T8	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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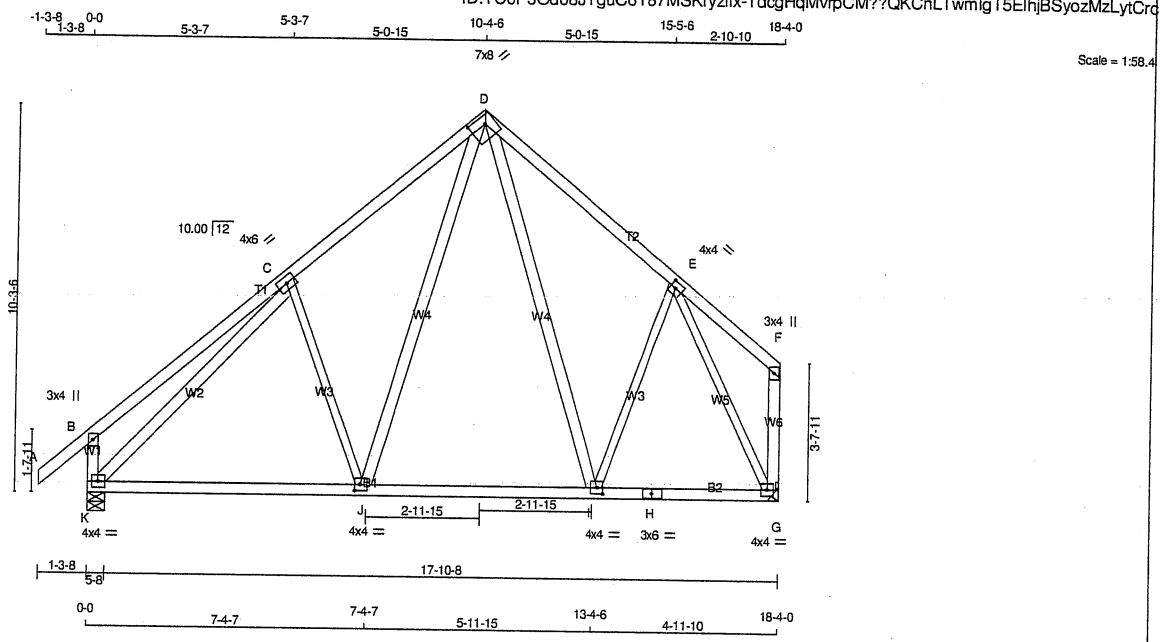

TOTAL WEIGHT = 3 X 101 = 303 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
E - G	2x3	DRY	No.2	SPF
C - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-h	MT20	7.0	8.0	2.50	5.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMWW-t	MT20	4.0	4.0	2.00	1.75
K	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1065	0	1065	0	0	5-8	5-8
G	947	0	947	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	754	491 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
G	673	427 / 0	0 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)		10.00
B- C	0 / 34	-84.9	-84.9	0.38 (1)		10.00
C- D	-760 / 0	-84.9	-84.9	0.30 (1)		6.25
D- E	-606 / 0	-84.9	-84.9	0.26 (1)		6.25
E- F	0 / 45	-84.9	-84.9	0.28 (1)		10.00
K- B	-281 / 0	0.0	0.0	0.03 (1)		7.81
G- F	-50 / 0	0.0	0.0	0.01 (1)		7.81
K- J	0 / 670	-18.5	-18.5	0.28 (4)		10.00
J- I	0 / 427	-18.5	-18.5	0.26 (4)		10.00
I- H	0 / 431	-18.5	-18.5	0.14 (4)		10.00
H- G	0 / 431	-18.5	-18.5	0.14 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 446	0.07 (1)
D- I	0 / 64	0.01 (4)
I- E	0 / 68	0.02 (4)
K- C	-990 / 0	0.81 (1)
E- G	-963 / 0	0.65 (1)
C- J	-304 / 0	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.28/1.00 (J-K:4) , WB=0.81/1.00 (C-K:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

Structural component only
DWG# T-2125154

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T8S	1	1	ROYAL PINE HOMES TRUSS DESC.	

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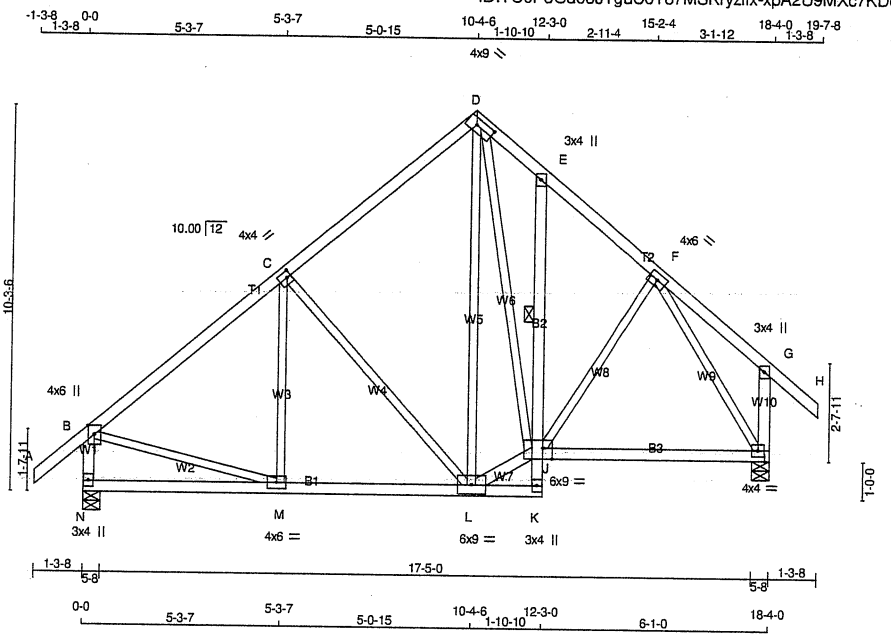

TOTAL WEIGHT = 106 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-I	MT20	4.0	9.0	2.00	5.75
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BVMWWW-I	MT20	6.0	9.0	Edge	3.00
K	BMV+p	MT20	3.0	4.0		
L	BMWWW-t	MT20	6.0	9.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125155

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1065	0	1065	0	0	5-8	5-8
I	1065	0	1065	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	754	491 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
I	754	491 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	M- C	-62 / 63	0.04 (1)
B- C	-853 / 0	-84.9	-84.9	0.31 (1)	6.23	C- L	-373 / 0	0.44 (1)
C- D	-599 / 0	-84.9	-84.9	0.30 (1)	6.25	L- D	0 / 118	0.03 (4)
D- E	-689 / 0	-84.9	-84.9	0.04 (1)	6.25	L- J	0 / 486	0.08 (1)
E- F	-695 / 0	-84.9	-84.9	0.09 (1)	6.25	D- J	0 / 463	0.10 (1)
F- G	0 / 17	-84.9	-84.9	0.12 (1)	10.00	J- F	0 / 52	0.02 (4)
G- H	0 / 38	-84.9	-84.9	0.12 (1)	10.00	B- M	0 / 701	0.16 (1)
N- B	-1025 / 0	0.0	0.0	0.11 (1)	7.74	F- I	-931 / 0	0.50 (1)
I- G	-220 / 0	0.0	0.0	0.03 (1)	7.81			
N- M	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
M- L	0 / 681	-18.5	-18.5	0.18 (4)	10.00			
L- K	0 / 12	-18.5	-18.5	0.07 (4)	10.00			
K- J	-4 / 0	0.0	0.0	0.01 (1)	10.00			
J- E	-203 / 0	0.0	0.0	0.02 (1)	6.25			
J- I	0 / 497	-18.5	-18.5	0.25 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.25/1.00 (I-J:4) , WB=0.50/1.00 (F-I:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T8S1	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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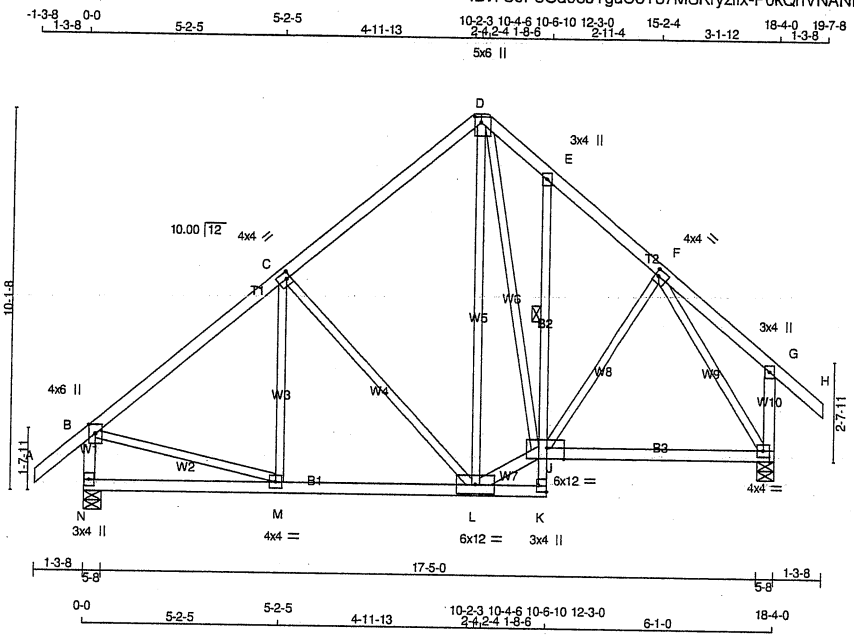

TOTAL WEIGHT = 103 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - E	2x3	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+p	MT20	5.0	6.0	1.75	2.00
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BVMWWW-I	MT20	6.0	12.0	Edge	5.50
K	BMV+p	MT20	3.0	4.0	2.00	Edge
L	BMWWW-t	MT20	6.0	12.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1065	0	1065	0	0	5-8	5-8
I	1065	0	1065	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	754	491 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
I	754	491 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-855 / 0	-84.9	-84.9	0.31 (1)	6.22
C- D	-598 / 0	-84.9	-84.9	0.30 (1)	6.25
D- E	-687 / 0	-84.9	-84.9	0.04 (1)	6.25
E- F	-697 / 0	-84.9	-84.9	0.09 (1)	6.25
F- G	0 / 17	-84.9	-84.9	0.12 (1)	10.00
G- H	0 / 38	-84.9	-84.9	0.12 (1)	10.00
N- B	-1026 / 0	0.0	0.0	0.11 (1)	7.74
I- G	-224 / 0	0.0	0.0	0.03 (1)	7.81
N- M	0 / 0	-18.5	-18.5	0.12 (4)	10.00
M- L	0 / 683	-18.5	-18.5	0.18 (4)	10.00
L- K	0 / 4	-18.5	-18.5	0.07 (4)	10.00
K- J	-4 / 0	0.0	0.0	0.01 (1)	10.00
J- E	-194 / 0	0.0	0.0	0.02 (1)	6.25
J- I	0 / 500	-18.5	-18.5	0.25 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-66 / 61	0.04 (1)
B- M	0 / 705	0.16 (1)
L- D	0 / 116	0.03 (4)
L- J	0 / 492	0.08 (1)
C- L	-370 / 0	0.44 (1)
J- F	0 / 47	0.02 (4)
F- I	-930 / 0	0.52 (1)
D- J	0 / 456	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.25/1.00 (I-J:4) , WB=0.52/1.00 (F-I:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125156

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T8S2	1	1	ROYAL PINE HOMES TRUSS DESC.	

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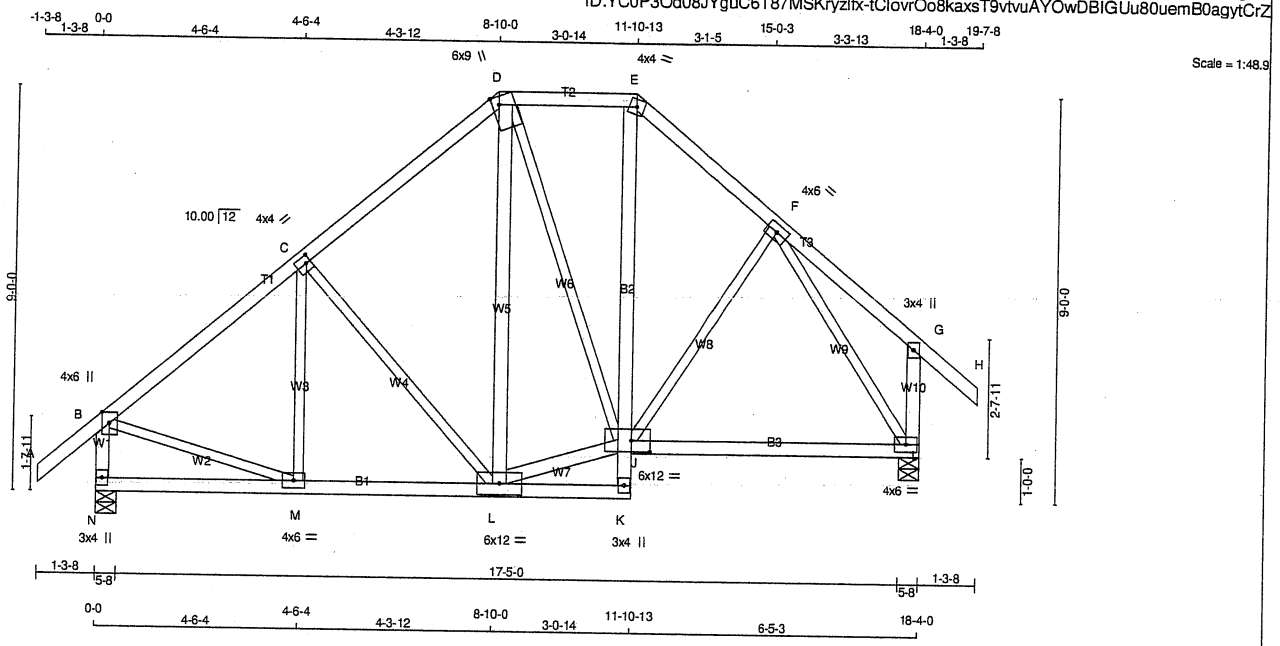

TOTAL WEIGHT = 106 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - D	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TTV-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BVMWWW-I	MT20	6.0	12.0	3.00	5.00
K	BMV+p	MT20	3.0	4.0		
L	BMWWW-t	MT20	6.0	12.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1065	0	1065	0	0	5-8	5-8
I	1065	0	1065	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	754	491 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
I	754	491 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-859 / 0	-84.9	-84.9	0.22 (1)	6.25
C- D	-682 / 0	-84.9	-84.9	0.22 (1)	6.25
D- E	-517 / 0	-84.9	-84.9	0.10 (1)	6.25
E- F	-691 / 0	-84.9	-84.9	0.11 (1)	6.25
F- G	0 / 20	-84.9	-84.9	0.14 (1)	10.00
G- H	0 / 38	-84.9	-84.9	0.12 (1)	10.00
N- B	-1030 / 0	0.0	0.0	0.11 (1)	7.73
I- G	-225 / 0	0.0	0.0	0.03 (1)	7.81
N- M	0 / 0	-18.5	-18.5	0.09 (4)	10.00
M- L	0 / 682	-18.5	-18.5	0.15 (1)	10.00
L- K	0 / 4	-18.5	-18.5	0.05 (4)	10.00
K- J	0 / 21	0.0	0.0	0.01 (1)	10.00
J- E	0 / 197	0.0	0.0	0.04 (1)	10.00
J- I	0 / 505	-18.5	-18.5	0.28 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-105 / 34	0.05 (1)
C- L	-279 / 0	0.23 (1)
L- D	0 / 109	0.02 (4)
L- J	0 / 526	0.08 (1)
D- J	0 / 46	0.01 (1)
J- F	0 / 39	0.01 (4)
B- M	0 / 710	0.16 (1)
F- I	-929 / 0	0.54 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.28/1.00 (I-J:4) , WB=0.54/1.00 (F-I:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125157

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T9	1	1	ROYAL PINE HOMES TRUSS DESC.	

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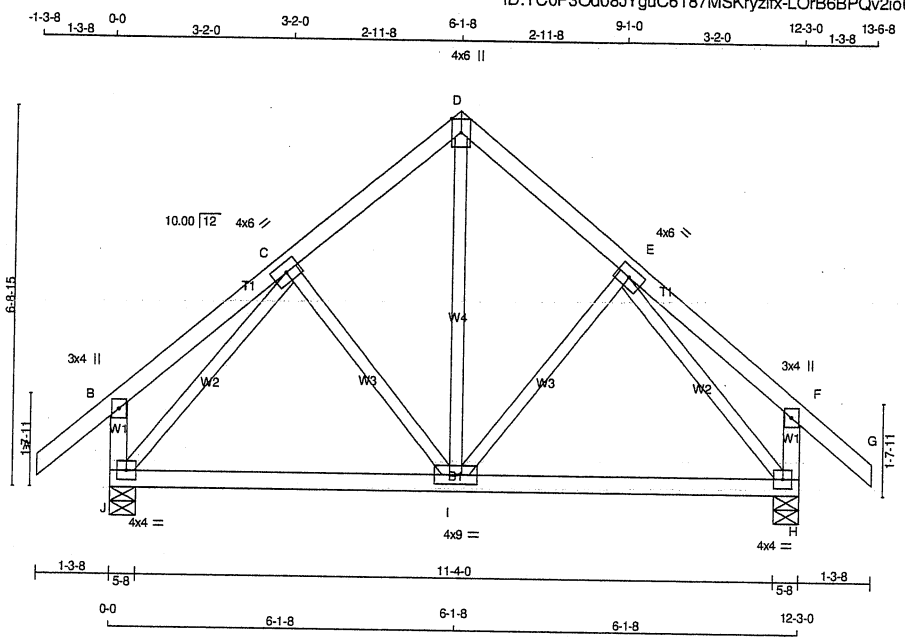


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	751	0	751	0	0	5-8	5-8
H	751	0	751	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)		10.00
B- C	0 / 19	-84.9	-84.9	0.12 (1)		10.00
C- D	-428 / 0	-84.9	-84.9	0.10 (1)		6.25
D- E	-428 / 0	-84.9	-84.9	0.10 (1)		6.25
E- F	0 / 19	-84.9	-84.9	0.12 (1)		10.00
F- G	0 / 38	-84.9	-84.9	0.12 (1)		10.00
J- B	-217 / 0	0.0	0.0	0.02 (1)		7.81
H- F	-217 / 0	0.0	0.0	0.02 (1)		7.81
J- I	0 / 388	-18.5	-18.5	0.23 (4)		10.00
I- H	0 / 388	-18.5	-18.5	0.23 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 320	0.07 (1)
I- E	-116 / 0	0.05 (1)
C- I	-116 / 0	0.05 (1)
J- C	-625 / 0	0.24 (1)
E- H	-625 / 0	0.24 (1)

TOTAL WEIGHT = 59 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (B-C:1) , BC=0.23/1.00 (I-J:4) , WB=0.24/1.00 (C-J:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

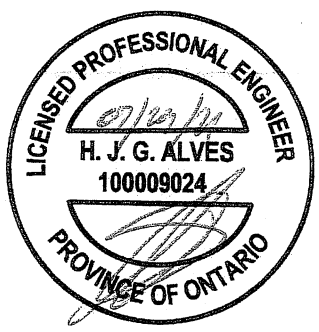
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (H) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)



Structural component only
DWG# T-2125158

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T10	1	1	ROYAL PINE HOMES TRUSS DESC.	

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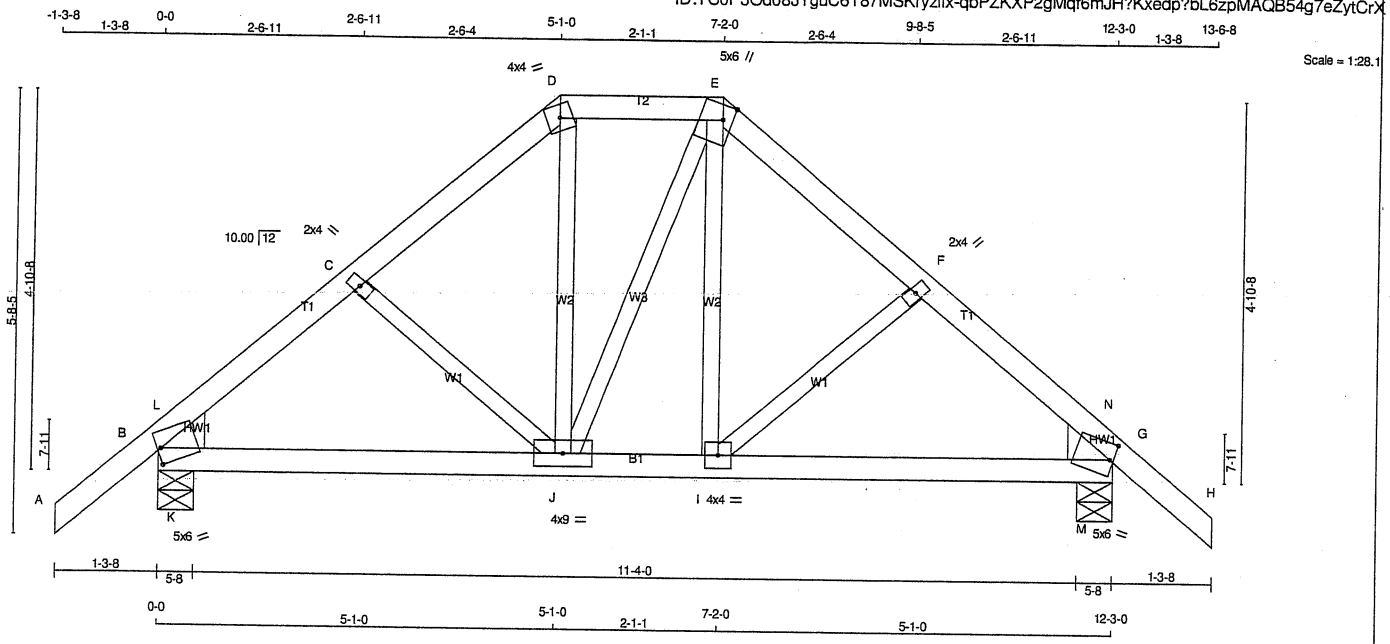

TOTAL WEIGHT = 54 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	5.0	6.0	2.50	0.50
C	TMW+w	MT20	2.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMBH1-m	MT20	5.0	6.0	2.50	0.50
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	4.0	9.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	751	0	751	0	0	5-8	5-8	2x6 L
G	751	0	751	0	0	5-8	5-8	2x6 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
G	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 31	-84.9	-84.9	0.12 (1)	10.00	C- J	-160 / 0	0.04 (1)
B- L	-951 / 0	-84.9	-84.9	0.07 (1)	6.25	J- D	0 / 161	0.04 (1)
L- C	-704 / 0	-84.9	-84.9	0.07 (1)	6.25	J- E	0 / 2	0.00 (1)
C- D	-548 / 0	-84.9	-84.9	0.07 (1)	6.25	I- E	0 / 158	0.04 (1)
D- E	-417 / 0	-84.9	-84.9	0.05 (1)	6.25	I- F	-161 / 0	0.04 (1)
E- F	-547 / 0	-84.9	-84.9	0.07 (1)	6.25	K- L	0 / 231	0.00 (1)
F- N	-703 / 0	-84.9	-84.9	0.07 (1)	6.25	M- N	0 / 236	0.00 (1)
N- G	-955 / 0	-84.9	-84.9	0.07 (1)	6.25			
G- H	0 / 31	-84.9	-84.9	0.12 (1)	10.00			
B- K	0 / 534	-18.5	-18.5	0.16 (1)	10.00			
K- J	0 / 534	-18.5	-18.5	0.16 (1)	10.00			
J- I	0 / 416	-18.5	-18.5	0.10 (4)	10.00			
I- M	0 / 534	-18.5	-18.5	0.16 (1)	10.00			
M- G	0 / 534	-18.5	-18.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.16/1.00 (I-M:1) , WB=0.04/1.00 (F-I:1) , SSI=0.15/1.00 (G-M:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125159

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T11	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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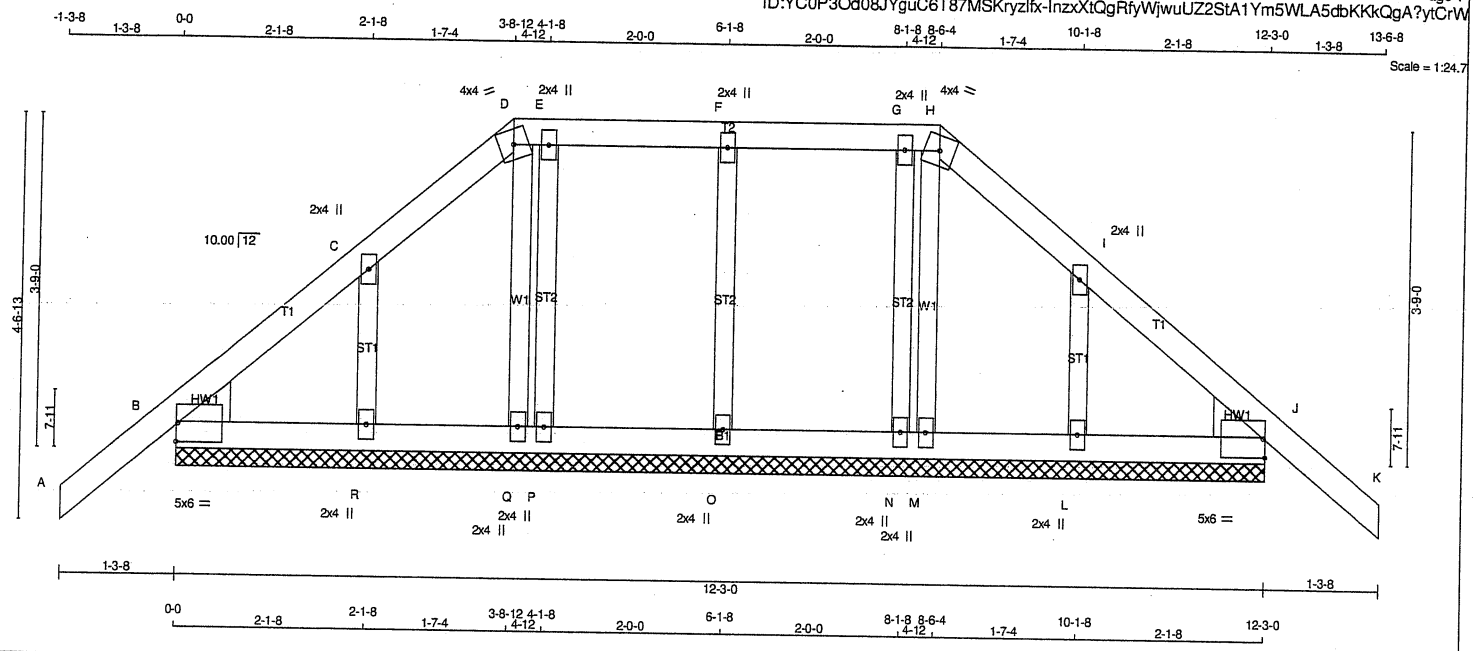

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	5.0	6.0		Edge
C, E, F, G, I						
C	TMW+w	MT20	2.0	4.0		
D	TTW-m	MT20	4.0	4.0		
H	TTW-m	MT20	4.0	4.0		
J	TMB1-I	MT20	5.0	6.0		Edge
L, M, N, O, P, Q, R						
L	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x6 L
2x6 R
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 31		-84.9	-84.9	0.12 (1)	10.00
B- C	-12 / 0		-84.9	-84.9	0.05 (1)	6.25
C- D	-25 / 0		-84.9	-84.9	0.05 (1)	6.25
D- E	-6 / 0		-84.9	-84.9	0.02 (1)	10.00
E- F	-8 / 0		-84.9	-84.9	0.05 (1)	10.00
F- G	-8 / 0		-84.9	-84.9	0.05 (1)	10.00
G- H	-6 / 0		-84.9	-84.9	0.02 (1)	10.00
H- I	-25 / 0		-84.9	-84.9	0.05 (1)	6.25
I- J	-12 / 0		-84.9	-84.9	0.05 (1)	6.25
J- K	0 / 31		-84.9	-84.9	0.12 (1)	10.00
B- R	0 / 18		-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 9		-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 8		-18.5	-18.5	0.01 (4)	10.00
P- O	0 / 8		-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 8		-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 8		-18.5	-18.5	0.01 (4)	10.00
M- L	0 / 9		-18.5	-18.5	0.02 (4)	10.00
L- J	0 / 18		-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
O- F	-191 / 0		0.04 (1)
P- E	-122 / 0		0.03 (1)
R- C	-188 / 0		0.03 (1)
N- G	-122 / 0		0.03 (1)
L- I	-188 / 0		0.03 (1)
Q- D	-29 / 0		0.01 (1)
M- H	-29 / 0		0.01 (1)

TOTAL WEIGHT = 53 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (B-R:4) , WB=0.04/1.00 (F-O:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.10 (I) (INPUT = 1.00)

Structural component only
DWG# T-2125160

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	T12	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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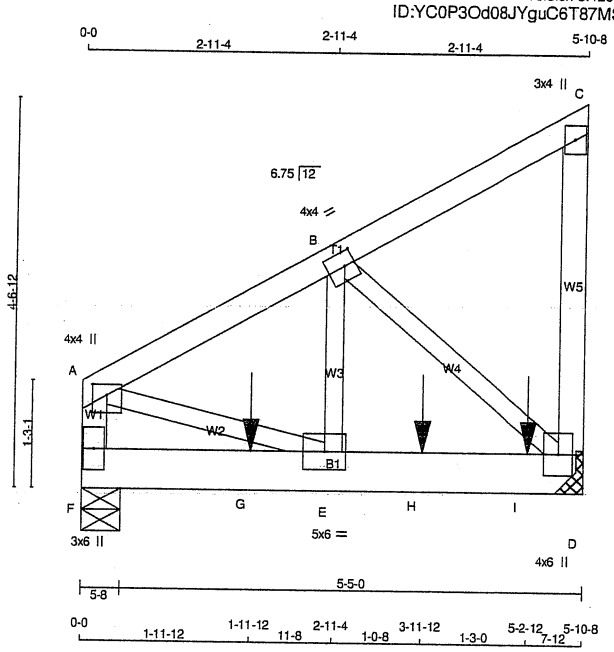

TOTAL WEIGHT = 2 X 30 = 60 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		

Structural component only
DWG# T-2125161

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	2075	0	2075	0	0	MECHANICAL	
F	1322	0	1322	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1470	950 / 0	0 / 0	0 / 0	0 / 0	521 / 0	0 / 0
F	937	605 / 0	0 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1405 / 0	-84.9	-84.9	0.06 (1)	6.25	E- B	0 / 1455	0.18 (1)
B- C	-11 / 0	-84.9	-84.9	0.05 (1)	6.25	B- D	-1623 / 0	0.21 (1)
D- C	-103 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 1285	0.16 (1)
F- A	-1157 / 0	0.0	0.0	0.06 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.18 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.18 (1)	10.00			
E- H	0 / 1234	-18.5	-18.5	0.30 (1)	10.00			
H- I	0 / 1234	-18.5	-18.5	0.30 (1)	10.00			
I- D	0 / 1234	-18.5	-18.5	0.30 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-12	-658	-658	---	BACK	VERT	TOTAL	---	C1
H	3-11-12	-658	-658	---	BACK	VERT	TOTAL	---	C1
I	5-2-12	-660	-660	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-F:1) , BC=0.30/1.00 (D-E:1) , WB=0.21/1.00 (B-D:1) , SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420226	T12	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125161

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T20	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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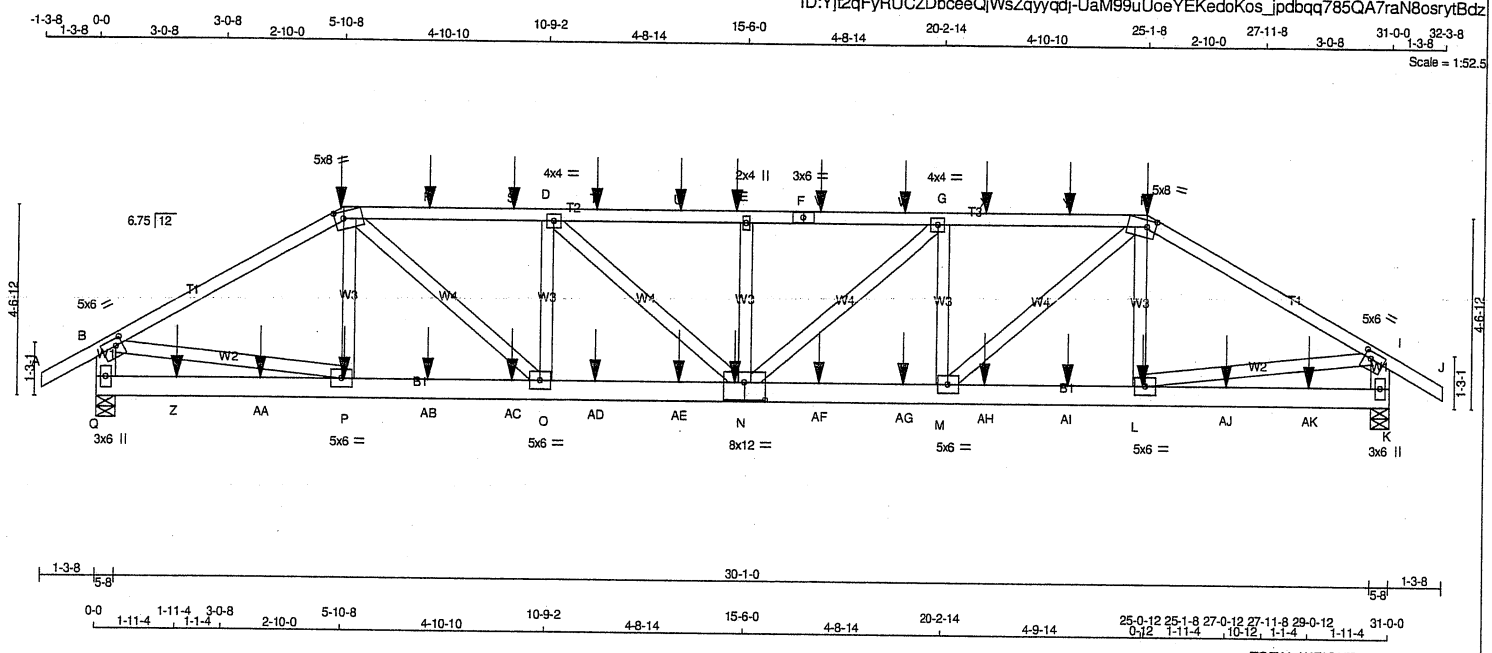

TOTAL WEIGHT = 2 X 160 = 320 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
Q- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
P- C	1	6	SIDE(2.5)
L- H	1	6	SIDE(2.5)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2125177

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2850	0	2850	0	0	5-8	5-8
K	2857	0	2857	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2022	1292 / 0	0 / 0	0 / 0	0 / 0	729 / 0	0 / 0
K	2027	1296 / 0	0 / 0	0 / 0	0 / 0	731 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.06 (1)	10.00	P- C	-296 / 43	0.03 (1)
B- C	-3913 / 0	-84.9	-84.9	0.42 (1)	4.41	L- H	-295 / 45	0.03 (1)
C- R	-4877 / 0	-84.9	-84.9	0.38 (1)	4.03	B- P	0 / 3443	0.30 (1)
R- S	-4877 / 0	-84.9	-84.9	0.38 (1)	4.03	L- I	0 / 3453	0.31 (1)
S- D	-4877 / 0	-84.9	-84.9	0.38 (1)	4.03	M- H	0 / 1940	0.17 (1)
D- T	-5329 / 0	-84.9	-84.9	0.39 (1)	3.85	C- O	0 / 1946	0.17 (1)
T- U	-5329 / 0	-84.9	-84.9	0.39 (1)	3.85	M- G	-1126 / 0	0.12 (1)
U- E	-5329 / 0	-84.9	-84.9	0.39 (1)	3.85	O- D	-1130 / 0	0.12 (1)
E- F	-5329 / 0	-84.9	-84.9	0.39 (1)	3.85	N- G	0 / 595	0.05 (1)
F- V	-5329 / 0	-84.9	-84.9	0.39 (1)	3.85	D- N	0 / 602	0.05 (1)
V- W	-5329 / 0	-84.9	-84.9	0.39 (1)	3.85	N- E	-624 / 0	0.07 (1)
W- G	-5329 / 0	-84.9	-84.9	0.39 (1)	3.85			
G- X	-4883 / 0	-84.9	-84.9	0.38 (1)	4.02			
X- Y	-4883 / 0	-84.9	-84.9	0.38 (1)	4.02			
Y- H	-4883 / 0	-84.9	-84.9	0.38 (1)	4.02			
H- I	-3925 / 0	-84.9	-84.9	0.42 (1)	4.41			
I- J	0 / 28	-84.9	-84.9	0.06 (1)	10.00			
Q- B	-2774 / 0	0.0	0.0	0.10 (1)	7.81			
K- I	-2781 / 0	0.0	0.0	0.10 (1)	7.81			
Q- Z	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
Z-AA	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AA- P	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
P-AB	0 / 3398	-18.5	-18.5	0.25 (1)	10.00			
AB-AC	0 / 3398	-18.5	-18.5	0.25 (1)	10.00			
AC- O	0 / 3398	-18.5	-18.5	0.25 (1)	10.00			
O-AD	0 / 4878	-18.5	-18.5	0.35 (1)	10.00			
AD-AE	0 / 4878	-18.5	-18.5	0.35 (1)	10.00			
AE- N	0 / 4878	-18.5	-18.5	0.35 (1)	10.00			
N-AF	0 / 4883	-18.5	-18.5	0.35 (1)	10.00			
AF-AG	0 / 4883	-18.5	-18.5	0.35 (1)	10.00			
AG- M	0 / 4883	-18.5	-18.5	0.35 (1)	10.00			
M-AH	0 / 3408	-18.5	-18.5	0.25 (1)	10.00			
AH-AI	0 / 3408	-18.5	-18.5	0.25 (1)	10.00			
AI- L	0 / 3408	-18.5	-18.5	0.25 (1)	10.00			
L-AJ	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AJ-AK	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AK- K	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-330	-330	---	FRONT	VERT	TOTAL	---	C1
E	15-3-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.42/1.00 (H-I:1) , BC=0.35/1.00 (M-N:1) , WB=0.31/1.00 (I-L:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420228	T20	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.00
C	TTWW-m	MT20	5.0	8.0	2.00	2.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	2.50
I	TMVW-t	MT20	5.0	6.0	2.00	2.00
K	BMV1+p	MT20	3.0	6.0		
L, M, O, P						
L	BMWW-t	MT20	5.0	6.0		
N	BSWWW-l	MT20	8.0	12.0	5.00	6.00
Q	BMV1+p	MT20	3.0	6.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	25-1-8	-330	-330	---	FRONT	VERT	TOTAL	---	C1
L	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	15-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
P	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	7-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
S	9-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
T	11-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
U	13-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
V	17-3-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
W	19-3-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
X	21-3-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Y	23-3-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Z	1-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AA	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AB	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AC	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AD	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AE	13-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AF	17-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	19-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	21-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AI	23-3-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	27-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	29-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2125177

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T20Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 14:31:29 2021 Page 1
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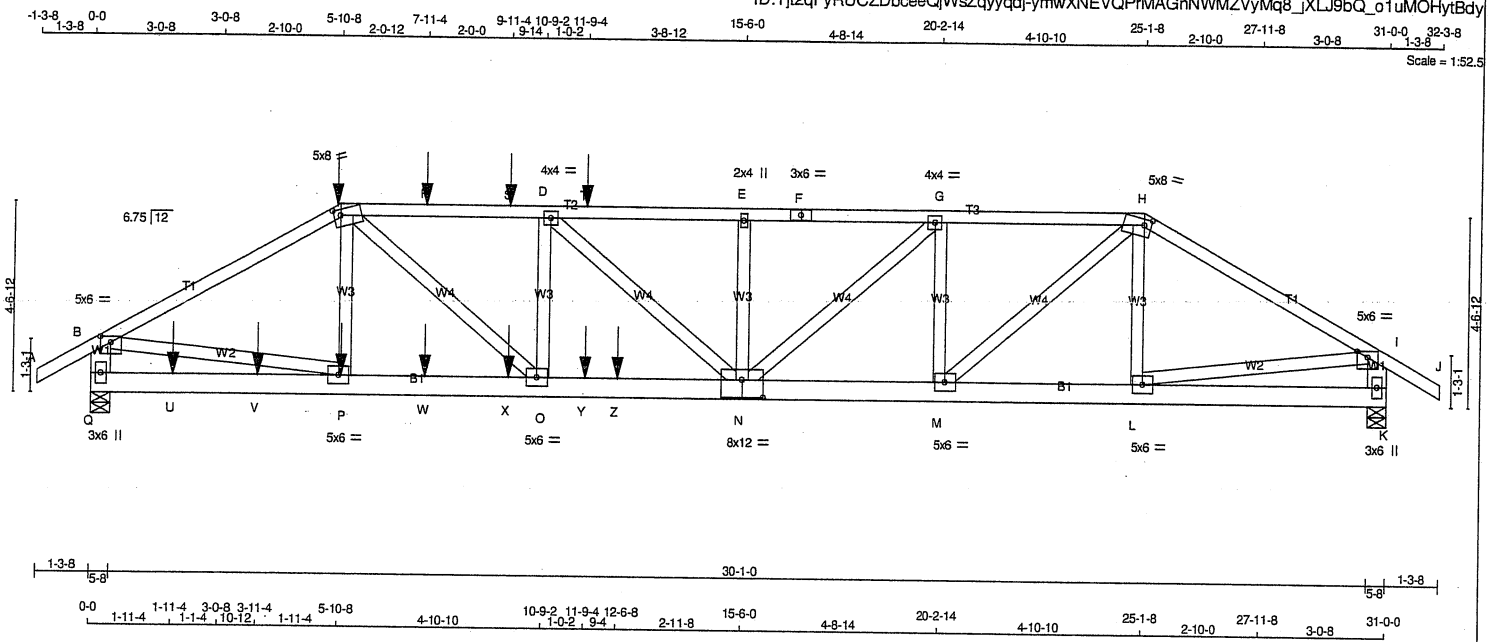

TOTAL WEIGHT = 2 X 160 = 320 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
H- J	1	12	TOP
Q- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	SIDE(183.1)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
P- C	1	6	SIDE(2.5)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2125178

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	3370	0	3370	0	0	5-8	5-8
K	2576	0	2576	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2389	1539 / 0	0 / 0	0 / 0	0 / 0	850 / 0	0 / 0
K	1825	1181 / 0	0 / 0	0 / 0	0 / 0	644 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.06 (1)	10.00
B- C	-4718 / 0	-84.9	-84.9	0.48 (1)	4.05
C- R	-6367 / 0	-84.9	-84.9	0.43 (1)	3.54
R- S	-6367 / 0	-84.9	-84.9	0.43 (1)	3.54
S- D	-6367 / 0	-84.9	-84.9	0.43 (1)	3.54
D- T	-6170 / 0	-84.9	-84.9	0.41 (1)	3.60
T- E	-6170 / 0	-84.9	-84.9	0.41 (1)	3.60
E- F	-6170 / 0	-84.9	-84.9	0.33 (1)	3.70
F- G	-6170 / 0	-84.9	-84.9	0.33 (1)	3.70
G- H	-4933 / 0	-84.9	-84.9	0.27 (1)	4.14
H- I	-3506 / 0	-84.9	-84.9	0.40 (1)	4.63
I- J	0 / 28	-84.9	-84.9	0.06 (1)	10.00
Q- B	-3270 / 0	0.0	0.0	0.12 (1)	7.68
K- I	-2524 / 0	0.0	0.0	0.09 (1)	7.81
Q- U	0 / 0	-18.5	-18.5	0.08 (4)	10.00
U- V	0 / 0	-18.5	-18.5	0.08 (4)	10.00
V- P	0 / 0	-18.5	-18.5	0.08 (4)	10.00
P- W	0 / 4088	-18.5	-18.5	0.51 (1)	10.00
W- X	0 / 4088	-18.5	-18.5	0.51 (1)	10.00
X- O	0 / 4088	-18.5	-18.5	0.51 (1)	10.00
O- Y	0 / 6368	-18.5	-18.5	0.93 (1)	10.00
Y- Z	0 / 6368	-18.5	-18.5	0.93 (1)	10.00
Z- N	0 / 6368	-18.5	-18.5	0.93 (1)	10.00
N- M	0 / 4933	-18.5	-18.5	0.35 (1)	10.00
M- L	0 / 3042	-18.5	-18.5	0.22 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-594 / 0	0.06 (1)
L- H	-320 / 0	0.03 (1)
B- P	0 / 4150	0.37 (1)
L- I	0 / 3084	0.27 (1)
M- H	0 / 2487	0.22 (1)
C- O	0 / 2999	0.27 (1)
M- G	-1549 / 0	0.17 (1)
O- D	-528 / 0	0.06 (1)
N- G	0 / 1650	0.15 (1)
D- N	-263 / 0	0.06 (1)
N- E	-365 / 0	0.04 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-330	-330	---	BACK	VERT	TOTAL	---	C1
P	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
R	7-11-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
S	9-11-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
T	11-9-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
U	1-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
V	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
W	7-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
X	9-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	11-9-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Z	12-6-8	-1110	-1110	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.93/1.00 (N-O:1) , WB=0.37/1.00 (B-P:1) , SSI=0.39/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.46 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	5.0	8.0	1.75	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	1.75	2.00
I	TMVW-p	MT20	5.0	6.0	1.75	3.00
K	BMV1+p	MT20	3.0	6.0		
L, M, O, P						
L	BMWW-t	MT20	5.0	6.0		
N	BSWWW-l	MT20	8.0	12.0	5.00	6.00
Q	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2125178

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420229	T20S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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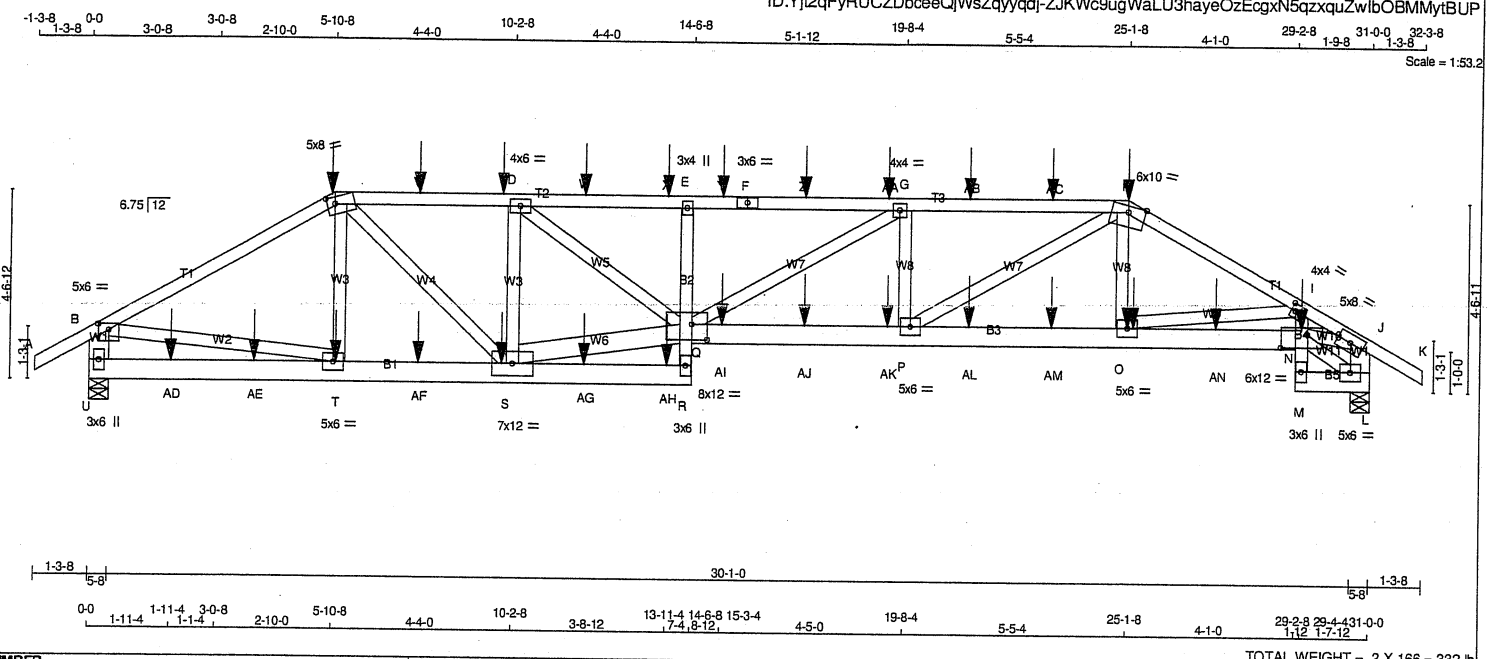


TOTAL WEIGHT = 2 X 166 = 332 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF

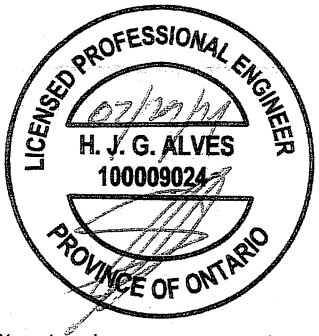
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- K	1	12	SIDE(61.0)
U- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- R	2	12	SIDE(183.1)
Q- N	2	12	SIDE(183.1)
M- L	2	12	TOP
R- E	1	12	TOP
I- M	1	12	SIDE(11.4)
WEBS : (0.122"X3") SPIRAL NAILS			
T- C	1	6	SIDE(2.5)
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.



Structural component only
DWG# T-2125193

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3002	0	3002	0	0	5-8	5-8
L	3414	0	3414	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2129	1363 / 0	0 / 0	0 / 0	0 / 0	766 / 0	0 / 0
L	.2417	1570 / 0	0 / 0	0 / 0	0 / 0	847 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.06 (1)	10.00
B- C	-4154 / 0	-84.9	-84.9	0.44 (1)	4.30
C- V	-5087 / 0	-84.9	-84.9	0.28 (1)	4.05
V- D	-5087 / 0	-84.9	-84.9	0.28 (1)	4.05
D- W	-7599 / 0	-84.9	-84.9	0.41 (1)	3.27
W- X	-7599 / 0	-84.9	-84.9	0.41 (1)	3.27
X- E	-7599 / 0	-84.9	-84.9	0.41 (1)	3.27
E- Y	-7652 / 0	-84.9	-84.9	0.63 (1)	3.06
Y- F	-7652 / 0	-84.9	-84.9	0.63 (1)	3.06
F- Z	-7652 / 0	-84.9	-84.9	0.63 (1)	3.06
Z-AA	-7652 / 0	-84.9	-84.9	0.63 (1)	3.06
AA- G	-7652 / 0	-84.9	-84.9	0.63 (1)	3.06
G-AB	-7374 / 0	-84.9	-84.9	0.62 (1)	3.14
AB-AC	-7374 / 0	-84.9	-84.9	0.62 (1)	3.14
AC- H	-7374 / 0	-84.9	-84.9	0.62 (1)	3.14
H- I	-5849 / 0	-84.9	-84.9	0.35 (1)	3.76
I- J	-7349 / 0	-84.9	-84.9	0.24 (1)	3.45
J- K	0 / 28	-84.9	-84.9	0.06 (1)	10.00
U- B	-2923 / 0	0.0	0.0	0.10 (1)	7.81
L- J	-3136 / 0	0.0	0.0	0.11 (1)	7.80
U-AD	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AD-AE	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AE- T	0 / 0	-18.5	-18.5	0.07 (4)	10.00
T-AF	0 / 3606	-18.5	-18.5	0.26 (1)	10.00
AF- S	0 / 3606	-18.5	-18.5	0.26 (1)	10.00
S-AG	0 / 168	-18.5	-18.5	0.04 (4)	10.00
AG-AH	0 / 168	-18.5	-18.5	0.04 (4)	10.00
AH- R	0 / 168	-18.5	-18.5	0.04 (4)	10.00
R- Q	0 / 82	0.0	0.0	0.13 (1)	10.00
Q- E	-616 / 0	0.0	0.0	0.13 (1)	7.81
Q-AI	0 / 7374	-18.5	-18.5	0.57 (1)	10.00
AI-AJ	0 / 7374	-18.5	-18.5	0.57 (1)	10.00
AJ-AK	0 / 7374	-18.5	-18.5	0.57 (1)	10.00
AK- P	0 / 7374	-18.5	-18.5	0.57 (1)	10.00
P-AL	0 / 5149	-18.5	-18.5	0.40 (1)	10.00
AL-AM	0 / 5149	-18.5	-18.5	0.40 (1)	10.00
AM- O	0 / 5149	-18.5	-18.5	0.40 (1)	10.00
O-AN	0 / 6946	-18.5	-18.5	0.49 (1)	10.00
AN- N	0 / 6946	-18.5	-18.5	0.49 (1)	10.00
M- N	0 / 17	0.0	0.0	0.32 (1)	10.00
N- I	0 / 906	0.0	0.0	0.40 (1)	10.00
M- L	0 / 412	-18.5	-18.5	0.03 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-347 / 20	0.04 (1)
C- S	0 / 2082	0.18 (1)
S- D	-2496 / 0	0.27 (1)
S- Q	0 / 5115	0.33 (1)
D- Q	0 / 3077	0.27 (1)
Q- G	0 / 324	0.03 (1)
P- G	-977 / 0	0.07 (1)
P- H	0 / 2577	0.23 (1)
O- H	0 / 1013	0.09 (1)
O- I	-1872 / 0	0.23 (1)
N- L	-488 / 0	0.03 (1)
N- J	0 / 6401	0.57 (1)
B- T	0 / 3655	0.32 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 960 (0.39")

CSI: TC=0.63/1.00 (E-G:1) , BC=0.57/1.00 (P-Q:1) , WB=0.57/1.00 (J-N:1) , SSI=0.25/1.00 (I-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420229	T20S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:Yjt2qFyRUCZDbceeQjWsZqyyqdj-ZJKWc9ugWaLU3hayeOzEcgxN5qzxquZwIbOBMMytBUP

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	5.0	8.0	2.00	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	10.0	Edge	
I	TMVW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-t	MT20	5.0	8.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-l	MT20	6.0	12.0	3.75	7.75
O, P, T						
O	BMWW-t	MT20	5.0	6.0		
Q	BVMWWW-l	MT20	8.0	12.0	4.50	4.50
R	BMV+p	MT20	3.0	6.0		
S	BMWWW-t	MT20	7.0	12.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-330	-330	---	FRONT	VERT	TOTAL	---	C1
D	9-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
H	25-1-8	-73	-73	---	FRONT	VERT	TOTAL	---	C1
N	29-4-4	-197	-197	---	FRONT	VERT	TOTAL	---	C1
O	25-1-8	-58	-58	---	FRONT	VERT	DEAD	---	C1
O	25-3-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
O	25-1-8	-223	-223	---	FRONT	VERT	SNOW	---	C1
S	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
W	11-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
X	13-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Y	15-3-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
Z	17-3-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AA	19-3-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AB	21-3-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AC	23-3-4	-67	-67	---	FRONT	VERT	TOTAL	---	C1
AD	1-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AE	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AF	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	13-11-4	-25	-25	---	FRONT	VERT	TOTAL	---	C1
AI	15-3-4	-66	-66	---	FRONT	VERT	TOTAL	---	C1
AJ	17-3-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AK	19-3-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AL	21-3-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AM	23-3-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AN	27-3-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.66 (J) (INPUT = 1.00)

Structural component only
DWG# T-2125193

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T21	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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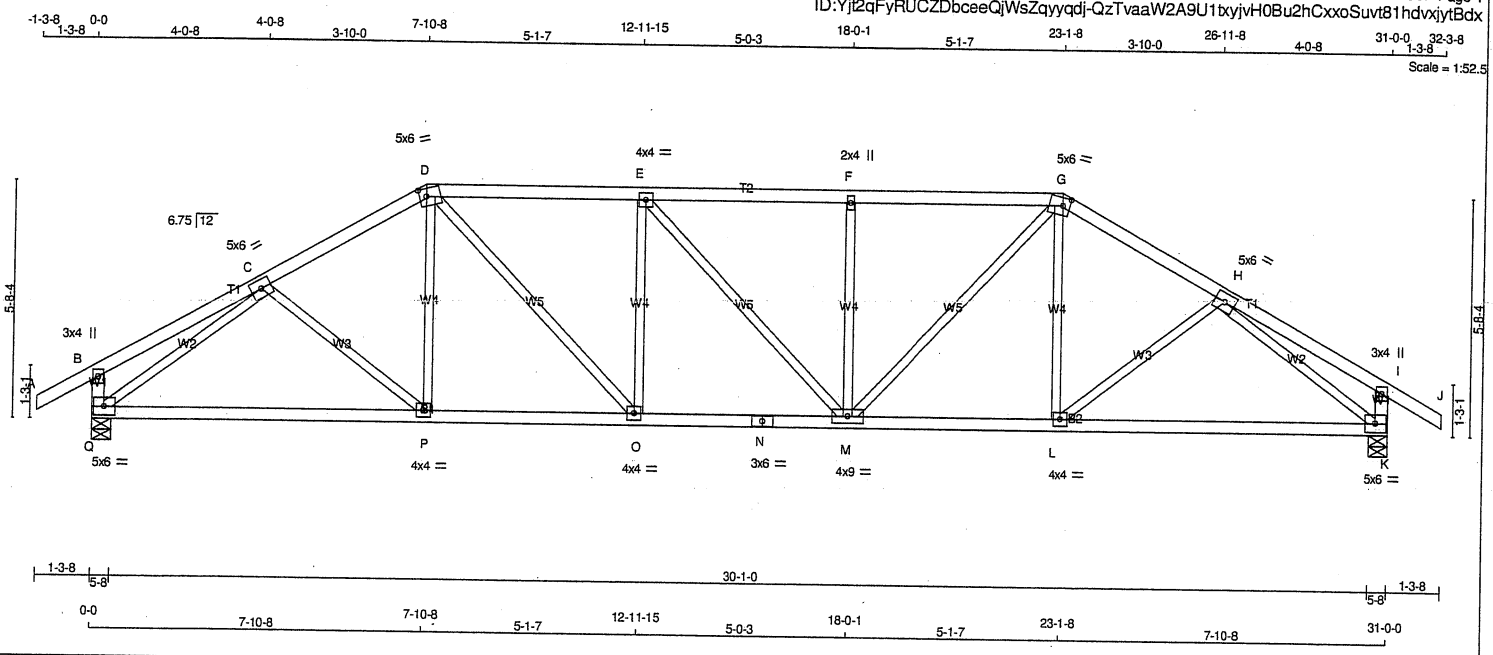

TOTAL WEIGHT = 2 X 128 = 256 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L, O, P						
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
Q	BMVW1-t	MT20	5.0	6.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1718	0	1718	0	0	5-8	5-8
K	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00		C- P	0 / 62	0.02 (4)
B- C	0 / 17	-84.9	-84.9	0.19 (1)	10.00		P- D	0 / 141	0.04 (4)
C- D	-2045 / 0	-84.9	-84.9	0.20 (1)	4.58		D- O	0 / 749	0.17 (1)
D- E	-2284 / 0	-84.9	-84.9	0.32 (1)	4.25		O- E	-465 / 0	0.23 (1)
E- F	-2283 / 0	-84.9	-84.9	0.29 (1)	4.28		E- M	-2 / 0	0.00 (1)
F- G	-2282 / 0	-84.9	-84.9	0.32 (1)	4.25		M- F	-464 / 0	0.23 (1)
G- H	-2045 / 0	-84.9	-84.9	0.20 (1)	4.58		M- G	0 / 747	0.17 (1)
H- I	0 / 17	-84.9	-84.9	0.19 (1)	10.00		L- G	0 / 142	0.04 (4)
I- J	0 / 28	-84.9	-84.9	0.11 (1)	10.00		L- H	0 / 62	0.02 (4)
Q- B	-249 / 0	0.0	0.0	0.03 (1)	7.81		Q- C	-2251 / 0	0.93 (1)
K- I	-249 / 0	0.0	0.0	0.03 (1)	7.81		H- K	-2251 / 0	0.93 (1)
Q- P	0 / 1756	-18.5	-18.5	0.42 (1)	10.00				
P- O	0 / 1768	-18.5	-18.5	0.42 (1)	10.00				
O- N	0 / 2284	-18.5	-18.5	0.42 (1)	10.00				
N- M	0 / 2284	-18.5	-18.5	0.42 (1)	10.00				
M- L	0 / 1768	-18.5	-18.5	0.42 (1)	10.00				
L- K	0 / 1757	-18.5	-18.5	0.42 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.32/1.00 (F-G:1) , BC=0.42/1.00 (M-O:1) , WB=0.93/1.00 (H-K:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.74 (N) (INPUT = 1.00)

Structural component only
DWG# T-2125179

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420229	T21S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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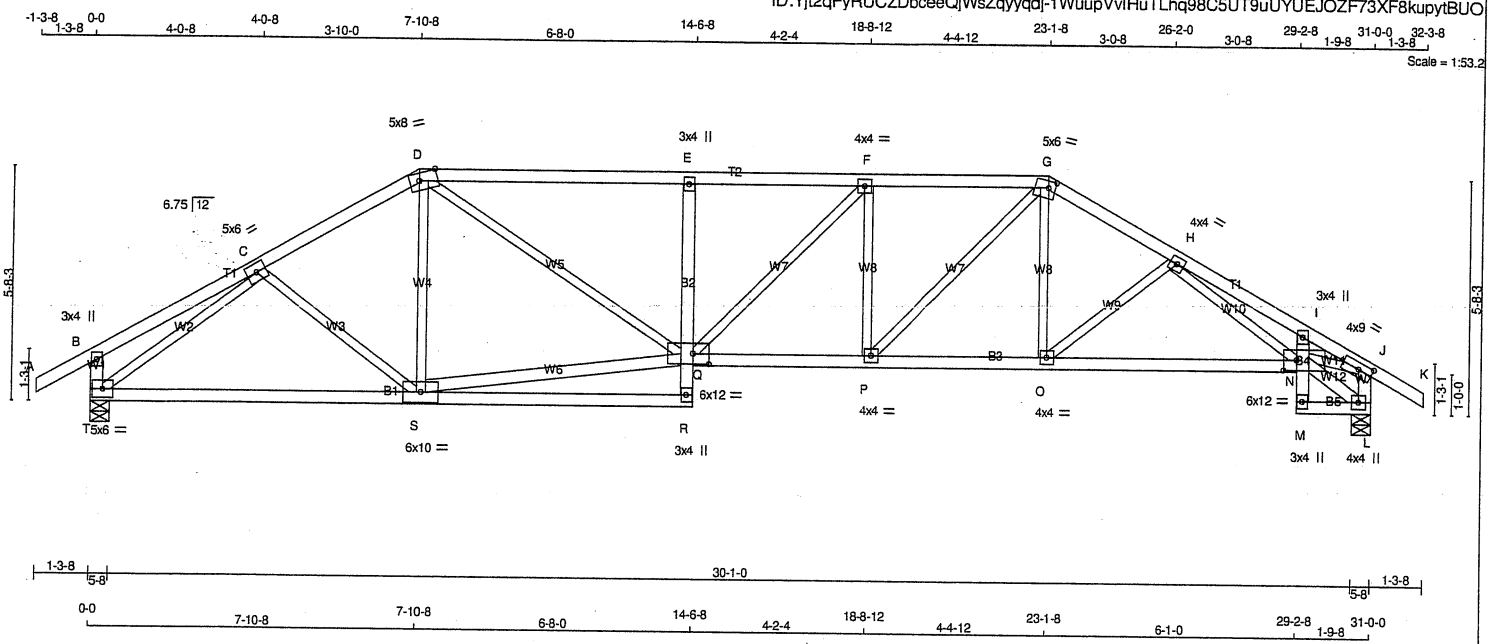

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, E, I						
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	Edge	5.25
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	2.00
H	TMWW-t	MT20	4.0	4.0		
J	TMVW-t	MT20	4.0	9.0	2.00	4.00
L	BMVW1+p	MT20	4.0	4.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWWW-I	MT20	6.0	12.0	3.00	3.75
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWWW-I	MT20	6.0	12.0	3.00	4.75
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	10.0		
T	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2125194

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1715	0	1715	0	0	5-8	5-8
L	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1215	784 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1219	787 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 17	-84.9	-84.9	0.19 (1)	10.00
C- D	-2045 / 0	-84.9	-84.9	0.20 (1)	4.58
D- E	-2855 / 0	-84.9	-84.9	0.59 (1)	3.56
E- F	-2873 / 0	-84.9	-84.9	0.42 (1)	3.71
F- G	-2721 / 0	-84.9	-84.9	0.29 (1)	3.97
G- H	-2476 / 0	-84.9	-84.9	0.18 (1)	4.24
H- I	-3044 / 0	-84.9	-84.9	0.26 (1)	3.81
I- J	-2919 / 0	-84.9	-84.9	0.18 (1)	3.95
J- K	0 / 28	-84.9	-84.9	0.11 (1)	10.00
T- B	-248 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-1608 / 0	0.0	0.0	0.16 (1)	6.52
T- S	0 / 1757	-18.5	-18.5	0.44 (1)	10.00
S- R	0 / 76	-18.5	-18.5	0.29 (4)	10.00
R- Q	0 / 47	0.0	0.0	0.11 (1)	10.00
Q- E	-547 / 0	0.0	0.0	0.14 (1)	7.81
Q- P	0 / 2721	-18.5	-18.5	0.49 (1)	10.00
P- O	0 / 2149	-18.5	-18.5	0.43 (1)	10.00
O- N	0 / 2458	-18.5	-18.5	0.48 (1)	10.00
M- N	0 / 17	0.0	0.0	0.22 (1)	10.00
N- I	-116 / 0	0.0	0.0	0.17 (1)	7.81
M- L	0 / 160	-18.5	-18.5	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	0 / 62	0.02 (4)
S- D	-101 / 51	0.05 (1)
S- Q	0 / 1710	0.27 (1)
D- Q	0 / 1313	0.30 (1)
Q- F	0 / 216	0.05 (1)
P- F	-498 / 0	0.16 (1)
P- G	0 / 808	0.18 (1)
O- G	0 / 373	0.08 (1)
T- C	-2252 / 0	0.93 (1)
N- L	-188 / 0	0.02 (1)
N- J	0 / 2500	0.56 (1)
O- H	-408 / 0	0.12 (1)
H- N	0 / 291	0.07 (1)

TOTAL WEIGHT = 137 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.59/1.00 (D-E:1) , BC=0.49/1.00 (P-Q:1) , WB=0.93/1.00 (C-T:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.51 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T22	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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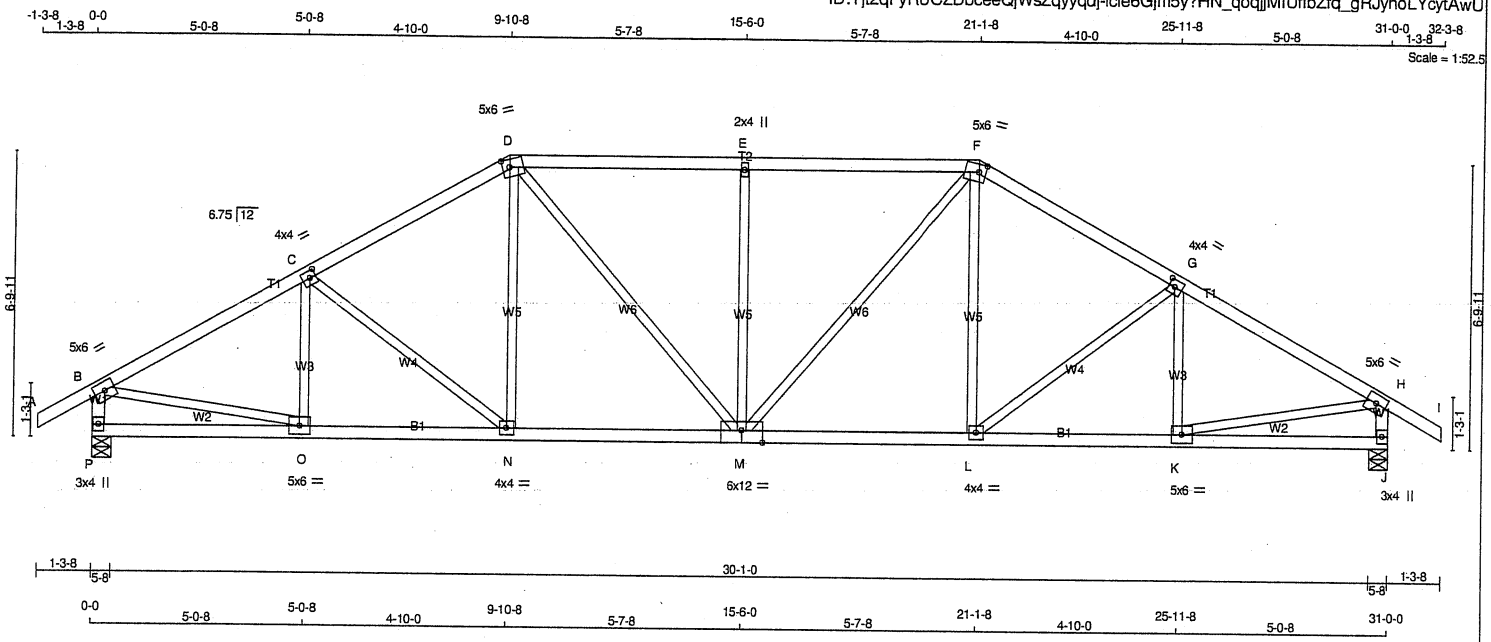

TOTAL WEIGHT = 2 X 131 = 262 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BSWWW-t	MT20	6.0	12.0	Edge	6.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1718	0	1718	0	0	5-8	5-8
J	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B- C	-2110 / 0	-84.9	-84.9	0.32 (1)	4.39
C- D	-1933 / 0	-84.9	-84.9	0.31 (1)	4.56
D- E	-1954 / 0	-84.9	-84.9	0.40 (1)	4.42
E- F	-1954 / 0	-84.9	-84.9	0.40 (1)	4.42
F- G	-1933 / 0	-84.9	-84.9	0.31 (1)	4.56
G- H	-2110 / 0	-84.9	-84.9	0.32 (1)	4.39
H- I	0 / 28	-84.9	-84.9	0.11 (1)	10.00
P- B	-1676 / 0	0.0	0.0	0.17 (1)	6.41
J- H	-1676 / 0	0.0	0.0	0.17 (1)	6.41
P- O	0 / 0	-18.5	-18.5	0.10 (4)	10.00
O- N	0 / 1859	-18.5	-18.5	0.37 (1)	10.00
N- M	0 / 1666	-18.5	-18.5	0.34 (1)	10.00
M- L	0 / 1666	-18.5	-18.5	0.34 (1)	10.00
L- K	0 / 1859	-18.5	-18.5	0.37 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-257 / 0	0.07 (1)
C- N	-249 / 0	0.17 (1)
N- D	0 / 264	0.06 (1)
D- M	0 / 441	0.10 (1)
M- E	-584 / 0	0.45 (1)
M- F	0 / 441	0.10 (1)
L- F	0 / 264	0.06 (1)
L- G	-249 / 0	0.17 (1)
K- G	-257 / 0	0.07 (1)
B- O	0 / 1891	0.43 (1)
K- H	0 / 1891	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.40/1.00 (E-F:1) , BC=0.37/1.00 (K-L:1) , WB=0.45/1.00 (E-M:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (O) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125180

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420229	T22S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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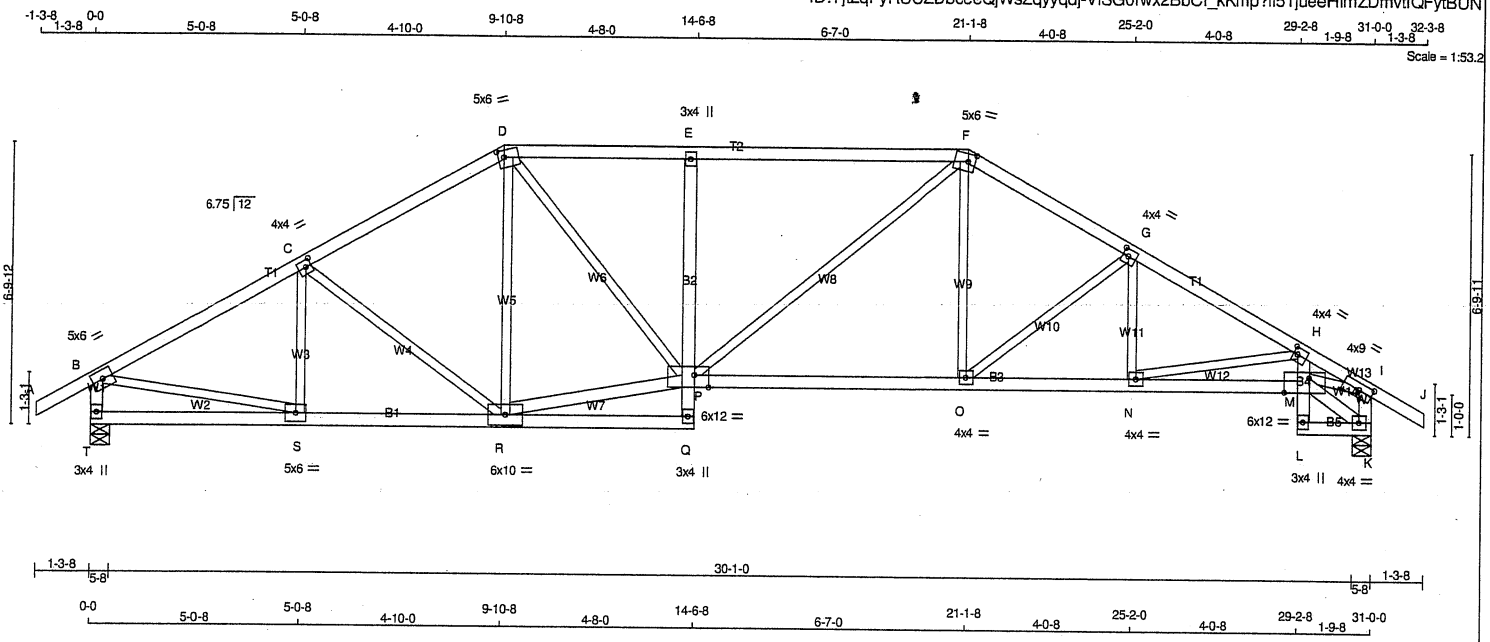


TOTAL WEIGHT = 139 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

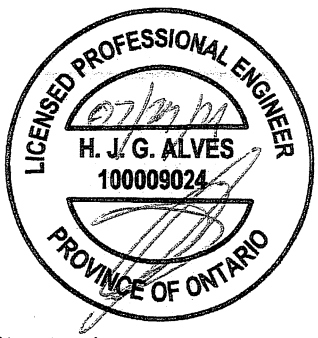
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW-t	MT20	4.0	9.0	2.00	4.00
K	BMVW1-t	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-I	MT20	6.0	12.0	4.25	7.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-I	MT20	6.0	12.0	Edge	4.25
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	6.0	10.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

~~NOTES~~ (1)
1)



Structural component only
DWG# T-2125195

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1715	0	1715	0	0	5-8	5-8
K	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1215	784 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
K	1219	787 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)		10.00
B- C	-2107 / 0	-84.9	-84.9	0.32 (1)		4.39
C- D	-1930 / 0	-84.9	-84.9	0.31 (1)		4.56
D- E	-2289 / 0	-84.9	-84.9	0.44 (1)		4.06
E- F	-2296 / 0	-84.9	-84.9	0.55 (1)		3.96
F- G	-2266 / 0	-84.9	-84.9	0.26 (1)		4.33
G- H	-2713 / 0	-84.9	-84.9	0.31 (1)		3.96
H- I	-3395 / 0	-84.9	-84.9	0.20 (1)		3.67
I- J	0 / 28	-84.9	-84.9	0.11 (1)		10.00
T- B	-1673 / 0	0.0	0.0	0.17 (1)		6.42
K- I	-1567 / 0	0.0	0.0	0.16 (1)		6.58
T- S	0 / 0	-18.5	-18.5	0.10 (4)		10.00
S- R	0 / 1857	-18.5	-18.5	0.36 (1)		10.00
R- Q	0 / 41	-18.5	-18.5	0.10 (4)		10.00
Q- P	0 / 37	0.0	0.0	0.06 (1)		10.00
P- E	-600 / 0	0.0	0.0	0.24 (1)		7.81
P- O	0 / 1962	-18.5	-18.5	0.41 (1)		10.00
O- N	0 / 2391	-18.5	-18.5	0.48 (1)		10.00
N- M	0 / 3233	-18.5	-18.5	0.57 (1)		10.00
L- M	0 / 17	0.0	0.0	0.31 (1)		10.00
M- H	0 / 381	0.0	0.0	0.37 (1)		10.00
L- K	0 / 199	-18.5	-18.5	0.04 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-255 / 0	0.07 (1)
C- R	-250 / 0	0.17 (1)
R- D	-105 / 26	0.08 (1)
R- P	0 / 1657	0.27 (1)
D- P	0 / 993	0.22 (1)
P- F	0 / 432	0.10 (1)
O- F	0 / 467	0.11 (1)
O- G	-555 / 0	0.25 (1)
N- G	0 / 125	0.03 (4)
N- H	-858 / 0	0.26 (1)
B- S	0 / 1889	0.43 (1)
M- K	-242 / 0	0.03 (1)
M- I	0 / 2940	0.66 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.55/1.00 (E-F:1) , BC=0.57/1.00 (M-N:1) , WB=0.66/1.00 (I-M:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.55 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T23	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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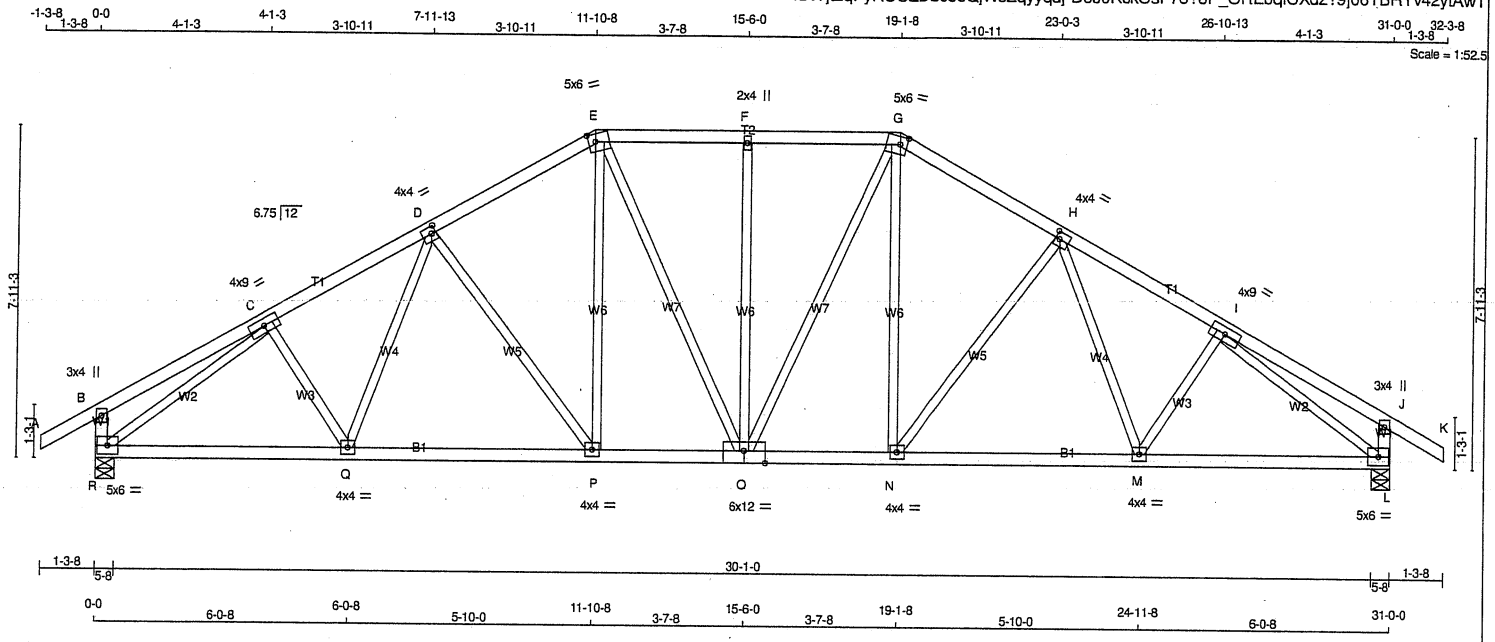

TOTAL WEIGHT = 2 X 144 = 288 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	9.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMWW-t	MT20	4.0	9.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M, N, P, Q						
M	BMWW-t	MT20	4.0	4.0		
O	BSWWW-l	MT20	6.0	12.0	Edge	6.00
R	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1718	0	1718	0	0	5-8	5-8
L	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 13	-84.9	-84.9	0.16 (1)	10.00
C- D	-2058 / 0	-84.9	-84.9	0.19 (1)	4.58
D- E	-1782 / 0	-84.9	-84.9	0.18 (1)	4.86
E- F	-1641 / 0	-84.9	-84.9	0.16 (1)	5.04
F- G	-1641 / 0	-84.9	-84.9	0.16 (1)	5.04
G- H	-1782 / 0	-84.9	-84.9	0.18 (1)	4.86
H- I	-2058 / 0	-84.9	-84.9	0.19 (1)	4.58
I- J	0 / 13	-84.9	-84.9	0.16 (1)	10.00
J- K	0 / 28	-84.9	-84.9	0.11 (1)	10.00
R- B	-258 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-258 / 0	0.0	0.0	0.03 (1)	7.81
R- Q	0 / 1769	-18.5	-18.5	0.37 (1)	10.00
Q- P	0 / 1767	-18.5	-18.5	0.36 (1)	10.00
P- O	0 / 1544	-18.5	-18.5	0.30 (1)	10.00
O- N	0 / 1544	-18.5	-18.5	0.30 (1)	10.00
N- M	0 / 1767	-18.5	-18.5	0.36 (1)	10.00
M- L	0 / 1769	-18.5	-18.5	0.37 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	0 / 69	0.02 (4)
Q- D	0 / 89	0.03 (4)
D- P	-386 / 0	0.33 (1)
P- E	0 / 407	0.09 (1)
E- O	0 / 226	0.05 (1)
O- F	-366 / 0	0.44 (1)
O- G	0 / 226	0.05 (1)
N- G	0 / 407	0.09 (1)
N- H	-386 / 0	0.33 (1)
H- M	0 / 89	0.03 (4)
M- I	0 / 69	0.02 (4)
R- C	-2264 / 0	0.94 (1)
I- L	-2264 / 0	0.94 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.19/1.00 (C-D:1) , BC=0.37/1.00 (Q-R:1) , WB=0.94/1.00 (C-R:1) , SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.40 (I) (INPUT = 1.00)

Structural component only
DWG# T-2125181

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420229	T23S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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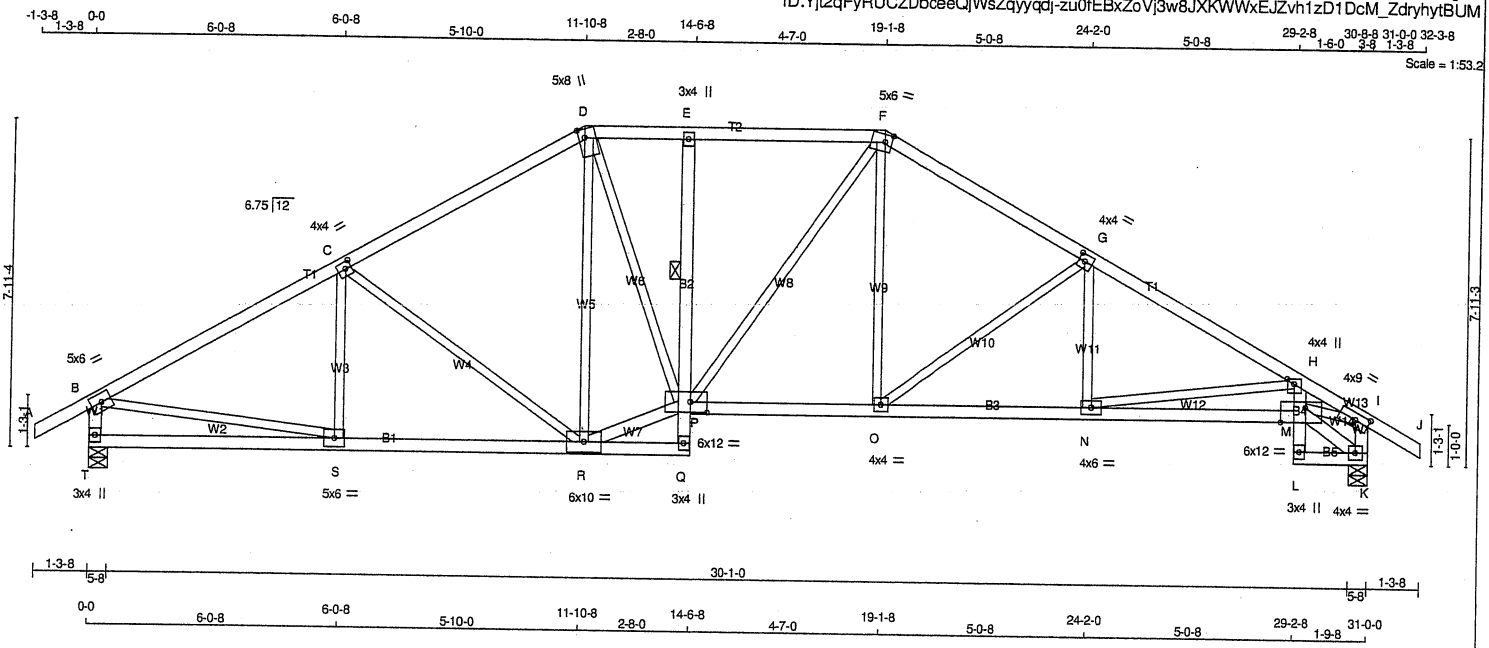

TOTAL WEIGHT = 145 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW+m	MT20	5.0	8.0	2.50	1.75
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW+p	MT20	4.0	4.0	1.50	2.00
I	TMVW-t	MT20	4.0	9.0	2.00	4.00
K	BMVW1-t	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-l	MT20	6.0	12.0	4.25	7.25
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-l	MT20	6.0	12.0	3.00	4.75
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	6.0	10.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125196

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1715	0	1715	0	0	5-8	5-8
K	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1215	784 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
K	1219	787 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)		10.00
B- C	-2134 / 0	-84.9	-84.9	0.48 (1)		4.19
C- D	-1794 / 0	-84.9	-84.9	0.44 (1)		4.53
D- E	-1873 / 0	-84.9	-84.9	0.20 (1)		4.73
E- F	-1877 / 0	-84.9	-84.9	0.26 (1)		4.66
F- G	-2049 / 0	-84.9	-84.9	0.35 (1)		4.40
G- H	-2632 / 0	-84.9	-84.9	0.41 (1)		3.92
H- I	-3447 / 0	-84.9	-84.9	0.20 (1)		3.64
I- J	0 / 28	-84.9	-84.9	0.11 (1)		10.00
T- B	-1668 / 0	0.0	0.0	0.17 (1)		6.42
K- I	-1565 / 0	0.0	0.0	0.16 (1)		6.59
T- S	0 / 0	-18.5	-18.5	0.16 (4)		10.00
S- R	0 / 1886	-18.5	-18.5	0.37 (1)		10.00
R- Q	0 / 29	-18.5	-18.5	0.10 (4)		10.00
Q- P	0 / 6	0.0	0.0	0.04 (1)		10.00
P- E	-389 / 0	0.0	0.0	0.05 (1)		6.25
P- O	0 / 1769	-18.5	-18.5	0.34 (1)		10.00
O- N	0 / 2319	-18.5	-18.5	0.43 (1)		10.00
N- M	0 / 3294	-18.5	-18.5	0.59 (1)		10.00
L- M	0 / 17	0.0	0.0	0.32 (1)		10.00
M- H	0 / 369	0.0	0.0	0.37 (1)		10.00
L- K	0 / 202	-18.5	-18.5	0.04 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-175 / 35	0.06 (1)
R- C	-433 / 0	0.46 (1)
R- D	-268 / 0	0.32 (1)
R- P	0 / 1627	0.26 (1)
D- P	0 / 957	0.22 (1)
P- F	0 / 186	0.04 (1)
O- F	0 / 510	0.11 (1)
O- G	-692 / 0	0.51 (1)
N- G	0 / 174	0.05 (4)
N- H	-988 / 0	0.46 (1)
B- S	0 / 1908	0.43 (1)
M- K	-246 / 0	0.03 (1)
M- I	0 / 2997	0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.59/1.00 (M-N:1) , WB=0.67/1.00 (I-M:1) , SSI=0.25/1.00 (H-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.56 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T24	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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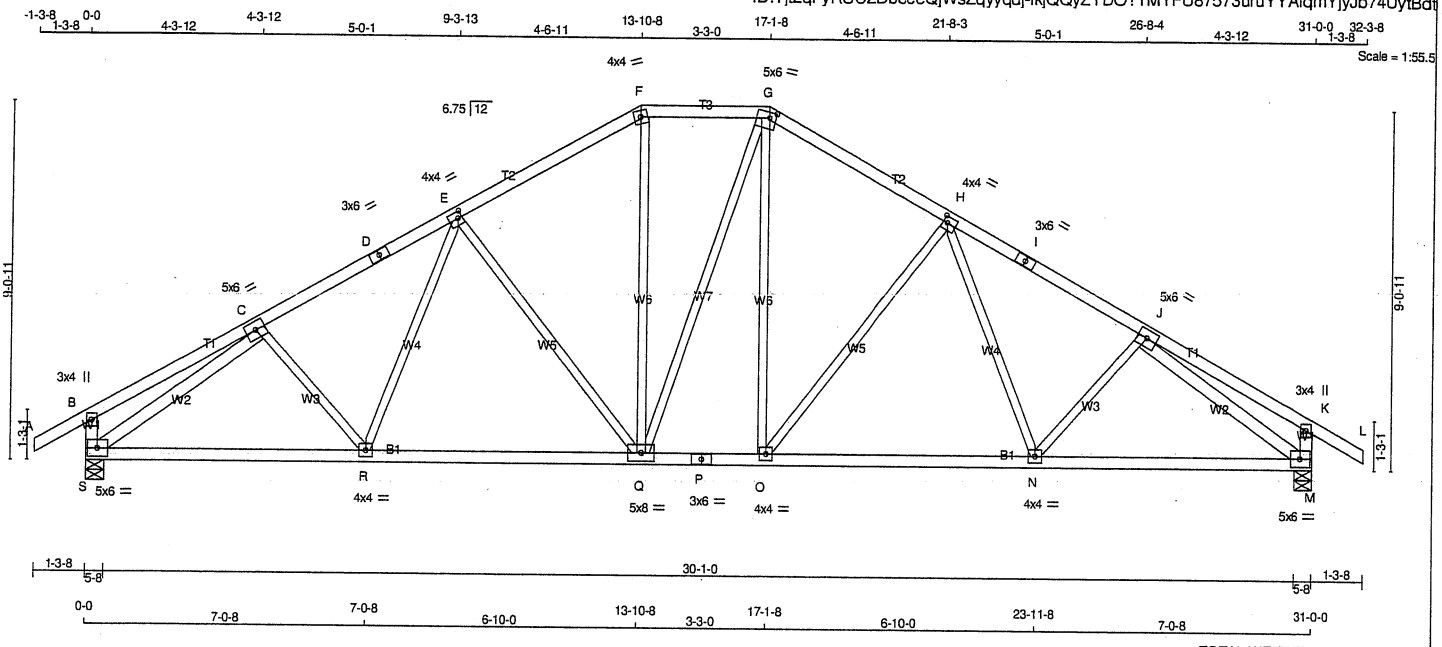

TOTAL WEIGHT = 2 X 142 = 285 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TTW-m	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	2.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	5.0	6.0		
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		
N, O, R						
N	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
S	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1718	0	1718	0	0	5-8	5-8
M	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
M	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	C- R	-22 / 52	0.02 (4)
B- C	0 / 19	-84.9	-84.9	0.22 (1)	10.00	R- E	0 / 174	0.05 (4)
C- D	-2035 / 0	-84.9	-84.9	0.28 (1)	4.52	E- Q	-522 / 0	0.64 (1)
D- E	-2035 / 0	-84.9	-84.9	0.28 (1)	4.52	Q- F	0 / 504	0.11 (1)
E- F	-1632 / 0	-84.9	-84.9	0.25 (1)	4.95	Q- G	0 / 4	0.00 (1)
F- G	-1408 / 0	-84.9	-84.9	0.14 (1)	5.36	O- G	0 / 500	0.11 (1)
G- H	-1630 / 0	-84.9	-84.9	0.25 (1)	4.95	O- H	-524 / 0	0.64 (1)
H- I	-2035 / 0	-84.9	-84.9	0.28 (1)	4.51	H- N	0 / 176	0.05 (4)
I- J	-2035 / 0	-84.9	-84.9	0.28 (1)	4.51	N- J	-22 / 52	0.02 (4)
J- K	0 / 19	-84.9	-84.9	0.22 (1)	10.00	S- C	-2279 / 0	0.76 (1)
K- L	0 / 28	-84.9	-84.9	0.11 (1)	10.00	J- M	-2279 / 0	0.76 (1)
S- B	-256 / 0	0.0	0.0	0.03 (1)	7.81			
M- K	-256 / 0	0.0	0.0	0.03 (1)	7.81			
S- R	0 / 1791	-18.5	-18.5	0.40 (1)	10.00			
R- Q	0 / 1716	-18.5	-18.5	0.39 (1)	10.00			
Q- P	0 / 1407	-18.5	-18.5	0.29 (1)	10.00			
P- O	0 / 1407	-18.5	-18.5	0.29 (1)	10.00			
O- N	0 / 1716	-18.5	-18.5	0.39 (1)	10.00			
N- M	0 / 1791	-18.5	-18.5	0.41 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.28/1.00 (H-J:1) , BC=0.41/1.00 (M-N:1) , WB=0.76/1.00 (J-M:1) , SSI=0.16/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (J) (INPUT = 0.90)
JSI METAL= 0.51 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125182

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T25	10	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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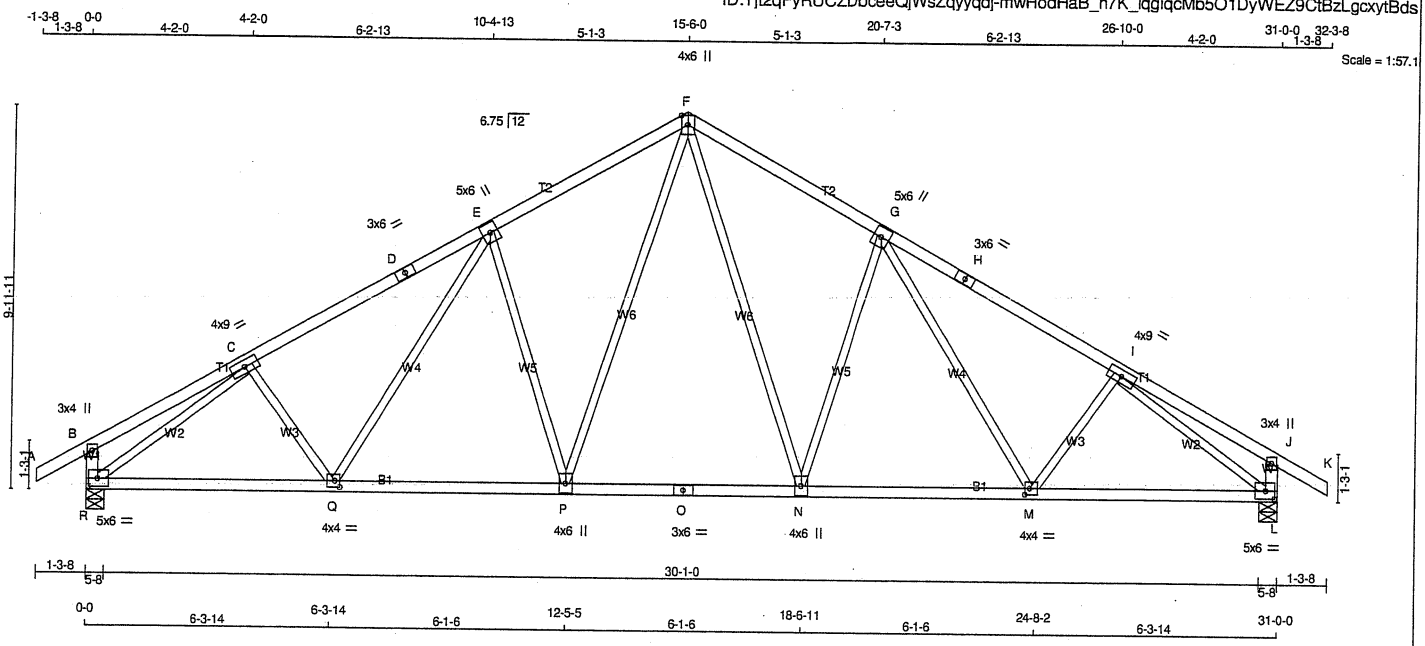

TOTAL WEIGHT = 10 X 135 = 1349 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	9.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	5.0	6.0		
F	TTWW+p	MT20	4.0	6.0	Edge	
G	TMWW+t	MT20	5.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	9.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0	2.00	1.50
R	BMVW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1718	0	1718	0	0	5-8	5-8
L	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	F- N	0 / 716	0.16 (1)
B- C	0 / 26	-84.9	-84.9	0.30 (1)	10.00	N- G	-596 / 0	0.58 (1)
C- D	-2024 / 0	-84.9	-84.9	0.41 (1)	4.39	G- M	0 / 197	0.04 (1)
D- E	-2024 / 0	-84.9	-84.9	0.41 (1)	4.39	M- I	-57 / 46	0.02 (4)
E- F	-1746 / 0	-84.9	-84.9	0.36 (1)	4.69	P- F	0 / 716	0.16 (1)
F- G	-1746 / 0	-84.9	-84.9	0.36 (1)	4.69	E- P	-596 / 0	0.58 (1)
G- H	-2024 / 0	-84.9	-84.9	0.41 (1)	4.39	Q- E	0 / 197	0.04 (1)
H- I	-2024 / 0	-84.9	-84.9	0.41 (1)	4.39	C- Q	-57 / 46	0.02 (4)
I- J	0 / 26	-84.9	-84.9	0.30 (1)	10.00	R- C	-2302 / 0	0.99 (1)
J- K	0 / 28	-84.9	-84.9	0.11 (1)	10.00	I- L	-2302 / 0	0.99 (1)
R- B	-235 / 0	0.0	0.0	0.02 (1)	7.81			
L- J	-235 / 0	0.0	0.0	0.02 (1)	7.81			
R- Q	0 / 1802	-18.5	-18.5	0.38 (1)	10.00			
Q- P	0 / 1669	-18.5	-18.5	0.34 (1)	10.00			
P- O	0 / 1282	-18.5	-18.5	0.28 (1)	10.00			
O- N	0 / 1282	-18.5	-18.5	0.28 (1)	10.00			
N- M	0 / 1669	-18.5	-18.5	0.34 (1)	10.00			
M- L	0 / 1802	-18.5	-18.5	0.38 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.41/1.00 (C-E:1) , BC=0.38/1.00 (Q-R:1) , WB=0.99/1.00 (C-R:1) , SSI=0.19/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.45 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125183

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T26	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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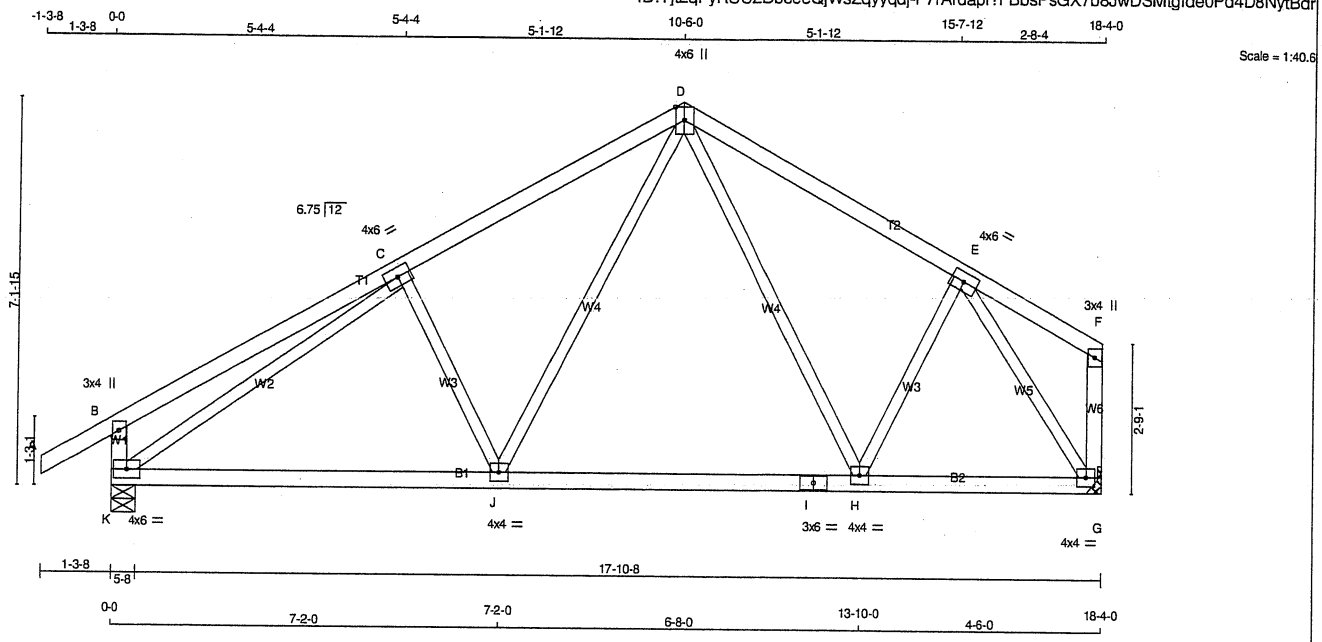

TOTAL WEIGHT = 78 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1063	0	1063	0	0	5-8	5-8
G	947	0	947	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	752	490 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
G	673	427 / 0	0 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 26	-84.9	-84.9	0.38 (1)	10.00
C- D	-978 / 0	-84.9	-84.9	0.31 (1)	5.92
D- E	-741 / 0	-84.9	-84.9	0.27 (1)	6.25
E- F	0 / 38	-84.9	-84.9	0.28 (1)	10.00
K- B	-285 / 0	0.0	0.0	0.03 (1)	7.81
G- F	-33 / 0	0.0	0.0	0.00 (1)	7.81
K- J	0 / 975	-18.5	-18.5	0.31 (4)	10.00
J- I	0 / 616	-18.5	-18.5	0.28 (4)	10.00
I- H	0 / 616	-18.5	-18.5	0.28 (4)	10.00
H- G	0 / 591	-18.5	-18.5	0.17 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-328 / 0	0.10 (1)
J- D	0 / 490	0.11 (1)
D- H	0 / 46	0.02 (4)
H- E	0 / 88	0.03 (4)
K- C	-1215 / 0	0.91 (1)
E- G	-1063 / 0	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.31/1.00 (J-K:4) , WB=0.91/1.00 (C-K:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (G) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)

Structural component only
DWG# T-2125184

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T27	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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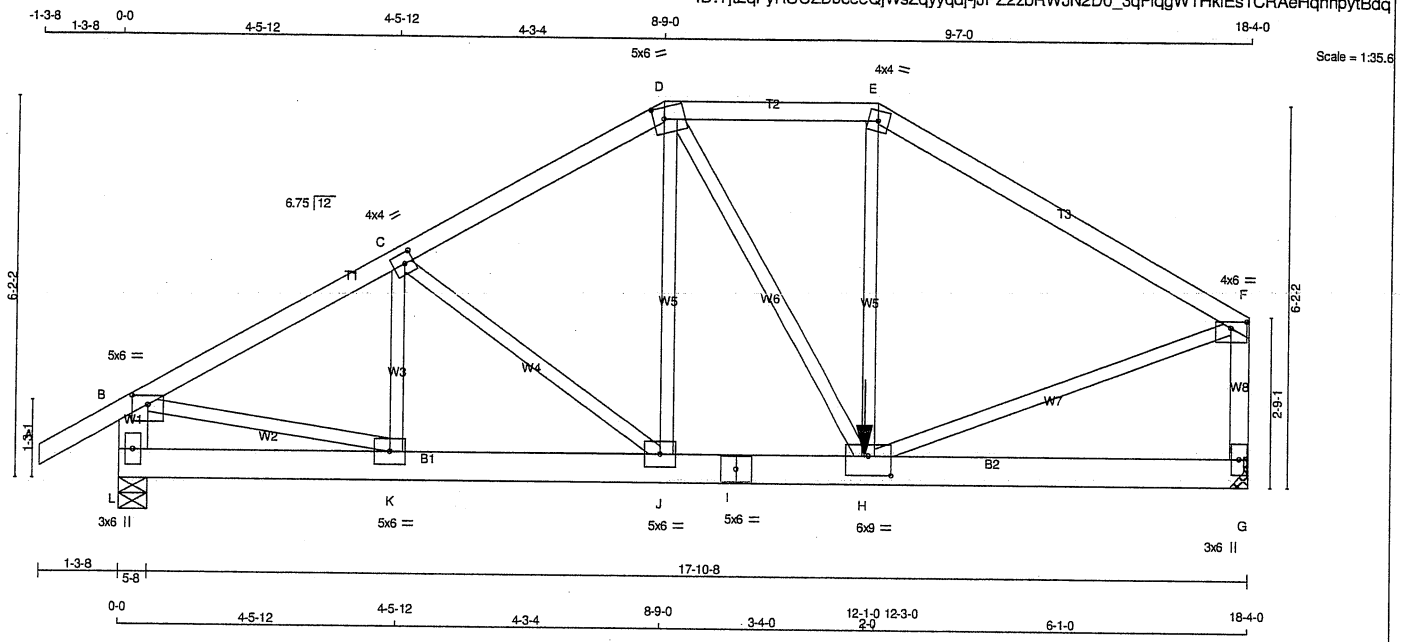

TOTAL WEIGHT = 91 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TTW-m	MT20	4.0	4.0		
F	TMVW-p	MT20	4.0	6.0	1.25	3.00
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	6.0	9.0	3.75	4.50
I	BS-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1519	0	1519	0	0	5-8	5-8
G	1843	0	1843	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1076	696 / 0	0 / 0	0 / 0	0 / 0	380 / 0	0 / 0
G	1308	831 / 0	0 / 0	0 / 0	0 / 0	477 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 28	-84.9	-84.9	0.12 (1)	10.00	K- C	-248 / 0	0.06 (1)
B- C	-1814 / 0	-84.9	-84.9	0.36 (1)	4.56	C- J	-200 / 0	0.10 (1)
C- D	-1675 / 0	-84.9	-84.9	0.35 (1)	4.72	J- D	0 / 221	0.05 (1)
D- E	-1623 / 0	-84.9	-84.9	0.24 (1)	4.91	D- H	0 / 355	0.09 (1)
E- F	-1854 / 0	-84.9	-84.9	0.79 (1)	3.89	H- E	0 / 504	0.12 (1)
L- B	-1470 / 0	0.0	0.0	0.10 (1)	7.81	B- K	0 / 1628	0.40 (1)
G- F	-1784 / 0	0.0	0.0	0.26 (1)	6.21	H- F	0 / 1728	0.43 (1)
L- K	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
K- J	0 / 1598	-18.5	-18.5	0.25 (1)	10.00			
J- I	0 / 1444	-18.5	-18.5	0.21 (1)	10.00			
I- H	0 / 1444	-18.5	-18.5	0.21 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	12-1-0	-959	-959	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.79/1.00 (E-F:1) , BC=0.25/1.00 (J-K:1) , WB=0.43/1.00 (F-H:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.38 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125185

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T28	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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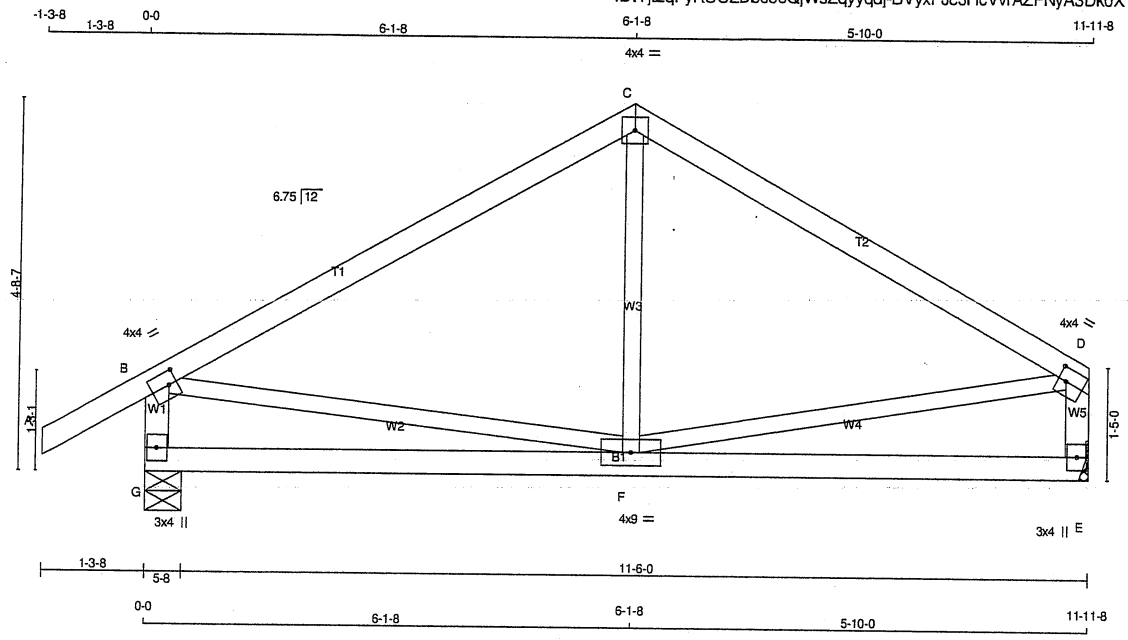


TOTAL WEIGHT = 2 X 46 = 92 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	734	0	734	0	0	5-8	5-8
E	618	0	618	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	519	342 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
E	439	278 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	F- C	-14 / 94	0.03 (4)
B- C	-503 / 0	-84.9	-84.9	0.41 (1)	6.25	B- F	0 / 443	0.10 (1)
C- D	-503 / 0	-84.9	-84.9	0.37 (1)	6.25	F- D	0 / 446	0.10 (1)
G- B	-689 / 0	0.0	0.0	0.07 (1)	7.81			
E- D	-577 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.19/1.00 (F-G:4) , WB=0.10/1.00 (D-F:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

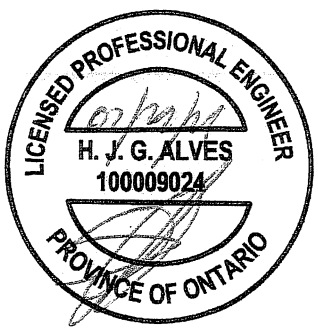
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125186

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T28C	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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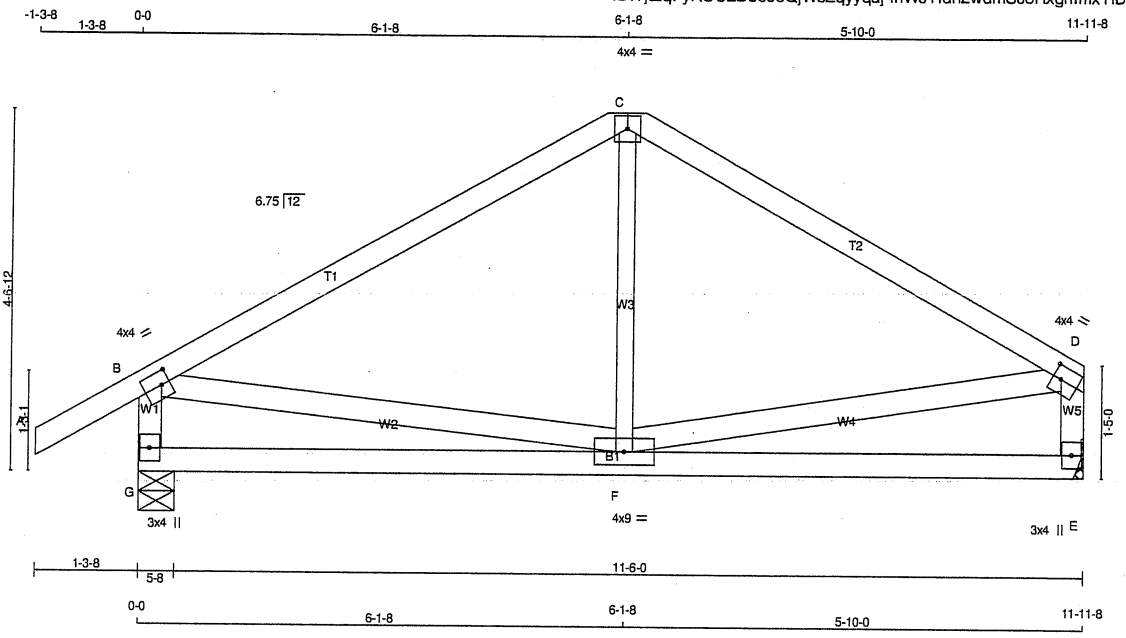

TOTAL WEIGHT = 49 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
F - C	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	734	0	734	0	0	5-8	5-8
E	618	0	618	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	519	342 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
E	439	278 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	F- C	-14 / 95	0.03 (4)
B- C	-503 / 0	-84.9	-84.9	0.41 (1)	6.25	B- F	0 / 444	0.07 (1)
C- D	-503 / 0	-84.9	-84.9	0.37 (1)	6.25	F- D	0 / 446	0.07 (1)
G- B	-690 / 0	0.0	0.0	0.07 (1)	7.81			
E- D	-577 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.19/1.00 (E-F:4) , WB=0.07/1.00 (D-F:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (D) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125187

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T30	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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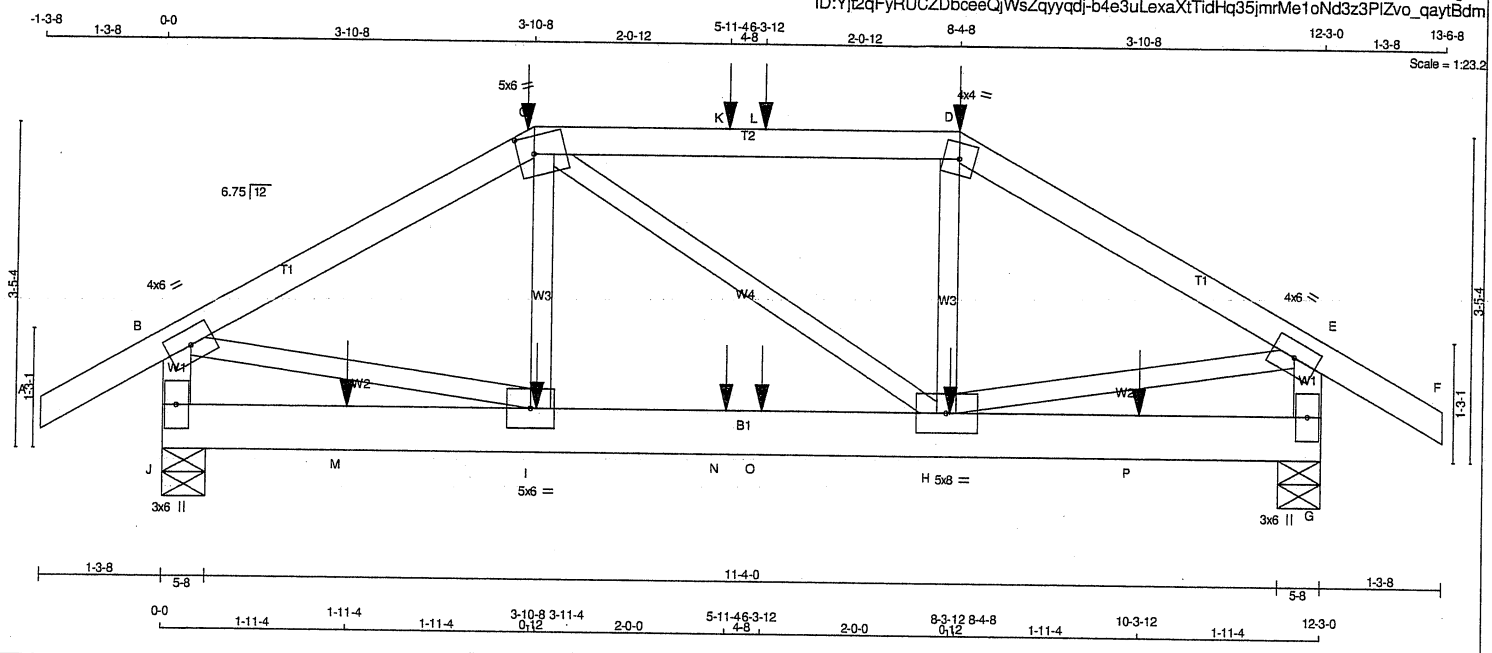

TOTAL WEIGHT = 57 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1051	0	1051	0	0	5-8	5-8
G	1051	0	1051	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	743	490 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
G	743	490 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.12 (1)	10.00	I- C	-95 / 55	0.02 (4)
B- C	-1075 / 0	-84.9	-84.9	0.27 (1)	5.72	C- H	0 / 3	0.00 (4)
C- K	-932 / 0	-84.9	-84.9	0.47 (1)	5.65	H- D	-94 / 57	0.02 (4)
K- L	-932 / 0	-84.9	-84.9	0.47 (1)	5.65	B- I	0 / 958	0.24 (1)
L- D	-932 / 0	-84.9	-84.9	0.47 (1)	5.65	H- E	0 / 959	0.24 (1)
D- E	-1078 / 0	-84.9	-84.9	0.27 (1)	5.71			
E- F	0 / 28	-84.9	-84.9	0.12 (1)	10.00			
J- B	-1011 / 0	0.0	0.0	0.11 (1)	7.75			
G- E	-1008 / 0	0.0	0.0	0.11 (1)	7.77			
J- M	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
M- I	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
I- N	0 / 933	-18.5	-18.5	0.17 (1)	10.00			
N- O	0 / 933	-18.5	-18.5	0.17 (1)	10.00			
O- H	0 / 933	-18.5	-18.5	0.17 (1)	10.00			
H- P	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
P- G	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-160	-160	---	BACK	VERT	TOTAL	---	C1
D	8-4-8	-160	-160	---	BACK	VERT	TOTAL	---	C1
H	8-3-12	-10	-10	---	BACK	VERT	TOTAL	---	C1
I	3-11-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
K	5-11-4	-27	-27	---	BACK	VERT	TOTAL	---	C1
L	6-3-12	-27	-27	---	BACK	VERT	TOTAL	---	C1
M	1-11-4	-8	-8	---	BACK	VERT	TOTAL	---	C1
N	5-11-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
O	6-3-12	-10	-10	---	BACK	VERT	TOTAL	---	C1
P	10-3-12	-8	-8	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.47/1.00 (C-D:1) , BC=0.17/1.00 (H-I:1) , WB=0.24/1.00 (E-H:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (D) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125188

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T31	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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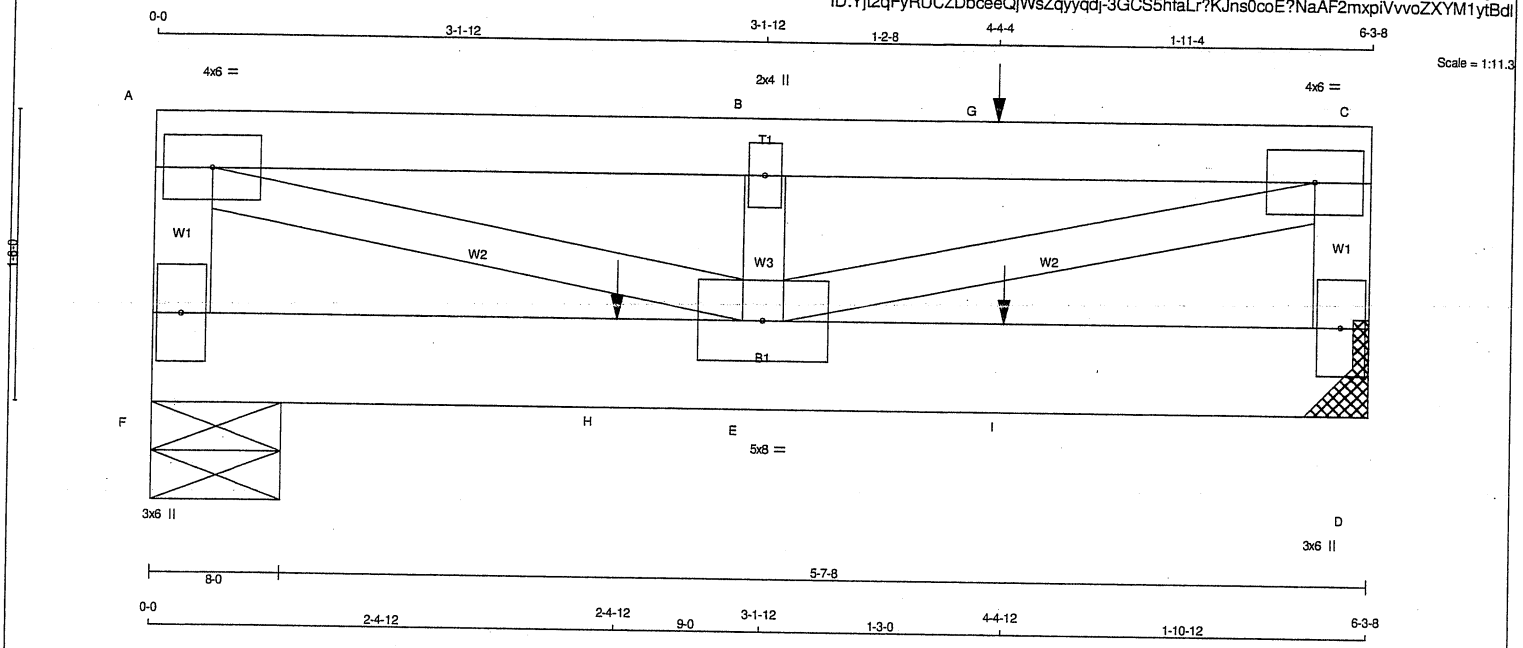

TOTAL WEIGHT = 2 X 25 = 51 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(14.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMWW-t	MT20	5.0	8.0		

Structural component only
DWG# T-2125189

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1094	0	1094	0	0	8-0	8-0
D	1370	0	1370	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	782	469 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
D	974	611 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-864 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 2307	0.29 (1)
A- B	-2172 / 0	-84.9	-84.9	0.13 (1)	5.90	E- B	-668 / 0	0.05 (1)
B- G	-2172 / 0	-84.9	-84.9	0.31 (1)	5.62	E- C	0 / 2307	0.29 (1)
G- C	-2172 / 0	-84.9	-84.9	0.31 (1)	5.62			
D- C	-1058 / 0	0.0	0.0	0.06 (1)	7.81			
F- H	0 / 0	-43.5	-43.5	0.16 (1)	10.00			
H- E	0 / 0	-43.5	-43.5	0.16 (1)	10.00			
E- I	0 / 0	-43.5	-43.5	0.20 (1)	10.00			
I- D	0 / 0	-18.5	-18.5	0.20 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	4-4-4	-356	-356	---	TOP	VERT	TOTAL	---	C1
H	2-4-12	-424	-424	---	BACK	VERT	TOTAL	---	C1
I	4-4-12	-424	-424	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.20/1.00 (D-E:1) , WB=0.29/1.00 (A-E:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (C) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420228	T31	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125189

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T32	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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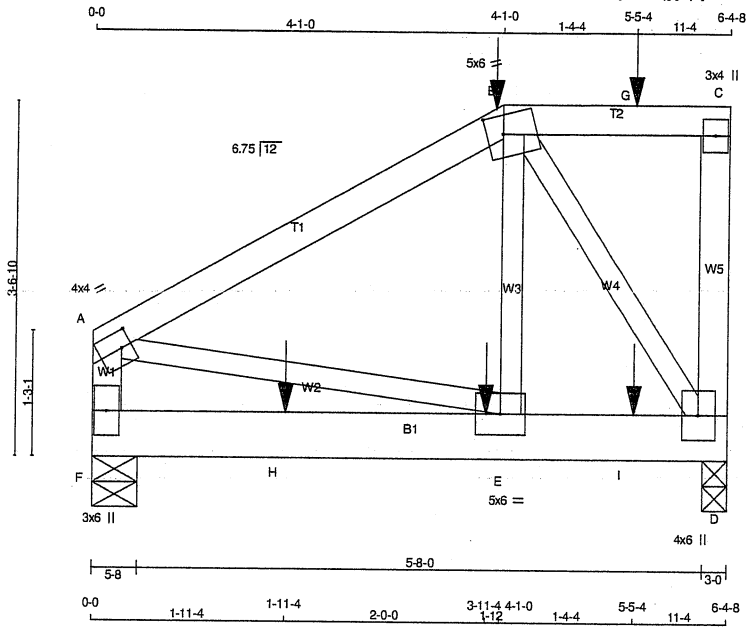

TOTAL WEIGHT = 32 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TTWW-m	MT20	5.0	6.0	2.25	2.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	588	0	588	0	0	3-0	3-0
F	459	0	459	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	415	276 / 0	0 / 0	0 / 0	0 / 0	139 / 0	0 / 0
F	325	210 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-359 / 0	-84.9	-84.9	0.27 (1)	6.25	E- B	0 / 93	0.04 (4)
B- G	0 / 0	-84.9	-84.9	0.12 (1)	10.00	B- D	-534 / 0	0.13 (1)
G- C	0 / 0	-84.9	-84.9	0.12 (1)	10.00	A- E	0 / 319	0.08 (1)
D- C	-126 / 0	0.0	0.0	0.02 (1)	7.81			
F- A	-414 / 0	0.0	0.0	0.05 (1)	7.81			
F- H	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
H- E	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
E- I	0 / 313	-18.5	-18.5	0.06 (1)	10.00			
I- D	0 / 313	-18.5	-18.5	0.06 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	4-1-0	-208	-208	---	BACK	VERT	TOTAL	---	C1
E	3-11-4	-11	-11	---	BACK	VERT	TOTAL	---	C1
G	5-5-4	-33	-33	---	BACK	VERT	TOTAL	---	C1
H	1-11-4	-9	-9	---	BACK	VERT	TOTAL	---	C1
I	5-5-4	-11	-11	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.27/1.00 (A-B:1) , BC=0.06/1.00 (E-F:4) , WB=0.13/1.00 (B-D:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (A) (INPUT = 0.90)
JSI METAL= 0.15 (A) (INPUT = 1.00)

Structural component only
DWG# T-2125190

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	T33	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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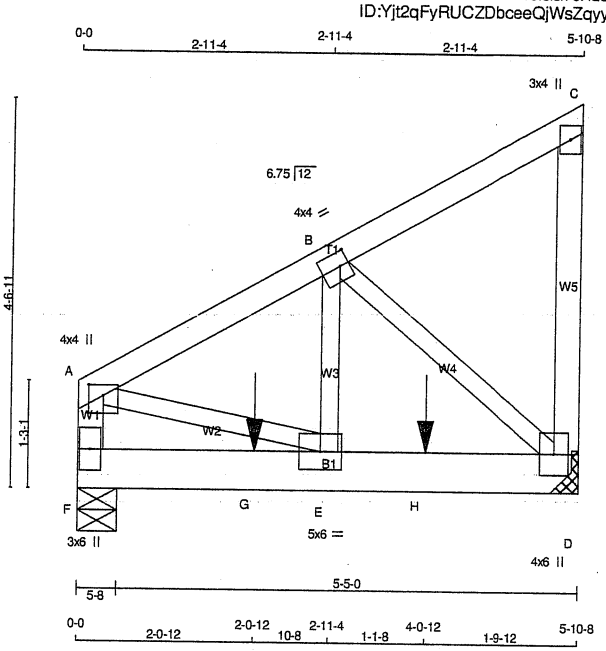

TOTAL WEIGHT = 2 X 30 = 60 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		

Structural component only
DWG# T-2125191

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1587	0	1587	0	0	MECHANICAL	
F	1774	0	1774	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1125	724 / 0	0 / 0	0 / 0	0 / 0	401 / 0	0 / 0
F	1258	808 / 0	0 / 0	0 / 0	0 / 0	450 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
A- B	-1708 / 0	-84.9	-84.9	0.06 (1)	6.25	E- B	0 / 1833	0.23 (1)
B- C	-10 / 0	-84.9	-84.9	0.05 (1)	10.00	B- D	-1970 / 0	0.25 (1)
D- C	-104 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 1560	0.19 (1)
F- A	-1384 / 0	0.0	0.0	0.08 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.28 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.28 (1)	10.00			
E- H	0 / 1497	-18.5	-18.5	0.29 (1)	10.00			
H- D	0 / 1497	-18.5	-18.5	0.29 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-1293	-1293	---	BACK	VERT	TOTAL	---	C1
H	4-0-12	-658	-658	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.08/1.00 (A-F:1) , BC=0.29/1.00 (D-E:1) , WB=0.25/1.00 (B-D:1) , SSI=0.49/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.34 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420228	T33	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:Yjt2qFyRUCZDbceeQjWsZqyyqdj-0fKCWMhqtSF2Z50PkDHTT?GeBacmAQzBFt0eQvytBdj

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125191

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420229	T34	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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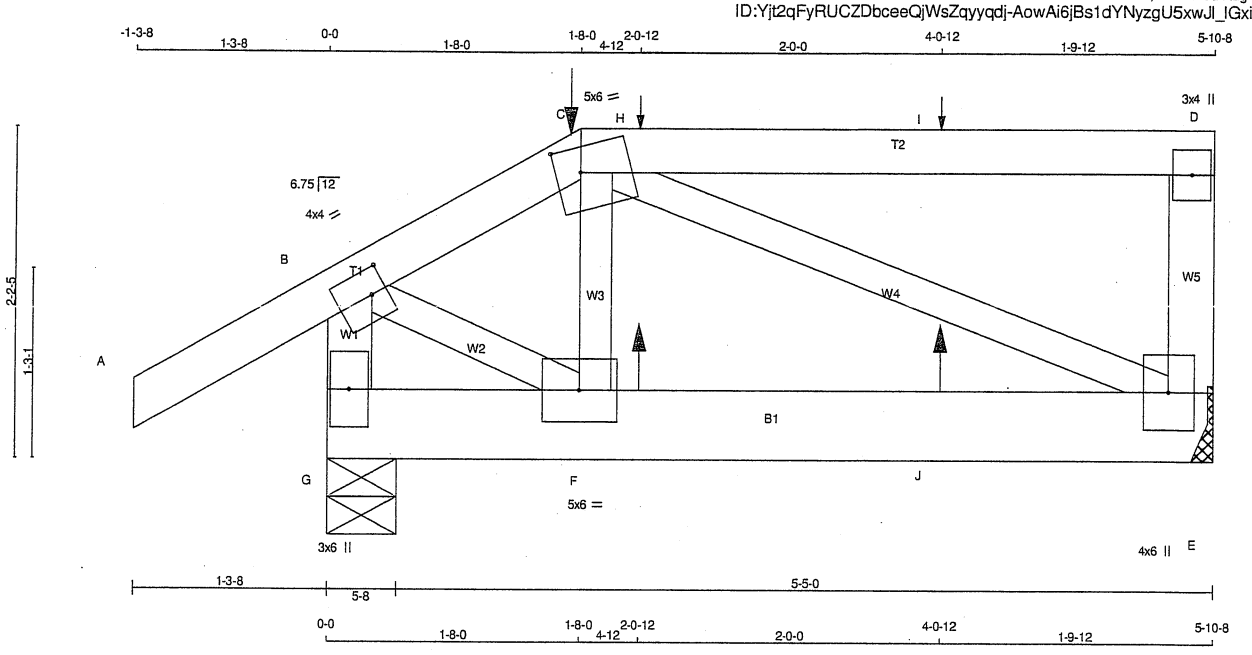

TOTAL WEIGHT = 27 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TMV+p	MT20	3.0	4.0		
E	BMVW1+p	MT20	4.0	6.0		
F	BMWW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	574	0	574	0	0	MECHANICAL	
G	715	0	715	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-12.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	403	284 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0
G	499	361 / 0	0 / 0	0 / 0	0 / 0	139 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.12 (1)	10.00	F- C	-186 / 9	0.03 (1)
B- C	-513 / 0	-84.9	-84.9	0.12 (1)	6.25	C- E	-451 / 0	0.14 (1)
C- H	0 / 0	-168.9	-168.9	0.56 (1)	10.00	B- F	0 / 473	0.12 (1)
H- I	0 / 0	-168.9	-168.9	0.56 (1)	10.00			
I- D	0 / 0	-168.9	-168.9	0.56 (1)	10.00			
E- D	-355 / 0	0.0	0.0	0.04 (1)	7.81			
G- B	-682 / 0	0.0	0.0	0.08 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.02 (1)	10.00			
F- J	0 / 415	-18.5	-18.5	0.09 (1)	10.00			
J- E	0 / 415	-18.5	-18.5	0.09 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-8-0	-3	-3	---	TOP	VERT	DEAD	---	C1
C	1-8-0	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	1-8-0	-9	-9	---	TOP	VERT	SNOW	---	C1
C	1-8-0	-115	-115	---	FRONT	VERT	SNOW	---	C1
F	2-0-12	5	1	8	FRONT	VERT	TOTAL	---	C1
H	2-0-12	1	1	51	FRONT	VERT	TOTAL	---	C1
I	4-0-12	1	1	65	FRONT	VERT	TOTAL	---	C1
J	4-0-12	5	1	8	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.56/1.00 (C-D:1) , BC=0.09/1.00 (E-F:1) , WB=0.14/1.00 (C-E:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2126374

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	PB1	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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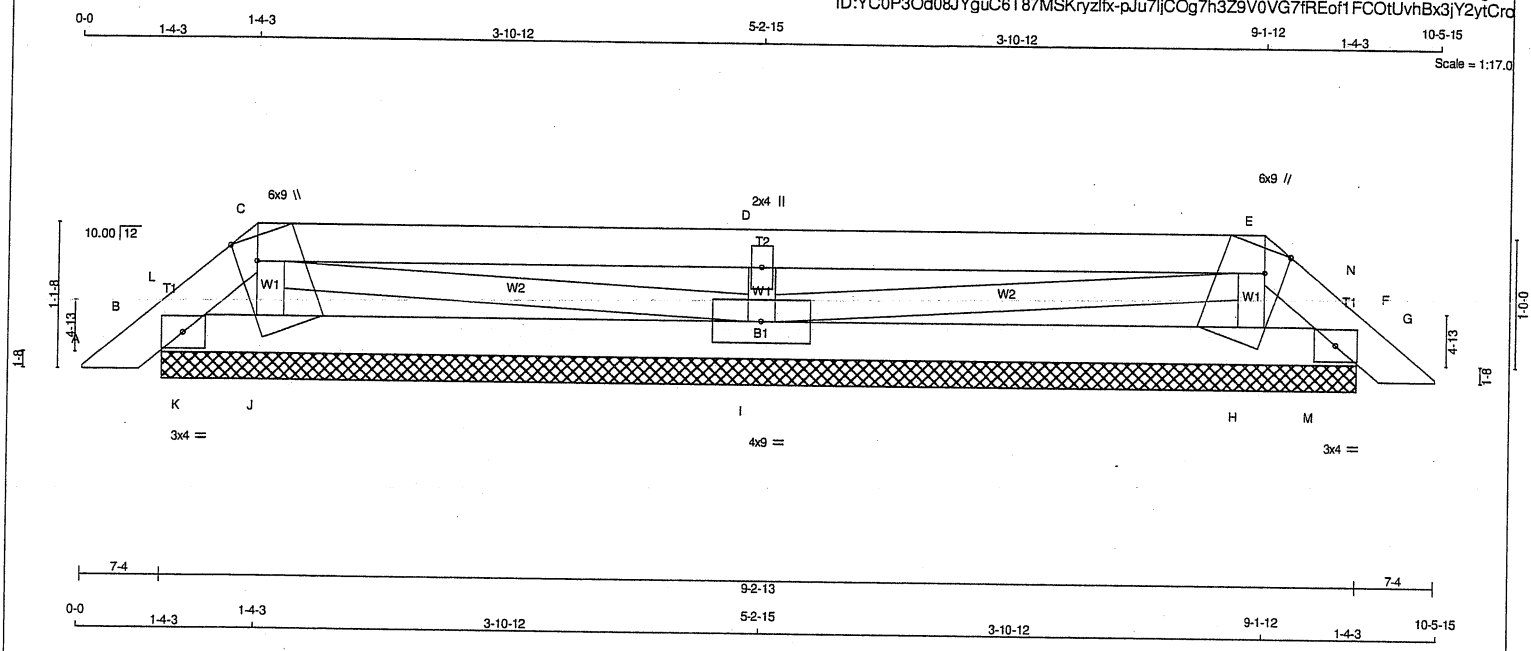

TOTAL WEIGHT = 2 X 30 = 61 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, E, H, J						
C						
C	TTBWW1"+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTBWW1"+m	MT20	6.0	9.0	Edge	1.75
F	TMB1-I	MT20	3.0	4.0		
I	BMWWW1-t	MT20	4.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	97	0	97	0	0	9-2-13	9-2-13
F	97	0	97	0	0	9-2-13	9-2-13
J	180	0	180	0	0	9-2-13	9-2-13
I	488	0	488	0	0	9-2-13	9-2-13
H	180	0	180	0	0	9-2-13	9-2-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	65	62 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
F	65	62 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
J	132	64 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
I	345	227 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0
H	132	64 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- L	-41 / 5	-84.9	-84.9	0.02 (1)	6.25
L- C	-39 / 0	-84.9	-84.9	0.01 (1)	6.25
C- D	0 / 0	-84.9	-84.9	0.22 (1)	10.00
D- E	0 / 0	-84.9	-84.9	0.22 (1)	10.00
E- N	-39 / 0	-84.9	-84.9	0.01 (1)	6.25
N- F	-41 / 5	-84.9	-84.9	0.02 (1)	6.25
F- G	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- K	0 / 21	-18.5	-18.5	0.00 (1)	10.00
K- J	0 / 21	-18.5	-18.5	0.04 (4)	10.00
J- I	0 / 4	-18.5	-18.5	0.06 (4)	10.00
I- H	0 / 4	-18.5	-18.5	0.06 (4)	10.00
H- M	0 / 21	-18.5	-18.5	0.04 (4)	10.00
M- F	0 / 21	-18.5	-18.5	0.00 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-110 / 0	0.02 (1)
C- I	-3 / 0	0.00 (1)
I- D	-413 / 0	0.06 (1)
I- E	-3 / 0	0.00 (1)
H- E	-110 / 0	0.02 (1)
K- L	-29 / 0	0.00 (1)
M- N	-29 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (D-E:1) , BC=0.06/1.00 (H-I:4) , WB=0.06/1.00 (D-I:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

Structural component only
DWG# T-2125143

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	PB2	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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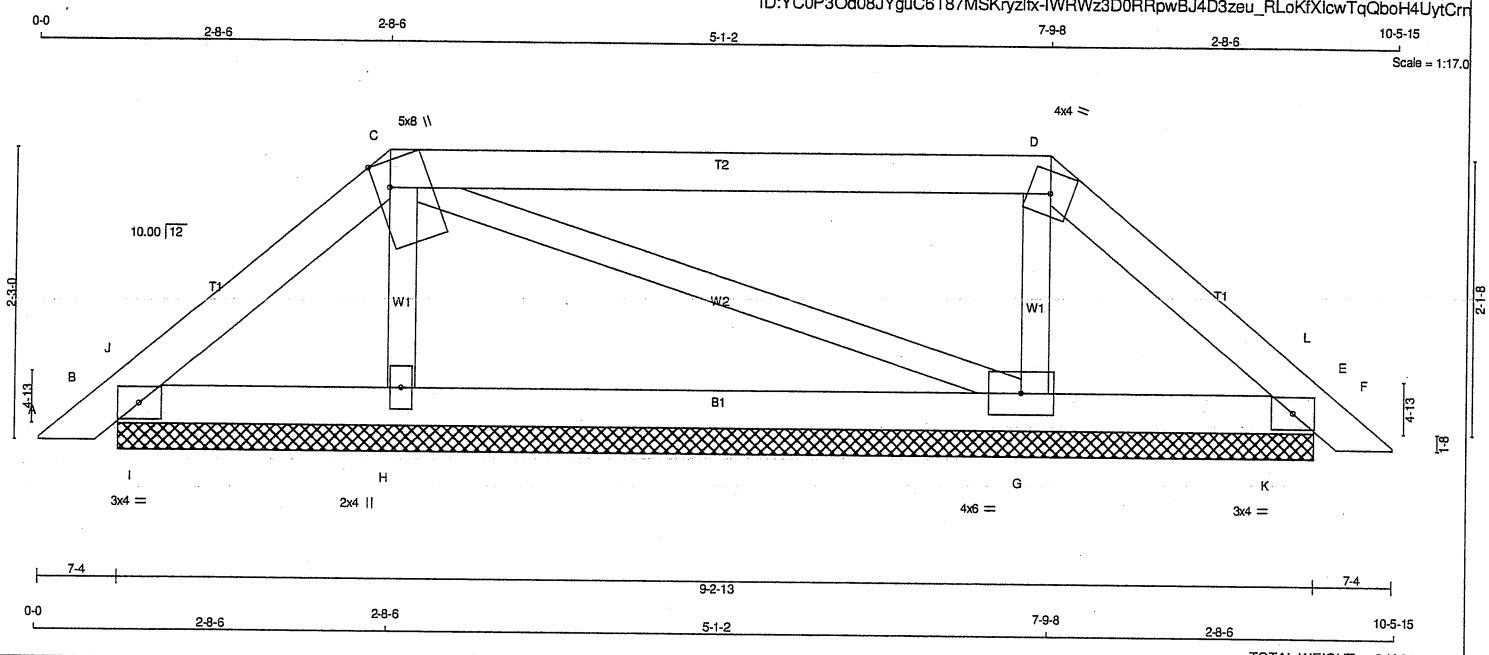

TOTAL WEIGHT = 2 X 31 = 62 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	8.0	Edge	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	194	0	194	0	0	9-2-13	9-2-13
E	183	0	183	0	0	9-2-13	9-2-13
H	323	0	323	0	0	9-2-13	9-2-13
G	344	0	344	0	0	9-2-13	9-2-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	134	104 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	127	98 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
H	232	133 / 0	0 / 0	0 / 0	0 / 0	99 / 0	0 / 0
G	246	144 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- J	-54 / 0	-84.9	-84.9	0.02 (1)	6.25
J- C	-90 / 0	-84.9	-84.9	0.04 (1)	6.25
C- D	-38 / 0	-84.9	-84.9	0.38 (1)	6.25
D- L	-73 / 0	-84.9	-84.9	0.04 (1)	6.25
L- E	-35 / 0	-84.9	-84.9	0.02 (1)	6.25
E- F	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- I	0 / 65	-18.5	-18.5	0.05 (1)	10.00
I- H	0 / 65	-18.5	-18.5	0.08 (4)	10.00
H- G	0 / 52	-18.5	-18.5	0.08 (4)	10.00
G- K	0 / 52	-18.5	-18.5	0.08 (4)	10.00
K- E	0 / 52	-18.5	-18.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-227 / 0	0.03 (1)
C- G	-15 / 0	0.01 (1)
G- D	-242 / 0	0.04 (1)
I- J	-137 / 0	0.00 (1)
K- L	-139 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.38/1.00 (C-D:1) , BC=0.08/1.00 (H-I:4) , WB=0.04/1.00 (D-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (D) (INPUT = 0.90)
JSI METAL= 0.05 (H) (INPUT = 1.00)

Structural component only
DWG# T-2125144

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420226	PB3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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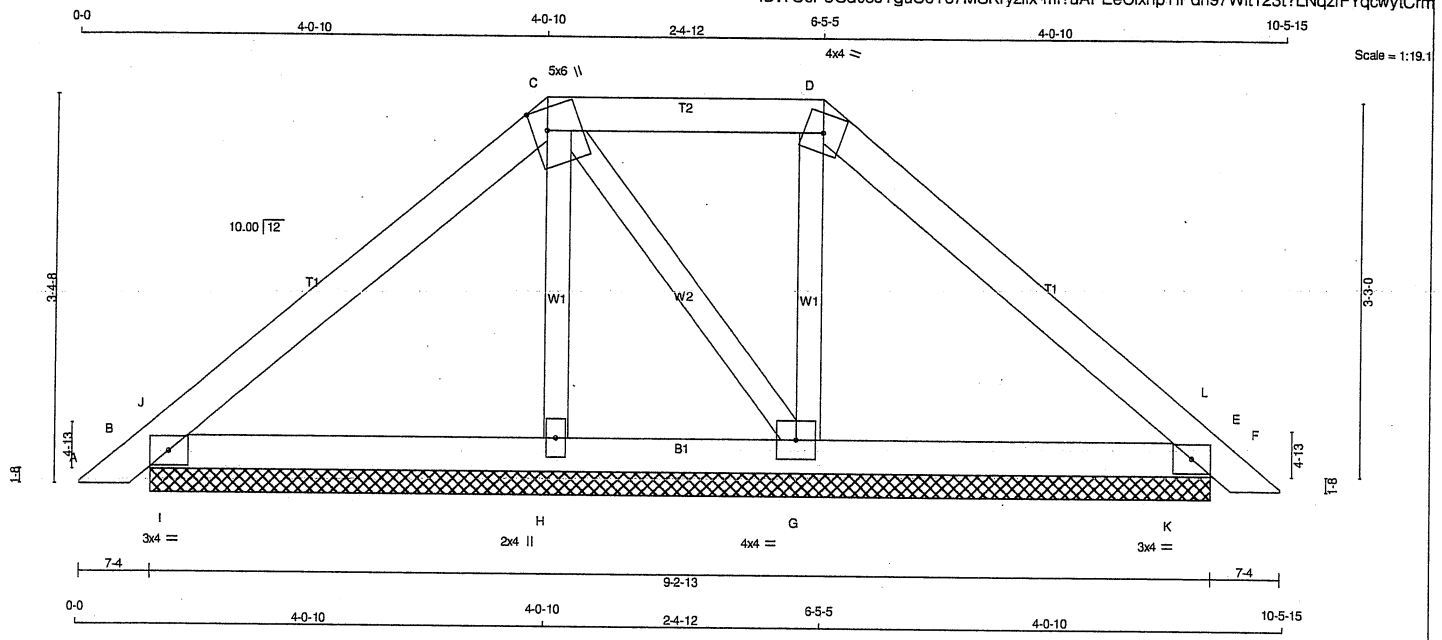

TOTAL WEIGHT = 2 X 32 = 65 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	300	0	300	0	0	9-2-13	9-2-13
E	281	0	281	0	0	9-2-13	9-2-13
H	190	0	190	0	0	9-2-13	9-2-13
G	272	0	272	0	0	9-2-13	9-2-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	211	146 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
E	198	135 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
H	137	76 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
G	193	122 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- J	-43 / 0	-84.9	-84.9	0.05 (1)	6.25
J- C	-141 / 0	-84.9	-84.9	0.12 (1)	6.25
C- D	-74 / 0	-84.9	-84.9	0.08 (1)	6.25
D- L	-112 / 0	-84.9	-84.9	0.12 (1)	6.25
L- E	-38 / 0	-84.9	-84.9	0.05 (1)	6.25
E- F	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- I	0 / 102	-18.5	-18.5	0.11 (1)	10.00
I- H	0 / 102	-18.5	-18.5	0.11 (1)	10.00
H- G	0 / 99	-18.5	-18.5	0.07 (1)	10.00
G- K	0 / 80	-18.5	-18.5	0.11 (1)	10.00
K- E	0 / 80	-18.5	-18.5	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-107 / 0	0.02 (1)
C- G	-40 / 0	0.01 (1)
G- D	-158 / 0	0.03 (1)
I- J	-285 / 0	0.00 (1)
K- L	-289 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (D-L:1) , BC=0.11/1.00 (H-I:1) , WB=0.03/1.00 (D-G:1) , SSI=0.23/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125145

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	PB4	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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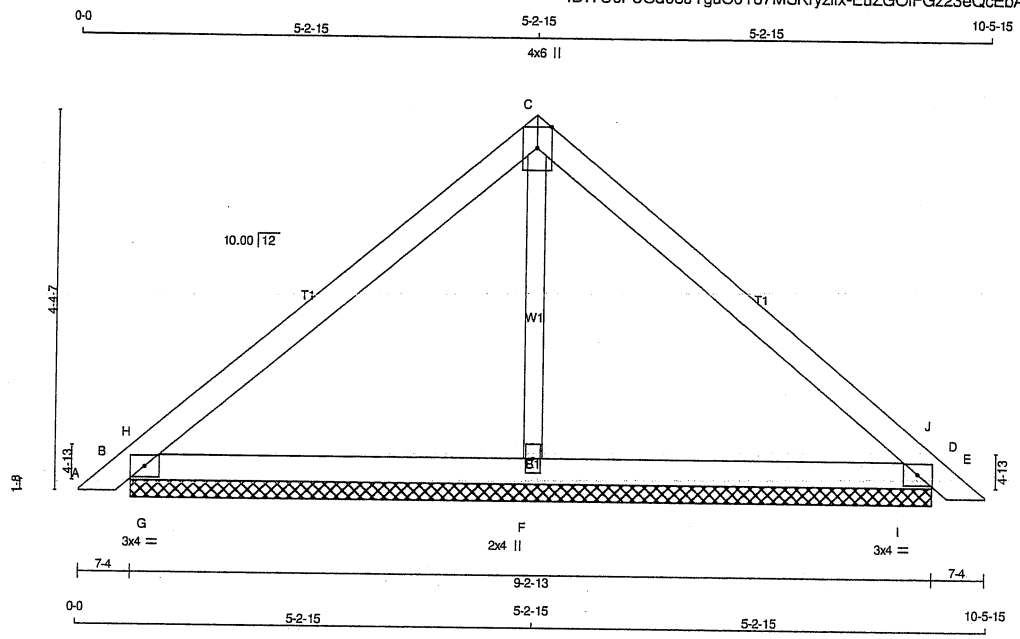

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	377	0	377	0	0	9-2-13	9-2-13
D	377	0	377	0	0	9-2-13	9-2-13
F	289	0	289	0	0	9-2-13	9-2-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	265	184 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
D	265	184 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
F	209	111 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- H	-33 / 80	-84.9	-84.9	0.14 (1)	6.25
H- C	-198 / 0	-84.9	-84.9	0.22 (1)	6.25
C- J	-198 / 0	-84.9	-84.9	0.22 (1)	6.25
J- D	-33 / 80	-84.9	-84.9	0.14 (1)	6.25
D- E	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- G	0 / 141	-18.5	-18.5	0.21 (1)	10.00
G- F	0 / 141	-18.5	-18.5	0.21 (1)	10.00
F- I	0 / 141	-18.5	-18.5	0.21 (1)	10.00
I- D	0 / 141	-18.5	-18.5	0.21 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-92 / 0	0.02 (1)
G- H	-572 / 0	0.00 (1)
I- J	-572 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (C-H:1) , BC=0.21/1.00 (F-G:1) , WB=0.02/1.00 (C-F:1) , SSI=0.44/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

Structural component only
DWG# T-2125146

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	J1	21	1	ROYAL PINE HOMES TRUSS DESC.	

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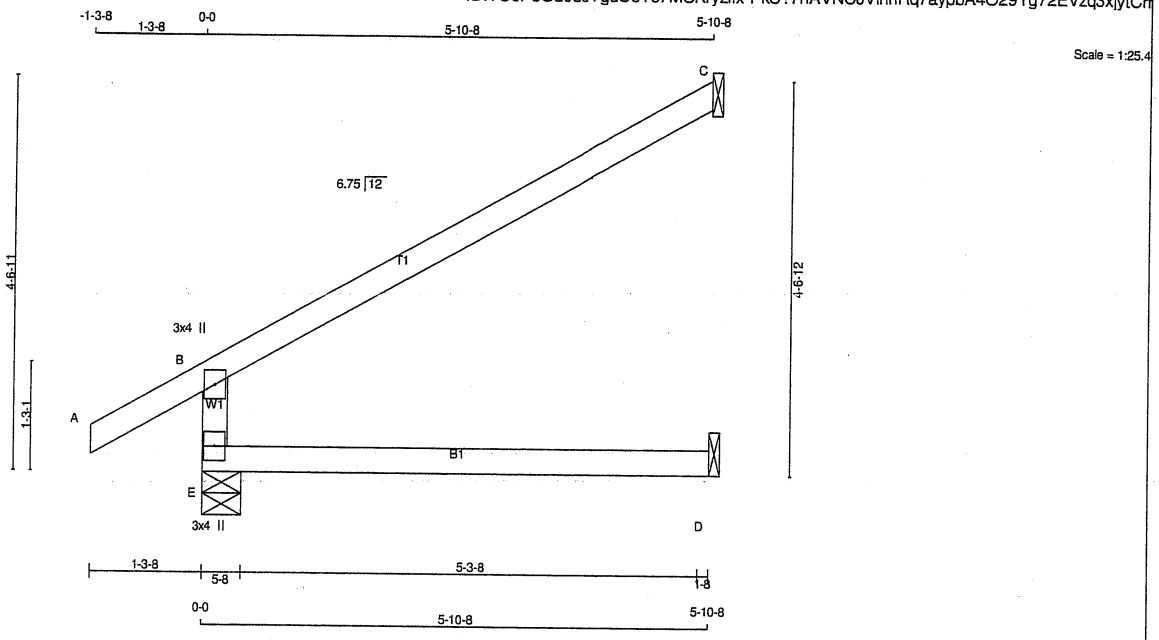

TOTAL WEIGHT = 21 X 17 = 361 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	491	0	491	0	0	5-8	5-8
C	187	0	187	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	346	234 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	129	103 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-427 / 0	0.0	0.0	0.13 (4)		7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)		10.00			
B- C	-31 / 0	-84.9	-84.9	0.50 (1)		6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125139

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420227	J1S	8	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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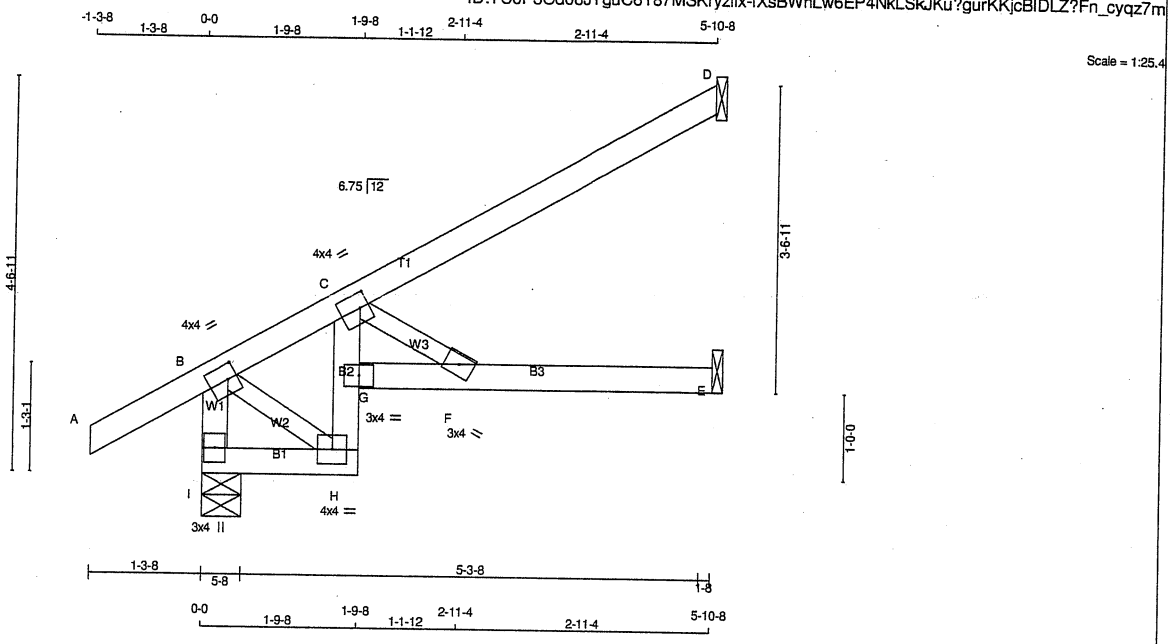

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW+w	MT20	3.0	4.0		
G	BVM-I	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	433	0	433	0	0	5-8	5-8
D	181	0	181	0	0	1-8	1-8
E	109	0	109	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	305	208 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	126	95 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	80	34 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-417 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B- C	-207 / 0	-84.9	-84.9	0.11 (1)	6.25
C- D	0 / 4	-84.9	-84.9	0.26 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00
H- G	-75 / 0	0.0	0.0	0.21 (1)	7.81
G- C	0 / 123	0.0	0.0	0.23 (1)	10.00
G- F	0 / 340	-18.5	-18.5	0.38 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.33 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- H	0 / 182	0.04 (1)
C- F	-410 / 0	0.06 (1)

TOTAL WEIGHT = 8 X 21 = 171 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 595 (0.12")

CSI: TC=0.26/1.00 (C-D:1) , BC=0.38/1.00 (F-G:1) , WB=0.06/1.00 (C-F:1) , SSI=0.15/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (C) (INPUT = 0.90)
JSI METAL= 0.15 (F) (INPUT = 1.00)

Structural component only
DWG# T-2126618

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	J2	7	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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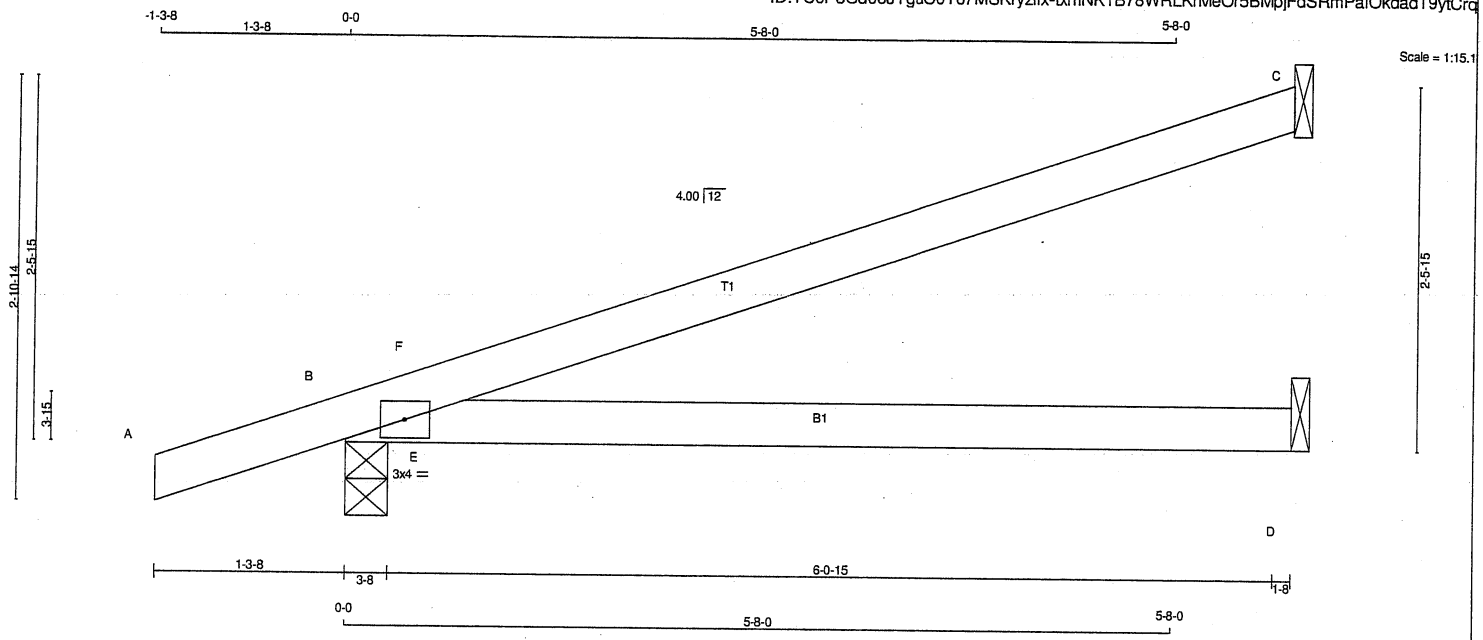

TOTAL WEIGHT = 7 X 17 = 118 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	242	0	242	0	0	1-8	1-8
B	449	0	449	0	0	3-8	3-8
D	94	0	94	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	168	128 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0
B	317	214 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
D	70	23 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.9	-84.9	0.11 (1)	10.00	E- F	-358 / 15	0.00 (1)
B- F	-22 / 27	-84.9	-84.9	0.07 (1)	6.25			
F- C	-2 / 2	-84.9	-84.9	0.47 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.31 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.32 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 826 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 388 (0.20")

CSI: TC=0.47/1.00 (C-F:1) , BC=0.32/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.30/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125140

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420226	J3	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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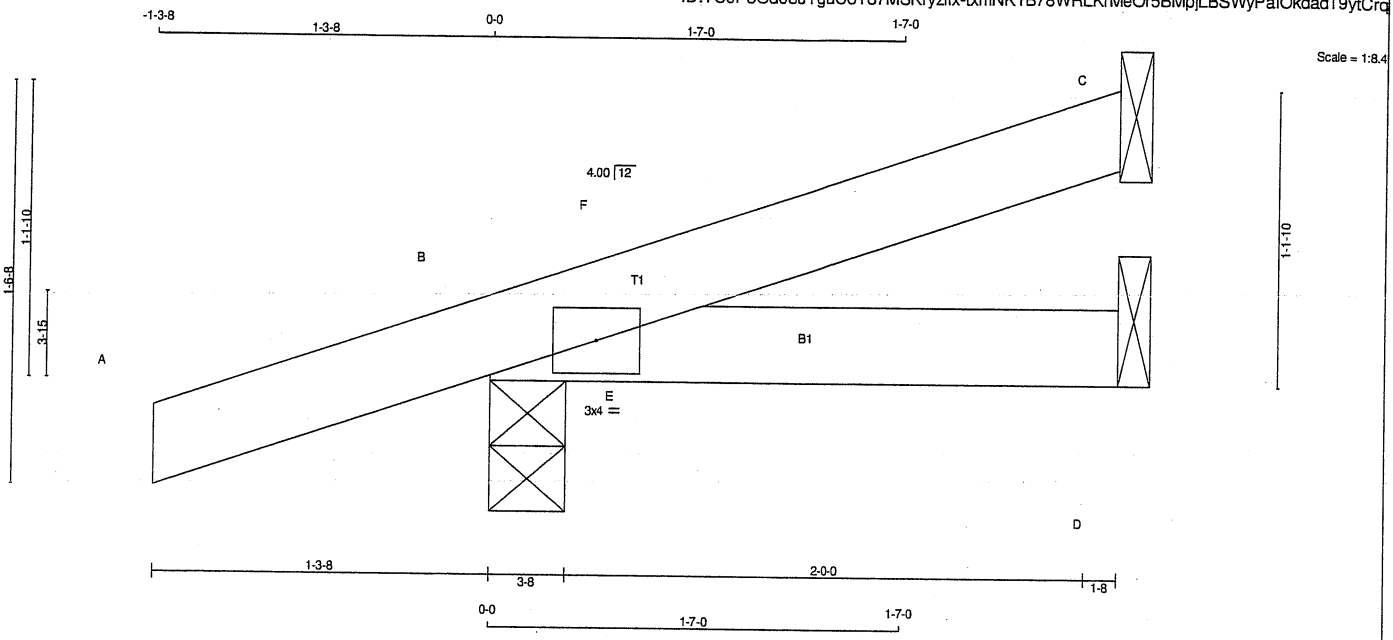

TOTAL WEIGHT = 3 X 7 = 22 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	86	0	86	0	0	1-8	1-8
B	238	0	238	0	0	3-8	3-8
D	39	0	39	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	60	45 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
B	167	119 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
D	29	11 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-84.9	-84.9	0.11 (5)	10.00
B- F	-10 / 0	-84.9	-84.9	0.02 (1)	10.00
F- C	0 / 3	-84.9	-84.9	0.06 (1)	10.00
B- E	0 / 0	-18.5	-18.5	0.05 (1)	10.00
E- D	0 / 0	-18.5	-18.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-66 / 3	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:5) , BC=0.05/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125141

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420226	J4	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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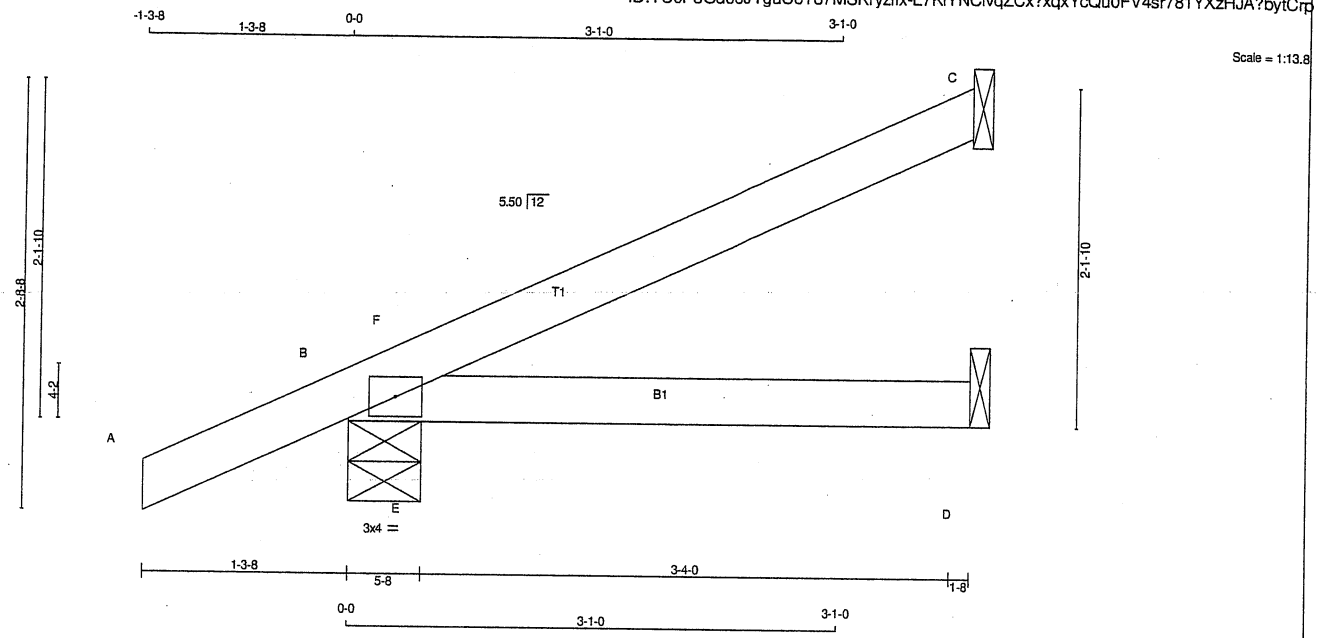

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	145	0	145	0	0	1-8	1-8
B	317	0	317	0	0	5-8	5-8
D	58	0	58	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	100	76 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
B	223	154 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
D	43	15 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 23	-84.9	-84.9	0.13 (5)	10.00	E- F	-191 / 6	0.00 (1)
B- F	-16 / 10	-84.9	-84.9	0.03 (4)	6.25			
F- C	-1 / 2	-84.9	-84.9	0.17 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.12 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.12 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 8 X 11 = 89 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.17/1.00 (C-F:1) , BC=0.12/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125142

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	J20	15	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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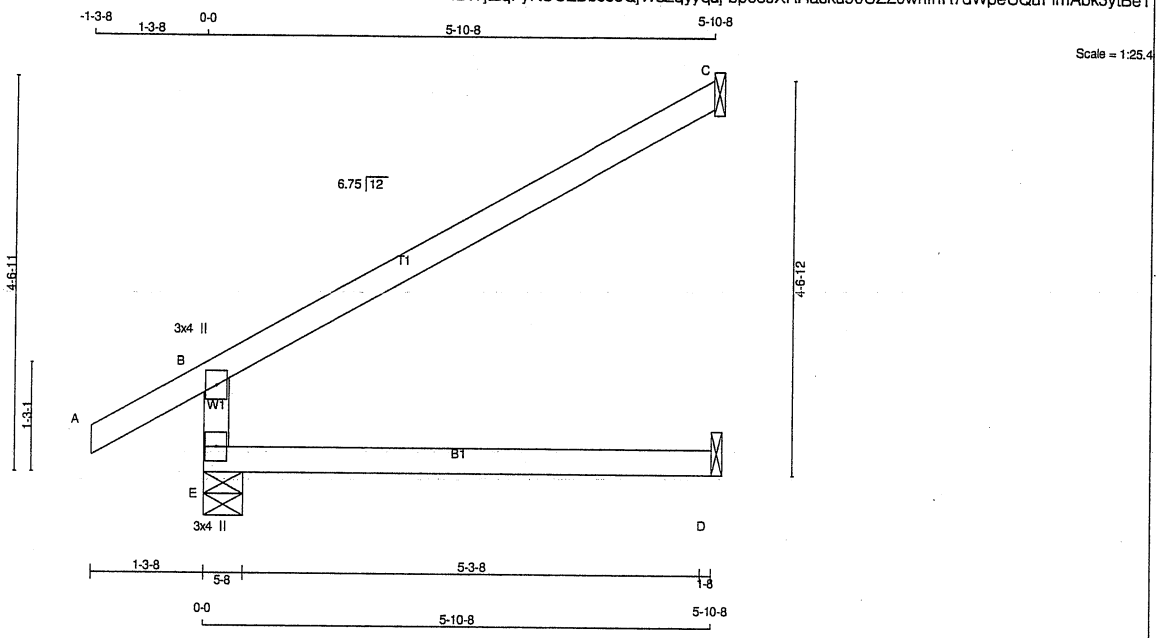

TOTAL WEIGHT = 15 X 17 = 258 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	491	0	491	0	0	5-8	5-8
C	187	0	187	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	346	234 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	129	103 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
E- B	-427 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B- C	-31 / 0	-84.9	-84.9	0.50 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125174

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420229	J20S	7	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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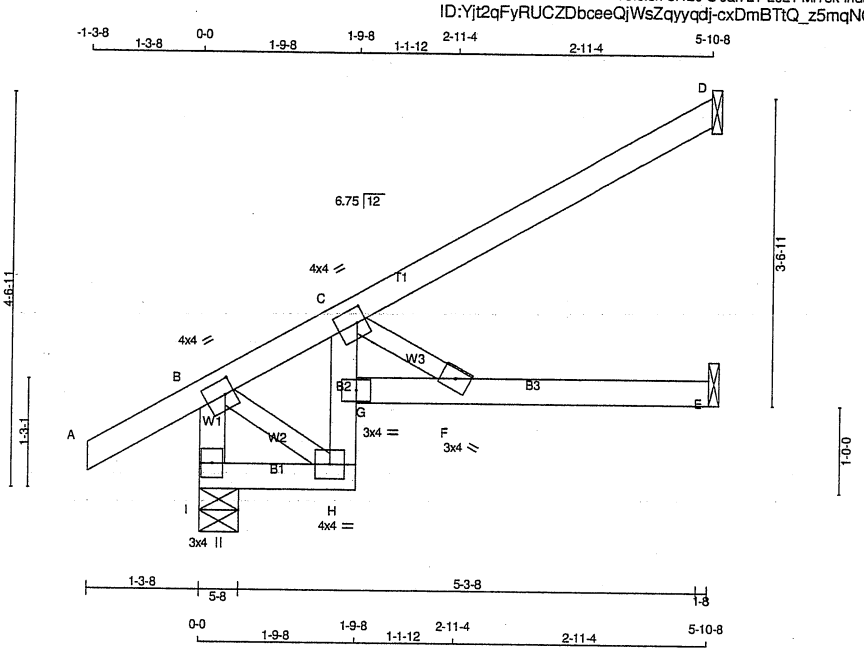

TOTAL WEIGHT = 7 X 21 = 150 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW+w	MT20	3.0	4.0		
G	BVM-l	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	433	0	433	0	0	5-8	5-8
D	181	0	181	0	0	1-8	1-8
E	109	0	109	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	305	208 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	126	95 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	80	34 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-417 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B- C	-207 / 0	-84.9	-84.9	0.11 (1)	6.25
C- D	0 / 4	-84.9	-84.9	0.26 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00
H- G	-75 / 0	0.0	0.0	0.21 (1)	7.81
G- C	0 / 123	0.0	0.0	0.23 (1)	10.00
G- F	0 / 340	-18.5	-18.5	0.38 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.33 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- H	0 / 182	0.04 (1)
C- F	-410 / 0	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 595 (0.12")

CSI: TC=0.26/1.00 (C-D:1) , BC=0.38/1.00 (F-G:1) , WB=0.06/1.00 (C-F:1) , SSI=0.15/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (C) (INPUT = 0.90)
JSI METAL= 0.15 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125192

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	J21	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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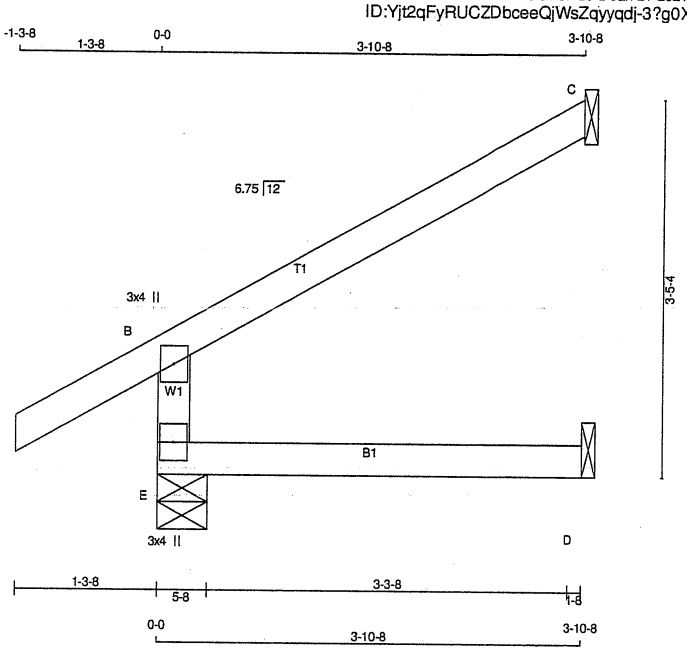

TOTAL WEIGHT = 4 X 12 = 49 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	362	0	362	0	0	5-8	5-8
C	123	0	123	0	0	1-8	1-8
D	31	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	254	176 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
C	85	68 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-321 / 0	0.0	0.0	0.05 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.13 (5)	10.00			
B- C	-20 / 0	-84.9	-84.9	0.22 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125175

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	J22	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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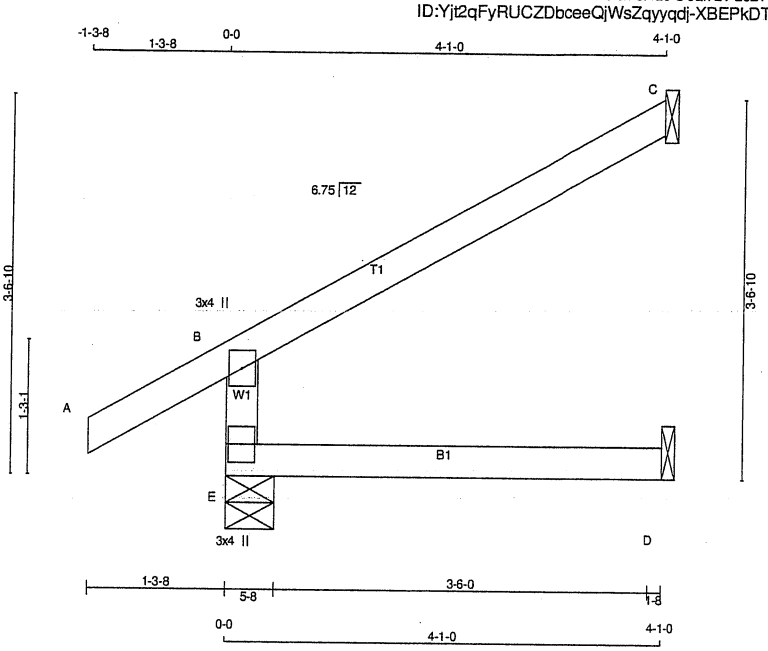

TOTAL WEIGHT = 2 X 13 = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	376	0	376	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	32	0	36	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	264	182 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	90	71 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	26	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-332 / 0	0.0	0.0	0.05 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B- C	-21 / 0	-84.9	-84.9	0.24 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.07/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125176

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420229	J23	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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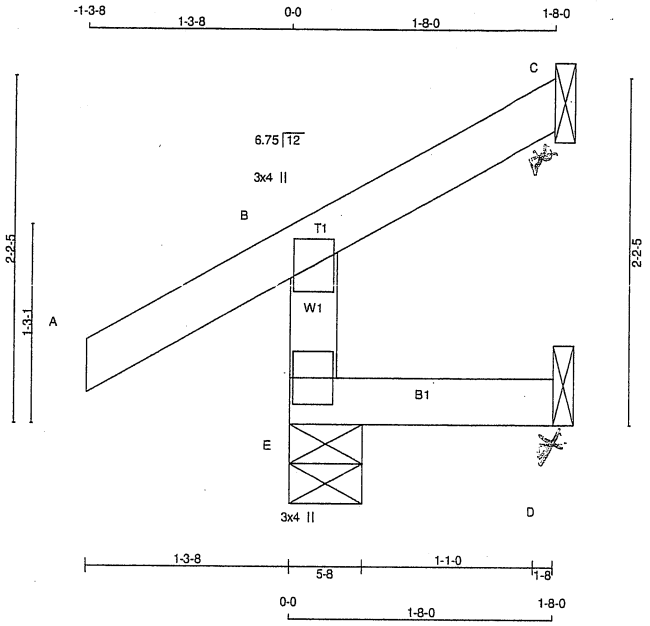

TOTAL WEIGHT = 2 X 7 = 14 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	249	0	249	0	0	5-8	5-8
C	34	0	34	0	-24	1-8	1-8
D	5	0	14	0	-3	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	174	128 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
C	23	18 / -18	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	5	0 / -8	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-224 / 0	0.0	0.0	0.03 (5)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B- C	-18 / 0	-84.9	-84.9	0.08 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2126373

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	C20	6	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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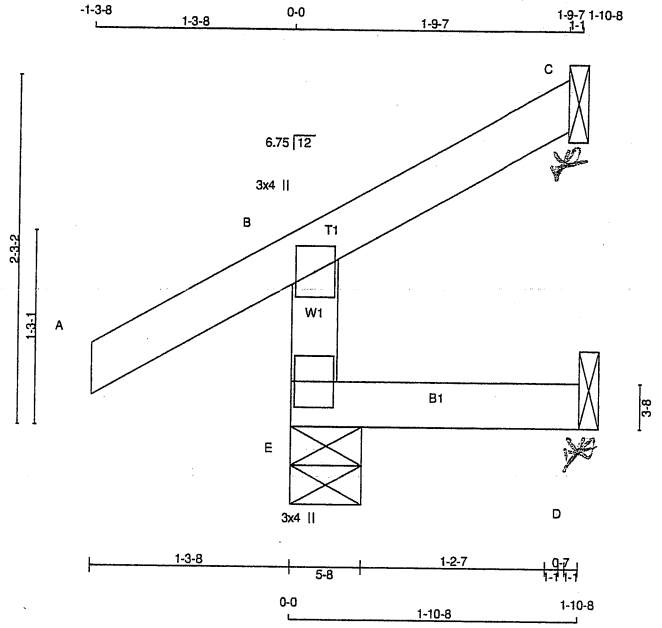

TOTAL WEIGHT = 6 X 7 = 44 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	253	0	253	0	0	5-8	5-8
C	41	0	41	0	-21	1-8	1-8
D	9	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	176	129 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	28	22 / -17	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	8	0 / -7	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-226 / 0	0.0	0.0	0.03 (5)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (5)	10.00			
B- C	-17 / 0	-84.9	-84.9	0.09 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:5) , BC=0.03/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125167

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420228	C21	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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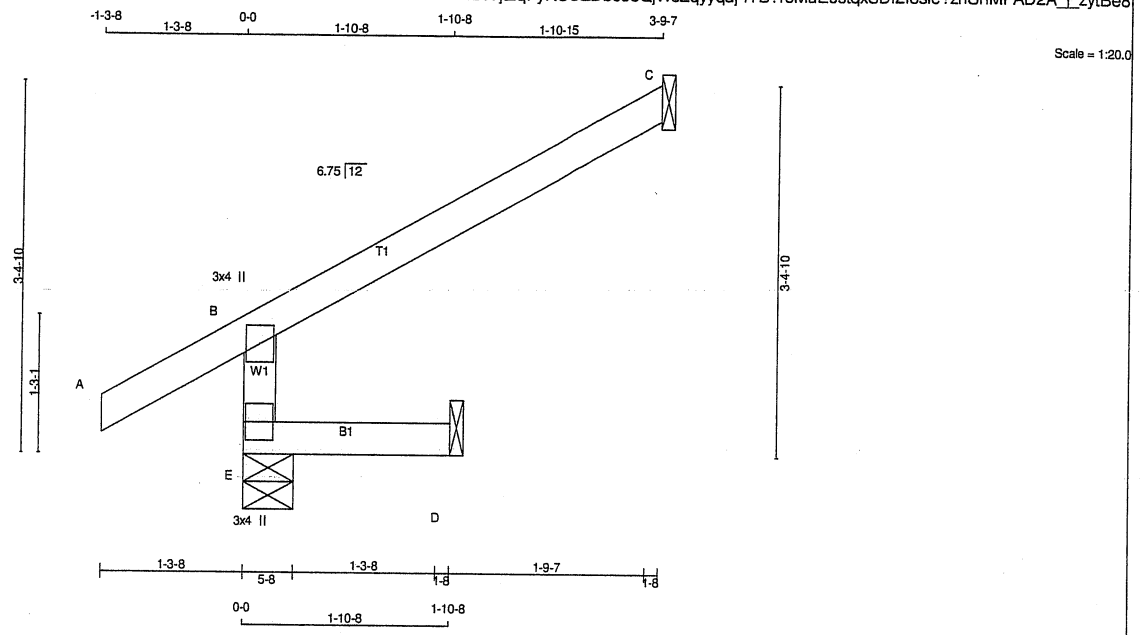

TOTAL WEIGHT = 3 X 10 = 30 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	335	0	335	0	0	5-8	5-8
C	121	0	121	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	234	174 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	83	66 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-316 / 0	0.0	0.0	0.01 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.12 (5)	10.00			
B- C	-20 / 0	-84.9	-84.9	0.21 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125168

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	C22	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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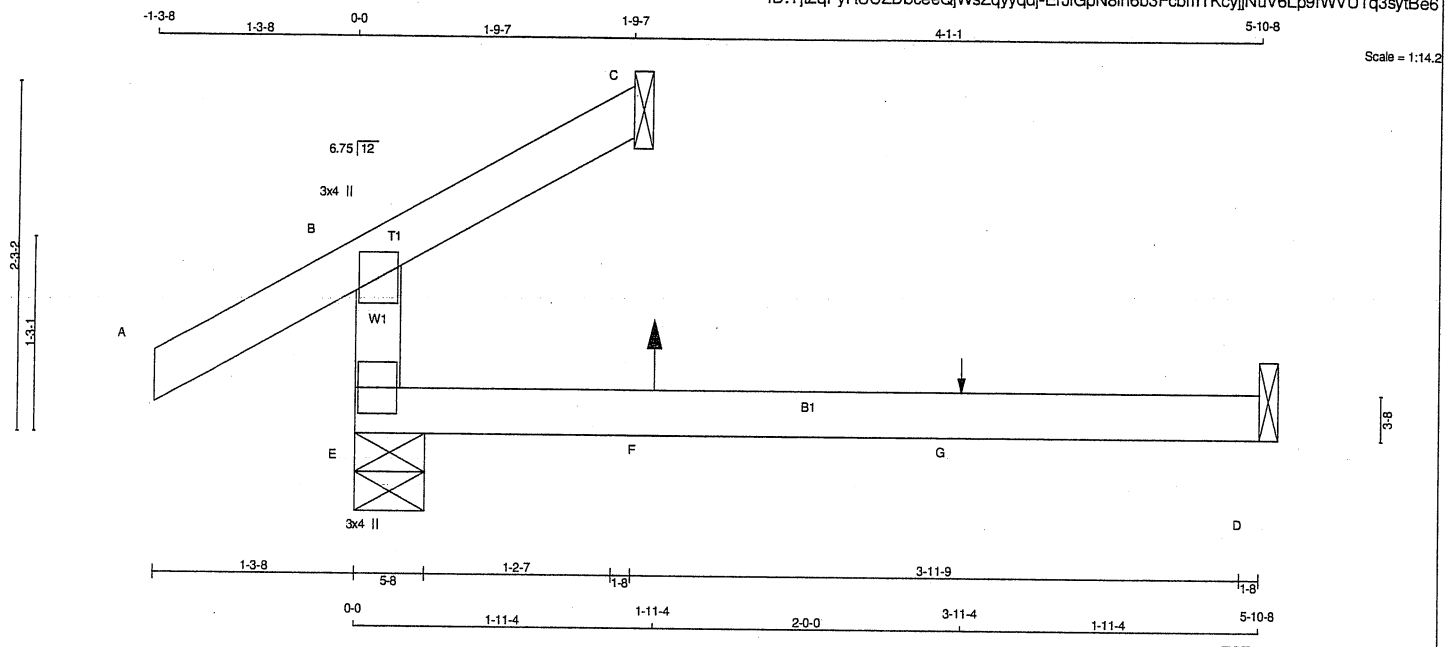

TOTAL WEIGHT = 3 X 12 = 35 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	267	0	267	0	0	5-8	5-8
C	59	0	59	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	126 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
C	43	19 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	36	0 / -2	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-210 / 0	0.0	0.0	0.11 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.12 (1)	10.00			
B- C	-9 / 10	-84.9	-84.9	0.08 (4)	10.00			
E- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
F- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	4	1	7	FRONT	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (D-E:4)

DOL LUMBER=0.97 NAIL=0.97 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125169

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	C23	3	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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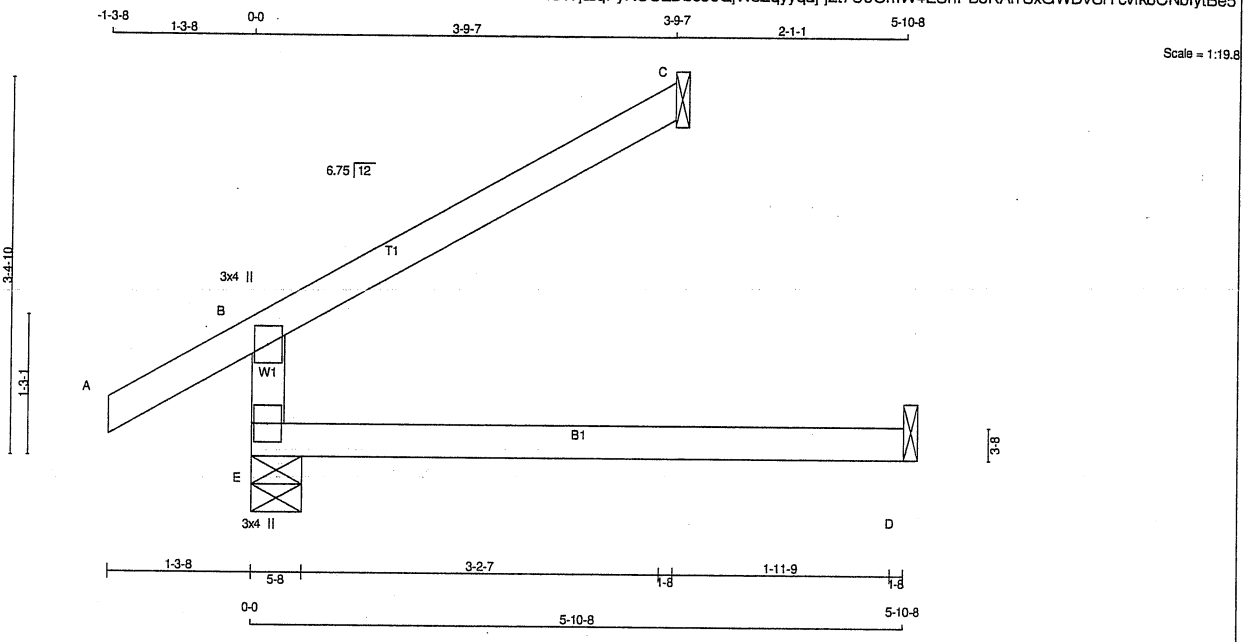

TOTAL WEIGHT = 3 X 14 = 43 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	380	0	380	0	0	5-8	5-8
C	121	0	121	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	269	174 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	83	66 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-316 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B- C	-20 / 0	-84.9	-84.9	0.21 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125170

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	C24	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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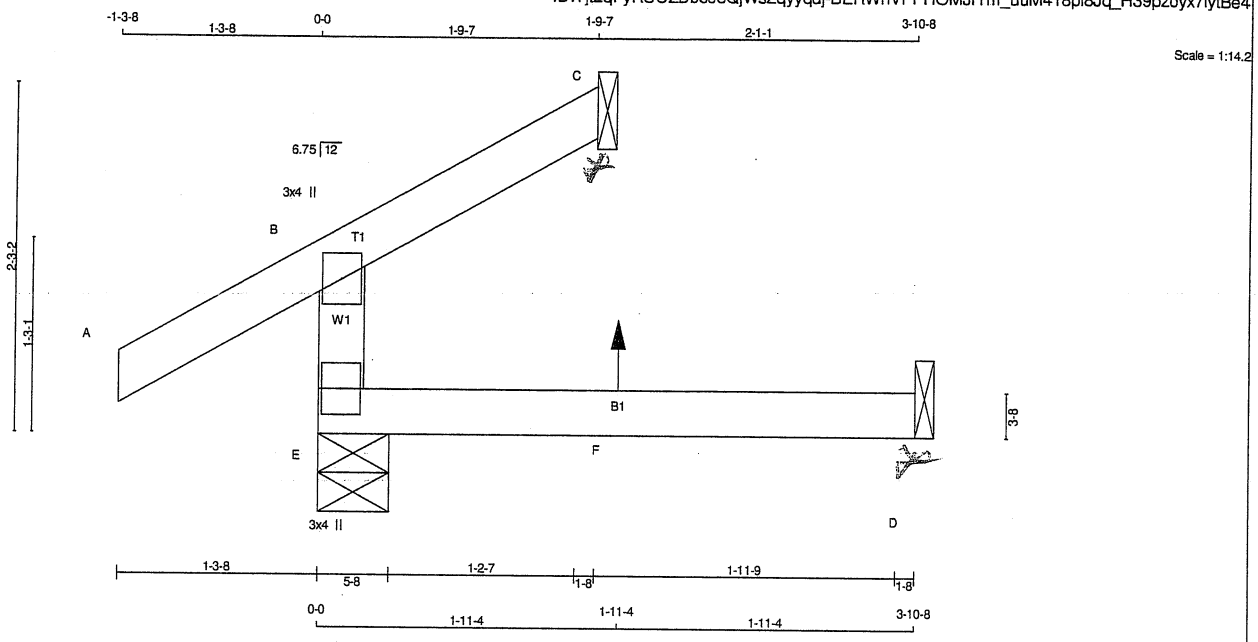

TOTAL WEIGHT = 2 X 10 = 19 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	261	0	261	0	0	5-8	5-8
C	45	0	45	0	-18	1-8	1-8
D	27	0	35	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	184	126 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
C	32	20 / -18	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
D	22	0 / -4	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (9)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-223 / 0	0.0	0.0	0.05 (5)	7.81			
A- B	0 / 28	-84.9	-84.9	0.13 (5)	10.00			
B- C	-15 / 1	-84.9	-84.9	0.08 (5)	6.25			
E- F	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
F- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	4	1	7	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125171

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420228	C25	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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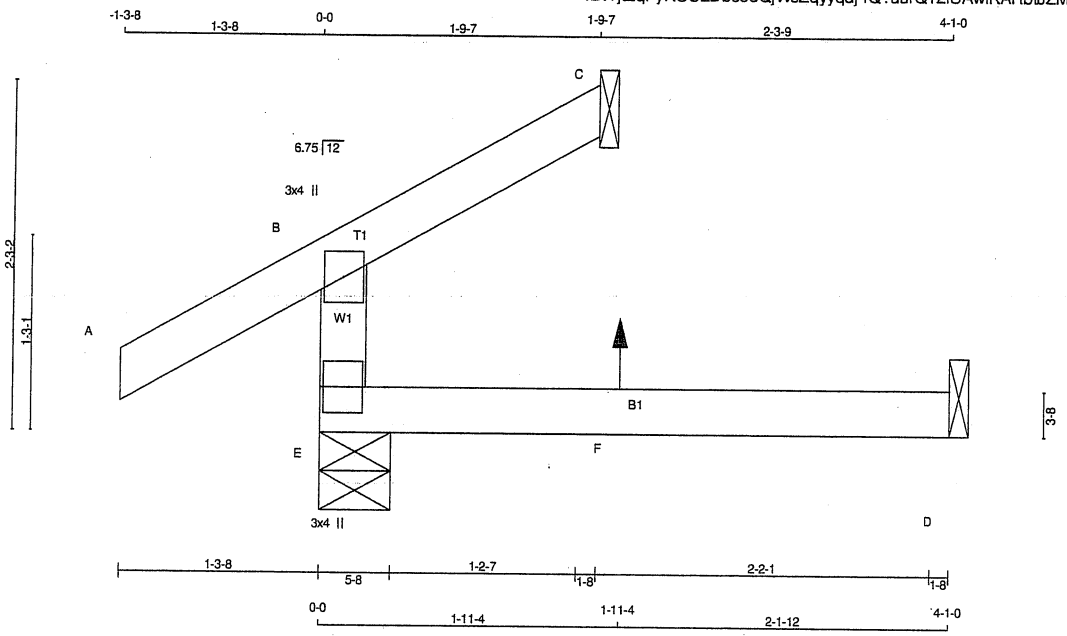


TOTAL WEIGHT = 10 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	262	0	262	0	0	5-8	5-8
C	46	0	46	0	0	1-8	1-8
D	28	0	37	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	184	126 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
C	33	20 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
D	23	0 / -4	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	CSI (LC)	WEBS MAX. MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		UNBRAC LENGTH	FR-TO	
E- B	-222 / 0	0.0 0.0	0.05 (4)	7.81		
A- B	0 / 28	-84.9 -84.9	0.11 (1)	10.00		
B- C	-15 / 2	-84.9 -84.9	0.07 (5)	6.25		
E- F	0 / 0	-18.5 -18.5	0.07 (4)	10.00		
F- D	0 / 0	-18.5 -18.5	0.07 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	4	1	7	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.07/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125172

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering m and General Safety notes.

T-1800218 Feb 09, 2018

MiTek®

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	LENGTH (in.)	DIA. (in.)	LATERAL RESISTANCE (lbs)		WITHDRAWAL RESISTANCE (lbs)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

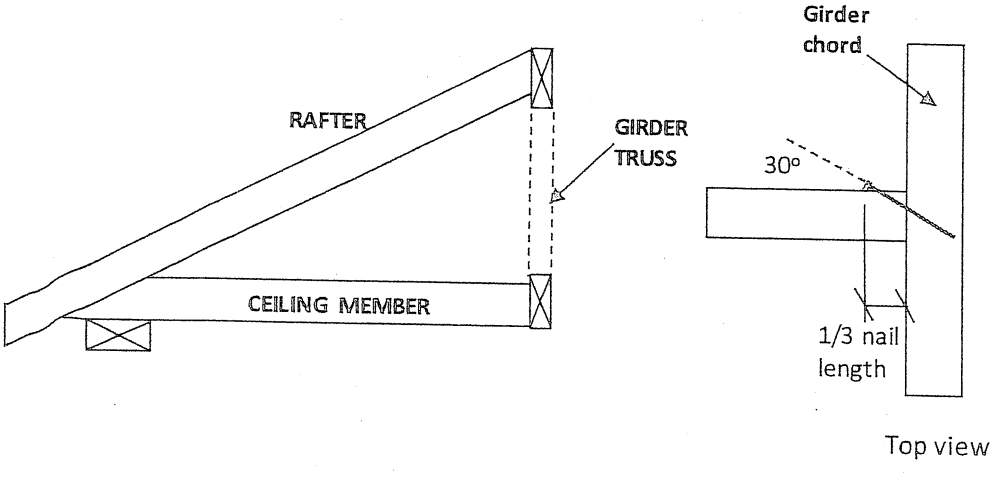
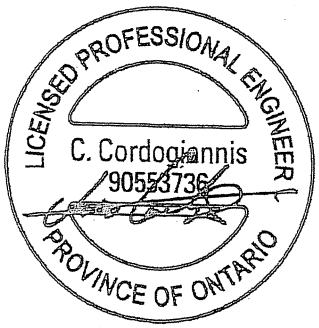


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485



December 21, 2020

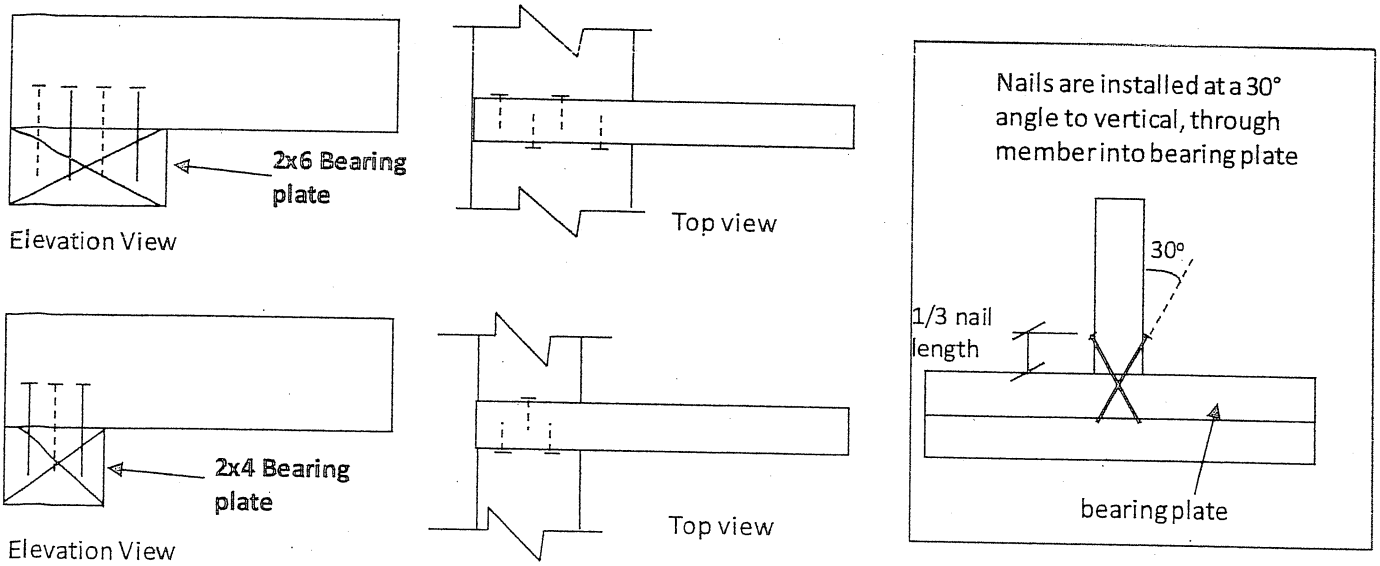
MiTek®

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).

2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).

3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.

4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).

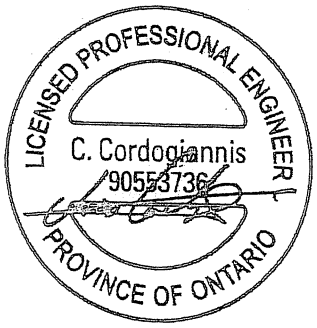
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.

6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.

7. Lumber must be dry (< 19% moisture content) at the time of nail installation.

8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



December 21, 2020

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

VALUE ENGINEERED

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

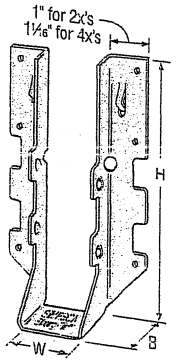
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

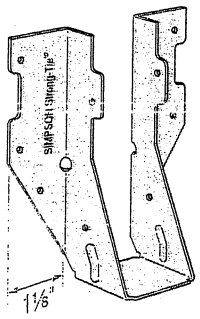
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

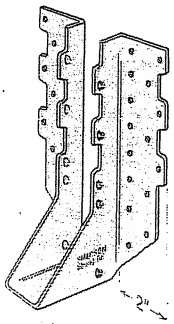
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



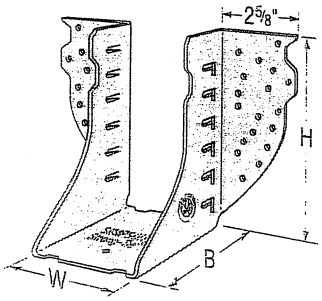
LUS28



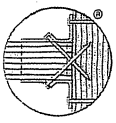
LU26L



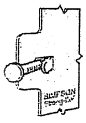
HUS210
(HUS26, HUS28, and HHUS similar)



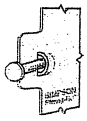
HGUS28-2



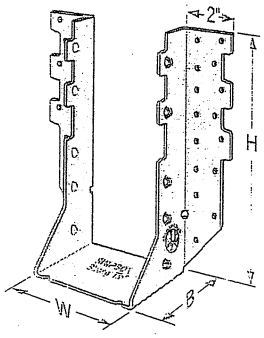
Double-Shear Nailing Top View



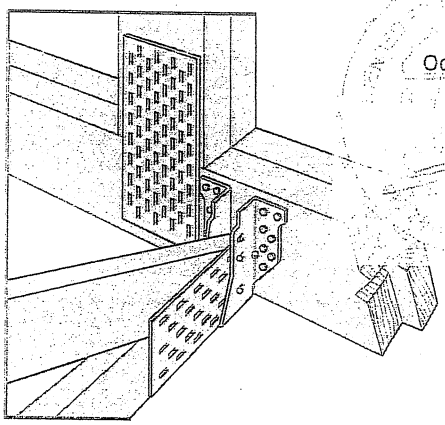
Double-Shear Nailing Side View; Do not bend tab



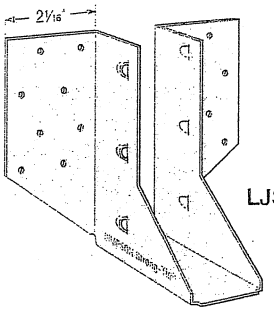
Dome Double-Shear Nailing Side View *(available on some models)*



HHUS210-2



Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

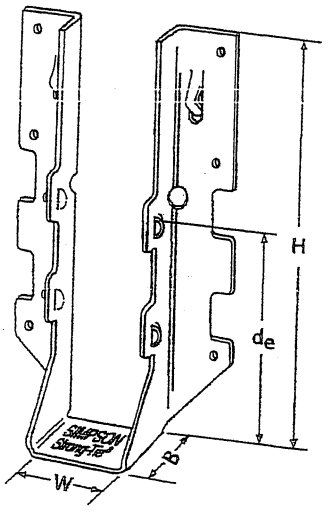
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

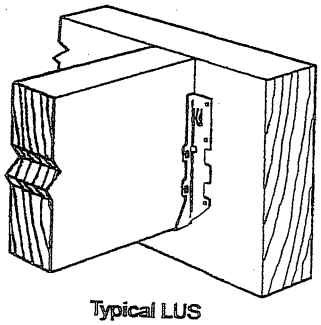
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

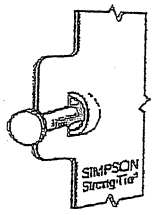


LUS28


Typical LUS Installation

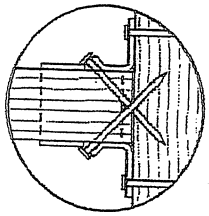
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

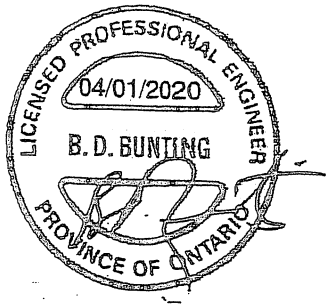
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

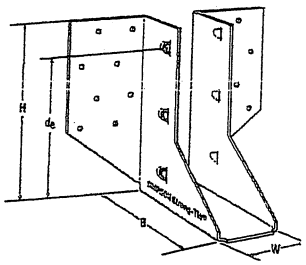
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

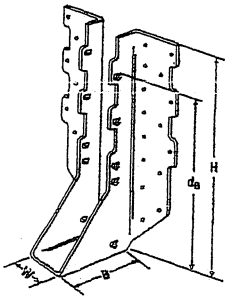
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

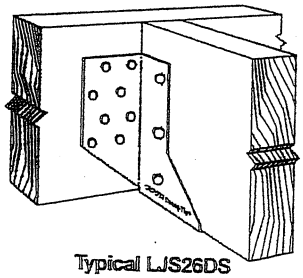
- See current catalogue for options



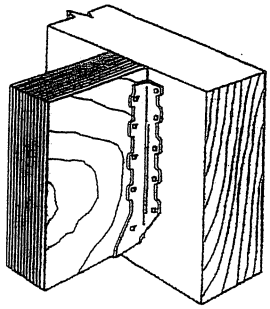
LJS26DS



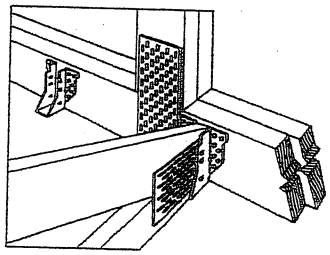
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



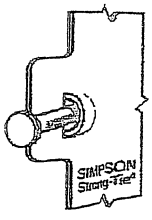
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

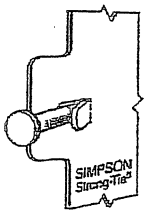
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

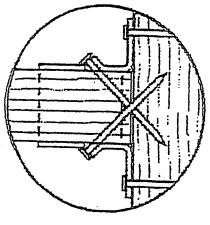


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

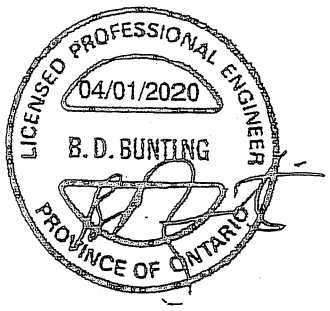
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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(800) 999-5099
Strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

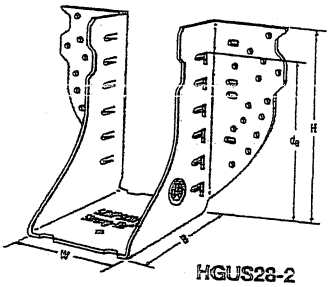
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

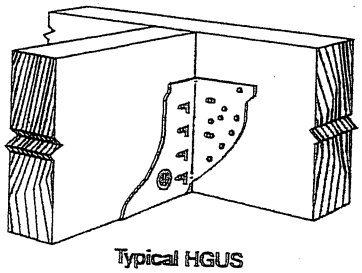
Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2



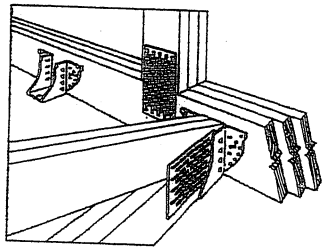
Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

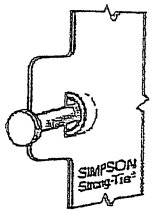
- See current catalogue for options



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

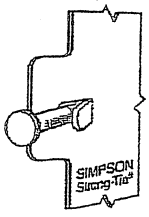
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10 5/8	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12 5/8	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

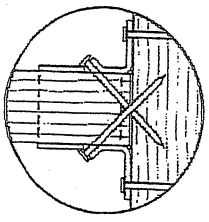


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

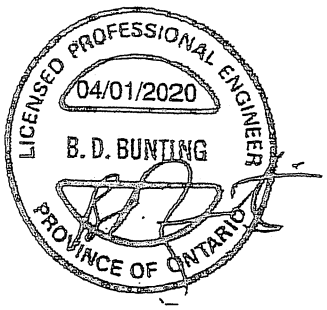
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



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Strongtie.com

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

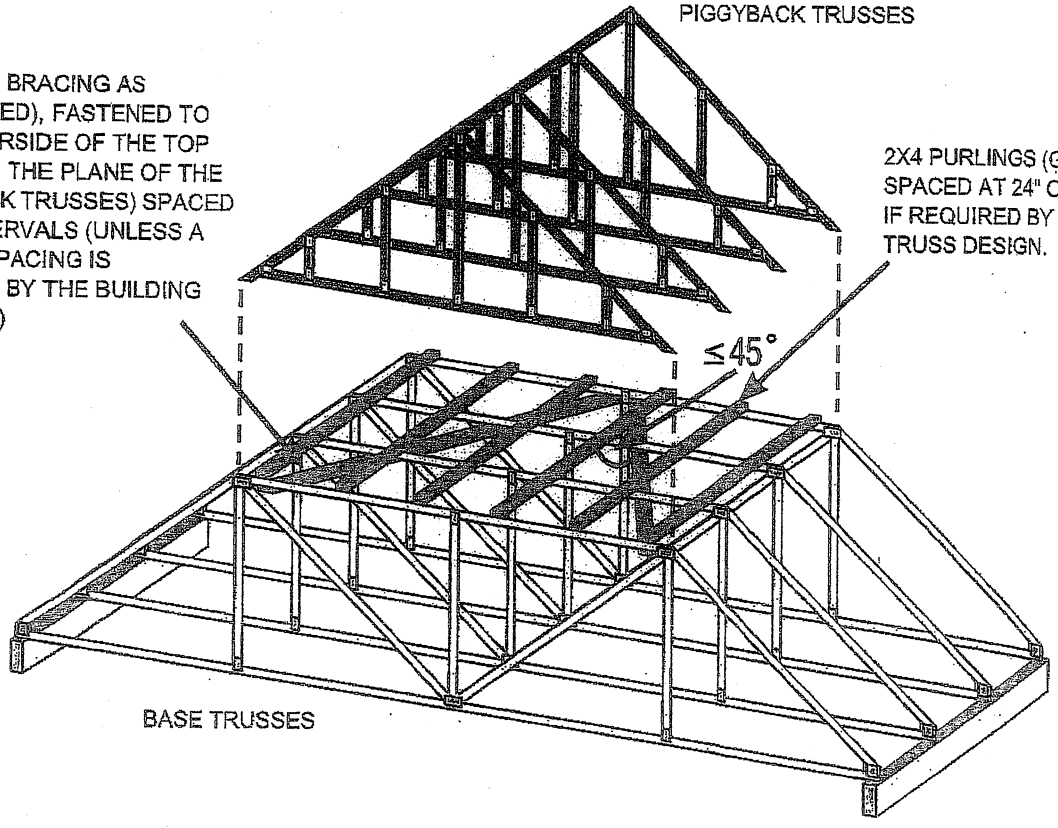
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

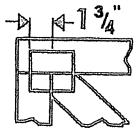


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

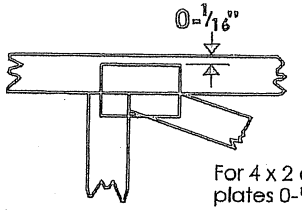
SKETCH FROM BCSI-CANADA 2013

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

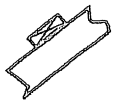
*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

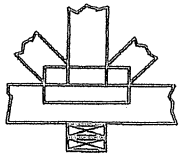
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

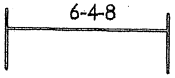
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

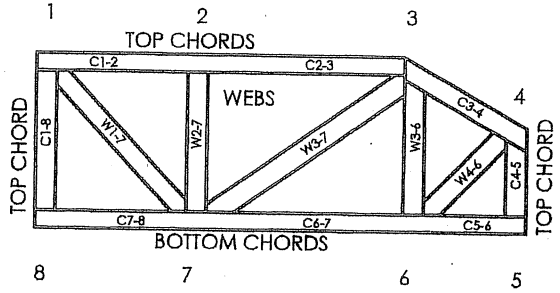
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.