

2011-MOD-CRN / B
413222

20-06-00

2008 / B
413223

19-08-00

2009-END / B
413224

20-01-00

3-03-00

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.4 psf | Sr = 8.4 psf

DESIGN LOADS:

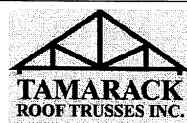
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

 **DENOTES:**
CONVENTIONAL
FRAMING

HARDWARE:

LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T - (I)
HTS16 - (S)

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

Job Track: **51012**Plan Log: **203568**

Layout ID: 413212

Builder / Location:	
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ROYAL PINE HOMES / RICHMOND HILL

Project: **CENTREFIELD**

Date: 2021-06-30

Sales:	Mario DiCano
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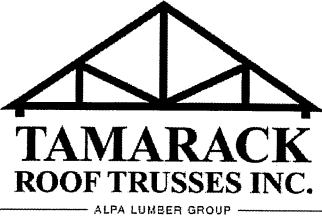
Designer: AC

Model / Elevation:

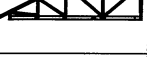
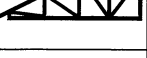
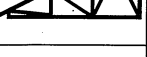
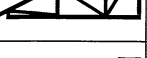
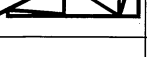
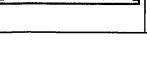
BLOCK 55 / UNITS 34 - 36

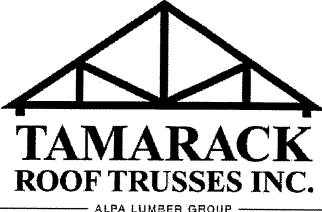
Mitek ver 8.4.2.286

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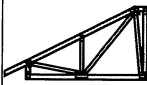
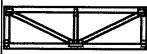
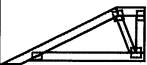
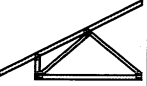
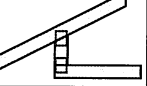
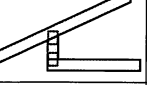
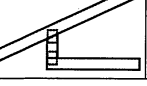
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 55 Lot #: Elevation: B / UNIT34BLK287						Job Track: 51012 PlanLog: 203568 Layout ID: 413222 Ref # Page: 1 of 3 Date: 05-28-2021 Designer: Andrew Conway Sales Rep: Mario DiCano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T10G GABLE	6 /12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	121.88 76.83		
	1	T10GA GABLE	6 /12	21-08-08	8-01-04	2 x 4		1-02-00 8-01-04	118.23 75.50		
	1 2-ply	T161 Half Hip Girder	8 /12	19-10-08	4-01-04	2 x 4	1-03-08	1-04-13 4-01-04	160 101.00		
	1	T162 Half Hip	8 /12	19-10-08	5-01-04	2 x 4	1-03-08	1-04-13 5-01-04	85.77 54.33		
	1	T163 Half Hip	8 /12	19-10-08	6-01-04	2 x 4	1-03-08	1-04-13 6-01-04	86.18 54.00		
	1	T164 Half Hip	8 /12	20-10-08	7-01-04	2 x 4		8-13 7-01-04	93.46 59.17		
	5	T165 Half Hip	6 /12	20-10-08	8-01-04	2 x 4		2-00 8-01-04	405.59 248.33		
	1	T166 Half Hip	6 /12	20-10-08	9-01-04	2 x 4		2-00 9-01-04	92.09 56.33		
	1 2-ply	T167 Half Hip Girder	6 /12	18-10-08	5-01-04	2 x 4 2 x 6	1-03-08	1-02-00 5-01-04	184.57 112.33		
	1	T168 Half Hip	6 /12	18-10-08	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	84.5 52.50		
	1	T169 Half Hip	6 /12	18-10-08	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	84.97 52.67		
	3	T170 Half Hip	6 /12	18-10-08	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	253.76 157.50		
	1	T171 Half Hip	6 /12	18-10-08	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	89.03 55.17		
	1 2-ply	T172 Flat Girder	0 /12	6-10-08	2-03-00	2 x 4 2 x 6		2-03-00 2-03-00	60.65 38.67		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 55 Lot #: Elevation: B / UNIT34BLK287	Job Track: 51012 PlanLog: 203568 Layout ID: 413222 Ref # Page: 2 of 3 Date: 05-28-2021 Designer: Andrew Conway Sales Rep: Mario DiCano

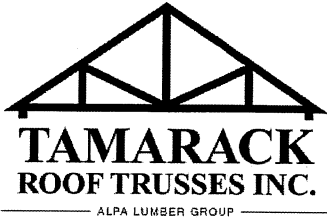
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T173 Half Hip	6 /12	7-08-00	4-07-00	2 x 4	1-03-08	1-02-00 4-07-00	37.51 25.00		
	1 2-ply	T174 Flat	0 /12	7-08-00	2-03-00	2 x 4		2-03-00 2-03-00	57.73 36.67		
	2	T175 Hip Girder	6 /12	9-11-00	2-02-15	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	81.16 50.67		
	2	T176 Hip	6 /12	9-11-00	3-02-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	83.37 54.33		
	2	PB121 Piggyback	6 /12	5-00-00	2-00-00	2 x 4		2-00-00	27.02 19.33		
	4	PB122 Piggyback	6 /12	5-00-00	2-06-00	2 x 4		2-06-00	52.01 34.67		
	2	PB123G GABLE	6 /12	8-10-00	2-06-00	2 x 4		2-06-00	53.32 33.67		
	10	J01 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	6	J61 Jack-Partial	6 /12	6-10-08	5-01-04	2 x 4	2-03-08	1-08-00 5-01-04	168.25 110.00		
	3	J62 Jack-Open	6 /12	1-07-12	1-11-14	2 x 4	1-03-08 4-04	1-02-00 1-11-14	20.96 14.00		
	3	J63 Jack-Open	5.53 /1 2	2-02-06	2-01-08	2 x 4	1-03-08 3-02	1-01-05 2-01-08	24.18 16.00		
	5	J64 Jack-Open	5.53 /1 2	2-05-08	2-02-15	2 x 4	1-03-08	1-01-05 2-02-15	41.93 30.00		

TOTAL # TRUSS= 65
TOTAL BFT OF ALL TRUSSES= 1725.34
BFT.
TOTAL WEIGHT OF ALL TRSSES 2736.06 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LUS24	
2	Hardware	HGUS26-2	



DELIVERY SHIPLIST

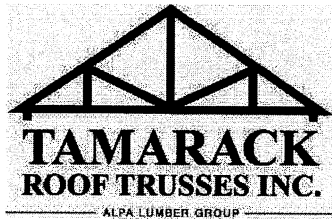
Lumber Yard: TAMARACK LUMBER
Builder: ROYAL PINE HOMES
Project: CENTREFIELD
Location: RICHMOND HILL
Model: BLOCK 55
Lot #:
Elevation: B / UNIT34BLK287

Job Track: 51012
PlanLog: 203568
Layout ID: 413222
Ref #
Page: 3 of 3
Date: 05-28-2021
Designer: Andrew Conway
Sales Rep: Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
19	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= **22**



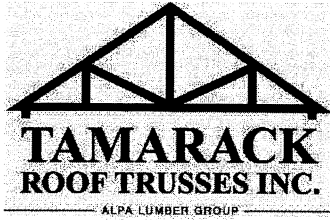
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 55
 Lot #:
 Elevation: B / UNIT35BLK287

Job Track: 51012
 PlanLog: 203568
 Layout ID: 413223
 Ref #
 Page: 1 of 2
 Date: 06-30-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T122 Hip Girder	6 /12	8-07-00	2-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	35.14 23.67		
	1	T141 Common	6 /12	8-07-00	3-03-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	34.58 22.17		
	1	T177G GABLE	6 /12	41-09-00	7-00-00	2 x 6		1-08-00 10-00	238.25 150.50		
	1	T178 Hip	6 /12	45-05-00	6-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	282.39 171.33		
	3	T179 Hip	6 /12	45-05-00	7-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	840.04 514.00		
	1	T179G GABLE	6 /12	45-05-00	7-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	260.2 168.00		
	1	T180 Hip	6 /12	45-05-00	8-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	282.59 172.00		
	1 3-ply	T181 Common Girder	6 /12	8-07-00	4-09-12	2 x 4 2 x 8	1-03-08 1-03-08	2-08-00 2-08-00	173.7 112.00		
	1	T183 Hip	6 /12	45-00-08	5-00-00	2 x 6	1-03-08	1-02-00 1-04-04	256.88 157.67		
	1 2-ply	T184 Hip Girder	6 /12	45-00-08	4-00-00	2 x 6	1-03-08	1-02-00 1-04-04	511.54 321.33		
	2	T185 Piggyback Base	6 /12	42-04-08	7-00-00	2 x 6		1-08-00 3-00-00	465.8 290.33		
	1	PB124G GABLE	6 /12	18-09-00	2-11-00	2 x 4			53.17 35.50		
	1	PB127 Piggyback	6 /12	22-01-00	2-00-00	2 x 4			63.6 40.67		
	1	PB128 Piggyback	6 /12	22-01-00	3-00-00	2 x 4			63.99 41.17		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: CENTREFIELD
 Location: RICHMOND HILL
 Model: BLOCK 55
 Lot #:
 Elevation: B / UNIT35BLK287

Job Track: 51012
 PlanLog: 203568
 Layout ID: 413223
 Ref #
 Page: 2 of 2
 Date: 06-30-2021
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	PB129G GABLE	6 /12	22-01-00	3-08-04	2 x 4			65.67 41.83		
	1	PB131 Piggyback	6 /12	18-09-00	3-11-00	2 x 4			51.26 32.50		
	1	PB132 Piggyback	6 /12	18-09-00	4-08-04	2 x 4			54.69 34.00		
	2	J11 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	15.69 10.67		
	2	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	17.09 12.00		
	2	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	14.65 9.33		
	1	J65 Jack-Open	6 /12	5-10-08	4-00-00	2 x 4		1-00-12 4-00-00	15.03 9.33		

TOTAL # TRUSS= 30

TOTAL BFT OF ALL TRUSSES= 2370

BFT.

TOTAL WEIGHT OF ALL TRSSES 3795.93 LBS

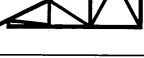
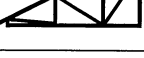
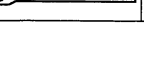
HARDWARE

QTY	TYPE	MODEL	LENGTH
13	Hardware	H2.5T	
2	Hardware	HGUS26-2	
4		HTS16	
5	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 24

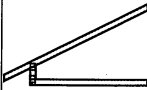
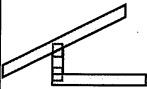
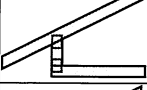
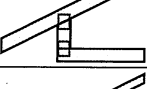
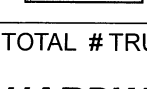
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER	Job Track: 51012	Page: 1 of 2 Date: 05-28-2021 Designer: Andrew Conway Sales Rep: Mario DiCano					
Builder: ROYAL PINE HOMES	PlanLog: 203568							
Project: CENTREFIELD	Layout ID: 413224							
Location: RICHMOND HILL	Ref #							
Model: BLOCK 55								
Lot #:								
Elevation: B / UNIT36BLK287								

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T10G GABLE	6 /12	22-08-08	8-01-04	2 x 4	1-05-00	1-02-00 8-01-04	243.75 153.67		
	1 2-ply	T101 Half Hip Girder	6 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	182.72 116.00		
	1 2-ply	T101Z Half Hip Girder	6 /12	19-05-08	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 4-01-04	182.72 116.00		
	2	T102 Half Hip	6 /12	19-05-08	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	159.13 101.33		
	2	T103 Half Hip	6 /12	19-05-08	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	172.24 109.67		
	2	T104 Half Hip	6 /12	19-05-08	7-01-04	2 x 4	1-03-08	1-02-00 7-01-04	173.04 108.00		
	8	T105 Half Hip	6 /12	19-05-08	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	689.37 434.67		
	2	T106 Half Hip	6 /12	19-05-08	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	181.02 114.00		
	1	T107 Hip Girder	6 /12	9-06-00	2-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	37.9 25.50		
	2	T108 Hip	6 /12	9-06-00	4-10-12	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	99.25 67.00		
	1	T109 Hip	6 /12	9-06-00	3-10-12	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	45.71 31.67		
	1 2-ply	T110 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	PB101 Piggyback	6 /12	5-07-00	2-00-00	2 x 4		2-00-00	32.19 22.67		
	4	PB102 Piggyback	6 /12	5-07-00	2-09-08	2 x 4		2-09-08	58.98 40.00		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: ROYAL PINE HOMES Project: CENTREFIELD Location: RICHMOND HILL Model: BLOCK 55 Lot #: Elevation: B / UNIT36BLK287	Job Track: 51012 PlanLog: 203568 Layout ID: 413224 Ref # Page: 2 of 2 Date: 05-28-2021 Designer: Andrew Conway Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB130G GABLE	6 /12	8-10-00	2-08-12	2 x 4		2-08-12	55 38.33		
	11	J01 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	184.74 117.33		
	5	J03 Jack-Open	4 /12	3-07-00	1-11-03	2 x 4	1-03-08	3-15 1-03-04	50.28 33.33		
	2	J11 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 6-09	1-02-00 2-01-08	15.69 10.67		
	2	J12 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	17.09 12.00		
	3	J13 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 1-01	1-02-00 2-01-08	21.97 14.00		
	1	J14 Jack-Open	6 /12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	9.87 6.00		
	1	J15 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 3-11-09	1-02-00 2-01-08	11.74 7.33		
	1	J16 Jack-Open	6 /12	3-10-15	3-01-08	2 x 4	1-03-08 1-11-09	1-02-00 3-01-08	14.29 8.67		

TOTAL # TRUSS= 62
TOTAL BFT OF ALL TRUSSES= 1725.51
BFT.
TOTAL WEIGHT OF ALL TRSSES 2697.1
LBS

HARDWARE

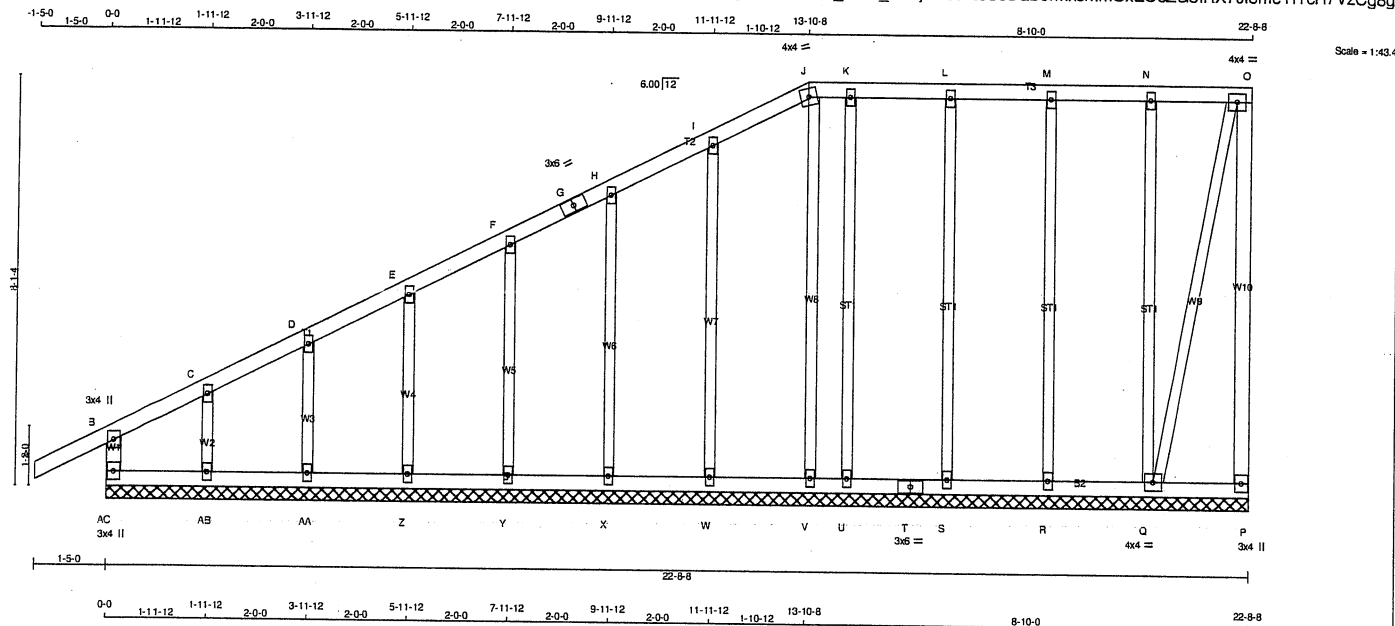
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	HGUS26-2	
20	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= 24

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T10G	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 10:38:11 2021 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
AC - B	2x4	DRY	No.2	SPF	
A - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
J - O	2x4	DRY	No.2	SPF	
P - O	2x4	DRY	No.2	SPF	
AC - T	2x4	DRY	No.2	SPF	
T - P	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			FACTORED			WEBS		
MEMB.	MAX. FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FORCE	FACTORED
	(LBS)	(PLF)	CSI (LC)		UNBRAC		(LBS)	MAX
FR-TO		FROM TO			LENGTH	FR-TO		CSI (LC)
AC-B	-271 / 0	0.0	0.0	0.03 (1)	7.81	Q-N	-200 / 0	0.25 (1)
A-B	0 / 30	-91.8	-91.8	0.14 (1)	10.00	R-M	-182 / 0	0.23 (1)
B-C	-21 / 0	-91.8	-91.8	0.14 (1)	6.25	S-L	-190 / 0	0.24 (1)
C-D	0 / 10	-91.8	-91.8	0.05 (1)	10.00	U-K	-149 / 0	0.19 (1)
D-E	0 / 9	-91.8	-91.8	0.05 (1)	10.00	V-J	-95 / 0	0.12 (1)
E-F	0 / 14	-91.8	-91.8	0.04 (1)	10.00	AB-C	-118 / 0	0.02 (1)
F-G	0 / 16	-91.8	-91.8	0.04 (1)	10.00	AA-D	-196 / 0	0.04 (1)
G-H	0 / 16	-91.8	-91.8	0.04 (1)	10.00	Z-E	-179 / 0	0.05 (1)
H-I	0 / 19	-91.8	-91.8	0.05 (1)	10.00	Y-F	-183 / 0	0.07 (1)
I-J	0 / 13	-91.8	-91.8	0.05 (1)	10.00	X-H	-180 / 0	0.11 (1)
J-K	0 / 20	-91.8	-91.8	0.03 (1)	10.00	W-I	-200 / 0	0.18 (1)
K-L	0 / 20	-91.8	-91.8	0.05 (1)	10.00	Q-O	-79 / 0	0.11 (1)
L-M	0 / 20	-91.8	-91.8	0.05 (1)	10.00			
M-N	0 / 20	-91.8	-91.8	0.05 (1)	10.00			
N-O	0 / 20	-91.8	-91.8	0.05 (1)	10.00			
P-O	0 / 0	0.0	0.0	0.00 (1)	10.00			
AC-AB	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
AB-AA	-4 / 0	-18.5	-18.5	0.02 (4)	10.00			
AA-Z	-9 / 0	-18.5	-18.5	0.01 (4)	10.00			
Z-Y	-12 / 0	-18.5	-18.5	0.01 (4)	6.25			
Y-X	-14 / 0	-18.5	-18.5	0.01 (4)	6.25			
X-W	-16 / 0	-18.5	-18.5	0.02 (4)	6.25			
W-V	-18 / 0	-18.5	-18.5	0.02 (4)	6.25			
V-U	-20 / 0	-18.5	-18.5	0.01 (4)	6.25			
U-T	-20 / 0	-18.5	-18.5	0.02 (4)	6.25			
T-S	-20 / 0	-18.5	-18.5	0.02 (4)	6.25			
S-R	-20 / 0	-18.5	-18.5	0.02 (4)	6.25			
R-Q	-20 / 0	-18.5	-18.5	0.02 (4)	6.25			
Q-P	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (Q-R:4),
WB=0.25/1.00 (N-Q:1), SS=0.09/1.00 (A-B:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (J) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00)

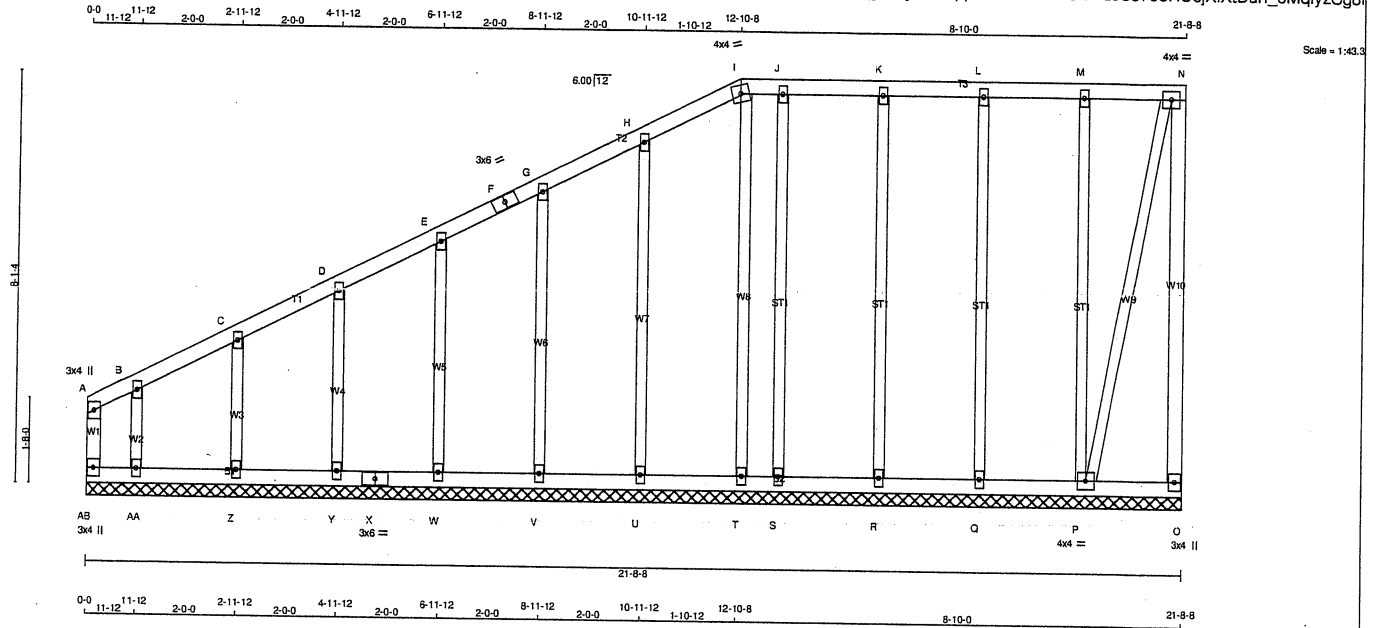


Structural component only
DWG# T-2115769

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T10GA	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Wed May 26 10:38:12 2021 Page 1
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TOTAL WEIGHT = 118 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
AB- A	2x4	DRY	No.2	SPF
A- F	2x4	DRY	No.2	SPF
F- I	2x4	DRY	No.2	SPF
I- N	2x4	DRY	No.2	SPF
O- N	2x4	DRY	No.2	SPF
AB- X	2x4	DRY	No.2	SPF
X- O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A TMV+p	MT20	3.0	4.0	
B, C, D, E, G, H, J, K, L, M	MT20	2.0	4.0	
B TMV+w	MT20	3.0	6.0	
F TS-t	MT20	4.0	4.0	
I TTV-m	MT20	4.0	4.0	
N TMVW-t	MT20	4.0	4.0	
O BMV1+p	MT20	3.0	4.0	
P BMVW1-t	MT20	4.0	4.0	
Q, R, S, T, U, V, W, Y, Z, AA	MT20	2.0	4.0	
Q BMV1+w	MT20	3.0	6.0	
X BS-t	MT20	3.0	6.0	
AB BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
AB-A	-20 / 0	0.0	0.0	0.00 (1)	7.81	P-M	-200 / 0
A-B	0 / 10	-91.8	-91.8	0.03 (1)	10.00	Q-L	-182 / 0
B-C	0 / 9	-91.8	-91.8	0.05 (1)	10.00	R-K	-190 / 0
C-D	0 / 10	-91.8	-91.8	0.05 (1)	10.00	S-J	-149 / 0
D-E	0 / 14	-91.8	-91.8	0.04 (1)	10.00	T-I	-96 / 0
E-F	0 / 17	-91.8	-91.8	0.04 (1)	10.00	AA-B	-147 / 0
F-G	0 / 17	-91.8	-91.8	0.04 (1)	10.00	Z-C	-189 / 0
G-H	0 / 20	-91.8	-91.8	0.05 (1)	10.00	Y-D	-180 / 0
H-I	0 / 14	-91.8	-91.8	0.05 (1)	10.00	W-E	-183 / 0
I-J	0 / 21	-91.8	-91.8	0.03 (1)	10.00	V-G	-180 / 0
J-K	0 / 20	-91.8	-91.8	0.05 (1)	10.00	U-H	-200 / 0
K-L	0 / 20	-91.8	-91.8	0.05 (1)	10.00	P-N	-82 / 0
L-M	0 / 20	-91.8	-91.8	0.05 (1)	10.00		
M-N	0 / 20	-91.8	-91.8	0.05 (1)	10.00		
O-N	0 / 4	0.0	0.0	0.00 (1)	10.00		
AB-AA	0 / 0	-18.5	-18.5	0.01 (4)	10.00		
AA-Z	-5 / 0	-18.5	-18.5	0.02 (4)	10.00		
Z-Y	-10 / 0	-18.5	-18.5	0.02 (4)	10.00		
Y-X	-13 / 0	-18.5	-18.5	0.01 (4)	6.25		
X-W	-13 / 0	-18.5	-18.5	0.01 (4)	6.25		
W-V	-15 / 0	-18.5	-18.5	0.01 (4)	6.25		
V-U	-17 / 0	-18.5	-18.5	0.02 (4)	6.25		
U-T	-19 / 0	-18.5	-18.5	0.02 (4)	6.25		
T-S	-20 / 0	-18.5	-18.5	0.01 (4)	6.25		
S-R	-20 / 0	-18.5	-18.5	0.02 (4)	6.25		
R-Q	-20 / 0	-18.5	-18.5	0.02 (4)	6.25		
Q-P	-20 / 0	-18.5	-18.5	0.02 (4)	6.25		
P-O	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (G-H:1), BC=0.02/1.00 (P-Q:4), WB=0.25/1.00 (M-P:1), SSI=0.08/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

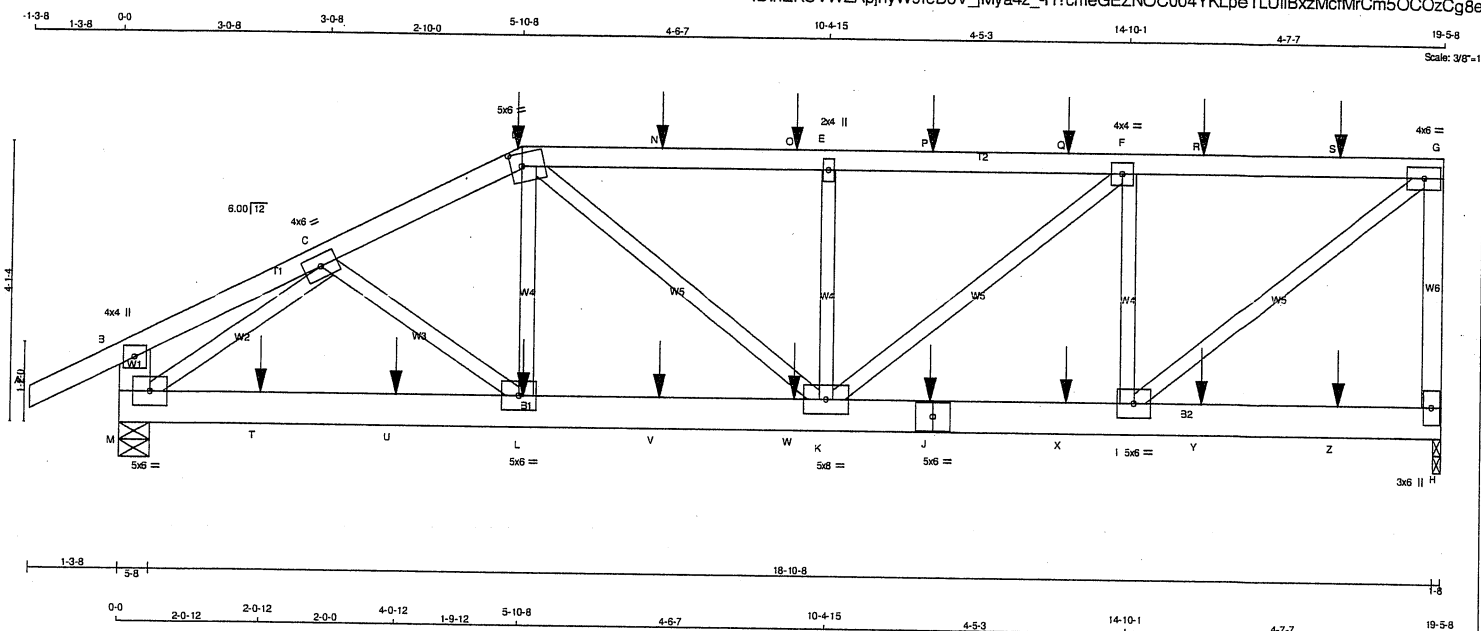
JSI GRIP= 0.59 (I) (INPUT = 0.90)
JSI METAL= 0.08 (H) (INPUT = 1.00)



Structural component only
DWG# T-2115770

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 91 = 183 lb

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 28	-91.8	-91.8 0.07 (1)	10.00	C-L	0 / 214	0.03 (1)
B-C	0 / 9	-91.8	-91.8 0.05 (1)	10.00	L-D	0 / 151	0.03 (4)
C-D	-2468 / 0	-91.8	-91.8 0.07 (1)	5.69	M-C	-2546 / 0	0.30 (1)
D-N	-2488 / 0	-91.8	-91.8 0.23 (1)	5.46	I-G	0 / 2332	0.29 (1)
N-O	-2488 / 0	-91.8	-91.8 0.23 (1)	5.46	D-K	0 / 375	0.05 (1)
E-O	-2488 / 0	-91.8	-91.8 0.23 (1)	5.46	I-F	-1302 / 0	0.17 (1)
P-P	-2488 / 0	-91.8	-91.8 0.26 (1)	5.42	K-E	-702 / 0	0.09 (1)
E-Q	-2488 / 0	-91.8	-91.8 0.26 (1)	5.42	K-F	0 / 880	0.11 (1)
Q-F	-2488 / 0	-91.8	-91.8 0.26 (1)	5.42			
F-R	-1814 / 0	-91.8	-91.8 0.25 (1)	6.13			
R-S	-1814 / 0	-91.8	-91.8 0.25 (1)	6.13			
S-G	-1814 / 0	-91.8	-91.8 0.25 (1)	6.13			
H-G	-1712 / 0	0.0	0.0 0.21 (1)	7.81			
M-B	-239 / 0	0.0	0.0 0.01 (1)	7.81			

M-T	0/2025	-18.5	-18.5	0.17 (1)	10.00
T-U	0/2025	-18.5	-18.5	0.17 (1)	10.00
L-L	0/2025	-18.5	-18.5	0.17 (1)	10.00
U-U	0/2198	-18.5	-18.5	0.17 (1)	10.00
V-V	0/2198	-18.5	-18.5	0.17 (1)	10.00
W-K	0/2198	-18.5	-18.5	0.17 (1)	10.00
K-J	0/1814	-18.5	-18.5	0.15 (1)	10.00
J-X	0/1814	-18.5	-18.5	0.15 (1)	10.00
X-I	0/1814	-18.5	-18.5	0.15 (1)	10.00
I-Y	0/0	-18.5	-18.5	0.04 (4)	10.00
Y-Z	0/0	-18.5	-18.5	0.04 (4)	10.00
Z-H	0/0	-18.5	-18.5	0.04 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)									
JOINT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
5-10-8	-317	-317	---	---	BACK	VERT	TOTAL	---	C1
11-11-4	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
5-11-4	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
7-11-4	-76	-76	---	---	BACK	VERT	TOTAL	---	C1
9-11-4	-76	-76	---	---	BACK	VERT	TOTAL	---	C1
11-11-4	-76	-76	---	---	BACK	VERT	TOTAL	---	C1
13-11-4	-76	-76	---	---	BACK	VERT	TOTAL	---	C1
15-11-4	-76	-76	---	---	BACK	VERT	TOTAL	---	C1
17-11-4	-76	-76	---	---	BACK	VERT	TOTAL	---	C1
2-0-12	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
4-0-12	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
7-1-14	-21	-21	---	---	BACK	VERT	TOTAL	---	C1
9-11-4	-21	-21	---	---	BACK	VERT	TOTAL	---	C1

SPECIFIED LOADS:			
TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD =		39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (0.65")
CALCULATED VERT. DEFL(LL) = L/999 (0.04")
ALLOWABLE DEFL(TL)= L/360 (0.65")
CALCULATED VERT. DEFL(TL) = L/999 (0.07")

CSL: TC=0.26/1.00 (E-F:1), BC=0.17/1.00 (L-M:1),
WB=0.30/1.00 (C-M:1), SSI=0.18/1.00 (D-E:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

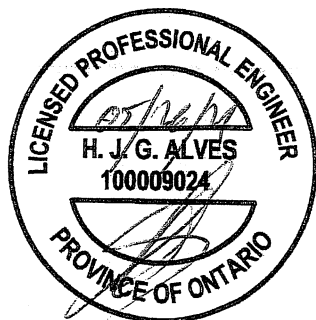
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (G) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115771 *///*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T101	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:13 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	13-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	15-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Z	17-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

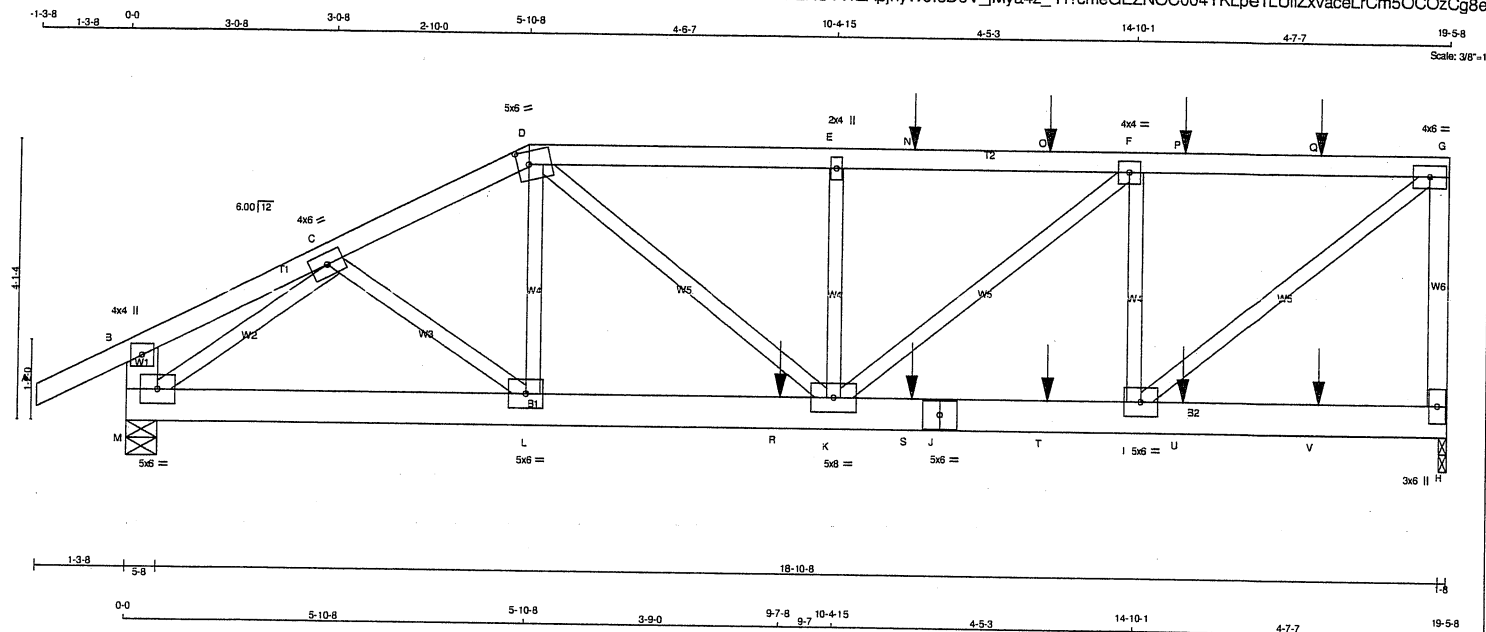


Structural component only
DWG# T-2115771 *2/2*

JOB NAME 413212	TRUSS NAME T101Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:13 2021 Page 1
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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x6	DRY	No.2
M - J	2x6	DRY	No.2
J - H	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2' SPF

EXCEPT DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-G 1	12	SIDE(0.0)
G-H 1	12	TOP
M-B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M-J 2	12	SIDE(0.0)
J-H 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	2170	0	2170	0
M	2037	0	2037	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1531	1022 / 0	0 / 0	0 / 0	0 / 0	509 / 0	0 / 0
M	1434	977 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.07 (1)	C-L	0 / 295	0.04 (1)	
B-C	0 / 9	-91.8 -91.8	0.06 (1)	L-D	0 / 176	0.02 (4)	
C-D	-2873 / 0	-91.8 -91.8	0.08 (1)	M-C	-2920 / 0	0.35 (1)	
D-E	-3565 / 0	-91.8 -91.8	0.19 (1)	I-G	0 / 2975	0.37 (1)	
E-N	-3565 / 0	-91.8 -91.8	0.30 (1)	D-K	0 / 1296	0.16 (1)	
N-O	-3565 / 0	-91.8 -91.8	0.30 (1)	I-F	-1812 / 0	0.23 (1)	
O-F	-3565 / 0	-91.8 -91.8	0.30 (1)	K-E	-529 / 0	0.07 (1)	
F-P	-2315 / 0	-91.8 -91.8	0.27 (1)	K-F	0 / 1635	0.20 (1)	
P-Q	-2315 / 0	-91.8 -91.8	0.27 (1)				
Q-G	-2315 / 0	-91.8 -91.8	0.27 (1)				
H-G	-2095 / 0	0.0 0.0	0.26 (1)				
M-B	-241 / 0	0.0 0.0	0.01 (1)				
M-L	0 / 2322	-18.5 -18.5	0.22 (1)				
L-R	0 / 2564	-18.5 -18.5	0.41 (1)				
R-K	0 / 2564	-18.5 -18.5	0.41 (1)				
K-S	0 / 2315	-18.5 -18.5	0.25 (1)				
S-J	0 / 2315	-18.5 -18.5	0.25 (1)				
J-T	0 / 2315	-18.5 -18.5	0.25 (1)				
T-I	0 / 2315	-18.5 -18.5	0.25 (1)				
I-U	0 / 0	-18.5 -18.5	0.04 (4)				
U-V	0 / 0	-18.5 -18.5	0.04 (4)				
V-H	0 / 0	-18.5 -18.5	0.04 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	11-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
O	13-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
P	15-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Q	17-6-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
R	9-7-8	-974	-974	---	FRONT	VERT	TOTAL	---	C1
S	11-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	13-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
U	15-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	17-6-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/999 (0.11")

CSI: TC=0.30/1.00 (E-F:1) , BC=0.41/1.00 (K-L:1) ,
WB=0.37/1.00 (G-I:1) , SS=0.40/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
		1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115772

CONTINUED ON PAGE 2

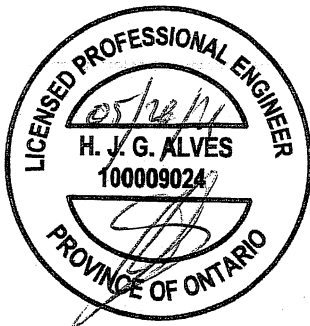
JOB NAME 413212	TRUSS NAME T101Z	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:13 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMVW1-t	MT20	5.0	6.0		

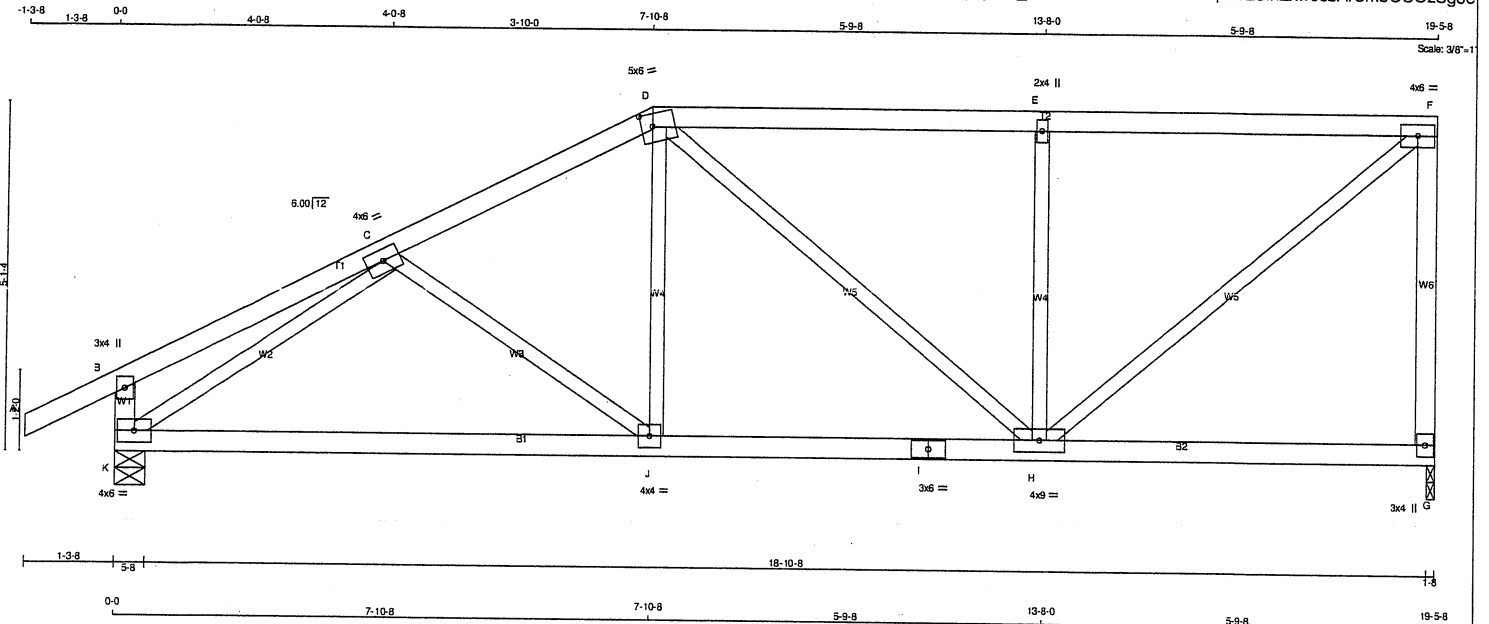


Structural component only
DWG# T-2115772 *JP*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T102	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF			
D - F	2x4	DRY	No.2	SPF			
G - F	2x4	DRY	No.2	SPF			
K - B	2x4	DRY	No.2	SPF			
K - I	2x4	DRY	No.2	SPF			
I - G	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
EXCEPT							
DRY: SEASONED LUMBER.							

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
G	1073	0	1073	0	1-8	1-8
K	1197	0	1197	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 CSI (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-J	-182 / 2	0.07 (1)
B-C	0 / 18	-91.8	-91.8 0.21 (1)	10.00	J-D	0 / 253	0.06 (4)
C-D	-1207 / 0	-91.8	-91.8 0.24 (1)	5.54	D-H	-94 / 0	0.10 (1)
D-E	-996 / 0	-91.8	-91.8 0.55 (1)	5.39	H-E	-658 / 0	0.25 (1)
E-F	-996 / 0	-91.8	-91.8 0.56 (1)	5.39	H-F	0 / 1291	0.29 (1)
G-F	-1026 / 0	0.0	0.0 0.45 (1)	7.74	K-C	-1491 / 0	0.57 (1)
K-B	-266 / 0	0.0	0.0 0.03 (1)	7.81			
K-J	0 / 1212	-18.5	-18.5 0.34 (4)	10.00			
J-I	0 / 1068	-18.5	-18.5 0.34 (4)	10.00			
I-H	0 / 1068	-18.5	-18.5 0.34 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

TOTAL WEIGHT = 2 X 80 = 159 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.56/1.00 (E-F:1), BC=0.34/1.00 (H-J:4),
WB=0.57/1.00 (C-K:1), SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

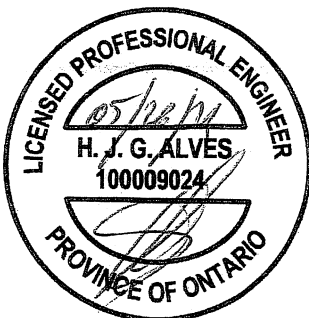
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.78 (F) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)

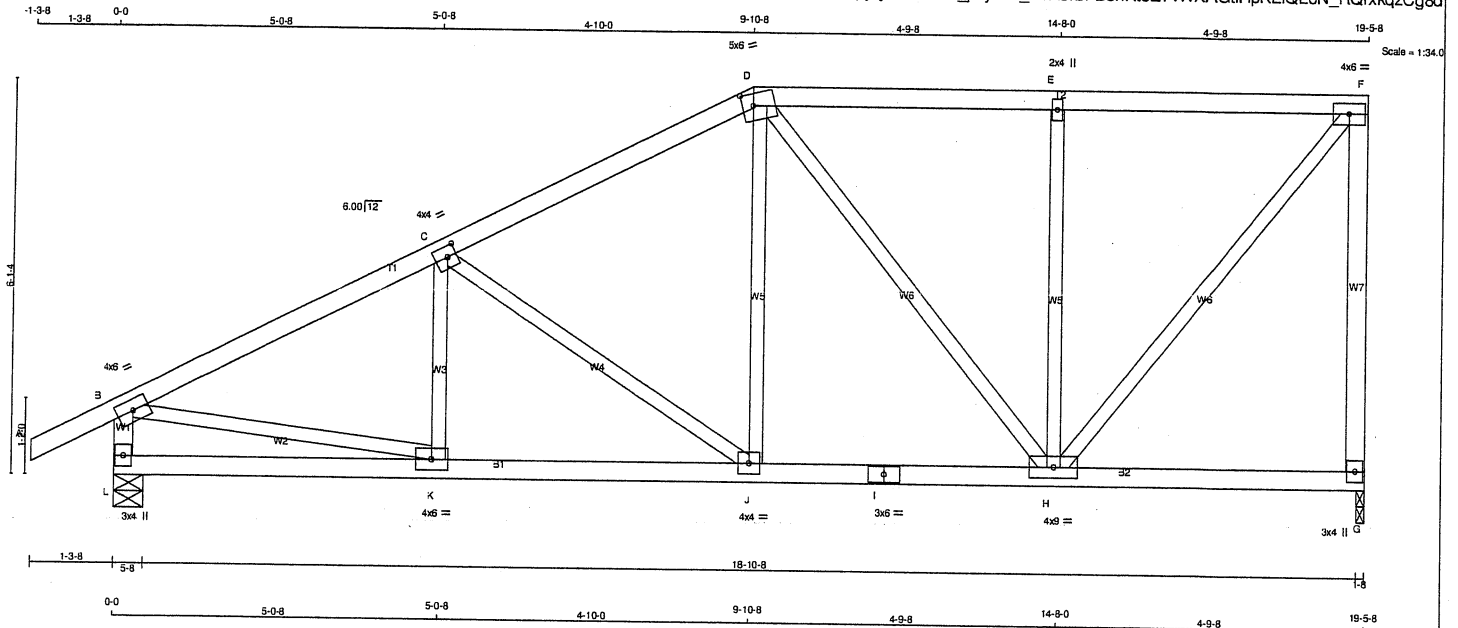


Structural component only
DWG# T-2115773

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T103	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
G	1073	0	1073	0
L	1197	0	1197	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
L	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	FR-TO
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	K-C	-118 / 43	0.03 (1)		
B-C	-1400 / 0	-91.8	-91.8	0.31 (1)	5.17	C-J	-454 / 0	0.28 (1)		
C-D	-1030 / 0	-91.8	-91.8	0.29 (1)	5.83	J-D	0 / 343	0.08 (1)		
D-E	-720 / 0	-91.8	-91.8	0.27 (1)	6.25	D-H	-292 / 0	0.32 (1)		
E-F	-720 / 0	-91.8	-91.8	0.27 (1)	6.25	H-E	-543 / 0	0.32 (1)		
G-F	-1035 / 0	0.0	0.0	0.75 (1)	7.72	H-F	0 / 1126	0.25 (1)		
L-B	-1157 / 0	0.0	0.0	0.12 (1)	7.40	B-K	0 / 1291	0.29 (1)		
L-K	0 / 0	-18.5	-18.5	0.10 (4)	10.00					
K-J	0 / 1273	-18.5	-18.5	0.25 (1)	10.00					
J-I	0 / 905	-18.5	-18.5	0.19 (1)	10.00					
I-H	0 / 905	-18.5	-18.5	0.19 (1)	10.00					
H-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00					

TOTAL WEIGHT = 2 X 86 = 172 lb
(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.75/1.00 (F-G:1), BC=0.25/1.00 (J-K:1),
WB=0.32/1.00 (E-H:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)

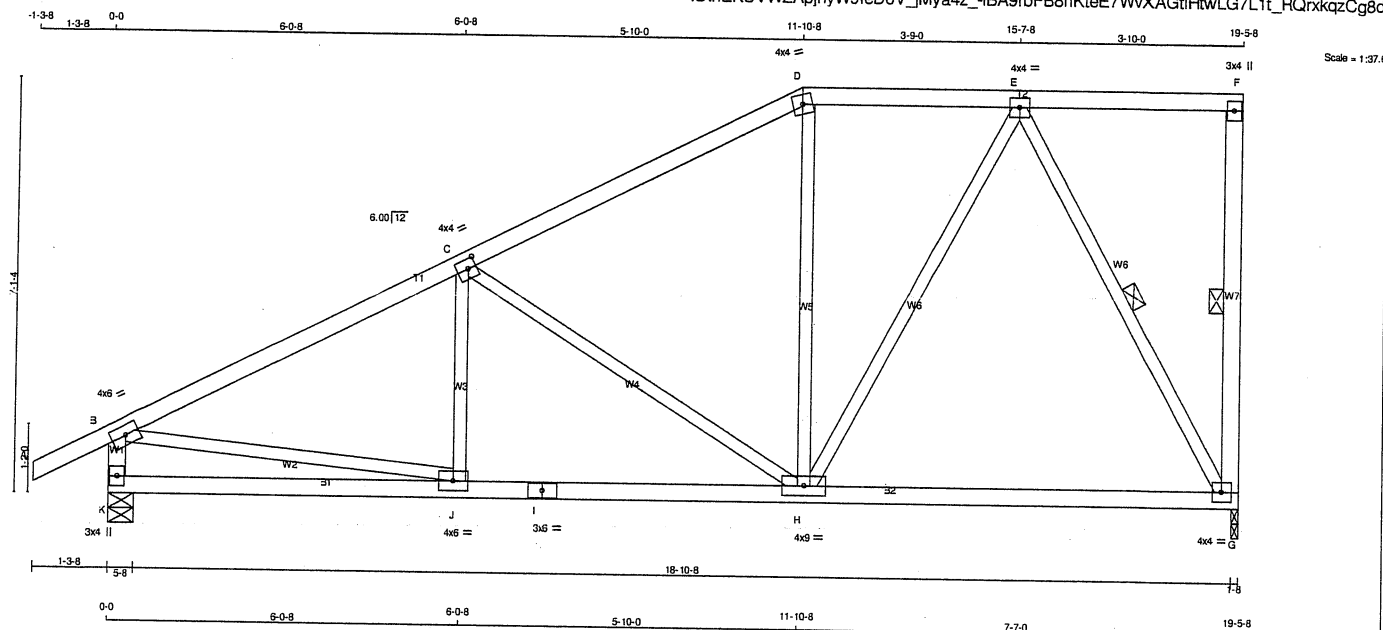


Structural component only
DWG# T-2115774

JOB NAME 413212	TRUSS NAME T104	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMV-p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVW1-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
G	1073	0	1073	0	0	1-8	1-8	
K	1197	0	1197	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	J-C	-70 / 86	0.02 (4)	
B-C	-1384 / 0	-91.8 -91.8	0.45 (1)	C-H	-629 / 0	0.61 (1)	
C-D	-855 / 0	-91.8 -91.8	0.42 (1)	H-D	0 / 89	0.02 (4)	
D-E	-738 / 0	-91.8 -91.8	0.17 (1)	H-E	0 / 512	0.12 (1)	
E-F	0 / 0	-91.8 -91.8	0.22 (1)	E-G	-1012 / 0	0.41 (1)	
G-F	-134 / 0	0.0 0.0	0.03 (1)	B-J	0 / 1276	0.29 (1)	
K-B	-1148 / 0	0.0 0.0	0.12 (1)				
K-J	0 / 0	-18.5 -18.5	0.14 (4)				
J-I	0 / 1264	-18.5 -18.5	0.33 (1)				
I-H	0 / 1264	-18.5 -18.5	0.33 (1)				
H-G	0 / 496	-18.5 -18.5	0.26 (4)				

TOTAL WEIGHT = 2 X 87 = 173 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.45/1.00 (B-C:1), BC=0.33/1.00 (H-J:1), WB=0.61/1.00 (C-H:1), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.37 (I) (INPUT = 1.00)

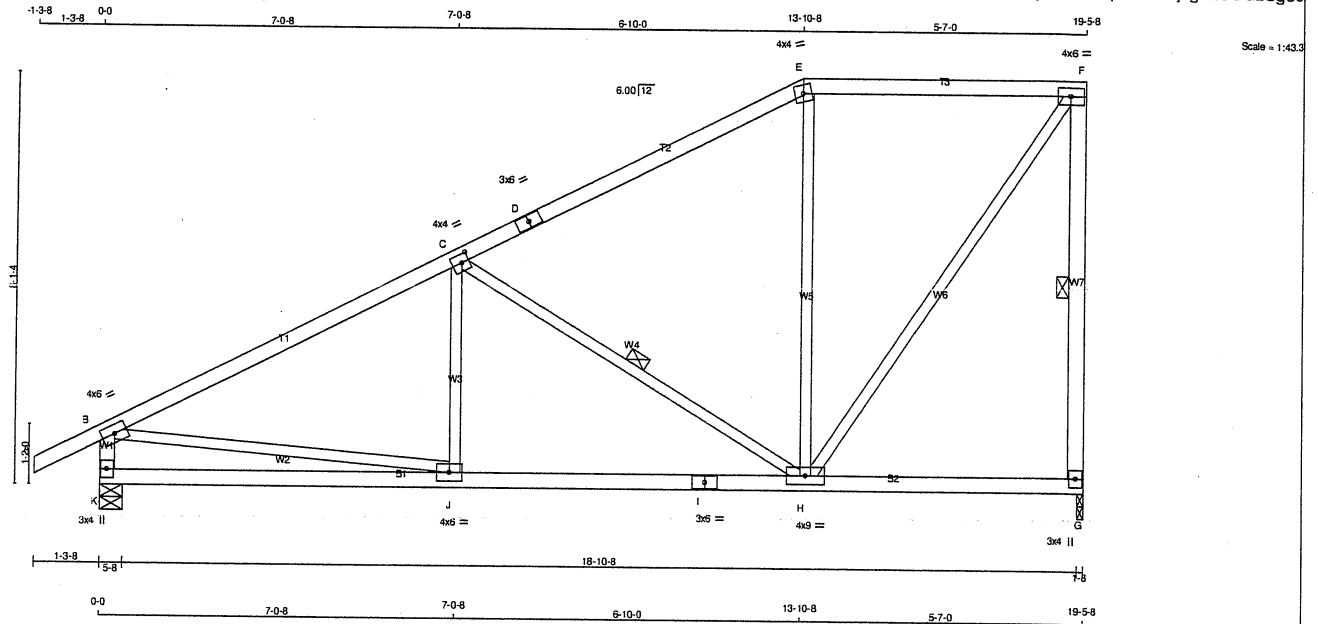


Structural component only
DWG# T-2115775

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T105	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 8 X 86 = 689 lb [M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - E	2x4 DRY	No.2	SPF		
E - F	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
K - B	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1-p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1073	0	1073	0
K	1197	0	1197	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	J-C	-8 / 113	0.04 (4)	
B-C	-1355 / 0	-91.8 -91.8	0.64 (1)	C-H	-821 / 0	0.38 (1)	
C-D	-649 / 0	-91.8 -91.8	0.58 (1)	H-E	-216 / 6	0.27 (1)	
D-E	-649 / 0	-91.8 -91.8	0.58 (1)	E-F	0 / 951	0.21 (1)	
E-F	-546 / 0	-91.8 -91.8	0.37 (1)	F-J	0 / 1252	0.28 (1)	
G-F	-1034 / 0	0.0 0.0	0.31 (1)				
K-B	-1144 / 0	0.0 0.0	0.12 (1)				
K-J	0 / 0	-18.5 -18.5	0.22 (4)				
J-I	0 / 1243	-18.5 -18.5	0.32 (4)				
I-H	0 / 1243	-18.5 -18.5	0.32 (4)				
H-G	0 / 0	-18.5 -18.5	0.15 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.64/1.00 (B-C:1), BC=0.32/1.00 (H-J:4),
WB=0.38/1.00 (C-H:1), SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.84 (H) (INPUT = 0.90)
JSI METAL = 0.41 (I) (INPUT = 1.00)

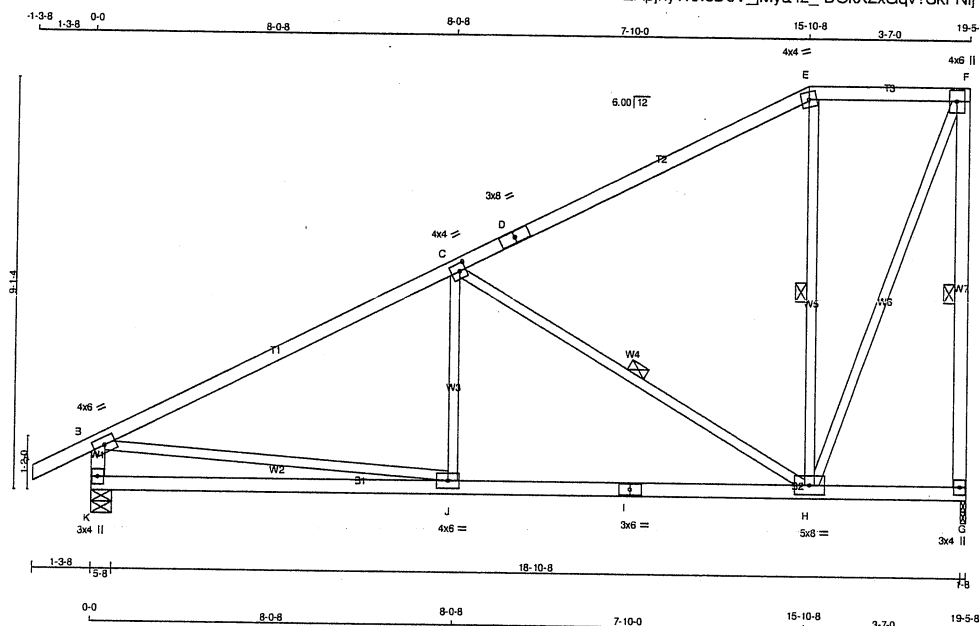


Structural component only
DWG# T-2115776

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T106	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/8" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - E	2x4 DRY	No.2	SPF		
E - F	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
K - B	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

LOADING DESIGNER							
<u>BEARINGS</u>							
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1073	0	1073	0	0	1-8	1-8
K	1197	0	1197	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	759	498 / 0	0 / 0	0 / 0	0 / 0	261 / 0	0 / 0
K	844	567 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO				FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	J-C	0 / 149	0.05 (4)
B-C	-1311 / 0	-91.8	-91.8 0.86 (1)	C-H	-1000 / 0	0.61 (1)
C-D	-439 / 0	-91.8	-91.8 0.77 (1)	H-E	-255 / 0	0.14 (1)
D-E	-439 / 0	-91.8	-91.8 0.77 (1)	H-F	0 / 961	0.22 (1)
E-F	-353 / 0	-91.8	-91.8 0.15 (1)	B-J	0 / 1214	0.27 (1)
G-F	-1057 / 0	0.0	0.0 0.41 (1)			
K-B	-1138 / 0	0.0	0.0 0.11 (1)			
K-J	0 / 0	-18.5	-18.5 0.30 (4)			
J-I	0 / 1208	-18.5	-18.5 0.40 (4)			
I-H	0 / 1208	-18.5	-18.5 0.40 (4)			
H-G	0 / 0	-18.5	-18.5 0.15 (4)			

TOTAL WEIGHT = 2 X 91 = 181 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.86/1.00 (B-C:1), BC=0.40/1.00 (H-J:4), WB=0.61/1.00 (C-H:1), SSI=0.32/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.49 (I) (INPUT = 1.00)

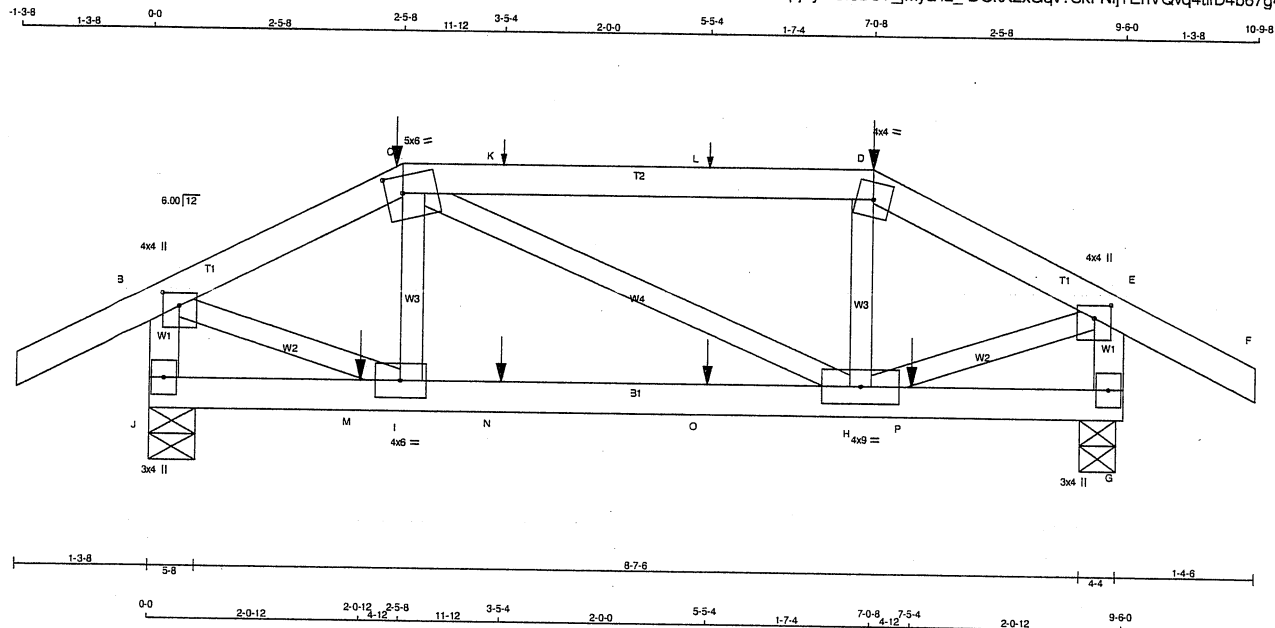


Structural component only
DWG# T-2115777

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T107	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/21.3

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER	DESCR	
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.50	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BMVWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	762	0	762	0
G	762	0	762	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	535	373 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0
G	535	373 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED MAX. CSI (LC)	MAX. UNBRACED LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. CSI (LC)
A-B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	I-C	-126 / 22	0.02 (1)		
B-C	-678 / 0	-91.8	-91.8	0.11 (1)	6.25	C-H	0 / 0	0.00 (1)		
C-K	-598 / 0	-91.8	-91.8	0.37 (1)	6.25	H-D	-127 / 21	0.02 (1)		
K-L	-598 / 0	-91.8	-91.8	0.37 (1)	6.25	B-I	0 / 638	0.16 (1)		
L-D	-598 / 0	-91.8	-91.8	0.37 (1)	6.25	H-E	0 / 638	0.16 (1)		
D-E	-677 / 0	-91.8	-91.8	0.11 (1)	6.25					
E-F	0 / 28	-91.8	-91.8	0.13 (1)	10.00					
J-B	-745 / 0	0.0	0.0	0.08 (1)	7.81					
G-E	-744 / 0	0.0	0.0	0.08 (1)	7.81					
J-M	0 / 0	-18.5	-18.5	0.06 (4)	10.00					
M-I	0 / 0	-18.5	-18.5	0.06 (4)	10.00					
I-N	0 / 599	-18.5	-18.5	0.15 (1)	10.00					
N-O	0 / 599	-18.5	-18.5	0.15 (1)	10.00					
O-H	0 / 599	-18.5	-18.5	0.15 (1)	10.00					
H-P	0 / 0	-18.5	-18.5	0.06 (4)	10.00					
P-G	0 / 0	-18.5	-18.5	0.06 (4)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-74	-74		FRONT	VERT	TOTAL	---	C1
D	7-0-8	-74	-74		FRONT	VERT	TOTAL	---	C1
K	3-5-4	1			FRONT	VERT	TOTAL	---	C1
L	5-5-4	1			FRONT	VERT	TOTAL	---	C1
M	2-0-12	-3	-3		FRONT	VERT	TOTAL	---	C1
N	3-5-4	-1	-1		FRONT	VERT	TOTAL	---	C1
O	5-5-4	-1	-1		FRONT	VERT	TOTAL	---	C1
P	7-5-4	-3	-3		FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.37/1.00 (C-D:1), BC=0.15/1.00 (H-I:1), WB=0.16/1.00 (B-I:1), SS=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

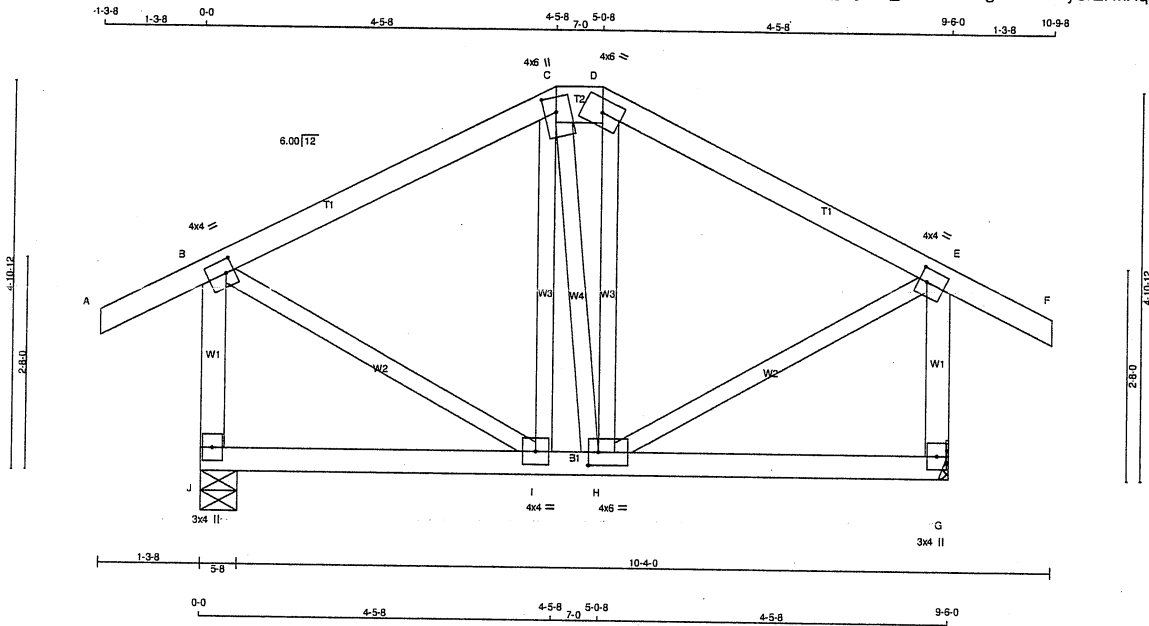


Structural component only
DWG# T-2115778

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T108	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 10:38:16 2021 Page 1
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Scale = 1/27.7

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x6	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	4.0	6.0	2.25	1.75
D	TTW-h	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UP	IN-SX
J	648	0	648	0
G	648	0	648	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	456	312 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
G	456	312 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH		(LBS)	CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	I-C	-88 / 7	0.03 (1)
B-C	-309 / 0	-91.8	-91.8	0.23 (1)	6.25	H-D	-96 / 7	0.03 (1)
C-D	-278 / 0	-91.8	-91.8	0.00 (1)	6.25	B-I	0 / 315	0.07 (1)
D-E	-307 / 0	-91.8	-91.8	0.23 (1)	6.25	H-E	0 / 314	0.07 (1)
E-F	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C-H	-7 / 0	0.00 (1)
J-B	-615 / 0	0.0	0.0	0.09 (1)	7.81			
G-E	-614 / 0	0.0	0.0	0.09 (1)	7.81			
J-I	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
I-H	0 / 279	-18.5	-18.5	0.10 (4)	10.00			
H-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

TOTAL WEIGHT = 2 X 50 = 99 lb
[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.32")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.32")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.10/1.00 (H-I:4), WB=0.07/1.00 (B-I:1), SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
650	371	1747
788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

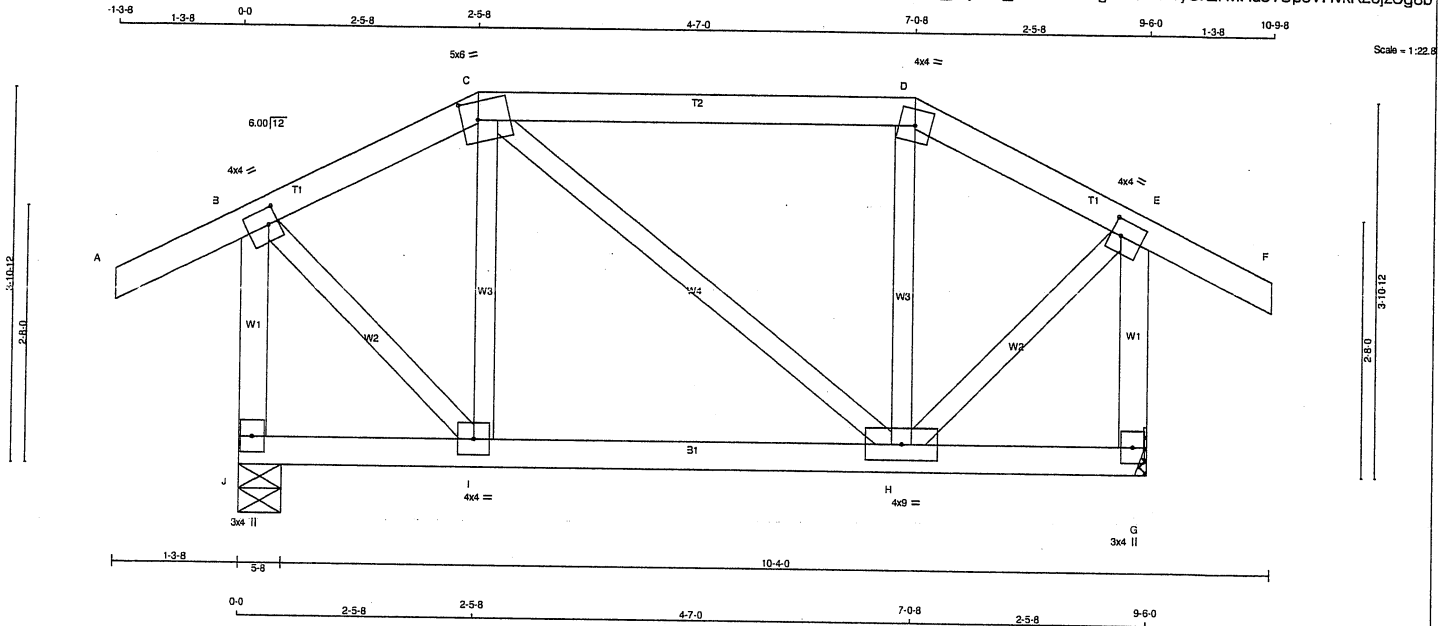


Structural component only
DWG# T-2115779

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T109	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:16 2021 Page 1
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TOTAL WEIGHT = 46 lb [M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	4.0	9.0	
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
J	648	0	648	0
G	648	0	648	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
J	456
G	456

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MEMB.	FORCE	MAX. FACTORED	MEMB.	FORCE
MEMB.	FORCE	VERT. LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00	I-C	-183/0
B-C	-311/0	-91.8	-91.8	0.07 (1)	6.25	C-H	0/0
C-D	-272/0	-91.8	-91.8	0.25 (1)	6.25	H-D	-183/0
D-E	-311/0	-91.8	-91.8	0.07 (1)	6.25	B-I	0/378
E-F	0/28	-91.8	-91.8	0.12 (1)	10.00	H-E	0/377
J-B	-633/0	0.0	0.0	0.09 (1)	7.81		
G-E	-632/0	0.0	0.0	0.09 (1)	7.81		
J-I	0/0	-18.5	-18.5	0.06 (4)	10.00		
I-H	0/273	-18.5	-18.5	0.08 (4)	10.00		
H-G	0/0	-18.5	-18.5	0.06 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.25/1.00 (C-D:1), BC=0.08/1.00 (H-I:4), WB=0.09/1.00 (B-I:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

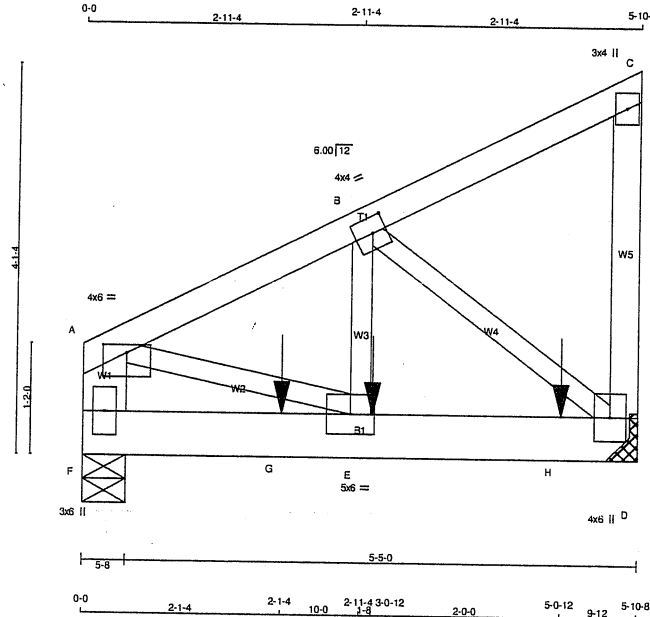


Structural component only
DWG# T-2115780

JOB NAME 413212	TRUSS NAME T110	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/2\"

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
SPACING (IN)		
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1 12	TOP	
C - D 1 12	TOP	
F - A 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2 12	SIDE(183.1)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
D	1408 0	1408 0	0	MECHANICAL	IN-SX
F	1130 0	1130 0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	989	688 / 0	0 / 0	0 / 0	0 / 0	301 / 0	0 / 0
F	794	550 / 0	0 / 0	0 / 0	0 / 0	244 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1250 / 0	-91.8 -91.8 0.06 (1)	6.25	E-B	0 / 1106	0.14 (1)	
B-C	-111 / 0	-91.8 -91.8 0.05 (1)	6.25	B-D	-1419 / 0	0.17 (1)	
D-C	-110 / 0	0.0 0.0 0.01 (1)	7.81	A-E	0 / 1166	0.14 (1)	
F-A	-971 / 0	0.0 0.0 0.03 (1)	7.81				
F-G	0 / 0	-18.5 -18.5 0.11 (1)	10.00				
G-E	0 / 0	-18.5 -18.5 0.11 (1)	10.00				
E-H	0 / 1128	-18.5 -18.5 0.20 (1)	10.00				
H-D	0 / 1128	-18.5 -18.5 0.20 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-441	-441	---	BACK	VERT	TOTAL	---	C1
G	2-1-4	-441	-441	---	BACK	VERT	TOTAL	---	C1
H	5-0-12	-443	-443	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.20/1.00 (D-E:1),
WB=0.17/1.00 (B-D:1), SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.25 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115781

CONTINUED ON PAGE 2

JOB NAME 413212	TRUSS NAME T110	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:17 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

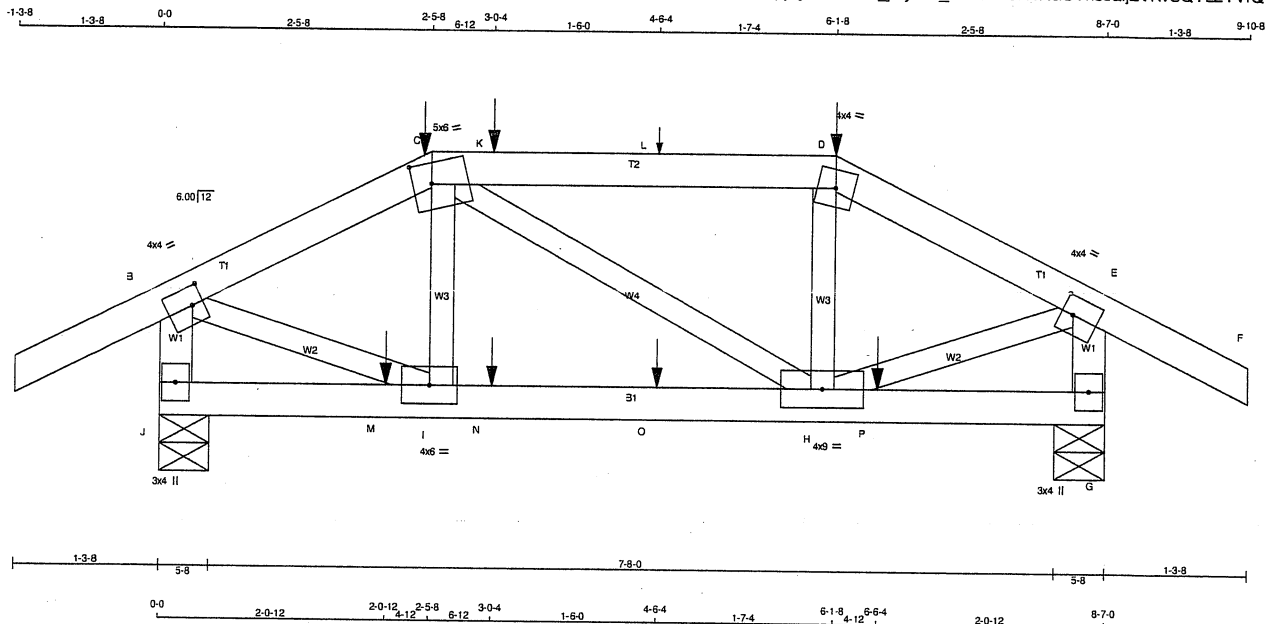


Structural component only
DWG# T-2115781 *en*

JOB NAME 413212	TRUSS NAME T122	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:17 2021 Page 1
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Scale = 1:19.6

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
J	720	0	720	0	5-8	5-8
G	716	0	716	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
J	505	354 / 0	0 / 0	0 / 0	151 / 0	0 / 0	0 / 0
G	502	352 / 0	0 / 0	0 / 0	150 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.13 (1)	I-C	-119 / 16	0.02 (1)	
B-C	-617 / 0	-91.8 -91.8	0.11 (1)	C-H	-6 / 0	0.00 (1)	
C-K	-540 / 0	-91.8 -91.8	0.24 (1)	H-D	-116 / 17	0.02 (1)	
K-L	-540 / 0	-91.8 -91.8	0.24 (1)	B-I	0 / 582	0.14 (1)	
L-D	-540 / 0	-91.8 -91.8	0.24 (1)	H-E	0 / 576	0.14 (1)	
D-E	-611 / 0	-91.8 -91.8	0.11 (1)				
E-F	0 / 28	-91.8 -91.8	0.13 (1)				
J-B	-700 / 0	0.0 0.0	0.08 (1)				
G-E	-695 / 0	0.0 0.0	0.08 (1)				
J-M	0 / 0	-18.5 -18.5	0.04 (4)				
M-I	0 / 0	-18.5 -18.5	0.04 (4)				
I-N	0 / 546	-18.5 -18.5	0.12 (1)				
N-O	0 / 546	-18.5 -18.5	0.12 (1)				
O-H	0 / 546	-18.5 -18.5	0.12 (1)				
H-P	0 / 0	-18.5 -18.5	0.04 (4)				
P-G	0 / 0	-18.5 -18.5	0.04 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-74	-74	---	BACK	VERT	TOTAL	---	C1
D	6-1-8	-74	-74	---	BACK	VERT	TOTAL	---	C1
K	3-0-4	-9	-9	---	BACK	VERT	TOTAL	---	C1
L	4-6-4	1	1	---	BACK	VERT	TOTAL	---	C1
M	2-0-12	-3	-3	---	BACK	VERT	TOTAL	---	C1
N	3-0-4	-1	-1	---	BACK	VERT	TOTAL	---	C1
O	4-6-4	-1	-1	---	BACK	VERT	TOTAL	---	C1
P	6-6-4	-3	-3	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 35 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL =	6.0	PSF
BOT CH.	LL = 0.0	PSF
DL =	7.4	PSF
TOTAL LOAD =	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.24/1.00 (C-D:1), BC=0.12/1.00 (H-I:1), WB=0.14/1.00 (B-I:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

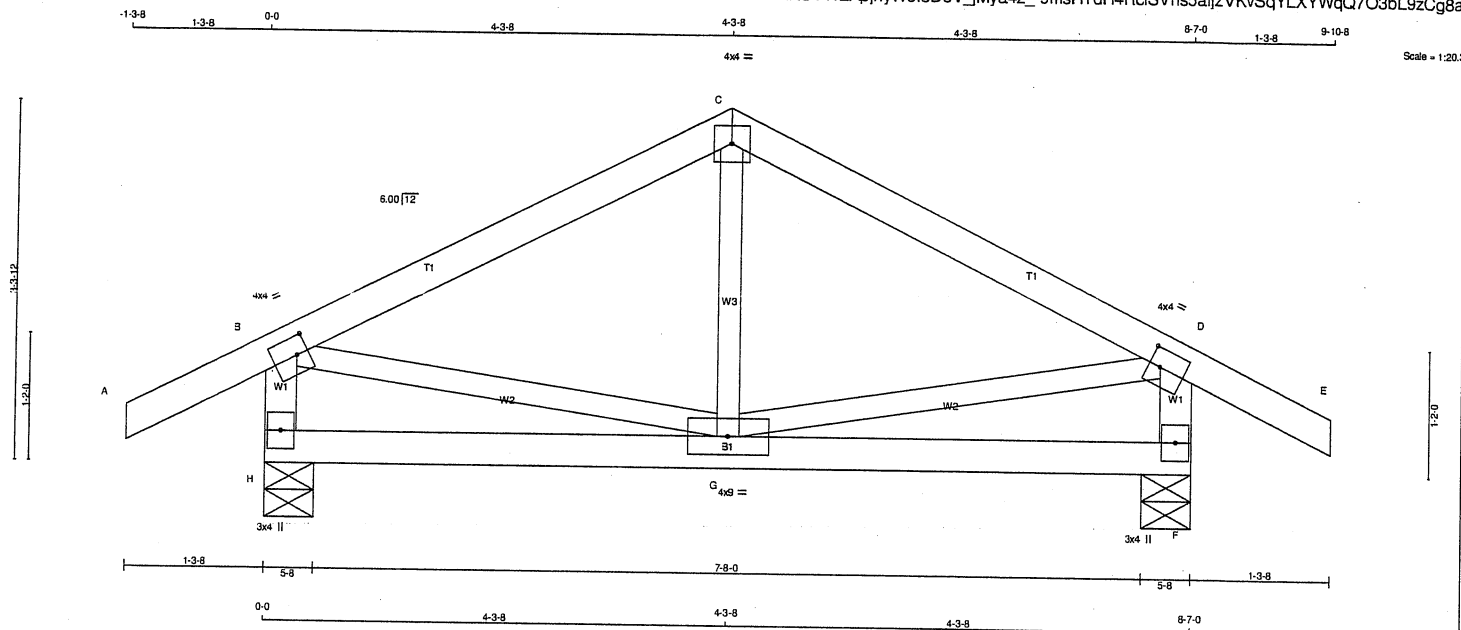
JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115782

JOB NAME 413212	TRUSS NAME T141	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:17 2021 Page 1	

ID:hEKUVWZApjnyW9leD6V_jMya4z_9msHTdH4RciSVhs5ajzVKvSqYLYWqQ7O3bL9zCg8a



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TTW-p	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	2.00 1.25
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW-t	MT20	4.0	9.0	
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	598	0	598	0
F	598	0	598	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	
H	420	289 / 0	0 / 0	0 / 0	131 / 0
F	420	289 / 0	0 / 0	0 / 0	131 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 28	-91.8 -91.8 0.12 (1)	G-C	-44 / 58
B-C	-391 / 0	-91.8 -91.8 0.22 (1)	B-G	0 / 357
C-D	-391 / 0	-91.8 -91.8 0.22 (1)	G-D	0 / 357
D-E	0 / 28	-91.8 -91.8 0.12 (1)		
H-B	-566 / 0	0.0 0.0 0.06 (1)		
F-D	-566 / 0	0.0 0.0 0.06 (1)		
H-G	0 / 0	-18.5 -18.5 0.09 (4)		
G-F	0 / 0	-18.5 -18.5 0.09 (4)		

TOTAL WEIGHT = 35 lb

[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF: PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.22/1.00 (C-D:1), BC=0.09/1.00 (F-G:4),
WB=0.08/1.00 (B-G:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (D) (INPUT = 0.90)
JSI METAL= 0.18 (D) (INPUT = 1.00)

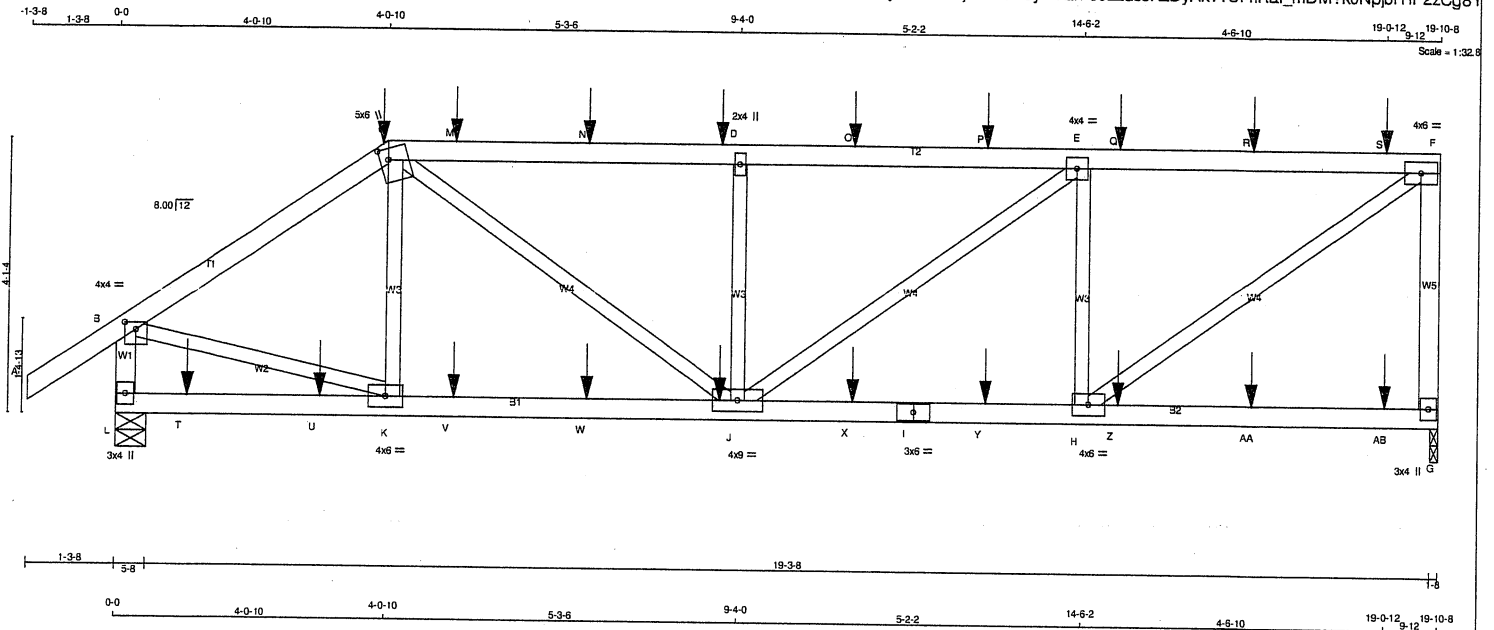


Structural component only
DWG# T-2115783

JOB NAME 413212	TRUSS NAME T161	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:19 2021 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - F	12	SIDE(61.0)
F - G	12	TOP
L - B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L - I	12	SIDE(61.0)
I - G	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
G	1841	0	1841	0	0	1-8	1-8		
L	1947	0	1947	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
G	1303	849 / 0	0 / 0	0 / 0	0 / 0	454 / 0	0 / 0		
L	1375	911 / 0	0 / 0	0 / 0	0 / 0	464 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO	CSI (LC)	LENGTH	FR-TO			
A - B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	K - C	-276 / 35	0.04 (1)	
B - C	-2052 / 0	-91.8	-91.8	0.16 (1)	5.98	B - K	0 / 1761	0.22 (1)	
C - M	-2533 / 0	-91.8	-91.8	0.32 (1)	5.32	H - F	0 / 2472	0.31 (1)	
M - N	-2533 / 0	-91.8	-91.8	0.32 (1)	5.32	C - J	0 / 1030	0.13 (1)	
N - D	-2533 / 0	-91.8	-91.8	0.32 (1)	5.32	H - E	-1242 / 0	0.17 (1)	
D - O	-2533 / 0	-91.8	-91.8	0.35 (1)	5.26	J - D	-830 / 0	0.11 (1)	
O - P	-2533 / 0	-91.8	-91.8	0.35 (1)	5.26	J - E	0 / 644	0.08 (1)	
P - E	-2533 / 0	-91.8	-91.8	0.35 (1)	5.26				
E - Q	-2015 / 0	-91.8	-91.8	0.34 (1)	5.75				
Q - R	-2015 / 0	-91.8	-91.8	0.34 (1)	5.75				
R - S	-2015 / 0	-91.8	-91.8	0.34 (1)	5.75				
S - F	-2015 / 0	-91.8	-91.8	0.34 (1)	5.75				
G - F	-1767 / 0	0.0	0.0	0.24 (1)	7.81				
L - B	-1898 / 0	0.0	0.0	0.11 (1)	7.81				

L - T	0 / 0	-18.5	-18.5	0.09 (4)	10.00
T - U	0 / 0	-18.5	-18.5	0.09 (4)	10.00
U - K	0 / 0	-18.5	-18.5	0.09 (4)	10.00
K - V	0 / 1697	-18.5	-18.5	0.20 (1)	10.00
V - W	0 / 1697	-18.5	-18.5	0.20 (1)	10.00
W - J	0 / 1697	-18.5	-18.5	0.20 (1)	10.00
J - X	0 / 2015	-18.5	-18.5	0.24 (1)	10.00
X - I	0 / 2015	-18.5	-18.5	0.24 (1)	10.00
I - Y	0 / 2015	-18.5	-18.5	0.24 (1)	10.00
Y - H	0 / 2015	-18.5	-18.5	0.24 (1)	10.00
H - Z	0 / 0	-18.5	-18.5	0.12 (4)	10.00
Z - AA	0 / 0	-18.5	-18.5	0.12 (4)	10.00
AA - AB	0 / 0	-18.5	-18.5	0.12 (4)	10.00
AB - G	0 / 0	-18.5	-18.5	0.12 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	4-0-10	-40	-40	---	FRONT	VERT	DEAD	---	C1
C	4-0-10	-175	-175	---	FRONT	VERT	SNOW	---	C1
D	9-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
J	9-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
M	5-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
N	7-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
O	11-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
P	13-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Q	15-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
R	17-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
S	19-0-12	-82	-82	---	BACK	VERT	TOTAL	---	C1

TOTAL WEIGHT = 2 X 80 = 160 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.35/1.00 (D-E:1), BC=0.24/1.00 (H-J:1),
WB=0.31/1.00 (F-H:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1857 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2115784

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T161	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:19 2021 Page 2
ID:5sPQm3ewyQLR2FGi1wwUPeyYVZn-69z2uJKzDyAk??Ui4iRaI mDM?k0NpbYiP2zCq8Y

PLATES (table is in inches)

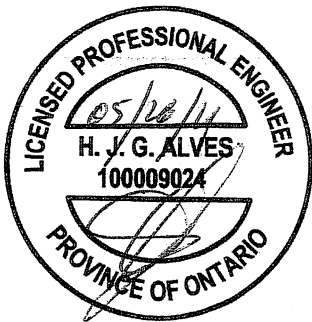
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	1-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
U	3-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
V	5-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
W	7-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
X	11-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	13-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Z	15-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AA	17-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AB	19-0-12	-23	-23	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

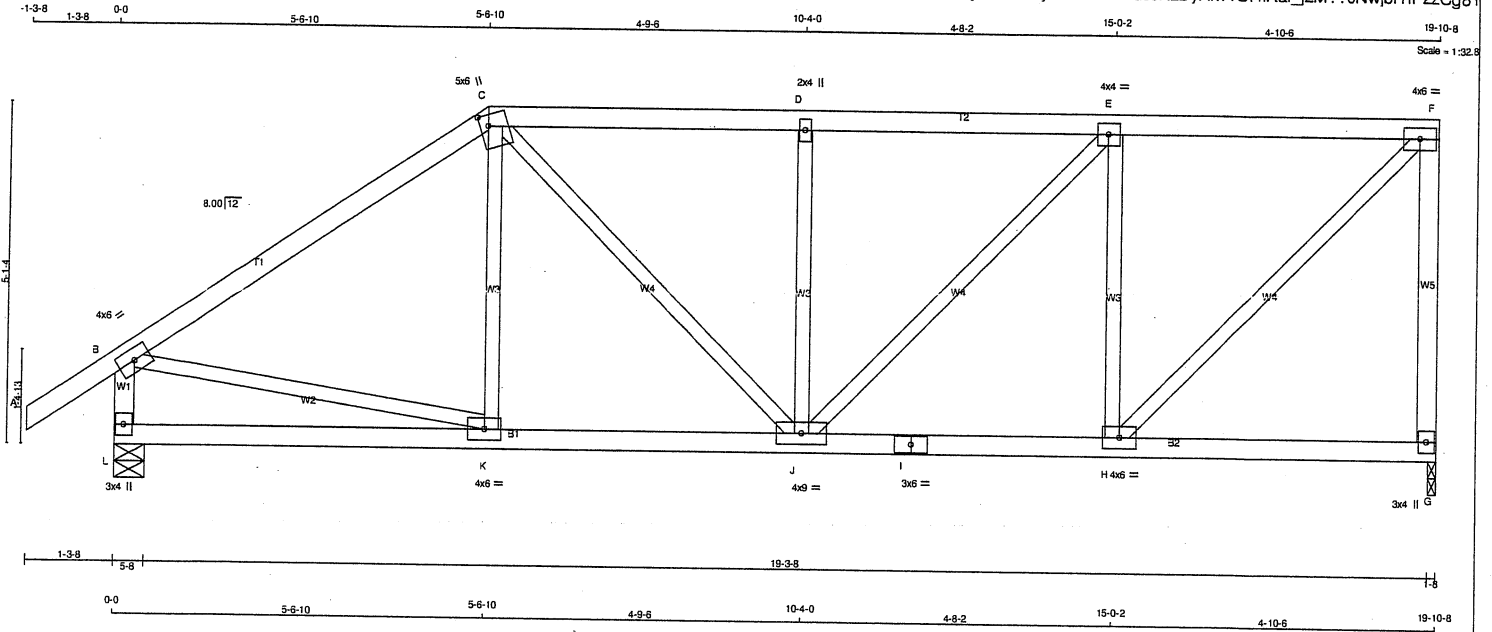


Structural component only
DWG# T-2115784 9/12

JOB NAME 413212	TRUSS NAME T162	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:19 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-69z2uJJKzDyAk??Uj4IRal_j2M??0NwibiYIP2zCg8Y



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	9.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
G	1096	0	0	1-8
L	1222	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	775	508 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0
L	862	579 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC1)	MAX. UNBRACED LENGTH (LC)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. UNBRACED LENGTH (LC1)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	K-C	-64 / 64	0.02 (1)
B-C	-1116 / 0	-91.8	-91.8	0.56 (1)	5.21	B-K	0 / 944	0.21 (1)
C-D	-1172 / 0	-91.8	-91.8	0.30 (1)	5.52	H-F	0 / 1248	0.28 (1)
D-E	-1173 / 0	-91.8	-91.8	0.31 (1)	5.49	C-J	0 / 349	0.08 (1)
E-F	-887 / 0	-91.8	-91.8	0.30 (1)	6.11	H-E	-776 / 0	0.30 (1)
G-F	-1059 / 0	0.0	0.0	0.47 (1)	7.65	J-D	-468 / 0	0.18 (1)
L-B	-1179 / 0	0.0	0.0	0.12 (1)	7.35	J-E	0 / 410	0.09 (1)
L-K	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
K-J	0 / 926	-18.5	-18.5	0.22 (1)	10.00			
J-I	0 / 887	-18.5	-18.5	0.20 (1)	10.00			
I-H	0 / 887	-18.5	-18.5	0.20 (1)	10.00			
H-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

TOTAL WEIGHT = 86 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.56/1.00 (B-C:1), BC=0.22/1.00 (J-K:1), WB=0.30/1.00 (E-H:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.31 (I) (INPUT = 1.00)

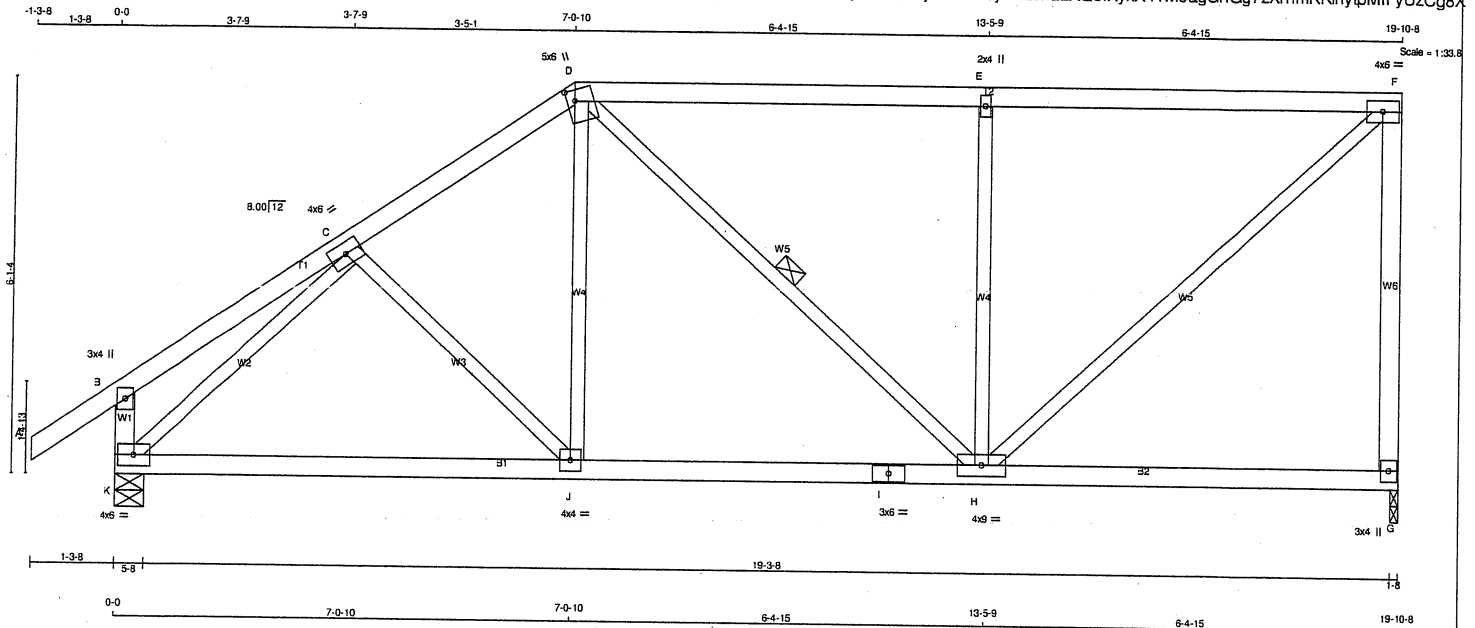


Structural component only
DWG# T-2115785

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T163	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:20 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-aLXQ6fKyX41M9agGnGg7zXrhmkKinytpMIFyUzCg8X



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1096	0	1096	0
K	1222	0	1222	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE
G	775	508 / 0	0 / 0	0 / 0
K	862	579 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	C-J	-93 / 16	0.04 (1)
B-C	0 / 20	-91.8	-91.8 0.17 (1)	10.00	J-D	0 / 203	0.05 (4)
C-D	-1069 / 0	-91.8	-91.8 0.19 (1)	5.87	D-H	-5 / 49	0.01 (1)
D-E	-911 / 0	-91.8	-91.8 0.69 (1)	5.25	H-E	-729 / 0	0.43 (1)
E-F	-911 / 0	-91.8	-91.8 0.69 (1)	5.25	F-F	0 / 1226	0.28 (1)
G-F	-1046 / 0	0.0	0.0 0.75 (1)	7.69	K-C	-1312 / 0	0.50 (1)
K-B	-252 / 0	0.0	0.0 0.03 (1)	7.81			
K-J	0 / 939	-18.5	-18.5 0.28 (4)	10.00			
J-I	0 / 875	-18.5	-18.5 0.28 (4)	10.00			
I-H	0 / 875	-18.5	-18.5 0.28 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.16 (4)	10.00			

TOTAL WEIGHT = 86 b [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.75/1.00 (F-G:1), BC=0.28/1.00 (H-J:4),
WB=0.50/1.00 (C-K:1), SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (F) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

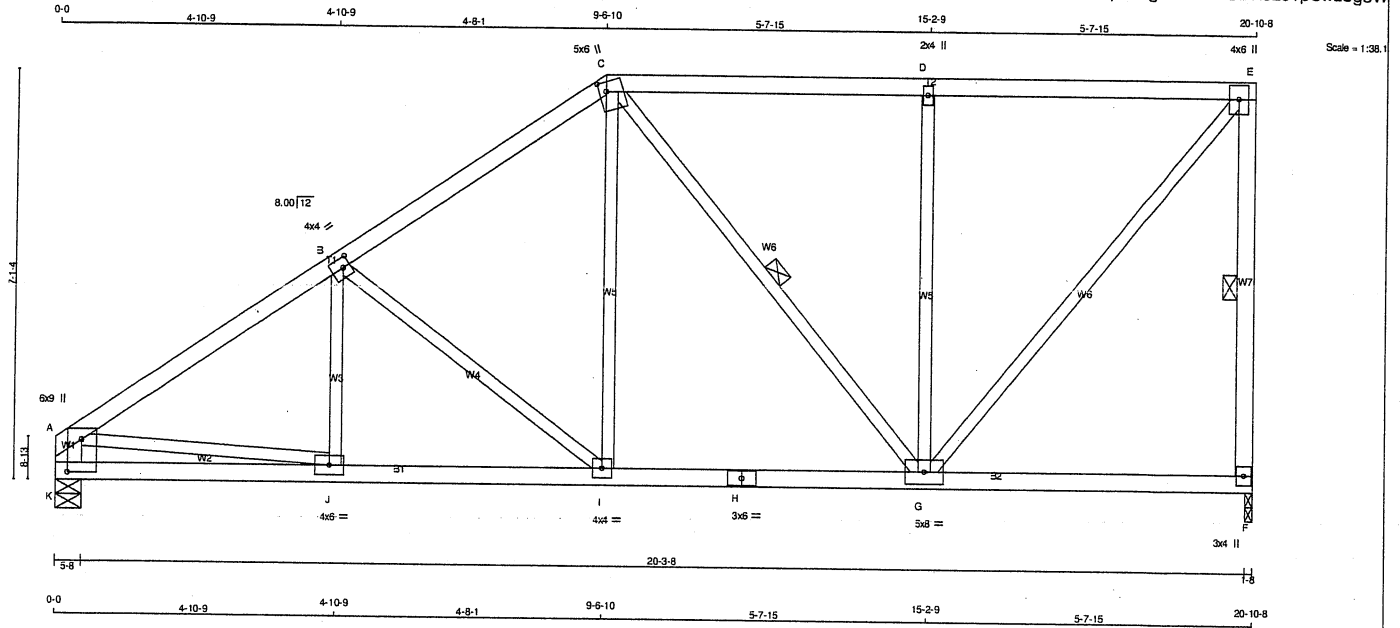


Structural component only
DWG# T-2115786

JOB NAME 413212	TRUSS NAME T164	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:21 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-2Y5oJ?KaVrCuzI9spVovgA4389h1UDH0201pUwzCg8W



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
K - A	2x6	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A						
B	TMBMVW1*+p	MT20	6.0	9.0	6.75	3.00
C	TMBVVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMW+W+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVVW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMVVW-t	MT20	4.0	4.0		
K	BMVVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	1151	0	1151	0	1-8	1-8
K	1151	0	1151	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
K	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-1471 / 0	-91.8	-91.8 0.41 (1)	4.93	J-B	-13 / 74	0.03 (4)
B-C	-1085 / 0	-91.8	-91.8 0.38 (1)	5.56	B-I	-467 / 0	0.29 (1)
C-D	-760 / 0	-91.8	-91.8 0.52 (1)	6.03	I-C	0 / 378	0.09 (1)
D-E	-761 / 0	-91.8	-91.8 0.52 (1)	6.03	C-G	-192 / 0	0.10 (1)
F-E	-1108 / 0	0.0	0.0 0.25 (1)	6.02	G-D	-643 / 0	0.56 (1)
K-A	-1111 / 0	0.0	0.0 0.07 (1)	7.81	G-E	0 / 1185	0.27 (1)
					A-J	0 / 1252	0.28 (1)
K-J	0 / 0	-18.5	-18.5 0.09 (4)	10.00			
J-I	0 / 1247	-18.5	-18.5 0.25 (1)	10.00			
I-H	0 / 883	-18.5	-18.5 0.22 (4)	10.00			
H-G	0 / 883	-18.5	-18.5 0.22 (4)	10.00			
G-F	0 / 0	-18.5	-18.5 0.15 (4)	10.00			

TOTAL WEIGHT = 93 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.52/1.00 (D-E:1), BC=0.25/1.00 (I-J:1), WB=0.56/1.00 (D-G:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (E) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)

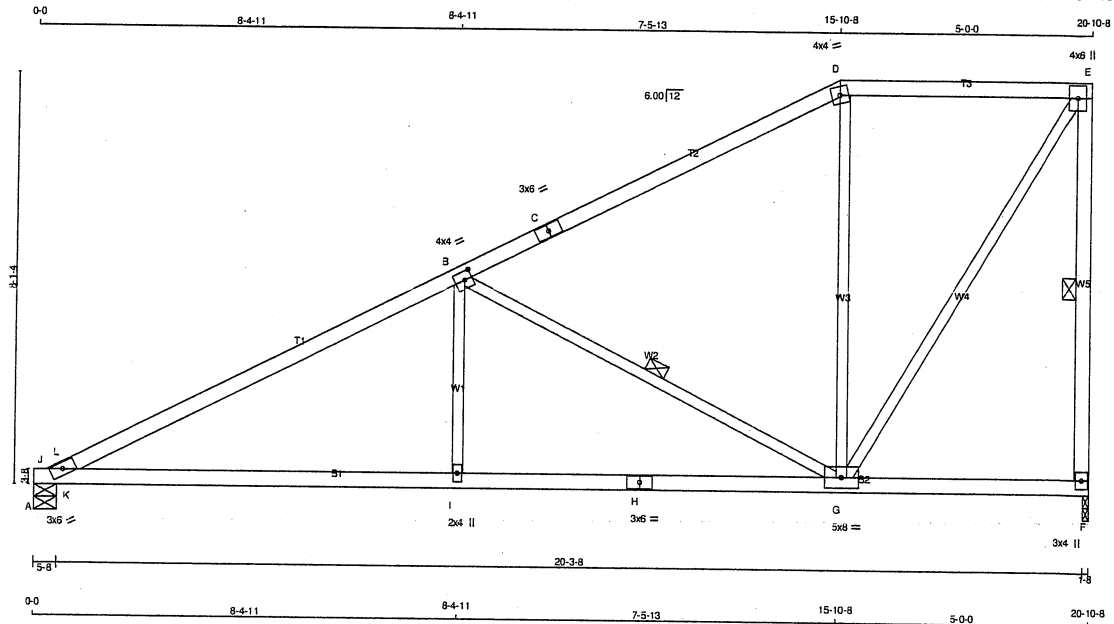


Structural component only
DWG# T-2115787

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T165	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:22 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-WkfAWLLDG8KlbSk3NCJ8COc9cZurDfW9HgnM0NzCg8V



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
J - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	6.0		
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TS-t	MT20	3.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMV+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1151	0	1151	0	0	1-8	1-8
A	1128	0	1128	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
A	798	521 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, B-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
J-L	-1861 / 0	-91.8	-91.8	0.14 (4)	4.85	I-B	0 / 244	0.07 (4)	
L-B	-1793 / 0	-91.8	-91.8	0.85 (1)	3.75	B-G	-1231 / 0	0.63 (1)	
B-C	-662 / 0	-91.8	-91.8	0.72 (1)	5.81	G-D	-206 / 12	0.26 (1)	
C-D	-662 / 0	-91.8	-91.8	0.72 (1)	5.81	G-E	0 / 1042	0.23 (1)	
D-E	-554 / 0	-91.8	-91.8	0.30 (1)	6.25	K-L	-216 / 107	0.00 (1)	
F-E	-1112 / 0	0.0	0.0	0.33 (1)	6.01				
A-J	0 / 0	-18.5	-18.5	0.38 (1)	10.00				
J-K	0 / 1642	-18.5	-18.5	0.79 (1)	10.00				
K-I	0 / 1642	-18.5	-18.5	0.79 (1)	10.00				
I-H	0 / 1642	-18.5	-18.5	0.56 (1)	10.00				
H-G	0 / 1642	-18.5	-18.5	0.56 (1)	10.00				
G-F	0 / 0	-18.5	-18.5	0.14 (4)	10.00				

TOTAL WEIGHT = 5 X 81 = 406 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL)= L/999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL)= L/529 (0.47")

CSI: TC=0.85/1.00 (B-L:1), BC=0.79/1.00 (J-K:1), WB=0.63/1.00 (B-G:1), SSI=0.88/1.00 (A-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

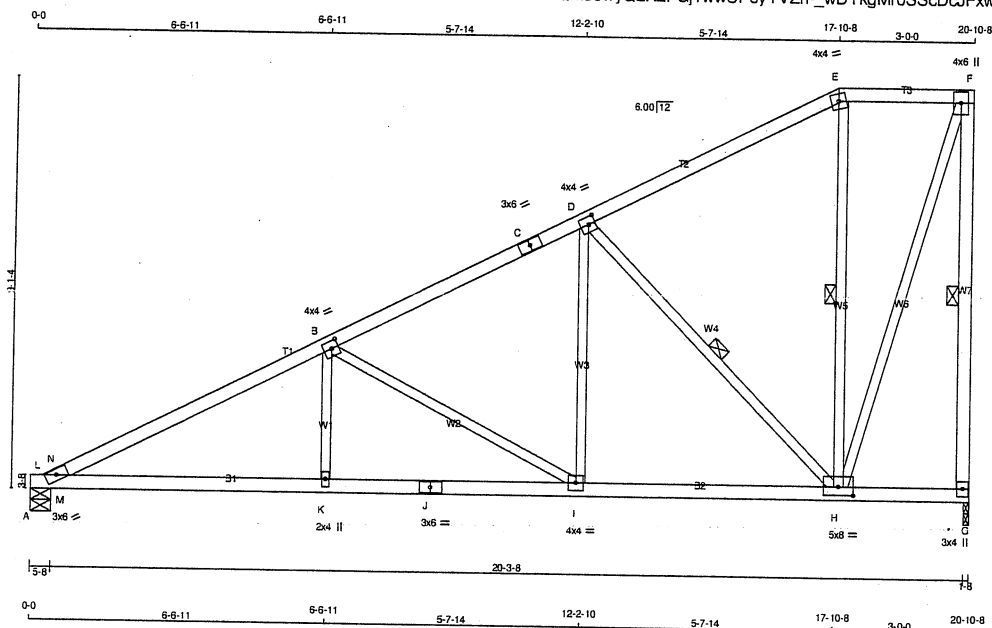
JSI GRIP= 0.77 (A) (INPUT = 0.90)
JSI METAL= 0.55 (A) (INPUT = 1.00)



Structural component only
DWG# T-2115788

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:23 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPayYVZn-wDYkgMr0SScDcJFwxqNlb9P0zE7y5AJWKWvYpzCg8U



TOTAL WEIGHT = 92 lb
(M/F)

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
L - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
A - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY			No.2	SPF
DRY: SEASONED LUMBER				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
G	1151	0	0	1151	0	0	1-8	1-8	
A	1128	0	0	1128	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	814	534 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
A	798	521 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX (LC)	
FR-TO		FROM	TO	FR-TO			
L-N	-2058 / 0	-91.8	-91.8 0.14 (1)	4.62	K-B	0 / 204	0.05 (4)
N-B	-1975 / 0	-91.8	-91.8 0.49 (1)	4.29	B-I	-875 / 0	0.67 (1)
B-C	-1153 / 0	-91.8	-91.8 0.36 (1)	5.49	I-D	0 / 509	0.11 (1)
C-D	-1153 / 0	-91.8	-91.8 0.36 (1)	5.49	D-H	-1005 / 0	0.47 (1)
D-E	-392 / 0	-91.8	-91.8 0.33 (1)	6.25	H-E	-182 / 0	0.10 (1)
E-F	-328 / 0	-91.8	-91.8 0.11 (1)	6.25	H-F	0 / 1052	0.24 (1)
G-F	-1136 / 0	0.0	0.0 0.44 (1)	5.96	M-N	-95 / 93	0.00 (1)
A-L	0 / 0	-18.5	-18.5 0.38 (1)	10.00			
L-M	0 / 1794	-18.5	-18.5 0.72 (1)	10.00			
M-K	0 / 1794	-18.5	-18.5 0.72 (1)	10.00			
K-J	0 / 1794	-18.5	-18.5 0.47 (1)	10.00			
J-I	0 / 1794	-18.5	-18.5 0.47 (1)	10.00			
I-H	0 / 1029	-18.5	-18.5 0.23 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL)= L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL)= L/930 (0.27")

CSI: TC=0.49/1.00 (B-N:1), BC=0.72/1.00 (L-M:1), WB=0.67/1.00 (B-I:1), SS=0.88/1.00 (A-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.60 (A) (INPUT = 1.00)



Structural component only
DWG# T-2115789

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T167	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:24 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	16-8-12	-168	-168	---	FRONT	VERT	TOTAL	---	C1
Y	18-2-12	-171	-171	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

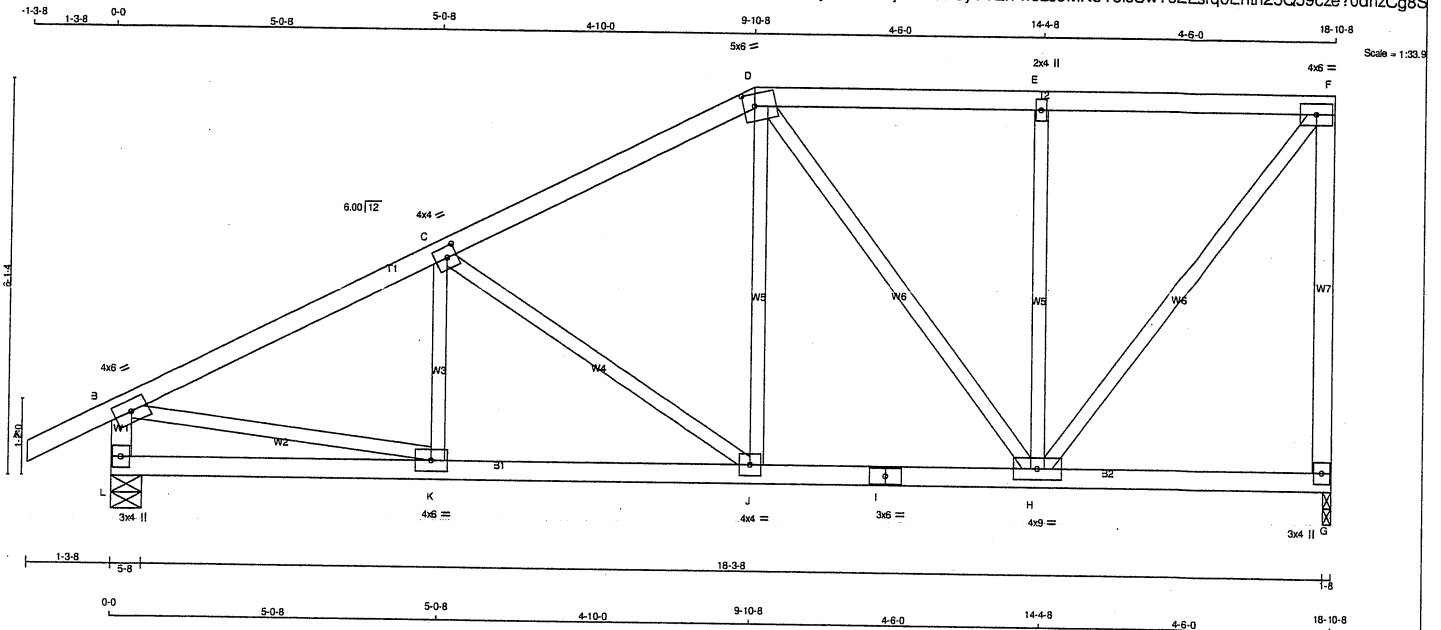


Structural component only
DWG# T-2115790 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T168	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	JT	DOWN	IN-SX		IN-SX	
G	1041	0	1041	0	1-8	1-8	
L	1165	0	1165	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	736	483 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
G	822	552 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE	MEMB.	FORCE
MAX. FACTORED		MAX. FACTORED	
FORCE		FORCE	
(LBS)		(LBS)	
FR-TO		FR-TO	
A-B	0 / 28	K-C	-109 / 47
B-C	-1347 / 0	C-J	-463 / 0
C-D	-968 / 0	J-D	0 / 346
D-E	-660 / 0	D-H	-311 / 0
E-F	-660 / 0	H-E	-510 / 0
G-F	-1005 / 0	H-F	0 / 1073
L-B	-1125 / 0	B-K	0 / 1243
L-K	0 / 0		
K-J	0 / 1225		
J-I	0 / 850		
I-H	0 / 850		
H-G	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.73/1.00 (F-G:1), BC=0.24/1.00 (J-K:1),
WB=0.31/1.00 (D-H:1), SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 1.00)

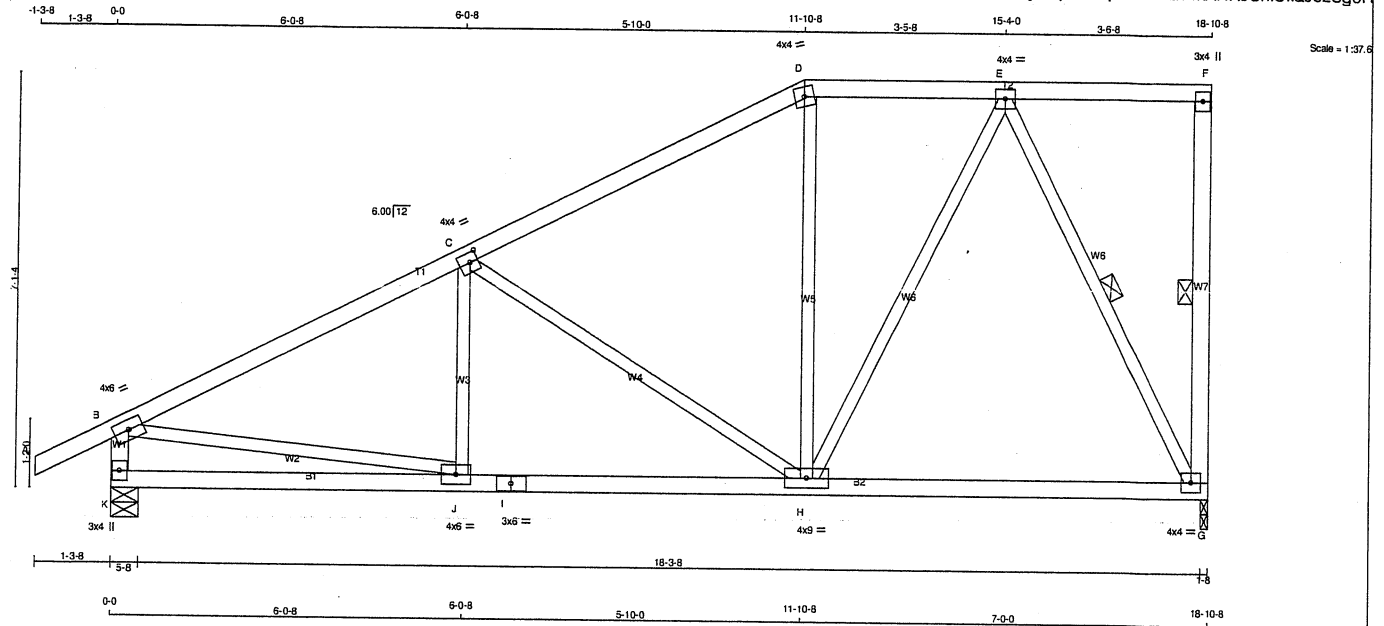


Structural component only
DWG# T-2115791

JOB NAME 413212	TRUSS NAME T169	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 85 lb [M/F]

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS				SPF	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
G	1041	0	0	1041	0	0	1-8	1-8	
K	1165	0	0	1165	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	736	483 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
G	822	552 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
FR-TO						FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	J-C	-60 / 72	0.02 (4)		
B-C	-1329 / 0	-91.8	-91.8 0.45 (1)	5.07	C-H	-640 / 0	0.82 (1)		
C-D	-790 / 0	-91.8	-91.8 0.42 (1)	6.20	H-D	0 / 61	0.02 (4)		
D-E	-679 / 0	-91.8	-91.8 0.15 (1)	6.25	H-E	0 / 521	0.12 (1)		
E-F	0 / 0	-91.8	-91.8 0.18 (1)	10.00	E-G	-974 / 0	0.38 (1)		
G-F	-124 / 0	0.0	0.0 0.03 (1)	6.25	B-J	0 / 1227	0.28 (1)		
K-B	-1117 / 0	0.0	0.0 0.11 (1)	7.50					
K-J	0 / 0	-18.5	-18.5 0.14 (4)	10.00					
J-I	0 / 1215	-18.5	-18.5 0.31 (1)	10.00					
I-H	0 / 1215	-18.5	-18.5 0.31 (1)	10.00					
H-G	0 / 449	-18.5	-18.5 0.23 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.45/1.00 (B-C:1), BC=0.31/1.00 (H-J:1),
WB=0.62/1.00 (C-H:1), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.71 (J) (INPUT = 0.90)
JSI METAL = 0.37 (I) (INPUT = 1.00)

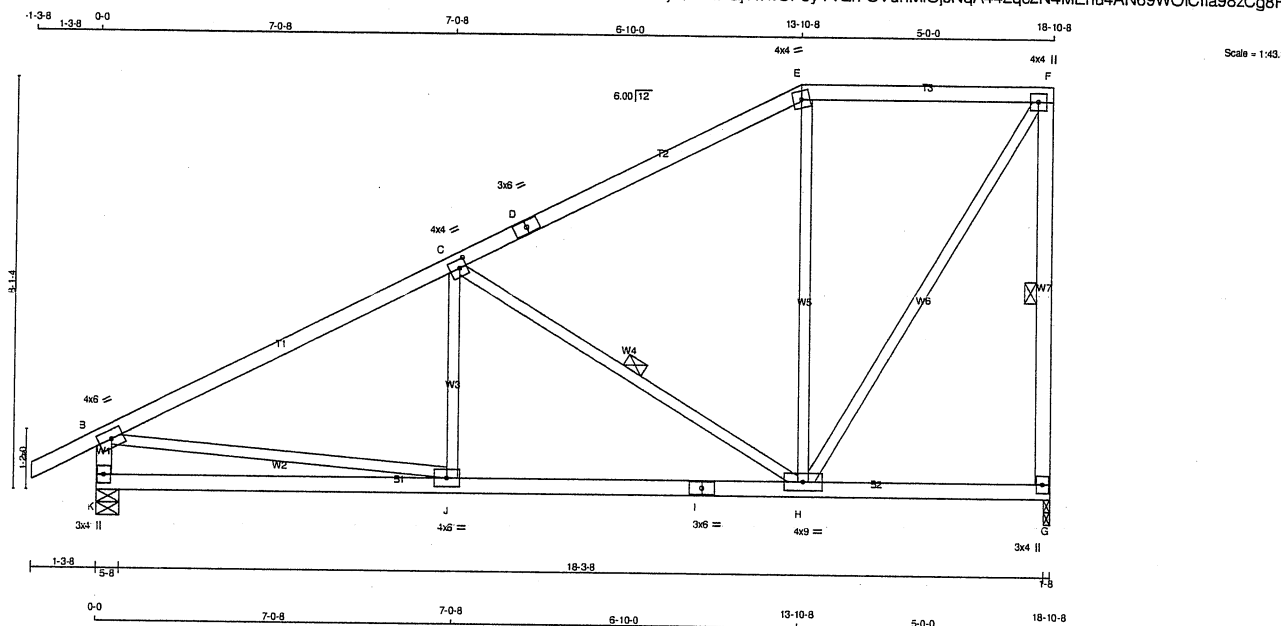


Structural component only
DWG# T-2115792

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T170	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:26 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-OVuhMiOjNqA442qc2N4MMenu4AN69WOIC1a98zCg8R



TOTAL WEIGHT = 3 X 85 = 254 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
G	1041	0	1041	0	1-8	1-8
K	1165	0	1165	0	5-8	5-8

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	736	483 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
K	822	552 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
		VERT. LOAD (PLF)	LC1 MAX (LC)				
FR-TO		FROM	TO		FR-TO		
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	J-C	0 / 116	0.04 (4)
B-C	-1298 / 0	-91.8	-91.8 0.63 (1)	4.77	C-H	-829 / 0	0.38 (1)
C-D	-584 / 0	-91.8	-91.8 0.58 (1)	6.25	H-E	-219 / 0	0.28 (1)
D-E	-584 / 0	-91.8	-91.8 0.58 (1)	6.25	H-F	0 / 918	0.21 (1)
E-F	-488 / 0	-91.8	-91.8 0.30 (1)	6.25	B-J	0 / 1200	0.27 (1)
G-F	-1007 / 0	0.0	0.0 0.30 (1)	6.24			
K-B	-1112 / 0	0.0	0.0 0.11 (1)	7.51			
K-J	0 / 0	-18.5	-18.5 0.22 (4)	10.00			
J-I	0 / 1192	-18.5	-18.5 0.32 (4)	10.00			
I-H	0 / 1192	-18.5	-18.5 0.32 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.63/1.00 (B-C:1), BC=0.32/1.00 (H-J:4),
WB=0.38/1.00 (C-H:1), SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

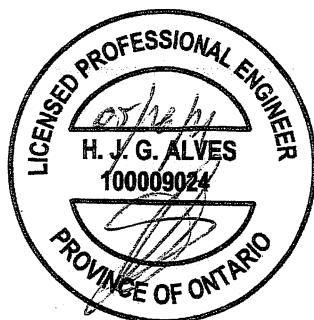
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.40 (I) (INPUT = 1.00)

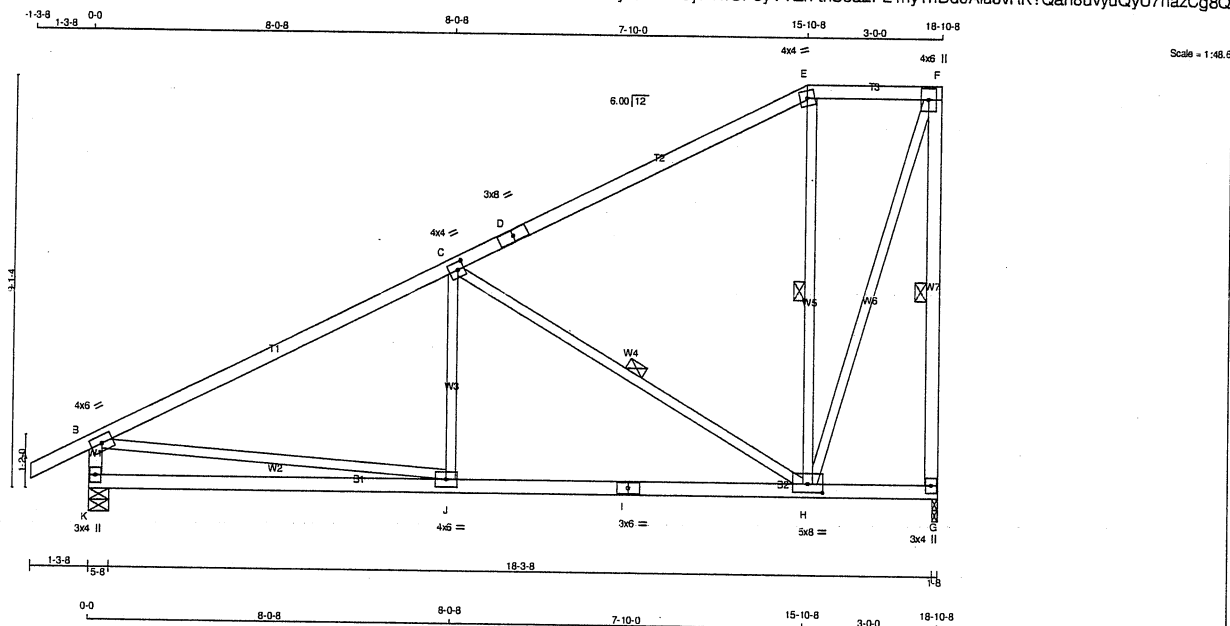


Structural component only
DWG# T-2115793

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T171	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:27 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-thS3a2PL4hy1hDd0AluJvRK?Qah8uvyUyQyU7hazCg8Q



TOTAL WEIGHT = 89 lb
(M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
G	1041	0	1041	0	1-8	1-8
K	1165	0	1165	0	5-8	5-8

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	736	483 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
K	822	552 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	J-C	0 / 151	0.05 (4)	
B-C	-1252 / 0	-91.8 -91.8	0.85 (1)	C-H	-1006 / 0	0.62 (1)	
C-D	-373 / 0	-91.8 -91.8	0.77 (1)	H-E	-258 / 0	0.14 (1)	
D-E	-373 / 0	-91.8 -91.8	0.77 (1)	H-F	0 / 943	0.21 (1)	
E-F	-295 / 0	-91.8 -91.8	0.11 (1)	B-J	0 / 1161	0.26 (1)	
G-F	-1033 / 0	0.0 0.0	0.40 (1)				
K-B	-1106 / 0	0.0 0.0	0.11 (1)				
K-J	0 / 0	-18.5 -18.5	0.30 (4)	10.00			
J-I	0 / 1155	-18.5 -18.5	0.40 (4)	10.00			
I-H	0 / 1155	-18.5 -18.5	0.40 (4)	10.00			
H-G	0 / 0	-18.5 -18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.63")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.63")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: $TC=0.85/1.00$ (B-C:1), $BC=0.40/1.00$ (H-J:4),
 $WB=0.82/1.00$ (C-H:1), $SSI=0.32/1.00$ (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

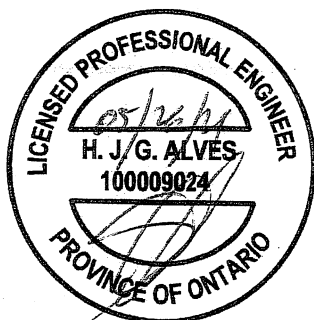
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.77 (H) (INPUT = 0.90)
JSI METAL = 0.48 (I) (INPUT = 1.00)

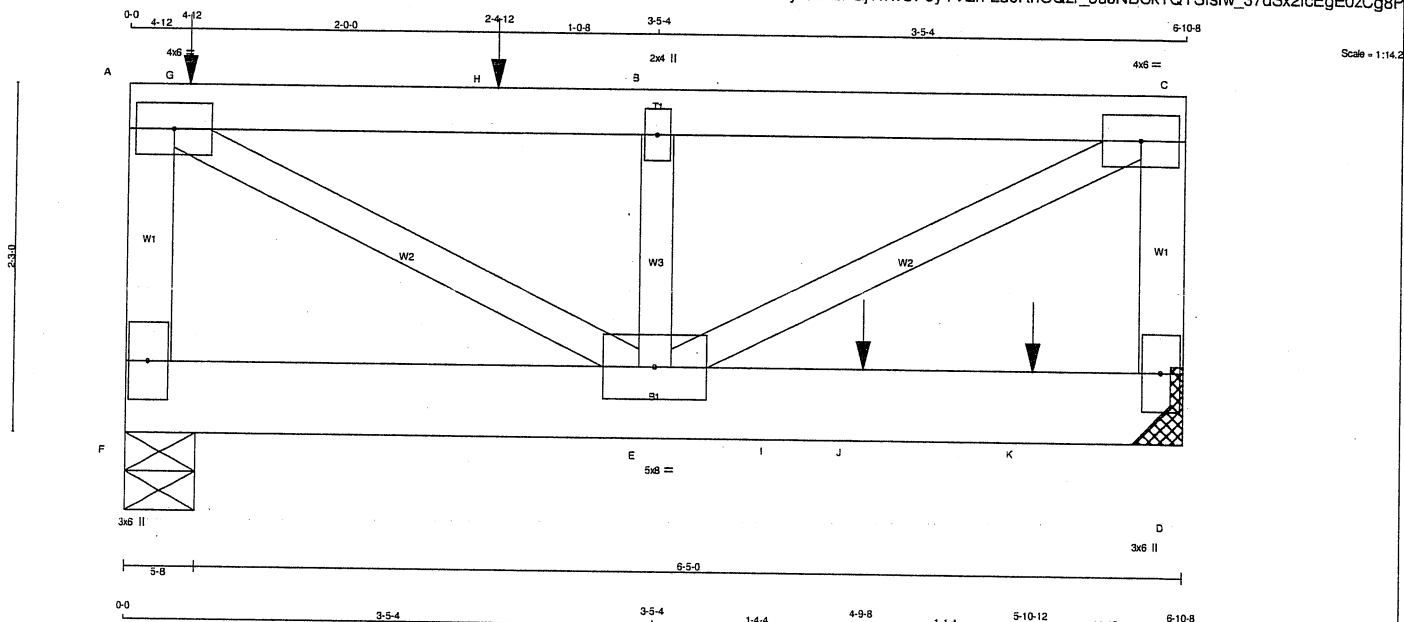


Structural component only
DWG# T-2115794

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T172	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:28 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-Lu0RnOQzr_5uJNBCKTQYSfslw_37dSx2fcEgEOzCg8P



CHORDS	SIZE	DRY	NO.2	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(17.5)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	
B	TMW+w	MT20	2.0	4.0	
C	TMVW-t	MT20	4.0	6.0	
D	BMV1+p	MT20	3.0	6.0	
E	BMVWW-t	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	UPLIFT	IN-SX
F 1799	0	1799	0
D 1622	0	1622	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN.	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
F 1281	793 / 0	0 / 0
D 1154	716 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED			WEBS		
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO		FROM	TO	UNBRAC LENGTH	FR-TO		
F-A	-1762 / 0	0.0	0.0	0.11 (1)	7.81	A-E	0 / 2001
A-G	-1757 / 0	-91.8	-91.8	0.35 (1)	6.00	E-B	-1059 / 0
G-H	-1757 / 0	-91.8	-91.8	0.35 (1)	6.00	E-C	0 / 2001
H-B	-1757 / 0	-91.8	-91.8	0.35 (1)	6.00		
B-C	-1757 / 0	-91.8	-91.8	0.27 (1)	6.15		
D-C	-1017 / 0	0.0	0.0	0.06 (1)	7.81		
F-E	0 / 0	-49.7	-49.7	0.06 (1)	10.00		
E-I	0 / 0	-49.7	-49.7	0.28 (1)	10.00		
I-J	0 / 0	-18.5	-18.5	0.28 (1)	10.00		
J-K	0 / 0	-18.5	-18.5	0.28 (1)	10.00		
K-D	0 / 0	-18.5	-18.5	0.28 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	4-12	-428	-428	---	TOP	VERT	TOTAL	---	C1
H	2-4-12	-482	-482	---	TOP	VERT	TOTAL	---	C1
J	4-9-8	-598	-598	---	BACK	VERT	TOTAL	---	C1
K	5-10-12	-284	-284	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.35/1.00 (A-B:1), BC=0.28/1.00 (D-E:1),
WB=0.25/1.00 (A-E:1), SSI=0.34/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (E) (INPUT = 0.90)
JSI METAL= 0.22 (A) (INPUT = 1.00)



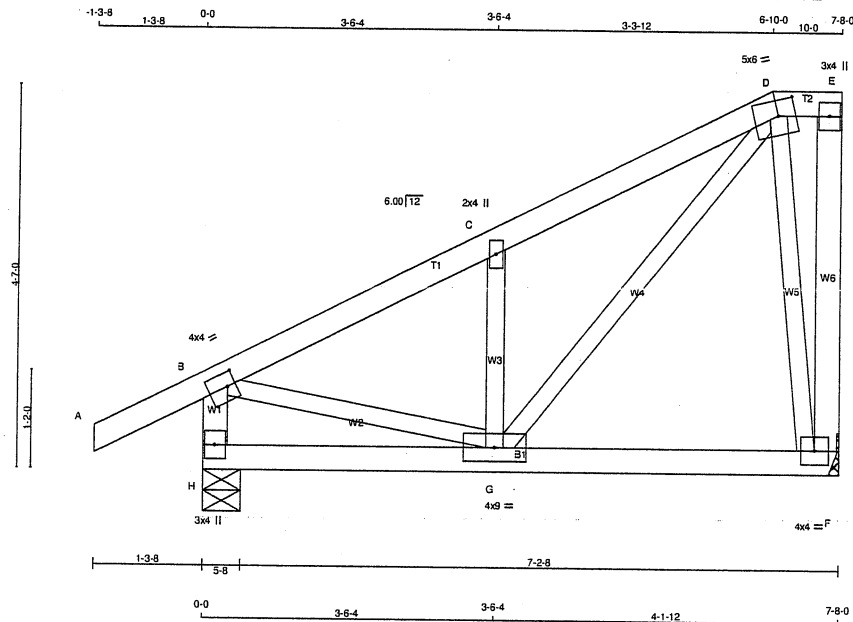
Structural component only
DWG# T-2115795

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T173	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-p4ap_kRcdDxXmPHAxn_sPWAORcMxIBuGzEmTzCg80



Scale = 1/25

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	4.0	4.0 2.00 1.25
C	TMVW-w	MT20	2.0	4.0
D	TTWW-m	MT20	5.0	6.0 2.25 2.50
E	TMVW-p	MT20	3.0	4.0
F	BMVW1-t	MT20	4.0	4.0
G	BMVWW-t	MT20	4.0	9.0
H	BMV1-p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	423	0	423	0	0	MECHANICAL
H	547	0	547	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	DOWN			
F	299	196 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
H	385	266 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	G-C	-377 / 0	0.07 (1)
B-C	-396 / 0	-91.8 -91.8	0.13 (1)	6.25	B-G	0 / 378	0.09 (1)
C-D	-406 / 0	-91.8 -91.8	0.13 (1)	6.25	D-F	-360 / 0	0.11 (1)
D-E	0 / 0	-91.8 -91.8	0.01 (1)	10.00	G-D	0 / 466	0.10 (1)
F-E	-37 / 0	0.0 0.0	0.01 (1)	7.81			
H-B	-520 / 0	0.0 0.0	0.05 (1)	7.81			
H-G	0 / 0	-18.5 -18.5	0.07 (4)	10.00			
G-F	0 / 66	-18.5 -18.5	0.08 (4)	10.00			

TOTAL WEIGHT = 38 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.26")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.26")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.13/1.00 (C-D:1), BC=0.08/1.00 (F-G:4), WB=0.11/1.00 (D-F:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.62 (B) (INPUT = 0.90)
JSI METAL = 0.18 (B) (INPUT = 1.00)

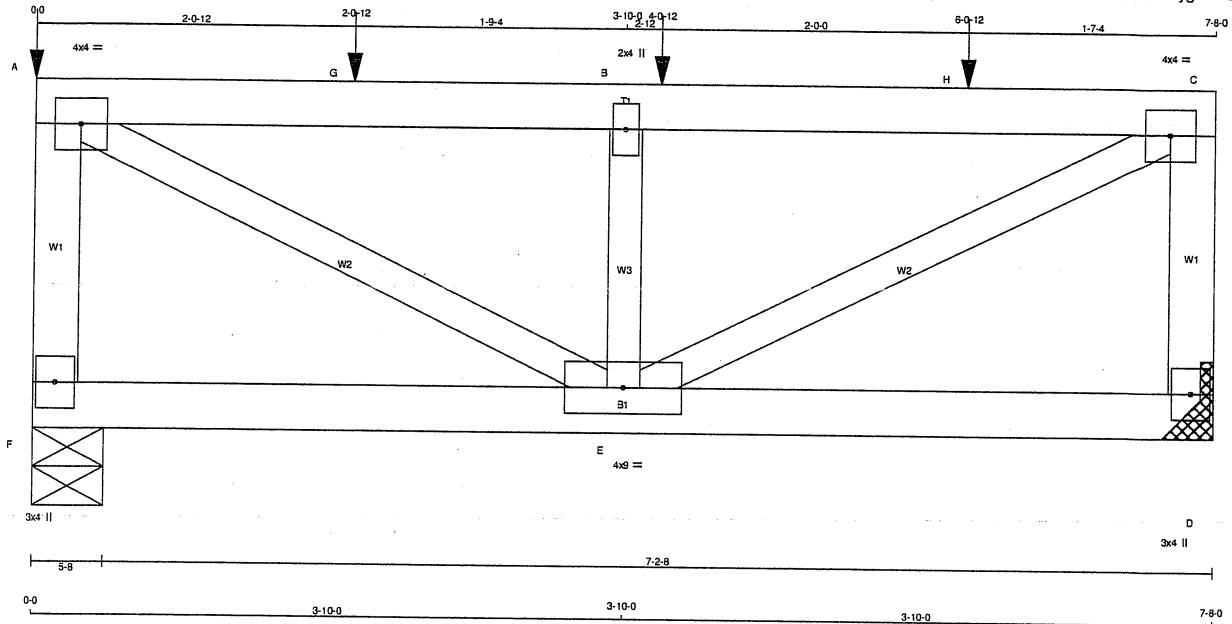


Structural component only
DWG# T-2115796

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T174	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 29 = 58 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 1	12	SIDE(17.5)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMVW-w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMV1+P	MT20	3.0	4.0		
E	BMVWVW-t	MT20	4.0	9.0		
F	BMV1+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1054	0	1054	0
D	852	0	852	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0".

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE
F	754	446 / 0	0 / 0	0 / 0
D	612	347 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
(LBS)	(LBS)	MAX	(LBS)	MAX
		FROM TO		FROM TO
FR-TO				
F-A	-973 / 0	0.0	0.0	0.06 (1)
A-G	-1076 / 0	-91.8	-91.8	0.21 (1)
G-B	-1076 / 0	-91.8	-91.8	0.21 (1)
B-H	-1076 / 0	-91.8	-91.8	0.21 (1)
H-C	-1076 / 0	-91.8	-91.8	0.21 (1)
D-C	-771 / 0	0.0	0.0	0.05 (1)
F-E	0 / 0	-49.7	-49.7	0.10 (4)
E-D	0 / 0	-49.7	-49.7	0.10 (4)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-158	-158	---	TOP	VERT	TOTAL	---	C1
B	4-0-12	-140	-140	---	TOP	VERT	TOTAL	---	C1
G	2-0-12	-140	-140	---	TOP	VERT	TOTAL	---	C1
H	6-0-12	-140	-140	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.21/1.00 (A-B:1), BC=0.10/1.00 (D-E:4), WB=0.15/1.00 (A-E:1), SSI=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

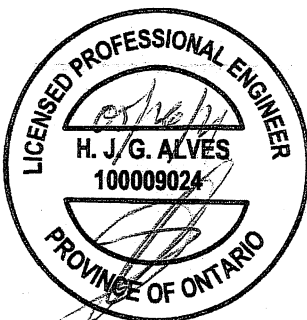
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

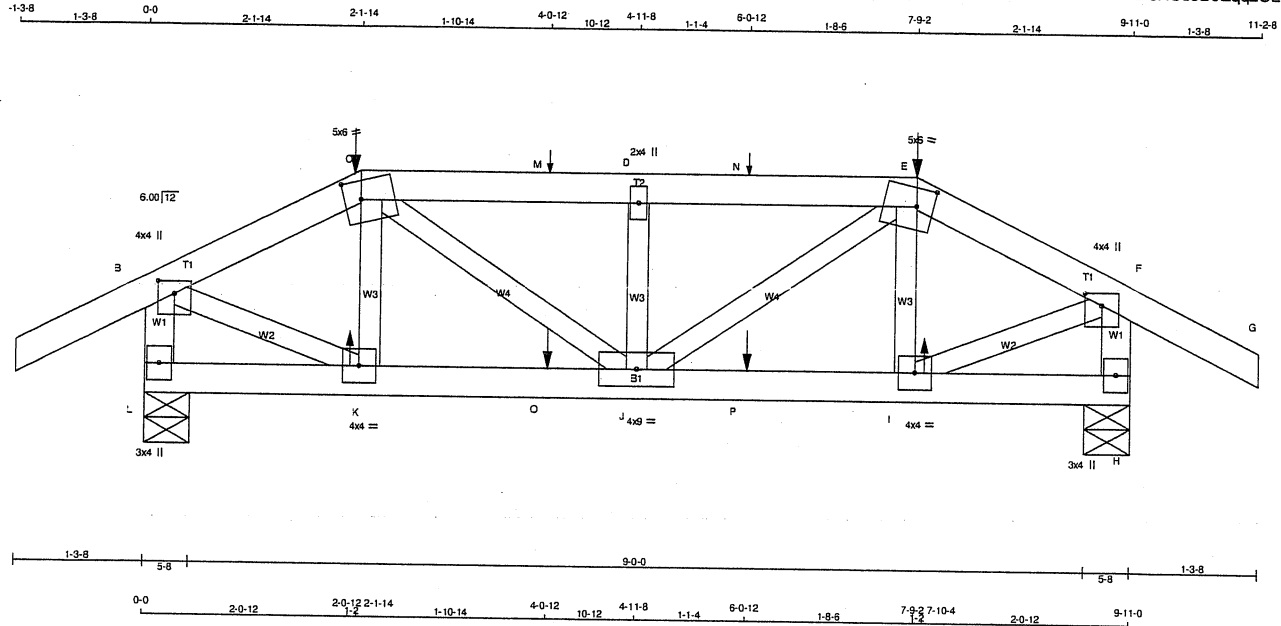
JSI GRIP= 0.62 (C) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 1.00)



Structural component only
DWG# T-2115797

JOB NAME 413212	TRUSS NAME T175	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.50 2.00
C	TTWW-m	MT20	5.0	6.0	2.25 2.00
D	TMVW-w	MT20	2.0	4.0	
E	TTWW-m	MT20	5.0	6.0	2.25 2.00
F	TMVW+p	MT20	4.0	4.0	1.50 2.00
H	BMV1+p	MT20	3.0	4.0	
I	BMVW-t	MT20	4.0	4.0	
J	BMVW-t	MT20	4.0	9.0	
K	BMVW-t	MT20	4.0	4.0	
L	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
L		776	0	776	0	0	5-8	5-8	5-8
H		776	0	776	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	545	378 / 0	0 / 0	0 / 0	0 / 0	167 / 0	0 / 0
H	545	378 / 0	0 / 0	0 / 0	0 / 0	167 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0 / 28	-91.8	-91.8 0.13 (1)	K-C	-178 / 0
B-C	-678 / 0	-91.8	-91.8 0.09 (1)	C-J	0 / 303
C-M	-845 / 0	-91.8	-91.8 0.12 (1)	J-D	-303 / 0
M-D	-845 / 0	-91.8	-91.8 0.12 (1)	D-E	0 / 304
D-N	-845 / 0	-91.8	-91.8 0.12 (1)	E-F	-177 / 0
N-E	-845 / 0	-91.8	-91.8 0.12 (1)	F-G	0 / 648
E-F	-678 / 0	-91.8	-91.8 0.09 (1)	G-H	0 / 647
F-G	0 / 28	-91.8	-91.8 0.13 (1)		
L-B	-759 / 0	0.0	0.0 0.08 (1)		
H-F	-758 / 0	0.0	0.0 0.08 (1)		
L-K	0 / 0	-18.5	-18.5 0.03 (4)		
K-O	0 / 596	-18.5	-18.5 0.13 (1)		
O-J	0 / 596	-18.5	-18.5 0.13 (1)		
J-P	0 / 596	-18.5	-18.5 0.13 (1)		
P-I	0 / 596	-18.5	-18.5 0.13 (1)		
I-H	0 / 0	-18.5	-18.5 0.03 (4)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	2-1-14	-73	-73	---	FRONT	VERT	TOTAL	C1
E	7-9-2	-72	-72	---	FRONT	VERT	TOTAL	C1
I	7-10-4	1	1	---	FRONT	VERT	TOTAL	C1
K	2-0-12	1	1	---	FRONT	VERT	TOTAL	C1
M	4-0-12	1	1	---	FRONT	VERT	TOTAL	C1
N	6-0-12	1	1	---	FRONT	VERT	TOTAL	C1
O	4-0-12	-1	-1	---	FRONT	VERT	TOTAL	C1
P	6-0-12	-1	-1	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 41 = 81 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.13/1.00 (F-G:1), BC=0.13/1.00 (I-J:1), WB=0.16/1.00 (B-K:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

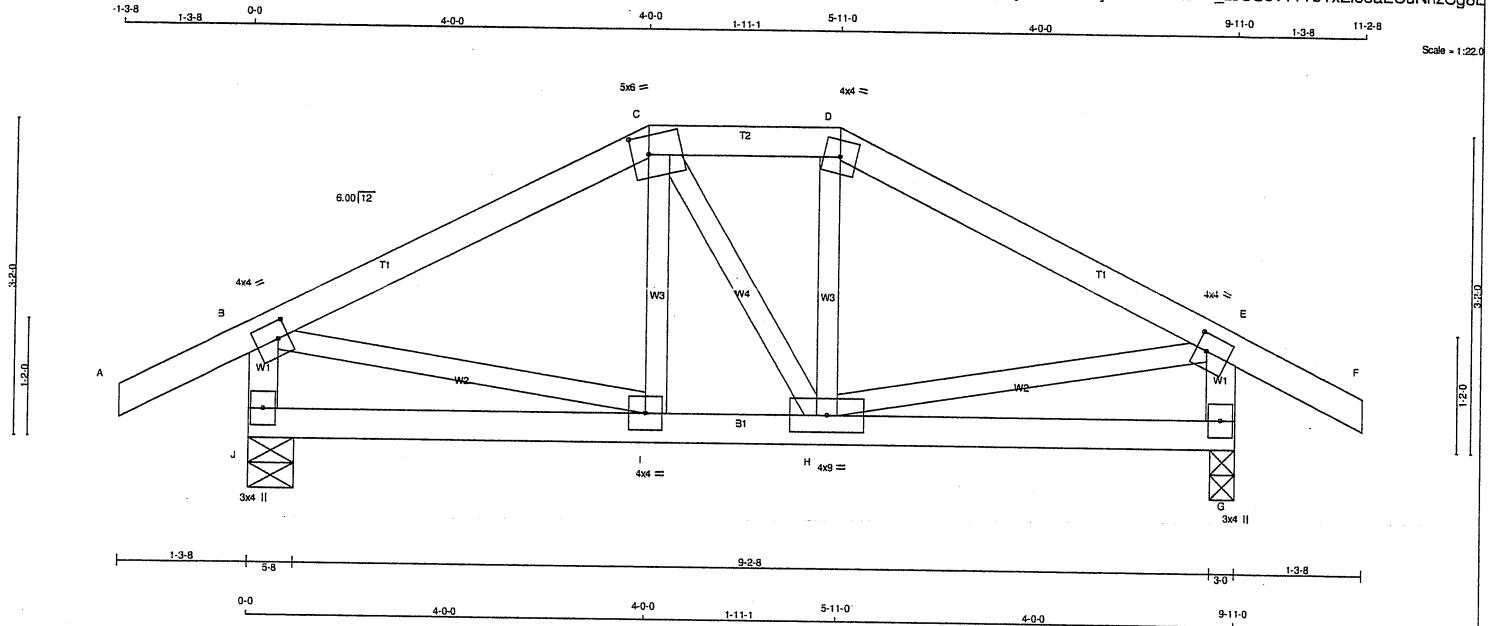
JSI GRIP= 0.74 (I) (INPUT = 0.90)
JSI METAL= 0.20 (K) (INPUT = 1.00)



Structural component only
DWG# T-2115798

JOB NAME 413212	TRUSS NAME T176	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 42 = 83 lb
(M/F)

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMW-t	MT20	4.0	4.0	2.00	1.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWVW-t	MT20	4.0	9.0		
I	BMWVW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
J	671	0	671	0	5-8	5-8
G	671	0	671	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
J	472	323 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
G	472	323 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	I-C	-39 / 31	0.01 (4)	
B-C	-519 / 0	-91.8 -91.8	0.19 (1)	C-H	0 / 2	0.00 (4)	
C-D	-463 / 0	-91.8 -91.8	0.05 (1)	H-D	-38 / 33	0.01 (4)	
D-E	-520 / 0	-91.8 -91.8	0.19 (1)	B-I	0 / 474	0.11 (1)	
E-F	0 / 28	-91.8 -91.8	0.12 (1)	H-E	0 / 475	0.11 (1)	
J-B	-638 / 0	0.0 0.0	0.06 (1)				
G-E	-637 / 0	0.0 0.0	0.06 (1)				
J-I	0 / 0	-18.5 -18.5	0.07 (4)				
I-H	0 / 462	-18.5 -18.5	0.10 (1)				
H-G	0 / 0	-18.5 -18.5	0.07 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.19/1.00 (D-E:1), BC=0.10/1.00 (H-I:1), WB=0.11/1.00 (E-H:1), SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (E) (INPUT = 0.90)
JSI METAL = 0.22 (E) (INPUT = 1.00)



Structural component only
DWG# T-2115799

JOB NAME 413212	TRUSS NAME T177G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 1 09:19:07 2021 Page 2
ID:5sPQm3ewyQLR2FGi1wwUPeyYVZn-2mytnHV A4MlwM3aiBCWA8y8rISiEovFvOmAs5z0Ugo

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	FACTORED FR-TO
FR-TO		FROM	TO				
AL-W	0 / 48	-18.5	-18.5 0.02 (1)			10.00	



Structural component only
DWG# T-2121140

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T178	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 5 Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:34 2021 Page 2
ID:5sPQm3ewyQLR2FGi1wwUPeyYVZn-92Ni2RUkRqr21lfM4kWyhw6F8PzB16VWw1Yh?RgzCq8J

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2115801 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T179	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:36 2021 Page 2

ID:5sPQm3ewyQLR2FGi1wwUPeyYVZn-6QVTT7W?zS5IGcplC8ZQmLCbZCiqV?qDVR5WZzCg8H

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2115802 *2/2*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T179G	1	1	TRUSS DESC.		

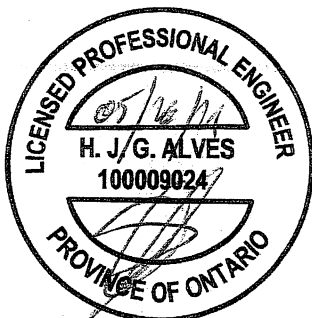
Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:37 2021 Page 2
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LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	FR-TO
AE-AD	0 / 13	-18.5	-18.5 0.01 (4)			10.00	
AD-AC	0 / 21	-18.5	-18.5 0.01 (4)			10.00	
AC-AB	0 / 29	-18.5	-18.5 0.03 (1)			10.00	



Structural component only
DWG# T-2115803

JOB NAME 413212	TRUSS NAME T180	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:39 2021 Page 2
ID:5sPQm3ewyQLR2FGi1wwUPeyYVZn-W?Bb59YtFNTK73XKtH67Ozp71QjCiHrfBpOm7tzCq8E

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2115804 *ML*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T181	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 11:02:48 2021 Page 2
ID:5sPQm3ewyQLR2FGi1wwUPeyYVZn-bmENCc5n0a1kTppF5cWdXKr4WJNik8xOlwvTOzCfmb

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	TMVW-p	MT20	4.0	6.0	1.00	3.00
F	BMV1+p	MT20	3.0	8.0		
G	BMWW+I	MT20	6.0	9.0	5.50	3.00
H	BMWW+I	MT20	6.0	9.0	5.50	3.00
I	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2115805 *me*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	T183	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 11:05:37 2021 Page 2
ID:5sPQm3ewyQLR2FGi1wwUPeyYVZn-8HyGgP7oDVkIFIEIf48ZzAqbkexjo46pFYLNu5zCtky

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT
(30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY
FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF
AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2115806 2/2

JOB NAME 413212	TRUSS NAME T184	QUANTITY 1	PLY 2	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:44 2021 Page 2 ID:5sPQm3ewyQLR2FGi1wwUPeyYVZn-tz U8sc04v6dErQHqit151X03RMENY0PL56Xo5zCg89		

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B						
C, L, N						
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	6.0	9.0	3.00	2.25
E, F, H, K						
E	TMWW-t	MT20	4.0	6.0		
G	TS-t	MT20	5.0	6.0		
I	TMW+w	MT20	3.0	6.0		
J	TS-t	MT20	5.0	6.0		
M	TTWW-m	MT20	6.0	9.0	2.75	2.00
O	TMV+p	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0	2.50	2.75
Q, V, X, Z						
Q	BMWW-t	MT20	4.0	6.0		
R, S, Y						
R	BMWW-t	MT20	5.0	6.0		
T	BS-t	MT20	5.0	6.0		
U	BMWWW-t	MT20	5.0	8.0		
W	BS-t	MT20	6.0	9.0		
AA	BMWW-t	MT20	8.0	9.0	4.25	4.50
AB						
AB	TBMVW1*+m	MT20	10.0	12.0	3.00	4.75

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-8-0	-58	-58	---	FRONT	VERT	DEAD	---	C1
D	5-8-0	-244	-244	---	FRONT	VERT	SNOW	---	C1
AA	2-6-8	-2694	-2694	626	FRONT	VERT	TOTAL	---	C1
AC	1-0-12	-45	-45	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.98 (T) (INPUT = 1.00)



Structural component only
DWG# T-2115807 *2/1*

PLATES (table is in inches)						
UT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	10.0		
B	TMVW-t	MT20	5.0	6.0		
C	TTVW+m	MT20	6.0	9.0	4.50	1.75
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMVW+t	MT20	4.0	6.0		
G	TTVW+m	MT20	6.0	9.0	4.50	2.00
H	TMW-t	MT20	5.0	6.0		
I	TMVW-m	MT20	7.0	12.0	3.75	5.25
J	TMW-t	MT20	3.0	6.0		
K	BMVW-1t	MT20	10.0	12.0	5.50	Edge
L	BMW+w	MT20	3.0	6.0		
M	BMVW+t	MT20	4.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMVWVW-t	MT20	5.0	8.0	2.50	3.25
R	BS-t	MT20	3.0	6.0		
S	BMVW+t	MT20	4.0	6.0		
T	BMVW-t	MT20	5.0	6.0		
U	BMV-t	MT20	5.0	10.0		
V	BMV-t	MT20	5.0	8.0	5.50	

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.96 (I) (INPUT = 1.00)



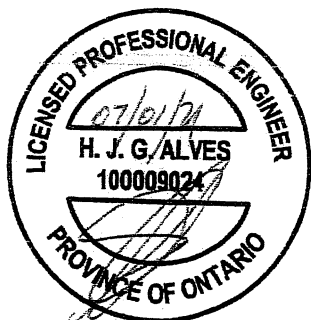
Structural component only
DWG# T-2121141

JOB NAME 413212	TRUSS NAME T185	QUANTITY 2	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 1 15:33:16 2021 Page 2 ID:5sPQm3ewyQLR2FGi1wwUPevYYZn-FbvXLNBSPYIVfHk5LNSuWBEPvd4Yac8Y1VGaIQz0PM1					

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



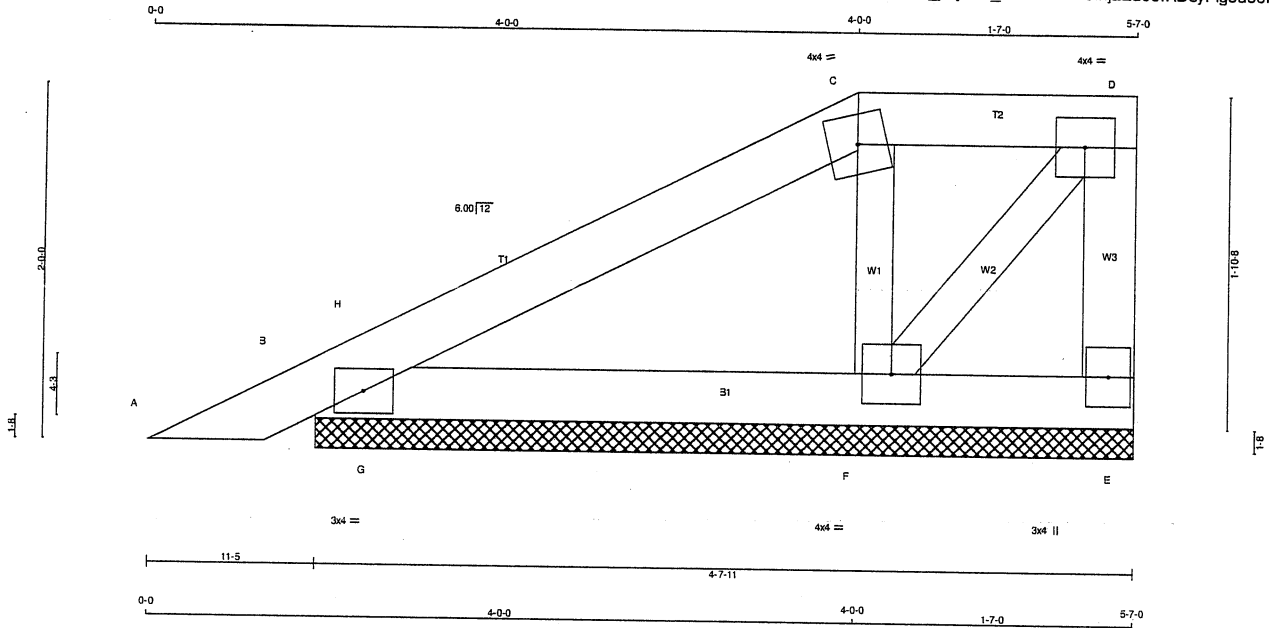
Structural component only
DWG# T-2121141 *me*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	PB101	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:01 2021 Page 1
ID:hEKUVWZApjnyW9leD6V_jMya4z_dhuE6951CihjaEd0eIRDsyFigSuoolk4SvBin4zCg8q

Scale = 1:12.4



LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
E	76	0	0	76	0	0	4-7-11	4-7-11	
B	252	0	0	252	0	0	4-7-11	4-7-11	
F	264	0	0	264	0	0	4-7-11	4-7-11	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	53	39 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
B	177	126 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
F	188	118 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH	FR-TO
A-B	0 / 17	-91.8	-91.8 0.05 (1)	10.00	F-C	-176 / 0	0.03 (1)
B-H	-18 / 0	-91.8	-91.8 0.02 (1)	6.25	F-D	0 / 24	0.01 (1)
H-C	-33 / 0	-91.8	-91.8 0.10 (1)	6.25	G-H	-170 / 0	0.00 (1)
C-D	-16 / 0	-91.8	-91.8 0.04 (1)	6.25			
E-D	-90 / 0	0.0	0.0 0.01 (1)	7.81			
B-G	0 / 28	-18.5	-18.5 0.09 (1)	10.00			
G-F	0 / 28	-18.5	-18.5 0.09 (1)	10.00			
F-E	0 / 0	-18.5	-18.5 0.06 (1)	10.00			

TOTAL WEIGHT = 2 X 16 = 32 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (C-H:1), BC=0.09/1.00 (B-G:1), WB=0.03/1.00 (C-F:1), SSI=0.13/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

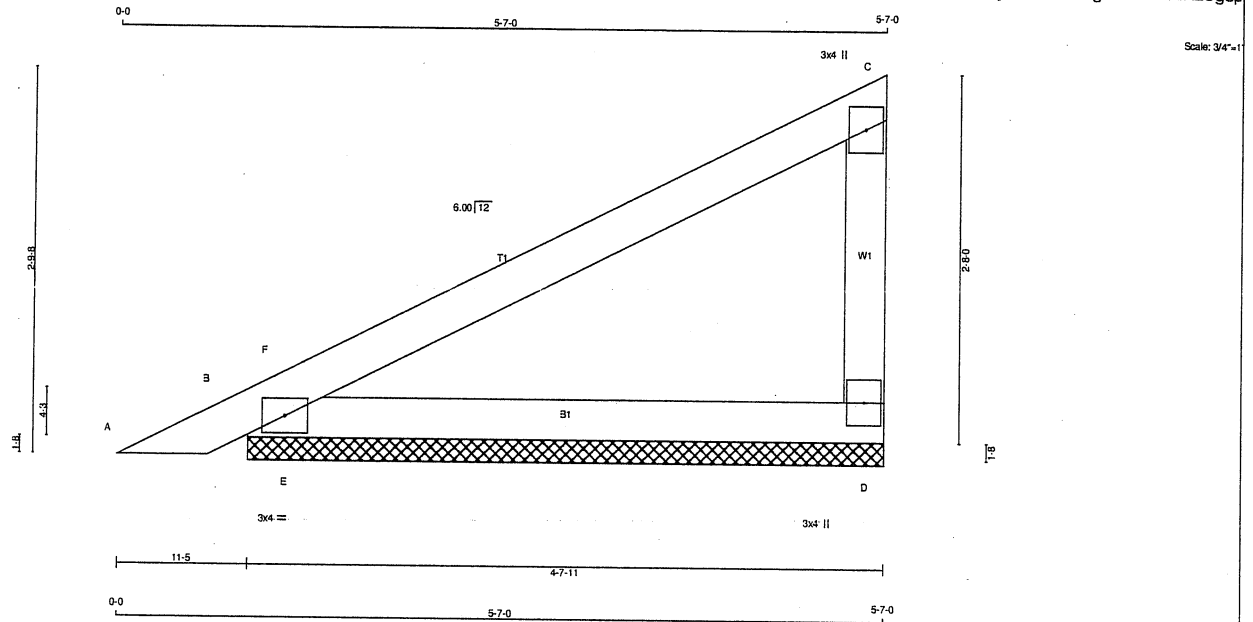


Structural component only
DWG# T-2115757

JOB NAME 413212	TRUSS NAME PB102	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:02 2021 Page 1
ID:hEKUVWZApjnyW9leD6V_jMya4z_-5uScKV6gz?paBOCDC?ySOAnu0VDgXIMdhZxIJZCg8p



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
D	256	0	256	0	0	4-7-11	4-7-11	4-7-11	4-7-11
B	336	0	336	0	0	4-7-11	4-7-11	4-7-11	4-7-11

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	181	119 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
B	236	164 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	10.00	E-F	-310 / 7	0.00 (1)
B-F	-21 / 45	-91.8 -91.8	0.06 (1)	6.25			
F-C	-4 / 2	-91.8 -91.8	0.26 (1)	10.00			
D-C	-166 / 0	0.0 0.0	0.02 (1)	7.81			
B-E	0 / 0	-18.5 -18.5	0.18 (1)	10.00			
E-D	0 / 0	-18.5 -18.5	0.18 (1)	10.00			

TOTAL WEIGHT = 4 X 15 = 59 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-F:1), BC=0.18/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.25/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

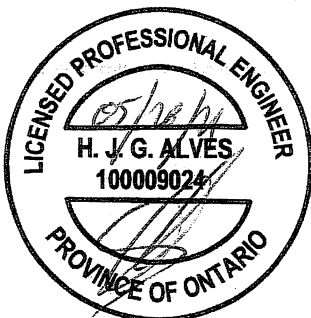
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

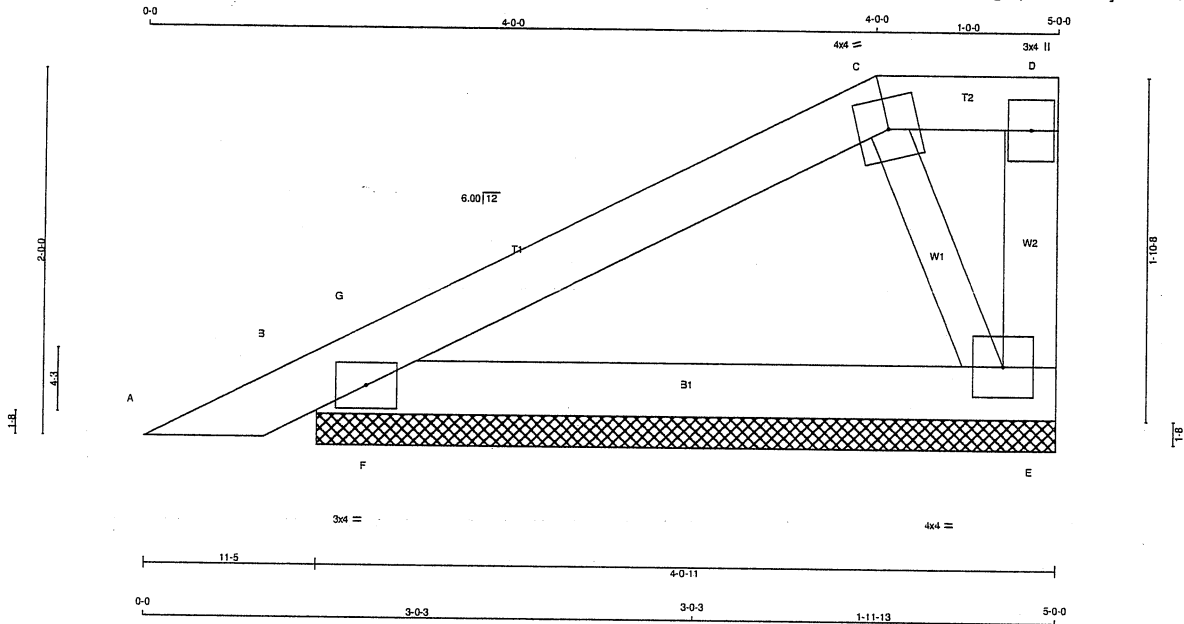


Structural component only
DWG# T-2115758

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	PB121	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 10:38:02 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-5uScKv6gz?paBOCDC?ySOAnw3VE3XIDhZxJXzCg8p



TOTAL WEIGHT = 2 X 14 = 27 lb [M]

LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS	
A - C 2x4 DRY No.2	SPF
C - D 2x4 DRY No.2	SPF
E - D 2x4 DRY No.2	SPF
B - E 2x4 DRY No.2	SPF
ALL WEBS 2x3 DRY No.2	SPF
DRY: SEASONED LUMBER.	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
E	224	0	224	0
B	304	0	304	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
E	158	104 / 0 0 / 0 0 / 0 0 / 0 54 / 0 0 / 0
B	214	149 / 0 0 / 0 0 / 0 0 / 0 65 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 17	-91.8	-91.8	0.05 (1)	10.00	C-E	-156 / 0	0.02 (1)	
B-G	-115 / 0	-91.8	-91.8	0.05 (4)	6.25	F-G	-88 / 43	0.00 (1)	
G-C	-88 / 0	-91.8	-91.8	0.12 (1)	6.25				
C-D	0 / 0	-91.8	-91.8	0.01 (1)	10.00				
E-D	-44 / 0	0.0	0.0	0.00 (1)	7.81				
B-F	0 / 81	-18.5	-18.5	0.08 (1)	10.00				
F-E	0 / 81	-18.5	-18.5	0.09 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (C-G:1), BC=0.09/1.00 (E-F:1), WB=0.02/1.00 (C-E:1), SSI=0.09/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

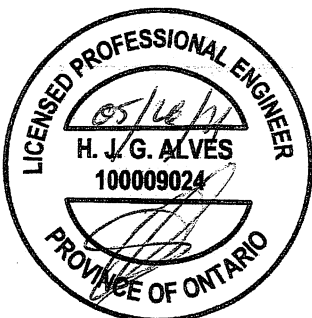
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)

JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115759

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:03 2021 Page 1
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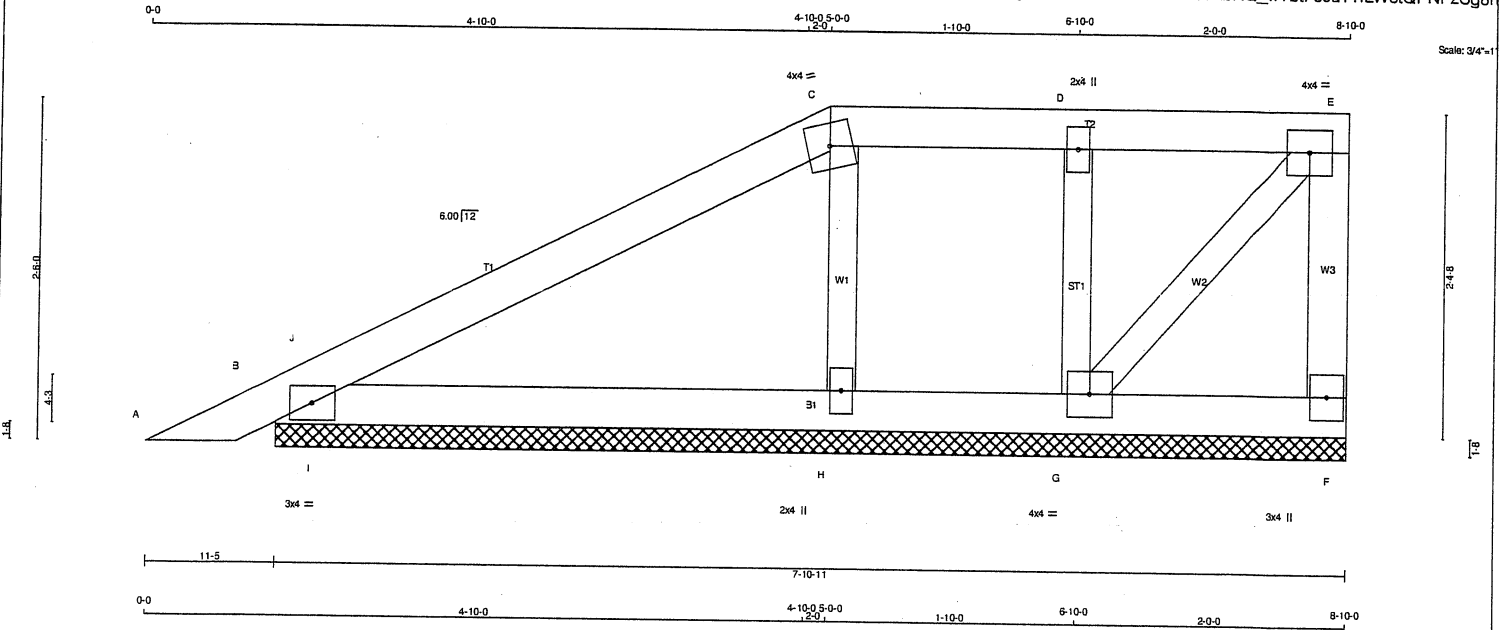


Structural component only
DWG# T-2115760

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	PB123G	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 10:38:04 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-2GZNIB7wVd3IRhMbKQ_wTbtFeJuT?fLW8tQPNNPzCg8r



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMW-w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWV1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	10.00	G-D	-219 / 0	0.03 (1)
B-J	-21 / 2	-91.8 -91.8	0.05 (1)	6.25	H-C	-200 / 0	0.03 (1)
J-C	-48 / 0	-91.8 -91.8	0.18 (1)	6.25	G-E	0 / 41	0.01 (1)
C-D	-28 / 0	-91.8 -91.8	0.06 (1)	6.25	I-J	-285 / 0	0.00 (1)
D-E	-28 / 0	-91.8 -91.8	0.06 (1)	6.25			
F-E	-101 / 0	0.0 0.0	0.01 (1)	7.81			
B-I	0 / 38	-18.5 -18.5	0.16 (1)	10.00			
I-H	0 / 38	-18.5 -18.5	0.16 (1)	10.00			
H-G	0 / 28	-18.5 -18.5	0.10 (1)	10.00			
G-F	0 / 0	-18.5 -18.5	0.02 (4)	10.00			

TOTAL WEIGHT = 2 X 27 = 53 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-J:1), BC=0.16/1.00 (B-I:1), WB=0.03/1.00 (D-G:1), SS=0.22/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

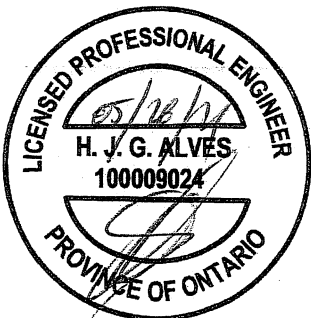
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.25 (B) (INPUT = 0.90)
JSI METAL = 0.08 (C) (INPUT = 1.00)

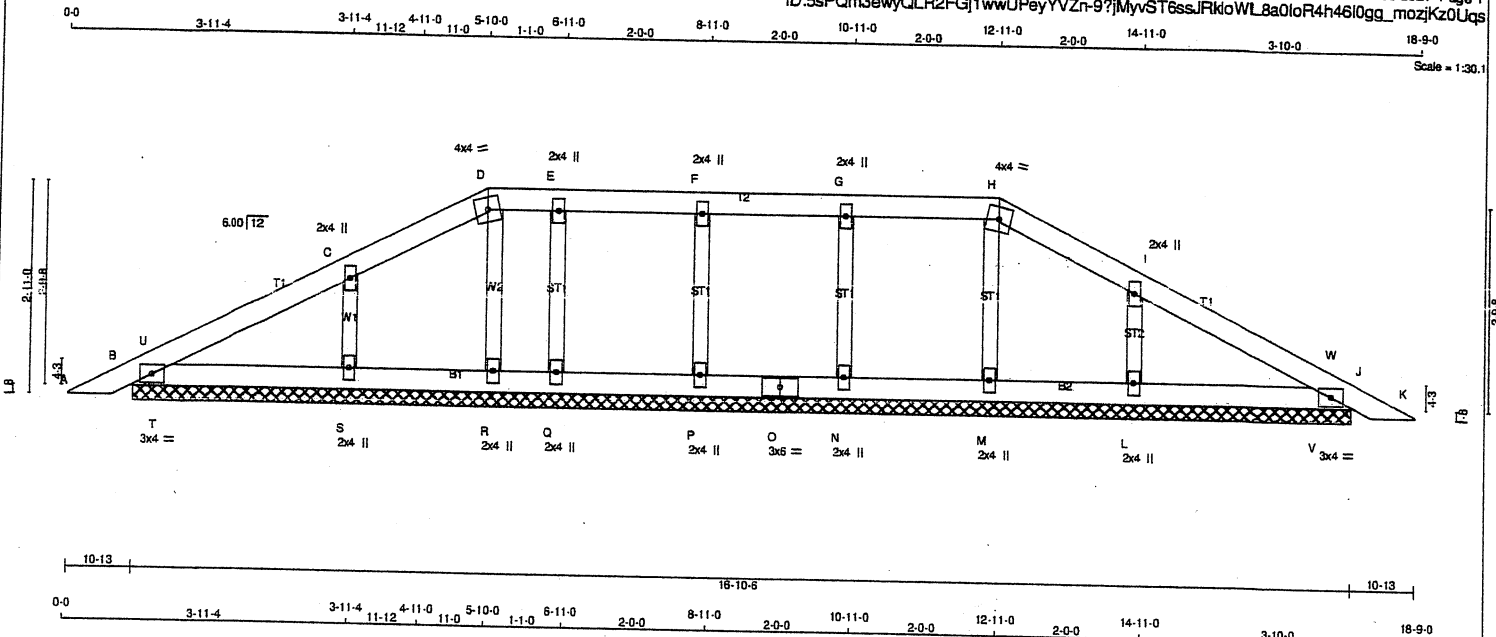


Structural component only
DWG# T-2115761

JOB NAME 413212	TRUSS NAME PB124G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 1 09:19:03 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPayYVZn-9?MyvST6ssJRKoWL8a0IoR4h46I0gg_mozjkZ0Uqs



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
B - O	2x4	DRY	No.2
O - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C, E, F, G, I				
C	TMW+w	MT20	2.0	4.0
D	TTW-m	MT20	4.0	4.0
H	TTW-m	MT20	4.0	4.0
J	TMB1-I	MT20	3.0	4.0
L, M, N, P, Q, R, S				
L	BMW1+w	MT20	2.0	4.0
O	BS-I	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
FR-TO		FROM TO		FR-TO			
A-B	0 / 16	-91.8 -91.8	0.05 (1)	10.00	M-H	-126 / 0	0.02 (1)
B-U	-34 / 0	-91.8 -91.8	0.02 (4)	6.25	N-G	-212 / 0	0.04 (1)
U-C	-19 / 0	-91.8 -91.8	0.09 (1)	6.25	P-F	-183 / 0	0.03 (1)
C-D	-32 / 0	-91.8 -91.8	0.03 (1)	6.25	Q-E	-156 / 0	0.03 (1)
D-E	-13 / 0	-91.8 -91.8	0.04 (1)	6.25	L-I	-259 / 0	0.04 (1)
E-F	-13 / 0	-91.8 -91.8	0.05 (1)	6.25	R-D	-72 / 0	0.01 (1)
F-G	-13 / 0	-91.8 -91.8	0.05 (1)	6.25	S-C	-258 / 0	0.04 (1)
G-H	-13 / 0	-91.8 -91.8	0.05 (1)	6.25	T-U	-102 / 4	0.00 (1)
H-I	-35 / 0	-91.8 -91.8	0.09 (1)	6.25	V-W	-100 / 4	0.00 (1)
I-W	-21 / 0	-91.8 -91.8	0.09 (1)	6.25			
W-J	-37 / 0	-91.8 -91.8	0.02 (4)	6.25			
J-K	0 / 16	-91.8 -91.8	0.05 (1)	10.00			
B-T	0 / 27	-18.5 -18.5	0.06 (1)	10.00			
T-S	0 / 27	-18.5 -18.5	0.06 (1)	10.00			
S-R	0 / 16	-18.5 -18.5	0.04 (1)	10.00			
R-Q	0 / 13	-18.5 -18.5	0.01 (4)	10.00			
Q-P	0 / 13	-18.5 -18.5	0.02 (4)	10.00			
P-O	0 / 13	-18.5 -18.5	0.02 (4)	10.00			
O-N	0 / 13	-18.5 -18.5	0.02 (4)	10.00			
N-M	0 / 13	-18.5 -18.5	0.02 (4)	10.00			
M-L	0 / 16	-18.5 -18.5	0.04 (1)	10.00			
L-V	0 / 29	-18.5 -18.5	0.06 (1)	10.00			
V-J	0 / 29	-18.5 -18.5	0.06 (1)	10.00			

TOTAL WEIGHT = 53 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (H-I), BC=0.06/1.00 (B-T), WB=0.04/1.00 (L-I), SS=0.10/1.00 (H-W)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.59 (D) (INPUT = 0.30)
JSI METAL = 0.11 (I) (INPUT = 1.00)

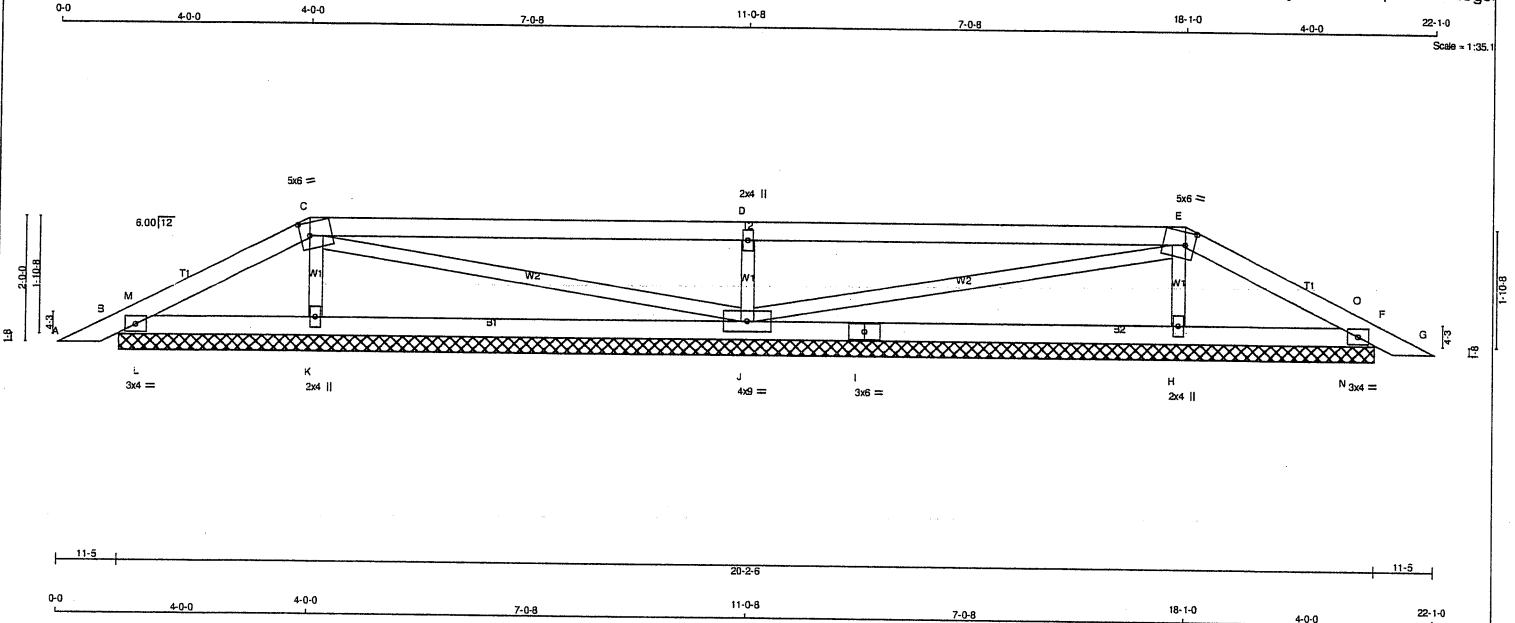


Structural component only
DWG# T-2121137

JOB NAME 413212	TRUSS NAME PB127	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Wed May 26 10:38:06 2021 Page 1
ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn_-fh7As9A1EJ0g?W_Rr0OZ0yRwGZNTYXpbBwWSizCg8



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
B - I	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	COMBINED	VERT	HORZ	DOWN	HORZ	UP/LIFT	IN-SX	BRG	IN-SX
B	255	0	255	0	0	0	20-2-6 (8-2-10)	2-6	2-6
K	466	0	466	0	0	0	20-2-6 (8-2-10)	2-6	2-6
J	947	0	947	0	0	0	20-2-6 (8-2-10)	2-6	2-6
H	466	0	466	0	0	0	20-2-6 (8-2-10)	2-6	2-6
F	255	0	255	0	0	0	20-2-6 (8-2-10)	2-6	2-6

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL
B	177	136 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0	0 / 0	0 / 0
K	333	200 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0	0 / 0	0 / 0
J	667	452 / 0	0 / 0	0 / 0	0 / 0	215 / 0	0 / 0	0 / 0	0 / 0
H	333	200 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0	0 / 0	0 / 0
F	177	136 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-91.8 -91.8	0.05 (1)	K-C	-328 / 0	0.05 (1)	
B-M	-33 / 0	-91.8 -91.8	0.03 (1)	C-J	-21 / 0	0.02 (1)	
M-C	-59 / 0	-91.8 -91.8	0.10 (1)	J-D	-807 / 0	0.12 (1)	
C-D	-8 / 0	-91.8 -91.8	0.77 (1)	D-E	-21 / 0	0.02 (1)	
D-E	-8 / 0	-91.8 -91.8	0.77 (1)	E-O	-328 / 0	0.05 (1)	
E-O	-59 / 0	-91.8 -91.8	0.10 (1)	O-F	-193 / 0	0.00 (1)	
O-F	-33 / 0	-91.8 -91.8	0.03 (1)	F-G	-193 / 0	0.00 (1)	
F-G	0 / 17	-91.8 -91.8	0.05 (1)				
B-L	0 / 50	-18.5 -18.5	0.10 (1)				
L-K	0 / 50	-18.5 -18.5	0.14 (4)				
K-J	0 / 28	-18.5 -18.5	0.19 (4)				
J-I	0 / 28	-18.5 -18.5	0.19 (4)				
I-H	0 / 28	-18.5 -18.5	0.19 (4)				
H-N	0 / 50	-18.5 -18.5	0.14 (4)				
N-F	0 / 50	-18.5 -18.5	0.10 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.77/1.00 (C-D:1), BC=0.19/1.00 (H-J:4), WB=0.12/1.00 (D-J:1), SSI=0.32/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE: GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)



Structural component only
DWG# T-2115763

amarack Roof Truss, Burlington



PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-l	MT20	3.0	4.0
C	TTWW-m	MT20	5.0	6.0 2.00 2.00
D	TMW+w	MT20	2.0	4.0
E	TTWW-m	MT20	5.0	6.0 2.00 2.00
F	TMB1-l	MT20	3.0	4.0
H	BMW1+w	MT20	2.0	4.0
J	BS-t	MT20	3.0	6.0
J	BMWWWW1-t	MT20	4.0	9.0
K	BMW1+w	MT20	2.0	4.0

JT	VERT	HORZ	DOWN		UP		BKG	
			HORZ	UPLIFT	IN-SX	IN-SX		
B	396	0	396	0	0	20-2-6	(8-2-1)20-2-6	
K	449	0	449	0	0	20-2-6	(8-2-1)20-2-6	
J	698	0	698	0	0	20-2-6	(8-2-1)20-2-6	
H	449	0	449	0	0	20-2-6	(8-2-1)20-2-6	
F	396	0	396	0	0	20-2-6	(8-2-1)20-2-6	

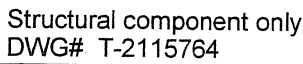
1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SC
K	277	197 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
B	320	195 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
J	490	338 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0
H	320	195 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
F	277	197 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (4)

C H O R D S				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 17	-91.8	-91.8	0.05 (1)	10.00	K-C	-284 / 0	0.05 (1)
B-M	-38 / 0	-91.8	-91.8	0.10 (1)	6.25	C-J	-74 / 0	0.04 (1)
M-C	-126 / 0	-91.8	-91.8	0.29 (1)	6.25	J-D	-578 / 0	0.10 (1)
C-D	-29 / 0	-91.8	-91.8	0.40 (1)	6.25	J-E	-74 / 0	0.04 (1)
D-E	-29 / 0	-91.8	-91.8	0.40 (1)	6.25	H-E	-284 / 0	0.05 (1)
E-O	-126 / 0	-91.8	-91.8	0.29 (1)	6.25	L-M	-419 / 0	0.00 (1)
O-F	-38 / 0	-91.8	-91.8	0.10 (1)	6.25	N-O	-419 / 0	0.00 (1)
F-G	0 / 17	-91.8	-91.8	0.05 (1)	10.00			
B-L	0 / 107	-18.5	-18.5	0.24 (1)	10.00			
L-K	0 / 107	-18.5	-18.5	0.24 (1)	10.00			
K-J	0 / 95	-18.5	-18.5	0.15 (1)	10.00			
J-I	0 / 95	-18.5	-18.5	0.15 (1)	10.00			
I-H	0 / 95	-18.5	-18.5	0.15 (1)	10.00			
H-N	0 / 107	-18.5	-18.5	0.24 (1)	10.00			
N-F	0 / 107	-18.5	-18.5	0.24 (1)	10.00			

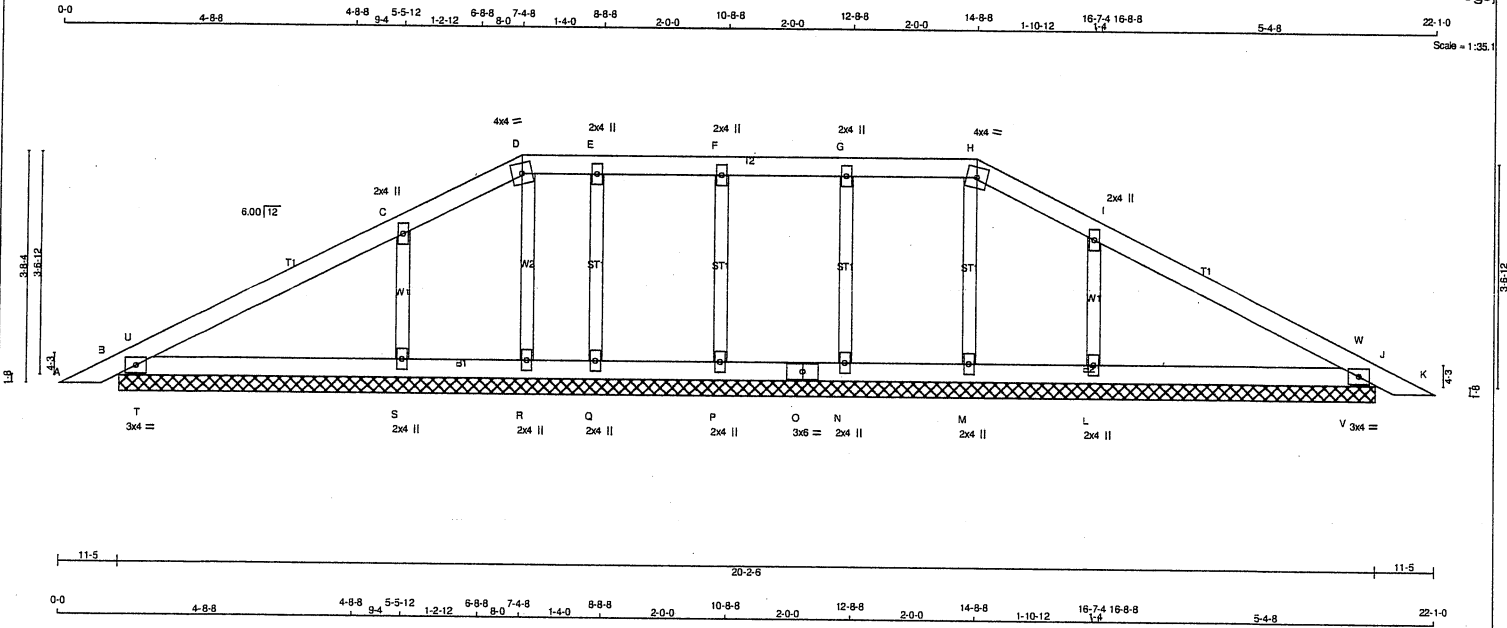
JSI GRIP= 0.65 (C) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)



JOB NAME 413212	TRUSS NAME PB129G	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
B - O	2x4	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0 0 OC.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C, E, F, G, I				
C	TMW+u	MT20	2.0	4.0
D	TTW-m	MT20	4.0	4.0
H	TTW-m	MT20	4.0	4.0
J	TMB1-I	MT20	3.0	4.0
L, M, N, P, Q, R, S				
L	BMW1+u	MT20	2.0	4.0
O	BS-I	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	UNBRAC LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	0 / 17	-91.8	-91.8 0.05 (1)	10.00	M-H	-80 / 0	0.02 (1)
B-U	-31 / 0	-91.8	-91.8 0.03 (1)	6.25	N-G	-211 / 0	0.04 (1)
U-C	-35 / 0	-91.8	-91.8 0.19 (1)	6.25	P-F	-183 / 0	0.04 (1)
C-D	-68 / 0	-91.8	-91.8 0.19 (1)	6.25	Q-E	-163 / 0	0.03 (1)
D-E	-31 / 0	-91.8	-91.8 0.03 (1)	6.25	R-D	-41 / 0	0.01 (1)
E-F	-31 / 0	-91.8	-91.8 0.04 (1)	6.25	S-C	-370 / 0	0.06 (1)
F-G	-31 / 0	-91.8	-91.8 0.05 (1)	6.25	L-I	-371 / 0	0.06 (1)
G-H	-31 / 0	-91.8	-91.8 0.05 (1)	6.25	T-U	-226 / 6	0.00 (1)
H-I	-70 / 0	-91.8	-91.8 0.19 (1)	6.25	V-W	-225 / 7	0.00 (1)
I-W	-37 / 0	-91.8	-91.8 0.19 (1)	6.25			
W-J	-31 / 0	-91.8	-91.8 0.03 (1)	6.25			
J-K	0 / 17	-91.8	-91.8 0.05 (1)	10.00			
B-T	0 / 43	-18.5	-18.5 0.13 (1)	10.00			
T-S	0 / 43	-18.5	-18.5 0.13 (1)	10.00			
S-R	0 / 33	-18.5	-18.5 0.10 (1)	10.00			
R-Q	0 / 31	-18.5	-18.5 0.03 (1)	10.00			
Q-P	0 / 31	-18.5	-18.5 0.02 (4)	10.00			
P-O	0 / 31	-18.5	-18.5 0.02 (4)	10.00			
O-N	0 / 31	-18.5	-18.5 0.02 (4)	10.00			
N-M	0 / 31	-18.5	-18.5 0.02 (4)	10.00			
M-L	0 / 34	-18.5	-18.5 0.10 (1)	10.00			
L-V	0 / 44	-18.5	-18.5 0.13 (1)	10.00			
V-J	0 / 44	-18.5	-18.5 0.13 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (I-W:1), BC=0.13/1.00 (S-T:1), WB=0.06/1.00 (H-L:1), SS=0.18/1.00 (B-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)

JSI METAL= 0.15 (I) (INPUT = 1.00)

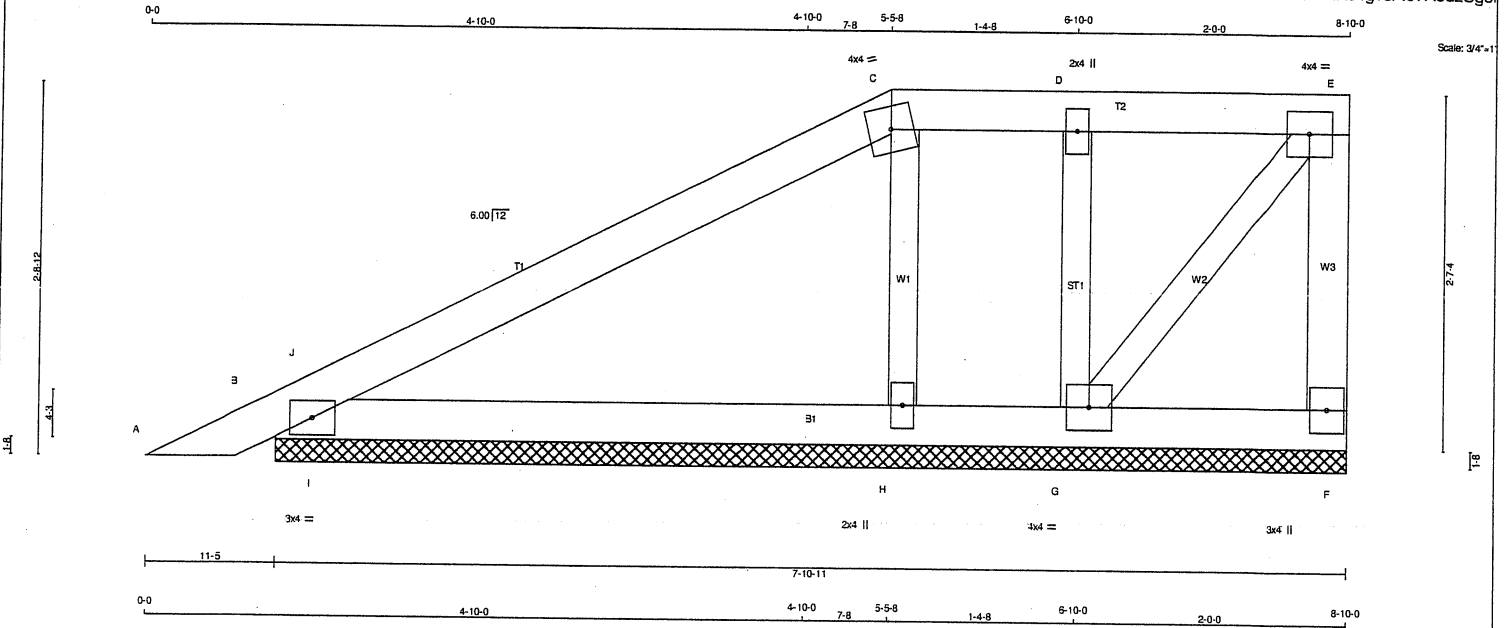


Structural component only
DWG# T-2115765

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	PB130G	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0-0 OC.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMW-w	MT20	2.0	4.0		
E	TMW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 17	-91.8	-91.8 0.05 (1)	10.00	G-D	-197 / 0	0.03 (1)
B-J	-22 / 20	-91.8	-91.8 0.08 (1)	6.25	H-C	-192 / 0	0.03 (1)
J-C	-54 / 0	-91.8	-91.8 0.23 (1)	6.25	G-E	0 / 52	0.01 (1)
C-D	-34 / 0	-91.8	-91.8 0.05 (1)	6.25	I-J	-352 / 0	0.00 (1)
D-E	-34 / 0	-91.8	-91.8 0.05 (1)	6.25			
F-E	-114 / 0	0.0	0.0 0.01 (1)	7.81			
B-I	0 / 43	-18.5	-18.5 0.19 (1)	10.00			
I-H	0 / 43	-18.5	-18.5 0.19 (1)	10.00			
H-G	0 / 34	-18.5	-18.5 0.14 (1)	10.00			
G-F	0 / 0	-18.5	-18.5 0.02 (4)	10.00			

TOTAL WEIGHT = 2 X 28 = 55 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-J:1), BC=0.19/1.00 (H-I:1), WB=0.03/1.00 (D-G:1), SS=0.27/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



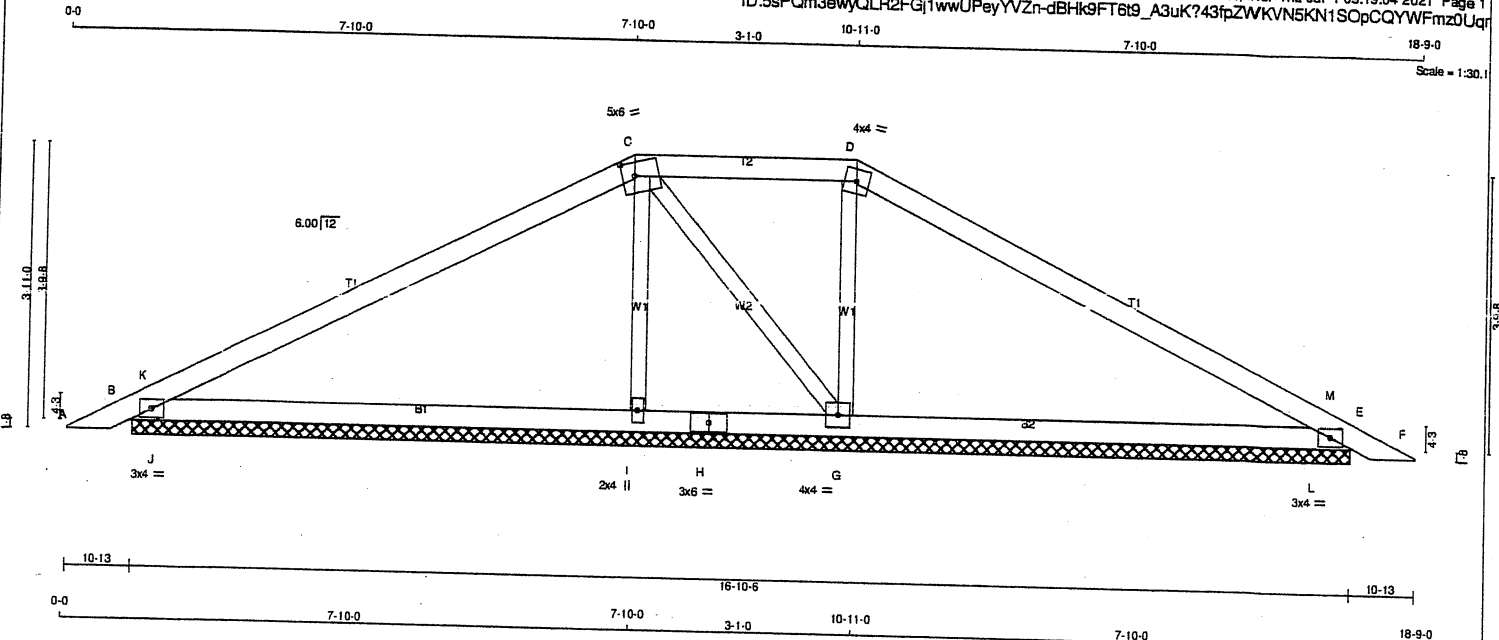
Structural component only
DWG# T-2115766

JOB NAME 413212	TRUSS NAME PB131	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	559	0	559	0	0	16-10-6 (8-10-6)	16-10-6 (8-10-6)	16-10-6 (8-10-6)	16-10-6 (8-10-6)
I	389	0	389	0	0	16-10-6 (8-10-6)	16-10-6 (8-10-6)	16-10-6 (8-10-6)	16-10-6 (8-10-6)
G	531	0	531	0	0	16-10-6 (8-10-6)	16-10-6 (8-10-6)	16-10-6 (8-10-6)	16-10-6 (8-10-6)
E	535	0	535	0	0	16-10-6 (8-10-6)	16-10-6 (8-10-6)	16-10-6 (8-10-6)	16-10-6 (8-10-6)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	393	274 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0	0 / 0
I	277	167 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0	0 / 0
G	375	248 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0	0 / 0
E	376	260 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I, G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 16	-91.8	-91.8 0.05 (1)	I-C	-231 / 0
B-K	-89 / 0	-91.8	-91.8 0.23 (1)	C-G	-78 / 0
K-C	-286 / 0	-91.8	-91.8 0.56 (1)	G-D	-314 / 0
C-D	-188 / 0	-91.8	-91.8 0.15 (1)	J-K	-735 / 0
D-M	-232 / 0	-91.8	-91.8 0.56 (1)	L-M	-738 / 0
M-E	-80 / 0	-91.8	-91.8 0.23 (1)		
E-F	0 / 16	-91.8	-91.8 0.05 (1)		
B-J	0 / 245	-18.5	-18.5 0.44 (1)		
J-I	0 / 245	-18.5	-18.5 0.44 (1)		
I-H	0 / 238	-18.5	-18.5 0.27 (1)		
H-G	0 / 238	-18.5	-18.5 0.27 (1)		
G-L	0 / 197	-18.5	-18.5 0.44 (1)		
L-E	0 / 197	-18.5	-18.5 0.44 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56/1.00 (C-K:1), BC=0.44/1.00 (B-J:1), WB=0.07/1.00 (D-G:1), SS=0.57/1.00 (E-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

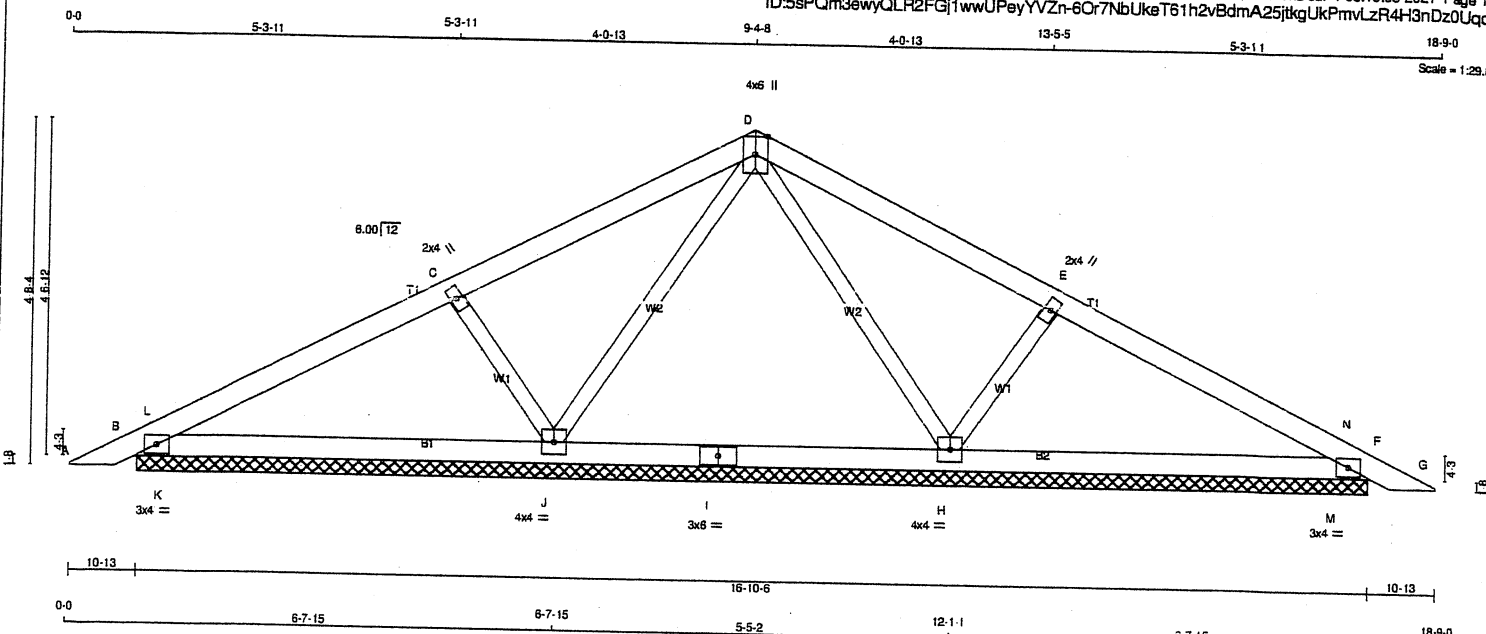
JSI GRIP = 0.78 (H) (INPUT = 0.30)
JSI METAL = 0.47 (H) (INPUT = 1.00)



Structural component only
DWG# T-2121138

JOB NAME 413212	TRUSS NAME PB132	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 1 09:19:05 2021 Page 1
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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
B - I	2x4	DRY No.2	SPF
I - F	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	355	0	355	0	16-10-6 (7-11)	16-10-6
H	652	0	652	0	16-10-6 (7-11)	16-10-6
J	652	0	652	0	16-10-6 (7-11)	16-10-6
F	355	0	355	0	16-10-6 (7-11)	16-10-6

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	250	173 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
H	461	302 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0
J	461	302 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0
F	250	173 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 16	-91.8 -91.8	0.05 (1)	D-H	-210 / 0	0.09 (1)	
B-L	-214 / 0	-91.8 -91.8	0.07 (4)	H-E	-423 / 0	0.07 (1)	
L-C	-157 / 0	-91.8 -91.8	0.27 (1)	J-D	-210 / 0	0.09 (1)	
C-D	0 / 56	-91.8 -91.8	0.27 (1)	C-J	-423 / 0	0.07 (1)	
D-E	0 / 56	-91.8 -91.8	0.27 (1)	K-L	-77 / 65	0.00 (1)	
E-N	-157 / 0	-91.8 -91.8	0.27 (1)	M-N	-77 / 65	0.00 (1)	
N-F	-214 / 0	-91.8 -91.8	0.07 (4)				
F-G	0 / 16	-91.8 -91.8	0.05 (1)				
B-K	0 / 162	-18.5 -18.5	0.09 (1)				
K-J	0 / 162	-18.5 -18.5	0.13 (4)				
J-I	0 / 44	-18.5 -18.5	0.13 (4)				
I-H	0 / 44	-18.5 -18.5	0.13 (4)				
H-M	0 / 162	-18.5 -18.5	0.13 (4)				
M-F	0 / 162	-18.5 -18.5	0.09 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (D-E:1), BC=0.13/1.00 (H-M:4), WB=0.09/1.00 (D-J:1), SS=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (H) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)

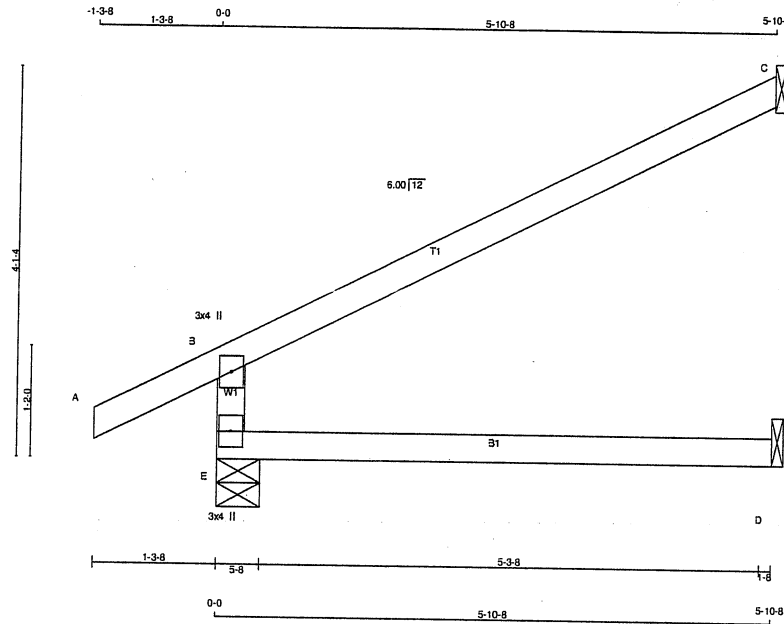


Structural component only
DWG# T-2121139

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	J01	21	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
E	525	0	525	0	0
C	202	0	202	0	0
D	45	0	50	0	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO	
E-B	-461 / 0	0.0 0.0 0.13 (4)	7.81		
A-B	0 / 28	-91.8 -91.8 0.12 (1)	10.00		
B-C	-30 / 0	-91.8 -91.8 0.54 (1)	6.25		
E-D	0 / 0	-18.5 -18.5 0.13 (4)	10.00		

TOTAL WEIGHT = 21 X 17 = 353 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



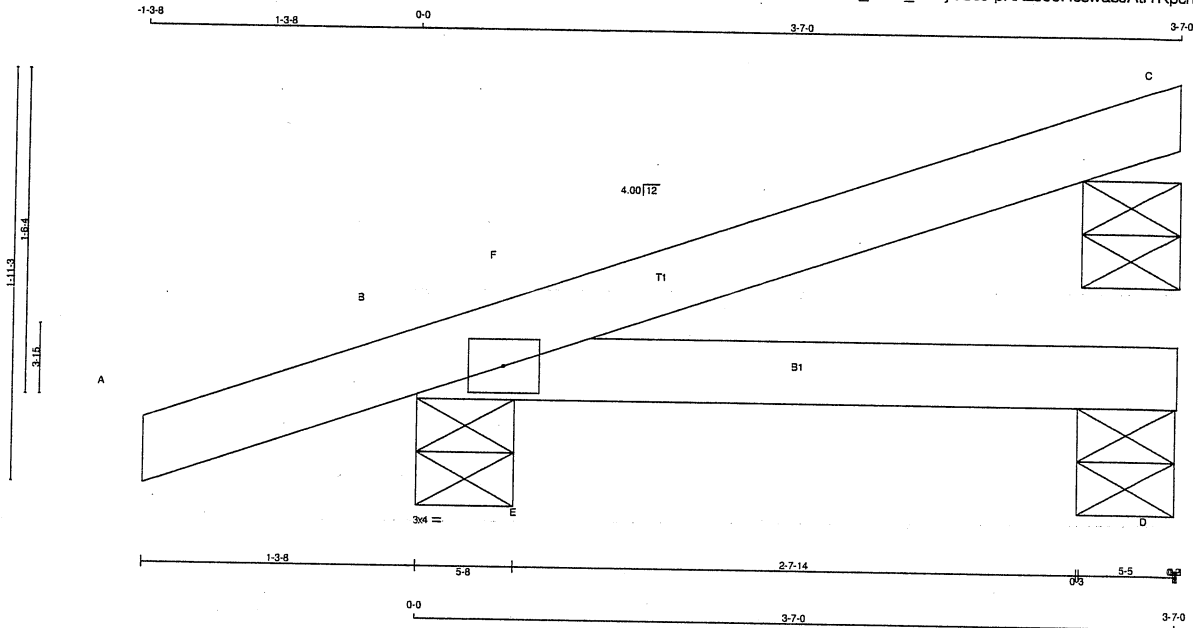
Structural component only
DWG# T-2115744

JOB NAME 413212	TRUSS NAME J03	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:10.0



LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
B - D 2x4 DRY
LUMBER No.2
DESCR. SPF
SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMB1-I MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
C	141	0	141	0	5-8 (5-7)	5-8
B	320	0	320	0	5-8	5-8
D	57	0	57	0	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
C	98	76 / 0	0 / 0	0 / 0	0 / 0	0 / 0	22 / 0
B	224	160 / 0	0 / 0	0 / 0	0 / 0	0 / 0	64 / 0
D	42	16 / 0	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX.	FORCE (LBS)	VERT.	LOAD LC1	MAX.	MEMB.	MAX.	FORCE (LBS)
FR-TO			FROM	TO		FR-TO		
A-B	0 / 18		-91.8	-91.8	0.13 (5)	10.00	E-F	-138 / 5
B-F	-10 / 0		-91.8	-91.8	0.04 (4)	6.25		
F-C	0 / 2		-91.8	-91.8	0.15 (1)	10.00		
B-E	0 / 0		-18.5	-18.5	0.12 (1)	10.00		
E-D	0 / 0		-18.5	-18.5	0.12 (1)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 10 = 50 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.15/1.00 (C-F:1), BC=0.12/1.00 (B-E:1),
WB=0.00/1.00 (E-F:1), SS=0.11/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

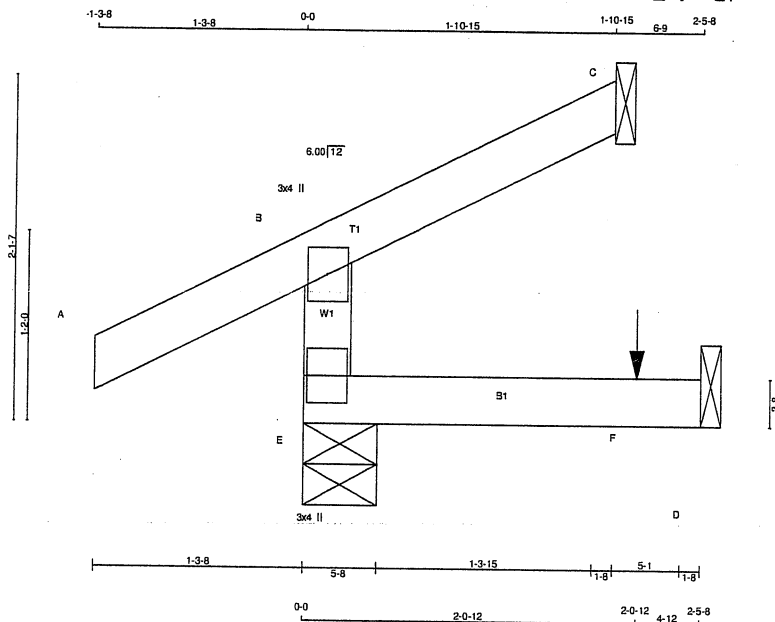


Structural component only
DWG# T-2115745

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	J11	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2			SPF
A - C	2x4	DRY	No.2			SPF
E - D	2x4	DRY	No.2			SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
E	260	0	260	0
C	66	0	66	0
D	23	0	26	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED								
E	182	130 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	18	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)
FR-TO										
E-B		-234 / 0	0.0	0.0	0.02 (4)	7.81				
A-B		0 / 28	-91.8	-91.8	0.13 (5)	10.00				
B-C		-10 / 0	-91.8	-91.8	0.06 (1)	10.00				
E-F		0 / 0	-18.5	-18.5	0.03 (4)	10.00				
F-D		0 / 0	-18.5	-18.5	0.03 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
					BACK	VERT	TOTAL		C1
F	2-0-12	-3	-3	---					

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 8 = 31 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B-5), BC=0.03/1.00 (D-E-4), WB=0.00/1.00 (n/a-0), SSI=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)

JSI METAL= 0.06 (B) (INPUT = 1.00)

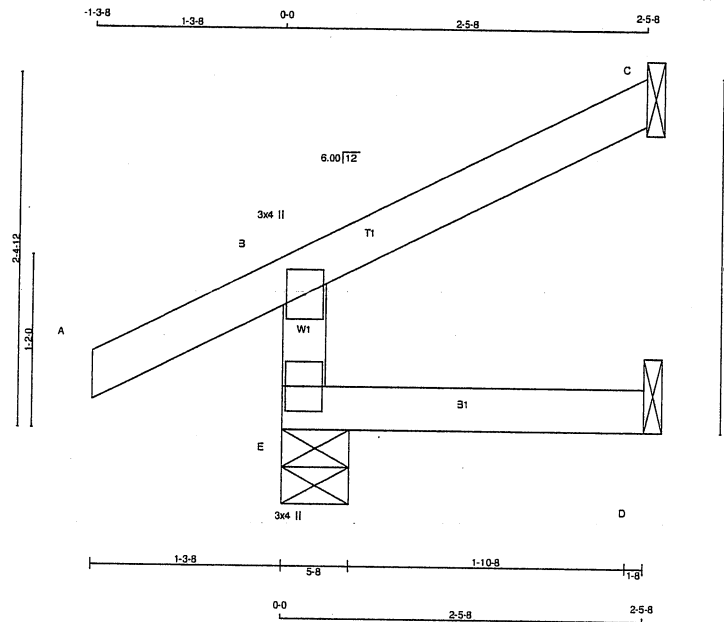


Structural component only
DWG# T-2115746

JOB NAME 413212	TRUSS NAME J12	QUANTITY 4	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:14.9

<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
E	291	0	291	0
C	85	0	85	0
D	20	0	22	0

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED						
E	203	148 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	58	47 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	16	0 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	MEMB. FORCE	VERT. LOAD	LC1	MAX	MAX. MEMB. FORCE	WEBS	MAX. FACTORED	MEMB. FORCE	MAX
	(LBS)	(PLF)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO											
E-B	-265 / 0	0.0	0.0	0.02	(4)	7.81					
A-B	0 / 28	-91.8	-91.8	0.12	(5)	10.00					
B-C	-12 / 0	-91.8	-91.8	0.09	(1)	6.25					
E-D	0 / 0	-18.5	-18.5	0.03	(4)	10.00					

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 9 = 34 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B-5), BC=0.03/1.00 (D-E-4),
WB=0.00/1.00 (n/a/0), SSI=0.10/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.11 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)

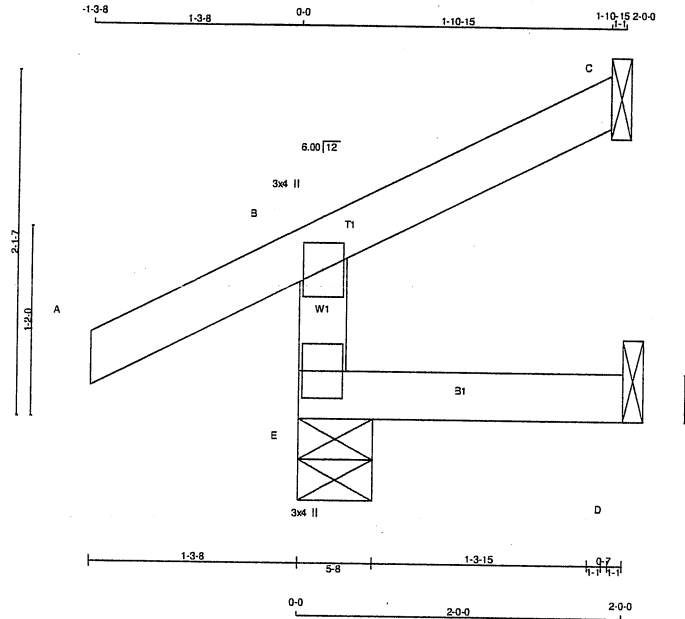


Structural component only
DWG# T-2115747

JOB NAME 413212	TRUSS NAME J13	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:12.5

LUMBER				DESCR.	SPF
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	254	0	254	0	5-8	5-8
C	66	0	66	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE			
E	177	130 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
E-B	-234 / 0	0.0	0.01 (4)	7.81
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00
B-C	-10 / 0	-91.8	-91.8 0.06 (1)	10.00
E-D	0 / 0	-18.5	-18.5 0.02 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

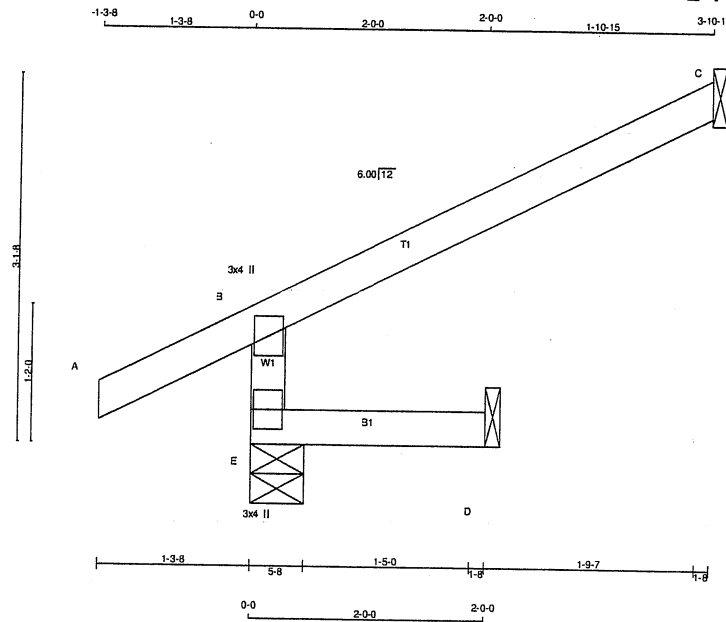


Structural component only
DWG# T-2115748

JOB NAME 413212	TRUSS NAME J14	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:18.6

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	369	0	369	0	5-8	5-8
C	135	0	135	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	256	194 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

FR-TO	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED MEMB. FORCE (LBS)
E-B	-349 / 0	0.0	0.0	0.01 (4)
A-B	0 / 28	-91.8	-91.8	0.13 (5)
B-C	-20 / 0	-91.8	-91.8	0.24 (1)
E-D	0 / 0	-18.5	-18.5	0.02 (4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 10 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.24/1.00 (B-C:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.10 (B) (INPUT = 1.00)

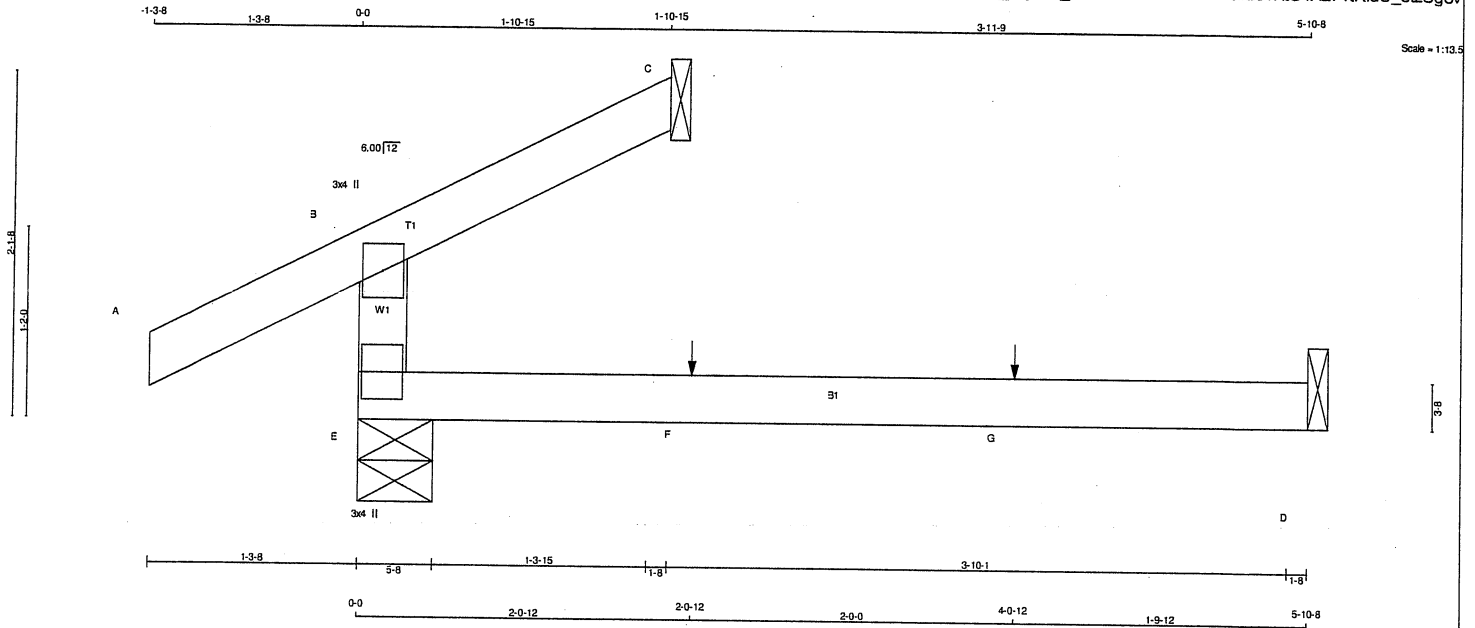


Structural component only
DWG# T-2115749

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	J15	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
E	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	297	0	297	0	5-8	5-8
C	66	0	66	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	212	130 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	46	37 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	WEBS		MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
				FROM	TO		
FR-TO							
E-B		-234 / 0	0.0	0.0	0.13 (4)	7.81	
A-B		0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C		-10 / 0	-91.8	-91.8	0.06 (1)	10.00	
E-F		0 / 0	-18.5	-18.5	0.13 (4)	10.00	
F-G		0 / 0	-18.5	-18.5	0.13 (4)	10.00	
G-D		0 / 0	-18.5	-18.5	0.13 (4)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
G	4-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.13/1.00 (B-E:4), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

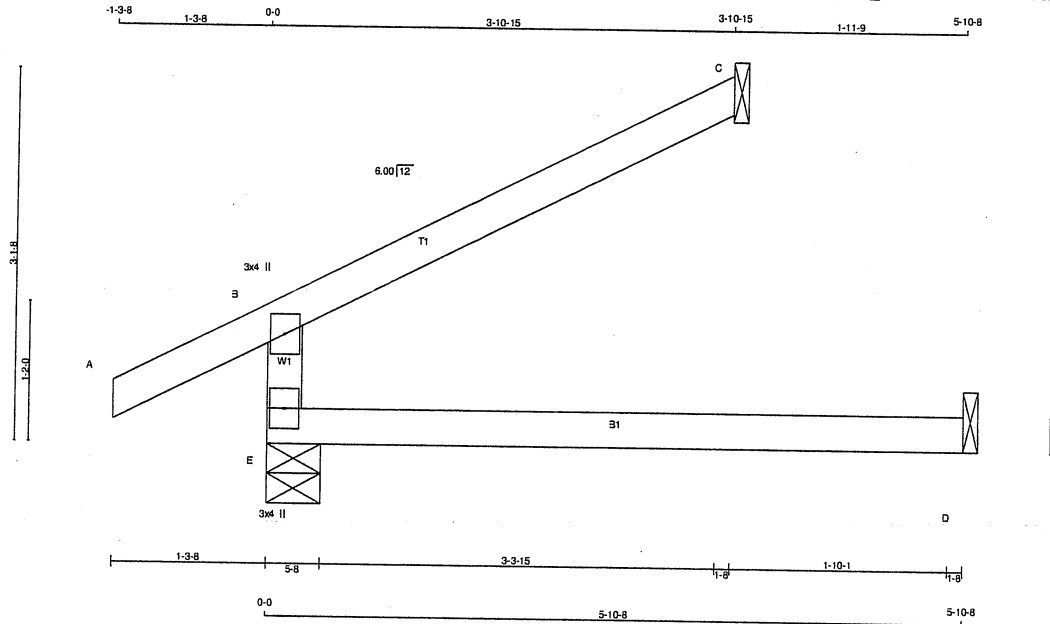
JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115750

JOB NAME 413212	TRUSS NAME J16	QUANTITY 1	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:18.4

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	412	0	412	0
C	135	0	135	0
D	45	0	50	0

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	291	194 / 0	0 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
C	93	75 / 0	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	UNBRAC LENGTH	FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX	CSI (LC)
E-B		-349 / 0	0.0	0.0	0.13 (4)	7.81					
A-B		0 / 28	-91.8	-91.8	0.12 (1)	10.00					
B-C		-20 / 0	-91.8	-91.8	0.24 (1)	6.25					
E-D		0 / 0	-18.5	-18.5	0.13 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.24/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

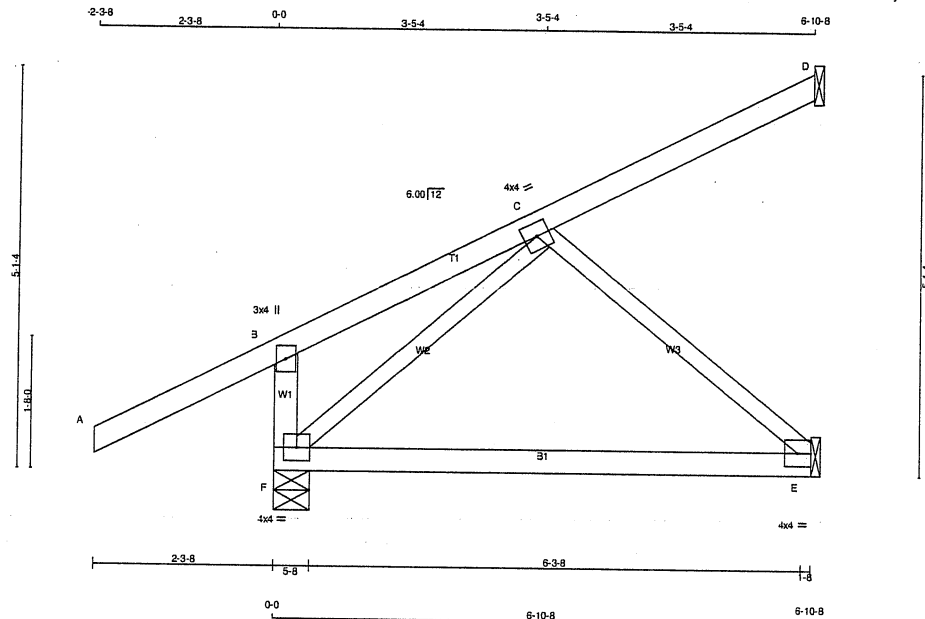
JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115751

JOB NAME 413212	TRUSS NAME J61	QUANTITY 6	PLY 1	JOB DESC. ROYAL PINE HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:5sPQm3ewyQLR2FGj1wwUPeyYVZn-D6C5U739vnJ9jmvRzAtWEKd9kupUbwgdmx5AmzCg8t



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
F	592	0	592	0
D	125	0	125	0
E	256	0	256	0

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE	
F	415	295 / 0	0 / 0	0 / 0	119 / 0
D	86	70 / 0	0 / 0	0 / 0	16 / 0
E	183	107 / 0	0 / 0	0 / 0	75 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO				FR-TO			
F-B	-331 / 0	0.0	0.04 (1)	F-C	-296 / 0	0.10 (1)	
A-B	0 / 48	-91.8	0.40 (5)	C-E	-292 / 0	0.11 (1)	
B-C	0 / 17	-91.8	0.18 (1)				
C-D	-17 / 0	-91.8	0.18 (1)				
F-E	0 / 219	-18.5	0.26 (4)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 6 X 28 = 168 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD = 39.0 PSF			

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/849 (0.10")

CSI: TC=0.40/1.00 (A-B:5), BC=0.26/1.00 (E-F:4),
WB=0.11/1.00 (C-E:1), SSI=0.17/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PL)	SECTION (PL)
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.29 (C) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)

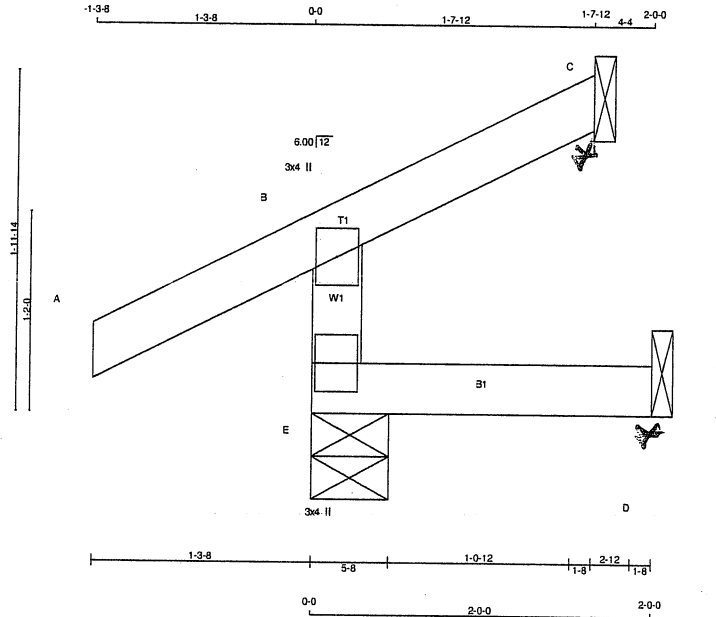


Structural component only
DWG# T-2115752

JOB NAME 413212	TRUSS NAME J62	QUANTITY 3	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 7 = 21 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS	2x4	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
E	268	0	268	0	5-8	5-8	5-8	5-8
C	36	0	36	0	-27	1-8	1-8	1-8
D	9	0	18	0	0	1-8	1-8	1-8

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	186	139 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	25	19 / -20	0 / 0	0 / 0	0 / 0	6 / 0	0 / 0
D	8	0 / -7	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC (LC)	MEMB. FORCE (LBS)	MAX. UNBRAC (LC)	MAX. FORCE (LBS)	MAX. UNBRAC (LC)
FR-TO		FROM TO			FR-TO			
E-B	-240 / 0	0.0	0.0	0.04 (5)	7.81			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-18 / 0	-91.8	-91.8	0.09 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.03/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

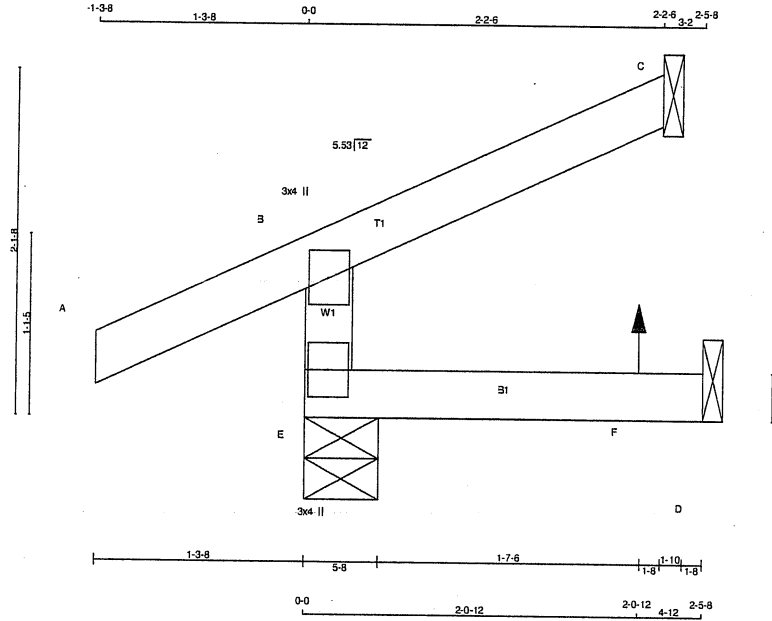


Structural component only
DWG# T-2115753

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES	DRWG NO.
413212	J63	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 8 = 24 lb [M]

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
E - B 2x4 DRY No.2 SPF
A - C 2x4 DRY No.2 SPF
E - D 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
JT				
E	275	0	275	0
C	76	0	76	0
D	17	0	25	0

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT							
E	192	138 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
C	52	42 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	14	0 / 4	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	UNBRAC LENGTH FR-TO	
E-B	-250 / 0	0.0	0.02 (4)	7.81
A-B	0 / 26	-91.8	0.13 (5)	10.00
B-C	-10 / 0	-91.8	0.07 (1)	6.25
E-F	0 / 0	-18.5	0.03 (4)	10.00
F-D	0 / 0	-18.5	0.03 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	2	1	2	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:5), BC=0.03/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

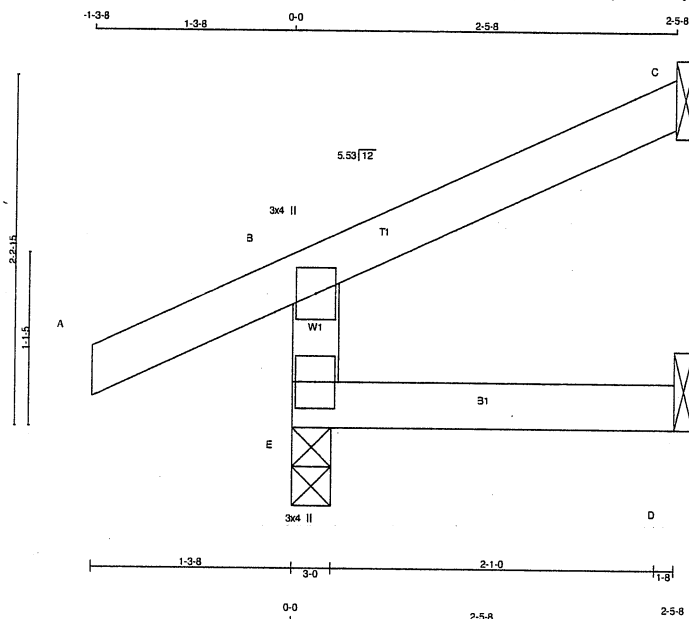


Structural component only
DWG# T-2115754

JOB NAME 413212	TRUSS NAME J64	QUANTITY 5	PLY 1	JOB DESC. ROYAL PINE HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:14.1

LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS SIZE	LUMBER
E - B 2x4 DRY	No.2
A - C 2x4 DRY	No.2
E - D 2x4 DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	290	0	290	0	3-0	3-0
C	85	0	85	0	1-8	1-8
D	20	0	22	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	203	148 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	58	47 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	16	0 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO		FROM	TO		FR-TO		
E-B	-265 / 0	0.0	0.0	0.02 (4)	7.81		
A-B	0 / 26	-91.8	-91.8	0.12 (5)	10.00		
B-C	-12 / 0	-91.8	-91.8	0.09 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 8 = 42 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B-5), BC=0.03/1.00 (D-E-4), WB=0.00/1.00 (n/a-0), SSI=0.10/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

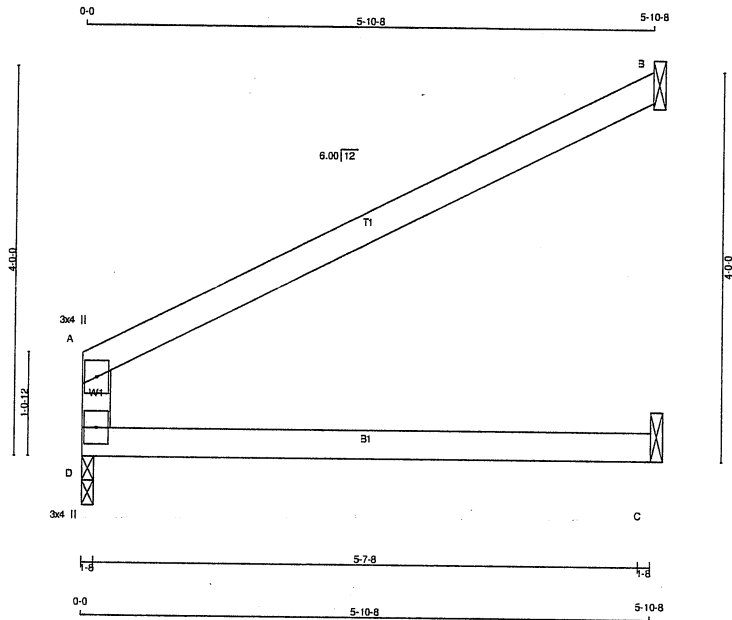
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2115755



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	3.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
D	324	0	324	0	1-8
B	245	0	245	0	1-8
C	79	0	79	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS				
D	229	150 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
B	169	134 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0
C	60	17 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO
D-A	-294 / 0	0.0 0.0 0.16 (1)	7.81
A-B	-11 / 0	-91.8 -91.8 0.45 (1)	6.25
D-C	0 / 0	-18.5 -18.5 0.23 (1)	10.00

TOTAL WEIGHT = 15 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 589 (0.12")

CSI: TC=0.45/1.00 (A-B:1) , BC=0.23/1.00 (C-D:1) ,
WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

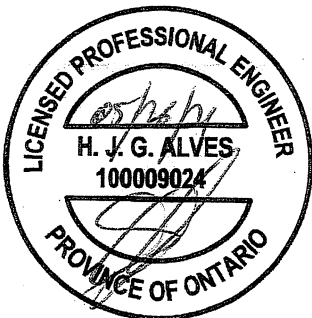
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (D) (INPUT = 0.90)
JSI METAL= 0.08 (A) (INPUT = 1.00)





Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MH7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900213

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
Minimum Number of Nails					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

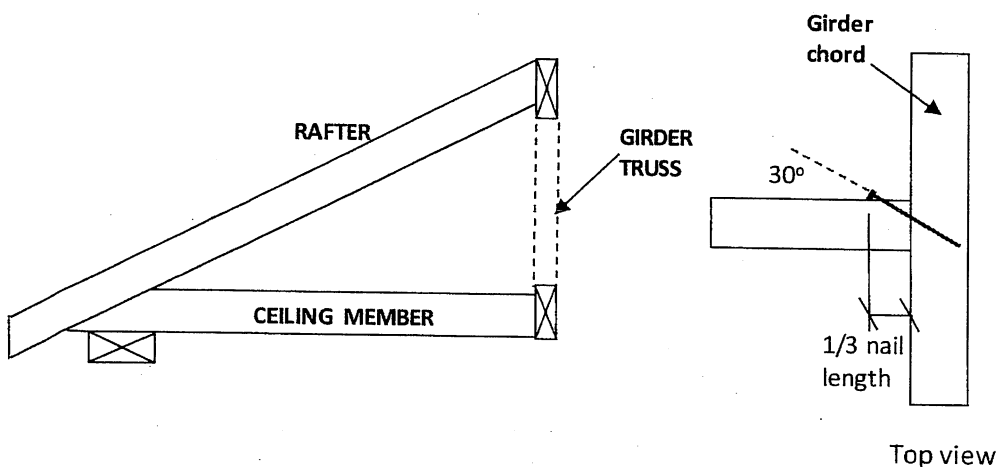
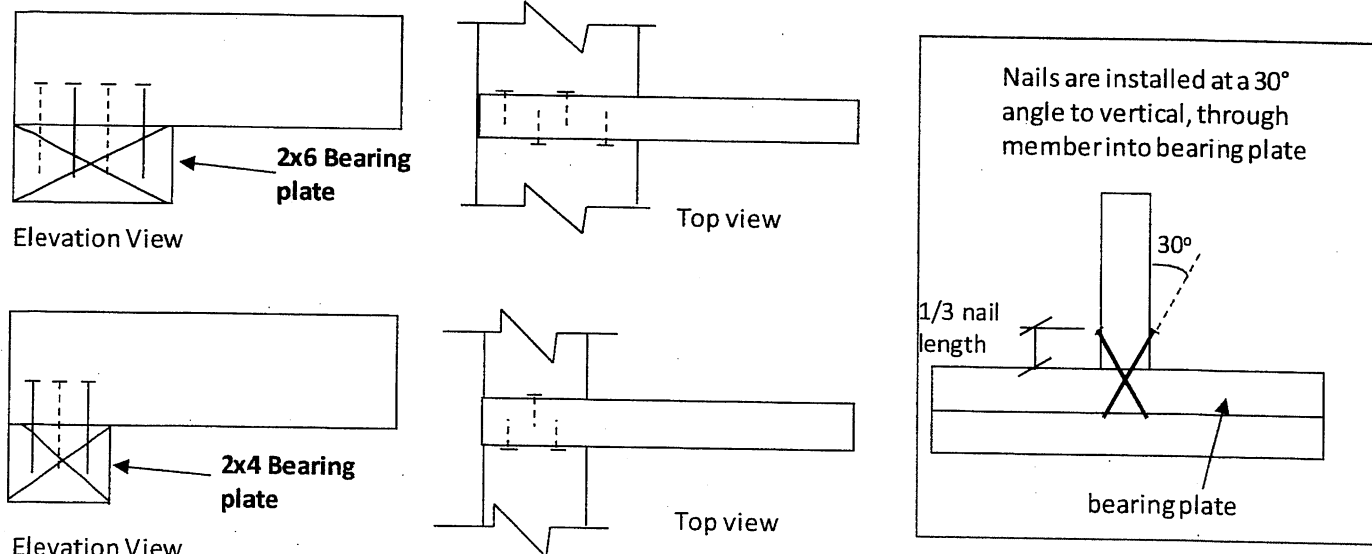


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
 a) easier installation, b) higher capacities, c) lower installed
 cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

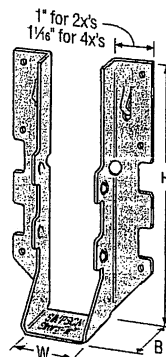
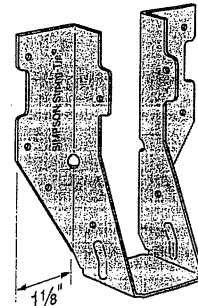
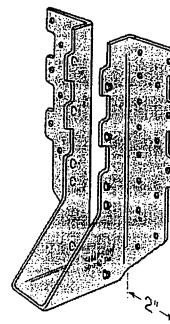
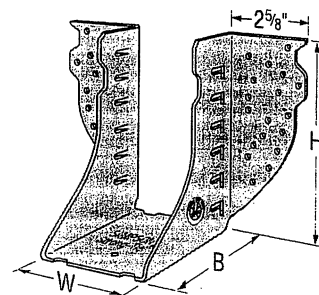
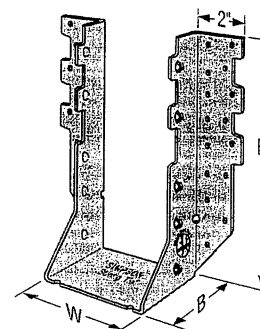
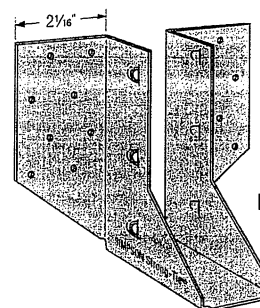
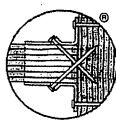
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

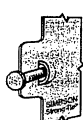
Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

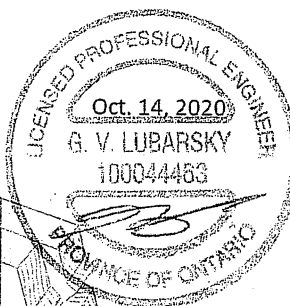
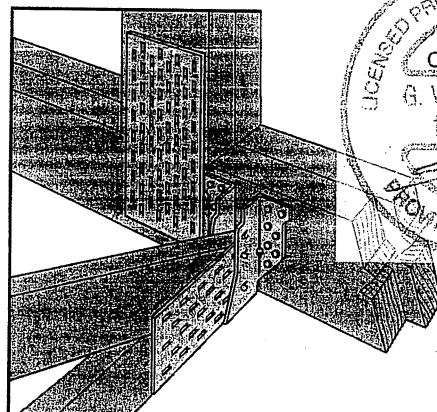
Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.


LUS28

LU26L

HUS210
 (HUS26, HUS28,
 and HHUS similar)

HGUS28-2

HHUS210-2

LJS26DS

 Double-Shear
 Nailing
 Top View

 Double-Shear
 Nailing
 Side View;
 Do not
 bend tab

 Dome Double-Shear
 Nailing
 Side View
 (available on
 some models)

**Typical HUS26
 Installation
 with Reduced
 Heel Height**
 (Truss Designer
 to provide
 fastener quantity
 for connecting
 multiple members
 together)



LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

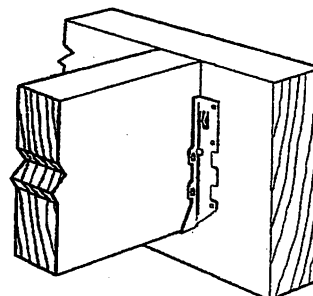
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

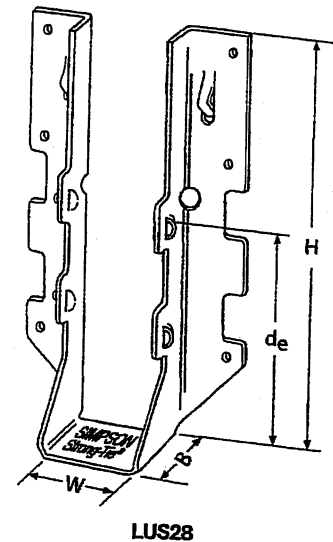
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

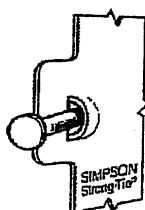


Typical LUS Installation



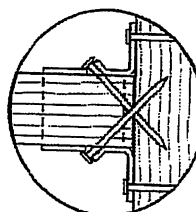
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1⅞	3⅞	1¾	1⅞	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅞	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⅞	4¾	1¾	3⅞	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅞	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅞	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅞	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅞	8⅞	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

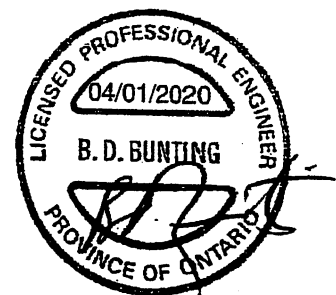


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

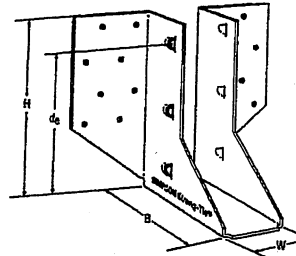
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

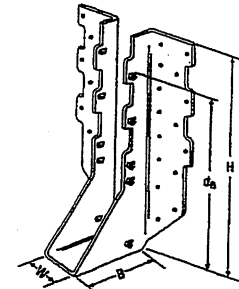
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

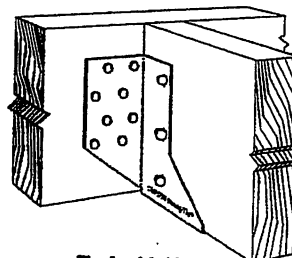
- See current catalogue for options



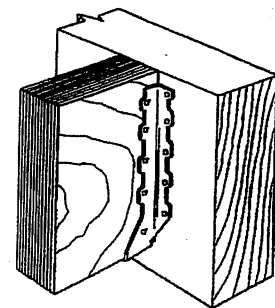
LJS26DS



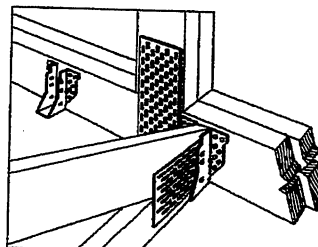
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



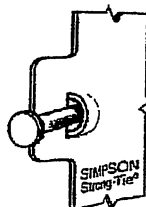
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

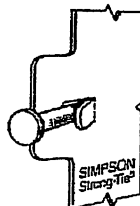
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 5/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 5/8	7 3/8	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 5/8	9 3/8	3	7 3/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

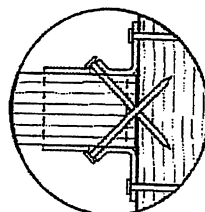


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

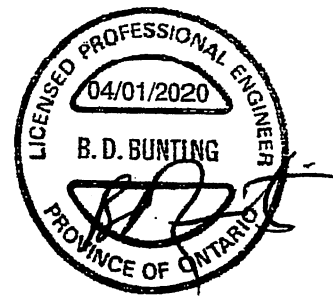
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

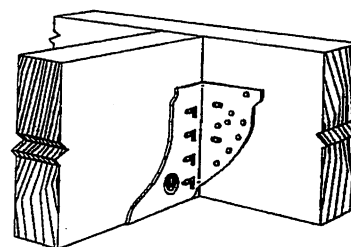
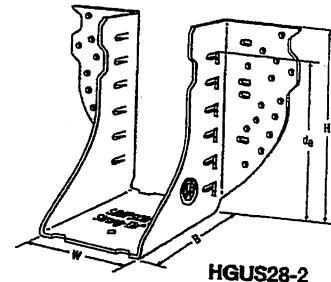
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

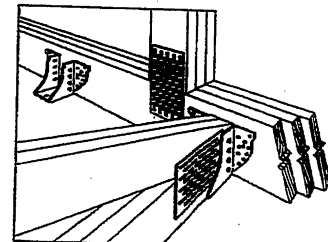
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



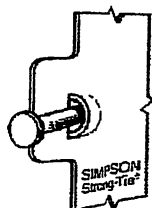
Typical HGUS Installation



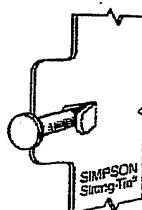
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _g =1.15)	Normal (K _g =1.00)	Uplift (K _g =1.15)	Normal (K _g =1.00)
HGUS26	12	1⅞	5⅞	5	4⅝	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅞	5⅞	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4⅞	5⅞	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4⅞	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅞	7⅞	5	6⅞	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅞	7⅞	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4⅞	7⅞	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6⅞	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅞	9⅞	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⅞	9⅞	4	8⅞	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4⅞	9⅞	4	8⅞	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅞	9⅞	4	8⅞	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅞	10⅞	4	10⅞	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅞	12⅞	4	11⅞	(66) 16d	(22) 16d	10130	16400	7195	11645

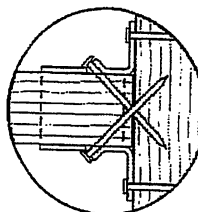
1. d_g is the distance from the seat of the hanger to the highest joist nail.



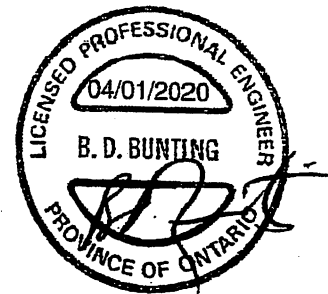
U.S. Patent
5,603,580



Double Shear Nailing Side View.
Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

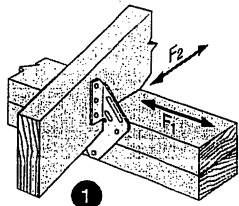
© 2020 Simpson Strong-Tie Company Inc.

T-SPECHGUS20 3/20 exp. 6/22

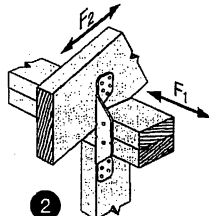
(800) 999-5099
strongtie.com

H/TSP

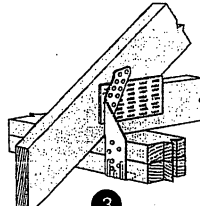
Seismic and Hurricane Ties (cont.)



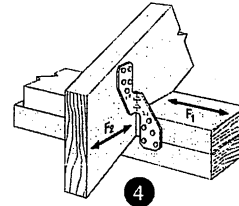
1 H1 Installation



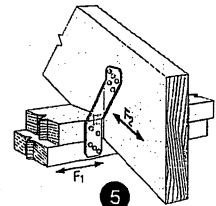
2 H2A Installation



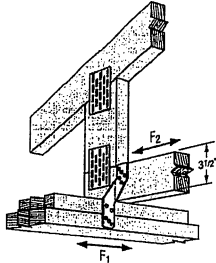
3 TSP Installation



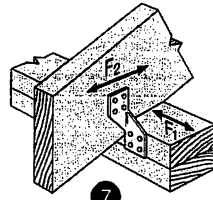
4 H2.5A Installation
(Nails into both top plates)



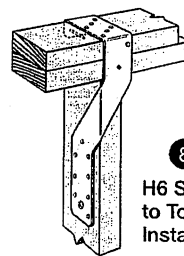
5 H2.5T Installation
(Nails into both top plates)



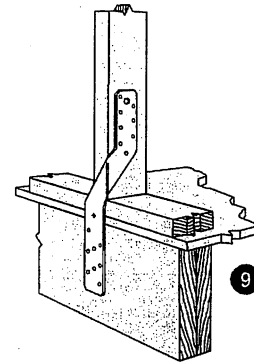
6 H2.5T Installation



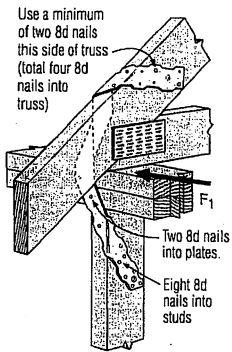
7 H3 Installation
(Nails into upper top plate)



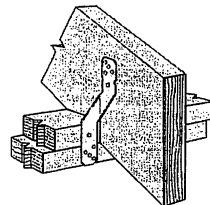
8 H6 Stud to Top Plate Installation



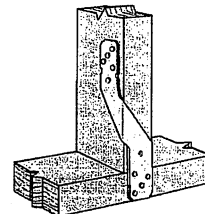
9 H6 Stud to Band Joist Installation



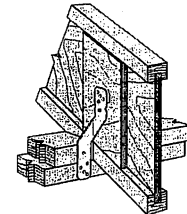
10 H7Z Installation



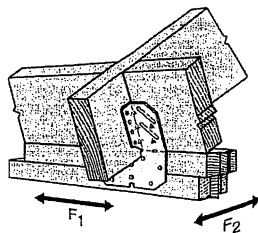
11 H8 Attaching Rafter to Double Top Plates



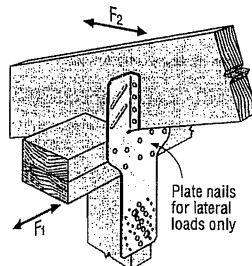
12 H8 attaching Stud to Sill
(4 8d into plate, 5 8d into stud)



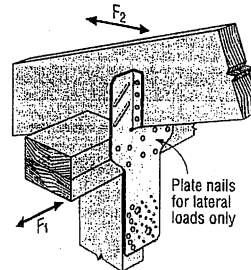
13 H8 attaching I-Joist to Double Top Plates



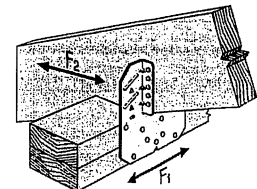
14 H10A Field-Bent Installation



15 H10S Installation

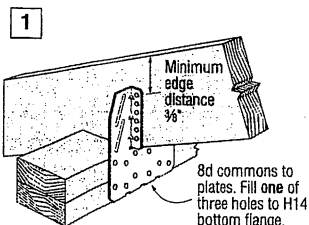


16 H10S Installation with Stud Offset

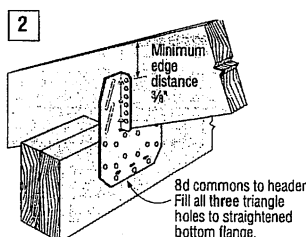


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

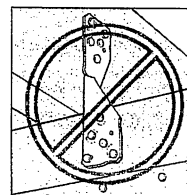


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

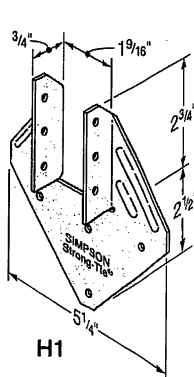
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

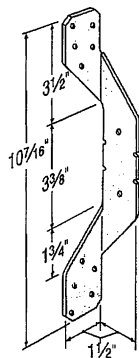
Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

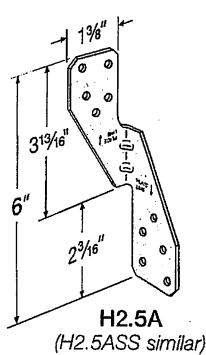
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



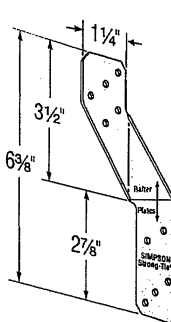
H1



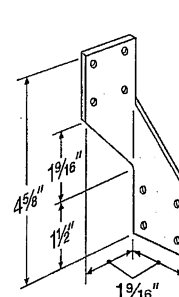
H2A
(H2ASS similar)



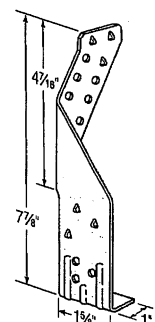
H2.5A
(H2.5ASS similar)



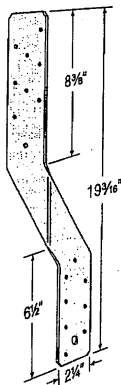
H2.5T



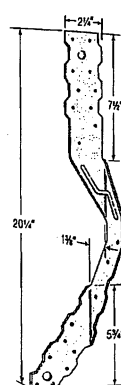
H3



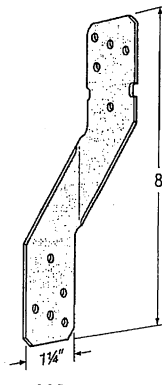
TSP



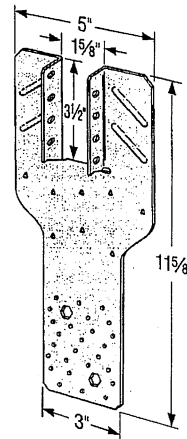
H6



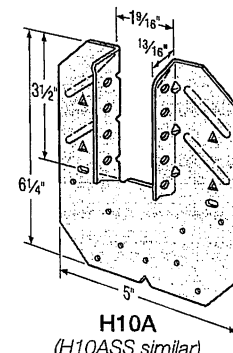
H7Z



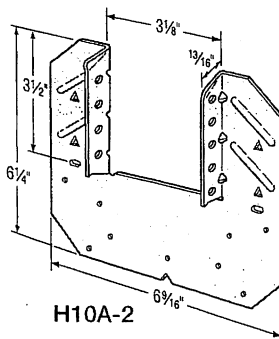
H8



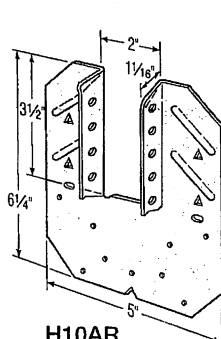
H10S



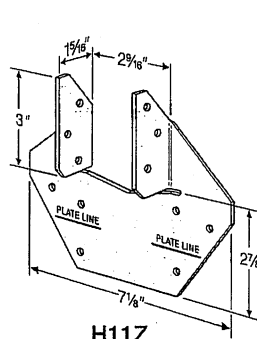
H10A
(H10ASS similar)



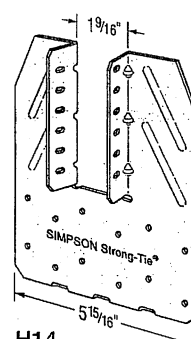
H10A-2



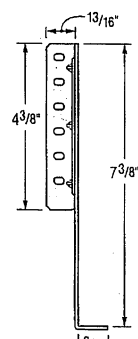
H10AR



H11Z



H14



H14
Profile

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

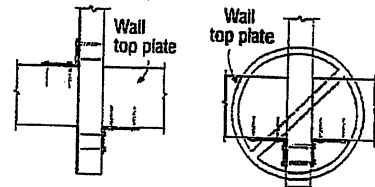
Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

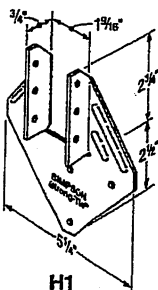
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

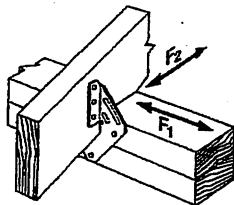


Install diagonally across from each other for minimum 2x truss.

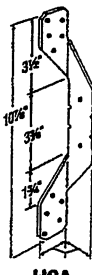
Nailing into both sides of a single ply 2x truss may cause the wood to split.



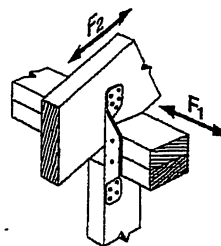
H1



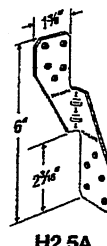
H1 Installation



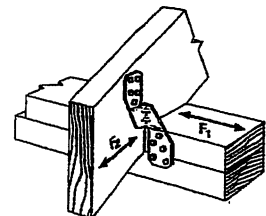
H2A



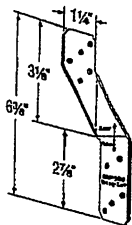
H2A Installation



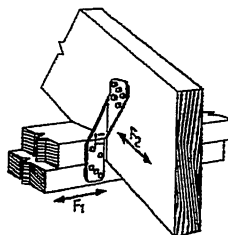
H2.5A



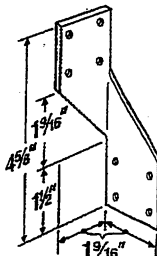
H2.5A Installation



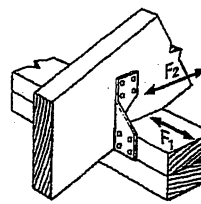
H2.5T



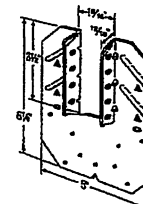
H2.5T Installation
(Nails into both top plates)



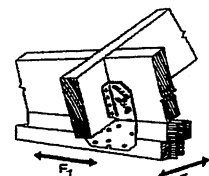
H3



H3 Installation



H10A

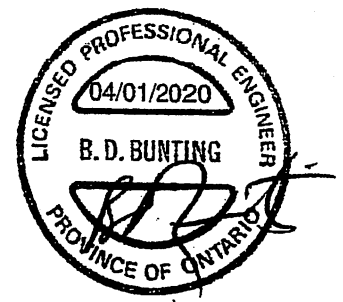


H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
										(K _o =1.15)
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

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T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

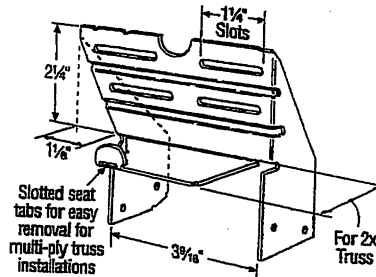
Design: Factored resistances are in accordance with CSA 086-14

Installation:

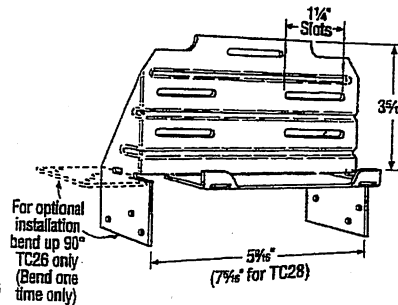
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

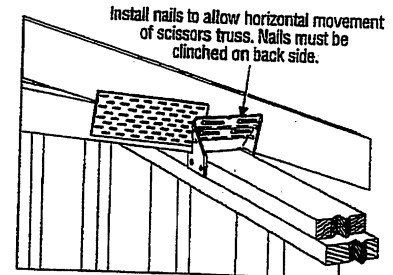
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



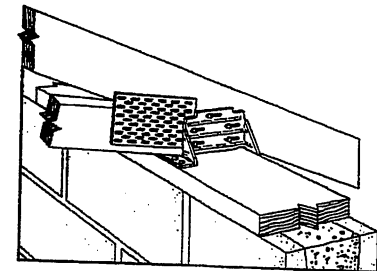
TC24
U.S. Patent 4,932,173



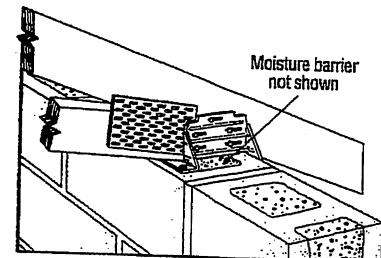
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
TC24	(4) 10d	(4) 10d	lb.	lb.
TC26	(5) 10d	(6) 10d	605	430
TC28	(5) 10d	(6) 10d	1015	720
			1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



**LIMIT
STATES
DESIGN**

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T-SPECTC20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

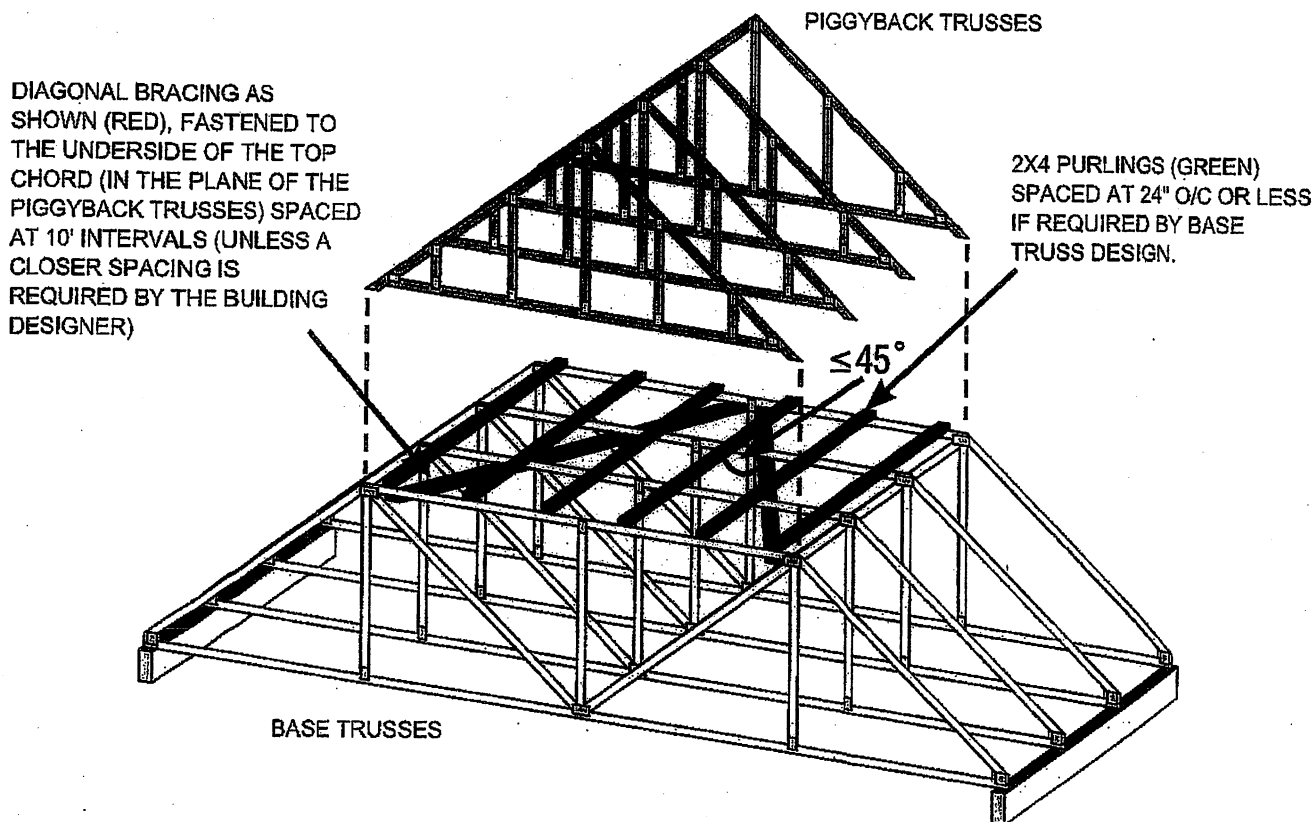
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

TS/LTS/MTS/HTS

Twist Straps

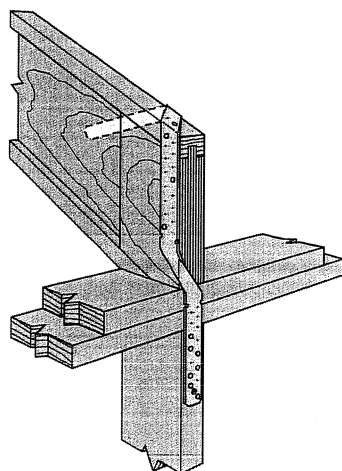
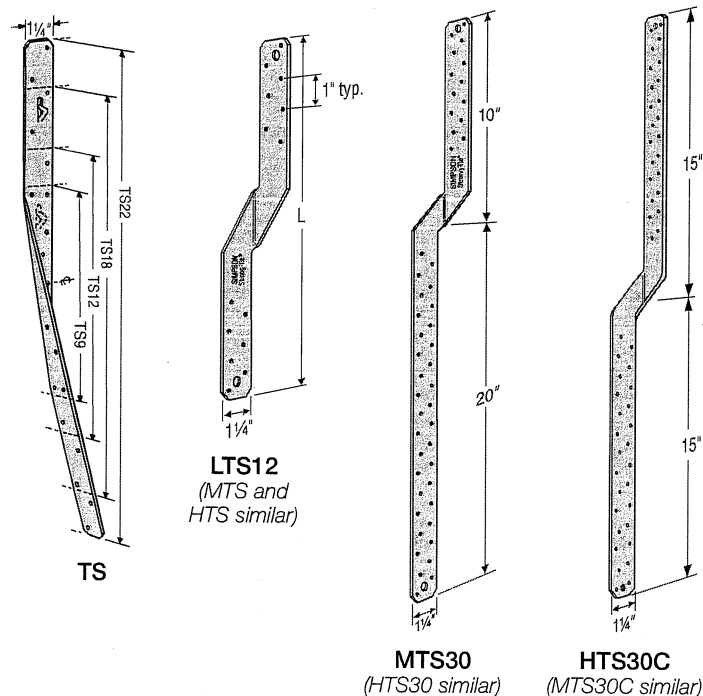
Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3"-bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

Material: LTS — 18 gauge; MTS/TS — 16 gauge; HTS — 14 gauge

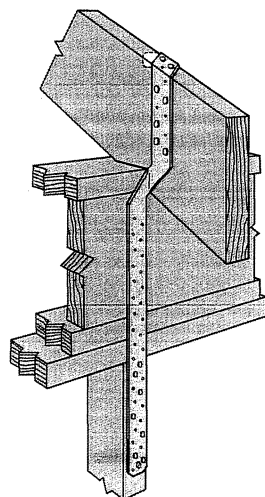
Finish: Galvanized. Some products available in stainless steel and ZMAX® coating; see Corrosion Information, pp.20–24.

Installation:

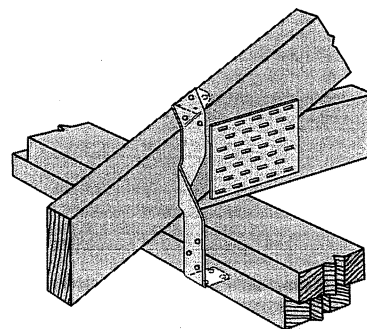
- Use all specified fasteners; see General Notes
- TS should be installed in pairs to reduce eccentricity
- When LTS/MTS is installed as truss-to-top plate tie, install (3) 10d x 1½" nails to the underside of the plate and (3) 10d x 1½" nails into the edge of the double top plate
- LTS, MTS and HTS are available with the bend reversed. Specify "-REV" after model number, such as MTS16-REV



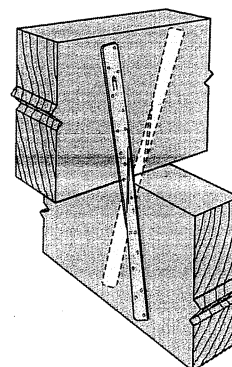
MTS30 Installation
with I-Joist Rafter



Typical MTS30
Installation



MTS Installation as a
Truss-to-Top Plate Tie



Typical TS
Installation

TS/LTS/MTS/HTS

Twist Straps (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

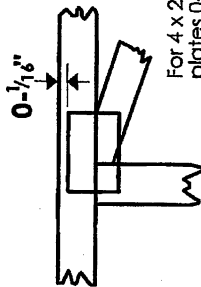
These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	L (in.)	Fasteners (Total)	Factored Resistance ($K_D = 1.15$)	
			D.Fir-L	S-P-F
			lb.	lb.
			kN	kN
TS9	9	(8) 16d	1125	1040
			5.00	4.63
TS12	11½	(10) 16d	1410	1300
			6.27	5.78
TS18	17¾	(14) 16d	1970	1820
			8.76	8.10
TS22	21¾	(18) 16d	2125	2125
			9.45	9.45
SS LTS12	12	(12) 10d x 1½"	1015	720
			4.52	3.20
LTS16	16	(12) 10d x 1½"	1015	720
			4.52	3.20
SS LTS18	18	(12) 10d x 1½"	1015	720
			4.52	3.20
LTS20	20	(12) 10d x 1½"	1015	720
			4.52	3.20
MTS12	12	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS16	16	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS18	18	(14) 10d x 1½"	1570	1180
			6.98	5.25
SS MTS20	20	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS30	30	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS24C	24	(14) 10d x 1½"	1570	1180
			6.98	5.25
MTS30C	30	(14) 10d x 1½"	1570	1180
			6.98	5.25
HTS16	16	(16) 10d x 1½"	2050	1455
			9.12	6.47
HTS20	20	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS24	24	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS28	28	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS30	30	(24) 10d x 1½"	2050	1455
			9.12	6.47
HTS30C	30	(24) 10d x 1½"	2050	1455
			9.12	6.47

1. LTS12 through LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading. No further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30 have the twist in the centre of the strap.
5. Twist straps do not have to be wrapped over the truss to achieve the load.
6. Optional nail holes are provided on some straps.
7. When used as a truss-to-top plate tie multiply the tabulated values by 0.95 for LTS and 0.74 for MTS. HTS cannot be used in this application.
8. **Nails:** 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

PLATE LOCATION AND ORIENTATION

Technical drawing of a mechanical part, likely a bracket or support. A dimension line indicates a length of $1\frac{3}{4}$ inches. The drawing shows a cross-section of the part with a central rectangular hole and a smaller square hole within it.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

4
x
4

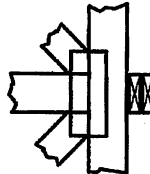
LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.



BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.



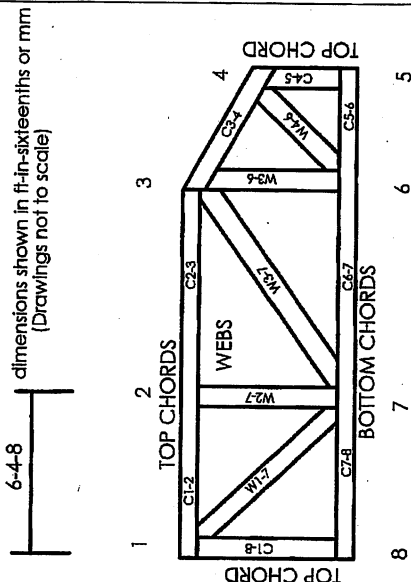
Industry Standards:

Truss Design Procedures and Specifications
for Light Metal Plate Connected Wood Trusses
Design Standard for Bracing
Building Component Safety Information,
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BC3.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Elliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, warts and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIIC Quality Criteria.



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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 MATtek Engineering Reference Sheet: MII-7473C rev. 10-'08