ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS24 -(O)
LJS26DS -(V)

DENOTE:

C- 10/12 CATH. CEILING

T-180485

DENOTES:
CONVENTIONAL
FRAMING

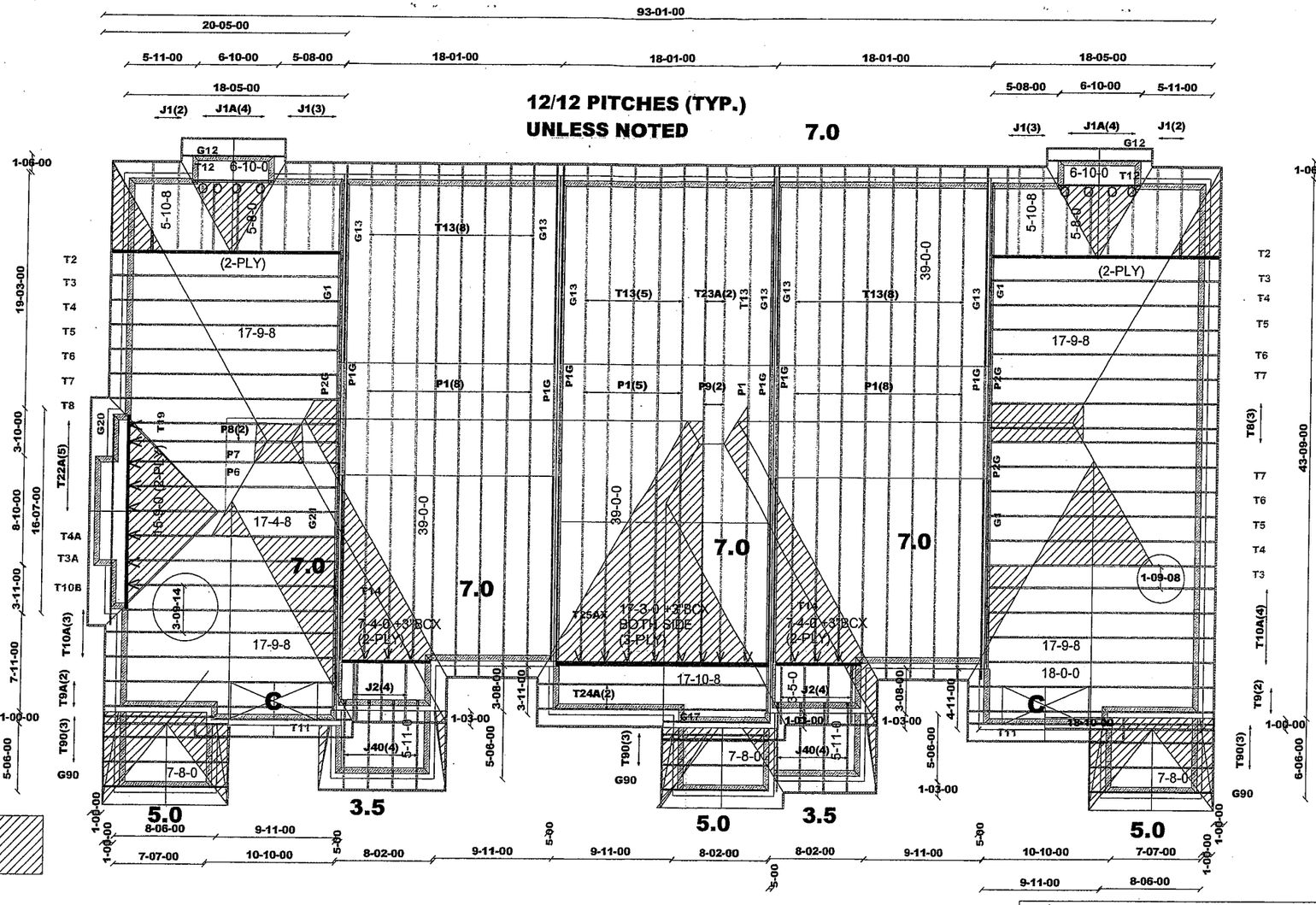
UNIT1804 CORNER
A -LOT1
402478

UNIT1802
A -LOT2
402479

UNIT1803
A1 (REV) -LOT3
402480

UNIT1802
A -LOT4
402479

UNIT1804 END
A(REV) -LOT5
402481



Lot # 1 DWG# , T-1900034 to T-1900037, T-1900039 to T-1900054, T-1900056 to T-1900059, T-1909771 to T-1909772, T-1800216 , T1800218

Lot 2 or 4 DWG#, T-1900060, T-1900063, T-1900064, T-1900066, T-1900089, T-1800216, T-1800218

lot # 3 DWG# , T-1900060, T-1900061, T-1900063 to T-1900069, T-190771 , T-1909772, T1800218

Lot # 5 DWG#, T-1900034 to T-1900035, T-1900039, T-1900040, T-1900042, T-1900044 to T-1900048, T-1900051, T-1900052 , T-1900056, T-1900082, T-1909771, T-1909772, T-1800216, T1800218

LICENSED PROFESSIONAL ENGINEER
04/24/19
H. J. G. ALVES
100098024
PROVINCE OF ONTARIO

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 23.3 PSF
TC DEAD 15 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Job Track: 50530
Layout ID: 200902
Plan Log: 402477

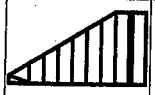
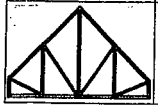
Builder / Location: ROYAL PINE HOMES / BRAMPTON
Project: FORESTSIDE PH.6
Date: 4/22/2019 Designer: S.V

Model / Elevation: BLOCK 1 / LOTS 1 TO 5 (REVISED)
Mitek ver 8.2.3.229

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50530
Builder:	ROYAL PINE HOMES-	PlanLog:	200902
Project:	FORESTSIDE PH.6	Layout ID:	402478
Location:	BRAMPTON	Ref #	
Model:	UNIT 1804 CORNER	Page:	1 of 3
Lot #:		Date:	04/18/2019
Elevation:	EL:A LOT1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

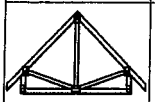
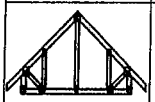
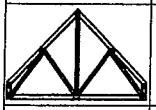
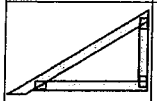
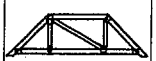
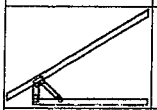
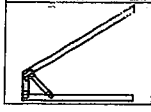
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G1 GABLE	7 /12	19-06-00	10-00-00	2 x 4		1-03-07 10-00-00	110.73 70.00		
	1	G21 GABLE	7 /12	23-02-00	9-08-08	2 x 4		1-03-07 9-08-08	131.02 84.17		
	1 2-ply	T2 Half Hip Girder	12 /12	17-09-08	4-08-09	2 x 4 2 x 6	1-03-08	1-10-08 4-08-09	199 125.33		
	1	T3 Half Hip	12 /12	17-09-08	5-10-09	2 x 4	1-03-08	1-10-08 5-10-09	78.18 49.00		
	1	T3A Half Hip	12 /12	17-04-08	5-10-09	2 x 4		1-10-08 5-10-09	75.26 47.67		
	1	T4 Half Hip	12 /12	17-09-08	7-00-09	2 x 4	1-03-08	1-10-08 7-00-09	83.47 52.33		
	1	T4A Half Hip	12 /12	17-04-08	7-00-09	2 x 4		1-10-08 7-00-09	80.51 50.50		
	1	T5 Half Hip	12 /12	17-09-08	8-02-09	2 x 4	1-03-08	1-10-08 8-02-09	92.67 58.67		
	1	T6 Half Hip	12 /12	17-09-08	9-04-09	2 x 4	1-03-08	1-10-08 9-04-09	105.42 66.00		
	1	T7 Half Hip	12 /12	17-09-08	10-06-09	2 x 4	1-03-08	1-10-08 10-06-09	104.32 63.83		
	1	T8 Half Hip	12 /12	17-09-08	11-08-09	2 x 4	1-03-08	1-10-08 11-08-09	116.92 72.00		
	2	T9A Roof Special	12 /12 10 /12	17-09-08	10-10-08	2 x 4	1-03-08	1-10-08 1-10-08	200.12 126.67		
	3	T10A Common	12 /12	17-09-08	10-10-08	2 x 4	1-03-08	1-10-08 2-01-00	286.15 179.00		
	1	T10B Common	12 /12	17-04-08	10-10-08	2 x 4		1-10-08 2-01-00	92.45 58.33		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50530
Builder:	ROYAL PINE HOMES	PlanLog:	200902
Project:	FORESTSIDE PH.6	Layout ID:	402478
Location:	BRAMPTON	Ref #	
Model:	UNIT 1804 CORNER	Page:	2 of 3
Lot #:		Date:	04/18/2019
Elevation:	EL:A LOT1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T11 Roof Special	12 /12 10 /12	**10-05-00**	7-01-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	55.81 36.33		
	1	T12 Common Girder	12 /12	**6-10-00**	4-10-08	2 x 4	10-08 10-08	1-05-08 1-05-08	33.29 23.17		
	1	G12 GABLE	12 /12	**6-10-00**	4-10-08	2 x 4	10-08 10-08	1-05-08 1-05-08	34.17 24.17		
	1 2-ply	T19 Common Girder	12 /12	**15-09-00**	9-09-00	2 x 6		1-10-08 1-10-08	222.79 134.67		
	1	G20 GABLE	12 /12	**15-09-00**	9-09-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	87.45 58.00		
	5	T22A Piggyback Base	12 /12	**17-04-08**	10-04-04	2 x 4		2-03-08 9-10-00	581.76 358.33		
	3	T90 Common	5 /12	**7-08-00**	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	73.47 47.00		
	1	G90 GABLE	5 /12	**7-08-00**	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	24.03 15.33		
	1	P2G GABLE	7 /12	**4-06-12**	2-07-15	2 x 4		2-07-15	12.46 8.00		
	1	P6 Piggyback	12 /12	**8-09-09**	1-02-00	2 x 4			20.63 13.67		
	1	P7 Piggyback	12 /12	**8-09-09**	2-04-00	2 x 4			26.94 18.50		
	2	P8 Piggyback	12 /12	**8-09-09**	2-06-06	2 x 4			54.45 34.67		
	5	J1 Jack-Open	7 /12	**5-10-08**	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	92.39 58.33		
	4	J1A Jack-Open	7 /12	**5-08-00**	4-08-09	2 x 4		1-04-14 4-08-09	65.83 41.33		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50530
Builder:	ROYAL PINE HOMES-	PlanLog:	200902
Project:	FORESTSIDE PH.6	Layout ID:	402478
Location:	BRAMPTON	Ref #	
Model:	UNIT 1804 CORNER	Page:	3 of 3
Lot #:		Date:	04/18/2019
Elevation:	EL:A LOT1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **47** TOTAL BFT OF ALL TRUSSES= **1975** BFT. TOTAL WEIGHT OF ALL TRSSES **3141.69** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
8	Hardware	LJS26DS	
4	Hardware	LUS24	

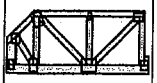
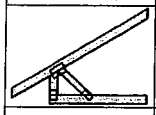
TOTAL NUMBER OF ITEMS= **12**

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50530
Builder:	ROYAL PINE HOMES	PlanLog:	200902
Project:	FORESTSIDE PH6	Layout ID:	402479
Location:	BRAMPTON	Ref #	
Model:	UNIT 1802	Page:	1 of 1
Lot #:		Date:	04/18/2019
Elevation:	EL:A LOT2 OR LOT4	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	8	T13 Piggyback Base	7 /12	**39-00-00**	10-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	1640.08 1016.00		
	2	G13 GABLE	7 /12	**39-00-00**	10-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	453.81 284.67		
	1 2-ply	T14 Half Hip Girder	12 /12	**7-04-00**	3-03-06	2 x 4 2 x 6		1-10-08 3-03-06	89.69 59.33		
	8	P1 Piggyback	7 /12	**9-01-08**	2-07-15	2 x 4			176.91 114.67		
	2	P1G GABLE	7 /12	**9-01-08**	2-07-15	2 x 4			46.84 30.67		
	4	J2 Jack-Open	7 /12	**3-05-00**	3-03-06	2 x 4	1-03-08	1-03-07 3-03-06	49.72 33.33		
	4	J40 Monopitch	3.5 /12	**5-11-00**	2-04-14	2 x 4	1-03-08	3-14 2-04-04	61.62 37.33		

TOTAL # TRUSS= **30** TOTAL BFT OF ALL TRUSSES= **1576** BFT. TOTAL WEIGHT OF ALL TRSSES **2518.67** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	

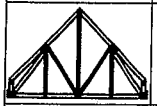
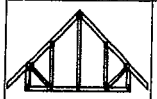
TOTAL NUMBER OF ITEMS= **3**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50530
Builder:	ROYAL PINE HOMES-	PlanLog:	200902
Project:	FORESTSIDE PH. 6	Layout ID:	402480
Location:	BRAMPTON	Ref #	
Model:	UNIT 1803	Page:	1 of 1
Lot #:		Date:	04/18/2019
Elevation:	EL:A1 LOT3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	6	T13 Piggyback Base	7 /12	**39-00-00**	10-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	1230.06 762.00		
	2	G13 GABLE	7 /12	**39-00-00**	10-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	453.81 284.67		
	2	T23A Piggyback Base	7 /12	**39-00-00**	10-00-00	2 x 4 2 x 6	1-03-08	1-03-07 3-05-02	424.23 264.33		
	2	T24A Common	12 /12	**17-10-08**	10-11-00	2 x 4	1-03-08	1-10-08 2-01-00	180.89 113.33		
	1 3-ply	T25AX Common Girder	12 /12	**17-03-00**	10-11-00	2 x 6	3-00 3-00	2-03-08 2-03-08	445.02 276.00		
	1	G17 GABLE	12 /12	**7-09-00**	5-09-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	42.13 28.67		
	3	T90 Common	5 /12	**7-08-00**	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	73.47 47.00		
	1	G90 GABLE	5 /12	**7-08-00**	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	24.03 15.33		
	6	P1 Piggyback	7 /12	**9-01-08**	2-07-15	2 x 4			132.68 86.00		
	2	P1G GABLE	7 /12	**9-01-08**	2-07-15	2 x 4			46.84 30.67		
	2	P9 Piggyback	7 /12	**12-09-08**	3-08-12	2 x 4			64.01 40.33		

TOTAL # TRUSS= **30** TOTAL BFT OF ALL TRUSSES= **1948.33** BFT. TOTAL WEIGHT OF ALL TRSSES **3117.19** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
8	Hardware	LJS26DS	

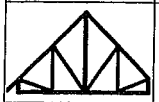
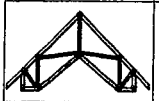
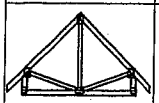
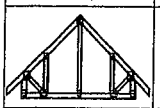
TOTAL NUMBER OF ITEMS= **8**

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50530
Builder:	ROYAL PINE HOMES	PlanLog:	200902
Project:	FORESTSIDE PH.6	Layout ID:	402481
Location:	BRAMPTON	Ref #	
Model:	UNIT1804 END	Page:	1 of 2
Lot #:		Date:	04/18/2019
Elevation:	EL:A LOT5	Designer:	
		Sales Rep:	Mario DiCano

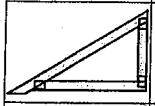
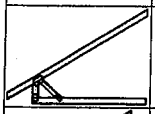
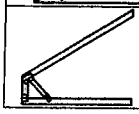
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	G1 GABLE	7 /12	**19-06-00**	10-00-00	2 x 4		1-03-07 10-00-00	221.47 140.00		
	1 2-ply	T2 Half Hip Girder	12 /12	**17-09-08**	4-08-09	2 x 4 2 x 6	1-03-08	1-10-08 4-08-09	199 125.33		
	2	T3 Half Hip	12 /12	**17-09-08**	5-10-09	2 x 4	1-03-08	1-10-08 5-10-09	156.36 98.00		
	2	T4 Half Hip	12 /12	**17-09-08**	7-00-09	2 x 4	1-03-08	1-10-08 7-00-09	166.94 104.67		
	2	T5 Half Hip	12 /12	**17-09-08**	8-02-09	2 x 4	1-03-08	1-10-08 8-02-09	185.34 117.33		
	2	T6 Half Hip	12 /12	**17-09-08**	9-04-09	2 x 4	1-03-08	1-10-08 9-04-09	210.83 132.00		
	2	T7 Half Hip	12 /12	**17-09-08**	10-06-09	2 x 4	1-03-08	1-10-08 10-06-09	208.65 127.67		
	3	T8 Half Hip	12 /12	**17-09-08**	11-08-09	2 x 4	1-03-08	1-10-08 11-08-09	350.76 216.00		
	2	T9 Roof Special	12 /12 10 /12	**18-00-00**	10-10-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	204.82 129.33		
	4	T10A Common	12 /12	**17-09-08**	10-10-08	2 x 4	1-03-08	1-10-08 2-01-00	381.53 238.67		
	1	T11 Roof Special	12 /12 10 /12	**10-05-00**	7-01-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	55.81 36.33		
	1	T12 Common Girder	12 /12	**6-10-00**	4-10-08	2 x 4	10-08 10-08	1-05-08 1-05-08	33.29 23.17		
	1	G12 GABLE	12 /12	**6-10-00**	4-10-08	2 x 4	10-08 10-08	1-05-08 1-05-08	34.17 24.17		
	3	T90 Common	5 /12	**7-08-00**	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	73.47 47.00		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50530
Builder:	ROYAL PINE HOMES	PlanLog:	200902
Project:	FORESTSIDE PH. 6	Layout ID:	402481
Location:	BRAMPTON	Ref #	
Model:	UNIT1804 END	Page:	2 of 2
Lot #:		Date:	04/18/2019
Elevation:	EL:A LOT5	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G90 GABLE	5 /12	**7-08-00**	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	24.03 15.33		
	2	P2G GABLE	7 /12	**4-06-12**	2-07-15	2 x 4		2-07-15	24.91 16.00		
	5	J1 Jack-Open	7 /12	**5-10-08**	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	92.39 58.33		
	4	J1A Jack-Open	7 /12	**5-08-00**	4-08-09	2 x 4		1-04-14 4-08-09	65.83 41.33		

TOTAL # TRUSS= **41** TOTAL BFT OF ALL TRUSSES= **1690.66** BFT. TOTAL WEIGHT OF ALL TRSSES **2689.61** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **4**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	G1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:24:25 2019 Page 1
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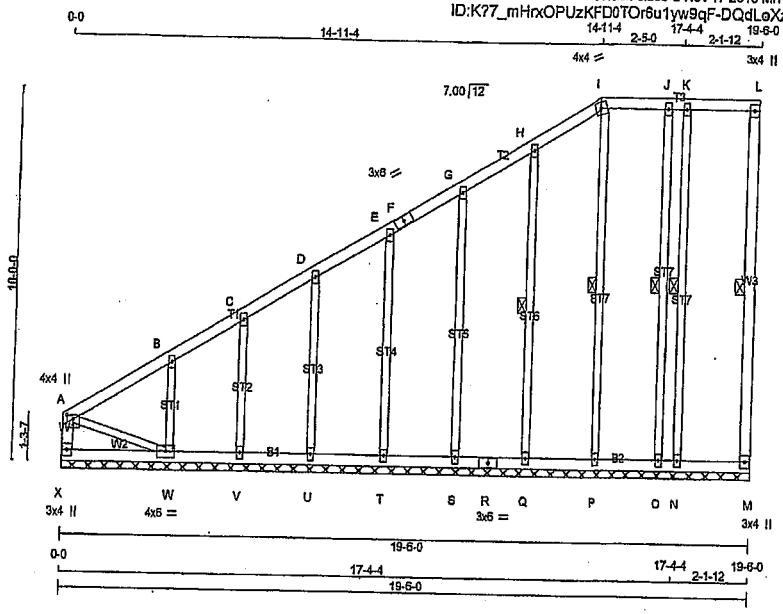

TOTAL WEIGHT = 111 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
X - A	2x4	DRY	No.2	SPF
X - R	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B, C, D, E, G, H, J, K						
B	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
I	TTW-m	MT20	4.0	4.0		
L	TMV+p	MT20	3.0	4.0		
M	BMV1+p	MT20	3.0	4.0		
N, O, P, Q, S, T, U, V						
N	BMW1+w	MT20	2.0	4.0		
R	BS-t	MT20	3.0	6.0		
W	BMWW1-t	MT20	4.0	6.0		
X	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-M, K-N, H-Q, I-P, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB., FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-10 / 0	-84.9	-84.9	0.09 (1)	6.25
B-C	-28 / 0	-84.9	-84.9	0.09 (1)	6.25
C-D	-8 / 0	-84.9	-84.9	0.04 (1)	10.00
D-E	-8 / 0	-84.9	-84.9	0.04 (1)	10.00
E-F	-6 / 0	-84.9	-84.9	0.04 (1)	10.00
F-G	-6 / 0	-84.9	-84.9	0.04 (1)	10.00
G-H	-2 / 0	-84.9	-84.9	0.04 (1)	10.00
H-I	-10 / 0	-84.9	-84.9	0.04 (1)	6.25
I-J	0 / 0	-84.9	-84.9	0.04 (1)	10.00
J-K	0 / 0	-84.9	-84.9	0.04 (1)	10.00
K-L	0 / 0	-84.9	-84.9	0.04 (1)	10.00
M-L	-72 / 0	0.0	0.0	0.03 (1)	6.25
X-A	-120 / 0	0.0	0.0	0.01 (1)	7.81
X-W	0 / 0	-38.5	-38.5	0.08 (3)	10.00
W-V	0 / 12	-38.5	-38.5	0.06 (3)	10.00
V-U	0 / 9	-38.5	-38.5	0.03 (3)	10.00
U-T	0 / 7	-38.5	-38.5	0.03 (3)	10.00
T-S	0 / 5	-38.5	-38.5	0.02 (3)	10.00
S-R	0 / 3	-38.5	-38.5	0.02 (3)	10.00
R-Q	0 / 3	-38.5	-38.5	0.02 (3)	10.00
Q-P	0 / 1	-38.5	-38.5	0.02 (3)	10.00
P-O	0 / 0	-38.5	-38.5	0.02 (3)	10.00
O-N	0 / 0	-38.5	-38.5	0.03 (3)	10.00
N-M	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-K	-121 / 0	0.08 (1)
W-B	-254 / 0	0.05 (1)
V-C	-139 / 0	0.04 (1)
U-D	-174 / 0	0.08 (1)
T-E	-168 / 0	0.12 (1)
S-G	-166 / 0	0.18 (1)
Q-H	-187 / 0	0.10 (1)
P-I	-135 / 0	0.09 (1)
O-J	-125 / 0	0.08 (1)
A-W	0 / 20	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (B-C:1) , BC=0.06/1.00 (V-W:3) , WB=0.18/1.00 (G-S:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (I) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1900034
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:45 2019 Page 1

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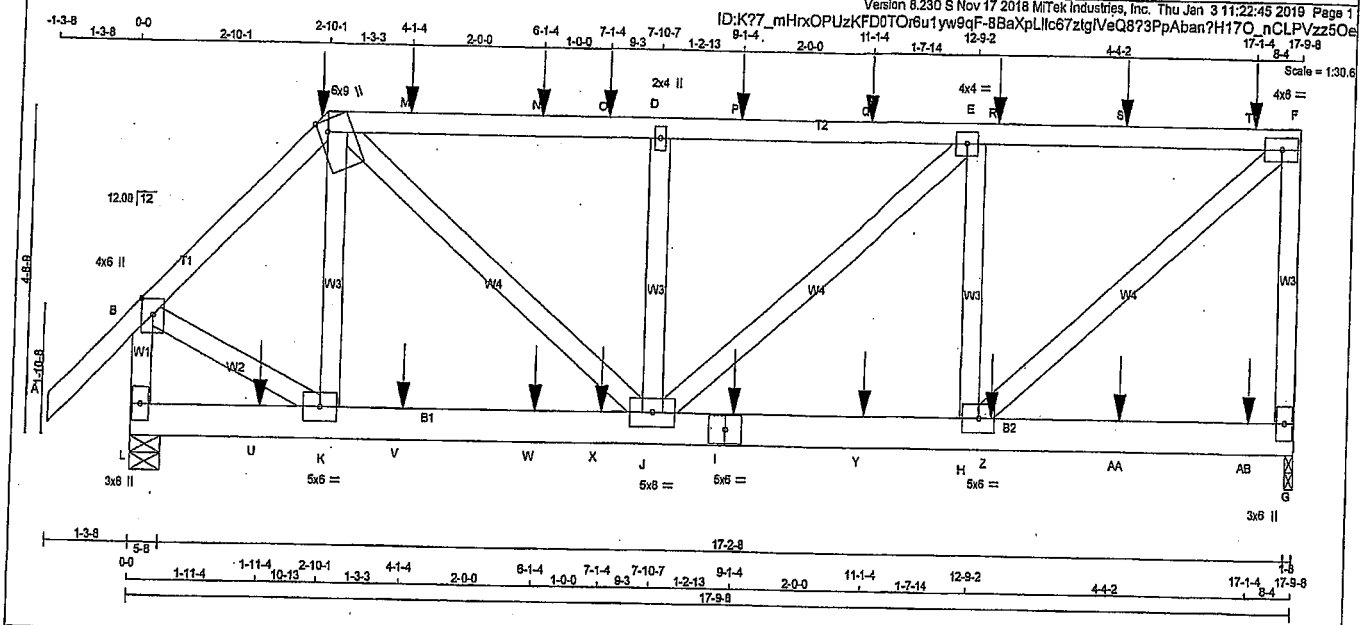

TOTAL WEIGHT = 2 X 99 = 199 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	2	12	SIDE(0.0)
I- G	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2244	0	2244	0	0	1-8	1-8
L	2221	0	2221	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1701	850 / 0	379 / 0	0 / 0	0 / 0	472 / 0	0 / 0
L	1676	862 / 0	355 / 0	0 / 0	0 / 0	459 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-84.9	-84.9	0.07 (1)	10.00	
B- C	-1854 / 0	-84.9	-84.9	0.08 (1)	6.25	
C- M	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35	
M- N	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35	
N- O	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35	
O- D	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35	
D- P	-2381 / 0	-84.9	-84.9	0.36 (1)	5.35	
P- Q	-2381 / 0	-84.9	-84.9	0.36 (1)	5.35	
Q- E	-2381 / 0	-84.9	-84.9	0.36 (1)	5.35	
E- R	-1958 / 0	-84.9	-84.9	0.32 (1)	5.35	
R- S	-1958 / 0	-84.9	-84.9	0.32 (1)	5.35	
S- T	-1958 / 0	-84.9	-84.9	0.32 (1)	5.35	
T- F	-1958 / 0	-84.9	-84.9	0.32 (1)	5.35	
G- F	-2065 / 0	0.0	0.0	0.35 (1)	7.69	
L- B	-2185 / 0	0.0	0.0	0.13 (1)	7.53	
L- U	0 / 0	-38.5	-38.5	0.07 (3)	10.00	
U- K	0 / 0	-38.5	-38.5	0.07 (3)	10.00	
K- V	0 / 1294	-38.5	-38.5	0.14 (2)	10.00	
V- W	0 / 1294	-38.5	-38.5	0.14 (2)	10.00	
W- X	0 / 1294	-38.5	-38.5	0.14 (2)	10.00	
X- J	0 / 1294	-38.5	-38.5	0.14 (2)	10.00	
J- I	0 / 1958	-38.5	-38.5	0.19 (2)	10.00	
I- Y	0 / 1958	-38.5	-38.5	0.19 (2)	10.00	
Y- H	0 / 1958	-38.5	-38.5	0.19 (2)	10.00	
H- Z	0 / 0	-38.5	-38.5	0.08 (3)	10.00	
Z-AA	0 / 0	-38.5	-38.5	0.08 (3)	10.00	
AA-AB	0 / 0	-38.5	-38.5	0.08 (3)	10.00	
AB- G	0 / 0	-38.5	-38.5	0.08 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-274 / 105	0.03 (1)
C- J	0 / 1431	0.13 (1)
J- D	-944 / 0	0.11 (1)
J- E	0 / 1585	0.85 (1)
H- E	0 / 0	0.14 (1)
H- R	0 / 1585	0.33 (1)
B- C	0 / 1447	0.13 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-10-1	-35	-39	—	FRONT	VERT	DEAD	—	—
C	2-10-1	-162	-162	—	FRONT	VERT	SNOW	—	—
I	9-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
M	4-1-4	-164	-164	—	BACK	VERT	TOTAL	—	—
N	6-1-4	-156	-156	—	BACK	VERT	TOTAL	—	—
O	7-1-4	-156	-156	—	BACK	VERT	TOTAL	—	—
P	9-1-4	-156	-156	—	BACK	VERT	TOTAL	—	—
Q	11-1-4	-156	-156	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/02/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1900039
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.19/1.00 (H-J:2) , WB=0.23/1.00 (F-H:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	13-1-4	-164	-164	—	BACK	VERT	TOTAL	—	—
S	15-1-4	-164	-164	—	BACK	VERT	TOTAL	—	—
T	17-1-4	-178	-178	—	BACK	VERT	TOTAL	—	—
U	1-11-4	-75	-95	—	BACK	VERT	TOTAL	—	—
V	4-1-4	-75	-95	—	BACK	VERT	TOTAL	—	—
W	6-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
X	7-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
Y	11-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
Z	13-1-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AA	15-1-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AB	17-1-4	-81	-103	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1900039
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:46 2019 Page 1

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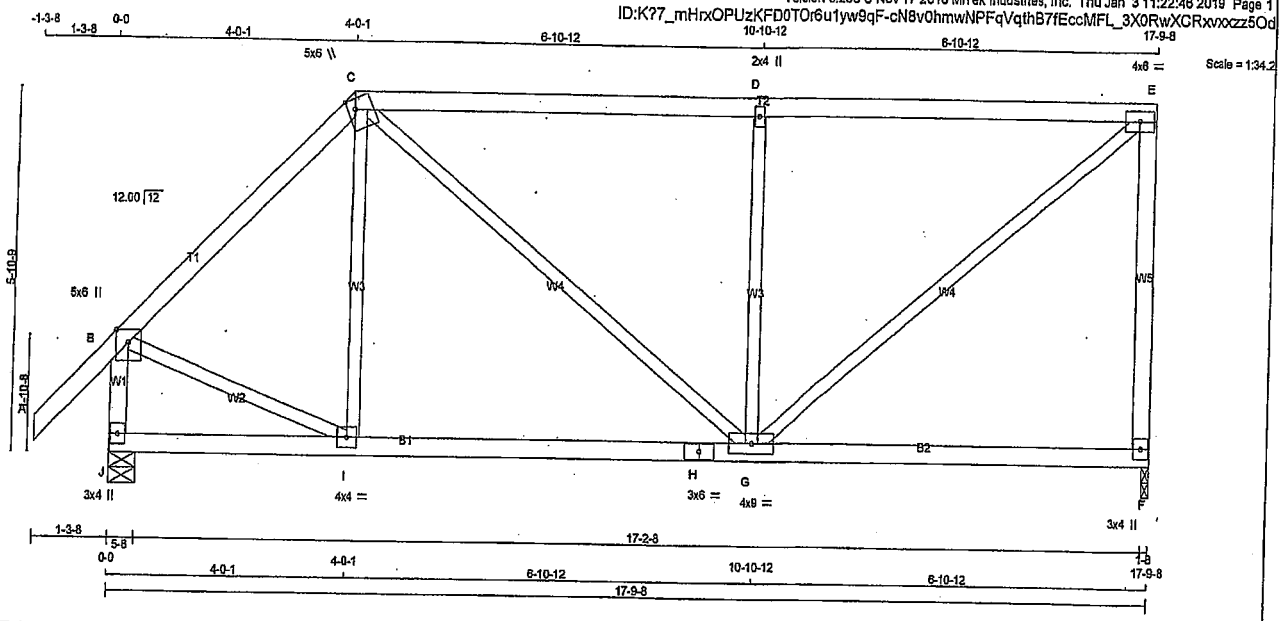


TOTAL WEIGHT = 78 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1097	0	1097	0	0	1-8	1-8
J	1216	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
J	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

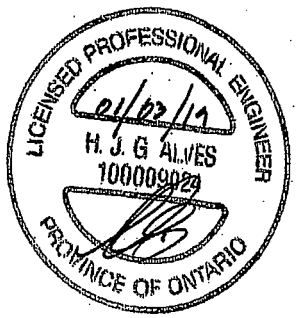
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	
B-C	-900 / 0	-84.9	-84.9	0.26 (1)	6.16	
C-D	-952 / 0	-84.9	-84.9	0.75 (1)	5.02	
D-E	-952 / 0	-84.9	-84.9	0.75 (1)	5.02	
F-E	-991 / 0	0.0	0.0	0.64 (1)	7.81	
J-B	-1162 / 0	0.0	0.0	0.13 (1)	7.39	
J-I	0 / 0	-38.5	-38.5	0.19 (3)	10.00	
I-H	0 / 634	-38.5	-38.5	0.42 (2)	10.00	
H-G	0 / 634	-38.5	-38.5	0.42 (2)	10.00	
G-F	0 / 0	-38.5	-38.5	0.36 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	-13 / 154	0.04 (3)
C-G	0 / 410	0.09 (1)
G-D	-723 / 0	0.39 (1)
G-E	0 / 1223	0.26 (1)
B-I	0 / 676	0.15 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.75/1.00 (D-E:1) , BC=0.42/1.00 (G-I:2) , WB=0.39/1.00 (D-G:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (I) (INPUT = 0.90)
JSI METAL= 0.44 (H) (INPUT = 1.00)

DWG NO. TAM T19000040
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T3A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:47 2019 Page 1

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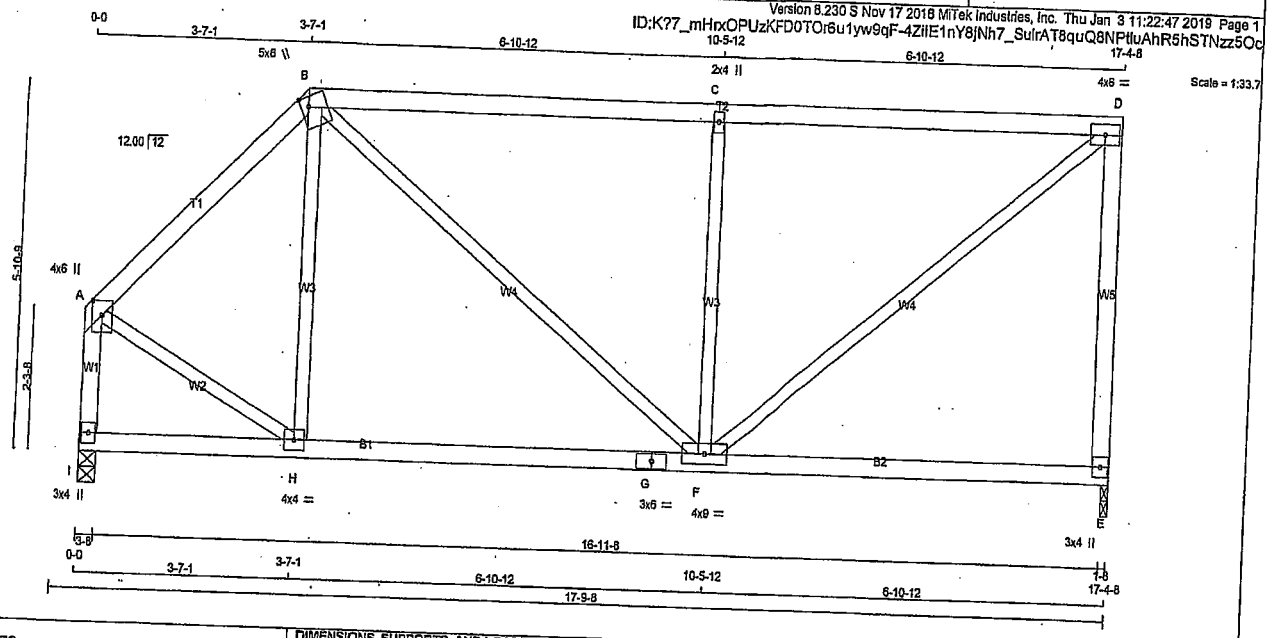


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.75	2.00
B	TTWW+m	MT20	5.0	6.0	2.00	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 75 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	813	405 / 0	182 / 0	0 / 0	0 / 0	226 / 0	0 / 0
I	813	405 / 0	182 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

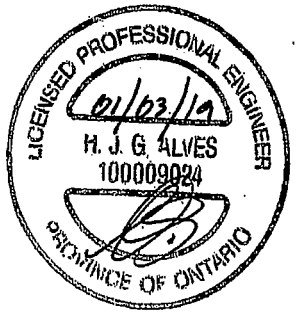
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-811 / 0	-84.9	-84.9	0.20 (1)	6.25	
B-C	-920 / 0	-84.9	-84.9	0.74 (1)	5.08	
C-D	-921 / 0	-84.9	-84.9	0.75 (1)	5.08	
E-D	-965 / 0	0.0	0.0	0.63 (1)	7.81	
I-A	-1028 / 0	0.0	0.0	0.13 (1)	7.74	
I-H	0 / 0	-38.5	-38.5	0.18 (3)	10.00	
H-G	0 / 569	-38.5	-38.5	0.42 (2)	10.00	
G-F	0 / 569	-38.5	-38.5	0.42 (2)	10.00	
F-E	0 / 0	-38.5	-38.5	0.36 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-B	-85 / 110	0.05 (1)
B-F	0 / 452	0.10 (1)
F-C	-723 / 0	0.39 (1)
F-D	0 / 1182	0.27 (1)
A-H	0 / 645	0.15 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.75/1.00 (C-D:1) , BC=0.42/1.00 (F-H:2) , WB=0.39/1.00 (C-F:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.42 (G) (INPUT = 1.00)

DWG NO. TAM T1900041
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:47 2019 Page 1
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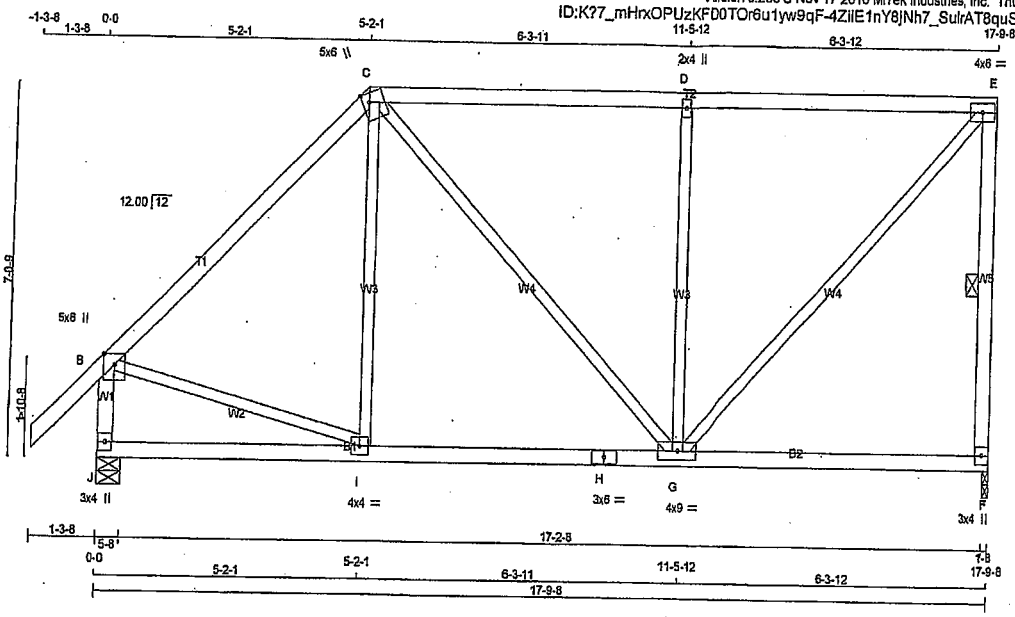


TOTAL WEIGHT = 83 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1097	0	1097	0	0	1-8	1-8
J	1216	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
J	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

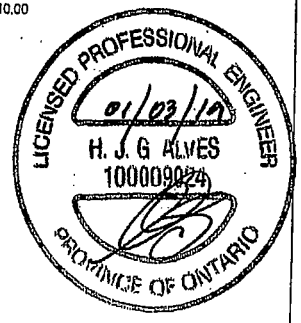
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)		10.00
B-C	-875 / 0	-84.9	-84.9	0.44 (1)		5.94
C-D	-746 / 0	-84.9	-84.9	0.60 (1)		5.88
D-E	-746 / 0	-84.9	-84.9	0.60 (1)		5.88
F-E	-999 / 0	0.0	0.0	0.22 (1)		6.25
J-B	-1136 / 0	0.0	0.0	0.13 (1)		7.45
J-I	0 / 0	-38.5	-38.5	0.21 (3)		10.00
I-H	0 / 618	-38.5	-38.5	0.36 (2)		10.00
H-G	0 / 618	-38.5	-38.5	0.36 (2)		10.00
G-F	0 / 0	-38.5	-38.5	0.29 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	0 / 204	0.05 (3)
C-G	0 / 189	0.04 (1)
G-D	-662 / 0	0.57 (1)
G-E	0 / 1090	0.25 (1)
B-I	0 / 642	0.14 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.60/1.00 (D-E:1) , BC=0.36/1.00 (G-I:2) , WB=0.57/1.00 (D-G:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (G) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)

DWG NO. TAM T1900042
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T4A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:48 2019 Page 1
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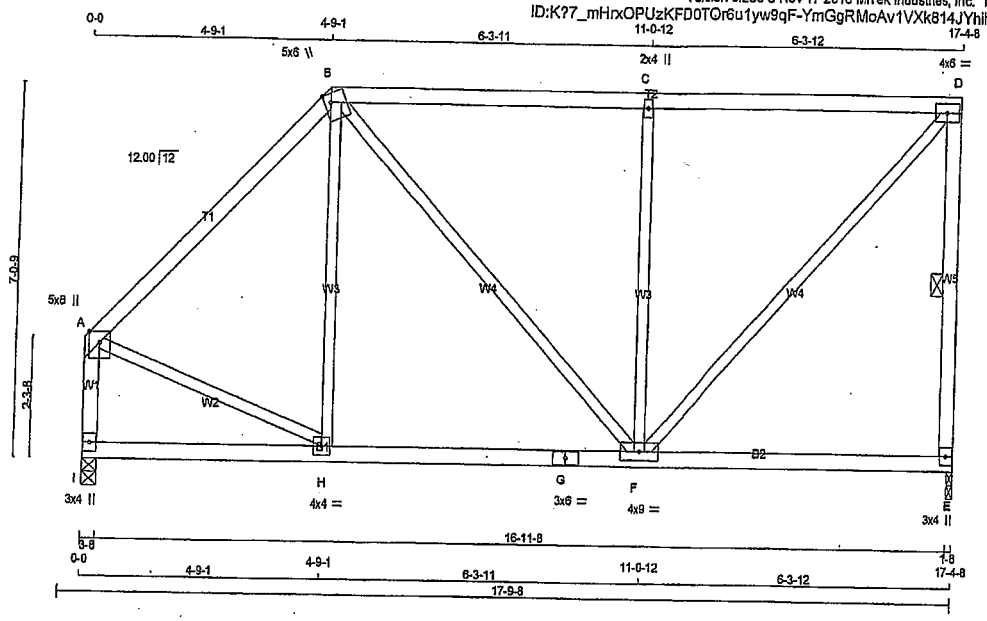


TOTAL WEIGHT = 81 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.00	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	813	405 / 0	182 / 0	0 / 0	0 / 0	226 / 0	0 / 0
I	813	405 / 0	182 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

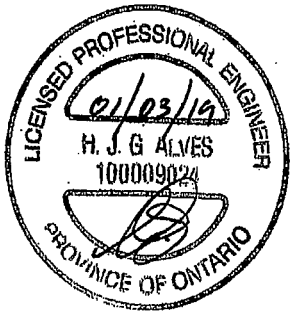
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-806 / 0	-84.9	-84.9	0.36 (1)	6.24	H- B	0 / 165	0.04 (3)
B- C	-722 / 0	-84.9	-84.9	0.60 (1)	5.96	B- F	0 / 227	0.05 (1)
C- D	-723 / 0	-84.9	-84.9	0.60 (1)	5.96	F- C	-662 / 0	0.57 (1)
E- D	-973 / 0	0.0	0.0	0.22 (1)	6.25	F- D	0 / 1056	0.24 (1)
I- A	-1000 / 0	0.0	0.0	0.12 (1)	7.81	A- H	0 / 611	0.14 (1)
I- H	0 / 0	-38.5	-38.5	0.19 (3)	10.00			
H- G	0 / 568	-38.5	-38.5	0.35 (2)	10.00			
G- F	0 / 568	-38.5	-38.5	0.35 (2)	10.00			
F- E	0 / 0	-38.5	-38.5	0.29 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.60/1.00 (C-D:1) , BC=0.35/1.00 (F-H:2) , WB=0.57/1.00 (C-F:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.28 (A) (INPUT = 1.00)

DWG NO. TAM T19 00043
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:49 2019 Page 1

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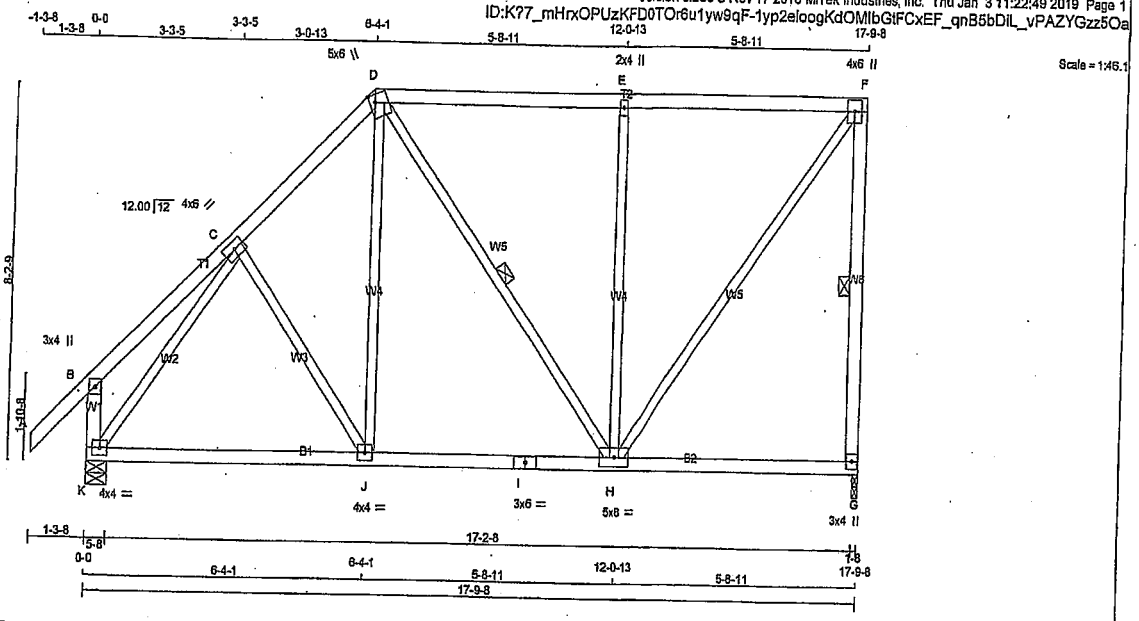


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1097	0	1097	0	0	1-8	1-8
K	1216	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
K	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

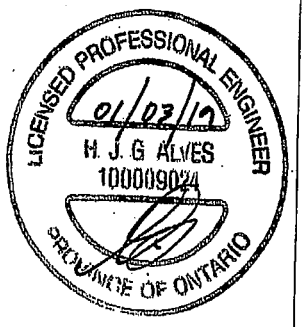
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-84.9	-84.9	0.12 (1)		10.00
B- C	0 / 21	-84.9	-84.9	0.13 (1)		10.00
C- D	-849 / 0	-84.9	-84.9	0.14 (1)		6.25
D- E	-598 / 0	-84.9	-84.9	0.48 (1)		6.25
E- F	-598 / 0	-84.9	-84.9	0.48 (1)		6.25
G- F	-1006 / 0	0.0	0.0	0.31 (1)		6.24
K- B	-221 / 0	0.0	0.0	0.02 (1)		7.81
K- J	0 / 600	-36.5	-36.5	0.34 (2)		10.00
J- I	0 / 587	-36.5	-36.5	0.34 (2)		10.00
I- H	0 / 567	-36.5	-36.5	0.34 (2)		10.00
H- G	0 / 0	-36.5	-36.5	0.21 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-29 / 57	0.02 (1)
J- D	0 / 322	0.07 (2)
D- H	-24 / 16	0.02 (3)
H- E	-600 / 0	0.79 (1)
H- F	0 / 1013	0.23 (1)
K- C	-1076 / 0	0.56 (1)



TOTAL WEIGHT = 93 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.48/1.00 (E-F:1) , BC=0.34/1.00 (H-J:2) , WB=0.79/1.00 (E-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.27 (F) (INPUT = 1.00)

DWG NO. TAM T1900044
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:50 2019 Page 1
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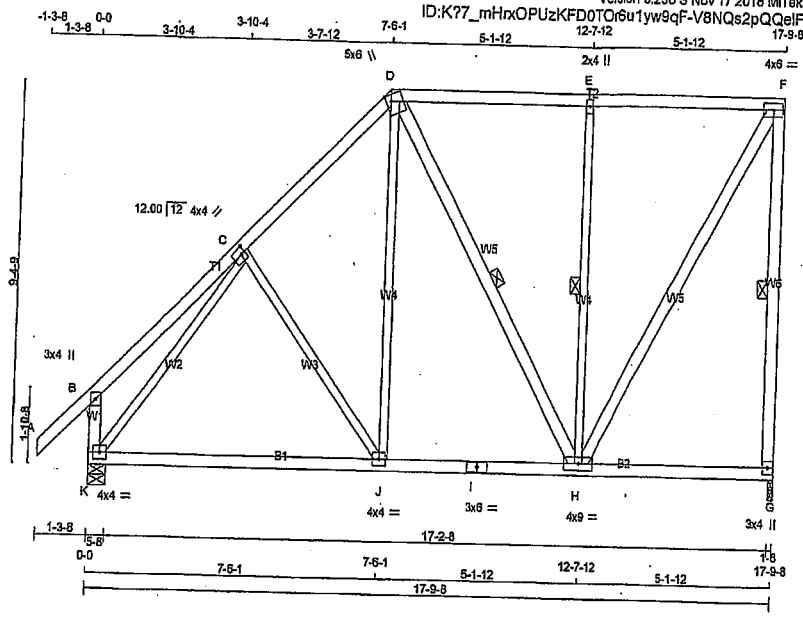

TOTAL WEIGHT = 105 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1097	0	1097	0	0	1-8	1-8
K	1216	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
K	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00
B-C	0 / 26	-84.9	-84.9	0.19 (1)	10.00
C-D	-798 / 0	-84.9	-84.9	0.20 (1)	6.25
D-E	-481 / 0	-84.9	-84.9	0.36 (1)	6.25
E-F	-481 / 0	-84.9	-84.9	0.38 (1)	6.25
G-F	-1011 / 0	0.0	0.0	0.42 (1)	6.23
K-B	-239 / 0	0.0	0.0	0.03 (1)	7.81
K-J	0 / 607	-38.5	-38.5	0.42 (2)	10.00
J-I	0 / 549	-38.5	-38.5	0.42 (2)	10.00
I-H	0 / 549	-38.5	-38.5	0.42 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-105 / 32	0.08 (1)
J-D	0 / 413	0.09 (2)
D-H	-145 / 0	0.08 (2)
H-E	-539 / 0	0.31 (1)
H-F	0 / 971	0.16 (1)
K-C	-1049 / 0	0.76 (1)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012 .
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. . RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 957 (0.22")

CSI: TC=0.42/1.00 (F-G:1) , BC=0.42/1.00 (J-K:2) , WB=0.76/1.00 (C-K:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

DWG NO. TAM T1900045
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:51 2019 Page 1
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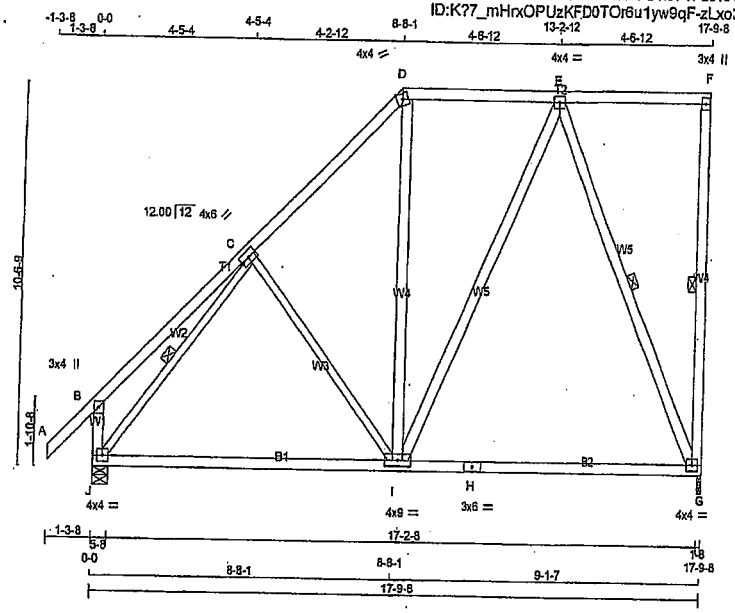


TOTAL WEIGHT = 108 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - I	2x3	DRY	No.2	SPF
J - C	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1097	0	1097	0	0	1-8	1-8
J	1216	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
J	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-G, C-J.

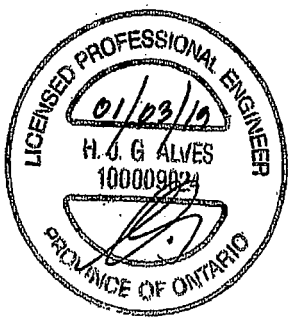
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00
B-C	0 / 31	-84.9	-84.9	0.28 (1)	10.00
C-D	-753 / 0	-84.9	-84.9	0.28 (1)	6.25
D-E	-512 / 0	-84.9	-84.9	0.30 (1)	6.25
E-F	0 / 0	-84.9	-84.9	0.29 (1)	10.00
G-F	-146 / 0	0.0	0.0	0.08 (1)	6.25
J-B	-256 / 0	0.0	0.0	0.03 (1)	7.81
J-I	0 / 614	-38.5	-38.5	0.74 (2)	10.00
I-H	0 / 365	-38.5	-38.5	0.72 (3)	10.00
H-G	0 / 365	-38.5	-38.5	0.72 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-I	-172 / 14	0.18 (1)
I-D	0 / 260	0.04 (2)
I-E	0 / 397	0.06 (2)
E-G	-896 / 0	0.62 (1)
J-C	-1034 / 0	0.36 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 693 (0.31")

CSI: TC=0.30/1.00 (D-E:1) , BC=0.74/1.00 (I-J:2) , WB=0.62/1.00 (E-G:1) , SSI=0.22/1.00 (G-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.80)
JSI METAL= 0.26 (C) (INPUT = 1.00)

DWG NO. TAM T19000046
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:51 2019 Page 1
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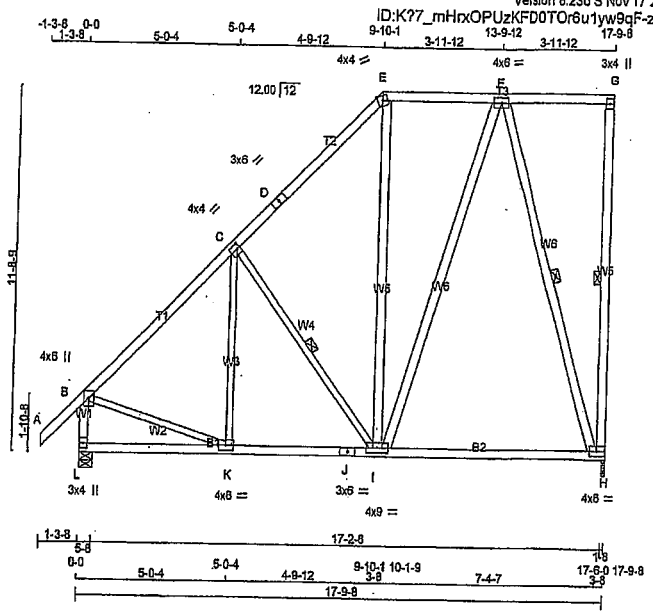


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - C	2x3	DRY	No.2	SPF
C - I	2x3	DRY	No.2	SPF
B - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1097	0	1097	0	0	1-8	1-8
L	1216	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS
1ST LCASE - MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
L	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

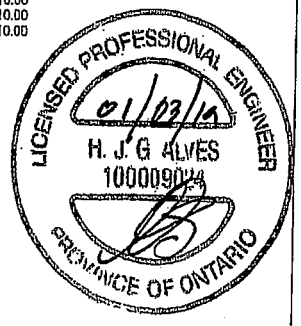
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-84.9	-84.9	0.12 (1)		10.00
B- C	-887 / 0	-84.9	-84.9	0.29 (1)		6.18
C- D	-657 / 0	-84.9	-84.9	0.27 (1)		6.25
D- E	-657 / 0	-84.9	-84.9	0.27 (1)		6.25
E- F	-439 / 0	-84.9	-84.9	0.17 (1)		6.25
F- G	0 / 0	-84.9	-84.9	0.22 (1)		10.00
H- G	-128 / 0	0.0	0.0	0.09 (1)		6.25
L- B	-1131 / 0	0.0	0.0	0.13 (1)		7.46
L- K	0 / 0	-38.5	-38.5	0.17 (3)		10.00
K- J	0 / 652	-38.5	-38.5	0.46 (2)		10.00
J- I	0 / 652	-38.5	-38.5	0.46 (2)		10.00
I- H	0 / 295	-38.5	-38.5	0.41 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-24 / 122	0.03 (3)
C- I	-356 / 0	0.16 (1)
I- E	0 / 195	0.03 (3)
B- K	0 / 681	0.15 (1)
I- F	0 / 457	0.07 (2)
F- H	-896 / 0	0.76 (1)



TOTAL WEIGHT = 117 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 898 (0.24")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.46/1.00 (I-K:2) , WB=0.76/1.00 (F-H:1) , SSI=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (J) (INPUT = 0.90)
JSI METAL= 0.52 (J) (INPUT = 1.00)

DWG NO. TAM T1900047
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
259671	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 13:33:22 2019 Page 1
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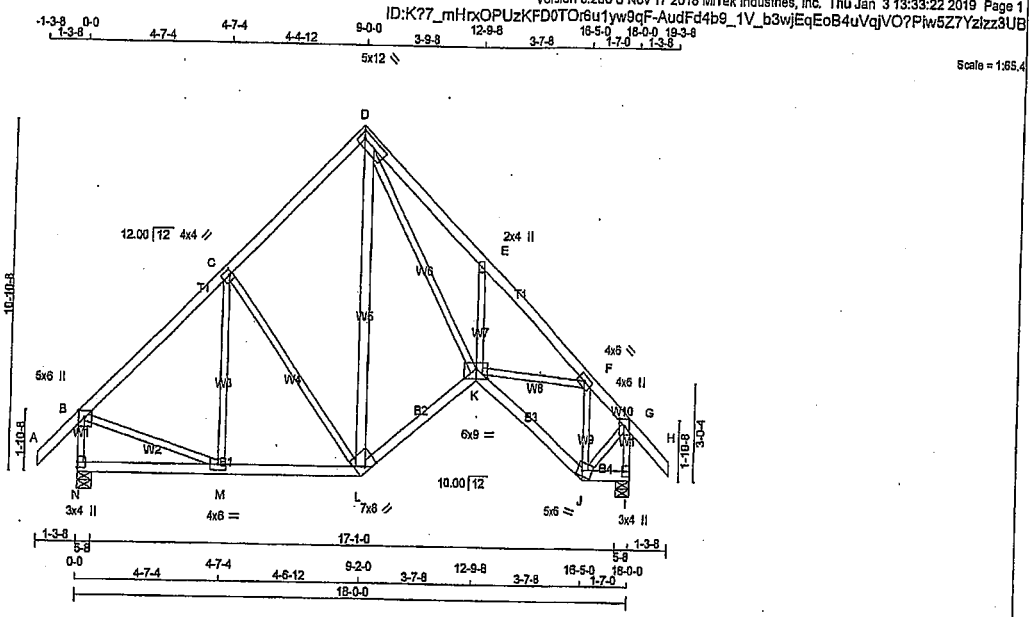


TOTAL WEIGHT = 2 X 101 = 203 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
N - B	2x3	DRY	No.2	SPF
I - G	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-t	MT20	5.0	12.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	6.0	2.75	2.00
I	BMV1+p	MT20	3.0	4.0	2.00	Edge
J	BBWW-m	MT20	5.0	8.0	2.00	2.00
K	BBWWW-p	MT20	6.0	9.0	Edge	4.50
L	BMBWW-h	MT20	7.0	8.0	Edge	4.50
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1224	0	1224	0	0	5-8	5-8
I	1239	0	1233	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	921	462 / 0	189 / 0	0 / 0	0 / 0	250 / 0	0 / 0
I	927	487 / 0	189 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

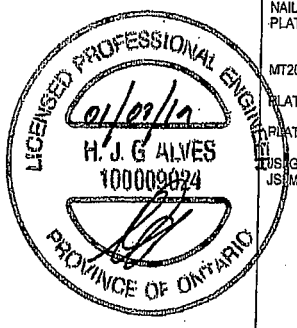
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-84.9	-84.9	0.12 (1)		10.00
B- C	-910 / 0	-84.9	-84.9	0.24 (1)		6.19
C- D	-693 / 0	-84.9	-84.9	0.23 (1)		6.25
D- E	-1609 / 0	-84.9	-84.9	0.20 (1)		5.04
E- F	-1606 / 0	-84.9	-84.9	0.15 (1)		5.10
F- G	-744 / 0	-84.9	-84.9	0.09 (1)		6.25
G- H	0 / 42	-84.9	-84.9	0.12 (1)		10.00
N- B	-1155 / 0	0.0	0.0	0.18 (1)		6.50
I- G	-1203 / 0	0.0	0.0	0.18 (1)		6.40
N- M	0 / 0	-38.5	-38.5	0.18 (3)		10.00
M- L	0 / 666	-38.5	-38.5	0.26 (2)		10.00
L- K	0 / 609	-38.5	-38.5	0.20 (2)		10.00
K- J	0 / 596	-38.5	-38.5	0.20 (2)		10.00
J- I	0 / 0	-38.5	-38.5	0.02 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-5 / 163	0.04 (3)
C- L	-336 / 0	0.39 (1)
L- D	0 / 110	0.02 (3)
D- K	0 / 1451	0.33 (1)
K- E	-348 / 0	0.09 (1)
K- F	0 / 689	0.15 (1)
J- F	-771 / 0	0.15 (1)
B- M	0 / 702	0.16 (1)
J- G	0 / 706	0.16 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.26/1.00 (L-M:2) , WB=0.39/1.00 (C-L:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

TUS GRIP= 0.88 (D) (INPUT = 0.90)
JS METAL= 0.34 (B) (INPUT = 1.00)

DWG NO. TAM T1900092
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T9A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:52 2019 Page 1

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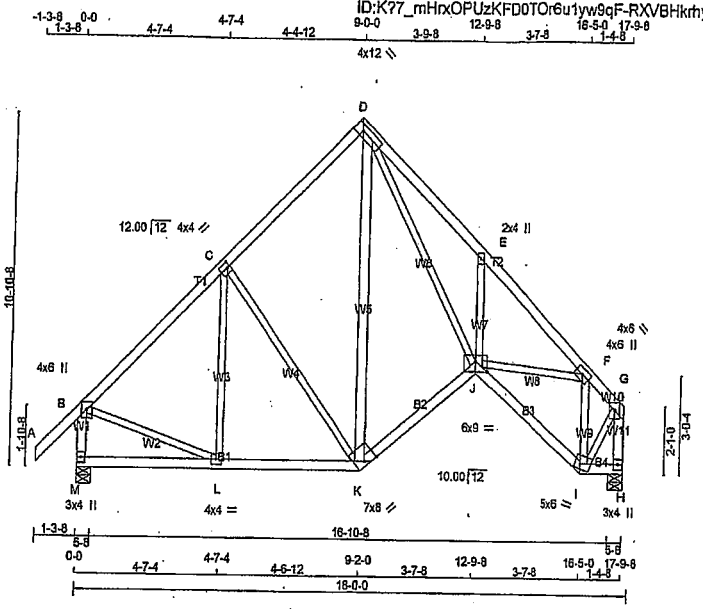


TOTAL WEIGHT = 2 X 100 = 200 lb [MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-l	MT20	4.0	12.0		Edge
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-m	MT20	5.0	6.0	1.75	2.25
J	BBWWW-p	MT20	6.0	9.0	Edge	4.50
K	BMBWW-h	MT20	7.0	8.0	Edge	4.50
L	BMWW-t	MT20	4.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1216	0	1216	0	0	5-8	5-8
H	1097	0	1097	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0
H	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

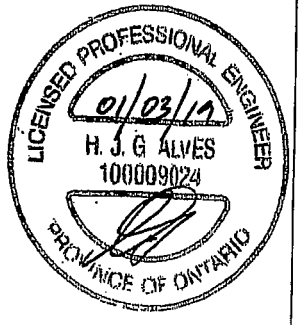
TOTAL LOAD CASES: (4).

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00
B-C	-902 / 0	-84.9	-84.9	0.24 (1)	6.21
C-D	-663 / 0	-84.9	-84.9	0.23 (1)	6.25
D-E	-1567 / 0	-84.9	-84.9	0.20 (1)	5.09
E-F	-1567 / 0	-84.9	-84.9	0.15 (1)	5.15
F-G	-640 / 0	-84.9	-84.9	0.06 (1)	6.25
M-B	-1147 / 0	0.0	0.0	0.13 (1)	7.43
H-G	-1071 / 0	0.0	0.0	0.13 (1)	7.61
M-L	0 / 0	-38.5	-38.5	0.17 (3)	10.00
L-K	0 / 660	-38.5	-38.5	0.26 (2)	10.00
K-J	0 / 600	-38.5	-38.5	0.20 (2)	10.00
J-I	0 / 542	-38.5	-38.5	0.19 (2)	10.00
I-H	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-2 / 164	0.04 (3)
C-K	-338 / 0	0.39 (1)
K-D	0 / 115	0.02 (3)
D-J	0 / 1402	0.32 (1)
J-E	-345 / 0	0.09 (1)
J-F	0 / 701	0.16 (1)
I-F	-846 / 0	0.16 (1)
B-L	0 / 695	0.16 (1)
I-G	0 / 759	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.26/1.00 (K-L:2) , WB=0.39/1.00 (C-K:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)

DWG NO. TAM T19000 48
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T10A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:53 2019 Page 1

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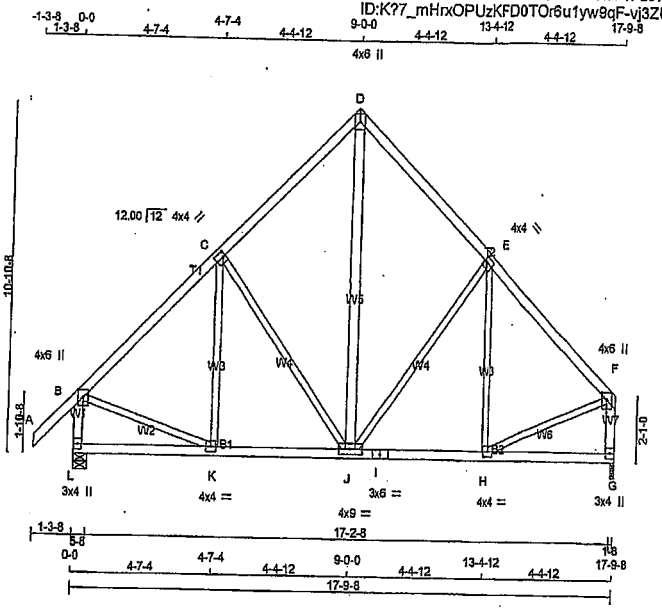


TOTAL WEIGHT = 3 X 95 = 286 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1216	0	1216	0	0	5-8	5-8
G	1097	0	1097	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0
G	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

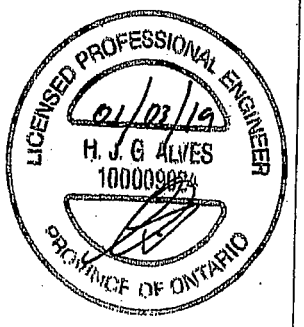
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00
B-C	-898 / 0	-84.9	-84.9	0.23 (1)	6.22
C-D	-896 / 0	-84.9	-84.9	0.23 (1)	6.25
D-E	-695 / 0	-84.9	-84.9	0.21 (1)	6.25
E-F	-869 / 0	-84.9	-84.9	0.22 (1)	6.25
L-B	-1144 / 0	0.0	0.0	0.13 (1)	7.43
G-F	-1029 / 0	0.0	0.0	0.12 (1)	7.74
L-K	0 / 0	-38.5	-38.5	0.15 (3)	10.00
K-J	0 / 657	-38.5	-38.5	0.23 (2)	10.00
J-I	0 / 637	-38.5	-38.5	0.22 (2)	10.00
I-H	0 / 637	-38.5	-38.5	0.22 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.14 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	-22 / 138	0.03 (3)
C-J	-323 / 0	0.37 (1)
J-D	0 / 656	0.11 (1)
J-E	-288 / 0	0.33 (1)
H-E	-61 / 114	0.04 (1)
B-K	0 / 692	0.16 (1)
H-F	0 / 664	0.15 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.23/1.00 (J-K:2) , WB=0.37/1.00 (C-J:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)

DWG NO. TAM T1000049
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T10B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:54 2019 Page 1
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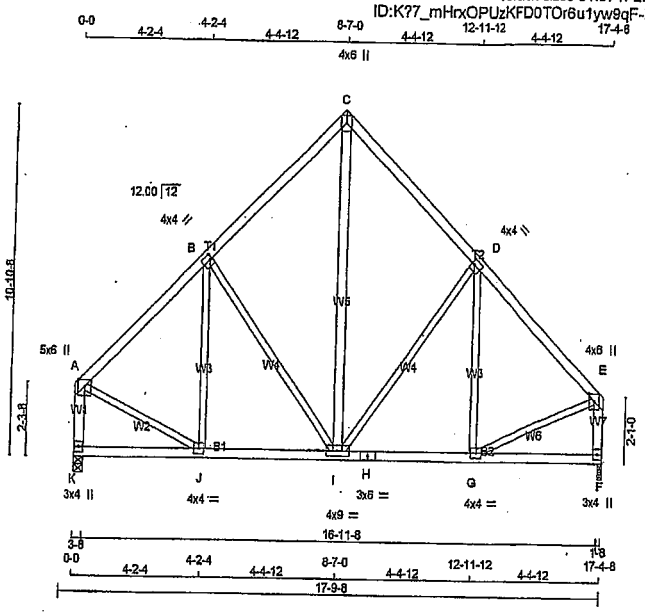


TOTAL WEIGHT = 92 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.00
C	TTW+p	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.00
E	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1072	0	1072	0	0	3-8	3-8
F	1072	0	1072	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	813	405 / 0	182 / 0	0 / 0	0 / 0	226 / 0	0 / 0
F	813	405 / 0	182 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-813 / 0	-84.9	-84.9	0.21 (1)	6.25	
B- C	-663 / 0	-84.9	-84.9	0.20 (1)	6.25	
C- D	-665 / 0	-84.9	-84.9	0.21 (1)	6.25	
D- E	-842 / 0	-84.9	-84.9	0.22 (1)	6.25	
K- A	-1007 / 0	0.0	0.0	0.13 (1)	7.80	
F- E	-1003 / 0	0.0	0.0	0.12 (1)	7.80	
K- J	0 / 0	-38.5	-38.5	0.13 (3)	10.00	
J- I	0 / 598	-38.5	-38.5	0.20 (2)	10.00	
I- H	0 / 618	-38.5	-38.5	0.22 (2)	10.00	
H- G	0 / 618	-38.5	-38.5	0.22 (2)	10.00	
G- F	0 / 0	-38.5	-38.5	0.14 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-93 / 93	0.06 (1)
B- I	-257 / 0	0.29 (1)
I- C	0 / 610	0.10 (1)
I- D	-293 / 0	0.33 (1)
G- D	-54 / 117	0.04 (1)
A- J	0 / 658	0.15 (1)
G- E	0 / 664	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.22/1.00 (D-E:1) , BC=0.22/1.00 (G-I:2) , WB=0.33/1.00 (D-I:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

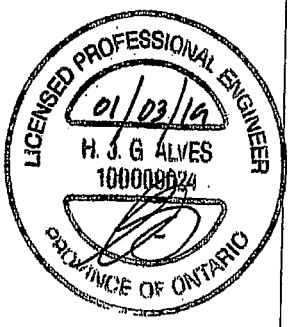
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (G) (INPUT = 0.90)
JSI METAL= 0.36 (E) (INPUT = 1.00)



DWG NO. TAM T1900050
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:55 2019 Page 1

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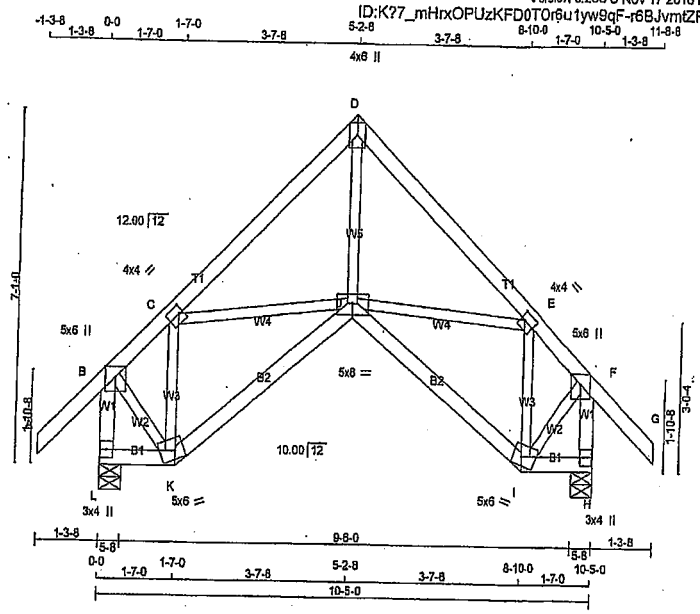


TOTAL WEIGHT = 56 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-m	MT20	5.0	6.0	2.00	2.25
J	BBWWW-p	MT20	5.0	8.0	2.75	4.00
K	BBWW-m	MT20	5.0	6.0	2.00	2.25
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	761	0	761	0	0	5-8	5-8
H	761	0	761	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	569	308 / 0	109 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	569	308 / 0	109 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

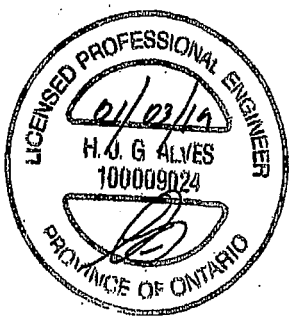
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00
B-C	-402 / 0	-84.9	-84.9	0.11 (1)	6.25
C-D	-632 / 0	-84.9	-84.9	0.15 (1)	6.25
D-E	-632 / 0	-84.9	-84.9	0.15 (1)	6.25
E-F	-402 / 0	-84.9	-84.9	0.11 (1)	6.25
F-G	0 / 42	-84.9	-84.9	0.12 (1)	10.00
L-B	-730 / 0	0.0	0.0	0.08 (1)	7.81
H-F	-730 / 0	0.0	0.0	0.08 (1)	7.81
L-K	0 / 0	-38.5	-38.5	0.02 (3)	10.00
K-J	0 / 342	-38.5	-38.5	0.16 (2)	10.00
J-I	0 / 342	-38.5	-38.5	0.16 (2)	10.00
I-H	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-D	0 / 594	0.13 (1)
J-E	0 / 172	0.04 (1)
I-E	-396 / 0	0.08 (1)
C-J	0 / 172	0.04 (1)
K-C	-396 / 0	0.08 (1)
B-K	0 / 400	0.09 (1)
I-F	0 / 400	0.09 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.15/1.00 (D-E:1) , BC=0.16/1.00 (J-K:2) , WB=0.13/1.00 (D-J:1) , SSI=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (I) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

DWG NO. TAM T19000 57
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:29:37 2019 Page 1

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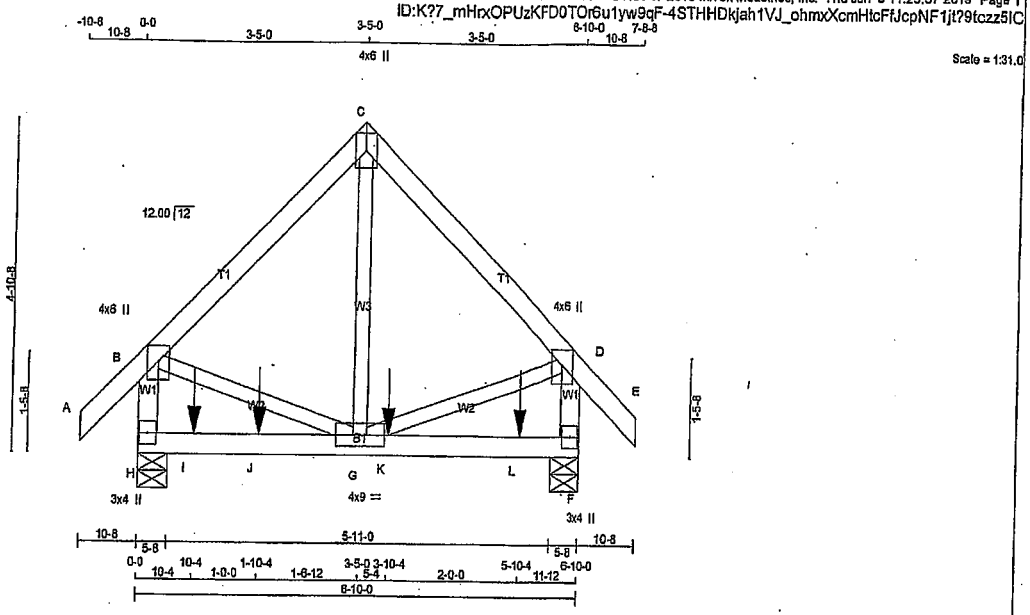


TOTAL WEIGHT = 33 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+p	MT20	4.0	6.0		
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1186	0	1186	0	0	5-8	5-8
F	1070	0	1070	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	882	493 / 0	157 / 0	0 / 0	0 / 0	232 / 0	0 / 0
F	796	444 / 0	142 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 29	-84.9	-84.9	0.07 (1)	10.00	G-C	0 / 633	0.16 (1)
B-C	-653 / 0	-84.9	-84.9	0.20 (1)	6.25	B-G	0 / 485	0.12 (1)
C-D	-653 / 0	-84.9	-84.9	0.20 (1)	6.25	G-D	0 / 485	0.12 (1)
D-E	0 / 29	-84.9	-84.9	0.07 (1)	10.00			
H-B	-839 / 0	0.0	0.0	0.10 (1)	7.81			
F-D	-839 / 0	0.0	0.0	0.10 (1)	7.81			
H-I	0 / 0	-38.5	-38.5	0.49 (1)	10.00			
I-J	0 / 0	-38.5	-38.5	0.49 (1)	10.00			
J-G	0 / 0	-38.5	-38.5	0.49 (1)	10.00			
G-K	0 / 0	-38.5	-38.5	0.49 (1)	10.00			
K-L	0 / 0	-38.5	-38.5	0.49 (1)	10.00			
L-F	0 / 0	-38.5	-38.5	0.49 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	10-4	-314	-314	—	FRONT	VERT	TOTAL	—	—
J	1-10-4	-311	-311	—	FRONT	VERT	TOTAL	—	—
K	3-10-4	-311	-311	—	FRONT	VERT	TOTAL	—	—
L	5-10-4	-311	-311	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.20/1.00 (C-D:1) , BC=0.49/1.00 (G-H:1) , WB=0.16/1.00 (C-G:1) , SSI=0.45/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

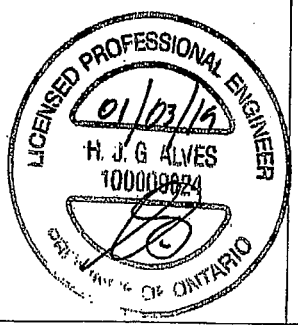
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)



DWG NO. TAM T19000 52
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	T13	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 12:17:49 2019 Page 1

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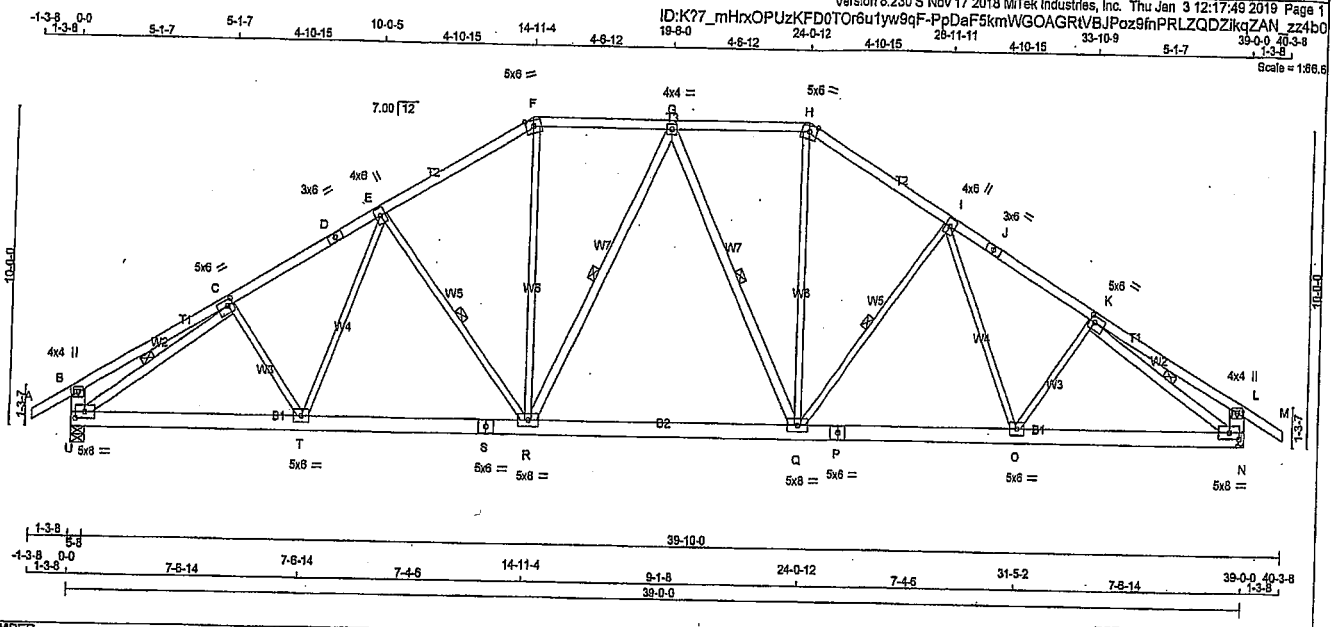

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
G - Q	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW-m	MT20	5.0	6.0	Edge	
G	TMWW-t	MT20	4.0	4.0		
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMWW+t	MT20	4.0	6.0		
J	TS-t	MT20	3.0	6.0		
K	TMWW-t	MT20	5.0	6.0	2.25	2.00
L	TMV+p	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	8.0	2.50	3.75
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	5.0	8.0	2.50	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2521	0	2521	0	0	5-8	5-8
N	2521	0	2521	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1905	972 / 0	410 / 0	0 / 0	0 / 0	523 / 0	0 / 0
N	1905	972 / 0	410 / 0	0 / 0	0 / 0	523 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, G-R, G-Q, I-Q, C-U, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 29	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 19	-84.9	-84.9	0.26 (1)	10.00
C-D	-3216 / 0	-84.9	-84.9	0.39 (1)	3.64
D-E	-3216 / 0	-84.9	-84.9	0.39 (1)	3.64
E-F	-2711 / 0	-84.9	-84.9	0.35 (1)	3.94
F-G	-2333 / 0	-84.9	-84.9	0.28 (1)	4.25
G-H	-2333 / 0	-84.9	-84.9	0.28 (1)	4.25
H-I	-2711 / 0	-84.9	-84.9	0.35 (1)	3.94
I-J	-3216 / 0	-84.9	-84.9	0.39 (1)	3.64
J-K	-3216 / 0	-84.9	-84.9	0.39 (1)	3.64
K-L	0 / 19	-84.9	-84.9	0.26 (1)	10.00
L-M	0 / 29	-84.9	-84.9	0.11 (1)	10.00
U-B	-290 / 0	0.0	0.0	0.02 (1)	7.81
N-L	-290 / 0	0.0	0.0	0.02 (1)	7.81
U-T	0 / 2763	-38.5	-38.5	0.47 (1)	10.00
T-S	0 / 2681	-38.5	-38.5	0.47 (2)	10.00
S-R	0 / 2681	-38.5	-38.5	0.47 (2)	10.00
R-Q	0 / 2440	-38.5	-38.5	0.44 (2)	10.00
Q-P	0 / 2681	-38.5	-38.5	0.47 (2)	10.00
P-O	0 / 2681	-38.5	-38.5	0.47 (2)	10.00
O-N	0 / 2763	-38.5	-38.5	0.47 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-T	0 / 147	0.03 (3)
T-E	0 / 330	0.07 (2)
E-R	-598 / 0	0.28 (1)
R-F	0 / 1036	0.23 (1)
R-G	-255 / 0	0.15 (1)
G-Q	-255 / 0	0.15 (1)
Q-H	0 / 1036	0.23 (1)
Q-I	-598 / 0	0.28 (1)
I-O	0 / 330	0.07 (2)
O-K	0 / 147	0.03 (3)
U-C	-3472 / 0	0.64 (1)
K-N	-3472 / 0	0.64 (1)

TOTAL WEIGHT = 6 X 209 = 1252 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.39/1.00 (I-K:1) , BC=0.47/1.00 (O-Q:2) , WB=0.64/1.00 (K-N:1) , SSI=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.90 (K) (INPUT = 0.90)
JS METAL= 0.86 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100099024
PROVINCE OF ONTARIO

DWG NO. TAM T1900066
STRUCTURAL
COMPONENT REVIEW

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 12:16:35 2019 Page 1
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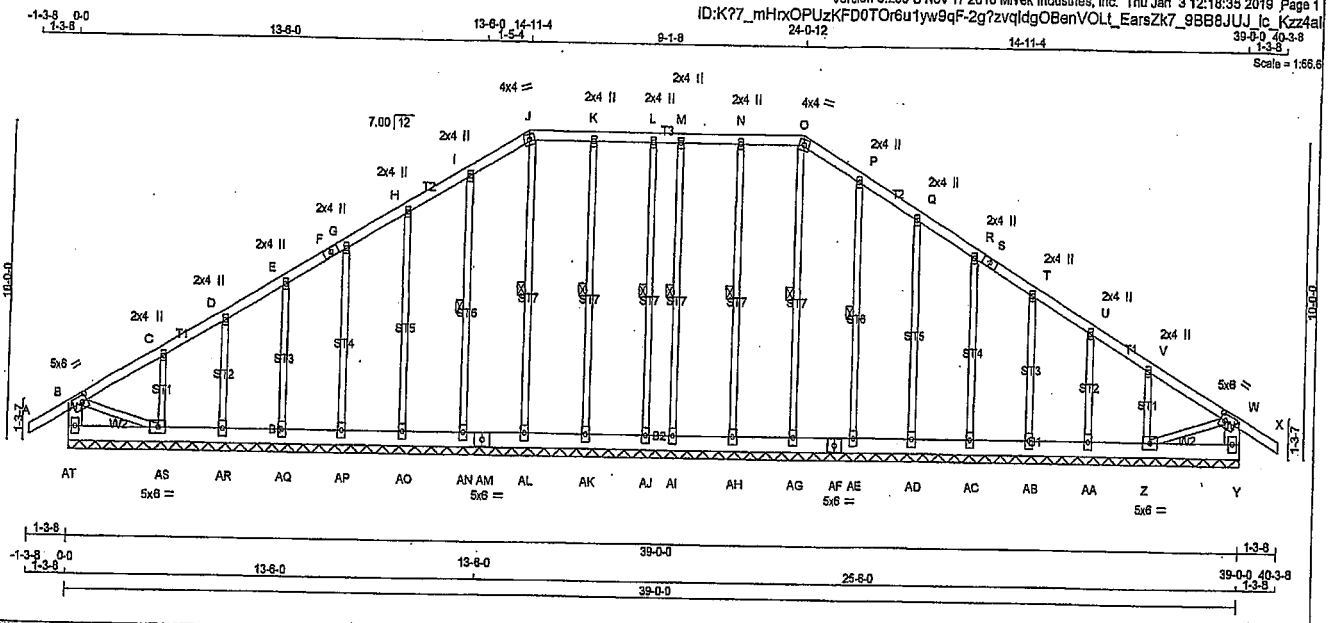


TOTAL WEIGHT = 2 X 227 = 454 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
O - S	2x4	DRY	No.2	SPF
S - X	2x4	DRY	No.2	SPF
AT- B	2x6	DRY	No.2	SPF
Y - W	2x6	DRY	No.2	SPF
AT- AM	2x6	DRY	No.2	SPF
AM- AF	2x6	DRY	No.2	SPF
AF- Y	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	2.00
C, D, E, G, H, I, K, L, M, N, P, Q, R, T, U, V						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
O	TTW-m	MT20	4.0	4.0		
S	TS-t	MT20	3.0	6.0		
W	TMWW-t	MT20	5.0	6.0	2.50	2.00
Y	BMV1+p	MT20	3.0	6.0		
Z	BMWW1-t	MT20	5.0	6.0		
AA, AB, AC, AD, AE, AG, AH, AI, AJ, AK, AL, AN, AO, AP, AQ, AR						
AA	BMW1+w	MT20	3.0	6.0		
AF	BS-t	MT20	5.0	6.0		
AM	BS-t	MT20	5.0	6.0		
AS	BMWW1-t	MT20	5.0	6.0		
AT	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-AJ, K-AK, J-AL, I-AN, N-AH, M-AI, O-AG, P-AE.

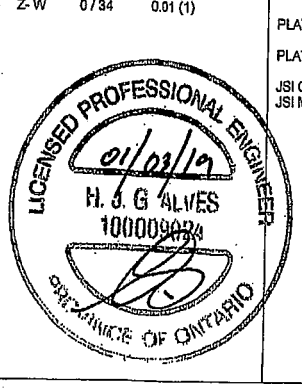
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 29	-84.9	-84.9	0.11 (1)	10.00
B- C	-26 / 0	-84.9	-84.9	0.09 (1)	6.25
C- D	-41 / 0	-84.9	-84.9	0.09 (1)	6.25
D- E	-23 / 0	-84.9	-84.9	0.04 (1)	6.25
E- F	-24 / 0	-84.9	-84.9	0.04 (1)	6.25
F- G	-24 / 0	-84.9	-84.9	0.04 (1)	6.25
G- H	-21 / 0	-84.9	-84.9	0.04 (1)	6.25
H- I	-18 / 0	-84.9	-84.9	0.04 (1)	6.25
I- J	-25 / 0	-84.9	-84.9	0.04 (1)	6.25
J- K	-13 / 0	-84.9	-84.9	0.05 (1)	6.25
K- L	-13 / 0	-84.9	-84.9	0.05 (1)	6.25
L- M	-13 / 0	-84.9	-84.9	0.03 (1)	6.25
M- N	-13 / 0	-84.9	-84.9	0.05 (1)	6.25
N- O	-13 / 0	-84.9	-84.9	0.05 (1)	6.25
O- P	-25 / 0	-84.9	-84.9	0.04 (1)	6.25
P- Q	-18 / 0	-84.9	-84.9	0.04 (1)	6.25
Q- R	-21 / 0	-84.9	-84.9	0.04 (1)	6.25
R- S	-24 / 0	-84.9	-84.9	0.04 (1)	6.25
S- T	-24 / 0	-84.9	-84.9	0.04 (1)	6.25
T- U	-23 / 0	-84.9	-84.9	0.04 (1)	6.25
U- V	-41 / 0	-84.9	-84.9	0.09 (1)	6.25
V- W	-26 / 0	-84.9	-84.9	0.09 (1)	6.25
W- X	0 / 29	-84.9	-84.9	0.11 (1)	10.00
AT- B	-247 / 0	0.0	0.0	0.02 (1)	7.81
Y- W	-247 / 0	0.0	0.0	0.02 (1)	7.81
AT-AS	0 / 0	-38.5	-38.5	0.03 (3)	10.00
AS-AR	0 / 25	-38.5	-38.5	0.03 (2)	10.00
AR-AQ	0 / 22	-38.5	-38.5	0.01 (2)	10.00
AQ-AP	0 / 20	-38.5	-38.5	0.01 (2)	10.00
AP-AO	0 / 18	-38.5	-38.5	0.01 (2)	10.00
AO-AN	0 / 16	-38.5	-38.5	0.01 (2)	10.00
AN-AM	0 / 15	-38.5	-38.5	0.01 (2)	10.00
AM-AL	0 / 15	-38.5	-38.5	0.01 (2)	10.00
AL-AK	0 / 13	-38.5	-38.5	0.01 (2)	10.00
AK-AJ	0 / 13	-38.5	-38.5	0.01 (2)	10.00
AJ-AI	0 / 13	-38.5	-38.5	0.01 (2)	10.00
AI-AH	0 / 13	-38.5	-38.5	0.01 (2)	10.00
AH-AG	0 / 13	-38.5	-38.5	0.01 (2)	10.00
AG-AF	0 / 15	-38.5	-38.5	0.01 (2)	10.00
AF-AE	0 / 15	-38.5	-38.5	0.01 (2)	10.00
AE-AD	0 / 16	-38.5	-38.5	0.01 (2)	10.00
AD-AC	0 / 18	-38.5	-38.5	0.01 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AJ- L	-117 / 0	0.08 (1)
AK- K	-198 / 0	0.13 (1)
AL- J	-131 / 0	0.09 (1)
AN- I	-166 / 0	0.09 (1)
AO- H	-166 / 0	0.17 (1)
AP- G	-166 / 0	0.11 (1)
AQ- E	-174 / 0	0.07 (1)
AR- D	-139 / 0	0.04 (1)
AS- C	-254 / 0	0.04 (1)
AH- N	-198 / 0	0.13 (1)
AI- M	-117 / 0	0.08 (1)
AG- O	-131 / 0	0.09 (1)
AE- P	-166 / 0	0.09 (1)
AD- Q	-166 / 0	0.17 (1)
AC- R	-166 / 0	0.11 (1)
AB- T	-174 / 0	0.07 (1)
AA- U	-139 / 0	0.04 (1)
Z- V	-254 / 0	0.04 (1)
B-AS	0 / 34	0.01 (1)
Z- W	0 / 34	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (Z-AA:2) , WB=0.17/1.00 (Q-AD:1) , SSI=0.10/1.00 (V-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1667	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

DWG NO. TAM T1900060
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AC-AB	0 / 20	-38.5	-38.5	0.01 (2)	10.00
AB-AA	0 / 22	-38.5	-38.5	0.01 (2)	10.00
AA-Z	0 / 25	-38.5	-38.5	0.03 (2)	10.00
Z-Y	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM T1900060
STRUCTURAL
CONFIDENTIALITY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
259672	T14	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Thu Jan 3 13:41:07 2019 Page 1
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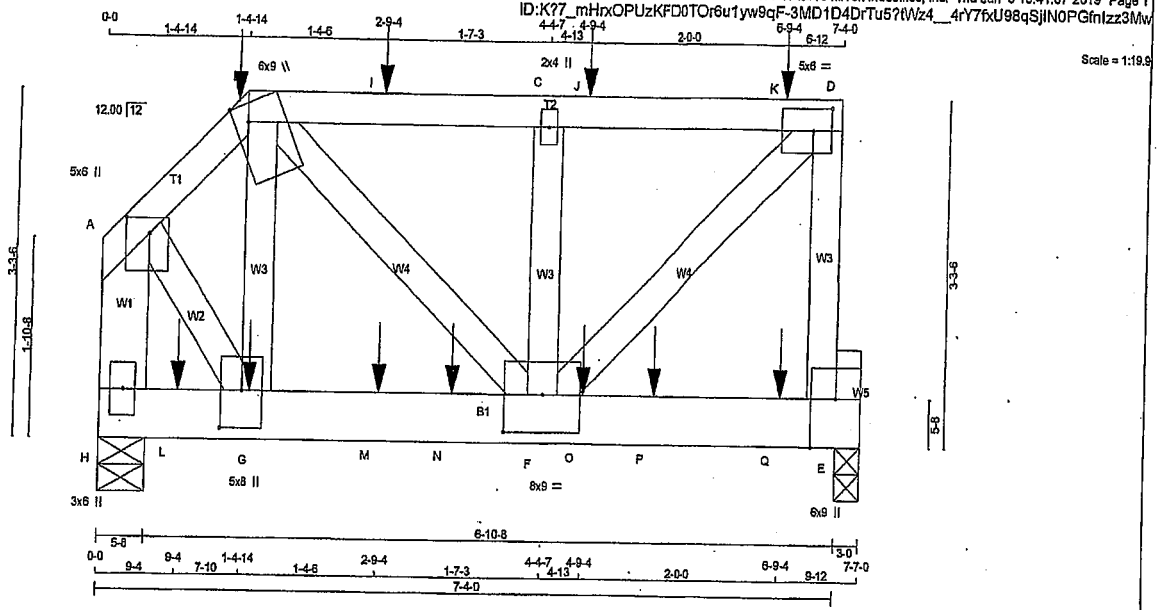

TOTAL WEIGHT = 2 X 45 = 90 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- D	1	12	SIDE(61.0)
D- E	1	12	TOP
H- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
G- B	1	3	SIDE(464.6)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TTWW+m	MT20	6.0	9.0	Edge	1.75
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	5.0	6.0	2.50	2.25
E	BMVW1+t	MT20	6.0	9.0	Edge	3.00
F	BMWWW-t	MT20	8.0	9.0	4.25	4.50
G	BMWW+t	MT20	5.0	6.0	4.25	2.50
H	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	4218	0	4218	0	0	3-0	3-0
H	4505	0	4505	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	3180	1642 / 0	669 / 0	0 / 0	0 / 0	869 / 0	0 / 0
H	3397	1751 / 0	717 / 0	0 / 0	0 / 0	929 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3307 / 0	-84.9	-84.9	0.05 (1)	5.10
B- I	-3440 / 0	-84.9	-84.9	0.12 (1)	4.94
I- C	-3440 / 0	-84.9	-84.9	0.12 (1)	4.94
C- J	-3440 / 0	-84.9	-84.9	0.12 (1)	4.95
J- K	-3440 / 0	-84.9	-84.9	0.12 (1)	4.95
K- D	-3440 / 0	-84.9	-84.9	0.12 (1)	4.95
E- D	-3549 / 0	0.0	0.0	0.30 (1)	6.23
H- A	-4570 / 0	0.0	0.0	0.18 (1)	6.74
H- L	0 / 0	-38.5	-38.5	0.06 (1)	10.00
L- G	0 / 0	-38.5	-38.5	0.06 (1)	10.00
G- M	0 / 2366	-38.5	-38.5	0.54 (1)	10.00
M- N	0 / 2366	-38.5	-38.5	0.54 (1)	10.00
N- F	0 / 2366	-38.5	-38.5	0.54 (1)	10.00
F- O	0 / 0	-38.5	-38.5	0.42 (1)	10.00
O- P	0 / 0	-38.5	-38.5	0.42 (1)	10.00
P- Q	0 / 0	-38.5	-38.5	0.42 (1)	10.00
Q- E	0 / 0	-38.5	-38.5	0.42 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	0 / 1040	0.09 (1)
B- F	0 / 1456	0.43 (1)
F- C	-3507 / 0	0.02 (1)
F- D	0 / 1456	0.43 (1)
A- G	0 / 3157	0.28 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HGR	CONN.
B	1-4-14	-9	-10	—	FRONT	VERT	DEAD	—	—
B	1-4-14	-41	-41	—	FRONT	VERT	SNOW	—	—
G	1-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
I	2-9-4	-60	-60	—	FRONT	VERT	TOTAL	—	—
J	4-9-4	-60	-60	—	FRONT	VERT	TOTAL	—	—
K	6-9-4	-79	-79	—	FRONT	VERT	TOTAL	—	—
L	9-4	-32	-40	—	FRONT	VERT	TOTAL	—	—
M	2-9-4	-27	-35	—	FRONT	VERT	TOTAL	—	—
N	3-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
O	4-9-4	-27	-35	—	FRONT	VERT	TOTAL	—	—
P	5-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
Q	6-9-4	-36	-45	—	FRONT	VERT	TOTAL	—	—

DWG NO. TAM T1900089
STRUCTURAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.30/1.00 (D-E:1) , BC=0.54/1.00 (F-G:1) , WB=0.43/1.00 (D-F:1) , SSI=0.69/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.88 (D) (INPUT= 0.90)
JSI METAL= 0.47 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100094824
01/03/19
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T19	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:57 2019 Page 1

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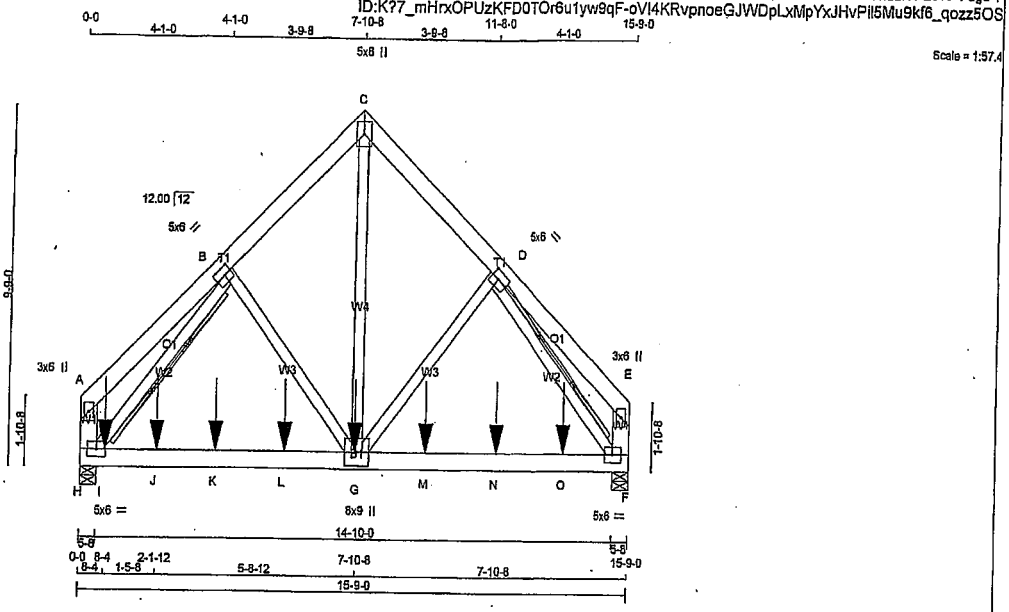


TOTAL WEIGHT = 2 X 111 = 223 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
H - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
H- A	2	3	SIDE(73.3)
F- E	2	3	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- F	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
G- C	1	6	SIDE(71.6)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWW-t	MT20	5.0	6.0	2.50	2.50
C	TTW+p	MT20	5.0	8.0		
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMV+p	MT20	3.0	6.0		
F	BMVW1-t	MT20	5.0	6.0		
G	BMWWW+t	MT20	8.0	9.0		
H	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	5584	0	5584	0	0	5-8	5-8
F	4630	0	4630	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	4211	2171 / 0	868 / 0	0 / 0	0 / 0	1152 / 0	0 / 0
F	3492	1799 / 0	737 / 0	0 / 0	0 / 0	955 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT B-H, D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-10 / 0	-84.9	-84.9	0.06 (1)	6.25
B- C	-3779 / 0	-84.9	-84.9	0.09 (1)	5.73
C- D	-3779 / 0	-84.9	-84.9	0.09 (1)	5.73
D- E	-10 / 0	-84.9	-84.9	0.06 (1)	6.25
H- A	-166 / 0	0.0	0.0	0.01 (1)	7.81
F- E	-166 / 0	0.0	0.0	0.01 (1)	7.81
H- I	0 / 2451	-38.5	-38.5	0.89 (1)	10.00
I- J	0 / 2451	-38.5	-38.5	0.89 (1)	10.00
J- K	0 / 2451	-38.5	-38.5	0.89 (1)	10.00
K- L	0 / 2451	-38.5	-38.5	0.89 (1)	10.00
L- G	0 / 2451	-38.5	-38.5	0.89 (1)	10.00
G- M	0 / 2451	-38.5	-38.5	0.89 (1)	10.00
M- N	0 / 2451	-38.5	-38.5	0.89 (1)	10.00
N- O	0 / 2451	-38.5	-38.5	0.89 (1)	10.00
O- F	0 / 2451	-38.5	-38.5	0.89 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 5002	0.44 (1)
G- D	0 / 365	0.03 (2)
B- G	97 / 365	0.03 (2)
H- B	-4103 / 0	0.63 (1)
D- F	-4103 / 0	0.63 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEI GHT	CONN
G	7-9-12	-1033	-1033	—	BACK	VERT	TOTAL	—	—
I	8-4	-1039	-1039	—	BACK	VERT	TOTAL	—	—
J	2-1-12	-1033	-1033	—	BACK	VERT	TOTAL	—	—
K	3-9-12	-1033	-1033	—	BACK	VERT	TOTAL	—	—
L	5-9-12	-1033	-1033	—	BACK	VERT	TOTAL	—	—
M	9-9-12	-1033	-1033	—	TOP	VERT	TOTAL	—	—
N	11-9-12	-1033	-1033	—	TOP	VERT	TOTAL	—	—
O	13-9-12	-1033	-1033	—	TOP	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 630 (0.30")

CSI: TC=0.09/1.00 (C-D:1) , BC=0.89/1.00 (G-H:1) , WB=0.63/1.00 (B-H:1) , SSI=0.77/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

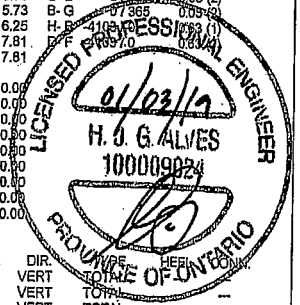
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.50 (D) (INPUT = 1.00)



DWG NO. TAM T19000053
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Thu Jan 3 12:18:24 2019 Page 1
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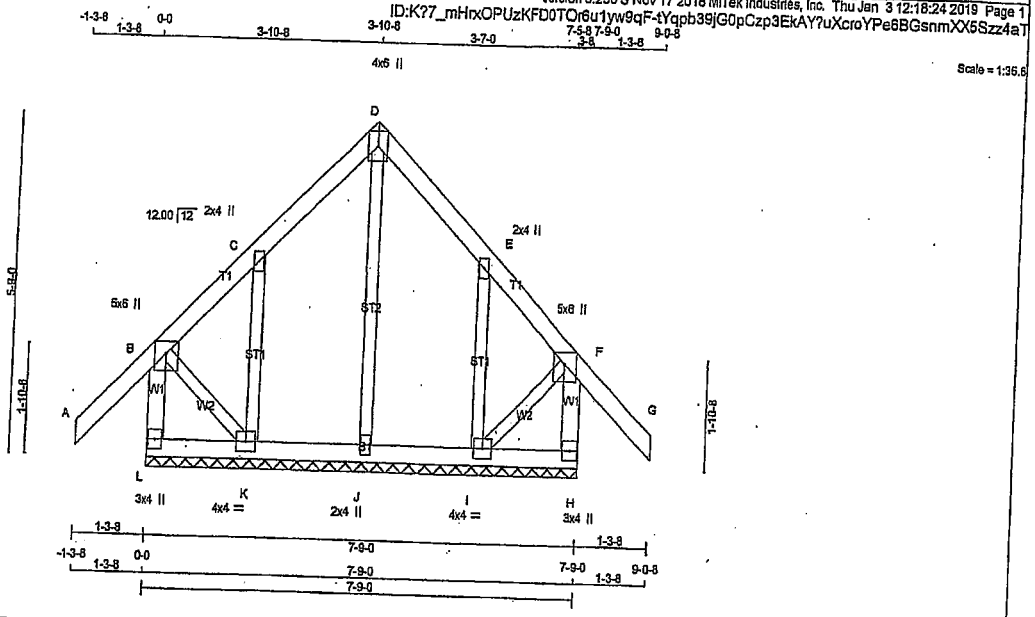

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
L-B	-253 / 0	0.0	0.0	0.03 (1)	7.81	
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	
B-C	-44 / 0	-84.9	-84.9	0.11 (1)	6.25	
C-D	-21 / 0	-84.9	-84.9	0.05 (1)	6.25	
D-E	-21 / 0	-84.9	-84.9	0.05 (1)	6.25	
E-F	-44 / 0	-84.9	-84.9	0.11 (1)	6.25	
F-G	0 / 42	-84.9	-84.9	0.12 (1)	10.00	
H-F	-253 / 0	0.0	0.0	0.03 (1)	7.81	
L-K	0 / 0	-38.5	-38.5	0.03 (3)	10.00	
K-J	0 / 10	-38.5	-38.5	0.03 (3)	10.00	
J-I	0 / 10	-38.5	-38.5	0.03 (3)	10.00	
I-H	0 / 0	-38.5	-38.5	0.03 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-D	-138 / 0	0.07 (1)
K-C	-137 / 0	0.03 (1)
I-E	-137 / 0	0.03 (1)
B-K	0 / 18	0.00 (1)
I-F	0 / 18	0.00 (1)

TOTAL WEIGHT = 42 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (F-G:1) , BC=0.03/1.00 (J-K:3) , WB=0.07/1.00 (D-J:1) , SSI=0.07/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1666

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/02/19
H. J G ALVES
100009602
PROVINCE OF ONTARIO

DWG NO. TAM 774000 61
STRUCTURAL
CONFIGURATION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	G12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:24:38 2019 Page 1

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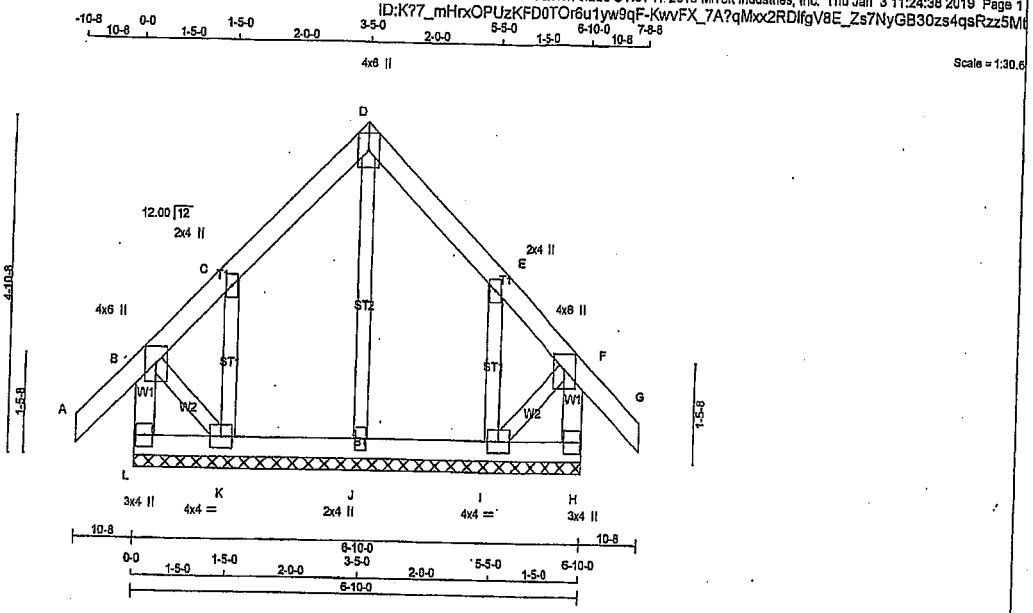

TOTAL WEIGHT = 34 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 29	-84.9	-84.9	0.08 (1)		10.00
B-C	-40 / 0	-84.9	-84.9	0.06 (1)		6.25
C-D	-33 / 0	-84.9	-84.9	0.05 (1)		6.25
D-E	-33 / 0	-84.9	-84.9	0.05 (1)		6.25
E-F	-40 / 0	-84.9	-84.9	0.06 (1)		6.25
F-G	0 / 29	-84.9	-84.9	0.06 (1)		10.00
L-B	-190 / 0	0.0	0.0	0.02 (1)		7.81
H-F	-190 / 0	0.0	0.0	0.02 (1)		7.81
L-K	0 / 0	-38.5	-38.5	0.02 (3)		10.00
K-J	0 / 17	-38.5	-38.5	0.03 (3)		10.00
J-I	0 / 17	-38.5	-38.5	0.03 (3)		10.00
I-H	0 / 0	-38.5	-38.5	0.02 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-D	-118 / 0	0.04 (1)
K-C	-142 / 0	0.02 (1)
I-E	-142 / 0	0.02 (1)
B-K	0 / 29	0.01 (1)
I-F	0 / 29	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B:1) , BC=0.03/1.00 (I-J:3) , WB=0.04/1.00 (D-J:1) , SSI=0.06/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (F) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1900035
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	G20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:24:47 2019 Page 1
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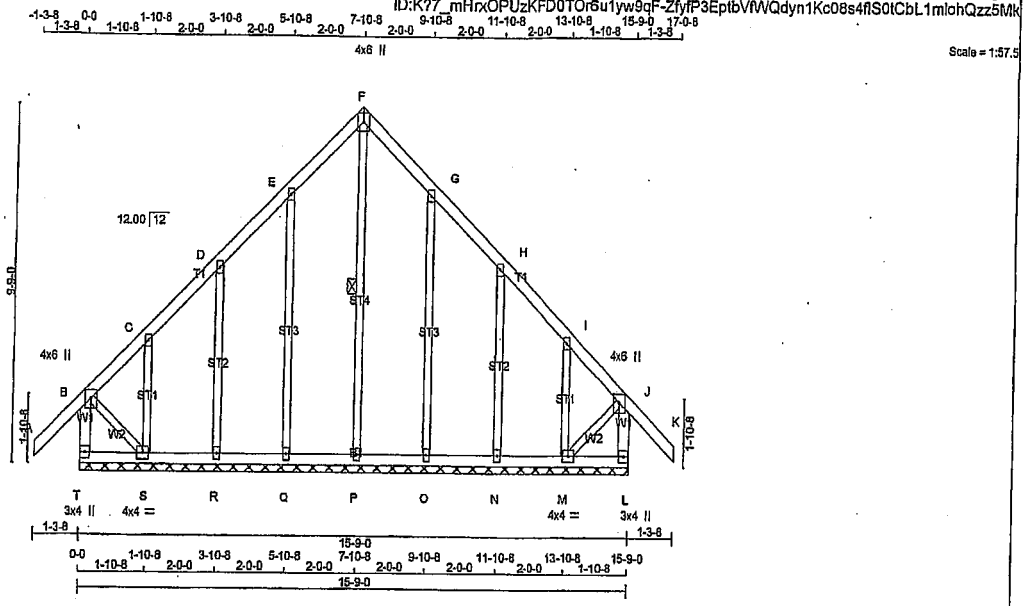


TOTAL WEIGHT = 87 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0		
J	TMVW+p	MT20	4.0	6.0	2.75	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

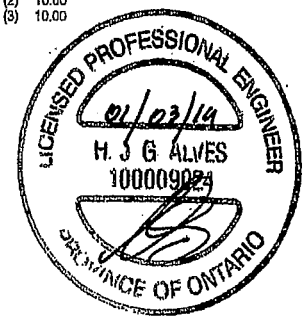
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)			FR-TO			
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	P-F	-116 / 0	0.07 (1)
B-C	-62 / 0	-84.9	-84.9	0.11 (1)	6.25	Q-E	-191 / 0	0.21 (1)
C-D	-22 / 0	-84.9	-84.9	0.04 (1)	6.25	R-D	-173 / 0	0.09 (1)
D-E	-22 / 0	-84.9	-84.9	0.05 (1)	6.25	S-C	-108 / 0	0.02 (1)
E-F	-32 / 0	-84.9	-84.9	0.05 (1)	6.25	O-G	-191 / 0	0.21 (1)
F-G	-32 / 0	-84.9	-84.9	0.05 (1)	6.25	N-H	-173 / 0	0.09 (1)
G-H	-22 / 0	-84.9	-84.9	0.05 (1)	6.25	M-I	-108 / 0	0.02 (1)
H-I	-22 / 0	-84.9	-84.9	0.04 (1)	6.25	B-S	0 / 31	0.01 (1)
I-J	-62 / 0	-84.9	-84.9	0.11 (1)	6.25	M-J	0 / 31	0.01 (1)
J-K	0 / 42	-84.9	-84.9	0.12 (1)	10.00			
T-B	-276 / 0	0.0	0.0	0.03 (1)	7.81			
L-J	-276 / 0	0.0	0.0	0.03 (1)	7.81			
T-S	0 / 0	-38.5	-38.5	0.03 (3)	10.00			
S-R	0 / 20	-38.5	-38.5	0.03 (2)	10.00			
R-Q	0 / 17	-38.5	-38.5	0.02 (2)	10.00			
Q-P	0 / 14	-38.5	-38.5	0.02 (3)	10.00			
P-O	0 / 14	-38.5	-38.5	0.02 (3)	10.00			
O-N	0 / 17	-38.5	-38.5	0.02 (2)	10.00			
N-M	0 / 20	-38.5	-38.5	0.03 (2)	10.00			
M-L	0 / 0	-38.5	-38.5	0.03 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (J-K:1) , BC=0.03/1.00 (M-N:2) , WB=0.21/1.00 (G-O:1) , SSI=0.07/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.10 (G) (INPUT = 1.00)

DWG NO. TAM T1900036
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	G21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:41 2019 Page 1
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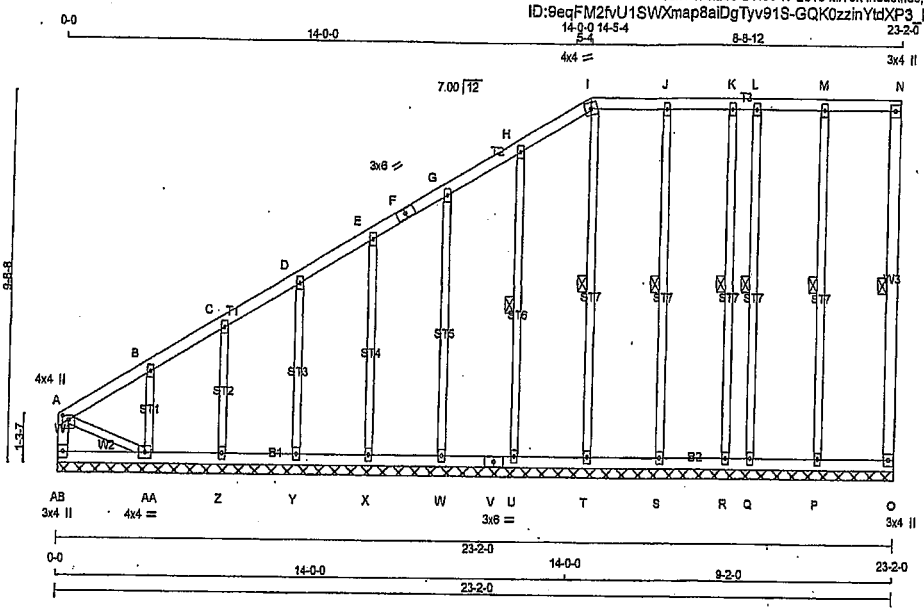

TOTAL WEIGHT = 131 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - N	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
AB- A	2x4	DRY	No.2	SPF
AB- V	2x4	DRY	No.2	SPF
V - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B, C, D, E, G, H, J, K, L, M						
B	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
I	TTW-m	MT20	4.0	4.0		
N	TMV+p	MT20	3.0	4.0		
O	BMV1+p	MT20	3.0	4.0		
P, Q, R, S, T, U, W, X, Y, Z						
P	BMW1+w	MT20	2.0	4.0		
V	BS-t	MT20	3.0	6.0		
AA	BMWW1-t	MT20	4.0	4.0		
AB	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF N-O, H-U, M-P, I-T, J-S, K-R, L-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-13 / 0	-84.9	-84.9	0.07 (1)		6.25
B-C	-21 / 0	-84.9	-84.9	0.07 (1)		6.25
C-D	-10 / 0	-84.9	-84.9	0.04 (1)		10.00
D-E	-8 / 0	-84.9	-84.9	0.04 (1)		10.00
E-F	-6 / 0	-84.9	-84.9	0.04 (1)		10.00
F-G	-6 / 0	-84.9	-84.9	0.04 (1)		10.00
G-H	-2 / 0	-84.9	-84.9	0.04 (1)		10.00
H-I	-10 / 0	-84.9	-84.9	0.04 (1)		6.25
I-J	0 / 0	-84.9	-84.9	0.04 (1)		10.00
J-K	0 / 0	-84.9	-84.9	0.04 (1)		10.00
K-L	0 / 0	-84.9	-84.9	0.03 (1)		10.00
L-M	0 / 0	-84.9	-84.9	0.04 (1)		10.00
M-N	0 / 0	-84.9	-84.9	0.05 (1)		10.00
O-N	-76 / 0	0.0	0.0	0.03 (1)		6.25
AB-A	-104 / 0	0.0	0.0	0.01 (1)		7.81
AB-AA	0 / 0	-38.5	-38.5	0.04 (3)		10.00
AA-Z	0 / 13	-38.5	-38.5	0.04 (3)		10.00
Z-Y	0 / 10	-38.5	-38.5	0.03 (3)		10.00
Y-X	0 / 7	-38.5	-38.5	0.03 (3)		10.00
X-W	0 / 5	-38.5	-38.5	0.02 (3)		10.00
W-V	0 / 3	-38.5	-38.5	0.02 (3)		10.00
V-U	0 / 3	-38.5	-38.5	0.02 (3)		10.00
U-T	0 / 2	-38.5	-38.5	0.02 (3)		10.00
T-S	0 / 0	-38.5	-38.5	0.02 (3)		10.00
S-R	0 / 0	-38.5	-38.5	0.02 (3)		10.00
R-Q	0 / 0	-38.5	-38.5	0.01 (3)		10.00
Q-P	0 / 0	-38.5	-38.5	0.03 (3)		10.00
P-O	0 / 0	-38.5	-38.5	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA-B	-218 / 0	0.04 (1)
Z-C	-153 / 0	0.04 (1)
Y-D	-171 / 0	0.07 (1)
X-E	-168 / 0	0.11 (1)
W-G	-166 / 0	0.17 (1)
U-H	-187 / 0	0.09 (1)
P-M	-192 / 0	0.12 (1)
T-I	-140 / 0	0.09 (1)
S-J	-192 / 0	0.12 (1)
R-K	-103 / 0	0.06 (1)
Q-L	-104 / 0	0.07 (1)
A-AA	0 / 21	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (B-C:1) , BC=0.04/1.00 (Z-AA:3) , WB=0.17/1.00 (G-W:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (AA) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H J G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1900037
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T22A	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:57 2019 Page 1
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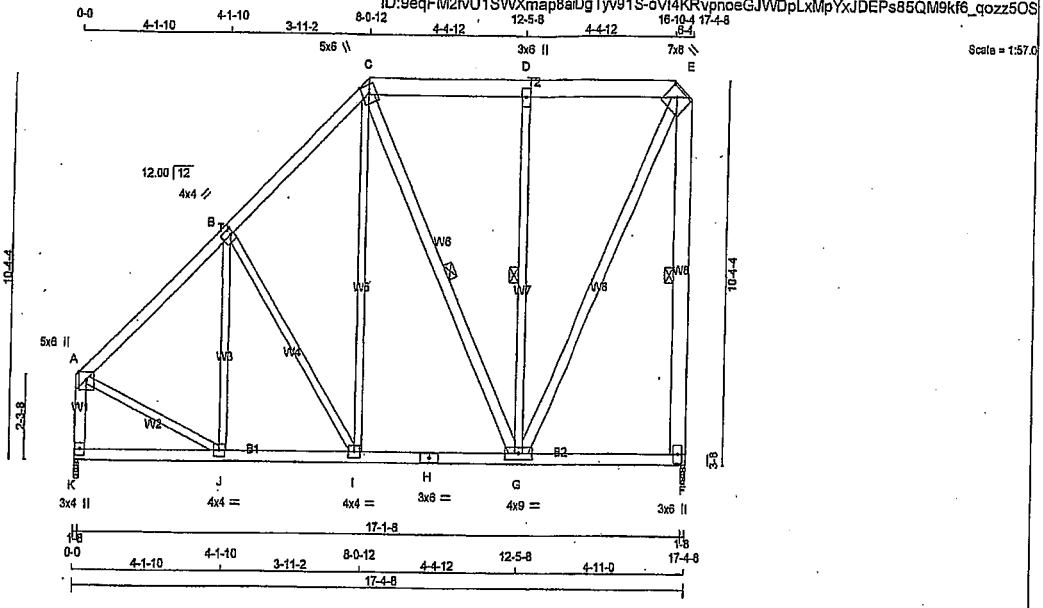


TOTAL WEIGHT = 5 X 123 = 613 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.00
C	TTWW+m	MT20	5.0	6.0	Edge	
D	TMW+w	MT20	3.0	6.0		
E	TMWWm	MT20	7.0	8.0	2.75	3.50
F	BMV1+p	MT20	3.0	6.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1072	0	1072	0	0	1-8	1-8
F	1072	0	1072	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	813	405 / 0	182 / 0	0 / 0	0 / 0	226 / 0	0 / 0
F	813	405 / 0	182 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-G, D-G, E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

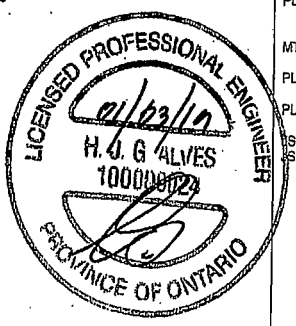
LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-810 / 0	-84.9	-84.9	0.25 (1)	6.25
B-C	-694 / 0	-84.9	-84.9	0.24 (1)	6.25
C-D	-405 / 0	-84.9	-84.9	0.13 (1)	6.25
D-E	-405 / 0	-84.9	-84.9	0.13 (1)	6.25
K-A	-1008 / 0	0.0	0.0	0.13 (1)	7.80
F-E	-996 / 0	0.0	0.0	0.32 (1)	6.25
K-J	0 / 0	-38.5	-38.5	0.11 (3)	10.00
J-I	0 / 593	-38.5	-38.5	0.19 (2)	10.00
I-H	0 / 467	-38.5	-38.5	0.23 (2)	10.00
H-G	0 / 467	-38.5	-38.5	0.23 (2)	10.00
G-F	0 / 0	-38.5	-38.5	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-B	-106 / 77	0.07 (1)
B-I	-229 / 0	0.23 (1)
I-C	0 / 364	0.08 (2)
C-G	-152 / 0	0.10 (1)
G-D	-477 / 0	0.34 (1)
A-J	0 / 655	0.15 (1)
G-E	0 / 921	0.15 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.32/1.00 (E-F:1) , BC=0.23/1.00 (G-I:2) , WB=0.34/1.00 (D-G:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.79 (G) (INPUT = 0.90)
SI METAL= 0.28 (A) (INPUT = 1.00)

DWG NO. TAM T1900054
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	T23A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 12:17:49 2019 Page 1

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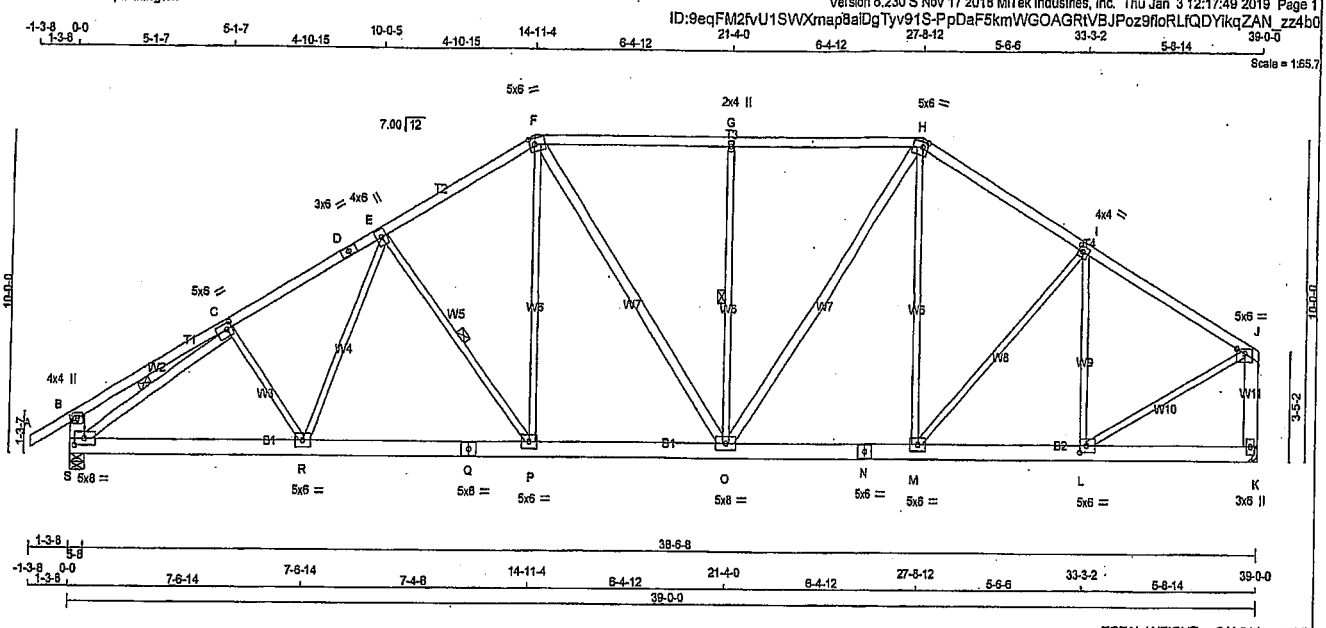


TOTAL WEIGHT = 2 X 214 = 428 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - O	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M, P, R						
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
S	BMVW1-t	MT20	5.0	8.0	2.50	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	2521	0	2521	0	0	5-8	5-8
K	2406	0	2406	0	0	MECHANICAL	IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1905	972 / 0	410 / 0	0 / 0	0 / 0	523 / 0	0 / 0
K	1825	908 / 0	410 / 0	0 / 0	0 / 0	507 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

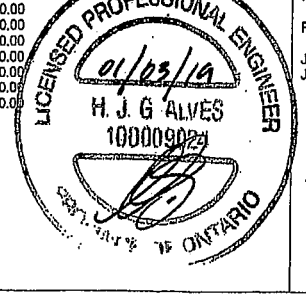
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, G-O, C-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 29	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 19	-84.9	-84.9	0.26 (1)	10.00
C-D	-3220 / 0	-84.9	-84.9	0.39 (1)	3.63
D-E	-3220 / 0	-84.9	-84.9	0.39 (1)	3.63
E-F	-2702 / 0	-84.9	-84.9	0.35 (1)	3.94
F-G	-2470 / 0	-84.9	-84.9	0.56 (1)	3.81
G-H	-2470 / 0	-84.9	-84.9	0.56 (1)	3.81
H-I	-2366 / 0	-84.9	-84.9	0.45 (1)	4.05
I-J	-2165 / 0	-84.9	-84.9	0.44 (1)	4.20
S-B	-290 / 0	0.0	0.0	0.02 (1)	7.81
K-J	-2310 / 0	0.0	0.0	0.28 (1)	6.75
S-R	0 / 2766	-38.5	-38.5	0.46 (1)	10.00
R-Q	0 / 2679	-38.5	-38.5	0.44 (2)	10.00
Q-P	0 / 2679	-38.5	-38.5	0.44 (2)	10.00
P-O	0 / 2322	-38.5	-38.5	0.37 (1)	10.00
O-N	0 / 2018	-38.5	-38.5	0.31 (2)	10.00
N-M	0 / 2018	-38.5	-38.5	0.31 (2)	10.00
M-L	0 / 1912	-38.5	-38.5	0.30 (1)	10.00
L-K	0 / 0	-38.5	-38.5	0.10 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-R	0 / 148	0.04 (3)
R-E	0 / 351	0.08 (2)
E-P	-609 / 0	0.29 (1)
P-F	0 / 755	0.17 (1)
F-O	0 / 266	0.04 (1)
O-G	-665 / 0	0.43 (1)
O-H	0 / 816	0.13 (1)
M-H	0 / 213	0.05 (3)
M-I	0 / 158	0.04 (1)
L-I	-777 / 0	0.56 (1)
S-C	-3478 / 0	0.65 (1)
L-J	-97 / 161	0.43 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.58/1.00 (F-G:1) , BC=0.46/1.00 (R-S:1) , WB=0.65/1.00 (C-S:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.86 (C) (INPUT = 1.00)

DWG NO. TAM T1900867
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	T24A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 12:17:50 2019 Page 1
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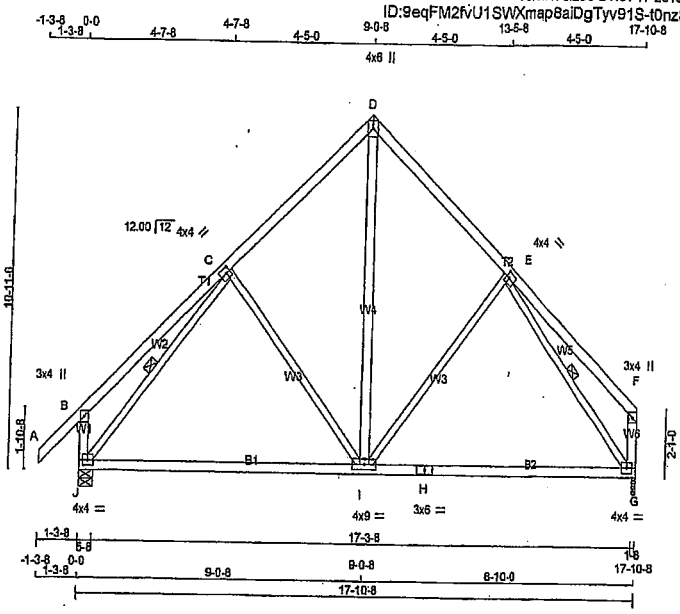


TOTAL WEIGHT = 2 X 90 = 181 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1221	0	1221	0	0	5-8	5-8
G	1103	0	1103	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	918	481 / 0	188 / 0	0 / 0	0 / 0	249 / 0	0 / 0
G	836	416 / 0	188 / 0	0 / 0	0 / 0	232 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

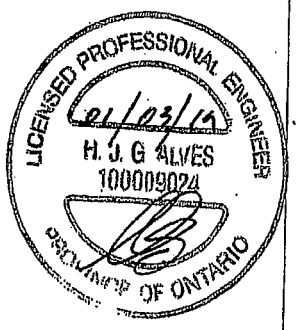
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-J, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	
B- C	0 / 32	-84.9	-84.9	0.28 (1)	10.00	
C- D	-742 / 0	-84.9	-84.9	0.23 (1)	6.25	
D- E	-740 / 0	-84.9	-84.9	0.22 (1)	6.25	
E- F	0 / 32	-84.9	-84.9	0.27 (1)	10.00	
J- B	-262 / 0	0.0	0.0	0.03 (1)	7.81	
G- F	-135 / 0	0.0	0.0	0.02 (1)	7.81	
J- I	0 / 616	-38.5	-38.5	0.74 (3)	10.00	
I- H	0 / 598	-38.5	-38.5	0.74 (3)	10.00	
H- G	0 / 598	-38.5	-38.5	0.74 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- I	-189 / 10	0.21 (1)
I- D	0 / 709	0.11 (2)
I- E	-158 / 22	0.18 (1)
J- C	-1031 / 0	0.39 (1)
E- G	-1031 / 0	0.37 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 662 (0.32")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.74/1.00 (I-J:3) , WB=0.39/1.00 (C-J:1) , SSI=0.23/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

ENG NO. IAM T1900063
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	T25AX	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 12:17:51 2019 Page 1
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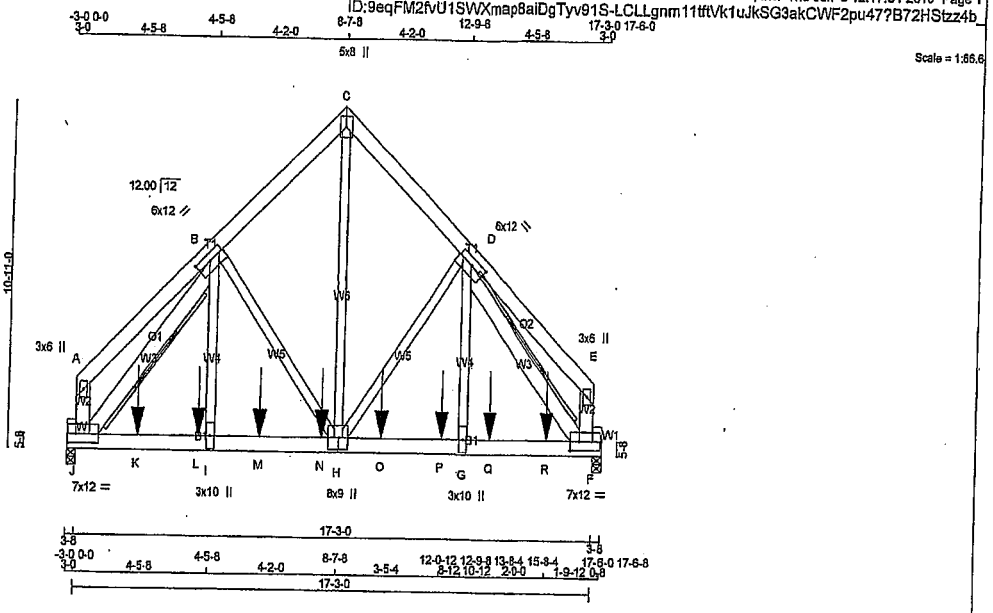

TOTAL WEIGHT = 3 X 149 = 446 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
H - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
J- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- H	2	6	SIDE(647.7)
H- F	2	6	SIDE(1235.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWWW-t	MT20	6.0	12.0	2.50	5.75
C	TTW+p	MT20	5.0	8.0		
D	TMWWW-t	MT20	6.0	12.0	2.50	5.75
E	TMV+p	MT20	3.0	6.0		
F	BMVWW1-t	MT20	7.0	12.0	3.50	3.50
G	BMW+w	MT20	3.0	10.0		
H	BSWWW+l	MT20	8.0	9.0		
I	BMW+w	MT20	3.0	10.0		
J	BMVWW1-t	MT20	7.0	12.0	3.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	10541	0	10541	0	0	3-8	3-8 & 2SBP
F	11219	0	11219	0	0	3-8	3-8 & 2SBP

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	7947	4102 / 0	1673 / 0	0 / 0	0 / 0	2172 / 0	0 / 0
F	8465	4349 / 0	1797 / 0	0 / 0	0 / 0	2319 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F
INSTALL TWO USP SBP4 BEARING PLATES AT JOINT(S) J, F OR SIMILAR

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT B-J, D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-29 / 0	-84.9	-84.9	0.08 (1)	6.25	
B- C	-6925 / 0	-84.9	-84.9	0.09 (1)	5.33	
C- D	-6923 / 0	-84.9	-84.9	0.09 (1)	5.33	
D- E	-30 / 0	-84.9	-84.9	0.07 (1)	6.25	
J- A	-222 / 0	0.0	0.0	0.01 (1)	7.81	
F- E	-224 / 0	0.0	0.0	0.01 (1)	7.81	
J- K	0 / 6569	-38.5	-38.5	0.38 (1)	10.00	
K- L	0 / 6569	-38.5	-38.5	0.38 (1)	10.00	
L- I	0 / 6569	-38.5	-38.5	0.38 (1)	10.00	
I- M	0 / 6686	-38.5	-38.5	0.39 (1)	10.00	
M- N	0 / 6686	-38.5	-38.5	0.39 (1)	10.00	
N- H	0 / 6686	-38.5	-38.5	0.39 (1)	10.00	
H- O	0 / 6836	-38.5	-38.5	0.42 (1)	10.00	
O- P	0 / 6836	-38.5	-38.5	0.42 (1)	10.00	
P- G	0 / 6836	-38.5	-38.5	0.42 (1)	10.00	
G- Q	0 / 6712	-38.5	-38.5	0.42 (1)	10.00	
Q- R	0 / 6712	-38.5	-38.5	0.42 (1)	10.00	
R- F	0 / 6712	-38.5	-38.5	0.42 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	0 / 6511	0.35 (1)
B- H	-3080 / 0	0.77 (1)
H- C	0 / 9390	0.50 (1)
H- D	-3341 / 0	0.83 (1)
G- D	-6927 / 0	0.84 (1)
J- B	-1300 / 0	0.56 (1)
D- G	-1300 / 0	0.56 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	2-0-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
L	4-0-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
M	6-0-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
N	8-0-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
O	10-0-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
P	12-0-12	-2367	-2367	—	BACK	VERT	TOTAL	—	—
Q	13-8-4	-2367	-2367	—	BACK	VERT	TOTAL	—	—
R	15-8-4	-2483	-2483	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.09/1.00 (C-D:1) , BC=0.42/1.00 (G-H:1) , WB=0.83/1.00 (D-H:1) , SSI=0.81/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.65 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G ALVES
100099877
PROVINCE OF ONTARIO

BWB NO. TAM T1900069
SEQUENCE:

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402477	T90	9	1	ROYAL PINE HOMES-BRAMPTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Apr 29 15:03:40 2019 Page 1
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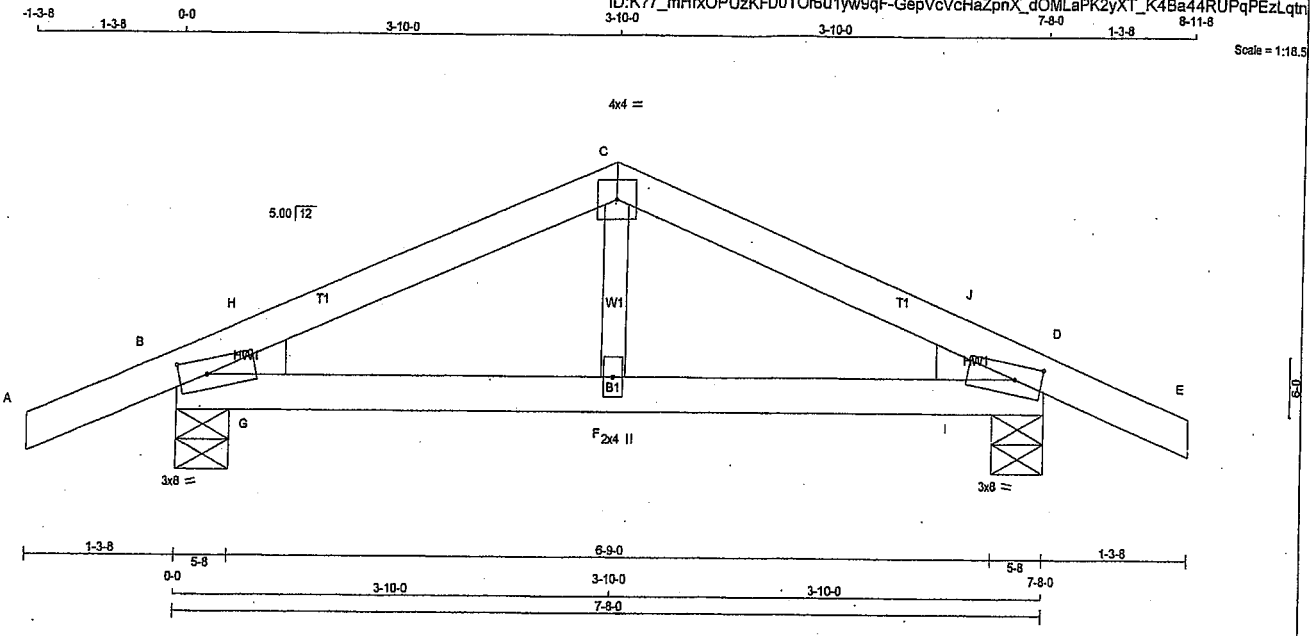

TOTAL WEIGHT = 9 X 24 = 220 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0	1.50	2.75
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-m	MT20	3.0	8.0	1.50	2.75
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	587	0	587	0	0	5-8	5-8	2x4 L
D	587	0	587	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	438	241 / 0	81 / 0	0 / 0	0 / 0	116 / 0	0 / 0
D	438	241 / 0	81 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.11 (1)	10.00	
B- H	-649 / 0	-84.9	-84.9	0.03 (1)	6.25	
H- C	-598 / 0	-84.9	-84.9	0.12 (1)	6.25	
C- J	-598 / 0	-84.9	-84.9	0.12 (1)	6.25	
J- D	-649 / 0	-84.9	-84.9	0.03 (1)	6.25	
D- E	0 / 15	-84.9	-84.9	0.11 (1)	10.00	
B- G	0 / 549	-38.5	-38.5	0.17 (1)	10.00	
G- F	0 / 549	-38.5	-38.5	0.20 (1)	10.00	
F- I	0 / 549	-38.5	-38.5	0.20 (1)	10.00	
I- D	0 / 549	-38.5	-38.5	0.17 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 229	0.05 (2)
G- H	-53 / 70	0.00 (1)
I- J	-53 / 70	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.12/1.00 (C-H:1) , BC=0.20/1.00 (F-I:1) , WB=0.05/1.00 (C-F:2) , SSI=0.10/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100089024
PROVINCE OF ONTARIO
04/29/19

DWG NO. TAM 19009772
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402477	G90	3	1	ROYAL PINE HOMES-BRAMPTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Apr 29 15:03:39 2019 Page 1
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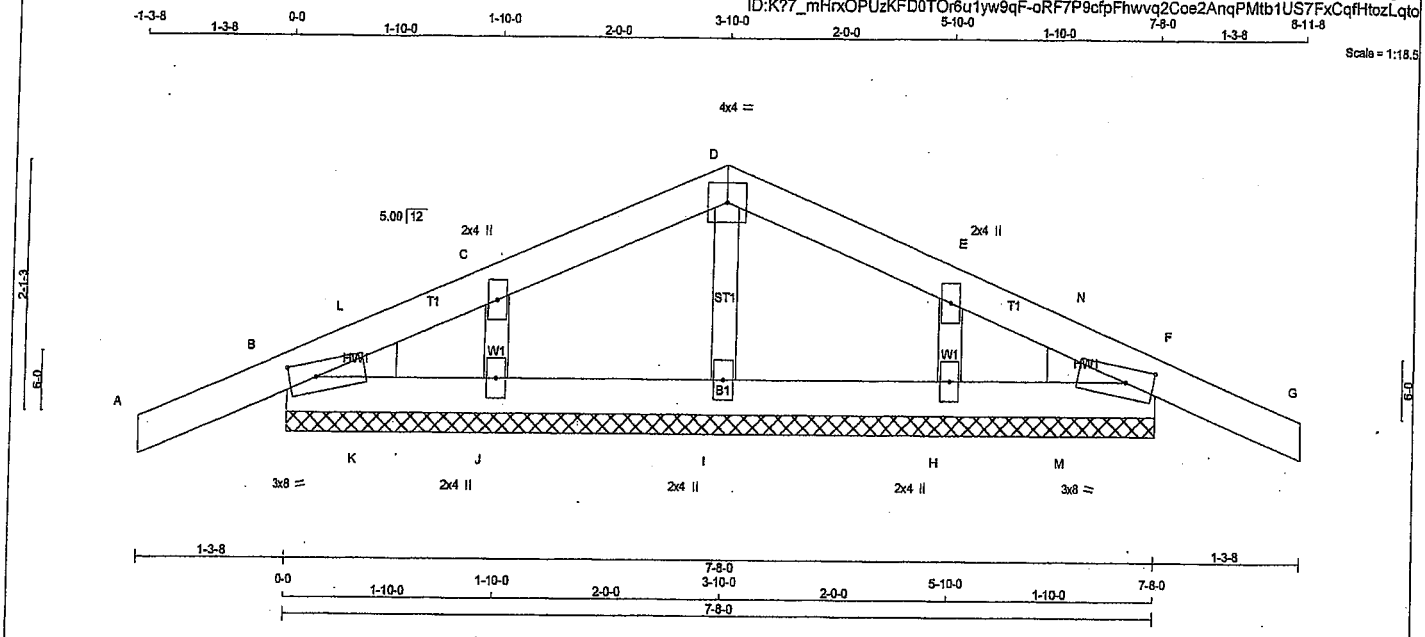


TOTAL WEIGHT = 3 X 26 = 77 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0	1.50	2.75
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-m	MT20	3.0	8.0	1.50	2.75
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
2x4 R
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

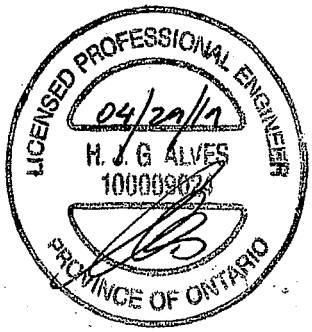
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.11 (1)	10.00	
B- L	-65 / 0	-84.9	-84.9	0.10 (1)	6.25	
L- C	-1 / 10	-84.9	-84.9	0.04 (1)	10.00	
C- D	-5 / 4	-84.9	-84.9	0.04 (1)	10.00	
D- E	-5 / 4	-84.9	-84.9	0.04 (1)	10.00	
E- N	-1 / 10	-84.9	-84.9	0.04 (1)	10.00	
N- F	-65 / 0	-84.9	-84.9	0.10 (1)	6.25	
F- G	0 / 15	-84.9	-84.9	0.11 (1)	10.00	
B- K	0 / 6	-38.5	-38.5	0.02 (1)	10.00	
K- J	0 / 6	-38.5	-38.5	0.02 (3)	10.00	
J- I	-9 / 3	-38.5	-38.5	0.03 (2)	10.00	
I- H	-9 / 3	-38.5	-38.5	0.03 (2)	10.00	
H- M	0 / 6	-38.5	-38.5	0.02 (3)	10.00	
M- F	0 / 6	-38.5	-38.5	0.02 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	-152 / 0	0.02 (1)
J- C	-173 / 0	0.02 (1)
H- E	-173 / 0	0.02 (1)
K- L	0 / 66	0.00 (1)
M- N	0 / 66	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (H-I:2) , WB=0.02/1.00 (C-J:1) , SSI=0.09/1.00 (B-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (E) (INPUT = 1.00)

DWG NO. TAM T1409721
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	P1	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 12:17:47 2019 Page 1

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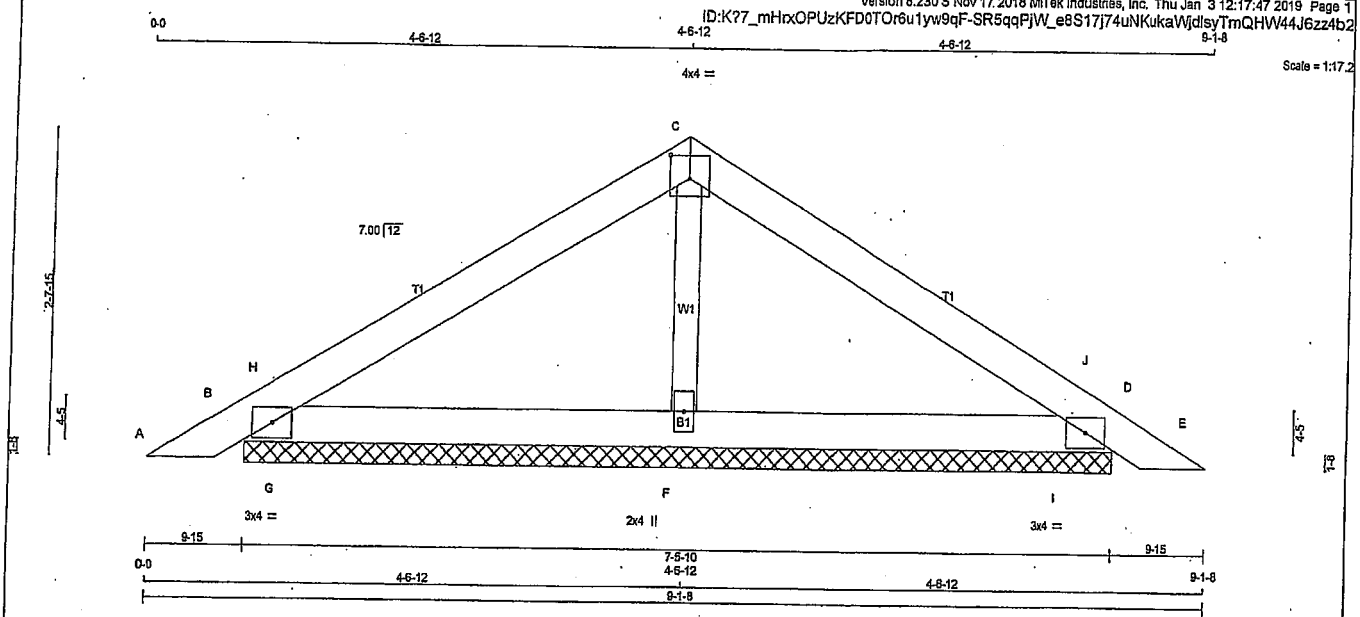


TOTAL WEIGHT = 6 X 22 = 133 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	322	0	322	0	0	7-5-10	7-5-10
D	322	0	322	0	0	7-5-10	7-5-10
F	404	0	404	0	0	7-5-10	7-5-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	237	141 / 0	36 / 0	0 / 0	0 / 0	60 / 0	0 / 0
D	237	141 / 0	36 / 0	0 / 0	0 / 0	60 / 0	0 / 0
F	313	136 / 0	85 / 0	0 / 0	0 / 0	92 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

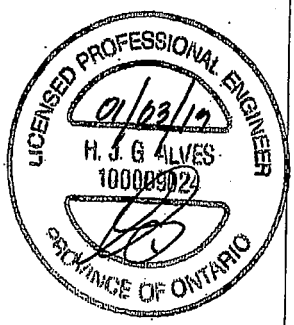
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 15	-84.9	-84.9	0.04 (1)	10.00	
B-H	-75 / 0	-84.9	-84.9	0.04 (1)	6.25	
H-C	-116 / 0	-84.9	-84.9	0.15 (1)	6.25	
C-J	-116 / 0	-84.9	-84.9	0.15 (1)	6.25	
J-D	-75 / 0	-84.9	-84.9	0.04 (1)	6.25	
D-E	0 / 15	-84.9	-84.9	0.04 (1)	10.00	
B-G	0 / 97	-38.5	-38.5	0.14 (1)	10.00	
G-F	0 / 97	-38.5	-38.5	0.16 (1)	10.00	
F-I	0 / 97	-38.5	-38.5	0.16 (1)	10.00	
I-D	0 / 97	-38.5	-38.5	0.14 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-156 / 0	0.02 (1)
G-H	-246 / 24	0.00 (1)
I-J	-246 / 24	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-J:1) , BC=0.16/1.00 (F-I:1) , WB=0.02/1.00 (C-F:1) , SSI=0.21/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)

DWG NO. TAM 17900063
STRUCTURAL
CONSTRUCT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	P1G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 12:17:47 2019 Page 1
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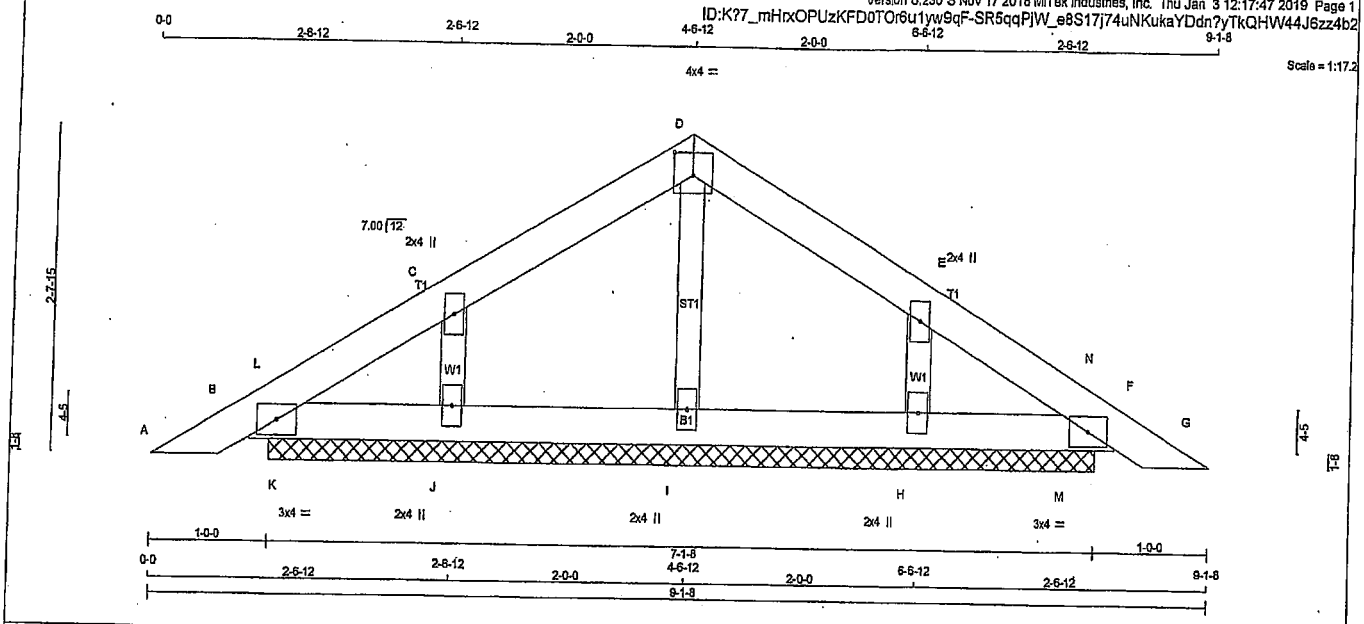


TOTAL WEIGHT = 2 X 23 = 47 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-84.9	-84.9	0.04 (1)	10.00
B-L	-53 / 0	-84.9	-84.9	0.01 (2)	6.25
L-C	-21 / 0	-84.9	-84.9	0.05 (1)	6.25
C-D	-31 / 0	-84.9	-84.9	0.05 (1)	6.25
D-E	-31 / 0	-84.9	-84.9	0.05 (1)	6.25
E-N	-21 / 0	-84.9	-84.9	0.05 (1)	6.25
N-F	-53 / 0	-84.9	-84.9	0.01 (2)	6.25
F-G	0 / 15	-84.9	-84.9	0.04 (1)	10.00
B-K	0 / 33	-38.5	-38.5	0.02 (1)	10.00
K-J	0 / 33	-38.5	-38.5	0.03 (2)	10.00
J-I	0 / 19	-38.5	-38.5	0.03 (2)	10.00
I-H	0 / 19	-38.5	-38.5	0.03 (2)	10.00
H-M	0 / 33	-38.5	-38.5	0.03 (2)	10.00
M-F	0 / 33	-38.5	-38.5	0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	-118 / 0	0.02 (1)
J-C	-188 / 0	0.03 (1)
H-E	-188 / 0	0.03 (1)
K-L	-11 / 17	0.00 (1)
M-N	-11 / 17	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-E:1) , BC=0.03/1.00 (J-K:2) , WB=0.03/1.00 (E-H:1) , SSI=0.07/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

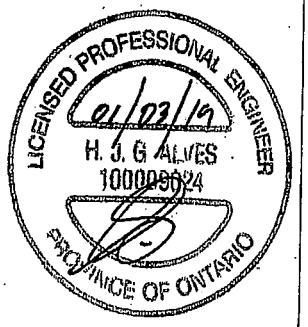
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (C) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



DWG NO. TAM T1900064
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	P2G	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Thu Jan 3 11:24:57 2019 Page 1

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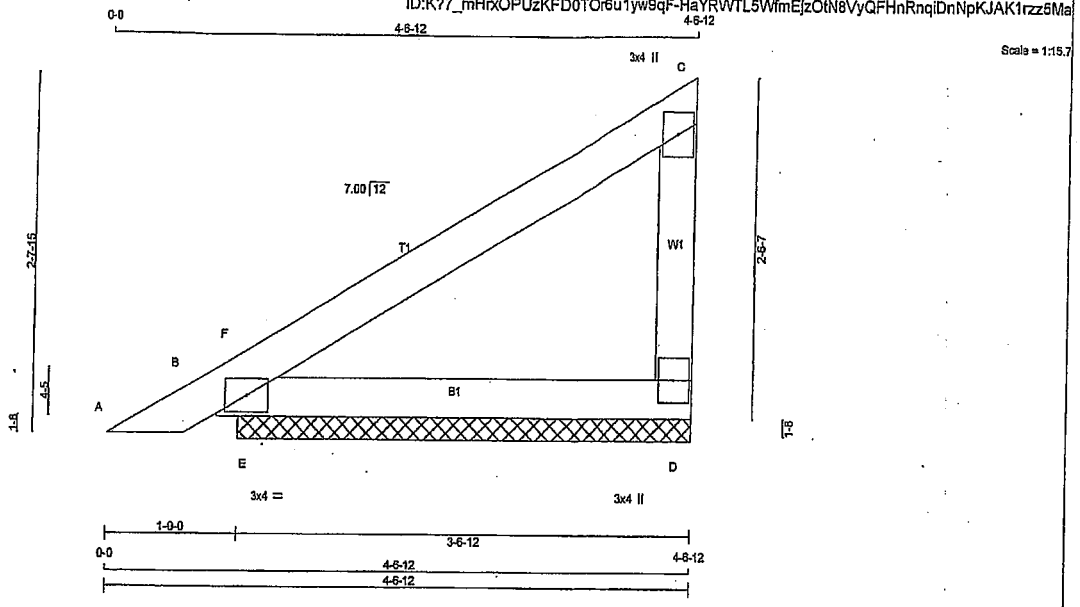


TOTAL WEIGHT = 12 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMV+p	MT20	3.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.9	-84.9	0.04	(1)	10.00	E- F	-165 / 86	0.00 (1)
B- F	-60 / 4	-84.9	-84.9	0.05	(3)	6.25			
F- C	0 / 5	-84.9	-84.9	0.16	(1)	10.00			
D- C	-143 / 0	0.0	0.0	0.02	(1)	7.81			
B- E	0 / 0	-38.5	-38.5	0.10	(1)	10.00			
E- D	0 / 0	-38.5	-38.5	0.13	(1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (C-F:1) , BC=0.13/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.17/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

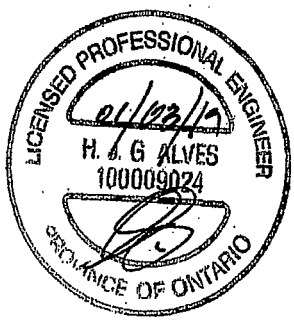
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.60)
JSI METAL= 0.04 (B) (INPUT = 1.00)



DWG NO. TAM T1900056
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	P6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 13:05:43 2019 Page 1
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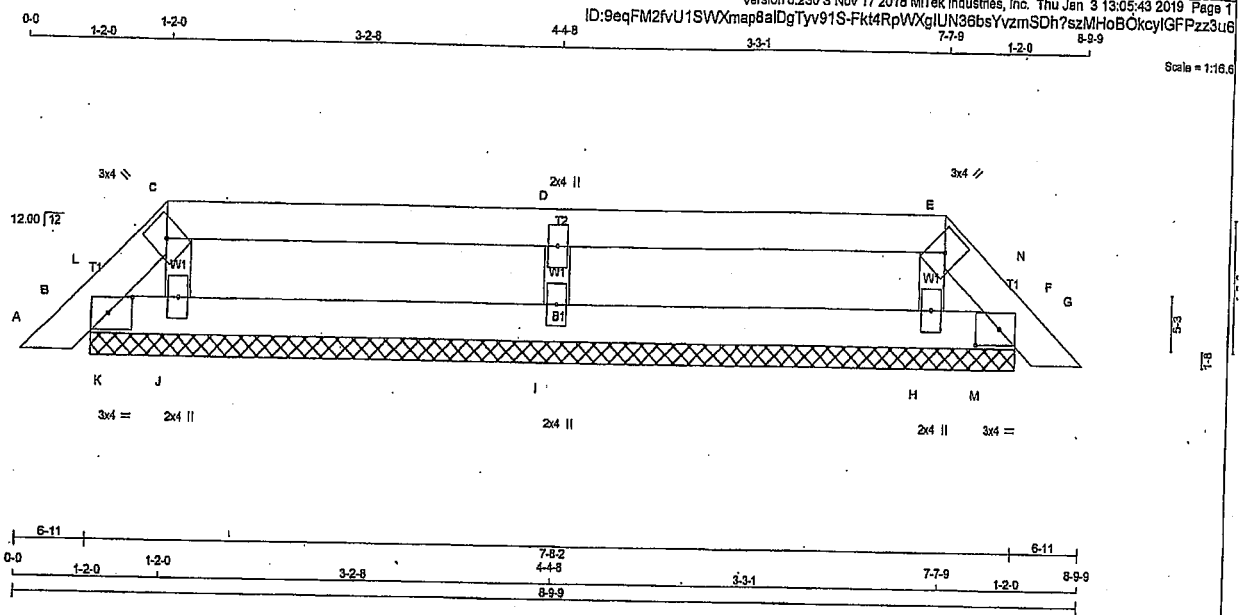


TOTAL WEIGHT = 21 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.25
C	TTW+h	MT20	3.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TTW+h	MT20	3.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.25
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	78	0	78	0	-11	7-8-2	7-8-2
F	77	0	77	0	-13	7-8-2	7-8-2
J	198	0	202	0	0	7-8-2	7-8-2
H	203	0	207	0	0	7-8-2	7-8-2
I	468	0	468	0	0	7-8-2	7-8-2

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	49	55 / 0	0 / -12	0 / 0	0 / 0	6 / 0	0 / 0
F	48	55 / 0	0 / -13	0 / 0	0 / 0	6 / 0	0 / 0
J	160	51 / 0	58 / 0	0 / 0	0 / 0	52 / 0	0 / 0
H	164	52 / 0	60 / 0	0 / 0	0 / 0	53 / 0	0 / 0
I	351	188 / 0	69 / 0	0 / 0	0 / 0	94 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

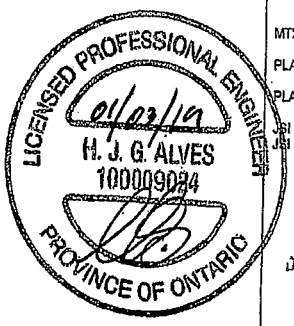
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-84.9	-84.9	0.01 (1)		10.00
B- L	-28 / 18	-84.9	-84.9	0.01 (1)		6.25
L- C	-38 / 0	-84.9	-84.9	0.01 (1)		6.25
C- D	-3 / 0	-84.9	-84.9	0.15 (1)		10.00
D- E	-3 / 0	-84.9	-84.9	0.15 (1)		10.00
E- N	-39 / 0	-84.9	-84.9	0.01 (2)		6.25
N- F	-28 / 19	-84.9	-84.9	0.01 (3)		6.25
F- G	0 / 12	-84.9	-84.9	0.01 (1)		10.00
B- K	0 / 15	-38.5	-38.5	0.00 (1)		10.00
K- J	0 / 15	-38.5	-38.5	0.05 (3)		10.00
J- I	0 / 4	-38.5	-38.5	0.06 (3)		10.00
I- H	0 / 4	-38.5	-38.5	0.06 (3)		10.00
H- M	0 / 16	-38.5	-38.5	0.05 (3)		10.00
M- F	0 / 16	-38.5	-38.5	0.00 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-83 / 0	0.01 (1)
H- E	-84 / 0	0.01 (1)
I- D	-342 / 0	0.05 (1)
K- L	-46 / 0	0.00 (1)
M- N	-48 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (D-E:1) , BC=0.06/1.00 (I-J:3) , WB=0.05/1.00 (D-I:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (E) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

DWG NO. TAM T19000057
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	P7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:43 2019 Page 1
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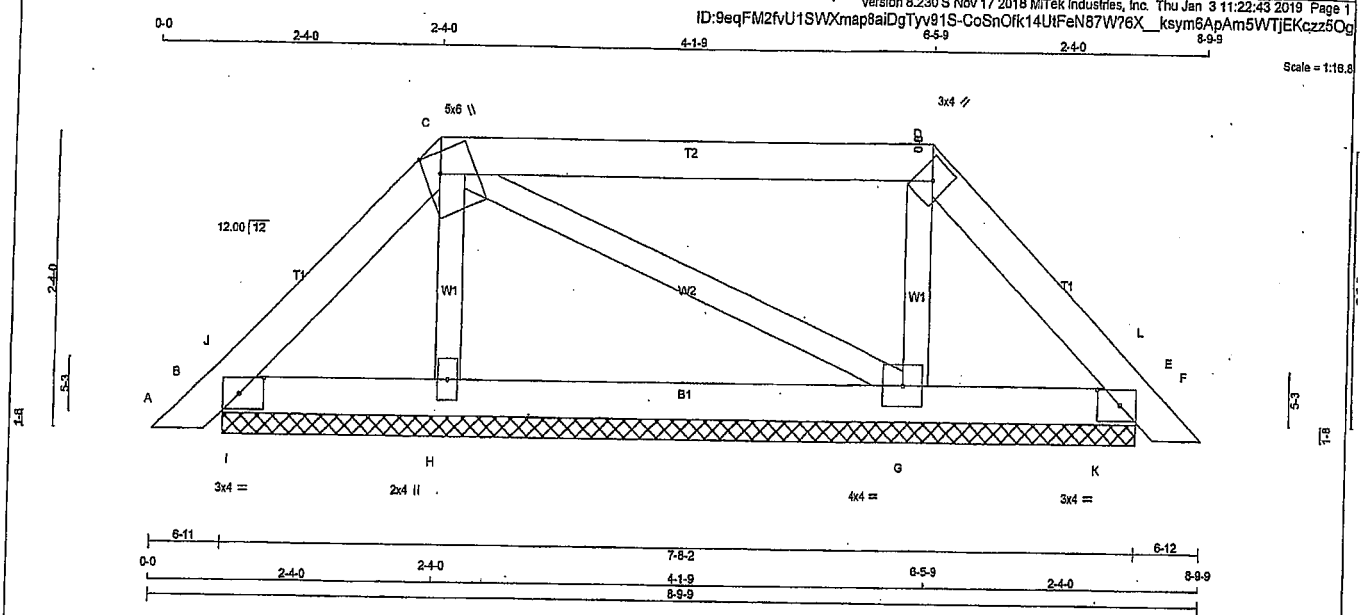


TOTAL WEIGHT = 27 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW+h	MT20	3.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.50
G	BMWW1-I	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	192	0	192	0	0	7-8-2	7-8-2
E	178	0	178	0	0	7-8-2	7-8-2
H	314	0	314	0	0	7-8-2	7-8-2
G	341	0	341	0	0	7-8-2	7-8-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	135	98 / 0	7 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	128	91 / 0	7 / 0	0 / 0	0 / 0	28 / 0	0 / 0
H	246	98 / 0	74 / 0	0 / 0	0 / 0	74 / 0	0 / 0
G	265	113 / 0	74 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

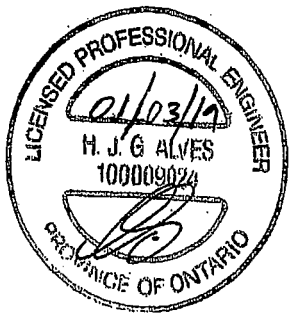
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FROM	TO	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-84.9	-84.9	0.01 (1)	10.00	
B- J	-57 / 0	-84.9	-84.9	0.02 (1)	6.25	
J- C	-92 / 0	-84.9	-84.9	0.03 (1)	6.25	
C- D	-38 / 0	-84.9	-84.9	0.25 (1)	6.25	
D- L	-74 / 0	-84.9	-84.9	0.03 (1)	6.25	
L- E	-35 / 3	-84.9	-84.9	0.02 (1)	6.25	
E- F	0 / 12	-84.9	-84.9	0.01 (1)	10.00	
B- I	0 / 61	-38.5	-38.5	0.04 (1)	10.00	
I- H	0 / 61	-38.5	-38.5	0.08 (2)	10.00	
H- G	0 / 52	-38.5	-38.5	0.08 (2)	10.00	
G- K	0 / 48	-38.5	-38.5	0.08 (2)	10.00	
K- E	0 / 48	-38.5	-38.5	0.04 (1)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-166 / 0	0.03 (1)
C- G	-15 / 0	0.00 (1)
G- D	-166 / 0	0.03 (1)
I- J	-119 / 0	0.00 (1)
K- L	-122 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (C-D:1) , BC=0.08/1.00 (H-I:2) , WB=0.03/1.00 (D-G:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (D) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM T190005B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	P8	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:44 2019 Page 1
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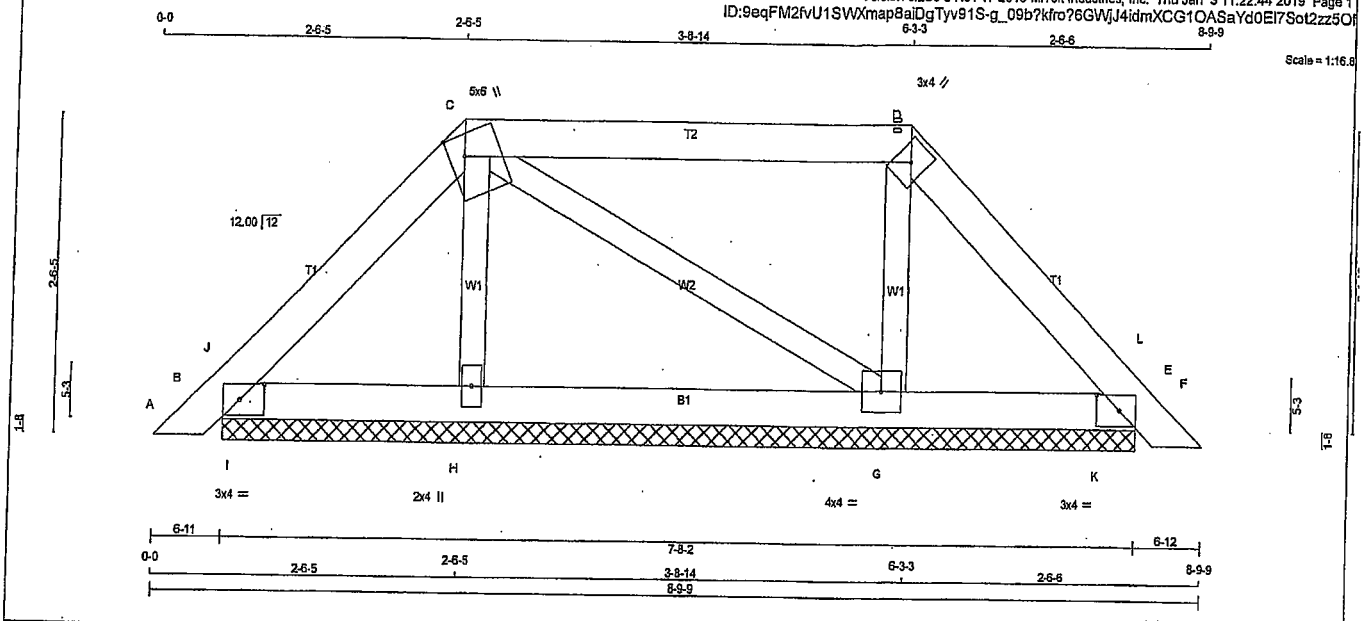

TOTAL WEIGHT = 2 X 27 = 54 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW+h	MT20	3.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.25
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	215	0	215	0	0	7-8-2	7-8-2
E	199	0	199	0	0	7-8-2	7-8-2
H	288	0	288	0	0	7-8-2	7-8-2
G	323	0	323	0	0	7-8-2	7-8-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	153	104 / 0	13 / 0	0 / 0	0 / 0	36 / 0	0 / 0
E	142	95 / 0	13 / 0	0 / 0	0 / 0	34 / 0	0 / 0
H	226	90 / 0	67 / 0	0 / 0	0 / 0	68 / 0	0 / 0
G	250	110 / 0	67 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 12	-84.9	-84.9	0.01 (1)	10.00
B-J	-67 / 0	-84.9	-84.9	0.02 (1)	6.25
J-C	-99 / 0	-84.9	-84.9	0.04 (1)	6.25
C-D	-42 / 0	-84.9	-84.9	0.20 (1)	6.25
D-L	-78 / 0	-84.9	-84.9	0.04 (1)	6.25
L-E	-42 / 0	-84.9	-84.9	0.02 (1)	6.25
E-F	0 / 12	-84.9	-84.9	0.01 (1)	10.00
B-I	0 / 66	-38.5	-38.5	0.05 (1)	10.00
I-H	0 / 66	-38.5	-38.5	0.07 (2)	10.00
H-G	0 / 59	-38.5	-38.5	0.07 (2)	10.00
G-K	0 / 51	-38.5	-38.5	0.07 (2)	10.00
K-E	0 / 51	-38.5	-38.5	0.04 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-150 / 0	0.02 (1)
C-G	-19 / 0	0.01 (1)
G-D	-175 / 0	0.03 (1)
I-J	-123 / 0	0.00 (1)
K-L	-127 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (C-D:1) , BC=0.07/1.00 (H-I:2) , WB=0.03/1.00 (D-G:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (D) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

ENG NO. TAM T1900059
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	P9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 12:17:48 2019 Page 1
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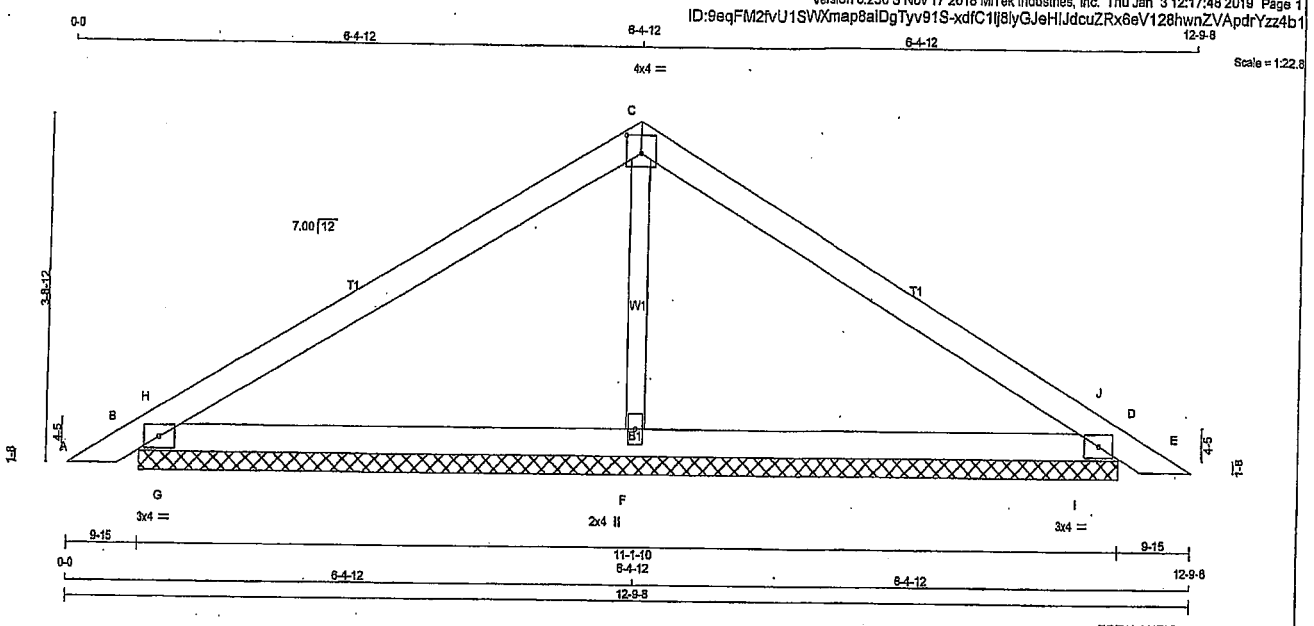


TOTAL WEIGHT = 2 X 32 = 64 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	475	0	475	0	0	11-1-10	11-1-10
D	475	0	475	0	0	11-1-10	11-1-10
F	551	0	551	0	0	11-1-10	11-1-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	351	204 / 0	56 / 0	0 / 0	0 / 0	90 / 0	0 / 0
D	351	204 / 0	56 / 0	0 / 0	0 / 0	90 / 0	0 / 0
F	429	180 / 0	121 / 0	0 / 0	0 / 0	127 / 0	0 / 0

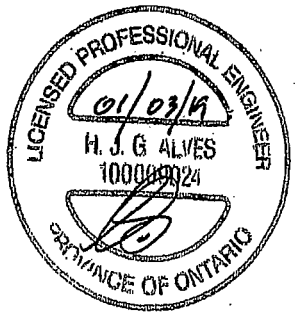
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B		0 / 15	-84.9	-84.9	0.04 (1)	10.00	F-C		-187 / 0	0.04 (1)
B-H		-129 / 0	-84.9	-84.9	0.13 (1)	6.25	G-H		-513 / 52	0.00 (1)
H-C		-226 / 0	-84.9	-84.9	0.34 (1)	6.25	I-J		-513 / 52	0.00 (1)
C-J		-226 / 0	-84.9	-84.9	0.34 (1)	6.25				
J-D		-129 / 0	-84.9	-84.9	0.13 (1)	6.25				
D-E		0 / 15	-84.9	-84.9	0.04 (1)	10.00				
B-G		0 / 187	-38.5	-38.5	0.29 (1)	10.00				
G-F		0 / 187	-38.5	-38.5	0.35 (1)	10.00				
F-I		0 / 187	-38.5	-38.5	0.35 (1)	10.00				
I-D		0 / 187	-38.5	-38.5	0.29 (1)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34/1.00 (C-H:1) , BC=0.35/1.00 (F-I:1) , WB=0.04/1.00 (C-F:1) , SSI=0.43/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

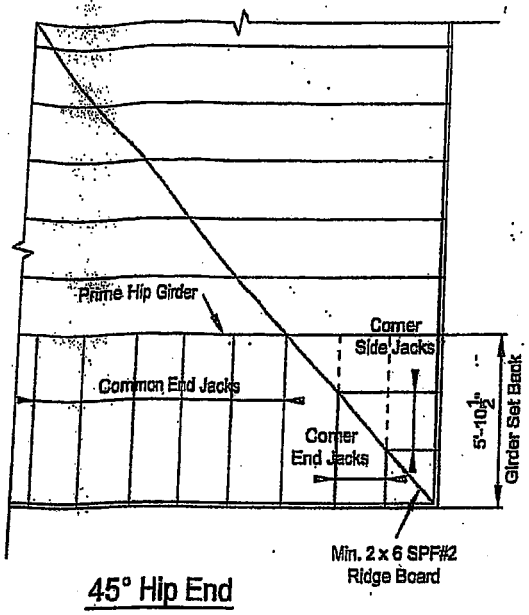
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

DWG NO. TAM 7900065
STRUCTURAL
GENERALITY ONLY



45° Hip End

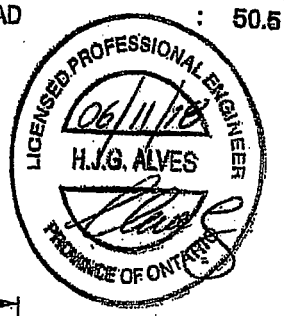
LUMBER SPECIFICATION

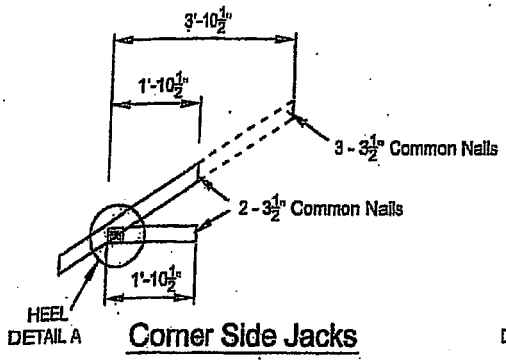
TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

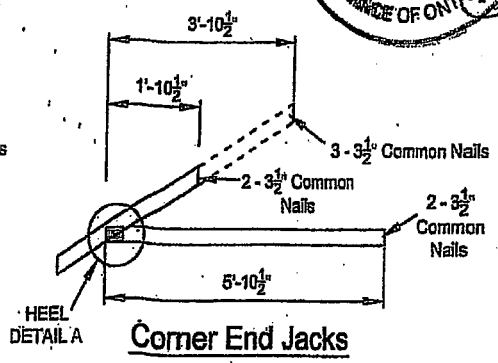
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

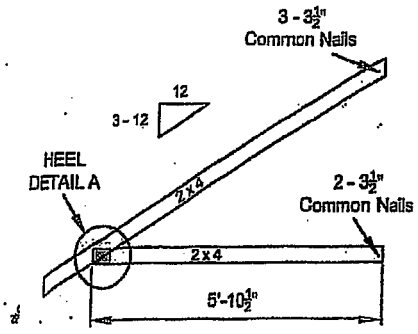




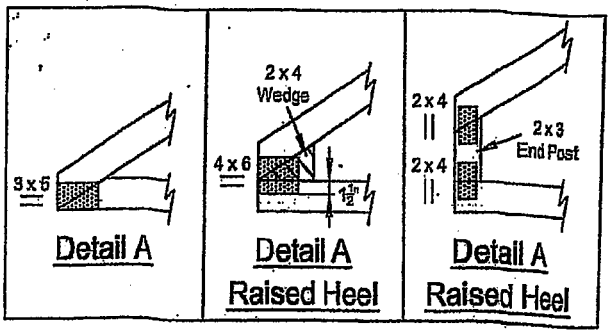
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

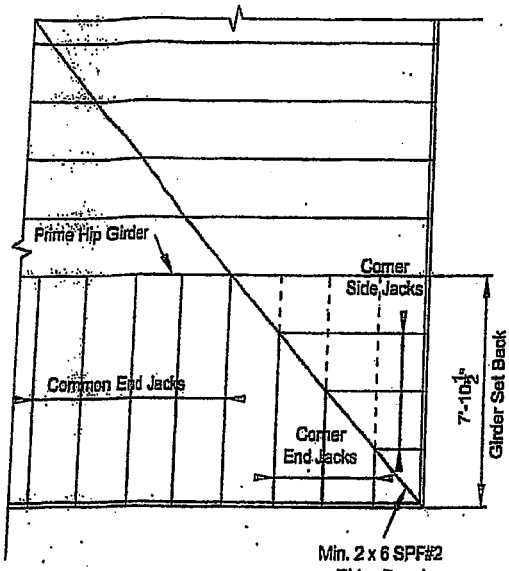
T-1800216

LUMBER SPECIFICATION

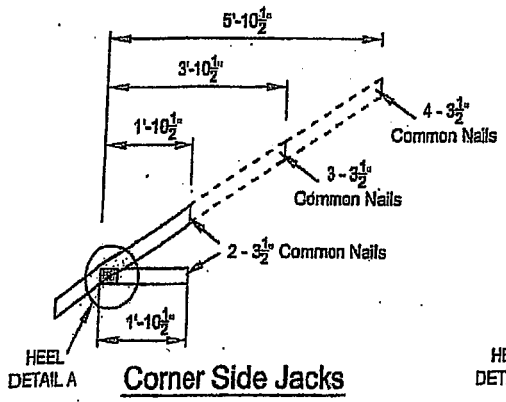
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

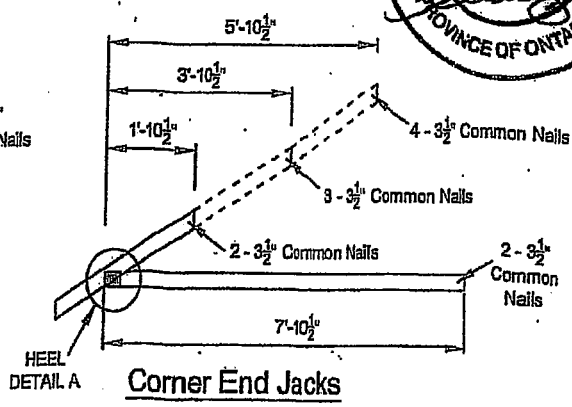
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F.



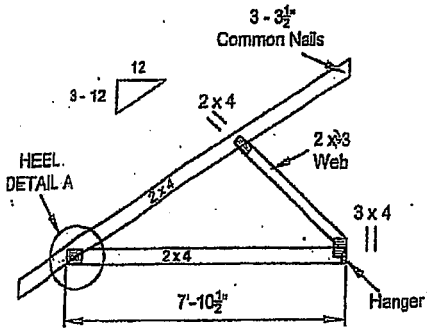
45° Hip End



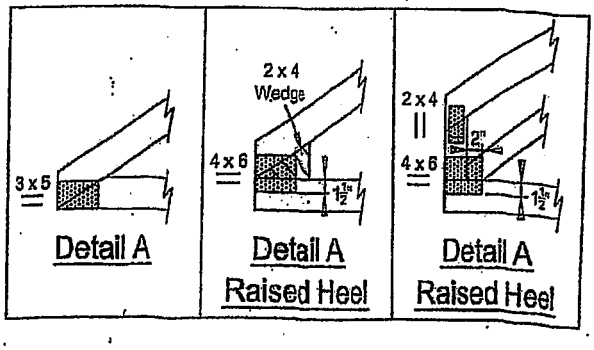
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

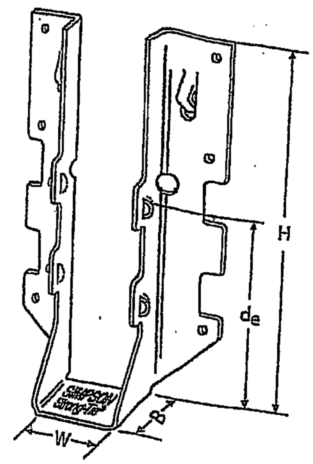
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

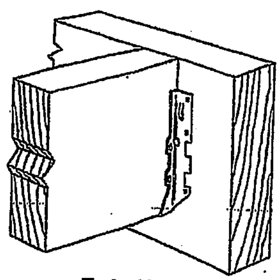
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



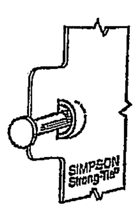
LUS28



Typical LUS Installation

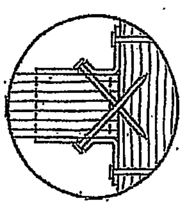
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

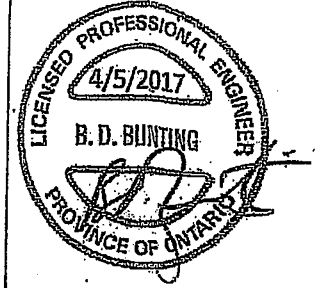


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

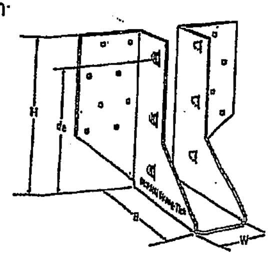
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

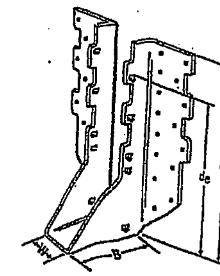
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

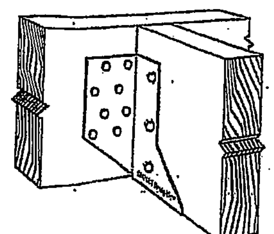
- See current catalogue for options



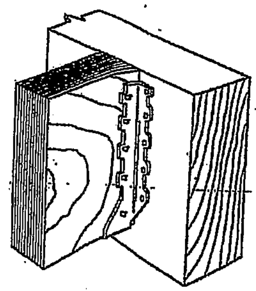
LJS26DS



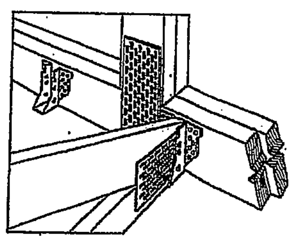
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



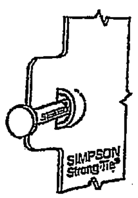
Typical HUS Installation



Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

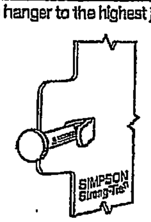
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7³⁄₃₂	3	6³⁄₃₂	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₃₂	3	7³¹⁄₃₂	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

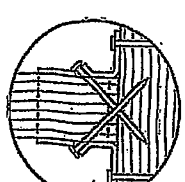


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/3/2017
B. D. BUNTING
PROVINCE OF ONTARIO

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

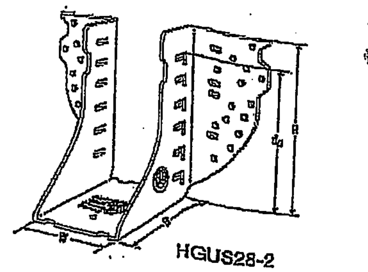
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

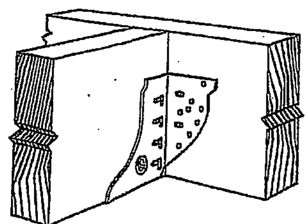
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

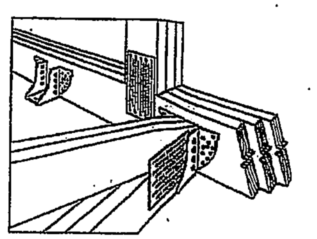
- See current catalogue for options



HGUS28-2



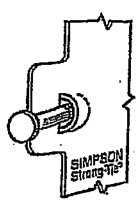
Typical HGUS Installation



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

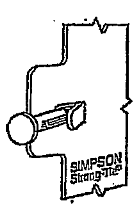
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.) D.Fir-L		Factored Resistance (lb.) S-P-F	
		W	H	B	d_e [1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⁵⁄₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⁹⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⁹⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

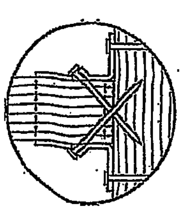


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

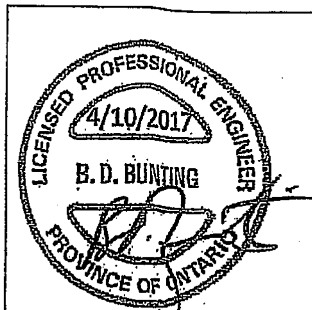
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.

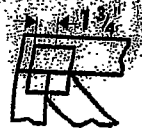


Double Shear Nailing Top View.

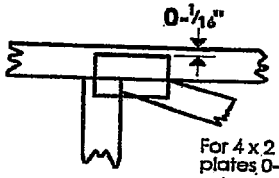


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

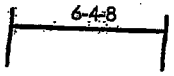
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

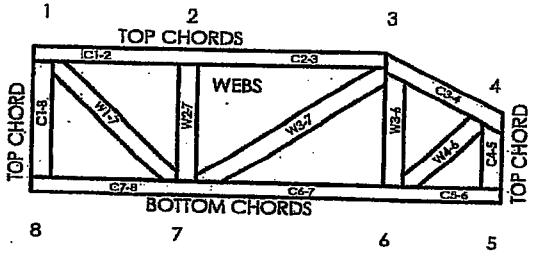
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

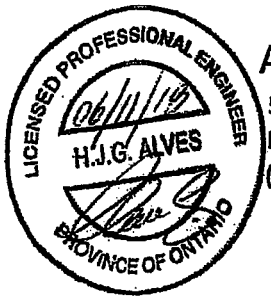
MiTek®

POWER TO PERFORM.™

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information			Application number:
Building number, street name			Unit no. UNITS 1 – 5 / Lot/con. BLOCK 1
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description	

B. Individual who reviews and takes responsibility for design activities			
Name SAM KATSOULAKOS		Firm MICRO CITY ENGINEERING SERVICES INC.	
Street address R.R #1, PO BOX 61			Unit no. / Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750		Cell number

C. Design activities undertaken by individual identified in Section B: [Building Code Table 3.5.2.1. of Division C]

☐ House	☐ HVAC – House	☒ Building Structural
☐ Small Buildings	☐ Building Services	☐ Plumbing – House
☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing – All Buildings
☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems

Description of designer's work **ROYAL PINE HOMES – FORESTSIDE PH6 – BLOCK 1–UNITS 1 - 5**
(SCHEDULE IS ISSUED AS LOT SPECIFIC)
REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM1137-19 DATED 1-18-19).
SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFED BY QUALIFIED BUILDING DESIGNER.

D. Declaration of Designer

I, **SAM KATSOULAKOS** declare that (choose one as appropriate):
(print name)

☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4.of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.

Individual BCIN: **26064**

Firm BCIN: **29991**

☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5.of Division C, of the Building Code.
Individual BCIN: _____

Basis for exemption from registration: _____

☐ The design work is exempt from the registration and qualification requirements of the Building Code.
Basis for exemption from registration and qualification: _____

I certify that:

1. The information contained in this schedule is true to the best of my knowledge.
2. I have submitted this application with the knowledge and consent of the firm.

Date 1-18-19 Signature of Designer

NOTE:

1. For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d).of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
2. Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

1-18-19

DWG #TAM 1137 -19-S
DWG #TAM 1138 -19-S