

		Products		
PlotID	Length	Product	Plies	Net Qty
J1	20-00-00	11 7/8" NI-40x	1	17
J1DJ	20-00-00	11 7/8" NI-40x	2	4
J2	18-00-00	11 7/8" NI-40x	1	2
J3	16-00-00	11 7/8" NI-40x	1	10
J4	14-00-00	11 7/8" NI-40x	1	16
J4DJ	14-00-00	11 7/8" NI-40x	2	12
J5	12-00-00	11 7/8" NI-40x	1	6
J6	8-00-00	11 7/8" NI-40x	1	2
J7	6-00-00	11 7/8" NI-40x	1	3
J8	4-00-00	11 7/8" NI-40x	1	4
·J9	2-00-00	11 7/8" NI-40x	1	2
B8≁	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B6-	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B5 ~	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B3 /	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B1-	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B4 <	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B2/	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary					
Qty	Manuf	Product			
3	H1	IUS2.56/11.88			
14	H1	IUS2.56/11.88			
10	H1	IUS2.56/11.88			
8	H1	IUS2.56/11.88			
2	H2	HUS1.81/10			
3	H4	HGUS410			

NOTES:

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.



FROM PLAN DATED: 2021/05

BUILDER:

ROYAL PINE HOMES

SITE:

VALES OF HUMBER NORTH

MODEL: 40-1

ELEVATION: A.B.

LOT:

CITY: BRAMPTON

SALESMAN: RICK DICIANO

DESIGNER: AJ REVISION:

DATE: 2021-08-30

1st FLOOR

ATE 901-1

SUBFLOOR: 3/4" GLUED AND NAILED

SNOW LOAD: 24.0 lb/ft²

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 20.0 lb/ff²

BCIN: 26064; FIRM: 29991

LOADING:

ENGINEERING ONLY - DIMENSIONS TO BE VERIFIED ON SITE SUPPORTING STRUCTURE TO BE VERIFIED BY QUALIFIED BUILDING DESIGNER. ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO JOIST(S) AND FLOOR BEAM(S) INSTALLATION, ALL NOTES DESIGNATING MORE OR LESS DAS PER PLAN WORKD DO NOT REPRESENT A PART OF THE SCOPE OF WORK WITHIN THE BOUNDARIES OF THE SEAL THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT, ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNER(S) PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE FLOOR SYSTEM COMPONENTS REVIEWED IN THIS SUBMISSION.

MUNICIPALITY HAVING JURISDICTION TO OBTAIN LOT SPECIFIC SCHEDULE 1 FORM FROM THIS OFFICE PRIOR TO BUILDING PERMIT APPROVAL.

INSTALLERS OF THIS FLOOR SYSTEM AND THEIR COMPANIES HAVE THE RESPONSIBILITY OF ENSURING THEY HAVE A COPY OF THE NORDIC INSTALLATION GUIDE AND ANY OTHER MANUFACTURER'S PRODUCT LITERATURE WHICH WILL AID IN THE OVERALL PROPER INSTALLATION OF THIS FLOOR SYSTEM. INSTALLERS ARE TO READ

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DWG# TAM 1803/2/THROUGH DWG# TAM 180364, INCLUSIVE DATED 8-144

SEALED STRUCTURAL COMPONENTS ONLY: + 16038-4
SEALED, THIRD PARTY LVL TYPE BEAMS, BUILT-UP CONVENTIONAL BEAMS, HEADERS, AND CONCENTRATED
LOADED NORDIC WOOD-I JOIST ONLY. 2 X 6 SQUASH BLOCK REQUIRED AT ALL EXTERIOR SUPPORTS OR AS PEF
PROJECT ENGINEER'S SPECIFICATIONS. WEB FILLER REINFORCEMENT REQUIRED AT ALL HANGER SUPPORTED
JOIST EXCEEDING A REACTION OF 1500 LBS (FACTORED)-SEE DETAILS.
A COMPLETE FRAMING PLAN REQUIRES THE NORDIC PUBLISHED LITERATURE, WHICH INCLUDES INSTALLATION

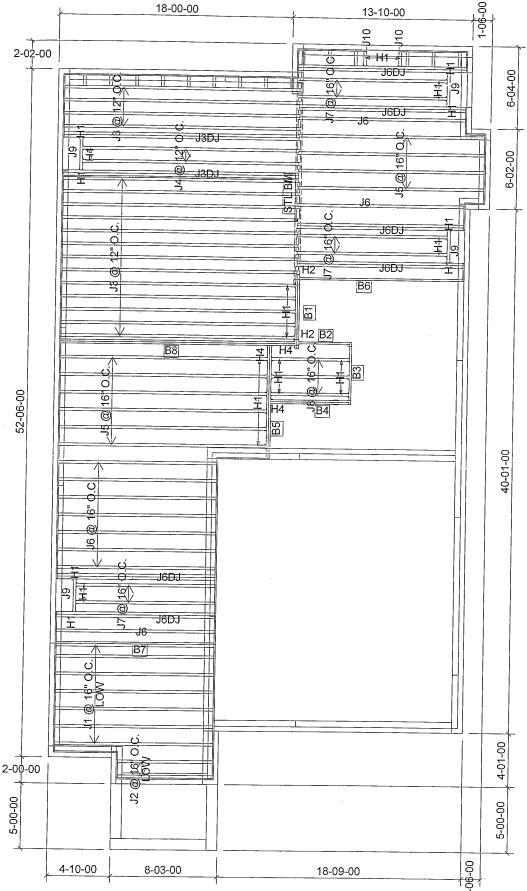
A COMPLETE FRAMING PLAN REQUIRES THE NORDIC PUBLISHED LITERATURE, WHICH INCLUDES INSTALLATION REQUIREMENTS, HANDLING AND STORAGE GUIDELINES, AND FORMS AN INTEGRAL PART OF THIS SEALED DOCUMENT. INSTALL SQUASH BLOCKS FOR TRANSFERRING POINT LOADS FROM GIRDER TRUSSES, HEADERS, AND BEAMS DOWN TO FOUNDATION COMPONENTS. FOR PROPER INSTALLATION, SEE NORDIC LITERATURE. PROVIDE 2 X 4 OR 2 X 6 STUD GRADE OR BETTER SQUASH BLOCKS, MATCHING SUPPORTED WALL WIDTH ABOVE BLOCKS. INSTALL SQUASH BLOCKS ON EACH SIDE OF JOIST. BLOCKING TO BE 1/16D DEEPER THAN JOIS DEPTH. SEE NORDIC LITERATURE FOR NAILING REQUIREMENT.

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION 3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND HE FIRM IS REGISTERED, IN APPROPRIATE CLASSES AND/OR CATEGORIES.

REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

DWG # TAM 195537
BCIN: 26064
FIRM: 29991
SEALED STRUCTURAL
COMPONENTS ONLY





		Products		
PlotID	Length	Product	Plies	Net Qty
J1	14-00-00	9 1/2" NI-40x	1	7
J2	8-00-00	9 1/2" NI-40x	1	2
J3	20-00-00	11 7/8" NI-40x	1	17
J3DJ	20-00-00	11 7/8" NI-40x	2	4
J4	18-00-00	11 7/8" NI-40x	1	2
J5	16-00-00	11 7/8" NI-40x	1	10
J6	14-00-00	11 7/8" NI-40x	1	10
J6DJ	14-00-00	11 7/8" NI-40x	2	12
J7	12-00-00	11 7/8" NI-40x	1	6
J8	6-00-00	11 7/8" NI-40x	1	3
J 9	4-00-00	11 7/8" NI-40x	1	4
J10	2-00-00	11 7/8" NI-40x	1	2
B8 ~	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B6≁	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B7.	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B5~	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B3~	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B1~	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B4/	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B2 🖊	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

	Connecto	r Summary
Qty	Manuf	Product
3	H1	IUS2.56/11.88
14	H1	IUS2.56/11.88
10	H1	IUS2.56/11.88
8	H1	IUS2.56/11.88
2	H2	HUS1.81/10
3	H4	HGUS410

NOTES:

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft2 DEAD LOAD: 20.0 lb/ft² SNOW LOAD: 24.0 lb/ft2

SUBFLOOR: 3/4" GLUED AND NAILED



FROM PLAN DATED: 2021/05

BUILDER:

ROYAL PINE HOMES

SITE:

VALES OF HUMBER NORTH

MODEL: 40-1

ELEVATION: A.B.

LOT:

CITY: BRAMPTON

SALESMAN: RICK DICIANO

DESIGNER: AJ **REVISION:**

DATE: 2021-08-30

1st FLOOR

SUNKEN

9014

BCIN: 26064; FIRM: 29991

ENGINEERING ONLY - DIMENSIONS TO BE VERIFIED ON SITE SUPPORTING STRUCTURE TO BE VERIFIED BY QUALIFIED BUILDING DESIGNER. ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO JOIST(S) AND FLOOR BEAM(S) INSTALLATION. ALL NOTES DESIGNATING MORE OR LESS DAS PER PLAN WORKD DO NOT REPRESENT A PART OF THE SCOPE OF WORK WITHIN THE BOUNDARIES OF THE SEAL THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT. ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNER'S) PRIOR TO FARRICATION TO ENSURE ADEQUATE LOAD THIS LAYOUT ARE BLAMS SPECIFIED BY BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNER(S) PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE FLOOR SYSTEM COMPONENTS REVIEWED IN THIS SUBMISSION.

MUNICIPALITY HAVING JURISDICTION TO OBTAIN LOT SPECIFIC SCHEDULE 1 FORM

MUNICIPALITY HAVING JURISDICTION TO OBTAIN LOT SPECIFIC SCHEDULE 1 FORM FROM THIS OFFICE PRIOR TO BUILDING PERMIT APROVAL. INSTALLERS OF THIS FLOOR SYSTEM AND THEIR COMPANIES HAVE THE RESPONSIBILITY OF ENSURING THEY HAVE A COPY OF THE NORDIC INSTALLATION GUIDE AND ANY OTHER MANUFACTURER'S PRODUCT LITERATURE WHICH WILL AID IN THE OVERALL PROPER INSTALLATION OF THIS FLOOR SYSTEM. INSTALLERS ARE TO READ ALL PRODUCT LITERATURE AND INSTALLATION GUIDELINES BEFORE PROCEEDING. THE SUPPLIER AND SEALING ENGINEER OF THIS FLOOR SYSTEM ARE NOT RESPONSIBLE FOR SURPLUS OR DEFICIT OF PRODUCTS AT PROJECT'S END. THIS LAYOUT IS A GUIDE ONLY. CONFIRMATION OF ALL QUANTITIES, LENGHTS, AND DETAILS, REMAINS THE RESPONSIBILITY OF THE FLOOR SYSTEM INSTALLATION CONTRACTOR.

DWG# TAM_1803/2/THROUGH DWG# TAM_180384 INCLUSIVE DATED_8142/

SEALED STRUCTURAL COMPONENTS ONLY:

SEALED, THIRD PARTY LVL TYPE BEAMS, BUILT-UP CONVENTIONAL BEAMS, HEADERS, AND CONCENTRATED LOADED NORDIC WOOD-I JOIST ONLY. 2 X 6 SQUASH BLOCK REQUIRED AT ALL EXTERIOR SUPPORTS OR AS PER PROJECT ENGINEER'S SPECIFICATIONS. WEB FILLER REINFORCEMENT REQUIRED AT ALL HANGER SUPPORTED JOIST EXCEEDING A REACTION OF 1500 LBS (FACTORED)-SEE DETAILS.

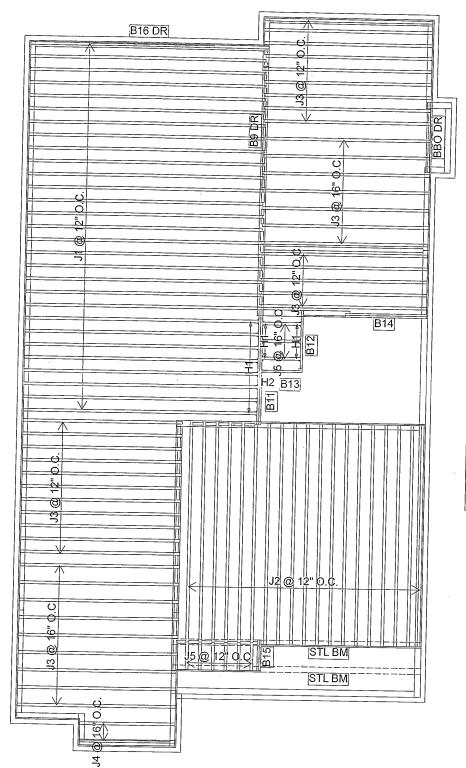
A COMPLETE FRAMING PLAN REQUIRES THE NORDIC PUBLISHED LITERATURE, WHICH INCLUDES INSTALLATION REQUIREMENTS, HANDLING AND STORAGE GUIDELINES, AND FORMS AN INTEGRAL PART OF THIS SEALED DOCUMENT. INSTALL SQUASH BLOCKS FOR TRANSFERRING POINT LOADS FROM GIRDER TRUSSES, HEADERS, AND BEAMS DOWN TO FOUNDATION COMPONENTS. FOR PROPER INSTALLATION, SEE NORDIC LITERATURE. PROVIDE 2 X 4 OR 2 X 6 STUD GRADE OR BETTER SQUASH BLOCKS, MATCHING SUPPORTED WALL WIDTH ABOVE BLOCKS. INSTALL SQUASH BLOCKS ON EACH SIDE OF JOIST. BLOCKING TO BE 1/16D DEEPER THAN JOIS DEPTH. SEE NORDIC LITERATURE FOR NAILING REQUIREMENT.

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION 3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND HE FIRM IS REGISTERED, IN APPROPRIATE CLASSES AND/OR CATEGORIES.

REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC

DWG # TAM BCIN: 26064 29991 SEALED STRUCTURAL COMPONENTS ONLY





		Products		
PlotID	Length	Product	Plies	Net Qty
J1	20-00-00	11 7/8" NI-40x	1	29
J2	18-00-00	11 7/8" NI-40x	1	19
J3	14-00-00	11 7/8" NI-40x	1	41
J4	8-00-00	11 7/8" NI-40x	1	2
J5	4-00-00	11 7/8" NI-40x	1	9
B16 DR~	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9 DR 🖊	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B11 /	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B14	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B12 ~	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B13 ′	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B15 ′	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
			_	

Connector Summary					
Qty	Manuf	Product			
3	H1	IUS2.56/11.88			
11	H1	IUS2.56/11.88			
1	H2	HUS1.81/10			

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.



FROM PLAN DATED: 2021/05

BUILDER:

ROYAL PINE HOMES

SITE:

VALES OF HUMBER NORTH

MODEL: 40-1

ELEVATION: A,B

LOT:

CITY: BRAMPTON

SALESMAN: RICK DICIANO

DESIGNER: AJ REVISION:

DATE: 2021-08-30

2nd FLOOR

DESIGN LOADS: L/480.000

LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 20.0 lb/ff²

SNOW LOAD: 24.0 lb/ft2

SUBFLOOR: 5/8" GLUED AND NAILED

LOADING:

BCIN: 26064; FIRM: 29991

ENGINEERING ONLY - DIMENSIONS TO BE VERIFIED ON SITE SUPPORTING STRUCTURE TO BE VERIFIED BY QUALIFIED BUILDING DESIGNER. ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO JOIST(S) AND FLOOR BEAM(S) INSTALLATION. ALL NOTES DESIGNATING MORE OR LESS DAS PER PLAN WORKD DO NOT REPRESENT A PART OF THE SCOPE OF WORK WITHIN THE BOUNDARIES OF THE SEAL THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT. ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNER(S) PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE FLOOR SYSTEM COMPONENTS REVIEWED IN THIS SUBMISSION.

MUNICIPALITY HAVING JURISDICTION TO OBTAIN LOT SPECIFIC SCHEDULE 1 FORM

FROM THIS OFFICE PRIOR TO BUILDING PERMIT APPROVAL.

INSTALLERS OF THIS FLOOR SYSTEM AND THEIR COMPANIES HAVE THE RESPONSIBILITY OF ENSURING THEY WHICH WILL AID IN THE OVERALL PROPER INSTALLATION GUIDE AND ANY OTHER MANUFACTURER'S PRODUCT LITERATURE WHICH WILL AID IN THE OVERALL PROPER INSTALLATION OF THIS FLOOR SYSTEM. INSTALLERS ARE TO READ ALL PRODUCT LITERATURE AND INSTALLATION GUIDELINES BEFORE PROCEEDING. THE SUPPLIER AND SEALING ENGINEER OF THIS FLOOR SYSTEM ARE NOT RESPONSIBLE FOR SURPLUS OR DEFICIT OF PRODUCTS AT PROJECT'S END. THIS LAYOUT IS A GUIDE ONLY. CONFIRMATION OF DEFICIT OF PRODUCTS AT PROJECT'S END. THIS LAYOUT IS A GUIDE ONLY. CONFIRMATION OF ALL QUANTITIES, LENGHTS, AND DETAILS, REMAINS THE RESPONSIBILITY OF THE FLOOR SYSTEM INSTALLATION CONTRACTOR.

DWG# TAM 1803924 THROUGH DWG# TAM 1804524, INCLUSIVE DATED 814-24

SEALED STRUCTURAL COMPONENTS ONLY:

SEALED, THIRD PARTY LVI. TYPE BEAMS, BUILT-UP CONVENTIONAL BEAMS, HEADERS, AND CONCENTRATED LOADED NORDIC WOOD-I JOIST ONLY, 2 X 6 SQUASH BLOCK REQUIRED AT ALL EXTERIOR SUPPORTS OR AS PEP PROJECT ENGINEER'S SPECIFICATIONS. WEB FILLER REINFORCEMENT REQUIRED AT ALL HANGER SUPPORTED JOIST EXCEEDING A REACTION OF 1500 LBS (FACTORED)-SEE DETAILS.

A COMPLETE FRAMING PLAN REQUIRES THE NORDIC PUBLISHED LITERATURE, WHICH INCLUDES INSTALLATION REQUIREMENTS, HANDLING AND STORAGE GUIDELINES, AND FORMS AN INTEGRAL PART OF THIS SEALED DOCUMENT. INSTALL SQUASH BLOCKS FOR TRANSFERRING POINT LOADS FROM GIRDER TRUSSES, HEADERS, AND BEAMS DOWN TO FOUNDATION COMPONENTS. FOR PROPER INSTALLATION, SEE NORDIC LITERATURE. PROVIDE 2 X 4 OR 2 X 6 STUD GRADE OR BETTER SQUASH BLOCKS, MATCHING SUPPORTED WALL WIDTH ABOVE BLOCKS. INSTALL SQUASH BLOCKS ON EACH SIDE OF JOIST. BLOCKING TO BE 1/16D DEEPER THAN JOIS DEPTH. SEE NORDIC LITERATURE FOR NAILING REQUIREMENT.

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION 3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND HE FIRM IS REGISTERED, IN APPROPRIATE CLASSES AND/OR CATEGORIES.

REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

BCIN: 26064 FIRM: 29991 26064 SEALED STRUCTURAL COMPONENTS ONLY



MORDIC

NORDIC JOIST

NS-GI33 **■**◆■

ENGLISH

Engineered Wood Products

BASIC INSTALLATION **GUIDE FOR** RESIDENTIAL **FLOORS**

JOIST

MORDIC

WEB STIFFENERS

nordic.ca

19

INSTALLING NORDIC I-JOISTS

- Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment
- Concentrated loads should only be applied to the top surface of the top flange. Concentrated loads should not be suspended from the bottom flange with the exception of light loads, such as ceiling fans or light fixtures
- I-joists must not be used in applications where they will be permanently exposed to weather, or will reach a moisture content of 15 percent or greater, such as in swimming pool or hot tub areas. They must not be installed where they will remain in direct contact with concrete or masonry.
- End bearing length must be at least 1-3/4 inch. For multiple-span joists, intermediate bearing length must be at least 3-1/2 inches.
- Ends of floor joists shall be restrained to prevent rollover. Use rim board or I-joist blocking panels.
- 1-joists installed beneath bearing walls perpendicular to the joists shall have full-depth blocking panels, rim board, or squash blocks (cripple blocks) to transfer gravity loads from above the floor system to the wall or foundation below.
- using a single 1-joist is 3,300 plf, and 6,600 plf if double 1-joists are used.
- Continuous lateral support of the I-joist's compression flange is required to prevent rotation and buckling. In simple span uses, lateral support of the top flange is normally supplied by the floor sheathing. In multiple-span or cantilever applications, bracing of the I-joist's bottom flange is also required at interior supports of multiple-span joists, and at the end support next to the cantilever extension. The ends of all cantilever extensions must be laterally braced as shown in details 3, 4, or 5.
- 2. Nails installed in flange face or edge shall be spaced in accordance
- with the applicable building code requirements or approved building plans, but should not be closer than those specified on page 3.3 of the Nordic Joist Technical Guide (NS-GT3).

15

NORDIC I-JOIST SERIES RESIDENTIAL SERIES

2×3 S-P-F No. 2

NI-40x

3/8 in. web

and 14 in.

Depths 9-1/2, 11-7/8

2×3 1950f MSR

- 3. Details 1 show only I-joist-specific fastener requirements. For other fastener requirements, see the applicable building code.
- 14. For proper temporary bracing of wood I-joists and placement of temporary construction loads, see APA Technical Note: Temporary Construction Loads over I-Joist Roofs and Floors, Form J735.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails.

SAFETY AND CONSTRUCTION PRECAUTIONS

l-joists are not stable until completely installed, and will not carry any load until fully brace

Avoid Accidents by Following these Important Guidelines:

- . Brace and nail each t-joist as it is installed, using hangers, blocking panels, rim board, and/ or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior
- When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2-inch nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two I-joists. Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet
- of I-joists at the end of the bay. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure namels
- rim board, or cross-bridging.
- Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- . Never install a damaged I-joist.

NI-60

2×3 2100f MSR

14 and 16 in. 33 pieces per uni

10

proper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation

2×4 2100f MSR 3/8 in. web

23 pieces per unit

Denths

2x plate flush with inside face of wall-or beam. 1/8" overhang allowed past inside face of wall or beam





Never stack building materials over unsheathed I-joists. Once sheathed, do no overstress I-joist with concentrated loads from building materials

RIM BOARDS

Width Length

APA Rim Board Plus

One 2-1/2" nail, one side only

2x4 2400f MSR 7/16 in. web

23 pieces per uni

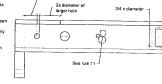
WEB HOLES IN I-JOISTS

- Rules for Cutting Holes in I-Joists
- I-joist top and bottom fianges must never be cut, notched or otherwise modified
- Whenever possible, field-cut holes should be centred on the middle of the web

- Holes measuring 1-1/2 inch or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size m be permitted subject to verification.

TABLE 6

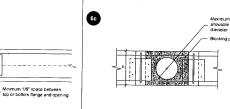
A group of round holes at approximate the same location shall be permitted if meets the requirements for a single



DUCT CHASE OPENINGS

6b

- Rules for Cutting Duct Chase Openings in I-loists
- The distance between the inside edge of the support and the centreline of a duct chase opening shall be in compliance with the requirements of Table 6.2.
- The maximum allowable hole size for a lateral-restraint-only blocking panel is 2.6 of the lesser dimension of the blocking's depth or length. Assuming the blocking panel is longer than its height (or depth), the table aside applies. For other applications, contact Nordic Structures. -joist top and bottom flanges must never be cut, notched or otherwise modifie . Holes cut into the blocking panels are subject to the following limitations The top and bottom flanges of an I-joist blocking panel must never be cut, notched or otherwise modified.
- The maximum depth of a duct chase opening that can be cut into an Lipist web shall equal the clear distinct between the flanges of the Lipist minus 148 inch a minimum of 1/8 inch should always be maintained between the top or bottom of the opening and the adjacent Lipist flange.
- Limit one maximum-size duct chase opening per spar



HOLES IN BLOCKING PANELS

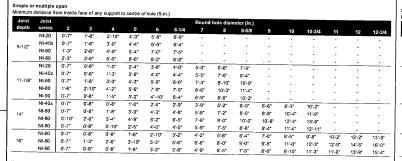
Field-cut holes must be centred in the blocking horizontally.

All holes must be cut in a workman-like manner in ac

While round holes are preferred, rectangle holes may be used provided the corners are not over cut. Slightly rounding corners or pre-drilling corner with a 1-inch-diameter bit is recommended

I-joist or rim board blocking depth (in.)	Maximum allowable hole diameter (in.) (*)
9-1/2	6-1/4
11-7/8	7-3/4
14	9-1/4
16	10-1/2

.1 -	LOCATION	OF	WEB	HOL	ES	

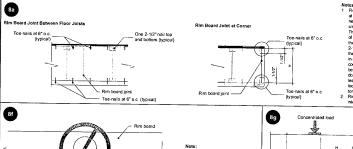


Live load = 40 psf and dead load = 15 psf L/480 under live load and L/240 under total load

TABLE 6.2 - LOCATION OF DUCT CHASE OPENINGS

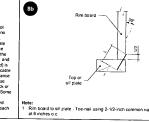
Jois					Duct c	hase leng	th (in.)			
dept	1 series	8 -	10	12	14	16	18	20	22	24
	NI-20	4'-1"	4'-5"	4'-10"	-				-	-
9-1/2	. NI-40x	5'-3"	5'-8"	6'-0"	6-5	6'-10"	7'-3"	7'-8"		
5-1/2	NI-60	5'-4"	5'-9"	6'-2"	6-7	7'-1"	7'-5"	8'-0"		
1	NI-80	5'-3"	5'-8"	6"-0"	6'-5*	6'-10"	7'-3"	7-8*	8'-2"	8'-6"
	NI-20	5'-9"	6'-2"	66.			-			
	NI-40x	6"-8"	7-2	7'-6"	8'-1"	8'-6"	9"-1"	9'-6"		
11-7/	B* NI-60	7-3*	7'-8"	8'-0"	8'-6"	9'-0"	9-3-	9'-9"		
	NI-80	7-2"	7-7	8'-0"	8"-5"	8'-10"	9'-3"	9'-8"	10'-2"	10'-8
	NI-90	7-6*	7-11"	8'-4"	8'-9"	9'-2"	9'-7"	10'-1"	10:-7	10'-1
1	NI-40x	8'-1"	8'-7"	9"-0"	9'-6"	10'-1"	10'-7"	11'-2"		-
14-	NI-60	8'-9"	9'-3"	9'-8"	10'-11"	10'-6"	11'-1"	11'-6"		
1	NI-80	9"-0"	9"-3"	9'-9"	10'-1"	10'-7"	11'-1"	11'-6"	12'-1"	12'-6'
	NI-90	9'-2"	9'-8"	10'-0"	10'-6"	10'-11"	11'-5"	11'-9"	12'-4"	12'-1
	NI-60	10'-3"	10'-8"	11'-2*	11'-6"	12"-1"	12"-6"	13'-2"		·
16*	NI-80	10"-4"	10'-9"	11'-3"	11'-9"	12'-1"	12'-7"	13'-1"	13"-8"	14'-4'
}	NI-90	10'-9"	11'-2"	11'-8"	12'-0"	12'-6"	13'-0"	13'-6"	14'-2"	14'-16

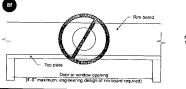
RIM BOARDS

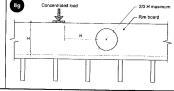


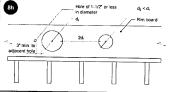
6-1/4 8-5/8

10-3/4

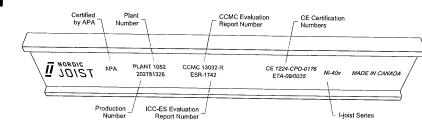


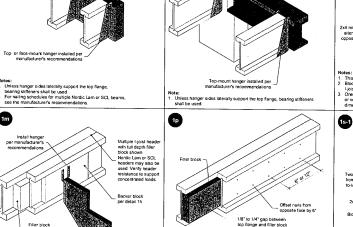


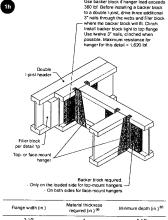




I-JOIST MARKING

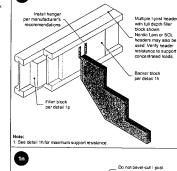


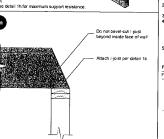


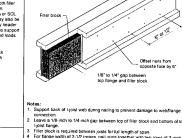


Flange width (in)	Material thickness required (in) (ii)	Minimum depth (in.)
2-1/2	1	5-1/2
3-1/2	1-1/2	7-1/4









connection 2. Leave a 186 inch to 144-inch gap between top of filer block and bottom of top i-poist fange.

3 Filer block is required between josts for full length of span

4 For fange width of 2.12 inches, nail josts together with two rows of 3 inch nails at 12 inches or, climited when possible) on each yade of the double-just folkid of four nails per foot). For fange width of 3.12 inches, use two rows of 3 inch nails at 6 inches or, on each side of the double-just (folial of eight nails per foot).

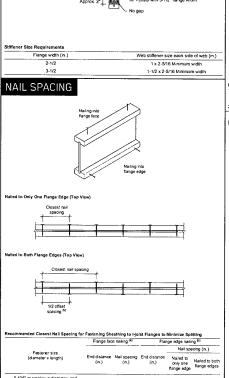
The maximum factored coad that may be applied to one side of the double. lange width (in.) Net depth (in.) Filler block size (in.) Example 9-1/2 2-1/8 to 2-1/4 x 6 2x6 + 5/8" or 3/4" snei 2-1/8 to 2-1/4 x 8 2x8 + 5/8" or 3/4" sheat: 2-1/8 to 2-1/4 x 10 2x10 + 5/8" or 3/4" sheat: 2-1/8 to 2-1/4 x 12 2x12 + 5/8" or 3/4" sheathir

3 x 12

11-7/8

2 x 2x12

FOR ALL construction details \rightarrow DC3



Greater than 0.128" up to 0.148" in diameter, and 3-1/4" or shorter in length

Schedule 1: Designer Information

Use one form for each individual who revie	ws and takes re	sponsibility for design esti	vitioo with rooms at t	a Alamana ta at	
A. Project Information	we are taken to	Application	number:	o the project.	
Building number, street name:		Application	Unit no.	1 -4/	
			Offictio.	Lot/con.	
Municipality	Postal code	Plan number/ other de	scription		
CITY OF BRAMPTON				_	
B. Individual who reviews and takes	s responsibili	ty for design activities	5		
Name SAM KATSOULAKOS		Firm			
Street address		MICRO CITY EN	SINEERING SERVI	CES INC.	
R.R #1, PO BOX 61			Unit no.	Lot/con.	
Municipality	Postal code	Province	F-mail moong	r@xplornet.com	
GLENCOE	NOL 1MO	ONTARIO	L man moeng	(@xpiornet.com	
Telephone number (519) 287-2242 Business	Fax number		Cell number		
C. Design activities undertaken by in	ndividual ider	ntified in Section B. [E	Building Code Ta	ble 3.5.2.1. of	
Division of					
☐ House	☐ HVAC -		☑ Building S	tructural	
☐ Small Buildings ☐ Large Buildings		Services	☐ Plumbing -		
☐ Large Buildings ☐ Complex Buildings	☐ Detectio	n, Lighting and Power	☐ Plumbing –	All Buildings	
Description of designer's work:	☐ Fire Pro	tection	☐ On-site Se	☐ On-site Sewage Systems	
ROYAL PINE HOMES—PROJECT: VALES OF HUMBE	ED NODTH MODE	1.40.4 51 51/4 05 5 10-5			
1, F 1, F 1, 1 1, F - F 1, O 1, F F 1, E D F F O O K 9 1 9 1 1	-1// (.()1//12()1/11-1	ハエ いちめかいがいてる マガレエック		DI ANI OUEDELIES SIL	
THE WALL CONTRACT THE PROPERTY OF THE PROPERTY	119000-ZI DA H	ED 9-01-21) SUPPORTI	NG STRUCTURE /	PLAN SUPPLIED BY	
AND VERIFED BY QUALIFIED BUILDING DE	SIGNER.		NO STRUCTURE (3) TO BE REVIEWED	
D. D. J. (i. c.)					
D. Declaration of Designer					
I, SAM KATSOULAKOS			leclare that (choose	one as appropriate):	
(print name)			icolare that (choose	one as appropriate):	
☑ I review and take responsibility	for the design v	vork on behalf of a firm red	nistered under subs	ection 2.2.4 of Division	
C, of the Building Code. I am qu	ualified, and the	firm is registered, in the a	ppropriate classes/c	categories	
				atogonoo.	
Individual BCIN: 26064					
Individual BCIN: <u>26064</u>					
Firm BCIN: 29991					
	-				
☐ I review and take responsibility	for the design ar	nd am qualified in the appr	onriate category as	an "other designer"	
didei subsection 3.2.3.01 DIVISIO	on C, of the Buil	ding Code.	opriate dategory as	an other designer	
Individual BCIN:					
Pagin for averagelian for					
Basis for exemption from re The design work is exempt from	gistration:	1 110			
and an individual state of the individual	the registration	and qualification requirem	nents of the Building	Code.	
Basis for exemption from re	gistration and q	ualification:			
I certify that:					
The information contained in this sch	edule is true to t	the best of my knowledge.			
I have submitted this application with	the knowledge	and consent of the firm.			
Date 9	nin .	Signature of D. :		The state of the s	
Date	v/ -/ S	Signature of Designer			

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d).of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- 2. Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG #TAM19553-21S

DWG #TAM19556-21S

Schedule 1: Designer Information

Use one form for each individual who revie A. Project Information	ws and takes re	coopsibility for the t		
	THE WIND LANCES TE	Application	ivities with respect to	the project.
Building number, street name:		Application		
			Unit no.	Lot/con.
Municipality	Postal code	Plan number/ other de	escription	
CITY OF BRAMPTON			•	
B. Individual who reviews and take	s responsibili	ty for design activities	S	
SAM KATSOULAKOS		Firm		
Street address		MICRO CITY EN	GINEERING SERVI	CES INC.
R.R #1, PO BOX 61			Unit no.	Lot/con.
Municipality	Postal code	Province	E mail mass	
GLENCOE	NOL 1MO	ONTARIO	E-mail incengi	@xplornet.com
Telephone number (519) 287-2242 Business	Fax number		Cell number	
			1	
C. Design activities undertaken by i Division C]	ndividual iden	tified in Section B. I	Building Code Tal	10 3 5 2 1 of
			- ununing oode Tal	JIE J.J.Z. [, O]
☐ House	☐ HVAC -			ructural
☐ Small Buildings	Building	Services	☐ Plumbing –	House
☐ Large Buildings ☐ Complex Buildings	Detection	n, Lighting and Power	☐ Plumbing –	All Buildings
☐ Complex Buildings Description of designer's work:	☐ Fire Prot	ection	☐ On-site Sewage Systems	
ROYAL PINE HOMES-PROJECT: VALES OF HUMBER	D HODEL			
ROYAL PINE HOMES-PROJECT: VALES OF HUMBE REVIEW PRE-ENGINEERED FLOOR SYSTI TAMARACK LUMBER INC. (SEE DWG #TAM	EK NOKTH-MODE	L: 40-1-ELEV.A OR B-1ST FL	OOR-SUNKEN-NOT LO	T SPECIFIC
TAMARACK LUMBER INC. (SEE DWG #TAM	119554 24 DATE	TO A 01 21) CURRORY	OUT PLACEMENT	PLAN SUPPLIED BY
AND VERIFED BY QUALIFIED BUILDING DE	SIGNER.	-D <u>3-01-21</u>). SUPPORT	NG STRUCTURE (8	S) TO BE REVIEWED
D. Declaration of Designer				
I, <u>SAM KATSOUL</u> AKOS			1 1 4	
(print name)		C	leclare that (choose	one as appropriate):
	for the decian u	ork on bobolf of a fin		
☑ I review and take responsibility C, of the Building Code. I am qu	alified, and the	firm is registered in the a	gistered under subse	ction 3.2.4.of Division
		min to registered, in the a	hbrobuste classes/ca	ategories.
L. P. L. L. Donn				
Individual BCIN:26064				
Firm BCIN: 29991				
7 IIII DOIN				
☐ I review and take responsibility t	or the decian on	dom munification		
☐ I review and take responsibility f under subsection 3.2.5.of Division	on the design an	u am qualified in the appr	opriate category as	an "other designer"
Individual BCIN:	o, or the bane	ing code.		
				
Basis for exemption from re	gistration:			
☐ The design work is exempt from	the registration a	and qualification requirem	ents of the Building	Code
pasis for exemption from re	gistration and qu	alification:		
I certify that:				
The information contained in this school	edule is true to the	ne best of mv knowledge		
I have submitted this application with	the knowledge a	and consent of the firm		
	. 5	or are mill,		
Data			0	
Date 9	olm s	ignature of Designer	L-	

NOTE

- 1. For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d).of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
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DWG #TAM19554-21S DWG #TAM19557-21S Schedule 1: Designer Information

Building number, street name:		esponsibility for design act Application	number:	
			Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other de	escription	Loucon.
B. Individual who reviews and tak	OC #00mon-il-ili			-
Name	es responsibili	ty for design activities	S	
SAM KATSOULAKOS		Firm		
Street address		MICRO CITY EN	GINEERING SERVI	CES INC.
R.R #1, PO BOX 61			Unit no.	Lot/con.
Municipality	Postal code	Province		
GLENCOE Telephone number	NOL 1MO	ONTARIO	⊏-mail mceng	r@xplornet.com
	Fax number		Cell number	
C. Design activities undertaken by Division C]	individual ider	tified in Section P	Puilding Carl T	
Jivision C]		va in oection B. [E	ounding Code Ta	ble 3.5.2.1. of
☐ House	☐ HVAC -			
☐ Small Buildings		Services	⊠ Building S	tructural
☐ Large Buildings		n, Lighting and Power	☐ Plumbing —	House
☐ Complex Buildings	☐ Fire Prot	ection	☐ Plumbing –	All Buildings
Description of designer's work: DYAL PINE HOMES-PROJECT:VALES OF HUMB EVIEW PRE-ENGINEERED FLOOR SYS			☐ On-site Sev	wage Systems
SAM KATSOULAKOS (print name		d	eclare that (choose	one as appropriate
I review and take responsibilit C, of the Building Code. I am o	y for the dealers			
				atogories.
Individual BCIN:2606	4			atogories.
Firm BCIN: 2999	l			
F' 50	for the design and	d am qualified in the approing Code.		
Firm BCIN: 2999? I review and take responsibility under subsection 3.2.5.of Divis Individual BCIN: Basis for exemption from receiving the design work is exempt from Basis for exemption from receiving the design work is exempt from Basis for exemption from receiving the design work is exemption.	for the design and ion C, of the Build egistration:	and qualification requirement	opriate category as a	an "other designer"
Firm BCIN: 2999? I review and take responsibility under subsection 3.2.5.of Divis Individual BCIN: Basis for exemption from recognition.	for the design and ion C, of the Build egistration: the registration and qualetter is true to the	and qualification requirementalification:	opriate category as a	an "other designer"

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DWG #TAM19555-21S DWG #TAM19558-21S

COMPANYJuly 9, 2021 14:58

PROJECT
J1 1ST FLOOR.wwb

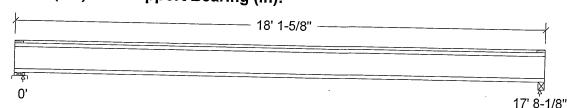
Design Check Calculation Sheet

Nordic Sizer - Canada 8.0

Loads:

Load	Type	Distribution	Pat- tern	Location Start	[ft] End		Unit
Load1 Load2	Dead Live	Full Area Full Area		Deare	End	Start End 15.00 40.00	psf psf

Maximum Reactions (lbs) and Support Bearing (in):



			17. 8-1/8
Unfactored:			
Dead Live Factored:	133 354		133
Total Bearing:	696		696
Capacity			
Joist	2336		
Support	7744		2138
Des ratio			-
Joist	0.30		
Support	0.09		0.33
Load case	#2		- 1
Length	4-3/8		#2
Min req'd	1-1/2		2-5/8
Stiffener	No		1-1/2
KD	1.00		No
KB support	1.00		1.00
fcp sup	769		-
Kzcp sup	1.15		-
		for joicto in 1.1/2" for outpring a	-

'Minimum bearing length for joists is 1-1/2" for exterior supports

Nordic Joist 11-7/8" NI-40x Floor joist @ 12" o.c.

Supports: 1 - Lumber Sill plate, No.1/No.2; 2 - Steel Beam, W; Total length: 18' 1-5/8"; Clear span: 17' 6-5/8"; 3/4" nailed and glued OSB sheathing This section PASSES the design code check.



P6 62

OWO NO.TAM/BOL9-21 STRUCTURAL COMPONENT ONLY

Nordic Sizer - Canada 8.0

Page 2

Limit States Design using CSA O86-14 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 696	Vr = 2336	lbs	$\frac{Marysis/Design}{Vf/Vr = 0.30}$
Moment(+)	Mf = 3076	Mr = 6255	lbs-ft	
Perm. Defl'n	$0.08 = \langle L/999 \rangle$	0.59 = L/360	in	Mf/Mr = 0.49
Live Defl'n	0.22 = L/953	0.44 = L/480		0.14
Total Defl'n	0.31 = L/693		in	0.50
Bare Defl'n	0.27 = L/798	0.88 = L/240	in	0.35
Vibration		0.59 = L/360	in	0.45
1	Lmax = 17'-8.1	Lv = 19'-6.3	ft	0.91
Defl'n	= 0.027	= 0.035	in	0.77

Additional Data:

FACTORS:	f/E	KD	KH	KZ	KL	KT	KS	KN	T C#
Vr	2336	1.00	1.00	_	_				тC#
Mr+						_	_		#2
			1.00	_	1.000		-	_	#2
EI	371.1 m	lllon	_		-	_	_	_	#2

CRITICAL LOAD COMBINATIONS:

Shear :	LC #2 = 1.25D + 1.5L
Moment(+):	LC #2 = 1.25D + 1.5L
Deflection:	LC $#1 = 1.0D$ (permanent)
	LC #2 = 1.0D + 1.0L (live)
	LC #2 = 1.0D + 1.0L (total)
	LC #2 = 1.0D + 1.0L (bare joist)
Bearing :	Support $1 - LC \#2 = 1.25D + 1.5L$
	Support $2 - LC \# 2 = 1.25D + 1.5L$

Load Types: D=dead L=live(use,occupancy)

Load Patterns: s=S/2 L=L+Ls _=no pattern load in this span All Load Combinations (LCs) are listed in the Analysis output

CALCULATIONS:

EIeff = 443.45 lb-in^2 K = 6.18e06 lbs GA = 0.77e06 lb

"Live" deflection is due to all non-dead loads (live, wind, snow...) CONFORMS TO OBC 2012

Design Notes:

AMENDED 2020

1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA 086-14 Engineering Design in Wood standard, Update No. 2 (June 2017).

2. Please verify that the default deflection limits are appropriate for your application.

3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.

4. Nordic I-joists are listed in CCMC evaluation report 13032-R.

- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. Allowable vibration-controlled span as per the Concluding Report, Development of Design Procedures for Vibration Controlled Spans using Engineered Wood Members, CWC et al for CCMC, 1997.
- 7. Floor vibration design from the CCMC Concluding Report (1997) on vibration controlled spans for engineered wood products.
- 8. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.

OF OTHER PORTS

UND HU. YAH 18029-21

STRUCTURAL

COMPONENT BNLY

COMPANYJuly 9, 2021 14:59

PROJECT
J1 2ND FLOOR.wwb

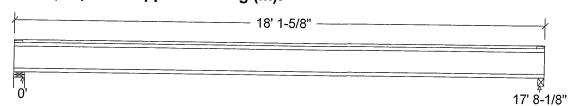
Design Check Calculation Sheet

Nordic Sizer - Canada 8.0

Loads:

Load	Туре	Distribution	Pat- tern	Location Start	[ft] End	Magnitude Start End	Unit
Load1 Load2	Dead Live	Full Area Full Area		20010	Bird	15.00 40.00	psf psf

Maximum Reactions (lbs) and Support Bearing (in):



Unfactored:]
Dead	133	13
Live	354	35
Factored:		
Total	696	696
Bearing:		050
Capacity		
Joist	2336	2138
Support	7744	4043
Des ratio		404
Joist	0.30	0.33
Support	0.09	0.17
Load case	#2	#2
Length	4-3/8	2-5/8
Min req'd	1-1/2	
Stiffener	No	1-1/2
KD	1.00	No.
KB support	-	1.00
fcp sup	769	1.00
Kzcp sup	- 1	769
48.6		1.00

*Minimum bearing length for joists is 1-1/2" for exterior supports

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

Nordic Joist 11-7/8" NI-40x Floor joist @ 12" o.c.

Supports: 1 - Lumber Wall, No.1/No.2; 2 - Lumber Beam, No.1/No.2; Total length: 18' 1-5/8"; Clear span: 17' 6-5/8"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling

This section PASSES the design code check.

Limit States Design using CSA 086-14 and Vibration Criterion:

Analysis Value	Design Value	Unit	Analysis/Design
Vf = 696	Vr = 2336	lbs	V£2Vr = 0.30
Mf = 3076	Mr = 6255	lbs-ft 4	ME/Mr = 0.49
0.09 = < L/999	0.59 = L/360	in /.	0.14
0.23 = L/933	0.44 = L/480	lin 🛭 🕉	
0.31 = L/678	0.88 = L/240	in 15	0.31
0.27 = L/800	0.59 = L/360		S. KATSOULAROS 0,45
Lmax = 17'-8.1	Lv = 18'-11.1	ft 🎒	1 /// # 1
= 0.029	= 0.035	in \	0, 93
	Vf = 696 Mf = 3076 0.09 = < L/999 0.23 = L/933 0.31 = L/678 0.27 = L/800 Lmax = 17'-8.1	Vf = 696 Mf = 3076 0.09 = < L/999 0.23 = L/933 0.31 = L/678 0.27 = L/800 0.28 = L/240 0.29 = L/360 0.29 = L/360 0.20 = L/360 0.21 = L/800 0.22 = L/800 0.23 = L/360 0.24 = L/240 0.25 = L/360 0.27 = L/800 0.29 = L/360 0.29 = L/360	Vf = 696 Mf = 3076 Mr = 6255 0.09 = < L/999 0.59 = L/360 0.23 = L/933 0.44 = L/480 0.31 = L/678 0.27 = L/800 0.59 = L/360 0.59 = L/360 0.71 = L/678 0.88 = L/240 0.72 = L/800 0.73 = L/800 0.74 = L/480 0.75 = L/360 0.75 = L/360

STRUCTURAL COMPONENT ONLY

WoodWorks® Sizer

for NORDIC STRUCTURES

J1 2ND FLOOR.wwb

Nordic Sizer - Canada 8.0

Page 2

Additiona	l Data:									
FACTORS:	f/E	KD	KH	KZ	KL	KT	KS	KN	LC#	
Vr	2336	1.00	1.00	-	-	_	_	-	#2	
ł	6255		1.00		1.000	_	-	_	#2	
EI	371.1 m			_	_	_	_		#2	
CRITICAL L	OAD COMB									
Shear	: LC #2		5D + 1.5							
) : LC #2									
Deflecti	on: LC #1									
			0 + 1.0L							
) + 1.0L							
			+ 1.0L							
Bearing	: Suppo:									
	Suppo	rt 2 - I	LC #2 = 1	1.25D +	1.5L					
Load Type	es: D=de	ad L=li	.ve(use,	occupand	cy)					
Load Patt	terns: s=	S/2 L=I	_=r	no patte	ern load	in this	s span			
	Combinat	ions (LC	cs) are	listed i	in the An	alysis	output			
CALCULATION										
Eleff = 4										
"Live" de	eflection	is due	to all r	non-deac	d loads (live, w	ind, sno	ow) CQ	nporms '	TO OBC 2012

Design Notes:

AMENDED 2020

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. Allowable vibration-controlled span as per the Concluding Report, Development of Design Procedures for Vibration Controlled Spans using Engineered Wood Members, CWC et al for CCMC, 1997.
- 7. Floor vibration design from the CCMC Concluding Report (1997) on vibration controlled spans for engineered wood products.
- 8. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.

S. KATSOULAKOS

S. KATSOULAKOS

SOUNCE OF ONTE

BWG NO. YAW 10030-21 STRUCTURAL COMPONENT ONLY





PASSED

1ST FLR FRAMING\Flush Beams\B1(i2741) (Flush Beam)

BC CALC® Member Report

Dry | 2 spans | L cant.

July 9, 2021 13:20:12

Build 7773 Job name:

Address:

Customer:

B2, 9-3/4"

Code reports:

City, Province, Postal Code: BRAMPTON

CCMC 12472-R

File name:

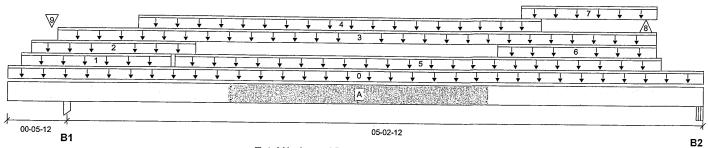
40-1 EL A,B SUNKEN.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B1(i2741)

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 05-08-08

Reaction Summary (Down / Uplift) (lbs)

7314 / 118

Bearing Live Dead Snow Wind B1, 5-1/2" 6235 / 0 3740 / 0

4437 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	•
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	05-08-08	Тор		12			00-00-00
1	4(i1098)	Unf. Lin. (lb/ft)	L	00-01-04	01-03-14	Top	295	148			n\a
2	4(i1098)	Unf. Lin. (lb/ft)	L	00-02-04	01-06-04	Top	189	95			n\a
3	4(i1098)	Unf. Lin. (lb/ft)	L	00-04-12	05-03-12	aoT		81			n\a
4	4(i1098)	Unf. Lin. (lb/ft)	L	01-00-10	04-04-04	•	555	278			n\a
5	Smoothed Load	Unf. Lin. (lb/ft)	L	01-04-04	05-04-04	Top	366	183			n\a
6	4(i1098)	Unf. Lin. (lb/ft)	L	03-11-14	05-03-12	•	271	136			n\a
7	4(i1098)	Unf. Lin. (lb/ft)	L	04-02-04	05-03-12		205	102			n\a
8	-	Conc. Pt. (lbs)	L	05-02-10	05-02-10	•	4827		0		
9	-	Conc. Pt. (lbs)	Ĺ	00-04-02	00-04-02	Top	4085	2439	J		n\a n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	5227 ft-lbs	35392 ft-lbs	14.8%	3	02-10-04
Neg. Moment	-1143 ft-lbs	-35392 ft-lbs	3.2%	1	00-05-12
End Shear	4131 lbs	14464 lbs	28.6%	3	03-10-14
Cont. Shear	4181 lbs	14464 lbs	28.9%	1	01-08-06
Total Load Deflection	L/999 (0.013")	n\a	n\a	10	02-08-07
Live Load Deflection	L/999 (0.009")	n\a	n\a	13	02-08-07
Total Neg. Defl.	2xL/1998 (-0.004")	n\a	n\a	10	00-00-00
Max Defl.	0.013"	n\a	n\a	10	02-08-07
Span / Depth	4.5				

Bearing	յ Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	5-1/2" x 3-1/2"	14027 lbs	89.7%	59.7%	Unspecified
B2	Beam	9-3/4" x 3-1/2"	16517 lbs	90.6%	39.7%	Unspecified

Cautions

Concentrated side load(s) 6,31 are closer than 18" from end of member. Please consult a technical representative or Professional of Record.







PASSED

1ST FLR FRAMING\Flush Beams\B1(i2741) (Flush Beam)

BC CALC® Member Report Build 7773

Dry | 2 spans | L cant.

July 9, 2021 13:20:12

Job name:

Address:

City, Province, Postal Code: BRAMPTON

40-1 EL A,B SUNKEN.mmdl

File name:

Description: 1ST FLR FRAMING\Flush Beams\B1(i2741)

Specifier:

Designer: Company: AJ

Customer: Code reports:

CCMC 12472-R

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

CONFORMS TO OBE 2012

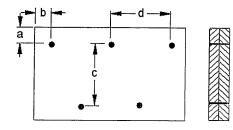
Importance Factor: Normal Part code: Part 9

AMENDED 2020

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-08-08.

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

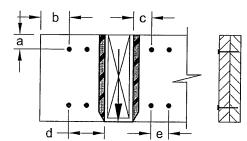
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c = 7-7/8" d = 🗩 8 c/

Calculated Side Load = 385.4 lb/ft Connectors are: 16d Nails 3½" ARDOX SPIRAL

Connection Diagrams: Concentrated Side Loads

Connection Tag: A Applies to load tag(s): 12+14+16



a minimum = 2"

b minimum = 4"

c minimum = 4"

d maximum = 12"

e minimum = 4"

Connectors are: 16d 1: Nails

ARDOX SPIRAL



DWG NO. TAM /803/-2 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B2(i2580) (Flush Beam)

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

July 9, 2021 13:20:12

Build 7773 Job name:

Address:

City, Province, Postal Code: BRAMPTON

Customer: Code reports:

CCMC 12472-R

File name:

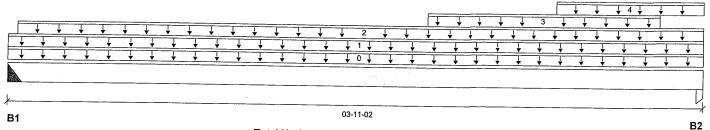
40-1 EL A,B SUNKEN.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B2(i2580)

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 03-11-02

Snow

Reaction Summary (Down / Uplift) (lbs)

Live B1, 2" 218/0 280 / 0 B2, 1-3/4" 629 / 0 498 / 0

Loa Tag	ad Summary Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-11-02	Тор		6	1.00	1.15	00-00-00
1	FC2 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	03-11-02	Тор	27	13			n\a
2	5(i1099)	Unf. Lin. (lb/ft)	L	00-00-10	03-11-02	Top		81			n\a
3	5(i1099)	Unf. Lin. (lb/ft)	L	02-04-02	03-08-02	Top	546	284			n\a
4	5(i1099)	Unf. Lin. (lb/ft)	L	03-01-00	03-11-02	Top	15	10			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1096 ft-lbs	17696 ft-lbs	6.2%	1	02-06-13
End Shear	1373 lbs	7232 lbs	19.0%	1	02-09-08
Total Load Deflection	L/999 (0.004")	n\a	n\a	4	02-01-07
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	02-01-14
Max Defl.	0.004"	n\a	n\a	4	02-01-07
Span / Depth	3.8			-	0 <u>2</u> -01-01

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 1-3/4"	678 lbs	n\a	15.9%	HUS1.81/10
B2	Column	1-3/4" x 1-3/4"	1566 lbs	63.0%	41.9%	Unspecified

Cautions

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

OVINCE OF ONE 846 HO, TAM 1853 LED 1 STRUCTURAL

COMPONENT ONLY





PASSED

1ST FLR FRAMING\Flush Beams\B2(i2580) (Flush Beam) **BC CALC® Member Report**

Dry | 1 span | No cant.

July 9, 2021 13:20:12

Build 7773 Job name:

Address:

Customer:

Code reports:

City, Province, Postal Code: BRAMPTON

CCMC 12472-R

File name:

40-1 EL A,B SUNKEN.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B2(i2580)

Specifier:

Designer: AJ

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 03-11-02.

CONFORMS TO OBC 2012

AMENDED 2020



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





PASSED

1ST FLR FRAMING\Flush Beams\B3(i2572) (Flush Beam)

Dry | 1 span | No cant.

July 9, 2021 13:20:12

Build 7773

Job name:

Address:

Customer:

Code reports:

Load Summary

Tag Description

City, Province, Postal Code: BRAMPTON

BC CALC® Member Report

CCMC 12472-R

File name:

40-1 EL A.B SUNKEN.mmdl

Live

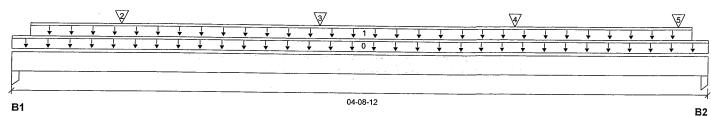
Description:

1ST FLR FRAMING\Flush Beams\B3(i2572)

Specifier:

Designer: AJ

Company:



Total Horizontal Product Length = 04-08-12

Reaction Summary (Down / Unlift) (lbs)

		pinity (188)		
Bearing	Live	Dead	Snow	Wind
B1, 3-1/2"	523 / 0	276 / 0	**************************************	
B2 3-1/2"	517 / 0	503 / 0		

rag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-08-12	Тор		6	· · · · · · · · · · · · · · · · · · ·		00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	00-01-07	04-07-06	Top	120	60			n\a
2	J8(i2654)	Conc. Pt. (lbs)	L	00-08-12	00-08-12	Top	130	65			n\a
3	J8(i2655)	Conc. Pt. (lbs)	L	02-00-12	02-00-12	Top	168	84			. n\a
4	J8(i2577)	Conc. Pt. (lbs)	L	03-04-12	03-04-12	Top	167	84		TESSIO	
5	5(i1099)	Conc. Pt. (lbs)	L	04-06-04	04-06-04	Тор	36	248			n\a
Со	ntrols Summary	Factored Demand	Factored Resistance	Dem: Resis	and/ stance	Case	Location		Z L	24/	

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1207 ft-lbs	17696 ft-lbs	6.8%	1	02-00-12
End Shear	733 lbs	7232 lbs	10.1%	1	03-05-06
Total Load Deflection	L/999 (0.006")	n\a	n\a	4	02-04-04
Live Load Deflection	L/999 (0.004")	n\a	n\a	5	02-04-04
Max Defl.	0.006"	n\a	n\a	4	02-04-04
Span / Depth	4.3				

Bea	ring Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	3-1/2" x 1-3/4"	1130 lbs	22.7%	15.1%	Unspecified
B2	Column	3-1/2" x 1-3/4"	1405 lbs	28.2%	18.8%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-01-08.

CONFORMS TO OBC 2012

AMENDED 2020



POVINCE OF ON

Disclosure

Snow

Wind

Tributary

Dead

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





PASSED

July 9, 2021 13:20:12

1ST FLR FRAMING\Flush Beams\B4(i2608) (Flush Beam) Dry | 1 span | No cant.

BC CALC® Member Report **Build 7773**

Job name:

Address:

City, Province, Postal Code: BRAMPTON

Customer: Code reports:

CCMC 12472-R

File name:

40-1 EL A,B SUNKEN.mmdl

Wind

Description: 1ST FLR FRAMING\Flush Beams\B4(i2608)

Specifier:

Designer: ΑJ

Company:

																											7	7				
	, †	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u>+</u>	<u> </u>	+		+	<u></u>	1	+	+	+	1 ↓			,	Į.	+		+	Ţ	+	+	¥	+	+	Ţ	1	¥
	, +		<u></u>	+	+	Ţ	+	+	+	+	¥	+	+	+	Ţ	0 1	,	,		Ţ	Ţ	Ţ	Ţ	+	¥	1	Ţ		Ţ	Ţ	Ţ	1
																1 1	77 . 7															
/																																
B 1															06	-00-00	כ															B2

Total Horizontal Product Length = 06-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow
B1, 4"	146 / 0	127 / 0	
B2. 1-3/4"	571 / 0	408 / 0	

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	06-00-00	Тор		12			00-00-00
1	FC2 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	06-00-00	Тор	15	7			n\a
2	PBO5(i1105)	Conc. Pt. (lbs)	L	05-00-02	05-00-02	Тор	630	419			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1215 ft-lbs	35392 ft-lbs	3.4%	1	05-00-02
End Shear	1098 lbs	14464 lbs	7.6%	1	04-10-06
Total Load Deflection	L/999 (0.004")	n\a	n\a	4	03-05-02
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	03-05-02
Max Defl.	0.004"	n\a	n\a	4	03-05-02
Span / Depth	5.7				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	4" x 3-1/2"	378 lbs	n\a	2.2%	HGUS410
B2	Column	1-3/4" x 3-1/2"	1367 lbs	27.5%	18.3%	Unspecified

Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CANYORMS TO OBC 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 06-00-00.

ove no. Tan 18039-21 STRUCTURAL COMPONENT ONLY





PASSED

July 9, 2021 13:20:12

1ST FLR FRAMING\Flush Beams\B4(i2608) (Flush Beam) Dry | 1 span | No cant.

BC CALC® Member Report Build 7773

Job name:

Address:

City, Province, Postal Code: BRAMPTON

Customer: Code reports:

CCMC 12472-R

File name:

40-1 EL A.B SUNKEN.mmdi

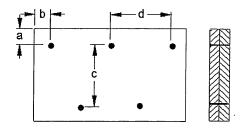
Description: 1ST FLR FRAMING\Flush Beams\B4(i2608)

Specifier.

Designer: ΑJ

Company:

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 200 61

Connectors are:

Re Nails ARDOX SPIRAL

POLINCE OF

DWG NO. TAM 1803/21 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





PASSED

July 9, 2021 13:20:12

1ST FLR FRAMING\Flush Beams\B5(i2726) (Flush Beam) Dry | 1 span | No cant.

BC CALC® Member Report Build 7773

Job name:

Address:

City, Province, Postal Code: BRAMPTON

Customer:

Code reports:

CCMC 12472-R

40-1 EL A,B SUNKEN.mmdl

File name:

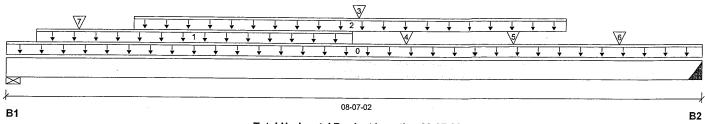
Wind

Description: 1ST FLR FRAMING\Flush Beams\B5(i2726)

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 08-07-02

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 4-3/8"	2184 / 0	1170 / 0
B2, 4"	1875 / 0	1015 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-07-02	Тор		12	· · · · · ·		00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	00-04-06	04-02-11	Тор	240	120			n\a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	01-06-10	06-10-10	Top	320	160			n\a
3	B4(i2608)	Conc. Pt. (lbs)	L	04-03-10	04-03-10	Top	138	120			n\a
4	J8(i2654)	Conc. Pt. (lbs)	L	04-10-10	04-10-10	Top	128	64			n\a
5	J8(i2655)	Conc. Pt. (lbs)	L	06-02-10	06-02-10	qoT	168	84			n\a
6	-	Conc. Pt. (lbs)	L	07-06-10	07-06-10	Top	564	283			n\a
7	J5(i2749)	Conc. Pt. (lbs)	L	00-10-10	00-10-10		402	201			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	9265 ft-lbs	35392 ft-lbs	26.2%	1	04-02-11
End Shear	3791 lbs	14464 lbs	26.2%	1	01-04-04
Total Load Deflection	L/999 (0.077")	n\a	n\a	4	04-03-10
Live Load Deflection	L/999 (0.05")	n\a	n\a	5	04-03-10
Max Defl.	0.077"	n\a	n\a	4	04-03-10
Span / Depth	8.1				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4-3/8" x 3-1/2"	4738 lbs	50.3%	25.4%	Spruce-Pine-Fir
B2	Hanger	4" x 3-1/2"	4081 lbs	n\a	23.9%	HGUS410

Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for) OU adequate capacity.

ONNOE OF ONT

ove no. Tan/80 STRUCTURAL

COMPONENT ONLY





PASSED

July 9, 2021 13:20:12

1ST FLR FRAMING\Flush Beams\B5(i2726) (Flush Beam) Dry | 1 span | No cant.

BC CALC® Member Report

Build 7773 Job name:

Address:

City, Province, Postal Code: BRAMPTON

Customer:

Code reports:

CCMC 12472-R

File name:

40-1 EL A,B SUNKEN.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B5(i2726)

Specifier:

Designer:

ΑJ

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

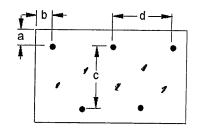
Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-01-08.

CONFORMS TO OBC 2012

AMENDED 2020

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 🚳 🛭 💘

Calculated Side Load = 905.3 lb/ft

Connectors are:

~ Nails

ARDOX SPIRAL

CUNICE OF ON THE

DWG NO. TAN 18035 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™. ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B6(i2743) (Flush Beam)

PASSED

BC CALC® Member Report

Build 7773

Dry | 1 span | No cant.

July 9, 2021 13:20:12

Job name:

Address:

Customer:

Code reports:

City, Province, Postal Code: BRAMPTON

CCMC 12472-R

File name:

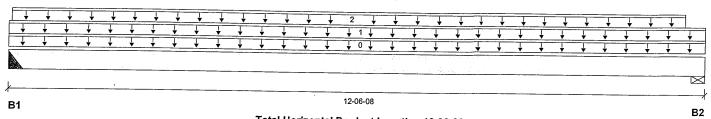
40-1 EL A,B SUNKEN.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B6(i2743)

Specifier:

Designer: AJ

Company:



Total Horizontal Product Length = 12-06-08

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead B1, 2" 152 / 0 509 / 0 B2, 4-3/8" 154 / 0 504/0

	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-06-08	Top		6			00-00-00
1	FC2 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	12-06-08	Тор	24	12	.com	والمراجد المستران فكالمنا فعستر	n\a
2	3(i1097)	Unf. Lin. (lb/ft)	L	00-00-10	12-02-02	Тор		65		£9910	, n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2136 ft-lbs	11502 ft-lbs	18.6%	0	06-02-01
End Shear	582 lbs	4701 lbs	12.4%	0	01-01-14
Total Load Deflection	L/999 (0.107")	n\a	n\a	4	06-02-01
Live Load Deflection	L/999 (0.024")	n\a	n\a	5	06-02-01
Max Defl.	0.107"	n\a	n\a	4	06-02-01
Span / Depth	12.3			•	00 02 01

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Hanger	2" x 1-3/4"	713 lbs	n\a	25.7%	HUS1.81/10
B2	Wall/Plate	4-3/8" x 1-3/4"	706 lbs	23.1%	11.6%	Spruce-Pine-Fir

Cautions

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Hanger Manufacturer: Unassigned

AMENDED 2020 Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 12-02-02.

DWG NO. TAN 180362 STRUCTURAL COMPONENT

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™. ALLJOIST®, BC RIM BOARD™. BCI®. BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B7(i2663) (Flush Beam)

PASSED

BC CALC® Member Report

Build 7773

Dry | 1 span | No cant.

July 9, 2021 13:20:12

Job name:

Address:

Customer:

Code reports:

City, Province, Postal Code: BRAMPTON

CCMC 12472-R

File name:

40-1 EL A, B SUNKEN.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B7(i2663)

Wind

ΑJ

Specifier:

Designer:

Company:

 † †	<u> </u>	<u> </u>	+	<u> </u>	<u>+</u>	+	↓	+	+	1	Ţ	Ţ	1 🗼	1	1	Ţ	1	Ţ	Ţ	Ţ	Ţ	Ţ	Ţ	Ţ	Ţ	Ţ	Ţ	=
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 							<u> </u>						22.5		42.	234.5	4,114											

Total Horizontal Product Length = 12-02-04

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow
B1, 4-3/8"	119 / 0	96 / 0	·
B2, 4-3/8"	119 / 0	96 / 0	

Loa Tag	ad Summary Description	Load Type	Def	O4 nort	P . 4		Live	Dead	Snow	Wind	Tributary
			Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
U	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-02-04	Top		6			00-00-00
1	FC2 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	12-02-04	Тор	20	10			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	824 ft-lbs	17696 ft-lbs	4.7%	1	06-01-02
End Shear	233 lbs	7232 lbs	3.2%	1	01-04-04
Total Load Deflection	L/999 (0.029")	n\a	n\a	4	06-01-02
Live Load Deflection	L/999 (0.016")	n\a	n\a	5	06-01-02
Max Defl.	0.029"	n\a	n\a	4	06-01-02
Span / Depth	11 7				· · · · ·

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4-3/8" x 1-3/4"	299 lbs	6.4%	3.2%	Spruce-Pine-Fir
B2	Wall/Plate	4-3/8" x 1-3/4"	299 lbs	6.4%	3.2%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086. Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

canforms to obc 2012

Calculations assume unbraced length of Top: 00-00-00, Bottom: 11-05-08.

AMENDED 2020



148 HD . FAM/1803/ STRUCTURAL COMPONENT ONLY

Disclosure

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BC CALC®, BC FRAMER® , AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®.





Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B8(i2733) (Flush Beam)

PASSED

BC CALC® Member Report

Build 7773

Dry | 1 span | No cant.

July 9, 2021 13:20:12

Job name:

Address:

Customer:

Code reports:

City, Province, Postal Code: BRAMPTON

CCMC 12472-R

File name:

40-1 EL A,B SUNKEN.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B8(i2733)

Specifier:

Designer: AJ

Company:

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																												Ā	(*Reach)		
B1								-								01-14												· · · · · · · · · · · · · · · · · · ·			—∤ B2

Total Horizontal Product Length = 18-01-14

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead B1, 5-1/2" 581 / 0 431 / 0 B2, 4" 1940 / 0 1146 / 0

	ad Summary Description	Load Type	Ref.	Stort	F1		Live	Dead	Snow	Wind	Tributary
_	Self-Weight		Kei.	Start	End	Loc.	1.00	0.65	1.00	1.15	
U		Unf. Lin. (lb/ft)	L	00-00-00	18-01-14	Top		12			00-00-00
1	FC2 Floor Decking (Plan	Unf. Lin. (lb/ft)	1	00-00-00	18-01-14	Ton	22				00-00-00
	View Fill)	(12/14)	-	00-00-00	10-01-14	тор	33	17			n\a
2	B5(i2726)	Conc. Pt. (lbs)	1	16-02-12	16-02-12	Т	4045	000			
2	E47(3004)	` '					1815	982			n\a
3	E17(i2204)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	Тор	108	78			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	7344 ft-lbs	35392 ft-lbs	20.7%	1	13-05-08
End Shear	4230 lbs	14464 lbs	29.2%	1	16-10-00
Total Load Deflection	L/726 (0.289")	n\a	33.1%	4	09-09-02
Live Load Deflection	L/1215 (0.173")	n\a	29.6%	5	09-11-15
Max Defl.	0.289"	n\a	n\a	4	09-09-02
Span / Depth	17.7			7	03-03-02

<u>Bearin</u>	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	5-1/2" x 3-1/2"	1410 lbs	11.9%	6.0%	Spruce-Pine-Fir
B2	Hanger	4" x 3-1/2"	4342 lbs	n\a	25.4%	HGUS410

Header for the hanger HGUS410 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Hanger Manufacturer: Unassigned

CONFORMS TO OBC 2012

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 15-07-08.

WINDE OF

DWO NO. TAN 16036 STRUCTURAL COMPONENT ONLY





Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B8(i2733) (Flush Beam)

PASSED

BC CALC® Member Report

Build 7773

Dry | 1 span | No cant.

July 9, 2021 13:20:12

Job name:

Address:

City, Province, Postal Code: BRAMPTON

Customer: Code reports:

CCMC 12472-R

File name:

40-1 EL A.B SUNKEN.mmdl

Description: 1ST FLR FRAMING\Flush Beams\B8(i2733)

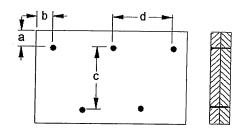
Specifier:

Designer:

ΑJ

Company:

Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8"

b minimum = 3"

d = 84

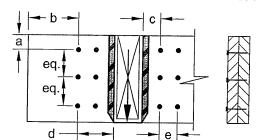
Connectors are:

1

312" ARDOX SPIRAL

Connection Diagrams: Concentrated Side Loads

Connection Tag: A Applies to load tag(s): 4



a minimum = 2"

b minimum = 4"

c minimum = 4"

d maximum = 12"

e minimum = 4" Connectors are: 16d

ARDOX SPIRAL



046 NO. TAM 1803821 STRUCTURAL COMPONENT ONLY

Disclosure

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2ND FLR FRAMING\Dropped Beams\B16 DR(i2762) (Dropped Beam)

Dry | 1 span | No cant.

July 9, 2021 13:20:12

PASSED

Build 7773

Job name: Address:

Customer:

Code reports:

City, Province, Postal Code: BRAMPTON

BC CALC® Member Report

CCMC 12472-R

File name:

40-1 EL A,B SUNKEN.mmdl

Description: 2ND FLR FRAMING\Dropped Beams\B16 DR(i2762)

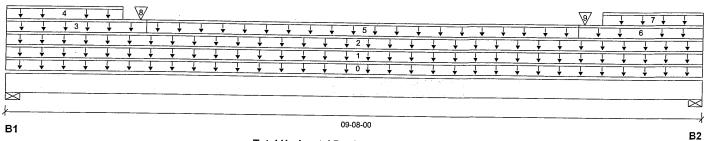
Specifier:

Designer:

ΑJ

Wind

Company:



Total Horizontal Product Length = 09-08-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead	Snow
B1, 4"	135 / 0	697 / 0	378 / 0
B2, 4"	135 / 0	694 / 0	376 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	, , , , , , , , , , , , , , , , , , , ,
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-08-00	Тор		10		1.10	00-00-00
1	J1(i2660)	Unf. Lin. (lb/ft)	L	00-00-00	09-08-00	Top	24	15			n\a
2	R1(i2718)	Unf. Lin. (lb/ft)	L	00-00-00	09-08-00	Тор	4	5			n\a
3	R1(i2718)	Unf. Lin. (lb/ft)	L	00-00-00		- 1-	,	81			n\a
4	R1(i2718)	Unf. Lin. (lb/ft)	L	00-00-00		Тор		42	78		n\a n\a
5	R1(i2718)	Unf. Lin. (lb/ft)	L	01-11-00	07-11-00	1-		51	70		n\a
6	R1(i2718)	Unf. Lin. (lb/ft)	L	07-11-00	09-08-00	•		81			n\a n\a
7	R1(i2718)	Unf. Lin. (lb/ft)	Ĺ	08-03-00	09-08-00	Тор		42	78		
8	R1(i2718)	Conc. Pt. (lbs)	Ĺ	01-10-00	01-10-00	Тор		42 191	76 261		n\a
9	R1(i2718)	Conc. Pt. (lbs)	Ĺ	08-00-00	08-00-00	Тор		189	259		n∖a n∖a
		` '				. 00		103	200		II\d

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2439 ft-lbs	16647 ft-lbs	14.6%	13	04-08-12
End Shear	1196 lbs	11571 lbs	10.3%	13	01-01-08
Total Load Deflection	L/999 (0.061")	n\a	n\a	35	04-09-14
Live Load Deflection	L/999 (0.024")	n\a	n\a	51	04-09-14
Max Defl.	0.061"	n\a	n\a	35	04-09-14
Span / Depth	11 5			3.0	

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4" x 3-1/2"	1573 lbs	8.4%	9.2%	Spruce-Pine-Fir
B2	Wall/Plate	4" x 3-1/2"	1568 lbs	8.4%	9.2%	Spruce-Pine-Fir



COMPONENT ONLY





Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Dropped Beams\B16 DR(i2762) (Dropped Beam)

PASSED

July 9, 2021 13:20:12

BC CALC® Member Report

Build 7773

Job name: Address:

City, Province, Postal Code: BRAMPTON

Customer: Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

40-1 EL A,B SUNKEN,mmdI

Description:

2ND FLR FRAMING\Dropped Beams\B16 DR(i2762)

Specifier:

Designer:

Company:

ΑJ

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

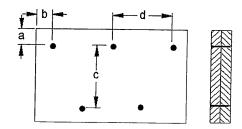
Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 09-08-00, Bottom: 09-08-00.

CONFORMS TO OBG 2012

AMENDED 2020

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 5-1/2" $d = \Re \mathcal{B}^{\prime\prime} \mathcal{B}^{\prime\prime}$

Connectors are: . . .

ARDOX SPIRAL

ONNCE OF ON DWG NO , TAM /BO342

STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS™ ALLJOIST® , BC RIM BOARD™, BCI® , BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

2ND FLR FRAMING\Dropped Beams\B9 DR(i2903) (Dropped Beam)

Dry | 1 span | No cant.

August 9, 2021 10:19:54

Build 7773 Job name:

Address:

Customer:

Code reports:

City, Province, Postal Code: BRAMPTON

BC CALC® Member Report

CCMC 12472-R

File name:

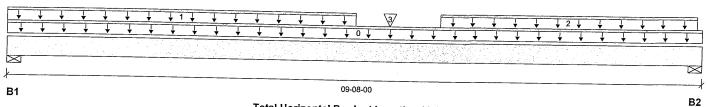
40-1 EL A,B SUNKEN.mmdl

Description: 2ND FLR FRAMING\Dropped Beams\B9 DR(i2903)

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 09-08-00

Reaction Summary (Down / Unlift) (lbs)

	(2000	JIII () (103)			
Bearing	Live	Dead	Snow	Wind	
B1, 4"	3245 / 0	1692 / 0	<u> </u>	AAIIIG	
B2, 4"	3010 / 0	1575 / 0			

	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	moduly
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-08-00			14	1.00	1.13	00.00.00
1	Smoothed Load	Unf. Lin. (lb/ft)	ı	00-00-00	04-09-08	1-	657				00-00-00
2	Smoothed Load	Unf. Lin. (lb/ft)	ī			[-	657	328			n∖a
2		` ,	L	05-11-08	09-07-02	Тор	674	337			n\a
3	•	Conc. Pt. (lbs)	L	05-03-02	05-03-02	Top	656	328			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	13716 ft-lbs	36222 ft-lbs	37.9%	1	05-01-02
End Shear	5454 lbs	17356 lbs	31.4%	1	·
Total Load Deflection	L/565 (0.194")	n\a		1	01-01-08
Live Load Deflection	` ,		42.5%	4	04-10-03
	L/861 (0.127")	n\a	41.8%	5	04-10-03
Max Defl.	0.194"	n\a	n\a	4	04-10-03
Span / Depth	11.5				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4" x 5-1/4"	6982 lbs	24.9%	27.3%	Spruce-Pine-Fir
B2	Wall/Plate	4" x 5-1/4"	6483 lbs	23.1%	25.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBE 2012

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-10-12, Bottom: 09-08-00.

144 NO. TAM/BOYO. 21 STRUCTURAL

COMPONENT ONLY





Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Dropped Beams\B9 DR(i2903) (Dropped Beam)

PASSED

BC CALC® Member Report

Build 7773 Job name:

Address:

Code reports:

City, Province, Postal Code: BRAMPTON Customer:

CCMC 12472-R

Dry | 1 span | No cant.

August 9, 2021 10:19:54

File name:

40-1 EL A,B SUNKEN.mmdl

Description: 2ND FLR FRAMING\Dropped Beams\B9 DR(i2903)

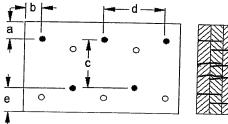
Specifier:

Designer:

Company:

ΑJ

Connection Diagram: Full Length of Member



4 rows

a minimum = 2" b minimum = 3"

e minimum = 2"

Nailing applies to both sides of the member Connectors are:

ARDÒX SPIRAL

POWNE OF

DWG NO. TAN 180421 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®. BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





2ND FLR FRAMING\Flush Beams\B11(i2522) (Flush Beam)

Dry | 1 span | No cant.

July 9, 2021 13:20:12

PASSED

Build 7773 Job name:

Address:

City, Province, Postal Code: BRAMPTON

BC CALC® Member Report

File name:

Description: Specifier:

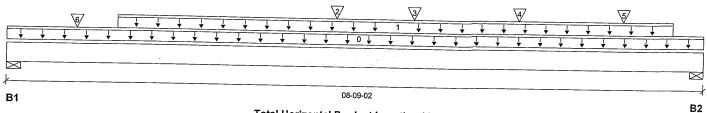
Customer: Code reports:

CCMC 12472-R

2ND FLR FRAMING\Flush Beams\B11(i2522) AJ

40-1 EL A,B SUNKEN.mmdl

Designer: Company:



Total Horizontal Product Length = 08-09-02

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 2-3/4"	1466 / 0	788 / 0
B2, 5-3/8"	1687 / 0	899 / 0

	ad Summary Description	Load Type	Ref.	Stort	. .		Live	Dead	Snow	Wind	Tributary
0	Self-Weight		Kei.	Start	End	Loc.	1.00	0.65	1.00	1.15	
4	•	Unf. Lin. (lb/ft)	L	00-00-00	08-09-02	Top		12			00-00-00
7	Smoothed Load	Unf. Lin. (lb/ft)	L	01-04-06	08-04-06	Top	363	181			
2	B13(i1104)	Conc. Pt. (lbs)	1	04-01-02			505				n\a
3	J5(i1171)	` '	-					24			n∖a
-	` '	Conc. Pt. (lbs)	L	05-00-12	05-00-12	Top	. 74	37			n\a
4	J5(i1173)	Conc. Pt. (lbs)	L	06-04-12	06-04-12	Top	87	43			
5	J5(i1172)	Conc. Pt. (lbs)	_			1-		43			n\a
6	,	` '	L	07-08-12	07-08-12	Тор	87	43			n\a
6	J1(i1697)	Conc. Pt. (lbs)	L	00-10-06	00-10-06	Top	320	160			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	7272 ft-lbs	35392 ft-lbs	20.5%	1	04-10-06
End Shear	3137 lbs	14464 lbs	21.7%	1	07-03-14
Total Load Deflection	L/999 (0.064")	n\a	n\a	4	04-03-14
Live Load Deflection	L/999 (0.042")	n\a	n\a	5	
Max Defl.	0.064"	n\a		-	04-03-07
Span / Depth	8.3	ma	n\a	4	04-03-07

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material `
B1	Wall/Plate	2-3/4" x 3-1/2"	3184 lbs	53.8%	27.1%	Spruce-Pine-Fir
B2	Wall/Plate	5-3/8" x 3-1/2"	3654 lbs	31.6%	15.9%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

danforms to obe 2012

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-09-08.

AMENDED 2020

DWS NO. TAM/804/-21 STRUCTURAL COMPONENT ONLY





2ND FLR FRAMING\Flush Beams\B11(i2522) (Flush Beam)

Dry | 1 span | No cant.

July 9, 2021 13:20:12

PASSED

Build 7773 Job name:

Address:

Customer:

Code reports:

City, Province, Postal Code: BRAMPTON

BC CALC® Member Report

CCMC 12472-R

File name:

40-1 EL A,B SUNKEN.mmdl

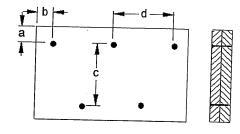
Description: 2ND FLR FRAMING\Flush Beams\B11(i2522)

Specifier:

Designer: AJ

Company:

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 8 6 4

Calculated Side Load = 779.3 lb/ft Connectors are: 16d / Nails

ARDOX SPIRAL

ONNOE OF ONE

OWE NO. TAMLEDY STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® . AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B12(i1103) (Flush Beam)

Dry | 1 span | No cant.

PASSED

July 9, 2021 13:20:12

BC CALC® Member Report

Build 7773 Job name:

Address:

City, Province, Postal Code: BRAMPTON

Customer:

Code reports:

Load Summany

CCMC 12472-R

File name:

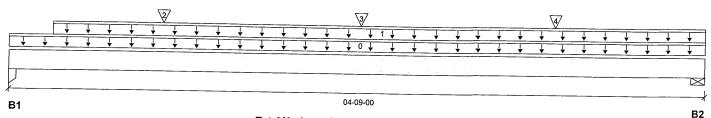
40-1 EL A,B SUNKEN.mmdl

2ND FLR FRAMING\Flush Beams\B12(i1103)

Description: Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 04-09-00

	ilinary (Down / U	piiπ) (ibs)			
Bearing	Live	Dead	Snow	Wind	
B1, 3-1/2"	597 / 0	313 / 0		VVIIIG	
B2, 5-1/2"	726 / 0	378 / 0		·	

	au Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	,
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-09-00			6	1.00	1.13	00-00-00
1	STAIR	Unf. Lin. (lb/ft)	L	00-03-08	04-09-00	Top	240	120			n\a
2	J5(i1171)	Conc. Pt. (lbs)	L	01-00-08		-	77	39			
3	J5(i1173)	Conc. Pt. (lbs)	L	02-04-08	02-04-08		87	43		المعاشدة والتوارد	n\a
4	J5(i1172)	Conc. Pt. (lbs)	L	03-08-08	03-08-08	Top	85	43	Æ.	LAVES!	n\a
		` '	_	77 77 77	00 00 00	ιορ	0.5	43			

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1414 ft-lbs	17696 ft-lbs	8.0%	1	02-04-08
End Shear	732 lbs	7232 lbs	10.1%	1	01-03-06
Total Load Deflection	L/999 (0.006")	n\a	n\a	4	02-03-11
Live Load Deflection	L/999 (0.004")	n\a	n\a	5	02-03-11
Max Defl.	0.006"	n\a	n\a	4	02-03-11
Span / Depth	4.2		1110	4	02-03-11

	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column	3-1/2" x 1-3/4"	1287 lbs	25.9%	17.2%	Unspecified
B2	Wall/Plate	5-1/2" x 1-3/4"	1561 lbs	26.4%	13.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO 0BC 2012

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-01-08.

AMENDED 2020



Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS $^{\text{TM}}$ ALLJOIST® , BC RIM BOARD™, BCI® , BOISE GLULAM™, BC FloorValue®. VERSA-LAM®, VERSA-RIM PLUS® ,





Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 2ND FLR FRAMING\Flush Beams\B13(i1104) (Flush Beam)

PASSED

July 9, 2021 13:20:12

BC CALC® Member Report

Build 7773 Job name:

Address:

City, Province, Postal Code: BRAMPTON

Customer:

Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

File name:

40-1 EL A,B SUNKEN.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B13(i1104)

Wind

Specifier:

Designer: ΑJ

Company:

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									·		-						·									

Total Horizontal Product Length = 02-11-10

Snow

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead
B1, 2"	31 / 0	25 / 0
B2, 1-3/4"	31 / 0	24/0

Loa	nd Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	inbutary
0	Self-Weight	Unf. Lin. (lb/ft)	L,	00-00-00	02-11-10	Тор		6	1.00	1.13	00-00-00
1	FC3 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	02-11-10	Тор	21	10			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	50 ft-lbs	17696 ft-lbs	0.3%	1	01-05-15
End Shear	18 lbs	7232 lbs	0.2%	1	01-03-13
Total Load Deflection	L/999 (0")	n\a	n\a	4	01-01-14
Live Load Deflection	L/999 (0")	n\a	n\a	5	01-05-15
Max Defl.	0"	n\a	n\a		
Span / Depth	2.8	ma	IIIa	4	01-05-15

_	Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
1	B1	Hanger	2" x 1-3/4"	77 11			
		riangei	2 X 1-3/4	77 lbs	n\a	1.8%	HUS1.81/10
- 1	B2	Column	1-3/4" x 1-3/4"	76 lbs	3.1%	2.0%	I Inama side al
				10 103	J. 1 /0	2.070	Unspecified

046 NO. TAN 1804321 STRUCTURAL COMPONENT ONLY

Cautions

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 02-11-10.

Disclosure

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BC CALC® Member Report



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

2ND FLR FRAMING\Flush Beams\B14(i1106) (Flush Beam)

Dry | 1 span | No cant.

July 9, 2021 13:20:12

PASSED

Build 7773 Job name:

Address:

Customer:

Code reports:

City, Province, Postal Code: BRAMPTON

CCMC 12472-R

File name:

40-1 EL A,B SUNKEN.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B14(i1106)

Specifier:

Designer: AJ

Company:

	F					-	 																										
_	-	*	*	<u> </u>	<u> </u>	<u>+</u>	 <u> </u>	+	<u>+</u>	<u></u>			+	+	,	,	¥ 2	+	Ţ	1	Ţ	Ţ	I	Ţ	I	I	I	1					
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																	06-0	2-06											 				_
																																	ı

Total Horizontal Product Length = 06-02-06

Reaction Summary (Down / Uplift) (lbs)

Bearing Dead Snow B1, 4" 46/0 225/0 B2, 4-3/8" 47/0 226 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00		Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	06-02-06		1.00		1.00	1.15	
1	FC3 Floor Decking (Plan		_			1-		12			00-00-00
	View Fill)	OTIL LITE (ID/IL)	L	00-00-00	06-02-06	Top	15	8			n\a
2	WALL	Unf. Lin. (lb/ft)	L	00-04-00	05-10-00	Top		60			
						. 00		00			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	440 ft-lbs	23005 ft-lbs	1.9%	0	03-01-00
End Shear	196 lbs	9401 lbs			
		9401108	2.1%	0	01-03-14
Total Load Deflection	L/999 (0.002")	n\a	n\a	4	03-01-00
Live Load Deflection	L/999 (0")	n\a	n\a	5	03-01-00
Max Defl.	0.002"	n\a	n\a	4	03-01-00
Span / Depth	5.7		ma	4	03-01-00

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4" x 3-1/2"	315 lbs	5.6%	2.8%	Spruce-Pine-Fir
B2	Wall/Plate	4-3/8" x 3-1/2"	316 lbs	5.2%	2.6%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CAMPARMS TO OBC 2012

Calculations assume unbraced length of Top: 00-00-00, Bottom: 05-10-00.

AMENDED 2020

due no. Tan 1804421

STRUCTURAL COMPONENT ONLY





2ND FLR FRAMING\Flush Beams\B14(i1106) (Flush Beam)

Dry | 1 span | No cant.

July 9, 2021 13:20:12

PASSED

BC CALC® Member Report Build 7773

Job name:

Address:
City, Province, Postal Code: BRAMPTON

City, Province, Postal Code: Bf Customer:

Code reports:

CCMC 12472-R

File name:

40-1 EL A,B SUNKEN.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B14(i1106)

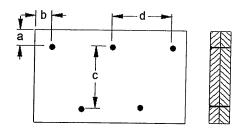
ΑJ

Specifier:

Designer:

Company:

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8'' d = ()

Connectors are: :

1 mater € ... Nail

3½" ARDÒX SPIRAL

S. KATSOULADAS S. KATSOULADAS PORCE OF COMPRESSIONAL PROPERTY OF COM

OWG NO. TAM 1604 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorVallue®, VERSA-LAM®, VERSA-RIM PLUS®,



BC CALC® Member Report



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

2ND FLR FRAMING\Flush Beams\B15(i1109) (Flush Beam)

Dry | 1 span | No cant.

July 9, 2021 13:20:12

PASSED

Build 7773 Job name:

Address:

Customer:

Code reports:

City, Province, Postal Code: BRAMPTON

CCMC 12472-R

File name:

40-1 EL A,B SUNKEN.mmdl Description: 2ND FLR FRAMING\Flush Beams\B15(i1109)

Specifier:

Designer: AJ

Company:

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Total Horizontal Product Length = 02-04-02

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Snow B1, 4-1/8' 19/0 23/0 B2, 5-1/4" 16/0 23/0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	,
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-04-02	Тор		12			00-00-00
1	FC3 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	02-01-08	Тор	11	6			n\a
2	FC3 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	01-10-14	Тор	6	3			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	18 ft-lbs	35392 ft-lbs	n\a	1	01-01-08
End Shear	11 lbs	14464 lbs	n\a	1	01-04-00
Span / Depth	1.7			•	

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam	4-1/8" x 3-1/2"	58 lbs	0.7%	0.3%	Unspecified
B2	Beam	5-1/4" x 3-1/2"	52 lbs	0.5%	0.2%	Unspecified

Notes

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

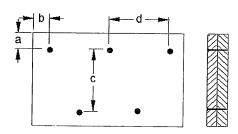
Importance Factor: Normal Part code: Part 9

COMPORMS TO OBG 2012

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-06-12.

AMENDED 2020

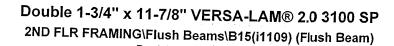
Connection Diagram: Full Length of Member





STRUCTURAL COMPONENT ONLY





Dry | 1 span | No cant.

PASSED

July 9, 2021 13:20:12

BC CALC® Member Report

City, Province, Postal Code: BRAMPTON

Build 7773 Job name:

Address:

Customer:

Code reports:

File name:

40-1 EL A,B SUNKEN.mmdl

Description:

2ND FLR FRAMING\Flush Beams\B15(i1109)

Specifier:

Designer: Company:

ΑJ

Connection Diagram: Full Length of Member

a minimum = 2"

c = 7-7/8"

b minimum = 3"

d = 208 9

Connectors are: .

√ Nails

CCMC 12472-R

312" ARDOX SPIRAL

POPPICE OF ON DWG NO. TAN 160 4521 STRUCTURAL

COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Maximum Floor Spans - S2.1

Design Criteria

Simple span

Loads: Live load = 40 psf and dead load = 15 psf Deflection limits:

L/480 under live load and L/240 under total load

Sheathing: 5/8 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			В	are			1/2 in. gyp	osum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-3"	13'-10"	_	15'-7"	14'-9"	14'-3"	-
9-1/2"	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-
9-1/2	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	-
	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	-
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	-
11-7/8"	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	-
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	-
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	-
	NI-40x	20'-1"	18'-8"	17'-10"	-	20'-10"	19'-4"	18'-6"	-
14"	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	-
14	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	-
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	-
	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	-
16"	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	-
	NI-90	24'-5"	22'-6"	21'-6"	-	25'-1"	23'-2"	22'-2"	_

		Mi	d-span blocking	with 1x4 inch s	trap	Mid-sp	an blocking an	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	-	16'-8"	15'-3"	14'-5"	-
9-1/2"	NI-40x	17'-11"	17'-0"	16'-1"	-	18'-5"	17'-1"	16'-1"	-
9-1/2	NI-60	18'-2"	17'-1"	16'-4"	-	18'-8"	17'-4"	16'-4"	-
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	-
	NI-20	19'-7"	18'-2"	17'-3"	-	19'-11"	18'-3"	17'-3"	-
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-2"	-
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-
	NI-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-
	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	21'-8"	-
14"	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-
14	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	-
	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-
16"	NI-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-
	NI-90	28'-8"	26'-6"	25'-3"	-	29'-3"	27'-2"	25'-11"	-

- 1. The tabulated clear spans are based on CSA 086-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - S4.1

Design Criteria

Spans: Simple span

Live load = 40 psf and dead load = 15 psf

Deflection limits: L/480 under live load and L/240 under total load

Sheathing: 3/4 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			В	are			1/2 in. gy	sum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
0.4/0"	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-2"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
14"	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10'
14	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
16"	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

		Mi	d-span blocking	with 1x4 inch	strap	Mid-sp	an blocking an	d 1/2 in. gypsur	n ceiling
Joist depth	Joist series		On cent	re spacing			On centr	e spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
9-1/2"	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"
11-7/8"	NI-60	22'-1"	20'-7"	19'-8"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
	N1-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-0"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-7"
	NI-40x	24'-5"	22'-9"	21'-9"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
14"	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10
14	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	24'-9"	23'-1"
16"	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

- 1. The tabulated clear spans are based on CSA 086-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - S6.1

Design Criteria

Spans: Simple span

Loads: Live load = 40 psf and dead load = 15 psf
Deflection limits: L/480 under live load and L/240 under total load
Sheathing: 5/8 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

			B	are			1/2 in. gyr	sum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	14'-11"	14'-1"	13'-7"	-	15'-4"	14'-6"	14'-1"	-
9-1/2"	NI-40x	15'-11"	15'-0"	14'-6"	-	16'-4"	15'-5"	14'-11"	-
5-1/2	NI-60	16'-1"	15'-2"	14'-8"	-	16'-6"	15'-7"	15'-1"	-
	NI-80	17'-1"	16'-1"	15'-6"	-	17'-5"	16'-5"	15'-10"	_
	NI-20	16'-9"	15'-10"	15'-4"	-	17'-4"	16'-4"	15'-10"	-
	NI-40x	17'-10"	16'-10"	16'-3"	-	18'-6"	17'-4"	16'-9"	-
11-7/8"	NI-60	18'-1"	17'-0"	16'-5"	-	18'-9"	17'-6"	16'-11"	-
	NI-80	19'-6"	18'-0"	17'-4"	-	20'-1"	18'-7"	17'-9"	-
	NI-90	19'-11"	18'-4"	17'-8"	-	20'-5"	18'-11"	18'-1"	-
	NI-40x	19'-10"	18'-4"	17'-8"	-	20'-6"	19'-1"	18'-3"	-
14"	NI-60	20'-2"	18'-8"	17'-11"	-	20'-10"	19'-4"	18'-6"	-
14	NI-80	21'-8"	20'-0"	19'-1"	-	22'-4"	20'-8"	19'-9"	_
	NI-90	22'-1"	20'-5"	19'-6"	-	22'-9"	21'-0"	20'-1"	-
	NI-60	22'-0"	20'-4"	19'-6"	-	22'-9"	21'-1"	20'-2"	-
16"	NI-80	23'-7"	21'-10"	20'-10"	-	24'-4"	22'-6"	21'-6"	_
	NI-90	24'-1"	22'-2"	21'-2"	-	24'-9"	22'-11"	21'-10"	_

		Mi	d-span blocking	with 1x4 inch s	trap	Mid-sp	an blocking and	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On centr	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-6"	15'-1"	14'-3"	-	16'-6"	15'-1"	14'-3"	-
9-1/2"	NI-40x	17'-9"	16'-10"	15'-11"	-	18'-2"	16'-11"	15'-11"	_
9-1/2	NI-60	17'-11"	16'-11"	16'-2"	-	18'-5"	17'-2"	16'-2"	-
	NI-80	19'-3"	17'-10"	17'-3"	-	19'-8"	18'-3"	17'-7"	-
	NI-20	19'-4"	18'-0"	17'-1"	-	19'-9"	18'-1"	17'-1"	-
	NI-40x	20'-10"	19'-4"	18'-6"	-	21'-5"	19'-11"	19'-0"	-
11-7/8"	NI-60	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-3"	_
	NI-80	22'-6"	20'-10"	19'-11"	-	23'-1"	21'-5"	20'-5"	-
	NI-90	23'-0"	21'-3"	20'-4"	-	23'-6"	21'-10"	20'-10"	-
	NI-40x	23'-5"	21'-8"	20'-9"	-	24'-0"	22'-5"	21'-5"	-
14"	NI-60	23'-9"	22'-0"	21'-0"	-	24'-5"	22'-8"	21'-8"	-
14	NI-80	25'-4"	23'-6"	22'-5"	-	25'-11"	24'-1"	23'-0"	-
	NI-90	25'-10"	23'-11"	22'-9"	-	26'-5"	24'-6"	23'-4"	-
	NI-60	26'-2"	24'-3"	23'-2"	-	26'-11"	25'-0"	23'-11"	-
16"	NI-80	27'-11"	25'-10"	24'-7"	-	28'-7"	26'-6"	25'-3"	-
	NI-90	28'-5"	26'-3"	25'-0"	-	29'-0"	26'-11"	25'-8"	_

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - S7.1

Design Criteria

Simple span

Loads:

Live load = 40 psf and dead load = 15 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

3/4 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

			Е	Bare			1/2 in. gy	osum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
9-1/2"	NI-40x	16'-11"	15'-11"	15'-4"	14'-9"	17'-4"	16'-4"	15'-9"	15'-1"
3-1/2	NI-60	17'-1"	16'-1"	15'-6"	14'-10"	17'-6"	16'-6"	15'-11"	15'-3"
	NI-80	18'-1"	17'-0"	16'-4"	15'-8"	18'-7"	17'-4"	16'-8"	16'-0"
	NI-20	17'-10"	16'-10"	16'-2"	15'-7"	18'-5"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-3"	17'-10"	17'-2"	16'-6"	19'-10"	18'-5"	17'-8"	16'-11'
11-7/8"	NI-60	19'-6"	18'-1"	17'-4"	16'-8"	20'-1"	18'-8"	17'-10"	17'-1"
	NI-80	20'-11"	19'-4"	18'-5"	17'-7"	21'-5"	19'-10"	18'-11"	17'-11'
	NI-90	21'-4"	19'-9"	18'-9"	17'-10"	21'-10"	20'-3"	19'-3"	18'-3"
	NI-40x	21'-4"	19'-9"	18'-10"	17'-11"	22'-0"	20'-5"	19'-6"	18'-6"
14"	NI-60	21'-8"	20'-1"	19'-2"	18'-2"	22'-4"	20'-9"	19'-9"	18'-9"
14	NI-80	23'-3"	21'-6"	20'-5"	19'-4"	23'-10"	22'-1"	21'-0"	19'-11'
	NI-90	23'-9"	21'-11"	20'-10"	19'-8"	24'-3"	22'-6"	21'-5"	20'-3"
	NI-60	23'-7"	21'-10"	20'-10"	19'-9"	24'-4"	22'-7"	21'-7"	20'-5"
16"	NI-80	25'-4"	23'-5"	22'-3"	21'-1"	26'-0"	24'-1"	22'-11"	21'-8"
	NI-90	25'-10"	23'-10"	22'-8"	21'-5"	26'-5"	24'-6"	23'-4"	22'-0"

		Mi	d-span blocking	with 1x4 inch	strap	Mid-sr	oan blocking an	d 1/2 in. gypsu	m ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
9-1/2"	NI-40x	18'-7"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
J-1/2	NI-60	18'-10"	17'-6"	16'-6"	15'-5"	19'-1"	17'-6"	16'-6"	15'-5"
	NI-80	20'-2"	18'-9"	17'-11"	16'-10"	20'-7"	19'-2"	18'-2"	16'-10'
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-9"	20'-3"	19'-4"	17'-8"	22'-4"	20'-5"	19'-4"	17'-8"
11-7/8"	NI-60	22'-0"	20'-6"	19'-7"	18'-4"	22'-7"	20'-10"	19'-8"	18'-4"
	NI-80	23'-6"	21'-10"	20'-10"	19'-9"	24'-0"	22'-5"	21'-4"	20'-0"
	NI-90	24'-0"	22'-4"	21'-3"	20'-1"	24'-6"	22'-10"	21'-9"	20'-7"
	NI-40x	24'-4"	22'-8"	21'-8"	19'-5"	25'-0"	23'-2"	21'-9"	19'-5"
14"	NI-60	24'-9"	23'-0"	22'-0"	20'-9"	25'-5"	23'-8"	22'-4"	20'-10'
14	NI-80	26'-5"	24'-6"	23'-4"	22'-1"	27'-0"	25'-2"	24'-0"	22'-8"
	NI-90	26'-11"	25'-0"	23'-10"	22'-6"	27'-5"	25'-7"	24'-5"	23'-1"
	NI-60	27'-2"	25'-4"	24'-2"	22'-10"	27'-11"	26'-1"	24'-9"	23'-1"
16"	NI-80	29'-0"	26'-11"	25'-8"	24'-3"	29'-7"	27'-7"	26'-4"	24'-11'
	NI-90	29'-6"	27'-5"	26'-1"	24'-8"	30'-1"	28'-1"	26'-9"	25'-4"

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.



Maximum Floor Spans - M2.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 20 psf

Deflection limits: Sheathing: L/480 under live load and L/240 under total load

Onoutining.

5/8 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			В	are			1/2 in. gy;	sum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-3"	13'-10"	-	15'-7"	14'-9"	14'-3"	_
9-1/2"	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	_
9-1/2	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	-
	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	-
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	-
11-7/8"	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	_
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	_
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	-
	NI-40x	20'-1"	18'-8"	17'-10"	-	20'-10"	19'-4"	18'-6"	-
14"	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	-
14	N1-80	21'-11"	20'-3"	19'-4"	_	22'-7"	20'-11"	20'-0"	-
	N1-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	-
	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	-
16"	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	_
	NI-90	24'-5"	22'-6"	21'-6"	_	25'-1"	23'-2"	22'-2"	_

		Mi	d-span blocking	with 1x4 inch s	trap	Mid-sp	an blocking an	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	-	16'-8"	15'-3"	14'-5"	_
9-1/2"	NI-40x	17'-11"	17'-0"	16'-1"	-	18'-5"	17'-1"	16'-1"	-
9-1/2	NI-60	18'-2"	17'-1"	16'-4"	-	18'-8"	17'-4"	16'-4"	-
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	-
	NI-20	19'-7"	18'-2"	17'-3"	-	19'-11"	18'-3"	17'-3"	-
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-0"	-
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	_
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-
	NI-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-
	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	20'-11"	-
14"	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	_
14	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	_
	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-
16"	N1-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-
	NI-90	28'-8"	26'-6"	25'-3"	-	29'-3"	27'-2"	25'-11"	_

- 1. The tabulated clear spans are based on CSA 086-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when L-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - M4.1

Design Criteria

Spans: Simple span

Loads: Live load = 40 psf and dead load = 20 psf Deflection limits:

L/480 under live load and L/240 under total load

Sheathing: 3/4 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			В	are			1/2 in. gyj	osum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
9-1/2"	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	14'-11
3-1/2	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	N1-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
14"	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10'
17	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
16"	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10'
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

		Mi	d-span blocking	with 1x4 inch	strap	Mid-sp	an blocking an	d 1/2 in. gypsur	m ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
9-1/2"	NI-40x	18'-8"	17'-2"	16'-3"	14'-11"	18'-10"	17'-2"	16'-3"	14'-11"
3-1/2	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
	NI-20	20'-1"	18'-5"	17'-5"	16'-1"	20'-1"	18'-5"	17'-5"	16'-1"
	NI-40x	21'-10"	20'-4"	19'-0"	17'-0"	22'-5"	20'-6"	19'-0"	17'-0"
11-7/8"	NI-60	22'-1"	20'-7"	19'-8"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-0"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-7"
	NI-40x	24'-5"	22'-9"	20'-11"	18'-8"	25'-1"	22'-11"	20'-11"	18'-8"
14"	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
14	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	24'-9"	23'-1"
16"	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - M6.1

Design Criteria

Spans:

Simple span

Loads: Deflection limits: Sheathing:

Live load = 40 psf and dead load = 20 psf

L/480 under live load and L/240 under total load 5/8 in nailed-glued Canadian softwood plywood

Maximum Floor Spans

			E	Bare			1/2 in. gy	psum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	14'-11"	14'-1"	13'-7"	-	15'-4"	14'-6"	14'-1"	_
9-1/2"	NI-40x	15'-11"	15'-0"	14'-6"	-	16'-4"	15'-5"	14'-11"	_
0 1/2	NI-60	16'-1"	15'-2"	14'-8"	-	16'-6"	15'-7"	15'-1"	_
	NI-80	17'-1"	16'-1"	15'-6"	-	17'-5"	16'-5"	15'-10"	_
	NI-20	16'-9"	15'-10"	15'-4"	-	17'-4"	16'-4"	15'-10"	
	NI-40x	17'-10"	16'-10"	16'-3"	_	18'-6"	17'-4"	16'-9"	_
11-7/8"	NI-60	18'-1"	17'-0"	16'-5"	-	18'-9"	17'-6"	16'-11"	_
	NI-80	19'-6"	18'-0"	17'-4"	-	20'-1"	18'-7"	17'-9"	_
	NI-90	19'-11"	18'-4"	17'-8"	-	20'-5"	18'-11"	18'-1"	_
	NI-40x	19'-10"	18'-4"	17'-8"	-	20'-6"	19'-1"	18'-3"	
14"	NI-60	20'-2"	18'-8"	17'-11"	-	20'-10"	19'-4"	18'-6"	_
• • • • • • • • • • • • • • • • • • • •	NI-80	21'-8"	20'-0"	19'-1"	-	22'-4"	20'-8"	19'-9"	_
	NI-90	22'-1"	20'-5"	19'-6"	-	22'-9"	21'-0"	20'-1"	_
	NI-60	22'-0"	20'-4"	19'-6"	-	22'-9"	21'-1"	20'-2"	-
16"	NI-80	23'-7"	21'-10"	20'-10"	-	24'-4"	22'-6"	21'-6"	_
	NI-90	24'-1"	22'-2"	21'-2"	-	24'-9"	22'-11"	21'-10"	

		Mi	d-span blocking	with 1x4 inch s	trap	Mid-s	oan blocking an	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing				re spacing	ŭ
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-6"	15'-1"	14'-3"	-	16'-6"	15'-1"	14'-3"	
9-1/2"	NI-40x	17'-9"	16'-10"	15'-11"	-	18'-2"	16'-11"	15'-11"	_
5 II.E	NI-60	17'-11"	16'-11"	16'-2"	-	18'-5"	17'-2"	16'-2"	_
	NI-80	19'-3"	17'-10"	17'-3"	-	19'-8"	18'-3"	17'-7"	_
	NI-20	19'-4"	18'-0"	17'-1"	-	19'-9"	18'-1"	17'-1"	
	NI-40x	20'-10"	19'-4"	18'-6"	-	21'-5"	19'-11"	19'-0"	_
11-7/8"	NI-60	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-3"	_
	NI-80	22'-6"	20'-10"	19'-11"	-	23'-1"	21'-5"	20'-5"	_
	NI-90	23'-0"	21'-3"	20'-4"	-	23'-6"	21'-10"	20'-10"	_
	NI-40x	23'-5"	21'-8"	20'-9"	-	24'-0"	22'-5"	20'-11"	
14"	NI-60	23'-9"	22'-0"	21'-0"	_	24'-5"	22'-8"	21'-8"	_
	NI-80	25'-4"	23'-6"	22'-5"	-	25'-11"	24'-1"	23'-0"	_
	NI-90	25'-10"	23'-11"	22'-9"	-	26'-5"	24'-6"	23'-4"	_
	NI-60	26'-2"	24'-3"	23'-2"	_	26'-11"	25'-0"	23'-11"	
16"	NI-80	27'-11"	25'-10"	24'-7"	_	28'-7"	26'-6"	25'-3"	_
_	NI-90	28'-5"	26'-3"	25'-0"	_	29'-0"	26'-11"	25'-8"	_

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - M7.1

Design Criteria

Spans: Simple span

 Loads:
 Live load = 40 psf and dead load = 20 psf

 Deflection limits:
 L/480 under live load and L/240 under total load

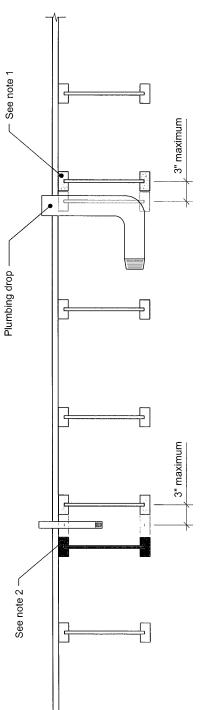
 Sheathing:
 3/4 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing			
Joist depth									
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	16'-11"	15'-11"	15'-4"	14'-9"	17'-4"	16'-4"	15'-9"	14'-11'
	NI-60	17'-1"	16'-1"	15'-6"	14'-10"	17'-6"	16'-6"	15'-11"	15'-3"
	NI-80	18'-1"	17'-0"	16'-4"	15'-8"	18'-7"	17'-4"	16'-8"	16'-0"
11-7/8"	NI-20	17'-10"	16'-10"	16'-2"	15'-7"	18'-5"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-3"	17'-10"	17'-2"	16'-6"	19'-10"	18'-5"	17'-8"	16'-11'
	NI-60	19'-6"	18'-1"	17'-4"	16'-8"	20'-1"	18'-8"	17'-10"	17'-1"
	NI-80	20'-11"	19'-4"	18'-5"	17'-7"	21'-5"	19'-10"	18'-11"	17'-11'
	NI-90	21'-4"	19'-9"	18'-9"	17'-10"	21'-10"	20'-3"	19'-3"	18'-3"
14"	NI-40x	21'-4"	19'-9"	18'-10"	17'-1.1"	22'-0"	20'-5"	19'-6"	18'-6"
	NI-60	21'-8"	20'-1"	19'-2"	18'-2"	22'-4"	20'-9"	19'-9"	18'-9"
	NI-80	23'-3"	21'-6"	20'-5"	19'-4"	23'-10"	22'-1"	21'-0"	19'-11'
	NI-90	23'-9"	21'-11"	20'-10"	19'-8"	24'-3"	22'-6"	21'-5"	20'-3"
16"	NI-60	23'-7"	21'-10"	20'-10"	19'-9"	24'-4"	22'-7"	21'-7"	20'-5"
	NI-80	25'-4"	23'-5"	22'-3"	21'-1"	26'-0"	24'-1"	22'-11"	21'-8"
	NI-90	25'-10"	23'-10"	22'-8"	21'-5"	26'-5"	24'-6"	23'-4"	22'-0"

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing			
		9-1/2"	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"
NI-40x	18'-7"		17'-2"	16'-3"	14'-11"	18'-10"	17'-2"	16'-3"	14'-11'
NI-60	18'-10"		17'-6"	16'-6"	15'-5"	19'-1"	17'-6"	16'-6"	15'-5"
NI-80	20'-2"		18'-9"	17'-11"	16'-10"	20'-7"	19'-2"	18'-2"	16'-10'
11-7/8"	NI-20	20'-1"	18'-5"	17'-5"	16'-1"	20'-1"	18'-5"	17'-5"	16'-1"
	NI-40x	21'-9"	20'-3"	19'-0"	17'-0"	22'-4"	20'-5"	19'-0"	17'-0"
	NI-60	22'-0"	20'-6"	19'-7"	18'-4"	22'-7"	20'-10"	19'-8"	18'-4"
	NI-80	23'-6"	21'-10"	20'-10"	19'-9"	24'-0"	22'-5"	21'-4"	20'-0"
	NI-90	24'-0"	22'-4"	21'-3"	20'-1"	24'-6"	22'-10"	21'-9"	20'-7"
14"	NI-40x	24'-4"	22'-8"	20'-11"	18'-8"	25'-0"	22'-11"	20'-11"	18'-8"
	NI-60	24'-9"	23'-0"	22'-0"	20'-9"	25'-5"	23'-8"	22'-4"	20'-10'
	NI-80	26'-5"	24'-6"	23'-4"	22'-1"	27'-0"	25'-2"	24'-0"	22'-8"
	NI-90	26'-11"	25'-0"	23'-10"	22'-6"	27'-5"	25'-7"	24'-5"	23'-1"
16"	NI-60	27'-2"	25'-4"	24'-2"	22'-10"	27'-11"	26'-1"	24'-9"	23'-1"
	NI-80	29'-0"	26'-11"	25'-8"	24'-3"	29'-7"	27'-7"	26'-4"	24'-11'
	NI-90	29'-6"	27'-5"	26'-1"	24'-8"	30'-1"	28'-1"	26'-9"	25'-4"

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.



- Notes:

 1. To prevent interference with plumbing, a joist may be shifted up to 3 inches if the edge of the floor panel is supported and the span rating is not exceeded.

 2. In all other cases, an additional joist is required.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity. DRAWING

NORDIC STRUCTURES nordic.ca

DETAILS NORDIC JOIST NS-DC3

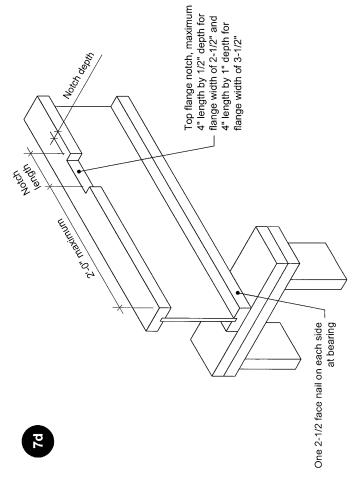
Openings for Vertical Elements Allowance for Piping CATEGORY

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2-1/2" and 1" depth for flange width of 3-1/2" Maximum 1/2" depth for flange width of Heat register

Notes:

- Blocking required at bearing for lateral support, not shown for clarity.
 The maximum dimensions for a notch on the side of the top flange are 4-inch length by 1/2-inch depth for flange width of 2-1/2 inches, and 4-inch length by 1-inch depth for flange width of 3-1/2 inches.
 This detail applies to simple-span joists and multiple-span joists where the notch is located at the end

half-span.
4. For other applications, contact Nordic Structures.

All nails shown in the delails are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity. DRAWING TITLE

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Notch in I-joist for Heat Register	CATEGORY	Openings for Vertical Elements

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