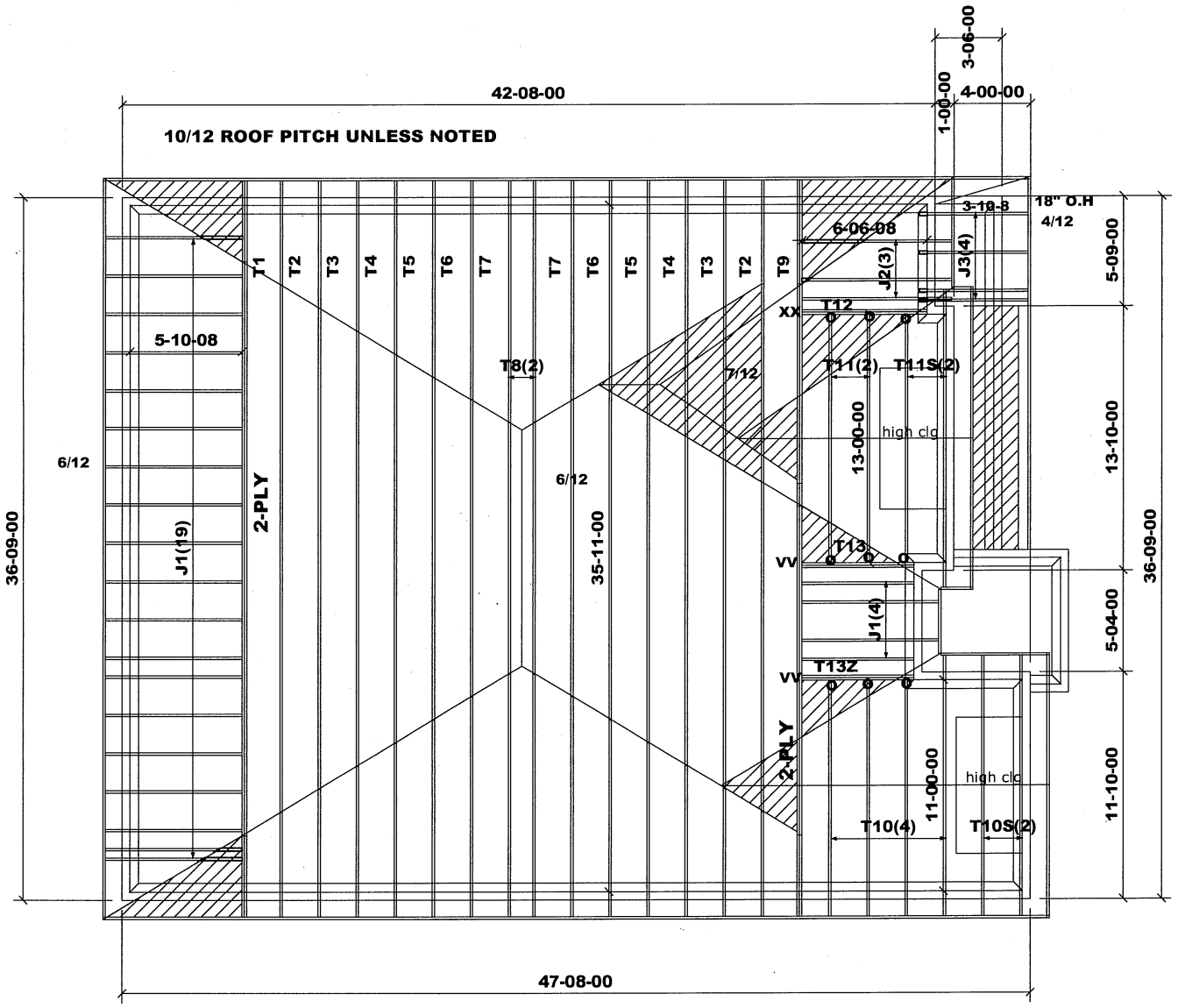


ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2- (VV)
HGUS26-2- (XX)

DENOTES:
CONVENTIONAL FRAMING

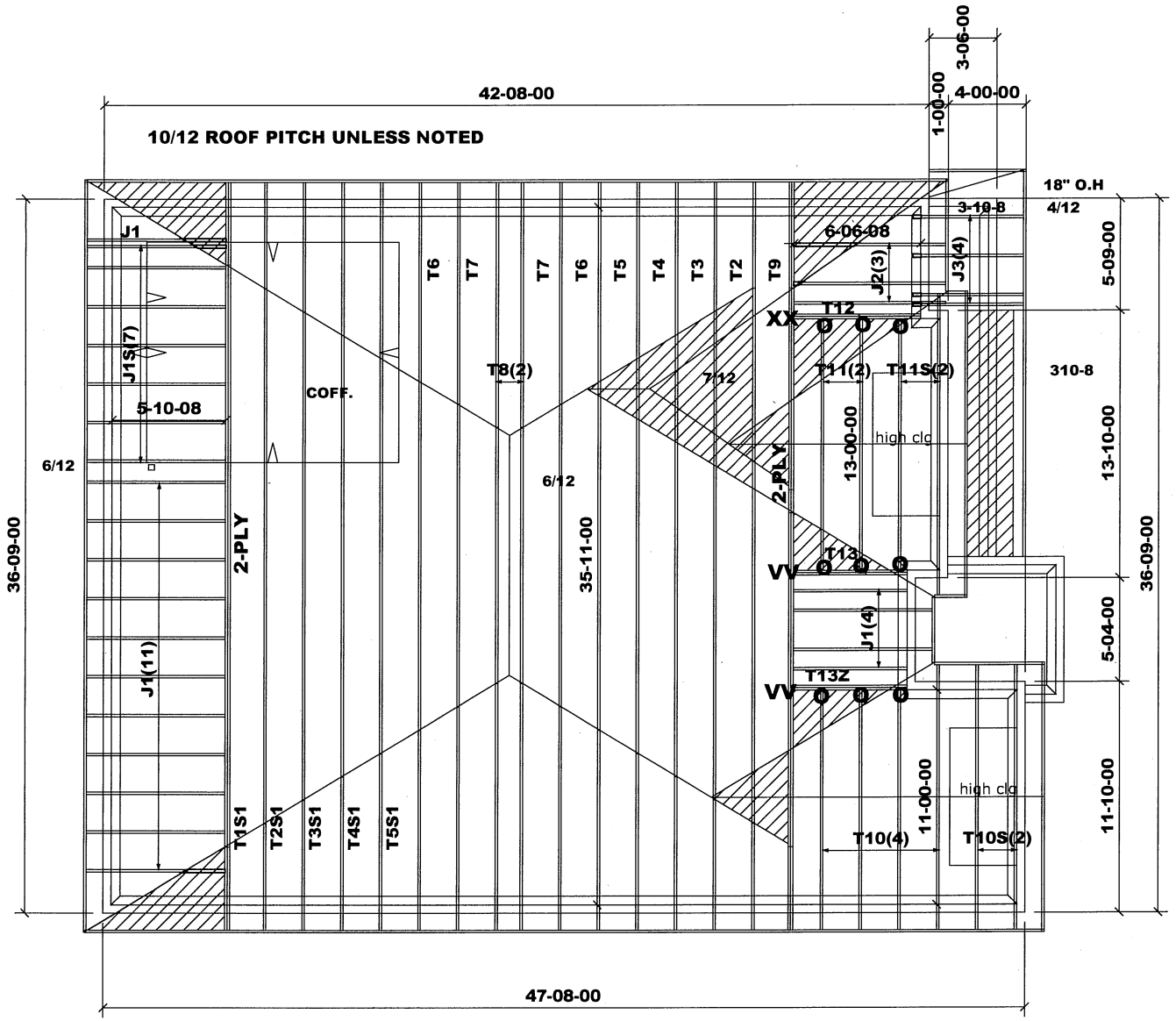
M14003

TAMARACK ROOF TRUSSES INC.

Job Track: **51004**	Builder / Location: **ROYAL PINE HOMES / RICHMOND HILL**	Model / Elevation: **UNIT-4502 / A-STD.OR 5 BRM**	
Plan Log: **203059**	Project: **CENTREFIELD**		
Layout ID: **418971**	Date: **5/26/2021**	Sales: Mario DiCano	Designer: JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 8.4.2.286



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2- (VV)
HGUS26-2 - (XX)

DENOTES:
CONVENTIONAL FRAMING

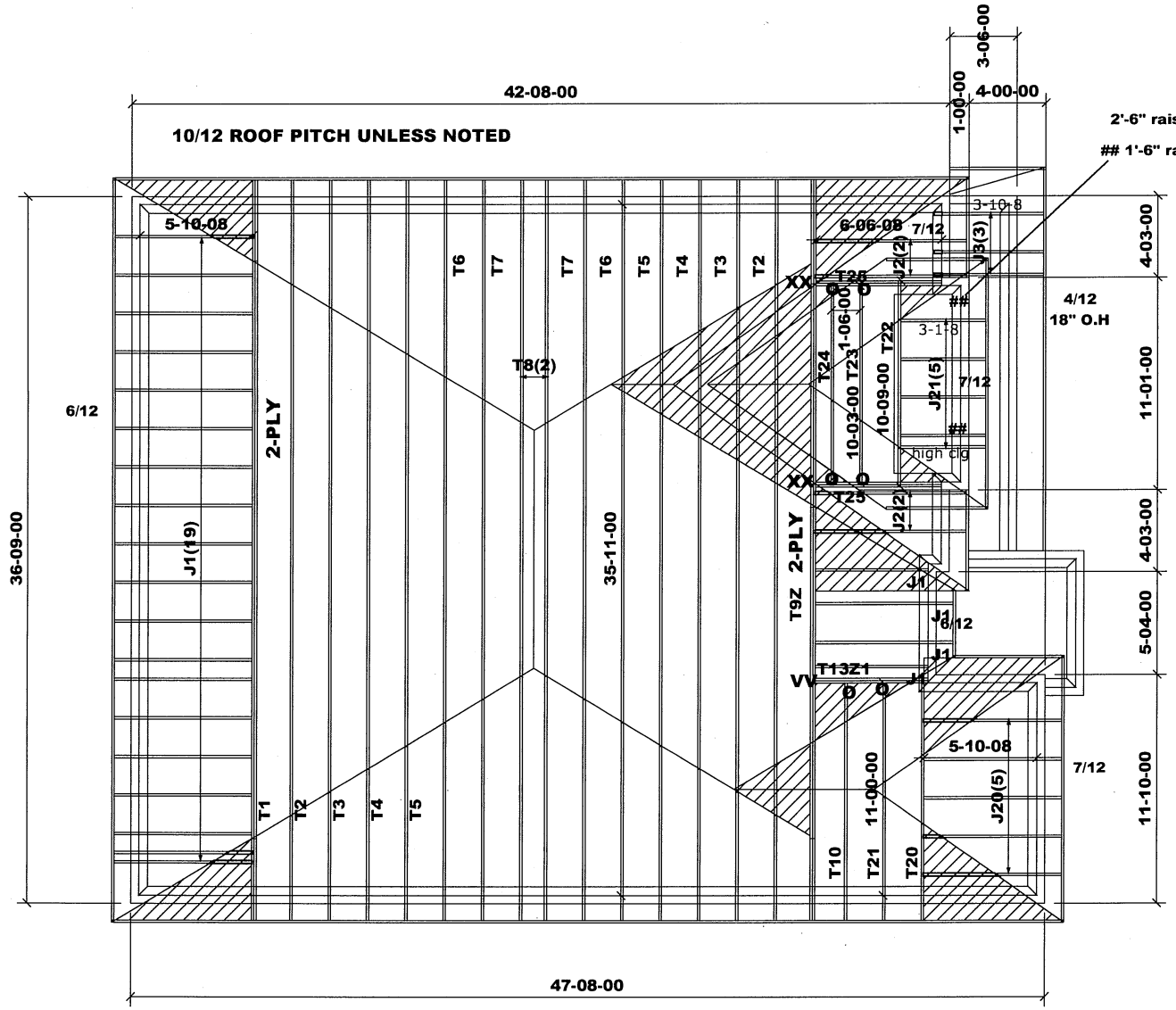
M14003

TAMARACK ROOF TRUSSES INC.

Job Track: **51004**	Builder / Location: **ROYAL PINE HOMES / RICHMOND HILL**	Model / Elevation: **UNIT-4502 / A-5 BRM. WITH OPT. COFF.**	
Plan Log: **203059**	Project: **CENTREFIELD**		
Layout ID: **410966**	Date: **5/26/2021**	Sales: Mario DiCano	Designer: JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 8.4.2.286



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2- (VV)
HGUS26-2 (XX)

DENOTES:
CONVENTIONAL FRAMING

M14003

TAMARACK ROOF TRUSSES INC.

Job Track:	**51004**
Plan Log:	**203059**
Layout ID:	**418972**

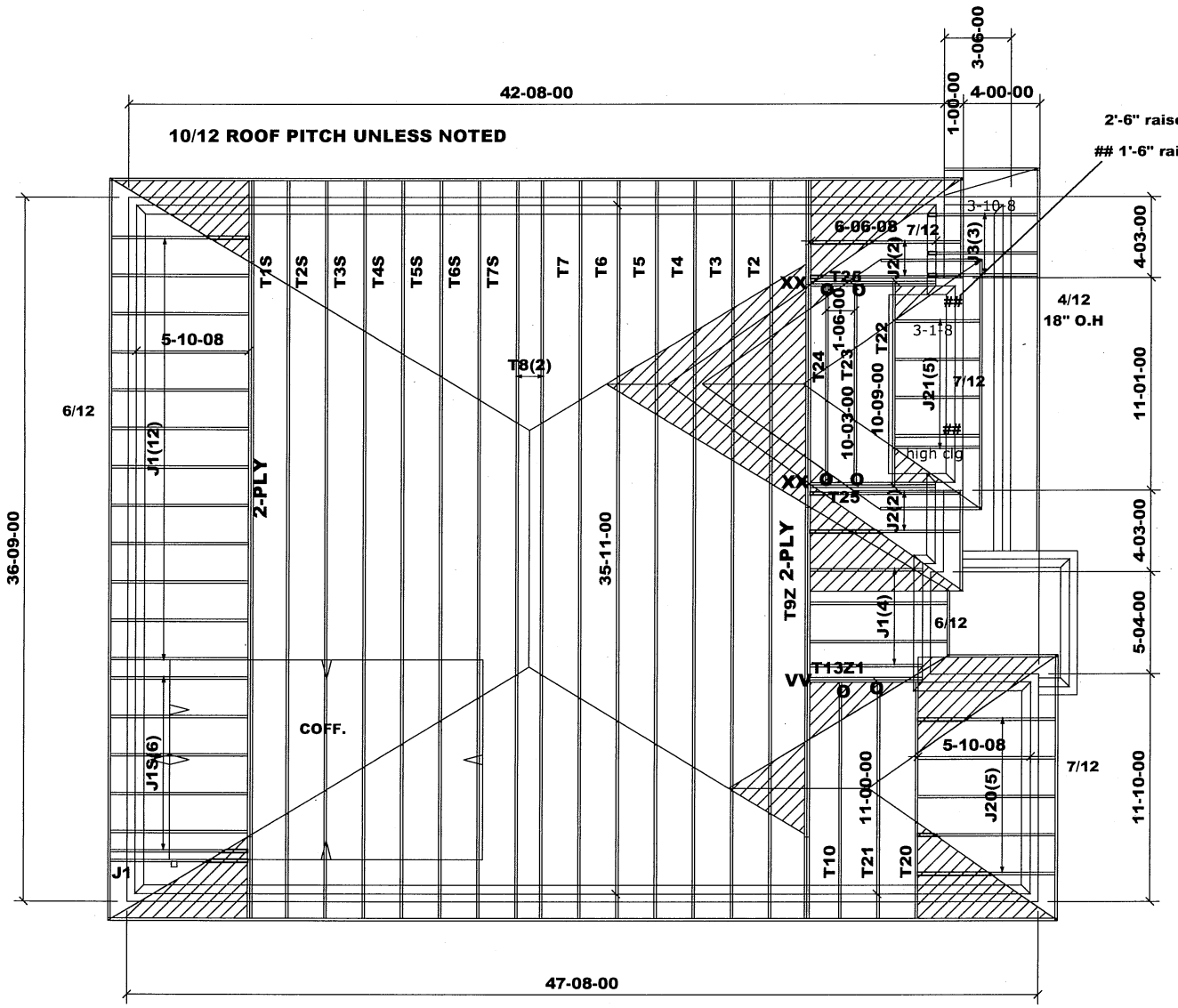
Builder / Location: **ROYAL PINE HOMES / RICHMOND HILL**

Model / Elevation: **UNIT-4502 / B-STD.OR 5 BRM**

Project: **CENTREFIELD**

Date: **5/26/2021** Sales: Mario DiCano Designer: JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2- (VV)
HGUS26-2 (XX)

DENOTES:
CONVENTIONAL FRAMING

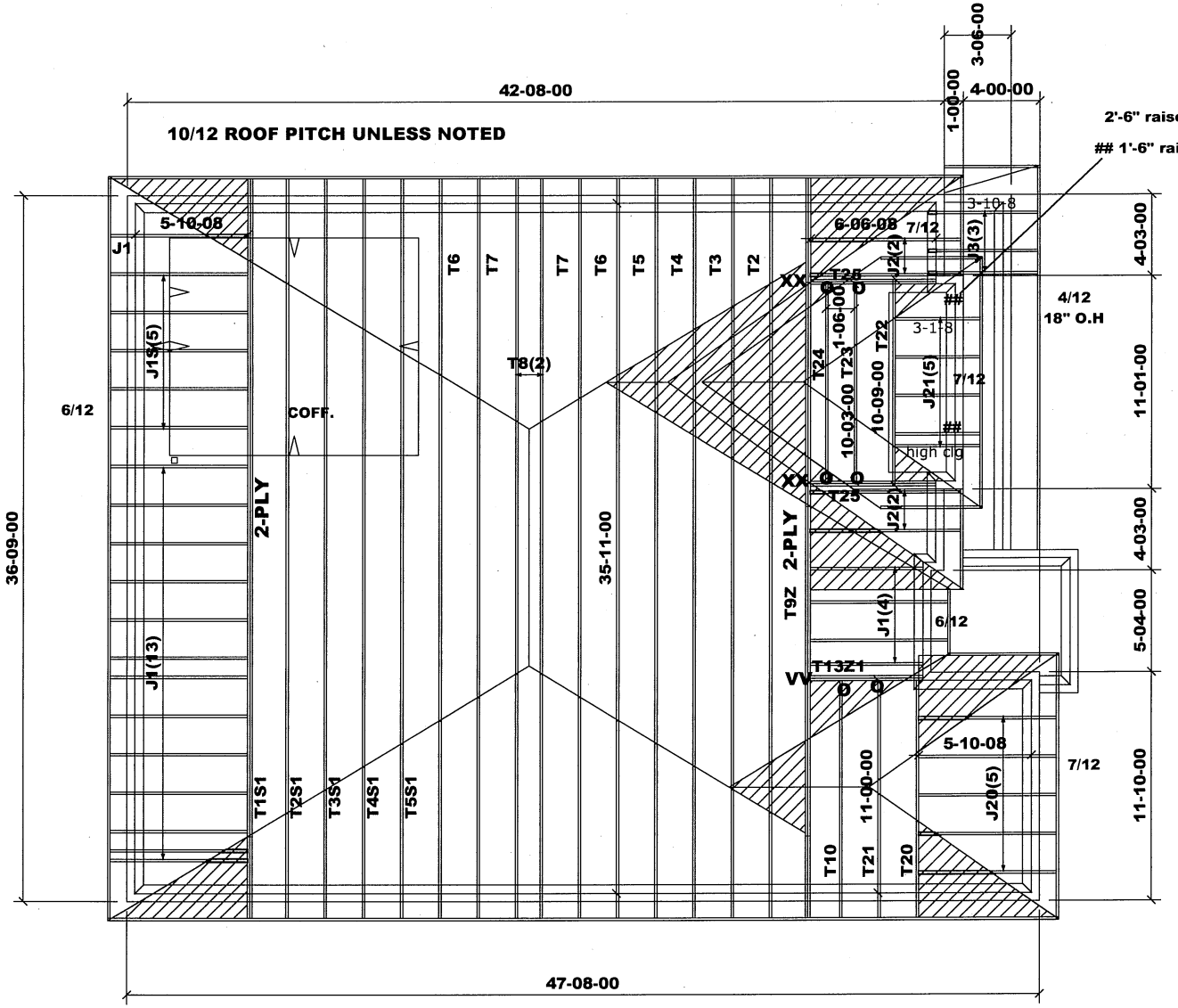
MH4003

TAMARACK ROOF TRUSSES INC.

Job Track: **51004**	Builder / Location: **ROYAL PINE HOMES / RICHMOND HILL**		Model / Elevation: **UNIT-4502 / B-STD.WITH OPT.COFF**
Plan Log: **203059**	Project: **CENTREFIELD**		
Layout ID: **410967**	Date: **5/26/2021**	Sales: Mario DiCano	Designer: JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

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ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2- (VV)
HGUS26-2 (XX)

DENOTES:
CONVENTIONAL FRAMING

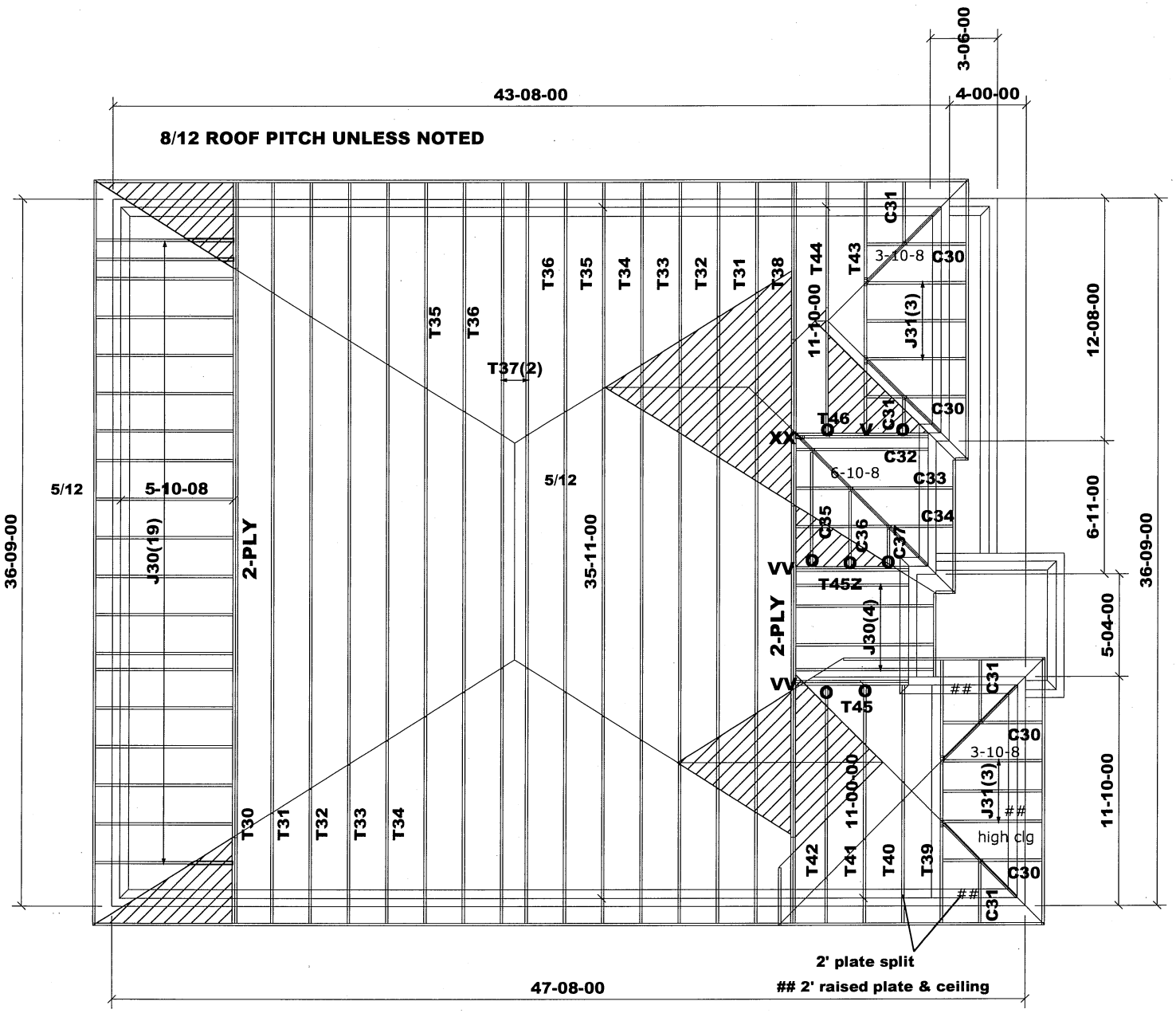
M14003

TAMARACK ROOF TRUSSES INC.

Job Track: **51004**	Builder / Location: **ROYAL PINE HOMES / RICHMOND HILL**		Model / Elevation: **UNIT-4502 / B-5 BRM WITH OPT.COFF.**
Plan Log: **203059**	Project: **CENTREFIELD**		
Layout ID: **410968**	Date: **5/26/2021**	Sales: Mario DiCano	Designer: JG

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ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2- (VV)

DENOTES:
CONVENTIONAL FRAMING

M11003

TAMARACK ROOF TRUSSES INC.

Job Track:	51004
Plan Log:	203059
Layout ID:	418973

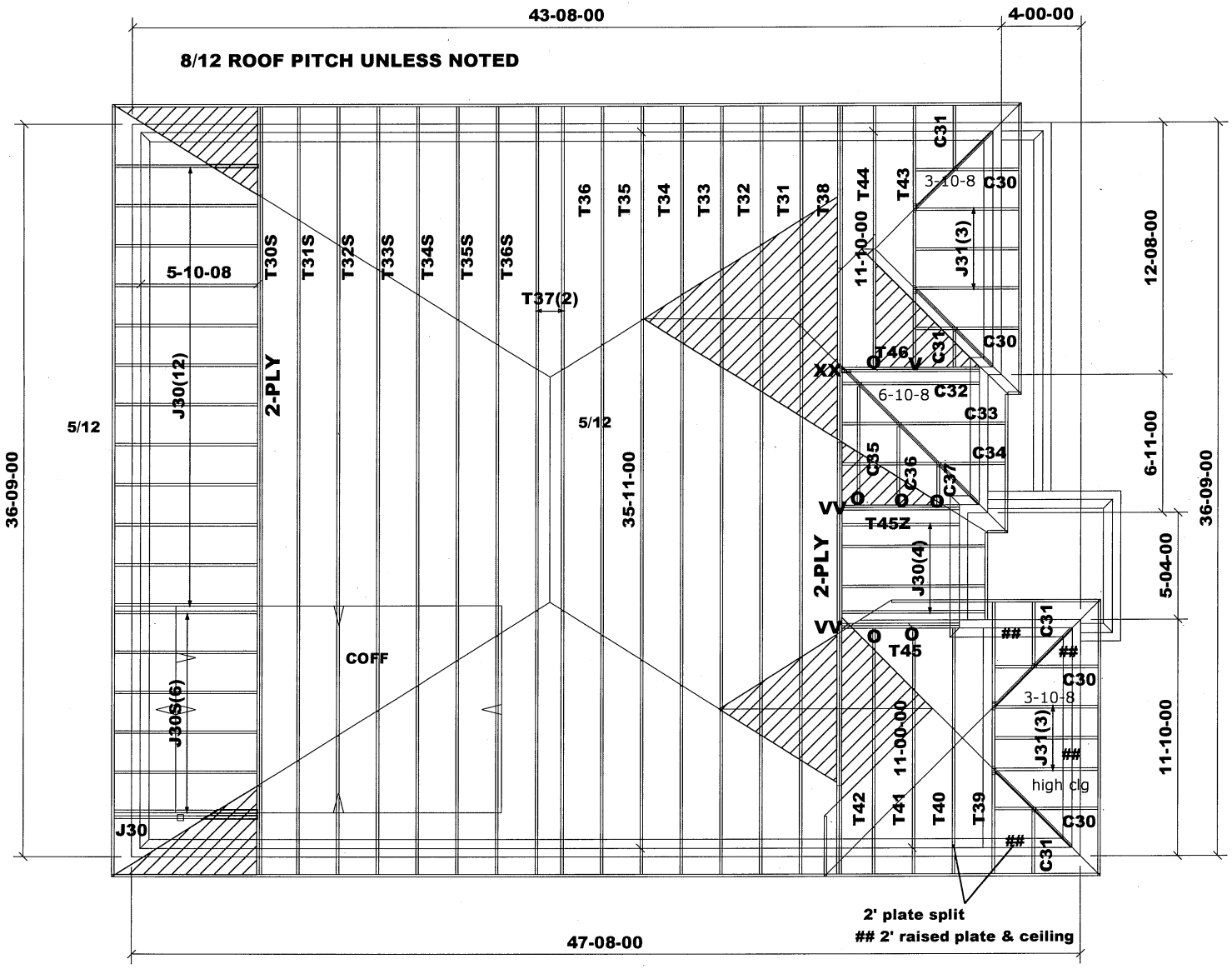
Builder / Location: **ROYAL PINE HOMES / RICHMOND HILL**

Model / Elevation: **UNIT-4502 / C-STD.OR 5 BRM**

Project: **CENTREFIELD**

Date: 5/26/2021 | Sales: Mario DiCano | Designer: JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2- (VV)
HGUS26-2 - (XX)

DENOTES:
CONVENTIONAL FRAMING

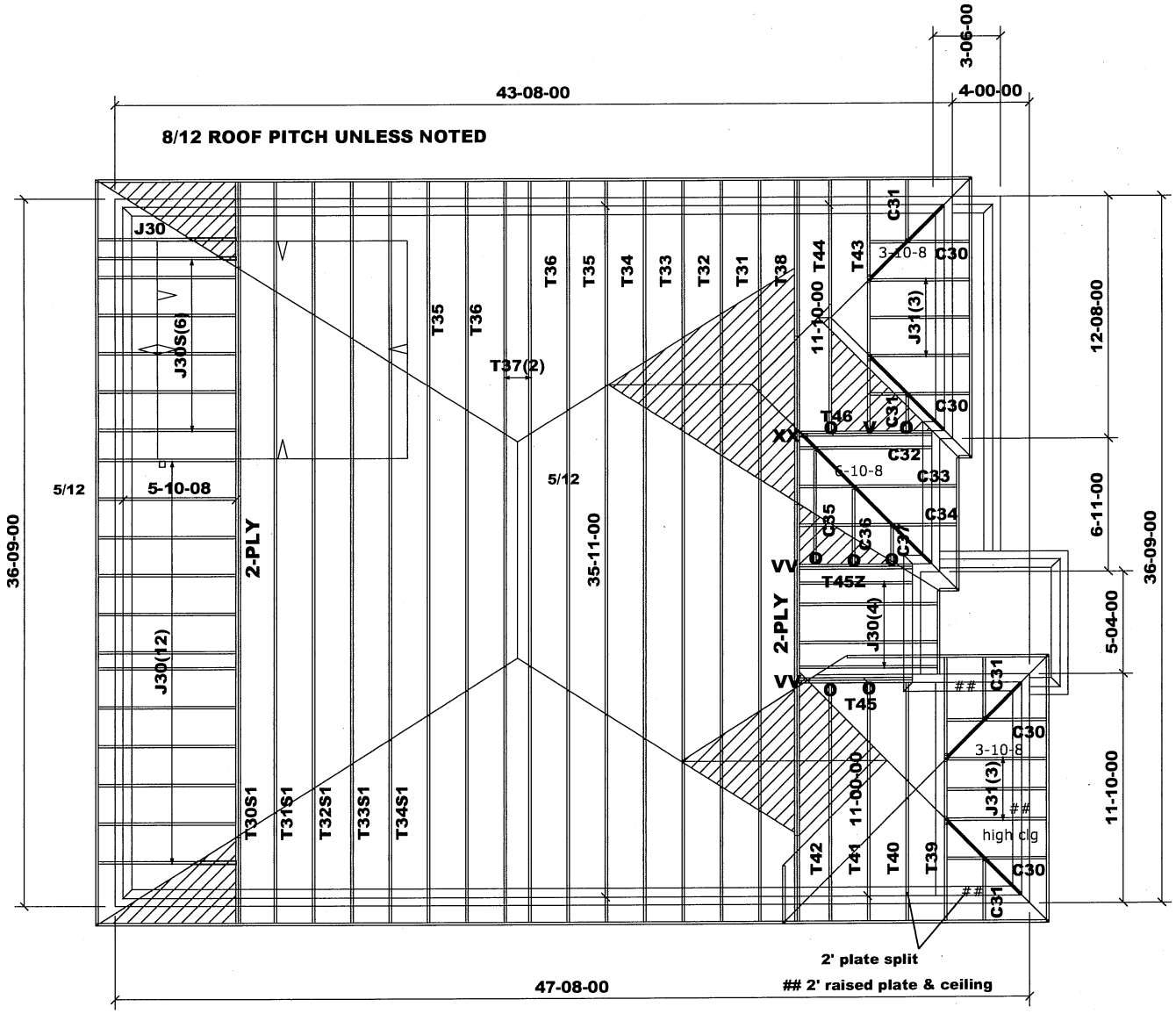
M1Y003

TAMARACK ROOF TRUSSES INC.

Job Track: **51004**	Builder / Location: **ROYAL PINE HOMES / RICHMOND HILL**	Model / Elevation: **UNIT-4502 / C- STD.WITH OPT.COFF.**	
Plan Log: **203059**	Project: **CENTREFIELD**		
Layout ID: **410969**	Date: **5/26/2021**	Sales: Mario DiCano	Designer: JG

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Mitek ver 8.4.2.286



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2- (VV)

DENOTES:
CONVENTIONAL FRAMING

M14003

TAMARACK ROOF TRUSSES INC.

Job Track:	51004
Plan Log:	203059
Layout ID:	410970

Builder / Location: **ROYAL PINE HOMES / RICHMOND HILL**

Model / Elevation: **UNIT-4502 / C-5 BRM WITH OPT. COFF.**

Project: **CENTREFIELD**

Date: **5/26/2021** | Sales: Mario DiCano | Designer: JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

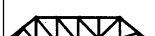
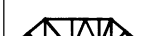
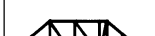
Mitek ver 8.4.2.286

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410967
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	1 of 3
Lot #:		Date:	05/26/2021
Elevation:	B-STD.WITH OPT.COFF	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

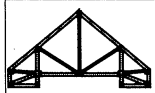
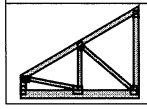
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S Hip Girder	10 /12	**35-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	396.4 251.33		
	1	T2 Hip	10 /12	**35-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	148.9 93.50		
	1	T2S Hip	10 /12	**35-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	162.94 105.17		
	1	T3 Hip	10 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.38 95.00		
	1	T3S Hip	10 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	164.05 104.50		
	1	T4 Hip	10 /12	**35-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.65 102.17		
	1	T4S Hip	10 /12	**35-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	171.65 108.33		
	1	T5 Hip	10 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	161.59 101.00		
	1	T5S Hip	10 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	179.97 115.83		
	1	T6 Hip	10 /12	**35-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	183.7 115.00		
	1	T6S Hip	10 /12	**35-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	193.96 123.33		
	1	T7 Hip	10 /12	**35-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	175.56 110.00		
	1	T7S Hip	10 /12	**35-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	199.53 125.83		
	2	T8 Hip	10 /12	**35-11-00**	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	384.63 238.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	418971
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	1 of 2
Lot #:		Date:	05/26/2021
Elevation:	A-STD.OR 5 BRM	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

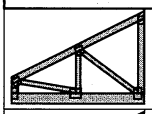
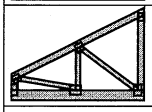
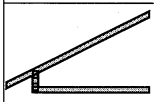
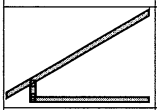
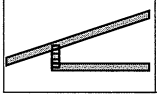
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	10 /12	35-11-00	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	371.79 222.00		
	2	T2 Hip	10 /12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	297.79 187.00		
	2	T3 Hip	10 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	300.76 190.00		
	2	T4 Hip	10 /12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	327.29 204.33		
	2	T5 Hip	10 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	323.18 202.00		
	2	T6 Hip	10 /12	35-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	367.4 230.00		
	2	T7 Hip	10 /12	35-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	351.11 220.00		
	2	T8 Hip	10 /12	35-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	384.63 238.00		
	1 2-ply	T9 Roof Special Girder	10 /12	35-11-00	5-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	398.56 245.00		
	4	T10 Common	10 /12	11-00-00	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	196.37 129.33		
	2	T10S Roof Special	10 /12	11-00-00	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	116.07 78.33		
	2	T11 Common	10 /12	13-00-00	7-00-11	2 x 4		1-07-11 1-07-11	116.53 73.67		
	2	T11S Roof Special	10 /12	13-00-00	7-00-11	2 x 4		1-07-11 1-07-11	130.91 88.67		
	1 2-ply	T12 Jack-Closed Girder	7 /12	6-06-08	5-01-04	2 x 4 2 x 6		1-03-07 5-01-04	67.4 42.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	418971
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	2 of 2
Lot #:		Date:	05/26/2021
Elevation:	A-STD.OR 5 BRM	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T13 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T13Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	23	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	386.27 245.33		
	3	J2 Jack-Open	7 /12	**6-06-08**	5-01-04	2 x 4	1-03-08	1-03-07 5-01-04	56.81 36.00		
	4	J3 Jack-Open	4 /12	**3-10-08**	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	48.09 32.00		

TOTAL # TRUSS= **64** TOTAL BFT OF ALL TRUSSES= **2738.33** BFT. TOTAL WEIGHT OF ALL TRSSES **4356.25** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
9	Hardware	LUS24	
2	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

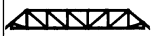
TOTAL NUMBER OF ITEMS= **12**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410965
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	1 of 3
Lot #:		Date:	05/26/2021
Elevation:	A-STD.WITH OPT.COFF	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

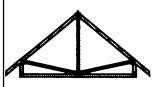
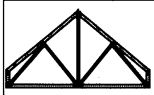
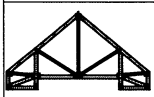
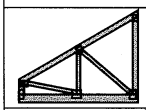
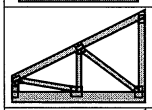
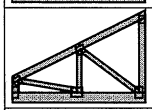
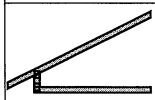
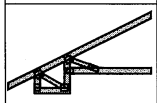
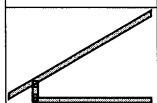
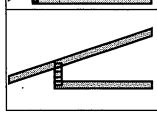
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S Hip Girder	10 /12	**35-11-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	396.4 251.33		
	1	T2 Hip	10 /12	**35-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	148.9 93.50		
	1	T2S Hip	10 /12	**35-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	162.94 105.17		
	1	T3 Hip	10 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.38 95.00		
	1	T3S Hip	10 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	164.05 104.50		
	1	T4 Hip	10 /12	**35-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.65 102.17		
	1	T4S Hip	10 /12	**35-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	171.65 108.33		
	1	T5 Hip	10 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	161.59 101.00		
	1	T5S Hip	10 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	179.97 115.83		
	1	T6 Hip	10 /12	**35-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	183.7 115.00		
	1	T6S Hip	10 /12	**35-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	193.96 123.33		
	1	T7 Hip	10 /12	**35-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	175.56 110.00		
	1	T7S Hip	10 /12	**35-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	199.53 125.83		
	2	T8 Hip	10 /12	**35-11-00**	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	384.63 238.00		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410965
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	2 of 3
Lot #:		Date:	05/26/2021
Elevation:	A-STD.WITH OPT.COFF	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T9 Roof Special Girder	10 /12	**35-11-00**	5-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	398.56 245.00		
	4	T10 Common	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	196.37 129.33		
	2	T10S Roof Special	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	116.07 78.33		
	2	T11 Common	10 /12	**13-00-00**	7-00-11	2 x 4		1-07-11 1-07-11	116.53 73.67		
	2	T11S Roof Special	10 /12	**13-00-00**	7-00-11	2 x 4		1-07-11 1-07-11	130.91 88.67		
	1 2-ply	T12 Jack-Closed Girder	7 /12	**6-06-08**	5-01-04	2 x 4 2 x 6		1-03-07 5-01-04	67.4 42.67		
	1 2-ply	T13 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T13Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	17	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		
	6	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	125.09 88.00		
	3	J2 Jack-Open	7 /12	**6-06-08**	5-01-04	2 x 4	1-03-08	1-03-07 5-01-04	56.81 36.00		
	4	J3 Jack-Open	4 /12	**3-10-08**	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	48.09 32.00		

TOTAL # TRUSS= **64** TOTAL BFT OF ALL TRUSSES= **2857.99** BFT. TOTAL WEIGHT OF ALL TRSSES **4493.51** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
9	Hardware	LUS24	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410965
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	3 of 3
Lot #:		Date:	05/26/2021
Elevation:	A-STD.WITH OPT.COFF	Designer:	
		Sales Rep:	Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS26-2	

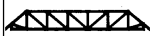
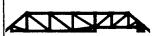
TOTAL NUMBER OF ITEMS= **12**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410966
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	1 of 2
Lot #:		Date:	05/26/2021
Elevation:	A-5 BRM. WITH OPT. COFF.	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

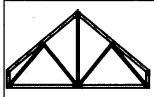
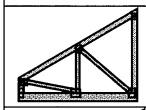
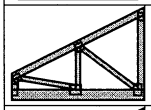
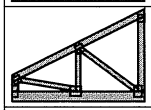
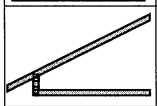
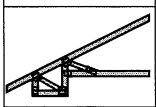
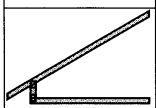
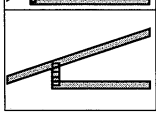
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S1 Hip Girder	10 /12	35-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	395 251.33		
	1	T2 Hip	10 /12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	148.9 93.50		
	1	T2S1 Hip	10 /12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	158.5 101.33		
	1	T3 Hip	10 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.38 95.00		
	1	T3S1 Hip	10 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.58 103.50		
	1	T4 Hip	10 /12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.65 102.17		
	1	T4S1 Hip	10 /12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	171.34 108.83		
	1	T5 Hip	10 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	161.59 101.00		
	1	T5S1 Hip	10 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	179.61 114.17		
	2	T6 Hip	10 /12	35-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	367.4 230.00		
	2	T7 Hip	10 /12	35-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	351.11 220.00		
	2	T8 Hip	10 /12	35-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	384.63 238.00		
	1 2-ply	T9 Roof Special Girder	10 /12	35-11-00	5-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	398.56 245.00		
	4	T10 Common	10 /12	11-00-00	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	196.37 129.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410966
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	2 of 2
Lot #:		Date:	05/26/2021
Elevation:	A-5 BRM. WITH OPT. COFF.	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T10S Roof Special	10 /12	11-00-00	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	116.07 78.33		
	2	T11 Common	10 /12	13-00-00	7-00-11	2 x 4		1-07-11 1-07-11	116.53 73.67		
	2	T11S Roof Special	10 /12	13-00-00	7-00-11	2 x 4		1-07-11 1-07-11	130.91 88.67		
	1 2-ply	T12 Jack-Closed Girder	7 /12	6-06-08	5-01-04	2 x 4 2 x 6		1-03-07 5-01-04	67.4 42.67		
	1 2-ply	T13 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T13Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	16	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		
	7	J1S Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	145.94 102.67		
	3	J2 Jack-Open	7 /12	6-06-08	5-01-04	2 x 4	1-03-08	1-03-07 5-01-04	56.81 36.00		
	4	J3 Jack-Open	4 /12	3-10-08	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	48.09 32.00		

TOTAL # TRUSS= **64** TOTAL BFT OF ALL TRUSSES= **2831.84** BFT. TOTAL WEIGHT OF ALL TRSSES **4456.36** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
9	Hardware	LUS24	
2	Hardware	LUS26-2	

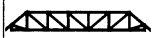
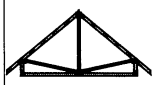
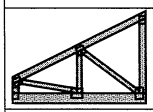
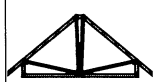
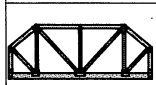
TOTAL NUMBER OF ITEMS= **12**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	418972
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	1 of 2
Lot #:		Date:	05/26/2021
Elevation:	B-STD.OR 5 BRM	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

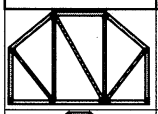
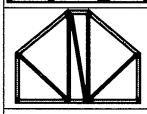
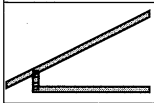
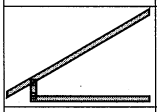
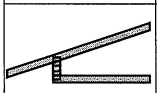
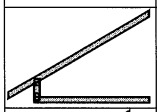
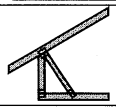
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	10 /12	**35-11-00**	4-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	370.4 221.33		
	2	T2 Hip	10 /12	**35-11-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	297.79 187.00		
	2	T3 Hip	10 /12	**35-11-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	300.76 190.00		
	2	T4 Hip	10 /12	**35-11-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	327.29 204.33		
	2	T5 Hip	10 /12	**35-11-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	323.18 202.00		
	2	T6 Hip	10 /12	**35-11-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	367.4 230.00		
	2	T7 Hip	10 /12	**35-11-00**	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	351.11 220.00		
	2	T8 Hip	10 /12	**35-11-00**	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	384.63 238.00		
	1 2-ply	T9Z Roof Special Girder	10 /12	**35-11-00**	5-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	398.56 245.00		
	1	T10 Common	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	49.09 32.33		
	1 2-ply	T13Z1 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T20 Hip Girder	10 /12	**11-00-00**	4-08-09	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	51.64 34.00		
	1	T21 Hip	10 /12	**11-00-00**	5-10-09	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	52.77 34.67		
	1	T22 Hip Girder	10 /12	**10-09-00**	4-01-05	2 x 4 2 x 6		2-05-03 2-05-03	57.35 37.33		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	418972
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	2 of 2
Lot #:		Date:	05/26/2021
Elevation:	B-STD.OR 5 BRM	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T23 Hip	10 /12	10-03-00	6-09-05	2 x 4		4-01-11 4-01-11	58.11 38.00		
	1	T24 Hip	10 /12	10-03-00	7-07-13	2 x 4		4-01-11 4-01-11	61.47 40.00		
	2 2-ply	T25 Flat Girder	0 /12	6-06-08	1-06-00	2 x 6		1-06-00 1-06-00	120.06 75.33		
	23	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	386.27 245.33		
	4	J2 Jack-Open	7 /12	6-06-08	5-01-04	2 x 4	1-03-08	1-03-07 5-01-04	75.75 48.00		
	3	J3 Jack-Open	4 /12	3-10-08	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	36.06 24.00		
	5	J20 Jack-Open	7 /12	5-10-08	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	86.49 53.33		
	5	J21 Jack-Open	7 /12	3-01-08	4-01-05	2 x 4	1-03-08	2-03-07 4-01-05	67.56 47.50		

TOTAL # TRUSS= **70** TOTAL BFT OF ALL TRUSSES= **2684.48** BFT. TOTAL WEIGHT OF ALL TRSSES **4281.4** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LUS24	
1	Hardware	LUS26-2	
2	Hardware	HGUS26-2	

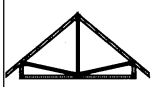
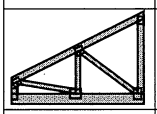
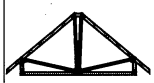
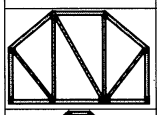
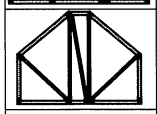
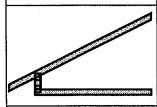
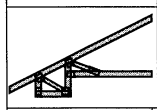
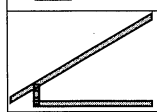
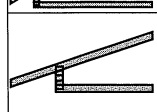
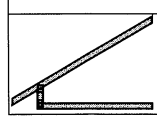
TOTAL NUMBER OF ITEMS= **9**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410967
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	2 of 3
Lot #:		Date:	05/26/2021
Elevation:	B-STD.WITH OPT.COFF	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

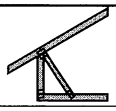
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T9Z Roof Special Girder	10 /12	**35-11-00**	5-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	398.56 245.00		
	1	T10 Common	10 /12	**11-00-00**	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	49.09 32.33		
	1 2-ply	T13Z1 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T20 Hip Girder	10 /12	**11-00-00**	4-08-09	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	51.64 34.00		
	1	T21 Hip	10 /12	**11-00-00**	5-10-09	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	52.77 34.67		
	1	T22 Hip Girder	10 /12	**10-09-00**	4-01-05	2 x 4 2 x 6		2-05-03 2-05-03	57.35 37.33		
	1	T23 Hip	10 /12	**10-03-00**	6-09-05	2 x 4		4-01-11 4-01-11	58.11 38.00		
	1	T24 Hip	10 /12	**10-03-00**	7-07-13	2 x 4		4-01-11 4-01-11	61.47 40.00		
	2 2-ply	T25 Flat Girder	0 /12	**6-06-08**	1-06-00	2 x 6		1-06-00 1-06-00	120.06 75.33		
	17	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		
	6	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 3-01-04	125.09 88.00		
	4	J2 Jack-Open	7 /12	**6-06-08**	5-01-04	2 x 4	1-03-08	1-03-07 5-01-04	75.75 48.00		
	3	J3 Jack-Open	4 /12	**3-10-08**	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	36.06 24.00		
	5	J20 Jack-Open	7 /12	**5-10-08**	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	86.49 53.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410967
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	3 of 3
Lot #:		Date:	05/26/2021
Elevation:	B-STD.WITH OPT.COFF	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	5	J21 Jack-Open	7 /12	3-01-08	4-01-05	2 x 4	1-03-08	2-03-07 4-01-05	67.56 47.50		

TOTAL # TRUSS= **70** TOTAL BFT OF ALL TRUSSES= **2804.81** BFT. TOTAL WEIGHT OF ALL TRSSES **4420.05** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
6	Hardware	LUS24	
1	Hardware	LUS26-2	

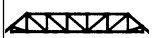
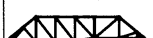
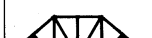
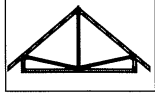
TOTAL NUMBER OF ITEMS= **9**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410968
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	1 of 3
Lot #:		Date:	05/26/2021
Elevation:	B-5 BRM WITH OPT.COFF.	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

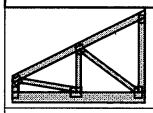
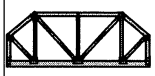
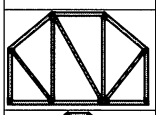
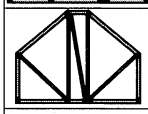
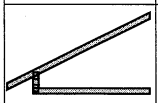
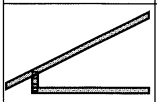
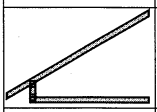
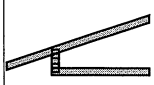
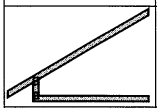
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1S1 Hip Girder	10 /12	35-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	395 251.33		
	1	T2 Hip	10 /12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	148.9 93.50		
	1	T2S1 Hip	10 /12	35-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	158.5 101.33		
	1	T3 Hip	10 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	150.38 95.00		
	1	T3S1 Hip	10 /12	35-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.58 103.50		
	1	T4 Hip	10 /12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	163.65 102.17		
	1	T4S1 Hip	10 /12	35-11-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	171.34 108.83		
	1	T5 Hip	10 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	161.59 101.00		
	1	T5S1 Hip	10 /12	35-11-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	179.61 114.17		
	2	T6 Hip	10 /12	35-11-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	367.4 230.00		
	2	T7 Hip	10 /12	35-11-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	351.11 220.00		
	2	T8 Hip	10 /12	35-11-00	11-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	384.63 238.00		
	1 2-ply	T9Z Roof Special Girder	10 /12	35-11-00	5-01-10	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	398.56 245.00		
	1	T10 Common	10 /12	11-00-00	6-02-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	49.09 32.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410968
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	2 of 3
Lot #:		Date:	05/26/2021
Elevation:	B-5 BRM WITH OPT.COFF.	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T13Z1 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1	T20 Hip Girder	10 /12	**11-00-00**	4-08-09	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	51.64 34.00		
	1	T21 Hip	10 /12	**11-00-00**	5-10-09	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	52.77 34.67		
	1	T22 Hip Girder	10 /12	**10-09-00**	4-01-05	2 x 4 2 x 6		2-05-03 2-05-03	57.35 37.33		
	1	T23 Hip	10 /12	**10-03-00**	6-09-05	2 x 4		4-01-11 4-01-11	58.11 38.00		
	1	T24 Hip	10 /12	**10-03-00**	7-07-13	2 x 4		4-01-11 4-01-11	61.47 40.00		
	2 2-ply	T25 Flat Girder	0 /12	**6-06-08**	1-06-00	2 x 6		1-06-00 1-06-00	120.06 75.33		
	18	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	302.3 192.00		
	5	J1S Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	83.97 53.33		
	4	J2 Jack-Open	7 /12	**6-06-08**	5-01-04	2 x 4	1-03-08	1-03-07 5-01-04	75.75 48.00		
	3	J3 Jack-Open	4 /12	**3-10-08**	2-04-11	2 x 4	1-09-08	1-01-03 2-04-11	36.06 24.00		
	5	J20 Jack-Open	7 /12	**5-10-08**	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	86.49 53.33		
	5	J21 Jack-Open	7 /12	**3-01-08**	4-01-05	2 x 4	1-03-08	2-03-07 4-01-05	67.56 47.50		

TOTAL # TRUSS= **70** TOTAL BFT OF ALL TRUSSES= **2750.65** BFT. TOTAL WEIGHT OF ALL TRSSES **4354.52** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410968
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	3 of 3
Lot #:		Date:	05/26/2021
Elevation:	B-5 BRM WITH OPT.COFF.	Designer:	
		Sales Rep:	Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
6	Hardware	LUS24	
1	Hardware	LUS26-2	

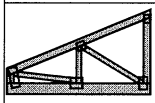
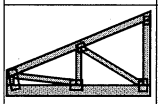
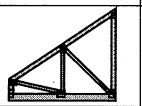
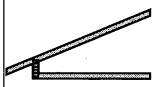
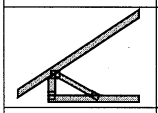
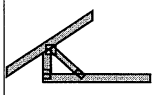
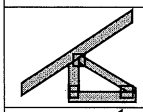
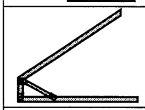
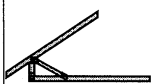
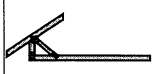
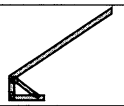
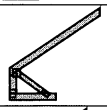
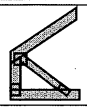
TOTAL NUMBER OF ITEMS= **9**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	418973
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	2 of 3
Lot #:		Date:	05/26/2021
Elevation:	C-STD.OR 5 BRM	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T44 Half Hip	8 /12	11-10-00	4-10-08	2 x 4	1-03-08	1-04-13 4-10-08	52.32 34.17		
	1 2-ply	T45 Jack-Closed Girder	5 /12	5-10-08	3-05-15	2 x 4 2 x 6		1-00-09 3-05-15	54.49 34.67		
	1 2-ply	T45Z Jack-Closed Girder	5 /12	5-10-08	3-05-15	2 x 4 2 x 6		1-00-09 3-05-15	54.49 34.67		
	1 2-ply	T46 Jack-Closed Girder	8 /12	6-10-08	5-11-13	2 x 4 2 x 6		1-04-13 5-11-13	74.36 47.33		
	23	J30 Jack-Open	5 /12	5-10-08	3-05-15	2 x 4	1-03-08	1-00-09 3-05-15	375.6 230.00		
	6	J31 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	87.32 57.00		
	4	C30 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-01-01	1-04-13 2-07-02	44.74 28.00		
	4	C31 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	36.68 22.67		
	1	C32 Jack-Open	8 /12	6-10-08	5-04-02	2 x 4		1-04-13 5-11-13	19 12.17		
	1	C33 Jack-Open	8 /12	3-10-15	4-00-02	2 x 4	1-03-08 2-11-09	1-04-13 4-00-02	18.02 11.50		
	1	C34 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 4-11-09	1-04-13 2-08-02	14.87 9.67		
	1	C35 Jack-Open	8 /12	2-00-00	5-04-02	2 x 4	3-10-15	1-04-13 2-08-13	13.08 8.33		
	1	C36 Jack-Open	8 /12	2-00-00	4-00-02	2 x 4	1-10-15	1-04-13 2-08-13	10.34 6.33		
	1	C37 Jack-Open	8 /12	2-00-00	2-08-02	2 x 4		1-04-13 2-08-13	7.6 5.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	418973
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	3 of 3
Lot #:		Date:	05/26/2021
Elevation:	C-STD.OR 5 BRM	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **73** TOTAL BFT OF ALL TRUSSES= **2511.35** BFT. TOTAL WEIGHT OF ALL TRSSES **4039.39** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LJS26DS	
7	Hardware	LUS24	
2	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

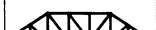
TOTAL NUMBER OF ITEMS= **11**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410969
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	1 of 3
Lot #:		Date:	05/26/2021
Elevation:	C- STD.WITH OPT.COFF.	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

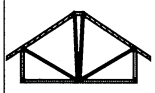
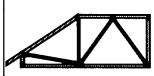
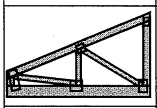
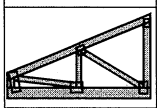
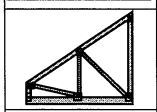
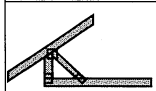
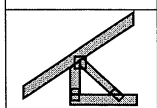
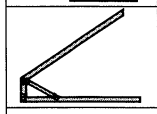
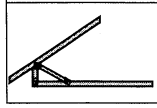
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T30S Hip Girder	8 /12	35-11-00	3-05-15	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	380.75 233.67		
	1	T31 Hip	8 /12	35-11-00	4-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	141.27 89.17		
	1	T31S Hip	8 /12	35-11-00	4-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	153.18 97.00		
	1	T32 Hip	8 /12	35-11-00	5-01-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	142.49 88.50		
	1	T32S Hip	8 /12	35-11-00	5-01-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	154.45 96.67		
	1	T33 Hip	8 /12	35-11-00	5-11-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	152.6 96.33		
	1	T33S Hip	8 /12	35-11-00	5-11-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	160.37 102.17		
	1	T34 Hip	8 /12	35-11-00	6-09-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	154.62 96.17		
	1	T34S Hip	8 /12	35-11-00	6-09-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	168.82 106.50		
	1	T35 Hip	8 /12	35-11-00	7-07-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	160.98 102.50		
	1	T35S Hip	8 /12	35-11-00	7-07-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	170.91 109.83		
	1	T36 Hip	8 /12	35-11-00	8-05-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	163.01 100.00		
	1	T36S Hip	8 /12	35-11-00	8-05-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	182.36 114.00		
	2	T37 Hip	8 /12	35-11-00	9-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	336.58 207.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410969
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	2 of 3
Lot #:		Date:	05/26/2021
Elevation:	C- STD.WITH OPT.COFF.	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

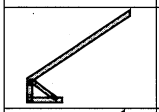
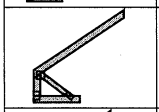
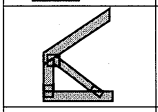
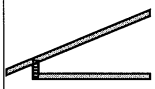
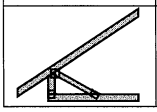
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T38 Roof Special Girder	8 /12	35-11-00	5-11-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	383.01 235.00		
	1	T39 Hip Girder	8 /12	11-00-00	3-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	48.21 31.17		
	1	T40 Hip	8 /12	11-00-00	6-08-13	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	57.61 35.67		
	1	T41 Hip	8 /12	11-00-00	5-04-13	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	56.98 36.17		
	1	T42 Hip	8 /12	11-00-00	4-00-13	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	53.76 33.33		
	1	T43 Hip Girder	8 /12	11-10-00	3-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	50.58 32.50		
	1	T44 Half Hip	8 /12	11-10-00	4-10-08	2 x 4	1-03-08	1-04-13 4-10-08	52.32 34.17		
	1 2-ply	T45 Jack-Closed Girder	5 /12	5-10-08	3-05-15	2 x 4 2 x 6		1-00-09 3-05-15	54.49 34.67		
	1 2-ply	T45Z Jack-Closed Girder	5 /12	5-10-08	3-05-15	2 x 4 2 x 6		1-00-09 3-05-15	54.49 34.67		
	1 2-ply	T46 Jack-Closed Girder	8 /12	6-10-08	5-11-13	2 x 4 2 x 6		1-04-13 5-11-13	74.36 47.33		
	4	C30 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-01-01	1-04-13 2-07-02	44.74 28.00		
	4	C31 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	35.62 22.67		
	1	C32 Jack-Open	8 /12	6-10-08	5-04-02	2 x 4		1-04-13 5-11-13	19 12.17		
	1	C33 Jack-Open	8 /12	3-10-15	4-00-02	2 x 4	1-03-08 2-11-09	1-04-13 4-00-02	18.02 11.50		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410969
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	3 of 3
Lot #:		Date:	05/26/2021
Elevation:	C- STD.WITH OPT.COFF.	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	C34 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 4-11-09	1-04-13 2-08-02	14.87 9.67		
	1	C35 Jack-Open	8 /12	2-00-00	5-04-02	2 x 4	3-10-15	1-04-13 2-08-13	13.08 8.33		
	1	C36 Jack-Open	8 /12	2-00-00	4-00-02	2 x 4	1-10-15	1-04-13 2-08-13	10.34 6.33		
	1	C37 Jack-Open	8 /12	2-00-00	2-08-02	2 x 4		1-04-13 2-08-13	7.6 5.00		
	17	J30 Jack-Open	5 /12	5-10-08	3-05-15	2 x 4	1-03-08	1-00-09 3-05-15	277.62 170.00		
	6	J30S Jack-Open	5 /12	5-10-08	3-05-15	2 x 4	1-03-08	1-00-09 2-05-15	117.58 84.00		
	6	J31 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	87.32 57.00		

TOTAL # TRUSS= **73** TOTAL BFT OF ALL TRUSSES= **2609.53** BFT. TOTAL WEIGHT OF ALL TRSSES **4153.98** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
6	Hardware	LUS24	
2	Hardware	LUS26-2	

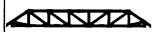
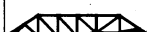
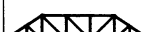
TOTAL NUMBER OF ITEMS= **10**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410970
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	1 of 3
Lot #:		Date:	05/26/2021
Elevation:	C-5 BRM WITH OPT. COFF.	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

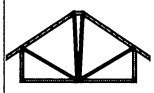
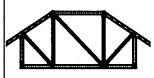
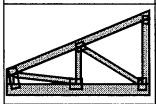
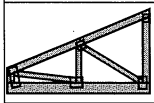
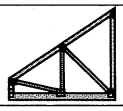
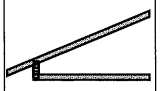
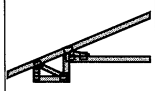
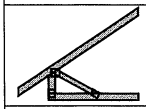
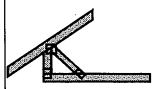
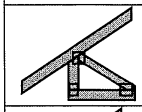
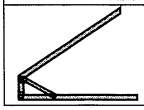
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T30S1 Hip Girder	8 /12	**35-11-00**	3-05-15	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	379.53 233.00		
	1	T31 Hip	8 /12	**35-11-00**	4-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	141.27 89.17		
	1	T31S1 Hip	8 /12	**35-11-00**	4-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	152.93 97.00		
	1	T32 Hip	8 /12	**35-11-00**	5-01-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	142.49 88.50		
	1	T32S1 Hip	8 /12	**35-11-00**	5-01-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	153.99 97.67		
	1	T33 Hip	8 /12	**35-11-00**	5-11-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	152.6 96.33		
	1	T33S1 Hip	8 /12	**35-11-00**	5-11-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	160.05 102.00		
	1	T34 Hip	8 /12	**35-11-00**	6-09-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	154.62 96.17		
	1	T34S1 Hip	8 /12	**35-11-00**	6-09-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	168.46 106.50		
	2	T35 Hip	8 /12	**35-11-00**	7-07-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	321.96 205.00		
	2	T36 Hip	8 /12	**35-11-00**	8-05-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	326.02 200.00		
	2	T37 Hip	8 /12	**35-11-00**	9-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	336.58 207.67		
	1 2-ply	T38 Roof Special Girder	8 /12	**35-11-00**	5-11-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	383.01 235.00		
	1	T39 Hip Girder	8 /12	**11-00-00**	3-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	48.21 31.17		

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		Sales Rep:	Mario DiCano

Roof Trusses

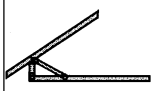
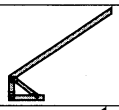
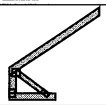
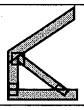
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T40 Hip	8 /12	11-00-00	6-08-13	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	57.61 35.67		
	1	T41 Hip	8 /12	11-00-00	5-04-13	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	56.98 36.17		
	1	T42 Hip	8 /12	11-00-00	4-00-13	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	53.76 33.33		
	1	T43 Hip Girder	8 /12	11-10-00	3-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	50.58 32.50		
	1	T44 Half Hip	8 /12	11-10-00	4-10-08	2 x 4	1-03-08	1-04-13 4-10-08	52.32 34.17		
	1 2-ply	T45 Jack-Closed Girder	5 /12	5-10-08	3-05-15	2 x 4 2 x 6		1-00-09 3-05-15	54.49 34.67		
	1 2-ply	T45Z Jack-Closed Girder	5 /12	5-10-08	3-05-15	2 x 4 2 x 6		1-00-09 3-05-15	54.49 34.67		
	1 2-ply	T46 Jack-Closed Girder	8 /12	6-10-08	5-11-13	2 x 4 2 x 6		1-04-13 5-11-13	74.36 47.33		
	17	J30 Jack-Open	5 /12	5-10-08	3-05-15	2 x 4	1-03-08	1-00-09 3-05-15	277.62 170.00		
	6	J30S Jack-Open	5 /12	5-10-08	3-05-15	2 x 4	1-03-08	1-00-09 2-05-15	117.58 84.00		
	6	J31 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	87.32 57.00		
	4	C30 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-01-01	1-04-13 2-07-02	44.74 28.00		
	4	C31 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	36.68 22.67		
	1	C32 Jack-Open	8 /12	6-10-08	5-04-02	2 x 4		1-04-13 5-11-13	19 12.17		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	410970
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	3 of 3
Lot #:		Date:	05/26/2021
Elevation:	C-5 BRM WITH OPT. COFF.	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	C33 Jack-Open	8 /12	3-10-15	4-00-02	2 x 4	1-03-08 2-11-09	1-04-13 4-00-02	18.02 11.50		
	1	C34 Jack-Open	8 /12	1-10-15	2-08-02	2 x 4	1-03-08 4-11-09	1-04-13 2-08-02	14.87 9.67		
	1	C35 Jack-Open	8 /12	2-00-00	5-04-02	2 x 4	3-10-15	1-04-13 2-08-13	13.08 8.33		
	1	C36 Jack-Open	8 /12	2-00-00	4-00-02	2 x 4	1-10-15	1-04-13 2-08-13	10.34 6.33		
	1	C37 Jack-Open	8 /12	2-00-00	2-08-02	2 x 4		1-04-13 2-08-13	7.6 5.00		

TOTAL # TRUSS= **73** TOTAL BFT OF ALL TRUSSES= **2588.36** BFT. TOTAL WEIGHT OF ALL TRSSES **4123.14** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
1	Hardware	LJS26DS	
7	Hardware	LUS24	
3	Hardware	LUS26-2	

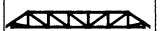
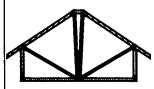
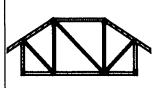
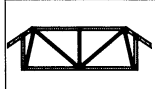
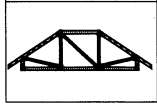
TOTAL NUMBER OF ITEMS= **12**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	51004
Builder:	ROYAL PINE HOMES	PlanLog:	203059
Project:	CENTREFIELD	Layout ID:	418973
Location:	RICHMOND HILL	Ref #	
Model:	UNIT-4502	Page:	1 of 3
Lot #:		Date:	05/26/2021
Elevation:	C-STD.OR 5 BRM	Designer:	
		Sales Rep:	Mario DiCano

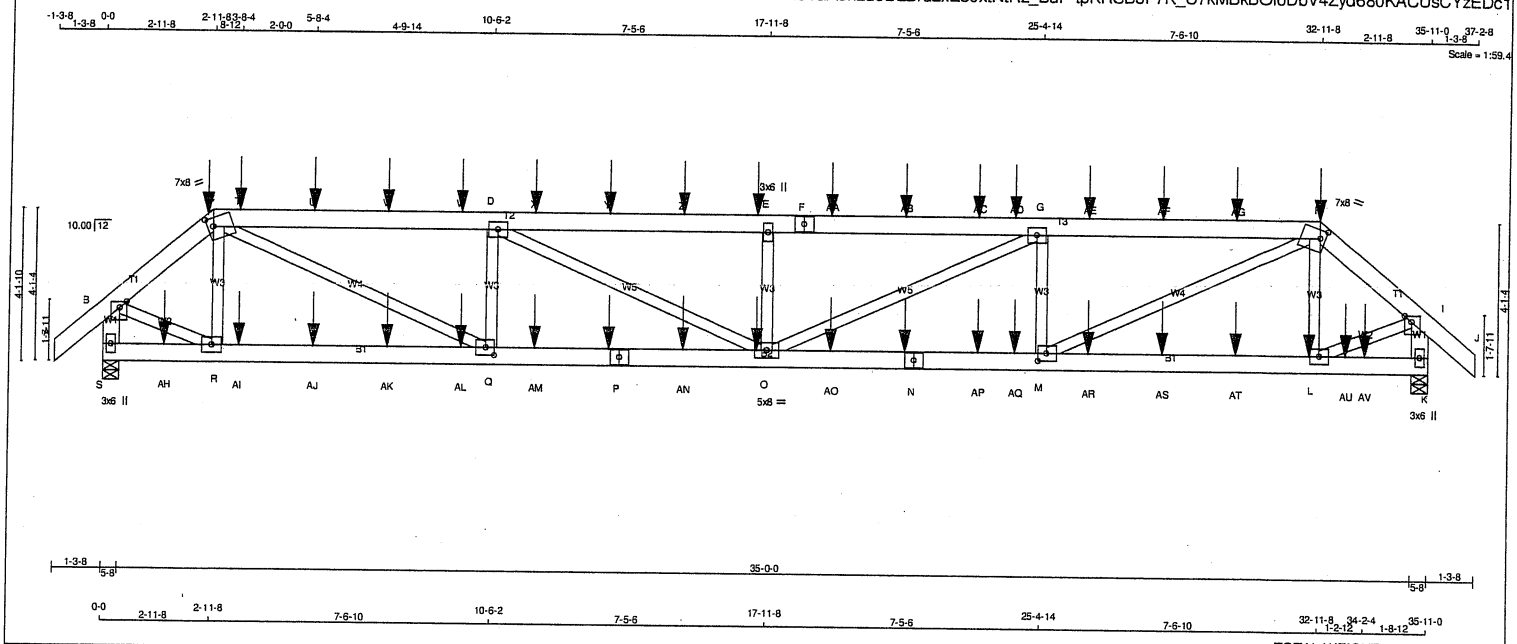
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T30 Hip Girder	8 /12	**35-11-00**	3-06-01	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	359.83 213.00		
	2	T31 Hip	8 /12	**35-11-00**	4-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	282.53 178.33		
	2	T32 Hip	8 /12	**35-11-00**	5-01-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	284.97 177.00		
	2	T33 Hip	8 /12	**35-11-00**	5-11-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	305.19 192.67		
	2	T34 Hip	8 /12	**35-11-00**	6-09-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	309.25 192.33		
	2	T35 Hip	8 /12	**35-11-00**	7-07-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	321.96 205.00		
	2	T36 Hip	8 /12	**35-11-00**	8-05-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	326.02 200.00		
	2	T37 Hip	8 /12	**35-11-00**	9-03-15	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	336.58 207.67		
	1 2-ply	T38 Roof Special Girder	8 /12	**35-11-00**	5-11-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	383.01 235.00		
	1	T39 Hip Girder	8 /12	**11-00-00**	3-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	48.21 31.17		
	1	T40 Hip	8 /12	**11-00-00**	6-08-13	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	57.61 35.67		
	1	T41 Hip	8 /12	**11-00-00**	5-04-13	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	56.98 36.17		
	1	T42 Hip	8 /12	**11-00-00**	4-00-13	2 x 4	1-03-08 1-03-08	3-04-13 3-04-13	53.76 33.33		
	1	T43 Hip Girder	8 /12	**11-10-00**	3-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	50.58 32.50		

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
418971	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:28:28 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-tpKRSB3F7K_C7kMBkBOloDbV4Zyd680KACUsCYzEDc1



TOTAL WEIGHT = 2 X 203 = 405 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- F	2	12	SIDE(183.1)
F- H	2	12	SIDE(61.0)
H- J	2	12	SIDE(122.0)
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	SIDE(0.0)
P- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.



Structural component only
DWG# T-2115333 //

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3446	0	3446	0	0	5-8	5-8
K	3520	0	3520	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2436	1606 / 0	0 / 0	0 / 0	0 / 0	830 / 0	0 / 0
K	2491	1625 / 0	0 / 0	0 / 0	0 / 0	865 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-91.8	-91.8	0.04 (1)	10.00
B- C	-3479 / 0	-91.8	-91.8	0.05 (1)	5.95
C- T	-6967 / 0	-91.8	-91.8	0.42 (1)	4.18
T- U	-6967 / 0	-91.8	-91.8	0.42 (1)	4.18
U- V	-6967 / 0	-91.8	-91.8	0.42 (1)	4.18
V- W	-6967 / 0	-91.8	-91.8	0.42 (1)	4.18
W- D	-6967 / 0	-91.8	-91.8	0.42 (1)	4.18
D- X	-8313 / 0	-91.8	-91.8	0.47 (1)	3.83
X- Y	-8313 / 0	-91.8	-91.8	0.47 (1)	3.83
Y- Z	-8313 / 0	-91.8	-91.8	0.47 (1)	3.83
Z- E	-8313 / 0	-91.8	-91.8	0.47 (1)	3.83
E- F	-8313 / 0	-91.8	-91.8	0.47 (1)	3.83
F-AA	-8313 / 0	-91.8	-91.8	0.47 (1)	3.83
AA-AB	-8313 / 0	-91.8	-91.8	0.47 (1)	3.83
AB-AC	-8313 / 0	-91.8	-91.8	0.47 (1)	3.83
AC-AD	-8313 / 0	-91.8	-91.8	0.47 (1)	3.83
AD- G	-8313 / 0	-91.8	-91.8	0.47 (1)	3.83
G-AE	-7058 / 0	-91.8	-91.8	0.43 (1)	4.14
AE-AF	-7058 / 0	-91.8	-91.8	0.43 (1)	4.14
AF-AG	-7058 / 0	-91.8	-91.8	0.43 (1)	4.14
AG- H	-7058 / 0	-91.8	-91.8	0.43 (1)	4.14
H- I	-3550 / 0	-91.8	-91.8	0.05 (1)	5.91
I- J	0 / 42	-91.8	-91.8	0.04 (1)	10.00
S- B	-3467 / 0	0.0	0.0	0.12 (1)	7.51
K- I	-3532 / 0	0.0	0.0	0.13 (1)	7.45
S-AH	0 / 0	-18.5	-18.5	0.08 (4)	10.00
AH- R	0 / 0	-18.5	-18.5	0.08 (4)	10.00
R-AI	0 / 2629	-18.5	-18.5	0.24 (1)	10.00
AI-AJ	0 / 2629	-18.5	-18.5	0.24 (1)	10.00
AJ-AK	0 / 2629	-18.5	-18.5	0.24 (1)	10.00
AK-AL	0 / 2629	-18.5	-18.5	0.24 (1)	10.00
AL- Q	0 / 2629	-18.5	-18.5	0.24 (1)	10.00
Q-AM	0 / 6967	-18.5	-18.5	0.52 (1)	10.00
AM- P	0 / 6967	-18.5	-18.5	0.52 (1)	10.00
P-AN	0 / 6967	-18.5	-18.5	0.52 (1)	10.00
AN- O	0 / 6967	-18.5	-18.5	0.52 (1)	10.00
O-AO	0 / 7059	-18.5	-18.5	0.53 (1)	10.00
AO- N	0 / 7059	-18.5	-18.5	0.53 (1)	10.00
N-AP	0 / 7059	-18.5	-18.5	0.53 (1)	10.00
AP-AQ	0 / 7059	-18.5	-18.5	0.53 (1)	10.00
AQ- M	0 / 7059	-18.5	-18.5	0.53 (1)	10.00
M-AR	0 / 2683	-18.5	-18.5	0.24 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-704 / 0	0.06 (1)
C- Q	0 / 4809	0.43 (1)
Q- D	-1868 / 0	0.16 (1)
D- O	0 / 1498	0.13 (1)
O- E	-1026 / 0	0.09 (1)
O- G	0 / 1397	0.12 (1)
M- G	-1871 / 0	0.16 (1)
M- H	0 / 4850	0.43 (1)
L- H	-699 / 0	0.06 (1)
B- R	0 / 2823	0.25 (1)
L- I	0 / 2881	0.25 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.47/1.00 (E-G:1) , BC=0.53/1.00 (M-O:1) , WB=0.43/1.00 (H-M:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.69 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
418971	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	7.0	8.0	2.75	1.75
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW-m	MT20	7.0	8.0	2.75	1.75
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

NOTES (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AR-AS	0 / 2683	-18.5	-18.5	0.24 (1)	10.00			
AS-AT	0 / 2683	-18.5	-18.5	0.24 (1)	10.00			
AT- L	0 / 2683	-18.5	-18.5	0.24 (1)	10.00			
L-AU	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
AU-AV	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
AV- K	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
E	17-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
H	32-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
H	32-11-8	-80	-80	---	FRONT	VERT	TOTAL	---	C1
H	32-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
L	32-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	21-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
O	17-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
P	13-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	3-8-4	-85	-85	---	FRONT	VERT	TOTAL	---	C1
U	5-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
V	7-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
W	9-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
X	11-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Y	13-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Z	15-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AA	19-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AB	21-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	23-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AD	24-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AE	26-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AF	28-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AG	30-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AH	1-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AI	3-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	5-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	7-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	9-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	11-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	15-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	19-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	23-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	24-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	26-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	28-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	30-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	33-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	34-2-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

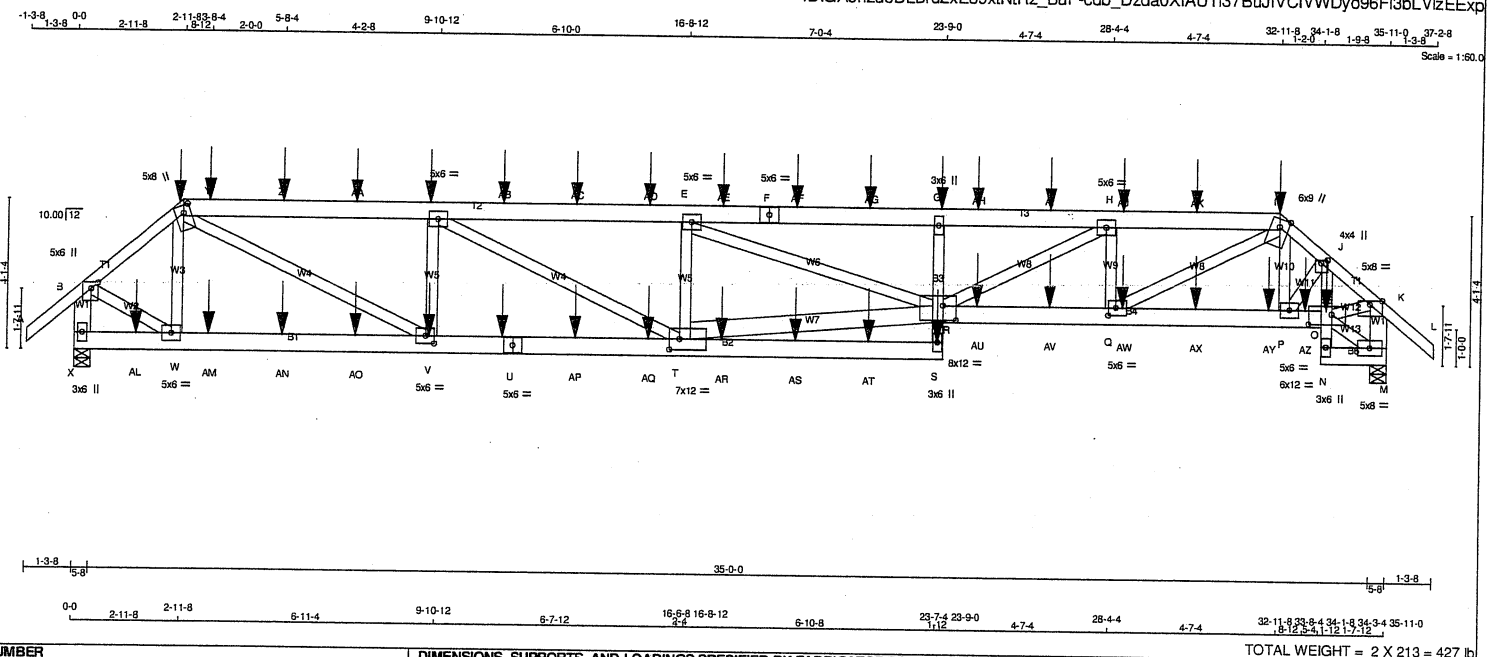


Structural component only
DWG# T-2115333

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410965	T1S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 213 = 427 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
I- L	1	12	SIDE(61.0)
C- F	2	12	SIDE(183.1)
F- I	2	12	SIDE(183.1)
X- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X- U	2	12	SIDE(183.1)
U- S	2	12	SIDE(122.0)
R- O	2	12	SIDE(183.1)
N- M	2	12	TOP
S- G	1	12	TOP
J- N	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.



Structural component only
DWG# T-2115258

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	3511	0	3511	0	0	5-8	5-8
M	3846	0	3846	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
X	2482	1635 / 0	0 / 0	0 / 0	0 / 0	847 / 0	0 / 0
M	2722	1776 / 0	0 / 0	0 / 0	0 / 0	946 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 41	-91.8	-91.8	0.07 (1)		10.00
B- C	-3360 / 0	-91.8	-91.8	0.12 (1)		5.00
C- Y	-6806 / 0	-91.8	-91.8	0.33 (1)		4.31
Y- Z	-6806 / 0	-91.8	-91.8	0.33 (1)		4.31
Z-AA	-6806 / 0	-91.8	-91.8	0.33 (1)		4.31
AA- D	-6806 / 0	-91.8	-91.8	0.33 (1)		4.31
D-AB	-8569 / 0	-91.8	-91.8	0.38 (1)		3.88
AB-AC	-8569 / 0	-91.8	-91.8	0.38 (1)		3.88
AC-AD	-8569 / 0	-91.8	-91.8	0.38 (1)		3.88
AD- E	-8569 / 0	-91.8	-91.8	0.38 (1)		3.88
E-AE	-10775 / 0	-91.8	-91.8	0.48 (1)		3.43
AE- F	-10775 / 0	-91.8	-91.8	0.48 (1)		3.43
F-AF	-10775 / 0	-91.8	-91.8	0.48 (1)		3.43
AF-AG	-10775 / 0	-91.8	-91.8	0.48 (1)		3.43
AG- G	-10775 / 0	-91.8	-91.8	0.48 (1)		3.43
G-AH	-10913 / 0	-91.8	-91.8	0.31 (1)		3.55
AH-AI	-10913 / 0	-91.8	-91.8	0.31 (1)		3.55
AI- H	-10913 / 0	-91.8	-91.8	0.31 (1)		3.55
H-AJ	-8217 / 0	-91.8	-91.8	0.24 (1)		4.08
AJ-AK	-8217 / 0	-91.8	-91.8	0.24 (1)		4.08
AK- I	-8217 / 0	-91.8	-91.8	0.24 (1)		4.08
I- J	-5042 / 0	-91.8	-91.8	0.14 (1)		4.21
J- K	-4861 / 0	-91.8	-91.8	0.14 (1)		4.27
K- L	0 / 41	-91.8	-91.8	0.07 (1)		10.00
X- B	-3529 / 0	0.0	0.0	0.13 (1)		7.45
M- K	-3762 / 0	0.0	0.0	0.14 (1)		7.28
X-AL	0 / 0	-18.5	-18.5	0.07 (4)		10.00
AL- W	0 / 0	-18.5	-18.5	0.07 (4)		10.00
W-AM	0 / 2625	-18.5	-18.5	0.23 (1)		10.00
AM-AN	0 / 2625	-18.5	-18.5	0.23 (1)		10.00
AN-AO	0 / 2625	-18.5	-18.5	0.23 (1)		10.00
AO- V	0 / 2625	-18.5	-18.5	0.23 (1)		10.00
V- U	0 / 6806	-18.5	-18.5	0.50 (1)		10.00
U-AP	0 / 6806	-18.5	-18.5	0.50 (1)		10.00
AP-AQ	0 / 6806	-18.5	-18.5	0.50 (1)		10.00
AQ- T	0 / 6806	-18.5	-18.5	0.50 (1)		10.00
T-AR	0 / 366	-18.5	-18.5	0.10 (4)		10.00
AR-AS	0 / 366	-18.5	-18.5	0.10 (4)		10.00
AS-AT	0 / 366	-18.5	-18.5	0.10 (4)		10.00
AT- S	0 / 366	-18.5	-18.5	0.10 (4)		10.00
S- R	0 / 131	0.0	0.0	0.28 (1)		10.00
R- G	-855 / 0	0.0	0.0	0.28 (1)		7.81

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
W- C	-896 / 0	0.08 (1)
R- H	0 / 3086	0.27 (1)
Q- H	-2229 / 0	0.14 (1)
Q- I	0 / 4956	0.44 (1)
P- I	0 / 113	0.02 (4)
P- J	0 / 294	0.03 (1)
B- W	0 / 2824	0.25 (1)
O- M	-128 / 0	0.01 (1)
O- K	0 / 3705	0.33 (1)
C- V	0 / 4750	0.42 (1)
T- E	-1883 / 0	0.16 (1)
V- D	-2071 / 0	0.18 (1)
D- T	0 / 2003	0.18 (1)
T- R	0 / 8373	0.54 (1)
E- R	0 / 2268	0.20 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 859 (0.50")

CSI: TC=0.48/1.00 (E-G:1) , BC=0.61/1.00 (Q-R:1) , WB=0.54/1.00 (R-T:1) , SSI=0.19/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	8.0	2.50	2.25
D, E, H						
D	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMVW1-t	MT20	5.0	8.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-l	MT20	6.0	12.0	3.25	7.50
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BVMWWW-l	MT20	8.0	12.0	4.75	4.25
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	7.0	12.0	3.50	3.25
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.75
W	BMWW-t	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
R-AU	0 / 8217	-18.5	-18.5	0.61 (1)	10.00
AU-AV	0 / 8217	-18.5	-18.5	0.61 (1)	10.00
AV- Q	0 / 8217	-18.5	-18.5	0.61 (1)	10.00
Q-AW	0 / 3928	-18.5	-18.5	0.33 (1)	10.00
AW-AX	0 / 3928	-18.5	-18.5	0.33 (1)	10.00
AX-AY	0 / 3928	-18.5	-18.5	0.33 (1)	10.00
AY- P	0 / 3928	-18.5	-18.5	0.33 (1)	10.00
P-AZ	0 / 3754	-18.5	-18.5	0.32 (1)	10.00
AZ- O	0 / 3754	-18.5	-18.5	0.32 (1)	10.00
N- O	0 / 17	0.0	0.0	0.09 (1)	10.00
O- J	-357 / 0	0.0	0.0	0.09 (1)	7.81
N- M	0 / 108	-18.5	-18.5	0.01 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
D	9-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
G	23-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
I	32-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
I	32-11-8	-77	-77	---	FRONT	VERT	TOTAL	---	C1
I	32-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
O	34-3-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
S	23-7-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
U	11-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	9-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	3-8-4	-85	-85	---	FRONT	VERT	TOTAL	---	C1
Z	5-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AA	7-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AB	11-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	13-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AD	15-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AE	17-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AF	19-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AG	21-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AH	24-8-4	-73	-73	---	FRONT	VERT	TOTAL	---	C1
AI	26-8-4	-73	-73	---	FRONT	VERT	TOTAL	---	C1
AJ	28-8-4	-73	-73	---	FRONT	VERT	TOTAL	---	C1
AK	30-8-4	-73	-73	---	FRONT	VERT	TOTAL	---	C1
AL	1-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	3-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	5-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	7-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	13-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	15-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	17-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	19-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	21-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	24-8-4	-68	-68	---	FRONT	VERT	TOTAL	---	C1
AV	26-8-4	-68	-68	---	FRONT	VERT	TOTAL	---	C1
AW	28-8-4	-68	-68	---	FRONT	VERT	TOTAL	---	C1
AX	30-8-4	-68	-68	---	FRONT	VERT	TOTAL	---	C1
AY	32-8-4	-68	-68	---	FRONT	VERT	TOTAL	---	C1
AZ	33-8-4	-71	-71	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.87 (V) (INPUT = 0.90)
JSI METAL= 0.64 (U) (INPUT = 1.00)

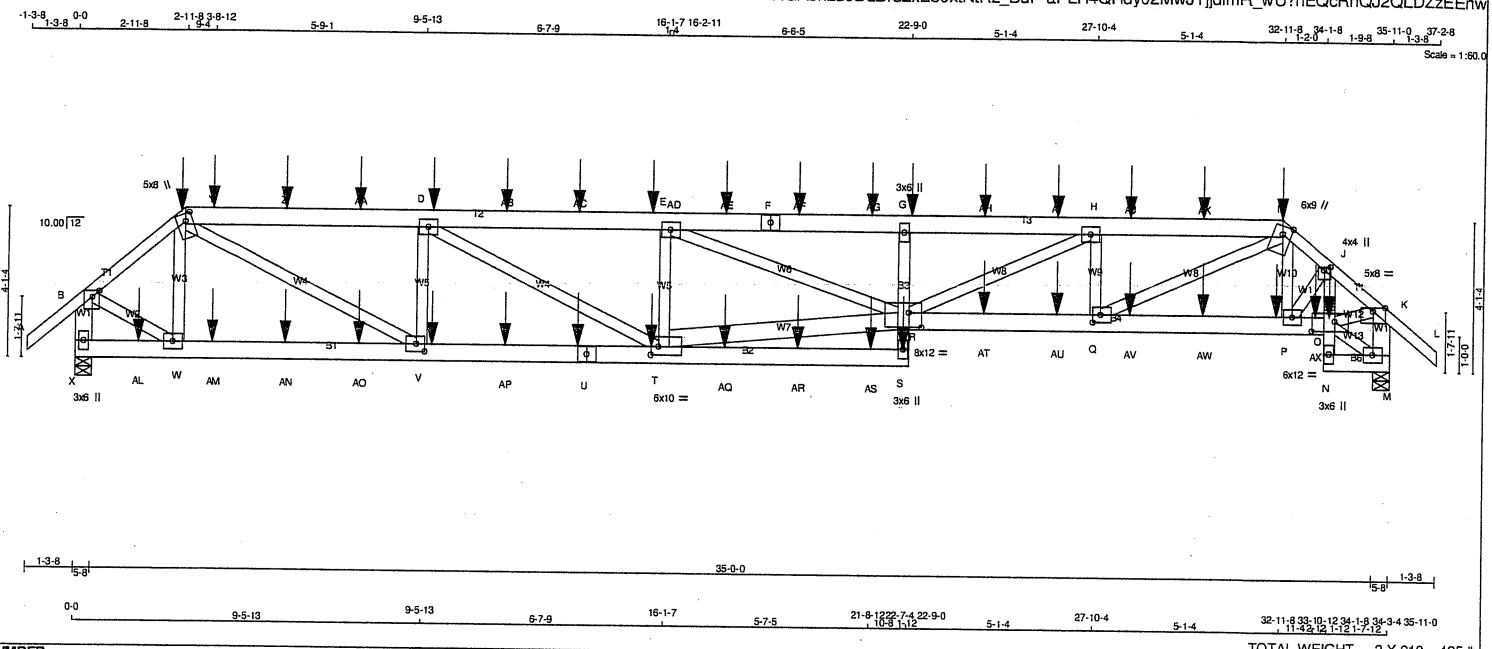
LICENSED PROFESSIONAL ENGINEER
05/22/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115258 JL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410966	T1S1	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 213 = 425 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
I- L	1	12	SIDE(61.0)
C- F	2	12	SIDE(183.1)
F- I	2	12	SIDE(183.1)
X- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X- U	2	12	SIDE(183.1)
U- S	2	12	SIDE(183.1)
R- O	2	12	SIDE(183.1)
N- M	2	12	TOP
S- G	1	12	TOP
J- N	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.



Structural component only
DWG# T-2115280 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	3532	0	3532	0	0	5-8	5-8
M	3889	0	3889	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	2497	1644 / 0	0 / 0	0 / 0	0 / 0	852 / 0	0 / 0
M	2752	1795 / 0	0 / 0	0 / 0	0 / 0	957 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.07 (1)	10.00
B- C	-3378 / 0	-91.8	-91.8	0.12 (1)	4.99
C- Y	-6712 / 0	-91.8	-91.8	0.31 (1)	4.36
Y- Z	-6712 / 0	-91.8	-91.8	0.31 (1)	4.36
Z-AA	-6712 / 0	-91.8	-91.8	0.31 (1)	4.36
AA- D	-6712 / 0	-91.8	-91.8	0.31 (1)	4.36
D-AB	-8580 / 0	-91.8	-91.8	0.35 (1)	3.90
AB-AC	-8580 / 0	-91.8	-91.8	0.35 (1)	3.90
AC-AD	-8580 / 0	-91.8	-91.8	0.35 (1)	3.90
AD- E	-8580 / 0	-91.8	-91.8	0.35 (1)	3.90
E-AE	-11258 / 0	-91.8	-91.8	0.46 (1)	3.37
AE- F	-11258 / 0	-91.8	-91.8	0.46 (1)	3.37
F-AF	-11258 / 0	-91.8	-91.8	0.46 (1)	3.37
AF-AG	-11258 / 0	-91.8	-91.8	0.46 (1)	3.37
AG- G	-11258 / 0	-91.8	-91.8	0.46 (1)	3.37
G-AH	-11395 / 0	-91.8	-91.8	0.37 (1)	3.43
AH-AI	-11395 / 0	-91.8	-91.8	0.37 (1)	3.43
AI- H	-11395 / 0	-91.8	-91.8	0.37 (1)	3.43
H-AJ	-8695 / 0	-91.8	-91.8	0.28 (1)	3.94
AJ-AK	-8695 / 0	-91.8	-91.8	0.28 (1)	3.94
AK- I	-8695 / 0	-91.8	-91.8	0.28 (1)	3.94
I- J	-5102 / 0	-91.8	-91.8	0.14 (1)	4.18
J- K	-4919 / 0	-91.8	-91.8	0.14 (1)	4.24
K- L	0 / 41	-91.8	-91.8	0.07 (1)	10.00
X- B	-3546 / 0	0.0	0.0	0.13 (1)	7.44
M- K	-3804 / 0	0.0	0.0	0.14 (1)	7.25
X-AL	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AL- W	0 / 0	-18.5	-18.5	0.07 (4)	10.00
W-AM	0 / 2640	-18.5	-18.5	0.22 (1)	10.00
AM-AN	0 / 2640	-18.5	-18.5	0.22 (1)	10.00
AN-AO	0 / 2640	-18.5	-18.5	0.22 (1)	10.00
AO- V	0 / 2640	-18.5	-18.5	0.22 (1)	10.00
V-AP	0 / 6711	-18.5	-18.5	0.49 (1)	10.00
AP- U	0 / 6711	-18.5	-18.5	0.49 (1)	10.00
U- T	0 / 6711	-18.5	-18.5	0.49 (1)	10.00
T-AQ	0 / 362	-18.5	-18.5	0.10 (4)	10.00
AQ-AR	0 / 362	-18.5	-18.5	0.10 (4)	10.00
AR-AS	0 / 362	-18.5	-18.5	0.10 (4)	10.00
AS- S	0 / 362	-18.5	-18.5	0.10 (4)	10.00
S- R	0 / 203	0.0	0.0	0.29 (1)	10.00
R- G	-834 / 0	0.0	0.0	0.28 (1)	7.81
R-AT	0 / 8695	-18.5	-18.5	0.65 (1)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
W- C	-910 / 0	0.08 (1)
W- H	0 / 3023	0.27 (1)
Q- H	-2189 / 0	0.14 (1)
Q- I	0 / 5326	0.47 (1)
P- I	0 / 129	0.02 (4)
P- J	0 / 298	0.03 (1)
B- W	0 / 2839	0.25 (1)
O- M	-130 / 0	0.01 (1)
O- K	0 / 3749	0.33 (1)
T- R	0 / 8403	0.55 (1)
E- R	0 / 2791	0.25 (1)
T- E	-2071 / 0	0.18 (1)
D- T	0 / 2145	0.19 (1)
C- V	0 / 4677	0.41 (1)
V- D	-2126 / 0	0.18 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	25.6 PSF
		DL	=	6.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.4 PSF
TOTAL	LOAD		=	39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 808 (0.53")

CSI: TC=0.46/1.00 (E-G:1) , BC=0.65/1.00 (Q-R:1) , WB=0.55/1.00 (R-T:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410966	T1S1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	8.0	2.50	2.25
D, E, H						
D	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-l	MT20	6.0	12.0	3.25	7.50
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BVMWWW-l	MT20	8.0	12.0	4.75	4.25
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	6.0	10.0	2.75	2.50
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.75
W	BMWW-t	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
AT-AU	0 / 8695	-18.5	-18.5	0.65 (1)	10.00			
AU- Q	0 / 8695	-18.5	-18.5	0.65 (1)	10.00			
Q-AV	0 / 3974	-18.5	-18.5	0.34 (1)	10.00			
AV-AW	0 / 3974	-18.5	-18.5	0.34 (1)	10.00			
AW- P	0 / 3974	-18.5	-18.5	0.34 (1)	10.00			
P-AX	0 / 3799	-18.5	-18.5	0.33 (1)	10.00			
AX- O	0 / 3799	-18.5	-18.5	0.33 (1)	10.00			
N- O	0 / 17	0.0	0.0	0.09 (1)	10.00			
O- J	-360 / 0	0.0	0.0	0.09 (1)	7.81			
N- M	0 / 110	-18.5	-18.5	0.01 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	2-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
D	9-8-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
G	22-9-12	-73	-73	---	BACK	VERT	TOTAL	---	C1
I	32-11-8	-30	-30	---	FRONT	VERT	DEAD	---	C1
I	32-11-8	-78	-78	---	BACK	VERT	TOTAL	---	C1
I	32-11-8	-127	-127	---	FRONT	VERT	SNOW	---	C1
O	34-3-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
P	32-9-12	-68	-68	---	BACK	VERT	TOTAL	---	C1
S	22-7-4	-74	-74	---	BACK	VERT	TOTAL	---	C1
T	15-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
U	13-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
V	9-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	3-8-12	-83	-83	---	BACK	VERT	TOTAL	---	C1
Z	5-8-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AA	7-8-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AB	11-8-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AC	13-8-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AD	15-8-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AE	17-8-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AF	19-8-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AG	21-8-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AH	24-9-12	-73	-73	---	BACK	VERT	TOTAL	---	C1
AI	26-9-12	-73	-73	---	BACK	VERT	TOTAL	---	C1
AJ	28-9-12	-73	-73	---	BACK	VERT	TOTAL	---	C1
AK	30-9-12	-73	-73	---	BACK	VERT	TOTAL	---	C1
AL	1-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AM	3-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AN	5-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AO	7-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AP	11-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AQ	17-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AR	19-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AS	21-8-12	-22	-22	---	BACK	VERT	TOTAL	---	C1
AT	24-9-12	-68	-68	---	BACK	VERT	TOTAL	---	C1
AU	26-9-12	-68	-68	---	BACK	VERT	TOTAL	---	C1
AV	28-9-12	-68	-68	---	BACK	VERT	TOTAL	---	C1
AW	30-9-12	-68	-68	---	BACK	VERT	TOTAL	---	C1
AX	33-10-12	-73	-73	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.62 (T) (INPUT = 1.00)

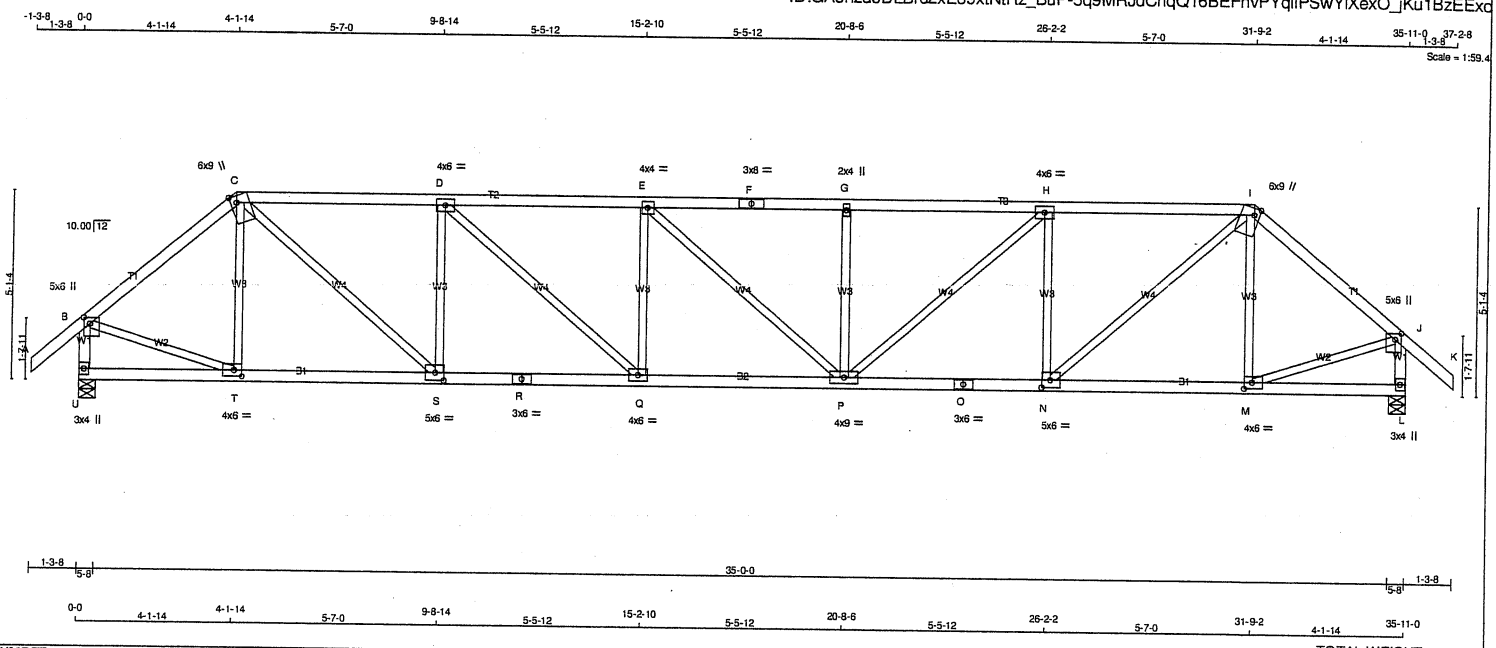


Structural component only
DWG# T-2115280 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMVW+p	MT20	5.0	6.0	2.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMWW-t	MT20	4.0	6.0	2.00	2.50
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115259

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2107	0	2107	0	0	5-8	5-8
L	2107	0	2107	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	-2007 / 0	-91.8	-91.8	0.36 (1)	4.41
C- D	-2985 / 0	-91.8	-91.8	0.62 (1)	3.48
D- E	-3657 / 0	-91.8	-91.8	0.72 (1)	3.09
E- F	-3656 / 0	-91.8	-91.8	0.59 (1)	3.26
F- G	-3656 / 0	-91.8	-91.8	0.59 (1)	3.26
G- H	-3656 / 0	-91.8	-91.8	0.72 (1)	3.09
H- I	-2986 / 0	-91.8	-91.8	0.62 (1)	3.48
I- J	-2007 / 0	-91.8	-91.8	0.36 (1)	4.41
J- K	0 / 41	-91.8	-91.8	0.13 (1)	10.00
U- B	-2078 / 0	0.0	0.0	0.22 (1)	5.87
L- J	-2078 / 0	0.0	0.0	0.22 (1)	5.87
U- T	0 / 0	-18.5	-18.5	0.10 (4)	10.00
T- S	0 / 1530	-18.5	-18.5	0.31 (1)	10.00
S- R	0 / 2986	-18.5	-18.5	0.53 (1)	10.00
R- Q	0 / 2986	-18.5	-18.5	0.53 (1)	10.00
Q- P	0 / 3657	-18.5	-18.5	0.64 (1)	10.00
P- O	0 / 2986	-18.5	-18.5	0.53 (1)	10.00
O- N	0 / 2986	-18.5	-18.5	0.53 (1)	10.00
N- M	0 / 1530	-18.5	-18.5	0.31 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-368 / 0	0.14 (1)
C- S	0 / 1918	0.43 (1)
S- D	-1150 / 0	0.44 (1)
D- Q	0 / 894	0.20 (1)
Q- E	-488 / 0	0.19 (1)
E- P	-2 / 0	0.00 (1)
P- G	-487 / 0	0.19 (1)
P- H	0 / 891	0.20 (1)
N- H	-1149 / 0	0.44 (1)
N- I	0 / 1919	0.43 (1)
M- I	-368 / 0	0.14 (1)
B- T	0 / 1608	0.36 (1)
M- J	0 / 1608	0.36 (1)

TOTAL WEIGHT = 149 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.42")

CSI: TC=0.72/1.00 (G-H:1) , BC=0.64/1.00 (P-Q:1) , WB=0.44/1.00 (D-S:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

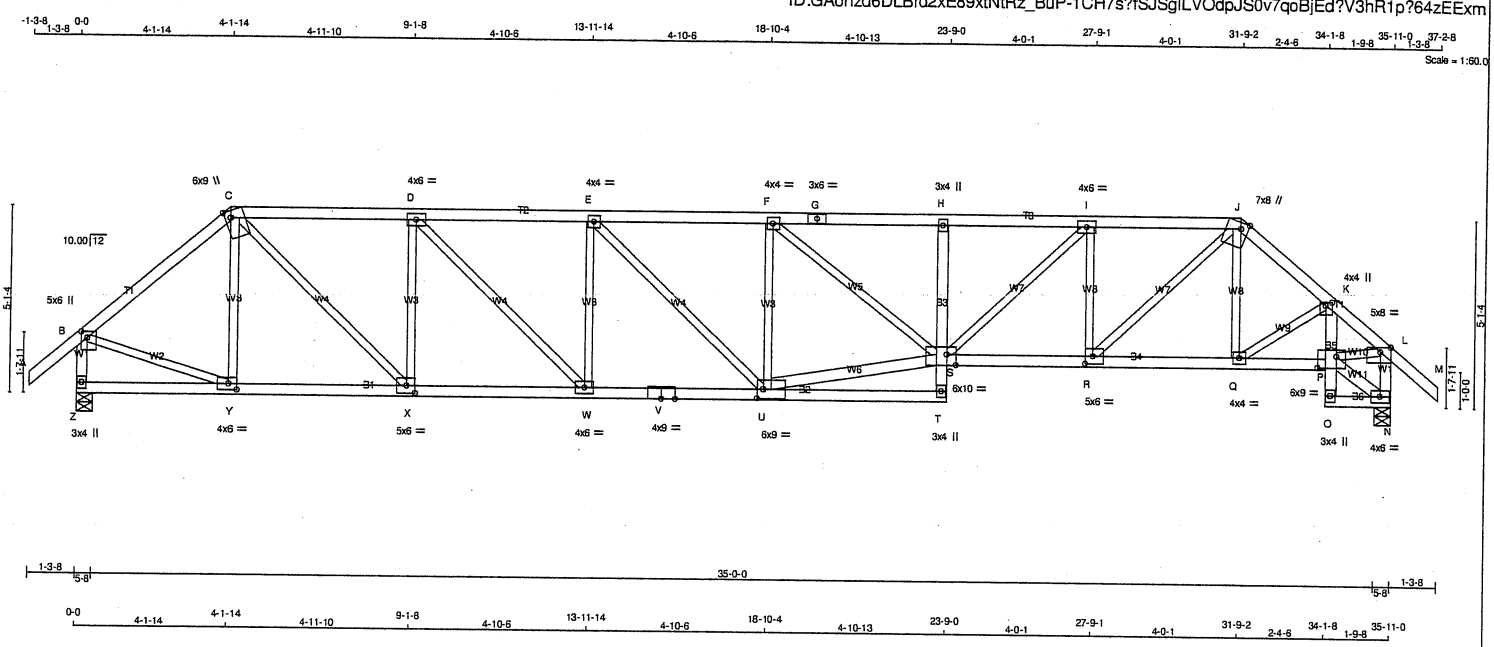
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.93 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T2S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Z - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Z - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - H	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	TMVW-p	MT20	5.0	8.0	1.50	3.50
N	BMVW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWW-l	MT20	6.0	9.0	3.75	6.00
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BVMWWW-l	MT20	6.0	10.0	Edge	3.00
T	BMV+p	MT20	3.0	4.0		
U	BMWWW-t	MT20	6.0	9.0	3.00	2.00
V	BS-t	MT20	4.0	9.0		
W	BMWW-t	MT20	4.0	6.0		



Structural component only
DWG# T-2115260

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	2105	0	2105	0	0	5-8	5-8
N	2109	0	2109	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	1486	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
N	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	-2003 / 0	-91.8	-91.8	0.36 (1)	4.42
C- D	-2846 / 0	-91.8	-91.8	0.48 (1)	3.70
D- E	-3542 / 0	-91.8	-91.8	0.56 (1)	3.29
E- F	-3718 / 0	-91.8	-91.8	0.52 (1)	3.27
F- G	-4185 / 0	-91.8	-91.8	0.58 (1)	3.04
G- H	-4185 / 0	-91.8	-91.8	0.58 (1)	3.04
H- I	-4215 / 0	-91.8	-91.8	0.47 (1)	3.11
I- J	-3326 / 0	-91.8	-91.8	0.37 (1)	3.57
J- K	-2493 / 0	-91.8	-91.8	0.18 (1)	4.22
K- L	-2540 / 0	-91.8	-91.8	0.19 (1)	4.17
L- M	0 / 41	-91.8	-91.8	0.13 (1)	10.00
Z- B	-2074 / 0	0.0	0.0	0.22 (1)	5.87
N- L	-2052 / 0	0.0	0.0	0.22 (1)	5.91
Z- Y	0 / 0	-18.5	-18.5	0.09 (4)	10.00
Y- X	0 / 1526	-18.5	-18.5	0.30 (1)	10.00
X- W	0 / 2847	-18.5	-18.5	0.50 (1)	10.00
W- V	0 / 3542	-18.5	-18.5	0.62 (1)	10.00
V- U	0 / 3542	-18.5	-18.5	0.62 (1)	10.00
U- T	0 / 115	-18.5	-18.5	0.11 (4)	10.00
T- S	0 / 40	0.0	0.0	0.16 (1)	10.00
S- H	-381 / 0	0.0	0.0	0.17 (1)	7.81
S- R	0 / 3326	-18.5	-18.5	0.58 (1)	10.00
R- Q	0 / 1908	-18.5	-18.5	0.35 (1)	10.00
Q- P	0 / 1972	-18.5	-18.5	0.36 (1)	10.00
O- P	0 / 17	0.0	0.0	0.10 (1)	10.00
P- K	-96 / 0	0.0	0.0	0.10 (1)	7.81
O- N	0 / 62	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-373 / 0	0.14 (1)
U- F	-828 / 0	0.32 (1)
U- S	0 / 3693	0.59 (1)
F- S	0 / 582	0.13 (1)
S- I	0 / 1222	0.28 (1)
R- I	-1235 / 0	0.32 (1)
R- J	0 / 1929	0.43 (1)
Q- J	0 / 52	0.02 (4)
Q- K	-73 / 0	0.01 (1)
B- Y	0 / 1605	0.36 (1)
P- N	-75 / 0	0.01 (1)
P- L	0 / 1933	0.43 (1)
E- U	0 / 251	0.06 (1)
C- X	0 / 1834	0.41 (1)
W- E	-603 / 0	0.23 (1)
X- D	-1185 / 0	0.46 (1)
D- W	0 / 978	0.22 (1)

TOTAL WEIGHT = 163 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 929 (0.46")

CSI: TC=0.58/1.00 (F-H:1) , BC=0.62/1.00 (U-W:1) , WB=0.59/1.00 (S-U:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (L) (INPUT = 0.90)
JSI METAL= 0.79 (V) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T2S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
X	BMWW-t	MT20	5.0	6.0	2.50	2.75
Y	BMWW-t	MT20	4.0	6.0	2.00	2.50
Z	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

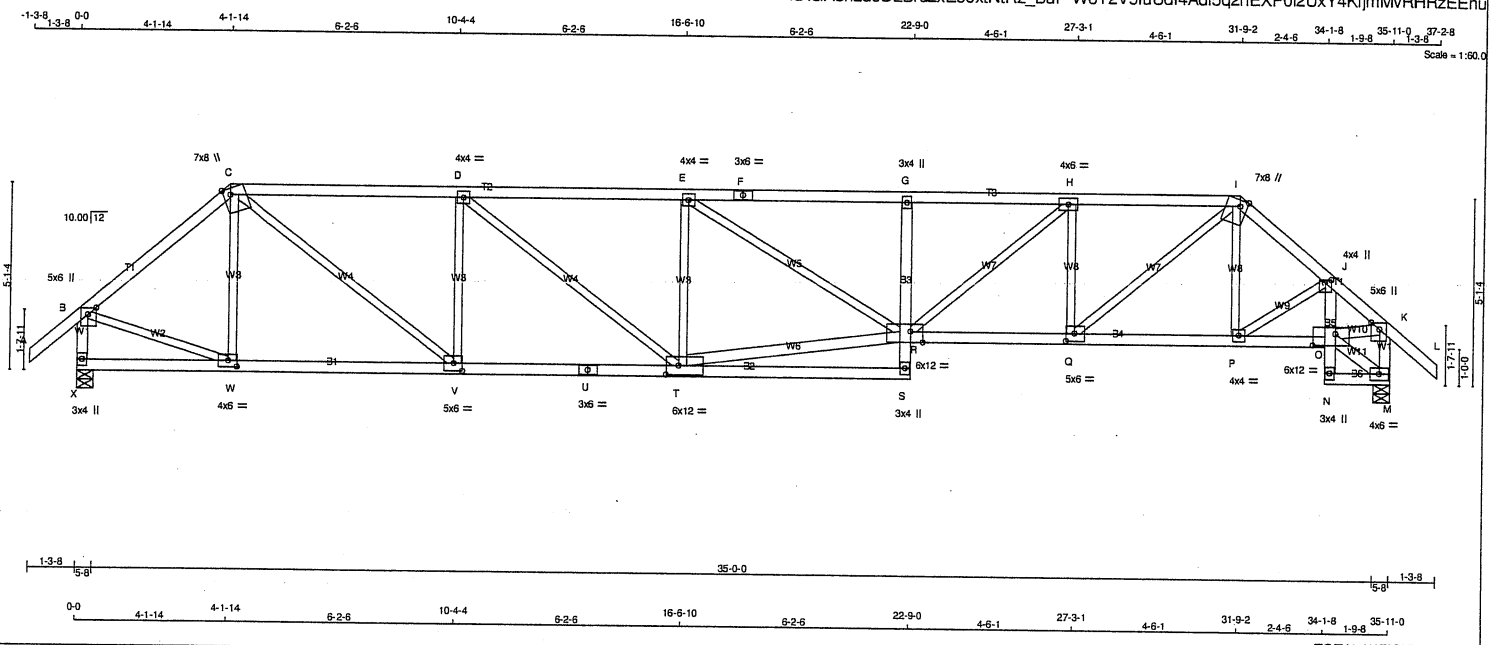
LICENSED PROFESSIONAL ENGINEER
05/22/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115260 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410966	T2S1	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 159 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
X - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW+p	MT20	5.0	6.0	Edge	2.75
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	3.75	7.50
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BVMWWW-l	MT20	6.0	12.0	Edge	4.25
S	BMV+p	MT20	3.0	4.0		
T	BMWWW-t	MT20	6.0	12.0	3.00	4.00
U	BS-t	MT20	3.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.75



Structural component only
DWG# T-2115281

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2105	0	2105	0	0	5-8	5-8
M	2109	0	2109	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1486	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	-2007 / 0	-91.8	-91.8	0.36 (1)	4.41
C- D	-3117 / 0	-91.8	-91.8	0.83 (1)	3.21
D- E	-3724 / 0	-91.8	-91.8	0.94 (1)	2.86
E- F	-4349 / 0	-91.8	-91.8	0.91 (1)	2.76
F- G	-4349 / 0	-91.8	-91.8	0.91 (1)	2.76
G- H	-4387 / 0	-91.8	-91.8	0.61 (1)	2.93
H- I	-3472 / 0	-91.8	-91.8	0.45 (1)	3.43
I- J	-2495 / 0	-91.8	-91.8	0.17 (1)	4.22
J- K	-2547 / 0	-91.8	-91.8	0.19 (1)	4.16
K- L	0 / 41	-91.8	-91.8	0.13 (1)	10.00
X- B	-2078 / 0	0.0	0.0	0.22 (1)	5.87
M- K	-2055 / 0	0.0	0.0	0.22 (1)	5.90
X- W	0 / 0	-18.5	-18.5	0.12 (4)	10.00
W- V	0 / 1530	-18.5	-18.5	0.32 (1)	10.00
V- U	0 / 3117	-18.5	-18.5	0.58 (1)	10.00
U- T	0 / 3117	-18.5	-18.5	0.58 (1)	10.00
T- S	0 / 143	-18.5	-18.5	0.17 (4)	10.00
S- R	0 / 51	0.0	0.0	0.20 (1)	10.00
R- G	-479 / 0	0.0	0.0	0.22 (1)	7.81
R- Q	0 / 3472	-18.5	-18.5	0.61 (1)	10.00
Q- P	0 / 1909	-18.5	-18.5	0.35 (1)	10.00
P- O	0 / 1974	-18.5	-18.5	0.36 (1)	10.00
N- O	0 / 17	0.0	0.0	0.09 (1)	10.00
O- J	-95 / 0	0.0	0.0	0.09 (1)	7.81
N- M	0 / 56	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-359 / 0	0.14 (1)
C- V	0 / 2000	0.45 (1)
V- D	-1110 / 0	0.43 (1)
D- T	0 / 775	0.17 (1)
T- E	-943 / 0	0.36 (1)
T- R	0 / 3646	0.59 (1)
E- R	0 / 719	0.16 (1)
R- H	0 / 1194	0.27 (1)
Q- H	-1201 / 0	0.31 (1)
Q- I	0 / 2023	0.46 (1)
P- I	0 / 57	0.02 (4)
P- J	-74 / 0	0.01 (1)
B- W	0 / 1608	0.36 (1)
O- M	-68 / 0	0.01 (1)
O- K	0 / 1939	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 840 (0.51")

CSI: TC=0.94/1.00 (D-E:1) , BC=0.61/1.00 (Q-R:1) , WB=0.59/1.00 (R-T:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.96 (U) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410966	T2S1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMWW-t	MT20	4.0	6.0	2.00	2.75
X	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

LICENSED PROFESSIONAL ENGINEER
05/20/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

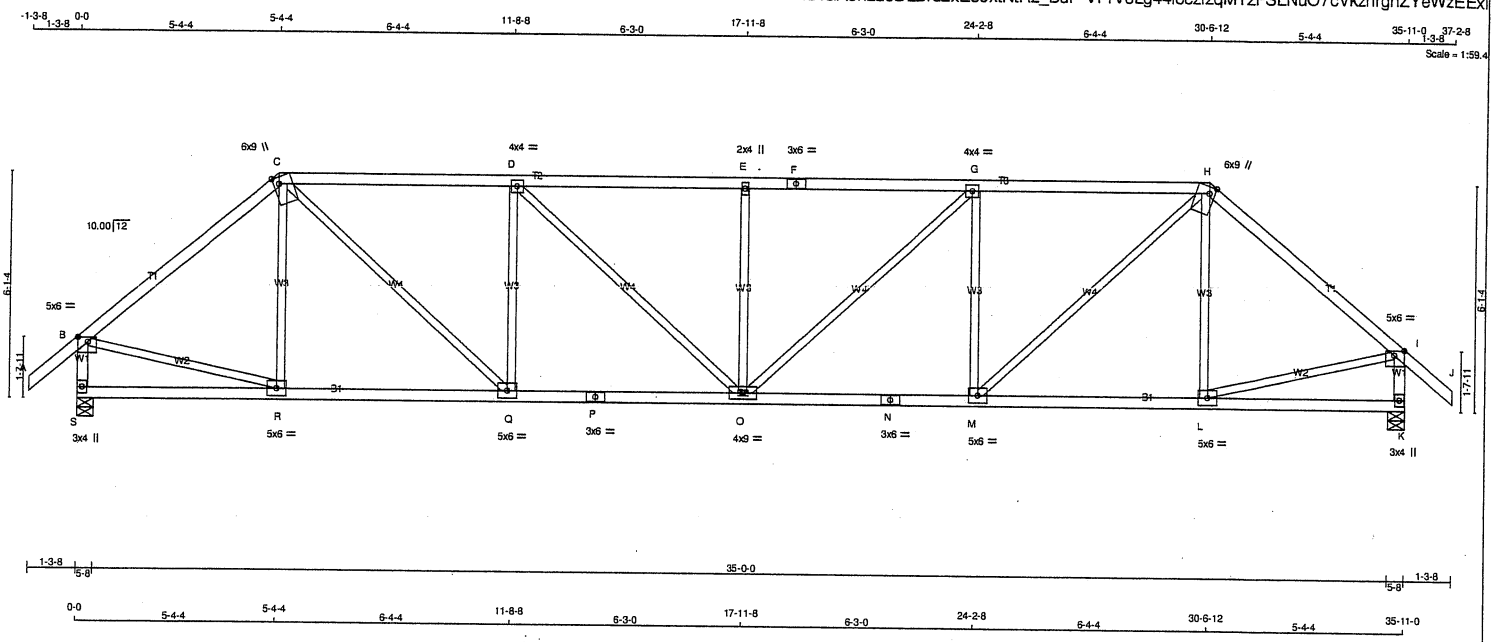
Structural component only
DWG# T-2115281

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410965	T3	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-p	MT20	5.0	6.0	1.50	3.25
K	BMV1+p	MT20	3.0	4.0		
L, M, Q, R						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115261

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2107	0	2107	0	0	5-8	5-8
K	2107	0	2107	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
K	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	R- C	-254 / 6	0.15 (1)
B- C	-2054 / 0	-91.8	-91.8	0.65 (1)	4.06	C- Q	0 / 1612	0.36 (1)
C- D	-2762 / 0	-91.8	-91.8	0.81 (1)	3.36	Q- D	-972 / 0	0.57 (1)
D- E	-3108 / 0	-91.8	-91.8	0.87 (1)	3.14	D- O	0 / 472	0.11 (1)
E- F	-3108 / 0	-91.8	-91.8	0.87 (1)	3.14	O- E	-529 / 0	0.31 (1)
F- G	-3108 / 0	-91.8	-91.8	0.87 (1)	3.14	O- G	0 / 472	0.11 (1)
G- H	-2762 / 0	-91.8	-91.8	0.81 (1)	3.36	M- G	-972 / 0	0.57 (1)
H- I	-2054 / 0	-91.8	-91.8	0.65 (1)	4.06	M- H	0 / 1612	0.36 (1)
I- J	0 / 41	-91.8	-91.8	0.13 (1)	10.00	L- H	-254 / 6	0.15 (1)
S- B	-2068 / 0	0.0	0.0	0.22 (1)	5.88	B- R	0 / 1619	0.36 (1)
K- I	-2068 / 0	0.0	0.0	0.22 (1)	5.88	L- I	0 / 1619	0.36 (1)
S- R	0 / 0	-18.5	-18.5	0.15 (4)	10.00			
R- Q	0 / 1570	-18.5	-18.5	0.34 (1)	10.00			
Q- P	0 / 2762	-18.5	-18.5	0.51 (1)	10.00			
P- O	0 / 2762	-18.5	-18.5	0.51 (1)	10.00			
O- N	0 / 2762	-18.5	-18.5	0.51 (1)	10.00			
N- M	0 / 2762	-18.5	-18.5	0.51 (1)	10.00			
M- L	0 / 1570	-18.5	-18.5	0.34 (1)	10.00			
L- K	0 / 0	-18.5	-18.5	0.15 (4)	10.00			

TOTAL WEIGHT = 150 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.87/1.00 (E-G:1) , BC=0.51/1.00 (M-O:1) , WB=0.57/1.00 (D-Q:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

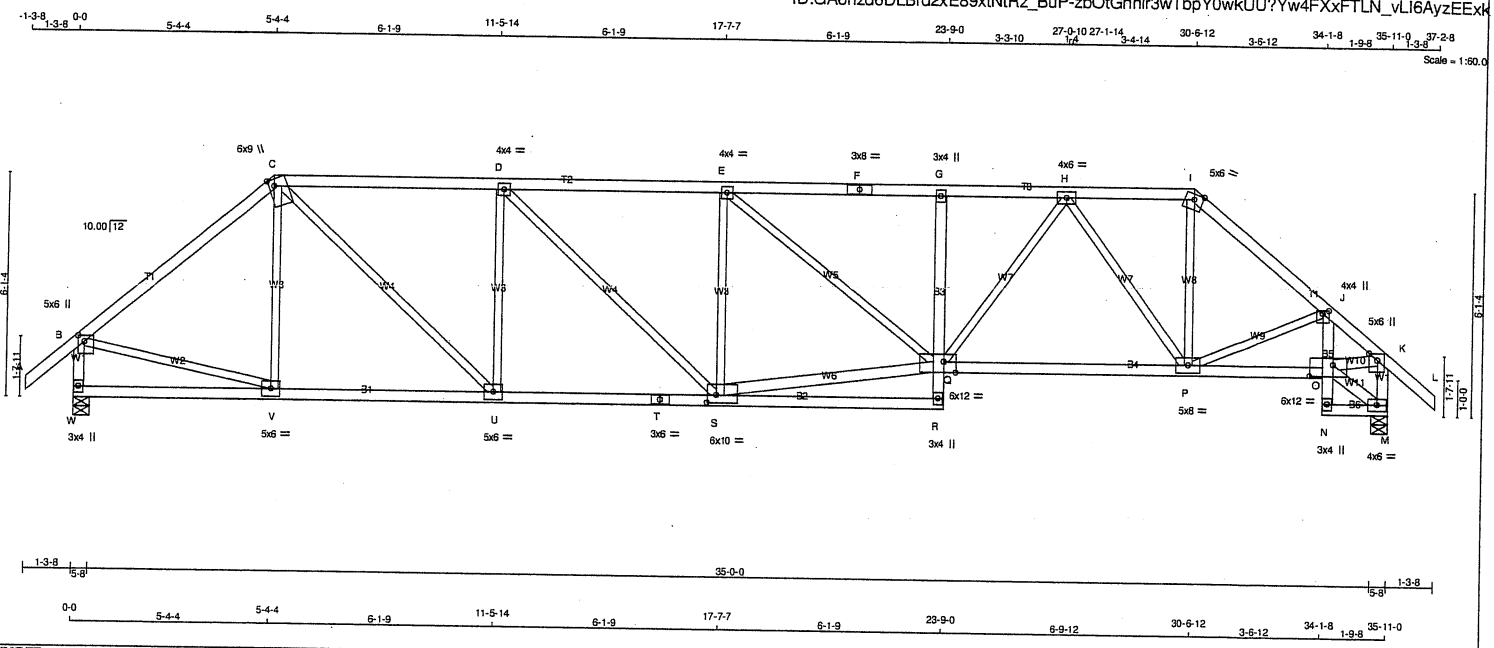
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.87 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410965	T3S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 164 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMV+p	MT20	3.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTW-m	MT20	5.0	6.0	Edge	
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW+p	MT20	5.0	6.0	Edge	2.75
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-I	MT20	6.0	12.0	3.75	7.50
P	BMWWW-t	MT20	5.0	8.0		
Q	BVMWWW-I	MT20	6.0	12.0	Edge	4.00
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	10.0	2.50	3.00
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0		



Structural component only
DWG# T-2115262

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2105	0	2105	0	0	5-8	5-8
M	2109	0	2109	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1486	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	-2051 / 0	-91.8	-91.8	0.64 (1)	4.06
C- D	-2730 / 0	-91.8	-91.8	0.74 (1)	3.47
D- E	-3103 / 0	-91.8	-91.8	0.79 (1)	3.23
E- F	-3337 / 0	-91.8	-91.8	0.69 (1)	3.29
F- G	-3337 / 0	-91.8	-91.8	0.69 (1)	3.29
G- H	-3356 / 0	-91.8	-91.8	0.42 (1)	3.46
H- I	-1910 / 0	-91.8	-91.8	0.20 (1)	4.69
I- J	-2468 / 0	-91.8	-91.8	0.28 (1)	4.15
J- K	-2575 / 0	-91.8	-91.8	0.19 (1)	4.14
K- L	0 / 41	-91.8	-91.8	0.13 (1)	10.00
W- B	-2066 / 0	0.0	0.0	0.22 (1)	5.89
M- K	-2055 / 0	0.0	0.0	0.22 (1)	5.90
W- V	0 / 0	-18.5	-18.5	0.15 (4)	10.00
V- U	0 / 1568	-18.5	-18.5	0.34 (1)	10.00
U- T	0 / 2730	-18.5	-18.5	0.52 (1)	10.00
T- S	0 / 2730	-18.5	-18.5	0.52 (1)	10.00
S- R	0 / 90	-18.5	-18.5	0.17 (4)	10.00
R- Q	0 / 50	0.0	0.0	0.13 (1)	10.00
Q- G	-452 / 0	0.0	0.0	0.18 (1)	7.81
Q- P	0 / 2734	-18.5	-18.5	0.54 (1)	10.00
P- O	0 / 2013	-18.5	-18.5	0.42 (1)	10.00
N- O	0 / 17	0.0	0.0	0.09 (1)	10.00
O- J	-127 / 0	0.0	0.0	0.09 (1)	7.81
N- M	0 / 56	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-254 / 6	0.15 (1)
C- U	0 / 1592	0.36 (1)
U- D	-983 / 0	0.58 (1)
D- S	0 / 519	0.12 (1)
S- E	-735 / 0	0.43 (1)
S- Q	0 / 3064	0.49 (1)
E- Q	0 / 288	0.06 (1)
Q- H	0 / 1068	0.24 (1)
H- P	-1425 / 0	0.86 (1)
P- I	0 / 1274	0.29 (1)
P- J	-142 / 0	0.04 (1)
B- V	0 / 1617	0.36 (1)
O- M	-69 / 0	0.01 (1)
O- K	0 / 1980	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.79/1.00 (D-E:1) , BC=0.54/1.00 (P-Q:1) , WB=0.86/1.00 (H-P:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.79 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T3S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

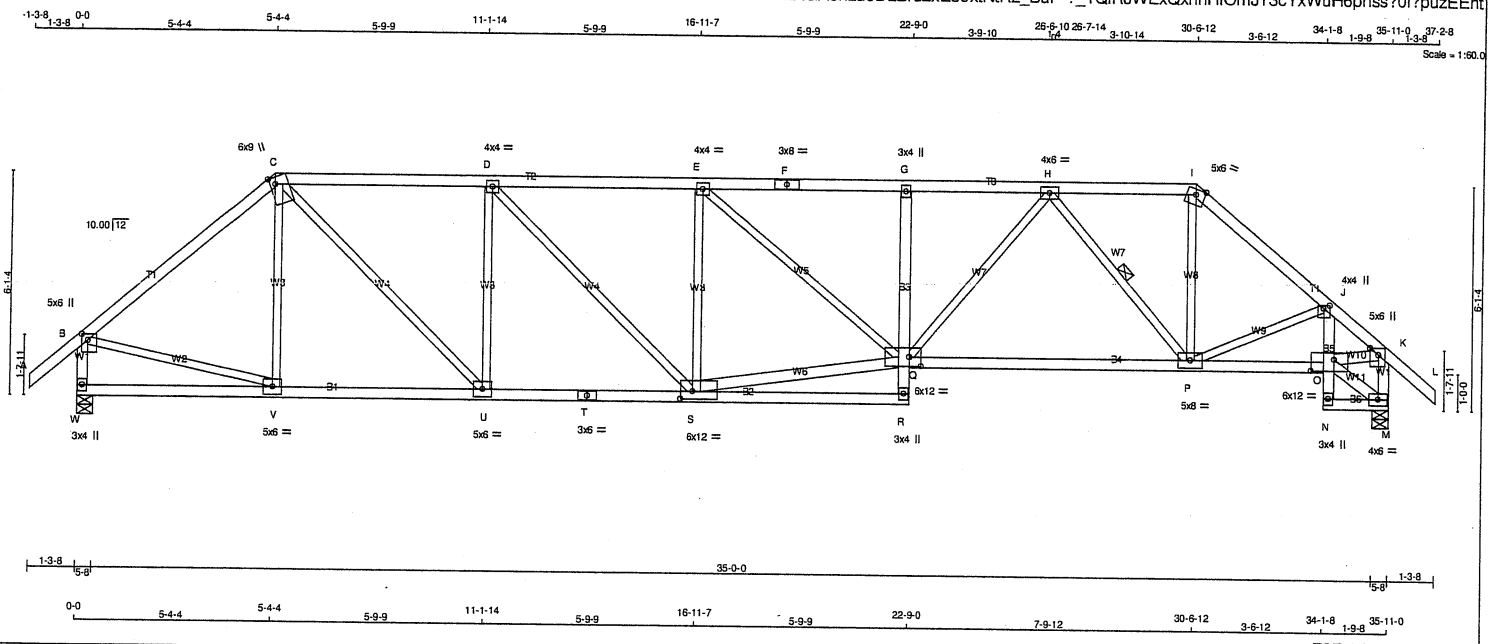
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115262

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410966	T3S1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 164 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMV+p	MT20	3.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTW-m	MT20	5.0	6.0	Edge	
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW+p	MT20	5.0	6.0	Edge	2.75
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-I	MT20	6.0	12.0	3.75	7.50
P	BMWWW-t	MT20	5.0	8.0		
Q	BVMWWW-I	MT20	6.0	12.0	3.00	4.00
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	12.0	2.50	4.00
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0		



Structural component only
DWG# T-2115282

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2105	0	2105	0	0	5-8	5-8
M	2109	0	2109	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1486	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 41	-91.8	-91.8	0.13 (1)		10.00
B- C		-2050 / 0	-91.8	-91.8	0.64 (1)		4.06
C- D		-2681 / 0	-91.8	-91.8	0.65 (1)		3.61
D- E		-3087 / 0	-91.8	-91.8	0.70 (1)		3.34
E- F		-3452 / 0	-91.8	-91.8	0.62 (1)		3.30
F- G		-3452 / 0	-91.8	-91.8	0.62 (1)		3.30
G- H		-3470 / 0	-91.8	-91.8	0.42 (1)		3.43
H- I		-1915 / 0	-91.8	-91.8	0.24 (1)		4.64
I- J		-2475 / 0	-91.8	-91.8	0.28 (1)		4.14
J- K		-2575 / 0	-91.8	-91.8	0.19 (1)		4.14
K- L		0 / 41	-91.8	-91.8	0.13 (1)		10.00
W- B		-2065 / 0	0.0	0.0	0.22 (1)		5.89
M- K		-2055 / 0	0.0	0.0	0.22 (1)		5.90
W- V		0 / 0	-18.5	-18.5	0.14 (4)		10.00
V- U		0 / 1567	-18.5	-18.5	0.33 (1)		10.00
U- T		0 / 2681	-18.5	-18.5	0.50 (1)		10.00
T- S		0 / 2681	-18.5	-18.5	0.50 (1)		10.00
S- R		0 / 88	-18.5	-18.5	0.15 (4)		10.00
R- Q		0 / 47	0.0	0.0	0.13 (1)		10.00
Q- G		-440 / 0	0.0	0.0	0.17 (1)		7.81
Q- P		0 / 2840	-18.5	-18.5	0.59 (1)		10.00
P- O		0 / 2013	-18.5	-18.5	0.46 (1)		10.00
N- O		0 / 17	0.0	0.0	0.09 (1)		10.00
O- J		-134 / 0	0.0	0.0	0.09 (1)		7.81
N- M		0 / 56	-18.5	-18.5	0.02 (4)		10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C		-257 / 2	0.15 (1)
C- U		0 / 1567	0.35 (1)
U- D		-1004 / 0	0.59 (1)
D- S		0 / 582	0.13 (1)
S- E		-824 / 0	0.48 (1)
S- Q		0 / 3055	0.49 (1)
E- Q		0 / 465	0.10 (1)
Q- H		0 / 993	0.22 (1)
H- P		-1468 / 0	0.40 (1)
P- I		0 / 1266	0.28 (1)
P- J		-136 / 0	0.04 (1)
B- V		0 / 1616	0.36 (1)
O- M		-69 / 0	0.01 (1)
O- K		0 / 1980	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.59/1.00 (P-Q:1) , WB=0.59/1.00 (D-U:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.83 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410966	T3S1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

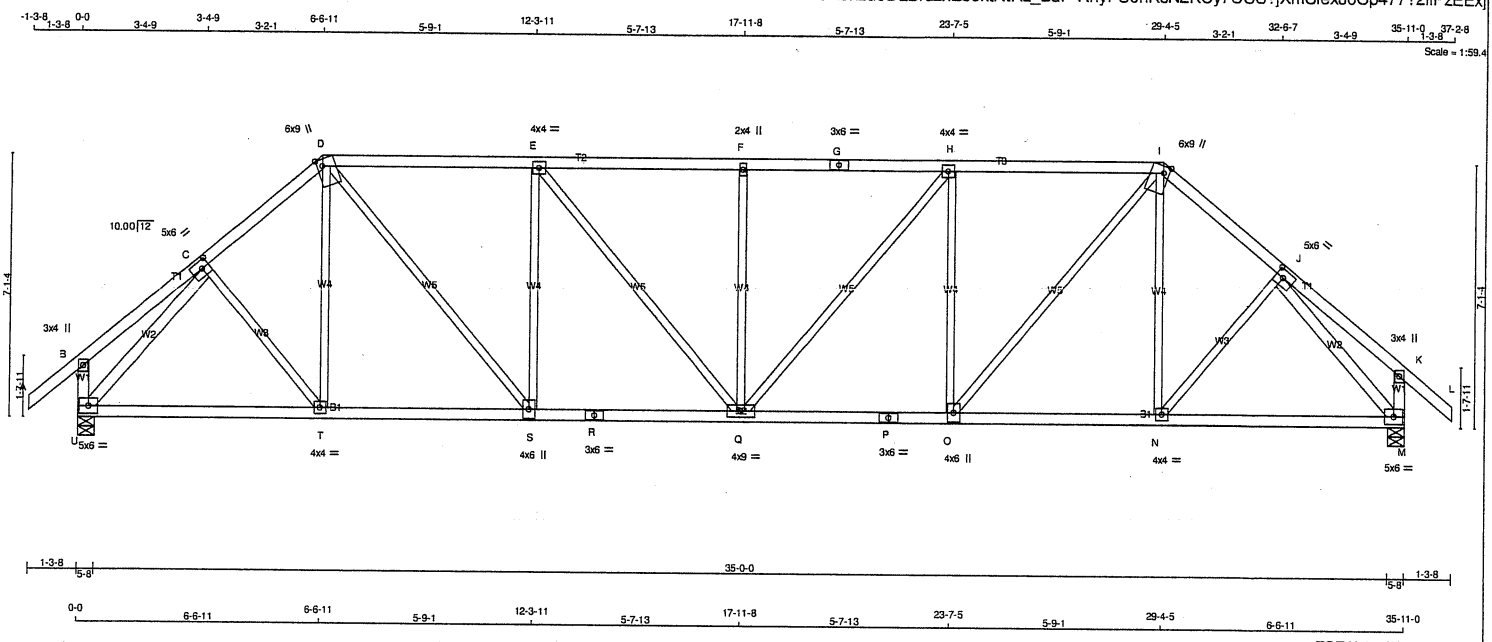
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115282

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410965	T4	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 15:57:04 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-RnyFU0hKcN2KCy7CUS?jXmSlexJ6Cp477?2fiPzEExj



TOTAL WEIGHT = 167 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

1)



Structural component only
DWG# T-2115263

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2107	0	2107	0	0	5-8	5-8
M	2107	0	2107	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	C- T	0 / 166	0.04 (1)
B- C	0 / 20	-91.8	-91.8	0.14 (1)	10.00	T- D	0 / 75	0.03 (4)
C- D	-2076 / 0	-91.8	-91.8	0.20 (1)	4.53	D- S	0 / 1280	0.29 (1)
D- E	-2402 / 0	-91.8	-91.8	0.60 (1)	3.81	S- E	-880 / 0	0.77 (1)
E- F	-2644 / 0	-91.8	-91.8	0.63 (1)	3.63	E- Q	0 / 378	0.09 (1)
F- G	-2644 / 0	-91.8	-91.8	0.63 (1)	3.63	Q- F	-477 / 0	0.42 (1)
G- H	-2644 / 0	-91.8	-91.8	0.63 (1)	3.63	Q- H	0 / 378	0.09 (1)
H- I	-2402 / 0	-91.8	-91.8	0.60 (1)	3.81	O- H	-880 / 0	0.77 (1)
I- J	-2076 / 0	-91.8	-91.8	0.20 (1)	4.53	O- I	0 / 1280	0.29 (1)
J- K	0 / 20	-91.8	-91.8	0.14 (1)	10.00	N- I	0 / 75	0.03 (4)
K- L	0 / 41	-91.8	-91.8	0.13 (1)	10.00	N- J	0 / 166	0.04 (1)
U- B	-244 / 0	0.0	0.0	0.03 (1)	7.81	U- C	-2332 / 0	0.72 (1)
M- K	-244 / 0	0.0	0.0	0.03 (1)	7.81	J- M	-2332 / 0	0.72 (1)
U- T	0 / 1467	-18.5	-18.5	0.34 (1)	10.00			
T- S	0 / 1575	-18.5	-18.5	0.36 (1)	10.00			
S- R	0 / 2402	-18.5	-18.5	0.44 (1)	10.00			
R- Q	0 / 2402	-18.5	-18.5	0.44 (1)	10.00			
Q- P	0 / 2402	-18.5	-18.5	0.44 (1)	10.00			
P- O	0 / 2402	-18.5	-18.5	0.44 (1)	10.00			
O- N	0 / 1575	-18.5	-18.5	0.36 (1)	10.00			
N- M	0 / 1467	-18.5	-18.5	0.34 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.63/1.00 (F-H:1) , BC=0.44/1.00 (O-Q:1) , WB=0.77/1.00 (E-S:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

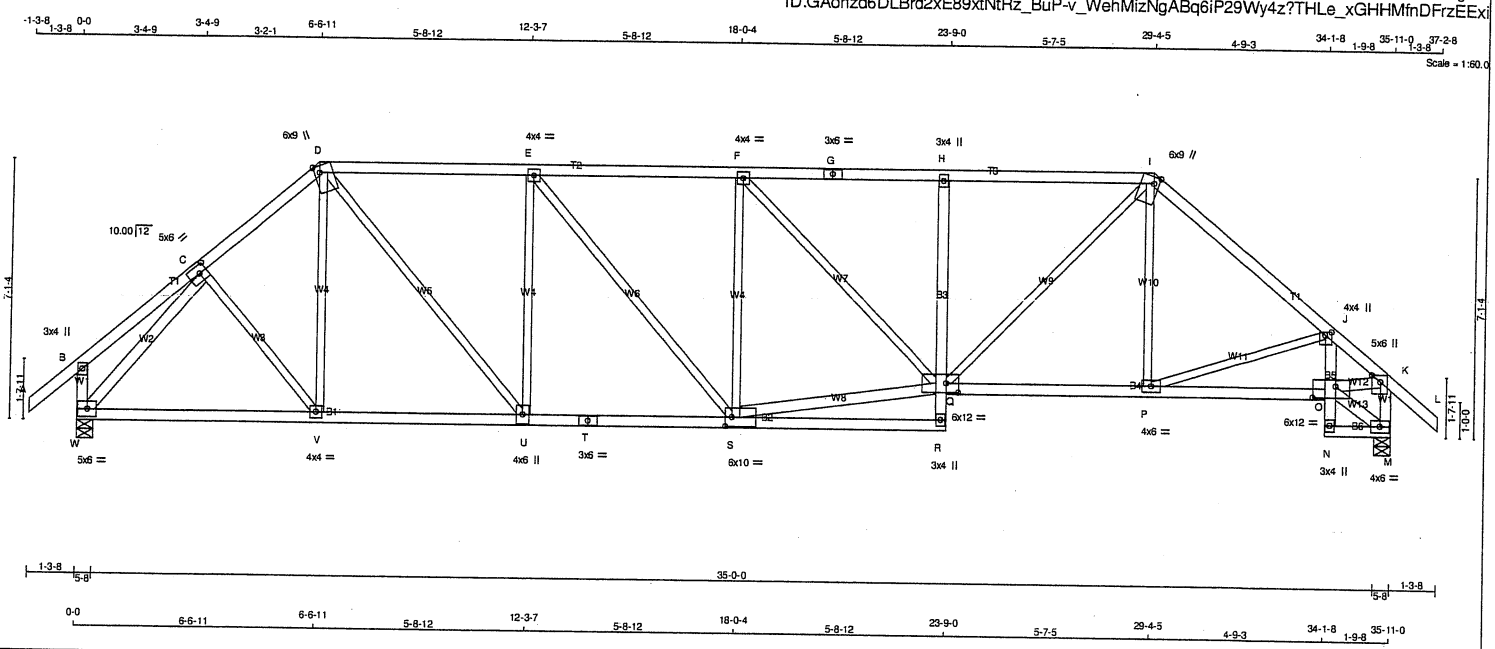
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.74 (R) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T4S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 15:57:05 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-v_WehMizNgABq6iP29Wy4z?THLe_xGHHMfnDFrzEExi



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
W - C	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW+p	MT20	5.0	6.0	Edge	2.75
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	3.75	7.50
P	BMWW-t	MT20	4.0	6.0		
Q	BVMWWW-l	MT20	6.0	12.0	3.00	4.00
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	10.0	3.00	2.25
T	BS-t	MT20	3.0	6.0		
U	BMWW+t	MT20	4.0	6.0		



Structural component only
DWG# T-2115264 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2105	0	2105	0	0	5-8	5-8
M	2109	0	2109	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1486	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	0 / 20	-91.8	-91.8	0.14 (1)	10.00
C- D	-2074 / 0	-91.8	-91.8	0.20 (1)	4.53
D- E	-2395 / 0	-91.8	-91.8	0.60 (1)	3.81
E- F	-2639 / 0	-91.8	-91.8	0.63 (1)	3.63
F- G	-2787 / 0	-91.8	-91.8	0.62 (1)	3.56
G- H	-2787 / 0	-91.8	-91.8	0.62 (1)	3.56
H- I	-2798 / 0	-91.8	-91.8	0.63 (1)	3.56
I- J	-2409 / 0	-91.8	-91.8	0.44 (1)	4.06
J- K	-2622 / 0	-91.8	-91.8	0.30 (1)	3.97
K- L	0 / 41	-91.8	-91.8	0.13 (1)	10.00
W- B	-244 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-2054 / 0	0.0	0.0	0.22 (1)	5.90
W- V	0 / 1465	-18.5	-18.5	0.34 (1)	10.00
V- U	0 / 1573	-18.5	-18.5	0.36 (1)	10.00
U- T	0 / 2395	-18.5	-18.5	0.46 (1)	10.00
T- S	0 / 2395	-18.5	-18.5	0.46 (1)	10.00
S- R	0 / 60	-18.5	-18.5	0.15 (4)	10.00
R- Q	0 / 46	0.0	0.0	0.09 (1)	10.00
Q- H	-578 / 0	0.0	0.0	0.31 (1)	7.81
Q- P	0 / 1832	-18.5	-18.5	0.38 (1)	10.00
P- O	0 / 2081	-18.5	-18.5	0.42 (1)	10.00
N- O	0 / 17	0.0	0.0	0.09 (1)	10.00
O- J	-133 / 0	0.0	0.0	0.09 (1)	7.81
N- M	0 / 58	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 166	0.04 (1)
V- D	0 / 76	0.03 (4)
D- U	0 / 1276	0.29 (1)
U- E	-883 / 0	0.77 (1)
E- S	0 / 381	0.09 (1)
S- F	-631 / 0	0.55 (1)
S- Q	0 / 2625	0.42 (1)
F- Q	0 / 205	0.05 (1)
Q- I	0 / 1369	0.31 (1)
P- I	0 / 147	0.05 (4)
P- J	-265 / 0	0.12 (1)
W- C	-2329 / 0	0.71 (1)
O- M	-70 / 0	0.01 (1)
O- K	0 / 2048	0.33 (1)

TOTAL WEIGHT = 174 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.63/1.00 (E-F:1) , BC=0.46/1.00 (S-U:1) , WB=0.77/1.00 (E-U:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.73 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T4S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

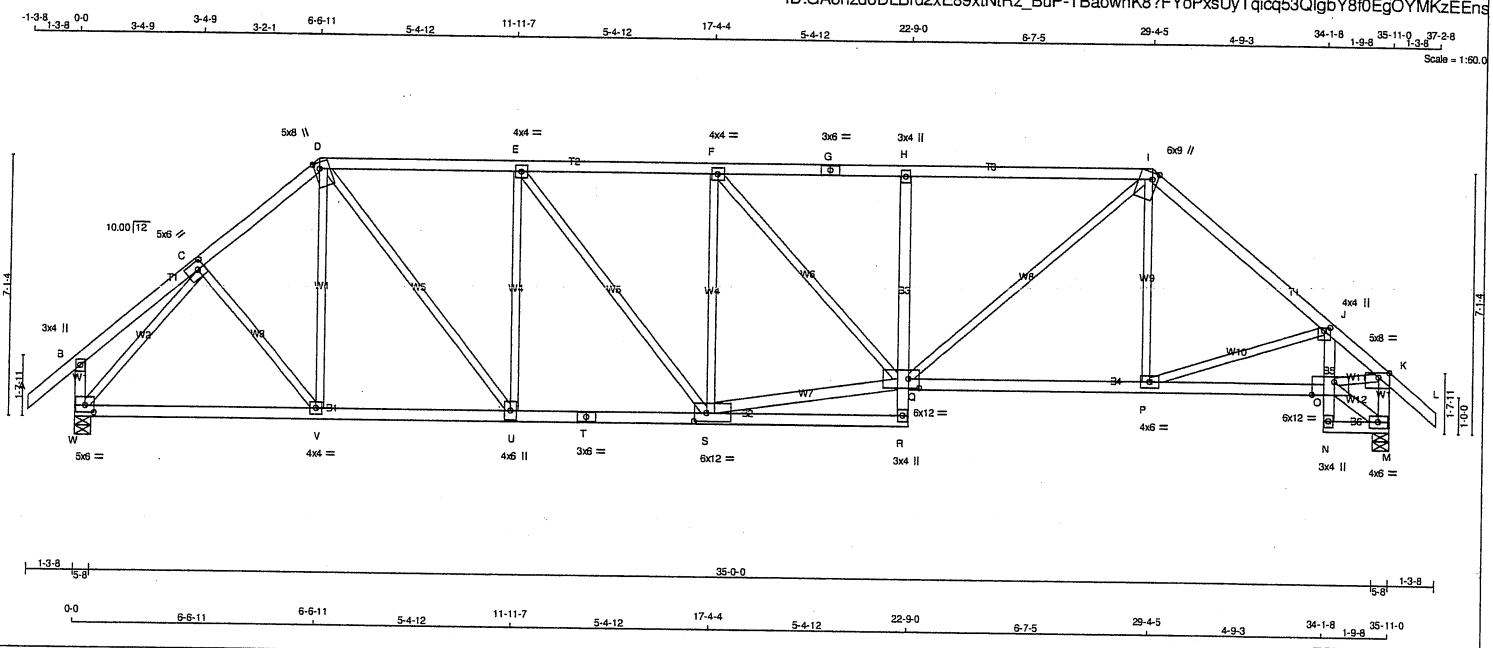
LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115264

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410966	T4S1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 16:07:35 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-TBaownK8?FYoPxsUyTqicq53QlgbY8f0EgOYMKzEEns



TOTAL WEIGHT = 171 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	5.0	8.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW-p	MT20	5.0	8.0	1.50	3.50
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	4.25	7.25
P	BMWW-t	MT20	4.0	6.0		
Q	BVMWWW-l	MT20	6.0	12.0	3.00	3.75
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	12.0	2.75	4.00
T	BS-t	MT20	3.0	6.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1-t	MT20	5.0	6.0	2.25	3.00



Structural component only
DWG# T-2115283

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2105	0	2105	0	0	5-8	5-8
M	2109	0	2109	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1486	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	0 / 19	-91.8	-91.8	0.14 (1)	10.00
C- D	-2072 / 0	-91.8	-91.8	0.20 (1)	4.54
D- E	-2364 / 0	-91.8	-91.8	0.54 (1)	3.91
E- F	-2626 / 0	-91.8	-91.8	0.57 (1)	3.71
F- G	-2899 / 0	-91.8	-91.8	0.74 (1)	3.27
G- H	-2899 / 0	-91.8	-91.8	0.74 (1)	3.27
H- I	-2909 / 0	-91.8	-91.8	0.88 (1)	3.26
I- J	-2412 / 0	-91.8	-91.8	0.45 (1)	4.06
J- K	-2616 / 0	-91.8	-91.8	0.30 (1)	3.97
K- L	0 / 41	-91.8	-91.8	0.13 (1)	10.00
W- B	-246 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-2050 / 0	0.0	0.0	0.22 (1)	5.91
W- V	0 / 1464	-18.5	-18.5	0.34 (1)	10.00
V- U	0 / 1572	-18.5	-18.5	0.35 (1)	10.00
U- T	0 / 2364	-18.5	-18.5	0.45 (1)	10.00
T- S	0 / 2364	-18.5	-18.5	0.45 (1)	10.00
S- R	0 / 60	-18.5	-18.5	0.13 (4)	10.00
R- Q	0 / 44	0.0	0.0	0.09 (1)	10.00
Q- H	-642 / 0	0.0	0.0	0.37 (1)	7.81
Q- P	0 / 1835	-18.5	-18.5	0.40 (1)	10.00
P- O	0 / 2080	-18.5	-18.5	0.44 (1)	10.00
N- O	0 / 17	0.0	0.0	0.10 (1)	10.00
O- J	-140 / 0	0.0	0.0	0.10 (1)	7.81
N- M	0 / 63	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 167	0.04 (1)
V- D	0 / 74	0.03 (4)
D- U	0 / 1266	0.28 (1)
U- E	-902 / 0	0.79 (1)
E- S	0 / 428	0.10 (1)
S- F	-710 / 0	0.62 (1)
S- Q	0 / 2619	0.42 (1)
F- Q	0 / 397	0.09 (1)
Q- I	0 / 1415	0.32 (1)
P- I	0 / 160	0.05 (4)
P- J	-260 / 0	0.12 (1)
W- C	-2326 / 0	1.00 (1)
O- M	-77 / 0	0.01 (1)
O- K	0 / 2042	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.88/1.00 (H-I:1) , BC=0.45/1.00 (S-U:1) , WB=1.00/1.00 (C-W:1) , SSI=0.29/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

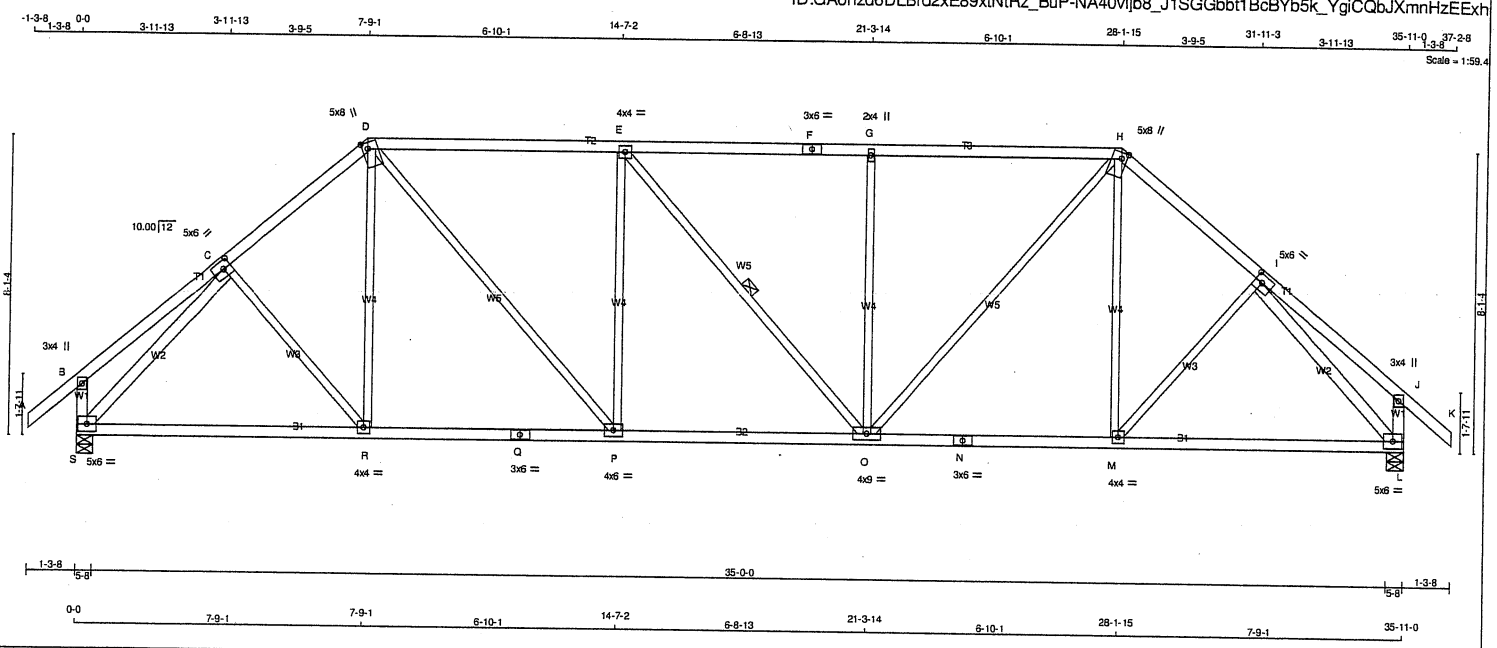
JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 0.73 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-NA40vijb8_J1SGGbbt1BcBYb5k_YgiCQbJXmnHzEExh



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	2100F 1.8E	SPF
I - L	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	8.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	8.0	2.00	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115265

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2107	0	2107	0	0	5-8	5-8
L	2107	0	2107	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	C- R	0 / 67	0.02 (4)
B- C	0 / 26	-91.8	-91.8	0.22 (1)	10.00	R- D	0 / 125	0.04 (4)
C- D	-2060 / 0	-91.8	-91.8	0.30 (1)	4.45	D- P	0 / 1055	0.24 (1)
D- E	-2256 / 0	-91.8	-91.8	0.82 (1)	3.63	P- E	-675 / 0	0.86 (1)
E- F	-2255 / 0	-91.8	-91.8	0.81 (1)	3.63	E- O	-2 / 0	0.00 (1)
F- G	-2255 / 0	-91.8	-91.8	0.81 (1)	3.63	O- G	-675 / 0	0.85 (1)
G- H	-2255 / 0	-91.8	-91.8	0.81 (1)	3.64	O- H	0 / 1053	0.24 (1)
H- I	-2060 / 0	-91.8	-91.8	0.30 (1)	4.45	M- H	0 / 125	0.04 (4)
I- J	0 / 26	-91.8	-91.8	0.22 (1)	10.00	M- I	0 / 67	0.02 (4)
J- K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	S- C	-2346 / 0	0.62 (1)
S- B	-263 / 0	0.0	0.0	0.03 (1)	7.81	I- L	-2346 / 0	0.62 (1)
L- J	-263 / 0	0.0	0.0	0.03 (1)	7.81			
S- R	0 / 1522	-18.5	-18.5	0.39 (1)	10.00			
R- Q	0 / 1560	-18.5	-18.5	0.40 (1)	10.00			
Q- P	0 / 1560	-18.5	-18.5	0.40 (1)	10.00			
P- O	0 / 2256	-18.5	-18.5	0.44 (1)	10.00			
O- N	0 / 1560	-18.5	-18.5	0.40 (1)	10.00			
N- M	0 / 1560	-18.5	-18.5	0.40 (1)	10.00			
M- L	0 / 1522	-18.5	-18.5	0.39 (1)	10.00			

TOTAL WEIGHT = 165 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.82/1.00 (D-E:1) , BC=0.44/1.00 (O-P:1) , WB=0.86/1.00 (E-P:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

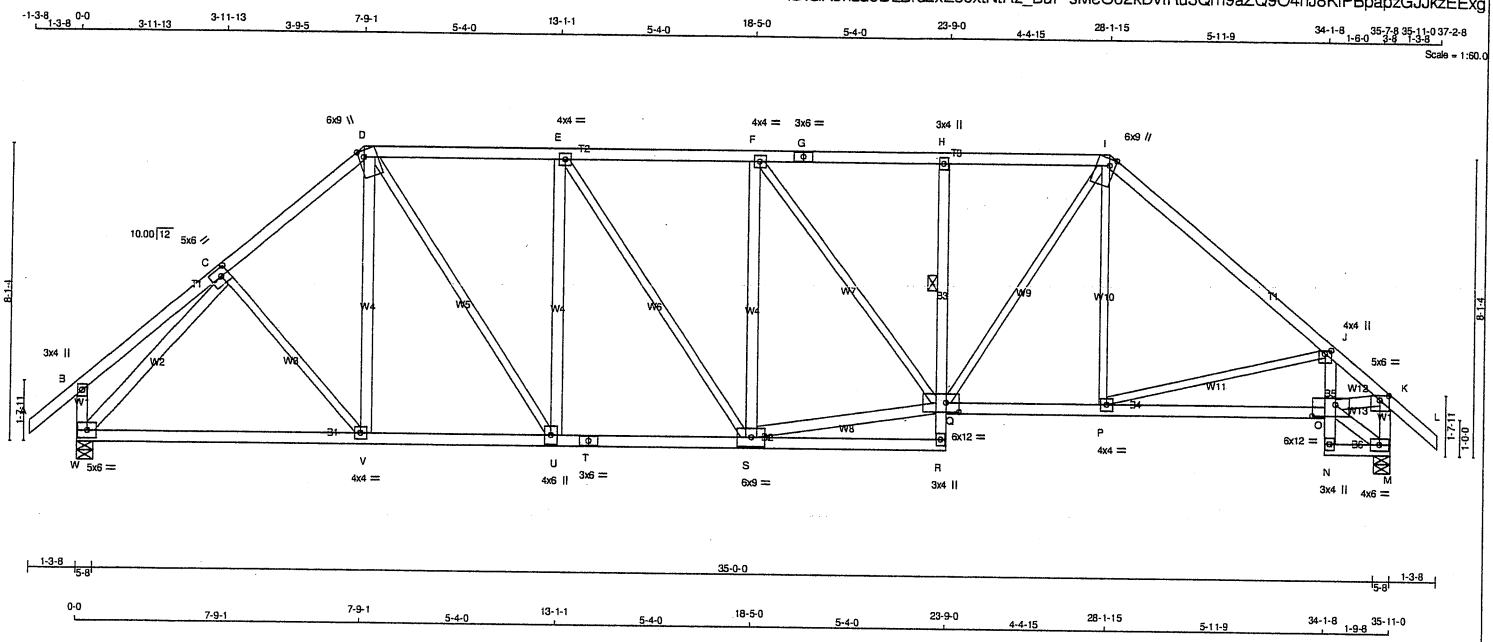
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.53 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T5S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - V	2x3	DRY	No.2	SPF
D - U	2x3	DRY	No.2	SPF
E - S	2x3	DRY	No.2	SPF
F - Q	2x3	DRY	No.2	SPF
Q - I	2x3	DRY	No.2	SPF
P - I	2x3	DRY	No.2	SPF
P - J	2x3	DRY	No.2	SPF
W - C	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	3.75	7.50
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWWW-l	MT20	6.0	12.0	3.00	4.25



Structural component only
DWG# T-2115266

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2105	0	2105	0	0	5-8	5-8
M	2109	0	2109	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1486	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	0 / 26	-91.8	-91.8	0.22 (1)	10.00
C- D	-2056 / 0	-91.8	-91.8	0.30 (1)	4.45
D- E	-2140 / 0	-91.8	-91.8	0.48 (1)	4.14
E- F	-2298 / 0	-91.8	-91.8	0.50 (1)	4.00
F- G	-2368 / 0	-91.8	-91.8	0.41 (1)	4.08
G- H	-2368 / 0	-91.8	-91.8	0.41 (1)	4.08
H- I	-2373 / 0	-91.8	-91.8	0.39 (1)	4.08
I- J	-2341 / 0	-91.8	-91.8	0.73 (1)	3.90
J- K	-2681 / 0	-91.8	-91.8	0.49 (1)	3.64
K- L	0 / 41	-91.8	-91.8	0.13 (1)	10.00
W- B	-263 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-2053 / 0	0.0	0.0	0.22 (1)	5.91
W- V	0 / 1519	-18.5	-18.5	0.39 (1)	10.00
V- U	0 / 1558	-18.5	-18.5	0.39 (1)	10.00
U- T	0 / 2140	-18.5	-18.5	0.41 (1)	10.00
T- S	0 / 2140	-18.5	-18.5	0.41 (1)	10.00
S- R	0 / 42	-18.5	-18.5	0.14 (4)	10.00
R- Q	0 / 42	0.0	0.0	0.06 (1)	10.00
Q- H	-484 / 0	0.0	0.0	0.07 (1)	6.25
Q- P	0 / 1774	-18.5	-18.5	0.38 (1)	10.00
P- O	0 / 2166	-18.5	-18.5	0.44 (1)	10.00
N- O	0 / 17	0.0	0.0	0.09 (1)	10.00
O- J	-139 / 3	0.0	0.0	0.09 (1)	7.81
N- M	0 / 59	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 67	0.02 (4)
V- D	0 / 120	0.03 (4)
D- U	0 / 1032	0.23 (1)
U- E	-778 / 0	0.70 (1)
E- S	0 / 282	0.06 (1)
S- F	-551 / 0	0.50 (1)
S- Q	0 / 2301	0.37 (1)
F- Q	0 / 108	0.02 (1)
Q- I	0 / 1076	0.24 (1)
P- I	0 / 182	0.05 (4)
P- J	-408 / 0	0.31 (1)
W- C	-2341 / 0	0.62 (1)
O- M	-72 / 0	0.01 (1)
O- K	0 / 2132	0.34 (1)

TOTAL WEIGHT = 189 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.73/1.00 (I-J:1) , BC=0.44/1.00 (O-P:1) , WB=0.70/1.00 (E-U:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (S) (INPUT = 0.90)
JSI METAL= 0.63 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T5S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	9.0		
T	BS-t	MT20	3.0	6.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

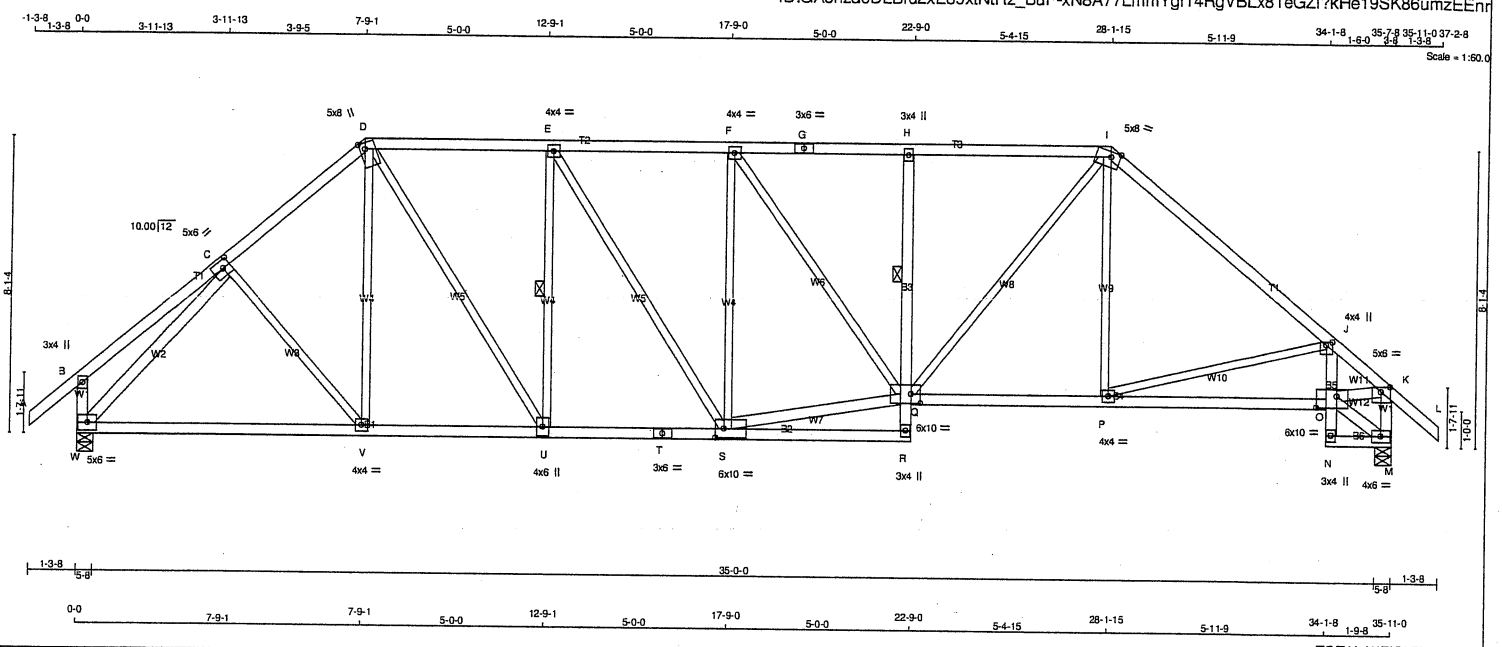
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115266 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410966	T5S1	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
W - C	2x4	DRY	2100F 1.8E	SPF
O - M	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	8.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW-m	MT20	5.0	8.0	Edge	3.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	10.0	3.75	6.50
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWWW-l	MT20	6.0	10.0	3.00	3.25
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	10.0	3.00	2.75
T	BS-t	MT20	3.0	6.0		
U	BMWW+t	MT20	4.0	6.0		



Structural component only
DWG# T-2115284 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2105	0	2105	0	0	5-8	5-8
M	2109	0	2109	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1486	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-Q, E-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	0 / 26	-91.8	-91.8	0.22 (1)	10.00
C- D	-2056 / 0	-91.8	-91.8	0.30 (1)	4.45
D- E	-2117 / 0	-91.8	-91.8	0.43 (1)	4.22
E- F	-2292 / 0	-91.8	-91.8	0.45 (1)	4.07
F- G	-2459 / 0	-91.8	-91.8	0.49 (1)	3.88
G- H	-2459 / 0	-91.8	-91.8	0.49 (1)	3.88
H- I	-2465 / 0	-91.8	-91.8	0.52 (1)	3.88
I- J	-2342 / 0	-91.8	-91.8	0.73 (1)	3.90
J- K	-2681 / 0	-91.8	-91.8	0.49 (1)	3.64
K- L	0 / 41	-91.8	-91.8	0.13 (1)	10.00
W- B	-263 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-2053 / 0	0.0	0.0	0.22 (1)	5.91
W- V	0 / 1519	-18.5	-18.5	0.38 (1)	10.00
V- U	0 / 1557	-18.5	-18.5	0.39 (1)	10.00
U- T	0 / 2117	-18.5	-18.5	0.40 (1)	10.00
T- S	0 / 2117	-18.5	-18.5	0.40 (1)	10.00
S- R	0 / 42	-18.5	-18.5	0.12 (4)	10.00
R- Q	0 / 40	0.0	0.0	0.06 (1)	10.00
Q- H	-541 / 0	0.0	0.0	0.08 (1)	6.25
Q- P	0 / 1775	-18.5	-18.5	0.39 (1)	10.00
P- O	0 / 2166	-18.5	-18.5	0.45 (1)	10.00
N- O	0 / 17	0.0	0.0	0.09 (1)	10.00
O- J	-140 / 1	0.0	0.0	0.09 (1)	7.81
N- M	0 / 59	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 67	0.02 (4)
V- D	0 / 118	0.04 (4)
D- U	0 / 1029	0.23 (1)
U- E	-797 / 0	0.33 (1)
E- S	0 / 330	0.07 (1)
S- F	-632 / 0	0.80 (1)
S- Q	0 / 2300	0.37 (1)
F- Q	0 / 281	0.06 (1)
Q- I	0 / 1090	0.25 (1)
P- I	0 / 191	0.06 (4)
P- J	-407 / 0	0.31 (1)
W- C	-2342 / 0	0.62 (1)
O- M	-72 / 0	0.01 (1)
O- K	0 / 2132	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.73/1.00 (I-J:1) , BC=0.45/1.00 (O-P:1) , WB=0.80/1.00 (F-S:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.64 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410966	T5S1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

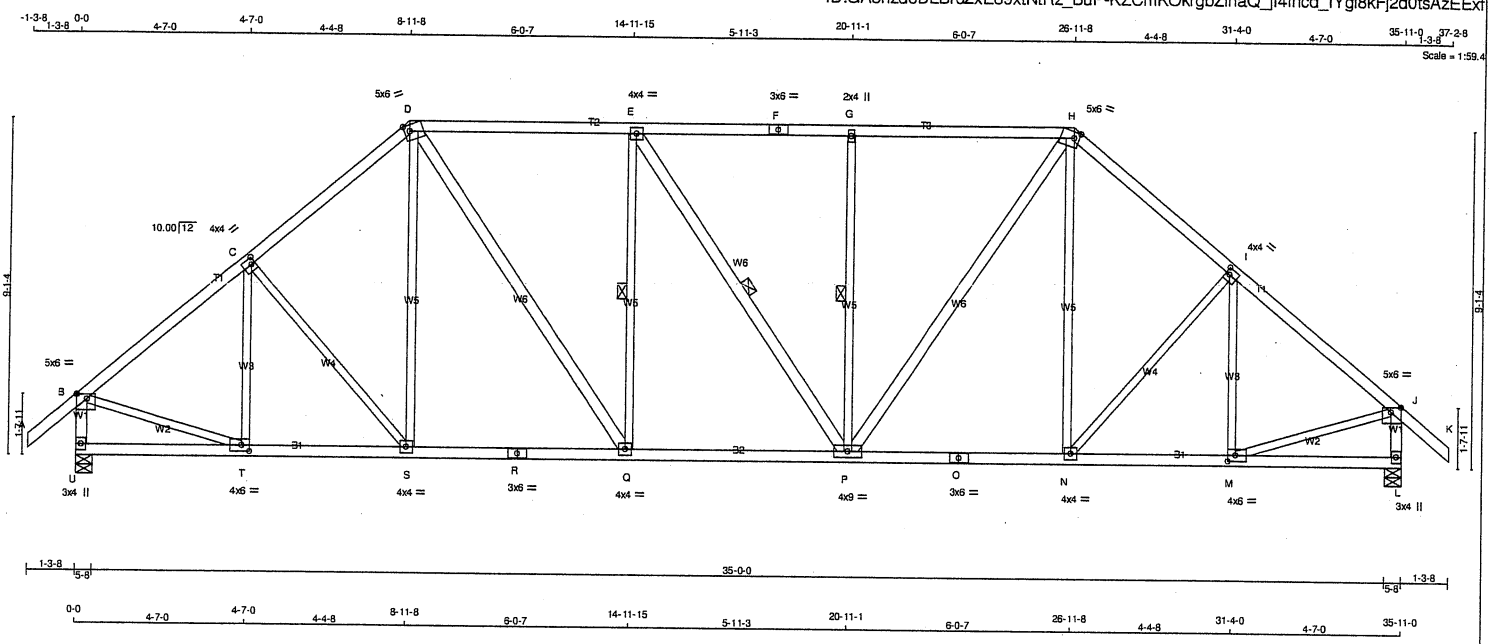


Structural component only
DWG# T-2115284

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 184 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	1.75
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	6.0	1.50	3.25
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N, Q, S						
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	6.0	2.00	2.50
U	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115267

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2107	0	2107	0	0	5-8	5-8
L	2107	0	2107	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	-2041 / 0	-91.8	-91.8	0.41 (1)	4.35
C- D	-2012 / 0	-91.8	-91.8	0.40 (1)	4.37
D- E	-2005 / 0	-91.8	-91.8	0.58 (1)	4.13
E- F	-2004 / 0	-91.8	-91.8	0.58 (1)	4.13
F- G	-2004 / 0	-91.8	-91.8	0.58 (1)	4.13
G- H	-2004 / 0	-91.8	-91.8	0.57 (1)	4.14
H- I	-2012 / 0	-91.8	-91.8	0.40 (1)	4.37
I- J	-2041 / 0	-91.8	-91.8	0.41 (1)	4.35
J- K	0 / 41	-91.8	-91.8	0.13 (1)	10.00
U- B	-2069 / 0	0.0	0.0	0.22 (1)	5.88
L- J	-2069 / 0	0.0	0.0	0.22 (1)	5.88
U- T	0 / 0	-18.5	-18.5	0.08 (4)	10.00
T- S	0 / 1591	-18.5	-18.5	0.31 (1)	10.00
S- R	0 / 1520	-18.5	-18.5	0.32 (1)	10.00
R- Q	0 / 1520	-18.5	-18.5	0.32 (1)	10.00
Q- P	0 / 2005	-18.5	-18.5	0.40 (1)	10.00
P- O	0 / 1520	-18.5	-18.5	0.31 (1)	10.00
O- N	0 / 1520	-18.5	-18.5	0.31 (1)	10.00
N- M	0 / 1591	-18.5	-18.5	0.32 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-369 / 0	0.16 (1)
C- S	-112 / 0	0.10 (1)
S- D	0 / 184	0.05 (4)
D- Q	0 / 858	0.14 (1)
Q- E	-594 / 0	0.32 (1)
E- P	-2 / 0	0.00 (1)
P- G	-593 / 0	0.32 (1)
P- H	0 / 855	0.14 (1)
N- H	0 / 185	0.05 (4)
N- I	-112 / 0	0.10 (1)
M- I	-370 / 0	0.17 (1)
B- T	0 / 1655	0.37 (1)
M- J	0 / 1654	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.58/1.00 (D-E:1) , BC=0.40/1.00 (P-Q:1) , WB=0.37/1.00 (B-T:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

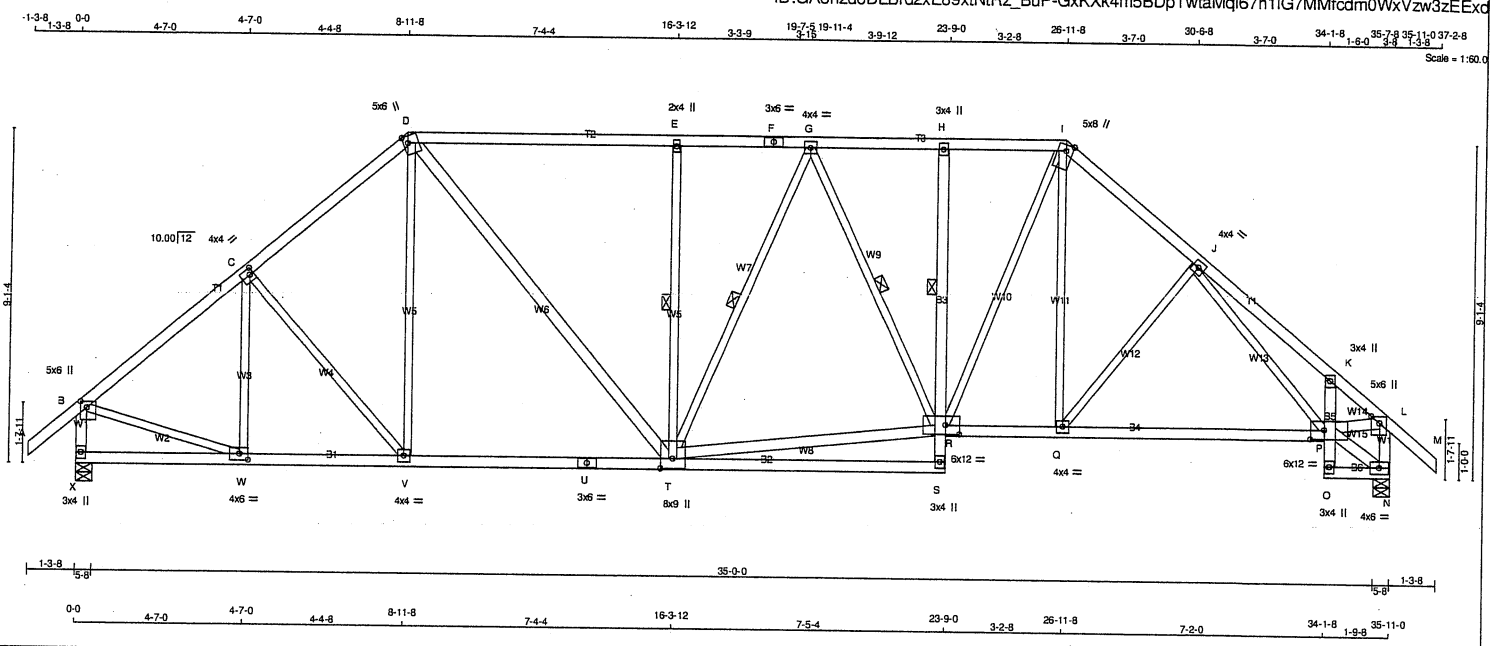
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.52 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T6S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-GxKXk4m5BDpTwtaMqi67n1iG7MMfcdm0WxVzw3zEExo



TOTAL WEIGHT = 194 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
X - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
D - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW+m	MT20	5.0	8.0	2.00	2.25
J	TMWW-t	MT20	4.0	4.0		
K	TMV+p	MT20	3.0	4.0		
L	TMVW+p	MT20	5.0	6.0	Edge	2.75
N	BMVW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-I	MT20	6.0	12.0	3.00	4.50
Q	BMWW-t	MT20	4.0	4.0		
R	BVMWWW-I	MT20	6.0	12.0	3.00	4.50
S	BMV+p	MT20	3.0	4.0		
T	BMWWWW*+m	MT20	8.0	9.0	3.25	4.00
U	BS-t	MT20	3.0	6.0		



Structural component only
DWG# T-2115268

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2105	0	2105	0	0	5-8	5-8
N	2109	0	2109	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1486	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
N	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-R, E-T, G-T, G-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)		10.00
B- C	-2038 / 0	-91.8	-91.8	0.41 (1)		4.35
C- D	-2010 / 0	-91.8	-91.8	0.40 (1)		4.38
D- E	-2056 / 0	-91.8	-91.8	0.88 (1)		3.69
E- F	-2059 / 0	-91.8	-91.8	0.70 (1)		3.68
F- G	-2059 / 0	-91.8	-91.8	0.70 (1)		3.68
G- H	-2053 / 0	-91.8	-91.8	0.22 (1)		4.52
H- I	-2058 / 0	-91.8	-91.8	0.22 (1)		4.52
I- J	-2254 / 0	-91.8	-91.8	0.28 (1)		4.30
J- K	-2327 / 0	-91.8	-91.8	0.29 (1)		4.24
K- L	-2349 / 0	-91.8	-91.8	0.18 (1)		4.32
L- M	0 / 41	-91.8	-91.8	0.13 (1)		10.00
X- B	-2066 / 0	0.0	0.0	0.22 (1)		5.88
N- L	-2061 / 0	0.0	0.0	0.22 (1)		5.89
X- W	0 / 0	-18.5	-18.5	0.08 (4)		10.00
W- V	0 / 1589	-18.5	-18.5	0.33 (1)		10.00
V- U	0 / 1519	-18.5	-18.5	0.38 (1)		10.00
U- T	0 / 1519	-18.5	-18.5	0.38 (1)		10.00
T- S	0 / 41	-18.5	-18.5	0.26 (4)		10.00
S- R	0 / 59	0.0	0.0	0.06 (1)		10.00
R- H	-374 / 0	0.0	0.0	0.07 (1)		6.25
R- Q	0 / 1713	-18.5	-18.5	0.40 (1)		10.00
Q- P	0 / 1893	-18.5	-18.5	0.43 (1)		10.00
O- P	0 / 17	0.0	0.0	0.08 (1)		10.00
P- K	-176 / 0	0.0	0.0	0.08 (1)		7.81
O- N	0 / 56	-18.5	-18.5	0.02 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-372 / 0	0.17 (1)
C- V	-110 / 0	0.09 (1)
V- D	0 / 192	0.05 (4)
R- I	0 / 874	0.20 (1)
Q- I	0 / 366	0.08 (1)
Q- J	-293 / 0	0.19 (1)
J- P	-145 / 0	0.09 (1)
B- W	0 / 1652	0.37 (1)
P- N	-65 / 0	0.01 (1)
P- L	0 / 1811	0.29 (1)
D- T	0 / 840	0.14 (1)
T- E	-679 / 0	0.37 (1)
T- R	0 / 2075	0.33 (1)
T- G	-99 / 0	0.06 (1)
G- R	-103 / 0	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.88/1.00 (D-E:1) , BC=0.43/1.00 (P-Q:1) , WB=0.37/1.00 (B-W:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (W) (INPUT = 0.90)
JSI METAL= 0.47 (U) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T6S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWW-t	MT20	4.0	4.0		
W	BMWW-t	MT20	4.0	6.0	2.00	2.75
X	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

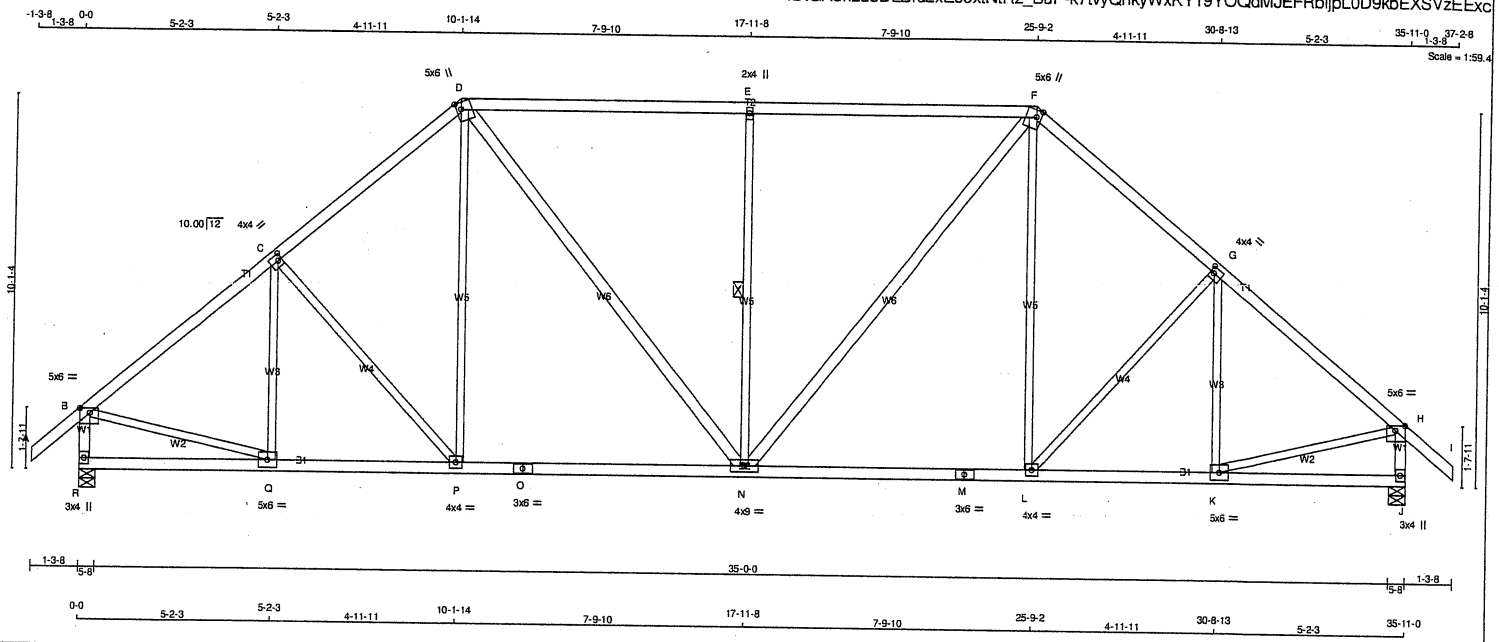
LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115268

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-k7tvyQnkyWxKY19YOQdMJEFRbljpL0D9kbEXSVzEExc



TOTAL WEIGHT = 176 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	6.0	1.50	3.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2107	0	2107	0	0	5-8	5-8
J	2107	0	2107	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
J	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	-2070 / 0	-91.8	-91.8	0.39 (1)	4.36
C- D	-1964 / 0	-91.8	-91.8	0.37 (1)	4.47
D- E	-1891 / 0	-91.8	-91.8	0.84 (1)	3.64
E- F	-1891 / 0	-91.8	-91.8	0.84 (1)	3.64
F- G	-1964 / 0	-91.8	-91.8	0.37 (1)	4.47
G- H	-2070 / 0	-91.8	-91.8	0.39 (1)	4.36
H- I	0 / 41	-91.8	-91.8	0.13 (1)	10.00
R- B	-2065 / 0	0.0	0.0	0.22 (1)	5.89
J- H	-2065 / 0	0.0	0.0	0.22 (1)	5.89
R- Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00
Q- P	0 / 1617	-18.5	-18.5	0.35 (1)	10.00
P- O	0 / 1481	-18.5	-18.5	0.37 (1)	10.00
O- N	0 / 1481	-18.5	-18.5	0.37 (1)	10.00
N- M	0 / 1481	-18.5	-18.5	0.37 (1)	10.00
M- L	0 / 1481	-18.5	-18.5	0.37 (1)	10.00
L- K	0 / 1617	-18.5	-18.5	0.35 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-314 / 0	0.17 (1)
C- P	-207 / 0	0.24 (1)
P- D	0 / 276	0.06 (1)
D- N	0 / 659	0.11 (1)
N- E	-881 / 0	0.61 (1)
N- F	0 / 659	0.11 (1)
L- F	0 / 276	0.06 (1)
L- G	-207 / 0	0.24 (1)
K- G	-314 / 0	0.17 (1)
B- Q	0 / 1667	0.38 (1)
K- H	0 / 1667	0.38 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.84/1.00 (D-E:1) , BC=0.37/1.00 (L-N:1) , WB=0.61/1.00 (E-N:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)

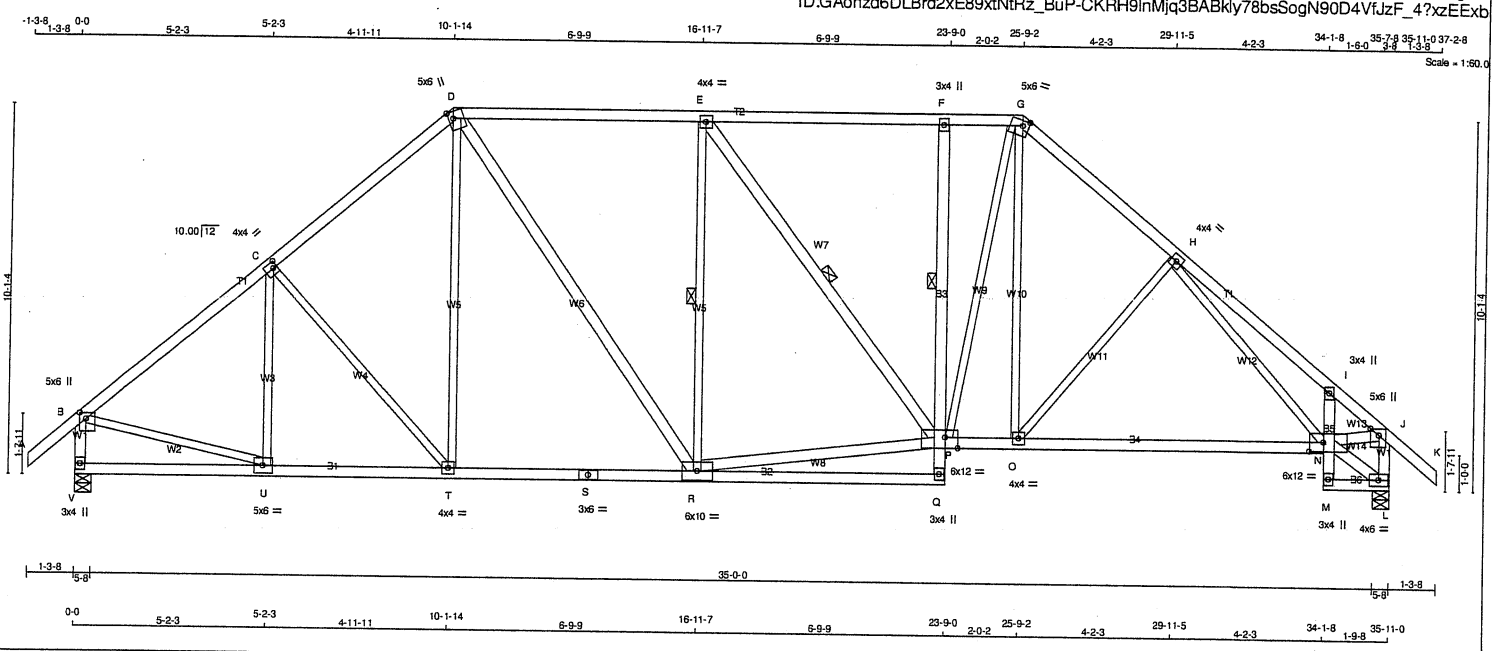
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115269

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T7S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 15:57:12 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-CKRH9InMjq3BABkly78bsSogN90D4VfJzF_4?xzEExb



TOTAL WEIGHT = 200 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	2.00
H	TMWW-t	MT20	4.0	4.0		
I	TMV+p	MT20	3.0	4.0		
J	TMVW+p	MT20	5.0	6.0	Edge	2.75
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWWW-l	MT20	6.0	12.0	3.00	4.50
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-l	MT20	6.0	12.0	Edge	4.25
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	6.0	10.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMWW-t	MT20	5.0	6.0		



Structural component only
DWG# T-2115270

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2105	0	2105	0	0	5-8	5-8
L	2109	0	2109	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1486	988 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1489	991 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P, E-R, E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	-2068 / 0	-91.8	-91.8	0.39 (1)	4.36
C- D	-1958 / 0	-91.8	-91.8	0.37 (1)	4.47
D- E	-1853 / 0	-91.8	-91.8	0.58 (1)	4.25
E- F	-1831 / 0	-91.8	-91.8	0.55 (1)	4.31
F- G	-1833 / 0	-91.8	-91.8	0.25 (1)	4.71
G- H	-2175 / 0	-91.8	-91.8	0.28 (1)	4.39
H- I	-2343 / 0	-91.8	-91.8	0.29 (1)	4.25
I- J	-2359 / 0	-91.8	-91.8	0.15 (1)	4.36
J- K	0 / 41	-91.8	-91.8	0.13 (1)	10.00
V- B	-2063 / 0	0.0	0.0	0.22 (1)	5.89
L- J	-2061 / 0	0.0	0.0	0.22 (1)	5.89
V- U	0 / 0	-18.5	-18.5	0.10 (4)	10.00
U- T	0 / 1615	-18.5	-18.5	0.32 (1)	10.00
T- S	0 / 1476	-18.5	-18.5	0.36 (1)	10.00
S- R	0 / 1476	-18.5	-18.5	0.36 (1)	10.00
R- Q	0 / 33	-18.5	-18.5	0.21 (4)	10.00
Q- P	0 / 55	0.0	0.0	0.05 (1)	10.00
P- F	-439 / 0	0.0	0.0	0.08 (1)	6.25
P- O	0 / 1650	-18.5	-18.5	0.46 (4)	10.00
O- N	0 / 1875	-18.5	-18.5	0.49 (1)	10.00
M- N	0 / 17	0.0	0.0	0.08 (1)	10.00
N- I	-216 / 0	0.0	0.0	0.06 (1)	7.81
M- L	0 / 56	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-308 / 0	0.17 (1)
C- T	-212 / 0	0.24 (1)
T- D	0 / 262	0.06 (1)
D- R	0 / 668	0.11 (1)
R- E	-679 / 0	0.47 (1)
R- P	0 / 1847	0.30 (1)
E- P	-48 / 0	0.03 (1)
P- G	0 / 773	0.17 (1)
O- G	0 / 450	0.10 (1)
O- H	-354 / 0	0.30 (1)
B- U	0 / 1666	0.37 (1)
N- L	-65 / 0	0.01 (1)
N- J	0 / 1827	0.29 (1)
H- N	-90 / 2	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP. CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.58/1.00 (D-E:1) , BC=0.49/1.00 (N-O:1) , WB=0.47/1.00 (E-R:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.49 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T7S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

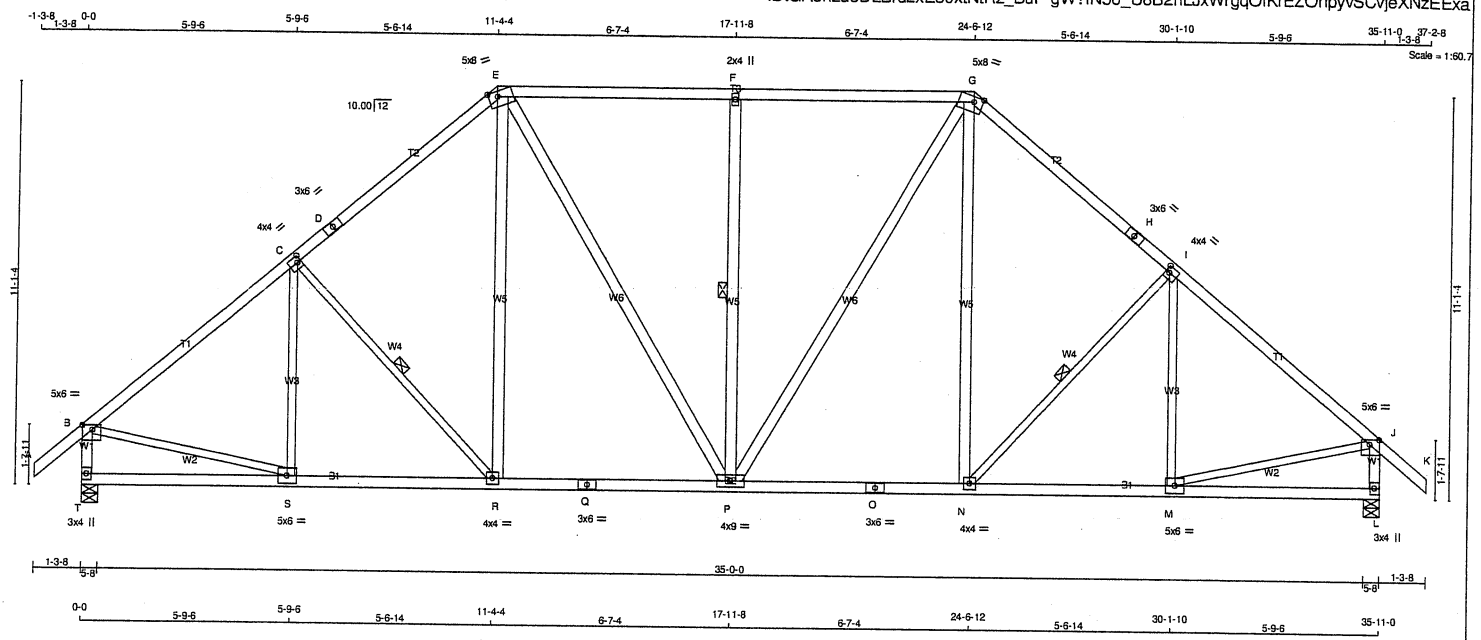
LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115270 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T8	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 192 = 385 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
C - R	2x3	DRY	No.2	SPF
N - I	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	6.0	1.50	3.25
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		



Structural component only
DWG# T-2115271

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2107	0	2107	0	0	5-8	5-8
L	2107	0	2107	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1488	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	S- C	-255 / 6	0.17 (1)	
B- C	-2089 / 0	-91.8	-91.8	0.50 (1)	4.22	C- R	-306 / 0	0.15 (1)	
C- D	-1901 / 0	-91.8	-91.8	0.47 (1)	4.41	R- E	0 / 336	0.05 (1)	
D- E	-1901 / 0	-91.8	-91.8	0.47 (1)	4.41	E- P	0 / 507	0.08 (1)	
E- F	-1695 / 0	-91.8	-91.8	0.57 (1)	4.40	P- F	-743 / 0	0.47 (1)	
F- G	-1695 / 0	-91.8	-91.8	0.57 (1)	4.40	P- G	0 / 507	0.08 (1)	
G- H	-1901 / 0	-91.8	-91.8	0.47 (1)	4.41	N- G	0 / 336	0.05 (1)	
H- I	-1901 / 0	-91.8	-91.8	0.47 (1)	4.41	N- I	-306 / 0	0.15 (1)	
I- J	-2089 / 0	-91.8	-91.8	0.50 (1)	4.22	M- I	-255 / 6	0.17 (1)	
J- K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	B- S	0 / 1677	0.38 (1)	
T- B	-2062 / 0	0.0	0.0	0.22 (1)	5.89	M- J	0 / 1677	0.38 (1)	
L- J	-2062 / 0	0.0	0.0	0.22 (1)	5.89				
T- S	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
S- R	0 / 1635	-18.5	-18.5	0.34 (1)	10.00				
R- Q	0 / 1431	-18.5	-18.5	0.32 (1)	10.00				
Q- P	0 / 1431	-18.5	-18.5	0.32 (1)	10.00				
P- O	0 / 1431	-18.5	-18.5	0.32 (1)	10.00				
O- N	0 / 1431	-18.5	-18.5	0.32 (1)	10.00				
N- M	0 / 1635	-18.5	-18.5	0.34 (1)	10.00				
M- L	0 / 0	-18.5	-18.5	0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.57/1.00 (E-F:1) , BC=0.34/1.00 (R-S:1) , WB=0.47/1.00 (F-P:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

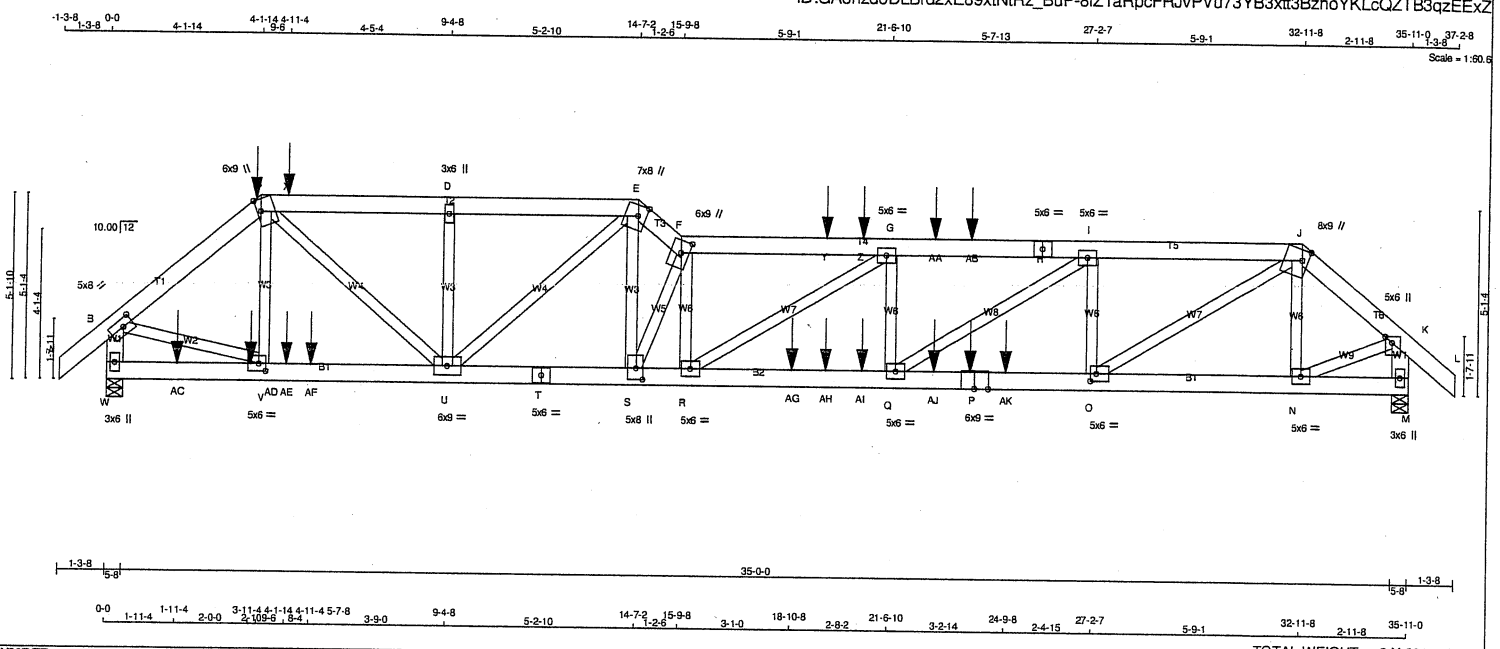
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.49 (Q) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410965	T9	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 221 = 441 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- E	2	12	SIDE(61.0)
E- F	2	12	TOP
F- H	2	12	SIDE(0.0)
H- J	2	12	TOP
J- L	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(197.8)
T- P	2	12	SIDE(0.0)
P- M	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
05/22/14
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115272

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	4994	0	4994	0	0	5-8	5-8
M	4108	0	4108	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3520	2375 / 0	0 / 0	0 / 0	0 / 0	1145 / 0	0 / 0
M	2896	1952 / 0	0 / 0	0 / 0	0 / 0	944 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 42	-91.8	-91.8	0.04 (1)		10.00
B- C	-5626 / 0	-91.8	-91.8	0.12 (1)		4.88
C- X	-6986 / 0	-91.8	-91.8	0.18 (1)		4.41
X- D	-6986 / 0	-91.8	-91.8	0.18 (1)		4.41
D- E	-6986 / 0	-91.8	-91.8	0.16 (1)		4.43
E- F	-10763 / 0	-91.8	-91.8	0.15 (1)		3.67
F- Y	-11345 / 0	-91.8	-91.8	0.37 (1)		3.41
Y- Z	-11345 / 0	-91.8	-91.8	0.37 (1)		3.41
Z- G	-11345 / 0	-91.8	-91.8	0.37 (1)		3.41
G-AA	-11719 / 0	-91.8	-91.8	0.35 (1)		3.38
AA-AB	-11719 / 0	-91.8	-91.8	0.35 (1)		3.38
AB- H	-11719 / 0	-91.8	-91.8	0.35 (1)		3.38
H- I	-11719 / 0	-91.8	-91.8	0.35 (1)		3.38
I- J	-8527 / 0	-91.8	-91.8	0.23 (1)		4.02
J- K	-4160 / 0	-91.8	-91.8	0.06 (1)		5.55
K- L	0 / 42	-91.8	-91.8	0.04 (1)		10.00
W- B	-5060 / 0	0.0	0.0	0.18 (1)		6.47
M- K	-4093 / 0	0.0	0.0	0.15 (1)		7.04
W-AC	0 / 0	-18.5	-18.5	0.11 (1)		10.00
AC-AD	0 / 0	-18.5	-18.5	0.11 (1)		10.00
AD- V	0 / 0	-18.5	-18.5	0.11 (1)		10.00
V-AE	0 / 4311	-18.5	-18.5	0.29 (1)		10.00
AE-AF	0 / 4311	-18.5	-18.5	0.29 (1)		10.00
AF- U	0 / 4311	-18.5	-18.5	0.29 (1)		10.00
U- T	0 / 8520	-18.5	-18.5	0.29 (1)		10.00
T- S	0 / 8520	-18.5	-18.5	0.29 (1)		10.00
S- R	0 / 11309	-18.5	-18.5	0.33 (1)		10.00
R-AG	0 / 11719	-18.5	-18.5	0.54 (1)		10.00
AG-AH	0 / 11719	-18.5	-18.5	0.54 (1)		10.00
AH-AI	0 / 11719	-18.5	-18.5	0.54 (1)		10.00
AI- Q	0 / 11719	-18.5	-18.5	0.54 (1)		10.00
Q-AJ	0 / 8527	-18.5	-18.5	0.45 (1)		10.00
AJ- P	0 / 8527	-18.5	-18.5	0.45 (1)		10.00
P-AK	0 / 8527	-18.5	-18.5	0.45 (1)		10.00
AK- O	0 / 8527	-18.5	-18.5	0.45 (1)		10.00
O- N	0 / 3133	-18.5	-18.5	0.14 (1)		10.00
N- M	0 / 0	-18.5	-18.5	0.01 (4)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	0 / 203	0.03 (4)
C- U	0 / 3569	0.32 (1)
U- D	-496 / 0	0.06 (1)
U- E	-2046 / 0	0.61 (1)
S- E	0 / 7952	0.70 (1)
S- F	-8719 / 0	0.78 (1)
R- F	0 / 895	0.08 (1)
N- J	-1089 / 0	0.09 (1)
B- V	0 / 4446	0.39 (1)
N- K	0 / 3376	0.30 (1)
O- J	0 / 6354	0.56 (1)
R- G	-444 / 0	0.13 (1)
O- I	-2655 / 0	0.23 (1)
Q- G	-600 / 0	0.05 (1)
Q- I	0 / 3815	0.34 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-1-14	-47	-47	---	FRONT	VERT	DEAD	---	C1
C	4-1-14	-97	-97	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 789 (0.55")

CSI: TC=0.37/1.00 (F-G:1) , BC=0.54/1.00 (Q-R:1) , WB=0.78/1.00 (F-S:1) , SSI=0.37/1.00 (U-V:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (V) (INPUT = 0.90)
JSI METAL= 0.83 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T9	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-8iZ1aRpcFRJvPVu73YB3xtt3BzhoYKLcQZTB3qzEExZ

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	6.0	9.0	4.00	1.25
D	TMW+w	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.75
F	TTWW+m	MT20	6.0	9.0	4.00	2.50
G	TMWW-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	9.0	3.50	2.00
K	TMVW+p	MT20	5.0	6.0	2.00	2.25
M	BMV1+p	MT20	3.0	6.0		
N, Q, R						
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.00
P	BS-t	MT20	6.0	9.0		
S	BMWW+t	MT20	5.0	8.0	3.75	2.25
T	BS-t	MT20	5.0	6.0		
U	BMWWW-t	MT20	6.0	9.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.50
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-1-14	-204	-204	---	FRONT	VERT	SNOW	---	C1
P	23-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
X	4-11-4	-99	-99	---	BACK	VERT	TOTAL	---	C1
Y	19-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Z	20-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AA	22-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AB	23-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AC	1-11-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
AD	3-11-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
AE	4-11-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
AF	5-7-8	-947	-947	---	BACK	VERT	TOTAL	---	C1
AG	18-10-8	-783	-783	---	BACK	VERT	TOTAL	---	C1
AH	19-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AI	20-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AJ	22-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AK	24-9-8	-794	-794	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

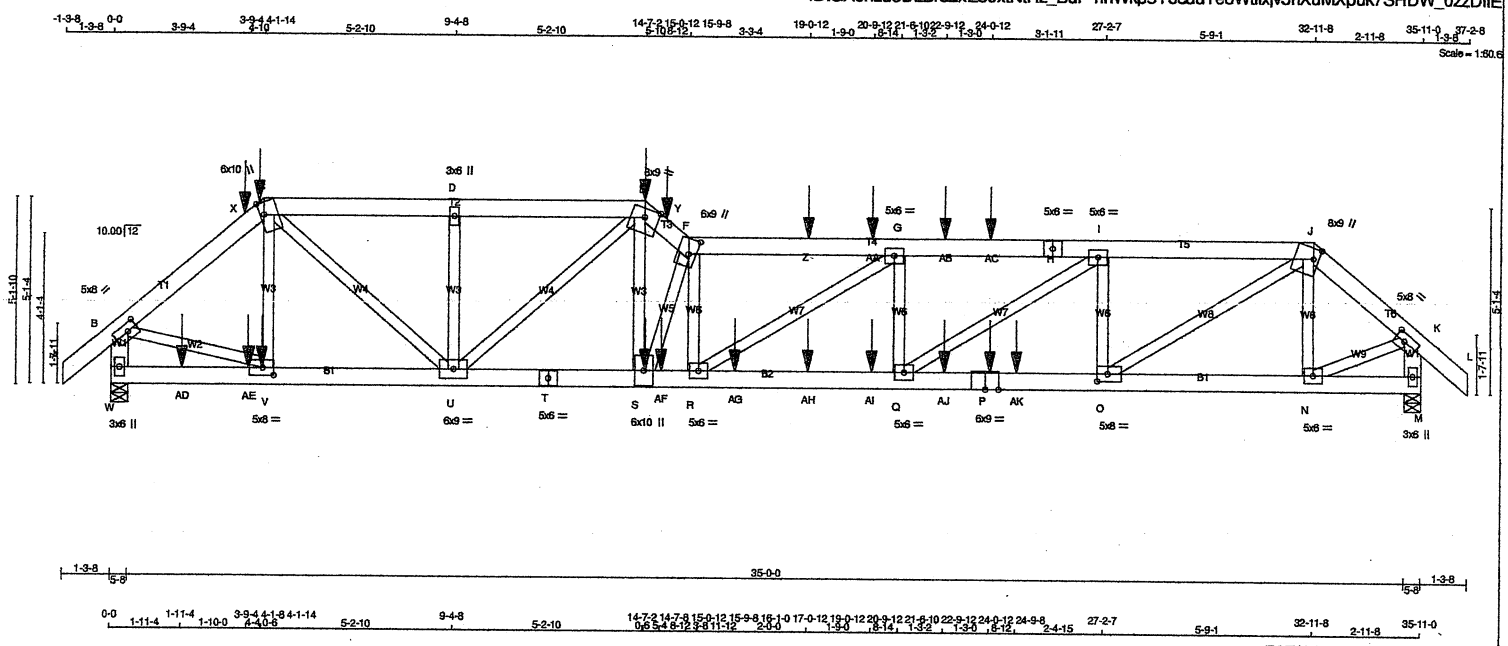
Structural component only
DWG# T-2115272

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410967	T9Z	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-nhWkp5Y8cduTeoWtilxjv3hXuMXpuk7SHDW_0zzDIIE



TOTAL WEIGHT = 2 X 221 = 441 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
V - C	2x4	DRY	2100F 1.8E	SPF
C - U	2x4	DRY	2100F 1.8E	SPF
U - D	2x4	DRY	2100F 1.8E	SPF
U - E	2x4	DRY	2100F 1.8E	SPF
S - E	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- E	2	12	SIDE(61.0)
E- F	2	12	SIDE(122.0)
F- H	2	12	SIDE(0.0)
H- J	2	12	TOP
J- L	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(197.8)
T- P	2	12	SIDE(197.8)
P- M	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
V- C	1	6	SIDE(184.8)
S- E	1	6	SIDE(184.8)
2x4	1	6	



Structural component only
DWG# T-2115285

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	5713	0	5713	0	0	5-8	5-8
M	4280	0	4280	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4028	2714 / 0	0 / 0	0 / 0	0 / 0	1314 / 0	0 / 0
M	3018	2031 / 0	0 / 0	0 / 0	0 / 0	987 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-91.8	-91.8	0.04 (1)	10.00
B- X	-6260 / 0	-91.8	-91.8	0.14 (1)	4.65
X- C	-6260 / 0	-91.8	-91.8	0.14 (1)	4.65
C- D	-7979 / 0	-91.8	-91.8	0.17 (1)	4.17
D- E	-7979 / 0	-91.8	-91.8	0.17 (1)	4.17
E- Y	-13199 / 0	-91.8	-91.8	0.24 (1)	3.24
Y- F	-13199 / 0	-91.8	-91.8	0.24 (1)	3.24
F- Z	-13077 / 0	-91.8	-91.8	0.45 (1)	3.12
Z-AA	-13077 / 0	-91.8	-91.8	0.45 (1)	3.12
AA- G	-13077 / 0	-91.8	-91.8	0.45 (1)	3.12
G-AB	-12317 / 0	-91.8	-91.8	0.37 (1)	3.28
AB-AC	-12317 / 0	-91.8	-91.8	0.37 (1)	3.28
AC- H	-12317 / 0	-91.8	-91.8	0.37 (1)	3.28
H- I	-12317 / 0	-91.8	-91.8	0.37 (1)	3.28
I- J	-8963 / 0	-91.8	-91.8	0.24 (1)	3.92
J- K	-4339 / 0	-91.8	-91.8	0.06 (1)	5.46
K- L	0 / 42	-91.8	-91.8	0.04 (1)	10.00
W- B	-5644 / 0	0.0	0.0	0.20 (1)	6.20
M- K	-4257 / 0	0.0	0.0	0.15 (1)	6.94
W-AD	0 / 0	-18.5	-18.5	0.02 (4)	10.00
AD-AE	0 / 0	-18.5	-18.5	0.02 (4)	10.00
AE- V	0 / 0	-18.5	-18.5	0.02 (4)	10.00
V- U	0 / 4832	-18.5	-18.5	0.14 (1)	10.00
U- T	0 / 10461	-18.5	-18.5	0.30 (1)	10.00
T- S	0 / 10461	-18.5	-18.5	0.30 (1)	10.00
S-AF	0 / 13116	-18.5	-18.5	0.48 (1)	10.00
AF- R	0 / 13116	-18.5	-18.5	0.48 (1)	10.00
R-AG	0 / 12317	-18.5	-18.5	0.46 (1)	10.00
AG-AH	0 / 12317	-18.5	-18.5	0.46 (1)	10.00
AH-AI	0 / 12317	-18.5	-18.5	0.46 (1)	10.00
AI- Q	0 / 12317	-18.5	-18.5	0.46 (1)	10.00
Q-AJ	0 / 8963	-18.5	-18.5	0.48 (1)	10.00
AJ- P	0 / 8963	-18.5	-18.5	0.48 (1)	10.00
P-AK	0 / 8963	-18.5	-18.5	0.48 (1)	10.00
AK- O	0 / 8963	-18.5	-18.5	0.48 (1)	10.00
O- N	0 / 3267	-18.5	-18.5	0.16 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	0 / 325	0.01 (4)
C- U	0 / 4198	0.17 (1)
U- D	-453 / 0	0.03 (1)
U- E	-3311 / 0	0.62 (1)
S- E	0 / 9877	0.41 (1)
S- F	-8509 / 0	0.78 (1)
R- F	-955 / 0	0.08 (1)
N- J	-1165 / 0	0.10 (1)
B- V	0 / 4981	0.44 (1)
N- K	0 / 3521	0.31 (1)
O- J	0 / 6711	0.59 (1)
R- G	0 / 907	0.08 (1)
O- I	-2745 / 0	0.23 (1)
Q- G	-1313 / 0	0.11 (1)
Q- I	0 / 4000	0.35 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-1-14	-47	-47	---	FRONT	VERT	DEAD	---	C1
C	4-1-14	-204	-204	---	FRONT	VERT	SNOW	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 778 (0.55")

CSI: TC=0.45/1.00 (F-G:1) , BC=0.48/1.00 (O-Q:1) , WB=0.78/1.00 (F-S:1) , SSI=0.23/1.00 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.92 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410967	T9Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	6.0	10.0	4.00	1.25
D	TMW+w	MT20	3.0	6.0		
E	TTWW-m	MT20	8.0	9.0	Edge	
F	TTWW+m	MT20	6.0	9.0	5.00	2.25
G	TMWW-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	9.0	3.50	1.75
K	TMW-t	MT20	5.0	8.0	2.50	3.25
M	BMV1+p	MT20	3.0	6.0		
N, Q, R						
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	8.0	2.50	3.50
P	BS-t	MT20	6.0	9.0		
S	BMWW+t	MT20	6.0	10.0		
T	BS-t	MT20	5.0	6.0		
U	BMWWW-t	MT20	6.0	9.0		
V	BMWW-t	MT20	5.0	8.0	2.50	3.75
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	14-7-2	-47	-47	---	FRONT	VERT	DEAD	---	C1
E	14-7-2	-204	-204	---	FRONT	VERT	SNOW	---	C1
P	24-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
S	14-7-8	-1035	-1035	---	BACK	VERT	TOTAL	---	C1
V	4-1-8	-1035	-1035	---	BACK	VERT	TOTAL	---	C1
X	3-9-4	-111	-111	---	BACK	VERT	TOTAL	---	C1
Y	15-0-12	-117	-117	---	BACK	VERT	TOTAL	---	C1
Z	19-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AA	20-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AB	22-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AC	24-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AD	1-11-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
AE	3-9-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
AF	15-0-12	-25	-25	---	BACK	VERT	TOTAL	---	C1
AG	17-0-12	-25	-25	---	BACK	VERT	TOTAL	---	C1
AH	19-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AI	20-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AJ	22-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AK	24-9-8	-773	-773	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

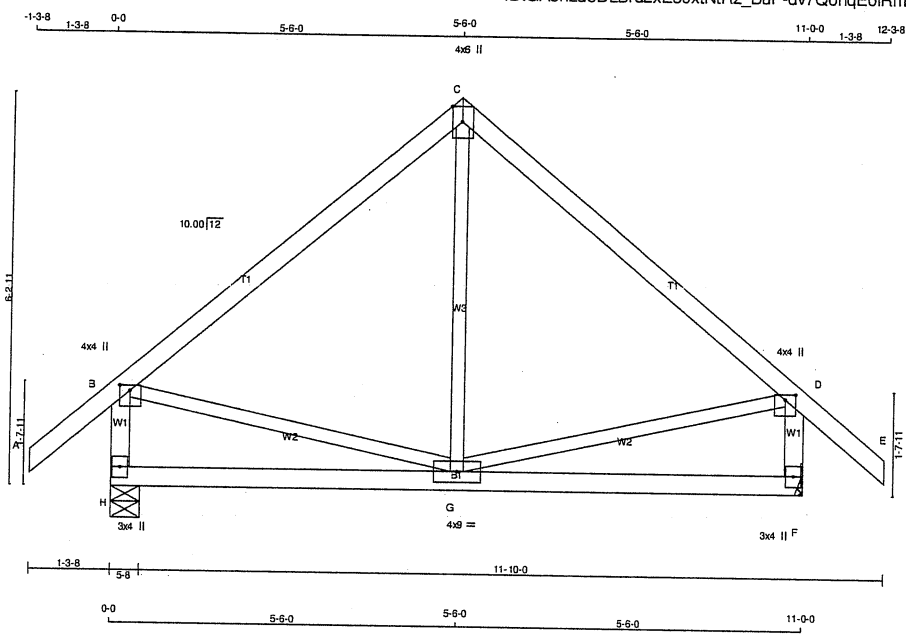
LICENSED PROFESSIONAL ENGINEER
05/24/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115285 3/4

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T10	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-dv7QonqE0lRm1fTKdGiIU4PE3N71HyklfDCkbGzEExY



TOTAL WEIGHT = 4 X 49 = 196 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	733	0	733	0	0	5-8	5-8
F	733	0	733	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
F	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	G- C	-14 / 90	0.03 (4)
B- C	-383 / 0	-91.8	-91.8	0.36 (1)	6.25	B- G	0 / 302	0.07 (1)
C- D	-383 / 0	-91.8	-91.8	0.36 (1)	6.25	G- D	0 / 302	0.07 (1)
D- E	0 / 41	-91.8	-91.8	0.13 (1)	10.00			
H- B	-694 / 0	0.0	0.0	0.07 (1)	7.81			
F- D	-694 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.16/1.00 (G-H:4) , WB=0.07/1.00 (D-G:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (D) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

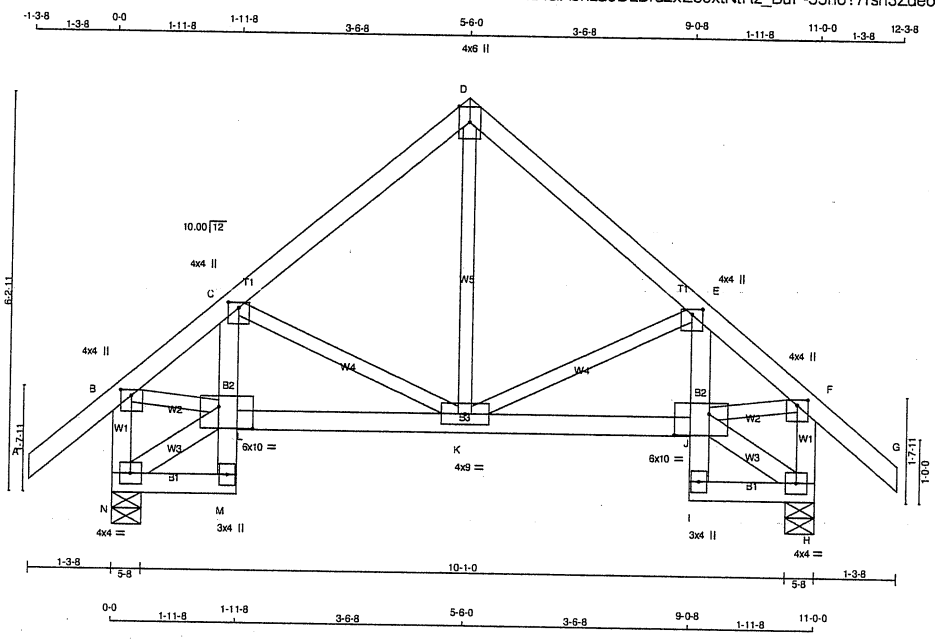


Structural component only
DWG# T-2115273

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410965	T10S	2	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-55ho?7rsn3Zdeo1WBzDX0lyS0mUo0PAvutyI8izEExX



TOTAL WEIGHT = 2 X 58 = 116 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B, C, E, F					
B TMVW+p	MT20	4.0	4.0	1.00	2.00
D TTW+p	MT20	4.0	6.0	Edge	
H BMVW1-t	MT20	4.0	4.0		
I BMV+p	MT20	3.0	4.0		
J BVMWW-l	MT20	6.0	10.0	4.00	6.50
K BMWWW-t	MT20	4.0	9.0		
L BVMWW-l	MT20	6.0	10.0	4.00	6.50
M BMV+p	MT20	3.0	4.0		
N BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	733	0	733	0	0	5-8	5-8
H	733	0	733	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
H	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	-691 / 0	-91.8	-91.8	0.10 (1)	6.25
C- D	-444 / 0	-91.8	-91.8	0.15 (1)	6.25
D- E	-444 / 0	-91.8	-91.8	0.15 (1)	6.25
E- F	-691 / 0	-91.8	-91.8	0.10 (1)	6.25
F- G	0 / 41	-91.8	-91.8	0.13 (1)	10.00
N- B	-706 / 0	0.0	0.0	0.08 (1)	7.81
H- F	-706 / 0	0.0	0.0	0.08 (1)	7.81
N- M	0 / 17	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 19	0.0	0.0	0.03 (1)	10.00
L- C	-3 / 32	0.0	0.0	0.02 (4)	10.00
L- K	0 / 543	-18.5	-18.5	0.13 (1)	10.00
K- J	0 / 543	-18.5	-18.5	0.13 (1)	10.00
I- J	0 / 19	0.0	0.0	0.03 (1)	10.00
J- E	-3 / 32	0.0	0.0	0.02 (4)	10.00
I- H	0 / 17	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 258	0.06 (1)
K- E	-238 / 0	0.07 (1)
C- K	-238 / 0	0.07 (1)
N- L	-20 / 0	0.00 (1)
B- L	0 / 534	0.12 (1)
J- H	-20 / 0	0.00 (1)
J- F	0 / 534	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (C-D:1) , BC=0.13/1.00 (K-L:1) , WB=0.12/1.00 (B-L:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

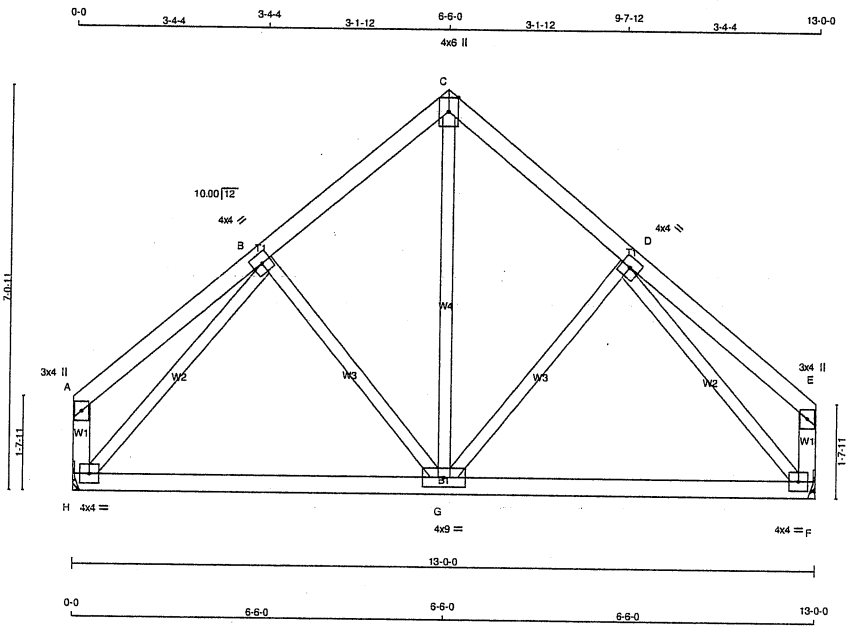


Structural component only
DWG# T-2115274

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-ZHFACTrUYMhUGycilgkmZVVdlAnzlpZ27Xhrg9zEExW



TOTAL WEIGHT = 2 X 58 = 117 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	717	0	717	0	0	MECHANICAL	
F	717	0	717	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	507	333 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0
F	507	333 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 22	-91.8	-91.8	0.15 (1)	10.00	G- C	0 / 366	0.08 (1)
B- C	-490 / 0	-91.8	-91.8	0.12 (1)	6.25	G- D	-141 / 0	0.06 (1)
C- D	-490 / 0	-91.8	-91.8	0.12 (1)	6.25	B- G	-141 / 0	0.06 (1)
D- E	0 / 22	-91.8	-91.8	0.15 (1)	10.00	H- B	-716 / 0	0.30 (1)
H- A	-114 / 0	0.0	0.0	0.01 (1)	7.81	D- F	-716 / 0	0.30 (1)
F- E	-114 / 0	0.0	0.0	0.01 (1)	7.81			
H- G	0 / 450	-18.5	-18.5	0.26 (4)	10.00			
G- F	0 / 450	-18.5	-18.5	0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.26/1.00 (F-G:4) , WB=0.30/1.00 (B-H:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

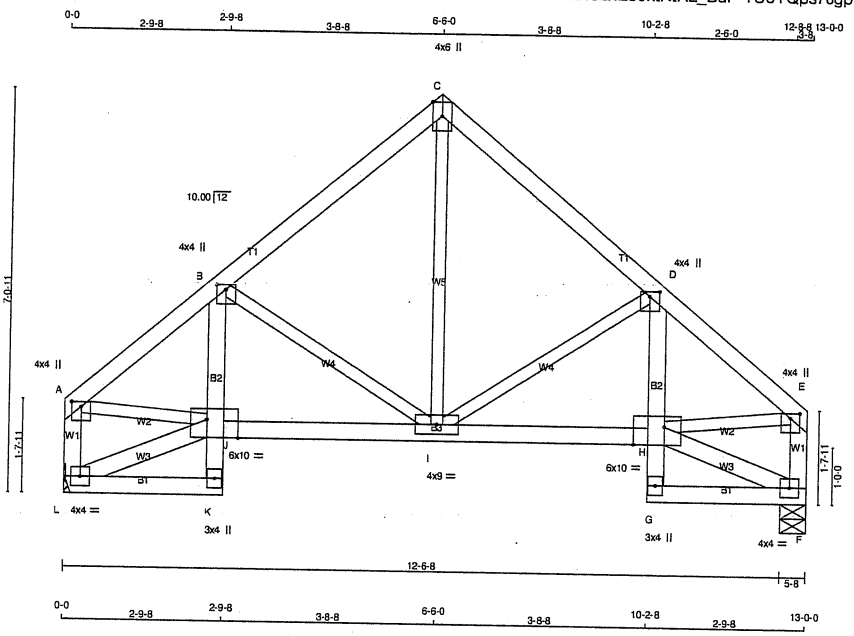


Structural component only
DWG# T-2115275

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T11S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, B, D, E	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
F	BMVW1-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWW-l	MT20	6.0	10.0	Edge	6.50
I	BMWWW-t	MT20	4.0	9.0		
J	BVMWW-l	MT20	6.0	10.0	Edge	6.50
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	717	0	717	0	0	IN-SX MECHANICAL	IN-SX
F	717	0	717	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	507	333 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0
F	507	333 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-831 / 0	-91.8	-91.8	0.13 (1)		6.25
B- C	-557 / 0	-91.8	-91.8	0.15 (1)		6.25
C- D	-557 / 0	-91.8	-91.8	0.15 (1)		6.25
D- E	-831 / 0	-91.8	-91.8	0.13 (1)		6.25
L- A	-686 / 0	0.0	0.0	0.07 (1)		7.81
F- E	-686 / 0	0.0	0.0	0.07 (1)		7.81
L- K	0 / 16	-18.5	-18.5	0.04 (4)		10.00
K- J	0 / 27	0.0	0.0	0.03 (1)		10.00
J- B	-2 / 40	0.0	0.0	0.02 (4)		10.00
J- I	0 / 668	-18.5	-18.5	0.15 (1)		10.00
I- H	0 / 668	-18.5	-18.5	0.15 (1)		10.00
G- H	0 / 27	0.0	0.0	0.03 (1)		10.00
H- D	-2 / 40	0.0	0.0	0.02 (4)		10.00
G- F	0 / 16	-18.5	-18.5	0.04 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 396	0.09 (1)
I- D	-305 / 0	0.11 (1)
B- I	-305 / 0	0.11 (1)
L- J	-17 / 0	0.00 (1)
A- J	0 / 664	0.15 (1)
H- F	-17 / 0	0.00 (1)
H- E	0 / 664	0.15 (1)

TOTAL WEIGHT = 2 X 65 = 131 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.15/1.00 (I-J:1) , WB=0.15/1.00 (A-J:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.23 (A) (INPUT = 1.00)

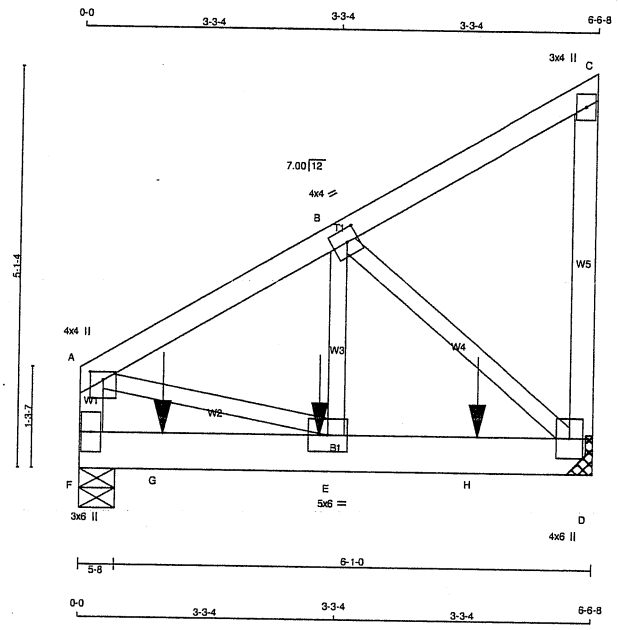


Structural component only
DWG# T-2115276

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T12	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 15:57:19 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-VgMwd9tl4_xBVGm5s5mFewa_L_TfDkYLarAyk1zEExU



TOTAL WEIGHT = 2 X 34 = 67 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		



Structural component only
DWG# T-2115277

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1452	0	1452	0	0	5-8	5-8
D	1363	0	1363	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1024	687 / 0	0 / 0	0 / 0	0 / 0	337 / 0	0 / 0
D	962	645 / 0	0 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1028 / 0	0.0	0.0	0.06 (1)	7.81	A- E	0 / 1102	0.14 (1)
A- B	-1216 / 0	-91.8	-91.8	0.08 (1)	6.25	E- B	0 / 1168	0.14 (1)
B- C	-16 / 0	-91.8	-91.8	0.08 (1)	6.25	B- D	-1400 / 0	0.21 (1)
D- C	-119 / 0	0.0	0.0	0.02 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
E- H	0 / 1064	-18.5	-18.5	0.24 (1)	10.00			
H- D	0 / 1064	-18.5	-18.5	0.24 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-492	-492	---	BACK	VERT	TOTAL	---	C1
G	1-0-12	-492	-492	---	BACK	VERT	TOTAL	---	C1
H	5-0-12	-492	-492	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.08/1.00 (A-B:1) , BC=0.24/1.00 (D-E:1) , WB=0.21/1.00 (B-D:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.24 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T12	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-VgMwd9tl4_xBVGm5s5mFewa_L_TfDkYLarAyk1zEExU

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2



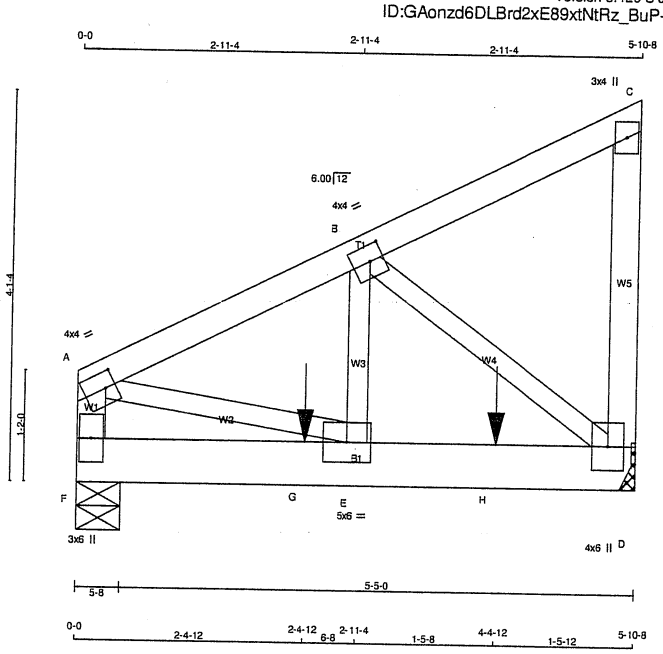
Structural component only

DWG# T-2115277

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T13	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 15:57:20 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-zswJrVuNrH327QLHQpIUB87AQOpIyCqUpVwVHTzEExT



TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		

LICENSED PROFESSIONAL ENGINEER
05/22/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115278 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	913	0	913	0	0	5-8	5-8
D	1131	0	1131	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	644	431 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
D	798	535 / 0	0 / 0	0 / 0	0 / 0	263 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-831 / 0	0.0	0.0	0.05 (1)	7.81
A- B	-1046 / 0	-91.8	-91.8	0.06 (1)	6.25
B- C	-12 / 0	-91.8	-91.8	0.06 (1)	6.25
D- C	-109 / 0	0.0	0.0	0.01 (1)	7.81
F- G	0 / 0	-18.5	-18.5	0.08 (1)	10.00
G- E	0 / 0	-18.5	-18.5	0.08 (1)	10.00
E- H	0 / 946	-18.5	-18.5	0.22 (1)	10.00
H- D	0 / 946	-18.5	-18.5	0.22 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 978	0.12 (1)
E- B	0 / 874	0.11 (1)
B- D	-1190 / 0	0.14 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-4-12	-492	-492	---	FRONT	VERT	TOTAL	---	C1
H	4-4-12	-492	-492	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.22/1.00 (D-E:1) , WB=0.14/1.00 (B-D:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (A) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T13	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-zswJrVuNrH327QLHQpIUB87AQOpIyCqUpVwVHTzEExT

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

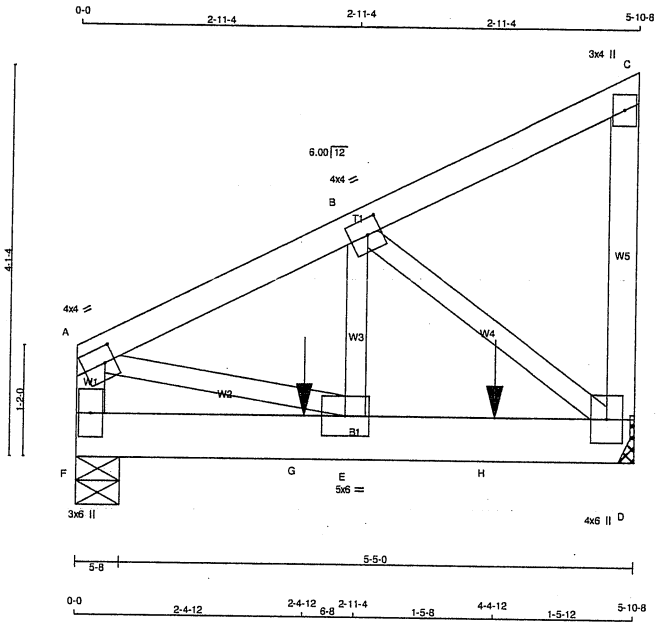
LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115278

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T13Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-R3Uh2qu?cbCvlawT_WpjjLfLAn9Thf1e18f3pwzEExS



TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		



Structural component only
DWG# T-2115279

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	927	0	927	0	0	IN-SX 5-8	IN-SX 5-8
D	1150	0	1150	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	652	448 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0
D	809	558 / 0	0 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-844 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 996	0.12 (1)
A- B	-1065 / 0	-91.8	-91.8	0.06 (1)	6.25	E- B	0 / 895	0.11 (1)
B- C	-12 / 0	-91.8	-91.8	0.06 (1)	6.25	B- D	-1211 / 0	0.14 (1)
D- C	-109 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.08 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.08 (1)	10.00			
E- H	0 / 963	-18.5	-18.5	0.22 (1)	10.00			
H- D	0 / 963	-18.5	-18.5	0.22 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-4-12	-502	-502	---	BACK	VERT	TOTAL	---	C1
H	4-4-12	-502	-502	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.22/1.00 (D-E:1) , WB=0.14/1.00 (B-D:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (A) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	T13Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

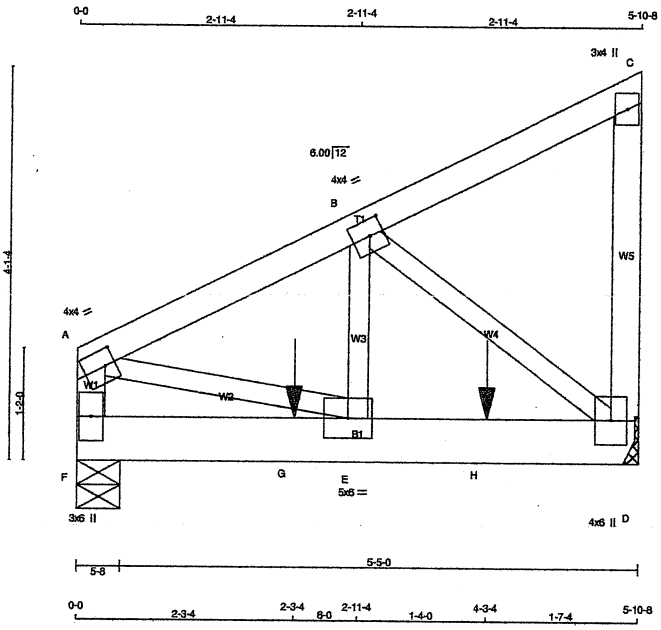
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115279

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410967	T13Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon May 24 12:26:24 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-Ft460RZmNx0KFy53GSSySHEnemwBdKlcWtFXYPzDIlD



TOTAL WEIGHT = 2 X 29 = 58 lb

[M

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		



Structural component only
DWG# T-2115286

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	958	0	958	0	0	5-8	5-8
D	1120	0	1120	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	674	463 / 0	0 / 0	0 / 0	0 / 0	211 / 0	0 / 0
D	787	543 / 0	0 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-852 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 1006	0.12 (1)
A- B	-1076 / 0	-91.8	-91.8	0.06 (1)	6.25	E- B	0 / 908	0.11 (1)
B- C	-12 / 0	-91.8	-91.8	0.06 (1)	6.25	B- D	-1224 / 0	0.15 (1)
D- C	-109 / 0	0.0	0.0	0.01 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.09 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.09 (1)	10.00			
E- H	0 / 973	-18.5	-18.5	0.21 (1)	10.00			
H- D	0 / 973	-18.5	-18.5	0.21 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-3-4	-502	-502	---	BACK	VERT	TOTAL	---	C1
H	4-3-4	-502	-502	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.21/1.00 (D-E:1) , WB=0.15/1.00 (B-D:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (A) (INPUT = 0.90)
JSI METAL= 0.22 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410967	T13Z1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

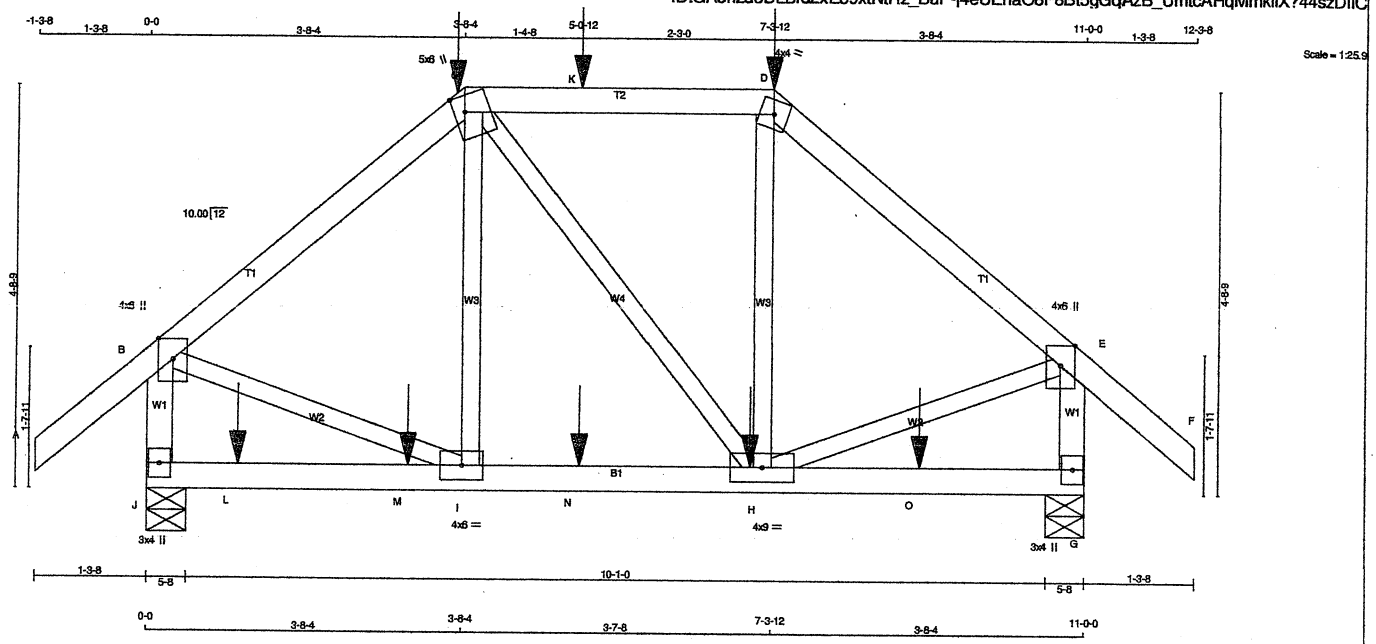


Structural component only
DWG# T-2115286

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410967	T20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon May 24 12:26:25 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-i4eUEnaO8F8Bt5gGqAzB_UmtcAHqMmklIX?44szDIIC



TOTAL WEIGHT = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1209	0	1209	0	0	5-8	5-8
G	1228	0	1228	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	852	576 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0
G	864	592 / 0	0 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.14 (1)	10.00
B- C	-952 / 0	-91.8	-91.8	0.26 (1)	6.00
C- K	-749 / 0	-91.8	-91.8	0.37 (1)	6.25
K- D	-749 / 0	-91.8	-91.8	0.37 (1)	6.25
D- E	-983 / 0	-91.8	-91.8	0.26 (1)	5.93
E- F	0 / 41	-91.8	-91.8	0.14 (1)	10.00
J- B	-1159 / 0	0.0	0.0	0.13 (1)	7.36
G- E	-1188 / 0	0.0	0.0	0.14 (1)	7.29
J- L	0 / 0	-18.5	-18.5	0.11 (4)	10.00
L- M	0 / 0	-18.5	-18.5	0.11 (4)	10.00
M- I	0 / 0	-18.5	-18.5	0.11 (4)	10.00
I- N	0 / 726	-18.5	-18.5	0.19 (1)	10.00
N- H	0 / 726	-18.5	-18.5	0.19 (1)	10.00
H- O	0 / 0	-18.5	-18.5	0.11 (4)	10.00
O- G	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-125 / 60	0.04 (1)
C- H	0 / 37	0.01 (1)
H- D	-166 / 50	0.06 (1)
B- I	0 / 771	0.19 (1)
H- E	0 / 796	0.20 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-8-4	-40	-40	---	FRONT	VERT	DEAD	---	C1
C	3-8-4	-170	-170	---	FRONT	VERT	SNOW	---	C1
D	7-3-12	-40	-40	---	FRONT	VERT	DEAD	---	C1
D	7-3-12	-81	-81	---	FRONT	VERT	TOTAL	---	C1
D	7-3-12	-170	-170	---	FRONT	VERT	SNOW	---	C1
H	7-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
K	5-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
L	1-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
M	3-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	5-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
O	9-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.19/1.00 (H-I:1) , WB=0.20/1.00 (E-H:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (E) (INPUT = 0.90)
JSI METAL= 0.49 (E) (INPUT = 1.00)

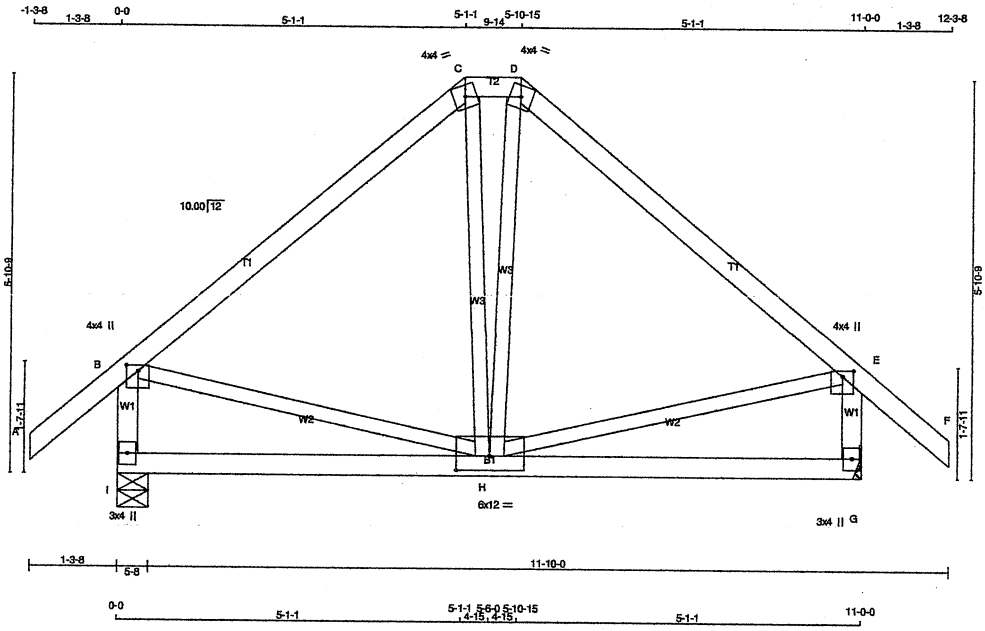
LICENSED PROFESSIONAL ENGINEER
05/24/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115287

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410967	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Mon May 24 12:26:26 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-BGBsR7b0vYG2VFFSOtUQXiJ3JadS5FyvzBkedlzDIlB



TOTAL WEIGHT = 53 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWWW*m	MT20	6.0	12.0	2.50	6.00
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	733	0	733	0	0	5-8	5-8
G	733	0	733	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
G	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41		-91.8	-91.8	0.13 (1)		10.00	B- H	0 / 319	0.07 (1)
B- C	-405 / 0		-91.8	-91.8	0.31 (1)		6.25	H- E	0 / 319	0.07 (1)
C- D	-310 / 0		-91.8	-91.8	0.01 (1)		6.25	C- H	-11 / 44	0.02 (4)
D- E	-405 / 0		-91.8	-91.8	0.31 (1)		6.25	H- D	-11 / 44	0.02 (4)
E- F	0 / 41		-91.8	-91.8	0.13 (1)		10.00			
I- B	-694 / 0		0.0	0.0	0.07 (1)		7.81			
G- E	-694 / 0		0.0	0.0	0.07 (1)		7.81			
I- H	0 / 0		-18.5	-18.5	0.16 (4)		10.00			
H- G	0 / 0		-18.5	-18.5	0.16 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.31/1.00 (D-E:1) , BC=0.16/1.00 (H-I:4) , WB=0.07/1.00 (B-H:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

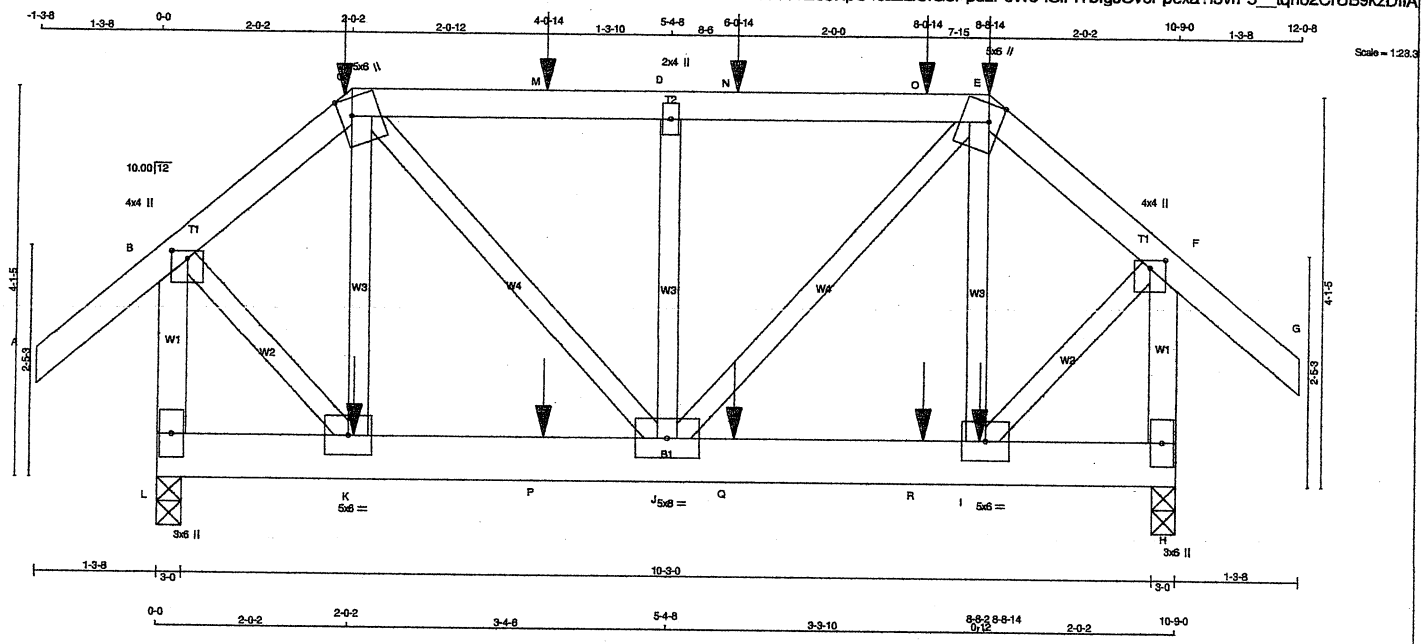


Structural component only
DWG# T-2115288

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410967	T22	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 61 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	939	0	939	0	0	3-0	3-0
H	971	0	971	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	661	450 / 0	0 / 0	0 / 0	0 / 0	211 / 0	0 / 0
H	683	466 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 41	-91.8	-91.8	0.14 (1)		10.00	K- C	-285 / 0	0.07 (1)
B- C	-518 / 0	-91.8	-91.8	0.07 (1)		6.25	C- J	0 / 365	0.09 (1)
C- M	-632 / 0	-91.8	-91.8	0.24 (1)		6.25	J- D	-478 / 0	0.12 (1)
M- D	-632 / 0	-91.8	-91.8	0.24 (1)		6.25	J- E	0 / 339	0.08 (1)
D- N	-633 / 0	-91.8	-91.8	0.24 (1)		6.25	I- E	-295 / 0	0.08 (1)
N- O	-633 / 0	-91.8	-91.8	0.24 (1)		6.25	B- K	0 / 546	0.14 (1)
O- E	-633 / 0	-91.8	-91.8	0.24 (1)		6.25	I- F	0 / 571	0.14 (1)
E- F	-541 / 0	-91.8	-91.8	0.07 (1)		6.25			
F- G	0 / 41	-91.8	-91.8	0.14 (1)		10.00			
L- B	-930 / 0	0.0	0.0	0.12 (1)		7.81			
H- F	-962 / 0	0.0	0.0	0.13 (1)		7.81			
L- K	0 / 0	-18.5	-18.5	0.02 (4)		10.00			
K- P	0 / 388	-18.5	-18.5	0.08 (1)		10.00			
P- J	0 / 388	-18.5	-18.5	0.08 (1)		10.00			
J- Q	0 / 405	-18.5	-18.5	0.08 (1)		10.00			
Q- R	0 / 405	-18.5	-18.5	0.08 (1)		10.00			
R- I	0 / 405	-18.5	-18.5	0.08 (1)		10.00			
I- H	0 / 0	-18.5	-18.5	0.02 (4)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-0-2	-9	-9	---	FRONT	VERT	DEAD	---	C1
C	2-0-2	-41	-41	---	BACK	VERT	TOTAL	---	C1
C	2-0-2	-36	-36	---	FRONT	VERT	SNOW	---	C1
E	8-8-14	-9	-9	---	FRONT	VERT	DEAD	---	C1
E	8-8-14	-41	-41	---	BACK	VERT	TOTAL	---	C1
E	8-8-14	-36	-36	---	FRONT	VERT	SNOW	---	C1
I	8-8-2	-8	-8	---	BACK	VERT	TOTAL	---	C1
K	2-0-14	-8	-8	---	BACK	VERT	TOTAL	---	C1
M	4-0-14	-36	-36	---	BACK	VERT	TOTAL	---	C1
N	6-0-14	-36	-36	---	BACK	VERT	TOTAL	---	C1
O	8-0-14	-46	-46	---	BACK	VERT	TOTAL	---	C1
P	4-0-14	-8	-8	---	BACK	VERT	TOTAL	---	C1
Q	6-0-14	-8	-8	---	BACK	VERT	TOTAL	---	C1
R	8-0-14	-8	-8	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.24/1.00 (D-E:1) , BC=0.08/1.00 (I-J:1) , WB=0.14/1.00 (F-I:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (F) (INPUT = 0.90)
JSI METAL= 0.19 (F) (INPUT = 1.00)

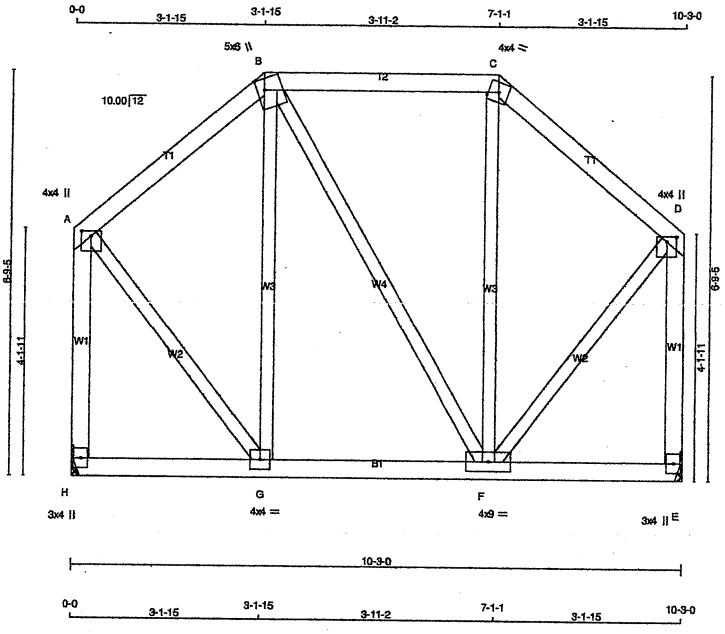
LICENSED PROFESSIONAL ENGINEER
05/24/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115289

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410967	T23	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 58 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	565	0	565	0	0	MECHANICAL	
E	565	0	565	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	400	262 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0
E	400	262 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-258 / 0	-91.8	-91.8	0.12 (1)	6.25	G- B	-158 / 0	0.12 (1)
B- C	-195 / 0	-91.8	-91.8	0.18 (1)	6.25	B- F	0 / 0	0.00 (1)
C- D	-257 / 0	-91.8	-91.8	0.12 (1)	6.25	F- C	-160 / 0	0.12 (1)
H- A	-541 / 0	0.0	0.0	0.16 (1)	7.81	A- G	0 / 303	0.07 (1)
E- D	-540 / 0	0.0	0.0	0.16 (1)	7.81	F- D	0 / 303	0.07 (1)
H- G	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
G- F	0 / 195	-18.5	-18.5	0.07 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.07/1.00 (F-G:4) , WB=0.12/1.00 (C-F:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (A) (INPUT = 0.90)
JSI METAL= 0.11 (A) (INPUT = 1.00)

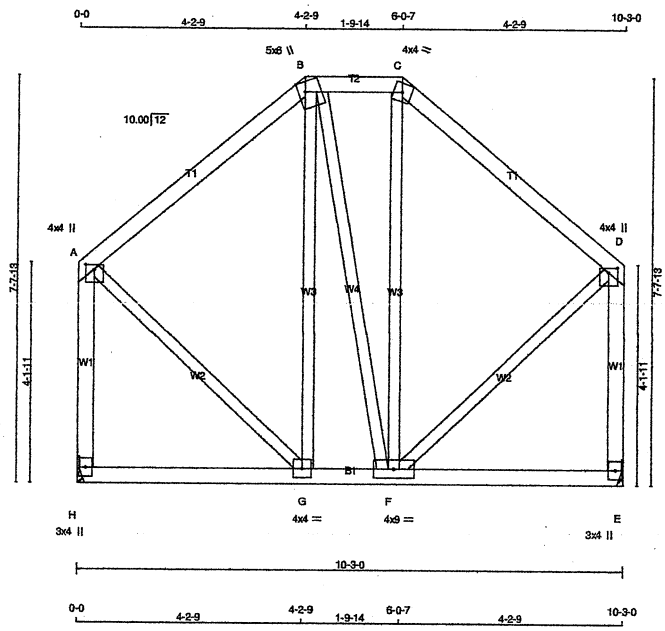
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO
05/24/21

Structural component only
DWG# T-2115290

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410967	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 61 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	565	0	565	0	0	MECHANICAL	
E	565	0	565	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	400	262 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0
E	400	262 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-257 / 0	-91.8	-91.8	0.21 (1)		6.25	G- B	-109 / 7	0.12 (1)
B- C	-194 / 0	-91.8	-91.8	0.04 (1)		6.25	B- F	-3 / 0	0.00 (1)
C- D	-256 / 0	-91.8	-91.8	0.21 (1)		6.25	F- C	-113 / 8	0.12 (1)
H- A	-532 / 0	0.0	0.0	0.15 (1)		7.81	A- G	0 / 262	0.06 (1)
E- D	-530 / 0	0.0	0.0	0.15 (1)		7.81	F- D	0 / 261	0.06 (1)
H- G	0 / 0	-18.5	-18.5	0.07 (4)		10.00			
G- F	0 / 195	-18.5	-18.5	0.08 (4)		10.00			
F- E	0 / 0	-18.5	-18.5	0.07 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (A-B:1) , BC=0.08/1.00 (F-G:4) , WB=0.12/1.00 (C-F:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (A) (INPUT = 0.90)
JSI METAL= 0.11 (A) (INPUT = 1.00)

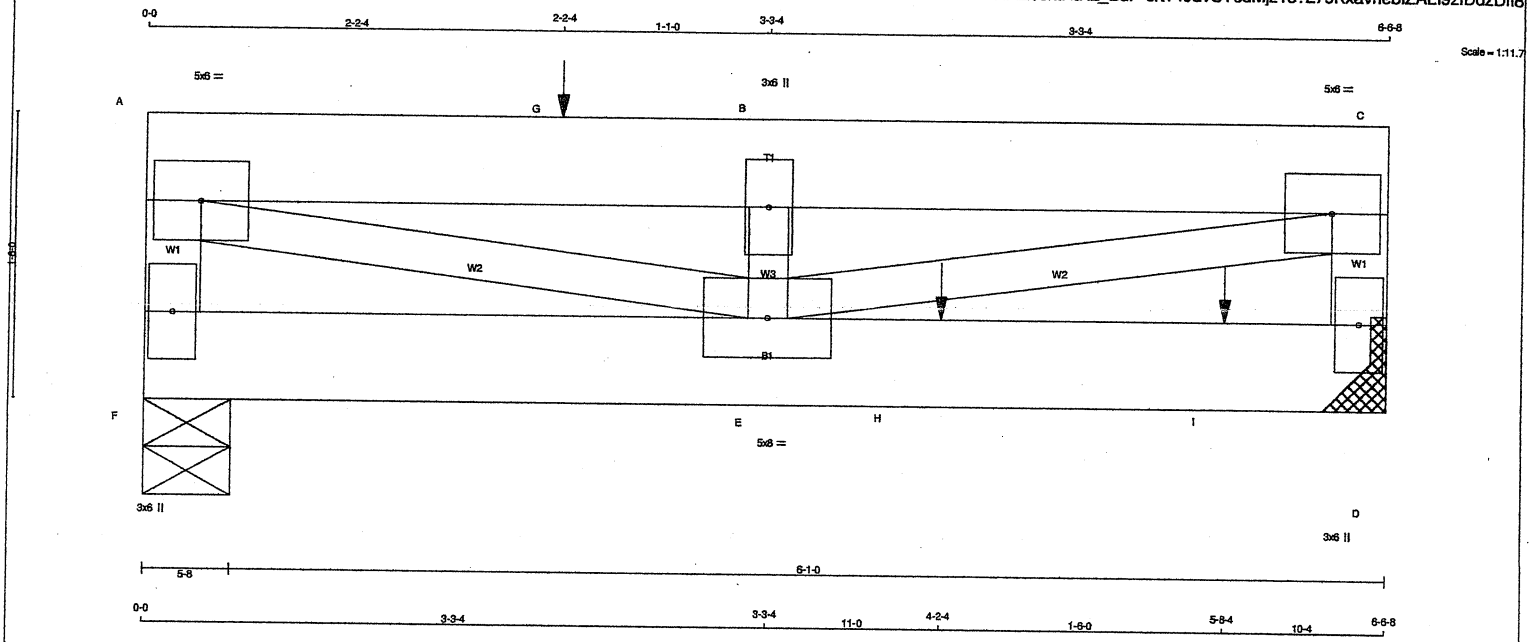


Structural component only
DWG# T-2115291

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410967	T25	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-crt?49dvCTedMjz13?279KxavneblZALf9zIDdzDll8



TOTAL WEIGHT = 4 X 30 = 120 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
C- D	1	12	TOP
A- C	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(14.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	5.0	6.0	
B	TMW+w	MT20	3.0	6.0	
C	TMVW-t	MT20	5.0	6.0	
D	BMV1+p	MT20	3.0	6.0	
E	BMWWW-t	MT20	5.0	8.0	



Structural component only
DWG# T-2115292

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1278	0	1278	0	0	5-8	5-8
D	1485	0	1485	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	908	569 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0
D	1050	689 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
F- A	-1168 / 0	0.0	0.0	0.06 (1)	7.81	A- E	0 / 2393	0.30 (1)
A- G	-2280 / 0	-91.8	-91.8	0.28 (1)	6.25	E- B	-843 / 0	0.07 (1)
G- B	-2280 / 0	-91.8	-91.8	0.28 (1)	6.25	E- C	0 / 2393	0.30 (1)
B- C	-2280 / 0	-91.8	-91.8	0.06 (1)	6.25			
D- C	-888 / 0	0.0	0.0	0.05 (1)	7.81			
F- E	0 / 0	-43.5	-43.5	0.05 (1)	10.00			
E- H	0 / 0	-43.5	-43.5	0.20 (1)	10.00			
H- I	0 / 0	-18.5	-18.5	0.20 (1)	10.00			
I- D	0 / 0	-18.5	-18.5	0.20 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-2-4	-598	-598	---	TOP	VERT	TOTAL	---	C1
H	4-2-4	-385	-385	---	BACK	VERT	TOTAL	---	C1
I	5-8-4	-386	-386	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.28/1.00 (A-B:1) , BC=0.20/1.00 (D-E:1) , WB=0.30/1.00 (A-E:1) , SSI=0.23/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (E) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410967	T25	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-crt?49dvCTedMjz13?279KxavneblZALf9zIDdzDII8

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

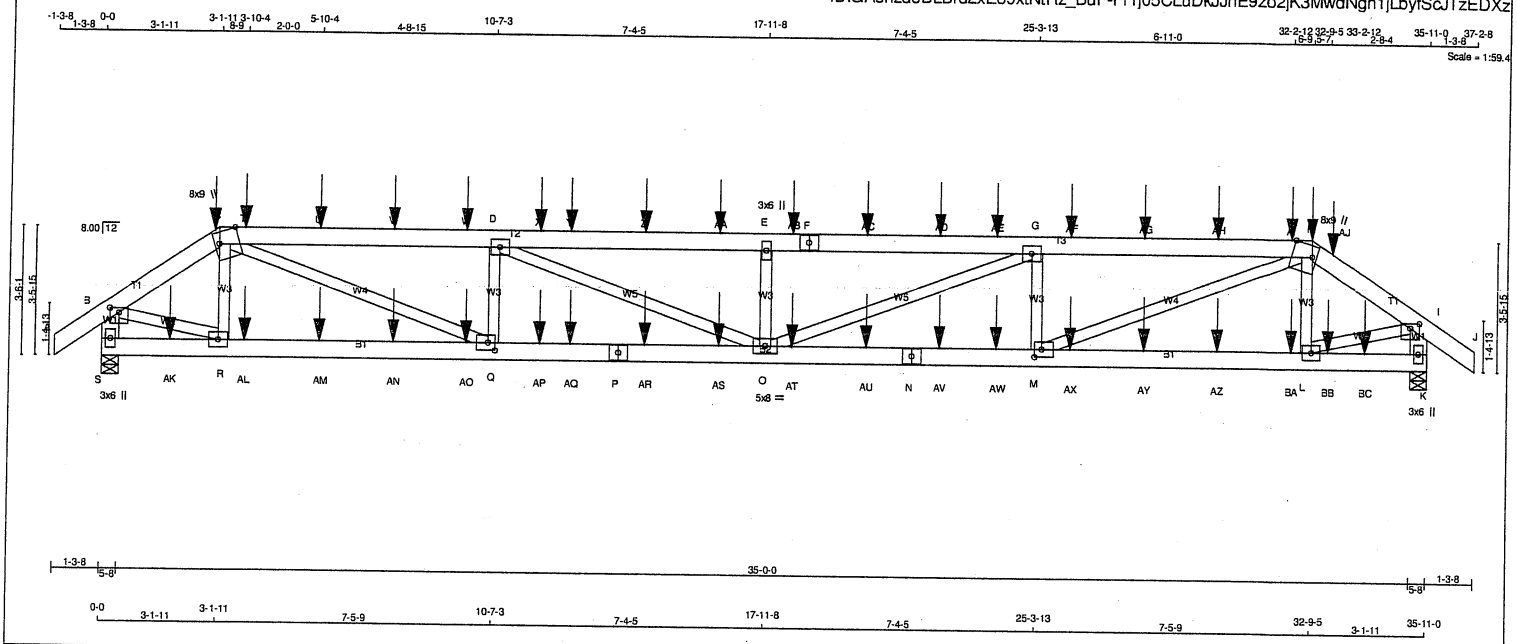
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115292

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
418973	T30	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-Fr1j05CLuDkJJhE9zo2jK3MwdNgh1jLbyfScJTzEDXz



TOTAL WEIGHT = 2 X 195 = 391 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- F	2	12	SIDE(61.0)
F- H	2	12	SIDE(61.0)
H- J	2	12	SIDE(122.0)
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	SIDE(0.0)
P- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.



Structural component only
DWG# T-2115334

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3484	0	3484	0	0	5-8	5-8
K	3622	0	3622	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2462	1626 / 0	0 / 0	0 / 0	0 / 0	836 / 0	0 / 0
K	2560	1691 / 0	0 / 0	0 / 0	0 / 0	869 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 36	-91.8	-91.8	0.04 (1)	10.00
B- C	-4113 / 0	-91.8	-91.8	0.06 (1)	5.57
C- T	-8562 / 0	-91.8	-91.8	0.46 (1)	3.80
T- U	-8562 / 0	-91.8	-91.8	0.46 (1)	3.80
U- V	-8562 / 0	-91.8	-91.8	0.46 (1)	3.80
V- W	-8562 / 0	-91.8	-91.8	0.46 (1)	3.80
W- D	-8562 / 0	-91.8	-91.8	0.46 (1)	3.80
D- X	-10110 / 0	-91.8	-91.8	0.53 (1)	3.47
X- Y	-10110 / 0	-91.8	-91.8	0.53 (1)	3.47
Y- Z	-10110 / 0	-91.8	-91.8	0.53 (1)	3.47
Z-AA	-10110 / 0	-91.8	-91.8	0.53 (1)	3.47
AA- E	-10110 / 0	-91.8	-91.8	0.53 (1)	3.47
E-AB	-10110 / 0	-91.8	-91.8	0.52 (1)	3.48
AB- F	-10110 / 0	-91.8	-91.8	0.52 (1)	3.48
F-AC	-10110 / 0	-91.8	-91.8	0.52 (1)	3.48
AC-AD	-10110 / 0	-91.8	-91.8	0.52 (1)	3.48
AD-AE	-10110 / 0	-91.8	-91.8	0.52 (1)	3.48
AE- G	-10110 / 0	-91.8	-91.8	0.52 (1)	3.48
G-AF	-8565 / 0	-91.8	-91.8	0.45 (1)	3.81
AF-AG	-8565 / 0	-91.8	-91.8	0.45 (1)	3.81
AG-AH	-8565 / 0	-91.8	-91.8	0.45 (1)	3.81
AH-AI	-8565 / 0	-91.8	-91.8	0.45 (1)	3.81
AI- H	-8565 / 0	-91.8	-91.8	0.45 (1)	3.81
H-AJ	-4228 / 0	-91.8	-91.8	0.08 (1)	5.50
AJ- I	-4228 / 0	-91.8	-91.8	0.08 (1)	5.50
I- J	0 / 36	-91.8	-91.8	0.04 (1)	10.00
S- B	-3503 / 0	0.0	0.0	0.12 (1)	7.48
K- I	-3634 / 0	0.0	0.0	0.13 (1)	7.37
S-AK	0 / 0	-18.5	-18.5	0.08 (4)	10.00
AK- R	0 / 0	-18.5	-18.5	0.08 (4)	10.00
R-AL	0 / 3378	-18.5	-18.5	0.29 (1)	10.00
AL-AM	0 / 3378	-18.5	-18.5	0.29 (1)	10.00
AM-AN	0 / 3378	-18.5	-18.5	0.29 (1)	10.00
AN-AO	0 / 3378	-18.5	-18.5	0.29 (1)	10.00
AO- Q	0 / 3378	-18.5	-18.5	0.29 (1)	10.00
Q-AP	0 / 8562	-18.5	-18.5	0.65 (1)	10.00
AP-AQ	0 / 8562	-18.5	-18.5	0.65 (1)	10.00
AQ- P	0 / 8562	-18.5	-18.5	0.65 (1)	10.00
P-AR	0 / 8562	-18.5	-18.5	0.65 (1)	10.00
AR-AS	0 / 8562	-18.5	-18.5	0.65 (1)	10.00
AS- O	0 / 8562	-18.5	-18.5	0.65 (1)	10.00
O-AT	0 / 8566	-18.5	-18.5	0.64 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-672 / 0	0.05 (1)
C- Q	0 / 5591	0.49 (1)
Q- D	-1890 / 0	0.13 (1)
D- O	0 / 1675	0.15 (1)
O- E	-1026 / 0	0.07 (1)
O- G	0 / 1671	0.15 (1)
M- G	-1852 / 0	0.13 (1)
M- H	0 / 5468	0.48 (1)
L- H	-682 / 0	0.05 (1)
B- R	0 / 3535	0.31 (1)
L- I	0 / 3657	0.32 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 788 (0.55")

CSI: TC=0.53/1.00 (D-E:1) , BC=0.65/1.00 (O-Q:1) , WB=0.49/1.00 (C-Q:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.83 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418973	T30	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge	6.50
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AT-AU	0 / 8566	-18.5	-18.5	0.64 (1)	10.00			
AU- N	0 / 8566	-18.5	-18.5	0.64 (1)	10.00			
N-AV	0 / 8566	-18.5	-18.5	0.64 (1)	10.00			
AV-AW	0 / 8566	-18.5	-18.5	0.64 (1)	10.00			
AW- M	0 / 8566	-18.5	-18.5	0.64 (1)	10.00			
M-AX	0 / 3495	-18.5	-18.5	0.30 (1)	10.00			
AX-AY	0 / 3495	-18.5	-18.5	0.30 (1)	10.00			
AY-AZ	0 / 3495	-18.5	-18.5	0.30 (1)	10.00			
AZ-BA	0 / 3495	-18.5	-18.5	0.30 (1)	10.00			
BA- L	0 / 3495	-18.5	-18.5	0.30 (1)	10.00			
L-BB	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
BB-BC	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
BC- K	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-1-11	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	3-1-11	-127	-127	---	FRONT	VERT	SNOW	---	C1
H	32-9-5	-30	-30	---	FRONT	VERT	DEAD	---	C1
H	32-9-5	-127	-127	---	FRONT	VERT	SNOW	---	C1
T	3-10-4	-85	-85	---	BACK	VERT	TOTAL	---	C1
U	5-10-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
V	7-10-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
W	9-10-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
X	11-10-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
Y	12-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
Z	14-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AA	16-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AB	18-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AC	20-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AD	22-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AE	24-2-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AF	26-2-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AG	28-2-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AH	30-2-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AI	32-2-12	-90	-90	---	BACK	VERT	TOTAL	---	C1
AJ	33-2-12	-93	-93	---	BACK	VERT	TOTAL	---	C1
AK	1-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AL	3-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AM	5-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AN	7-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AO	9-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AP	11-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AQ	12-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AR	14-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AS	16-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AT	18-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AU	20-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AV	22-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AW	24-2-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AX	26-2-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AY	28-2-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AZ	30-2-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
BA	32-2-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
BB	33-2-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
BC	34-2-12	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

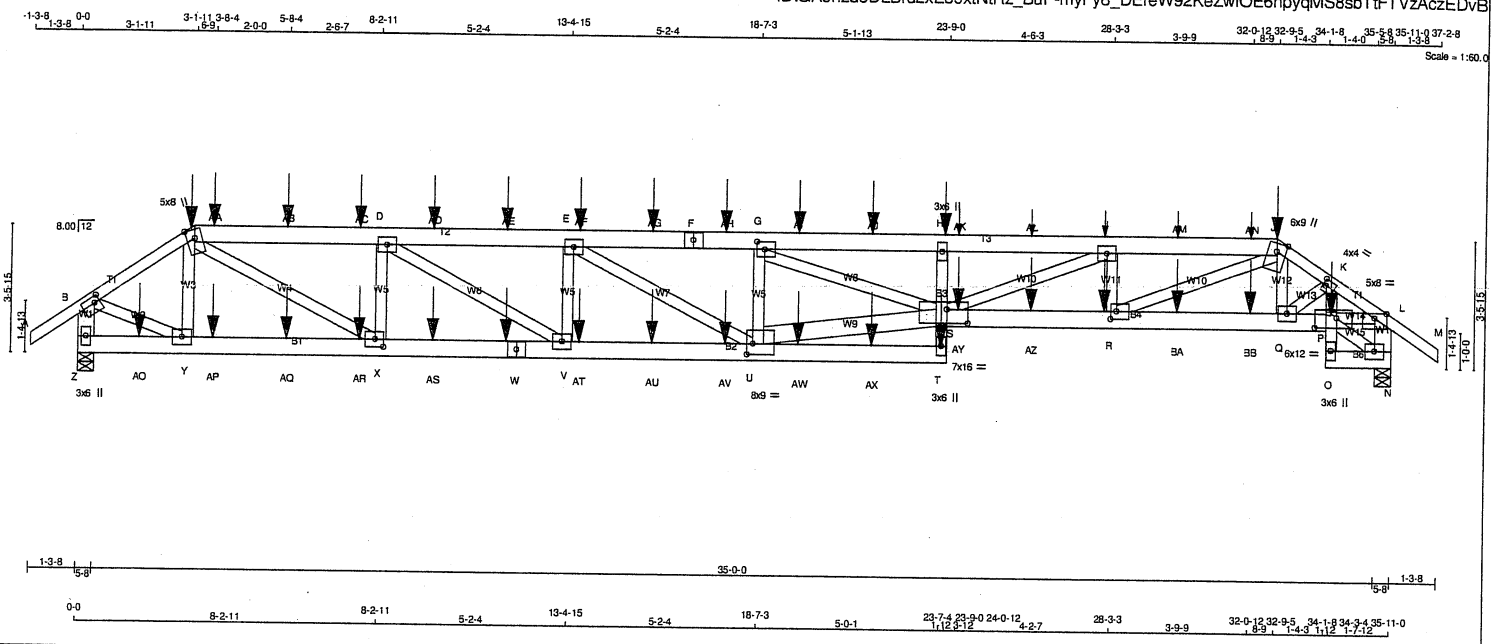


Structural component only
DWG# T-2115334

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410969	T30S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Z - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Z - W	2x6	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - H	2x4	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
J- M	1	12	SIDE(61.0)
C- F	2	12	SIDE(183.1)
F- J	2	12	SIDE(183.1)
Z- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Z- W	2	12	SIDE(0.0)
W- T	2	12	SIDE(183.1)
S- P	2	12	SIDE(183.1)
O- N	2	12	TOP
T- H	1	12	TOP
K- O	1	12	SIDE(12.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.



Structural component only
DWG# T-2115304

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Z	3586	0	3586	0	0	5-8	5-8
N	4135	0	4135	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	2535	1670 / 0	0 / 0	0 / 0	0 / 0	865 / 0	0 / 0
N	2926	1911 / 0	0 / 0	0 / 0	0 / 0	1016 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B- C	-3988 / 0	-91.8	-91.8	0.14 (1)	4.64
C-AA	-7432 / 0	-91.8	-91.8	0.24 (1)	4.25
AA-AB	-7432 / 0	-91.8	-91.8	0.24 (1)	4.25
AB-AC	-7432 / 0	-91.8	-91.8	0.24 (1)	4.25
AC- D	-7432 / 0	-91.8	-91.8	0.24 (1)	4.25
D-AD	-9788 / 0	-91.8	-91.8	0.28 (1)	3.75
AD-AE	-9788 / 0	-91.8	-91.8	0.28 (1)	3.75
AE- E	-9788 / 0	-91.8	-91.8	0.28 (1)	3.75
E-AF	-10740 / 0	-91.8	-91.8	0.35 (1)	3.53
AF-AG	-10740 / 0	-91.8	-91.8	0.35 (1)	3.53
AG- F	-10740 / 0	-91.8	-91.8	0.35 (1)	3.53
F-AH	-10740 / 0	-91.8	-91.8	0.35 (1)	3.53
AH- G	-10740 / 0	-91.8	-91.8	0.35 (1)	3.53
G-AI	-14527 / 0	-91.8	-91.8	0.58 (1)	2.85
AI-AJ	-14527 / 0	-91.8	-91.8	0.58 (1)	2.85
AJ- H	-14527 / 0	-91.8	-91.8	0.58 (1)	2.85
H-AK	-14768 / 0	-91.8	-91.8	0.56 (1)	2.84
AK-AL	-14768 / 0	-91.8	-91.8	0.56 (1)	2.84
AL- I	-14768 / 0	-91.8	-91.8	0.56 (1)	2.84
I-AM	-11374 / 0	-91.8	-91.8	0.33 (1)	3.46
AM-AN	-11374 / 0	-91.8	-91.8	0.33 (1)	3.46
AN- J	-11374 / 0	-91.8	-91.8	0.33 (1)	3.46
J- K	-6814 / 0	-91.8	-91.8	0.27 (1)	3.53
K- L	-6909 / 0	-91.8	-91.8	0.28 (1)	3.50
L- M	0 / 35	-91.8	-91.8	0.07 (1)	10.00
Z- B	-3571 / 0	0.0	0.0	0.13 (1)	7.42
N- L	-3963 / 0	0.0	0.0	0.14 (1)	7.13
Z-AO	0 / 0	-18.5	-18.5	0.04 (4)	10.00
AO- Y	0 / 0	-18.5	-18.5	0.04 (4)	10.00
Y-AP	0 / 3376	-18.5	-18.5	0.26 (1)	10.00
AP-AQ	0 / 3376	-18.5	-18.5	0.26 (1)	10.00
AQ-AR	0 / 3376	-18.5	-18.5	0.26 (1)	10.00
AR- X	0 / 3376	-18.5	-18.5	0.26 (1)	10.00
X-AS	0 / 7432	-18.5	-18.5	0.54 (1)	10.00
AS- W	0 / 7432	-18.5	-18.5	0.54 (1)	10.00
W- V	0 / 7432	-18.5	-18.5	0.54 (1)	10.00
V-AT	0 / 9788	-18.5	-18.5	0.70 (1)	10.00
AT-AU	0 / 9788	-18.5	-18.5	0.70 (1)	10.00
AU-AV	0 / 9788	-18.5	-18.5	0.70 (1)	10.00
AV- U	0 / 9788	-18.5	-18.5	0.70 (1)	10.00
U-AW	0 / 491	-18.5	-18.5	0.07 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
Y- C	-934 / 0	0.07 (1)
U- G	-2414 / 0	0.17 (1)
U- S	0 / 10567	0.69 (1)
G- S	0 / 3954	0.35 (1)
S- I	0 / 3704	0.33 (1)
R- I	-2017 / 0	0.12 (1)
R- J	0 / 6210	0.55 (1)
Q- J	0 / 599	0.05 (1)
Q- K	-200 / 0	0.01 (1)
B- Y	0 / 3511	0.31 (1)
P- N	-293 / 0	0.02 (1)
P- L	0 / 5679	0.50 (1)
E- U	0 / 1109	0.10 (1)
C- X	0 / 4725	0.42 (1)
V- E	-1260 / 0	0.09 (1)
X- D	-2278 / 0	0.16 (1)
D- V	0 / 2741	0.24 (1)

TOTAL WEIGHT = 2 X 205 = 410 lb [M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	25.6 PSF
		DL	=	6.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.4 PSF
TOTAL	LOAD		=	39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.41")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 567 (0.76")

CSI: TC=0.58/1.00 (G-H:1) , BC=0.89/1.00 (R-S:1) , WB=0.69/1.00 (S-U:1) , SSI=0.21/1.00 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410969	T30S	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	1.75
C	TTWW+m	MT20	5.0	8.0	Edge	
D, E, I						
D	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	6.0		
J	TTWW+m	MT20	6.0	9.0	Edge	
K	TMVW-t	MT20	4.0	4.0	2.00	1.00
L	TMVW-p	MT20	5.0	8.0	Edge	
N	BMVW1-t	MT20	5.0	6.0		
O	BMV+p	MT20	3.0	6.0		
P	BVMWW-t	MT20	6.0	12.0	3.25	7.00
Q, V, Y						
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BVMWWW-t	MT20	7.0	16.0	4.50	7.00
T	BMV+p	MT20	3.0	6.0		
U	BMWWW-t	MT20	8.0	9.0	3.50	2.00
W	BS-t	MT20	5.0	6.0		
X	BMWW-t	MT20	5.0	6.0	2.50	2.75
Z	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
AW-AX	0 / 491	-18.5	-18.5	0.07 (1)	10.00			
AX- T	0 / 491	-18.5	-18.5	0.07 (1)	10.00			
T- S	0 / 109	0.0	0.0	0.38 (1)	10.00			
S- H	-460 / 0	0.0	0.0	0.37 (1)	7.81			
S-AY	0 / 11374	-18.5	-18.5	0.89 (1)	10.00			
AY-AZ	0 / 11374	-18.5	-18.5	0.89 (1)	10.00			
AZ- R	0 / 11374	-18.5	-18.5	0.89 (1)	10.00			
R-BA	0 / 5727	-18.5	-18.5	0.50 (1)	10.00			
BA-BB	0 / 5727	-18.5	-18.5	0.50 (1)	10.00			
BB- Q	0 / 5727	-18.5	-18.5	0.50 (1)	10.00			
Q- P	0 / 5919	-18.5	-18.5	0.51 (1)	10.00			
O- P	0 / 17	0.0	0.0	0.20 (1)	10.00			
P- K	0 / 295	0.0	0.0	0.22 (1)	10.00			
O- N	0 / 247	-18.5	-18.5	0.02 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-1-11	-31	-31	---	FRONT	VERT	DEAD	---	C1
C	3-1-11	-128	-128	---	FRONT	VERT	SNOW	---	C1
H	23-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
I	28-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
J	32-9-5	-31	-31	---	FRONT	VERT	DEAD	---	C1
J	32-9-5	-128	-128	---	FRONT	VERT	SNOW	---	C1
P	34-3-4	-204	-204	---	FRONT	VERT	TOTAL	---	C1
R	28-0-12	-171	-171	---	FRONT	VERT	TOTAL	---	C1
T	23-7-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
W	11-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AA	3-8-4	-90	-90	---	FRONT	VERT	TOTAL	---	C1
AB	5-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	7-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AD	9-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AE	11-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AF	13-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AG	15-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AH	17-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AI	19-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AJ	21-8-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AK	24-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
AL	26-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
AM	30-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
AN	32-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
AO	1-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	3-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	5-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	7-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	9-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	13-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	15-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	17-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AW	19-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AX	21-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AY	24-0-12	-175	-175	---	FRONT	VERT	TOTAL	---	C1
AZ	26-0-12	-171	-171	---	FRONT	VERT	TOTAL	---	C1
BA	30-0-12	-171	-171	---	FRONT	VERT	TOTAL	---	C1
BB	32-0-12	-171	-171	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.89 (S) (INPUT = 1.00)

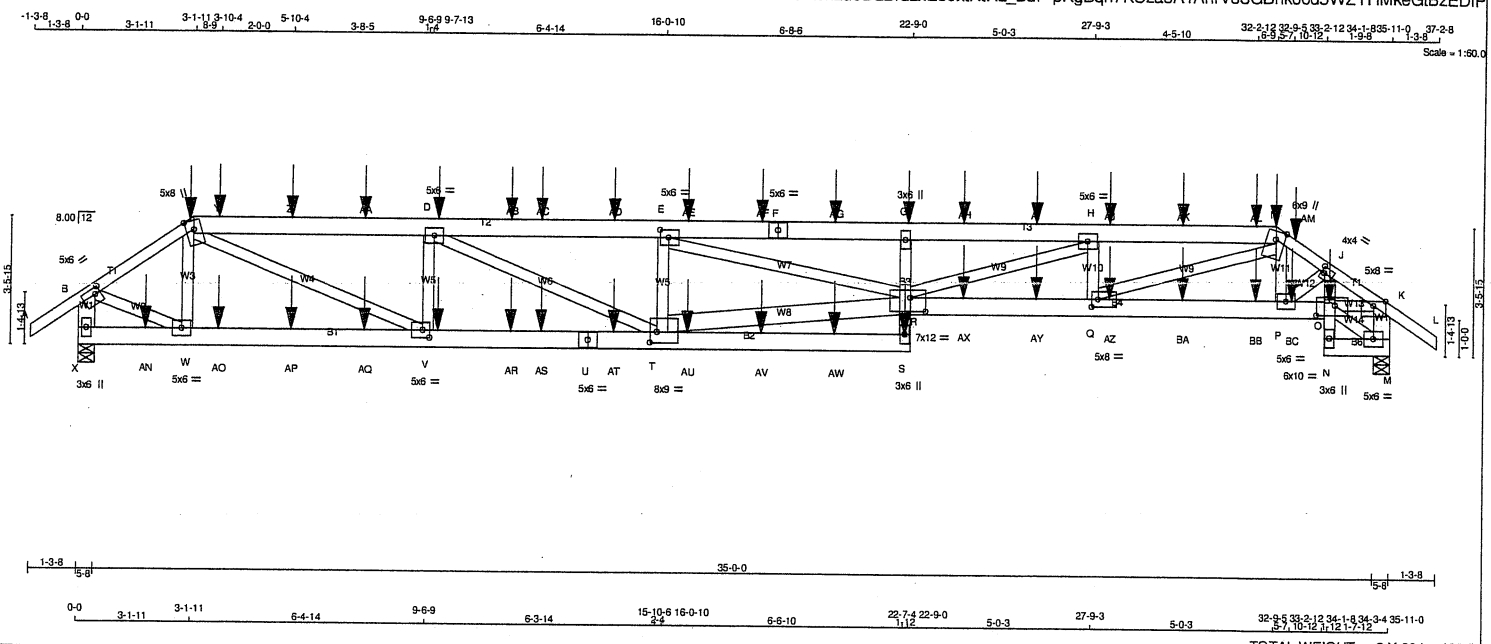
LICENSED PROFESSIONAL ENGINEER
05/12/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115304

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410970	T30S1	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
I- L	1	12	SIDE(61.0)
C- F	2	12	SIDE(183.1)
F- I	2	12	SIDE(183.1)
X- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X- U	2	12	SIDE(183.1)
U- S	2	12	SIDE(122.0)
R- O	2	12	SIDE(183.1)
N- M	2	12	TOP
S- G	1	12	TOP
J- N	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.



Structural component only
DWG# T-2115328

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	3555	0	3555	0	0	5-8	5-8
M	3939	0	3939	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	2512	1657 / 0	0 / 0	0 / 0	0 / 0	856 / 0	0 / 0
M	2784	1833 / 0	0 / 0	0 / 0	0 / 0	951 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B- C	-3982 / 0	-91.8	-91.8	0.14 (1)	4.64
C- Y	-8209 / 0	-91.8	-91.8	0.36 (1)	3.96
Y- Z	-8209 / 0	-91.8	-91.8	0.36 (1)	3.96
Z-AA	-8209 / 0	-91.8	-91.8	0.36 (1)	3.96
AA- D	-8209 / 0	-91.8	-91.8	0.36 (1)	3.96
D-AB	-10367 / 0	-91.8	-91.8	0.44 (1)	3.51
AB-AC	-10367 / 0	-91.8	-91.8	0.44 (1)	3.51
AC-AD	-10367 / 0	-91.8	-91.8	0.44 (1)	3.51
AD- E	-10367 / 0	-91.8	-91.8	0.44 (1)	3.51
E-AE	-14383 / 0	-91.8	-91.8	0.66 (1)	2.83
AE-AF	-14383 / 0	-91.8	-91.8	0.66 (1)	2.83
AF- F	-14383 / 0	-91.8	-91.8	0.66 (1)	2.83
F-AG	-14383 / 0	-91.8	-91.8	0.66 (1)	2.83
AG- G	-14383 / 0	-91.8	-91.8	0.66 (1)	2.83
G-AH	-14681 / 0	-91.8	-91.8	0.58 (1)	2.84
AH-AI	-14681 / 0	-91.8	-91.8	0.58 (1)	2.84
AI- H	-14681 / 0	-91.8	-91.8	0.58 (1)	2.84
H-AJ	-11352 / 0	-91.8	-91.8	0.40 (1)	3.39
AJ-AK	-11352 / 0	-91.8	-91.8	0.40 (1)	3.39
AK-AL	-11352 / 0	-91.8	-91.8	0.40 (1)	3.39
AL- I	-11352 / 0	-91.8	-91.8	0.40 (1)	3.39
I-AM	-6565 / 0	-91.8	-91.8	0.24 (1)	3.63
AM- J	-6565 / 0	-91.8	-91.8	0.24 (1)	3.63
J- K	-6572 / 0	-91.8	-91.8	0.25 (1)	3.63
K- L	0 / 35	-91.8	-91.8	0.07 (1)	10.00
X- B	-3566 / 0	0.0	0.0	0.13 (1)	7.42
M- K	-3774 / 0	0.0	0.0	0.14 (1)	7.26
X-AN	0 / 0	-18.5	-18.5	0.07 (4)	10.00
AN- W	0 / 0	-18.5	-18.5	0.07 (4)	10.00
W-AO	0 / 3368	-18.5	-18.5	0.27 (1)	10.00
AO-AP	0 / 3368	-18.5	-18.5	0.27 (1)	10.00
AP-AQ	0 / 3368	-18.5	-18.5	0.27 (1)	10.00
AQ- V	0 / 3368	-18.5	-18.5	0.27 (1)	10.00
V-AR	0 / 8208	-18.5	-18.5	0.60 (1)	10.00
AR-AS	0 / 8208	-18.5	-18.5	0.60 (1)	10.00
AS- U	0 / 8208	-18.5	-18.5	0.60 (1)	10.00
U-AT	0 / 8208	-18.5	-18.5	0.60 (1)	10.00
AT- T	0 / 8208	-18.5	-18.5	0.60 (1)	10.00
T-AU	0 / 607	-18.5	-18.5	0.10 (4)	10.00
AU-AV	0 / 607	-18.5	-18.5	0.10 (4)	10.00
AV-AW	0 / 607	-18.5	-18.5	0.10 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
W- C	-879 / 0	0.06 (1)
R- H	0 / 3580	0.32 (1)
Q- H	-2204 / 0	0.13 (1)
Q- I	0 / 6288	0.56 (1)
P- I	0 / 401	0.04 (1)
P- J	-93 / 0	0.01 (1)
B- W	0 / 3505	0.31 (1)
O- M	-277 / 0	0.02 (1)
O- K	0 / 5405	0.48 (1)
C- V	0 / 5359	0.47 (1)
T- E	-2287 / 0	0.16 (1)
V- D	-2134 / 0	0.15 (1)
D- T	0 / 2401	0.21 (1)
T- R	0 / 10009	0.65 (1)
E- R	0 / 4055	0.36 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 963 (0.45")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 516 (0.84")

CSI: TC=0.66/1.00 (E-G:1) , BC=0.86/1.00 (Q-R:1) , WB=0.65/1.00 (R-T:1) , SSI=0.26/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410970	T30S1	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	1.75
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0	2.50	2.75
F	TS-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	
J	TMVW-t	MT20	4.0	4.0	2.00	1.00
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-I	MT20	6.0	10.0	3.25	6.00
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	2.00
R	BVMWWW-I	MT20	7.0	12.0	4.50	5.25
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	8.0	9.0	3.50	2.25
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.25
W	BMWW-t	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
AW- S	0 / 607	-18.5	-18.5	0.10 (4)	10.00
S- R	0 / 127	0.0	0.0	0.47 (1)	10.00
R- G	-754 / 0	0.0	0.0	0.46 (1)	7.81
R-AX	0 / 11352	-18.5	-18.5	0.86 (1)	10.00
AX-AY	0 / 11352	-18.5	-18.5	0.86 (1)	10.00
AY- Q	0 / 11352	-18.5	-18.5	0.86 (1)	10.00
Q-AZ	0 / 5537	-18.5	-18.5	0.46 (1)	10.00
AZ-BA	0 / 5537	-18.5	-18.5	0.46 (1)	10.00
BA-BB	0 / 5537	-18.5	-18.5	0.46 (1)	10.00
BB- P	0 / 5537	-18.5	-18.5	0.46 (1)	10.00
P-BC	0 / 5633	-18.5	-18.5	0.47 (1)	10.00
BC- O	0 / 5633	-18.5	-18.5	0.47 (1)	10.00
N- O	0 / 17	0.0	0.0	0.18 (1)	10.00
O- J	0 / 106	0.0	0.0	0.19 (1)	10.00
N- M	0 / 234	-18.5	-18.5	0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-1-11	-30	-30	---	FRONT	VERT	DEAD	---	C1
C	3-1-11	-127	-127	---	FRONT	VERT	SNOW	---	C1
D	9-10-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
G	22-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
I	32-9-5	-30	-30	---	FRONT	VERT	DEAD	---	C1
I	32-9-5	-127	-127	---	FRONT	VERT	SNOW	---	C1
O	34-3-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
S	22-7-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
V	9-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	3-10-4	-85	-85	---	BACK	VERT	TOTAL	---	C1
Z	5-10-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AA	7-10-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AB	11-10-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AC	12-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AD	14-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AE	16-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AF	18-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AG	20-8-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AH	24-2-12	-79	-79	---	BACK	VERT	TOTAL	---	C1
AI	26-2-12	-79	-79	---	BACK	VERT	TOTAL	---	C1
AJ	28-2-12	-79	-79	---	BACK	VERT	TOTAL	---	C1
AK	30-2-12	-79	-79	---	BACK	VERT	TOTAL	---	C1
AL	32-2-12	-93	-93	---	BACK	VERT	TOTAL	---	C1
AM	33-2-12	-96	-96	---	BACK	VERT	TOTAL	---	C1
AN	1-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AO	3-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AP	5-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AQ	7-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AR	11-10-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AS	12-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AT	14-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AU	16-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AV	18-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AW	20-8-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AX	24-2-12	-63	-63	---	BACK	VERT	TOTAL	---	C1
AY	26-2-12	-63	-63	---	BACK	VERT	TOTAL	---	C1
AZ	28-2-12	-63	-63	---	BACK	VERT	TOTAL	---	C1
BA	30-2-12	-63	-63	---	BACK	VERT	TOTAL	---	C1
BB	32-2-12	-63	-63	---	BACK	VERT	TOTAL	---	C1
BC	33-2-12	-63	-63	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.78 (I) (INPUT = 1.00)

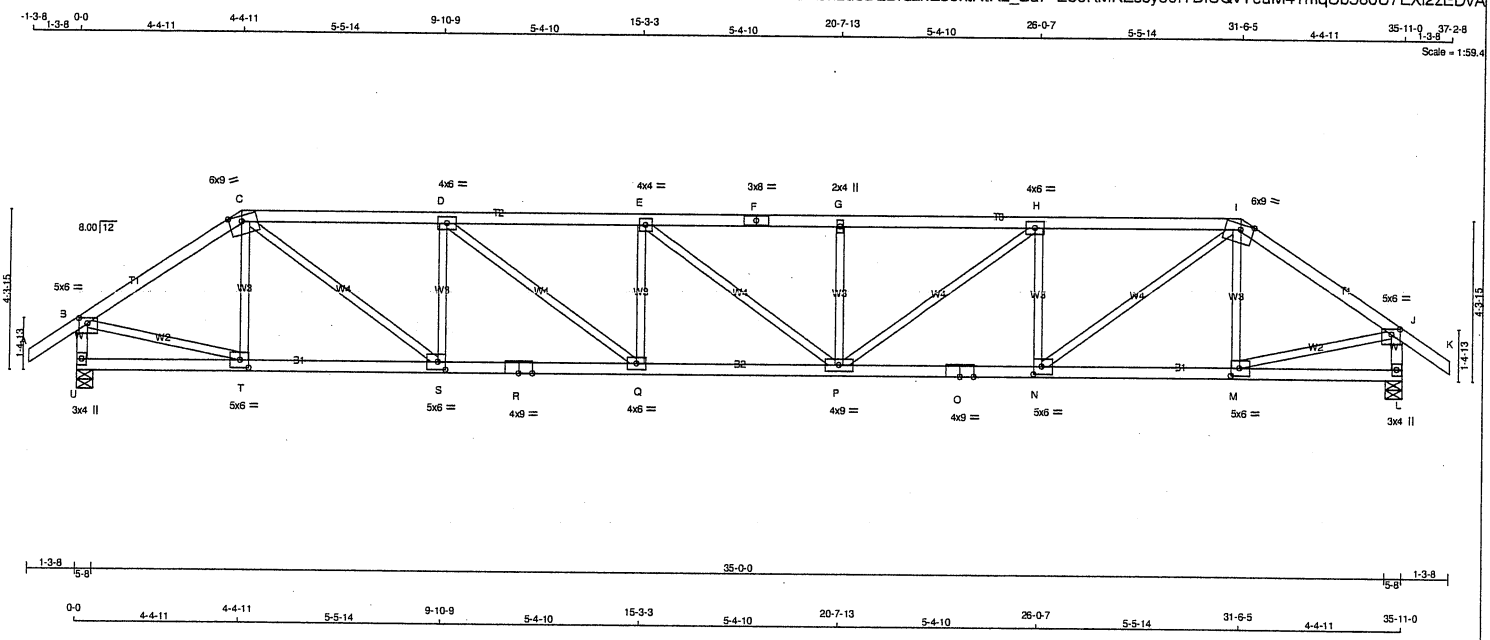
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115328

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:08:03 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-E8oKMKEscye0fTDIUQvTeuM41mqUb580U7EXi2zEDvA



TOTAL WEIGHT = 141 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TTWW-m	MT20	6.0	9.0	1.75	4.00
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW-m	MT20	6.0	9.0	1.75	4.00
J	TMVW-p	MT20	5.0	6.0	1.75	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BS-t	MT20	4.0	9.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BS-t	MT20	4.0	9.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115305

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2106	0	2106	0	0	5-8	5-8
L	2106	0	2106	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2310 / 0	-91.8	-91.8	0.42 (1)	4.10
C- D	-3585 / 0	-91.8	-91.8	0.68 (1)	3.16
D- E	-4359 / 0	-91.8	-91.8	0.80 (1)	2.78
E- F	-4358 / 0	-91.8	-91.8	0.67 (1)	2.94
F- G	-4358 / 0	-91.8	-91.8	0.67 (1)	2.94
G- H	-4358 / 0	-91.8	-91.8	0.80 (1)	2.77
H- I	-3586 / 0	-91.8	-91.8	0.68 (1)	3.16
I- J	-2310 / 0	-91.8	-91.8	0.42 (1)	4.10
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-2074 / 0	0.0	0.0	0.21 (1)	5.87
L- J	-2074 / 0	0.0	0.0	0.21 (1)	5.87
U- T	0 / 0	-18.5	-18.5	0.11 (4)	10.00
T- S	0 / 1910	-18.5	-18.5	0.37 (1)	10.00
S- R	0 / 3585	-18.5	-18.5	0.63 (1)	10.00
R- Q	0 / 3585	-18.5	-18.5	0.63 (1)	10.00
Q- P	0 / 4359	-18.5	-18.5	0.76 (1)	10.00
P- O	0 / 3586	-18.5	-18.5	0.63 (1)	10.00
O- N	0 / 3586	-18.5	-18.5	0.63 (1)	10.00
N- M	0 / 1910	-18.5	-18.5	0.37 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-356 / 0	0.10 (1)
C- S	0 / 2077	0.47 (1)
S- D	-1131 / 0	0.32 (1)
D- Q	0 / 967	0.22 (1)
Q- E	-480 / 0	0.13 (1)
E- P	-2 / 0	0.00 (1)
P- G	-479 / 0	0.13 (1)
P- H	0 / 965	0.22 (1)
N- H	-1131 / 0	0.32 (1)
N- I	0 / 2078	0.47 (1)
M- I	-357 / 0	0.10 (1)
B- T	0 / 1974	0.44 (1)
M- J	0 / 1974	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 776 (0.56")

CSI: TC=0.80/1.00 (G-H:1) , BC=0.76/1.00 (P-Q:1) , WB=0.47/1.00 (I-N:1) , SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

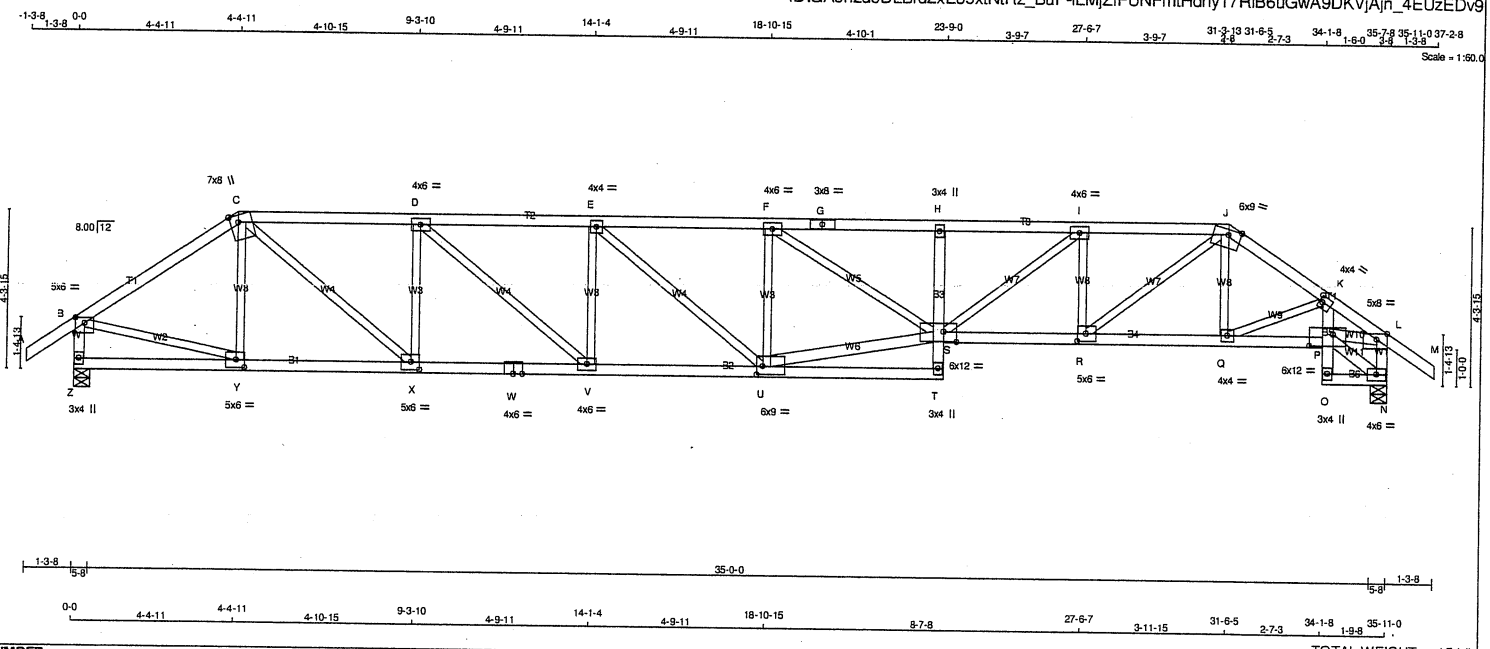
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.82 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T31S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-iLMjZfFUNFmtHdny17RiB6uGwA9DKVjAjn_4EUzEDv9



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Z - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Z - W	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - H	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D, F, I						
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMV+p	MT20	3.0	4.0		
J	TTWW-m	MT20	6.0	9.0	1.75	4.00
K	TMVW-t	MT20	4.0	4.0	2.00	1.00
L	TMVW-p	MT20	5.0	8.0	Edge	3.50
N	BMVW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWW-I	MT20	6.0	12.0	3.75	7.75
Q	BMWW-t	MT20	4.0	4.0		
R, X, Y						
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BVMWWW-I	MT20	6.0	12.0	3.25	4.25
T	BMV+p	MT20	3.0	4.0		
U	BMWWW-t	MT20	6.0	9.0	2.75	2.00
V	BMWW-t	MT20	4.0	6.0		



Structural component only
DWG# T-2115306

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	2103	0	2103	0	0	5-8	5-8
N	2108	0	2108	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Z	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
N	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	1 (1)	0.12	10.00
B- C	-2305 / 0	-91.8	-91.8	1 (1)	0.42	4.10
C- D	-3432 / 0	-91.8	-91.8	1 (1)	0.53	3.37
D- E	-4231 / 0	-91.8	-91.8	1 (1)	0.63	2.96
E- F	-4430 / 0	-91.8	-91.8	1 (1)	0.61	2.94
F- G	-5229 / 0	-91.8	-91.8	1 (1)	0.73	2.59
G- H	-5229 / 0	-91.8	-91.8	1 (1)	0.73	2.59
H- I	-5286 / 0	-91.8	-91.8	1 (1)	0.58	2.69
I- J	-4207 / 0	-91.8	-91.8	1 (1)	0.45	3.14
J- K	-3014 / 0	-91.8	-91.8	1 (1)	0.25	3.81
K- L	-3365 / 0	-91.8	-91.8	1 (1)	0.24	3.64
L- M	0 / 35	-91.8	-91.8	1 (1)	0.12	10.00
Z- B	-2070 / 0	0.0	0.0	1 (1)	0.21	5.88
N- L	-2015 / 0	0.0	0.0	1 (1)	0.21	5.95
Z- Y	0 / 0	-18.5	-18.5	1 (4)	0.09	10.00
Y- X	0 / 1905	-18.5	-18.5	1 (1)	0.36	10.00
X- W	0 / 3432	-18.5	-18.5	1 (1)	0.60	10.00
W- V	0 / 3432	-18.5	-18.5	1 (1)	0.60	10.00
V- U	0 / 4231	-18.5	-18.5	1 (1)	0.72	10.00
U- T	0 / 174	-18.5	-18.5	1 (1)	0.11	10.00
T- S	0 / 40	0.0	0.0	1 (1)	0.24	10.00
S- H	-368 / 0	0.0	0.0	1 (1)	0.24	7.81
S- R	0 / 4207	-18.5	-18.5	1 (1)	0.73	10.00
R- Q	0 / 2511	-18.5	-18.5	1 (1)	0.44	10.00
Q- P	0 / 2892	-18.5	-18.5	1 (1)	0.50	10.00
O- P	0 / 17	0.0	0.0	1 (1)	0.18	10.00
P- K	0 / 203	0.0	0.0	1 (1)	0.21	10.00
O- N	0 / 112	-18.5	-18.5	1 (1)	0.03	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-362 / 0	0.10 (1)
U- F	-967 / 0	0.27 (1)
U- S	0 / 4368	0.70 (1)
F- S	0 / 928	0.21 (1)
Q- J	0 / 94	0.03 (4)
Q- K	-412 / 0	0.08 (1)
B- Y	0 / 1969	0.44 (1)
P- N	-137 / 0	0.01 (1)
P- L	0 / 2767	0.44 (1)
E- U	0 / 261	0.06 (1)
C- X	0 / 1973	0.44 (1)
V- E	-588 / 0	0.16 (1)
X- D	-1164 / 0	0.32 (1)
D- V	0 / 1044	0.23 (1)
R- J	0 / 2127	0.48 (1)
S- I	0 / 1364	0.31 (1)
R- I	-1221 / 0	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 658 (0.65")

CSI: TC=0.73/1.00 (F-H:1) , BC=0.73/1.00 (R-S:1) , WB=0.70/1.00 (S-U:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.78 (W) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T31S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BS-t	MT20	4.0	6.0		
Z	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

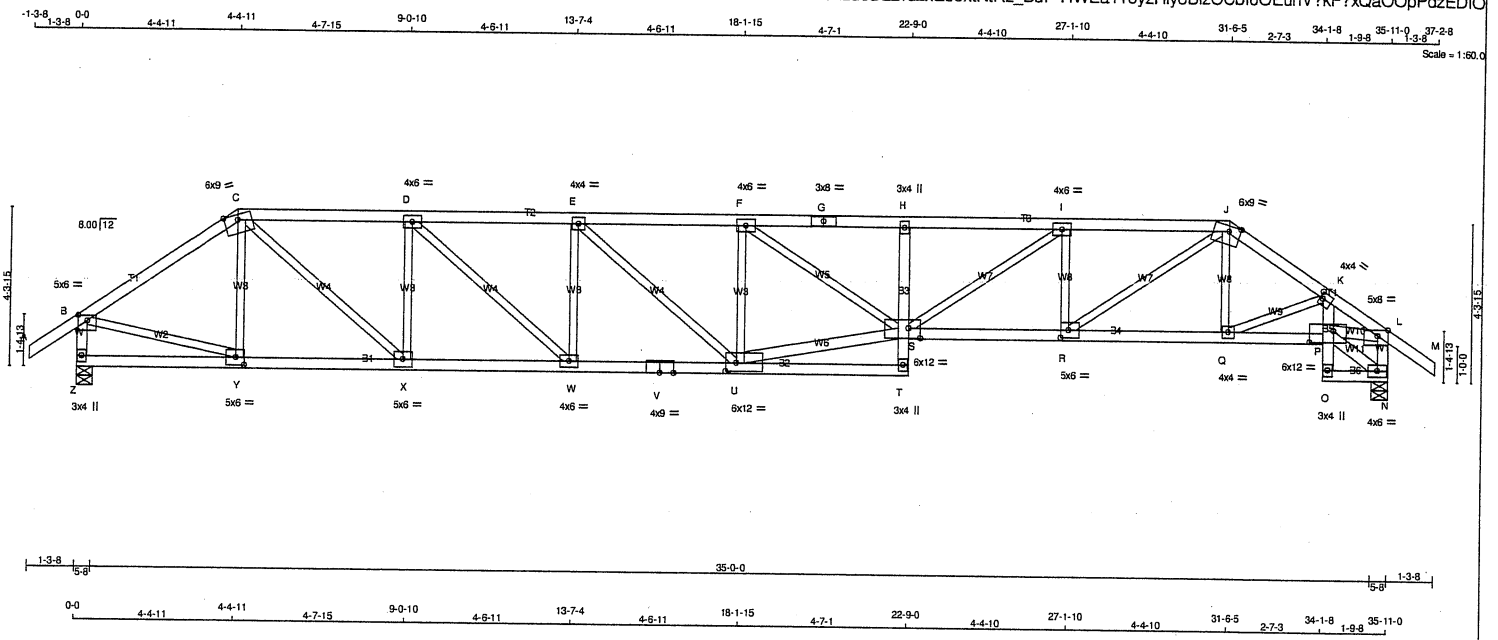
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115306

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410970	T31S1	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Z - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Z - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - H	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	6.0	9.0	Edge	
D, F, I						
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMV+p	MT20	3.0	4.0		
J	TTWW-m	MT20	6.0	9.0	1.75	3.75
K	TMVW-t	MT20	4.0	4.0	2.00	1.00
L	TMVW-p	MT20	5.0	8.0	Edge	3.50
N	BMVW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWW-I	MT20	6.0	12.0	3.75	7.75
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BVMWWW-I	MT20	6.0	12.0	3.25	4.00
T	BMV+p	MT20	3.0	4.0		
U	BMWWW-t	MT20	6.0	12.0	2.75	3.50
V	BS-t	MT20	4.0	9.0		
W	BMWW-t	MT20	4.0	6.0		



Structural component only
DWG# T-2115329

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	2103	0	2103	0	0	5-8	5-8
N	2108	0	2108	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
N	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2304 / 0	-91.8	-91.8	0.42 (1)	4.10
C- D	-3365 / 0	-91.8	-91.8	0.47 (1)	3.45
D- E	-4171 / 0	-91.8	-91.8	0.57 (1)	3.04
E- F	-4434 / 0	-91.8	-91.8	0.55 (1)	2.98
F- G	-5417 / 0	-91.8	-91.8	0.70 (1)	2.56
G- H	-5417 / 0	-91.8	-91.8	0.70 (1)	2.56
H- I	-5474 / 0	-91.8	-91.8	0.73 (1)	2.51
I- J	-4393 / 0	-91.8	-91.8	0.56 (1)	2.98
J- K	-3016 / 0	-91.8	-91.8	0.26 (1)	3.81
K- L	-3365 / 0	-91.8	-91.8	0.24 (1)	3.64
L- M	0 / 35	-91.8	-91.8	0.12 (1)	10.00
Z- B	-2070 / 0	0.0	0.0	0.21 (1)	5.88
N- L	-2015 / 0	0.0	0.0	0.21 (1)	5.95
Z- Y	0 / 0	-18.5	-18.5	0.09 (4)	10.00
Y- X	0 / 1905	-18.5	-18.5	0.36 (1)	10.00
X- W	0 / 3365	-18.5	-18.5	0.59 (1)	10.00
W- V	0 / 4171	-18.5	-18.5	0.71 (1)	10.00
V- U	0 / 4171	-18.5	-18.5	0.71 (1)	10.00
U- T	0 / 173	-18.5	-18.5	0.10 (4)	10.00
T- S	0 / 38	0.0	0.0	0.24 (1)	10.00
S- H	-374 / 0	0.0	0.0	0.24 (1)	7.81
S- R	0 / 4394	-18.5	-18.5	0.77 (1)	10.00
R- Q	0 / 2513	-18.5	-18.5	0.45 (1)	10.00
Q- P	0 / 2892	-18.5	-18.5	0.51 (1)	10.00
O- P	0 / 17	0.0	0.0	0.18 (1)	10.00
P- K	0 / 201	0.0	0.0	0.21 (1)	10.00
O- N	0 / 112	-18.5	-18.5	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-364 / 0	0.10 (1)
U- F	-1091 / 0	0.30 (1)
U- S	0 / 4386	0.70 (1)
F- S	0 / 1165	0.26 (1)
S- I	0 / 1310	0.29 (1)
R- I	-1191 / 0	0.23 (1)
R- J	0 / 2266	0.51 (1)
Q- J	0 / 99	0.03 (4)
Q- K	-410 / 0	0.08 (1)
B- Y	0 / 1969	0.44 (1)
P- N	-137 / 0	0.01 (1)
P- L	0 / 2767	0.44 (1)
E- U	0 / 354	0.08 (1)
C- X	0 / 1929	0.43 (1)
W- E	-636 / 0	0.18 (1)
X- D	-1180 / 0	0.33 (1)
D- W	0 / 1076	0.24 (1)

TOTAL WEIGHT = 153 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.37")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 629 (0.69")

CSI: TC=0.73/1.00 (H-I:1) , BC=0.77/1.00 (R-S:1) , WB=0.70/1.00 (S-U:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (X) (INPUT = 0.90)
JSI METAL= 0.92 (V) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410970	T31S1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
X	BMWW-t	MT20	5.0	6.0		
Y	BMWW-t	MT20	5.0	6.0	2.50	2.75
Z	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

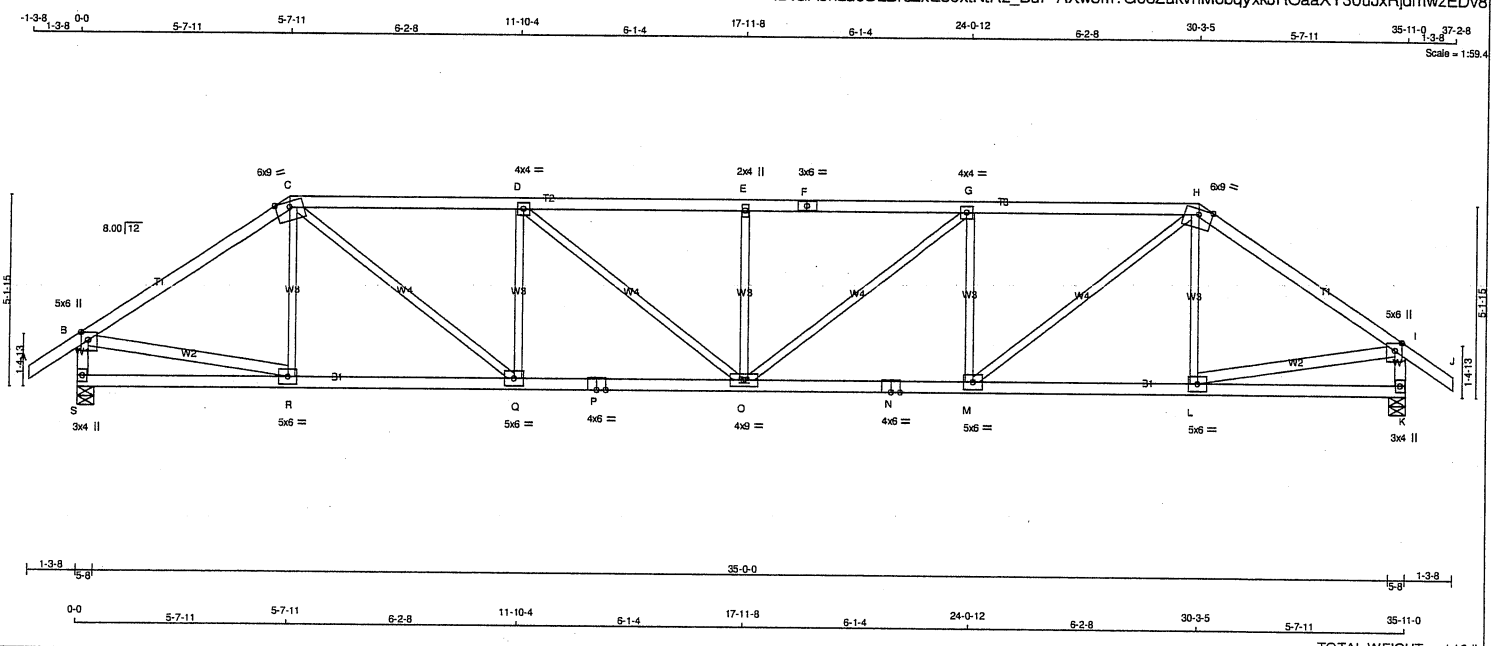
LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115329

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 146 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - R	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW+p	MT20	5.0	6.0	2.50	2.25
K	BMV1+p	MT20	3.0	4.0		
L, M, Q, R						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	4.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BS-t	MT20	4.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115307

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2106	0	2106	0	0	5-8	5-8
K	2106	0	2106	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
K	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	R- C	-244 / 11	0.10 (1)
B- C	-2357 / 0	-91.8	-91.8	0.74 (1)	3.70	C- Q	0 / 1722	0.39 (1)
C- D	-3310 / 0	-91.8	-91.8	0.86 (1)	3.08	Q- D	-952 / 0	0.37 (1)
D- E	-3705 / 0	-91.8	-91.8	0.93 (1)	2.86	D- O	0 / 505	0.11 (1)
E- F	-3705 / 0	-91.8	-91.8	0.93 (1)	2.86	O- E	-517 / 0	0.20 (1)
F- G	-3705 / 0	-91.8	-91.8	0.93 (1)	2.86	O- G	0 / 505	0.11 (1)
G- H	-3310 / 0	-91.8	-91.8	0.86 (1)	3.08	M- G	-952 / 0	0.37 (1)
H- I	-2357 / 0	-91.8	-91.8	0.74 (1)	3.70	M- H	0 / 1722	0.39 (1)
I- J	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L- H	-244 / 11	0.10 (1)
S- B	-2064 / 0	0.0	0.0	0.21 (1)	5.89	B- R	0 / 1993	0.32 (1)
K- I	-2064 / 0	0.0	0.0	0.21 (1)	5.89	L- I	0 / 1993	0.32 (1)
S- R	0 / 0	-18.5	-18.5	0.15 (4)	10.00			
R- Q	0 / 1954	-18.5	-18.5	0.40 (1)	10.00			
Q- P	0 / 3310	-18.5	-18.5	0.59 (1)	10.00			
P- O	0 / 3310	-18.5	-18.5	0.59 (1)	10.00			
O- N	0 / 3310	-18.5	-18.5	0.59 (1)	10.00			
N- M	0 / 3310	-18.5	-18.5	0.59 (1)	10.00			
M- L	0 / 1954	-18.5	-18.5	0.40 (1)	10.00			
L- K	0 / 0	-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.93/1.00 (D-E:1) , BC=0.59/1.00 (O-Q:1) , WB=0.39/1.00 (H-M:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

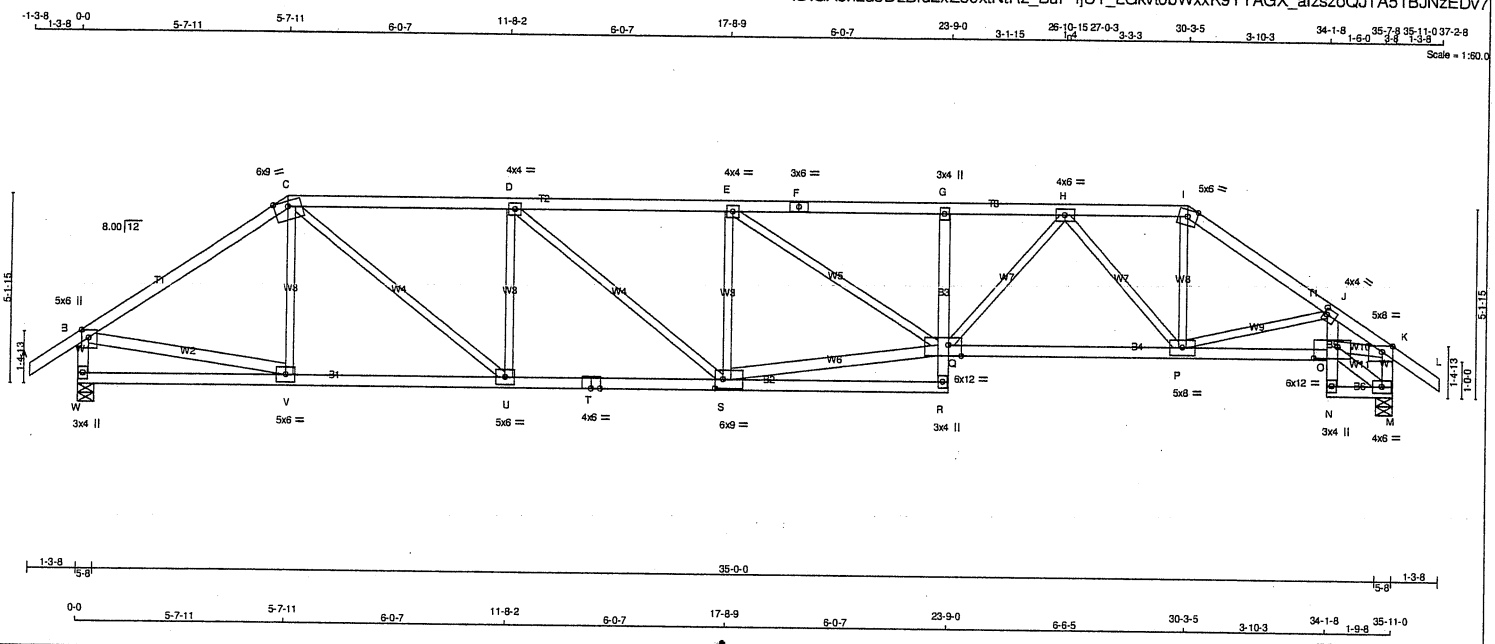
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.95 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T32S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-fjUT_LGkvt0bWxxK9YTAGX_alzszoQJTA5TBJNzEDv7



TOTAL WEIGHT = 157 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
B - V	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTW-m	MT20	5.0	6.0	Edge	
J	TMVW-t	MT20	4.0	4.0	2.00	1.00
K	TMVW-p	MT20	5.0	8.0	Edge	3.50
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	3.75	7.75
P	BMWWW-t	MT20	5.0	8.0		
Q	BVMWWW-l	MT20	6.0	12.0	Edge	4.25
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	9.0	3.00	2.50
T	BS-t	MT20	4.0	6.0		
U	BMWW-t	MT20	5.0	6.0		



Structural component only
DWG# T-2115308 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2103	0	2103	0	0	5-8	5-8
M	2108	0	2108	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
M	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C	-2354 / 0	-91.8	-91.8	0.74 (1)		3.70
C- D	-3281 / 0	-91.8	-91.8	0.80 (1)		3.16
D- E	-3701 / 0	-91.8	-91.8	0.87 (1)		2.93
E- F	-4135 / 0	-91.8	-91.8	0.81 (1)		2.89
F- G	-4135 / 0	-91.8	-91.8	0.81 (1)		2.89
G- H	-4169 / 0	-91.8	-91.8	0.48 (1)		3.07
H- I	-2494 / 0	-91.8	-91.8	0.23 (1)		4.17
I- J	-2937 / 0	-91.8	-91.8	0.37 (1)		3.75
J- K	-3423 / 0	-91.8	-91.8	0.24 (1)		3.60
K- L	0 / 35	-91.8	-91.8	0.12 (1)		10.00
W- B	-2062 / 0	0.0	0.0	0.21 (1)		5.89
M- K	-2014 / 0	0.0	0.0	0.21 (1)		5.95
W- V	0 / 0	-18.5	-18.5	0.16 (4)		10.00
V- U	0 / 1951	-18.5	-18.5	0.40 (1)		10.00
U- T	0 / 3281	-18.5	-18.5	0.60 (1)		10.00
T- S	0 / 3281	-18.5	-18.5	0.60 (1)		10.00
S- R	0 / 132	-18.5	-18.5	0.16 (4)		10.00
R- Q	0 / 50	0.0	0.0	0.19 (1)		10.00
Q- G	-440 / 0	0.0	0.0	0.20 (1)		7.81
Q- P	0 / 3435	-18.5	-18.5	0.65 (1)		10.00
P- O	0 / 2961	-18.5	-18.5	0.56 (1)		10.00
N- O	0 / 17	0.0	0.0	0.18 (1)		10.00
O- J	0 / 168	0.0	0.0	0.20 (1)		10.00
N- M	0 / 114	-18.5	-18.5	0.03 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-243 / 12	0.10 (1)
C- U	0 / 1701	0.38 (1)
U- D	-960 / 0	0.38 (1)
D- S	0 / 545	0.12 (1)
S- E	-824 / 0	0.32 (1)
S- Q	0 / 3633	0.58 (1)
E- Q	0 / 502	0.11 (1)
Q- H	0 / 1132	0.25 (1)
H- P	-1478 / 0	0.63 (1)
P- I	0 / 1312	0.30 (1)
P- J	-545 / 0	0.17 (1)
B- V	0 / 1990	0.32 (1)
O- M	-139 / 0	0.01 (1)
O- K	0 / 2836	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 884 (0.49")

CSI: TC=0.87/1.00 (D-E:1) , BC=0.65/1.00 (P-Q:1) , WB=0.63/1.00 (H-P:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.94 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T32S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-fjUT_LGkvt0bWxxK9YTAGX_alzszoQJTA5TBJNzEDv7

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

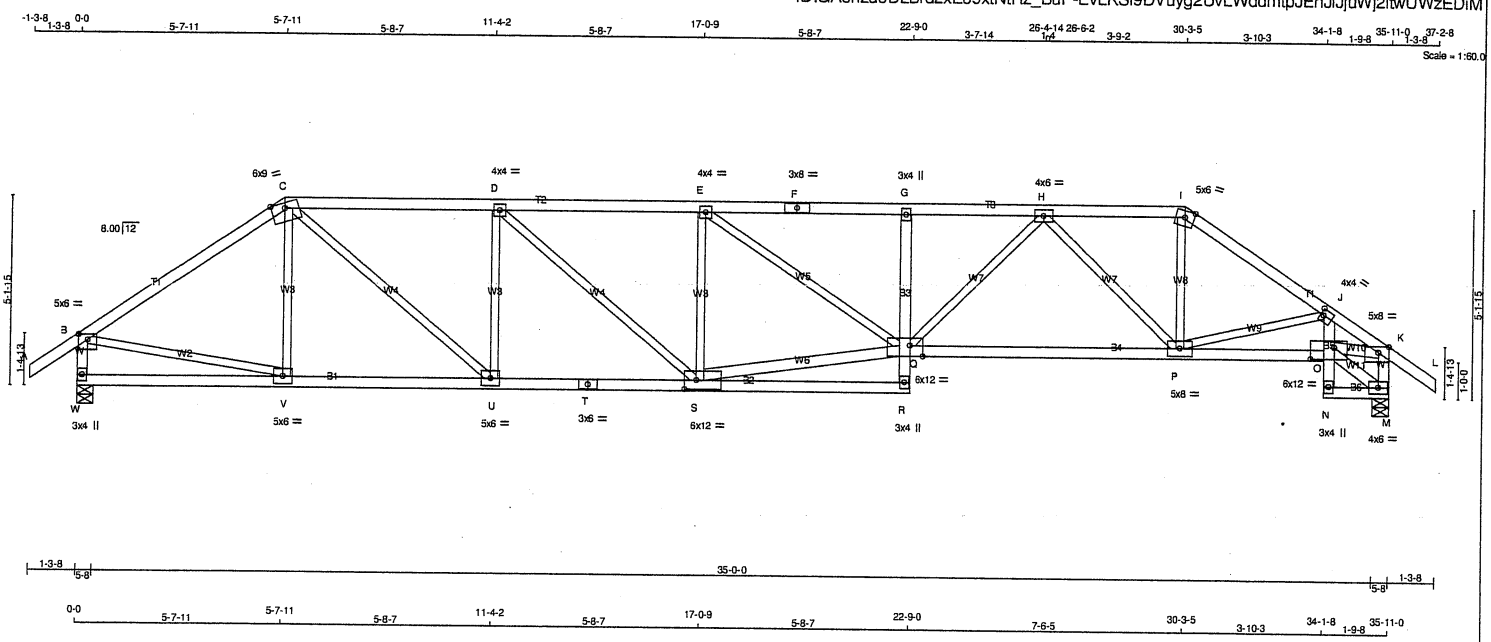


Structural component only
DWG# T-2115308

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410970	T32S1	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMV+p	MT20	3.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTW-m	MT20	5.0	6.0	Edge	
J	TMVW-t	MT20	4.0	4.0	2.00	1.00
K	TMVW-p	MT20	5.0	8.0	Edge	3.50
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	3.75	7.75
P	BMWWW-t	MT20	5.0	8.0		
Q	BVMWWW-l	MT20	6.0	12.0	Edge	4.50
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	12.0	3.00	4.00
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0		



Structural component only
DWG# T-2115330

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2103	0	2103	0	0	5-8	5-8
M	2108	0	2108	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
M	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C	-2353 / 0	-91.8	-91.8	0.74 (1)		3.71
C- D	-3224 / 0	-91.8	-91.8	0.69 (1)		3.30
D- E	-3684 / 0	-91.8	-91.8	0.76 (1)		3.03
E- F	-4278 / 0	-91.8	-91.8	0.74 (1)		2.90
F- G	-4278 / 0	-91.8	-91.8	0.74 (1)		2.90
G- H	-4312 / 0	-91.8	-91.8	0.48 (1)		3.05
H- I	-2501 / 0	-91.8	-91.8	0.26 (1)		4.13
I- J	-2943 / 0	-91.8	-91.8	0.38 (1)		3.75
J- K	-3423 / 0	-91.8	-91.8	0.24 (1)		3.60
K- L	0 / 35	-91.8	-91.8	0.12 (1)		10.00
W- B	-2061 / 0	0.0	0.0	0.21 (1)		5.89
M- K	-2014 / 0	0.0	0.0	0.21 (1)		5.95
W- V	0 / 0	-18.5	-18.5	0.15 (4)		10.00
V- U	0 / 1950	-18.5	-18.5	0.39 (1)		10.00
U- T	0 / 3224	-18.5	-18.5	0.58 (1)		10.00
T- S	0 / 3224	-18.5	-18.5	0.58 (1)		10.00
S- R	0 / 130	-18.5	-18.5	0.15 (4)		10.00
R- Q	0 / 47	0.0	0.0	0.18 (1)		10.00
Q- G	-428 / 0	0.0	0.0	0.20 (1)		7.81
Q- P	0 / 3565	-18.5	-18.5	0.70 (1)		10.00
P- O	0 / 2961	-18.5	-18.5	0.59 (1)		10.00
N- O	0 / 17	0.0	0.0	0.18 (1)		10.00
O- J	0 / 163	0.0	0.0	0.20 (1)		10.00
N- M	0 / 114	-18.5	-18.5	0.03 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-246 / 8	0.10 (1)
C- U	0 / 1667	0.38 (1)
U- D	-981 / 0	0.39 (1)
D- S	0 / 611	0.14 (1)
S- E	-919 / 0	0.36 (1)
S- Q	0 / 3625	0.58 (1)
E- Q	0 / 705	0.16 (1)
Q- H	0 / 1065	0.24 (1)
H- P	-1543 / 0	0.76 (1)
P- I	0 / 1303	0.29 (1)
P- J	-540 / 0	0.16 (1)
B- V	0 / 1989	0.45 (1)
O- M	-139 / 0	0.01 (1)
O- K	0 / 2836	0.46 (1)

TOTAL WEIGHT = 154 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 861 (0.50")

CSI: TC=0.76/1.00 (D-E:1) , BC=0.70/1.00 (P-Q:1) , WB=0.76/1.00 (H-P:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)
JSI METAL= 0.99 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410970	T32S1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-EvLKSi9DVuyg2UvLWddmtpJEnJiJjuWj2itwUWzEDiM

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

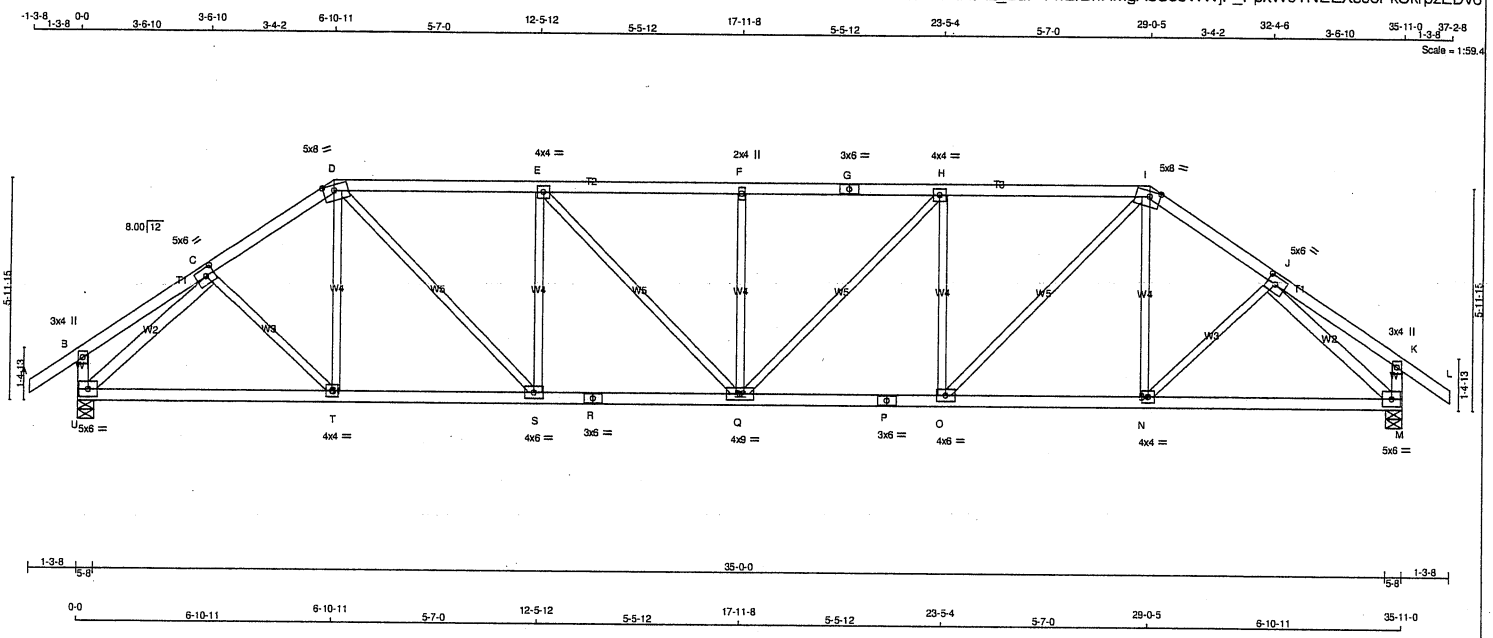
LICENSED PROFESSIONAL ENGINEER
05/22/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115330 R12

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-7w2rBhHMgA8S85WWjF_PpkWoTNEEXsocPkCkrpzEDv6



TOTAL WEIGHT = 155 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	1.75	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.50
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115309

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2106	0	2106	0	0	5-8	5-8
M	2106	0	2106	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
M	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 18	-91.8	-91.8	0.15 (1)	10.00
C- D	-2370 / 0	-91.8	-91.8	0.23 (1)	4.26
D- E	-2882 / 0	-91.8	-91.8	0.62 (1)	3.52
E- F	-3155 / 0	-91.8	-91.8	0.65 (1)	3.35
F- G	-3155 / 0	-91.8	-91.8	0.65 (1)	3.35
G- H	-3155 / 0	-91.8	-91.8	0.65 (1)	3.35
H- I	-2882 / 0	-91.8	-91.8	0.62 (1)	3.52
I- J	-2370 / 0	-91.8	-91.8	0.23 (1)	4.26
J- K	0 / 18	-91.8	-91.8	0.15 (1)	10.00
K- L	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-251 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-251 / 0	0.0	0.0	0.03 (1)	7.81
U- T	0 / 1836	-18.5	-18.5	0.40 (1)	10.00
T- S	0 / 1953	-18.5	-18.5	0.42 (1)	10.00
S- R	0 / 2882	-18.5	-18.5	0.52 (1)	10.00
R- Q	0 / 2882	-18.5	-18.5	0.52 (1)	10.00
Q- P	0 / 2882	-18.5	-18.5	0.52 (1)	10.00
P- O	0 / 2882	-18.5	-18.5	0.52 (1)	10.00
O- N	0 / 1953	-18.5	-18.5	0.42 (1)	10.00
N- M	0 / 1836	-18.5	-18.5	0.40 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 161	0.04 (1)
T- D	0 / 83	0.03 (4)
D- S	0 / 1326	0.30 (1)
S- E	-856 / 0	0.48 (1)
E- Q	0 / 394	0.09 (1)
Q- F	-463 / 0	0.26 (1)
Q- H	0 / 394	0.09 (1)
O- H	-856 / 0	0.48 (1)
O- I	0 / 1326	0.30 (1)
N- I	0 / 83	0.03 (4)
N- J	0 / 161	0.04 (1)
U- C	-2572 / 0	0.68 (1)
J- M	-2572 / 0	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.65/1.00 (E-F:1) , BC=0.52/1.00 (Q-S:1) , WB=0.68/1.00 (C-U:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

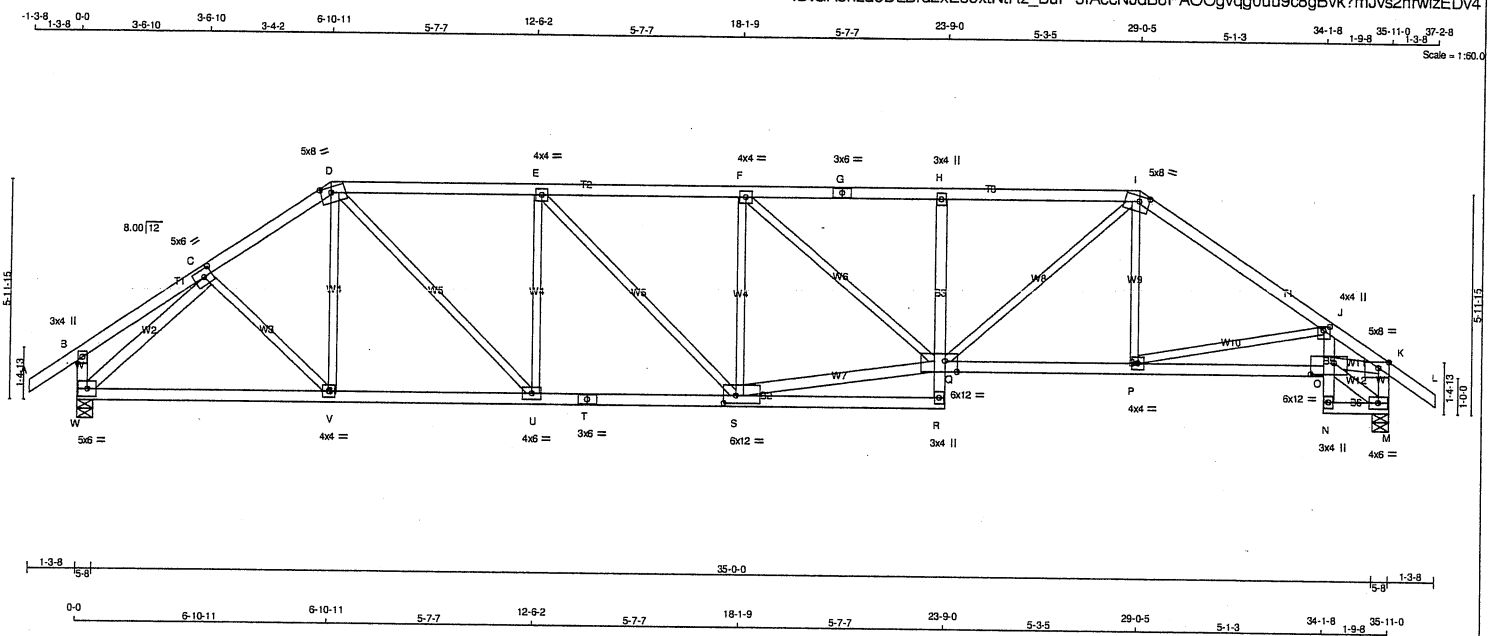
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.87 (R) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T33S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:08:09 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-3IAccNJdBoPAOOgvqg0uu9c8gBvk?mJvs2hrwizEDv4



TOTAL WEIGHT = 162 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
W - C	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	1.75	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.25
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	3.50
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	3.75	7.75
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWWW-l	MT20	6.0	12.0	Edge	4.00
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	12.0	2.50	4.00
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	4.0	6.0		



Structural component only
DWG# T-2115310

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2103	0	2103	0	0	5-8	5-8
M	2108	0	2108	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
M	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 18	-91.8	-91.8	0.15 (1)	10.00
C- D	-2366 / 0	-91.8	-91.8	0.23 (1)	4.26
D- E	-2887 / 0	-91.8	-91.8	0.64 (1)	3.50
E- F	-3150 / 0	-91.8	-91.8	0.67 (1)	3.33
F- G	-3432 / 0	-91.8	-91.8	0.64 (1)	3.25
G- H	-3432 / 0	-91.8	-91.8	0.64 (1)	3.25
H- I	-3450 / 0	-91.8	-91.8	0.64 (1)	3.24
I- J	-2840 / 0	-91.8	-91.8	0.57 (1)	3.66
J- K	-3513 / 0	-91.8	-91.8	0.34 (1)	3.44
K- L	0 / 35	-91.8	-91.8	0.12 (1)	10.00
W- B	-251 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-2012 / 0	0.0	0.0	0.21 (1)	5.95
W- V	0 / 1834	-18.5	-18.5	0.41 (1)	10.00
V- U	0 / 1951	-18.5	-18.5	0.42 (1)	10.00
U- T	0 / 2887	-18.5	-18.5	0.53 (1)	10.00
T- S	0 / 2887	-18.5	-18.5	0.53 (1)	10.00
S- R	0 / 87	-18.5	-18.5	0.15 (4)	10.00
R- Q	0 / 46	0.0	0.0	0.13 (1)	10.00
Q- H	-552 / 0	0.0	0.0	0.19 (1)	7.81
Q- P	0 / 2351	-18.5	-18.5	0.46 (1)	10.00
P- O	0 / 3072	-18.5	-18.5	0.58 (1)	10.00
N- O	0 / 17	0.0	0.0	0.18 (1)	10.00
O- J	0 / 166	0.0	0.0	0.21 (1)	10.00
N- M	0 / 117	-18.5	-18.5	0.03 (1)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
C- V	0 / 160	0.04 (1)
V- D	0 / 85	0.03 (4)
D- U	0 / 1326	0.30 (1)
U- E	-852 / 0	0.48 (1)
E- S	0 / 379	0.09 (1)
S- F	-710 / 0	0.40 (1)
S- Q	0 / 3121	0.50 (1)
F- Q	0 / 360	0.08 (1)
Q- I	0 / 1453	0.33 (1)
P- I	0 / 199	0.05 (4)
P- J	-741 / 0	0.38 (1)
W- C	-2569 / 0	0.68 (1)
O- M	-143 / 0	0.02 (1)
O- K	0 / 2943	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.67/1.00 (E-F:1) , BC=0.58/1.00 (O-P:1) , WB=0.68/1.00 (C-W:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.87 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T33S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-3IAccNJdBoPAOOgvqg0uu9c8gBvk?mJvs2hrwizEDv4

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

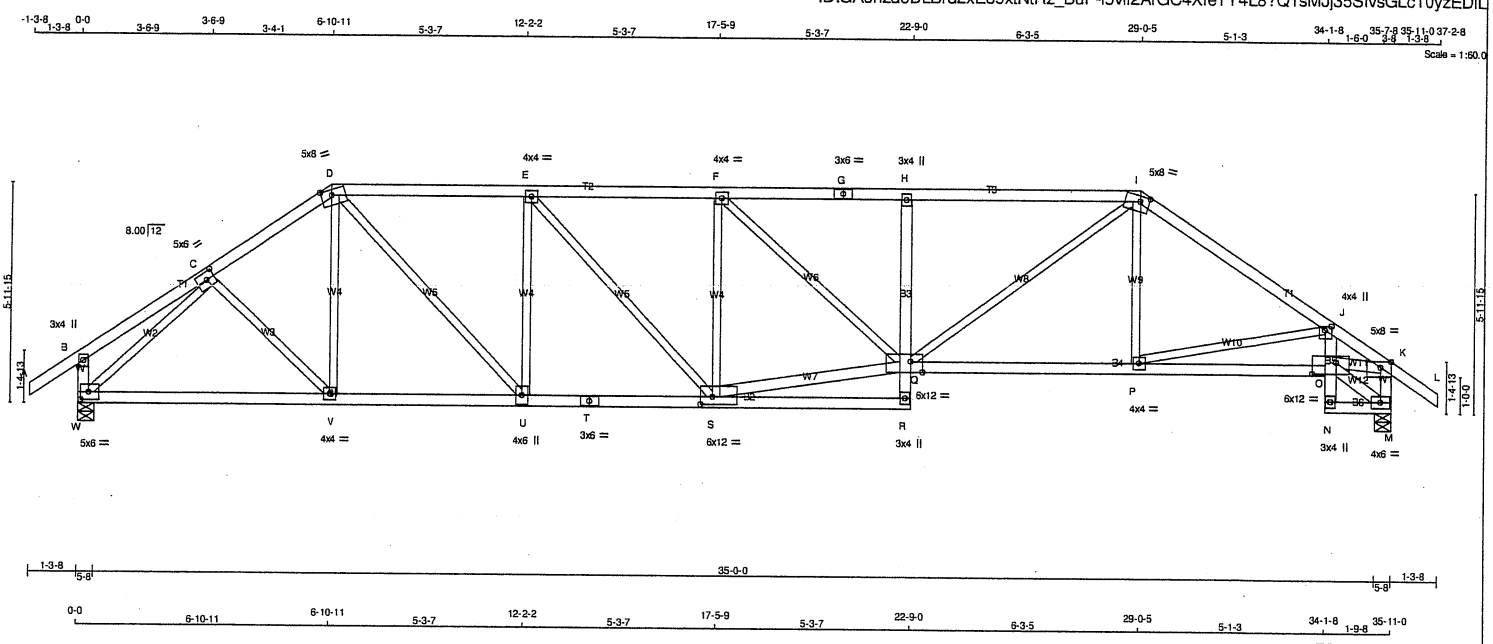
LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115310

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410970	T33S1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-i5vif2ArGC4XfeTY4L8?Q1sMJj35SIvsGLcT0yzEDiL



TOTAL WEIGHT = 160 lb [M]

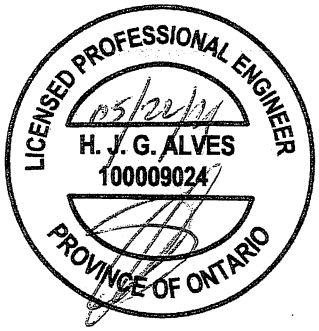
LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	1.75	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW-m	MT20	5.0	8.0	1.75	3.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	TMVW-p	MT20	5.0	8.0	Edge	3.50
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	3.75	7.75
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWWW-l	MT20	6.0	12.0	Edge	4.00
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	12.0	2.50	4.00
T	BS-t	MT20	3.0	6.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMWW-t	MT20	4.0	4.0		



Structural component only
DWG# T-2115331

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2103	0	2103	0	0	5-8	5-8
M	2108	0	2108	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
M	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 17	-91.8	-91.8	0.15 (1)	10.00
C- D	-2365 / 0	-91.8	-91.8	0.22 (1)	4.27
D- E	-2845 / 0	-91.8	-91.8	0.57 (1)	3.59
E- F	-3137 / 0	-91.8	-91.8	0.60 (1)	3.41
F- G	-3569 / 0	-91.8	-91.8	0.76 (1)	2.99
G- H	-3569 / 0	-91.8	-91.8	0.76 (1)	2.99
H- I	-3587 / 0	-91.8	-91.8	0.90 (1)	2.98
I- J	-2844 / 0	-91.8	-91.8	0.57 (1)	3.65
J- K	-3514 / 0	-91.8	-91.8	0.34 (1)	3.44
K- L	0 / 35	-91.8	-91.8	0.12 (1)	10.00
W- B	-253 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-2012 / 0	0.0	0.0	0.21 (1)	5.95
W- V	0 / 1832	-18.5	-18.5	0.40 (1)	10.00
V- U	0 / 1950	-18.5	-18.5	0.42 (1)	10.00
U- T	0 / 2845	-18.5	-18.5	0.52 (1)	10.00
T- S	0 / 2845	-18.5	-18.5	0.52 (1)	10.00
S- R	0 / 86	-18.5	-18.5	0.13 (4)	10.00
R- Q	0 / 43	0.0	0.0	0.12 (1)	10.00
Q- H	-612 / 0	0.0	0.0	0.21 (1)	7.81
Q- P	0 / 2355	-18.5	-18.5	0.48 (1)	10.00
P- O	0 / 3071	-18.5	-18.5	0.60 (1)	10.00
N- O	0 / 17	0.0	0.0	0.18 (1)	10.00
O- J	0 / 163	0.0	0.0	0.21 (1)	10.00
N- M	0 / 117	-18.5	-18.5	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 162	0.04 (1)
V- D	0 / 82	0.03 (4)
D- U	0 / 1308	0.29 (1)
U- E	-874 / 0	0.49 (1)
E- S	0 / 435	0.10 (1)
S- F	-800 / 0	0.45 (1)
S- Q	0 / 3117	0.50 (1)
F- Q	0 / 571	0.13 (1)
Q- I	0 / 1526	0.34 (1)
P- I	0 / 211	0.06 (4)
P- J	-738 / 0	0.38 (1)
W- C	-2566 / 0	0.95 (1)
O- M	-143 / 0	0.02 (1)
O- K	0 / 2943	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.90/1.00 (H-I:1) , BC=0.60/1.00 (O-P:1) , WB=0.95/1.00 (C-W:1) , SSI=0.27/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (W) (INPUT = 0.90)
JSI METAL= 0.87 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410970	T33S1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-i5vif2ArGC4XfeTY4L8?Q1sMJj35SIvsGLcT0yzEDiL

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMVW1-t	MT20	5.0	6.0	2.50	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

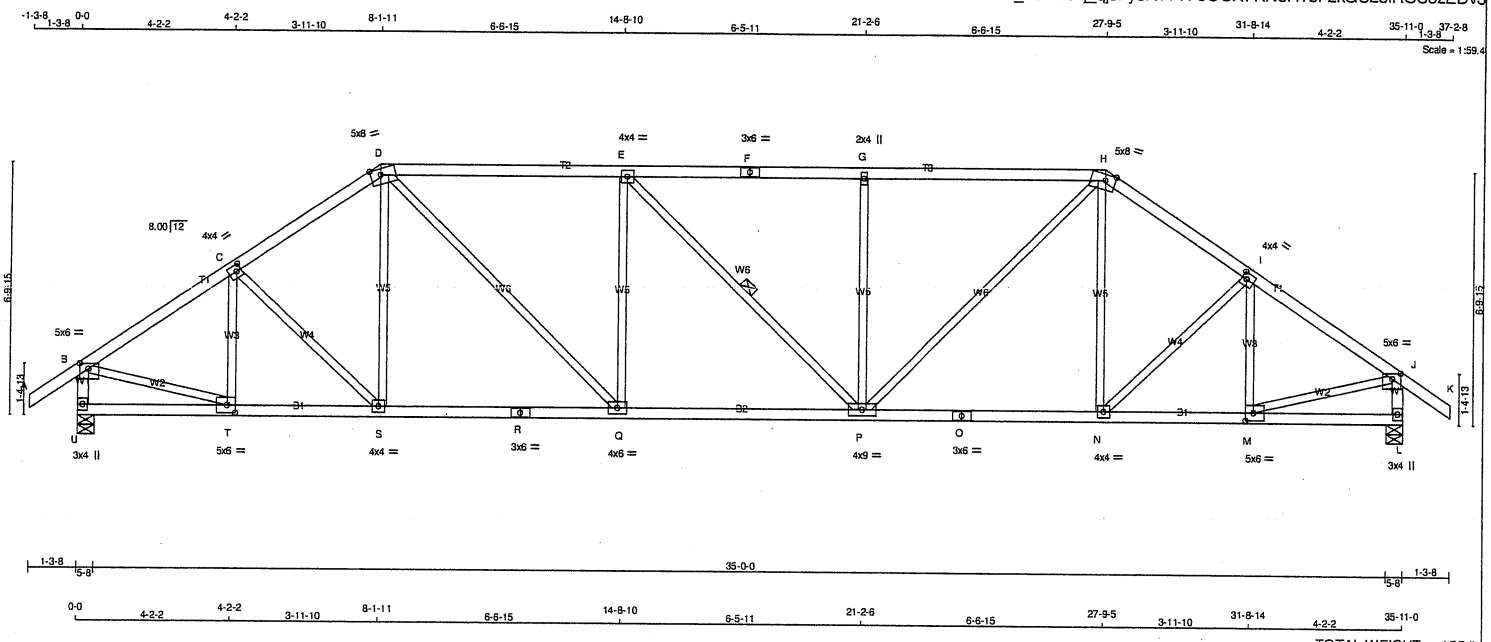
LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115331

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410969	T34	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	6.0	1.75	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115311

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2106	0	2106	0	0	5-8	5-8
L	2106	0	2106	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2304 / 0	-91.8	-91.8	0.33 (1)	4.21
C- D	-2333 / 0	-91.8	-91.8	0.33 (1)	4.18
D- E	-2699 / 0	-91.8	-91.8	0.82 (1)	3.40
E- F	-2697 / 0	-91.8	-91.8	0.81 (1)	3.40
F- G	-2697 / 0	-91.8	-91.8	0.81 (1)	3.40
G- H	-2697 / 0	-91.8	-91.8	0.81 (1)	3.41
H- I	-2334 / 0	-91.8	-91.8	0.33 (1)	4.18
I- J	-2304 / 0	-91.8	-91.8	0.33 (1)	4.21
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-2068 / 0	0.0	0.0	0.21 (1)	5.88
L- J	-2068 / 0	0.0	0.0	0.21 (1)	5.88
U- T	0 / 0	-18.5	-18.5	0.07 (4)	10.00
T- S	0 / 1936	-18.5	-18.5	0.38 (1)	10.00
S- R	0 / 1920	-18.5	-18.5	0.39 (1)	10.00
R- Q	0 / 1920	-18.5	-18.5	0.39 (1)	10.00
Q- P	0 / 2699	-18.5	-18.5	0.51 (1)	10.00
P- O	0 / 1920	-18.5	-18.5	0.38 (1)	10.00
O- N	0 / 1920	-18.5	-18.5	0.38 (1)	10.00
N- M	0 / 1936	-18.5	-18.5	0.38 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-435 / 0	0.12 (1)
C- S	-25 / 2	0.01 (1)
S- D	0 / 125	0.04 (4)
D- Q	0 / 1096	0.25 (1)
Q- E	-650 / 0	0.51 (1)
E- P	-2 / 0	0.00 (1)
P- G	-650 / 0	0.51 (1)
P- H	0 / 1094	0.25 (1)
N- H	0 / 126	0.04 (4)
N- I	-24 / 2	0.01 (1)
M- I	-436 / 0	0.12 (1)
B- T	0 / 2000	0.45 (1)
M- J	0 / 2000	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.82/1.00 (D-E:1) , BC=0.51/1.00 (P-Q:1) , WB=0.51/1.00 (E-Q:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

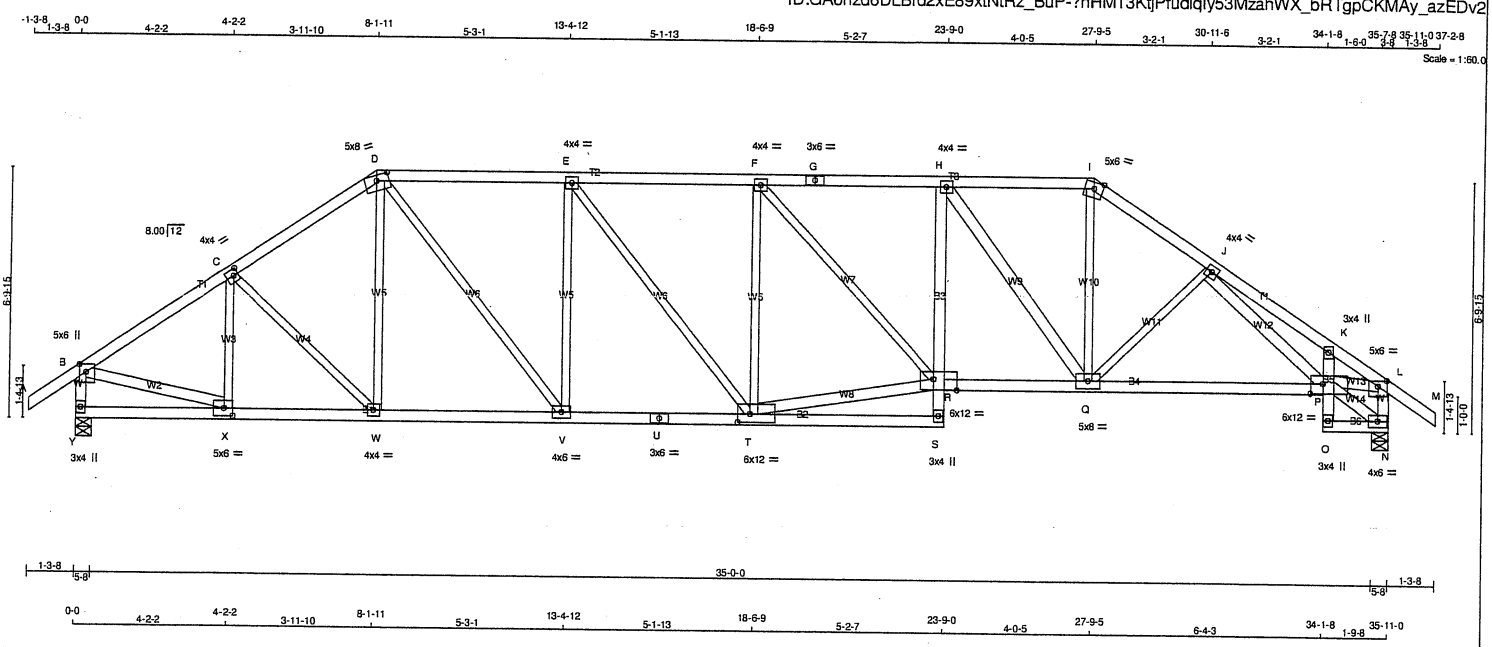
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.65 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T34S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:08:11 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-?hHM13KtjPfudiqly53MzahWX_bRTgpCKMAy_azEDv2



TOTAL WEIGHT = 176 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
H - Q	2x4	DRY	No.2	SPF
B - X	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
J - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	1.75	4.00
E, F, J						
E	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTW-m	MT20	5.0	6.0	Edge	
K	TMV+p	MT20	3.0	4.0		
L	TMVW-p	MT20	5.0	6.0	1.75	3.00
N	BMWW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-t	MT20	6.0	12.0	3.25	4.00
Q	BMWWW-t	MT20	5.0	8.0		
R	BVMWW-t	MT20	6.0	12.0	3.50	7.75
S	BMV+p	MT20	3.0	4.0		
T	BMWWW-t	MT20	6.0	12.0	2.75	4.00



Structural component only
DWG# T-2115312

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	2103	0	2103	0	0	5-8	5-8
N	2108	0	2108	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
N	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2303 / 0	-91.8	-91.8	0.34 (1)	4.20
C- D	-2327 / 0	-91.8	-91.8	0.33 (1)	4.18
D- E	-2583 / 0	-91.8	-91.8	0.50 (1)	3.81
E- F	-2742 / 0	-91.8	-91.8	0.52 (1)	3.70
F- G	-2917 / 0	-91.8	-91.8	0.41 (1)	3.74
G- H	-2917 / 0	-91.8	-91.8	0.41 (1)	3.74
H- I	-2275 / 0	-91.8	-91.8	0.33 (1)	4.21
I- J	-2731 / 0	-91.8	-91.8	0.25 (1)	3.99
J- K	-3018 / 0	-91.8	-91.8	0.27 (1)	3.80
K- L	-2993 / 0	-91.8	-91.8	0.21 (1)	3.86
L- M	0 / 35	-91.8	-91.8	0.12 (1)	10.00
Y- B	-2067 / 0	0.0	0.0	0.21 (1)	5.88
N- L	-2032 / 0	0.0	0.0	0.21 (1)	5.93
Y- X	0 / 0	-18.5	-18.5	0.07 (4)	10.00
X- W	0 / 1936	-18.5	-18.5	0.36 (1)	10.00
W- V	0 / 1915	-18.5	-18.5	0.36 (1)	10.00
V- U	0 / 2583	-18.5	-18.5	0.47 (1)	10.00
U- T	0 / 2583	-18.5	-18.5	0.47 (1)	10.00
T- S	0 / 59	-18.5	-18.5	0.12 (4)	10.00
S- R	0 / 43	0.0	0.0	0.09 (1)	10.00
R- H	0 / 430	0.0	0.0	0.15 (1)	10.00
R- Q	0 / 2928	-18.5	-18.5	0.56 (1)	10.00
Q- P	0 / 2497	-18.5	-18.5	0.50 (1)	10.00
O- P	0 / 17	0.0	0.0	0.14 (1)	10.00
P- K	-144 / 0	0.0	0.0	0.14 (1)	7.81
O- N	0 / 100	-18.5	-18.5	0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-429 / 0	0.11 (1)
C- W	-31 / 0	0.02 (1)
W- D	0 / 115	0.03 (4)
T- F	-637 / 0	0.50 (1)
T- R	0 / 2743	0.44 (1)
F- R	0 / 250	0.06 (1)
H- Q	-1103 / 0	0.68 (1)
Q- I	0 / 1238	0.28 (1)
B- X	0 / 1999	0.32 (1)
P- N	-117 / 0	0.01 (1)
P- L	0 / 2456	0.39 (1)
E- T	0 / 260	0.06 (1)
D- V	0 / 1065	0.24 (1)
V- E	-738 / 0	0.58 (1)
Q- J	-343 / 0	0.13 (1)
J- P	0 / 50	0.01 (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.52/1.00 (E-F:1) , BC=0.56/1.00 (Q-R:1) , WB=0.68/1.00 (H-Q:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.91 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T34S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-?hHM13KtjPfudiqIy53MzahWX_bRTgpCKMAy_azEDv2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BS-t	MT20	3.0	6.0		
V	BMWW-t	MT20	4.0	6.0		
W	BMWW-t	MT20	4.0	4.0		
X	BMWW-t	MT20	5.0	6.0	2.50	2.75
Y	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

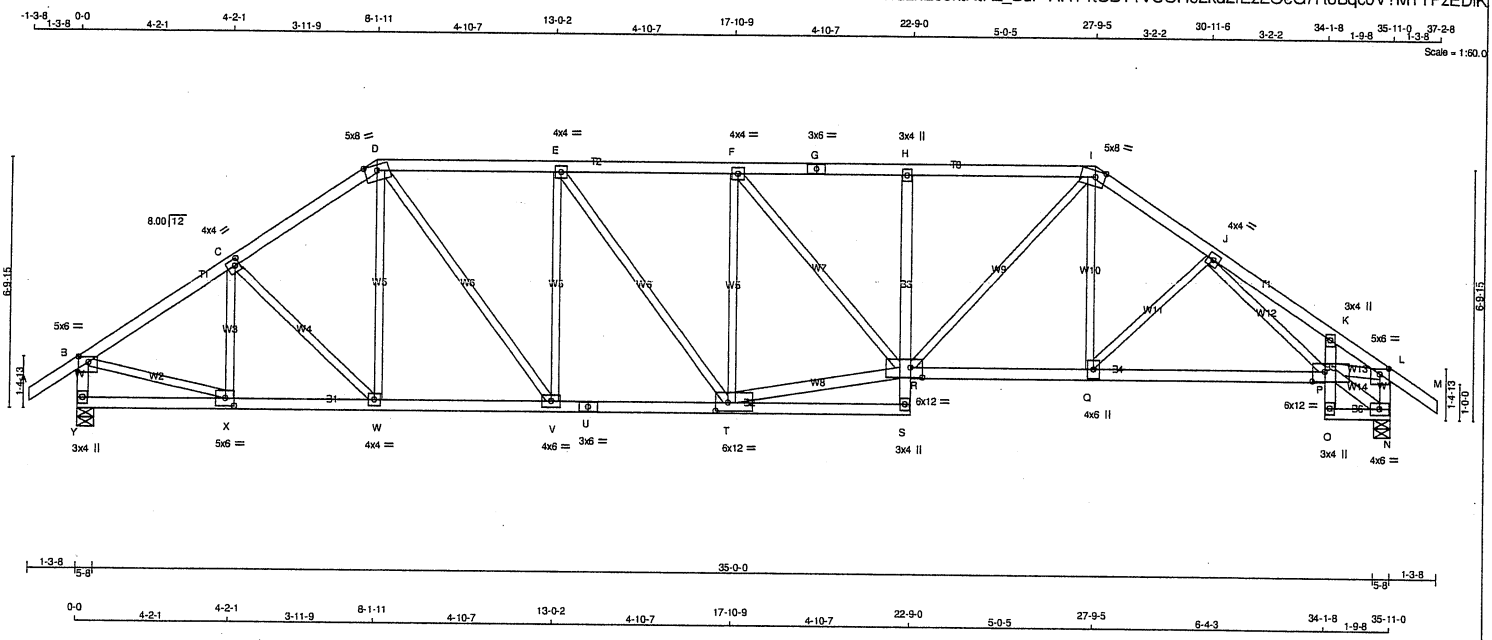
LICENSED PROFESSIONAL ENGINEER
05/22/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115312

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410970	T34S1	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-AHT4tOBT1VCOHo2kd2fEzEOeG7RoBqc0V?M1YPzEDiK



TOTAL WEIGHT = 171 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	1.75	4.00
E, F, J						
E	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW-m	MT20	5.0	8.0	2.00	3.25
K	TMV+p	MT20	3.0	4.0		
L	TMVW-p	MT20	5.0	6.0	1.75	3.00
N	BMVW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-I	MT20	6.0	12.0	3.25	4.00
Q	BMWW+t	MT20	4.0	6.0		
R	BVMWWW-I	MT20	6.0	12.0	3.25	4.00
S	BMV+p	MT20	3.0	4.0		
T	BMWWW-t	MT20	6.0	12.0	2.75	4.00
U	BS-t	MT20	3.0	6.0		
V	BMWW-t	MT20	4.0	6.0		
W	BMWW-t	MT20	4.0	4.0		



Structural component only
DWG# T-2115332

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	2103	0	2103	0	0	5-8	5-8
N	2108	0	2108	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
N	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2303 / 0	-91.8	-91.8	0.33 (1)	4.21
C- D	-2327 / 0	-91.8	-91.8	0.33 (1)	4.19
D- E	-2551 / 0	-91.8	-91.8	0.44 (1)	3.90
E- F	-2738 / 0	-91.8	-91.8	0.46 (1)	3.77
F- G	-3016 / 0	-91.8	-91.8	0.48 (1)	3.58
G- H	-3016 / 0	-91.8	-91.8	0.48 (1)	3.58
H- I	-3027 / 0	-91.8	-91.8	0.51 (1)	3.57
I- J	-2733 / 0	-91.8	-91.8	0.26 (1)	3.98
J- K	-3014 / 0	-91.8	-91.8	0.28 (1)	3.79
K- L	-2991 / 0	-91.8	-91.8	0.21 (1)	3.86
L- M	0 / 35	-91.8	-91.8	0.12 (1)	10.00
Y- B	-2067 / 0	0.0	0.0	0.21 (1)	5.88
N- L	-2032 / 0	0.0	0.0	0.21 (1)	5.93
Y- X	0 / 0	-18.5	-18.5	0.07 (4)	10.00
X- W	0 / 1935	-18.5	-18.5	0.36 (1)	10.00
W- V	0 / 1914	-18.5	-18.5	0.36 (1)	10.00
V- U	0 / 2551	-18.5	-18.5	0.46 (1)	10.00
U- T	0 / 2551	-18.5	-18.5	0.46 (1)	10.00
T- S	0 / 60	-18.5	-18.5	0.10 (4)	10.00
S- R	0 / 40	0.0	0.0	0.09 (1)	10.00
R- H	-508 / 0	0.0	0.0	0.22 (1)	7.81
R- Q	0 / 2260	-18.5	-18.5	0.46 (1)	10.00
Q- P	0 / 2500	-18.5	-18.5	0.50 (1)	10.00
O- P	0 / 17	0.0	0.0	0.14 (1)	10.00
P- K	-140 / 0	0.0	0.0	0.14 (1)	7.81
O- N	0 / 100	-18.5	-18.5	0.02 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-429 / 0	0.11 (1)
W- C	-30 / 0	0.02 (1)
W- D	0 / 112	0.03 (4)
D- V	0 / 1057	0.24 (1)
V- E	-760 / 0	0.60 (1)
E- T	0 / 317	0.07 (1)
T- F	-719 / 0	0.56 (1)
T- R	0 / 2742	0.44 (1)
F- R	0 / 418	0.09 (1)
R- I	0 / 1122	0.25 (1)
Q- I	0 / 375	0.08 (1)
Q- J	-345 / 0	0.13 (1)
B- X	0 / 1998	0.45 (1)
P- N	-117 / 0	0.01 (1)
P- L	0 / 2454	0.39 (1)
J- P	0 / 47	0.02 (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.51/1.00 (H-I:1) , BC=0.50/1.00 (P-Q:1) , WB=0.60/1.00 (E-V:1) , SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.73 (U) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410970	T34S1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-AHT4tOBT1VCOHo2kd2fEzEOeG7RoBqc0V?M1YPzEDiK

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
X	BMWW-t	MT20	5.0	6.0	2.50	2.75
Y	BMV1+p	MT20	3.0	4.0		

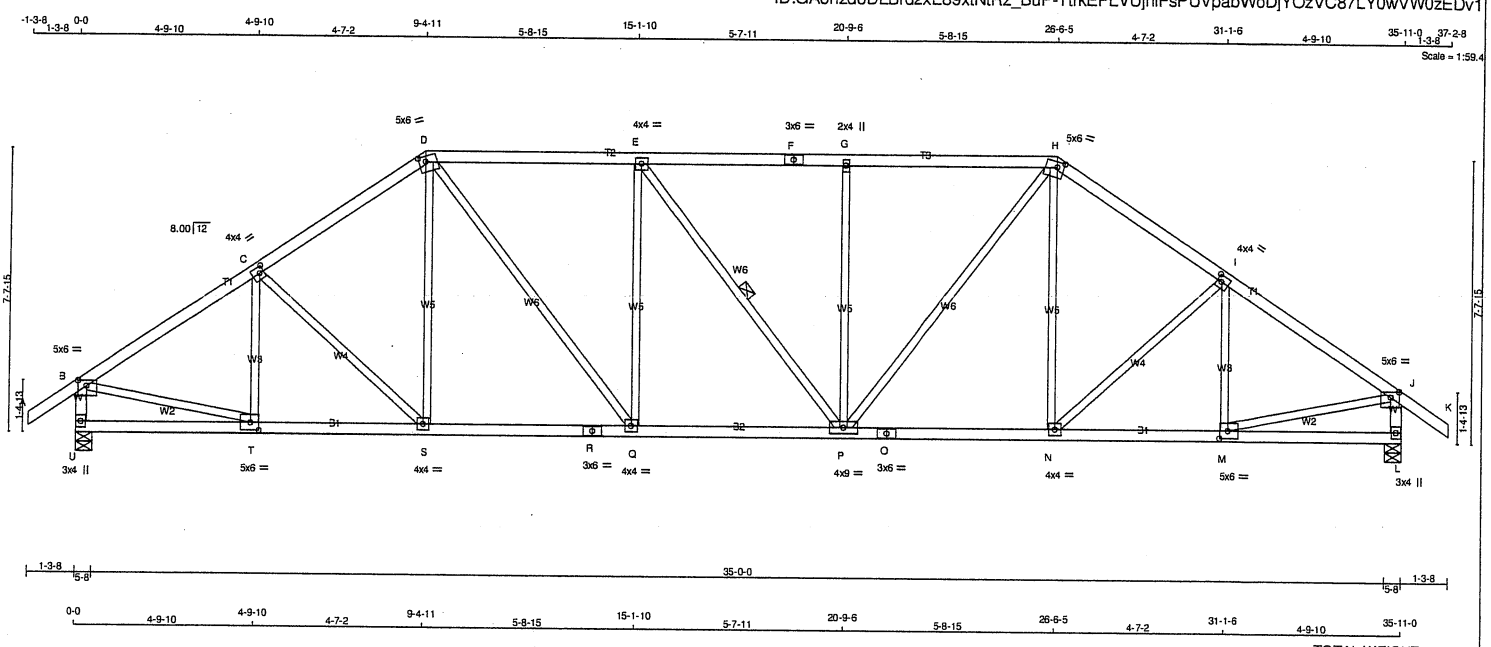
LICENSED PROFESSIONAL ENGINEER
05/22/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115332

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410969	T35	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-TtrkEPLVUjnlFsPUVpabWoDjYOzVC87LY0wVW0zEDv1



TOTAL WEIGHT = 161 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	1.75	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	2.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	6.0	1.75	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N, Q, S						
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2115313

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2106	0	2106	0	0	5-8	5-8
L	2106	0	2106	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2356 / 0	-91.8	-91.8	0.34 (1)	4.18
C- D	-2280 / 0	-91.8	-91.8	0.33 (1)	4.25
D- E	-2398 / 0	-91.8	-91.8	0.43 (1)	4.03
E- F	-2397 / 0	-91.8	-91.8	0.40 (1)	4.08
F- G	-2397 / 0	-91.8	-91.8	0.40 (1)	4.08
G- H	-2397 / 0	-91.8	-91.8	0.43 (1)	4.03
H- I	-2280 / 0	-91.8	-91.8	0.33 (1)	4.25
I- J	-2356 / 0	-91.8	-91.8	0.34 (1)	4.18
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
U- B	-2066 / 0	0.0	0.0	0.21 (1)	5.89
L- J	-2066 / 0	0.0	0.0	0.21 (1)	5.89
U- T	0 / 0	-18.5	-18.5	0.09 (4)	10.00
T- S	0 / 1983	-18.5	-18.5	0.38 (1)	10.00
S- R	0 / 1874	-18.5	-18.5	0.36 (1)	10.00
R- Q	0 / 1874	-18.5	-18.5	0.36 (1)	10.00
Q- P	0 / 2398	-18.5	-18.5	0.45 (1)	10.00
P- O	0 / 1874	-18.5	-18.5	0.36 (1)	10.00
O- N	0 / 1874	-18.5	-18.5	0.36 (1)	10.00
N- M	0 / 1983	-18.5	-18.5	0.38 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.09 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-359 / 0	0.11 (1)
C- S	-152 / 0	0.11 (1)
S- D	0 / 203	0.05 (4)
D- Q	0 / 852	0.19 (1)
Q- E	-566 / 0	0.61 (1)
E- P	-2 / 0	0.00 (1)
P- G	-565 / 0	0.61 (1)
P- H	0 / 849	0.19 (1)
N- H	0 / 204	0.05 (4)
N- I	-152 / 0	0.11 (1)
M- I	-359 / 0	0.11 (1)
B- T	0 / 2033	0.46 (1)
M- J	0 / 2033	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.45/1.00 (P-Q:1) , WB=0.61/1.00 (E-Q:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

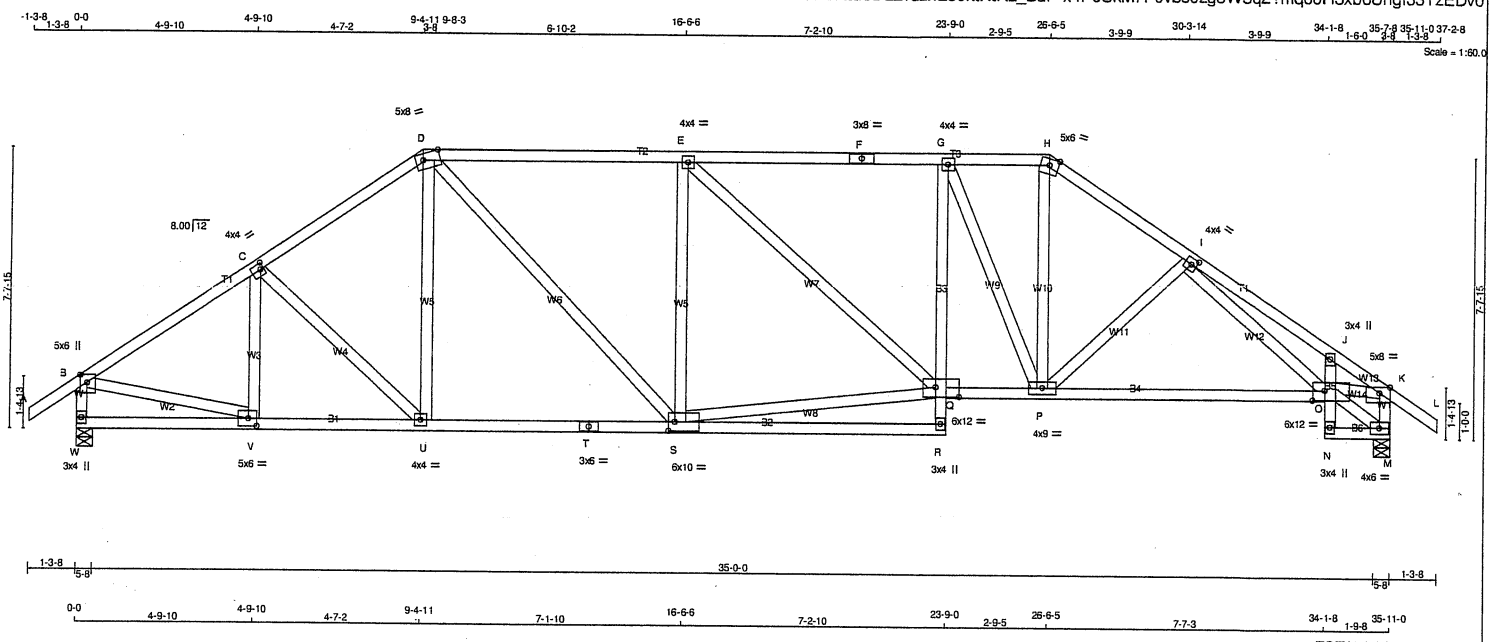
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (T) (INPUT = 0.90)
JSI METAL= 0.55 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410969	T35S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-x4P6SkM7F0vbs0zg3W5q2?mq6oH3xb6Ungf33TzEDv0



TOTAL WEIGHT = 194 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-m	MT20	5.0	8.0	Edge	5.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMV+p	MT20	3.0	4.0		
K	TMVW-p	MT20	5.0	8.0	Edge	3.50
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWWW-I	MT20	6.0	12.0	3.25	4.00
P	BMWWW-t	MT20	4.0	9.0		
Q	BVMWW-I	MT20	6.0	12.0	3.25	7.75
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	10.0	3.00	2.00
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	4.0	4.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.75
W	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



Structural component only
DWG# T-2115314

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2103	0	2103	0	0	5-8	5-8
M	2108	0	2108	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
M	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2354 / 0	-91.8	-91.8	0.34 (1)	4.18
C- D	-2276 / 0	-91.8	-91.8	0.33 (1)	4.25
D- E	-2465 / 0	-91.8	-91.8	0.70 (1)	3.61
E- F	-2541 / 0	-91.8	-91.8	0.69 (1)	3.59
F- G	-2541 / 0	-91.8	-91.8	0.69 (1)	3.59
G- H	-2187 / 0	-91.8	-91.8	0.31 (1)	4.32
H- I	-2622 / 0	-91.8	-91.8	0.26 (1)	4.07
I- J	-3035 / 0	-91.8	-91.8	0.29 (1)	3.79
J- K	-3007 / 0	-91.8	-91.8	0.18 (1)	3.89
K- L	0 / 35	-91.8	-91.8	0.12 (1)	10.00
W- B	-2064 / 0	0.0	0.0	0.21 (1)	5.89
M- K	-2032 / 0	0.0	0.0	0.21 (1)	5.93
W- V	0 / 0	-18.5	-18.5	0.09 (4)	10.00
V- U	0 / 1981	-18.5	-18.5	0.38 (1)	10.00
U- T	0 / 1872	-18.5	-18.5	0.43 (1)	10.00
T- S	0 / 1872	-18.5	-18.5	0.43 (1)	10.00
S- R	0 / 60	-18.5	-18.5	0.24 (4)	10.00
R- Q	0 / 59	0.0	0.0	0.09 (1)	10.00
Q- G	0 / 338	0.0	0.0	0.14 (1)	10.00
Q- P	0 / 2550	-18.5	-18.5	0.55 (1)	10.00
P- O	0 / 2463	-18.5	-18.5	0.54 (1)	10.00
N- O	0 / 17	0.0	0.0	0.14 (1)	10.00
O- J	-182 / 0	0.0	0.0	0.11 (1)	7.81
N- M	0 / 100	-18.5	-18.5	0.02 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-359 / 0	0.08 (1)
C- U	-153 / 0	0.08 (1)
U- D	0 / 209	0.04 (4)
G- P	-909 / 0	0.58 (1)
P- H	0 / 1247	0.20 (1)
P- I	-412 / 0	0.15 (1)
I- O	0 / 117	0.02 (1)
B- V	0 / 2031	0.33 (1)
O- M	-118 / 0	0.01 (1)
O- K	0 / 2474	0.40 (1)
D- S	0 / 861	0.14 (1)
S- E	-814 / 0	0.63 (1)
S- Q	0 / 2440	0.39 (1)
E- Q	0 / 87	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.55/1.00 (P-Q:1) , WB=0.63/1.00 (E-S:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

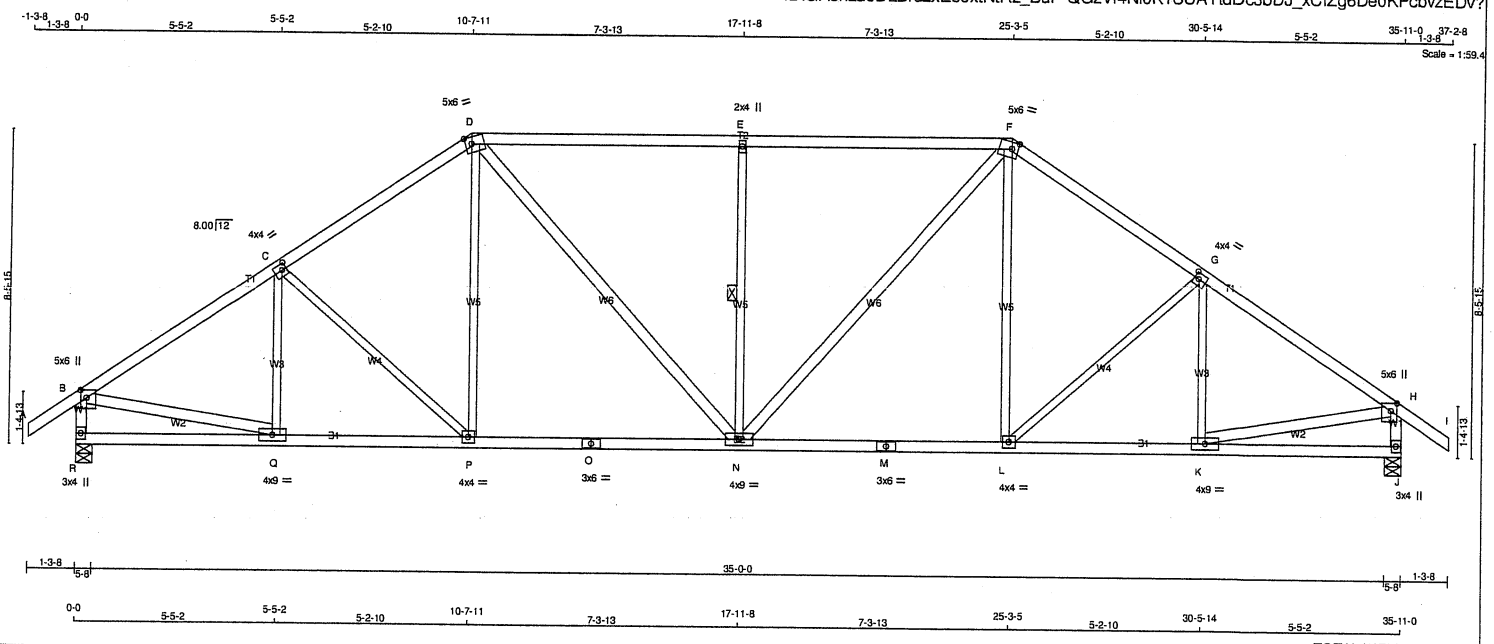
JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.94 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
B - Q	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	2.50	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	4.0	9.0		
R	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	2106	0	2106	0	0	5-8	5-8
J	2106	0	2106	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
J	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	Q- C	-301 / 0	0.11 (1)
B- C	-2389 / 0	-91.8	-91.8	0.44 (1)	4.05	C- P	-266 / 0	0.26 (1)
C- D	-2217 / 0	-91.8	-91.8	0.41 (1)	4.20	P- D	0 / 294	0.07 (1)
D- E	-2250 / 0	-91.8	-91.8	0.76 (1)	3.60	D- N	0 / 647	0.10 (1)
E- F	-2250 / 0	-91.8	-91.8	0.76 (1)	3.60	N- E	-827 / 0	0.38 (1)
F- G	-2217 / 0	-91.8	-91.8	0.41 (1)	4.20	N- F	0 / 647	0.10 (1)
G- H	-2389 / 0	-91.8	-91.8	0.44 (1)	4.05	L- F	0 / 294	0.07 (1)
H- I	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L- G	-266 / 0	0.26 (1)
R- B	-2062 / 0	0.0	0.0	0.21 (1)	5.89	K- G	-301 / 0	0.11 (1)
J- H	-2062 / 0	0.0	0.0	0.21 (1)	5.89	B- Q	0 / 2054	0.33 (1)
						K- H	0 / 2054	0.33 (1)
R- Q	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
Q- P	0 / 2014	-18.5	-18.5	0.41 (1)	10.00			
P- O	0 / 1819	-18.5	-18.5	0.40 (1)	10.00			
O- N	0 / 1819	-18.5	-18.5	0.40 (1)	10.00			
N- M	0 / 1819	-18.5	-18.5	0.40 (1)	10.00			
M- L	0 / 1819	-18.5	-18.5	0.40 (1)	10.00			
L- K	0 / 2014	-18.5	-18.5	0.41 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

TOTAL WEIGHT = 166 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.76/1.00 (D-E:1) , BC=0.41/1.00 (P-Q:1) , WB=0.38/1.00 (E-N:1) , SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.64 (O) (INPUT = 1.00)

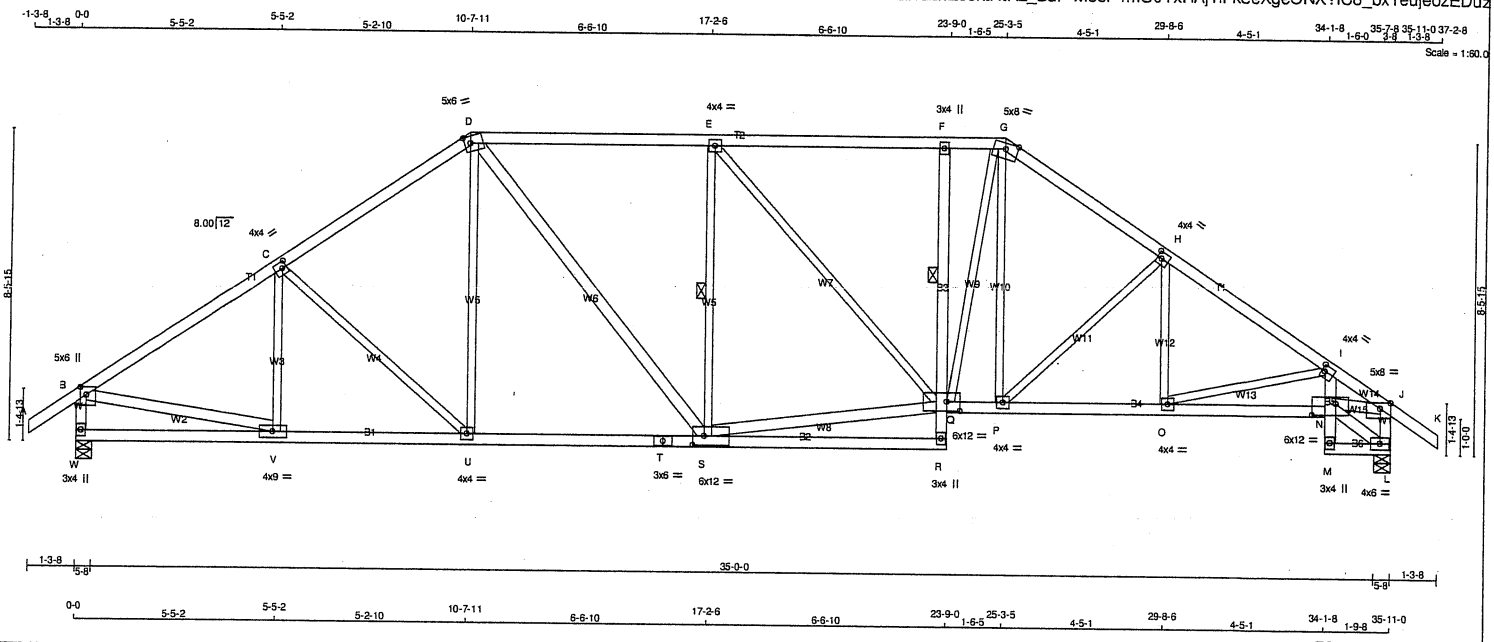


Structural component only
DWG# T-2115315

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410969	T36S	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-Me5F4mO0YxHAjTiFkeeXgeONX?IC8_bxTeujeozEDuz



TOTAL WEIGHT = 184 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - F	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
B - V	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-t	MT20	4.0	4.0	2.00	1.00
J	TMVW-p	MT20	5.0	8.0	Edge	3.50
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-l	MT20	6.0	12.0	3.75	7.75
O, P, U						
O	BMWW-t	MT20	4.0	4.0		
Q	BVMWWW-l	MT20	6.0	12.0	3.00	4.25
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	6.0	12.0	3.00	3.75
T	BS-t	MT20	3.0	6.0		
V	BMWW-t	MT20	4.0	9.0		



Structural component only
DWG# T-2115316

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2103	0	2103	0	0	5-8	5-8
L	2108	0	2108	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1485	988 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0
L	1489	990 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-Q, E-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2387 / 0	-91.8	-91.8	0.44 (1)	4.05
C- D	-2211 / 0	-91.8	-91.8	0.41 (1)	4.21
D- E	-2215 / 0	-91.8	-91.8	0.56 (1)	3.98
E- F	-2236 / 0	-91.8	-91.8	0.54 (1)	3.99
F- G	-2241 / 0	-91.8	-91.8	0.24 (1)	4.34
G- H	-2497 / 0	-91.8	-91.8	0.31 (1)	4.10
H- I	-2918 / 0	-91.8	-91.8	0.35 (1)	3.81
I- J	-3406 / 0	-91.8	-91.8	0.21 (1)	3.66
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
W- B	-2060 / 0	0.0	0.0	0.21 (1)	5.89
L- J	-2014 / 0	0.0	0.0	0.21 (1)	5.95
W- V	0 / 0	-18.5	-18.5	0.12 (4)	10.00
V- U	0 / 2012	-18.5	-18.5	0.39 (1)	10.00
U- T	0 / 1814	-18.5	-18.5	0.40 (1)	10.00
T- S	0 / 1814	-18.5	-18.5	0.40 (1)	10.00
S- R	0 / 46	-18.5	-18.5	0.20 (4)	10.00
R- Q	0 / 53	0.0	0.0	0.07 (1)	10.00
Q- F	-424 / 0	0.0	0.0	0.07 (1)	6.25
Q- P	0 / 2059	-18.5	-18.5	0.37 (1)	10.00
P- O	0 / 2448	-18.5	-18.5	0.44 (1)	10.00
O- N	0 / 2940	-18.5	-18.5	0.52 (1)	10.00
M- N	0 / 17	0.0	0.0	0.18 (1)	10.00
N- I	0 / 173	0.0	0.0	0.20 (1)	10.00
M- L	0 / 114	-18.5	-18.5	0.03 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-296 / 0	0.11 (1)
C- U	-270 / 0	0.26 (1)
U- D	0 / 283	0.06 (1)
D- S	0 / 648	0.10 (1)
S- E	-707 / 0	0.33 (1)
S- Q	0 / 2203	0.35 (1)
E- Q	0 / 19	0.00 (1)
Q- G	0 / 808	0.18 (1)
P- G	0 / 447	0.10 (1)
P- H	-541 / 0	0.37 (1)
O- H	0 / 122	0.04 (4)
O- I	-507 / 0	0.19 (1)
B- V	0 / 2052	0.33 (1)
N- L	-138 / 0	0.01 (1)
N- J	0 / 2816	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.56/1.00 (D-E:1) , BC=0.52/1.00 (N-O:1) , WB=0.45/1.00 (J-N:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.56 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T36S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-Me5F4mO0YxHAiTiFkeeXgeONX?IC8_bxTeujeozEDuz

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

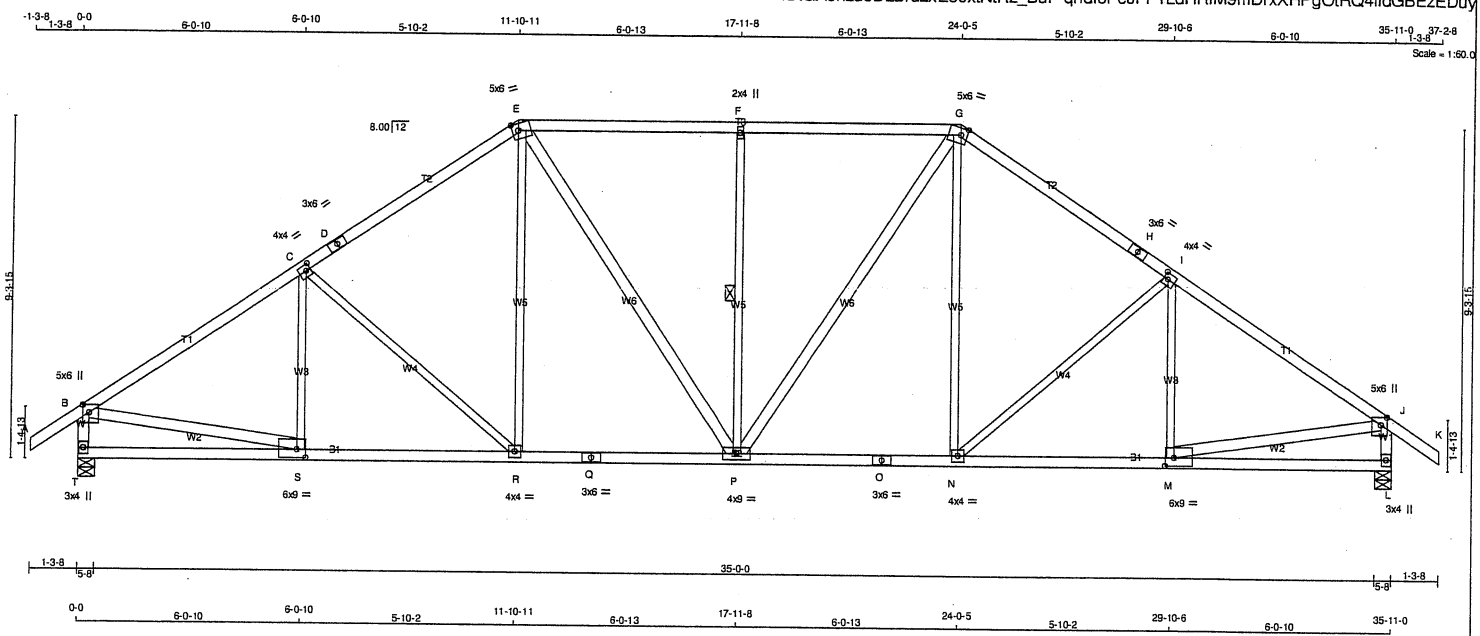
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115316

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T37	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 172 = 344 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF
B - S	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW+p	MT20	5.0	6.0	2.50	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	6.0	9.0	2.75	3.00
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	6.0	9.0	2.75	3.00
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1)



Structural component only
DWG# T-2115317

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2106	0	2106	0	0	5-8	5-8
L	2106	0	2106	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1487	989 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2410 / 0	-91.8	-91.8	0.56 (1)	3.89
C- D	-2140 / 0	-91.8	-91.8	0.52 (1)	4.13
D- E	-2140 / 0	-91.8	-91.8	0.52 (1)	4.13
E- F	-2018 / 0	-91.8	-91.8	0.50 (1)	4.22
F- G	-2018 / 0	-91.8	-91.8	0.50 (1)	4.22
G- H	-2140 / 0	-91.8	-91.8	0.52 (1)	4.13
H- I	-2140 / 0	-91.8	-91.8	0.52 (1)	4.13
I- J	-2410 / 0	-91.8	-91.8	0.56 (1)	3.89
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
T- B	-2059 / 0	0.0	0.0	0.21 (1)	5.89
L- J	-2059 / 0	0.0	0.0	0.21 (1)	5.89
T- S	0 / 0	-18.5	-18.5	0.15 (4)	10.00
S- R	0 / 2035	-18.5	-18.5	0.40 (1)	10.00
R- Q	0 / 1752	-18.5	-18.5	0.35 (1)	10.00
Q- P	0 / 1752	-18.5	-18.5	0.35 (1)	10.00
P- O	0 / 1752	-18.5	-18.5	0.35 (1)	10.00
O- N	0 / 1752	-18.5	-18.5	0.35 (1)	10.00
N- M	0 / 2035	-18.5	-18.5	0.40 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-243 / 16	0.11 (1)
C- R	-379 / 0	0.48 (1)
R- E	0 / 357	0.08 (1)
E- P	0 / 477	0.08 (1)
P- F	-682 / 0	0.39 (1)
P- G	0 / 477	0.08 (1)
N- G	0 / 357	0.08 (1)
N- I	-379 / 0	0.48 (1)
M- I	-243 / 16	0.11 (1)
B- S	0 / 2067	0.33 (1)
M- J	0 / 2067	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.40/1.00 (R-S:1) , WB=0.48/1.00 (C-R:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

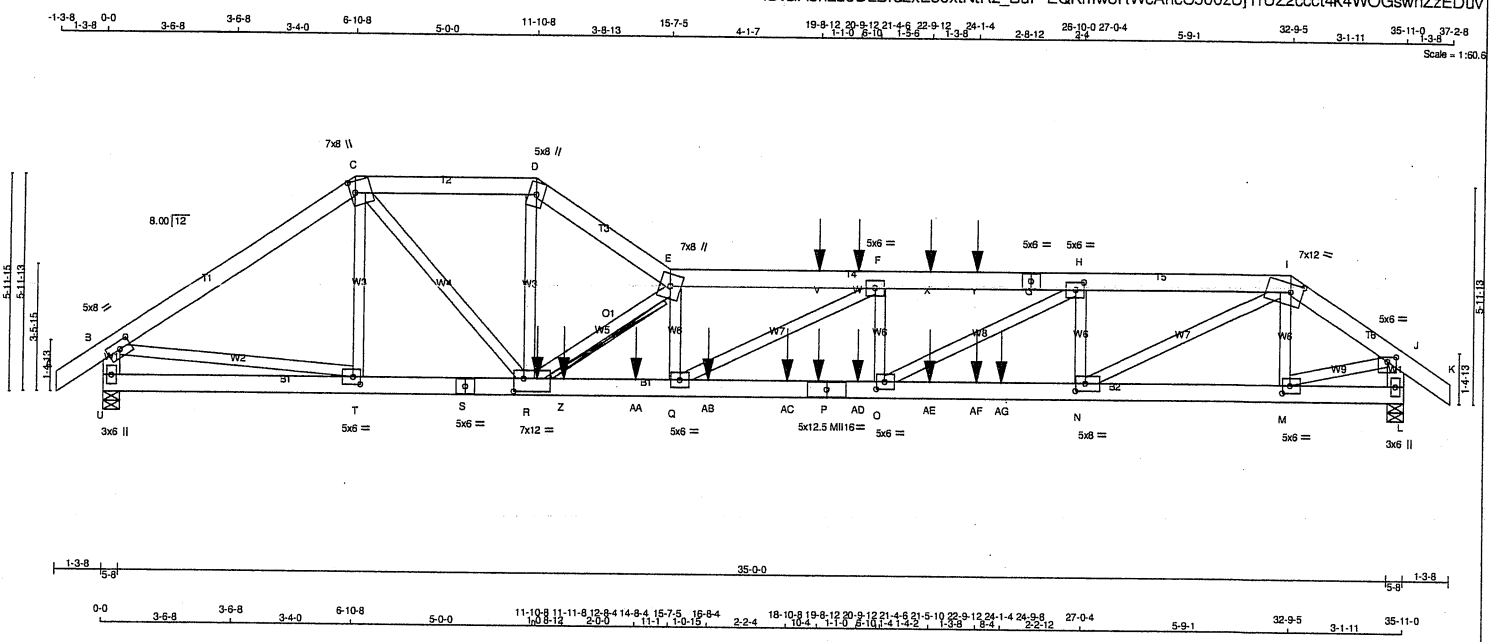
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.55 (Q) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410969	T38	1	2	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-EQKmw8RWcAncC500zUjTrUZ2ccct4k4WOGswnZzEDuv



TOTAL WEIGHT = 2 X 210 = 421 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - S	2x6	DRY	1650F 1.5E	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - L	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
R - E	2x4	DRY	2100F 1.8E	SPF

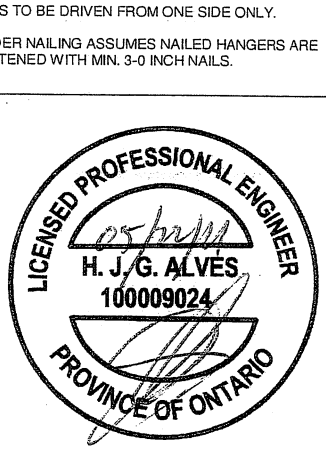
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- D	2	12	TOP
D- E	2	12	TOP
E- G	2	12	SIDE(0.0)
G- I	2	12	TOP
I- K	2	12	TOP
U- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- S	2	12	TOP
S- P	2	12	SIDE(183.1)
P- L	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
R- D	1	6	SIDE(374.6)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.



Structural component only
DWG# T-2115318

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	4492	0	4492	0	0	5-8	5-8
L	4297	0	4297	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	3164	2150 / 0	0 / 0	0 / 0	0 / 0	1014 / 0	0 / 0
L	3027	2049 / 0	0 / 0	0 / 0	0 / 0	978 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 36	-91.8	-91.8	0.04 (1)	10.00
B- C	-6021 / 0	-91.8	-91.8	0.29 (1)	4.58
C- D	-7897 / 0	-91.8	-91.8	0.20 (1)	4.16
D- E	-9338 / 0	-91.8	-91.8	0.18 (1)	3.89
E- V	-15302 / 0	-91.8	-91.8	0.56 (1)	2.78
V- W	-15302 / 0	-91.8	-91.8	0.56 (1)	2.78
W- F	-15302 / 0	-91.8	-91.8	0.56 (1)	2.78
F- X	-14934 / 0	-91.8	-91.8	0.53 (1)	2.84
X- Y	-14934 / 0	-91.8	-91.8	0.53 (1)	2.84
Y- G	-14934 / 0	-91.8	-91.8	0.53 (1)	2.84
G- H	-14934 / 0	-91.8	-91.8	0.53 (1)	2.84
H- I	-10959 / 0	-91.8	-91.8	0.32 (1)	3.52
I- J	-5094 / 0	-91.8	-91.8	0.08 (1)	5.12
J- K	0 / 36	-91.8	-91.8	0.04 (1)	10.00
U- B	-4436 / 0	0.0	0.0	0.16 (1)	6.82
L- J	-4274 / 0	0.0	0.0	0.15 (1)	6.93
U- T	0 / 0	-18.5	-18.5	0.04 (4)	10.00
T- S	0 / 4989	-18.5	-18.5	0.23 (1)	10.00
S- R	0 / 4989	-18.5	-18.5	0.23 (1)	10.00
R- Z	0 / 15295	-18.5	-18.5	0.67 (1)	10.00
Z-AA	0 / 15295	-18.5	-18.5	0.67 (1)	10.00
AA- Q	0 / 15295	-18.5	-18.5	0.67 (1)	10.00
Q-AB	0 / 14934	-18.5	-18.5	0.80 (1)	10.00
AB-AC	0 / 14934	-18.5	-18.5	0.80 (1)	10.00
AC- P	0 / 14934	-18.5	-18.5	0.80 (1)	10.00
P-AD	0 / 14934	-18.5	-18.5	0.80 (1)	10.00
AD- O	0 / 14934	-18.5	-18.5	0.80 (1)	10.00
O-AE	0 / 10959	-18.5	-18.5	0.75 (1)	10.00
AE-AF	0 / 10959	-18.5	-18.5	0.75 (1)	10.00
AF-AG	0 / 10959	-18.5	-18.5	0.75 (1)	10.00
AG- N	0 / 10959	-18.5	-18.5	0.75 (1)	10.00
N- M	0 / 4171	-18.5	-18.5	0.24 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-490 / 0	0.09 (1)
C- R	0 / 4396	0.39 (1)
R- D	0 / 4781	0.42 (1)
R- E	-9528 / 0	0.61 (1)
Q- E	0 / 157	0.02 (4)
M- I	-1106 / 0	0.08 (1)
B- T	0 / 5043	0.45 (1)
M- J	0 / 4378	0.39 (1)
N- I	0 / 7650	0.68 (1)
Q- F	0 / 418	0.04 (1)
N- H	-2750 / 0	0.20 (1)
O- F	-1015 / 0	0.07 (1)
O- H	0 / 4528	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.39")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 602 (0.72")

CSI: TC=0.56/1.00 (E-F:1) , BC=0.80/1.00 (O-Q:1) , WB=0.68/1.00 (I-N:1) , SSI=0.23/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MII16	438	302	2547	1256	4283	1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.97 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T38	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW+m	MT20	7.0	8.0	3.75	1.50
D	TTW+m	MT20	5.0	8.0		
E	TTWW+m	MT20	7.0	8.0		
F	TMWW-t	MT20	5.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0	2.50	2.75
I	TTWW-m	MT20	7.0	12.0	2.75	4.00
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BMWW-t	MT20	5.0	8.0	2.50	3.25
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P	BS-t	MII16	5.0	12.5		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	7.0	12.0	4.25	3.25
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	19-8-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
R	11-11-8	-1493	-1493	---	BACK	VERT	TOTAL	---	C1
V	19-8-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
W	20-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
X	22-9-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Y	24-1-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
Z	12-8-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AA	14-8-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AB	16-8-4	-37	-37	---	BACK	VERT	TOTAL	---	C1
AC	18-10-8	-450	-450	---	BACK	VERT	TOTAL	---	C1
AD	20-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	22-9-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AF	24-1-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AG	24-9-8	-772	-772	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

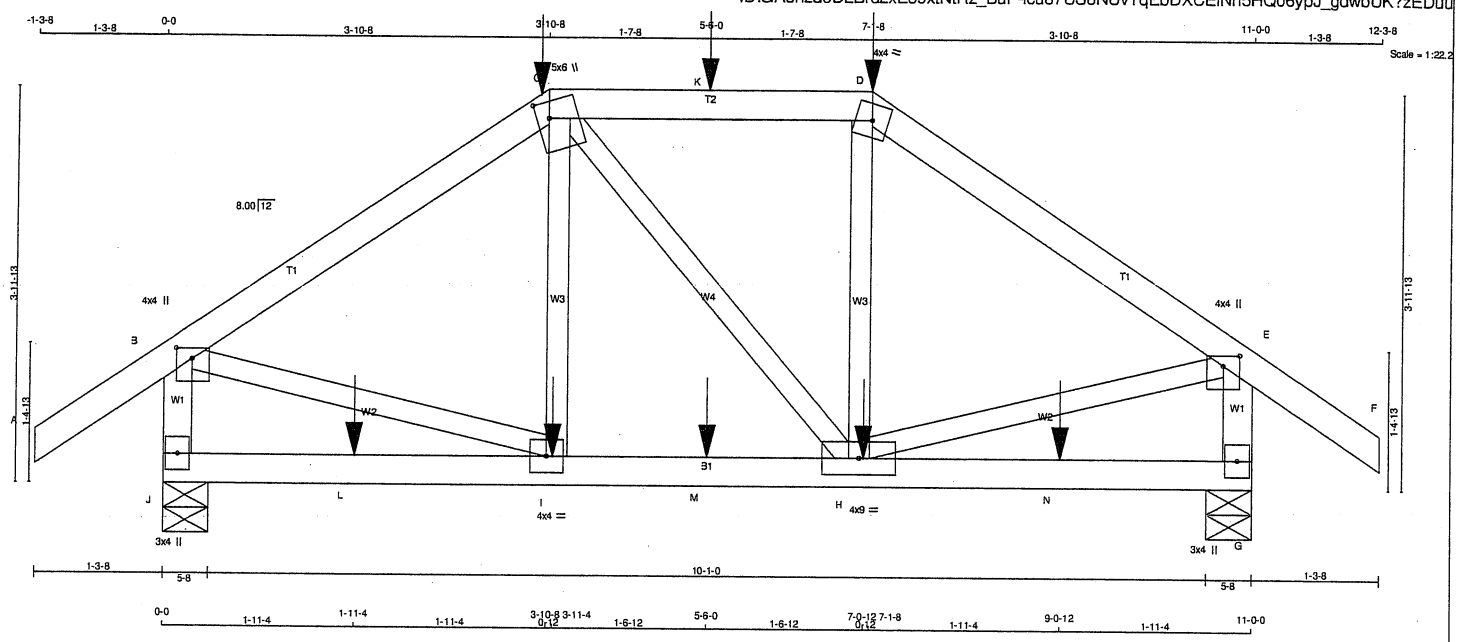
LICENSED PROFESSIONAL ENGINEER
05/22/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115318

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T39	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 48 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1028	0	1028	0	0	5-8	5-8
G	1028	0	1028	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	725	488 / 0	0 / 0	0 / 0	0 / 0	236 / 0	0 / 0
G	724	488 / 0	0 / 0	0 / 0	0 / 0	236 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.14 (1)		10.00
B- C	-875 / 0	-91.8	-91.8	0.28 (1)		6.15
C- K	-725 / 0	-91.8	-91.8	0.30 (1)		6.25
K- D	-725 / 0	-91.8	-91.8	0.30 (1)		6.25
D- E	-878 / 0	-91.8	-91.8	0.28 (1)		6.15
E- F	0 / 35	-91.8	-91.8	0.14 (1)		10.00
J- B	-989 / 0	0.0	0.0	0.11 (1)		7.81
G- E	-988 / 0	0.0	0.0	0.11 (1)		7.81
J- L	0 / 0	-18.5	-18.5	0.10 (4)		10.00
L- I	0 / 0	-18.5	-18.5	0.10 (4)		10.00
I- M	0 / 725	-18.5	-18.5	0.17 (1)		10.00
M- H	0 / 725	-18.5	-18.5	0.17 (1)		10.00
H- N	0 / 0	-18.5	-18.5	0.10 (4)		10.00
N- G	0 / 0	-18.5	-18.5	0.10 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-92 / 51	0.02 (1)
C- H	0 / 2	0.00 (4)
H- D	-90 / 53	0.02 (1)
B- I	0 / 753	0.19 (1)
H- E	0 / 755	0.19 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-145	-145	---	FRONT	VERT	TOTAL	---	C1
D	7-1-8	-145	-145	---	FRONT	VERT	TOTAL	---	C1
H	7-0-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
I	3-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
K	5-6-0	-59	-59	---	FRONT	VERT	TOTAL	---	C1
L	1-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
M	5-6-0	-14	-14	---	FRONT	VERT	TOTAL	---	C1
N	9-0-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.17/1.00 (H-I:1) , WB=0.19/1.00 (E-H:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.25 (I) (INPUT = 1.00)

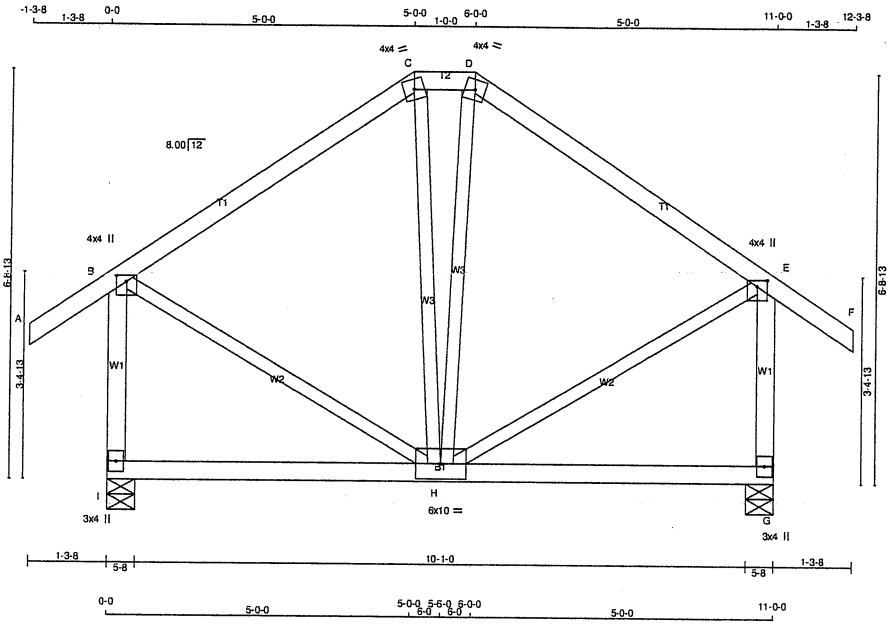


Structural component only
DWG# T-2115319

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T40	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-BoSWLpTn8n1KROAP5vlxwveSCQSIYn2psaL1sRzEDut



TOTAL WEIGHT = 58 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWWW"-I	MT20	6.0	10.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	732	0	732	0	0	5-8	5-8
G	732	0	732	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
G	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-331 / 0	-91.8	-91.8	0.29 (1)	6.25
C- D	-268 / 0	-91.8	-91.8	0.01 (1)	6.25
D- E	-331 / 0	-91.8	-91.8	0.29 (1)	6.25
E- F	0 / 35	-91.8	-91.8	0.12 (1)	10.00
I- B	-693 / 0	0.0	0.0	0.14 (1)	7.81
G- E	-693 / 0	0.0	0.0	0.14 (1)	7.81
I- H	0 / 0	-18.5	-18.5	0.16 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.16 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- H	0 / 315	0.07 (1)
H- E	0 / 315	0.07 (1)
C- H	-91 / 16	0.07 (1)
H- D	-91 / 16	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.16/1.00 (G-H:4) , WB=0.07/1.00 (E-H:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

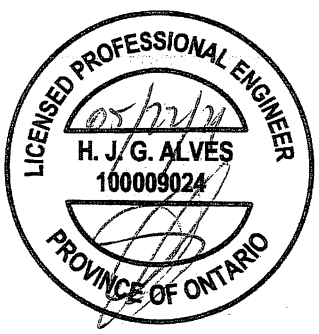
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

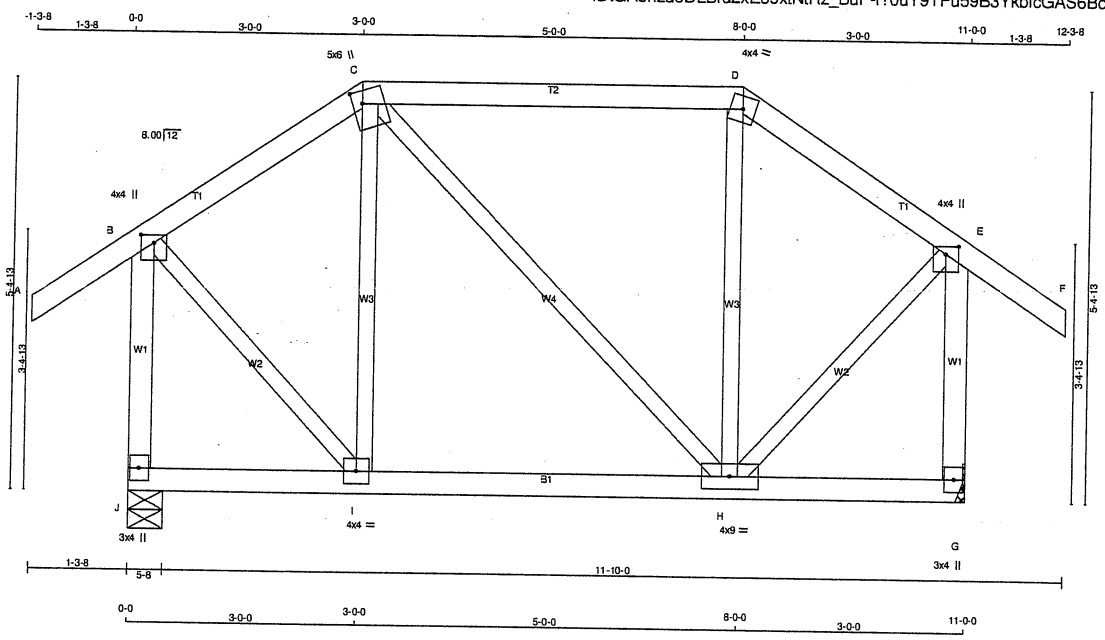


Structural component only
DWG# T-2115320

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T41	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-f?0uY9TPu59B3YkbfcGAS6BdzqoZHE3z4E4aOuzEDus



TOTAL WEIGHT = 57 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	732	0	732	0	0	5-8	5-8
G	732	0	732	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
G	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-325 / 0	-91.8	-91.8	0.11 (1)	6.25
C- D	-266 / 0	-91.8	-91.8	0.29 (1)	6.25
D- E	-325 / 0	-91.8	-91.8	0.11 (1)	6.25
E- F	0 / 35	-91.8	-91.8	0.12 (1)	10.00
J- B	-713 / 0	0.0	0.0	0.14 (1)	7.81
G- E	-712 / 0	0.0	0.0	0.14 (1)	7.81
J- I	0 / 0	-18.5	-18.5	0.07 (4)	10.00
I- H	0 / 266	-18.5	-18.5	0.09 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-185 / 0	0.08 (1)
C- H	0 / 0	0.00 (1)
H- D	-186 / 0	0.08 (1)
B- I	0 / 380	0.09 (1)
H- E	0 / 380	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.29/1.00 (C-D:1) , BC=0.09/1.00 (H-I:4) , WB=0.09/1.00 (B-I:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

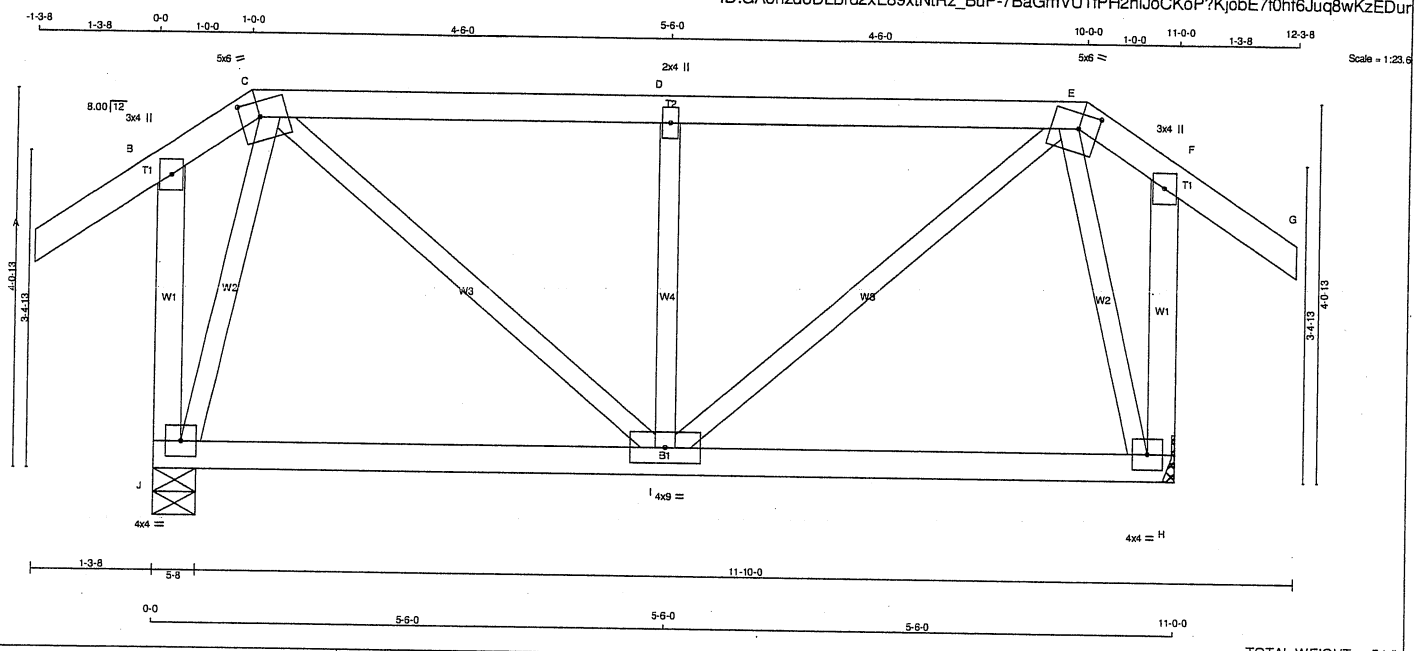


Structural component only
DWG# T-2115321

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 54 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.00	2.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	2.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	732	0	732	0	0	IN-SX	IN-SX
H	732	0	732	0	0	5-8	5-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
H	516	352 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-46 / 0	-91.8	-91.8	0.12 (1)	6.25
C- D	-492 / 0	-91.8	-91.8	0.30 (1)	6.25
D- E	-492 / 0	-91.8	-91.8	0.30 (1)	6.25
E- F	-46 / 0	-91.8	-91.8	0.12 (1)	6.25
F- G	0 / 35	-91.8	-91.8	0.12 (1)	10.00
J- B	-257 / 0	0.0	0.0	0.05 (1)	7.81
H- F	-257 / 0	0.0	0.0	0.05 (1)	7.81
J- I	0 / 121	-18.5	-18.5	0.17 (4)	10.00
I- H	0 / 121	-18.5	-18.5	0.17 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	-506 / 0	0.13 (1)
J- C	-452 / 0	0.12 (1)
E- H	-452 / 0	0.12 (1)
C- I	0 / 487	0.11 (1)
I- E	0 / 487	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.17/1.00 (I-J:4) , WB=0.13/1.00 (D-I:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (E) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)

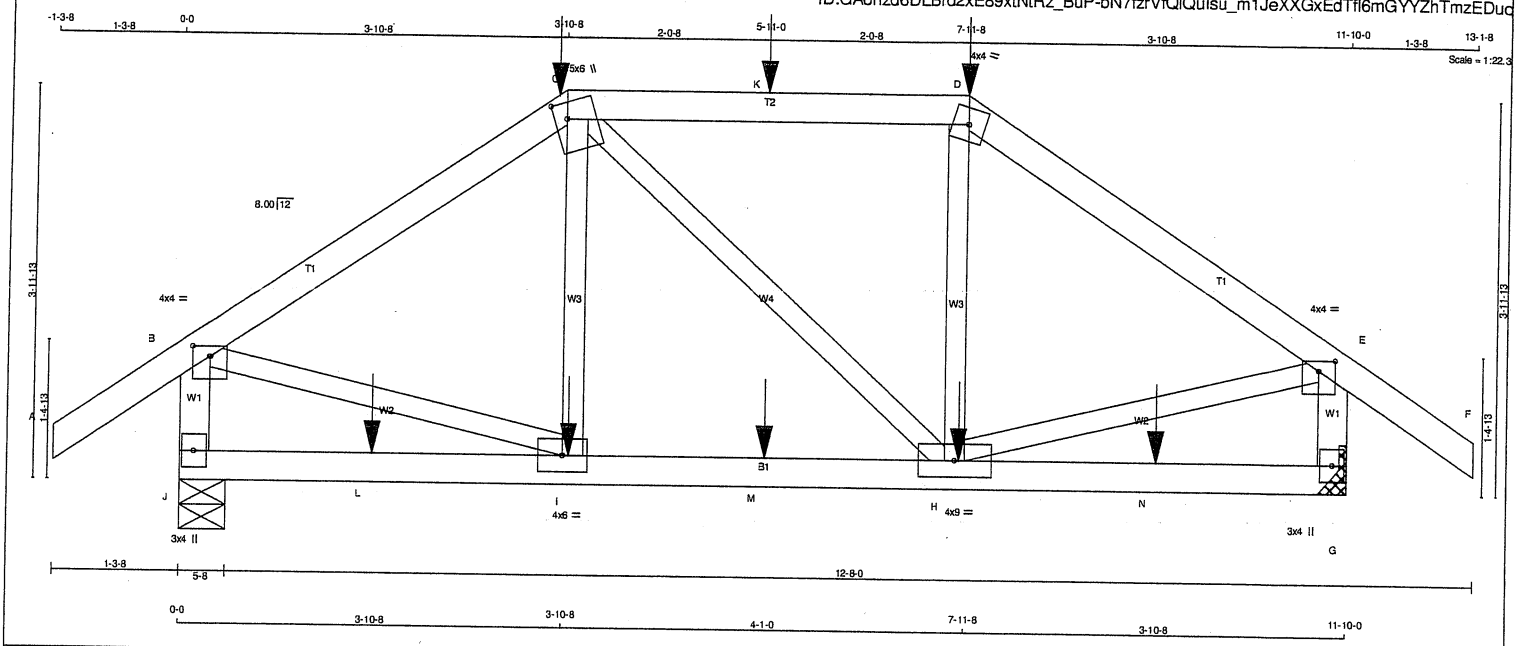


Structural component only
DWG# T-2115322

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T43	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-bN7fzrVfQiQuIsu_m1JeXXGxEdTfl6mGYYZhTmzEDuc



TOTAL WEIGHT = 51 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW-p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1074	0	1074	0	0	5-8	5-8
G	1073	0	1073	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	757	510 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0
G	757	510 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00
B- C	-936 / 0	-91.8	-91.8	0.28 (1)	6.00
C- K	-776 / 0	-91.8	-91.8	0.44 (1)	6.12
K- D	-776 / 0	-91.8	-91.8	0.44 (1)	6.12
D- E	-939 / 0	-91.8	-91.8	0.28 (1)	5.99
E- F	0 / 35	-91.8	-91.8	0.14 (1)	10.00
J- B	-1036 / 0	0.0	0.0	0.12 (1)	7.69
G- E	-1035 / 0	0.0	0.0	0.12 (1)	7.69
J- L	0 / 0	-18.5	-18.5	0.09 (4)	10.00
L- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00
I- M	0 / 774	-18.5	-18.5	0.18 (1)	10.00
M- H	0 / 774	-18.5	-18.5	0.18 (1)	10.00
H- N	0 / 0	-18.5	-18.5	0.10 (4)	10.00
N- G	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-97 / 56	0.03 (1)
C- H	0 / 2	0.00 (4)
H- D	-94 / 58	0.02 (1)
B- I	0 / 805	0.20 (1)
H- E	0 / 807	0.20 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-145	-145	---	BACK	VERT	TOTAL	---	C1
D	7-11-8	-145	-145	---	BACK	VERT	TOTAL	---	C1
H	7-10-12	-14	-14	---	BACK	VERT	TOTAL	---	C1
I	3-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
K	5-11-0	-59	-59	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
M	5-11-0	-14	-14	---	BACK	VERT	TOTAL	---	C1
N	9-10-12	-14	-14	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 .

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.44/1.00 (C-D:1) , BC=0.18/1.00 (H-I:1) , WB=0.20/1.00 (E-H:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

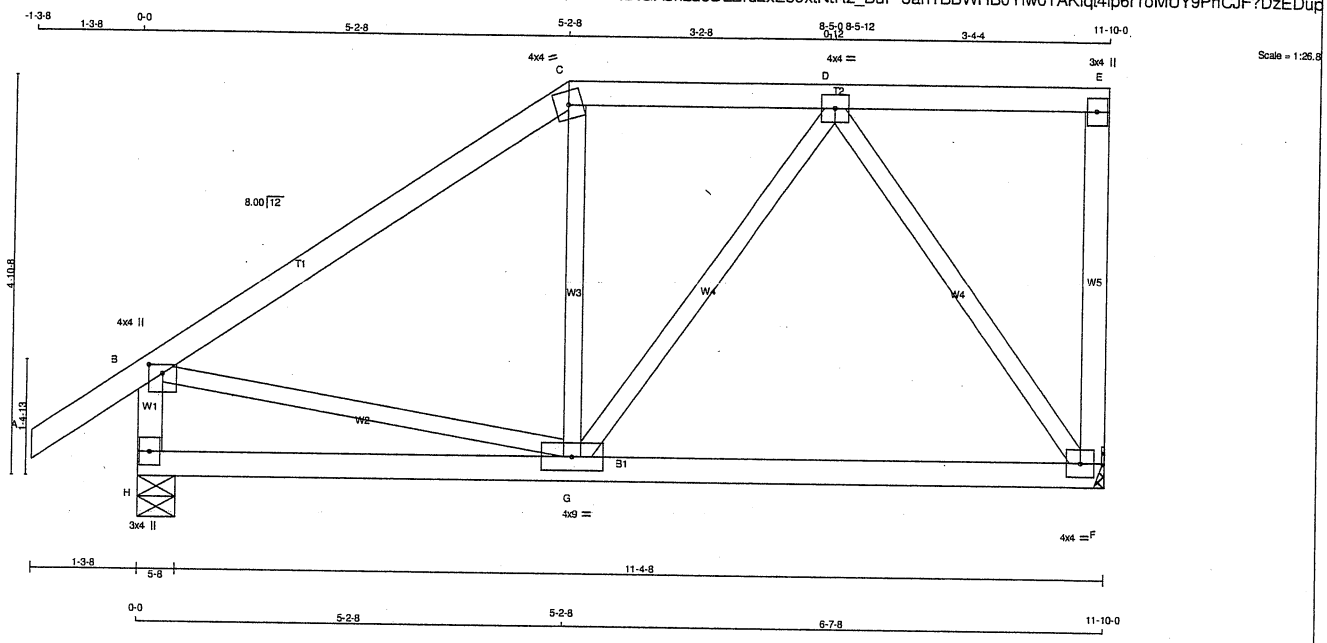
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115323

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410969	T44	1	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	652	0	652	0	0	MECHANICAL	
H	778	0	778	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	461	303 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0
H	548	373 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-523 / 0	-91.8	-91.8	0.45 (1)	6.25
C- D	-432 / 0	-91.8	-91.8	0.17 (1)	6.25
D- E	0 / 0	-91.8	-91.8	0.16 (1)	10.00
F- E	-118 / 0	0.0	0.0	0.05 (1)	7.81
H- B	-743 / 0	0.0	0.0	0.08 (1)	7.81
H- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00
G- F	0 / 356	-18.5	-18.5	0.22 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-63 / 42	0.02 (1)
G- D	0 / 135	0.03 (4)
D- F	-603 / 0	0.32 (1)
B- G	0 / 443	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.22/1.00 (F-G:4) , WB=0.32/1.00 (D-F:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

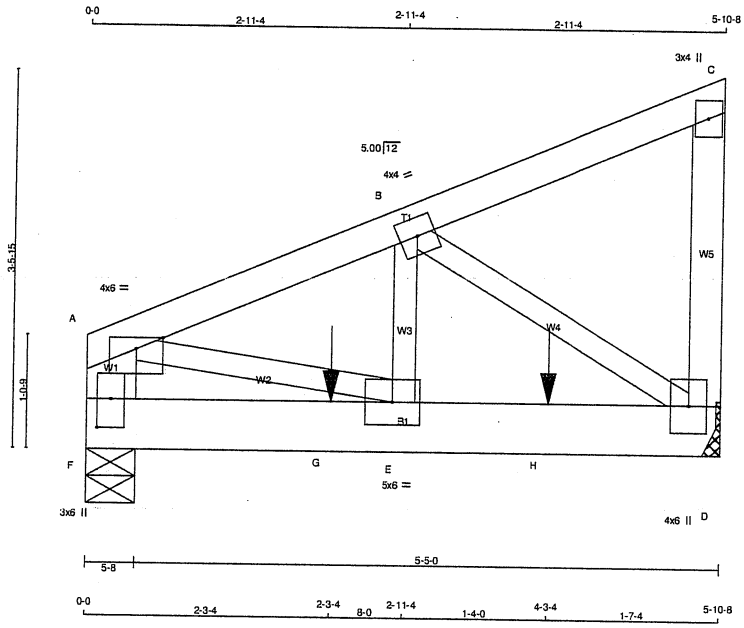
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115324

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T45	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:08:27 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-XmFPOXXvyKgcY92MuSL6dyLNfR8PD10Y?s2oXfzEDuc



TOTAL WEIGHT = 2 X 28 = 55 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		



Structural component only
DWG# T-2115325 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	957	0	957	0	0	5-8	5-8
D	1119	0	1119	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	673	462 / 0	0 / 0	0 / 0	0 / 0	211 / 0	0 / 0
D	787	542 / 0	0 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-842 / 0	0.0	0.0	0.03 (1)	7.81		A- E	0 / 1168	0.14 (1)
A- B	-1226 / 0	-91.8	-91.8	0.06 (1)	6.25		E- B	0 / 890	0.11 (1)
B- C	-9 / 0	-91.8	-91.8	0.05 (1)	10.00		B- D	-1354 / 0	0.15 (1)
D- C	-112 / 0	0.0	0.0	0.01 (1)	7.81				
F- G	0 / 0	-18.5	-18.5	0.08 (1)	10.00				
G- E	0 / 0	-18.5	-18.5	0.08 (1)	10.00				
E- H	0 / 1140	-18.5	-18.5	0.23 (1)	10.00				
H- D	0 / 1140	-18.5	-18.5	0.23 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-3-4	-501	-501	---	BACK	VERT	TOTAL	---	C1
H	4-3-4	-501	-501	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.23/1.00 (D-E:1) , WB=0.15/1.00 (B-D:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.90)
JSI METAL= 0.26 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T45	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0	3.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

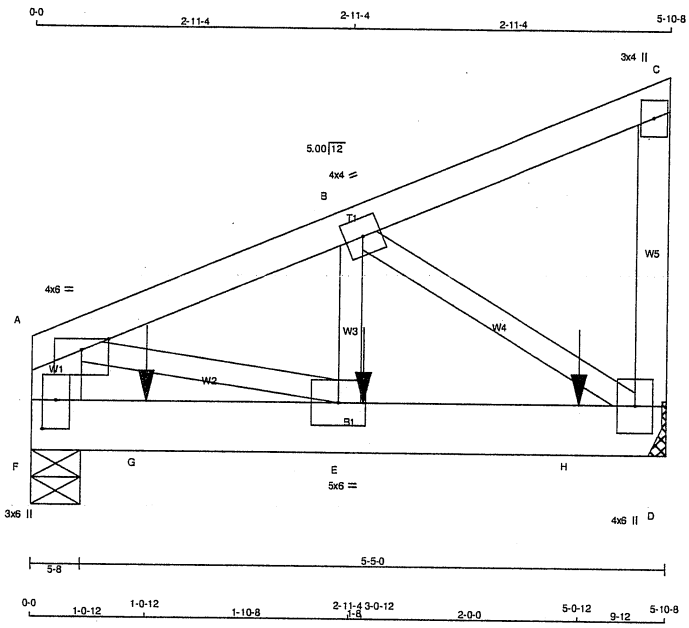


Structural component only
DWG# T-2115325 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T45Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:08:28 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-?ypnbtXXjdoT9JdZRAsL9AuXHrWhyVXiEWoL35zEDun



TOTAL WEIGHT = 2 X 28 = 55 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		



Structural component only
DWG# T-2115326

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	525	0	525	0	0	5-8	5-8
D	666	0	666	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	368	261 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
D	465	340 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-447 / 0	0.0	0.0	0.02 (1)	7.81
A- B	-566 / 0	-91.8	-91.8	0.06 (1)	6.25
B- C	-11 / 0	-91.8	-91.8	0.06 (1)	6.25
D- C	-106 / 0	0.0	0.0	0.01 (1)	7.81
F- G	0 / 0	-18.5	-18.5	0.03 (1)	10.00
G- E	0 / 0	-18.5	-18.5	0.03 (1)	10.00
E- H	0 / 533	-18.5	-18.5	0.10 (1)	10.00
H- D	0 / 533	-18.5	-18.5	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 546	0.07 (1)
E- B	0 / 236	0.03 (1)
B- D	-633 / 0	0.07 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-124	-124	---	FRONT	VERT	TOTAL	---	C1
G	1-0-12	-61	-61	---	FRONT	VERT	TOTAL	---	C1
H	5-0-12	-189	-189	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.10/1.00 (D-E:1) , WB=0.07/1.00 (B-D:1) , SSI=0.07/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T45Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:GAonzd6DLBrd2xE89xtNtRz_BuP-?ypnbtXXjdoT9JdZRAsL9AuXHrWhyVXiEWoL35zEDun

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	6.0	3.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

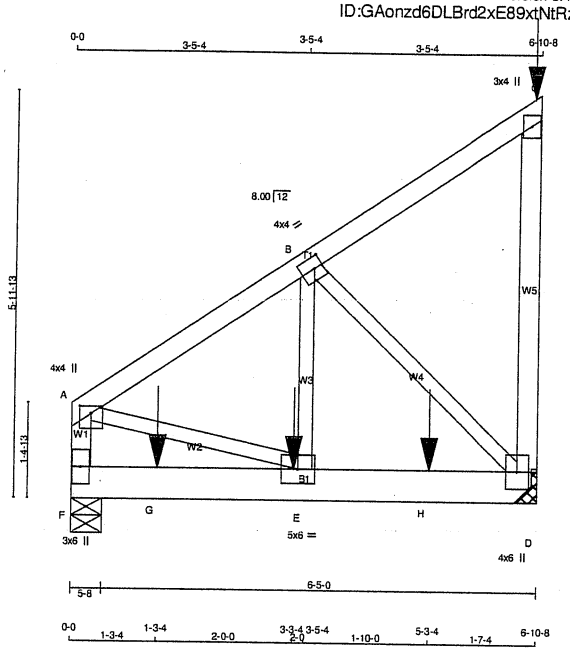
LICENSED PROFESSIONAL ENGINEER
05/22/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115326

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T46	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-U9N9pDYAUxwKnTCl?tNaiNRfzFpXhwurTAXvcXzEDum



TOTAL WEIGHT = 2 X 37 = 74 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- C	1	12	SIDE(61.0)
C- D	1	12	SIDE(125.8)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(92.1)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		



Structural component only
DWG# T-2115327

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1265	0	1265	0	0	5-8	5-8
D	2156	0	2156	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	889	618 / 0	0 / 0	0 / 0	0 / 0	271 / 0	0 / 0
D	1507	1088 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-1116 / 0	0.0	0.0	0.06 (1)	7.81
A- B	-1229 / 0	-91.8	-91.8	0.10 (1)	6.25
B- C	-20 / 0	-91.8	-91.8	0.09 (1)	6.25
D- C	-838 / 0	0.0	0.0	0.26 (1)	7.81
F- G	0 / 0	-18.5	-18.5	0.06 (1)	10.00
G- E	0 / 0	-18.5	-18.5	0.06 (1)	10.00
E- H	0 / 1039	-18.5	-18.5	0.25 (1)	10.00
H- D	0 / 1039	-18.5	-18.5	0.25 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 1082	0.13 (1)
E- B	0 / 1301	0.16 (1)
B- D	-1438 / 0	0.26 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	6-10-8	-493	-493	---	BACK	VERT	TOTAL	---	C1
E	3-3-4	-742	-742	---	FRONT	VERT	TOTAL	---	C1
G	1-3-4	-179	-179	---	TOP	VERT	TOTAL	---	C1
H	5-3-4	-447	-447	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.26/1.00 (C-D:1) , BC=0.25/1.00 (D-E:1) , WB=0.26/1.00 (B-D:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.23 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	T46	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

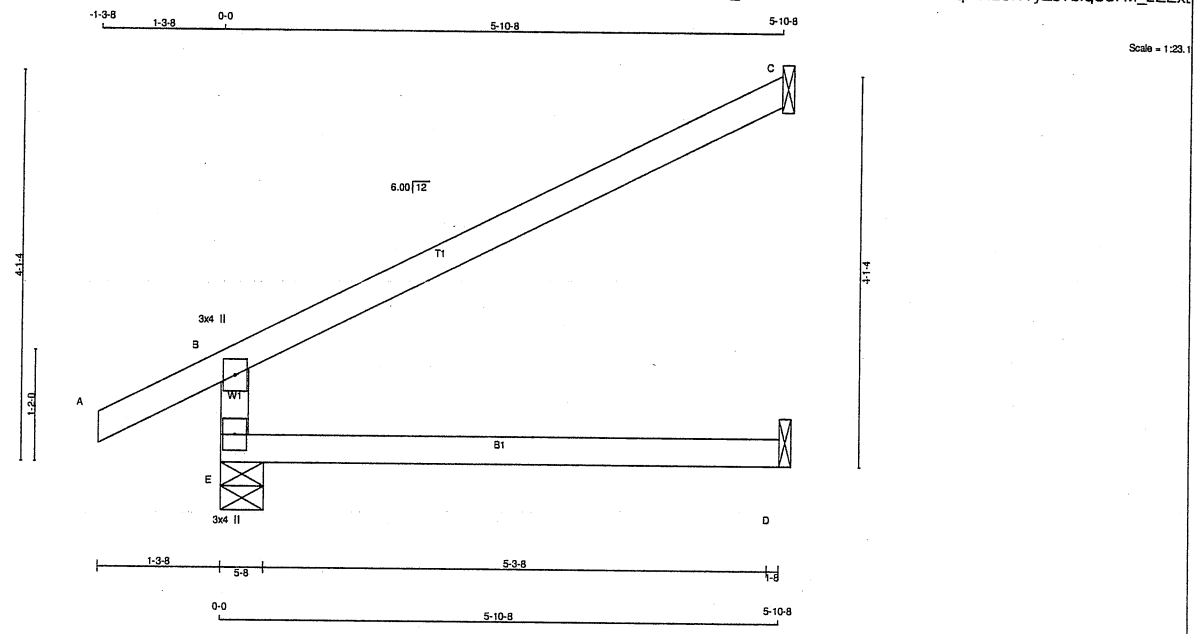


Structural component only
DWG# T-2115327

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	J1	17	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-ksMTOca3zIok?QMHuLqN7f2cWvyZsTbfqSd7M_zEExt



TOTAL WEIGHT = 17 X 17 = 286 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-461 / 0	0.0	0.0	0.13	(4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12	(1)	10.00			
B- C	-30 / 0	-91.8	-91.8	0.54	(1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

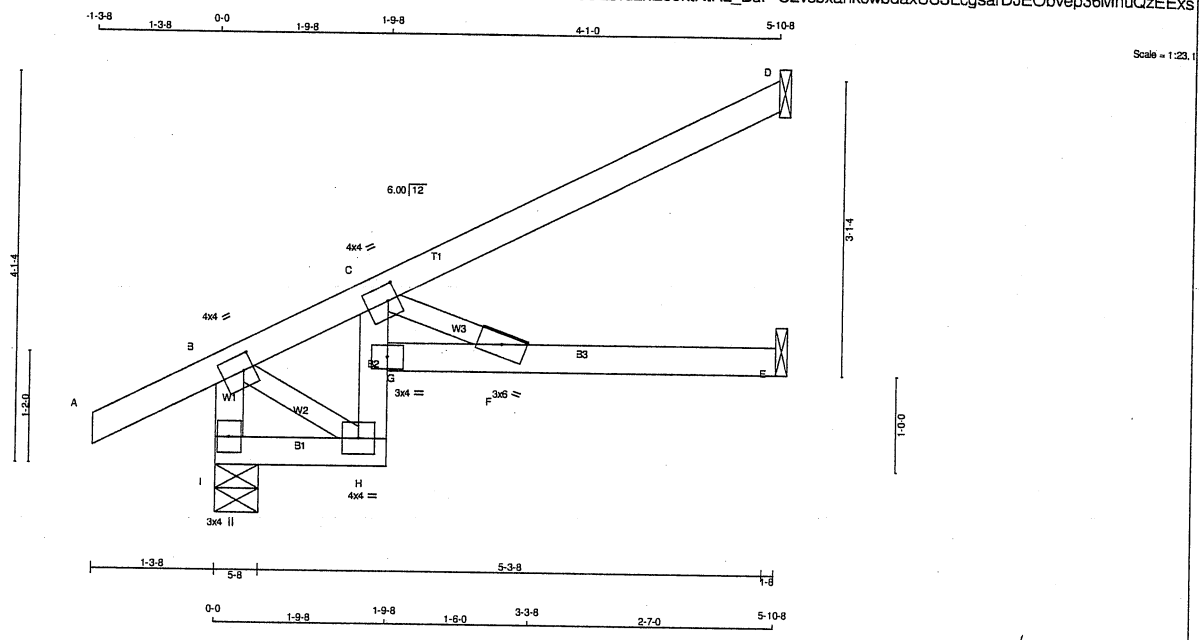


Structural component only
DWG# T-2115254

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	J1S	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 6 X 21 = 125 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMW+w	MT20	3.0	6.0		
G	BVM-I	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	463	0	463	0	0	5-8	5-8
D	196	0	196	0	0	1-8	1-8
E	113	0	113	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	325	228 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	136	105 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	83	37 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
I- B	-448 / 0	0.0	0.0	0.05 (1)	7.81
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-234 / 0	-91.8	-91.8	0.12 (1)	6.25
C- D	0 / 4	-91.8	-91.8	0.29 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00
H- G	-82 / 0	0.0	0.0	0.25 (1)	7.81
G- C	0 / 116	0.0	0.0	0.27 (1)	10.00
G- F	0 / 460	-18.5	-18.5	0.41 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.34 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- H	0 / 210	0.05 (1)
C- F	-515 / 0	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 567 (0.12")

CSI: TC=0.29/1.00 (C-D:1) , BC=0.41/1.00 (F-G:1) , WB=0.08/1.00 (C-F:1) , SSI=0.21/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (C) (INPUT = 0.90)
JSI METAL= 0.16 (G) (INPUT = 1.00)

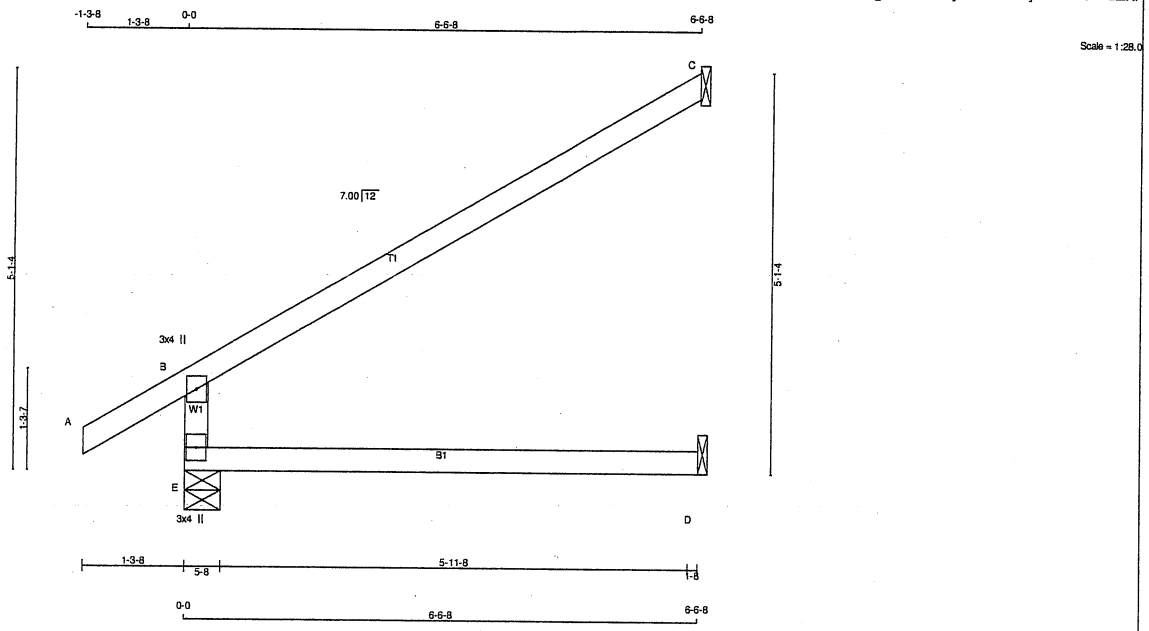


Structural component only
DWG# T-2115255

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410965	J2	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 19 = 57 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	571	0	571	0	0	5-8	5-8
C	225	0	225	0	0	1-8	1-8
D	50	0	56	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	401	279 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0
C	155	126 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-500 / 0	0.0	0.0	0.14 (4)		7.81			
A- B	0 / 32	-91.8	-91.8	0.12 (1)		10.00			
B- C	-38 / 0	-91.8	-91.8	0.52 (1)		6.25			
E- D	0 / 0	-18.5	-18.5	0.16 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.16/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (E) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

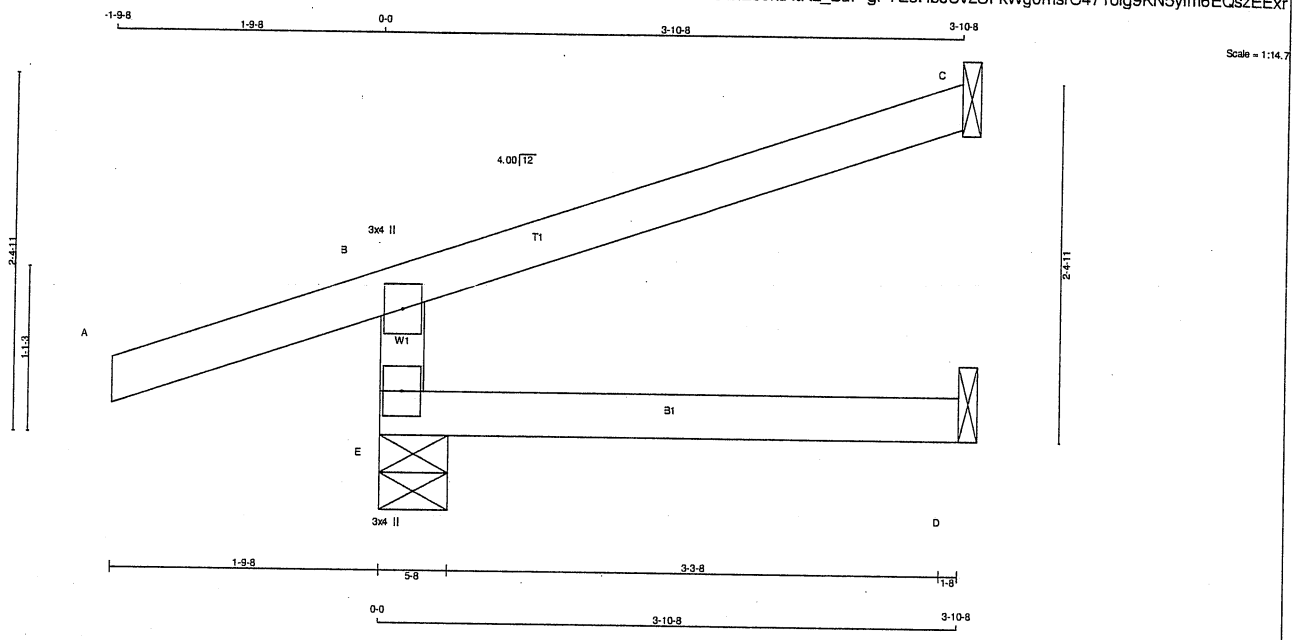
LICENSED PROFESSIONAL ENGINEER
05/22/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115256

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410965	J3	4	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	432	0	432	0	0	5-8	5-8
C	133	0	133	0	0	1-8	1-8
D	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	302	218 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
C	92	74 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-391 / 0	0.0	0.0	0.05 (4)	7.81
A- B	0 / 27	-91.8	-91.8	0.23 (5)	10.00
B- C	-14 / 0	-91.8	-91.8	0.23 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 12 = 48 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (E) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

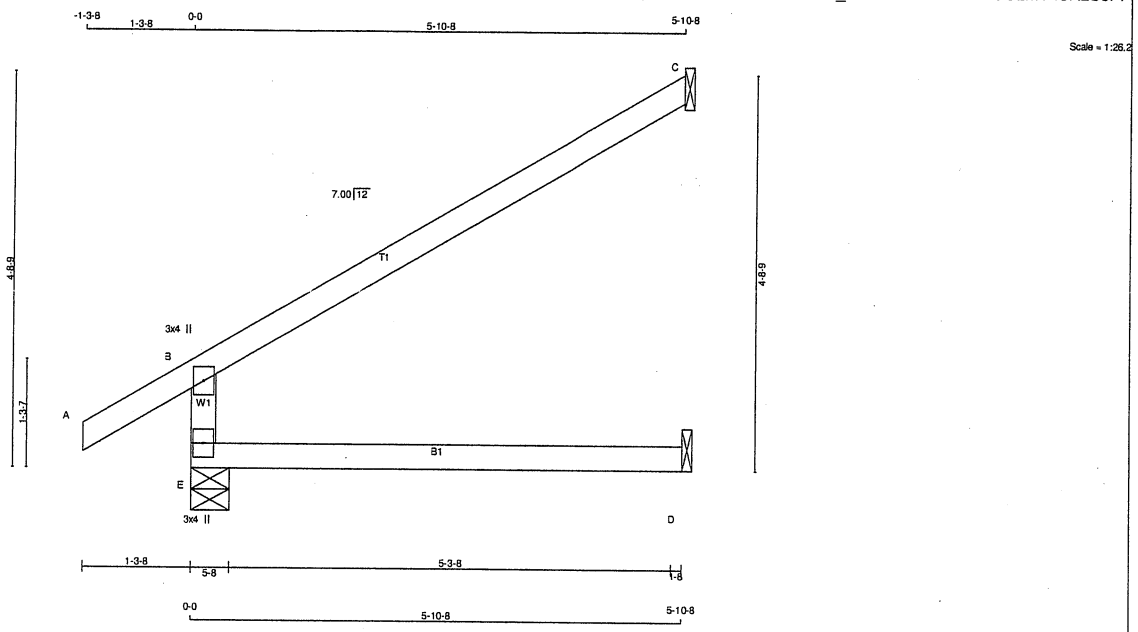
LICENSED PROFESSIONAL ENGINEER
05/22/21
H. J. G. ALVES
100090247
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115257

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
418972	J20	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:53:24 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-UaGz4dXdchDZO2Y_nvrWOGNEsub5VPUUzMV45RzCcPP



TOTAL WEIGHT = 5 X 17 = 86 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	526	0	526	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	258 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-462 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 32	-91.8	-91.8	0.12 (1)	10.00			
B- C	-34 / 0	-91.8	-91.8	0.54 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

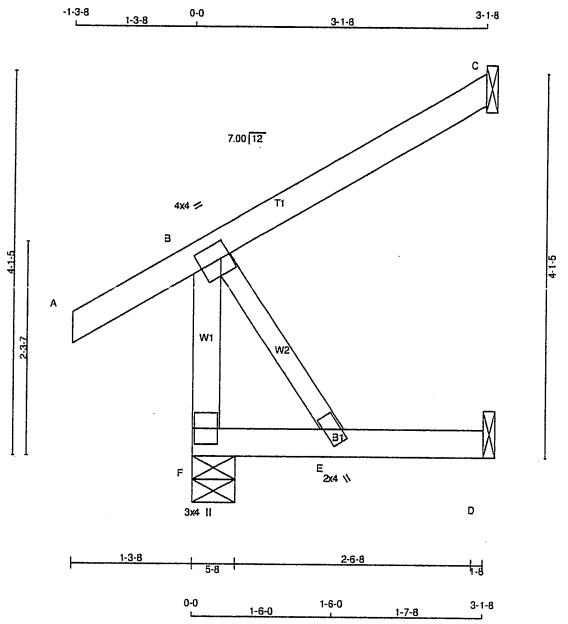


Structural component only
DWG# T-2115888

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
418972	J21	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed May 26 14:53:25 2021 Page 1
ID:w1TZe5RpU4szZzlCfG5PpdzFeW5-ynqLHzYFN?LQ0C6ALcMlwUwVflzXEskdC0FddtzCcPO



TOTAL WEIGHT = 5 X 14 = 68 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	298	0	298	0	0	5-8	5-8
C	143	0	143	0	0	1-8	1-8
D	29	0	32	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	208	150 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
C	99	80 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
D	23	0 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-269 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 32	-91.8	-91.8	0.13 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.15 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.05/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

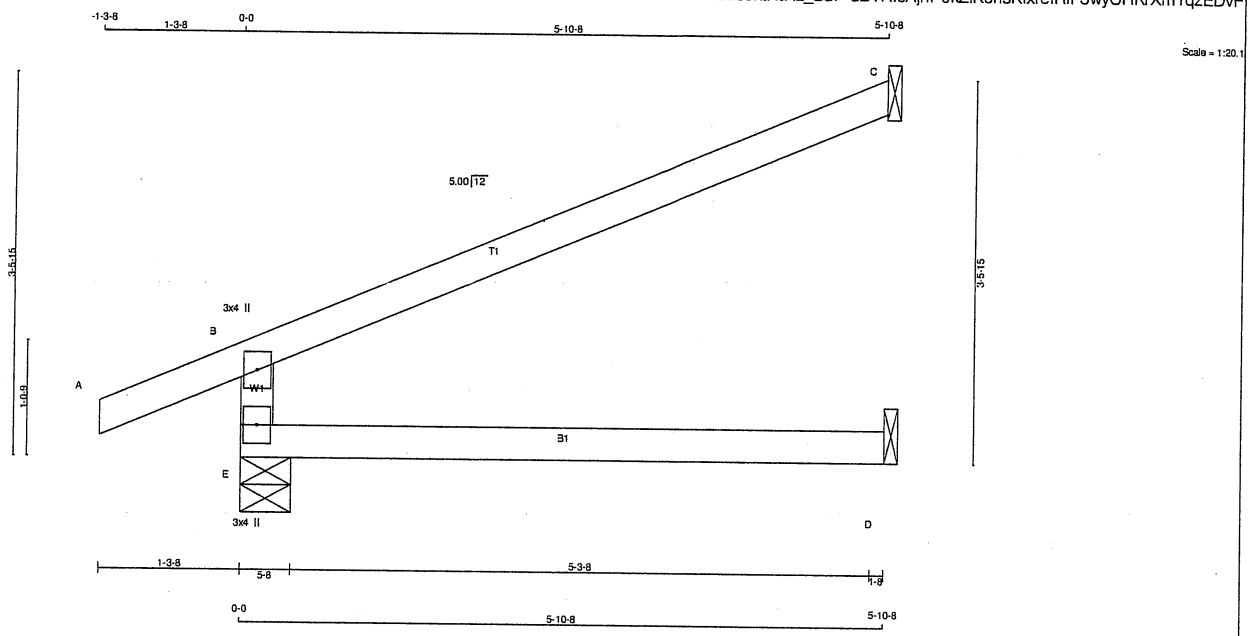


Structural component only
DWG# T-2115889

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	J30	17	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-uB?RIcAjnP0kZiKohsKlxreIRIF3wyCHKrXm1qzEDvF



TOTAL WEIGHT = 17 X 16 = 278 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	525	0	525	0	0	5-8	5-8
C	202	0	202	0	0	1-8	1-8
D	44	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	368	257 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-461 / 0	0.0	0.0	0.14 (4)	7.81			
A- B	0 / 24	-91.8	-91.8	0.12 (1)	10.00			
B- C	-26 / 0	-91.8	-91.8	0.54 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

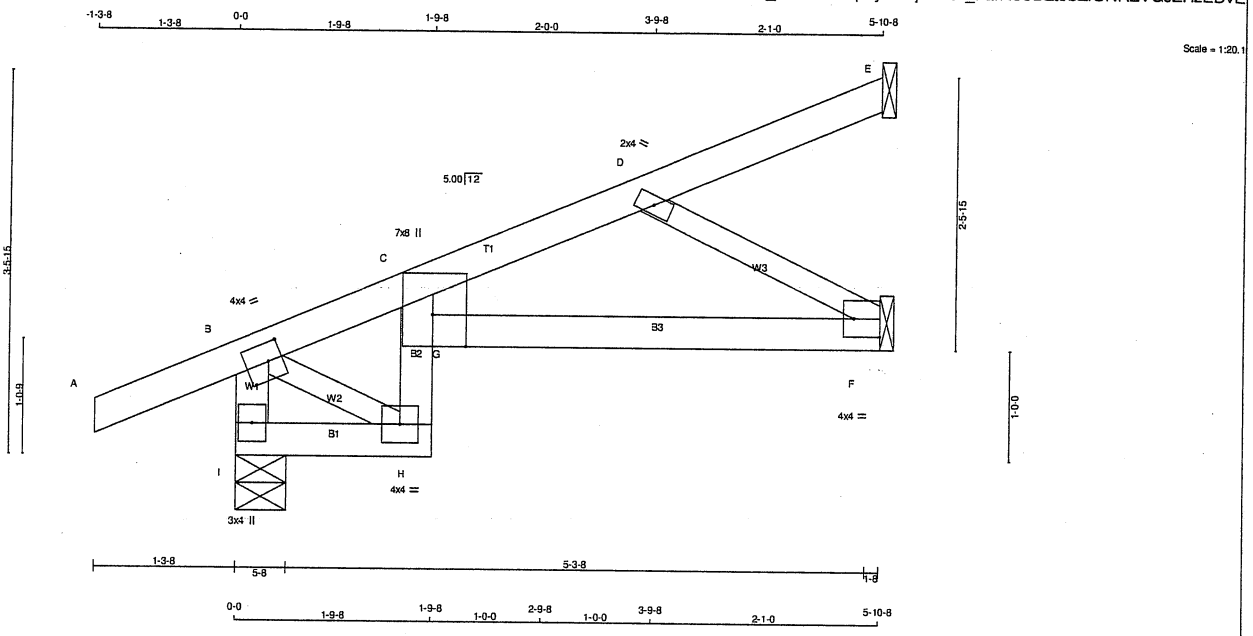
LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115301

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	J30S	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:07:59 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-MNZqWyBLYj8bBsv_FarXU3BZt9bZfONRZVGJZHzEDvE



TOTAL WEIGHT = 6 X 21 = 124 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C						
C	TMBV*m	MT20	7.0	8.0	3.75	3.50
D	TMW+w	MT20	2.0	4.0		
F	BMW1-t	MT20	4.0	4.0	2.00	Edge
G						
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	456	0	456	0	0	5-8	5-8
E	53	0	53	0	0	1-8	1-8
F	259	0	259	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) E , F

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	320	225 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
E	36	35 / 0	0 / 0	0 / 0	0 / 0	1 / 0	0 / 0
F	186	110 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-466 / 0	0.0	0.0	0.02 (4)	7.81	B- H	0 / 185	0.04 (1)
A- B	0 / 24	-91.8	-91.8	0.12 (1)	10.00	D- F	-388 / 0	0.07 (1)
B- C	-235 / 0	-91.8	-91.8	0.14 (1)	6.25			
C- D	-313 / 0	-91.8	-91.8	0.18 (1)	6.25			
D- E	-17 / 0	-91.8	-91.8	0.13 (1)	6.25			
I- H	0 / 18	-18.5	-18.5	0.06 (1)	10.00			
H- G	-36 / 7	0.0	0.0	0.18 (1)	7.81			
G- C	0 / 54	0.0	0.0	0.12 (1)	10.00			
G- F	0 / 326	-18.5	-18.5	0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.18/1.00 (C-D:1) , BC=0.18/1.00 (G-H:1) , WB=0.07/1.00 (D-F:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.19 (D) (INPUT = 1.00)

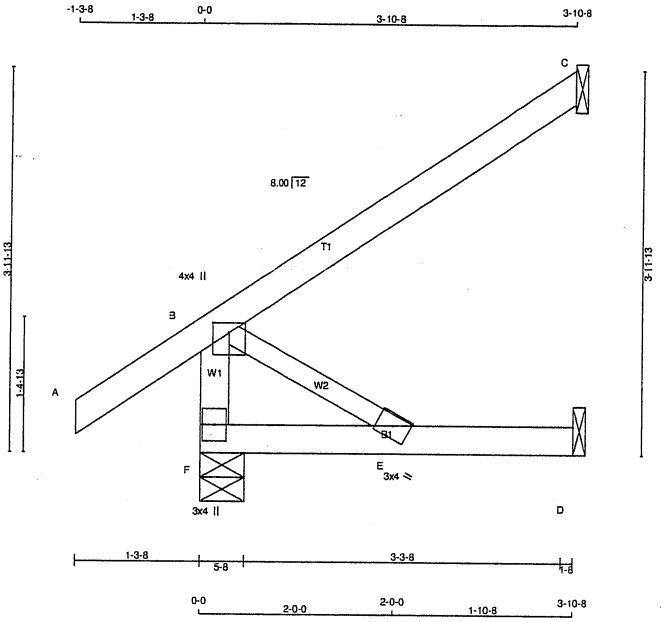


Structural component only
DWG# T-2115302

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
410969	J31	6	1	ROYAL PINE HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-qZ7CjICzJ1GSo0UAoHMm1GkjhZyNOshao90t5jzEDvD



TOTAL WEIGHT = 6 X 15 = 87 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	340	0	340	0	0	5-8	5-8
C	178	0	178	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	238	169 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	122	99 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-304 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.14 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.23 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.08/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

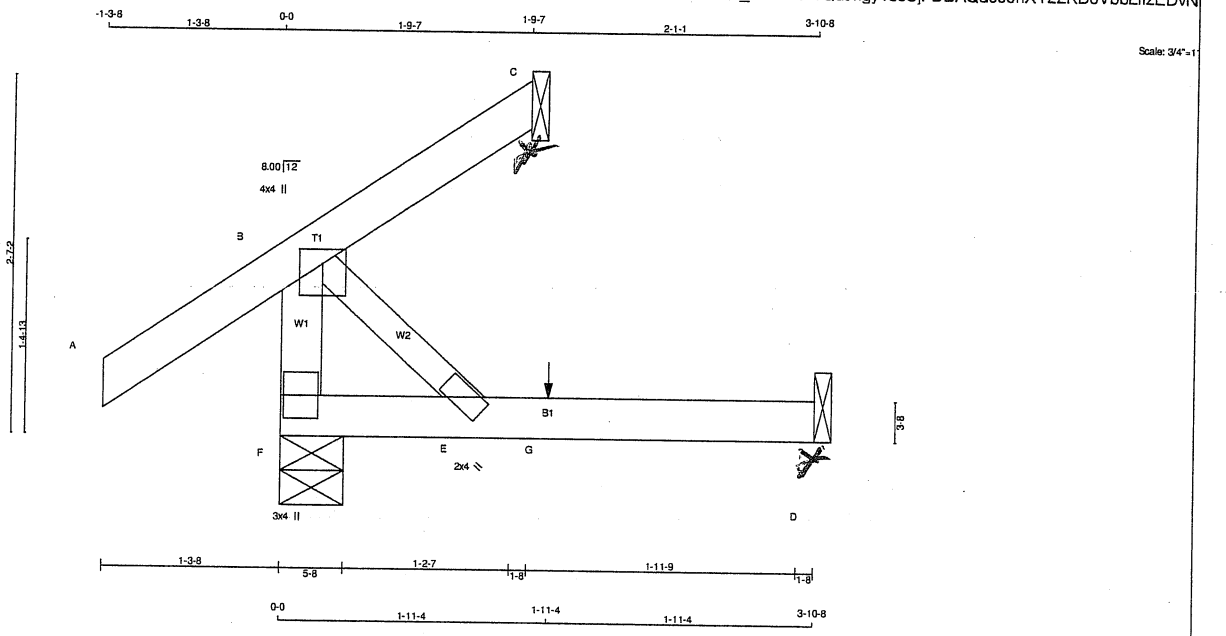


Structural component only
DWG# T-2115303

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	C30	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-7eWQdt4igy?scUjFDBAQd9J0hXY22KD6VbbLIlzEDvN



TOTAL WEIGHT = 4 X 11 = 45 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	292	0	292	0	0	5-8	5-8
C	34	0	34	0	-39	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	205	143 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.14 (5)	10.00			
B- C	-27 / 0	-91.8	-91.8	0.13 (5)	6.25			
F- E	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

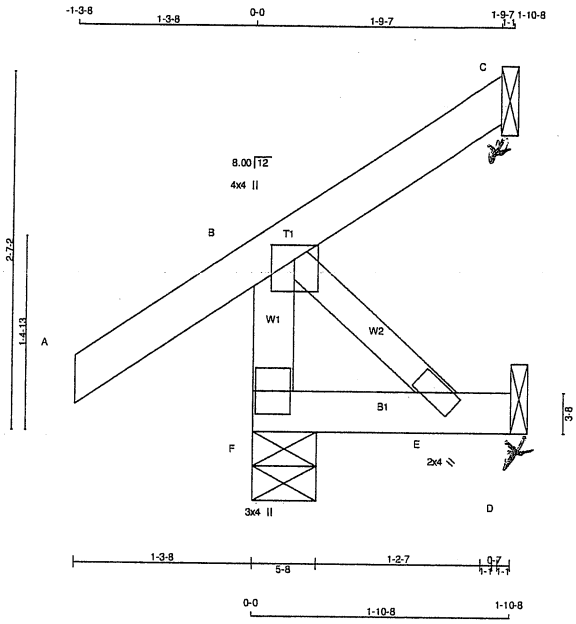
LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115293

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	C31	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-br4oqD5KRG7jDdISnuif9NsCiwuEnnTFjFKuHkzEDvM



TOTAL WEIGHT = 4 X 9 = 36 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	274	0	274	0	0	5-8	5-8
C	34	0	34	0	-39	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-256 / 0	0.0	0.0	0.03 (1)		7.81
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C	-27 / 0	-91.8	-91.8	0.12 (1)		6.25
F- E	0 / 0	-18.5	-18.5	0.02 (4)		10.00
E- D	0 / 0	-18.5	-18.5	0.02 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

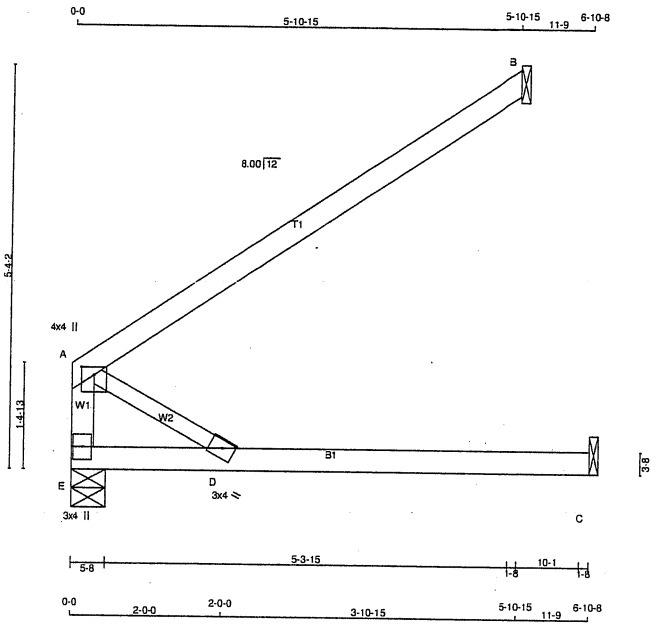


Structural component only
DWG# T-2115294

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	C32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:07:52 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-31eA2Z5yCZFarnteKcDuiaOGsKAmWEjPyv4RpBzEDvL



TOTAL WEIGHT = 19 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMW+w	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	335	0	335	0	0	5-8	5-8
B	271	0	271	0	0	1-8	1-8
C	64	0	71	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	238	151 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
B	187	151 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
C	51	0 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-271 / 0	0.0	0.0	0.03 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.55 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.21 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 819 (0.10")

CSI: TC=0.55/1.00 (A-B:1) , BC=0.26/1.00 (C-D:4) , WB=0.00/1.00 (A-D:1) , SSI=0.18/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (A) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)

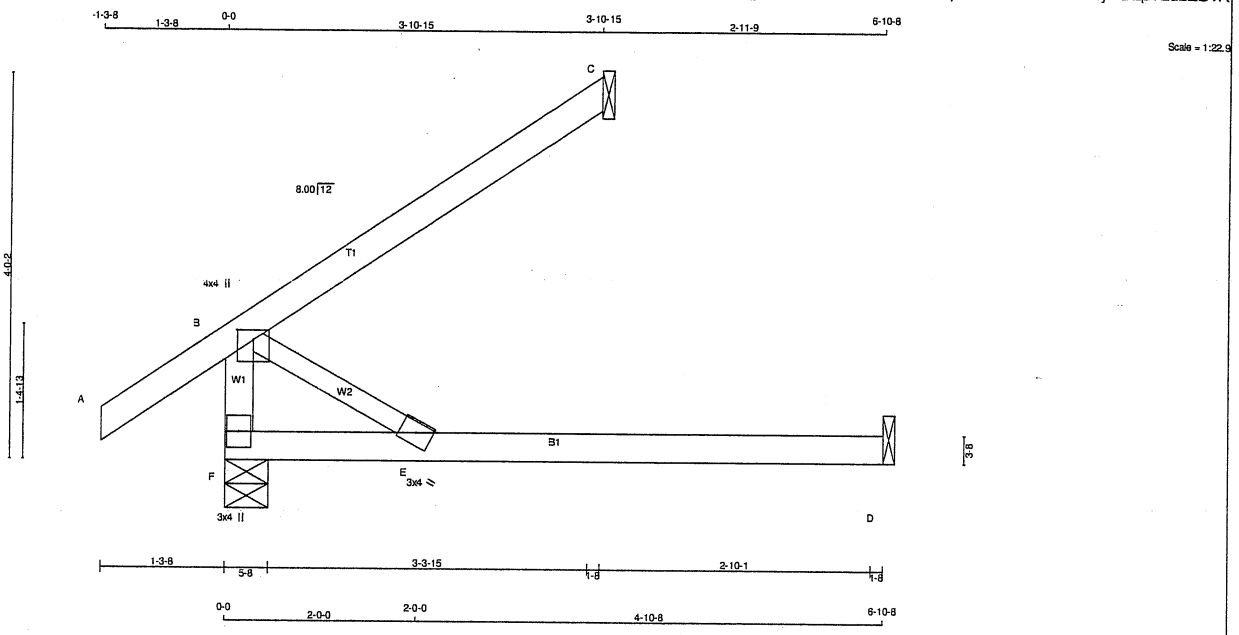
LICENSED PROFESSIONAL ENGINEER
05/21/21
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115295

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	C33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:07:53 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-XDCYFv6aztNRTxSquJk7EoxWOkW?FhyYBZp?LdzEDvK



TOTAL WEIGHT = 18 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	369	0	369	0	0	5-8	5-8
C	179	0	179	0	0	1-8	1-8
D	64	0	71	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	261	170 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0
C	124	100 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	51	0 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-305 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.24 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.21 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 819 (0.10")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.26/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

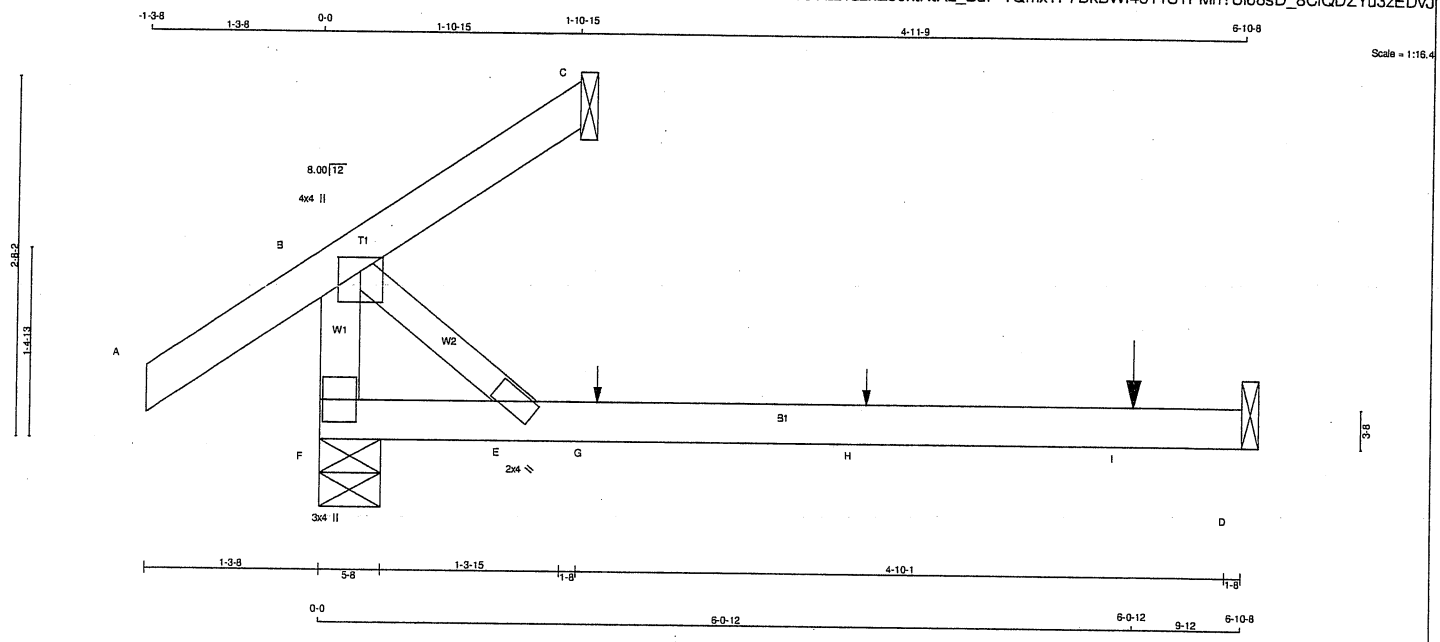
LICENSED PROFESSIONAL ENGINEER
05/22/14
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2115296

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	C34	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:07:54 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-?QmxTF7DkBWI4511S1FMn?Uio8sD_8CiQDZYu3zEDvJ



TOTAL WEIGHT = 15 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	323	0	323	0	0	5-8	5-8
C	42	0	42	0	0	1-8	1-8
D	65	0	73	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	229	144 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
C	29	24 / 0	0 / 0	0 / 0	0 / 0	6 / 0	0 / 0
D	52	0 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-259 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.13 (1)	10.00			
B- C	-25 / 0	-91.8	-91.8	0.10 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
H- I	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
I- D	0 / 0	-18.5	-18.5	0.26 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	1	1	---	BACK	VERT	TOTAL	---	C1
H	4-0-12	1	1	---	BACK	VERT	TOTAL	---	C1
I	6-0-12	-1	-1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 813 (0.10")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.26/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.93 NAIL=0.93 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

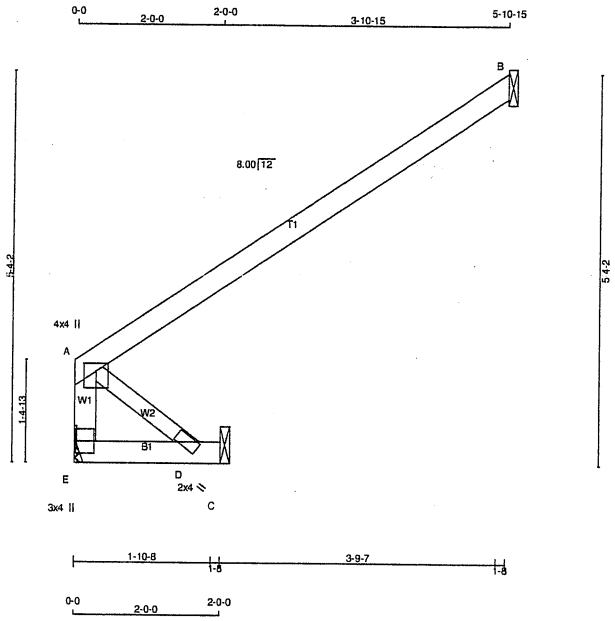


Structural component only
DWG# T-2115297

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	C35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:GAonzd6DLBrd2xE89xtNtRz_BuP-TcJJgb8rUUe9iFbD0kmbKD0n5YG8jbSretl6QWzEDvl



TOTAL WEIGHT = 13 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	290	0	290	0	0	MECHANICAL	
B	271	0	271	0	0	1-8	1-8
C	19	0	21	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	202	151 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
B	187	151 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
C	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-271 / 0	0.0	0.0	0.03 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.55 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.55/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (A-D:1) , SSI=0.18/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (A) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)

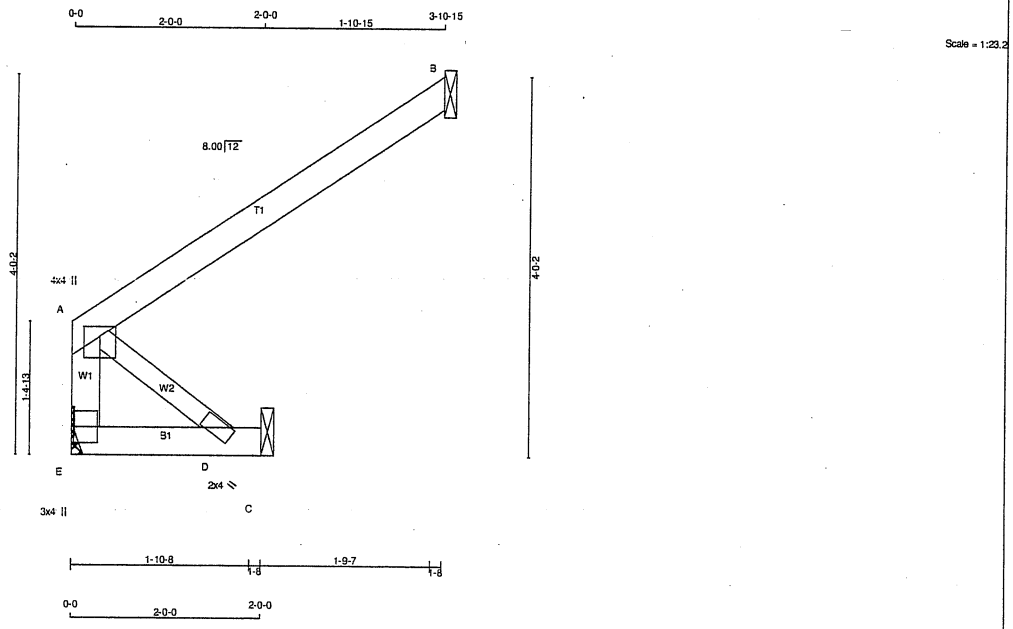
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO
05/21/21

Structural component only
DWG# T-2115298

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	C36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:07:55 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-TcJJgb8rUUe9iFbD0kmbKD0suYG8jbSretl6QWzEDvl



TOTAL WEIGHT = 10 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	198	0	198	0	0	MECHANICAL	
B	179	0	179	0	0	1-8	1-8
C	19	0	21	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	138	100 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
B	124	100 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
C	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-179 / 0	0.0	0.0	0.02 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.24 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.24/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (A-D:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)

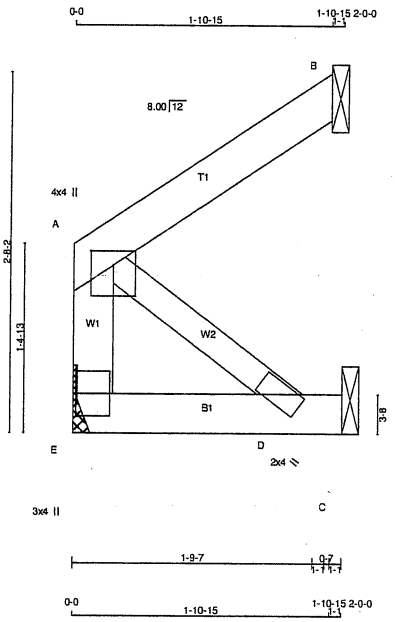


Structural component only
DWG# T-2115299

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. ROYAL PINE HOMES	DRWG NO.
410969	C37	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri May 21 17:07:56 2021 Page 1
ID:GAonzd6DLBrd2xE89xtNtRz_BuP-xothuw9TFom0KPAPZSHqsQZ4TxcNS2i_tX2fyyzEDvH



TOTAL WEIGHT = 8 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	106	0	106	0	0	MECHANICAL	
B	88	0	88	0	0	1-8	1-8
C	19	0	21	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	75	49 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
B	60	49 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
C	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-88 / 0	0.0	0.0	0.01 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.06 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (A-D:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

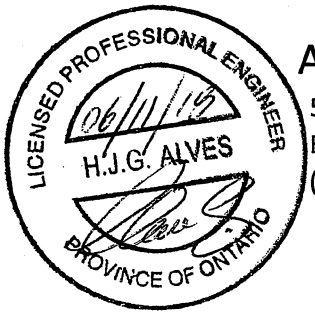
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



Structural component only
DWG# T-2115300



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213 Feb 09, 2018

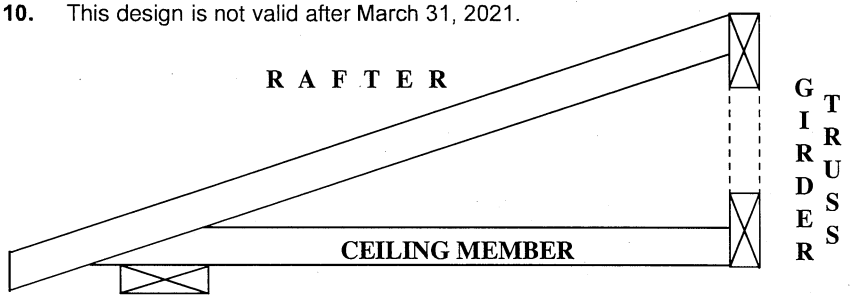
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

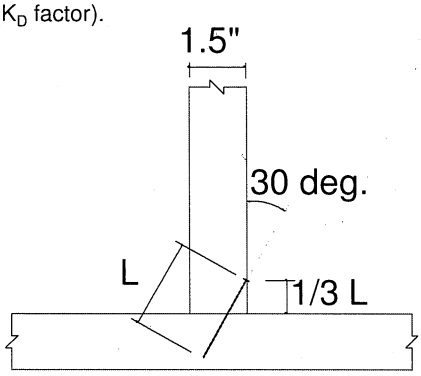
B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.





TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

MiTek®

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

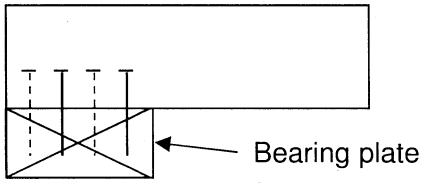
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

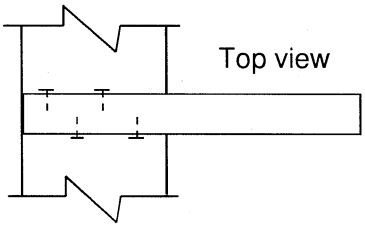
NOTES:

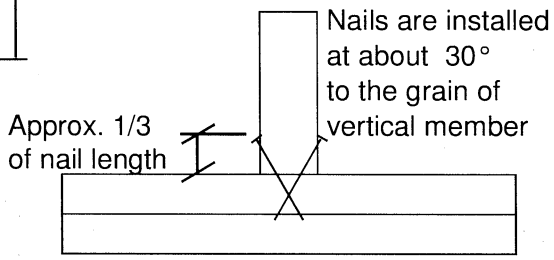
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



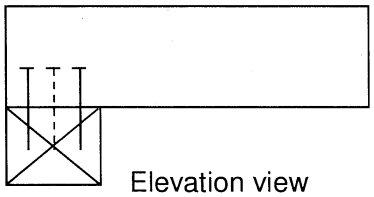
Elevation view



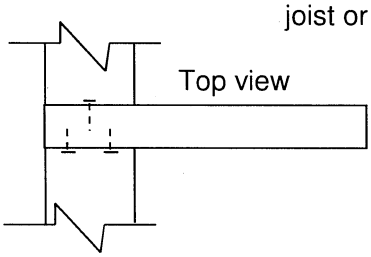


Toe-nailing viewed from end of joist or truss

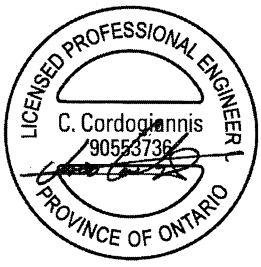
Toe-nailing on 2x4 Bearing Plate



Elevation view



PEO
Certificate No. 10889485



MiTek®

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

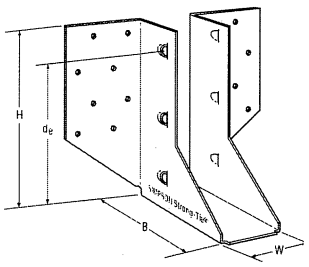
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

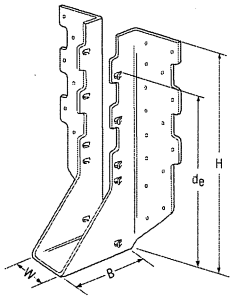
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

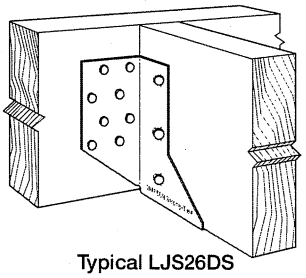
- See current catalogue for options



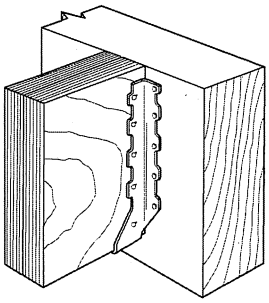
LJS26DS



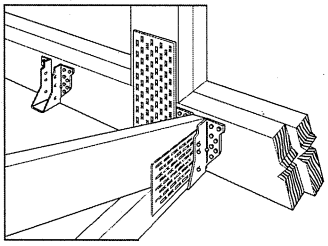
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



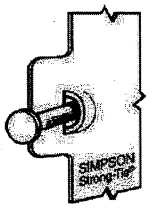
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

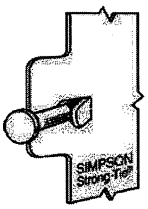
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

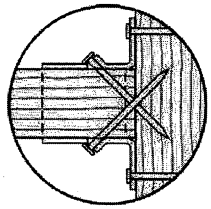


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022 and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

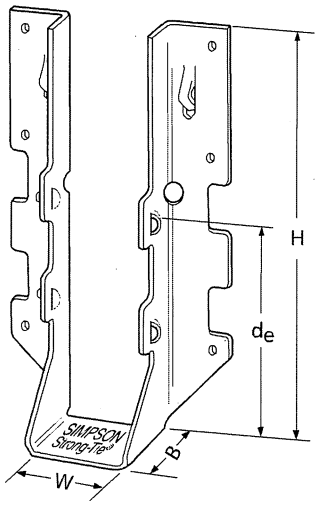
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

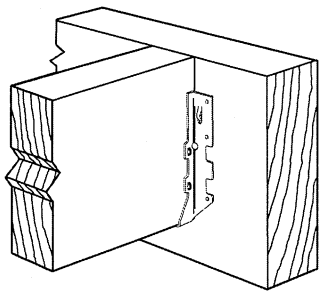
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

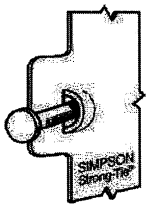


LUS28


Typical LUS Installation

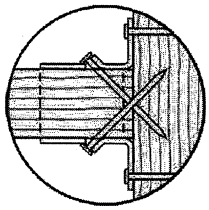
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS20 3/20 exp. 6/22

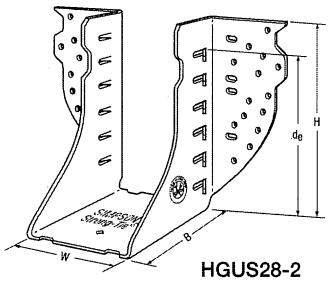
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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



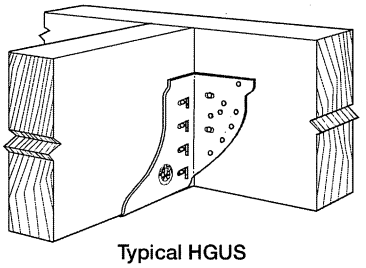
HGUS28-2

Material: 12 gauge

Finish: G90 galvanized

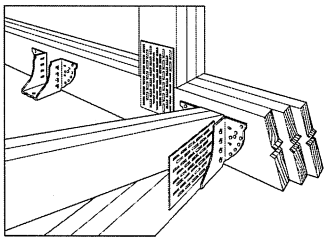
Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.


Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

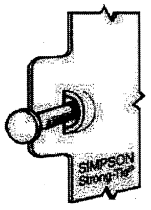

Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Options:

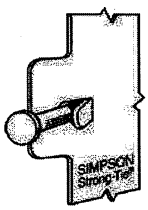
- See current catalogue for options

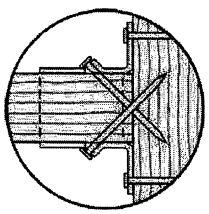
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 9/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 9/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 5/16	9 9/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 9/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10 5/8	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12 5/8	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

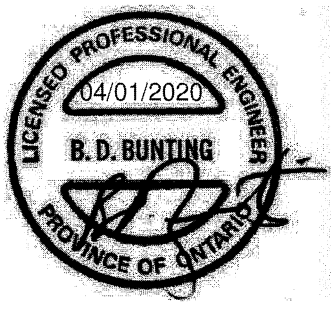
1. d_e is the distance from the seat of the hanger to the highest joist nail.


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECHGUS20 3/20 exp. 6/22

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TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON Strong-Tie

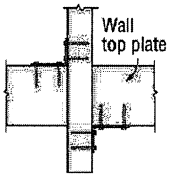
The H connector series provides wind and seismic ties for trusses and rafters.
Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14
- Factored resistances have been increased 15%. No further increase is permitted.

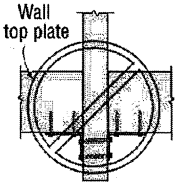
Installation: • Use all specified fasteners
- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

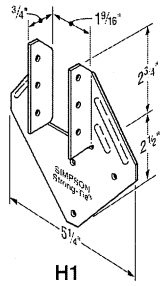
Hurricane Tie Installations to Achieve Twice the Load (Top View)



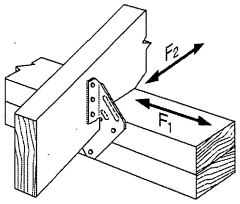
Install diagonally across from each other for minimum 2x truss.



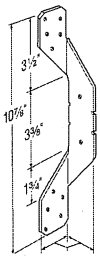
Nailing into both sides of a single ply 2x truss may cause the wood to split.



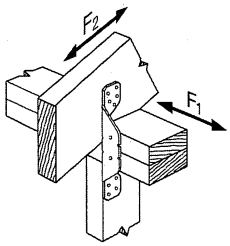
H1



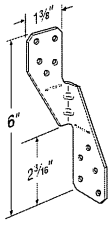
H1 Installation



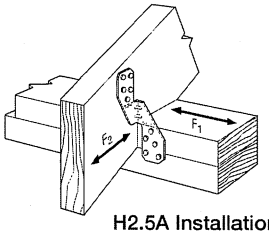
H2A



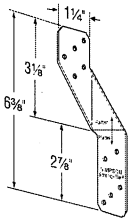
H2A Installation



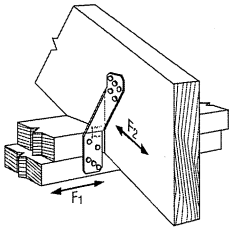
H2.5A



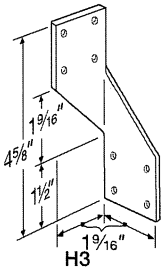
H2.5A Installation



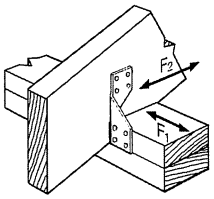
H2.5T



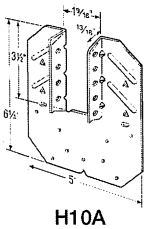
H2.5T Installation
(Nails into both top plates)



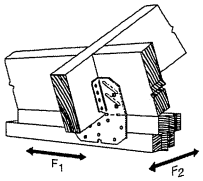
H3



H3 Installation



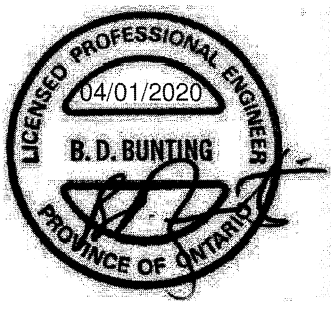
H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
		To Rafter	To Plates	To Studs		F_1	F_2		F_1	F_2
					(K_D=1.15)			(K_D=1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT STATES DESIGN

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T-SPECH20 3/20 exp. 6/22

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TC – Truss Connectors

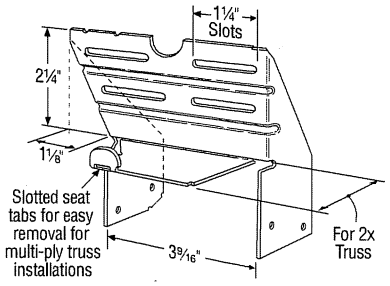
SIMPSON Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

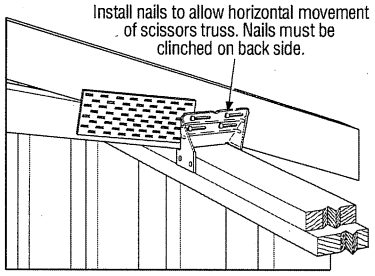
Material: 16 gauge

Finish: G90 galvanized

Design: Factored resistances are in accordance with CSA 086-14



TC24
U.S. Patent 4,932,173



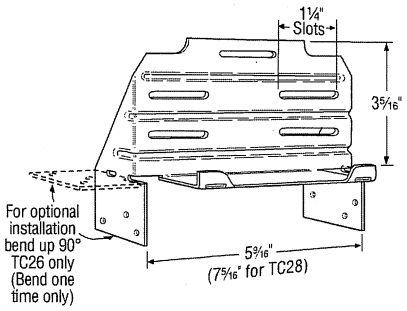
Typical TC24 Installation

Installation:

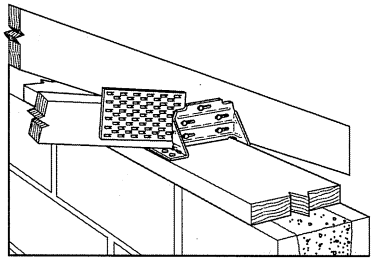
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

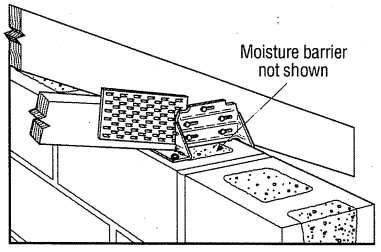
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



TC26
(TC28 Similiar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



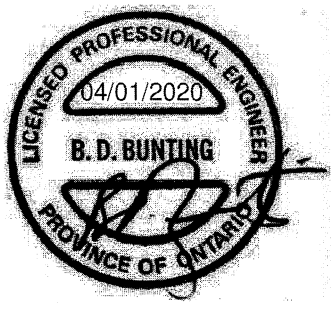
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



LIMIT STATES DESIGN

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TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

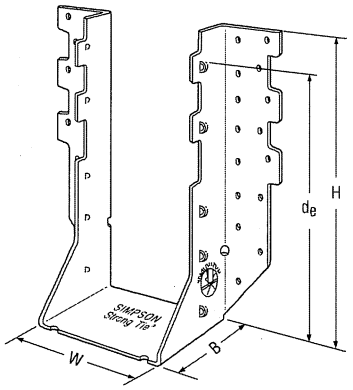
All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

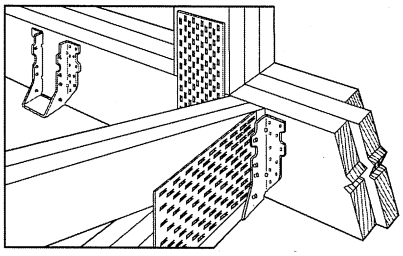
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HHUS410

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

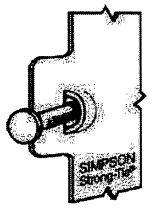

Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Options:

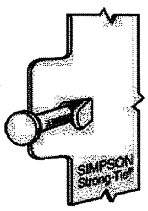
- See current catalogue for options

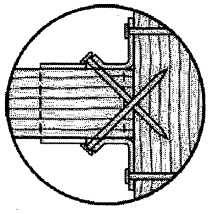
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 5/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 5/8	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	(30) 16d	(10) 16d	4670	10155	3370	7210

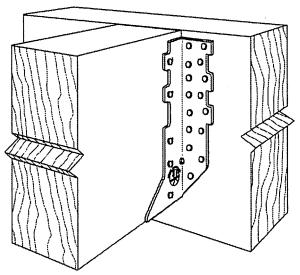
1. d_e is the distance from the seat of the hanger to the highest joist nail.

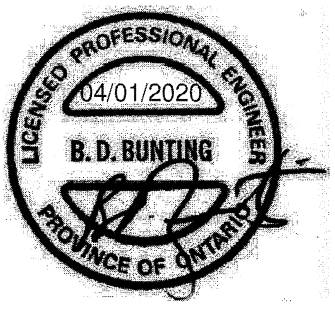

Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.


Typical HHUS Installation



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

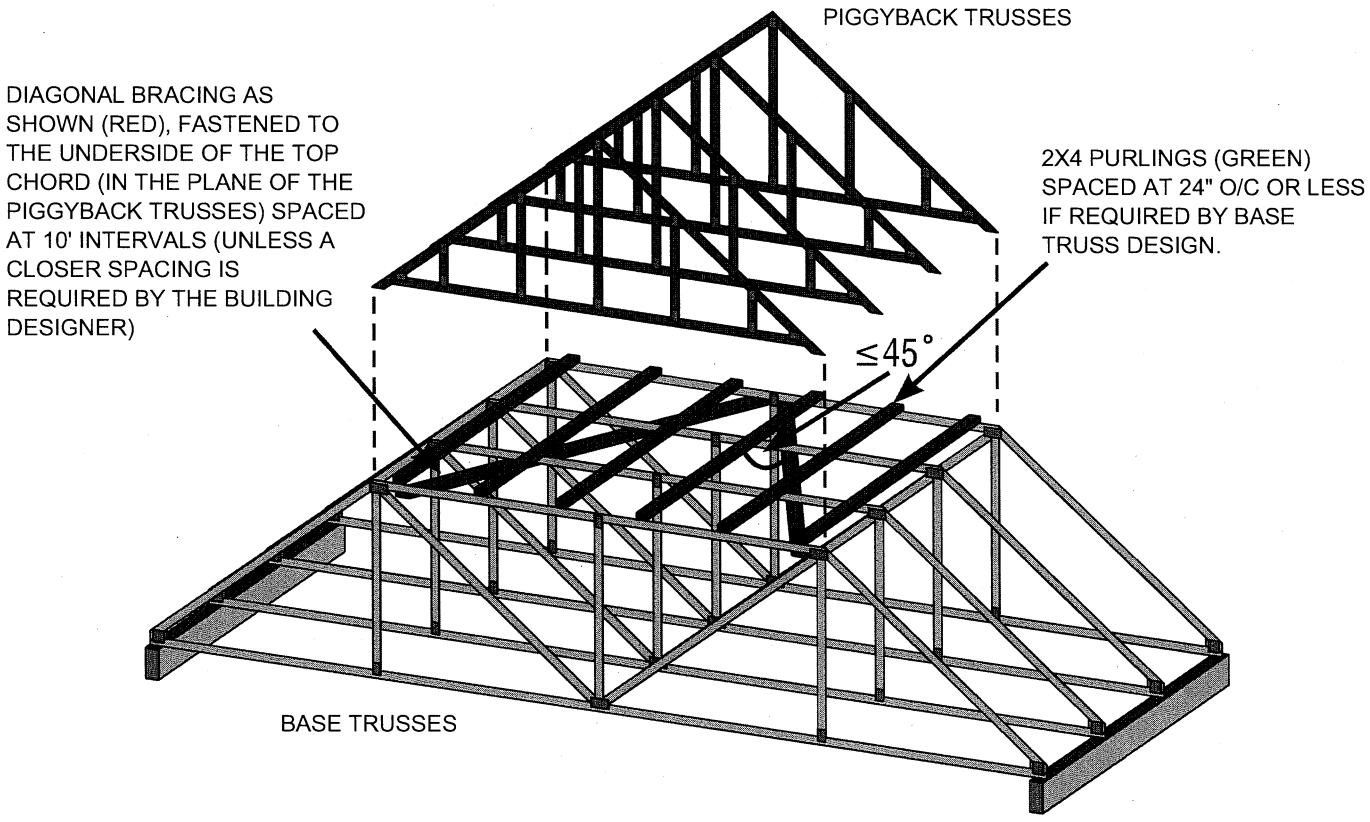
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

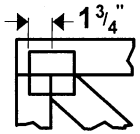
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

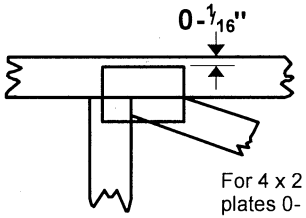
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

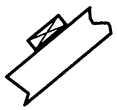
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

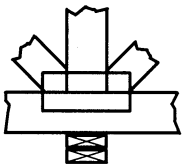
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

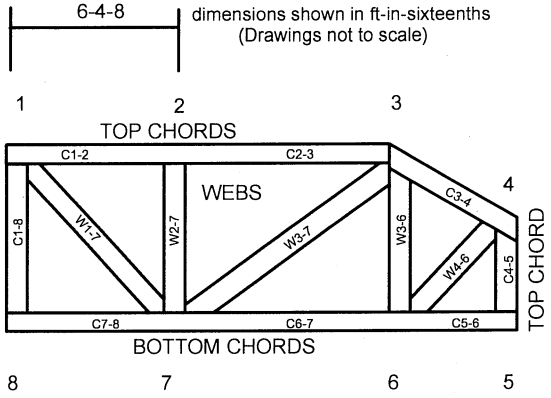


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.