

		Products		
PlotID	Length	Product	Plies	Net Qty
J1	10-00-00	11 7/8" NI-40x	1	8
J1DJ	10-00-00	11 7/8" NI-40x	2	2
J2	8-00-00	11 7/8" NI-40x	1	6
J3	2-00-00	11 7/8" NI-40x	1	4
J4	20-00-00	11 7/8" NI-80	1	16
B2	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B1	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	_ 1	1
B3	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

Connector Summary						
Qty Manuf Product						
6	H1	IUS2.56/11.88				
4	H1	IUS2.56/11.88				
1	H2	HUS1.81/10				



FROM PLAN DATED:

BUILDER: ROYAL PINE HOMES

SITE: CENTREFIELD WEST GORMLEY

MODEL: 2007

ELEVATION: A,B

LOT:

CITY: RICHMAND HILL

SALESMAN: MARIO DICIANO

DESIGNER: AJ REVISION:

NOTES:

REFER TO THE **NORDIC INSTALLATION**GUIDE FOR PROPER STORAGE AND

INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.

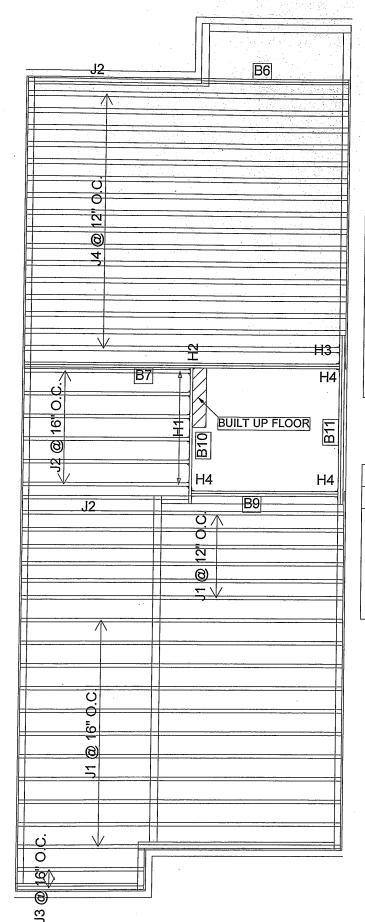
LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 20.0 lb/ft²

SUBFLOOR: 3/4" GLUED AND NAILED

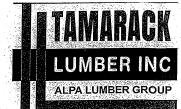
DATE: 2021-06-23

1st FLOOR



Products								
PlotID	Length	Product	Plies	Net Qty				
J1	20-00-00	11 7/8" NI-40x	1	17				
J2	12-00-00	11 7/8" NI-40x	1	8				
J3	8-00-00	11 7/8" NI-40x	1	2				
J4	20-00-00	11 7/8" NI-80	1	16				
B7	20-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2				
B11	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2				
B6	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2				
B9	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2				
B10	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1				

Connector Summary							
Qty	Manuf	Product					
6	H1	IUS2.56/11.88					
1	H2	HUS1.81/10					
1	H3	IUS3.56/11.88					
1	H4	HGUS410					
2	H4	HGUS410					



FROM PLAN DATED:

BUILDER: ROYAL PINE HOMES

SITE: CENTREFIELD WEST GORMLEY

MODEL: 2007

ELEVATION: A,B

LOT:

CITY: RICHMAND HILL

SALESMAN: MARIO DICIANO

DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC INSTALLATION **GUIDE** FOR PROPER STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4. 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE **SQUASH BLOCKS** REQ'D UNDER CONCENTRATED LOADS, SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' **OVER BRICK** REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD **CUT OPENINGS** SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 20.0 lb/ft²

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 2021-06-07

2nd FLOOR

NORDIC

INSTALLATION GUIDE NORDIC JOIST

NS-GI33 **■**◆ ENGLISH

Engineered Wood Products

BASIC INSTALLATION **GUIDE FOR** RESIDENTIAL **FLOORS**

JOIST

NORDIC

NAIL SPACING

nordic.ca

1 x 2-5/16 Minimum widt

1-1/2 x 2-5/16 Minimum width

INSTALLING NORDIC I-JOISTS

- 2. Except for cutting to length, I-joist flanges should never be cut, drilled or notche Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
- Concentrated loads should only be applied to the top surface of the top flange. Concentrated loads should not be bottom flange with the exception of light loads, such as ceiling fans or light fixtures.
- I-joists must not be used in applications where they will be permanently exposed to weather, or will reach a moisture content of 15 percent or greater, such as in swimming pool or hot tub areas. They must not be installed where they will remain in direct contact wi
- End bearing length must be at least 1-3/4 inch. For multiple-span joists, intermediate bearing length must be at least 3-1/2 inches
- 8. Ends of floor joists shall be restrained to prevent rollover. Use rim board or I-joist blocking panels.
- (cripple blocks) to transfer gravity loads from above the floor system to the wall or foundation below.
- 0. For I-joists installed directly beneath bearing walls parallel to the joists or used as rim board or blocking panels, using a single I-joist is 3,300 plf, and 6,600 plf if double I-joists are used.
- . Continuous (ateral support of the I-joist's compression flange is required to prevent rotation and buckling. In simple span uses, latera support of the top flange is normally supplied by the floor sheathing. In multiple-span or cantilever applications, bracing of the I-joist's bottom flange is also required at interior supports of multiple-span joists, and at the end support next to the cantilever extension. The ends of all cantilever extensions must be laterally braced as shown in details 3, 4, or 5,
- 2. Nails installed in flange face or edge shall be spaced in accordance
- with the applicable building code requirements or approved building plans, but should not be closer than those specified on page 3.3 of the Nordic Joist Technical Guide (NS-GT3).
- 3. Details 1 show only I-joist-specific fastener requirements. For other fastener requirements, see the applicable building code.
- 4. For proper temporary bracing of wood I-joists and placement of temporary construction loads, see APA Technical Note: Temporary Construction Loads over I-Joist Roofs and Floors, Form J735.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity.

SAFETY AND CONSTRUCTION PRECAUTIONS

- Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/ or cross-bridging at joist ends. When I-joists are applied continuous over interior supports support.
- When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2-inch nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the
- end of each bay. Lap ends of adjoining bracing over at least two I-loists. . Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet
- For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panel rim board, or cross-bridging.
- . Install and fully nail permanent sheathing to each I-joist before placing loads on the floo. system. Then, stack building materials over beams or walls only.
- . Never install a damaged I-loist.

NI-60

33 pieces per unit

RESIDENTIAL SERIES

2x8 1950f MSR 3/8 in. web

33 pieces per unit

mproper storage or installation, failure to follow applicable building codes, failure to follow an ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure ouse web stiffeners when required can result in serious accidents. Follow these installation

2×4 2100f MSR

23 pieces per un



WEB HOLES IN I-JOISTS

- Holes measuring 1-1/2 inch or smalle be permitted subject to verification

A 1-1/2 inch hole or smaller can be staced anywhere in the web provide

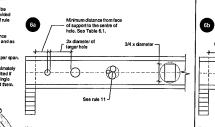
Never stack building materials over unsheathed I-joists. Once sheathed, de

RIM BOARDS

Width Length 1-1/8 in. 16 ft

APA Rim Board Plu

Do not walk on I-joist until fully fastened an braced, or serious injuries can result.



DUCT CHASE OPENINGS HOLES IN BLOCKING PANELS

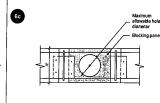
rum depth of a duct chase opening that can be cut into an 1-joist equal the clear distance between the flanges of the 1-joist minus

All openings shall be cut in accordance with the restrictions listed above and as illustrated in detail 6b.

tules for Cutting Duct Chase Openings in 1-jols

- - The maximum allowable hole size for a lateral-restraint-only blocking panel is 2/3 of the lesser dimension of the blocking's depth or length. Assuming the blocking panel is longer than its height (or depth), For other applications, contact Nordic Structures.

 - Field-cut holes must be centred in the blocking horizontally
 - While round holes are preferred, rectangle holes may be used provided the corners are not over cut. Slightly rounding corners or pre-drilling con-with a 1-inch-diameter bit is recommended.
 - All holes must be cut in a workman-like manner in accordance with the limitations listed above.

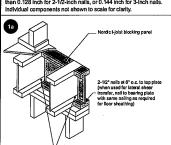


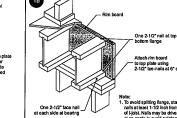
I-joist or rim board blocking depth (in.)	Maximum allowable hole diameter (in.) (*)
9-1/2	6-1/4
11-7/8	7-3/4
14	9-1/4
16	10-1/2



19

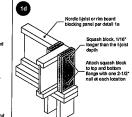
1

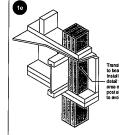




NORDIC I-JOIST SERIES

NI-20 2x8 S-P-F No. 2

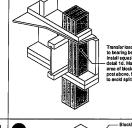


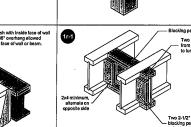


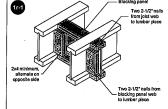
NI-90

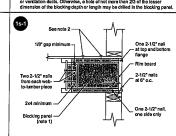
2×4 2400f MSF

23 pieces per uni







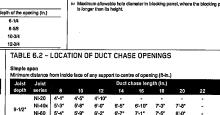




construction details <u>→DC3</u>

TABLE 6.1 - LOCATION OF WEB HOLES

Joist spacing

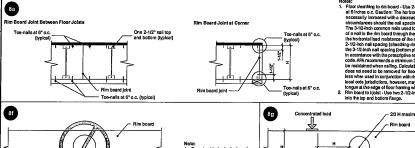


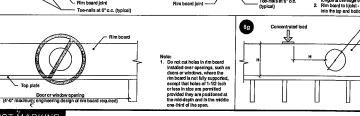
Joist spacing Up to 24 inches

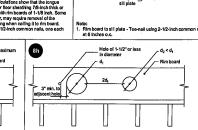
Loads Live load = 40 psf and dead load = 15 psf

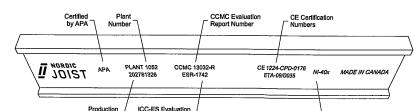
Deflection limits L/480 under live load and L/240 under total

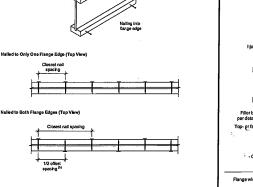
RIM BOARDS

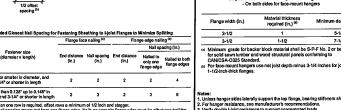


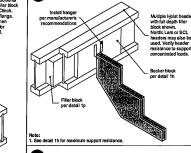


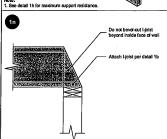


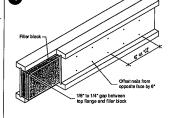












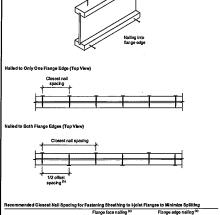
- Support cauch or Fjoss was suring insuling to prevent currings to veronizing to Leave at 196, the 144-inch gap between top of filler block and bottom of top I-joist flange.

Filler block its required between joists for full length of span.

For flange width of 2-1/2 inches, nell joists (opether with two rows of 3-inch malls at 12 Inches o... (clinched when possible) on each side of the duble.



ICC-ES Evaluation



COMPANYJuly 22, 2020 08:23

PROJECT
J5 1ST FLOOR.wwb

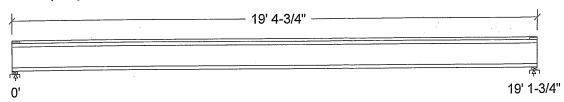
Design Check Calculation Sheet

Nordic Sizer - Canada 7.2

Loads:

Load	Туре	Distribution	Pat-	Location	[ft]	Magnitude		Unit
12044	-21		tern	Start	End	Start	End	
Load1	Dead	Full Area				20.00		psf
Load2	Live	Full Area				40.00		psf

Maximum Reactions (lbs) and Support Bearing (in):



Unfactored: Dead Live	191 383	191 383
Factored: Total	814	814
Bearing: Capacity Joist Support	2188 5573	2188 5573
Des ratio Joist Support Load case	0.37 0.15 #2	0.37 0.15 #2
Length Min req'd	2-3/8 1-3/4 No	2-3/8 1-3/4 No
Stiffener KD KB support	1.00 1.00	1.00
fcp sup Kzcp sup	769 1.09	769 1.09

Nordic Joist 11-7/8" NI-80 Floor joist @ 12" o.c.

Supports: All - Lumber Sill plate, No.1/No.2

Total length: 19' 4-3/4"; Clear span: 19'; 3/4" nailed and glued OSB sheathing

This section PASSES the design code check.

Limit States Design using CSA 086-14 and Vibration Criterion:

Analysis Value	Design Value	Unit	Analysis/Design
Vf = 814	Vr = 2336		Vf/Vr = 0.35
Mf = 3894	Mr = 11609	lbs-ft	ME/MD 0.34
$0.11 = \langle L/999 \rangle$	0.64 = L/360	in 🔏	0.17
0.22 = < L/999	0.48 = L/480	in 🎉	4161 0.35
0.33 = L/690	0.96 = L/240	in	0.35
0.25 = L/920	0.64 = L/360	in 🕼	WOS 0 839
Lmax = 19'-1.8	Lv = 21'-2.7	ft 🖁	S. KATSOULAHOS 0 139
= 0.026	= 0.033	in l	0.78
	Vf = 814 Mf = 3894 0.11 = < L/999 0.22 = < L/999 0.33 = L/690 0.25 = L/920 Lmax = 19'-1.8	Vf = 814	Vf = 814

STRUCTURAL COMPONENT ONLY

WoodWorks® Sizer

for NORDIC STRUCTURES

J5 1ST FLOOR.wwb

Nordic Sizer - Canada 7.2

Page 2

```
Additional Data:
                                                                         LC#
                                                                   KN
FACTORS:
                     KD
                            KH
                                             KL
                                                           KS
                                                                         #2
            2336
                    1.00
                           1.00
 Vr
                                                                         #2
                                           1.000
           11609
                    1.00
                           1.00
Mr+
                                                                         #2
           547.1 million
F.T
CRITICAL LOAD COMBINATIONS:
          : LC \#2 = 1.25D + 1.5L
 Shear
 Moment(+): LC #2
                   = 1.25D + 1.5L
                            (permanent)
 Deflection: LC #1
                    = 1.0D
                    = 1.0D + 1.0L (live)
             LC #2
                    = 1.0D + 1.0L
             LC #2
                                   (total)
                   = 1.0D + 1.0L (bare joist)
             LC #2
           : Support 1 - LC \# 2 = 1.25D + 1.5L
 Bearing
             Support 2 - LC \# 2 = 1.25D + 1.5L
 Load Types: D=dead W=wind S=snow H=earth, groundwater E=earthquake
             L=live(use, occupancy) Ls=live(storage, equipment)
 Load Patterns: s=S/2 L=L+Ls _=no pattern load in this span
 All Load Combinations (LCs) are listed in the Analysis output
CALCULATIONS:
 Eleff = 625.37 lb-in^2 K = 6.18e06 lbs
 "Live" deflection is due to all non-dead loads (live, wind, snow...) CONFORMS TO OBC 2012
```

Design Notes:

AMENDED 2020

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.





COMPANY July 22, 2020 08:27 **PROJECT**J5 2ND FLOOR.wwb

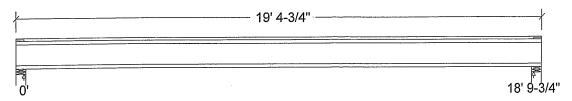
Design Check Calculation Sheet

Nordic Sizer - Canada 7.2

Loads:

Load	Type	Distribution	Pat-	Location	[ft]	Magnitude		Unit
1			tern	Start	End	Start	End	
Load1	Dead	Full Area				20.00		psf
Load2	Live	Full Area				40.00		psf

Maximum Reactions (Ibs) and Support Bearing (in):



Unfactored: Dead Live	188 376	188 376
Factored: Total	800	800
Bearing: Capacity Joist Support	2336 10841	2336 10841
Des ratio Joist Support Load case Length	0.34 0.07 #2 4-3/8	0.34 0.07 #2 4-3/8
Min req'd Stiffener KD	1-3/4 No 1.00	1-3/4 No 1.00
KB support	769 -	769

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

Nordic Joist 11-7/8" NI-80 Floor joist @ 12" o.c.

Supports: All - Lumber Wall, No.1/No.2

Total length: 19' 4-3/4"; Clear span: 18' 8"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling

This section PASSES the design code check.

Limit States Design using CSA 086-14 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 800	Vr = 2336	lbs	VI/VI = 0.34
Moment(+)	Mf = 3760	Mr = 11609	lbs-ft	Mf/Mi 0,32
Perm. Defl'n	0.11 = < L/999	0.63 = L/360	in ,	19 1111 0117
Live Defl'n	0.21 = < L/999	0.47 = L/480	in /	0.45
Total Defl'n	0.32 = L/712	0.94 = L/240	in //	S. KATSOULAKOS : 34
Bare Defl'n	0.23 = L/966	0.63 = L/360	in 🖁	S. KATSOULAKOS:
Vibration	Lmax = 18'-9.8	Lv = 20'-5.8	ft 🕻	0.92
Defl'n	= 0.027	= 0.033	in 🔻	181
				The second of the second

NACE OF WEND. TAN 9/48-21
STRUCTURAL

WoodWorks® Sizer

for NORDIC STRUCTURES

J5 2ND FLOOR.wwb

Nordic Sizer - Canada 7.2

Page 2

3											
Additiona	l Data:										
FACTORS:			KH		KL	KT	KS	KN	LC#		
	2336				-	-	-	-	#2		
	11609			-	1.000	-	_	-	#2		
	547.1 m			-	-	_	-	-	#2		
CRITICAL L	OAD COMB	INATIONS	S:								
Shear	: LC #2	= 1.25	5D + 1.5I								
Moment (+) : LC #2	= 1.25	5D + 1.5I		,						
Deflecti	on: LC #1	= 1.01) (perma	anent)							
·	LC #2	= 1.01	+ 1.0L	(live)						
			+ 1.0L								
	LC #2	= 1.01	+ 1.0L	(bare	joist)						
Bearing	: Suppo	rt 1 - I	LC #2 = 3	1.25D +	1.5L						
1	Suppo	rt 2 - I	10 #2 = 1	1.25D +	1.5L						
Load Typ	es: D=dea	d W=wir	nd S=sno	ow H=ea	arth,grou	.ndwater	r E=ear	thquake			
	L=liv	e(use,oc	ccupancy) Ls=1:	ive(stora	.ge,equi	.pment)	f=fire			
Load Pat	terns: s=	S/2 L=I	L+Ls _=r	no patte	ern load	in this	s span				
All Load	Combinat	ions (LO	Cs) are	listed :	in the An	alysis.	output				
CALCULAT											
ETeff =	613.27 lb	-in^2 F	<= 6.18e	e06 lbs							
"Live" d	eflection	is due	to all r	non-dead	d loads (live, w	ind, sno	ow) cal	iporms 1	in obc	2012
								o eq.			
1									AMBRUBE	'A AAA	.a. I

Design Notes:

AMENDED 2020

- 1. WoodWorks analysis and design are in accordance with the 2015 National Building Code of Canada (NBC), Division B, Part 4, and the CSA O86-14 Engineering Design in Wood standard, Update No. 2 (June 2017).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to Nordic Structures technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.

S KATSOULAKOS S

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OWB NO. TAM 9/46 -21

STRUCTURAL

COMPONENT ONLY





PASSED

1ST FLR FRAMING\Flush Beams\B1(i416) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 09:22:00

Build 7493

Job name:

Address: City, Province, Postal Code: RICHMAND HILL File name: Description: UNIT 2007 EL A, B. mmdl

1ST FLR FRAMING\Flush Beams\B1(i416)

Specifier:

ΑJ

Wind

Code reports:

BC CALC® Member Report

Customer:

CCMC 12472-R

Designer: Company:

在1920年1920年1920年1920年1920年1年1920年1920年192	commence and a substitution of the contract of
08-03-12	
_	08-03-12

Total Horizontal Product Length = 08-03-12

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 1-7/8"	37 / 0	43 / 0
B2. 4-3/8"	39 / 0	45 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-03-12	Тор		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-03-12	Тор	9	5			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	212 ft-lbs	17696 ft-lbs	1.2%	1	04-00-10
End Shear	78 lbs	7232 lbs	1.1%	1	01-01-12
Total Load Deflection	L/999 (0.004")	n\a	n\a ·	4	04-00-10
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	04-00-10
Max Defl.	0.004"	n\a	n\a	4	04-00-10
Span / Depth	8.0				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 1-3/4"	109 lbs	5.4%	2.7%	Spruce-Pine-Fir
B2	Wall/Plate	4-3/8" x 1-3/4"	115 lbs	2.4%	1.2%	Spruce-Pine-Fir

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBC 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B2(i573) (Flush Beam)

PASSED

BC CALC® Member Report

Build 7773

Dry | 1 span | No cant.

June 23, 2021 10:22:26

Job name:

Address:

Customer:

Code reports:

City, Province, Postal Code: RICHMAND HILL

CCMC 12472-R

File name: Description:

UNIT 2007 EL A, B.mmdl

1ST FLR FRAMING\Flush Beams\B2(i573)

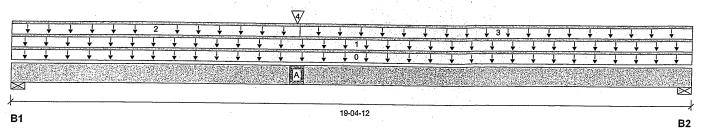
Specifier:

Designer:

AJ

Wind

Company:



Total Horizontal Product Length = 19-04-12

Snow

Reaction Summary (Down / Uplift) (Ibs)

Bearing Live Dead B1, 1-7/8" 1229 / 0 745 / 0 B2, 1-7/8" 892/0 572 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	-
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-04-12	Тор		12			00-00-00
1	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	19-04-12	Тор	20	10			n\a
2	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	00-00-00	08-01-02	Тор	22	11			n\a
3	FC1 Floor Decking (Plan View Fill)	Unf. Lin. (lb/ft)	L	08-01-02	19-04-12	Тор	6	3			n\a
4	B3(i574)	Conc. Pt. (lbs)	L	08-00-04	08-00-04	Тор	1488	767			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	18632 ft-lbs	35392 ft-lbs	52.6%	1	08-00-04
End Shear	2655 lbs	14464 lbs	18.4%	1	01-01-12
Total Load Deflection	L/306 (0.753")	n\a	78.4%	4	09-02-08
Live Load Deflection	L/487 (0.473")	n\a	73.9%	5	09-02-08
Max Defl.	0.753"	n\a	n\a	4	09-02-08
Span / Depth	19.4				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	1-7/8" x 3-1/2"	2774 lbs	68.7%	34.7%	Spruce-Pine-Fir
B2	Wall/Plate	1-7/8" x 3-1/2"	2052 lbs	50.8%	25.6%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

CONFORMS TO OBE 2012

Design meets Code minimum (L/360) Live load deflection criteria.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 11-01-12.



000 NO. TAM 13692-21 STRUCTURAL COMPONENT ONLY





Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B2(i573) (Flush Beam)

PASSED

BC CALC® Member Report

Build 7773

Dry | 1 span | No cant.

June 23, 2021 10:22:26

Job name:

Address:

City, Province, Postal Code: RICHMAND HILL

Customer:

Code reports:

CCMC 12472-R

File name:

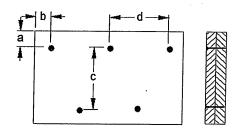
UNIT 2007 EL A, B. mmdl Description: 1ST FLR FRAMING\Flush Beams\B2(i573)

Specifier:

Designer: AJ

Company:

Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8"

b minimum = 3"

d= 100 11

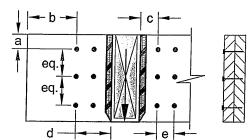
Connectors are:

- **1** ... Nails

ARDOX SPIRAL

Connection Diagrams: Concentrated Side Loads

Connection Tag: A Applies to load tag(s): 3



a minimum = 2"

b minimum = 4"

c minimum = 4"

d maximum = 12"

e minimum = 4"

Connectors are: 5

Nails

ARDOX SPIRAL



OWO NO. TAM 1369221 STRUCTURAL COMPONENT ONLY

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BC CALC®, BC FRAMER® , AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1ST FLR FRAMING\Flush Beams\B3(i574) (Flush Beam)

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

June 23, 2021 10:22:26

Build 7773

Job name:

Address:

City, Province, Postal Code: RICHMAND HILL

Customer: Code reports:

CCMC 12472-R

UNIT 2007 EL A, B. mmdl

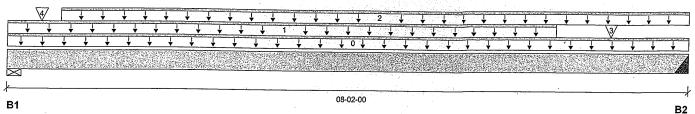
Description: 1ST FLR FRAMING\Flush Beams\B3(i574)

File name:

Specifier:

Designer:

AJ Company:



Total Horizontal Product Length = 08-02-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead		
B1, 5-1/2"	1906 / 0	1029 / 0		
B2, 2"	1517 / 0	782 / 0		

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	_
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-02-00	Top		6			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-00-00	06-06-08	Тор	216	96			n\a
2	STAIR	Unf. Lin. (lb/ft)	L	00-07-08	08-02-00	Top	240	120			n\a
3	J2(i550)	Conc. Pt. (lbs)	L	07-02-08	07-02-08	Тор	199	100		1000	n\a
4	1(i140)	Conc. Pt. (lbs)	L	00-04-12	00-04-12	Тор		126	·	SOFE!	SS/O/V _{UD} Ta

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	6310 ft-lbs	17696 ft-lbs	35.7%	1	04-05-08
End Shear	2920 lbs	7232 lbs	40.4%	1	01-05-06
Total Load Deflection	L/999 (0.096")	n\a	n\a	4	04-02-08
Live Load Deflection	L/999 (0.064")	n\a	n\a	5	Ó4-02-08
Max Defl.	0.096"	n\a	n\a	4	04-02-08
Span / Depth	7.7				

Bearing	յ Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	5-1/2" x 1-3/4"	4145 lbs	70.0%	35.3%	Spruce-Pine-Fir
B2	Hanger	2" x 1-3/4"	3253 lbs	n\a	76.2%	HUS1.81/10

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Hanger Manufacturer: Unassigned

CONFORMS TO DECEMBE AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Calculations assume unbraced length of Top: 00-00-00, Bottom: 01-01-08.

COMPONENT ONLY Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a Olygualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

STRUCTURAL

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®. VERSA-LAM®, VERSA-RIM PLUS®.







PASSED

2ND FLR FRAMING\Flush Beams\B6(i267) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 08:51:14

BC CALC® Member Report Build 7493

Job name:

Address:

Customer: Code reports:

City, Province, Postal Code:

CCMC 12472-R

File name:

UNIT 2007 EL A,B.mmdl

Description: 2ND FLR FRAMING\Flush Beams\B6(i267)

Specifier:

Designer: Company:

V V V V V V V V V V V V V V V V V V V

В1

Total Horizontal Product Length = 08-10-14

Summary (Down / Unlift) (lhs)

Reaction Sui	Illiary (Down / Of	mit (iba)			
Bearing	Live	Dead	Snow	Wind	
B1, 5-1/2"	1281 / 0	1544 / 0	2490 / 0	·	
B2, 4-3/8"	1249 / 0	1507 / 0	2427 / 0		

١٨	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf, Lin. (lb/ft)	L	00-00-00	08-10-14	Тор		12			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-10-14	Top	20	10			n\a
2	E26(i263)	Unf. Lin. (lb/ft)	L	00-00-00	03-07-00	Тор		81			n\a
3	E26(i263)	Unf. Lin. (lb/ft)	L	00-00-00	03-03-00	Top	264	240	552		n\a
4	E27(i275)	Unf. Lin. (lb/ft)	L	03-07-00	06-01-00	Top		61			n\a
5	E28(i276)	Unf. Lin. (lb/ft)	L	06-01-00	08-10-14	Top		81			n\a
6	E28(i276)	Unf. Lin. (lb/ft)	L	06-05-00	08-10-14	Тор	264	240	552		n\a
7	E26(i263)	Conc. Pt. (lbs)	L	03-06-00	03-06-00	Top	426	411	891		n\a
, 8	E28(i276)	Conc. Pt. (lbs)	L	06-02-00	06-02-00	Тор	410	396	857		n\a

Cantuala Cummanu	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Controls Summary	Factored Demand				
Pos. Moment	12287 ft-lbs	35392 ft-lbs	34.7%	13	03-06-00
End Shear	4715 lbs	14464 lbs	32.6%	13	01-05-06
Total Load Deflection	L/999 (0.118")	n\a	n\a	35	04-05-05
Live Load Deflection	L/999 (0.084")	n\a	n\a	51	04-05-05
Max Defl.	0.118"	n\a	n\a	35	04-05-05
Span / Depth	8.3				

Bearin	g Supports	Dim. (LxW)	Demand	Resistance Support	Resistance Member	Material
B1	Wall/Plate	5-1/2" x 3-1/2"	6946 lbs	58.7%	29.6%	Spruce-Pine-Fir
B2	Wall/Plate	4-3/8" x 3-1/2"	6773 lbs	71.9%	36.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

CONFORMS TO OBE 2012

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Unbalanced snow loads determined from building geometry were used in selected product's

verification.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9



846 NO. TAM 9/56 STRUCTURAL COMPONENT ONLY





PASSED

2ND FLR FRAMING\Flush Beams\B6(i267) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 08:51:14

BC CALC® Member Report

Build 7493 Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports:

CCMC 12472-R

File name:

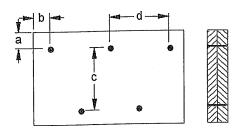
UNIT 2007 EL A,B.mmdi

Description: 2ND FLR FRAMING\Flush Beams\B6(i267)

Specifier: Designer:

Company:

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 24 8 "

Connectors are:

Nails ARDOX SPIRAL



OWG NO. TAM 9156-21 STRUCTURAL COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



PASSED

July 27, 2020 08:17:43

2ND FLR FRAMING\Flush Beams\B7(i472) (Flush Beam)

BC CALC® Member Report

Build 7493

Job name: Address:

Dry | 1 span | No cant.

UNIT 2007 EL A,B.mmdl

File name: Description:

ΑJ

2ND FLR FRAMING\Flush Beams\B7(i472)

Specifier:

Designer:

Customer: Code reports:

CCMC 12472-R

Company:

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2																															
					_									1	9-00-	06															

Total Horizontal Product Length = 19-00-06

City, Province, Postal Code: RICHMAND HILL

Reaction Sun	nmary (Down / U)	plift) (lbs)		,	
Bearing	Live	Dead	Snow	Wind	
B1. 4-3/8"	911 / 0	581 / 0			
B2, 4"	982 / 0	617 / 0			

1		d Cummary						Live	Dead	Snow	Wind	Tributary
		d Summary Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
-	ι ας Ί	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-00-06	Тор		12			00-00-00
,	1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	10-02-06	Top	27	13			n\a
٠,	ו כ	FC2 Floor Material	Unf. Lin. (lb/ft)	L	10-02-06	19-00-06	Top	23	11	•		n\a
4	_			1	10-01-08	10-01-08	Top	1422	734			n\a
- 3	3	B10(i465)	Conc. Pt. (lbs)	L	10-01-00	10-01-00	ioh	1422	104			ma

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	16887 ft-lbs	35392 ft-lbs	47.7%	1	10-01-08
End Shear	2161 lbs	14464 lbs	14.9%	1	17-08-08
Total Load Deflection	L/352 (0.629")	n\a	68.1%	4	09-08-09
Live Load Deflection	L/562 (0.394")	n\a	64.1%	5	09-08-09
Max Defl.	0.629"	n\a	n\a	4	09-08-09
Snan / Denth	18.7				

Rearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	4-3/8" x 3-1/2"	2093 lbs	22.2%	11.2%	Spruce-Pine-Fir
B2	Hanger	4" x 3-1/2"	2244 lbs	n\a	13.1%	HGUS410

Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CONFORMS TO OBC 2012

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

AMENDED 2020

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9



OWO NO. TAM 9157 -21 STRUCTURAL COMPONENT ONLY



PASSED

July 27, 2020 08:17:43

2ND FLR FRAMING\Flush Beams\B7(i472) (Flush Beam)

BC CALC® Member Report

Build 7493

Job name:

Address:

City, Province, Postal Code: RICHMAND HILL

Customer: Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

UNIT 2007 EL A, B. mmdl

File name: Description:

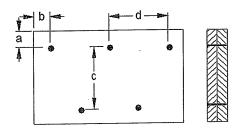
2ND FLR FRAMING\Flush Beams\B7(i472)

Specifier:

ΑJ Designer:

Company:

Connection Diagram: Full Length of Member

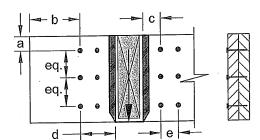


a minimum = 2" b minimum = 3" c = 7-7/8" d = 26'8''

Connectors are: ARDOX SPIRAL

Connection Diagrams: Concentrated Side Loads

Applies to load tag(s): 2 Connection Tag. A



a minimum = 2"

b minimum = 4"

c minimum = 4"

d maximum = 12"

e minimum = 4"

Nails

ARDOX SPIRAL



Disclosure

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BC CALC®, BC FRAMER® , AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



PASSED

B2

July 22, 2020 08:51:14

2ND FLR FRAMING\Flush Beams\B9(i260) (Flush Beam)

BC CALC® Member Report

Build 7493

Job name:

Address: City, Province, Postal Code:

Customer: Code reports: Dry | 1 span | No cant.

UNIT 2007 EL A,B.mmdl

File name:

Description: 2ND FLR FRAMING\Flush Beams\B9(i260)

Specifier:

Designer:

Company:

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- 	ŢŢ	Ť	Į.	Į Į	Ţ	To the second second	¥	¥	ţο	<u> </u>	į į	22452-24522	AND TORK	1	Taxana	Į	į .		1 1	Ţ		Ţ	SERVICE CONTRACTOR

B1

Total Horizontal Product Length = 08-10-00

Reaction Summary (Down / Unlift) (lbs)

CCMC 12472-R

Neaction of	Allilliary (Down 1 o	אווינין (וויסטו			
Bearing	Live	Dead	Snow	Wind	
B1, 4"	85 / 0	360 / 0			
B2, 4"	85 / 0	360 / 0			

Los	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-10-00	Тор		12			00-00-00
1	WALL	Unf. Lin. (lb/ft)	L	00-00-00	08-10-00	Top		60			n\a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	08-10-00	Тор	19	10			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	982 ft-lbs	23005 ft-lbs	4.3%	0	04-05-00
End Shear	354 lbs	9401 lbs	3.8%	0	01-03-14
Total Load Deflection	L/999 (0.011")	n\a	n\a	4	04-05-00
Live Load Deflection	L/999 (0.002")	n\a	n\a	5	04-05-00
Max Defl.	0.011"	n\a	n\a	4	04-05-00
Span / Depth	8.4				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material	
B1	Hanger	4" x 3-1/2"	505 lbs	n\a	4.5%	HGUS410	
B2	Hanger	4" x 3-1/2"	505 lbs	n\a	4.5%	HGUS410	

Cautions

Header for the hanger HGUS410 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HGUS410 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

CONFORMS TO OBC 2012 AMENDED 2020

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

DANCE OF ON

146 NO. TAM 9/59 STRUCTURAL COMPONENT ONLY





PASSED

2ND FLR FRAMING\Flush Beams\B9(i260) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 08:51:14

BC CALC® Member Report Build 7493

Job name: Address:

City, Province, Postal Code:

Customer: Code reports:

CCMC 12472-R

UNIT 2007 EL A,B.mmdl File name:

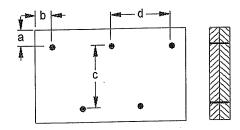
Description: 2ND FLR FRAMING\Flush Beams\B9(i260)

Specifier:

Designer:

Company:

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 20 8 "

Connectors are: ...

ARDOX SPIRAL

PONNICE OF CHILD'S 146 NO. TAN 9159 STRUCTURAL

COMPONENT ONLY

Disclosure

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PASSED

2ND FLR FRAMING\Flush Beams\B10(i465) (Flush Beam)

Dry | 1 span | No cant. **BC CALC® Member Report**

July 27, 2020 08:17:43

Build 7493

Job name:

Address:

City, Province, Postal Code: RICHMAND HILL

Customer: CCMC 12472-R Code reports:

UNIT 2007 EL A, B. mmdl

File name: 2ND FLR FRAMING\Flush Beams\B10(i465) Description:

Wind

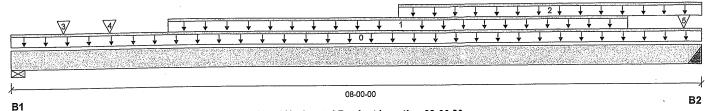
Live

Dead

Specifier:

Designer: ΑJ

Company:



Total Horizontal Product Length = 08-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead
B1, 5-1/2"	995 / 0	841 / 0
B2 2"	1435 / 0	740 / 0

1.0	ad Summary						Live	Dead	Snow	vvina	Tributa
Tag	ad Summary Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-00-00	Тор		6			00-00-0
1	Smoothed Load	Unf. Lin. (lb/ft)	L	01-09-08	07-01-08	Top	201	101			r
2	STAIR	Unf. Lin. (lb/ft)	L	04-05-01	08-00-00	Тор	240	120			r
3	B9(i345)	Conc. Pt. (lbs)	L	00-07-04	00-07-04	Тор	114	375		British Charles	in r
	J2(i477)	Conc. Pt. (lbs)	L	01-01-08	01-01-08	Top	210	105	45 F. 1831	Carly In the same	i Vermen
4 5	J2(1477) J2(1452)	Conc. Pt. (lbs)	L	07-09-08	07-09-08	Top	170	85	19	No. of the last of	'n
ິນ	JZ(143Z)	00,,0,, , , , , , , , , , , , , , , , ,				•			19 L	4/6	
			Factored	Dem	and/	_					61/00

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4677 ft-lbs	17696 ft-lbs	26.4%	1	04-11-06
End Shear	2133 lbs	7232 lbs	29.5%	1	06-10-02
Total Load Deflection	L/999 (0.067")	n\a	n\a	4	04-03-03
Live Load Deflection	L/999 (0.044")	n\a	n\a	5	04-03-03
Max Defl.	0.067"	n\a	n\a	4	04-03-03
Span / Depth	7.6				

Rearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1		5-1/2" x 1-3/4"	2543 lbs	43.0%	21.7%	Spruce-Pine-Fir
B2	Hanger	2" x 1-3/4"	3077 lbs	n\a	72.1%	HUS1.81/10

Cautions

Header for the hanger HUS1.81/10 is a Double 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user. Hanger has not been analyzed for adequate capacity.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00. GQNY 0 RMS TO 0BC 2012

Hanger Manufacturer: Unassigned

15

AMENDED 2020

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

DWG NO. TAM9/60 -21 STRUCTURAL COMPONENT ONLY

POPMOE OF CHILD

Wind

Snow

Tributary

00-00-00 n\a n∖a

n\a

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PASSED

2ND FLR FRAMING\Flush Beams\B11(i258) (Flush Beam)

Dry | 1 span | No cant.

July 22, 2020 08:51:14

BC CALC® Member Report Build 0

Job name:

Address:

City, Province, Postal Code:

Customer:

Code reports:

CCMC 12472-R

File name:

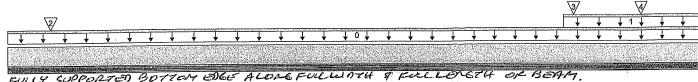
UNIT 2007 EL A, B. mmdi

Description: 2ND FLR FRAMING\Flush Beams\B11(i258)

Specifier:

Designer:

Company:



Total Horizontal Product Length = 09-11-12

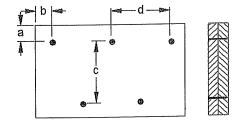
Los	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End .	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-11-12	Тор		12			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	08-00-00	09-11-12	Top	6				n\a
2	B9(i260)	Conc. Pt. (lbs)	L	00-07-04	00-07-04	Top	87	362			n\a
3	B7(i144)	Conc. Pt. (lbs)	L	08-01-12	08-01-12	Top	96	98			n\a
4	J5(i200)	Conc. Pt. (lbs)	L	09-01-08	09-01-08	Top	397	198			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Loca	tion	
Dist, Load	13.28 lb/ft	57645.00 lb/ft	n\a				
Conc. Load	843 lbs	16813 lbs	5.0%	的的相談	6 5111 C	T A	ΩD

CONFORMS TO OBC 2012

Connection Diagram: Full Length of Member

AMENDED 2020



a minimum = 2" b minimum = 3"

 $c = 7 - \frac{7}{8}$ d = 2 6 4

Calculated Side Load = 421.5 lb/ft Connectors are: 16d And Nails

ARDOX SPIRAL

POMANCE OF ON

DWE NO. TAN916/-28 STRUCTURAL COMPONENT ONLY

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Maximum Floor Spans - S2.1

Design Criteria

Spans:

Simple span

Loads: Deflection limits:

Live load = 40 psf and dead load = 15 psf

L/480 under live load and L/240 under total load

5/8 in. nailed-glued oriented strand board (OSB) sheathing Sheathing:

			E	Bare			1/2 in. gy	psum ceiling			
Joist depth	Joist series		On cent	re spacing		On centre spacing					
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-1"	14'-3"	13'-10"	-	15'-7"	14'-9"	14'-3"	-		
0.4/01	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-		
9-1/2"	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-		
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	-		
	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	-		
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	-		
11-7/8"	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	-		
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	-		
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	-		
	NI-40x	20'-1"	18'-8"	17'-10"	-	20'-10"	19'-4"	18'-6"	-		
4.411	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	-		
14"	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	-		
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	-		
	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	-		
16"	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	-		
	NI-90	24'-5"	22'-6"	21'-6"	_	25'-1"	23'-2"	22'-2"	_		

		Mi	d-span blocking	g with 1x4 inch s	trap	Mid-sp	oan blocking ar	d 1/2 in. gypsum	ceiling	
Joist depth	Joist series		On cent	re spacing		On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-8"	15'-3"	14'-5"	-	16'-8"	15'-3"	14'-5"	-	
0.4/01	NI-40x	17'-11"	17'-0"	16'-1"	-	18'-5"	17'-1"	16'-1"	-	
, 9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	-	18'-8"	17'-4"	16'-4"	-	
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	-	
	NI-20	19'-7"	18'-2"	17'-3"	-	19'-11"	18'-3"	17'-3"	-	
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-2"	-	
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-	
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-	
	N!-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-	
-	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	21'-8"	-	
4.411	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-	
14"	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-	
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	-	
	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-	
16"	NI-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-	
	NI-90	28'-8"	26'-6"	25'-3"	-	29'-3"	27'-2"	25'-11"	_	

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - S4.1

Design Criteria

Spans:

Simple span

Loads: Deflection limits: Live load = 40 psf and dead load = 15 psf

L/480 under live load and L/240 under total load

Sheathing:

3/4 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

			E	Bare			1/2 in. gy	psum ceiling			
Joist depth	Joist series		On cent	re spacing		On centre spacing					
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"		
0.4/00	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	15'-2"		
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"		
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"		
	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-2"		
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"		
11-7/8"	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"		
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"		
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"		
	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"		
14"	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10'		
14	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"		
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"		
	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"		
16"	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10"		
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"		

		Mi	d-span blocking	with 1x4 inch	strap	Mid-s	oan blocking an	d 1/2 in. gypsu	m ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
0.4/00	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10'
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"
11-7/8"	NI-60	22'-1"	20'-7"	19'-8"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
11-7/8"	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-0"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-7"
	NI-40x	24'-5"	22'-9"	21'-9"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
4.411	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
14"	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	24'-9"	23'-1"
16"	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

Notes:

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - S6.1

Design Criteria

Spans:

Simple span

Loads: Deflection limits:

Sheathing:

Live load = 40 psf and dead load = 15 psf

L/480 under live load and L/240 under total load 5/8 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

			E	Bare			1/2 in. gy	psum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	14'-11"	14'-1"	13'-7"	-	15'-4"	14'-6"	14'-1"	_
0.4/01	NI-40x	15'-11"	15'-0"	14'-6"	-	16'-4"	15'-5"	14'-11"	-
9-1/2"	NI-60	16'-1"	15'-2"	14'-8"	-	16'-6"	15'-7"	15'-1"	-
	NI-80	17'-1"	16'-1"	15'-6"	-	17'-5"	16'-5"	15'-10"	-
	NI-20	16'-9"	15'-10"	15'-4"	-	17'-4"	16'-4"	15'-10"	-
	NI-40x	17'-10"	16'-10"	16'-3"	-	18'-6"	17 '-4 "	16'-9"	_
11-7/8"	NI-60	18'-1"	17'-0"	16'-5"	-	18'-9"	17'-6"	16'-11"	-
	NI-80	19'-6"	18'-0"	17'-4"	-	20'-1"	18'-7"	17'-9"	-
	NI-90	19'-11"	18'-4"	17'-8"	-	20'-5"	18'-11"	18'-1"	-
	NI-40x	19'-10"	18'-4"	17'-8"	-	20'-6"	19'-1"	18'-3"	-
4.41	NI-60	20'-2"	18'-8"	17'-11"	_	20'-10"	19'-4"	18'-6"	-
14"	NI-80	21'-8"	20'-0"	19'-1"	-	22'-4"	20'-8"	19'-9"	-
	NI-90	22'-1"	20'-5"	19'-6"	-	22'-9"	21'-0"	20'-1"	-
	NI-60	22'-0"	20'-4"	19'-6"	-	22'-9"	21'-1"	20'-2"	-
16"	NI-80	23'-7"	21'-10"	20'-10"	-	24'-4"	22'-6"	21'-6"	-
	NI-90	24'-1"	22'-2"	21'-2"	_	24'-9"	22'-11"	21'-10"	-

		Mi	d-span blocking	g with 1x4 inch s	trap	Mid-s	pan blocking an	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-6"	15'-1"	14'-3"	-	16'-6"	15'-1"	14'-3"	-
0.4/0!	NI-40x	17'-9"	16'-10"	15'-11"	-	18'-2"	16'-11"	15'-11"	-
9-1/2"	NI-60	17'-11"	16'-11"	16'-2"	-	18'-5"	17'-2"	16'-2"	-
	NI-80	19'-3"	17'-10"	17'-3"	-	19'-8"	18'-3"	17'-7"	-
	NI-20	19'-4"	18'-0"	17'-1"	-	19'-9"	18'-1"	17'-1"	-
	NI-40x	20'-10"	19'-4"	18'-6"	-	21'-5"	19'-11"	19'-0"	_
11-7/8"	NI-60	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-3"	-
	NI-80	22'-6"	20'-10"	19'-11"	-	23'-1"	21'-5"	20'-5"	-
	NI-90	23'-0"	21'-3"	20'-4"	-	23'-6"	21'-10"	20'-10"	-
	NI-40x	23'-5"	21'-8"	20'-9"	-	24'-0"	22'-5"	21'-5"	-
4.40	NI-60	23'-9"	22'-0"	21'-0"	-	24'-5"	22'-8"	21'-8"	-
14"	NI-80	25'-4"	23'-6"	22'-5"	-	25'-11"	24'-1"	23'-0"	_
	NI-90	25'-10"	23'-11"	22'-9"	-	26'-5"	24'-6"	23'-4"	-
	NI-60	26'-2"	24'-3"	23'-2"	-	26'-11"	25'-0"	23'-11"	-
16"	NI-80	27'-11"	25'-10"	24'-7"	-	28'-7"	26'-6"	25'-3"	-
	NI-90	28'-5"	26'-3"	25'-0"	-	29'-0"	26'-11"	25'-8"	_

Notes:

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - S7.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 15 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

3/4 in. nailed-glued Canadian softwood plywood

Maximum Floor Spans

			E	Bare			1/2 in. gy	psum ceiling		
Joist depth	Joist series		On cent	re spacing		On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"	
9-1/2"	NI-40x	16'-11"	15'-11"	15'-4"	14'-9"	17'-4"	16'-4"	15'-9"	15'-1"	
9-1/2	NI-60	17'-1"	16'-1"	15'-6"	14'-10"	17'-6"	16'-6"	15'-11"	15'-3"	
	NI-80	18'-1"	17'-0"	16'-4"	15'-8"	18'-7"	17'-4"	19.2" 14'-6" 15'-9"	16'-0"	
	NI-20	17'-10"	16'-10"	16'-2"	15'-7"	18'-5"	17'-4"	16'-9"	16'-1"	
	NI-40x	19'-3"	17'-10"	17'-2"	16'-6"	19'-10"	18'-5"	17'-8"	16'-11'	
11-7/8"	NI-60	19'-6"	18'-1"	17'-4"	16'-8"	20'-1"	18'-8"	17'-10"	17'-1"	
	NI-80	20'-11"	19'-4"	18'-5"	17'-7"	21'-5"	19'-10"	5" 14'-6" 4" 15'-9" 6" 15'-11" 4" 16'-8" 4" 16'-9" 5" 17'-8" 8" 17'-10" 10" 18'-11" 3" 19'-3" 5" 19'-6" 9" 19'-9" 1" 21'-0" 6" 21'-5" 7" 21'-7"	17'-11'	
	NI-90	21'-4"	19'-9"	18'-9"	17'-10"	21'-10"	20'-3"		18'-3"	
	NI-40x	21'-4"	19'-9"	18'-10"	17'-11"	22'-0"	20'-5"	19'-6"	18'-6"	
14"	NI-60	21'-8"	20'-1"	19'-2"	18'-2"	22'-4"	20'-9"	19'-9"	18'-9"	
14	NI-80	23'-3"	21'-6"	20'-5"	19'-4"	23'-10"	22'-1"	21'-0"	19'-11"	
	NI-90	23'-9"	21'-11"	20'-10"	19'-8"	24'-3"	22'-6"	21'-5"	20'-3"	
	NI-60	23'-7"	21'-10"	20'-10"	19'-9"	24'-4"	22'-7"	21'-7"	20'-5"	
16"	NI-80	25'-4"	23'-5"	22'-3"	21'-1"	26'-0"	24'-1"	22'-11"	21'-8"	
	NI-90	25'-10"	23'-10"	22'-8"	21'-5"	26'-5"	24'-6"	23'-4"	22'-0"	

		Mi	d-span blockin	g with 1x4 inch	strap	Mid-s	oan blocking ar	ıd 1/2 in. gypsı	ım ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
9-1/2"	NI-40x	18'-7"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2"	NI-60	18'-10"	17'-6"	16'-6"	15'-5"	19'-1"	17'-6"	16'-6"	15'-5"
	NI-80	20'-2"	18'-9"	17'-11"	16'-10"	20'-7"	19'-2"	18'-2"	16'-10'
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-9"	20'-3"	19'-4"	17'-8"	22'-4"	20'-5"	19'-4"	17'-8"
11-7/8"	NI-60	22'-0"	20'-6"	19'-7"	18'-4"	22'-7"	20'-10"	19'-8"	18'-4"
11-7/8"	NI-80	23'-6"	21'-10"	20'-10"	19'-9"	24'-0"	22'-5"	21'-4"	20'-0"
	NI-90	24'-0"	22'-4"	21'-3"	20'-1"	24'-6"	22'-10"	21'-9"	20'-7"
	NI-40x	24'-4"	22'-8"	21'-8"	19'-5"	25'-0"	23'-2"	21'-9"	19'-5"
14"	NI-60	24'-9"	23'-0"	22'-0"	20'-9"	25'-5"	23'-8"	22'-4"	20'-10"
14	NI-80	26'-5"	24'-6"	23'-4"	22'-1"	27'-0"	25'-2"	24'-0"	22'-8"
	NI-90	26'-11"	25'-0"	23'-10"	22'-6"	27'-5"	25'-7"	24'-5"	23'-1"
	NI-60	27'-2"	25'-4"	24'-2"	22'-10"	27'-11"	26'-1"	24'-9"	23'-1"
16"	NI-80	29'-0"	26'-11"	25'-8"	24'-3"	29'-7"	27'-7"	26'-4"	24'-11"
	NI-90	29'-6"	27'-5"	26'-1"	24'-8"	30'-1"	28'-1"	26'-9"	25'-4"

Notes:

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - M2.1

Design Criteria

Spans:

Simple span Loads:

Live load = 40 psf and dead load = 20 psf Deflection limits: L/480 under live load and L/240 under total load

5/8 in. nailed-glued oriented strand board (OSB) sheathing Sheathing:

Maximum Floor Spans

			В	are			1/2 in. gyį	osum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
•		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-3"	13'-10"	-	15'-7"	14'-9"	14'-3"	- "
	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-
9-1/2"	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	-
	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	-
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	-
11-7/8"	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	-
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	-
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	-
	NI-40x	20'-1"	18'-8"	17'-10"	_	20'-10"	19'-4"	18'-6"	-
	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	-
14"	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	-
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	-
	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	-
16"	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	-
	NI-90	24'-5"	22'-6"	21'-6"	_	25'-1"	23'-2"	22'-2"	-

		Mi	d-span blocking	with 1x4 inch s	rap	Mid-sp	an blocking an	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
•		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	-	16'-8"	15'-3"	14'-5"	-
	NI-40x	17'-11"	17'-0"	16'-1"	-	18'-5"	17'-1"	16'-1"	-
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	-	18'-8"	17'-4"	16'-4"	-
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	
	NI-20	19'-7"	18'-2"	17'-3"	-	19'-11"	18'-3"	17'-3"	-
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-0"	-
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-
	NI-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-
	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	20'-11"	-
	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-
14"	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	-
	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-
16"	N!-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-
	NI-90	28'-8"	26'-6"	25'-3"	-	29'-3"	27'-2"	25'-11"	-

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.



Maximum Floor Spans - M4.1

Design Criteria

Live load = 40 psf and dead load = 20 psf Loads:

Deflection limits: L/480 under live load and L/240 under total load

3/4 in. nailed-glued oriented strand board (OSB) sheathing

			В	are			1/2 in. gy	psum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	14'-11'
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10'
14"	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
16"	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10'
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

		Mi	d-span blocking	with 1x4 inch	strap	Mid-sp	oan blocking an	d 1/2 in. gypsui	m ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	14'-11"	18'-10"	17'-2"	16'-3"	14'-11"
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
	NI-20	20'-1"	18'-5"	17'-5"	16'-1"	20'-1"	18'-5"	17'-5"	16'-1"
	NI-40x	21'-10"	20'-4"	19'-0"	17'-0"	22'-5"	20'-6"	19'-0"	17'-0"
11-7/8"	NI-60	22'-1"	20'-7"	19'-8"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
11-7/8"	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-0"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-7"
	NI-40x	24'-5"	22'-9"	20'-11"	18'-8"	25'-1"	22'-11"	20'-11"	18'-8"
	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
14"	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
16"	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	24'-9"	23'-1"
	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

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Maximum Floor Spans – M6.1

Design Criteria Spans:

Simple span

Live load = 40 psf and dead load = 20 psf Deflection limits: L/480 under live load and L/240 under total load

Sheathing:

5/8 in. nailed-glued Canadian softwood plywood

			E	Bare			1/2 in. gy	psum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	14'-11"	14'-1"	13'-7"	-	15'-4"	14'-6"	14'-1"	-
0.4/0!	NI-40x	15'-11"	15'-0"	14'-6"	-	16'-4"	15'-5"	14'-11"	-
9-1/2"	NI-60	16'-1"	15'-2"	14'-8"	-	16'-6"	15'-7"	15'-1"	-
	NI-80	17'-1"	16'-1"	15'-6"	-	17'-5"	16'-5"	15'-10"	-
	NI-20	16'-9"	15'-10"	15'-4"	-	17'-4"	16'-4"	15'-10"	-
	NI-40x	17'-10"	16'-10"	16'-3"	-	18'-6"	17'-4"	16'-9"	-
11-7/8"	NI-60	18'-1"	17'-0"	16'-5"	-	18'-9"	17'-6"	16'-11"	-
	NI-80	19'-6"	18'-0"	17'-4"	-	20'-1"	18'-7"	17'-9"	-
	NI-90	19'-11"	18'-4"	17'-8"	_	20'-5"	18'-11"	18'-1"	-
	NI-40x	19'-10"	18'-4"	17'-8"	_	20'-6"	19'-1"	18'-3"	-
	NI-60	20'-2"	18'-8"	17'-11"	-	20'-10"	19'-4"	18'-6"	-
14"	NI-80	21'-8"	20'-0"	19'-1"	-	22'-4"	20'-8"	19'-9"	-
	NI-90	22'-1"	20'-5"	19'-6"	-	22'-9"	21'-0"	20'-1"	-
	NI-60	22'-0"	20'-4"	19'-6"	_	22'-9"	21'-1"	20'-2"	-
16"	NI-80	23'-7"	21'-10"	20'-10"	-	24'-4"	22'-6"	21'-6"	-
	NI-90	24'-1"	22'-2"	21'-2"	_	24'-9"	22'-11"	21'-10"	_

		Mi	d-span blocking	with 1x4 inch s	trap	Mid-s	oan blocking an	d 1/2 in. gypsum	ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
·		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-6"	15'-1"	14'-3"	-	16'-6"	15'-1"	14'-3"	-
0.410#	NI-40x	17'-9"	16'-10"	15'-11"	-	18'-2"	16'-11"	15'-11"	-
9-1/2"	NI-60	17'-11"	16'-11"	16'-2"	-	18'-5"	17'-2"	16'-2"	-
	NI-80	19'-3"	17'-10"	17'-3"	-	19'-8"	18'-3"	17'-7"	-
	NI-20	19'-4"	18'-0"	17'-1"	-	19'-9"	18'-1"	17'-1"	-
	NI-40x	20'-10"	19'-4"	18'-6"	-	21'-5"	19'-11"	19'-0"	-
11-7/8"	NI-60	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-3"	-
	NI-80	22'-6"	20'-10"	19'-11"	-	23'-1"	21'-5"	20'-5"	-
	NI-90	23'-0"	21'-3"	20'-4"	-	23'-6"	21'-10"	20'-10"	-
	NI-40x	23'-5"	21'-8"	20'-9"	-	24'-0"	22'-5"	20'-11"	-
	NI-60	23'-9"	22'-0"	21'-0"	-	24'-5"	22'-8"	21'-8"	-
14"	NI-80	25'-4"	23'-6"	22'-5"	-	25'-11"	24'-1"	23'-0"	-
	NI-90	25'-10"	23'-11"	22'-9"	-	26'-5"	24'-6"	23'-4"	-
	NI-60	26'-2"	24'-3"	23'-2"	-	26'-11"	25'-0"	23'-11"	-
16"	NI-80	27'-11"	25'-10"	24'-7"	-	28'-7"	26'-6"	25'-3"	-
	NI-90	28'-5"	26'-3"	25'-0"	-	29'-0"	26'-11"	25'-8"	-

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

Maximum Floor Spans - M7.1

Design Criteria

Spans:

Simple span

Loads:

Live load = 40 psf and dead load = 20 psf

Deflection limits:

L/480 under live load and L/240 under total load

Sheathing:

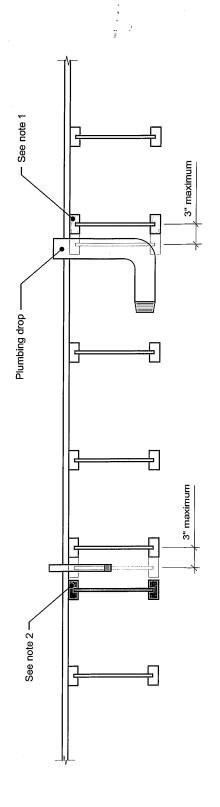
3/4 in. nailed-glued Canadian softwood plywood

			В	are			1/2 in. gy _l	osum ceiling	
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
•		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	16'-11"	15'-11"	15'-4"	14'-9"	17'-4"	16 '-4"	15'-9"	14'-11'
9-1/2"	NI-60	17'-1"	16'-1"	15'-6"	14'-10"	17'-6"	16'-6"	15'-11"	15'-3"
	NI-80	18'-1"	17'-0"	16'-4"	15'-8"	18'-7"	17'-4"	16'-8"	16'-0"
	NI-20	17'-10"	16'-10"	16'-2"	15'-7"	18'-5"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-3"	17'-10"	17'-2"	16'-6"	19'-10"	18'-5"	17'-8"	16'-11
11-7/8"	NI-60	19'-6"	18'-1"	17'-4"	16'-8"	20'-1"	18'-8"	17'-10"	17'-1"
	NI-80	20'-11"	19'-4"	18'-5"	17'-7"	21'-5"	19'-10"	18'-11"	17'-11'
	NI-90	21'-4"	19'-9"	18'-9"	17'-10"	21'-10"	20'-3"	19'-3"	18'-3"
	NI-40x	21'-4"	19'-9"	18'-10"	17'-11"	22'-0"	20'-5"	19'-6"	18'-6"
	NI-60	21'-8"	20'-1"	19'-2"	18'-2"	22'-4"	20'-9"	19'-9"	18'-9"
14"	NI-80	23'-3"	21'-6"	20'-5"	19'-4"	23'-10"	22'-1"	21'-0"	19'-11'
	NI-90	23'-9"	21'-11"	20'-10"	19'-8"	24'-3"	22'-6"	21'-5"	20'-3"
	NI-60	23'-7"	21'-10"	20'-10"	19'-9"	24'-4"	22'-7"	21'-7"	20'-5"
16"	NI-80	25'-4"	23'-5"	22'-3"	21'-1"	26'-0"	24'-1"	22'-11"	21'-8"
	NI-90	25'-10"	23'-10"	22'-8"	21'-5"	26'-5"	24'-6"	23'-4"	22'-0"

		Mi	d-span blocking	with 1x4 inch	strap	Mid-sp	oan blocking an	d 1/2 in. gypsui	m ceiling
Joist depth	Joist series		On cent	re spacing			On cent	re spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-7"	17'-2"	16'-3"	14'-11"	18'-10"	17'-2"	16'-3"	14'-11'
9-1/2"	NI-60	18'-10"	17'-6"	16'-6"	15'-5"	19'-1"	17'-6"	16'-6"	15'-5"
	NI-80	20'-2"	18'-9"	17'-11"	16'-10"	20'-7"	19'-2"	18'-2"	16'-10'
	NI-20	20'-1"	18'-5"	17'-5"	16'-1"	20'-1"	18'-5"	17'-5"	16'-1"
	NI-40x	21'-9"	20'-3"	19'-0"	17'-0"	22'-4"	20'-5"	19'-0"	17'-0"
11-7/8"	NI-60	22'-0"	20'-6"	19'-7"	18'-4"	22'-7"	20'-10"	19'-8"	18'-4"
	NI-80	23'-6"	21'-10"	20'-10"	19'-9"	24'-0"	22'-5"	21'-4"	20'-0"
	NI-90	24'-0"	22'-4"	21'-3"	20'-1"	24'-6"	22'-10"	21'-9"	20'-7"
	NI-40x	24'-4"	22'-8"	20'-11"	18'-8"	25'-0"	22'-11"	20'-11"	18'-8"
	NI-60	24'-9"	23'-0"	22'-0"	20'-9"	25'-5"	23'-8"	22'-4"	20'-10'
14"	NI-80	26'-5"	24'-6"	23'-4"	22'-1"	27'-0"	25'-2"	24'-0"	22'-8"
	NI-90	26'-11"	25'-0"	23'-10"	22'-6"	27'-5"	25'-7"	24'-5"	23'-1"
	NI-60	27'-2"	25'-4"	24'-2"	22'-10"	27'-11"	26'-1"	24'-9"	23'-1"
16"	NI-80	29'-0"	26'-11"	25'-8"	24'-3"	29'-7"	27'-7"	26'-4"	24'-11'
	NI-90	29'-6"	27'-5"	26'-1"	24'-8"	30'-1"	28'-1"	26'-9"	25'-4"

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.





- Notes:

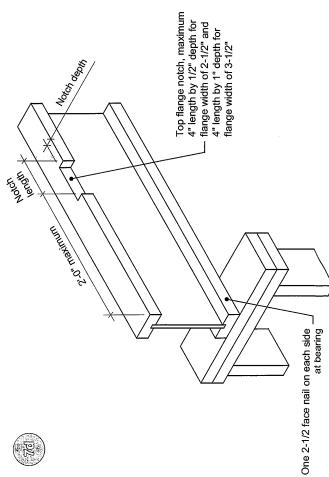
 1. To prevent interference with plumbing, a joist may be shifted up to 3 inches if the edge of the floor panel is supported and the span rating is not exceeded.

 2. In all other cases, an additional joist is required.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails. or 0.144 inch for 3-inch

NORDIC STRUCTURES	NS-DC3	MORDIC NS-DC3 I♣ Allowance for Piping TRUCTURES	Madual components not shown	n to scale for clarity. DRAWING 7 C	
:	DETAILS		SCALE	DATE	PAGE
nordic.ca	NORDIC JOIST	Openings for Vertical Elements		2020-10-01 3.10	3.10





2-1/2" and 1" depth for flange width of 3-1/2" Maximum 1/2" depth for flange width of — Heat register

- Blocking required at bearing for lateral support, not shown for clarity.
 The maximum dimensions for a notch on the side of the top flange are 4-inch length by 1/2-inch depth for flange width of 2-1/2 inches, and 4-inch length by 1-inch depth for flange width of 3-1/2 inches.
 This detail applies to simple-span joists and multiple-span joists where the notch is located at the end
 - half-span.
 4. For other applications, contact Nordic Structures.

All nails shown in the details are assumed to be common nails unless otherwise noted. Nails shall have a diameter not less than 0.128 inch for 2-1/2-inch nails, or 0.144 inch for 3-inch nails. Individual components not shown to scale for clarity.

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CATEGORY	Openings for Vertical Elements

for Heat Register		DRAWING 7d	
	SCALE	DATE	PAGE
/ertical Elements		2020-10-01	3.11