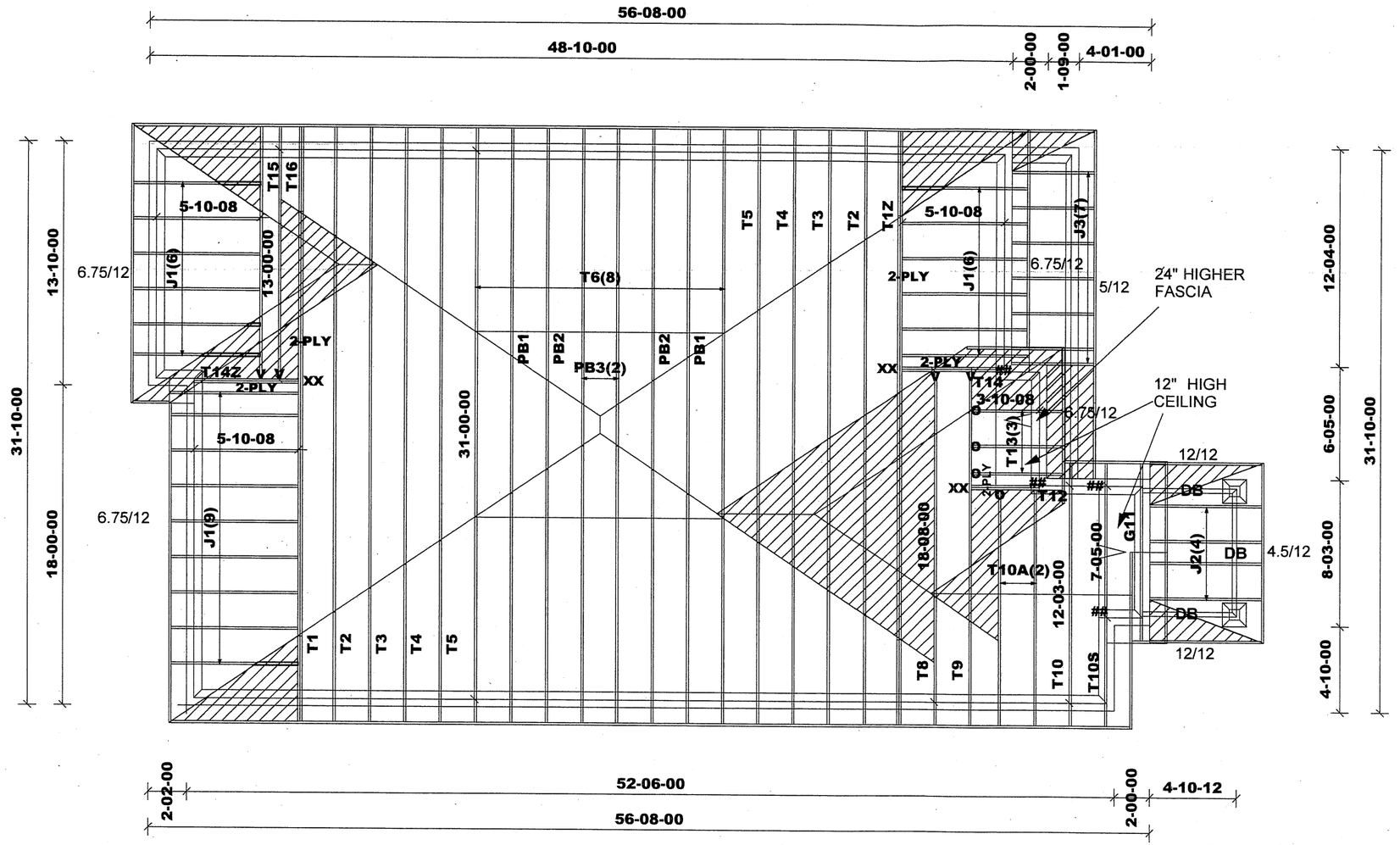
10/12 ROOF PITCHES UNLESS OTHERWISE NOTED



All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127264
STRUCTURAL
COMPONENT ONLY

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DENOTES:
CONVENTIONAL
FRAMING

DB - DROPPED BEAMS BY OTHERS

- 12" RAISED PLATES

DWG# T-2125076 to T-2125100, T1800218

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

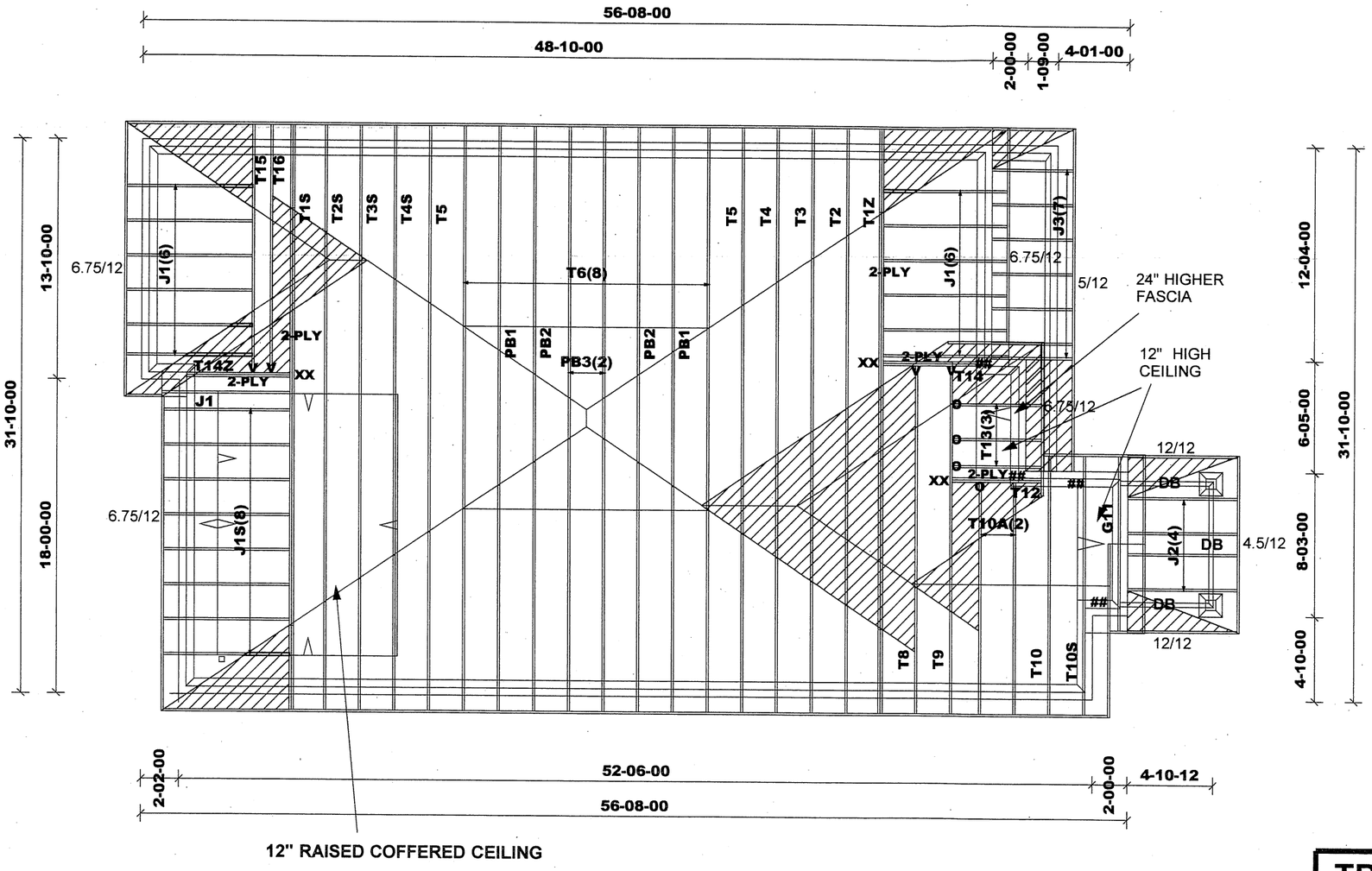
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

M14211

TAMARACK ROOF TRUSSES INC.

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: MODEL 40-1 / A STD	Mitek ver 8.4.2.286
Layout ID: 420178	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 204990	Date: 2021-08-04	Designer: JC	

10/12 ROOF PITCHES UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DENOTES:
CONVENTIONAL FRAMING

DB - DROPPED BEAMS BY OTHERS

- 12" PLATE DIFF.

DWG# T-2125076 to T-2125082, T-2125084 to T-2125105, T1800218

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127205
STRUCTURAL COMPONENT ONLY

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

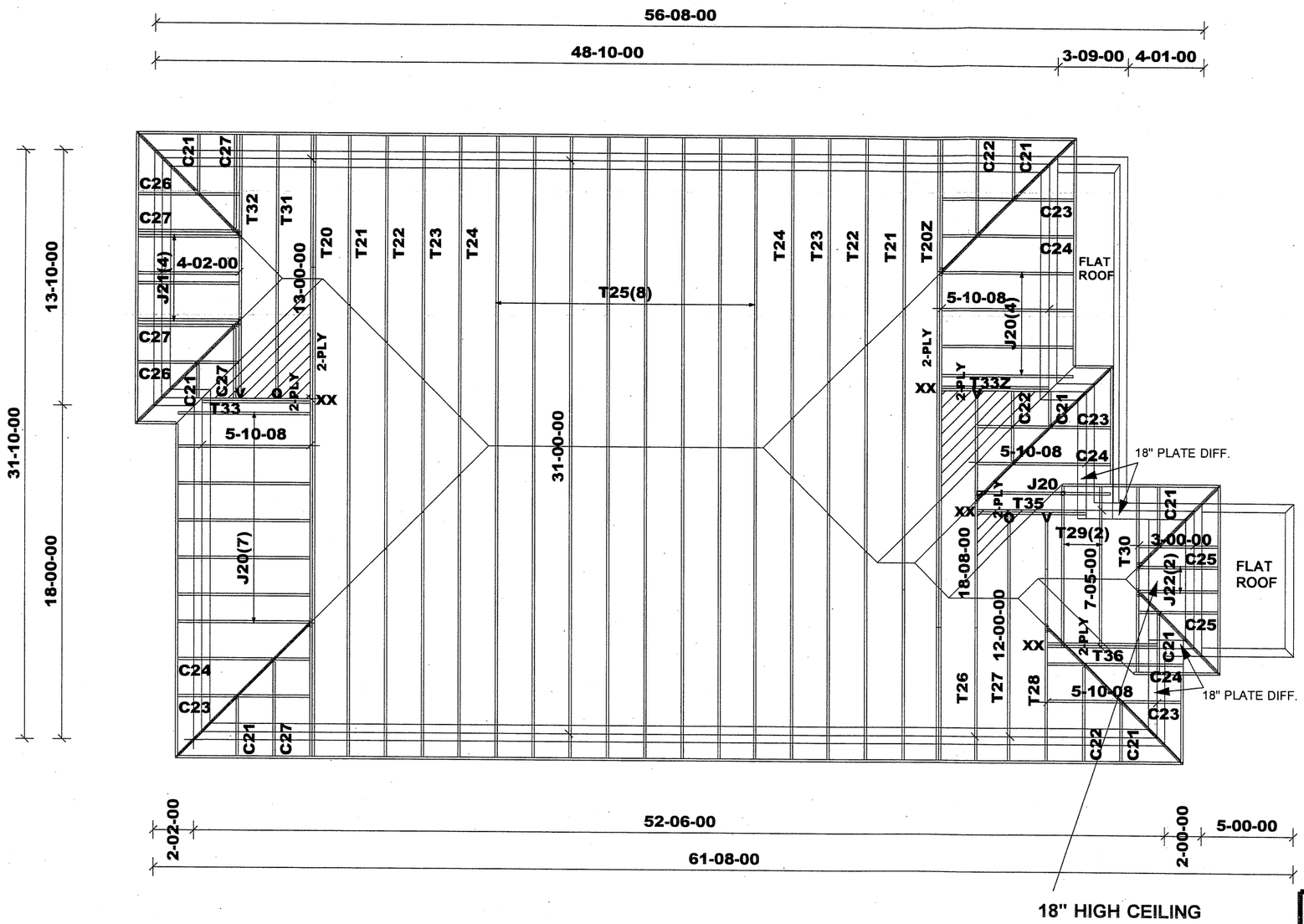
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

M14211

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: MODEL 40-1 / A OPT.	Mitek ver 8.4.2.286
Layout ID: 420185	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 204990	Date: 2021-08-04 Designer: JC		

6.75/12 ROOF PITCHES UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)

LICENSED PROFESSIONAL ENGINEER
08/10/21
H. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127206
STRUCTURAL
COMPONENT ONLY

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DENOTES:
CONVENTIONAL
FRAMING

DWG# T-2125106 to T-2125133, T1800218

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

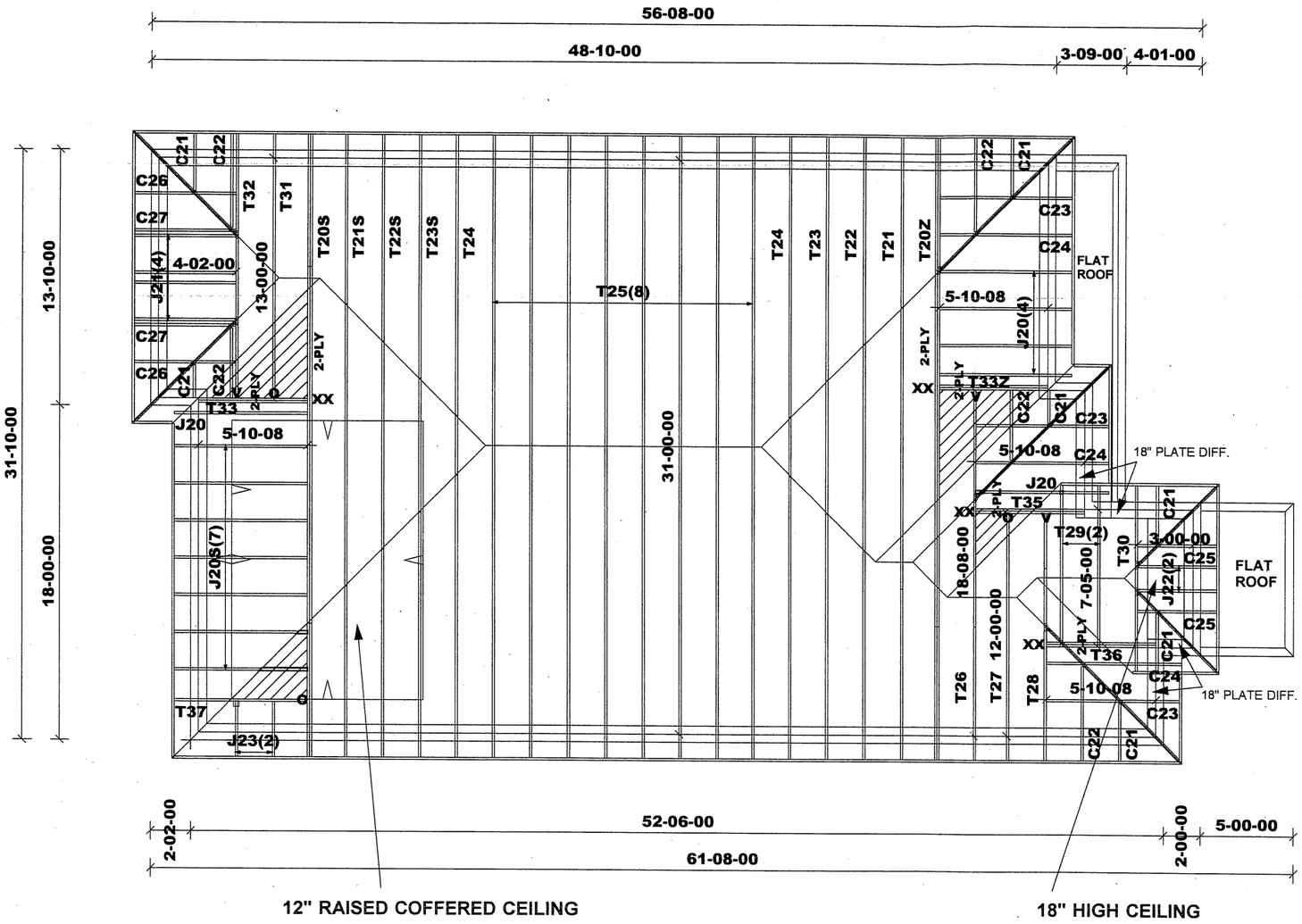
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

M14211

TAMARACK ROOF TRUSSES INC.
ALMA LUMBER GROUP

Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: MODEL 40-1 / B STD.	Mitek ver 8.4.2.286
Layout ID: 420186	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 204990	Date: 2021-08-04	Designer: JC	

6.75/12 ROOF PITCHES UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
2019 (AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 27.2 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 23.3 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2 - (**XX**)

LICENSED PROFESSIONAL ENGINEER
09/10/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T-2127207
STRUCTURAL
COMPONENT ONLY

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DENOTES:
CONVENTIONAL
FRAMING

DWG# T-2125106 to T-2125115, T-2125117 to T-2125138, T-2126371 to T-2126372, T1800218

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

M14211

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

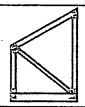
Job Track: 52430	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: MODEL 40-1 / B OPT	Mitek ver 8.4.2.286
Layout ID: 420187	Project: VALES OF CASTLEMORE NORTH		
Plan Log: 204990	Date: 2021-08-04	Designer: JC	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420178
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	1 of 2
Lot #:		Date:	08-04-2021
Elevation:	A STD	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

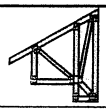
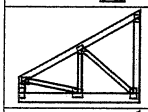
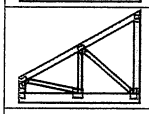
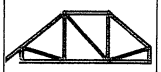
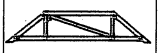
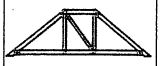
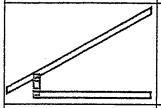
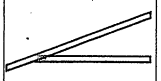
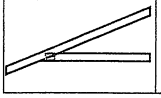
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	10 /12	**31-00-00**	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	344.48 212.00		
	1 2-ply	T1Z Hip Girder	10 /12	**31-00-00**	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	344.48 212.00		
	2	T2 Hip	10 /12	**31-00-00**	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	266.14 173.33		
	2	T3 Hip	10 /12	**31-00-00**	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	294.81 187.00		
	2	T4 Hip	10 /12	**31-00-00**	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	313.76 199.33		
	2	T5 Hip	10 /12	**31-00-00**	9-00-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	308.44 194.33		
	8	T6 Hip	10 /12	**31-00-00**	10-02-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	1294.75 810.67		
	1	T8 Hip	10 /12	**18-08-00**	6-06-12	2 x 4	1-03-08	1-07-11 3-07-11	88.32 55.83		
	1	T9 Hip Girder	10 /12	**18-08-00**	5-05-04	2 x 4 2 x 6	1-03-08	1-07-11 3-07-11	101.37 64.17		
	1	T10 Common	10 /12	**12-03-00**	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	59.1 38.17		
	2	T10A Common	10 /12	**12-00-00**	6-08-15	2 x 4	1-03-08	1-10-03 1-07-11	113.28 72.33		
	1	T10S Roof Special	10 /12	**12-03-00**	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 7-11	65.91 43.33		
	1	G11 GABLE	10 /12	**7-05-00**	4-06-09	2 x 4	1-03-08 1-03-08	7-11 7-11	30.73 20.67		
	1 2-ply	T12 Monopitch Girder	6.75 /1 2	**3-10-08**	5-05-04	2 x 4 2 x 6		3-03-01 5-05-04	47.85 30.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420178
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	2 of 2
Lot #:		Date:	08-04-2021
Elevation:	A STD	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	3	T13 Monopitch	6.75 /12	3-10-08	5-05-04	2 x 4	1-03-08	2-03-01 5-05-04	91.82 62.50		
	1 2-ply	T14 Monopitch Girder	6.75 /12	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1 2-ply	T14Z Monopitch Girder	6.75 /12	5-10-08	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1	T15 Hip Girder	10 /12	13-00-00	4-06-12	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	67.79 43.67		
	1	T16 Hip	10 /12	13-00-00	5-01-12	2 x 4	1-03-08	1-07-11 1-07-11	56.92 37.50		
	2	PB1 Piggyback	10 /12	10-05-15	1-01-08	2 x 4			60.53 40.33		
	2	PB2 Piggyback	10 /12	10-05-15	2-03-00	2 x 4			61.66 41.00		
	2	PB3 Piggyback	10 /12	10-05-15	3-04-08	2 x 4			64.57 43.33		
	21	J1 Jack-Open	6.75 /12	5-10-08	4-06-12	2 x 4	1-03-08	1-03-01 4-06-12	360.5 224.00		
	4	J2 Jack-Open	4.5 /12	4-10-12	2-11-13	2 x 4	1-04-12	4-07 2-02-08	61.34 37.33		
	7	J3 Jack-Open	5 /12	3-04-00	2-07-01	2 x 4	1-03-08	4-01 1-08-11	81.43 56.00		

TOTAL # TRUSS= **76** TOTAL BFT OF ALL TRUSSES= **2975.15** BFT. TOTAL WEIGHT OF ALL TRSSES **4700.12** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
4	Hardware	LUS24	

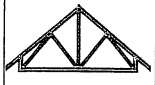
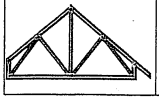
TOTAL NUMBER OF ITEMS= **11**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420185
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	1 of 3
Lot #:		Date:	08-04-2021
Elevation:	A OPT.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

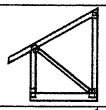
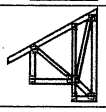
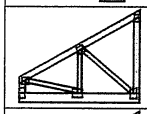
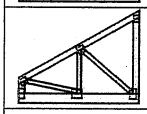
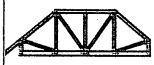
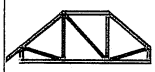
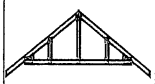
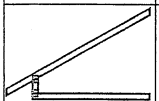
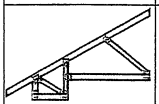
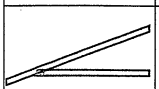
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	353.79 223.33		
	1 2-ply	T1Z Hip Girder	10 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	344.48 212.00		
	1	T2 Hip	10 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	133.07 86.67		
	1	T2S Hip	10 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	141.06 92.33		
	1	T3 Hip	10 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	147.41 93.50		
	1	T3S Hip	10 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	152.05 97.67		
	1	T4 Hip	10 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	156.88 99.67		
	1	T4S Hip	10 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	153.97 97.50		
	2	T5 Hip	10 /12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	308.44 194.33		
	8	T6 Hip	10 /12	31-00-00	10-02-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	1294.75 810.67		
	1	T8 Hip	10 /12	18-08-00	6-06-12	2 x 4	1-03-08	1-07-11 3-07-11	88.32 55.83		
	1	T9 Hip Girder	10 /12	18-08-00	5-05-04	2 x 4 2 x 6	1-03-08	1-07-11 3-07-11	101.37 64.17		
	1	T10 Common	10 /12	12-03-00	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	59.1 38.17		
	2	T10A Common	10 /12	12-00-00	6-08-15	2 x 4	1-03-08	1-10-03 1-07-11	113.28 72.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420185
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	2 of 3
Lot #:		Date:	08-04-2021
Elevation:	A OPT.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

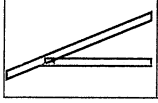
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T10S Roof Special	10 /12	**12-03-00**	6-08-15	2 x 4	1-03-08 1-03-08	1-07-11 7-11	65.91 43.33		
	1 2-ply	T12 Monopitch Girder	6.75 /1 2	**3-10-08**	5-05-04	2 x 4 2 x 6	1-03-08	3-03-01 5-05-04	51.23 33.00		
	3	T13 Monopitch	6.75 /1 2	**3-10-08**	5-05-04	2 x 4	1-03-08	2-03-01 5-05-04	87.81 60.00		
	1 2-ply	T14 Monopitch Girder	6.75 /1 2	**5-10-08**	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1 2-ply	T14Z Monopitch Girder	6.75 /1 2	**5-10-08**	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1	T15 Hip Girder	10 /12	**13-00-00**	4-06-12	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	67.79 43.67		
	1	T16 Hip	10 /12	**13-00-00**	5-01-12	2 x 4	1-03-08	1-07-11 1-07-11	56.92 37.50		
	1	G11 GABLE	10 /12	**7-05-00**	4-06-09	2 x 4	1-03-08 1-03-08	7-11 7-11	30.73 20.67		
	2	PB1 Piggyback	10 /12	**10-05-15**	1-01-08	2 x 4			60.53 40.33		
	2	PB2 Piggyback	10 /12	**10-05-15**	2-03-00	2 x 4			61.66 41.00		
	2	PB3 Piggyback	10 /12	**10-05-15**	3-04-08	2 x 4			64.57 43.33		
	13	J1 Jack-Open	6.75 /1 2	**5-10-08**	4-06-12	2 x 4	1-03-08	1-03-01 4-06-12	223.17 138.67		
	8	J1S Jack-Open	6.75 /1 2	**5-10-08**	4-06-12	2 x 4	1-03-08	1-03-01 3-06-12	176.1 118.67		
	4	J2 Jack-Open	4.5 /12	**4-10-12**	2-11-13	2 x 4	1-04-12	4-07 2-02-08	61.34 37.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420185
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	3 of 3
Lot #:		Date:	08-04-2021
Elevation:	A OPT.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	7	J3 Jack-Open	5 /12	**3-04-00**	2-07-01	2 x 4	1-03-08	4-01 1-08-11	81.43 56.00		

TOTAL # TRUSS= **76** TOTAL BFT OF ALL TRUSSES= **3027.67** BFT. TOTAL WEIGHT OF ALL TRSSES **4757.3** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
4	Hardware	LJS26DS	
4	Hardware	LUS24	

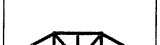
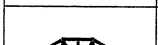
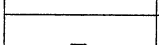
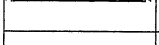
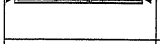
TOTAL NUMBER OF ITEMS= **11**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420186
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	1 of 3
Lot #:		Date:	08-04-2021
Elevation:	B STD.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

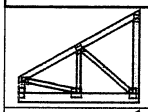
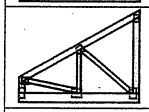
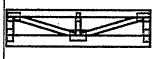
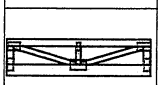
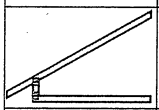
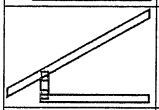
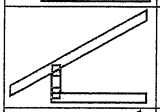
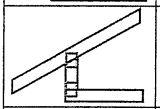
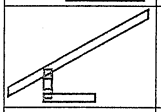
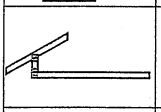
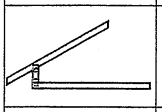
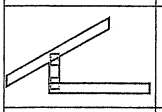
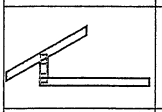
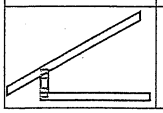
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T20 Hip Girder	6.75 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	320.2 193.33		
	1 2-ply	T20Z Hip Girder	6.75 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	320.2 193.33		
	2	T21 Hip	6.75 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	256.4 164.00		
	2	T22 Hip	6.75 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	262.3 165.00		
	2	T23 Hip	6.75 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	277.51 178.67		
	2	T24 Hip	6.75 /12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	277.97 175.67		
	8	T25 Common	6.75 /12	31-00-00	9-11-11	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	1078.84 666.67		
	1	T26 Hip Girder	6.75 /12	18-08-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	89.3 55.67		
	1	T27 Common	6.75 /12	12-00-00	5-05-06	2 x 4	1-03-08	1-03-01 2-10-12	50.29 32.00		
	1	T28 Hip Girder	6.75 /12	12-00-00	4-06-12	2 x 4 2 x 6	1-03-08	1-03-01 2-10-12	59.94 37.83		
	2	T29 Common	6.75 /12	7-05-00	3-04-02	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	62.96 40.33		
	1	T30 Hip Girder	6.75 /12	7-05-00	2-11-05	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	38 26.17		
	1	T31 Common	6.75 /12	13-00-00	4-08-11	2 x 4	1-03-08	1-03-01 1-03-01	51.66 33.50		
	1	T32 Hip Girder	6.75 /12	13-00-00	3-07-03	2 x 4 2 x 6	1-03-08	1-03-01 1-03-01	58.21 36.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420186
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	2 of 3
Lot #:		Date:	08-04-2021
Elevation:	B STD.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T33 Monopitch Girder	6.75 /12	**5-10-08**	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1 2-ply	T33Z Monopitch Girder	6.75 /12	**5-10-08**	4-06-12	2 x 4 2 x 6		1-03-01 4-06-11	60.07 38.00		
	1 2-ply	T35 Flat Girder	0 /12	**5-10-08**	1-06-00	2 x 4 2 x 6		1-06-00 1-06-00	47.73 29.67		
	1 2-ply	T36 Flat Girder	0 /12	**5-10-08**	1-06-00	2 x 4 2 x 6		1-06-00 1-06-00	47.73 29.67		
	12	J20 Jack-Open	6.75 /12	**5-10-08**	4-06-12	2 x 4	1-03-08	1-03-01 4-06-12	206 128.00		
	4	J21 Jack-Open	6.75 /12	**4-02-00**	3-07-03	2 x 4	1-03-08	1-03-01 3-07-03	51.94 34.67		
	2	J22 Jack-Open	6.75 /12	**3-00-00**	2-11-05	2 x 4	1-03-08	1-03-01 2-11-05	20.25 13.33		
	8	C21 Jack-Open	6.75 /12	**1-10-08**	2-03-02	2 x 4	1-03-08	1-03-01 2-03-12	58.05 37.33		
	3	C22 Jack-Open	6.75 /12	**1-10-08**	3-04-10	2 x 4	1-03-08 1-10-15	1-03-01 2-03-12	29.62 18.00		
	4	C23 Jack-Open	6.75 /12	**1-09-07**	2-03-02	2 x 4	1-03-08 4-01-01	1-03-01 2-03-02	47.27 29.33		
	4	C24 Jack-Open	6.75 /12	**3-09-07**	3-04-10	2 x 4	1-03-08 2-01-01	1-03-01 3-04-10	57.74 34.67		
	2	C25 Jack-Open	6.75 /12	**1-09-07**	2-03-02	2 x 4	1-03-08 1-02-09	1-03-01 2-03-02	17.08 10.67		
	2	C26 Jack-Open	6.75 /12	**1-09-07**	2-03-02	2 x 4	1-03-08 2-04-09	1-03-01 2-03-02	19.74 13.33		
	5	C27 Jack-Open	6.75 /12	**3-09-07**	3-04-10	2 x 4	1-03-08 4-09	1-03-01 3-04-10	62.43 40.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420186
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	3 of 3
Lot #:		Date:	08-04-2021
Elevation:	B STD.	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **82** TOTAL BFT OF ALL TRUSSES= **2493.51** BFT. TOTAL WEIGHT OF ALL TRSSES **3989.5** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
3	Hardware	LJS26DS	
4	Hardware	HGUS26-2	

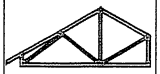
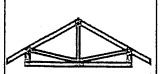
TOTAL NUMBER OF ITEMS= **9**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420187
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	1 of 3
Lot #:		Date:	08-04-2021
Elevation:	B OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

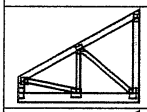
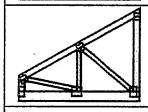
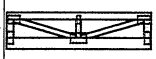
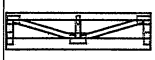
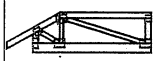
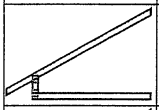
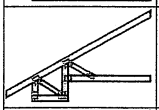
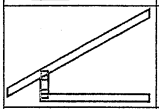
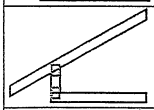
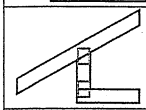
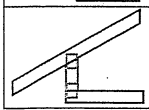
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T20S Hip Girder	6.75 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	331.49 208.00		
	1 2-ply	T20Z Hip Girder	6.75 /12	31-00-00	4-06-12	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	320.2 193.33		
	1	T21 Hip	6.75 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	124.56 77.67		
	1	T21S Hip	6.75 /12	31-00-00	5-08-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	138.06 87.50		
	1	T22 Hip	6.75 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	131.15 82.50		
	1	T22S Hip	6.75 /12	31-00-00	6-09-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	138.66 87.00		
	1	T23 Hip	6.75 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	138.76 89.33		
	1	T23S Hip	6.75 /12	31-00-00	7-11-04	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	144.33 92.83		
	2	T24 Hip	6.75 /12	31-00-00	9-00-12	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	277.97 175.67		
	8	T25 Common	6.75 /12	31-00-00	9-11-11	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	1078.95 666.67		
	1	T26 Hip Girder	6.75 /12	18-08-00	4-06-12	2 x 4 2 x 6	1-03-08	1-03-01 1-03-01	87.61 54.33		
	1	T27 Common	6.75 /12	12-00-00	5-05-06	2 x 4	1-03-08	1-03-01 2-10-12	50.29 32.00		
	1	T28 Hip Girder	6.75 /12	12-00-00	4-06-12	2 x 4 2 x 6	1-03-08	1-03-01 2-10-12	61.82 39.50		
	2	T29 Common	6.75 /12	7-05-00	3-04-02	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	62.96 40.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420187
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	2 of 3
Lot #:		Date:	08-04-2021
Elevation:	B OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

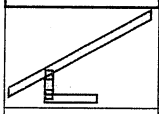
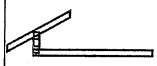
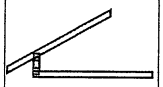
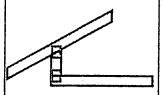
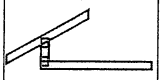
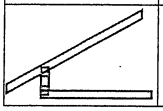
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T30 Hip Girder	6.75 /12	**7-05-00**	2-11-05	2 x 4 2 x 6	1-03-08 1-03-08	1-03-01 1-03-01	38 26.17		
	1	T31 Hip	6.75 /12	**13-00-00**	4-08-11	2 x 4	1-03-08 1-03-08	1-03-01 1-03-01	53.51 33.83		
	1	T32 Hip Girder	6.75 /12	**13-00-00**	3-07-03	2 x 4 2 x 6	1-03-08	1-03-01 1-03-01	58.21 36.67		
	1 2-ply	T33 Monopitch Girder	6.75 /12	**5-10-08**	4-06-12	2 x 4 2 x 6		1-03-01 4-06-12	60.07 38.00		
	1 2-ply	T33Z Monopitch Girder	6.75 /12	**5-10-08**	4-06-12	2 x 4 2 x 6		1-03-01 4-06-11	60.07 38.00		
	1 2-ply	T35 Flat Girder	0 /12	**5-10-08**	1-06-00	2 x 4 2 x 6		1-06-00 1-06-00	47.73 29.67		
	1 2-ply	T36 Flat Girder	0 /12	**5-10-08**	1-06-00	2 x 4 2 x 6		1-06-00 1-06-00	47.73 29.67		
	1	T37 Half Hip Girder	6.75 /12	**5-10-08**	2-00-01	2 x 4 2 x 6	1-03-08	1-03-01 2-00-01	26.93 18.50		
	6	J20 Jack-Open	6.75 /12	**5-10-08**	4-06-12	2 x 4	1-03-08	1-03-01 4-06-12	103 64.00		
	7	J20S Jack-Open	6.75 /12	**5-10-08**	4-06-12	2 x 4	1-03-08	1-03-01 3-06-12	150.02 102.67		
	4	J21 Jack-Open	6.75 /12	**4-02-00**	3-07-03	2 x 4	1-03-08	1-03-01 3-07-03	51.94 34.67		
	2	J22 Jack-Open	6.75 /12	**3-00-00**	2-11-05	2 x 4	1-03-08	1-03-01 2-11-05	20.25 13.33		
	2	J23 Jack-Open	6.75 /12	**1-04-00**	2-00-01	2 x 4	1-03-08	1-03-01 2-00-01	12.09 9.33		
	7	C21 Jack-Open	6.75 /12	**1-09-07**	2-03-02	2 x 4	1-03-08 1-01	1-03-01 2-03-02	50.8 32.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	52430
Builder:	ROYAL PINE HOMES	PlanLog:	204990
Project:	VALES OF CASTLEMORE NORTH	Layout ID:	420187
Location:	BRAMPTON	Ref #	
Model:	MODEL 40-1	Page:	3 of 3
Lot #:		Date:	08-04-2021
Elevation:	B OPT	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	5	C22 Jack-Open	6.75 /12	1-10-08	3-04-10	2 x 4	1-03-08 1-10-15	1-03-01 2-03-12	49.37 30.00		
	3	C23 Jack-Open	6.75 /12	1-09-07	2-03-02	2 x 4	1-03-08 4-01-01	1-03-01 2-03-02	35.45 22.00		
	3	C24 Jack-Open	6.75 /12	3-09-07	3-04-10	2 x 4	1-03-08 2-01-01	1-03-01 3-04-10	43.3 26.00		
	2	C25 Jack-Open	6.75 /12	1-09-07	2-03-02	2 x 4	1-03-08 1-02-09	1-03-01 2-03-02	17.08 10.67		
	2	C26 Jack-Open	6.75 /12	1-09-07	2-03-02	2 x 4	1-03-08 2-04-09	1-03-01 2-03-02	19.74 13.33		
	2	C27 Jack-Open	6.75 /12	3-09-07	3-04-10	2 x 4	1-03-08 4-09	1-03-01 3-04-10	24.97 16.00		

TOTAL # TRUSS= **82** TOTAL BFT OF ALL TRUSSES= **2551.84** BFT. TOTAL WEIGHT OF ALL TRSSES **4057.05** LBS

HARDWARE

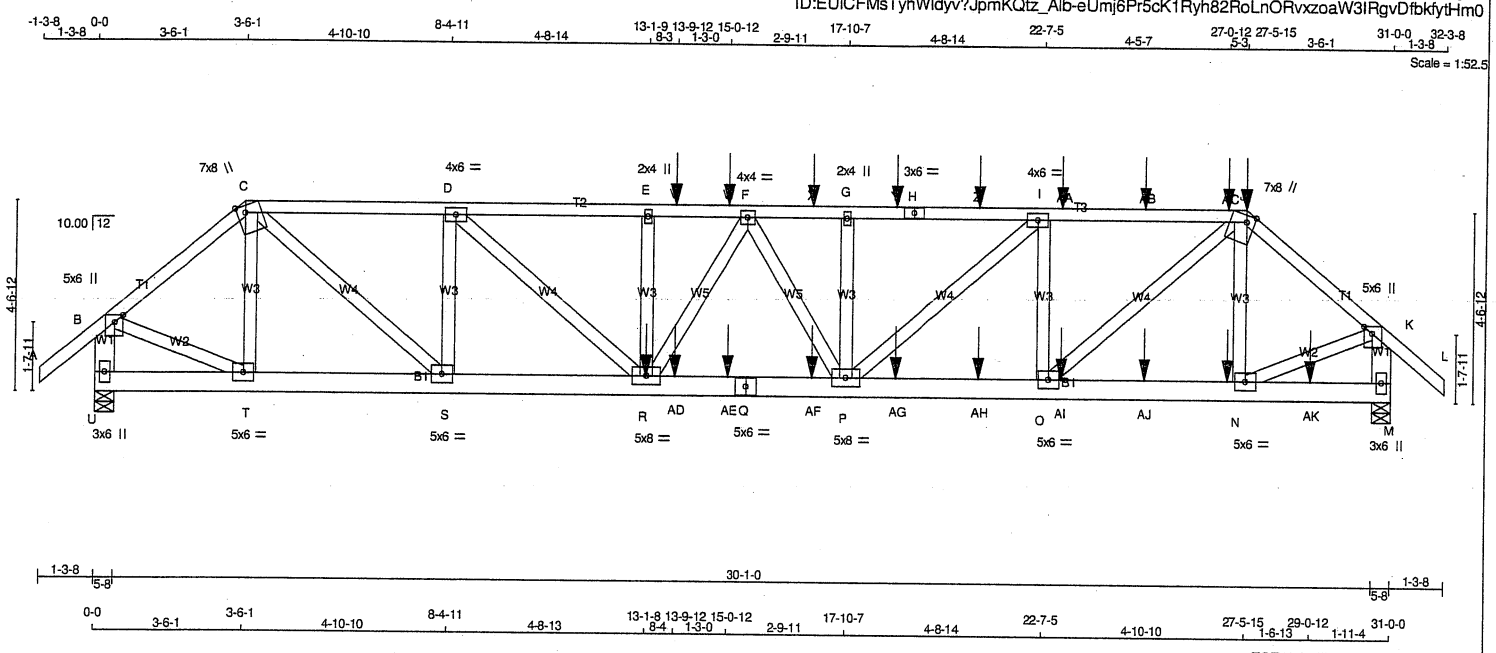
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
3	Hardware	LJS26DS	
4	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **10**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T1	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 29 07:33:17 2021 Page 1
ID:EUlCFMsTyhWldyv?JpmKQtz_Alb-eUmj6Pr5cK1Ryh82RoLnORvxzoaW3IRgvDfbkfytHm0



TOTAL WEIGHT = 2 X 172 = 344 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- H	1	12	SIDE(0.0)
H- J	1	12	SIDE(61.0)
J- L	1	12	TOP
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(183.1)
Q- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
R- E	1	6	SIDE(236.0)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2125083

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3038	0	3038	0	0	5-8	5-8
M	3332	0	3332	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2152	1392 / 0	0 / 0	0 / 0	0 / 0	760 / 0	0 / 0
M	2361	1525 / 0	0 / 0	0 / 0	0 / 0	835 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.06 (1)	10.00
B- C	-3058 / 0	-84.9	-84.9	0.14 (1)	5.16
C- D	-5029 / 0	-84.9	-84.9	0.28 (1)	4.10
D- E	-7024 / 0	-84.9	-84.9	0.40 (1)	3.44
E- V	-7024 / 0	-84.9	-84.9	0.23 (1)	3.56
V- W	-7024 / 0	-84.9	-84.9	0.23 (1)	3.56
W- F	-7024 / 0	-84.9	-84.9	0.23 (1)	3.56
F- X	-6496 / 0	-84.9	-84.9	0.25 (1)	3.66
X- G	-6496 / 0	-84.9	-84.9	0.25 (1)	3.66
G- Y	-6496 / 0	-84.9	-84.9	0.48 (1)	3.45
Y- H	-6496 / 0	-84.9	-84.9	0.48 (1)	3.45
H- Z	-6496 / 0	-84.9	-84.9	0.48 (1)	3.45
Z- I	-6496 / 0	-84.9	-84.9	0.48 (1)	3.45
I-AA	-5072 / 0	-84.9	-84.9	0.40 (1)	3.94
AA-AB	-5072 / 0	-84.9	-84.9	0.40 (1)	3.94
AB-AC	-5072 / 0	-84.9	-84.9	0.40 (1)	3.94
AC- J	-5072 / 0	-84.9	-84.9	0.40 (1)	3.94
J- K	-3374 / 0	-84.9	-84.9	0.15 (1)	4.96
K- L	0 / 38	-84.9	-84.9	0.06 (1)	10.00
U- B	-3016 / 0	0.0	0.0	0.11 (1)	7.81
M- K	-3301 / 0	0.0	0.0	0.12 (1)	7.64
U- T	0 / 0	-18.5	-18.5	0.02 (4)	10.00
T- S	0 / 2318	-18.5	-18.5	0.17 (1)	10.00
S- R	0 / 5029	-18.5	-18.5	0.37 (1)	10.00
R-AD	0 / 6845	-18.5	-18.5	0.51 (1)	10.00
AD-AE	0 / 6845	-18.5	-18.5	0.51 (1)	10.00
AE- Q	0 / 6845	-18.5	-18.5	0.51 (1)	10.00
Q-AF	0 / 6845	-18.5	-18.5	0.51 (1)	10.00
AF- P	0 / 6845	-18.5	-18.5	0.51 (1)	10.00
P-AG	0 / 5072	-18.5	-18.5	0.37 (1)	10.00
AG-AH	0 / 5072	-18.5	-18.5	0.37 (1)	10.00
AH- O	0 / 5072	-18.5	-18.5	0.37 (1)	10.00
O-AI	0 / 2560	-18.5	-18.5	0.19 (1)	10.00
AI-AJ	0 / 2560	-18.5	-18.5	0.19 (1)	10.00
AJ- N	0 / 2560	-18.5	-18.5	0.19 (1)	10.00
N-AK	0 / 0	-18.5	-18.5	0.03 (4)	10.00
AK- M	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-681 / 0	0.07 (1)
N- J	-700 / 0	0.08 (1)
B- T	0 / 2470	0.22 (1)
N- K	0 / 2725	0.24 (1)
O- J	0 / 3301	0.29 (1)
C- S	0 / 3561	0.31 (1)
O- I	-2004 / 0	0.21 (1)
S- D	-2216 / 0	0.24 (1)
P- I	0 / 1900	0.17 (1)
D- R	0 / 2661	0.24 (1)
P- G	-474 / 0	0.05 (1)
R- E	-377 / 0	0.04 (1)
R- F	0 / 364	0.03 (1)
F- P	-707 / 0	0.10 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	27-5-15	-35	-35	---	TOP	VERT	DEAD	---	C1
J	27-5-15	-133	-133	---	TOP	VERT	SNOW	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.48/1.00 (G-I:1) , BC=0.51/1.00 (P-R:1) , WB=0.31/1.00 (C-S:1) , SSI=0.18/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI) MAX	MIN	SECTION (PLI) MAX	MIN	MAX	MIN
MT20		650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (P) (INPUT = 0.90)
JSI METAL= 0.65 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420178	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:EUICFMsTyhWldyv?JpmKQtz_Alb-eUmj6Pr5cK1Ryh82RoLnORvxzoaW3IRgvDfbkfytHm0

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	27-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	13-1-8	-1135	-1135	---	FRONT	VERT	TOTAL	---	C1
V	13-9-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
W	15-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
X	17-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Y	19-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
Z	21-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AA	23-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AB	25-0-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
AC	27-0-12	-87	-87	---	FRONT	VERT	TOTAL	---	C1
AD	13-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AE	15-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AF	17-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AG	19-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AH	21-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AI	23-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	29-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2125083

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T1Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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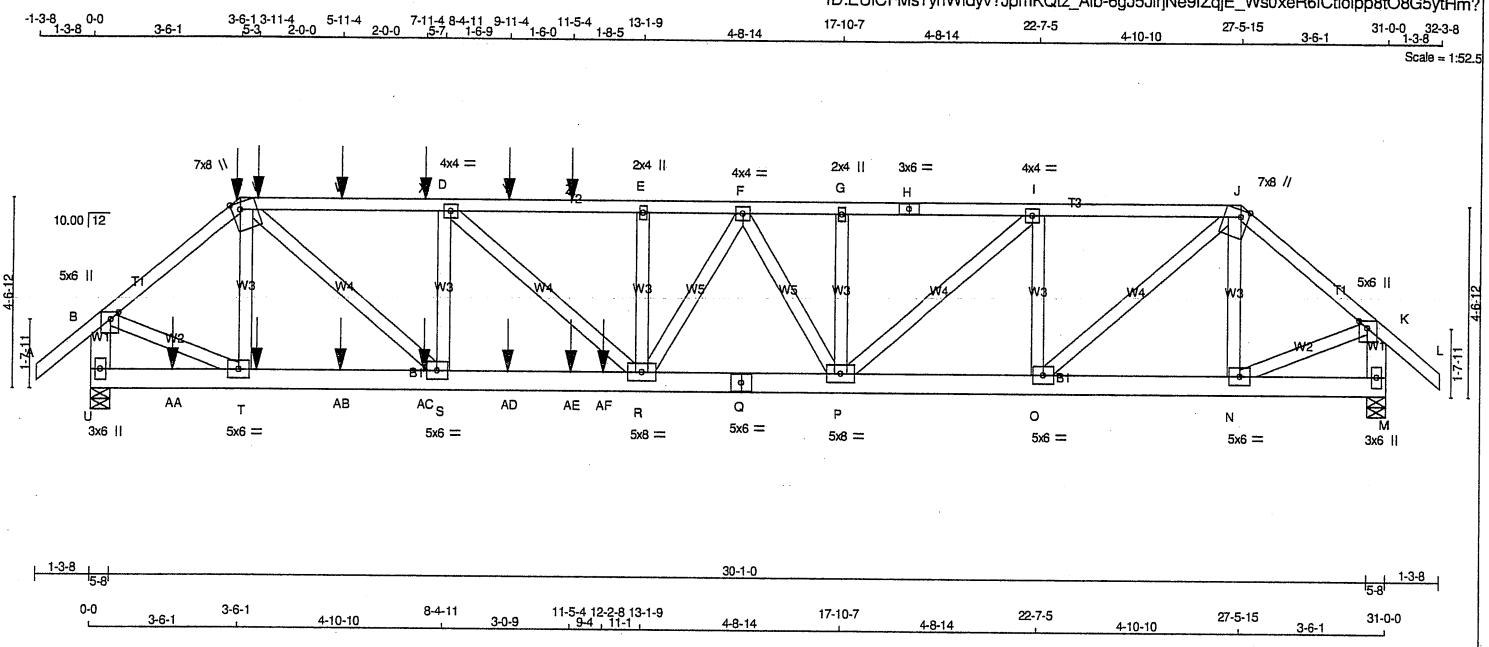

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- H	1	12	SIDE(61.0)
H- J	1	12	TOP
J- L	1	12	TOP
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(183.1)
Q- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2125084

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3379	0	3379	0	0	5-8	5-8
M	2509	0	2509	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2392	1555 / 0	0 / 0	0 / 0	0 / 0	837 / 0	0 / 0
M	1777	1153 / 0	0 / 0	0 / 0	0 / 0	624 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.06 (1)	10.00
B- C	-3407 / 0	-84.9	-84.9	0.15 (1)	4.94
C- V	-5224 / 0	-84.9	-84.9	0.42 (1)	3.87
V- W	-5224 / 0	-84.9	-84.9	0.42 (1)	3.87
W- X	-5224 / 0	-84.9	-84.9	0.42 (1)	3.87
X- D	-5224 / 0	-84.9	-84.9	0.42 (1)	3.87
D- Y	-6392 / 0	-84.9	-84.9	0.48 (1)	3.47
Y- Z	-6392 / 0	-84.9	-84.9	0.48 (1)	3.47
Z- E	-6392 / 0	-84.9	-84.9	0.48 (1)	3.47
E- F	-6392 / 0	-84.9	-84.9	0.24 (1)	3.70
F- G	-5353 / 0	-84.9	-84.9	0.16 (1)	4.08
G- H	-5353 / 0	-84.9	-84.9	0.30 (1)	3.97
H- I	-5353 / 0	-84.9	-84.9	0.30 (1)	3.97
I- J	-3977 / 0	-84.9	-84.9	0.24 (1)	4.55
J- K	-2467 / 0	-84.9	-84.9	0.13 (1)	5.63
K- L	0 / 38	-84.9	-84.9	0.06 (1)	10.00
U- B	-3331 / 0	0.0	0.0	0.12 (1)	7.61
M- K	-2485 / 0	0.0	0.0	0.09 (1)	7.81
U-AA	0 / 0	-18.5	-18.5	0.02 (4)	10.00
AA- T	0 / 0	-18.5	-18.5	0.02 (4)	10.00
T-AB	0 / 2582	-18.5	-18.5	0.25 (1)	10.00
AB-AC	0 / 2582	-18.5	-18.5	0.25 (1)	10.00
AC- S	0 / 2582	-18.5	-18.5	0.25 (1)	10.00
S-AD	0 / 5224	-18.5	-18.5	0.64 (1)	10.00
AD-AE	0 / 5224	-18.5	-18.5	0.64 (1)	10.00
AE-AF	0 / 5224	-18.5	-18.5	0.64 (1)	10.00
AF- R	0 / 5224	-18.5	-18.5	0.64 (1)	10.00
R- Q	0 / 5905	-18.5	-18.5	0.53 (1)	10.00
Q- P	0 / 5905	-18.5	-18.5	0.53 (1)	10.00
P- O	0 / 3977	-18.5	-18.5	0.32 (1)	10.00
O- N	0 / 1870	-18.5	-18.5	0.13 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-788 / 0	0.08 (1)
N- J	-541 / 0	0.06 (1)
B- T	0 / 2752	0.24 (1)
N- K	0 / 1992	0.18 (1)
O- J	0 / 2768	0.24 (1)
C- S	0 / 3471	0.31 (1)
O- I	-1673 / 0	0.18 (1)
S- D	-1795 / 0	0.19 (1)
P- I	0 / 1836	0.16 (1)
D- R	0 / 1559	0.14 (1)
P- G	-311 / 0	0.03 (1)
R- E	-406 / 0	0.04 (1)
R- F	0 / 989	0.09 (1)
F- P	-1119 / 0	0.15 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-6-1	-35	-35	---	TOP	VERT	DEAD	---	C1
C	3-6-1	-133	-133	---	TOP	VERT	SNOW	---	C1
T	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
V	3-11-4	-87	-87	---	BACK	VERT	TOTAL	---	C1
W	5-11-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
X	7-11-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
Y	9-11-4	-71	-71	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.48/1.00 (D-E:1) , BC=0.64/1.00 (R-S:1) , WB=0.31/1.00 (C-S:1) , SSI=0.44/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.52 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420178	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:EUlCFMsTyhWldyv?JpmKQtz_Alb-6qJ5JlrjNe9IZqjE_Ws0xeR6iCtiolpp8tO8G5ytHm?

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D, F, I						
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Z	11-5-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
AA	1-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AB	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	7-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	9-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	11-5-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AF	12-2-8	-1067	-1067	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125084

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420185	T1S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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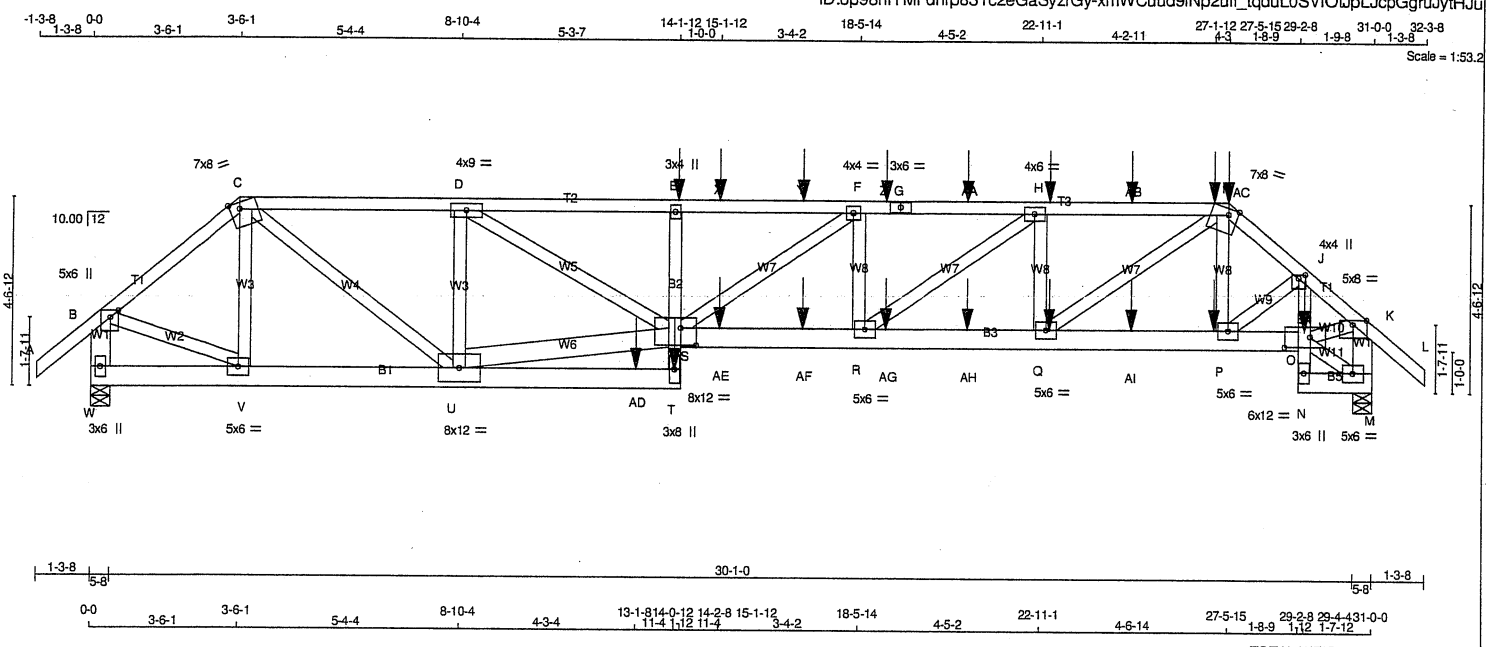

TOTAL WEIGHT = 2 X 176 = 352 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - E	2x4	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- G	1	12	SIDE(61.0)
G- I	1	12	SIDE(61.0)
I- L	1	12	SIDE(61.0)
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(122.0)
S- O	2	12	SIDE(183.1)
N- M	2	12	TOP
T- E	1	12	TOP
J- N	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2125102

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3171	0	3171	0	0	5-8	5-8
M	3687	0	3687	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2247	1452 / 0	0 / 0	0 / 0	0 / 0	795 / 0	0 / 0
M	2612	1689 / 0	0 / 0	0 / 0	0 / 0	923 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 38	-84.9	-84.9	0.06 (1)	10.00	V- C	-831 / 0	0.09 (1)
B- C	-3176 / 0	-84.9	-84.9	0.14 (1)	5.08	C- U	0 / 4047	0.36 (1)
C- D	-5570 / 0	-84.9	-84.9	0.37 (1)	3.86	U- D	-3007 / 0	0.32 (1)
D- E	-9758 / 0	-84.9	-84.9	0.70 (1)	2.72	U- S	0 / 5509	0.36 (1)
E- X	-9834 / 0	-84.9	-84.9	0.59 (1)	2.75	D- S	0 / 4809	0.43 (1)
X- Y	-9834 / 0	-84.9	-84.9	0.59 (1)	2.75	P- I	0 / 127	0.02 (4)
Y- F	-9834 / 0	-84.9	-84.9	0.59 (1)	2.75	P- J	0 / 183	0.02 (1)
F- Z	-9050 / 0	-84.9	-84.9	0.56 (1)	2.89	B- V	0 / 2565	0.23 (1)
Z- G	-9050 / 0	-84.9	-84.9	0.56 (1)	2.89	O- M	-122 / 0	0.01 (1)
G-AA	-9050 / 0	-84.9	-84.9	0.56 (1)	2.89	O- K	0 / 3523	0.31 (1)
AA- H	-9050 / 0	-84.9	-84.9	0.56 (1)	2.89	Q- I	0 / 4070	0.36 (1)
H-AB	-7062 / 0	-84.9	-84.9	0.43 (1)	3.39	S- F	0 / 966	0.09 (1)
AB-AC	-7062 / 0	-84.9	-84.9	0.43 (1)	3.39	Q- H	-2067 / 0	0.16 (1)
AC- I	-7062 / 0	-84.9	-84.9	0.43 (1)	3.39	R- F	-1166 / 0	0.09 (1)
I- J	-4837 / 0	-84.9	-84.9	0.13 (1)	4.30	R- H	0 / 2448	0.22 (1)
J- K	-4605 / 0	-84.9	-84.9	0.13 (1)	4.38			
K- L	0 / 38	-84.9	-84.9	0.06 (1)	10.00			
W- B	-3123 / 0	0.0	0.0	0.12 (1)	7.80			
M- K	-3608 / 0	0.0	0.0	0.13 (1)	7.39			
W- V	0 / 0	-18.5	-18.5	0.02 (1)	10.00			
V- U	0 / 2403	-18.5	-18.5	0.29 (1)	10.00			
U-AD	0 / 242	-18.5	-18.5	0.46 (1)	10.00			
AD- T	0 / 242	-18.5	-18.5	0.46 (1)	10.00			
T- S	0 / 1340	0.0	0.0	0.30 (1)	10.00			
S- E	-570 / 0	0.0	0.0	0.19 (1)	7.81			
S-AE	0 / 9050	-18.5	-18.5	0.66 (1)	10.00			
AE-AF	0 / 9050	-18.5	-18.5	0.66 (1)	10.00			
AF- R	0 / 9050	-18.5	-18.5	0.66 (1)	10.00			
R-AG	0 / 7062	-18.5	-18.5	0.54 (1)	10.00			
AG-AH	0 / 7062	-18.5	-18.5	0.54 (1)	10.00			
AH- Q	0 / 7062	-18.5	-18.5	0.54 (1)	10.00			
Q-AI	0 / 3714	-18.5	-18.5	0.28 (1)	10.00			
AI- P	0 / 3714	-18.5	-18.5	0.28 (1)	10.00			
P- O	0 / 3564	-18.5	-18.5	0.27 (1)	10.00			
N- O	0 / 17	0.0	0.0	0.08 (1)	10.00			
O- J	-371 / 0	0.0	0.0	0.08 (1)	7.81			
N- M	0 / 104	-18.5	-18.5	0.01 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	14-1-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
H	23-1-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 814 (0.46")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.66/1.00 (R-S:1) , WB=0.43/1.00 (D-S:1) , SSI=0.43/1.00 (T-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420185	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	7.0	8.0	Edge	2.75
D	TMWW-t	MT20	4.0	9.0		
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW-m	MT20	7.0	8.0	Edge	2.75
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-t	MT20	6.0	12.0	3.00	7.50
P, Q, R, V						
P	BMWW-t	MT20	5.0	6.0		
S	BVMWWW-t	MT20	8.0	12.0	5.00	4.50
T	BMV+p	MT20	3.0	8.0		
U	BMWWW-t	MT20	8.0	12.0		
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	27-5-15	-35	-35	---	FRONT	VERT	DEAD	---	C1
I	27-5-15	-133	-133	---	FRONT	VERT	SNOW	---	C1
O	29-4-4	-70	-70	---	FRONT	VERT	TOTAL	---	C1
P	27-1-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
Q	23-1-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
T	14-0-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
X	15-1-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
Y	17-1-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
Z	19-1-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
AA	21-1-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
AB	25-1-12	-69	-69	---	FRONT	VERT	TOTAL	---	C1
AC	27-1-12	-88	-88	---	FRONT	VERT	TOTAL	---	C1
AD	13-1-8	-1136	-1136	---	FRONT	VERT	TOTAL	---	C1
AE	15-1-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AF	17-1-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AG	19-1-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AH	21-1-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1
AI	25-1-12	-64	-64	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.52 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125102

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T2	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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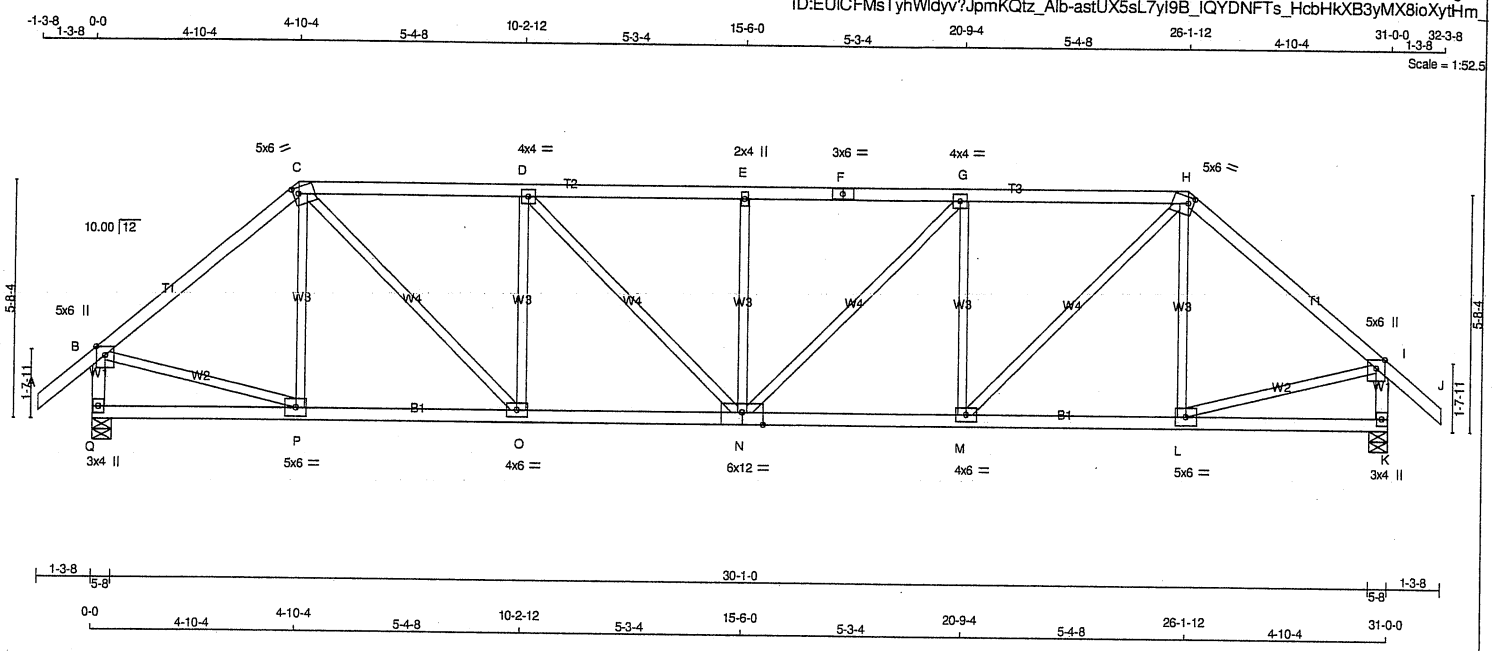

TOTAL WEIGHT = 2 X 133 = 266 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	1.50
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BSWWW-t	MT20	6.0	12.0	Edge	6.00
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125085

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1720	0	1720	0	0	5-8	5-8
K	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)				FR-TO		
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	P- C	-220 / 0	0.11 (1)
B- C	-1611 / 0	-84.9	-84.9	0.44 (1)	4.74	C- O	0 / 1214	0.27 (1)
C- D	-2089 / 0	-84.9	-84.9	0.45 (1)	4.23	O- D	-754 / 0	0.37 (1)
D- E	-2331 / 0	-84.9	-84.9	0.47 (1)	4.02	D- N	0 / 345	0.08 (1)
E- F	-2331 / 0	-84.9	-84.9	0.47 (1)	4.02	N- E	-412 / 0	0.20 (1)
F- G	-2331 / 0	-84.9	-84.9	0.47 (1)	4.02	N- G	0 / 345	0.08 (1)
G- H	-2089 / 0	-84.9	-84.9	0.45 (1)	4.23	M- G	-754 / 0	0.37 (1)
H- I	-1611 / 0	-84.9	-84.9	0.44 (1)	4.74	M- H	0 / 1214	0.27 (1)
I- J	0 / 38	-84.9	-84.9	0.12 (1)	10.00	L- H	-220 / 0	0.11 (1)
Q- B	-1683 / 0	0.0	0.0	0.18 (1)	6.40	B- P	0 / 1277	0.29 (1)
K- I	-1683 / 0	0.0	0.0	0.18 (1)	6.40	L- I	0 / 1277	0.29 (1)
Q- P	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
P- O	0 / 1231	-18.5	-18.5	0.26 (1)	10.00			
O- N	0 / 2090	-18.5	-18.5	0.40 (1)	10.00			
N- M	0 / 2090	-18.5	-18.5	0.40 (1)	10.00			
M- L	0 / 1231	-18.5	-18.5	0.26 (1)	10.00			
L- K	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.47/1.00 (E-G:1) , BC=0.40/1.00 (M-N:1) , WB=0.37/1.00 (G-M:1) , SSI=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.60 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420185	T2S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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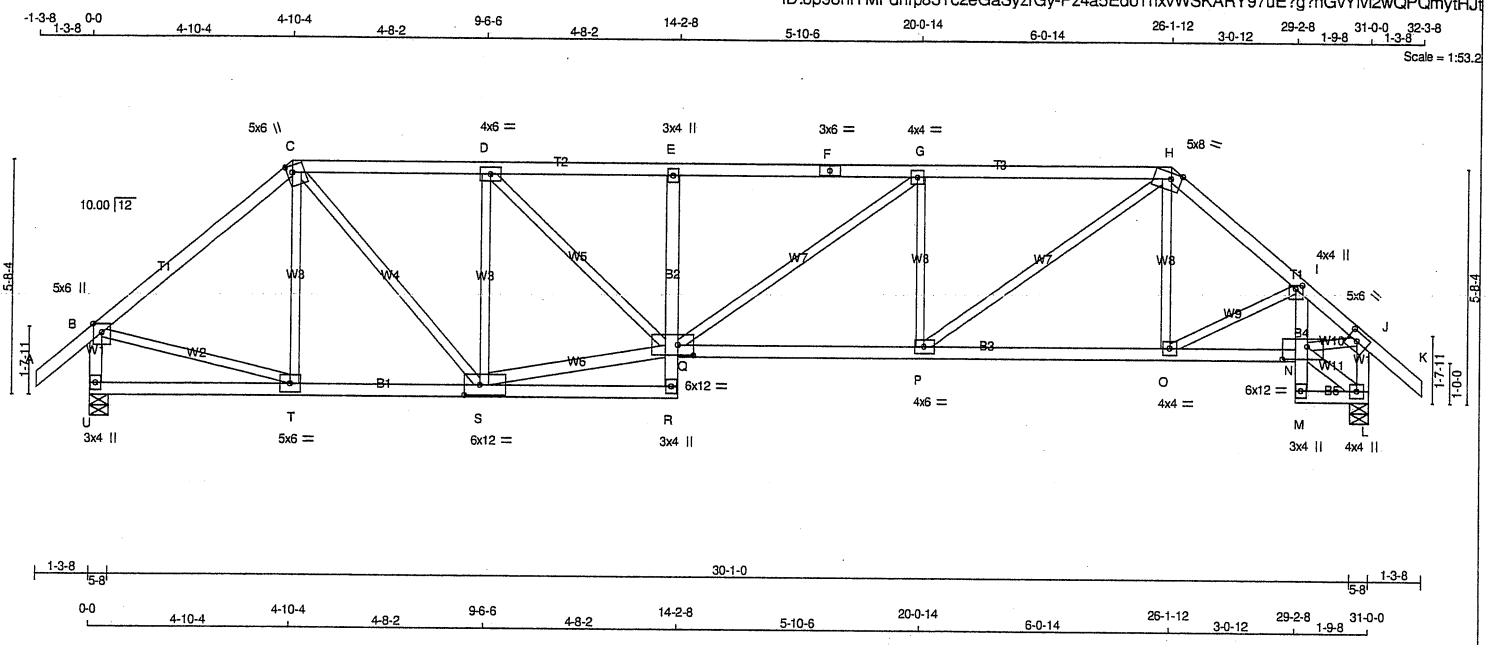

TOTAL WEIGHT = 141 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMVW1+p	MT20	4.0	4.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-I	MT20	6.0	12.0	3.75	7.25
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BVMWWW-I	MT20	6.0	12.0	3.00	4.50
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	12.0	3.00	4.50
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2125103

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1717	0	1717	0	0	5-8	5-8
L	1722	0	1722	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1220	788 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1607 / 0	-84.9	-84.9	0.43 (1)	4.74
C- D	-1983 / 0	-84.9	-84.9	0.31 (1)	4.49
D- E	-2814 / 0	-84.9	-84.9	0.38 (1)	3.82
E- F	-2830 / 0	-84.9	-84.9	0.70 (1)	3.46
F- G	-2830 / 0	-84.9	-84.9	0.70 (1)	3.46
G- H	-2655 / 0	-84.9	-84.9	0.68 (1)	3.58
H- I	-1958 / 0	-84.9	-84.9	0.18 (1)	4.66
I- J	-2053 / 0	-84.9	-84.9	0.16 (1)	4.59
J- K	0 / 38	-84.9	-84.9	0.12 (1)	10.00
U- B	-1679 / 0	0.0	0.0	0.18 (1)	6.41
L- J	-1672 / 0	0.0	0.0	0.18 (1)	6.42
U- T	0 / 0	-18.5	-18.5	0.10 (4)	10.00
T- S	0 / 1227	-18.5	-18.5	0.25 (1)	10.00
S- R	0 / 68	-18.5	-18.5	0.09 (4)	10.00
R- Q	0 / 39	0.0	0.0	0.10 (1)	10.00
Q- E	-416 / 0	0.0	0.0	0.12 (1)	7.81
Q- P	0 / 2655	-18.5	-18.5	0.51 (1)	10.00
P- O	0 / 1496	-18.5	-18.5	0.31 (1)	10.00
O- N	0 / 1601	-18.5	-18.5	0.32 (1)	10.00
M- N	0 / 17	0.0	0.0	0.08 (1)	10.00
N- I	-87 / 0	0.0	0.0	0.08 (1)	7.81
M- L	0 / 50	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-226 / 0	0.11 (1)
C- S	0 / 1161	0.26 (1)
S- D	-1212 / 0	0.60 (1)
S- Q	0 / 1975	0.32 (1)
D- Q	0 / 1136	0.26 (1)
Q- G	0 / 216	0.05 (1)
P- G	-713 / 0	0.23 (1)
P- H	0 / 1428	0.32 (1)
O- H	0 / 95	0.03 (4)
O- I	-119 / 0	0.03 (1)
B- T	0 / 1273	0.29 (1)
N- L	-61 / 0	0.01 (1)
N- J	0 / 1570	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.70/1.00 (E-G:1) , BC=0.51/1.00 (P-Q:1) , WB=0.60/1.00 (D-S:1) , SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.60 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T3	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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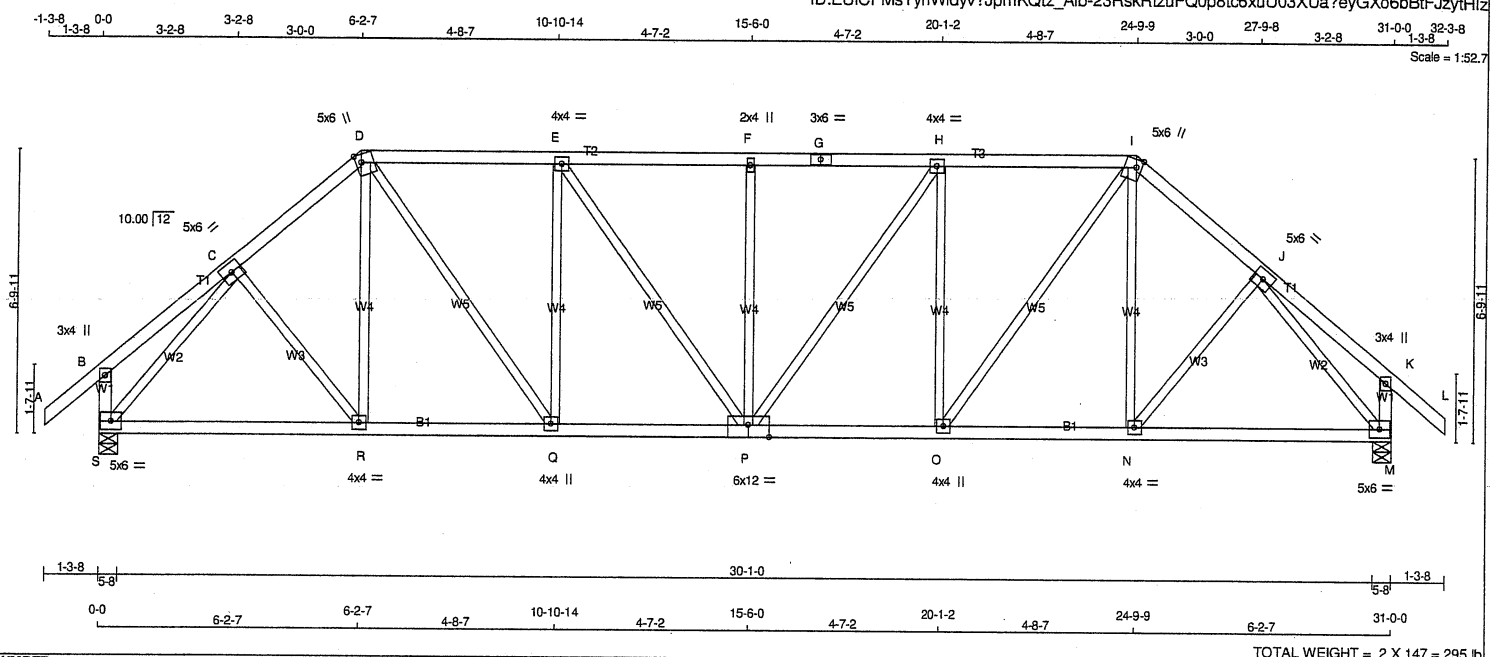

TOTAL WEIGHT = 2 X 147 = 295 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.25	1.50
J	TMWW-t	MT20	5.0	6.0		
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	4.0		
P	BSWWW-t	MT20	6.0	12.0	Edge	6.00
Q	BMWW+t	MT20	4.0	4.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1720	0	1720	0	0	5-8	5-8
M	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
M	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	C- R	0 / 127	0.03 (1)
B- C	0 / 17	-84.9	-84.9	0.11 (1)	10.00	R- D	0 / 70	0.02 (4)
C- D	-1628 / 0	-84.9	-84.9	0.15 (1)	5.05	D- Q	0 / 915	0.21 (1)
D- E	-1770 / 0	-84.9	-84.9	0.32 (1)	4.68	Q- E	-658 / 0	0.51 (1)
E- F	-1923 / 0	-84.9	-84.9	0.33 (1)	4.52	E- P	0 / 264	0.06 (1)
F- G	-1923 / 0	-84.9	-84.9	0.33 (1)	4.52	P- F	-358 / 0	0.28 (1)
G- H	-1923 / 0	-84.9	-84.9	0.33 (1)	4.52	P- H	0 / 264	0.06 (1)
H- I	-1770 / 0	-84.9	-84.9	0.32 (1)	4.68	O- H	-658 / 0	0.51 (1)
I- J	-1628 / 0	-84.9	-84.9	0.15 (1)	5.05	O- I	0 / 915	0.21 (1)
J- K	0 / 17	-84.9	-84.9	0.11 (1)	10.00	N- I	0 / 70	0.02 (4)
K- L	0 / 38	-84.9	-84.9	0.12 (1)	10.00	N- J	0 / 127	0.03 (1)
S- B	-222 / 0	0.0	0.0	0.02 (1)	7.81	S- C	-1852 / 0	0.72 (1)
M- K	-222 / 0	0.0	0.0	0.02 (1)	7.81	J- M	-1852 / 0	0.72 (1)
S- R	0 / 1152	-18.5	-18.5	0.27 (1)	10.00			
R- Q	0 / 1234	-18.5	-18.5	0.28 (1)	10.00			
Q- P	0 / 1770	-18.5	-18.5	0.33 (1)	10.00			
P- O	0 / 1770	-18.5	-18.5	0.33 (1)	10.00			
O- N	0 / 1234	-18.5	-18.5	0.28 (1)	10.00			
N- M	0 / 1152	-18.5	-18.5	0.27 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.33/1.00 (E-F:1) , BC=0.33/1.00 (P-Q:1) , WB=0.72/1.00 (J-M:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Q) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

Structural component only
DWG# T-2125086

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420185	T3S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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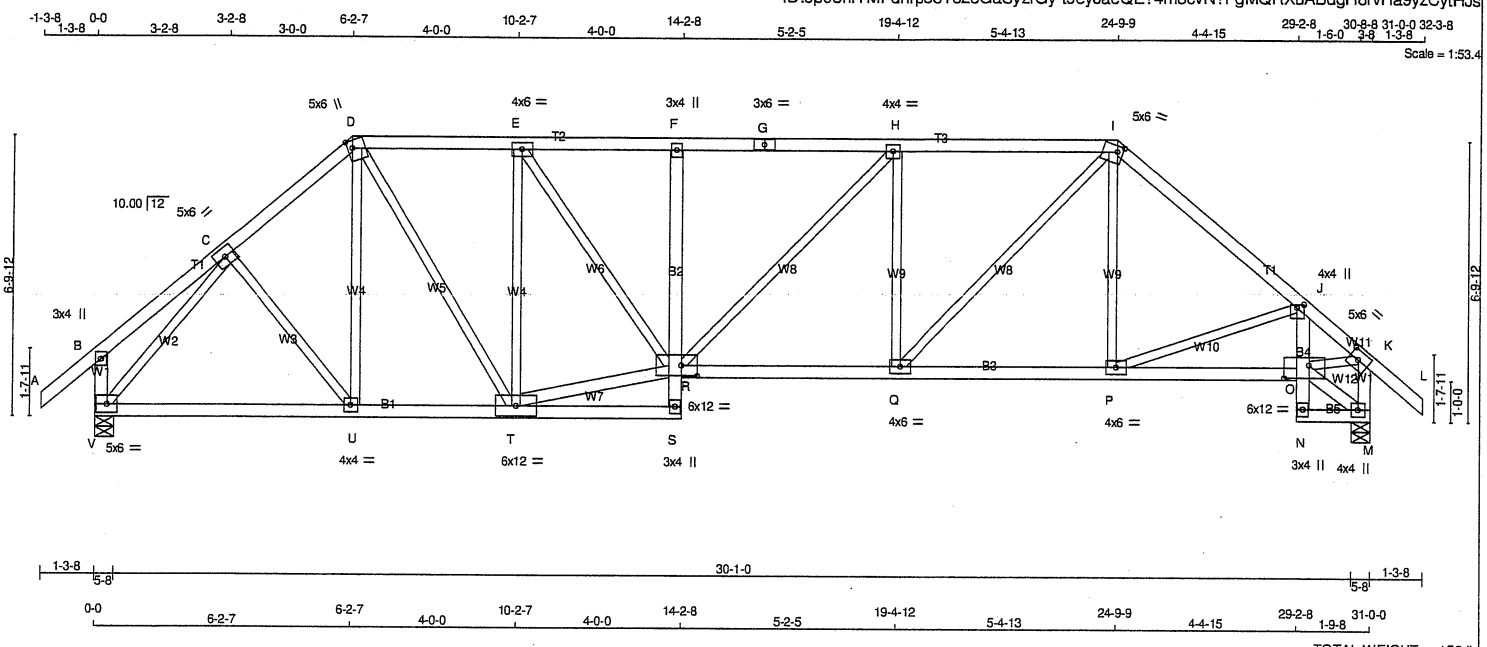

TOTAL WEIGHT = 152 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	6.0	1.75	1.75
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW-t	MT20	5.0	6.0	2.50	2.75
M	BMVW1+p	MT20	4.0	4.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-t	MT20	6.0	12.0	3.75	7.25
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BVMWWW-t	MT20	6.0	12.0	3.00	4.75
S	BMV+p	MT20	3.0	4.0		
T	BMWWW-t	MT20	6.0	12.0		
U	BMWW-t	MT20	4.0	4.0		
V	BMVW1-t	MT20	5.0	6.0		

NOTES (1)
1)

Structural component only
DWG# T-2125104

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1717	0	1717	0	0	5-8	5-8
M	1722	0	1722	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
M	1220	788 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 17	-84.9	-84.9	0.11 (1)	10.00
C- D	-1625 / 0	-84.9	-84.9	0.15 (1)	5.06
D- E	-1693 / 0	-84.9	-84.9	0.21 (1)	4.90
E- F	-2238 / 0	-84.9	-84.9	0.24 (1)	4.35
F- G	-2246 / 0	-84.9	-84.9	0.48 (1)	4.06
G- H	-2246 / 0	-84.9	-84.9	0.48 (1)	4.06
H- I	-2153 / 0	-84.9	-84.9	0.48 (1)	4.14
I- J	-1898 / 0	-84.9	-84.9	0.31 (1)	4.60
J- K	-2094 / 0	-84.9	-84.9	0.22 (1)	4.47
K- L	0 / 38	-84.9	-84.9	0.12 (1)	10.00
V- B	-222 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-1672 / 0	0.0	0.0	0.18 (1)	6.42
V- U	0 / 1150	-18.5	-18.5	0.27 (1)	10.00
U- T	0 / 1231	-18.5	-18.5	0.28 (1)	10.00
T- S	0 / 45	-18.5	-18.5	0.07 (4)	10.00
S- R	0 / 35	0.0	0.0	0.07 (1)	10.00
R- F	-365 / 0	0.0	0.0	0.13 (1)	7.81
R- Q	0 / 2154	-18.5	-18.5	0.41 (1)	10.00
Q- P	0 / 1443	-18.5	-18.5	0.29 (1)	10.00
P- O	0 / 1660	-18.5	-18.5	0.33 (1)	10.00
N- O	0 / 17	0.0	0.0	0.08 (1)	10.00
O- J	-96 / 3	0.0	0.0	0.08 (1)	7.81
N- M	0 / 51	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- U	0 / 126	0.03 (4)
U- D	0 / 67	0.02 (4)
D- T	0 / 890	0.20 (1)
T- E	-1119 / 0	0.87 (1)
T- R	0 / 1711	0.28 (1)
E- R	0 / 933	0.21 (1)
R- H	0 / 133	0.03 (1)
Q- H	-617 / 0	0.32 (1)
Q- I	0 / 1013	0.23 (1)
P- I	0 / 130	0.04 (4)
P- J	-232 / 0	0.09 (1)
V- C	-1848 / 0	0.72 (1)
O- M	-62 / 0	0.01 (1)
O- K	0 / 1630	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.48/1.00 (F-H:1) , BC=0.41/1.00 (Q-R:1) , WB=0.87/1.00 (E-T:1) , SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.50 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420178	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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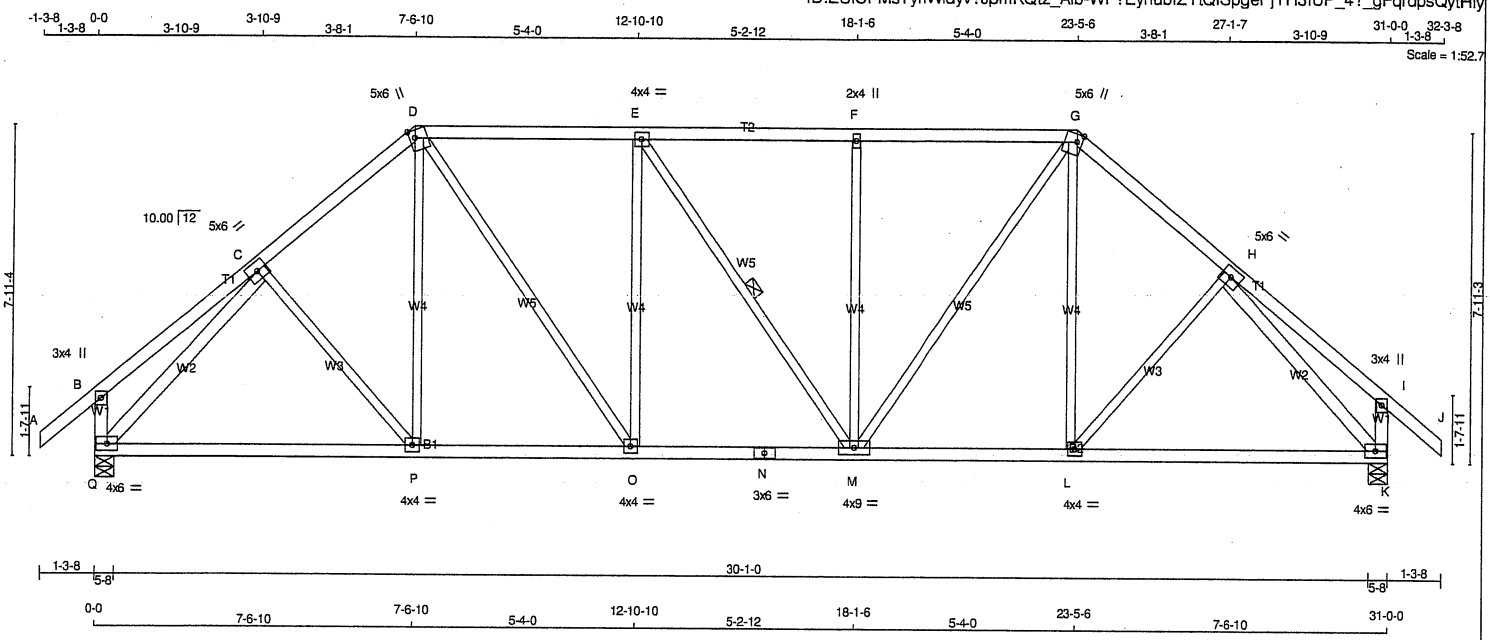

TOTAL WEIGHT = 2 X 150 = 300 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		
L, O, P						
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
Q	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1720	0	1720	0	0	5-8	5-8
K	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	C- P	0 / 51	0.02 (4)
B- C	0 / 23	-84.9	-84.9	0.19 (1)	10.00	P- D	0 / 133	0.04 (4)
C- D	-1606 / 0	-84.9	-84.9	0.23 (1)	4.98	D- O	0 / 694	0.16 (1)
D- E	-1612 / 0	-84.9	-84.9	0.38 (1)	4.77	O- E	-484 / 0	0.58 (1)
E- F	-1611 / 0	-84.9	-84.9	0.38 (1)	4.78	E- M	-3 / 0	0.00 (1)
F- G	-1611 / 0	-84.9	-84.9	0.38 (1)	4.79	M- F	-483 / 0	0.58 (1)
G- H	-1606 / 0	-84.9	-84.9	0.23 (1)	4.98	M- G	0 / 691	0.16 (1)
H- I	0 / 23	-84.9	-84.9	0.19 (1)	10.00	L- G	0 / 134	0.04 (4)
I- J	0 / 38	-84.9	-84.9	0.12 (1)	10.00	L- H	0 / 51	0.02 (4)
Q- B	-240 / 0	0.0	0.0	0.03 (1)	7.81	Q- C	-1861 / 0	0.75 (1)
K- I	-240 / 0	0.0	0.0	0.03 (1)	7.81	H- K	-1862 / 0	0.75 (1)
Q- P	0 / 1202	-18.5	-18.5	0.33 (4)	10.00			
P- O	0 / 1214	-18.5	-18.5	0.34 (4)	10.00			
O- N	0 / 1612	-18.5	-18.5	0.31 (1)	10.00			
N- M	0 / 1612	-18.5	-18.5	0.31 (1)	10.00			
M- L	0 / 1214	-18.5	-18.5	0.34 (4)	10.00			
L- K	0 / 1202	-18.5	-18.5	0.33 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.34/1.00 (L-M:4) , WB=0.75/1.00 (H-K:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)

Structural component only
DWG# T-2125087

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420185	T4S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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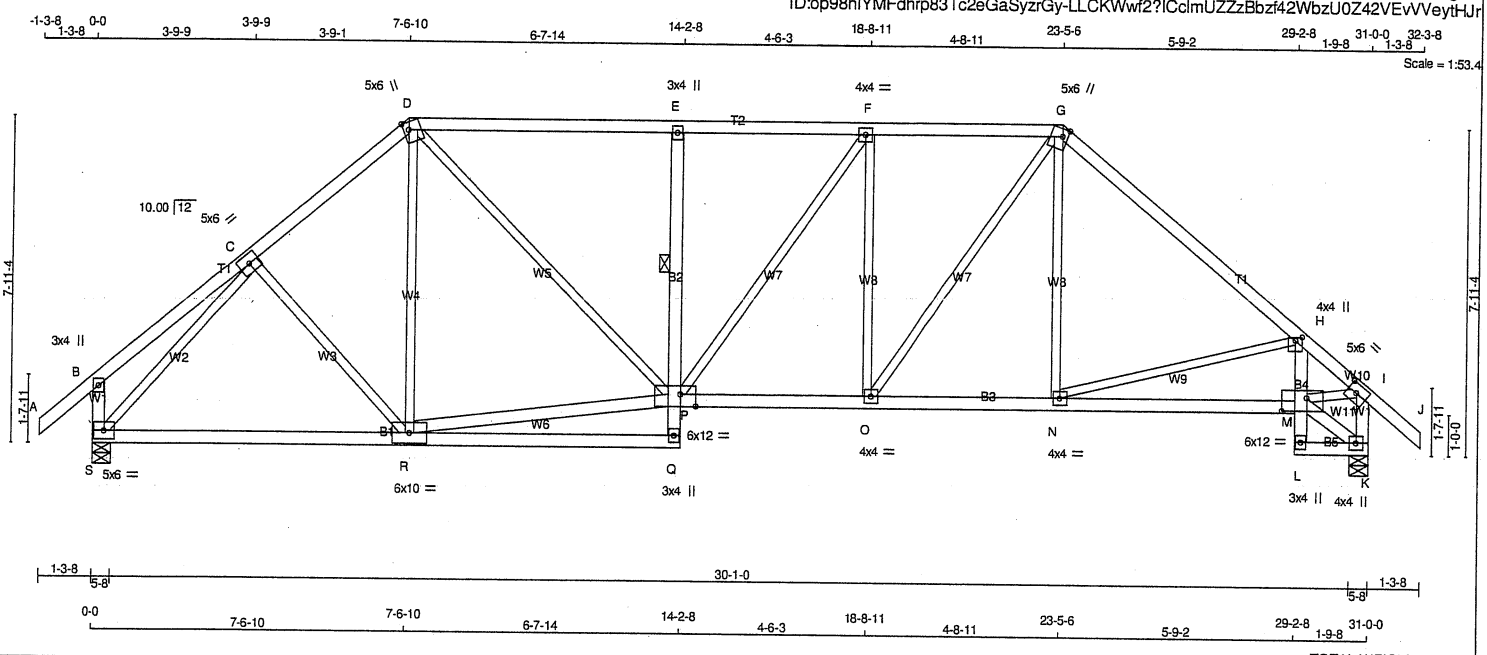

TOTAL WEIGHT = 154 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
I	TMVW-t	MT20	5.0	6.0	2.50	2.75
K	BMVW1+p	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-l	MT20	6.0	12.0	3.75	7.25
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-l	MT20	6.0	12.0	Edge	4.50
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	6.0	10.0		
S	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1)

Structural component only
DWG# T-2125105

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1717	0	1717	0	0	5-8	5-8
K	1722	0	1722	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1220	788 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 23	-84.9	-84.9	0.19 (1)	10.00
C- D	-1605 / 0	-84.9	-84.9	0.23 (1)	4.99
D- E	-1889 / 0	-84.9	-84.9	0.57 (1)	4.27
E- F	-1894 / 0	-84.9	-84.9	0.51 (1)	4.26
F- G	-1807 / 0	-84.9	-84.9	0.29 (1)	4.68
G- H	-1827 / 0	-84.9	-84.9	0.53 (1)	4.47
H- I	-2151 / 0	-84.9	-84.9	0.40 (1)	4.16
I- J	0 / 38	-84.9	-84.9	0.12 (1)	10.00
S- B	-237 / 0	0.0	0.0	0.03 (1)	7.81
K- I	-1671 / 0	0.0	0.0	0.18 (1)	6.42
S- R	0 / 1198	-18.5	-18.5	0.37 (4)	10.00
R- Q	0 / 34	-18.5	-18.5	0.27 (4)	10.00
Q- P	0 / 47	0.0	0.0	0.05 (1)	10.00
P- E	-552 / 0	0.0	0.0	0.07 (1)	6.25
P- O	0 / 1807	-18.5	-18.5	0.34 (1)	10.00
O- N	0 / 1383	-18.5	-18.5	0.30 (1)	10.00
N- M	0 / 1743	-18.5	-18.5	0.36 (1)	10.00
L- M	0 / 17	0.0	0.0	0.08 (1)	10.00
M- H	-102 / 11	0.0	0.0	0.08 (1)	7.81
L- K	0 / 52	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 54	0.02 (4)
R- D	-36 / 74	0.04 (1)
R- P	0 / 1192	0.19 (1)
D- P	0 / 966	0.22 (1)
P- F	0 / 152	0.03 (1)
O- F	-509 / 0	0.42 (1)
O- G	0 / 730	0.16 (1)
N- G	0 / 175	0.05 (4)
N- H	-377 / 0	0.26 (1)
S- C	-1861 / 0	1.00 (1)
M- K	-64 / 0	0.01 (1)
M- I	0 / 1712	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.57/1.00 (D-E:1) , BC=0.37/1.00 (R-S:4) , WB=1.00/1.00 (C-S:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.52 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T5	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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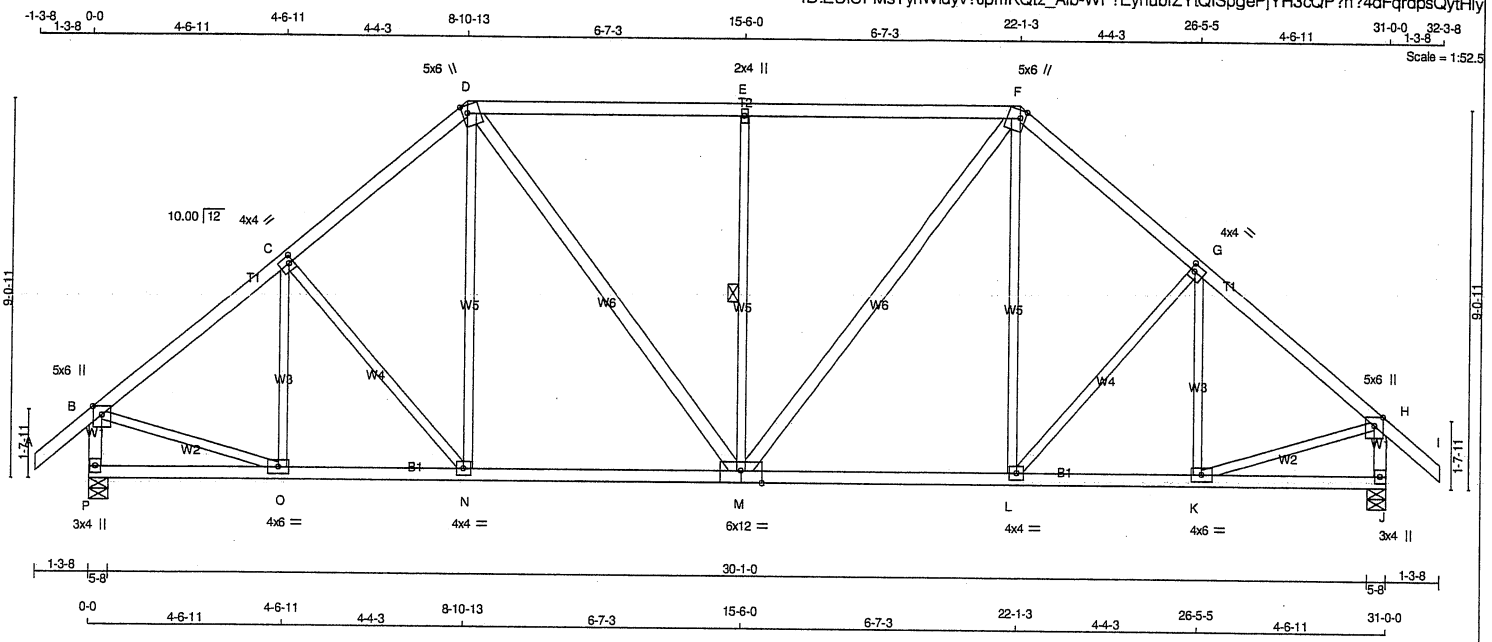

TOTAL WEIGHT = 2 X 154 = 308 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BSWWW-t	MT20	6.0	12.0	Edge	6.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125088

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1720	0	1720	0	0	5-8	5-8
J	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1616 / 0	-84.9	-84.9	0.25 (1)	4.97
C- D	-1554 / 0	-84.9	-84.9	0.25 (1)	5.05
D- E	-1467 / 0	-84.9	-84.9	0.52 (1)	4.76
E- F	-1467 / 0	-84.9	-84.9	0.52 (1)	4.76
F- G	-1554 / 0	-84.9	-84.9	0.25 (1)	5.05
G- H	-1616 / 0	-84.9	-84.9	0.25 (1)	4.97
H- I	0 / 38	-84.9	-84.9	0.12 (1)	10.00
P- B	-1681 / 0	0.0	0.0	0.18 (1)	6.40
J- H	-1681 / 0	0.0	0.0	0.18 (1)	6.40
P- O	0 / 0	-18.5	-18.5	0.08 (4)	10.00
O- N	0 / 1263	-18.5	-18.5	0.30 (1)	10.00
N- M	0 / 1171	-18.5	-18.5	0.28 (1)	10.00
M- L	0 / 1171	-18.5	-18.5	0.28 (1)	10.00
L- K	0 / 1263	-18.5	-18.5	0.30 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-286 / 0	0.13 (1)
C- N	-143 / 0	0.12 (1)
N- D	0 / 232	0.06 (4)
D- M	0 / 492	0.08 (1)
M- E	-687 / 0	0.37 (1)
M- F	0 / 492	0.08 (1)
L- F	0 / 232	0.06 (4)
L- G	-143 / 0	0.12 (1)
K- G	-286 / 0	0.13 (1)
B- O	0 / 1314	0.30 (1)
K- H	0 / 1314	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.52/1.00 (D-E:1) , BC=0.30/1.00 (N-O:1) , WB=0.37/1.00 (E-M:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.60 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T6	8	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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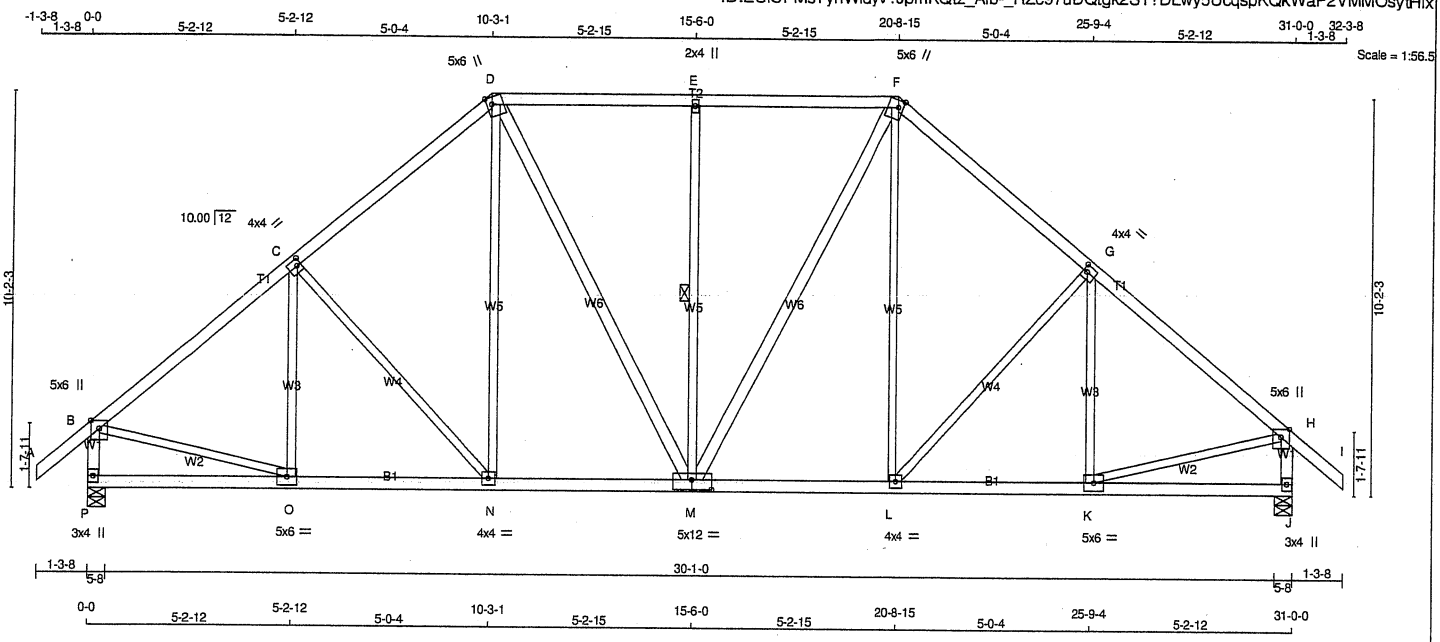

TOTAL WEIGHT = 8 X 162 = 1295 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BSWWW-t	MT20	5.0	12.0	3.00	6.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125089

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1720	0	1720	0	0	5-8	5-8
J	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1218	786 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-1638 / 0	-84.9	-84.9	0.34 (1)	4.84
C- D	-1490 / 0	-84.9	-84.9	0.33 (1)	5.04
D- E	-1283 / 0	-84.9	-84.9	0.32 (1)	5.33
E- F	-1283 / 0	-84.9	-84.9	0.32 (1)	5.33
F- G	-1490 / 0	-84.9	-84.9	0.33 (1)	5.04
G- H	-1638 / 0	-84.9	-84.9	0.34 (1)	4.84
H- I	0 / 38	-84.9	-84.9	0.12 (1)	10.00
P- B	-1678 / 0	0.0	0.0	0.18 (1)	6.41
J- H	-1678 / 0	0.0	0.0	0.18 (1)	6.41
P- O	0 / 0	-18.5	-18.5	0.11 (4)	10.00
O- N	0 / 1284	-18.5	-18.5	0.27 (1)	10.00
N- M	0 / 1119	-18.5	-18.5	0.24 (1)	10.00
M- L	0 / 1119	-18.5	-18.5	0.24 (1)	10.00
L- K	0 / 1284	-18.5	-18.5	0.27 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-219 / 0	0.12 (1)
C- N	-250 / 0	0.29 (1)
N- D	0 / 292	0.07 (1)
D- M	0 / 350	0.06 (1)
M- E	-541 / 0	0.39 (1)
M- F	0 / 350	0.06 (1)
L- F	0 / 292	0.07 (1)
L- G	-250 / 0	0.29 (1)
K- G	-219 / 0	0.12 (1)
B- O	0 / 1323	0.30 (1)
K- H	0 / 1323	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.34/1.00 (B-C:1) , BC=0.27/1.00 (N-O:1) , WB=0.39/1.00 (E-M:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.62 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T8	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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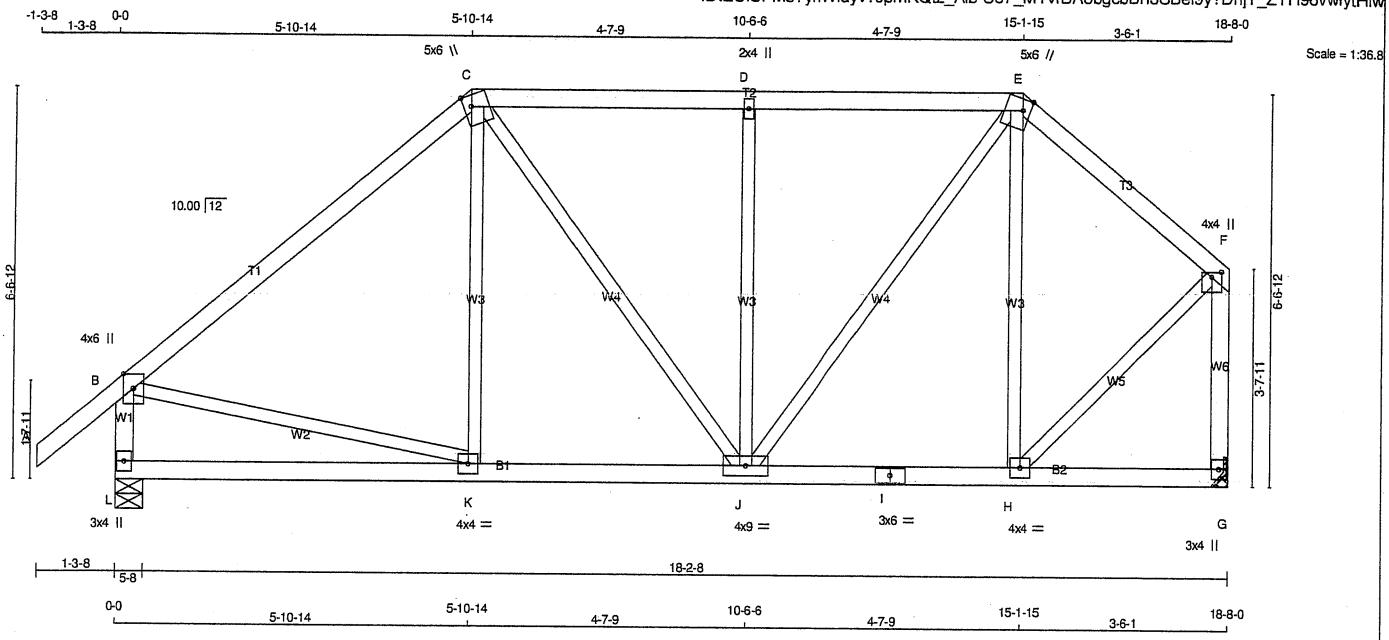


TOTAL WEIGHT = 88 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

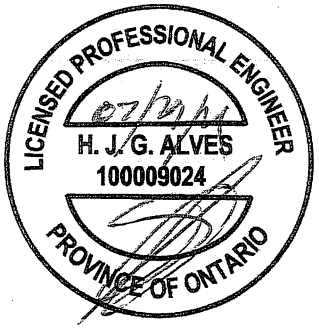
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2125090

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1082	0	1082	0	0	IN-SX	IN-SX
G	965	0	965	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	766	499 / 0	0 / 0	0 / 0	0 / 0	267 / 0	0 / 0
G	685	435 / 0	0 / 0	0 / 0	0 / 0	250 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	
B- C	-827 / 0	-84.9	-84.9	0.57 (1)	5.80	
C- D	-743 / 0	-84.9	-84.9	0.31 (1)	6.25	
D- E	-743 / 0	-84.9	-84.9	0.31 (1)	6.25	
E- F	-589 / 0	-84.9	-84.9	0.19 (1)	6.25	
L- B	-1037 / 0	0.0	0.0	0.11 (1)	7.71	
G- F	-941 / 0	0.0	0.0	0.21 (1)	7.81	
L- K	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
K- J	0 / 634	-18.5	-18.5	0.20 (4)	10.00	
J- I	0 / 446	-18.5	-18.5	0.12 (1)	10.00	
I- H	0 / 446	-18.5	-18.5	0.12 (1)	10.00	
H- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-20 / 79	0.03 (4)
C- J	0 / 183	0.04 (1)
J- D	-479 / 0	0.34 (1)
J- E	0 / 499	0.11 (1)
H- E	-324 / 0	0.23 (1)
B- K	0 / 649	0.15 (1)
H- F	0 / 612	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.20/1.00 (J-K:4) , WB=0.34/1.00 (D-J:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (K) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T9	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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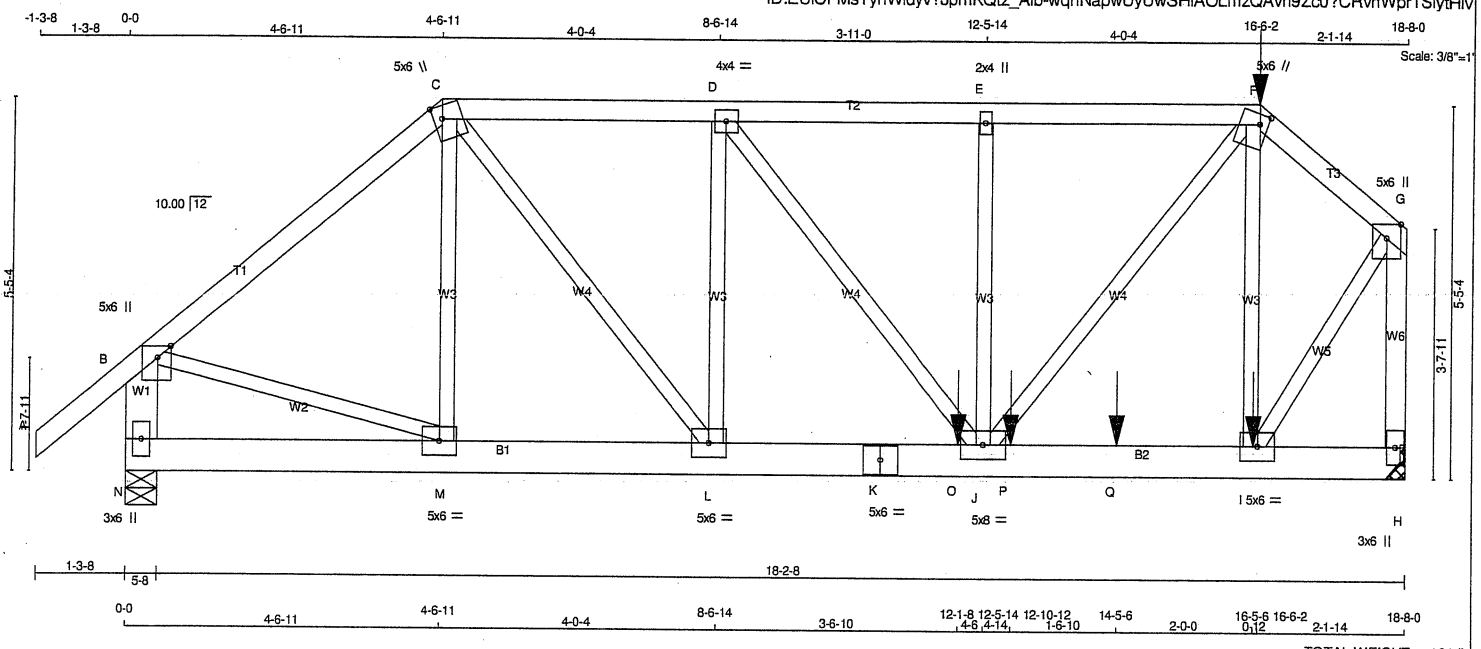

TOTAL WEIGHT = 101 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	1.75	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I, L, M						
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1397	0	1397	0	0	5-8	5-8
H	1813	0	1813	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	987	652 / 0	0 / 0	0 / 0	0 / 0	335 / 0	0 / 0
H	1280	853 / 0	0 / 0	0 / 0	0 / 0	427 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX - CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.13 (1)	10.00	M- C	-163 / 13	0.07 (1)
B- C	-1244 / 0	-84.9	-84.9	0.39 (1)	5.25	C- L	0 / 796	0.20 (1)
C- D	-1446 / 0	-84.9	-84.9	0.23 (1)	5.15	L- D	-520 / 0	0.22 (1)
D- E	-1566 / 0	-84.9	-84.9	0.24 (1)	4.99	D- J	0 / 196	0.05 (1)
E- F	-1566 / 0	-84.9	-84.9	0.22 (1)	5.00	J- E	-353 / 0	0.15 (1)
F- G	-1006 / 0	-84.9	-84.9	0.09 (1)	6.12	J- F	0 / 1302	0.32 (1)
N- B	-1360 / 0	0.0	0.0	0.10 (1)	7.81	I- F	-701 / 0	0.30 (1)
H- G	-1837 / 0	0.0	0.0	0.39 (1)	6.13	B- M	0 / 985	0.24 (1)
						I- G	0 / 1340	0.33 (1)
N- M	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
M- L	0 / 950	-18.5	-18.5	0.14 (1)	10.00			
L- K	0 / 1446	-18.5	-18.5	0.26 (1)	10.00			
K- O	0 / 1446	-18.5	-18.5	0.26 (1)	10.00			
O- J	0 / 1446	-18.5	-18.5	0.26 (1)	10.00			
J- P	0 / 755	-18.5	-18.5	0.21 (1)	10.00			
P- Q	0 / 755	-18.5	-18.5	0.21 (1)	10.00			
Q- I	0 / 755	-18.5	-18.5	0.21 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.07 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	16-6-2	-13	-13	---	FRONT	VERT	DEAD	---	C1
F	16-6-2	-49	-49	---	FRONT	VERT	SNOW	---	C1
I	16-5-6	-127	-127	---	FRONT	VERT	TOTAL	---	C1
O	12-1-8	-372	-372	---	FRONT	VERT	TOTAL	---	C1
P	12-10-12	-127	-127	---	FRONT	VERT	TOTAL	---	C1
Q	14-5-6	-127	-127	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.39/1.00 (G-H:1) , BC=0.26/1.00 (J-L:1) , WB=0.33/1.00 (G-I:1) , SSI=0.35/1.00 (J-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.35 (G) (INPUT = 1.00)

Structural component only
DWG# T-2125091

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T10	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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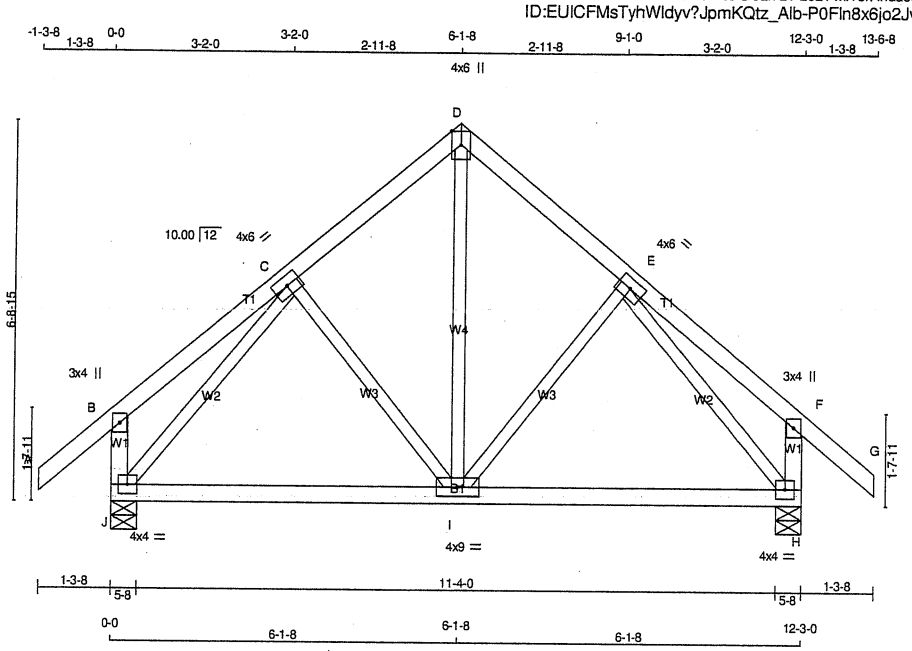

TOTAL WEIGHT = 59 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	751	0	751	0	0	5-8	5-8
H	751	0	751	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	0 / 19	-84.9	-84.9	0.12 (1)	10.00
C- D	-428 / 0	-84.9	-84.9	0.10 (1)	6.25
D- E	-428 / 0	-84.9	-84.9	0.10 (1)	6.25
E- F	0 / 19	-84.9	-84.9	0.12 (1)	10.00
F- G	0 / 38	-84.9	-84.9	0.12 (1)	10.00
J- B	-217 / 0	0.0	0.0	0.02 (1)	7.81
H- F	-217 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 388	-18.5	-18.5	0.23 (4)	10.00
I- H	0 / 388	-18.5	-18.5	0.23 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 320	0.07 (1)
I- E	-116 / 0	0.05 (1)
C- I	-116 / 0	0.05 (1)
J- C	-625 / 0	0.24 (1)
E- H	-625 / 0	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (B-C:1) , BC=0.23/1.00 (I-J:4) , WB=0.24/1.00 (C-J:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (H) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)

Structural component only
DWG# T-2125092

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T10A	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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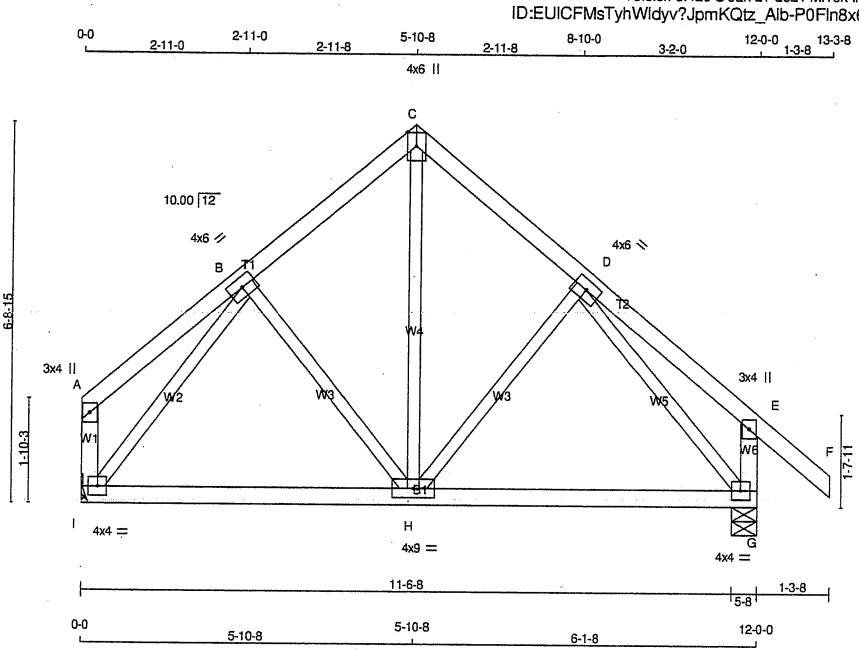

TOTAL WEIGHT = 2 X 57 = 113 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	620	0	620	0	0	MECHANICAL	
G	738	0	738	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	440	279 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0
G	521	344 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-84.9	-84.9	0.11 (1)	10.00
B- C	-410 / 0	-84.9	-84.9	0.09 (1)	6.25
C- D	-412 / 0	-84.9	-84.9	0.10 (1)	6.25
D- E	0 / 19	-84.9	-84.9	0.12 (1)	10.00
E- F	0 / 38	-84.9	-84.9	0.12 (1)	10.00
I- A	-89 / 0	0.0	0.0	0.01 (1)	7.81
G- E	-217 / 0	0.0	0.0	0.02 (1)	7.81
I- H	0 / 356	-18.5	-18.5	0.22 (4)	10.00
H- G	0 / 377	-18.5	-18.5	0.22 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- H	-86 / 3	0.03 (1)
H- C	0 / 297	0.07 (1)
H- D	-120 / 0	0.05 (1)
I- B	-606 / 0	0.22 (1)
D- G	-608 / 0	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (D-E:1) , BC=0.22/1.00 (G-H:4) , WB=0.23/1.00 (D-G:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (G) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

Structural component only
DWG# T-2125093

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T10S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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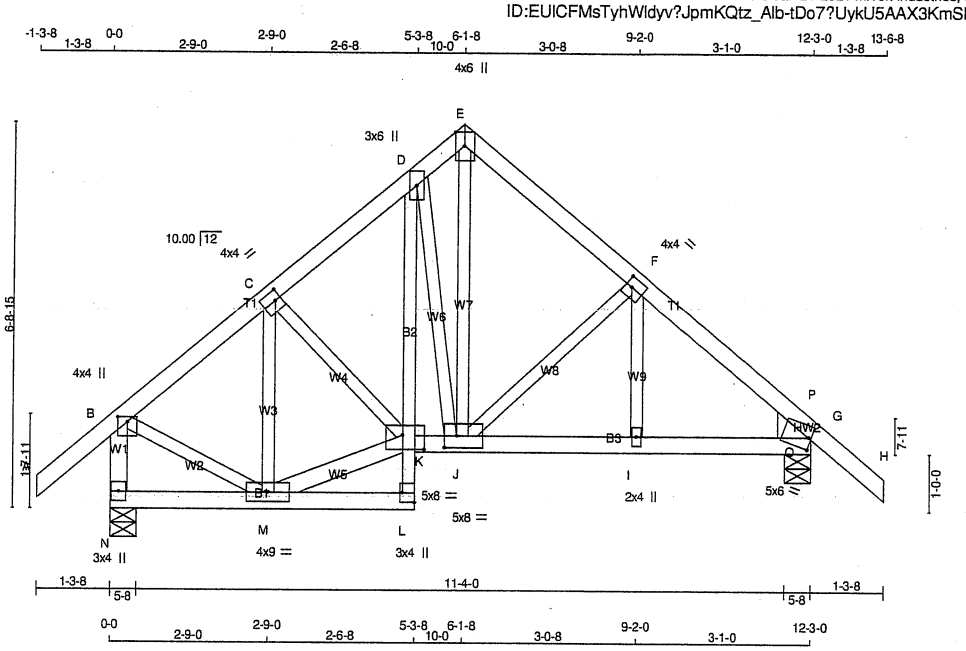

TOTAL WEIGHT = 67 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - D	2x3	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMVW+p	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMBH1-m	MT20	5.0	6.0	2.50	0.50
I	BMW+w	MT20	2.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.50	2.50
K	BVMWW-I	MT20	5.0	8.0	3.00	4.50
L	BMV+p	MT20	3.0	4.0	2.00	Edge
M	BMWWW-t	MT20	4.0	9.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
N	751	0	751	0	0	5-8	5-8	
G	751	0	751	0	0	5-8	5-8	2x6 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
G	531	350 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	M- C	-301 / 0	0.07 (1)
B- C	-502 / 0	-84.9	-84.9	0.08 (1)	6.25	M- K	0 / 429	0.07 (1)
C- D	-549 / 0	-84.9	-84.9	0.08 (1)	6.25	C- K	0 / 15	0.00 (1)
D- E	-470 / 0	-84.9	-84.9	0.01 (4)	6.25	D- J	-246 / 0	0.10 (1)
E- F	-486 / 0	-84.9	-84.9	0.09 (1)	6.25	J- E	0 / 460	0.10 (1)
F- P	-685 / 0	-84.9	-84.9	0.07 (1)	6.25	J- F	-222 / 0	0.06 (1)
P- G	-919 / 0	-84.9	-84.9	0.08 (1)	6.25	I- F	0 / 51	0.02 (4)
G- H	0 / 31	-84.9	-84.9	0.12 (1)	10.00	B- M	0 / 441	0.10 (1)
N- B	-729 / 0	0.0	0.0	0.08 (1)	7.81	O- P	0 / 191	0.00 (1)
N- M	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
M- L	0 / 3	-18.5	-18.5	0.03 (4)	10.00			
L- K	0 / 19	0.0	0.0	0.01 (1)	10.00			
K- D	0 / 170	0.0	0.0	0.05 (1)	10.00			
K- J	0 / 401	-18.5	-18.5	0.10 (1)	10.00			
J- I	0 / 525	-18.5	-18.5	0.12 (1)	10.00			
I- O	0 / 525	-18.5	-18.5	0.14 (1)	10.00			
O- G	0 / 525	-18.5	-18.5	0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.14/1.00 (I-O:1) , WB=0.10/1.00 (E-J:1) , SSI=0.11/1.00 (G-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.17 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/29/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125094

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	G11	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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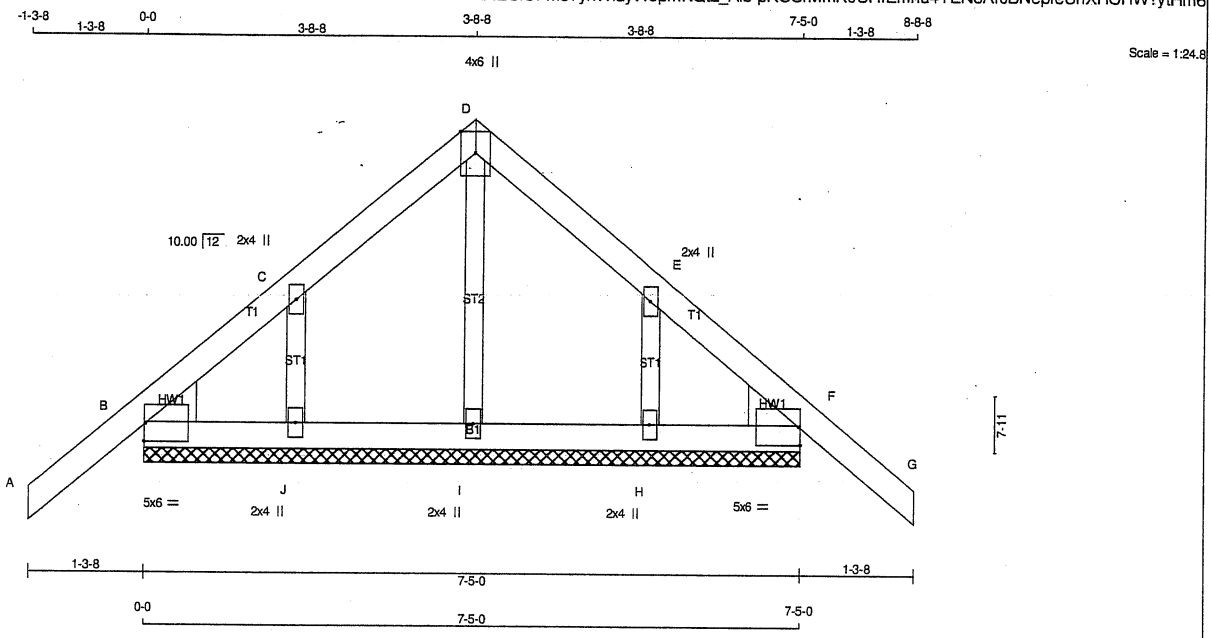

TOTAL WEIGHT = 31 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	5.0	6.0		Edge
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	5.0	6.0		Edge
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125076

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x6 L
2x6 R
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 31	-84.9	-84.9	0.12 (1)		10.00
B- C	-75 / 0	-84.9	-84.9	0.11 (1)		6.25
C- D	-41 / 0	-84.9	-84.9	0.05 (1)		6.25
D- E	-41 / 0	-84.9	-84.9	0.05 (1)		6.25
E- F	-75 / 0	-84.9	-84.9	0.11 (1)		6.25
F- G	0 / 31	-84.9	-84.9	0.12 (1)		10.00
B- J	0 / 33	-18.5	-18.5	0.02 (4)		10.00
J- I	0 / 27	-18.5	-18.5	0.02 (4)		10.00
I- H	0 / 27	-18.5	-18.5	0.02 (4)		10.00
H- F	0 / 33	-18.5	-18.5	0.02 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	-114 / 0	0.02 (1)
J- C	-116 / 0	0.02 (1)
H- E	-116 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (F-G:1) , BC=0.02/1.00 (B-J:4) , WB=0.02/1.00 (D-I:1) , SSI=0.08/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (F) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420178	T12	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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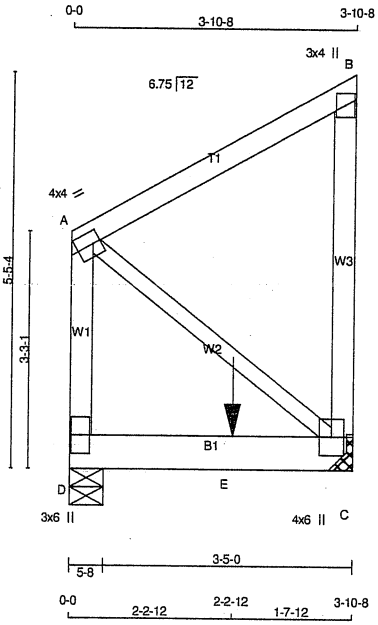

TOTAL WEIGHT = 2 X 24 = 48 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	TOP
D- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125095

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	546	0	546	0	0	MECHANICAL	
D	456	0	456	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	387	251 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0
D	323	209 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 0	-84.9	-84.9	0.12 (1)	10.00	A- C	0 / 0	0.00 (1)
C- B	-164 / 0	0.0	0.0	0.04 (1)	7.81			
D- A	-164 / 0	0.0	0.0	0.01 (1)	7.81			
D- E	0 / 0	-18.5	-18.5	0.22 (1)	10.00			
E- C	0 / 0	-18.5	-18.5	0.22 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-2-12	-425	-425	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.22/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420178	T13	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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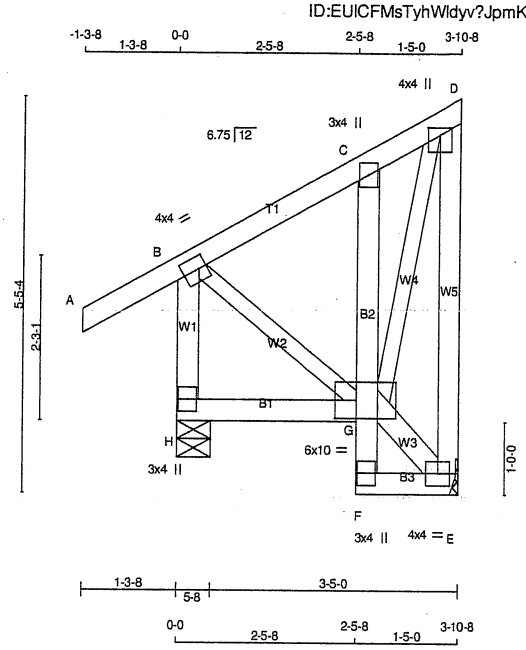

TOTAL WEIGHT = 3 X 31 = 92 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMVW1-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-I	MT20	6.0	10.0	3.00	3.50
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	200	0	200	0	0	MECHANICAL	
H	316	0	316	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	142	90 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
H	222	154 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.13 (5)	10.00
B- C	-66 / 0	-84.9	-84.9	0.07 (1)	6.25
C- D	-81 / 0	-84.9	-84.9	0.06 (1)	6.25
E- D	-181 / 0	0.0	0.0	0.09 (1)	7.81
H- B	-292 / 0	0.0	0.0	0.04 (1)	7.81
H- G	0 / 0	-18.5	-18.5	0.04 (4)	10.00
F- G	0 / 13	0.0	0.0	0.02 (1)	10.00
G- C	-207 / 0	0.0	0.0	0.02 (1)	7.81
F- E	0 / 10	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- D	0 / 195	0.04 (1)
B- G	0 / 81	0.02 (1)
G- E	-12 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.04/1.00 (G-H:4) , WB=0.04/1.00 (D-G:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

Structural component only
DWG# T-2125096

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420178	T14	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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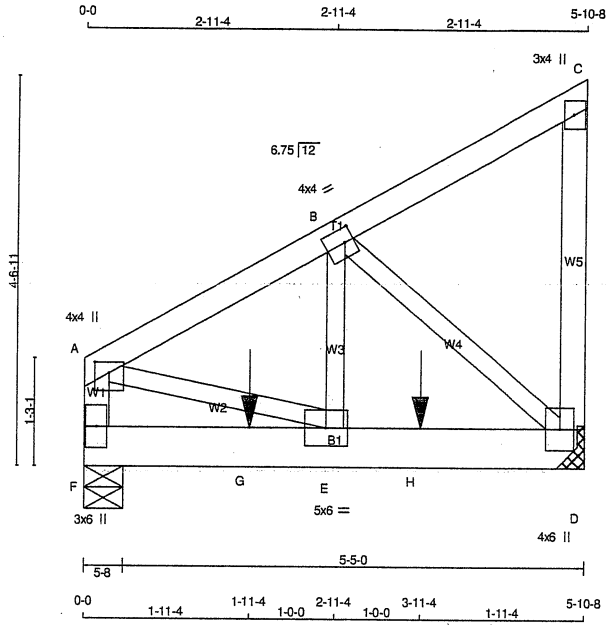

TOTAL WEIGHT = 2 X 30 = 60 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		

Structural component only
DWG# T-2125097

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1529	0	1529	0	0	MECHANICAL	
F	1819	0	1819	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1082	709 / 0	0 / 0	0 / 0	0 / 0	372 / 0	0 / 0
F	1285	852 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1684 / 0	-84.9	-84.9	0.06	(1)	6.25	E- B	0 / 1802	0.22 (1)
B- C	-10 / 0	-84.9	-84.9	0.05	(1)	10.00	B- D	-1942 / 0	0.25 (1)
D- C	-104 / 0	0.0	0.0	0.02	(1)	7.81	A- E	0 / 1538	0.19 (1)
F- A	-1366 / 0	0.0	0.0	0.08	(1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.31	(1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.31	(1)	10.00			
E- H	0 / 1476	-18.5	-18.5	0.30	(1)	10.00			
H- D	0 / 1476	-18.5	-18.5	0.30	(1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-1265	-1265	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	-670	-670	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.08/1.00 (A-F:1) , BC=0.31/1.00 (E-F:1) , WB=0.25/1.00 (B-D:1) , SSI=0.46/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.33 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420178	T14	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07/29/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125097

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420178	T14Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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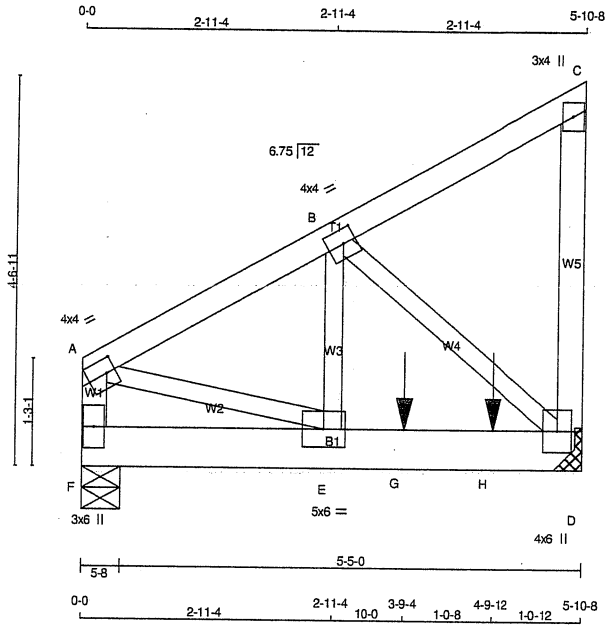

TOTAL WEIGHT = 2 X 30 = 60 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		

Structural component only
DWG# T-2125098

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1625	0	1625	0	0	MECHANICAL	
F	860	0	860	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1150	750 / 0	0 / 0	0 / 0	0 / 0	400 / 0	0 / 0
F	609	395 / 0	0 / 0	0 / 0	0 / 0	214 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1126 / 0	-84.9	-84.9	0.06 (1)	6.25	E- B	0 / 1108	0.14 (1)
B- C	-12 / 0	-84.9	-84.9	0.05 (1)	6.25	B- D	-1304 / 0	0.16 (1)
D- C	-101 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 1033	0.13 (1)
F- A	-948 / 0	0.0	0.0	0.05 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.12 (1)	10.00			
E- G	0 / 991	-18.5	-18.5	0.33 (1)	10.00			
G- H	0 / 991	-18.5	-18.5	0.33 (1)	10.00			
H- D	0 / 991	-18.5	-18.5	0.33 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	3-9-4	-866	-866	---	BACK	VERT	TOTAL	---	C1
H	4-9-12	-462	-462	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.33/1.00 (D-E:1) , WB=0.16/1.00 (B-D:1) , SSI=0.42/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420178	T14Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125098

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T15	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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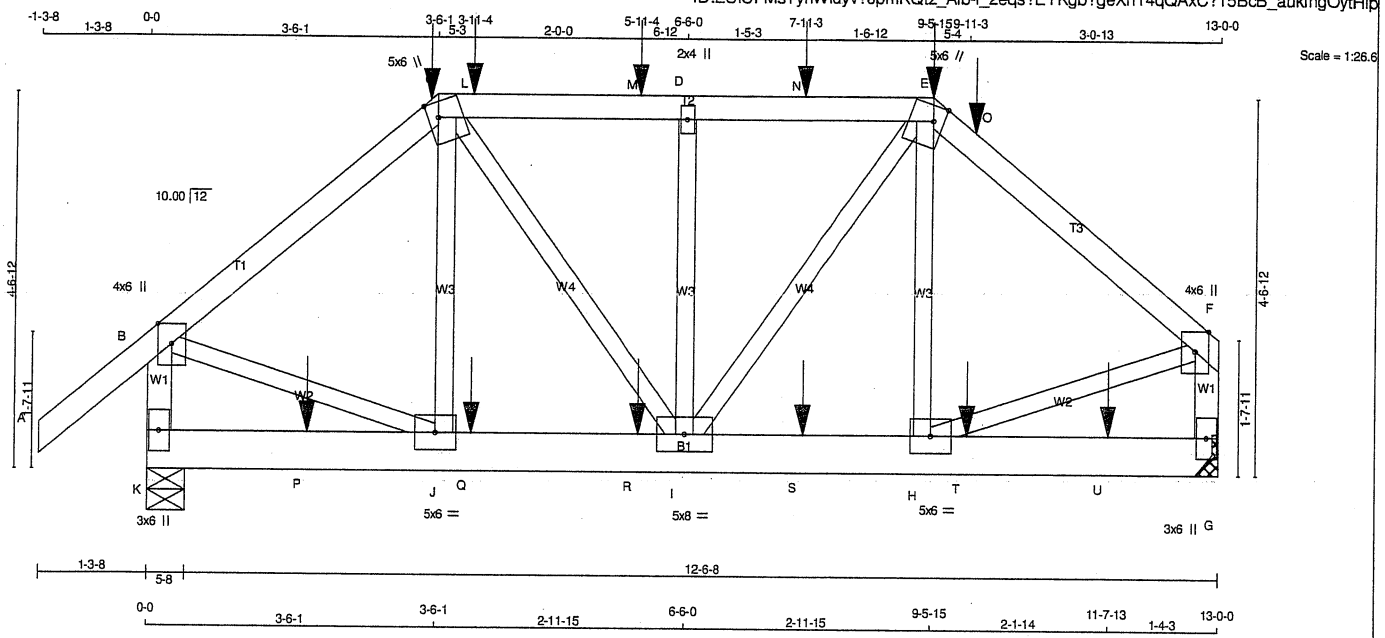

TOTAL WEIGHT = 68 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125099

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1321	0	1321	0	0	5-8	5-8
G	1243	0	1243	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	933	617 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0
G	881	569 / 0	0 / 0	0 / 0	0 / 0	311 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.13 (1)	10.00
B- C	-1126 / 0	-84.9	-84.9	0.22 (1)	5.69
C- L	-1057 / 0	-84.9	-84.9	0.23 (1)	5.80
L- M	-1057 / 0	-84.9	-84.9	0.23 (1)	5.80
M- D	-1057 / 0	-84.9	-84.9	0.23 (1)	5.80
D- N	-1057 / 0	-84.9	-84.9	0.23 (1)	5.80
N- E	-1057 / 0	-84.9	-84.9	0.23 (1)	5.80
E- O	-1114 / 0	-84.9	-84.9	0.27 (1)	5.64
O- F	-1114 / 0	-84.9	-84.9	0.27 (1)	5.64
K- B	-1282 / 0	0.0	0.0	0.15 (1)	7.08
G- F	-1197 / 0	0.0	0.0	0.14 (1)	7.28
K- P	0 / 0	-18.5	-18.5	0.05 (4)	10.00
P- J	0 / 0	-18.5	-18.5	0.05 (4)	10.00
J- Q	0 / 858	-18.5	-18.5	0.15 (1)	10.00
Q- R	0 / 858	-18.5	-18.5	0.15 (1)	10.00
R- I	0 / 858	-18.5	-18.5	0.15 (1)	10.00
I- S	0 / 872	-18.5	-18.5	0.15 (1)	10.00
S- H	0 / 872	-18.5	-18.5	0.15 (1)	10.00
H- T	0 / 0	-18.5	-18.5	0.06 (4)	10.00
T- U	0 / 0	-18.5	-18.5	0.06 (4)	10.00
U- G	0 / 0	-18.5	-18.5	0.06 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-161 / 34	0.05 (1)
C- I	0 / 341	0.08 (1)
I- D	-487 / 0	0.15 (1)
I- E	0 / 317	0.08 (1)
H- E	-161 / 38	0.05 (1)
B- J	0 / 910	0.23 (1)
H- F	0 / 925	0.23 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-6-1	-35	-35	---	TOP	VERT	DEAD	---	C1
C	3-6-1	-133	-133	---	TOP	VERT	SNOW	---	C1
E	9-5-15	-35	-35	---	TOP	VERT	DEAD	---	C1
E	9-5-15	-133	-133	---	TOP	VERT	SNOW	---	C1
L	3-11-4	-87	-87	---	FRONT	VERT	TOTAL	---	C1
M	5-11-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
N	7-11-3	-71	-71	---	FRONT	VERT	TOTAL	---	C1
O	9-11-3	-87	-87	---	FRONT	VERT	TOTAL	---	C1
P	1-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Q	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	7-11-3	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	9-11-3	-21	-21	---	FRONT	VERT	TOTAL	---	C1
U	11-7-13	-21	-21	---	FRONT	VERT	TOTAL	---	C1

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.27/1.00 (E-F:1) , BC=0.15/1.00 (I-J:1) , WB=0.23/1.00 (F-H:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	T16	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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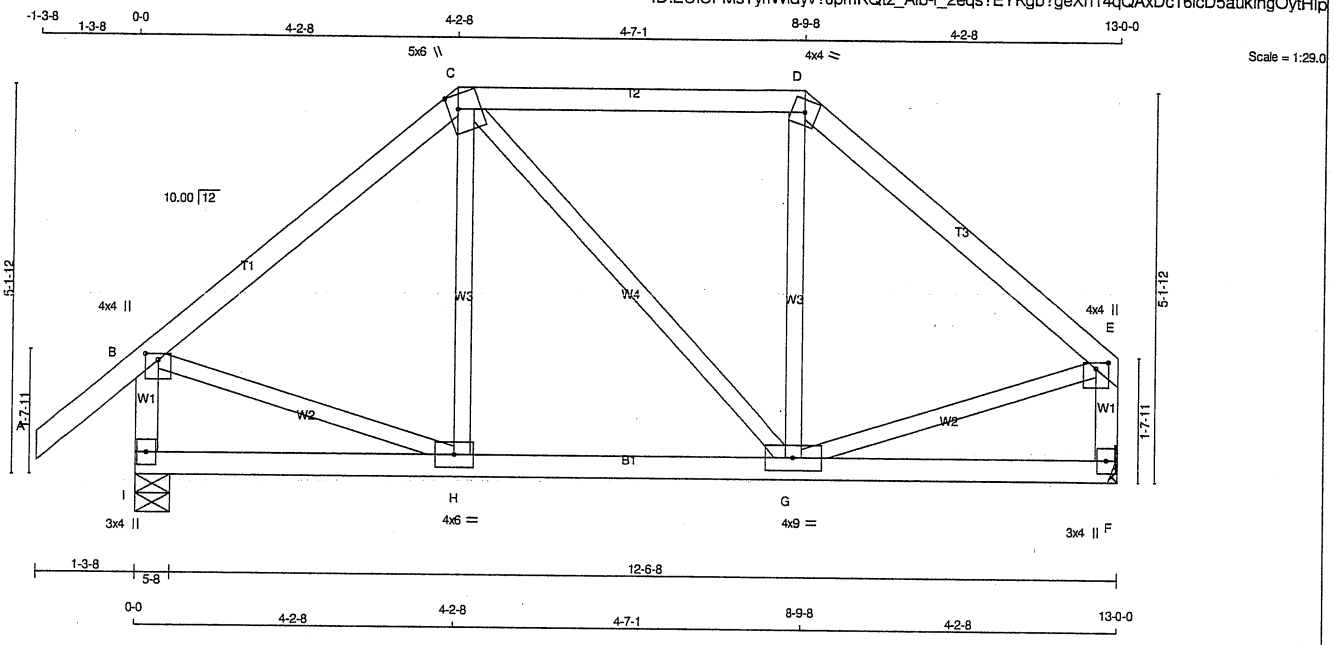

TOTAL WEIGHT = 57 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	789	0	789	0	0	5-8	5-8
F	672	0	672	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	558	367 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0
F	477	303 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)	10.00
B- C	-527 / 0	-84.9	-84.9	0.20 (1)	6.25
C- D	-403 / 0	-84.9	-84.9	0.23 (1)	6.25
D- E	-527 / 0	-84.9	-84.9	0.20 (1)	6.25
I- B	-757 / 0	0.0	0.0	0.08 (1)	7.81
F- E	-639 / 0	0.0	0.0	0.07 (1)	7.81
I- H	0 / 0	-18.5	-18.5	0.08 (4)	10.00
H- G	0 / 403	-18.5	-18.5	0.12 (4)	10.00
G- F	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-35 / 53	0.02 (4)
C- G	0 / 0	0.00 (1)
G- D	-35 / 53	0.02 (4)
B- H	0 / 422	0.09 (1)
G- E	0 / 422	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (C-D:1) , BC=0.12/1.00 (G-H:4) , WB=0.09/1.00 (B-H:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125100

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420186	T20	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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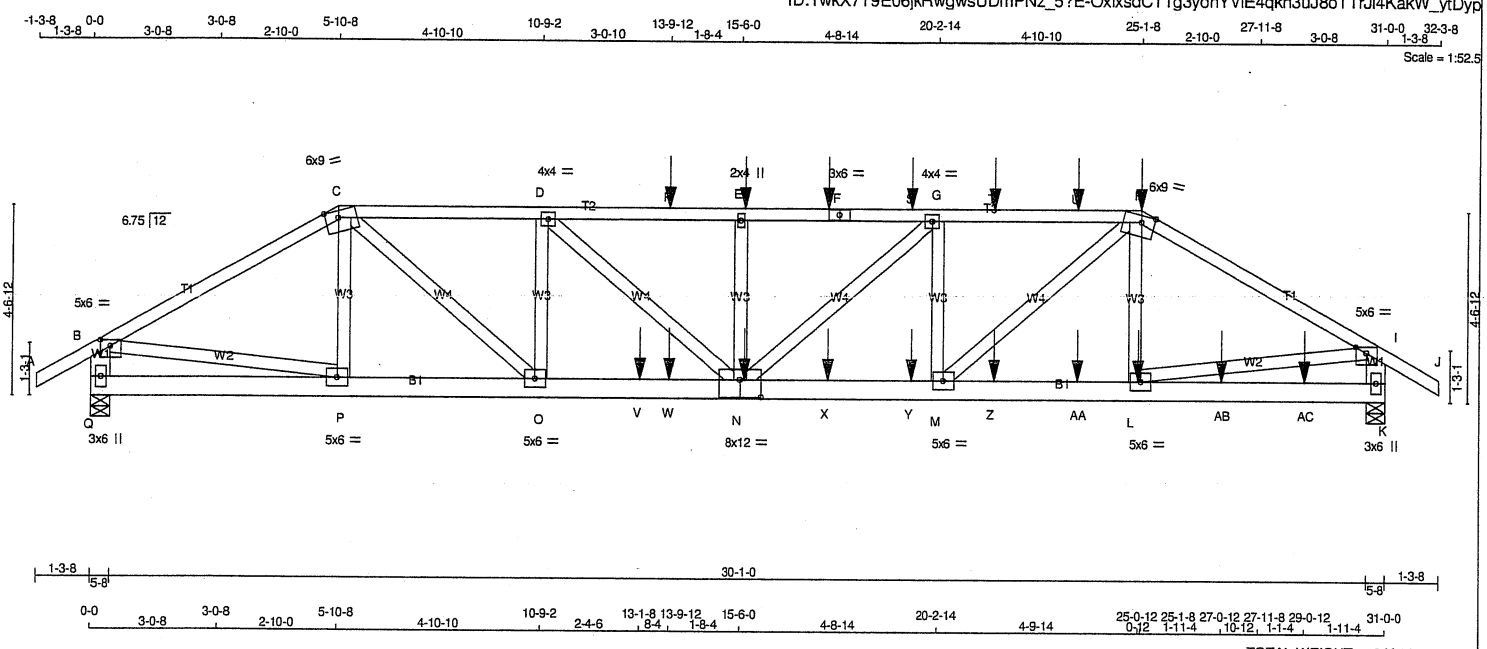

TOTAL WEIGHT = 2 X 160 = 320 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-F	1	12	SIDE(61.0)
F-H	1	12	SIDE(61.0)
H-J	1	12	SIDE(61.0)
Q-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q-N	2	12	SIDE(61.0)
N-K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only

DWG# T-2125116

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2853	0	2853	0	0	5-8	5-8
K	3136	0	3136	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2020	1311 / 0	0 / 0	0 / 0	0 / 0	709 / 0	0 / 0
K	2223	1426 / 0	0 / 0	0 / 0	0 / 0	798 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 28	-84.9	-84.9	0.06 (1)	10.00	P- C	-538 / 0	0.06 (1)
B- C		-3922 / 0	-84.9	-84.9	0.42 (1)	4.41	L- H	-345 / 32	0.04 (1)
C- D		-5782 / 0	-84.9	-84.9	0.32 (1)	3.81	B- P	0 / 3450	0.31 (1)
D- R		-6434 / 0	-84.9	-84.9	0.35 (1)	3.61	L- I	0 / 3850	0.34 (1)
R- E		-6434 / 0	-84.9	-84.9	0.35 (1)	3.61	M- H	0 / 2418	0.21 (1)
E- F		-6434 / 0	-84.9	-84.9	0.46 (1)	3.49	C- O	0 / 3137	0.28 (1)
F- S		-6434 / 0	-84.9	-84.9	0.46 (1)	3.49	M- G	-1441 / 0	0.15 (1)
S- G		-6434 / 0	-84.9	-84.9	0.46 (1)	3.49	O- D	-1058 / 0	0.11 (1)
G- T		-5638 / 0	-84.9	-84.9	0.41 (1)	3.75	N- G	0 / 1061	0.09 (1)
T- U		-5638 / 0	-84.9	-84.9	0.41 (1)	3.75	D- N	0 / 869	0.08 (1)
U- H		-5638 / 0	-84.9	-84.9	0.41 (1)	3.75	N- E	-595 / 0	0.06 (1)
H- I		-4377 / 0	-84.9	-84.9	0.45 (1)	4.19			
I- J		0 / 28	-84.9	-84.9	0.06 (1)	10.00			
Q- B		-2780 / 0	0.0	0.0	0.10 (1)	7.81			
K- I		-3060 / 0	0.0	0.0	0.11 (1)	7.81			
Q- P		0 / 0	-18.5	-18.5	0.05 (1)	10.00			
P- O		0 / 3397	-18.5	-18.5	0.41 (1)	10.00			
O- V		0 / 5782	-18.5	-18.5	0.86 (1)	10.00			
V- W		0 / 5782	-18.5	-18.5	0.86 (1)	10.00			
W- N		0 / 5782	-18.5	-18.5	0.86 (1)	10.00			
N- X		0 / 5638	-18.5	-18.5	0.40 (1)	10.00			
X- Y		0 / 5638	-18.5	-18.5	0.40 (1)	10.00			
Y- M		0 / 5638	-18.5	-18.5	0.40 (1)	10.00			
M- Z		0 / 3800	-18.5	-18.5	0.28 (1)	10.00			
Z-AA		0 / 3800	-18.5	-18.5	0.28 (1)	10.00			
AA- L		0 / 3800	-18.5	-18.5	0.28 (1)	10.00			
L-AB		0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AB-AC		0 / 0	-18.5	-18.5	0.07 (4)	10.00			
AC- K		0 / 0	-18.5	-18.5	0.07 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	15-7-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
F	17-7-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
H	25-1-8	-294	-294	---	FRONT	VERT	TOTAL	---	C1
L	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	15-7-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	13-9-12	-71	-71	---	FRONT	VERT	TOTAL	---	C1
S	19-7-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
T	21-7-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
U	23-7-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1
V	13-1-8	-901	-901	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.46/1.00 (E-G:1) , BC=0.86/1.00 (N-O:1) , WB=0.34/1.00 (I-L:1) , SSI=0.27/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.43 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420186	T20	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:TwkX7T9E06jkRwgwsUDmPNz_5?E-OxlxsdCT1g3yohYVlE4gkn3uJ8oTTrJi4KakW_ytDyp

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	6.0	9.0	2.00	3.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	2.00	3.75
I	TMVW-p	MT20	5.0	6.0	1.75	3.00
K	BMV1+p	MT20	3.0	6.0		
L, M, O, P						
L	BMWW-t	MT20	5.0	6.0		
N	BSWWW-l	MT20	8.0	12.0	5.00	6.00
Q	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	13-9-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	17-7-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	19-7-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Z	21-7-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AA	23-7-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AB	27-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AC	29-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125116

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T20Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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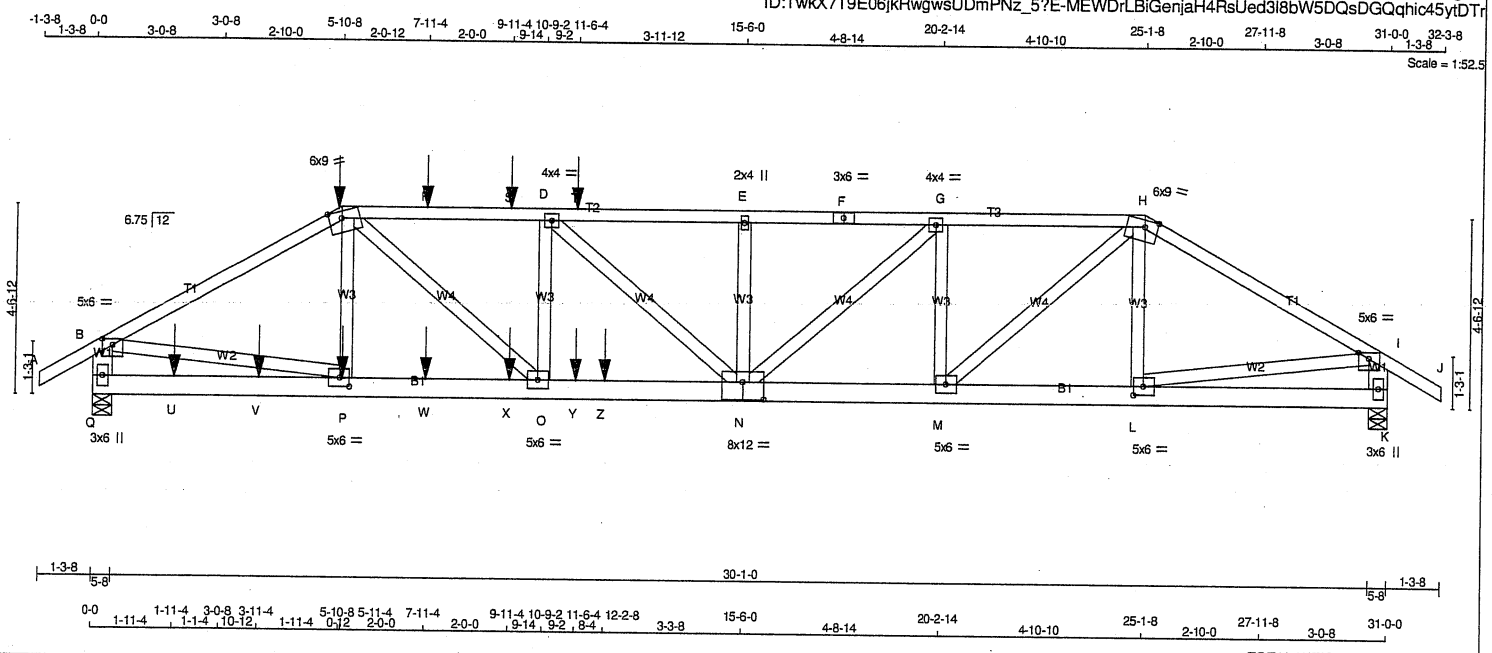

TOTAL WEIGHT = 2 X 160 = 320 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
H- J	1	12	TOP
Q- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	SIDE(183.1)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2125117

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	3530	0	3530	0	0	5-8	5-8
K	2668	0	2668	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2501	1617 / 0	0 / 0	0 / 0	0 / 0	884 / 0	0 / 0
K	1889	1228 / 0	0 / 0	0 / 0	0 / 0	660 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.06 (1)	10.00
B- C	-4972 / 0	-84.9	-84.9	0.50 (1)	3.94
C- R	-6856 / 0	-84.9	-84.9	0.46 (1)	3.40
R- S	-6856 / 0	-84.9	-84.9	0.46 (1)	3.40
S- D	-6856 / 0	-84.9	-84.9	0.46 (1)	3.40
D- T	-6511 / 0	-84.9	-84.9	0.42 (1)	3.51
T- E	-6511 / 0	-84.9	-84.9	0.42 (1)	3.51
E- F	-6511 / 0	-84.9	-84.9	0.35 (1)	3.59
F- G	-6511 / 0	-84.9	-84.9	0.35 (1)	3.59
G- H	-5170 / 0	-84.9	-84.9	0.28 (1)	4.05
H- I	-3656 / 0	-84.9	-84.9	0.40 (1)	4.55
I- J	0 / 28	-84.9	-84.9	0.06 (1)	10.00
Q- B	-3426 / 0	0.0	0.0	0.12 (1)	7.54
K- I	-2616 / 0	0.0	0.0	0.09 (1)	7.81
Q- U	0 / 0	-18.5	-18.5	0.08 (1)	10.00
U- V	0 / 0	-18.5	-18.5	0.08 (1)	10.00
V- P	0 / 0	-18.5	-18.5	0.08 (1)	10.00
P- W	0 / 4308	-18.5	-18.5	0.55 (1)	10.00
W- X	0 / 4308	-18.5	-18.5	0.55 (1)	10.00
X- O	0 / 4308	-18.5	-18.5	0.55 (1)	10.00
O- Y	0 / 6856	-18.5	-18.5	0.97 (1)	10.00
Y- Z	0 / 6856	-18.5	-18.5	0.97 (1)	10.00
Z- N	0 / 6856	-18.5	-18.5	0.97 (1)	10.00
N- M	0 / 5170	-18.5	-18.5	0.36 (1)	10.00
M- L	0 / 3172	-18.5	-18.5	0.23 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-648 / 0	0.07 (1)
L- H	-339 / 0	0.04 (1)
B- P	0 / 4374	0.39 (1)
L- I	0 / 3216	0.28 (1)
M- H	0 / 2628	0.23 (1)
C- O	0 / 3352	0.30 (1)
M- G	-1641 / 0	0.18 (1)
O- D	-399 / 0	0.04 (1)
N- G	0 / 1789	0.16 (1)
D- N	-461 / 0	0.11 (1)
N- E	-359 / 0	0.04 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-294	-294	---	BACK	VERT	TOTAL	---	C1
P	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
R	7-11-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
S	9-11-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
T	11-6-4	-71	-71	---	BACK	VERT	TOTAL	---	C1
U	1-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
V	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
W	7-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
X	9-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	11-6-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Z	12-2-8	-1323	-1323	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.97/1.00 (N-O:1) , WB=0.39/1.00 (B-P:1) , SSI=0.50/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.49 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T20Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	6.0	9.0	2.00	3.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	2.00	3.75
I	TMVW-p	MT20	5.0	6.0	1.75	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	5.0	6.0		
N	BSWWW-I	MT20	8.0	12.0	5.00	6.00
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	3.0	6.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2125117

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420187	T20S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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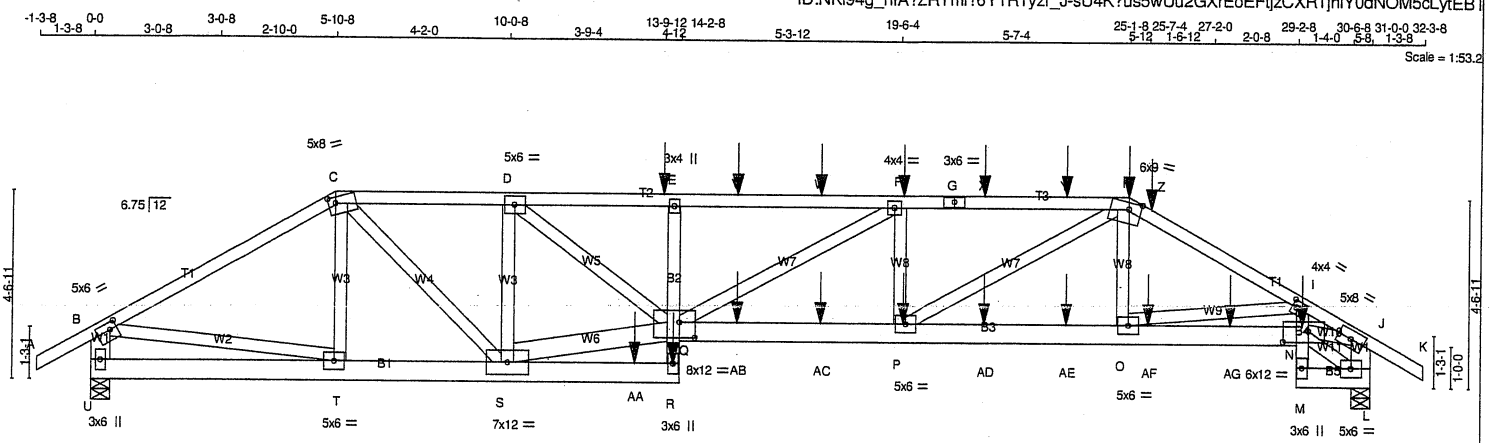

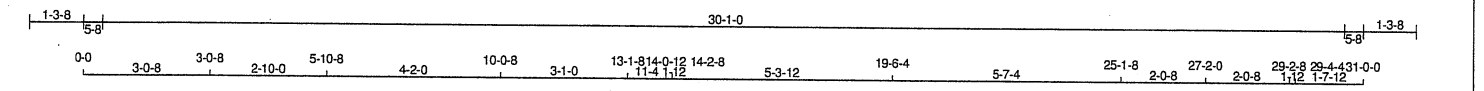

TOTAL WEIGHT = 2 X 166 = 331 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- G	1	12	SIDE(61.0)
G- H	1	12	SIDE(61.0)
H- K	1	12	SIDE(61.0)
U- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- R	2	12	SIDE(122.0)
Q- N	2	12	SIDE(183.1)
M- L	2	12	TOP
R- E	1	12	TOP
I- M	1	12	SIDE(9.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2125135

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2817	0	2817	0	0	5-8	5-8
L	3580	0	3580	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1973	1400 / 0	0 / 0	0 / 0	0 / 0	573 / 0	0 / 0
L	2514	1748 / 0	0 / 0	0 / 0	0 / 0	767 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 23	-69.9	-69.9	0.05 (1)	10.00
B- C	-3982 / 0	-69.9	-69.9	0.37 (1)	4.45
C- D	-5506 / 0	-69.9	-69.9	0.21 (1)	4.00
D- E	-8861 / 0	-69.9	-69.9	0.40 (1)	3.05
E- V	-8921 / 0	-69.9	-69.9	0.75 (1)	2.76
V- W	-8921 / 0	-69.9	-69.9	0.75 (1)	2.76
W- F	-8921 / 0	-69.9	-69.9	0.75 (1)	2.76
F- G	-8228 / 0	-69.9	-69.9	0.70 (1)	2.94
G- X	-8228 / 0	-69.9	-69.9	0.70 (1)	2.94
X- Y	-8228 / 0	-69.9	-69.9	0.70 (1)	2.94
Y- H	-8228 / 0	-69.9	-69.9	0.70 (1)	2.94
H- Z	-6289 / 0	-69.9	-69.9	0.37 (1)	3.61
Z- I	-6289 / 0	-69.9	-69.9	0.37 (1)	3.61
I- J	-7817 / 0	-69.9	-69.9	0.27 (1)	3.31
J- K	0 / 23	-69.9	-69.9	0.05 (1)	10.00
U- B	-2752 / 0	0.0	0.0	0.10 (1)	7.81
L- J	-3286 / 0	0.0	0.0	0.12 (1)	7.66
U- T	0 / 0	-18.5	-18.5	0.04 (4)	10.00
T- S	0 / 3450	-18.5	-18.5	0.33 (1)	10.00
S-AA	0 / 191	-18.5	-18.5	0.35 (1)	10.00
AA- R	0 / 191	-18.5	-18.5	0.35 (1)	10.00
R- Q	0 / 1022	0.0	0.0	0.23 (1)	10.00
Q- E	-527 / 0	0.0	0.0	0.15 (1)	7.81
Q-AB	0 / 8228	-18.5	-18.5	0.62 (1)	10.00
AB-AC	0 / 8228	-18.5	-18.5	0.62 (1)	10.00
AC- P	0 / 8228	-18.5	-18.5	0.62 (1)	10.00
P-AD	0 / 5537	-18.5	-18.5	0.43 (1)	10.00
AD-AE	0 / 5537	-18.5	-18.5	0.43 (1)	10.00
AE- O	0 / 5537	-18.5	-18.5	0.43 (1)	10.00
O-AF	0 / 7390	-18.5	-18.5	0.53 (1)	10.00
AF-AG	0 / 7390	-18.5	-18.5	0.53 (1)	10.00
AG- N	0 / 7390	-18.5	-18.5	0.53 (1)	10.00
M- N	0 / 17	0.0	0.0	0.34 (1)	10.00
N- I	0 / 946	0.0	0.0	0.43 (1)	10.00
M- L	0 / 437	-18.5	-18.5	0.03 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-498 / 0	0.05 (1)
S- C	0 / 2947	0.26 (1)
S- D	-2889 / 0	0.31 (1)
S- Q	0 / 5542	0.36 (1)
D- Q	0 / 4191	0.37 (1)
Q- F	0 / 803	0.07 (1)
P- F	-1203 / 0	0.09 (1)
P- H	0 / 3092	0.27 (1)
O- H	0 / 624	0.06 (1)
B- T	0 / 3503	0.31 (1)
N- L	-519 / 0	0.03 (1)
N- J	0 / 6812	0.60 (1)
O- I	-1912 / 0	0.23 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	13-9-12	-83	-83	---	FRONT	VERT	TOTAL	---	C1
F	19-7-4	-79	-79	---	FRONT	VERT	TOTAL	---	C1
H	25-1-8	-58	-58	---	FRONT	VERT	DEAD	---	C1
H	25-1-8	-223	-223	---	FRONT	VERT	SNOW	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 849 (0.44")

CSI: TC=0.75/1.00 (E-F:1) , BC=0.62/1.00 (P-Q:1) , WB=0.60/1.00 (J-N:1) , SSI=0.33/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420187	T20S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.00
C	TTWW-m	MT20	5.0	8.0	1.75	2.00
D	TMWW-t	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TTWW-m	MT20	6.0	9.0	2.00	3.50
I	TMVW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-t	MT20	5.0	8.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-l	MT20	6.0	12.0	3.25	7.25
O, P, T						
O	BMWW-t	MT20	5.0	6.0		
Q	BVMWWW-l	MT20	8.0	12.0	4.50	4.50
R	BMV+p	MT20	3.0	6.0		
S	BMWWW-t	MT20	7.0	12.0		
U	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS-(LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	29-4-4	-192	-192	---	FRONT	VERT	TOTAL	---	C1
P	19-7-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
R	14-0-12	-27	-27	---	FRONT	VERT	TOTAL	---	C1
V	15-7-4	-79	-79	---	FRONT	VERT	TOTAL	---	C1
W	17-7-4	-79	-79	---	FRONT	VERT	TOTAL	---	C1
X	21-7-4	-79	-79	---	FRONT	VERT	TOTAL	---	C1
Y	23-7-4	-79	-79	---	FRONT	VERT	TOTAL	---	C1
Z	25-7-4	-92	-92	---	FRONT	VERT	TOTAL	---	C1
AA	13-1-8	-929	-929	---	FRONT	VERT	TOTAL	---	C1
AB	15-7-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AC	17-7-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AD	21-7-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AE	23-7-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AF	25-7-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1
AG	27-7-4	-65	-65	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.70 (J) (INPUT = 1.00)

Structural component only
DWG# T-2125135

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T21	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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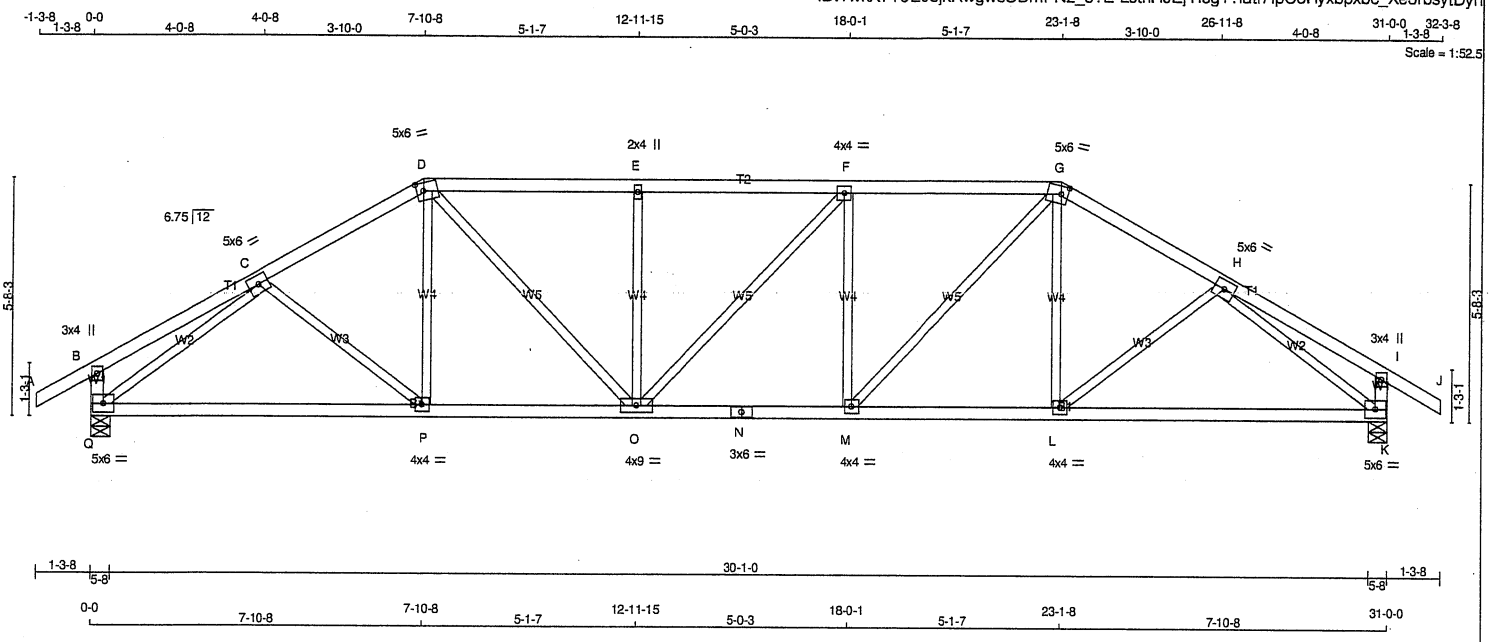

TOTAL WEIGHT = 2 X 128 = 256 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L, M, P						
L	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
Q	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1718	0	1718	0	0	5-8	5-8
K	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
K	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	C- P	0 / 62		0.02 (4)
B- C	0 / 17	-84.9	-84.9	0.19 (1)	10.00	P- D	0 / 142		0.04 (4)
C- D	-2045 / 0	-84.9	-84.9	0.20 (1)	4.58	L- G	0 / 141		0.04 (4)
D- E	-2282 / 0	-84.9	-84.9	0.32 (1)	4.25	L- H	0 / 62		0.02 (4)
E- F	-2283 / 0	-84.9	-84.9	0.29 (1)	4.28	Q- C	-2251 / 0		0.93 (1)
F- G	-2284 / 0	-84.9	-84.9	0.32 (1)	4.25	H- K	-2251 / 0		0.93 (1)
G- H	-2045 / 0	-84.9	-84.9	0.20 (1)	4.58	M- G	0 / 749		0.17 (1)
H- I	0 / 17	-84.9	-84.9	0.19 (1)	10.00	D- O	0 / 747		0.17 (1)
I- J	0 / 28	-84.9	-84.9	0.11 (1)	10.00	M- F	-465 / 0		0.23 (1)
Q- B	-249 / 0	0.0	0.0	0.03 (1)	7.81	O- E	-464 / 0		0.23 (1)
K- I	-248 / 0	0.0	0.0	0.03 (1)	7.81	O- F	-2 / 0		0.00 (1)
Q- P	0 / 1757	-18.5	-18.5	0.42 (1)	10.00				
P- O	0 / 1768	-18.5	-18.5	0.42 (1)	10.00				
O- N	0 / 2284	-18.5	-18.5	0.42 (1)	10.00				
N- M	0 / 2284	-18.5	-18.5	0.42 (1)	10.00				
M- L	0 / 1768	-18.5	-18.5	0.42 (1)	10.00				
L- K	0 / 1756	-18.5	-18.5	0.42 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.42/1.00 (M-O:1) , WB=0.93/1.00 (C-Q:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.75 (N) (INPUT = 1.00)

Structural component only
DWG# T-2125118

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420187	T21S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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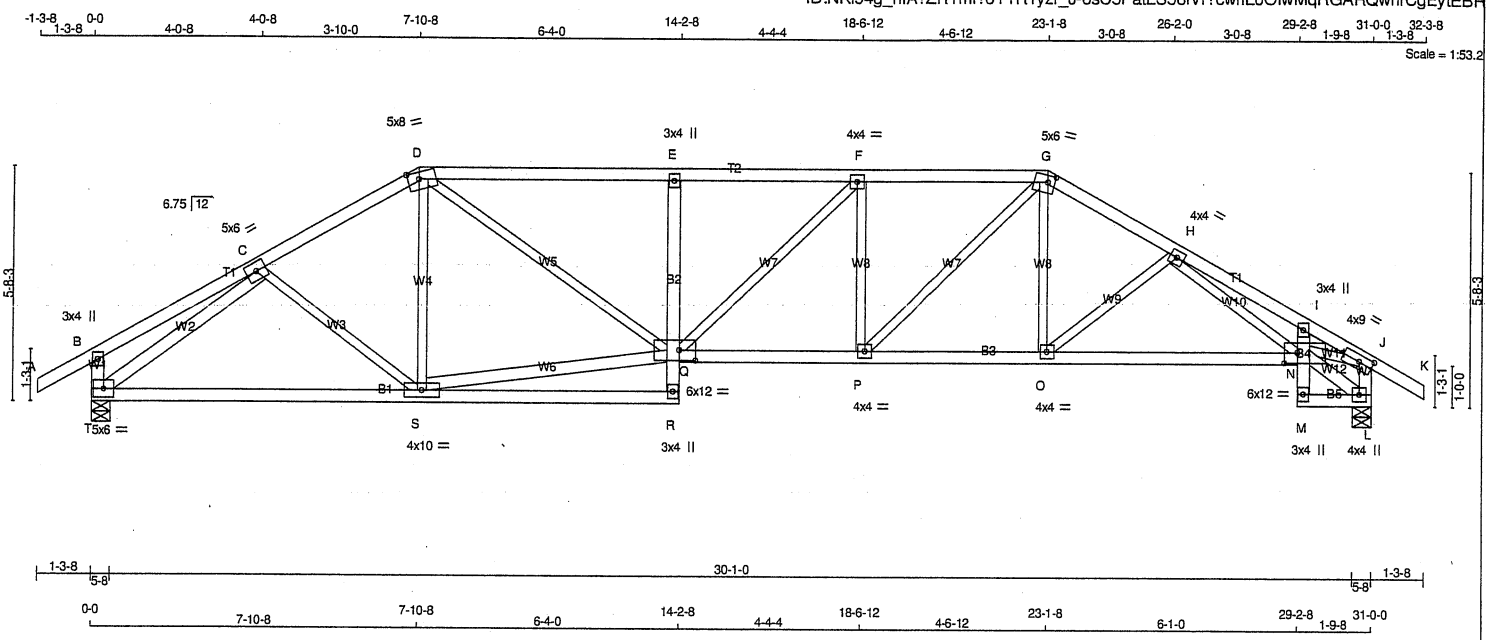

TOTAL WEIGHT = 138 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, E, I						
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	2.00
H	TMWW-t	MT20	4.0	4.0		
J	TMVW-t	MT20	4.0	9.0	2.00	4.00
L	BMVW1+p	MT20	4.0	4.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWWW-I	MT20	6.0	12.0	3.00	3.75
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWWW-I	MT20	6.0	12.0	3.00	4.75
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	4.0	10.0		
T	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125136

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1715	0	1715	0	0	5-8	5-8
L	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1215	784 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1219	787 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 18	-84.9	-84.9	0.20 (1)	10.00
C- D	-2046 / 0	-84.9	-84.9	0.20 (1)	4.57
D- E	-2839 / 0	-84.9	-84.9	0.53 (1)	3.64
E- F	-2856 / 0	-84.9	-84.9	0.39 (1)	3.77
F- G	-2739 / 0	-84.9	-84.9	0.31 (1)	3.94
G- H	-2476 / 0	-84.9	-84.9	0.18 (1)	4.24
H- I	-3044 / 0	-84.9	-84.9	0.26 (1)	3.81
I- J	-2919 / 0	-84.9	-84.9	0.18 (1)	3.95
J- K	0 / 28	-84.9	-84.9	0.11 (1)	10.00
T- B	-247 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-1608 / 0	0.0	0.0	0.16 (1)	6.52
T- S	0 / 1759	-18.5	-18.5	0.43 (1)	10.00
S- R	0 / 73	-18.5	-18.5	0.28 (4)	10.00
R- Q	0 / 43	0.0	0.0	0.11 (1)	10.00
Q- E	-524 / 0	0.0	0.0	0.13 (1)	7.81
Q- P	0 / 2739	-18.5	-18.5	0.49 (1)	10.00
P- O	0 / 2149	-18.5	-18.5	0.43 (1)	10.00
O- N	0 / 2458	-18.5	-18.5	0.48 (1)	10.00
M- N	0 / 17	0.0	0.0	0.22 (1)	10.00
N- I	-116 / 0	0.0	0.0	0.17 (1)	7.81
M- L	0 / 160	-18.5	-18.5	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	0 / 61	0.02 (4)
S- D	-117 / 43	0.06 (1)
S- Q	0 / 1714	0.28 (1)
D- Q	0 / 1317	0.30 (1)
Q- F	0 / 164	0.04 (1)
P- F	-490 / 0	0.16 (1)
P- G	0 / 818	0.18 (1)
O- G	0 / 374	0.08 (1)
T- C	-2254 / 0	0.67 (1)
N- L	-188 / 0	0.02 (1)
N- J	0 / 2500	0.56 (1)
O- H	-408 / 0	0.12 (1)
H- N	0 / 291	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.53/1.00 (D-E:1) , BC=0.49/1.00 (P-Q:1) , WB=0.67/1.00 (C-T:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.51 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T22	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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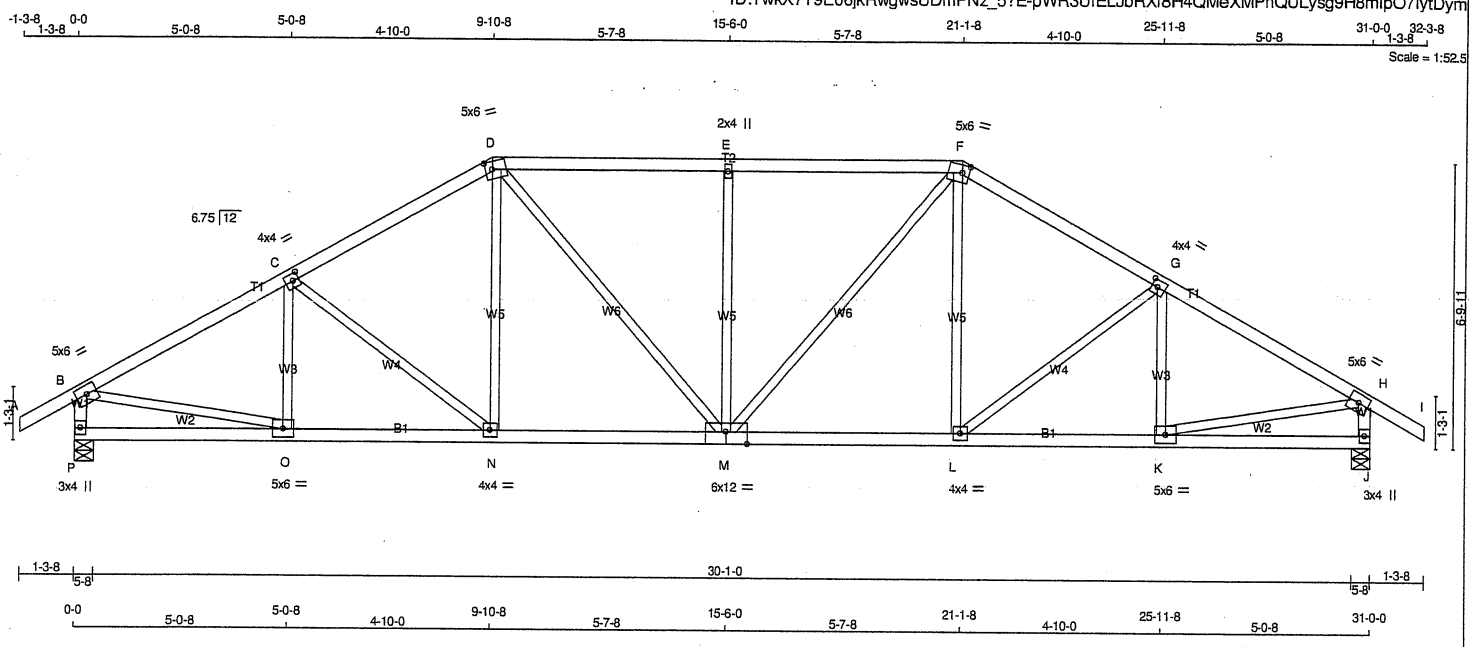

TOTAL WEIGHT = 2 X 131 = 262 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BSWWW-t	MT20	6.0	12.0	Edge	6.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1718	0	1718	0	0	5-8	5-8
J	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B- C	-2110 / 0	-84.9	-84.9	0.32 (1)	4.39
C- D	-1933 / 0	-84.9	-84.9	0.31 (1)	4.56
D- E	-1954 / 0	-84.9	-84.9	0.40 (1)	4.42
E- F	-1954 / 0	-84.9	-84.9	0.40 (1)	4.42
F- G	-1933 / 0	-84.9	-84.9	0.31 (1)	4.56
G- H	-2110 / 0	-84.9	-84.9	0.32 (1)	4.39
H- I	0 / 28	-84.9	-84.9	0.11 (1)	10.00
P- B	-1676 / 0	0.0	0.0	0.17 (1)	6.41
J- H	-1676 / 0	0.0	0.0	0.17 (1)	6.41
P- O	0 / 0	-18.5	-18.5	0.10 (4)	10.00
O- N	0 / 1859	-18.5	-18.5	0.37 (1)	10.00
N- M	0 / 1666	-18.5	-18.5	0.34 (1)	10.00
M- L	0 / 1666	-18.5	-18.5	0.34 (1)	10.00
L- K	0 / 1859	-18.5	-18.5	0.37 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
O- C	-257 / 0	0.07 (1)
C- N	-249 / 0	0.17 (1)
N- D	0 / 264	0.06 (1)
D- M	0 / 441	0.10 (1)
M- E	-584 / 0	0.45 (1)
M- F	0 / 441	0.10 (1)
L- F	0 / 264	0.06 (1)
L- G	-249 / 0	0.17 (1)
K- G	-257 / 0	0.07 (1)
B- O	0 / 1891	0.43 (1)
K- H	0 / 1891	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.37/1.00 (N-O:1) , WB=0.45/1.00 (E-M:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (O) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125119

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420187	T22S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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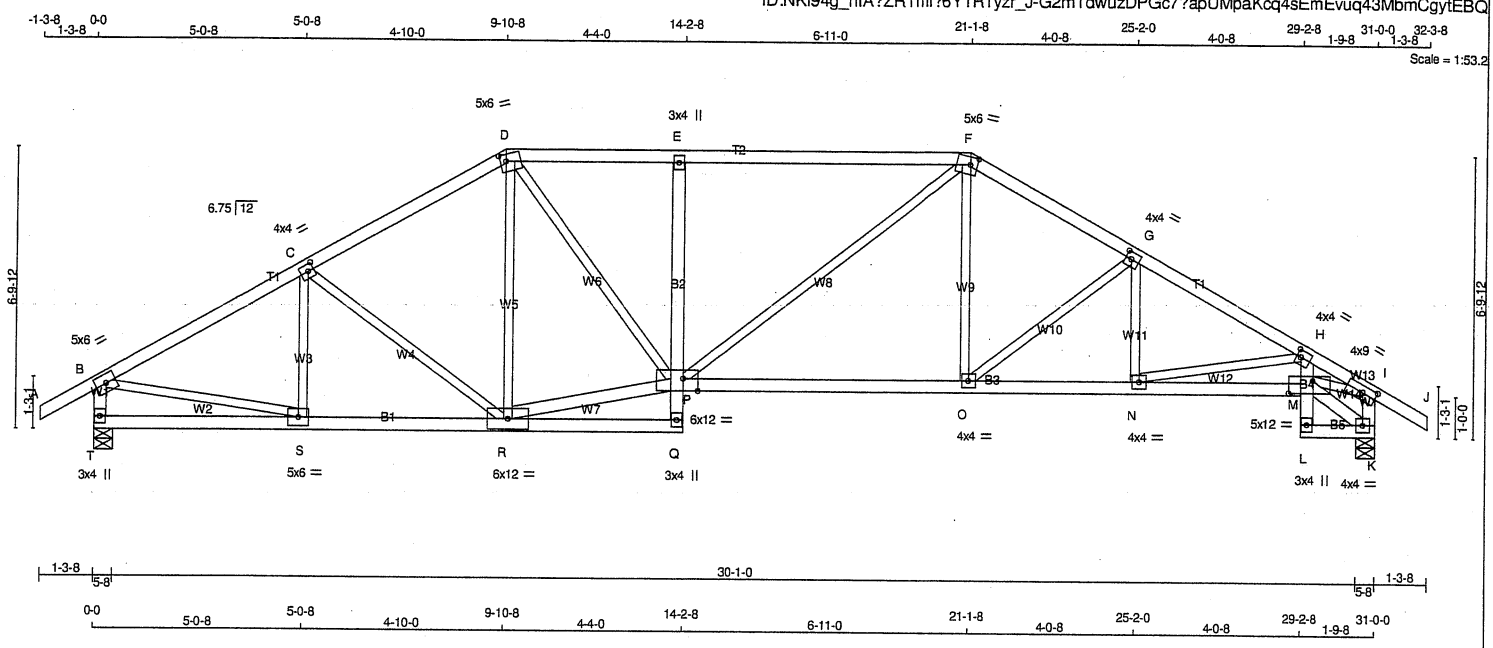

TOTAL WEIGHT = 139 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW-t	MT20	4.0	9.0	2.00	4.00
K	BMVW1-t	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-I	MT20	5.0	12.0	3.75	7.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-I	MT20	6.0	12.0	Edge	4.25
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	6.0	12.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

~~NOTES~~ (1)
1)

Structural component only
DWG# T-2125137

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1715	0	1715	0	0	5-8	5-8
K	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1215	784 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
K	1219	787 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B- C	-2107 / 0	-84.9	-84.9	0.32 (1)	4.39
C- D	-1929 / 0	-84.9	-84.9	0.31 (1)	4.56
D- E	-2286 / 0	-84.9	-84.9	0.46 (1)	4.03
E- F	-2293 / 0	-84.9	-84.9	0.61 (1)	3.87
F- G	-2267 / 0	-84.9	-84.9	0.26 (1)	4.33
G- H	-2712 / 0	-84.9	-84.9	0.31 (1)	3.96
H- I	-3395 / 0	-84.9	-84.9	0.20 (1)	3.67
I- J	0 / 28	-84.9	-84.9	0.11 (1)	10.00
T- B	-1673 / 0	0.0	0.0	0.17 (1)	6.42
K- I	-1567 / 0	0.0	0.0	0.16 (1)	6.58
T- S	0 / 0	-18.5	-18.5	0.09 (4)	10.00
S- R	0 / 1857	-18.5	-18.5	0.35 (1)	10.00
R- Q	0 / 40	-18.5	-18.5	0.09 (4)	10.00
Q- P	0 / 33	0.0	0.0	0.06 (1)	10.00
P- E	-613 / 0	0.0	0.0	0.24 (1)	7.81
P- O	0 / 1963	-18.5	-18.5	0.42 (1)	10.00
O- N	0 / 2390	-18.5	-18.5	0.49 (1)	10.00
N- M	0 / 3233	-18.5	-18.5	0.57 (1)	10.00
L- M	0 / 17	0.0	0.0	0.31 (1)	10.00
M- H	0 / 381	0.0	0.0	0.37 (1)	10.00
L- K	0 / 199	-18.5	-18.5	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-254 / 0	0.06 (1)
C- R	-251 / 0	0.17 (1)
R- D	-137 / 12	0.11 (1)
R- P	0 / 1664	0.27 (1)
D- P	0 / 1037	0.23 (1)
P- F	0 / 418	0.09 (1)
O- F	0 / 472	0.11 (1)
O- G	-552 / 0	0.25 (1)
N- G	0 / 123	0.03 (4)
N- H	-859 / 0	0.26 (1)
B- S	0 / 1889	0.43 (1)
M- K	-242 / 0	0.03 (1)
M- I	0 / 2940	0.66 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.61/1.00 (E-F:1) , BC=0.57/1.00 (M-N:1) , WB=0.66/1.00 (I-M:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.57 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T23	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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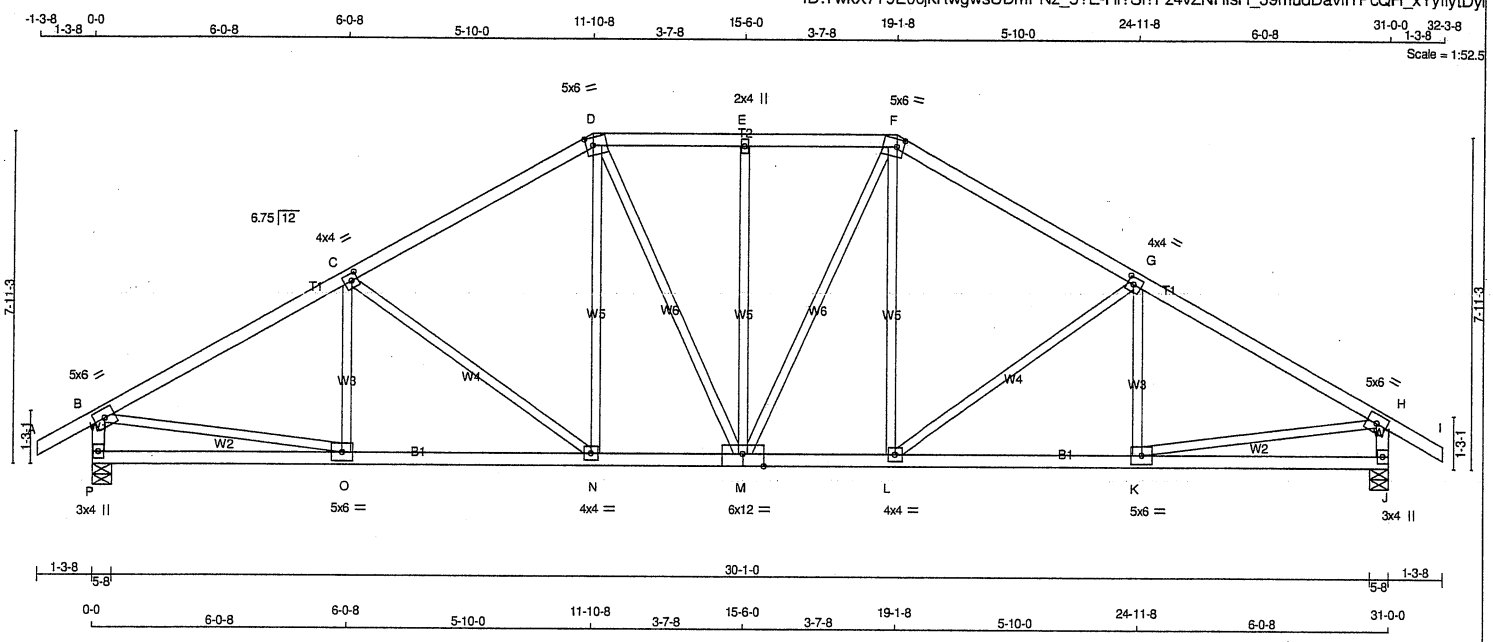

TOTAL WEIGHT = 2 X 139 = 278 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BSWWW-t	MT20	6.0	12.0	Edge	6.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1718	0	1718	0	0	5-8	5-8
J	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	
B- C	-2139 / 0	-84.9	-84.9	0.48 (1)	4.19	
C- D	-1797 / 0	-84.9	-84.9	0.44 (1)	4.53	
D- E	-1641 / 0	-84.9	-84.9	0.16 (1)	5.04	
E- F	-1641 / 0	-84.9	-84.9	0.16 (1)	5.04	
F- G	-1797 / 0	-84.9	-84.9	0.44 (1)	4.53	
G- H	-2139 / 0	-84.9	-84.9	0.48 (1)	4.19	
H- I	0 / 28	-84.9	-84.9	0.11 (1)	10.00	
P- B	-1671 / 0	0.0	0.0	0.17 (1)	6.42	
J- H	-1671 / 0	0.0	0.0	0.17 (1)	6.42	
P- O	0 / 0	-18.5	-18.5	0.16 (4)	10.00	
O- N	0 / 1889	-18.5	-18.5	0.38 (1)	10.00	
N- M	0 / 1544	-18.5	-18.5	0.30 (1)	10.00	
M- L	0 / 1544	-18.5	-18.5	0.30 (1)	10.00	
L- K	0 / 1889	-18.5	-18.5	0.38 (1)	10.00	
K- J	0 / 0	-18.5	-18.5	0.16 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-173 / 35	0.06 (1)
C- N	-434 / 0	0.46 (1)
N- D	0 / 353	0.08 (1)
D- M	0 / 226	0.05 (1)
M- E	-366 / 0	0.44 (1)
M- F	0 / 226	0.05 (1)
L- F	0 / 353	0.08 (1)
L- G	-434 / 0	0.46 (1)
K- G	-173 / 35	0.06 (1)
B- O	0 / 1912	0.43 (1)
K- H	0 / 1912	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.38/1.00 (N-O:1) , WB=0.46/1.00 (C-N:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125120

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420187	T23S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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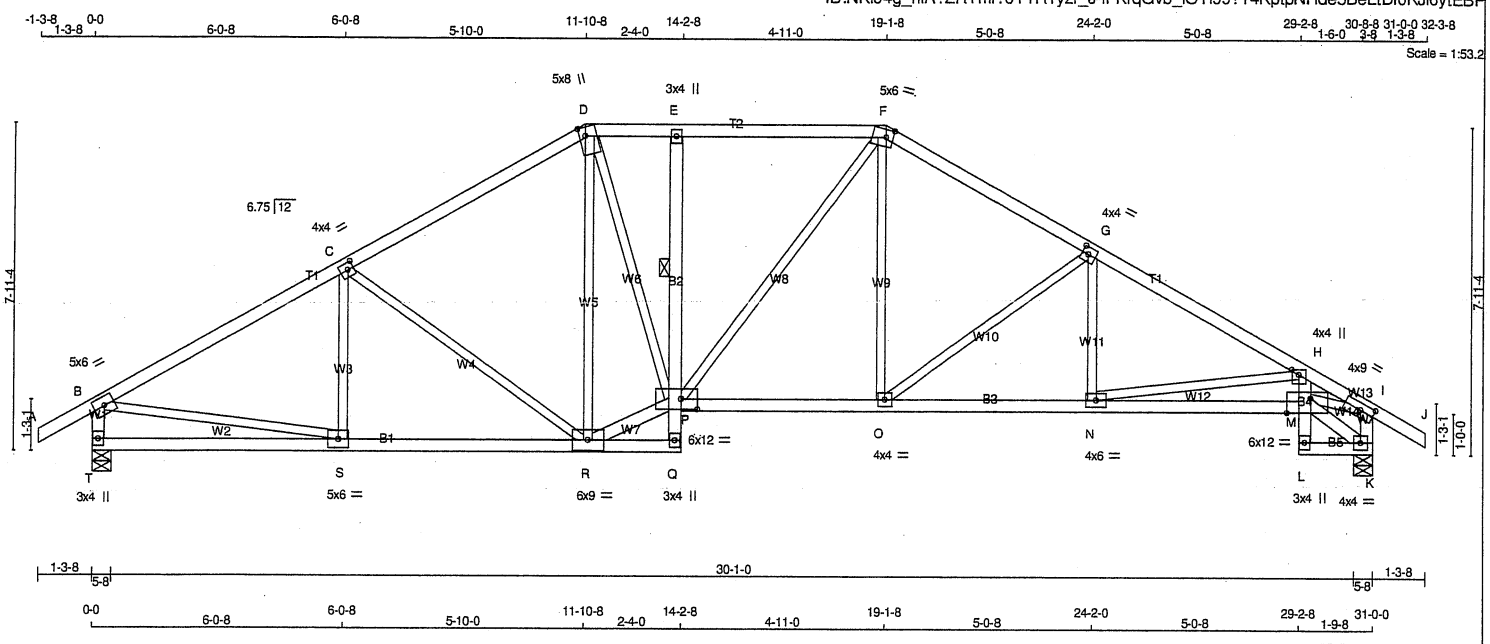

TOTAL WEIGHT = 144 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW+m	MT20	5.0	8.0	2.50	1.75
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW+p	MT20	4.0	4.0	1.50	2.00
I	TMVW-t	MT20	4.0	9.0	2.00	4.00
K	BMVW1-t	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-l	MT20	6.0	12.0	4.25	7.00
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-l	MT20	6.0	12.0	3.00	4.75
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	6.0	9.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125138

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1715	0	1715	0	0	5-8	5-8
K	1720	0	1720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1215	784 / 0	0 / 0	0 / 0	0 / 0	431 / 0	0 / 0
K	1219	787 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B- C	-2134 / 0	-84.9	-84.9	0.48 (1)	4.19
C- D	-1795 / 0	-84.9	-84.9	0.44 (1)	4.53
D- E	-1869 / 0	-84.9	-84.9	0.22 (1)	4.71
E- F	-1874 / 0	-84.9	-84.9	0.30 (1)	4.62
F- G	-2050 / 0	-84.9	-84.9	0.35 (1)	4.40
G- H	-2631 / 0	-84.9	-84.9	0.41 (1)	3.92
H- I	-3447 / 0	-84.9	-84.9	0.20 (1)	3.64
I- J	0 / 28	-84.9	-84.9	0.11 (1)	10.00
T- B	-1667 / 0	0.0	0.0	0.17 (1)	6.42
K- I	-1565 / 0	0.0	0.0	0.16 (1)	6.59
T- S	0 / 0	-18.5	-18.5	0.15 (4)	10.00
S- R	0 / 1885	-18.5	-18.5	0.38 (1)	10.00
R- Q	0 / 33	-18.5	-18.5	0.10 (4)	10.00
Q- P	-8 / 0	0.0	0.0	0.03 (1)	10.00
P- E	-411 / 0	0.0	0.0	0.05 (1)	6.25
P- O	0 / 1770	-18.5	-18.5	0.34 (1)	10.00
O- N	0 / 2319	-18.5	-18.5	0.43 (1)	10.00
N- M	0 / 3294	-18.5	-18.5	0.59 (1)	10.00
L- M	0 / 17	0.0	0.0	0.32 (1)	10.00
M- H	0 / 370	0.0	0.0	0.37 (1)	10.00
L- K	0 / 202	-18.5	-18.5	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-177 / 34	0.06 (1)
C- R	-431 / 0	0.46 (1)
R- D	-362 / 0	0.43 (1)
R- P	0 / 1663	0.27 (1)
D- P	0 / 1076	0.24 (1)
P- F	0 / 172	0.04 (1)
O- F	0 / 514	0.12 (1)
O- G	-691 / 0	0.51 (1)
N- G	0 / 173	0.05 (4)
N- H	-988 / 0	0.46 (1)
B- S	0 / 1908	0.43 (1)
M- K	-246 / 0	0.03 (1)
M- I	0 / 2997	0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.59/1.00 (M-N:1) , WB=0.67/1.00 (I-M:1) , SSI=0.25/1.00 (H-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.56 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T24	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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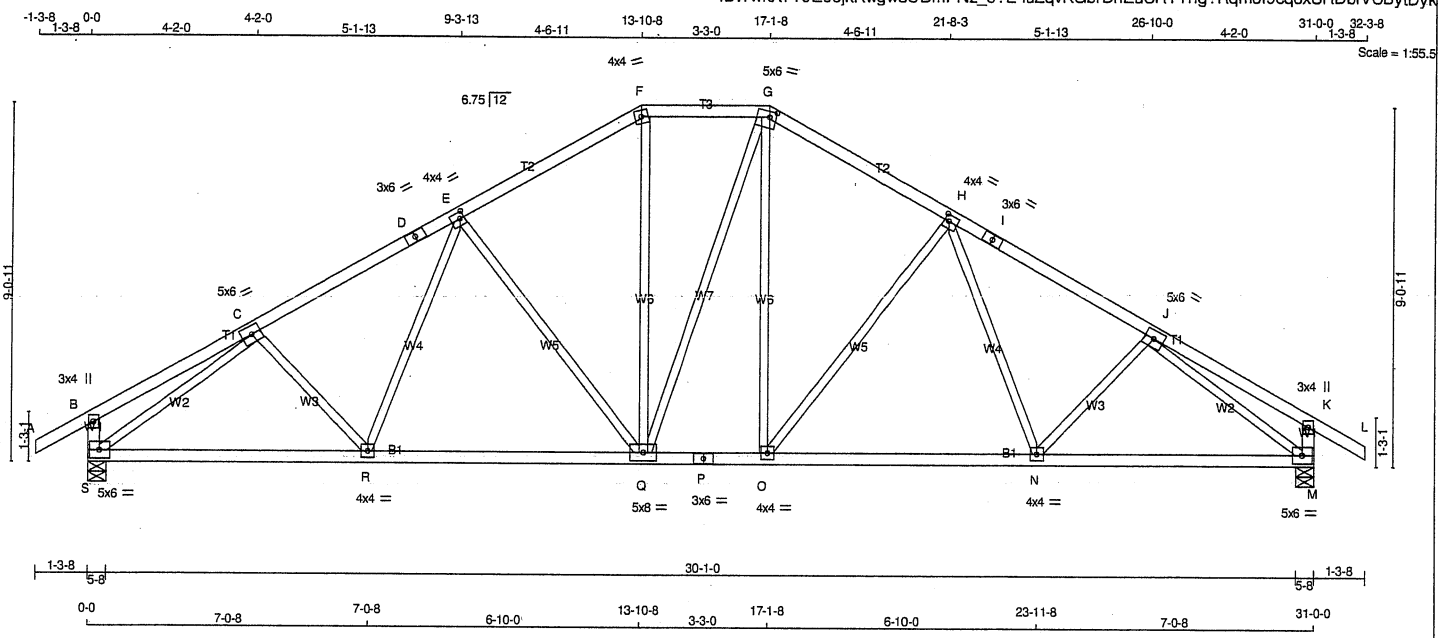

TOTAL WEIGHT = 2 X 139 = 278 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TTW-m	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	2.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	5.0	6.0		
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		
N, O, R						
N	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
S	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125121

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1718	0	1718	0	0	5-8	5-8
M	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
M	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	C- R	-11 / 56	0.02 (4)
B- C	0 / 19	-84.9	-84.9	0.22 (1)	10.00	R- E	0 / 164	0.05 (4)
C- D	-2031 / 0	-84.9	-84.9	0.29 (1)	4.50	E- Q	-524 / 0	0.64 (1)
D- E	-2031 / 0	-84.9	-84.9	0.29 (1)	4.50	Q- F	0 / 506	0.11 (1)
E- F	-1633 / 0	-84.9	-84.9	0.26 (1)	4.94	Q- G	0 / 4	0.00 (1)
F- G	-1408 / 0	-84.9	-84.9	0.14 (1)	5.36	O- G	0 / 502	0.11 (1)
G- H	-1631 / 0	-84.9	-84.9	0.26 (1)	4.94	O- H	-526 / 0	0.64 (1)
H- I	-2032 / 0	-84.9	-84.9	0.29 (1)	4.50	H- N	0 / 166	0.05 (4)
I- J	-2032 / 0	-84.9	-84.9	0.29 (1)	4.50	N- J	-11 / 56	0.02 (4)
J- K	0 / 19	-84.9	-84.9	0.22 (1)	10.00	S- C	-2275 / 0	0.99 (1)
K- L	0 / 28	-84.9	-84.9	0.11 (1)	10.00	J- M	-2276 / 0	0.99 (1)
S- B	-249 / 0	0.0	0.0	0.03 (1)	7.81			
M- K	-249 / 0	0.0	0.0	0.03 (1)	7.81			
S- R	0 / 1781	-18.5	-18.5	0.40 (1)	10.00			
R- Q	0 / 1718	-18.5	-18.5	0.39 (1)	10.00			
Q- P	0 / 1407	-18.5	-18.5	0.29 (1)	10.00			
P- O	0 / 1407	-18.5	-18.5	0.29 (1)	10.00			
O- N	0 / 1717	-18.5	-18.5	0.39 (1)	10.00			
N- M	0 / 1782	-18.5	-18.5	0.40 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.29/1.00 (H-J:1) , BC=0.40/1.00 (M-N:1) , WB=0.99/1.00 (J-M:1) , SSI=0.16/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.51 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T25	8	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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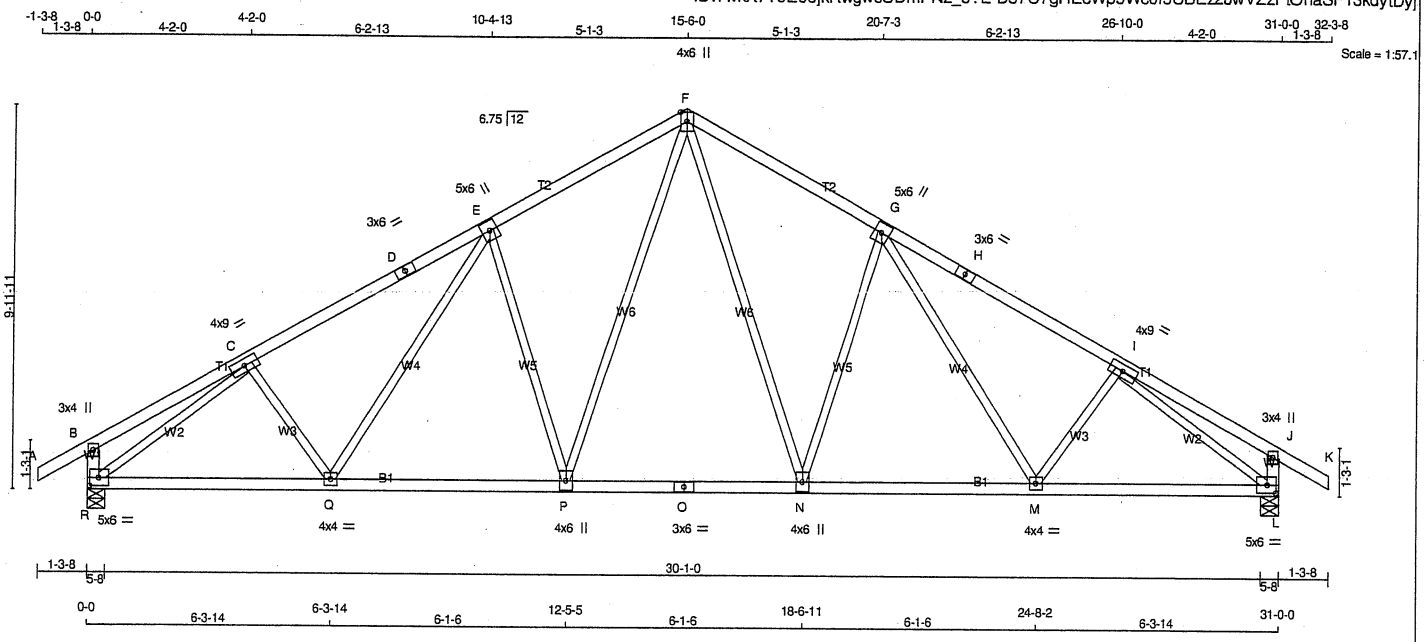

TOTAL WEIGHT = 8 X 135 = 1079 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	9.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	5.0	6.0		
F	TTWW+p	MT20	4.0	6.0	Edge	
G	TMWW+t	MT20	5.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	9.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125122

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1718	0	1718	0	0	5-8	5-8
L	1718	0	1718	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1217	785 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	F- N	0 / 716	0.16 (1)
B- C	0 / 26	-84.9	-84.9	0.30 (1)	10.00	N- G	-595 / 0	0.58 (1)
C- D	-2026 / 0	-84.9	-84.9	0.41 (1)	4.39	G- M	0 / 198	0.04 (1)
D- E	-2026 / 0	-84.9	-84.9	0.41 (1)	4.39	M- I	-56 / 46	0.02 (4)
E- F	-1747 / 0	-84.9	-84.9	0.36 (1)	4.69	P- F	0 / 716	0.16 (1)
F- G	-1747 / 0	-84.9	-84.9	0.36 (1)	4.69	E- P	-595 / 0	0.58 (1)
G- H	-2026 / 0	-84.9	-84.9	0.41 (1)	4.39	Q- E	0 / 198	0.04 (1)
H- I	-2026 / 0	-84.9	-84.9	0.41 (1)	4.39	C- Q	-56 / 46	0.02 (4)
I- J	0 / 26	-84.9	-84.9	0.30 (1)	10.00	R- C	-2303 / 0	0.99 (1)
J- K	0 / 28	-84.9	-84.9	0.11 (1)	10.00	I- L	-2303 / 0	0.99 (1)
R- B	-235 / 0	0.0	0.0	0.02 (1)	7.81			
L- J	-235 / 0	0.0	0.0	0.02 (1)	7.81			
R- Q	0 / 1803	-18.5	-18.5	0.38 (1)	10.00			
Q- P	0 / 1669	-18.5	-18.5	0.34 (1)	10.00			
P- O	0 / 1282	-18.5	-18.5	0.28 (1)	10.00			
O- N	0 / 1282	-18.5	-18.5	0.28 (1)	10.00			
N- M	0 / 1669	-18.5	-18.5	0.34 (1)	10.00			
M- L	0 / 1803	-18.5	-18.5	0.38 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.41/1.00 (C-E:1) , BC=0.38/1.00 (Q-R:1) , WB=0.99/1.00 (I-L:1) , SSI=0.19/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.44 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T26	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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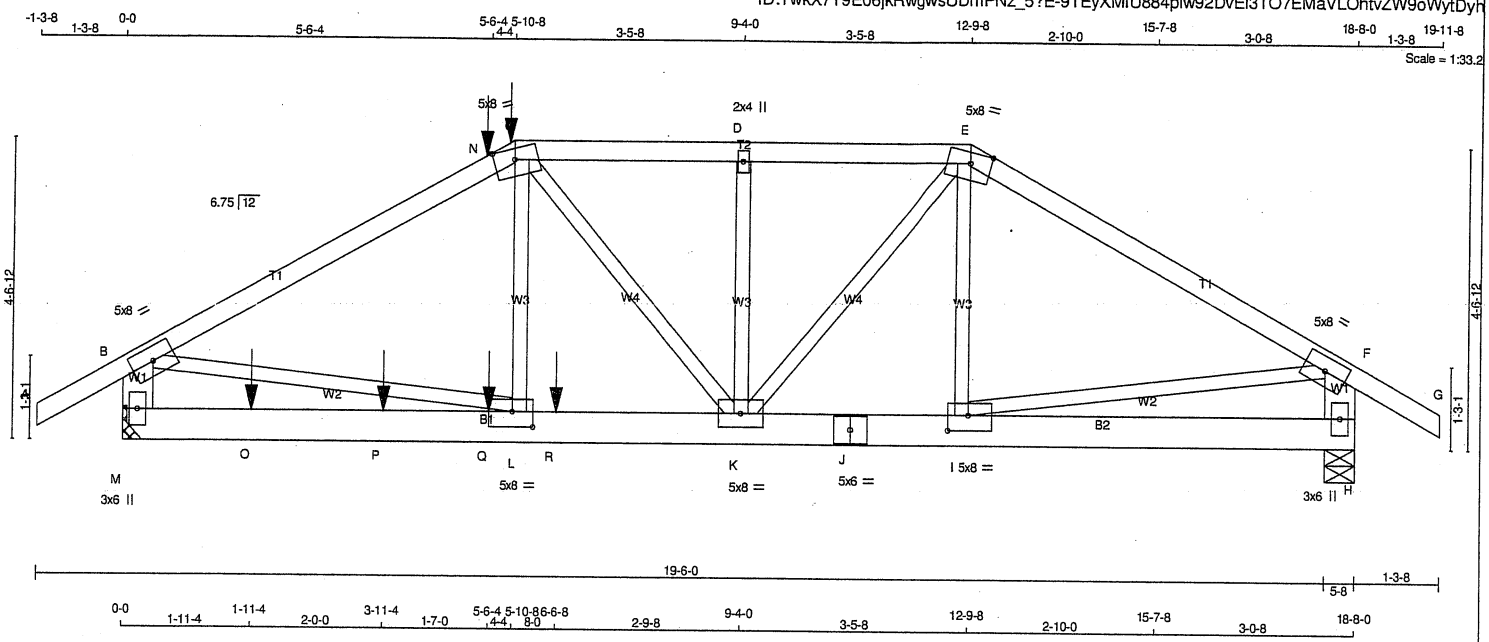

TOTAL WEIGHT = 89 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	2.00	3.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.00	3.75
F	TMVW-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	8.0	2.75	3.75
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	8.0	2.75	3.75
M	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125123

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	2231	0	2231	0	0	MECHANICAL	
H	1655	0	1655	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1580	1026 / 0	0 / 0	0 / 0	0 / 0	554 / 0	0 / 0
H	1171	768 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.12 (1)	10.00	L- C	0 / 815	0.20 (1)
B- N	-2888 / 0	-84.9	-84.9	0.97 (1)	3.03	C- K	-243 / 0	0.11 (1)
N- C	-2888 / 0	-84.9	-84.9	0.97 (1)	3.03	K- D	-339 / 0	0.10 (1)
C- D	-2405 / 0	-84.9	-84.9	0.24 (1)	4.17	K- E	0 / 1041	0.26 (1)
D- E	-2405 / 0	-84.9	-84.9	0.24 (1)	4.17	I- E	-190 / 21	0.06 (1)
E- F	-2004 / 0	-84.9	-84.9	0.76 (1)	3.84	B- L	0 / 2566	0.64 (1)
F- G	0 / 28	-84.9	-84.9	0.12 (1)	10.00	I- F	0 / 1764	0.44 (1)
M- B	-2171 / 0	0.0	0.0	0.15 (1)	6.88			
H- F	-1601 / 0	0.0	0.0	0.11 (1)	7.74			
M- O	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
O- P	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
P- Q	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
Q- L	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
L- R	0 / 2560	-18.5	-18.5	0.66 (1)	10.00			
R- K	0 / 2560	-18.5	-18.5	0.66 (1)	10.00			
K- J	0 / 1741	-18.5	-18.5	0.29 (1)	10.00			
J- I	0 / 1741	-18.5	-18.5	0.29 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-214	-214	---	BACK	VERT	TOTAL	---	C1
N	5-6-4	-89	-89	---	BACK	VERT	TOTAL	---	C1
O	1-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
P	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Q	5-6-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
R	6-6-8	-854	-854	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.97/1.00 (B-C:1) , BC=0.66/1.00 (K-L:1) , WB=0.64/1.00 (B-L:1) , SSI=0.70/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.66 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T27	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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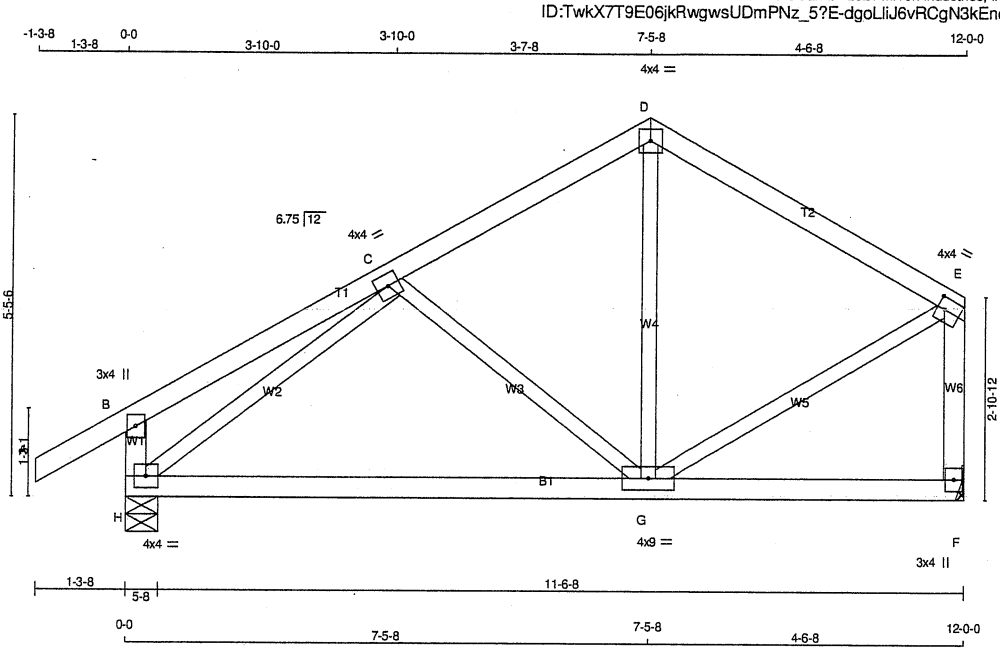

TOTAL WEIGHT = 50 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	620	0	620	0	0	MECHANICAL	
H	736	0	736	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	440	279 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0
H	520	343 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	C- G	-237 / 0	0.09 (1)
B- C	0 / 18	-84.9	-84.9	0.19 (1)	10.00	G- D	0 / 100	0.03 (4)
C- D	-429 / 0	-84.9	-84.9	0.15 (1)	6.25	H- C	-699 / 0	0.26 (1)
D- E	-411 / 0	-84.9	-84.9	0.23 (1)	6.25	G- E	0 / 412	0.09 (1)
H- B	-237 / 0	0.0	0.0	0.02 (1)	7.81			
F- E	-598 / 0	0.0	0.0	0.09 (1)	7.81			
H- G	0 / 542	-18.5	-18.5	0.27 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.22 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.23/1.00 (D-E:1) , BC=0.27/1.00 (G-H:4) , WB=0.26/1.00 (C-H:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (H) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

Structural component only
DWG# T-2125124

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T28	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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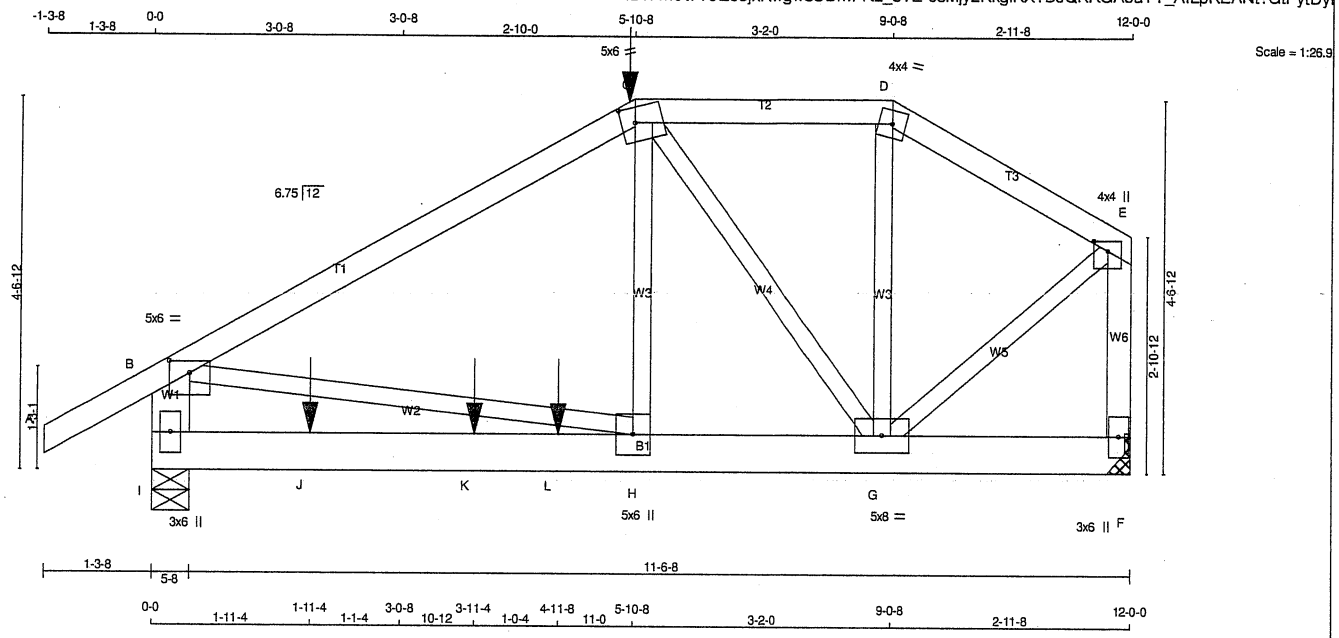

TOTAL WEIGHT = 60 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.50	2.00
F	BMV1+p	MT20	3.0	6.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMWW+t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1542	0	1542	0	0	IN-SX	IN-SX
F	1214	0	1214	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1095	690 / 0	0 / 0	0 / 0	0 / 0	405 / 0	0 / 0
F	861	547 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.12 (1)	10.00	H- C	0 / 1055	0.26 (1)
B- C	-1632 / 0	-84.9	-84.9	0.70 (1)	4.24	C- G	-1179 / 0	0.50 (1)
C- D	-751 / 0	-84.9	-84.9	0.17 (1)	6.25	G- D	0 / 163	0.04 (1)
D- E	-858 / 0	-84.9	-84.9	0.15 (1)	6.25	G- E	0 / 964	0.24 (1)
I- B	-1371 / 0	0.0	0.0	0.10 (1)	7.81	B- H	0 / 1436	0.36 (1)
F- E	-1157 / 0	0.0	0.0	0.18 (1)	7.37			
I- J	0 / 0	-18.5	-18.5	0.39 (1)	10.00			
J- K	0 / 0	-18.5	-18.5	0.39 (1)	10.00			
K- L	0 / 0	-18.5	-18.5	0.39 (1)	10.00			
L- H	0 / 0	-18.5	-18.5	0.39 (1)	10.00			
H- G	0 / 1448	-18.5	-18.5	0.57 (1)	10.00			
G- F	0 / 0	-18.5	-18.5	0.06 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-218	-218	---	FRONT	VERT	TOTAL	---	C1
J	1-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
K	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
L	4-11-8	-737	-737	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.70/1.00 (B-C:1) , BC=0.57/1.00 (G-H:1) , WB=0.50/1.00 (C-G:1) , SSI=0.68/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (H) (INPUT = 0.90)
JSI METAL= 0.52 (H) (INPUT = 1.00)

Structural component only
DWG# T-2125125

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T29	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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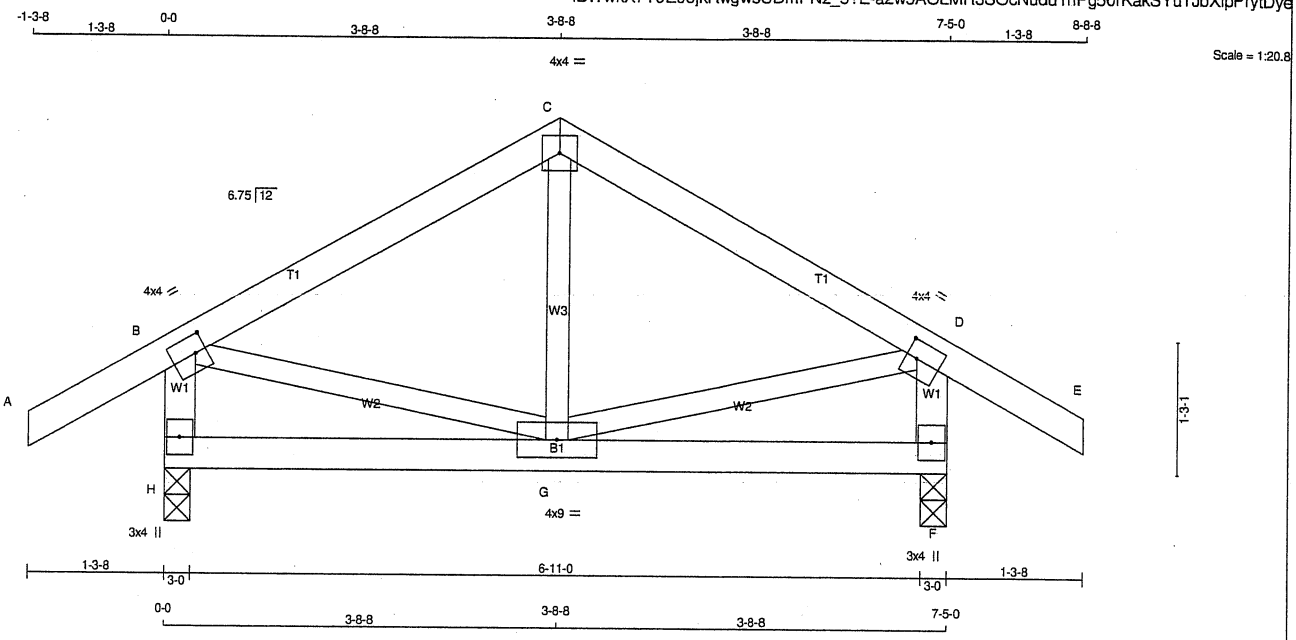

TOTAL WEIGHT = 2 X 31 = 63 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	499	0	499	0	0	3-0	3-0
F	499	0	499	0	0	3-0	3-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	352	236 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
F	352	236 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	G- C	-41 / 46	0.02 (4)
B- C	-279 / 0	-84.9	-84.9	0.15 (1)	6.25	B- G	0 / 251	0.06 (1)
C- D	-279 / 0	-84.9	-84.9	0.15 (1)	6.25	G- D	0 / 251	0.06 (1)
D- E	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
H- B	-472 / 0	0.0	0.0	0.05 (1)	7.81			
F- D	-472 / 0	0.0	0.0	0.05 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.07/1.00 (G-H:4) , WB=0.06/1.00 (D-G:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (D) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125126

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T30	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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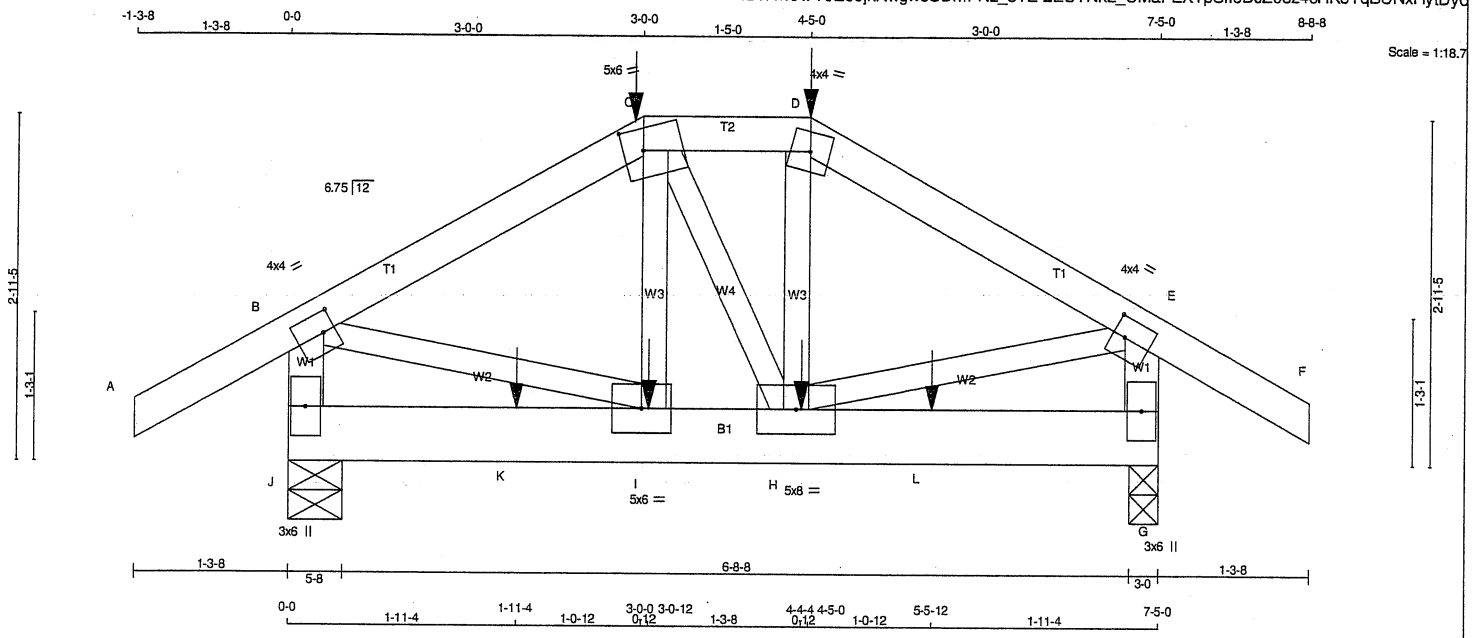

TOTAL WEIGHT = 38 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	655	0	655	0	0	5-8	5-8
G	655	0	655	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	462	311 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0
G	462	311 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-84.9	-84.9	0.12 (1)	10.00	I- C	-75 / 12	0.01 (1)
B- C	-517 / 0	-84.9	-84.9	0.15 (1)	6.25	C- H	-8 / 2	0.00 (1)
C- D	-443 / 0	-84.9	-84.9	0.03 (1)	6.25	H- D	-84 / 15	0.02 (1)
D- E	-515 / 0	-84.9	-84.9	0.15 (1)	6.25	B- I	0 / 467	0.12 (1)
E- F	0 / 28	-84.9	-84.9	0.12 (1)	10.00	H- E	0 / 465	0.12 (1)
J- B	-622 / 0	0.0	0.0	0.07 (1)	7.81			
G- E	-619 / 0	0.0	0.0	0.07 (1)	7.81			
J- K	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
K- I	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
I- H	0 / 447	-18.5	-18.5	0.07 (1)	10.00			
H- L	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
L- G	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-0-0	-104	-104	---	BACK	VERT	TOTAL	---	C1
D	4-5-0	-104	-104	---	BACK	VERT	TOTAL	---	C1
H	4-4-4	-5	-5	---	BACK	VERT	TOTAL	---	C1
I	3-0-12	-5	-5	---	BACK	VERT	TOTAL	---	C1
K	1-11-4	-1	-1	---	BACK	VERT	TOTAL	---	C1
L	5-5-12	-1	-1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.07/1.00 (H-I:1) , WB=0.12/1.00 (B-I:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125127

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420186	T31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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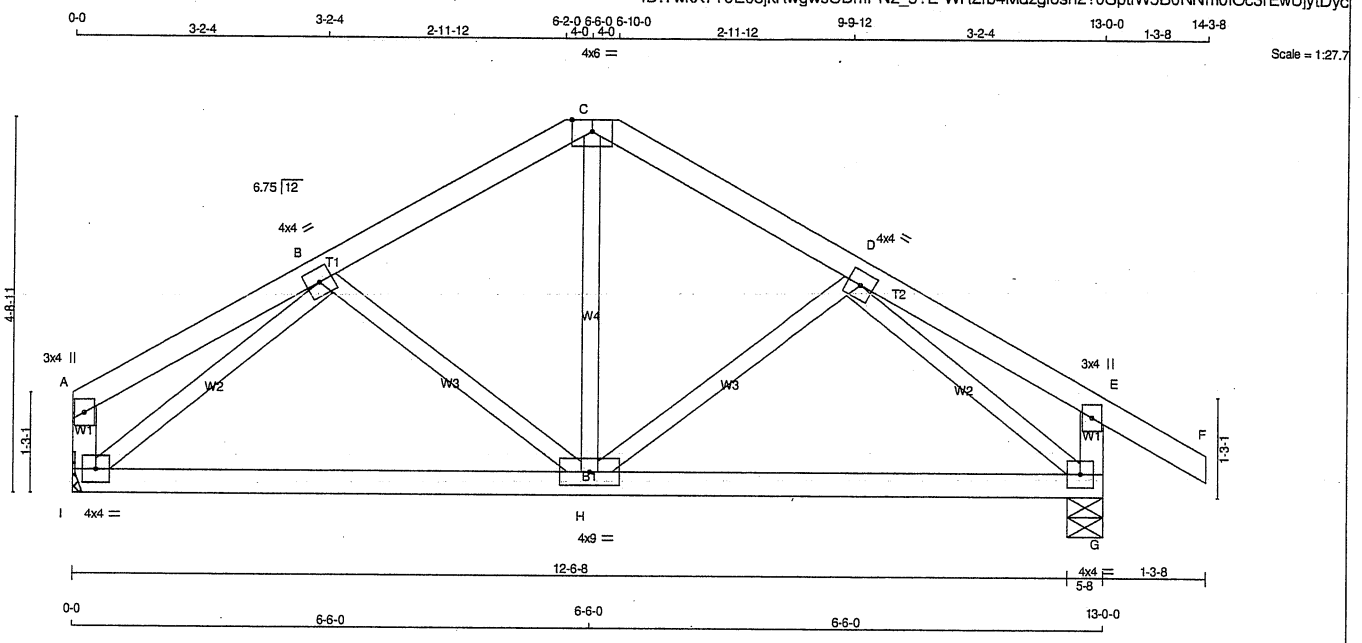

TOTAL WEIGHT = 52 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TMTMWm	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	672	0	672	0	0	MECHANICAL	
G	788	0	788	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	477	303 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0
G	557	366 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 16	-84.9	-84.9	0.14 (1)	10.00	B- H	-143 / 0	0.05 (1)
B- C	-581 / 0	-84.9	-84.9	0.11 (1)	6.25	I- B	-803 / 0	0.22 (1)
C- D	-581 / 0	-84.9	-84.9	0.11 (1)	6.25	H- C	0 / 324	0.07 (1)
D- E	0 / 16	-84.9	-84.9	0.14 (1)	10.00	H- D	-143 / 0	0.05 (1)
E- F	0 / 28	-84.9	-84.9	0.11 (1)	10.00	D- G	-803 / 0	0.22 (1)
I- A	-99 / 0	0.0	0.0	0.01 (1)	7.81			
G- E	-215 / 0	0.0	0.0	0.02 (1)	7.81			
I- H	0 / 607	-18.5	-18.5	0.27 (4)	10.00			
H- G	0 / 607	-18.5	-18.5	0.27 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.14/1.00 (D-E:1) , BC=0.27/1.00 (G-H:4) , WB=0.22/1.00 (D-G:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (G) (INPUT = 0.90)
JSI METAL= 0.25 (D) (INPUT = 1.00)

Structural component only
DWG# T-2125128

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T32	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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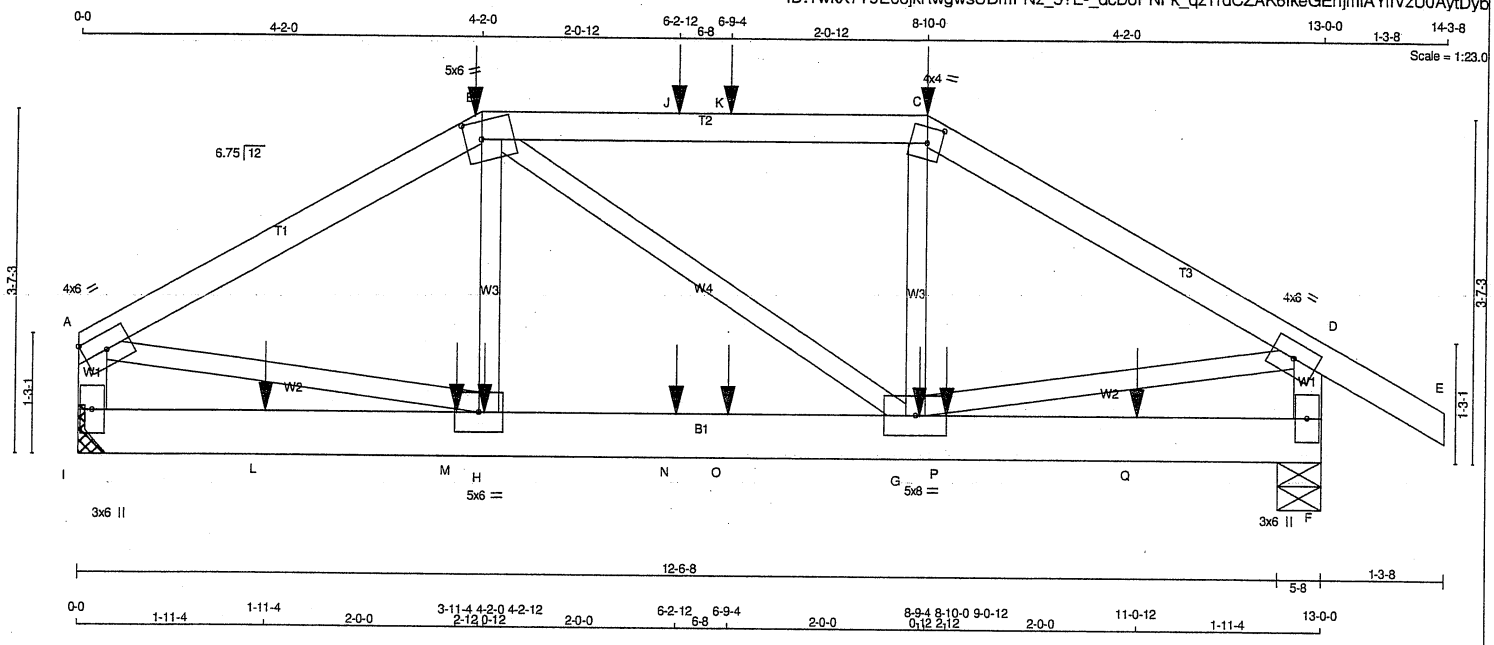

TOTAL WEIGHT = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B	TTWW-m	MT20	5.0	6.0	2.25	2.00
C	TTW-m	MT20	4.0	4.0	2.00	1.75
D	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	6.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2125129

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1470	0	1470	0	0	MECHANICAL	
F	1585	0	1585	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1039	686 / 0	0 / 0	0 / 0	0 / 0	353 / 0	0 / 0
F	1118	750 / 0	0 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1618 / 0	-84.9	-84.9	0.34 (1)		4.79	H- B	0 / 108	0.04 (4)
B- J	-1410 / 0	-84.9	-84.9	0.55 (1)		4.66	B- G	0 / 3	0.00 (4)
J- K	-1410 / 0	-84.9	-84.9	0.55 (1)		4.66	G- C	0 / 110	0.04 (4)
K- C	-1410 / 0	-84.9	-84.9	0.55 (1)		4.66	A- H	0 / 1437	0.36 (1)
C- D	-1618 / 0	-84.9	-84.9	0.34 (1)		4.79	G- D	0 / 1435	0.36 (1)
D- E	0 / 28	-84.9	-84.9	0.12 (1)		10.00			
I- A	-1256 / 0	0.0	0.0	0.14 (1)		7.14			
F- D	-1365 / 0	0.0	0.0	0.15 (1)		6.91			
I- L	0 / 0	-18.5	-18.5	0.28 (1)		10.00			
L- M	0 / 0	-18.5	-18.5	0.28 (1)		10.00			
M- H	0 / 0	-18.5	-18.5	0.28 (1)		10.00			
H- N	0 / 1409	-18.5	-18.5	0.26 (1)		10.00			
N- O	0 / 1409	-18.5	-18.5	0.26 (1)		10.00			
O- G	0 / 1409	-18.5	-18.5	0.26 (1)		10.00			
G- P	0 / 0	-18.5	-18.5	0.29 (1)		10.00			
P- Q	0 / 0	-18.5	-18.5	0.29 (1)		10.00			
Q- F	0 / 0	-18.5	-18.5	0.29 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	4-2-0	-240	-240	---	BACK	VERT	TOTAL	---	C1
C	8-10-0	-240	-240	---	BACK	VERT	TOTAL	---	C1
G	8-9-4	-11	-11	---	BACK	VERT	TOTAL	---	C1
J	6-2-12	-33	-33	---	BACK	VERT	TOTAL	---	C1
K	6-9-4	-33	-33	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-254	-254	---	BACK	VERT	TOTAL	---	C1
M	3-11-4	-11	-11	---	BACK	VERT	TOTAL	---	C1
M	4-2-12	-11	-11	---	BACK	VERT	TOTAL	---	C1
N	6-2-12	-11	-11	---	BACK	VERT	TOTAL	---	C1
O	6-9-4	-11	-11	---	BACK	VERT	TOTAL	---	C1
P	9-0-12	-11	-11	---	BACK	VERT	TOTAL	---	C1
Q	11-0-12	-254	-254	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.29/1.00 (F-G:1) , WB=0.36/1.00 (A-H:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.41 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420186	T33	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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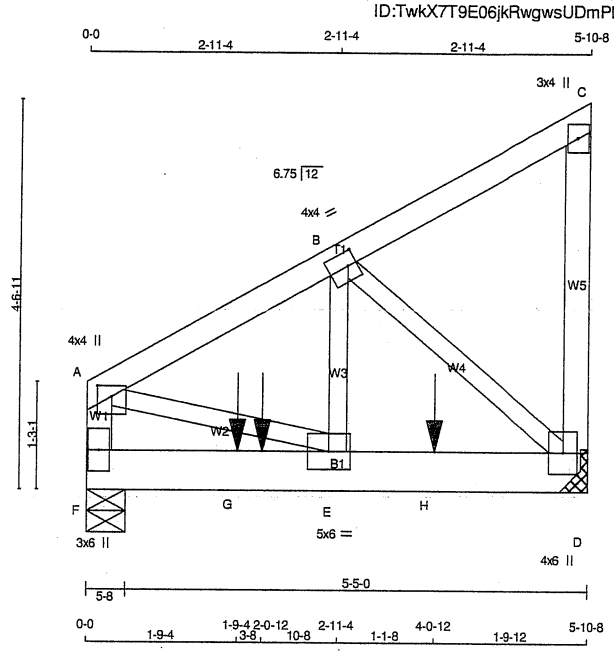

TOTAL WEIGHT = 2 X 30 = 60 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125130

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1296	0	1296	0	0	MECHANICAL	
F	1758	0	1758	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	916	605 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0
F	1240	831 / 0	0 / 0	0 / 0	0 / 0	409 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
A- B	-1455 / 0	-84.9 -84.9	0.06 (1)	6.25	E- B	0 / 1517	0.19 (1)
B- C	-11 / 0	-84.9 -84.9	0.05 (1)	6.25	B- D	-1680 / 0	0.21 (1)
D- C	-103 / 0	0.0 0.0	0.02 (1)	7.81	A- E	0 / 1330	0.16 (1)
F- A	-1195 / 0	0.0 0.0	0.07 (1)	7.81			
F- G	0 / 0	-18.5 -18.5	0.36 (1)	10.00			
G- E	0 / 0	-18.5 -18.5	0.36 (1)	10.00			
E- H	0 / 1277	-18.5 -18.5	0.27 (1)	10.00			
H- D	0 / 1277	-18.5 -18.5	0.27 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-9-4	-239	-239	---	TOP	VERT	TOTAL	---	C1
G	2-0-12	-1024	-1024	---	BACK	VERT	TOTAL	---	C1
H	4-0-12	-462	-462	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.07/1.00 (A-F:1) , BC=0.36/1.00 (E-F:1) , WB=0.21/1.00 (B-D:1) , SSI=0.43/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (A) (INPUT = 0.90)
JSI METAL= 0.29 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420186	T33	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only

DWG# T-2125130

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420186	T33Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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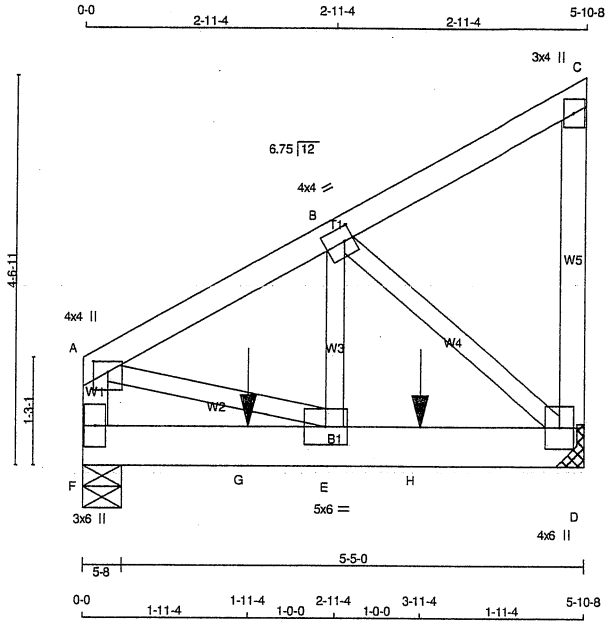


TOTAL WEIGHT = 2 X 30 = 60 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

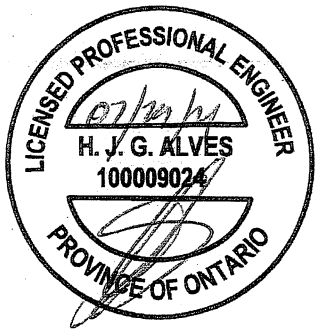
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		



Structural component only
DWG# T-2125131

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1892	0	1892	0	0	MECHANICAL	
F	1248	0	1248	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1338	882 / 0	0 / 0	0 / 0	0 / 0	456 / 0	0 / 0
F	880	591 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1573 / 0	-84.9	-84.9	0.06 (1)	6.25	E- B	0 / 1665	0.21 (1)
B- C	-10 / 0	-84.9	-84.9	0.05 (1)	6.25	B- D	-1816 / 0	0.23 (1)
D- C	-104 / 0	0.0	0.0	0.02 (1)	7.81	A- E	0 / 1438	0.18 (1)
F- A	-1284 / 0	0.0	0.0	0.07 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
E- H	0 / 1380	-18.5	-18.5	0.51 (1)	10.00			
H- D	0 / 1380	-18.5	-18.5	0.51 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-222	-222	---	TOP	VERT	TOTAL	---	C1
H	3-11-4	-1565	-1565	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.07/1.00 (A-F:1) , BC=0.51/1.00 (D-E:1) , WB=0.23/1.00 (B-D:1) , SSI=0.55/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (A) (INPUT = 0.90)
JSI METAL= 0.31 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420186	T33Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125131

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T35	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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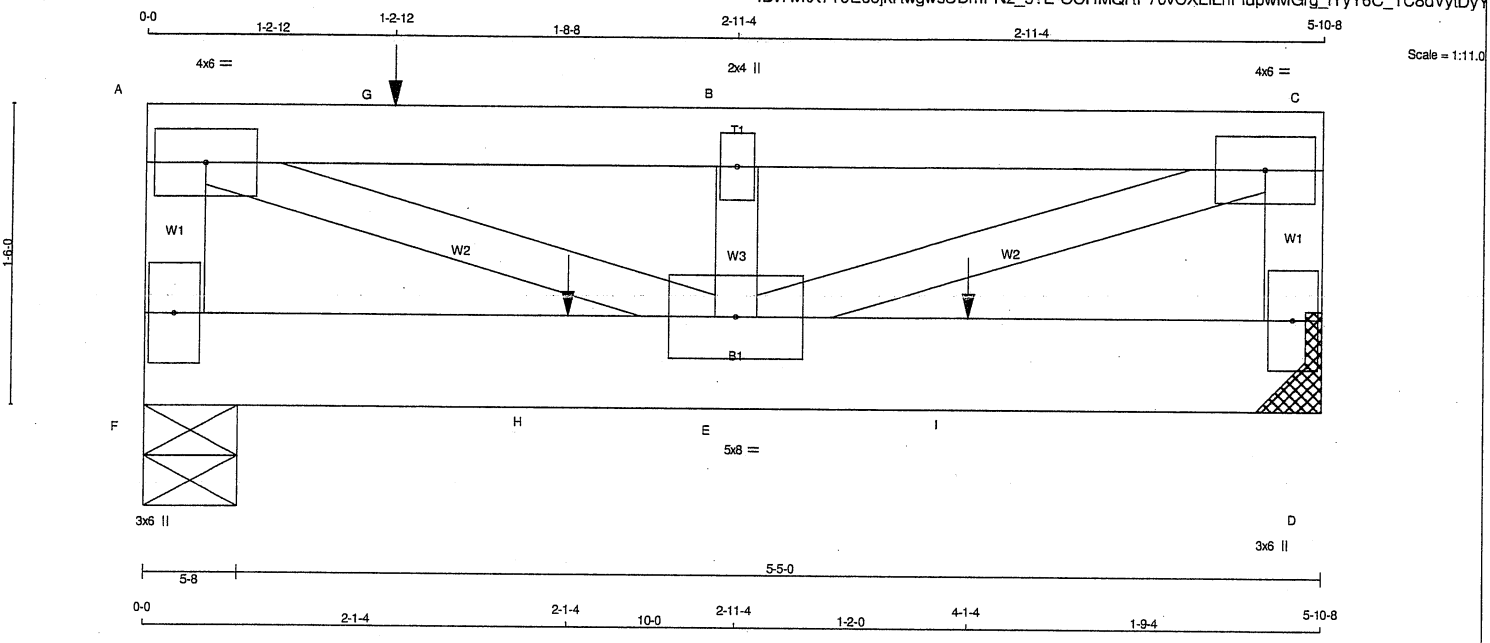

TOTAL WEIGHT = 2 X 24 = 48 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(14.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	5.0	8.0		

Structural component only
DWG# T-2125132

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1642	0	1642	0	0	5-8	5-8
D	1226	0	1226	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1167	732 / 0	0 / 0	0 / 0	0 / 0	436 / 0	0 / 0
D	869	558 / 0	0 / 0	0 / 0	0 / 0	311 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1218 / 0	0.0	0.0	0.07 (1)	7.81		A- E	0 / 2456	0.30 (1)
A- G	-2294 / 0	-84.9	-84.9	0.29 (1)	5.54		E- B	-475 / 0	0.04 (1)
G- B	-2294 / 0	-84.9	-84.9	0.29 (1)	5.54		E- C	0 / 2456	0.30 (1)
B- C	-2294 / 0	-84.9	-84.9	0.08 (1)	5.84				
D- C	-977 / 0	0.0	0.0	0.05 (1)	7.81				
F- H	0 / 0	-43.5	-43.5	0.28 (1)	10.00				
H- E	0 / 0	-18.5	-18.5	0.28 (1)	10.00				
E- I	0 / 0	-18.5	-18.5	0.15 (1)	10.00				
I- D	0 / 0	-18.5	-18.5	0.15 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-2-12	-293	-293	---	TOP	VERT	TOTAL	---	C1
H	2-1-4	-847	-847	---	BACK	VERT	TOTAL	---	C1
I	4-1-4	-425	-425	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.29/1.00 (A-B:1) , BC=0.28/1.00 (E-F:1) , WB=0.30/1.00 (A-E:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420186	T35	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:TwkX7T9E06jkRwgwsUDmPNz_5?E-OCHMQRP70vCXLILnFlupwMGrg_IYyY6C_TC8dVytDyY

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only

DWG# T-2125132

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	T36	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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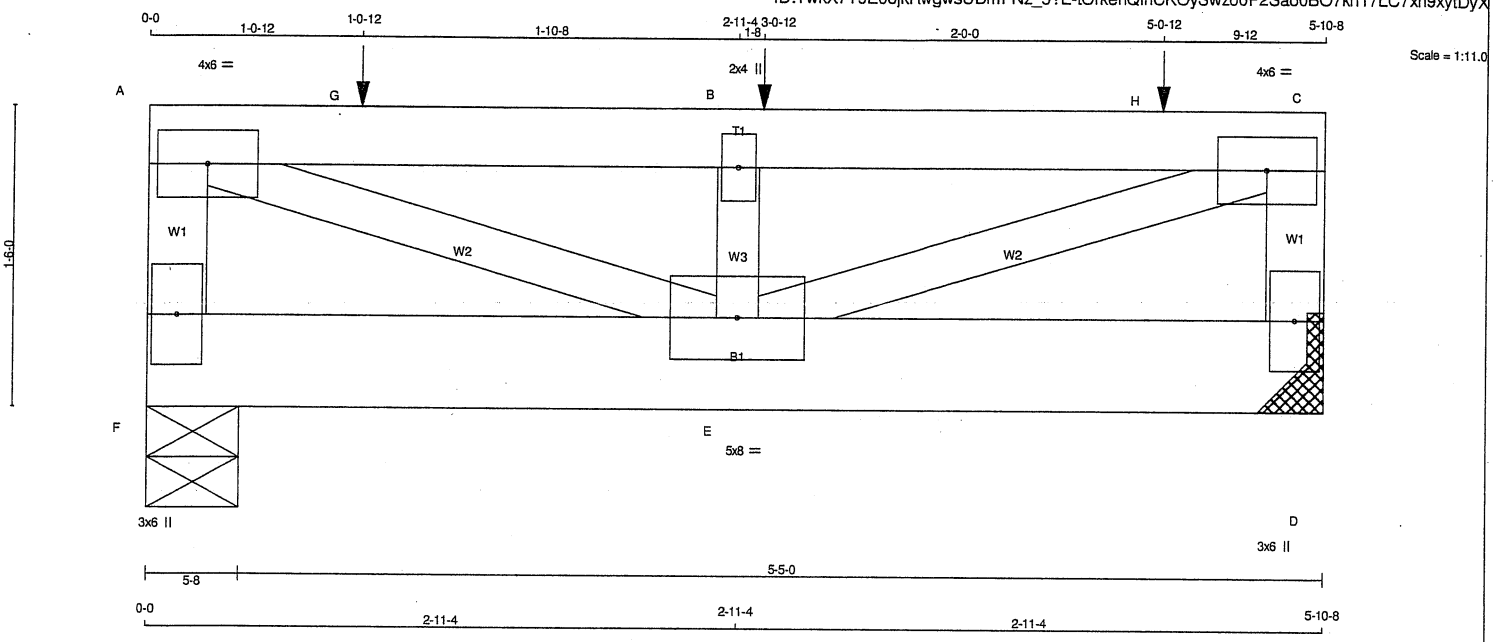

TOTAL WEIGHT = 2 X 24 = 48 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	SIDE(0.0)
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(14.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	5.0	8.0		

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125133

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1109	0	1109	0	0	5-8	5-8
D	1051	0	1051	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	792	475 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
D	751	447 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-961 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 1534	0.19 (1)
A- G	-1433 / 0	-84.9	-84.9	0.30 (1)	6.25	E- B	-1139 / 0	0.09 (1)
G- B	-1433 / 0	-84.9	-84.9	0.30 (1)	6.25	E- C	0 / 1534	0.19 (1)
B- H	-1433 / 0	-84.9	-84.9	0.20 (1)	6.25			
H- C	-1433 / 0	-84.9	-84.9	0.20 (1)	6.25			
D- C	-902 / 0	0.0	0.0	0.05 (1)	7.81			
F- E	0 / 0	-43.5	-43.5	0.09 (1)	10.00			
E- D	0 / 0	-43.5	-43.5	0.09 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-0-12	-293	-293	---	TOP	VERT	TOTAL	---	C1
G	1-0-12	-403	-403	---	FRONT	VERT	TOTAL	---	C1
H	5-0-12	-299	-299	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.09/1.00 (E-F:1) , WB=0.19/1.00 (A-E:1) , SSI=0.18/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (E) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420186	T36	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:TwkX7T9E06jkRwgwsUDmPNz_5?E-tOrkenQlnCKOySwzo0P2Sao0BO7kh17LC7xh9xytDyX

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009023
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125133

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420187	T37	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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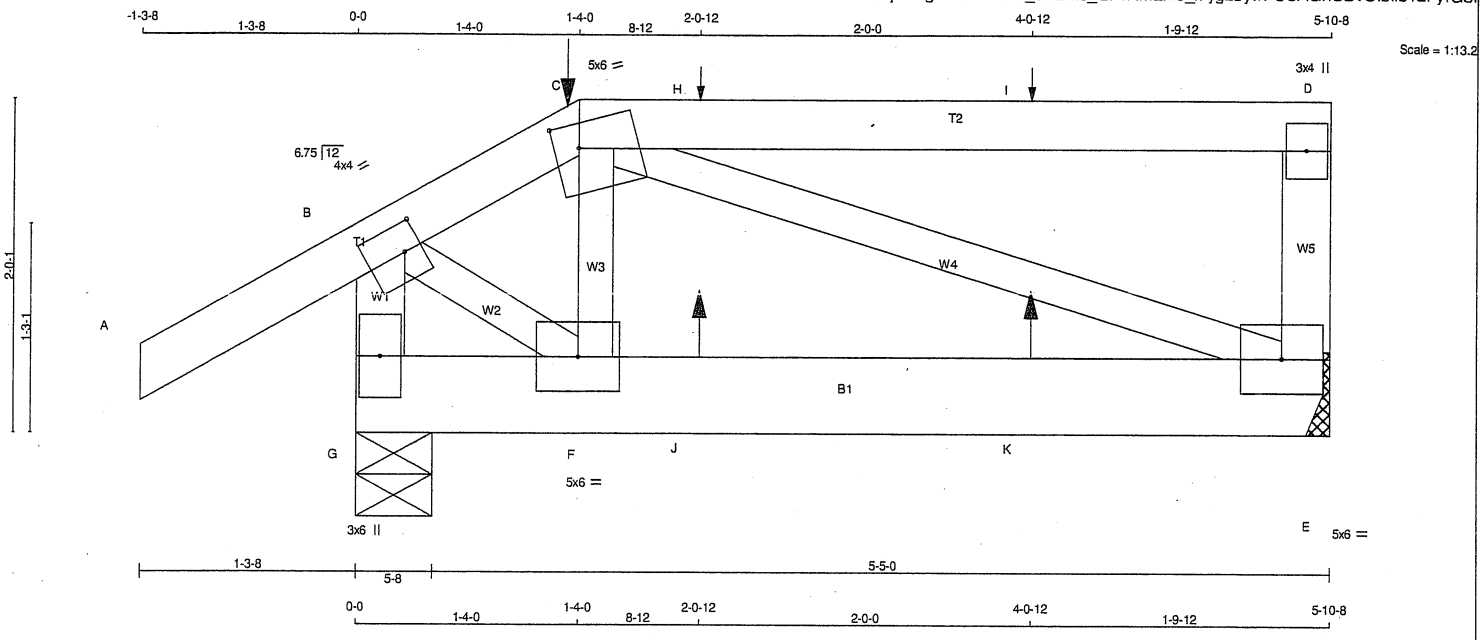

TOTAL WEIGHT = 27 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	5.0	6.0		
F	BMWW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	574	0	574	0	0	MECHANICAL	
G	638	0	638	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-12.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	403	285 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
G	450	306 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 28	-84.9	-84.9	0.12 (1)	10.00	F- C	-172 / 0	0.03 (1)	
B- C	-404 / 0	-84.9	-84.9	0.12 (1)	6.25	C- E	-334 / 0	0.12 (1)	
C- H	0 / 0	-186.9	-186.9	0.72 (1)	10.00	B- F	0 / 379	0.09 (1)	
H- I	0 / 0	-186.9	-186.9	0.72 (1)	10.00				
I- D	0 / 0	-186.9	-186.9	0.72 (1)	10.00				
E- D	-424 / 0	0.0	0.0	0.05 (1)	7.81				
G- B	-611 / 0	0.0	0.0	0.07 (1)	7.81				
G- F	0 / 0	-18.5	-18.5	0.02 (4)	10.00				
F- J	0 / 314	-18.5	-18.5	0.07 (1)	10.00				
J- K	0 / 314	-18.5	-18.5	0.07 (1)	10.00				
K- E	0 / 314	-18.5	-18.5	0.07 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-4-0	-1	-1	---	TOP	VERT	DEAD	---	C1
C	1-4-0	-32	-32	---	FRONT	VERT	DEAD	---	C1
C	1-4-0	-4	-4	---	TOP	VERT	SNOW	---	C1
H	2-0-12	1	1	64	FRONT	VERT	TOTAL	---	C1
I	4-0-12	1	1	70	FRONT	VERT	TOTAL	---	C1
J	2-0-12	7	1	9	FRONT	VERT	TOTAL	---	C1
K	4-0-12	7	1	9	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.72/1.00 (C-D:1) , BC=0.07/1.00 (E-F:1) , WB=0.12/1.00 (C-E:1) , SSI=0.36/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

Structural component only
DWG# T-2126372

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	PB1	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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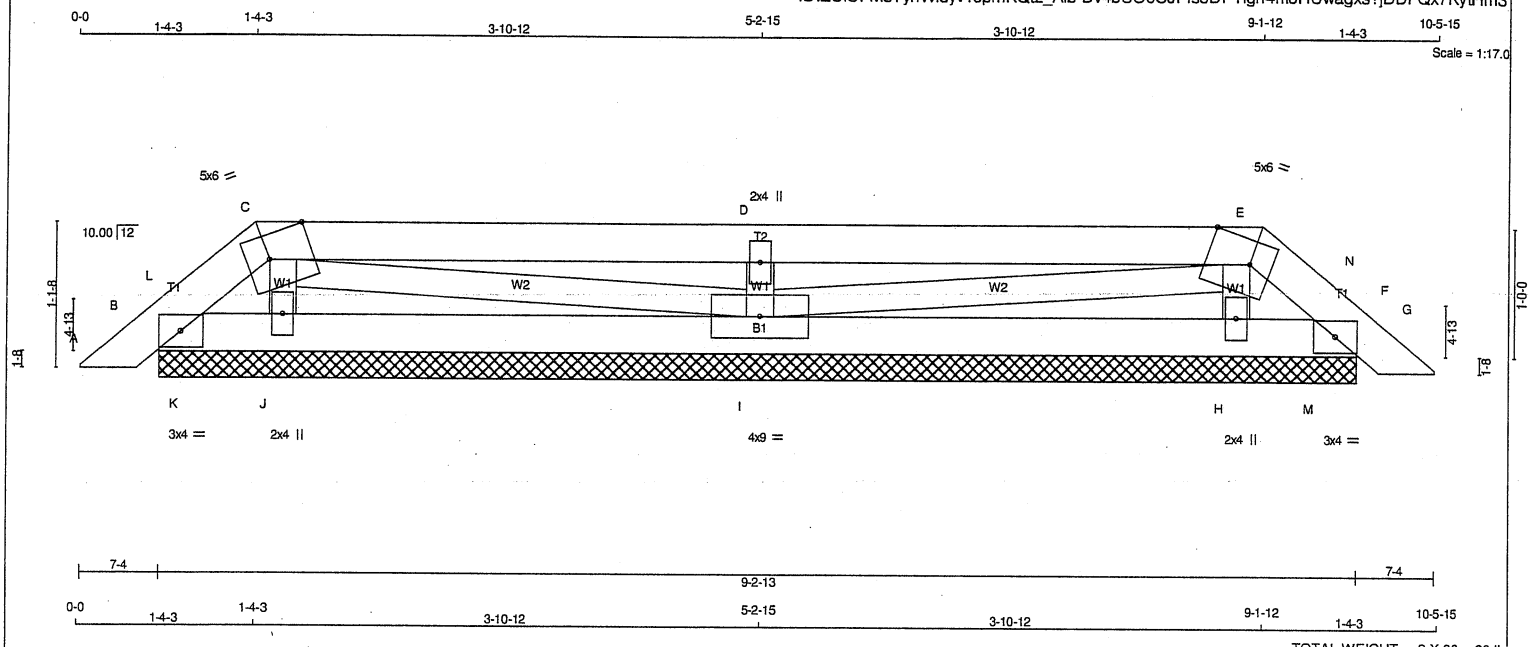

TOTAL WEIGHT = 2 X 30 = 60 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	Edge	4.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	Edge	4.00
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	9.0		
J	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	96	0	96	0	0	9-2-13	9-2-13
F	96	0	96	0	0	9-2-13	9-2-13
J	185	0	185	0	0	9-2-13	9-2-13
I	480	0	480	0	0	9-2-13	9-2-13
H	185	0	185	0	0	9-2-13	9-2-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	65	58 / 0	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
F	65	58 / 0	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
J	134	69 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
I	339	224 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
H	134	69 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- L	-30 / 6	-84.9	-84.9	0.01 (4)	6.25
L- C	-43 / 0	-84.9	-84.9	0.01 (4)	6.25
C- D	0 / 0	-84.9	-84.9	0.21 (1)	10.00
D- E	0 / 0	-84.9	-84.9	0.21 (1)	10.00
E- N	-43 / 0	-84.9	-84.9	0.01 (4)	6.25
N- F	-30 / 6	-84.9	-84.9	0.01 (4)	6.25
F- G	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- K	0 / 29	-18.5	-18.5	0.01 (1)	10.00
K- J	0 / 29	-18.5	-18.5	0.04 (4)	10.00
J- I	0 / 4	-18.5	-18.5	0.05 (4)	10.00
I- H	0 / 4	-18.5	-18.5	0.05 (4)	10.00
H- M	0 / 29	-18.5	-18.5	0.04 (4)	10.00
M- F	0 / 29	-18.5	-18.5	0.01 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-114 / 0	0.02 (1)
C- I	-4 / 0	0.00 (1)
I- D	-408 / 0	0.06 (1)
I- E	-4 / 0	0.00 (1)
H- E	-114 / 0	0.02 (1)
K- L	-51 / 0	0.00 (1)
M- N	-51 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (D-E:1) , BC=0.05/1.00 (I-J:4) , WB=0.06/1.00 (D-I:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

Structural component only
DWG# T-2125080

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	PB2	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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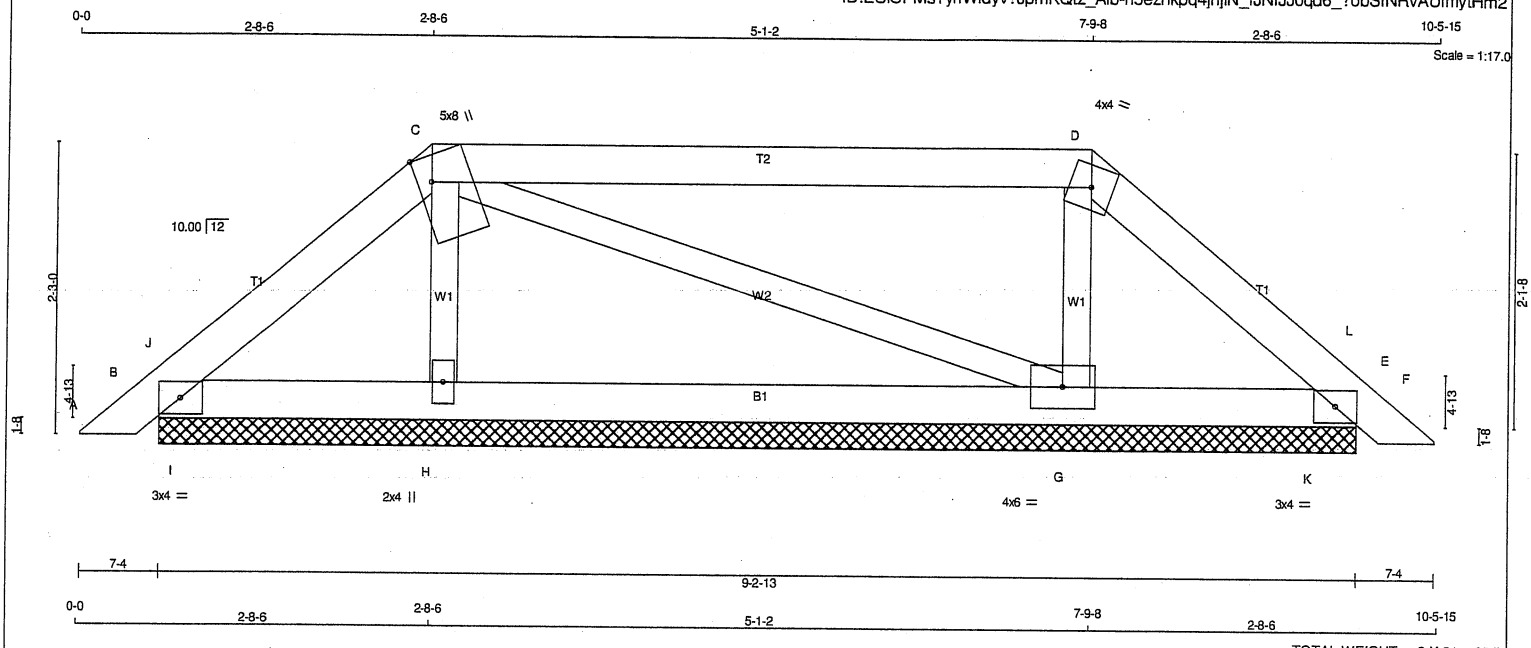

TOTAL WEIGHT = 2 X 31 = 62 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	8.0	Edge	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	194	0	194	0	0	9-2-13	9-2-13
E	183	0	183	0	0	9-2-13	9-2-13
H	323	0	323	0	0	9-2-13	9-2-13
G	344	0	344	0	0	9-2-13	9-2-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	134	104 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	127	98 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
H	232	133 / 0	0 / 0	0 / 0	0 / 0	99 / 0	0 / 0
G	246	144 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- J	-54 / 0	-84.9	-84.9	0.02 (1)	6.25
J- C	-90 / 0	-84.9	-84.9	0.04 (1)	6.25
C- D	-38 / 0	-84.9	-84.9	0.38 (1)	6.25
D- L	-73 / 0	-84.9	-84.9	0.04 (1)	6.25
L- E	-35 / 0	-84.9	-84.9	0.02 (1)	6.25
E- F	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- I	0 / 65	-18.5	-18.5	0.05 (1)	10.00
I- H	0 / 65	-18.5	-18.5	0.08 (4)	10.00
H- G	0 / 52	-18.5	-18.5	0.08 (4)	10.00
G- K	0 / 52	-18.5	-18.5	0.08 (4)	10.00
K- E	0 / 52	-18.5	-18.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-227 / 0	0.03 (1)
C- G	-15 / 0	0.01 (1)
G- D	-242 / 0	0.04 (1)
I- J	-137 / 0	0.00 (1)
K- L	-139 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.38/1.00 (C-D:1) , BC=0.08/1.00 (H-I:4) , WB=0.04/1.00 (D-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (D) (INPUT = 0.90)
JSI METAL= 0.05 (H) (INPUT = 1.00)

Structural component only
DWG# T-2125081

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	PB3	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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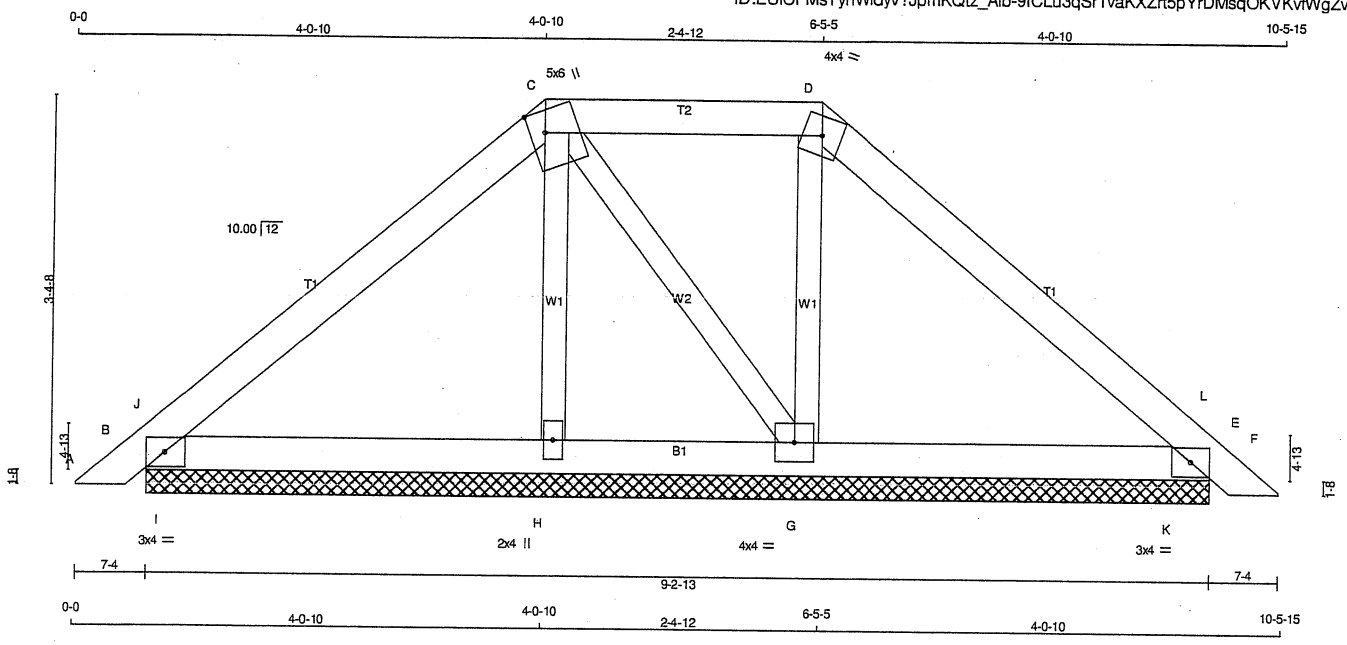

TOTAL WEIGHT = 2 X 32 = 65 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	300	0	300	0	0	9-2-13	9-2-13
E	281	0	281	0	0	9-2-13	9-2-13
H	190	0	190	0	0	9-2-13	9-2-13
G	272	0	272	0	0	9-2-13	9-2-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	211	146 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
E	198	135 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
H	137	76 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
G	193	122 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- J	-43 / 0	-84.9	-84.9	0.05 (1)	6.25
J- C	-141 / 0	-84.9	-84.9	0.12 (1)	6.25
C- D	-74 / 0	-84.9	-84.9	0.08 (1)	6.25
D- L	-112 / 0	-84.9	-84.9	0.12 (1)	6.25
L- E	-38 / 0	-84.9	-84.9	0.05 (1)	6.25
E- F	0 / 13	-84.9	-84.9	0.02 (1)	10.00
B- I	0 / 102	-18.5	-18.5	0.11 (1)	10.00
I- H	0 / 102	-18.5	-18.5	0.11 (1)	10.00
H- G	0 / 99	-18.5	-18.5	0.07 (1)	10.00
G- K	0 / 80	-18.5	-18.5	0.11 (1)	10.00
K- E	0 / 80	-18.5	-18.5	0.11 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-107 / 0	0.02 (1)
C- G	-40 / 0	0.01 (1)
G- D	-158 / 0	0.03 (1)
I- J	-285 / 0	0.00 (1)
K- L	-289 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (D-L:1) , BC=0.11/1.00 (B-I:1) , WB=0.03/1.00 (D-G:1) , SSI=0.23/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125082

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420178	J1	21	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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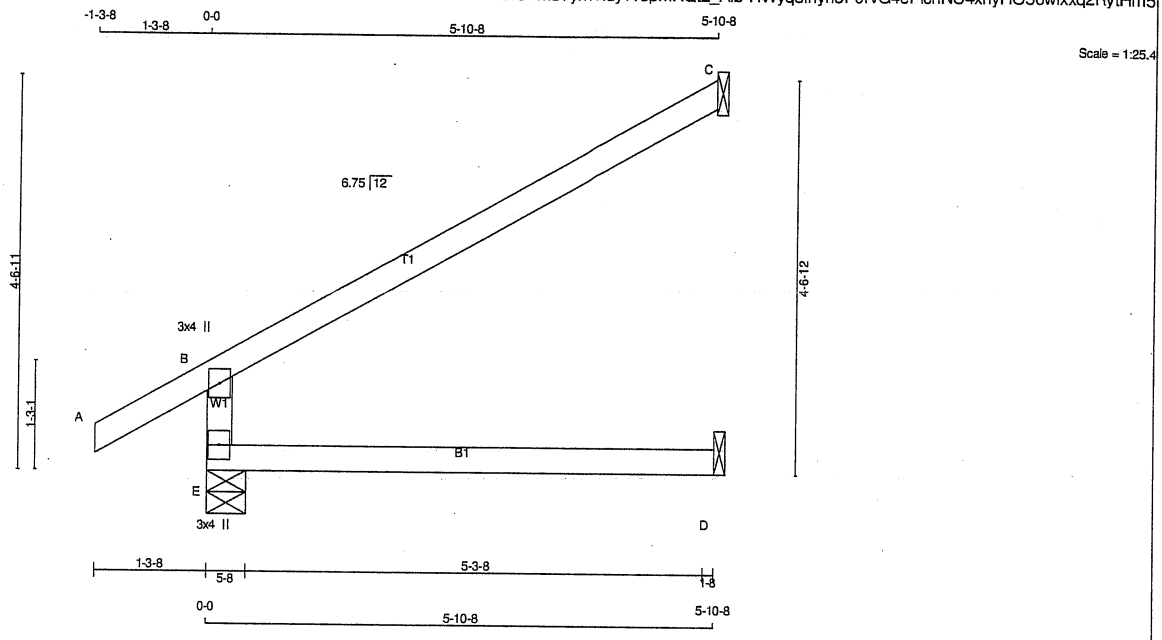

TOTAL WEIGHT = 21 X 17 = 361 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	491	0	491	0	0	5-8	5-8
C	187	0	187	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	346	234 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	129	103 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-427 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B- C	-31 / 0	-84.9	-84.9	0.50 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125077

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420185	J1S	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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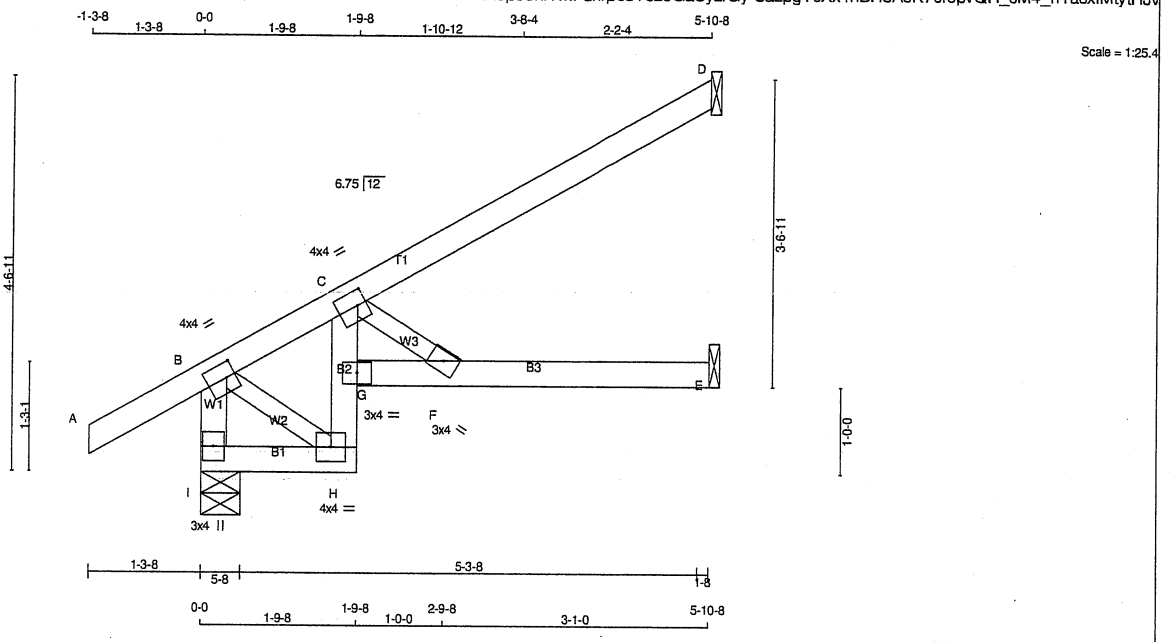

TOTAL WEIGHT = 8 X 21 = 170 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW+w	MT20	3.0	4.0		
G	BVM-I	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	433	0	433	0	0	5-8	5-8
D	183	0	183	0	0	1-8	1-8
E	107	0	107	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	305	208 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	127	96 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	79	34 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-417 / 0	0.0	0.0	0.04 (1)	7.81	B- H	0 / 178	0.04 (1)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	C- F	-417 / 0	0.06 (1)
B- C	-205 / 0	-84.9	-84.9	0.11 (1)	6.25			
C- D	0 / 4	-84.9	-84.9	0.27 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
H- G	-73 / 0	0.0	0.0	0.21 (1)	7.81			
G- C	0 / 149	0.0	0.0	0.23 (1)	10.00			
G- F	0 / 332	-18.5	-18.5	0.38 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 579 (0.12")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.38/1.00 (F-G:1) , WB=0.06/1.00 (C-F:1) , SSI=0.17/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (C) (INPUT = 0.90)
JSI METAL= 0.14 (F) (INPUT = 1.00)

Structural component only
DWG# T-2125101

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	J2	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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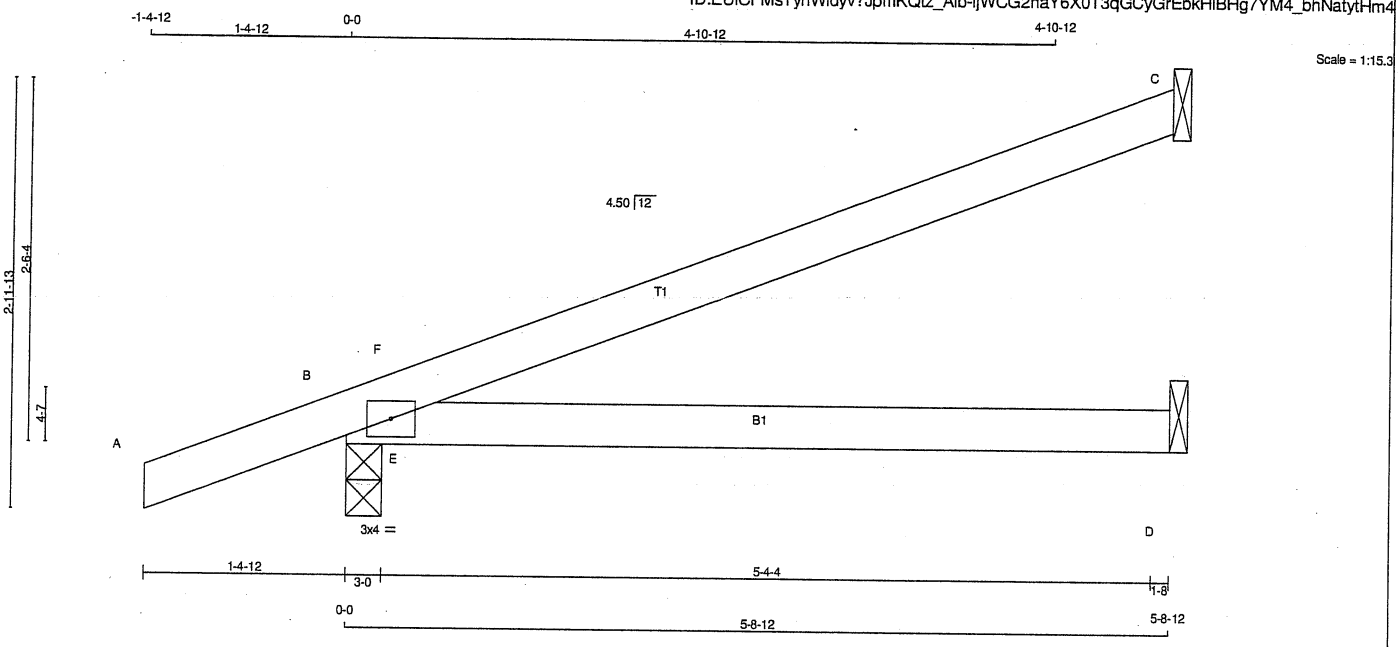

TOTAL WEIGHT = 4 X 15 = 61 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	214	0	214	0	0	1-8	1-8
B	419	0	419	0	0	3-0	3-0
D	82	0	82	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	149	113 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
B	295	201 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
D	62	20 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-84.9	-84.9	0.12 (1)	10.00	E- F	-360 / 14	0.00 (1)
B- F	-26 / 50	-84.9	-84.9	0.08 (1)	6.25			
F- C	-3 / 2	-84.9	-84.9	0.37 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.24 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.25 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 571 (0.12")

CSI: TC=0.37/1.00 (C-F:1) , BC=0.25/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.30/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/29/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125078

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420178	J3	7	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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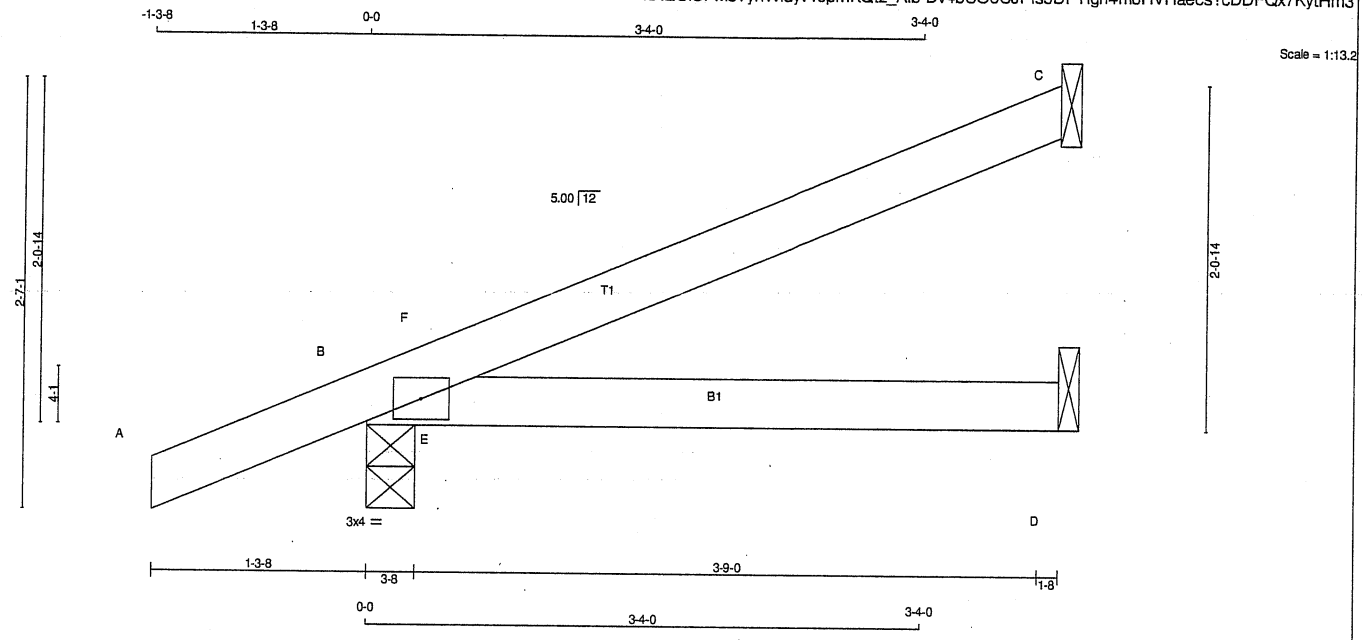

TOTAL WEIGHT = 7 X 12 = 81 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	154	0	154	0	0	1-8	1-8
B	330	0	330	0	0	3-8	3-8
D	61	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	107	81 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
B	232	160 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
D	46	16 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 21	-84.9	-84.9	0.11 (1)	10.00	E- F	-195 / 7	0.00 (1)
B- F	-16 / 7	-84.9	-84.9	0.04 (4)	6.25			
F- C	0 / 2	-84.9	-84.9	0.19 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.14 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.19/1.00 (C-F:1) , BC=0.14/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125079

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	J20	12	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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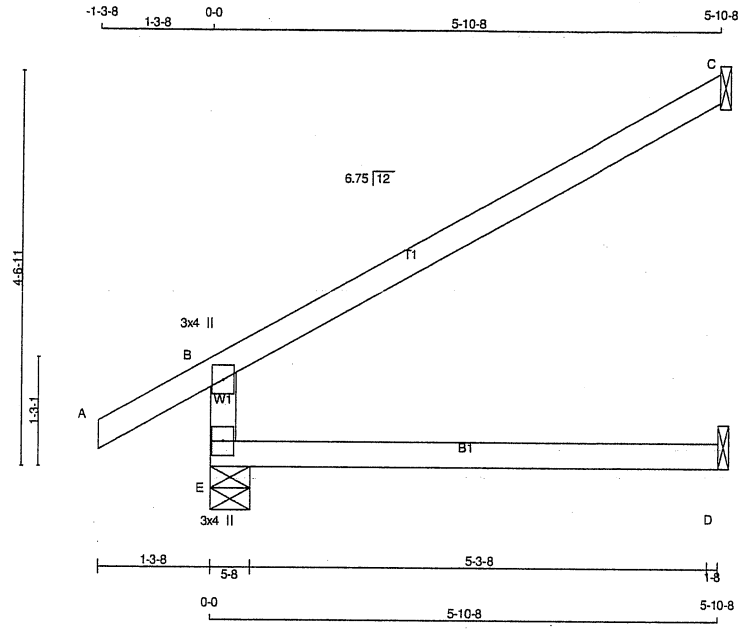

TOTAL WEIGHT = 12 X 17 = 206 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	491	0	491	0	0	5-8	5-8
C	187	0	187	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	346	234 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	129	103 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-427 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B- C	-31 / 0	-84.9	-84.9	0.50 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125113

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	J21	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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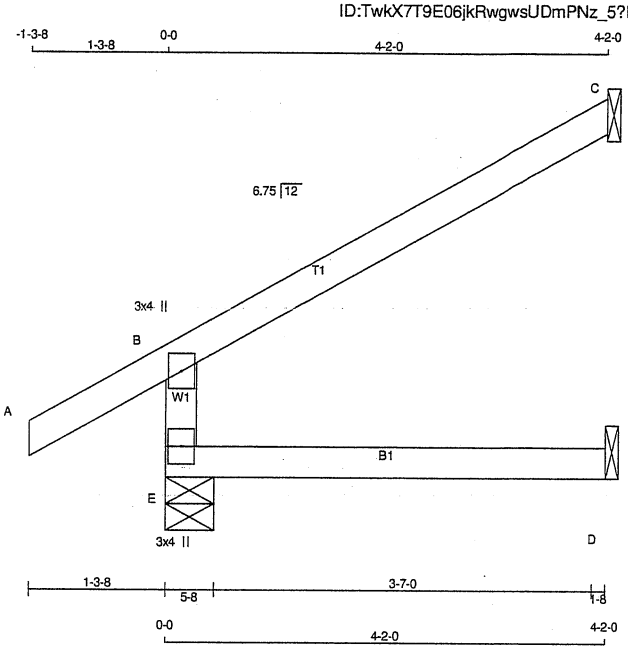

TOTAL WEIGHT = 4 X 13 = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	381	0	381	0	0	5-8	5-8
C	133	0	133	0	0	1-8	1-8
D	33	0	37	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	268	185 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
C	92	73 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
D	26	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-337 / 0	0.0	0.0	0.06 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B- C	-22 / 0	-84.9	-84.9	0.25 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.07/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125114

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	J22	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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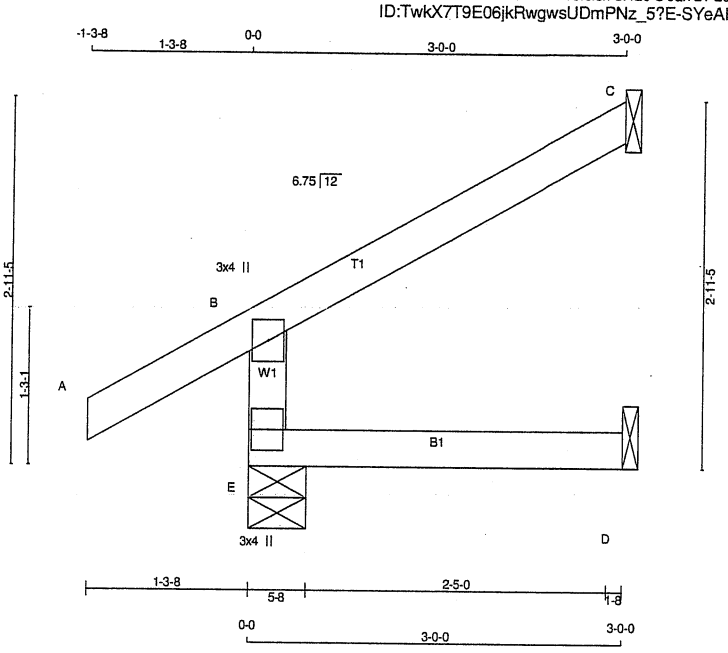


TOTAL WEIGHT = 2 X 10 = 20 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	306	0	306	0	0	5-8	5-8
C	96	0	96	0	0	1-8	1-8
D	24	0	27	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	215	151 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
C	66	53 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	19	0 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-275 / 0	0.0	0.0	0.03 (4)	7.81
A- B	0 / 28	-84.9	-84.9	0.12 (5)	10.00
B- C	-16 / 0	-84.9	-84.9	0.13 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.04/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

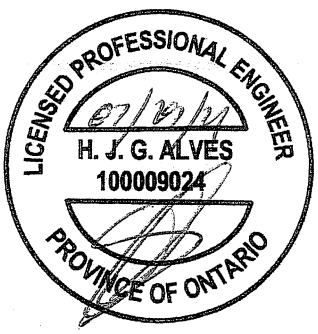
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2125115

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420187	J20S	7	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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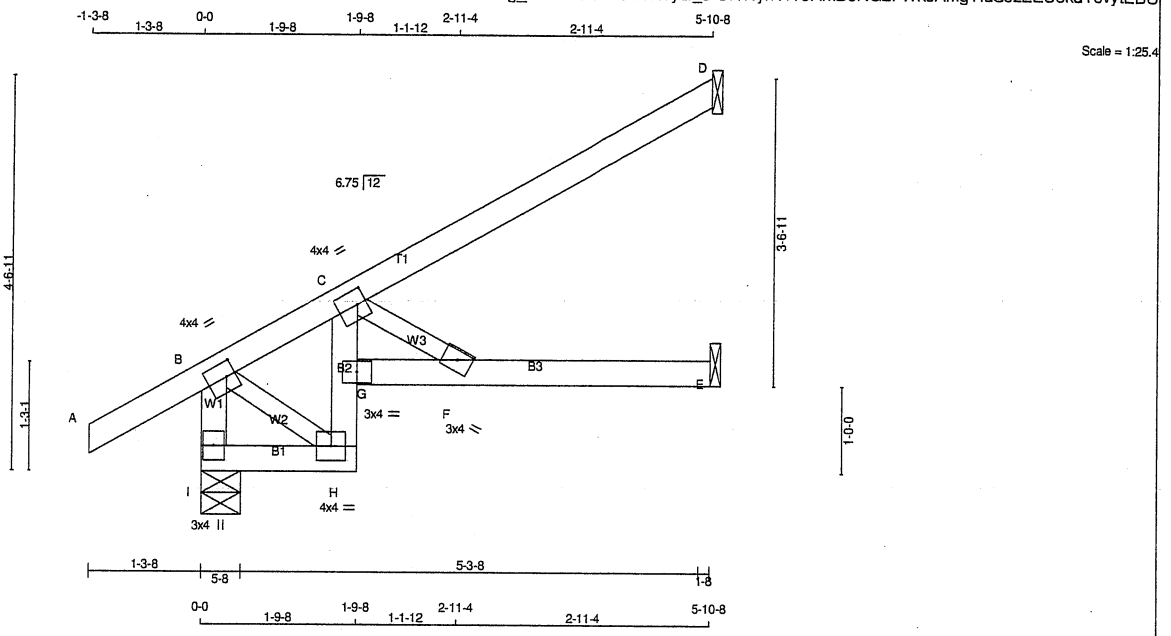

TOTAL WEIGHT = 7 X 21 = 150 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW+w	MT20	3.0	4.0		
G	BVM-I	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	433	0	433	0	0	5-8	5-8
D	181	0	181	0	0	1-8	1-8
E	109	0	109	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	305	208 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	126	95 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	80	34 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-417 / 0	0.0	0.0	0.04 (1)	7.81	B- H	0 / 182	0.04 (1)
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	C- F	-410 / 0	0.06 (1)
B- C	-207 / 0	-84.9	-84.9	0.11 (1)	6.25			
C- D	0 / 4	-84.9	-84.9	0.26 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
H- G	-75 / 0	0.0	0.0	0.21 (1)	7.81			
G- C	0 / 123	0.0	0.0	0.23 (1)	10.00			
G- F	0 / 340	-18.5	-18.5	0.38 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 595 (0.12")

CSI: TC=0.26/1.00 (C-D:1) , BC=0.38/1.00 (F-G:1) , WB=0.06/1.00 (C-F:1) , SSI=0.15/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (C) (INPUT = 0.90)
JSI METAL= 0.15 (F) (INPUT = 1.00)

Structural component only
DWG# T-2125134

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420187	J23	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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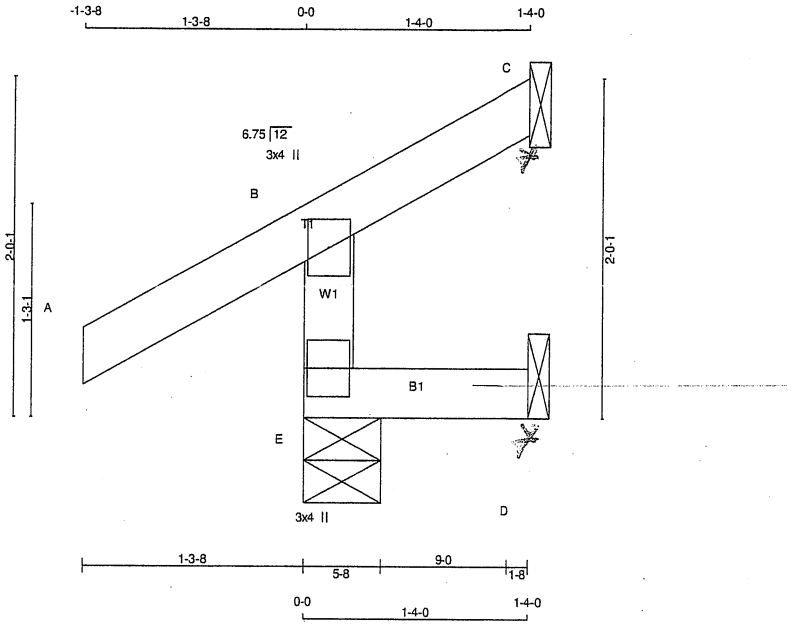

TOTAL WEIGHT = 2 X 6 = 12 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	244	0	244	0	0	5-8	5-8
C	11	0	11	0	-36	1-8	1-8
D	0	0	11	0	-7	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	170	127 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
C	7	5 / -24	0 / 0	0 / 0	0 / 0	2 / 0	0 / 0
D	0	0 / -9	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-218 / 0	0.0	0.0	0.03 (5)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B- C	-22 / 0	-84.9	-84.9	0.08 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2126371

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	C21	8	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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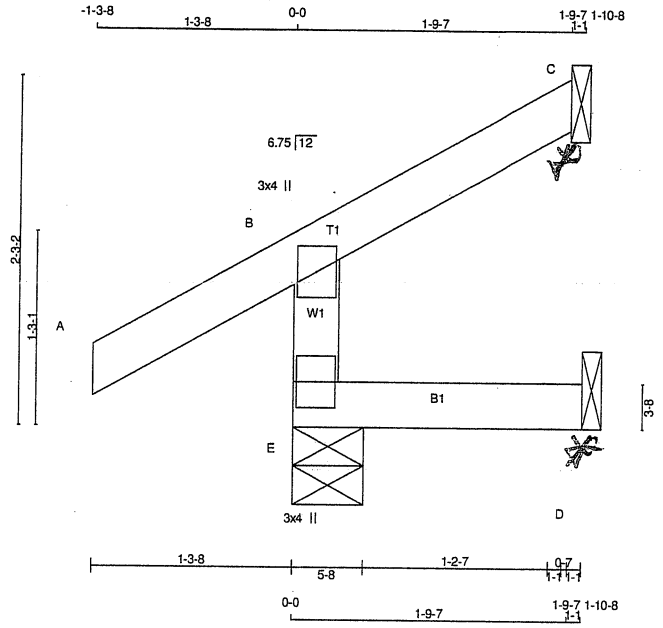

TOTAL WEIGHT = 8 X 7 = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	253	0	253	0	0	5-8	5-8
C	41	0	41	0	-21	1-8	1-8
D	9	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	176	129 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	28	22 / -17	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	8	0 / -7	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-226 / 0	0.0	0.0	0.03 (5)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (5)	10.00			
B- C	-17 / 0	-84.9	-84.9	0.09 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:5) , BC=0.03/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125106

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
420186	C22	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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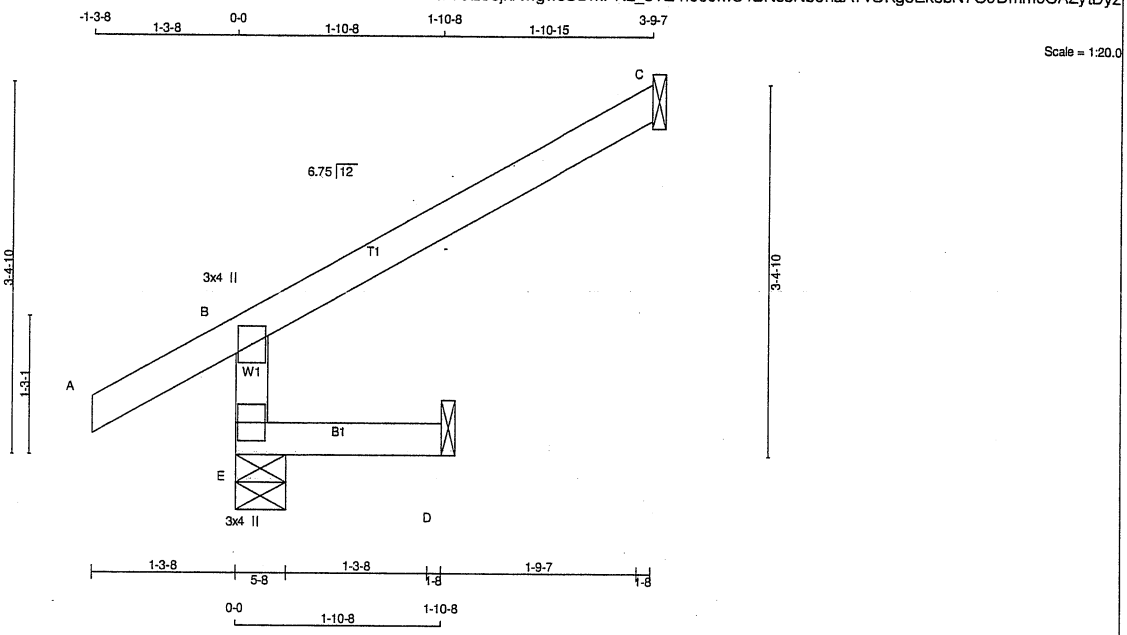

TOTAL WEIGHT = 3 X 10 = 30 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	335	0	335	0	0	5-8	5-8
C	121	0	121	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	234	174 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	83	66 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-316 / 0	0.0	0.0	0.01 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.12 (5)	10.00			
B- C	-20 / 0	-84.9	-84.9	0.21 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125107

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	C23	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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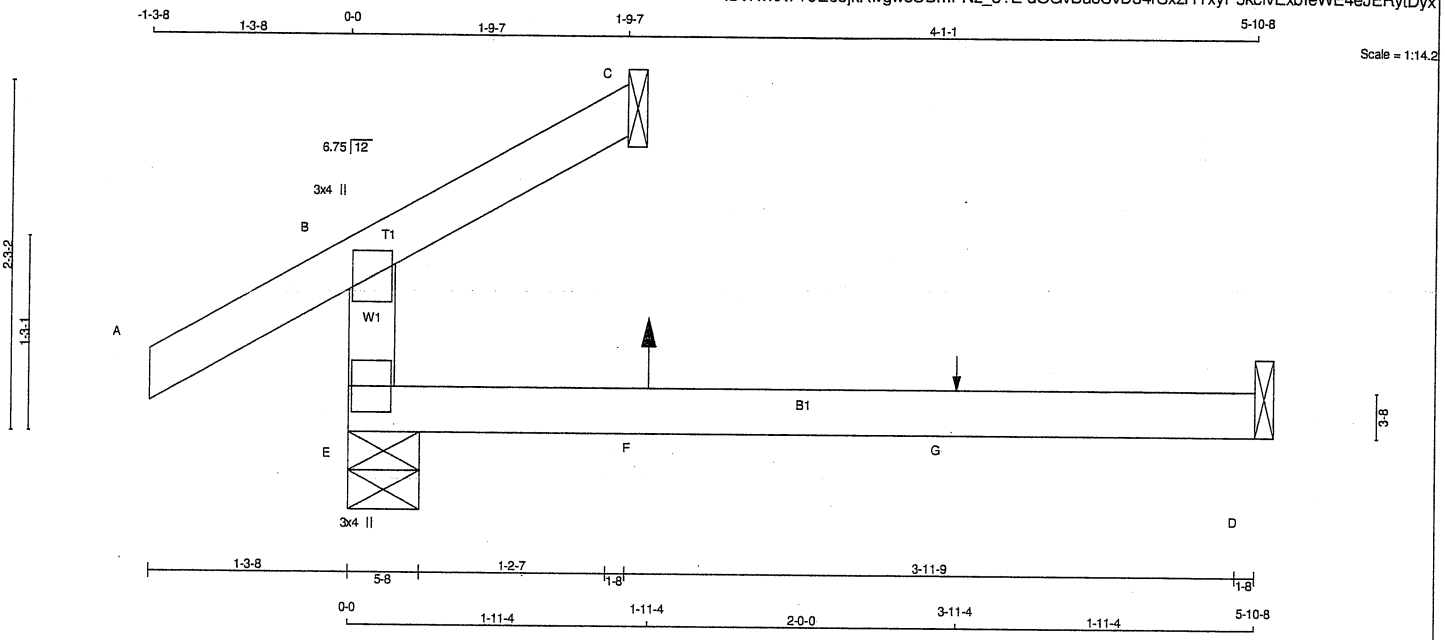

TOTAL WEIGHT = 4 X 12 = 47 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	267	0	267	0	0	5-8	5-8
C	59	0	59	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	126 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
C	43	19 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	36	0 / -2	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-210 / 0	0.0	0.0	0.11 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.12 (1)	10.00			
B- C	-9 / 10	-84.9	-84.9	0.08 (4)	10.00			
E- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
F- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	4	1	7	FRONT	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (D-E:4)

DOL LUMBER=0.97 NAIL=0.97 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125108

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	C24	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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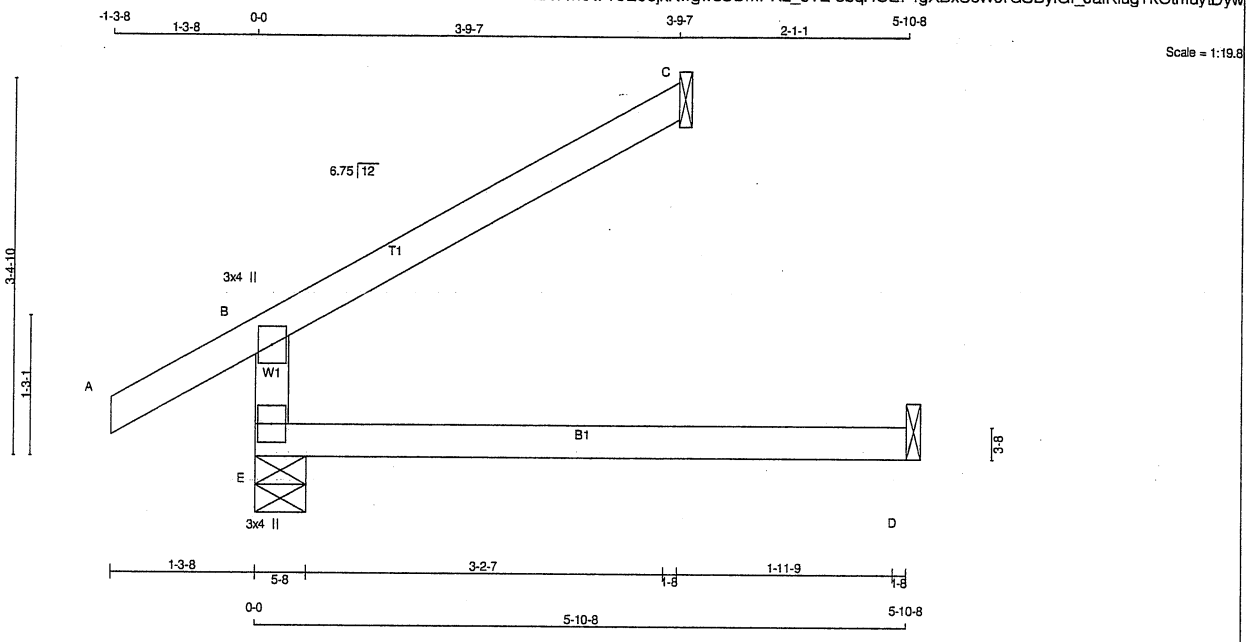

TOTAL WEIGHT = 4 X 14 = 58 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	380	0	380	0	0	5-8	5-8
C	121	0	121	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	269	174 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	83	66 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-316 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B- C	-20 / 0	-84.9	-84.9	0.21 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125109

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	C25	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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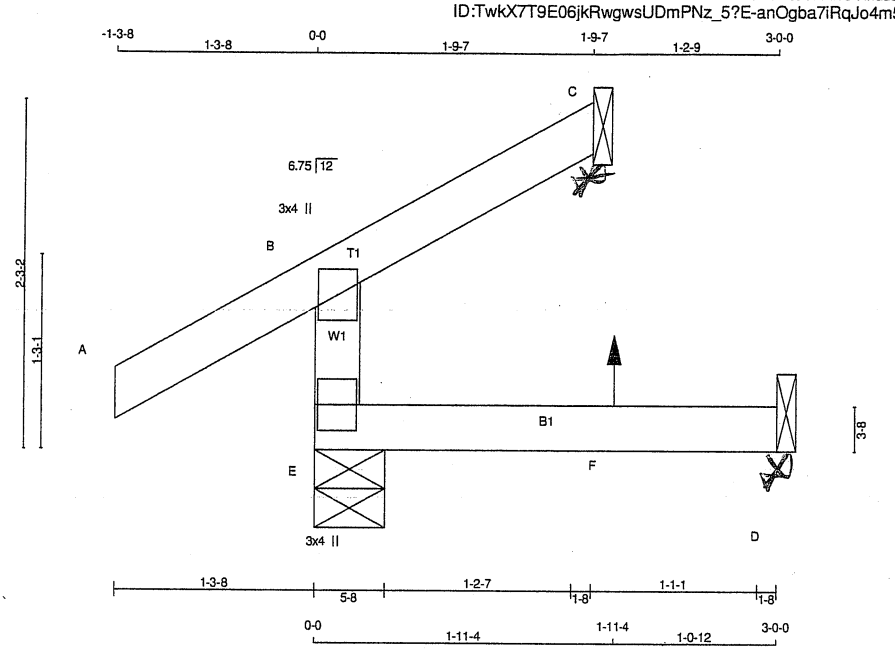

TOTAL WEIGHT = 2 X 9 = 17 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	257	0	257	0	0	5-8	5-8
C	42	0	42	0	-20	1-8	1-8
D	17	0	27	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	180	127 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
C	30	21 / -18	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	15	0 / -6	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (9)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-225 / 0	0.0	0.0	0.04 (5)	7.81			
A- B	0 / 28	-84.9	-84.9	0.13 (5)	10.00			
B- C	-17 / 0	-84.9	-84.9	0.08 (6)	6.25			
E- F	0 / 0	-18.5	-18.5	0.04 (5)	10.00			
F- D	0 / 0	-18.5	-18.5	0.04 (5)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	4	1	7	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC,2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125110

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	C26	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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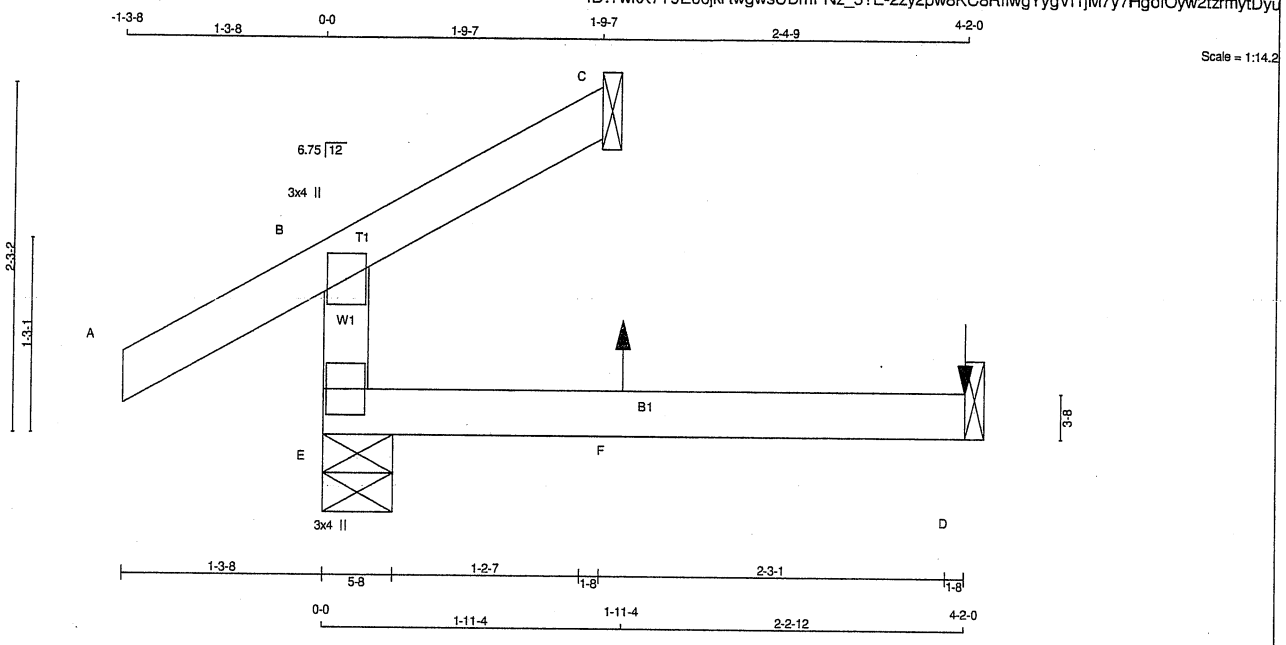

TOTAL WEIGHT = 2 X 10 = 20 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	262	0	262	0	0	5-8	5-8
C	47	0	47	0	0	1-8	1-8
D	379	0	379	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	185	126 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
C	33	20 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	269	171 / -4	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	UNBRAC LENGTH
E- B	-222 / 0	0.0	0.0	0.05 (4)				7.81
A- B	0 / 28	-84.9	-84.9	0.11 (1)				10.00
B- C	-15 / 2	-84.9	-84.9	0.07 (5)				6.25
E- F	0 / 0	-18.5	-18.5	0.07 (4)				10.00
F- D	0 / 0	-18.5	-18.5	0.07 (4)				10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	4-2-0	-245	-245	---	TOP	VERT	TOTAL	---	C1
F	1-11-4	4	1	7	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.07/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-2125111

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
420186	C27	5	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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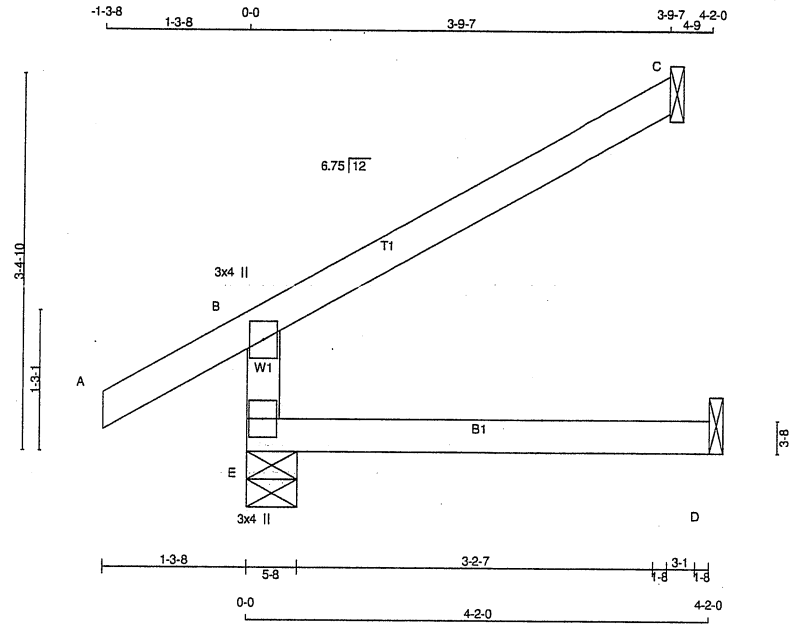


TOTAL WEIGHT = 5 X 12 = 62 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	361	0	361	0	0	5-8	5-8
C	121	0	121	0	0	1-8	1-8
D	33	0	37	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	254	174 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
C	83	66 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	26	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C .

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-316 / 0	0.0	0.0	0.06 (4)	7.81			
A- B	0 / 28	-84.9	-84.9	0.11 (1)	10.00			
B- C	-20 / 0	-84.9	-84.9	0.21 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	36.7	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.07/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2125112

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering m and General Safety notes.

T-1800218 Feb 09, 2018

MiTek®

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	LENGTH (in.)	DIA. (in.)	LATERAL RESISTANCE PER NAIL (lbs)		WITHDRAWAL RESISTANCE PER NAIL (lbs)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MINIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

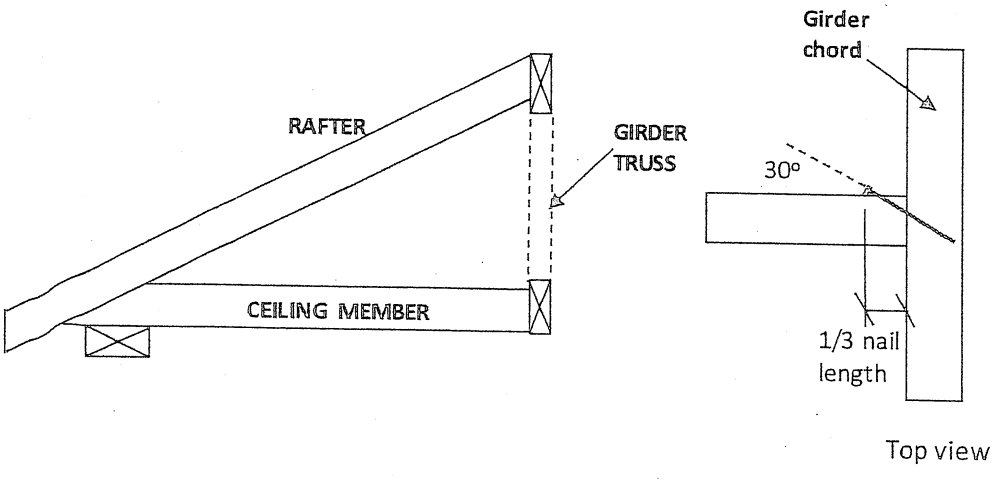
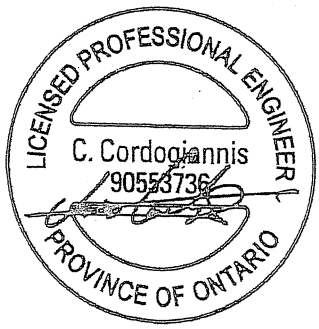


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485



December 21, 2020

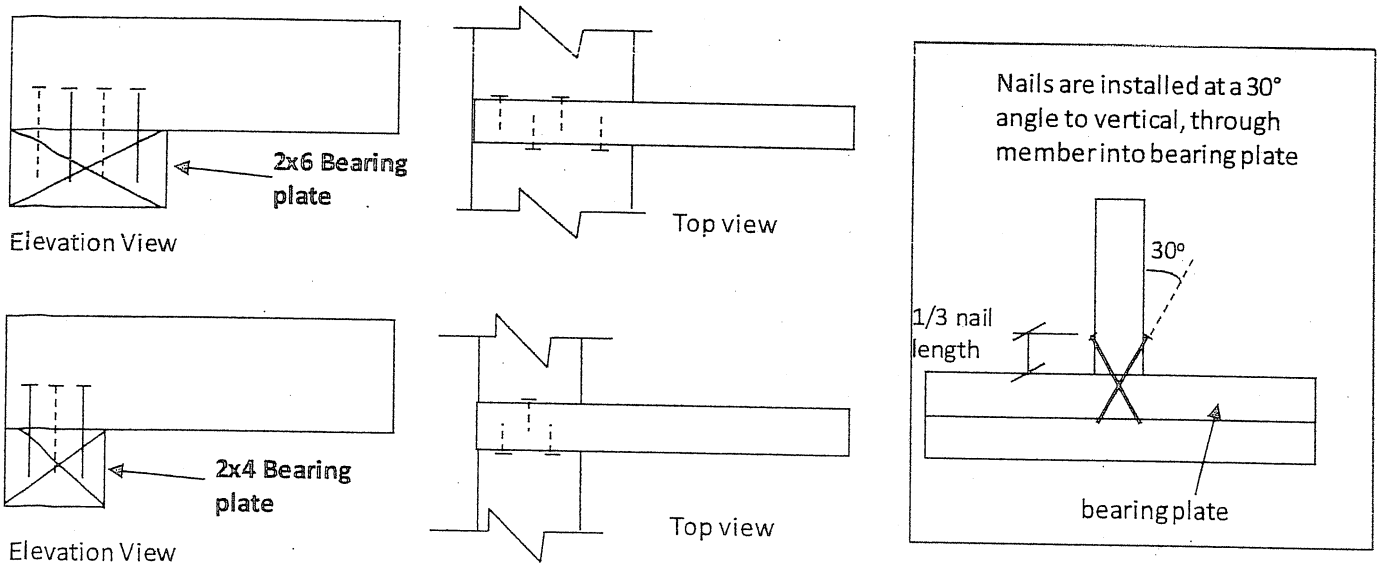
MiTek®

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).

2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).

3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.

4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).

5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.

6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.

7. Lumber must be dry (< 19% moisture content) at the time of nail installation.

8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485

December 21, 2020

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

VALUE ENGINEERED

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

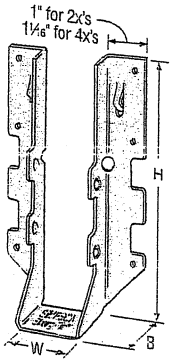
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

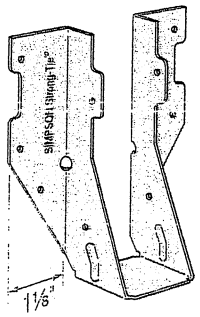
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

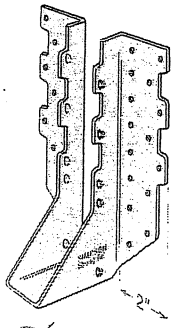
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



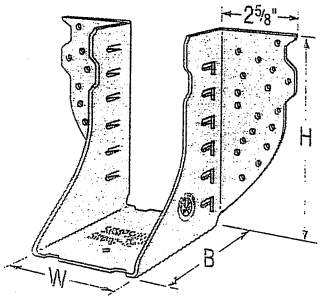
LUS28



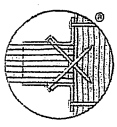
LU26L



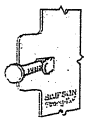
HUS210
(HUS26, HUS28, and HHUS similar)



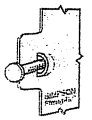
HGUS28-2



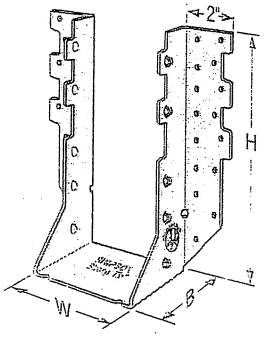
Double-Shear Nailing Top View



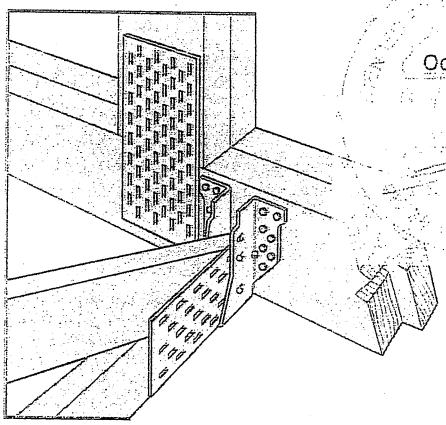
Double-Shear Nailing Side View; Do not bend tab



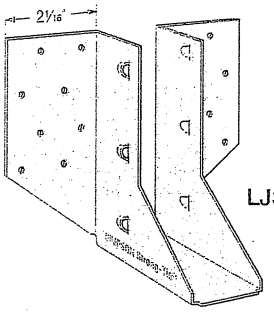
Dome Double-Shear Nailing Side View *(available on some models)*



HHUS210-2



Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

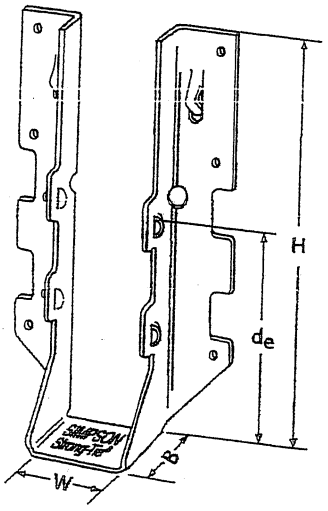
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

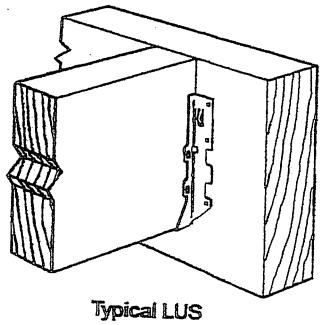
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

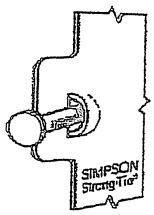


LUS28


Typical LUS Installation

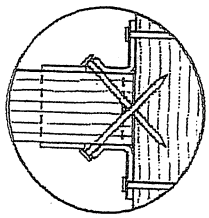
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

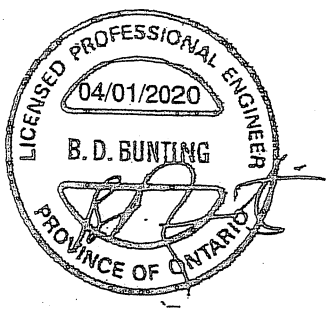
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

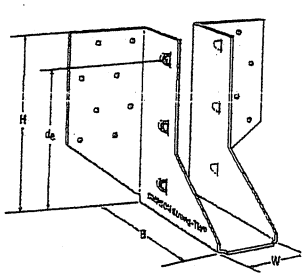
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

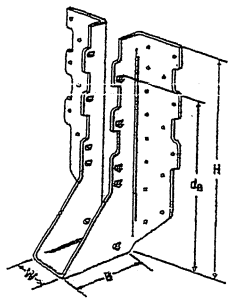
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

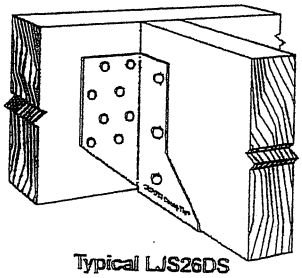
- See current catalogue for options



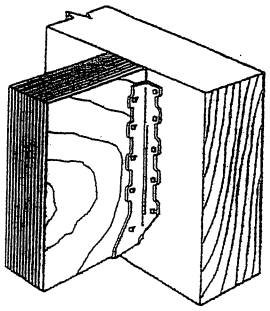
LJS26DS



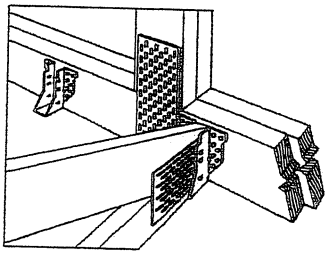
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



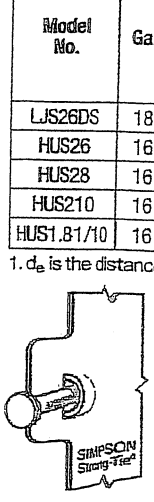
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

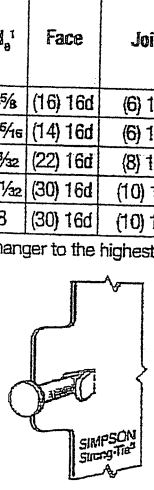
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

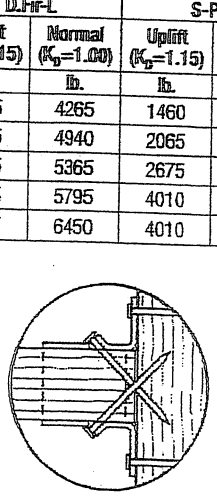


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

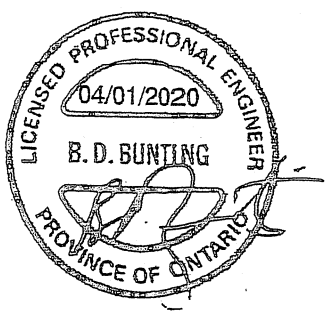
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

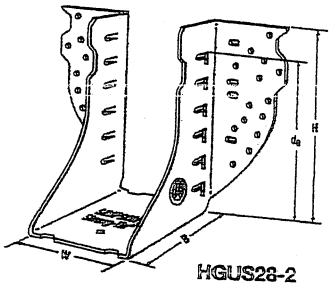
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

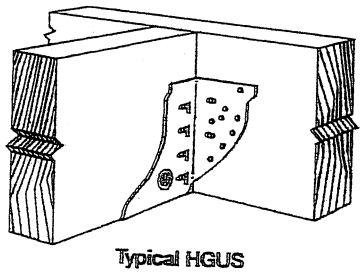
Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2



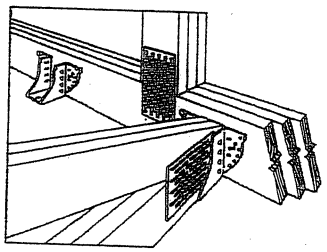
Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

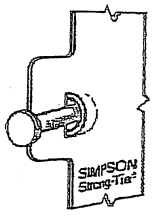
- See current catalogue for options



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

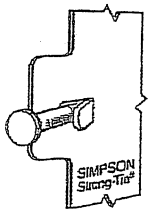
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10 5/8	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12 5/8	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

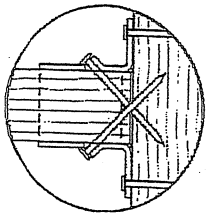


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

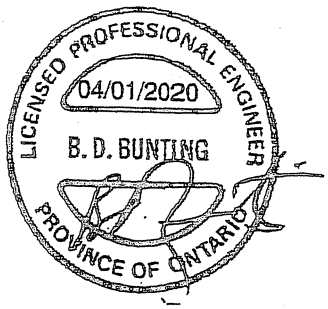
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

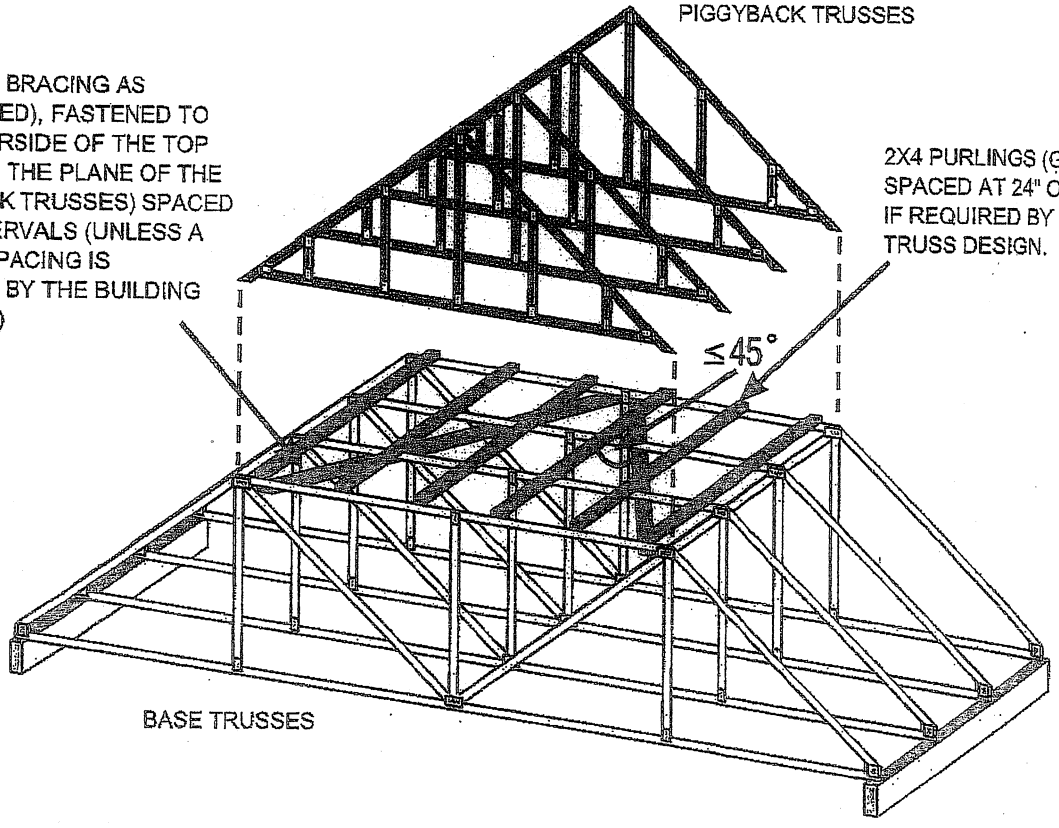
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

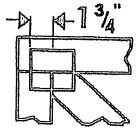


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

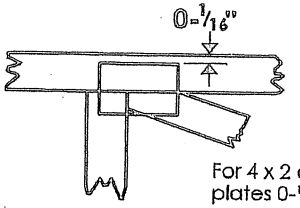
SKETCH FROM BCSI-CANADA 2013

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

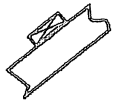
*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

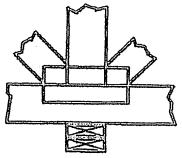
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

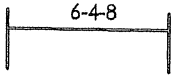
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

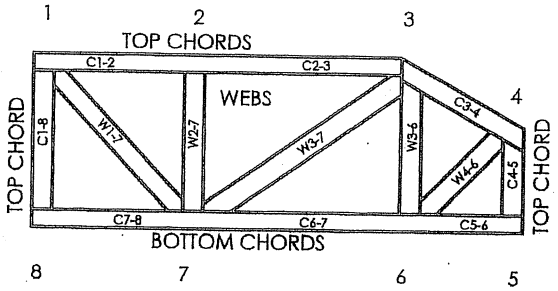
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.