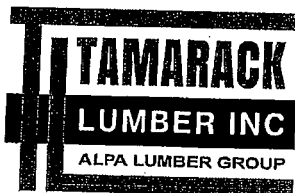


THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



Delivery Shiplist

Page 1 of 2

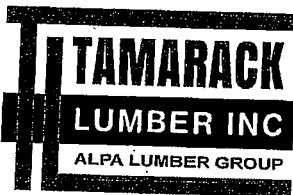
DATE	09/14/18
SALES REP	Mario

JOB TRACK: 41239	LAYOUT ID: 300799	LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT	SUB-BUILDER:	
MODEL: UNIT1804 END	ELEVATION: B LOT1 OR LOT6	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	G40 GABLE	7.00 0.00	13-08-00	09-03-02	2 X 4 2 X 4	00-00-00 00-00-00	01-03-07 09-03-02	68.36 43.83		
	1	G34 GABLE	7.00 0.00	28-08-00	10-00-00	2 X 4 2 X 4	01-03-08 00-00-00	01-03-07 09-07-03	171.71 109.00		
	1 2 Ply	T2 HALF HIP	12.00 0.00	17-09-08	04-08-09	2 X 4 2 X 6	01-03-08 00-00-00	01-10-08 04-08-09	199.00 121.34		
	2	T3 HALF HIP	12.00 0.00	17-09-08	05-10-09	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 05-10-09	156.36 98.00		
	2	T4 HALF HIP	12.00 0.00	17-09-08	07-00-09	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 07-00-09	166.94 104.66		
	2	T5 HALF HIP	12.00 0.00	17-09-08	08-02-09	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 08-02-09	185.34 117.34		
	7	T6 HALF HIP	12.00 0.00	17-09-08	09-04-09	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 09-04-09	737.94 462.00		
	2	T7 HALF HIP	12.00 0.00	17-09-08	10-06-09	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 10-06-09	208.64 127.66		
	1 2 Ply	T26A HIP GIRDER	12.00 0.00	17-09-08	05-09-00	2 X 4 2 X 6	01-03-08 00-00-00	01-10-08 02-01-00	205.94 124.00		
	1	T27A HIP	12.00 0.00	17-09-08	07-09-00	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	80.84 51.67		
	1	T29A COMMON	12.00 0.00	10-02-08	07-01-00	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	47.71 31.00		
	1	T30 ROOF	12.00 8.00	10-05-00	07-01-00	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	55.38 36.33		
	1	T30A ROOF	12.00 8.00	10-02-08	07-01-00	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	53.04 35.00		
	1	T12 COMMON	12.00 0.00	06-10-00	04-10-08	2 X 4 2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	33.29 23.17		
	1	G12 GABLE	12.00 0.00	06-10-00	04-10-08	2 X 4 2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	34.17 24.17		
	2	P10 PIGGYBACK	12.00 0.00	09-08-12	02-02-08	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 02-02-08	67.46 44.00		
	3	P11 PIGGYBACK	12.00 0.00	09-08-12	03-04-08	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 03-04-08	110.34 69.51		
	1	P12G PIGGYBACK	7.00 0.00	11-04-10	03-08-02	2 X 4 2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	35.84 22.67		



Delivery Shiplist

Page 2 of 2

DATE	09/14/18
SALES REP	Mario

JOB TRACK:41239	LAYOUT ID: 300799	LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT	SUB-BUILDER:	
MODEL: UNIT1804 END	ELEVATION: B LOT1 OR LOT6	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	5	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-04-15	86.50 53.35		
	4	J1A JACK-OPEN	7.00 0.00	05-08-00	04-08-09	2 X 4	2 X 4	00-00-00 00-00-00	01-04-14 04-08-09	60.88 37.32		
	2	J3 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	29.50 18.66		
	1	J4 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 02-01-01	01-10-08 00-03-08	11.39 7.33		
	1	J5 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 00-01-01	01-10-08 00-03-08	9.10 6.00		

TOTAL # TRUSS= 46

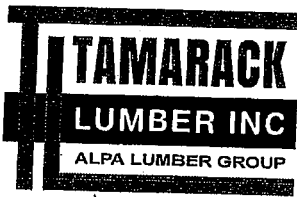
TOTAL BFT OF ALL TRUSSES=

1768.01 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2815.67 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
6	Hangers	LUS24	

TOTAL # ITEMS= 7



Delivery Shiplist

Page 1 of 2

DATE	09/14/18
SALES REP	Mario

JOB TRACK:41239

LAYOUT ID: 300800

LOCATION:

BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT

SUB-BUILDER:

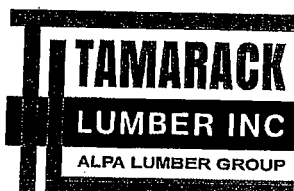
MODEL: UNIT1803

ELEVATION: B LOT2 OR LOT5

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T33 PIGGYBACK	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 07-03-12	150.22 94.33		
	1	G33 GABLE	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 07-03-12	167.03 106.17		
	7	T34 PIGGYBACK	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 09-07-03	1055.25 659.19		
	1	G34 GABLE	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 09-07-03	171.71 109.00		
	1 2 Ply	T35 FLAT GIRDER	0.00 0.00	17-08-00	04-08-09	2 X 6	2 X 6	00-00-00 00-00-00	04-08-09 04-08-09	220.58 134.68		
	1	T36 FLAT	0.00 0.00	17-08-00	05-10-09	2 X 4	2 X 4	00-00-00 00-00-00	05-10-09 05-10-09	79.76 50.00		
	1	T37 FLAT	0.00 0.00	17-08-00	07-00-09	2 X 4	2 X 4	00-00-00 00-00-00	07-00-09 07-00-09	86.38 53.83		
	1	T38 FLAT	0.00 0.00	17-08-00	08-02-09	2 X 4	2 X 4	00-00-00 00-00-00	08-02-09 08-02-09	93.18 57.67		
	1 3 Ply	T39X FLAT GIRDER	0.00 0.00	17-05-00	09-04-09	2 X 6	2 X 6	00-00-00 00-00-00	00-05-08 00-05-08	498.06 311.01		
	1	T42 SCISSOR	12.00 8.00	07-09-00	05-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	41.86 26.83		
	1	T46A SCISSOR	12.00 8.00	07-06-08	09-05-00	2 X 4	2 X 4	00-00-00 00-00-00	01-10-08 09-05-00	46.44 29.50		
	2	T140 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	93.70 59.66		
	1	G40 GABLE	7.00 0.00	13-08-00	09-03-02	2 X 4	2 X 4	00-00-00 00-00-00	01-03-07 09-03-02	68.36 43.83		
	1	G41 GABLE	0.00 0.00	13-08-00	09-05-04	2 X 4	2 X 4	00-00-00 00-00-00	09-05-04 09-05-04	94.63 57.67		
	1	P1 PIGGYBACK	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	22.11 14.33		
	1	P1G GABLE	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	23.42 15.33		
	7	P12 PIGGYBACK	7.00 0.00	11-04-10	03-08-02	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	228.76 144.69		
	1	P12G GABLE	7.00 0.00	11-04-10	03-08-02	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	35.84 22.67		



Delivery Shiplist

DATE	09/14/18
SALES REP	Mario

JOB TRACK: 41239	LAYOUT ID: 300800	LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT	SUB-BUILDER:	
MODEL: UNIT1803	ELEVATION: B LOT2 OR LOT5	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	5	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4 2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	86.50 53.35		

TOTAL # TRUSS= 39

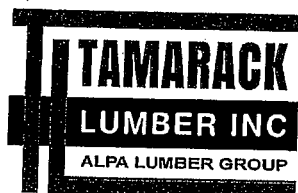
TOTAL BFT OF ALL TRUSSES=

2043.74 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3263.79 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	
8	Hangers	LJS26DS	

TOTAL # ITEMS= 12



Delivery Shiplist

DATE	09/14/18
SALES REP	Mario

JOB TRACK: 41239	LAYOUT ID: 300784	LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT	SUB-BUILDER:	
MODEL: UNIT1801 OR UNIT1802	ELEVATION: B LOT3 OR LOT4	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	8	T13 PIGGYBACK	7.00 0.00	39-00-00	10-00-00	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	1640.08 1016.00		
	2	G13 GABLE	7.00 0.00	39-00-00	10-00-00	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	453.88 284.66		
	1	T140 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	46.85 29.83		
	1	G140 GABLE	12.00 0.00	07-06-08	09-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	48.08 29.83		
	1 2 Ply	T141 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4	2 X 6	00-00-00 00-00-00	01-10-08 09-05-00	109.56 66.66		
	8	P1 PIGGYBACK	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	176.88 114.64		
	2	P1G GABLE	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	46.84 30.66		

TOTAL # TRUSS= 24

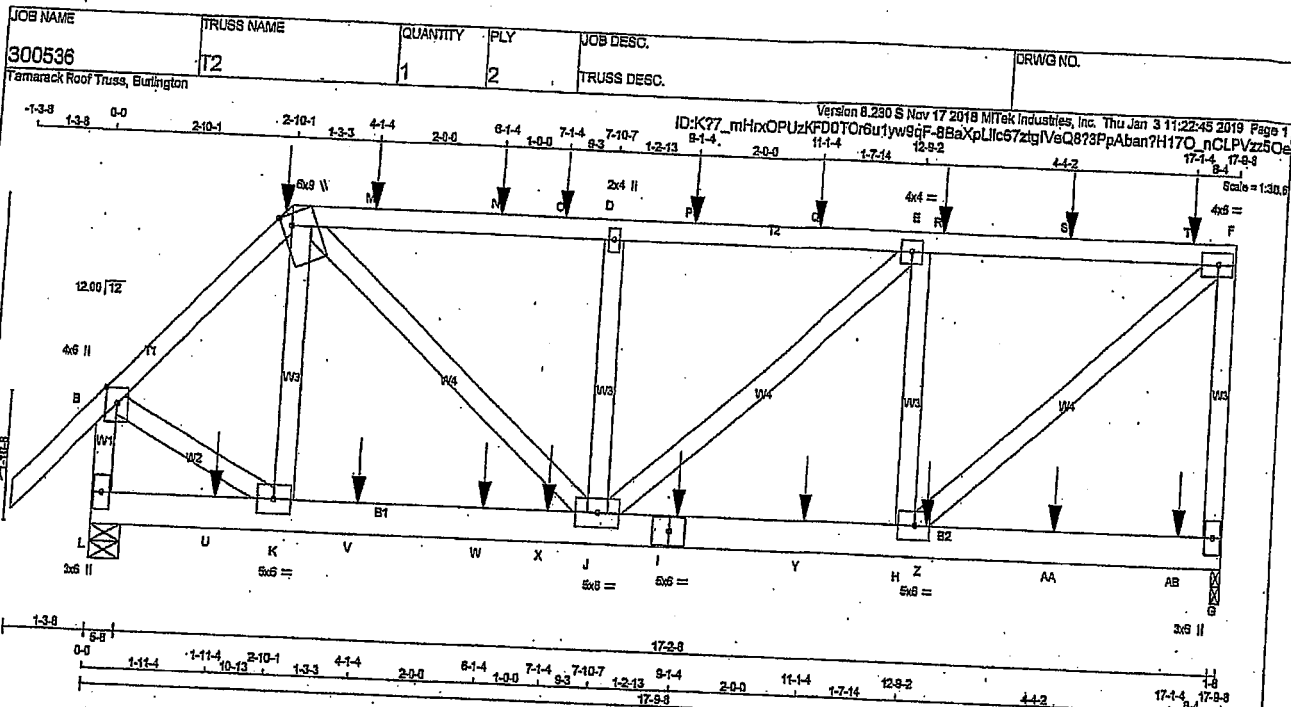
TOTAL BFT OF ALL TRUSSES=

1572.28 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2522.17 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LJS26DS	

TOTAL # ITEMS= 3



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - F	2x4	DRY No.2
G - F	2x4	DRY No.2
L - B	2x4	DRY No.2
L - I	2x6	DRY No.2
I - G	2x6	DRY No.2

ALL WEBS 2x4 DRY No.2
 DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'x3') SPIRAL NAILS		
A-C	12	SIDE(81.0)
C-F	12	SIDE(81.0)
F-G	12	TOP
L-B	12	TOP
BOTTOM CHORDS : (0.122'x3') SPIRAL NAILS		
L-I	12	SIDE(81.0)
I-G	12	SIDE(183.1)
WEBS : (0.122'x3') SPIRAL NAILS		
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	4.0	8.0	2.75	2.00
C	TMWV+m	MT20	6.0	9.0	Edge	1.75
D	TMWV+w	MT20	2.0	4.0		
E	TMWV+t	MT20	4.0	4.0		
F	TMWV+t	MT20	4.0	6.0		
G	BMVH+p	MT20	3.0	6.0		
H	BMWV+t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWVW-t	MT20	5.0	8.0		
K	BMWVW-t	MT20	5.0	6.0		
L	BMVH+p	MT20	6.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	DOWN	BRG	BRG
G	2244	0	0	1-8
L	2221	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LOASE	MAX MIN. COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
G	COMBINED	SNOW	LIVE	0/0	0/0	472/0
L	1676	850/0	378/0	0/0	0/0	459/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO	A-B	0/42	-84.9	0.07 (1)	10.00	K-C	-274/105	0.03 (1)
	B-C	-1854/0	-84.9	0.08 (1)	8.25	C-J	0/1431	0.13 (1)
	C-M	-2381/0	-84.9	0.37 (1)	5.35	J-D	0/1431	0.13 (1)
	M-N	-2381/0	-84.9	0.37 (1)	5.35	J-E	0/1431	0.13 (1)
	N-O	-2381/0	-84.9	0.37 (1)	5.35	J-F	0/1431	0.13 (1)
	O-D	-2381/0	-84.9	0.37 (1)	5.35	J-G	0/1431	0.13 (1)
	D-P	-2381/0	-84.9	0.37 (1)	5.35	J-H	0/1431	0.13 (1)
	P-Q	-2381/0	-84.9	0.37 (1)	5.35	J-I	0/1431	0.13 (1)
	Q-E	-2381/0	-84.9	0.37 (1)	5.35	J-J	0/1431	0.13 (1)
	E-R	-1958/0	-84.9	0.37 (1)	5.35	J-K	0/1431	0.13 (1)
	R-S	-1958/0	-84.9	0.37 (1)	5.35	J-L	0/1431	0.13 (1)
	S-T	-1958/0	-84.9	0.37 (1)	5.35	J-M	0/1431	0.13 (1)
	T-F	-1958/0	-84.9	0.37 (1)	5.35	J-N	0/1431	0.13 (1)
	G-F	-2065/0	-84.9	0.37 (1)	5.35	J-O	0/1431	0.13 (1)
	L-B	-2185/0	0.0	0.0 (1)	7.6	J-P	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-Q	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-R	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-S	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-T	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-U	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-V	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-W	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-X	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-Y	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-Z	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-AA	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-AB	0/1431	0.13 (1)
			0.0	0.0 (1)	7.6	J-AC	0/1431	0.13 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-10-1	-35	-39		FRONT	VERT	DEAD		
C	2-10-1	-162	-162		FRONT	VERT	SNOW		
I	9-1-4	-71	-80		BACK	VERT	TOTAL		
M	4-1-4	-164	-164		BACK	VERT	TOTAL		
N	6-1-4	-156	-156		BACK	VERT	TOTAL		
O	7-1-4	-156	-156		BACK	VERT	TOTAL		
P	9-1-4	-156	-156		BACK	VERT	TOTAL		
Q	11-1-4	-156	-156		BACK	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 23.3 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 10.5 PSF
	DL	= 7.0 PSF
TOTAL LOAD		= 46.8 PSF

SPACING = 24.0 IN/GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 -PART 9 OF BCBC 2010, CBC 2012
 -CSA 086-09, CSA 086-14
 -TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.59")
 CALCULATED VERT. DEFL. (LL) = L/899 (0.03")
 ALLOWABLE DEFL. (TL) = L/360 (0.59")
 CALCULATED VERT. DEFL. (TL) = L/899 (0.05")

CS1: TC=0.37/1.00 (C-D:1), SC=0.19/1.00 (H-I:2),
 WS=0.23/1.00 (F-H:1), SS=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1657

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 6.0 Deg.

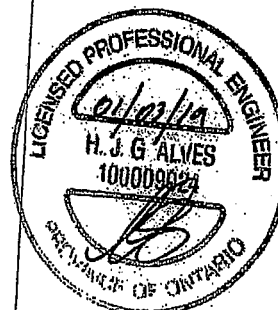
JSI GRIP= 0.77 (B) (INPUT = 0.90)
 JSI METAL= 0.35 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME 300536 Tamarack Roof Truss, Burlington	TRUSS NAME T2	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
--	-------------------------	----------------------	-----------------	---------------------------------	----------

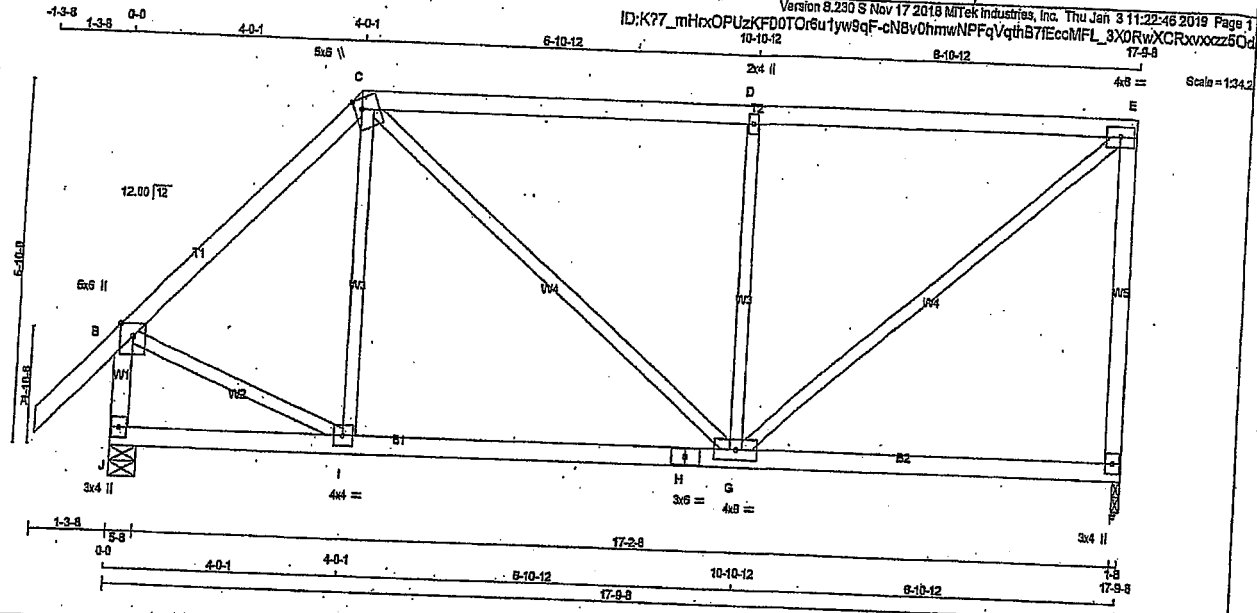
Version 8.230 S Nov-17 2018 Mitek Industries, Inc. Thu Jan 3 11:22:45 2018 Page 2
ID:K77 mHxOPUzKFD0TOr8u1vw9aF-8BaXpLlC67zIqIVeQ873PpAben?H17O nCLPVzz5Oe

FACTORED CONCENTRATED LOADS (LBS)									
	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	13-1-4	-184	-184	---	BACK	VERT	TOTAL	---	---
R	15-1-4	-184	-184	---	BACK	VERT	TOTAL	---	---
S	17-1-4	-178	-178	---	BACK	VERT	TOTAL	---	---
T	1-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
U	4-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
V	6-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
W	7-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
X	11-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
Y	13-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
Z	15-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
AA	17-1-4	-81	-103	---	BACK	VERT	TOTAL	---	---
AB									



DWG NO. TAM **71900039**
STRUCTURAL
COMPONENT ONLY **2/2**

JOB NAME 300536	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N.L.G.A. RULES	SIZE	LUMBER
CHORDS		
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
F - E	2x4	DRY No.2
J - B	2x4	DRY No.2
J - H	2x4	DRY No.2
H - F	2x4	DRY No.2
ALL WEBS EXCEPT	2x3	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TMVW+w	MT20	2.0	4.0	
E	TMVW+t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW+t	MT20	4.0	9.0	
H	BS+t	MT20	3.0	6.0	
I	BMVWW+t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	1097	0	1097	0	1-8	1-8
J	1216	0	1216	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
F	832	414/0	187/0	0/0	0/0	0/0	231/0	0/0
J	914	479/0	187/0	0/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.
A-B	0/142	-84.9	-84.9 0.12 (1)	10.00	I-C	-13/154	0.04 (3)
B-C	-800/0	-84.9	-84.9 0.28 (1)	6.16	C-G	0/410	0.03 (1)
C-D	-852/0	-84.9	-84.9 0.75 (1)	5.02	G-D	-723/0	0.39 (1)
D-E	-852/0	-84.9	-84.9 0.75 (1)	5.02	G-E	0/1223	0.28 (1)
E-F	-891/0	0.0	0.0 0.64 (1)	7.81	B-I	0/576	0.15 (1)
F-G	-1162/0	0.0	0.0 0.13 (1)	7.39			
J-I	0/0	-38.5	-38.5 0.19 (3)	10.00			
I-H	0/634	-38.5	-38.5 0.42 (2)	10.00			
H-G	0/634	-38.5	-38.5 0.42 (2)	10.00			
G-F	0/0	-38.5	-38.5 0.36 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 DL/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.59")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.09")
ALLOWABLE DEFL. (TL) = 1/360 (0.59")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.14")

CSI: TC=0.75/1.00 (D-E-1), BC=0.42/1.00 (G-I-2), WB=0.39/1.00 (D-G-1), GS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

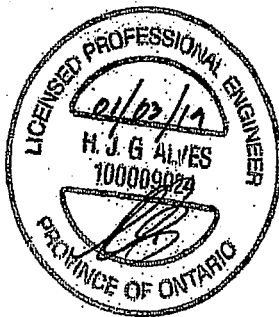
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1667 788 1987 1658	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.79 (I) (INPUT = 0.90)
JSI METAL = 0.44 (F) (INPUT = 1.00)

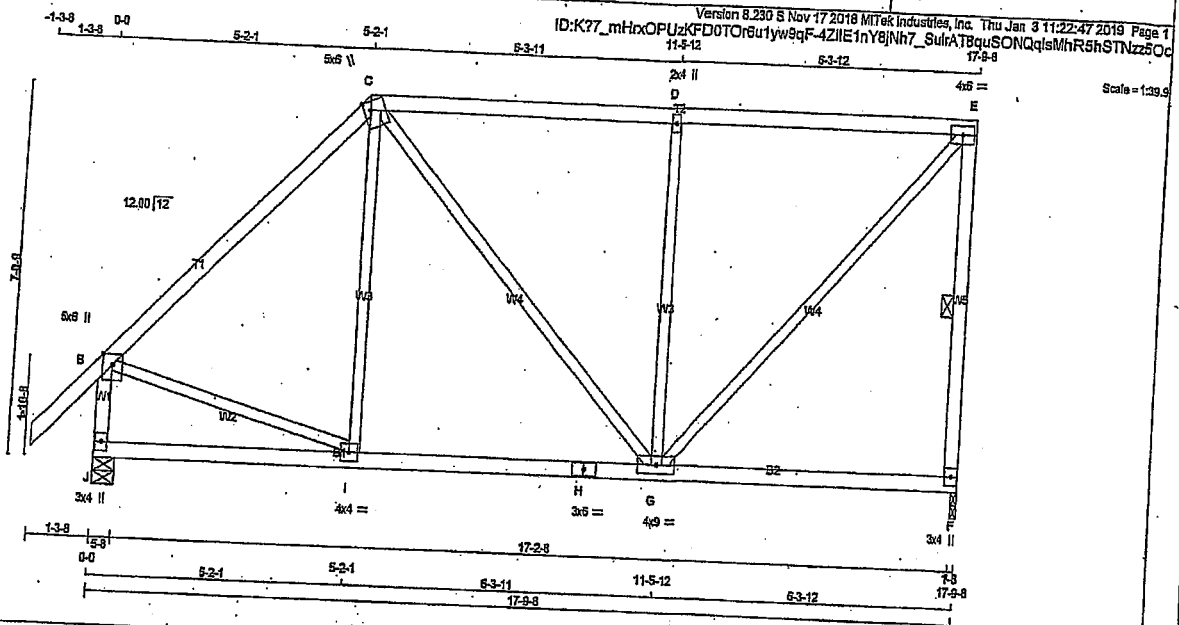


DWG NO. TAM 71900040
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
300536	T4	1	1			

Tamarack Roof Truss, Burlington

Version 8.230 5 Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:22:47 2019 Page 1
ID:K77_mHxOPuzKFD0TOr6utyw8qF-4ZIE1nY8JNh7_SulrAT8quSONQqIsMhRshSTNzz8Oc
17-9-8



Scale = 1/32" = 1'-0"

TOTAL WEIGHT = 83 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS EXCEPT 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	5.0	6.0	Edge	
C TTW+m	MT20	5.0	6.0	2.00	1.50
D TMW+w	MT20	2.0	4.0		
E TMW+t	MT20	4.0	6.0		
F BMV1+p	MT20	3.0	4.0		
G BMWVW+t	MT20	4.0	9.0		
H BS-t	MT20	3.0	6.0		
I BMWVW-t	MT20	4.0	4.0		
J BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	1097	0	1097	0	1-8	1-8
J	1216	0	1216	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM/LIVE			
F	832	414/0	187/0	0/0	0/0	231/0	0/0
J	914	479/0	187/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS		MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
		VERT. LOAD (LBS)	FACTORED (PLF)	FACTORED (PLF)	UNBRACED LENGTH (FT)			
FR-TO					FR-TO			
A-B	0/142				I-C	0/204	0.05 (3)	
B-C	-875/0	-84.9	-84.9 0.12 (1)	10.00	C-G	0/189	0.04 (1)	
C-D	-745/0	-84.9	-84.9 0.60 (1)	5.88	G-D	-682/0	0.57 (1)	
D-E	-745/0	-84.9	-84.9 0.60 (1)	5.88	G-E	0/1060	0.25 (1)	
E-F	-989/0	0.0	0.0 0.22 (1)	6.25	B-I	0/642	0.14 (1)	
F-G	-1136/0	0.0	0.0 0.13 (1)	7.45				
J-I	0/0	-38.5	-38.5 0.21 (3)	10.00				
I-H	0/618	-38.5	-38.5 0.36 (2)	10.00				
H-G	0/618	-38.5	-38.5 0.36 (2)	10.00				
G-F	0/0	-38.5	-38.5 0.29 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TFC 2011, TFC 2014

(55% OF 27.2 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.59")
CALCULATED VERT. DEFL. (LL) = L/999 (0.06")
ALLOWABLE DEFL. (TL) = L/360 (0.59")
CALCULATED VERT. DEFL. (TL) = L/999 (0.11")

CSI: TC=0.60/1.00 (D-E-1), BC=0.36/1.00 (G-1,2), WB=0.57/1.00 (D-G-1), SS=0.26/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

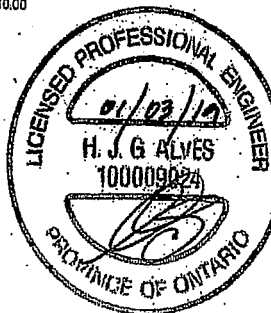
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
- MAX MIN MAX MIN MAX MIN
MT20 618 394 1687 738 1987 1555

PLATE PLACEMENT TOL. = 0.250 inches

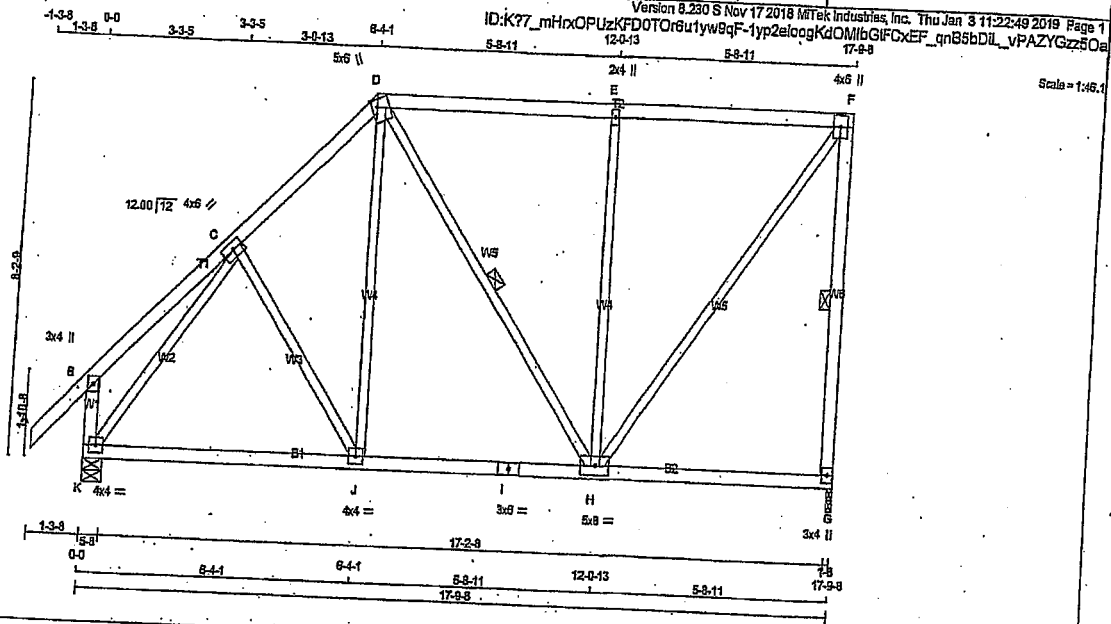
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.81 (3) (INPUT = 0.90)
JSI METAL = 0.83 (3) (INPUT = 1.00)



DRWG NO. TAM 71900042
STRUCTURAL
COMPONENT ONLY

JOB NAME 300536	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS		No.2	
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+u	MT20	4.0	6.0	
D	TTWW+m	MT20	5.0	6.0	2.00 1.50
E	TMVW+u	MT20	2.0	4.0	
F	TMVW+p	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BRVWW+u	MT20	5.0	8.0	
I	BS+u	MT20	3.0	6.0	
J	BMVW+u	MT20	4.0	4.0	
K	BMVW1+u	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1097	0	1097	0	1-8	1-8	
K	1216	0	1216	0	5-8	5-8	

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS		1ST LCASE		COMBINED	
JT	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
G	832	4147/0	187/0	0/0	0/0	231/0	0/0
K	914	4791/0	187/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CS (LC)
FR-TO		FROM	TO
A-B	0/42	-84.9	-84.9 0.12 (1)
B-C	0/21	-84.9	-84.9 0.13 (1)
C-D	-849/0	-84.9	-84.9 0.14 (1)
D-E	-849/0	-84.9	-84.9 0.48 (1)
E-F	-849/0	-84.9	-84.9 0.48 (1)
G-F	-1008/0	0.0	0.0 0.31 (1)
K-B	-221/0	0.0	0.0 0.02 (1)
K-J	6/600	-38.5	-38.5 0.34 (2)
J-I	0/587	-38.5	-38.5 0.34 (2)
I-H	0/587	-38.5	-38.5 0.34 (2)
H-G	0/0	-38.5	-38.5 0.21 (3)

WEBS		FACTORED	
MEMB.	FORCE (LBS)	MAX. CS (LC)	MAX. UNBRACED LENGTH (FT)
C-J	-29/57	0.02 (1)	10.00
J-D	0/322	0.07 (2)	10.00
D-H	-24/16	0.02 (3)	6.25
H-E	-600/0	0.79 (1)	6.25
H-F	0/1013	0.23 (1)	6.24
K-C	-1076/0	0.58 (1)	7.81

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBC 2012
- CSA 086-09, CSA 086-14
- TPO 2011, TPO 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.59")
CALCULATED VERT. DEF. (LL) = L/999 (0.07")
ALLOWABLE DEF. (TL) = L/360 (0.59")
CALCULATED VERT. DEF. (TL) = L/999 (0.12")

CSI: TC=0.48/1.00 (E-F-1), EC=0.34/1.00 (H-I-2), WS=0.79/1.00 (E-F-1), SS=0.24/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

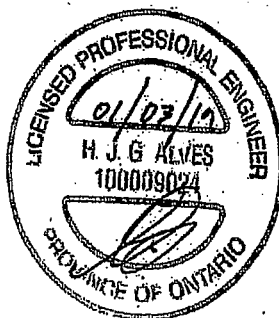
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

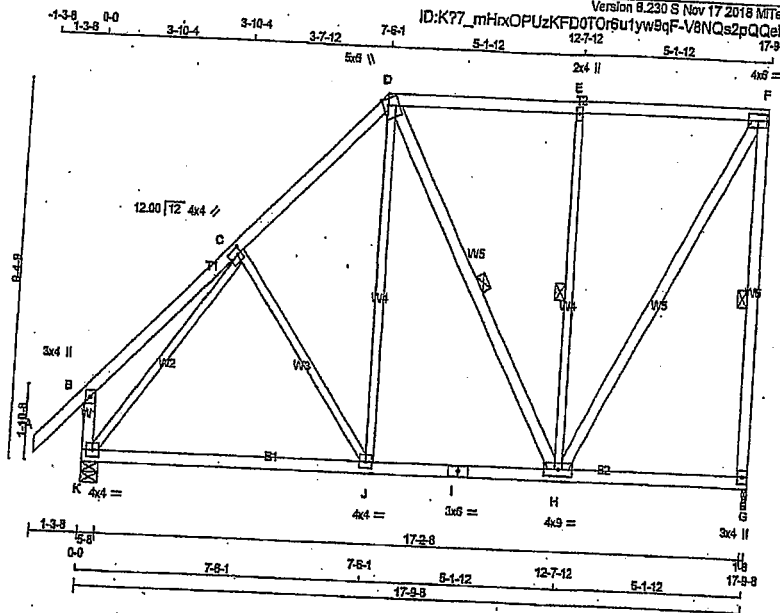
ISI GRIP = 0.82 (K) (INPUT = 0.90)
ISI METAL = 0.27 (F) (INPUT = 1.00)



DRWG NO. TAM 71900044
STRUCTURAL
COMPONENT ONLY

JOB NAME 300536	TRUSS NAME T6	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
---------------------------	-------------------------	----------------------	-----------------	---------------------------------	----------

Tamarack Roof Truss, Burlington



ID:K77_mHixOPUzKFD0T0r6u1ywsqF-V8Nqs2pQqelf RATQzAmSW0vbQUyA7773y64izzOZ
Version 9.230 S Nov 17 2018 MTek Industries, Inc. Thu Jan 3 11:22:50 2019 Page 1
Scale = 1/8" = 1'-0"

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
G - F	2x4	DRY No.2
K - B	2x4	DRY No.2
K - J	2x4	DRY No.2
I - G	2x4	DRY No.2
ALL WEBS EXCEPT	2x3	DRY No.2
D - H	2x4	DRY No.2
H - F	2x4	DRY No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

SPF		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
SPF	G	1097	0	1097	0	1-8	1-8
SPF	K	1218	0	1218	0	5-8	5-8

SPF

UNFACTORED REACTIONS							
		1ST LOOSE		MAX. MIN. COMPONENT REACTIONS			
SPF	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
SPF	G	832	414/0	187/0	0/0	0/0	231/0
	K	914	479/0	187/0	0/0	0/0	248/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.59")
CALCULATED VERT. DEFL. (LL) = L/959 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (0.59")
CALCULATED VERT. DEFL. (TL) = L/957 (0.22")

CSL TC=0.42/1.00 (F-G-1), BC=0.42/1.00 (H-K-2), WB=0.76/1.00 (C-K-1), SS=0.21/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.83 (C) (INPUT = 0.90)
JSI METAL=0.39 (C) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMAV-p	MT20	3.0	4.0		
C	TMAVW-1	MT20	4.0	4.0	2.00	1.25
D	TTWWV-m	MT20	5.0	6.0	2.00	1.50
E	TMAVW-1	MT20	2.0	4.0		
F	TMAVW-1	MT20	4.0	6.0		
G	BMV1-p	MT20	3.0	4.0		
H	BMVWW-1	MT20	4.0	6.0		
I	BS-1	MT20	3.0	6.0		
J	BMVWW-1	MT20	4.0	4.0		
K	BMVWW-1	MT20	4.0	4.0		

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

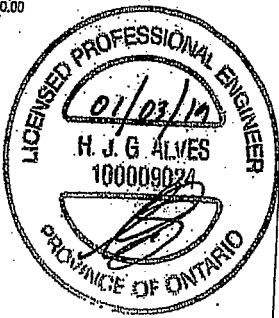
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

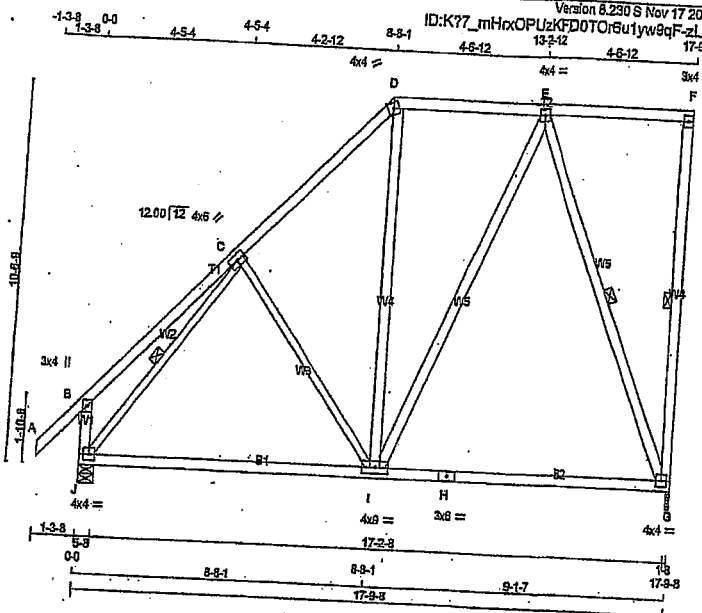
TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX		MAX. UNBRAC. LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC. LENGTH FR-TO
		FROM	TO				
A-B	0/42	-84.9	-84.9	0.12 (1)	10.00	C-J	-105/32
B-C	0/26	-84.9	-84.9	0.18 (1)	10.00	D-H	0/413
C-D	-795/0	-84.9	-84.9	0.29 (1)	6.25	D-H	-145/0
D-E	-481/0	-84.9	-84.9	0.39 (1)	6.25	H-E	-539/0
E-F	-481/0	-84.9	-84.9	0.38 (1)	6.25	H-F	0/971
G-F	-1011/0	0.0	0.0	0.42 (1)	6.25	K-C	-1049/0
K-B	-239/0	0.0	0.0	0.03 (1)	7.81		
K-J	0/607	-38.5	-38.5	0.42 (2)	10.00		
J-I	0/549	-38.5	-38.5	0.42 (2)	10.00		
I-H	0/549	-38.5	-38.5	0.42 (2)	10.00		
H-G	0/0	-38.5	-38.5	0.17 (3)	10.00		



DRWG NO. TAM **T1900045**
STRUCTURAL
COMPONENT ONLY

JOB NAME 300536 Tamarack Roof Truss, Burlington	TRUSS NAME T7	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
--	-------------------------	----------------------	-----------------	---------------------------------	----------



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	NO.2
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2

ALL WEBS 2x4 DRY No.2

EXCEPT

C - I 2x3 DRY No.2

J - C 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMA+p	MT20	3.0	4.0		
C TMAWW-t	MT20	4.0	6.0		
D TTVW-t	MT20	4.0	4.0		Edge
E TMAWW-t	MT20	4.0	4.0		
F TMA+p	MT20	3.0	4.0		
G BIAWW-t	MT20	4.0	4.0		
H BS-t	MT20	3.0	6.0		
I BIAWW-t	MT20	4.0	9.0		
J BIAWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UP/LT	IN-SX
G	1097	0	1-8	1-8
J	1216	0	5-8	5-8

UNFACTORED REACTIONS

SPF	1ST LCASE	MAX MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
G	832	414/0 187/0 0/0 0/0 231/0 0/0
J	914	479/0 187/0 0/0 0/0 248/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G, C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0/42	-84.9	0.12 (1)	10.00	C-I	-172/14	0.18 (1)
B-C	0/31	-84.9	0.28 (1)	10.00	I-D	0/280	0.04 (2)
C-D	-793/0	-84.9	0.28 (1)	6.25	E-E	0/397	0.06 (2)
D-E	-512/0	-84.9	0.30 (1)	6.25	E-G	-893/0	0.62 (1)
E-F	0/0	-84.9	0.29 (1)	10.00	J-C	-1034/0	0.36 (1)
G-F	-148/0	0.0	0.08 (1)	6.25			
J-B	-256/0	0.0	0.03 (1)	7.81			
J-I	0/614	-38.5	0.74 (2)	10.00			
I-H	0/355	-38.5	0.72 (3)	10.00			
H-G	0/355	-38.5	0.72 (3)	10.00			

TOTAL WEIGHT = 108 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010; NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, CBC 2012
- CSA 088-09, CSA 088-14
- TFC 2011, TFC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.59")
CALCULATED VERT. DEFL. (LL) = L/699 (0.16")
ALLOWABLE DEFL. (TL) = L/360 (0.59")
CALCULATED VERT. DEFL. (TL) = L/693 (0.31")

CSI: TC=0.30/1.00 (D-E-1), BC=0.74/1.00 (I-J-2), WB=0.62/1.00 (E-G-1), SS=0.22/1.00 (G-I-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

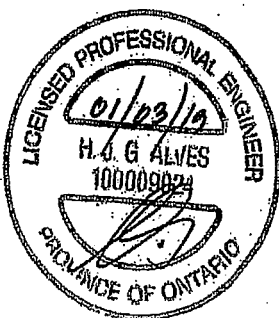
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (FS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 768 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

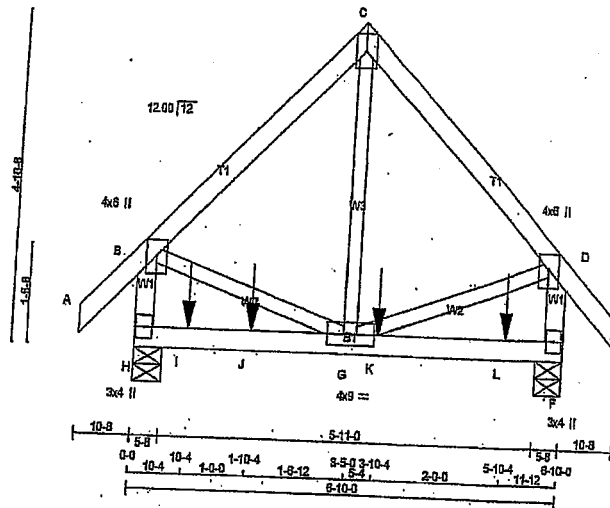
JSI GRIP=0.80 (J) (INPUT = 0.80)
JSI METAL=0.25 (C) (INPUT = 1.00)



DRWG NO. TAM 71900046
STRUCTURAL
COMPONENT ONLY

JOB NAME 300536	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. Tamarack Roof Truss, Burlington	DRWG NO.
---------------------------	--------------------------	----------------------	-----------------	---	----------

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:28:37 2019 Page 1
 ID:K??_mHxOPuzKFDOTOr6u1yw9qF-4STHHDkjh1VJ_ohmxXcmHbFtJepNF1j79tzzz5IC
 Scale = 1:31.0



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Inches in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTVW+p	MT20	4.0	6.0		
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMVW+p	MT20	3.0	4.0		
G	BMVWVW+p	MT20	4.0	9.0		
H	BMVW+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
H	1186	0	1186	0	5-8
F	1070	0	1070	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	862	493/0	157/0	0/0	0/0	232/0	0/0
F	786	444/0	142/0	0/0	0/0	209/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. (LC)	MEMB.	MAX. FACTORED (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0/29	-84.9	-84.9 0.07 (1)	G-C	0/633
B-C	-653/0	-84.9	-84.9 0.20 (1)	B-G	0/485
C-D	-653/0	-84.9	-84.9 0.20 (1)	G-D	0/485
D-E	0/29	-84.9	-84.9 0.07 (1)		
H-B	-839/0	0.0	0.0 0.10 (1)		
F-D	-839/0	0.0	0.0 0.10 (1)		
H-I	0/0	-38.5	-38.5 0.49 (1)		
I-J	0/0	-38.5	-38.5 0.49 (1)		
J-G	0/0	-38.5	-38.5 0.49 (1)		
G-K	0/0	-38.5	-38.5 0.49 (1)		
K-L	0/0	-38.5	-38.5 0.49 (1)		
L-F	0/0	-38.5	-38.5 0.49 (1)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	10-4	-314	-314		FRONT	VERT.	TOTAL		
J	1-10-4	-311	-311		FRONT	VERT.	TOTAL		
K	3-10-4	-311	-311		FRONT	VERT.	TOTAL		
L	5-10-4	-311	-311		FRONT	VERT.	TOTAL		

TOTAL WEIGHT = 33 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.8 PSF

SPACING = 240 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2010, OSC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.23")
 CALCULATED VERT. DEFL. (LL) = L/699 (0.03")
 ALLOWABLE DEFL. (TL) = L/360 (0.23")
 CALCULATED VERT. DEFL. (TL) = L/699 (0.06")

CSI: TC=0.20/1.00 (C-D:1), BC=0.49/1.00 (G-H:1), VB=0.18/1.00 (C-G:1), SS=0.45/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

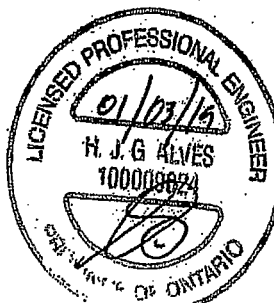
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1667 1668

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

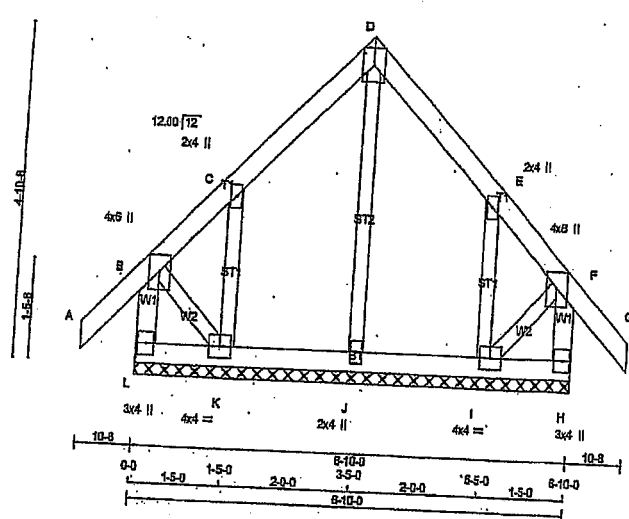
JSI GRIP = 0.59 (D) (INPUT = 0.80)
 JSI METAL = 0.30 (D) (INPUT = 1.00)



DRWG NO. TAM T1900062
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 300536 Tamarack Roof Truss, Burlington	TRUSS NAME G12	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
--	--------------------------	----------------------	-----------------	---------------------------------	----------

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:24:38 2019 Page 1
 ID:K77_mHxOPUzKF0TQr8u1yw9qF-KwvFX_7A?Qmox2RDIfgV8E_Zs7NyGB30zs4qsRzz5M
 Scale = 1:30.6



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESC
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3			
ALL GABLE WEBS		DRY	No.2	SPF
	2x3	DRY		
DRY, SEASONED LUMBER			No.2	SPF
GABLE STUDS SPACED AT 2'-0" O.C.				

DESIGNER
BUILDING DESIGNER
BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
 THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6'-25" FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10'-00" FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED (LBS)	MEMB.	MAX. FACTORED (LBS)
FR-TO				FR-TO	
A-B	0/29	-84.9	-84.9 0.08 (1)	J-D	-118/0
B-C	-40/0	-84.9	-84.9 0.08 (1)	K-C	-142/0
C-D	-33/0	-84.9	-84.9 0.08 (1)	L-E	-142/0
D-E	-33/0	-84.9	-84.9 0.08 (1)	B-K	0/29
E-F	-40/0	-84.9	-84.9 0.08 (1)	L-F	0/29
F-G	0/29	-84.9	-84.9 0.08 (1)		
L-B	-190/0	0.0	0.0 0.02 (1)		
H-F	-190/0	0.0	0.0 0.02 (1)		
L-K	0/0	-38.5	-38.5 0.02 (3)		
K-J	0/17	-38.5	-38.5 0.03 (3)		
J-I	0/17	-38.5	-38.5 0.03 (3)		
I-H	0/0	-38.5	-38.5 0.02 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCQ 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 OVERHANG NOT TO BE ALTERED OR CUT OFF.
 (5% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

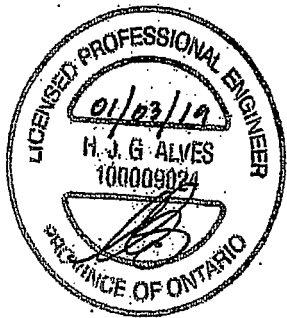
CSI: TO=0.08/1.00 (A-B:1), BC=0.03/1.00 (J-K:1), WB=0.04/1.00 (D-E:1), SF=0.03/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	4.0	6.0	2.75	2.00
C TMW+p	MT20	2.0	4.0		
D TMW+p	MT20	4.0	6.0		
E TMW+p	MT20	2.0	4.0		
F TMW+p	MT20	4.0	6.0	2.75	2.00
H BMW+p	MT20	3.0	4.0		
I BMW+p	MT20	4.0	4.0		
J BMW+p	MT20	2.0	4.0		
K BMW+p	MT20	4.0	4.0		
L BMW+p	MT20	3.0	4.0		



TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	610	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

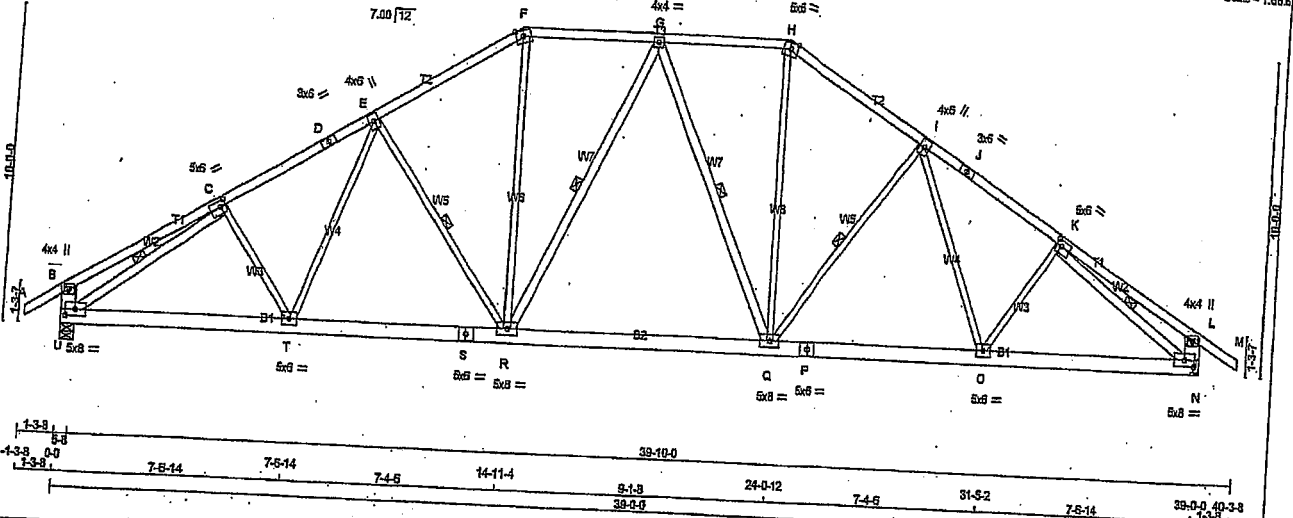
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.13 (F) (INPUT = 0.90)
 JSI METAL = 0.07 (C) (INPUT = 1.00)

DRWG NO. TAM T1900025
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 300539	TRUSS NAME T13	QUANTITY 6	PLY 1	JOB DESC. Tamarack Roof Truss, Burlington	TRUSS DESC.	DRWG NO.
--------------------	-------------------	---------------	----------	--	-------------	----------

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 12:17:49 2019 Page 1
 ID:K77_mHxOPUzKFD0TOr6u1yw9qF-PpDaF5kmWGOAGRWBjPoz9mPRLZQDZKqZAN_zz4b0
 19-8-0 4-5-12 24-0-12 4-10-15 33-10-8 5-1-7 39-0-0 40-3-8
 1-3-8 0-0 1-3-8 5-1-7 5-1-7 4-10-15 10-0-5 4-10-15 14-11-4 4-5-12 4-5-12 24-0-12 4-10-15 33-10-8 5-1-7 39-0-0 40-3-8
 Scale = 1:600



TOTAL WEIGHT = 6 X 209 = 1252 lb

LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - M	2x4	DRY	No.2
U - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
U - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
ALL WEBS EXCEPT			
R - G	2x4	DRY	No.2
U - Q	2x4	DRY	No.2
K - N	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMW+P	MT20	4.0	4.0
C	TMW+P	MT20	5.0	6.0 2.25 2.00
D	TS-1	MT20	3.0	6.0
E	TMW+P	MT20	4.0	6.0
F	TMW+P	MT20	5.0	6.0 Edge
G	TMW+P	MT20	4.0	4.0
H	TMW+P	MT20	5.0	6.0 Edge
I	TS-1	MT20	4.0	6.0
J	TMW+P	MT20	3.0	6.0
K	TMW+P	MT20	5.0	6.0 2.25 2.00
L	TMW+P	MT20	4.0	4.0
N	BMW+P	MT20	5.0	8.0 2.50 3.75
O	BMW+P	MT20	5.0	6.0
P	BS-1	MT20	5.0	6.0
Q	BMW+P	MT20	5.0	8.0
R	BMW+P	MT20	5.0	8.0
S	BS-1	MT20	5.0	6.0
T	BMW+P	MT20	5.0	6.0
U	BMW+P	MT20	5.0	8.0 2.50 3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG
U	2521	0	2521	0	0	5-8	5-8
N	2521	0	2521	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS						
SPF	JT	1ST LGASE COMBINED	SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL
SPF	U	1805	972/0	410/0	0/0	0/0	523/0	0/0
SPF	N	1805	972/0	410/0	0/0	0/0	523/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.64 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, G-R, G-Q, H-Q, C-U, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO		
A-B	0/29	-84.9	-84.9 0.11 (1)	10.00	C-T	0/147	0.03 (3)
B-C	0/19	-84.9	-84.9 0.26 (1)	10.00	T-E	0/230	0.07 (2)
C-D	-3216/0	-84.9	-84.9 0.39 (1)	3.64	E-R	-598/0	0.28 (1)
D-E	-3216/0	-84.9	-84.9 0.39 (1)	3.64	R-F	0/1036	0.23 (1)
E-F	-2711/0	-84.9	-84.9 0.35 (1)	3.64	G-Q	-255/0	0.15 (1)
F-G	-2333/0	-84.9	-84.9 0.28 (1)	4.25	Q-H	-255/0	0.15 (1)
G-H	-2333/0	-84.9	-84.9 0.28 (1)	4.25	H-I	0/1036	0.23 (1)
H-I	-2711/0	-84.9	-84.9 0.35 (1)	3.64	I-J	-598/0	0.28 (1)
I-J	-3216/0	-84.9	-84.9 0.39 (1)	3.64	J-K	0/330	0.07 (2)
J-K	-3216/0	-84.9	-84.9 0.39 (1)	3.64	K-L	0/147	0.03 (3)
K-L	0/19	-84.9	-84.9 0.26 (1)	10.00	L-M	-3472/0	0.64 (1)
L-M	0/29	-84.9	-84.9 0.11 (1)	10.00	M-N	-3472/0	0.64 (1)
M-N	-290/0	0.0	0.0 0.02 (1)	7.81	N-L	-290/0	0.02 (1)
N-L	-290/0	0.0	0.0 0.02 (1)	7.81			
U-T	0/2783	-38.5	-38.5 0.47 (1)	10.00			
T-S	0/2681	-38.5	-38.5 0.47 (2)	10.00			
S-R	0/2681	-38.5	-38.5 0.47 (2)	10.00			
R-Q	0/2440	-38.5	-38.5 0.44 (2)	10.00			
Q-P	0/2681	-38.5	-38.5 0.47 (2)	10.00			
P-O	0/2681	-38.5	-38.5 0.47 (2)	10.00			
O-N	0/2783	-38.5	-38.5 0.47 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 48.8 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012
 - CSA 086-09, CSA 086-14
 - TRC 2011, TRC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.30")
 CALCULATED VERT. DEFL. (LL) = $L/999$ (0.15")
 ALLOWABLE DEFL. (TL) = $L/360$ (1.30")
 CALCULATED VERT. DEFL. (TL) = $L/599$ (0.26")

CS1: TO=0.39/1.00 (K-K1), BC=0.47/1.00 (C-Q2), WB=0.64/1.00 (K-K1), SS=0.19/1.00 (G-H1)

DGL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

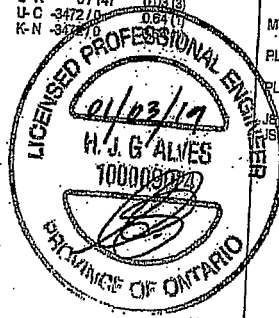
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1697 788 1697 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

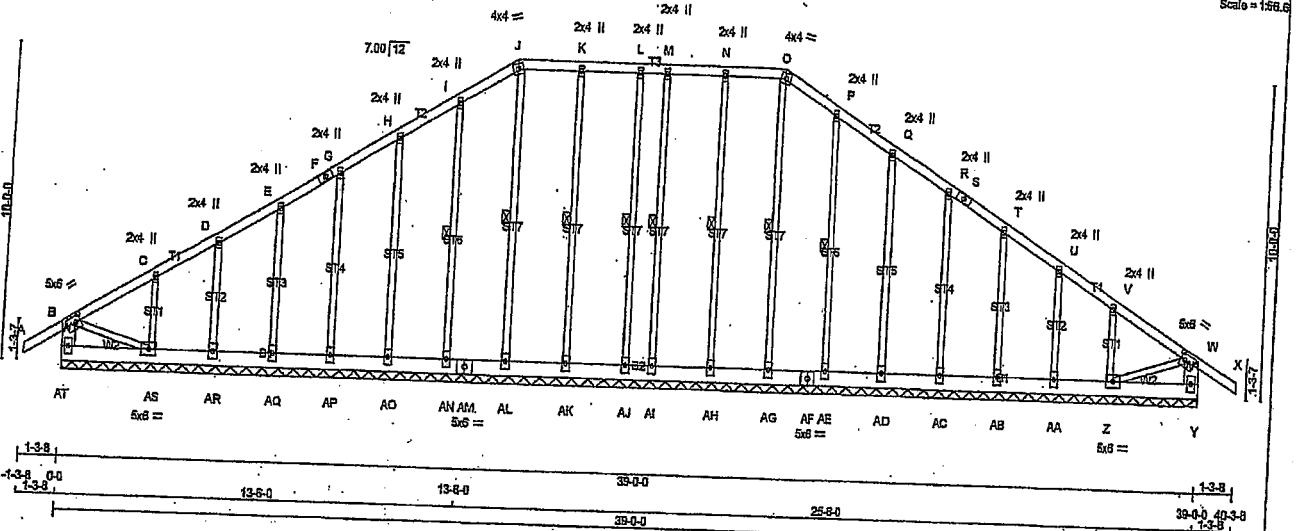
GRIP=0.80 (K) (INPUT=0.80)
 METAL=0.86 (K) (INPUT=1.00)



DRWG NO. TAM T19 000 66
 STRUCTURAL
 COMPANY

JOB NAME 300539 Tamarack Roof Truss, Burlington	TRUSS NAME G13	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
--	--------------------------	----------------------	-----------------	--------------------------	----------

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 12:18:35 2019 Page 1
ID:K77_mHoxOPUzKF0D0T0r6u1ywsqF-2g7zvqldgOBenVOLt_EarsZk7_9BB8JUU_b_Kzz4a1
39-0-0 40-3-8
1-3-8 1-3-8
Scale = 1/8"=1'-0"



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
J - O	2x4	DRY	No.2
O - S	2x4	DRY	No.2
S - X	2x4	DRY	No.2
AT - B	2x6	DRY	No.2
Y - W	2x6	DRY	No.2
AT - AM	2x6	DRY	No.2
AM - AF	2x6	DRY	No.2
AF - Y	2x6	DRY	No.2
ALL WEBS	2x8	DRY	No.2
ALL GABLE WEBS	2x8	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-00 O.C.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMW-4	MT20	5.0	6.0	2.50	2.00
C, D, E, G, H, I, K, L, M, N, P, Q, R, T, U, V					
F TS-1	MT20	2.0	4.0		
J TTW-m	MT20	3.0	6.0		
O TTW-m	MT20	4.0	4.0		
S TS-1	MT20	3.0	6.0		
W TMW-1	MT20	5.0	6.0	2.50	2.00
Y BMV1-p	MT20	3.0	6.0		
Z BMWV1-t	MT20	5.0	6.0		
AA, AB, AC, AD, AE, AG, AH, AI, AJ, AK, AL, AN, AO, AP, AQ, AR					
AA BMV1-w	MT20	3.0	6.0		
AF BS-1	MT20	5.0	6.0		
AM BS-1	MT20	5.0	6.0		
AS BMWV1-t	MT20	5.0	6.0		
AT BMV1-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-AJ, K-AK, J-AL, I-AN, N-AH, M-AI, O-AG, P-AE.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0/29	-84.9	-84.9 0.11 (1)	10.00	AJ-L	-117/0	0.08 (1)
B-C	-26/0	-84.9	-84.9 0.09 (1)	6.25	AK-K	-198/0	0.13 (1)
C-D	-41/0	-84.9	-84.9 0.09 (1)	6.25	AL-J	-131/0	0.09 (1)
D-E	-23/0	-84.9	-84.9 0.04 (1)	6.25	AN-I	-166/0	0.09 (1)
E-F	-24/0	-84.9	-84.9 0.04 (1)	6.25	AO-H	-166/0	0.17 (1)
F-G	-24/0	-84.9	-84.9 0.04 (1)	6.25	AP-G	-169/0	0.11 (1)
G-H	-21/0	-84.9	-84.9 0.04 (1)	6.25	AQ-E	-174/0	0.07 (1)
H-I	-16/0	-84.9	-84.9 0.04 (1)	6.25	AR-D	-139/0	0.04 (1)
I-J	-25/0	-84.9	-84.9 0.04 (1)	6.25	AS-C	-254/0	0.04 (1)
J-K	-13/0	-84.9	-84.9 0.05 (1)	6.25	AH-N	-198/0	0.13 (1)
K-L	-13/0	-84.9	-84.9 0.05 (1)	6.25	AI-M	-117/0	0.08 (1)
L-M	-13/0	-84.9	-84.9 0.03 (1)	6.25	AG-O	-131/0	0.09 (1)
M-N	-13/0	-84.9	-84.9 0.05 (1)	6.25	AP-P	-169/0	0.09 (1)
N-O	-13/0	-84.9	-84.9 0.05 (1)	6.25	AR-Q	-169/0	0.17 (1)
O-P	-25/0	-84.9	-84.9 0.04 (1)	6.25	AS-R	-169/0	0.11 (1)
P-Q	-18/0	-84.9	-84.9 0.04 (1)	6.25	AB-T	-174/0	0.07 (1)
Q-R	-21/0	-84.9	-84.9 0.04 (1)	6.25	AA-U	-139/0	0.04 (1)
R-S	-24/0	-84.9	-84.9 0.04 (1)	6.25	Z-V	-254/0	0.04 (1)
S-T	-24/0	-84.9	-84.9 0.04 (1)	6.25	BA-S	0/34	0.01 (1)
T-U	-23/0	-84.9	-84.9 0.04 (1)	6.25	Z-W	0/34	0.01 (1)
U-V	-41/0	-84.9	-84.9 0.09 (1)	6.25			
V-W	-26/0	-84.9	-84.9 0.09 (1)	6.25			
W-X	0/29	-84.9	-84.9 0.11 (1)	10.00			
AT-B	-247/0	0.0	0.0 0.02 (1)	7.81			
Y-W	-247/0	0.0	0.0 0.02 (1)	7.81			
AT-AS	0/0	-38.5	-38.5 0.03 (3)	10.00			
AS-AQ	0/25	-38.5	-38.5 0.03 (2)	10.00			
AR-AQ	0/22	-38.5	-38.5 0.01 (2)	10.00			
AQ-AP	0/20	-38.5	-38.5 0.01 (2)	10.00			
AP-AO	0/18	-38.5	-38.5 0.01 (2)	10.00			
AO-AN	0/16	-38.5	-38.5 0.01 (2)	10.00			
AN-AM	0/15	-38.5	-38.5 0.01 (2)	10.00			
AM-AL	0/15	-38.5	-38.5 0.01 (2)	10.00			
AL-AK	0/13	-38.5	-38.5 0.01 (2)	10.00			
AK-AJ	0/13	-38.5	-38.5 0.01 (2)	10.00			
AJ-AI	0/13	-38.5	-38.5 0.01 (2)	10.00			
AI-AH	0/13	-38.5	-38.5 0.01 (2)	10.00			
AH-AG	0/13	-38.5	-38.5 0.01 (2)	10.00			
AG-AF	0/15	-38.5	-38.5 0.01 (2)	10.00			
AF-AE	0/15	-38.5	-38.5 0.01 (2)	10.00			
AE-AD	0/16	-38.5	-38.5 0.01 (2)	10.00			
AD-AC	0/18	-38.5	-38.5 0.01 (2)	10.00			

LICENSED PROFESSIONAL

01/03/19

H. J. G. ALVES

100009004

OFFICE OF CIVIL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.11/1.00 (A-B-1), BC=0.09/1.00 (Z-AA-2), WB=0.17/1.00 (Q-AD-1), SS=0.10/1.00 (V-W-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

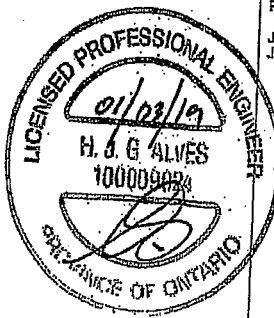
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1657 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT= 0.80)

JSI METAL= 0.12 (C) (INPUT= 1.00)



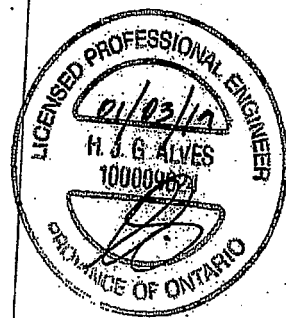
DRWG NO. TAM T1400060
STRUCTURAL
COMPONENTS 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Thu Jan 3 12:18:35 2019 Page 2
 ID:K77 mHixOPUzKFD0TOr6u1vw9qF-2q?zvqldqOBenVOLT EarsZk7 9BB6JUJ ic Kzz4ai

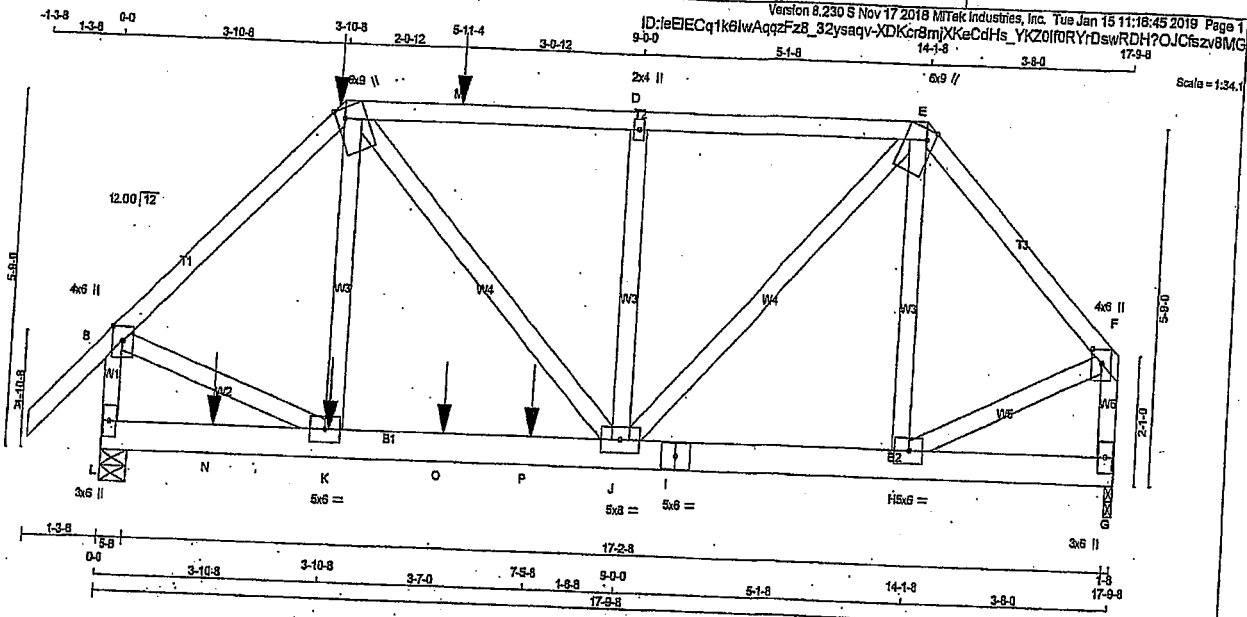
LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED UNBRAC LENGTH	FR-TO MAX CSI (LC)
FR-TO	0/20	-38.5	-38.5 0.01 (2)	10.00			
AB-AB	0/22	-38.5	-38.5 0.01 (2)	10.00			
AA-Z	0/25	-38.5	-38.5 0.03 (2)	10.00			
Z-Y	0/0	-38.5	-38.5 0.03 (3)	10.00			



DESIGNED BY TAM T1900060
 STRUCTURAL
 CHECKED BY 4/2

JOB NAME 300783	TRUSS NAME T26A	QUANTITY 1	PLY 2	JOB DESC: TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
L - I	2x6	DRY No.2	SPF
I - G	2x6	DRY No.2	SPF

ALL WEBS 2x4 DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	SIDE (61.0)
C-E 1	12	SIDE (61.0)
E-F 1	12	TOP
L-B 1	12	TOP
G-F 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-I 2	12	SIDE (183.1)
I-G 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT - TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75 2.00
C	TTVW+m	MT20	6.0	9.0	Edge 1.75
D	TMVW+h	MT20	2.0	4.0	
E	TTVW+m	MT20	6.0	9.0	Edge 1.75
F	TMVW+p	MT20	4.0	6.0	2.75 2.00
G	BMV1+p	MT20	3.0	6.0	
H	BMVW-t	MT20	5.0	6.0	
I	BS-t	MT20	5.0	6.0	
J	BMVW+t	MT20	5.0	6.0	
K	BMVW-t	MT20	5.0	6.0	
L	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2130	2130	0	0
L	0	2130	0	0
G	1615	1615	0	0

UNFACTORED REACTIONS

JT	1ST LOASE	MAX	MIN	COMPONENT REACTIONS
L	1591	864/0	300/0	PERM LIVE WIND DEAD SOIL
G	1214	636/0	249/0	0/0 0/0 427/0 0/0 331/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)
FR-TO				FR-TO	
A-B	0/42	-84.9	-84.9 0.07 (1)	10.00	K-C 0/257 0.03 (3)
B-C	-1911/0	-84.9	-84.9 0.14 (1)	6.17	C-J 0/777 0.07 (1)
C-M	-1887/0	-84.9	-84.9 0.25 (1)	6.04	J-E -560/0 0.10 (1)
M-D	-1887/0	-84.9	-84.9 0.25 (1)	6.04	E-H 0/1358 0.12 (1)
D-E	-1887/0	-84.9	-84.9 0.25 (1)	6.04	H-K -341/0 0.06 (1)
E-F	-1359/0	-84.9	-84.9 0.12 (1)	6.25	K-L 0/1422 0.06 (1)
L-B	-2121/0	0.0	0.0 0.12 (1)	7.61	L-H 0/1422 0.06 (1)
G-F	-1635/0	0.0	0.0 0.09 (1)	7.61	H-I 0/1422 0.06 (1)
L-N	0/0	-38.5	-38.5 0.13 (1)	10.00	
N-K	0/0	-38.5	-38.5 0.13 (1)	10.00	
K-O	0/1350	-38.5	-38.5 0.37 (1)	10.00	
O-P	0/1350	-38.5	-38.5 0.37 (1)	10.00	
P-J	0/1350	-38.5	-38.5 0.37 (1)	10.00	
J-I	0/1948	-38.5	-38.5 0.22 (1)	10.00	
I-H	0/1948	-38.5	-38.5 0.22 (1)	10.00	
H-G	0/0	-38.5	-38.5 0.03 (2)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.
C	3-10-8	-271	-271	-	FRONT	VERT
K	3-11-4	-36	-46	-	FRONT	VERT
M	5-11-4	-30	-80	-	FRONT	VERT
N	1-11-4	-36	-46	-	FRONT	VERT
O	5-11-4	-36	-46	-	FRONT	VERT
P	7-5-8	-972	-972	-	TOP	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBCC 2015
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.99")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.99")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSK TC=0.25/1.00 (C-D-1), BC=0.37/1.00 (J-K-1), WB=0.13/1.00 (B-K-1), SS=0.28/1.00 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	619 354 1867 788 1987 1656	MAX MIN MAX MIN MAX MIN

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

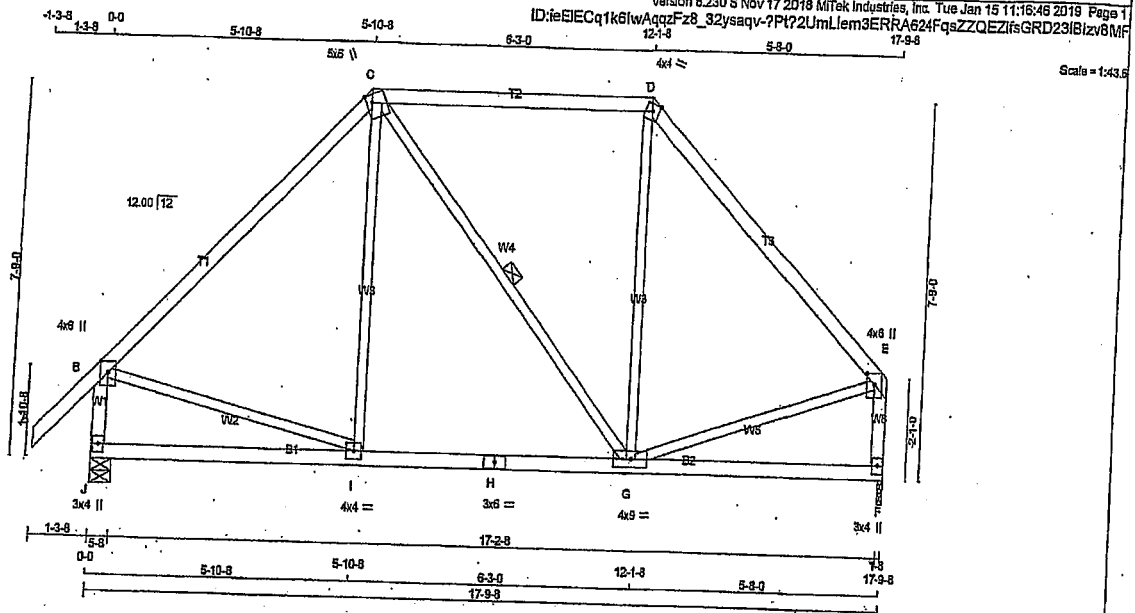
JSI GRIP = 0.74 (B) (INPUT = 0.90)
JSI METAL = 0.39 (B) (INPUT = 1.00)

DWG NO. TAM 17901093
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300783	T27A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Jan 15 11:16:46 2019 Page 1
ID:ieEIECq1k6lwAgqzFz8_32ysaqv-?P172UmlLem3ERRA624FqsZZQEzifsGRD23I8Izv8MF



Scale = 1/4\"/>

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS EXCEPT
2x3 DRY No.2
EXCEPT
SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMWW+p	MT20	4.0	6.0	2.75	2.00	
C TTWW+m	MT20	5.0	6.0	2.00	1.50	
D TTWW-m	MT20	4.0	4.0	Edge		
E TMWW+p	MT20	4.0	6.0	2.75	2.00	
F BMV1+p	MT20	3.0	4.0			
G BMWWW-t	MT20	4.0	9.0			
H BS-t	MT20	3.0	6.0			
I BMWW-t	MT20	4.0	4.0			
J BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORIZ	UPLIFT
J 1216	0	1216	0
F 1097	0	1097	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM.LIVE
J 914	479/0	187/0	0/0
F 832	414/0	187/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	UNBRAC	MEMB.	MAX. FACTORED
MEMB.	FORCE	(PLF)	(LBS)		CS1 (LC)	CS2 (LC)			FORCE
FR-TO		FROM	TO				LENGTH	FR-TO	
A-B	0/42	-84.9	-84.9	0.12 (1)	10.00	I-C	0/243	0.08 (3)	
B-C	-851/0	-84.9	-84.9	0.40 (1)	6.09	C-G	-18/0	0.01 (1)	
C-D	-580/0	-84.9	-84.9	0.43 (1)	6.25	G-D	0/230	0.05 (3)	
D-E	-834/0	-84.9	-84.9	0.37 (1)	6.18	B-I	0/819	0.14 (1)	
E-F	-1123/0	0.0	0.0	0.13 (1)	7.48	G-E	0/814	0.14 (1)	
F-G	-1009/0	0.0	0.0	0.12 (1)	7.79				
J-I	0/0	-38.5	-38.5	0.25 (3)	10.00				
I-H	0/601	-38.5	-38.5	0.32 (2)	10.00				
H-G	0/601	-38.5	-38.5	0.32 (2)	10.00				
G-F	0/0	-38.5	-38.5	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF CBC 2018, OBC 2012
-CSA 086-09, CSA 086-14
-TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CS: TC=0.43/1.00 (C-D:1), BC=0.32/1.00 (G-I:2), WB=0.14/1.00 (B-I:1), SS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

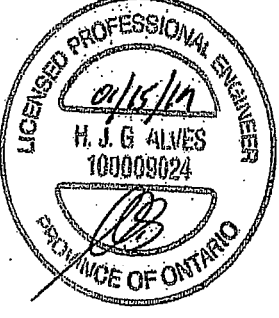
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1887 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)



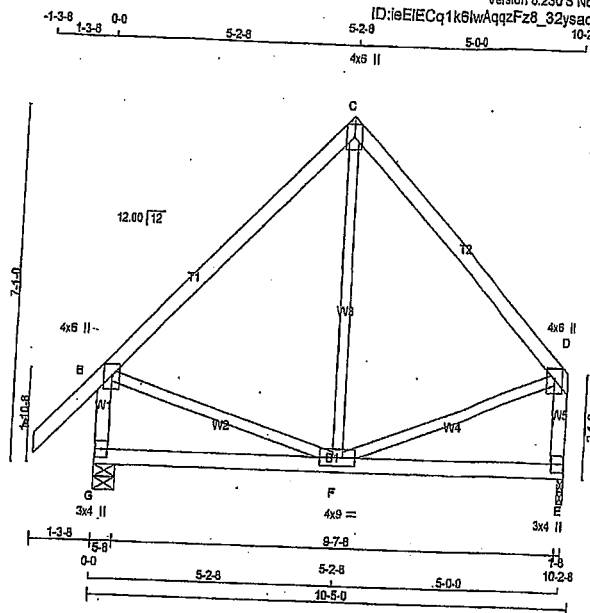
DWG NO. TAM T27A1094
STRUCTURAL
COMPONENT ONLY

JOB NAME 300799 Tamarack Roof Truss, Burlington	TRUSS NAME T29A	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
---	--------------------	---------------	----------	--------------------------	----------

Version 8.230.8 Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 16:51:10 2019 Page 1

ID:ieEIECq1k6lwaAqzFz8_32ysaqv-h6krS9DY2T12PvCeUJ9uCoReqn2R1fwo8??YVwszz0a

Scale = 1:42.3



LUMBER	N. L. G. A. RULES	SIZE	LUMBER
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE				
B TMVW+p	MT20	4.0	8.0	2.75 2.00
C TTW+p	MT20	4.0	8.0	
D TMVW+p	MT20	4.0	8.0	2.75 2.00
E BMV1+p	MT20	3.0	4.0	
F BMVWW+L	MT20	4.0	9.0	
G BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
G	VERT	DOWN	HORIZ	UPLIFT
G	748	0	748	0
E	630	0	630	0

UNFACTORED REACTIONS	1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
G	559	303/0 107/0 0/0 0/0 149/0 0/0
E	478	238/0 107/0 0/0 0/0 133/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1 MAX	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0/42	-84.9 -84.9 0.12 (1)	F-C	0/217
B-C	-363/0	-84.9 -84.9 0.30 (1)	B-F	0/267
C-D	-363/0	-84.9 -84.9 0.27 (1)	F-D	0/271
G-B	-671/0	0.0 0.0 0.07 (1)		
E-D	-558/0	0.0 0.0 0.07 (1)		
G-F	0/0	-38.5 -38.5 0.23 (3)		
F-E	0/0	-38.5 -38.5 0.23 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL: BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 089-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.30/1.00 (B-C:1), BC=0.23/1.00 (E-F:3), WB=0.08/1.00 (D-F:1), SSI=0.13/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

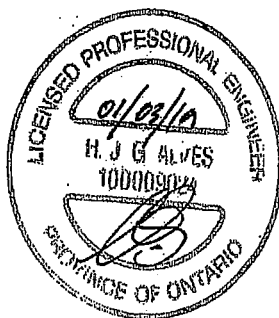
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)	
	MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 788	1667 1656	

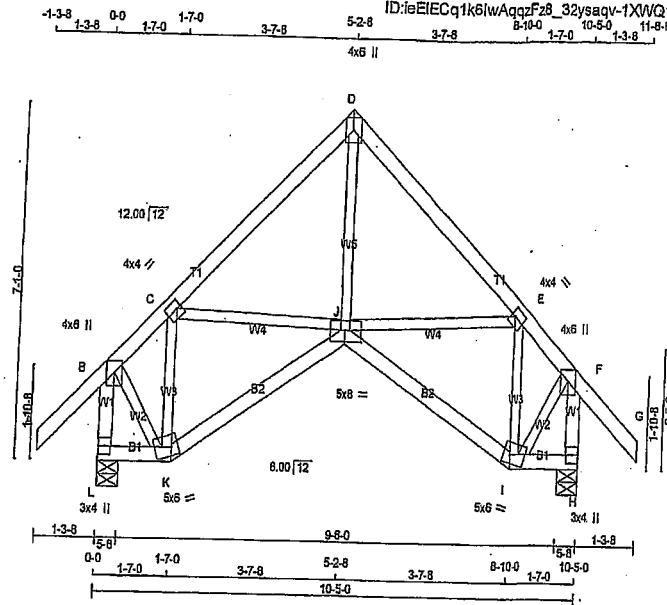
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.50)
JSI METAL= 0.22 (B) (INPUT = 1.00)



DRWG NO. TAM T1900119
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 55 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW-l	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0		
E	TMVW-l	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-m	MT20	5.0	6.0	2.00	1.75
J	BBWW-p	MT20	5.0	8.0	2.75	4.00
K	BBWW-m	MT20	5.0	6.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	761	0	5-8	5-8
H	761	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	569	308/0	109/0	0/0	0/0	152/0	0/0
H	569	308/0	109/0	0/0	0/0	152/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED
	FORCE	VERT. LOAD	MAX.		FORCE
	(LBS)	LOC1	CSI (LC)		(LBS)
FR-TO		(PLF)	UNBRAC	FR-TO	MAX
		FROM	LENGTH		CSI (LC)
A-B	0/42	-84.9	-84.9 0.12 (1)	J-D	0/473 0.11 (2)
B-C	-401/0	-84.9	-84.9 0.11 (1)	J-E	0/107 0.02 (1)
C-D	-543/0	-84.9	-84.9 0.15 (1)	I-E	-354/0 0.07 (1)
D-E	-543/0	-84.9	-84.9 0.15 (1)	C-J	0/107 0.02 (1)
E-F	-401/0	-84.9	-84.9 0.11 (1)	K-C	-354/0 0.07 (1)
F-G	0/42	-84.9	-84.9 0.12 (1)	B-K	0/401 0.09 (1)
L-B	-730/0	0.0	0.0 0.08 (1)	I-F	0/401 0.09 (1)
H-F	-730/0	0.0	0.0 0.08 (1)		
L-K	0/0	-38.5	-38.5 0.02 (3)		
K-J	0/319	-38.5	-38.5 0.16 (2)		
J-I	0/319	-38.5	-38.5 0.16 (2)		
I-H	0/0	-38.5	-38.5 0.02 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL =	6.0	PSF
BOT CH.	LL = 10.5	PSF
DL =	7.0	PSF
TOTAL LOAD	= 46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.35")
 CALCULATED VERT. DEFL. (LL) = $L/889$ (0.02")
 ALLOWABLE DEFL. (TL) = $L/360$ (0.35")
 CALCULATED VERT. DEFL. (TL) = $L/989$ (0.03")

CSI: TO=0.15/1.00 (C-D:1), BC=0.16/1.00 (I-J:2),
 WB=0.11/1.00 (D-J:2), SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

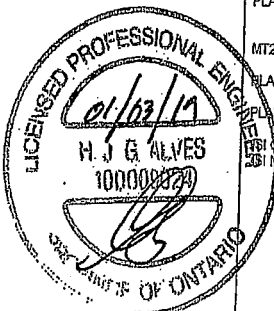
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
		786	1987
			1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRIP = 0.67 (K) (INPUT = 0.90)
 RETAL = 0.20 (F) (INPUT = 1.00)

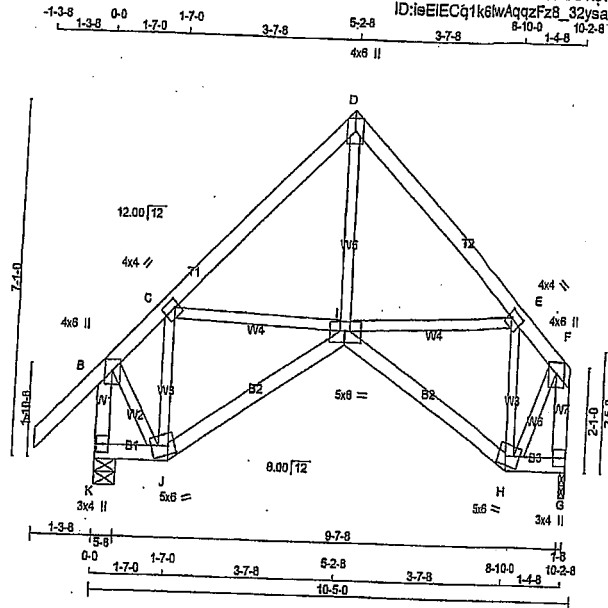


ENGINEER 71900150
 STRUCTURAL
 CALCULATED ONLY

JOB NAME 300799	TRUSS NAME T30A	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.230 8 Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:51:11 2019 Page 1
ID: IsEIECq1k6lwAgqzFzB_32ysaqv-9IIIDgVEApntv13nr11g7k?zruBPmRTVxkNel52lzzOak

Scale = 1/42.3



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY, SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
K	756	0	756	0	5-8	5-8	
G	622	0	622	0	1-8	1-8	

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	565	307/0	107/0	0/0	0/0	151/0	0/0
G	472	233/0	107/0	0/0	0/0	132/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
A-B	0/42	-84.9	-84.9 0.12 (1)	10.00	I-D	0/462	0.10 (2)
B-C	-398/0	-84.9	-84.9 0.11 (1)	6.25	I-E	0/112	0.03 (1)
C-D	-535/0	-84.9	-84.9 0.15 (1)	6.25	H-E	-432/0	0.08 (1)
D-E	-537/0	-84.9	-84.9 0.14 (1)	6.25	C-I	0/104	0.02 (1)
E-F	-336/0	-84.9	-84.9 0.12 (1)	6.25	J-C	-351/0	0.07 (1)
F-G	-726/0	0.0	0.0 0.08 (1)	7.81	B-J	0/397	0.09 (1)
G-F	-595/0	0.0	0.0 0.07 (1)	7.81	H-F	0/456	0.10 (1)
K-J	0/0	-38.5	-38.5 0.02 (3)	10.00			
J-I	0/316	-38.5	-38.5 0.16 (2)	10.00			
I-H	0/306	-38.5	-38.5 0.15 (2)	10.00			
H-G	0/0	-38.5	-38.5 0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.15/1.00 (C-D:1), BC=0.16/1.00 (I-J:2), WB=0.10/1.00 (D-E:2), SS=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

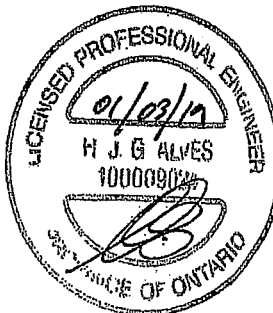
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 Inches

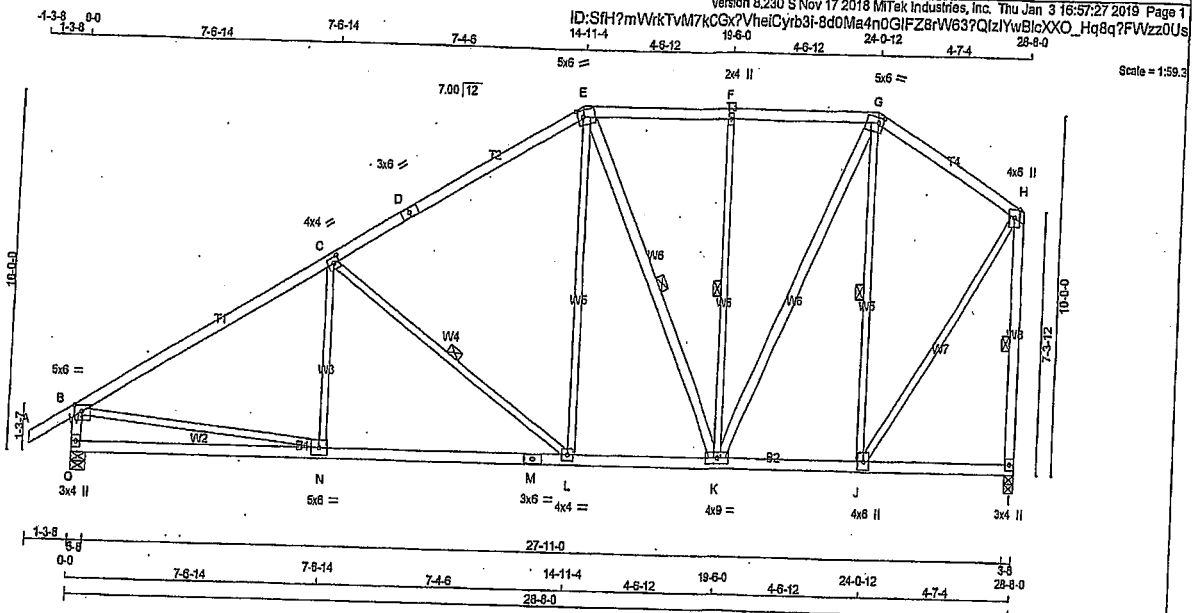
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.79 (H) (INPUT = 0.60)
JSI METAL = 0.19 (B) (INPUT = 1.00)



DRWG NO. TAM T1900120
STRUCTURAL
COMPONENT ONLY

JOB NAME 300800	TRUSS NAME T33	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - K	2x4	DRY	No.2
K - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN Y	X
B TMVW-p	MT20	5.0	6.0	2.00 2.75
C TMVW-t	MT20	4.0	4.0	2.00 1.75
D TS-t	MT20	3.0	6.0	
E TMVW-m	MT20	5.0	6.0	2.25 2.00
F TMVW-w	MT20	2.0	4.0	
G TMVW-m	MT20	5.0	6.0	1.75 1.75
H TMVW-p	MT20	4.0	6.0	Edge
I BMV-t+p	MT20	3.0	4.0	
J BMVW-t	MT20	4.0	6.0	
K BMVW-t	MT20	4.0	9.0	
L BMVW-t	MT20	4.0	4.0	
M BS-t	MT20	3.0	6.0	
N BMVW-t	MT20	5.0	6.0	
O BMV-t+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD BRG IN-SX
O	1884 0	1884 0	5-8
I	1768 0	1768 0	3-8

UNFACTORED REACTIONS						
JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
O	1421	731/0	301/0	0/0	0/0	389/0
I	1341	668/0	301/0	0/0	0/0	373/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-K, F-K, G-J, H-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	WEBS MAX. FACTORED FORCE (LBS)
FR-TO			
A-B	0/29	-84.9 -84.9 0.11 (1)	10.00 N-C
B-C	-2250/0	-84.9 -84.9 0.84 (1)	3.60 C-L
C-D	-1557/0	-84.9 -84.9 0.71 (1)	4.33 L-E
D-E	-1557/0	-84.9 -84.9 0.71 (1)	4.33 E-K
E-F	-1161/0	-84.9 -84.9 0.23 (1)	5.65 K-F
F-G	-1161/0	-84.9 -84.9 0.23 (1)	5.65 K-G
G-H	-840/0	-84.9 -84.9 0.24 (1)	6.25 J-G
C-B	-1768/0	0.0 0.0 0.18 (1)	6.27 B-N
I-H	-1697/0	0.0 0.0 0.42 (1)	5.10 J-H
O-N	0/0	-38.5 -38.5 0.43 (3)	10.00
N-M	0/1977	-38.5 -38.5 0.68 (2)	10.00
M-L	0/1977	-38.5 -38.5 0.68 (2)	10.00
L-K	0/1318	-38.5 -38.5 0.40 (2)	10.00
K-J	0/715	-38.5 -38.5 0.28 (2)	10.00
J-I	0/0	-38.5 -38.5 0.16 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSL: TC=0.84/1.00 (B-C:1), BC=0.68/1.00 (L-N:2), WB=0.59/1.00 (G-J:1), SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

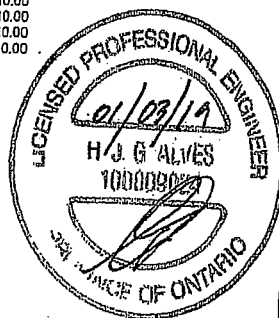
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (K) (INPUT=0.90)

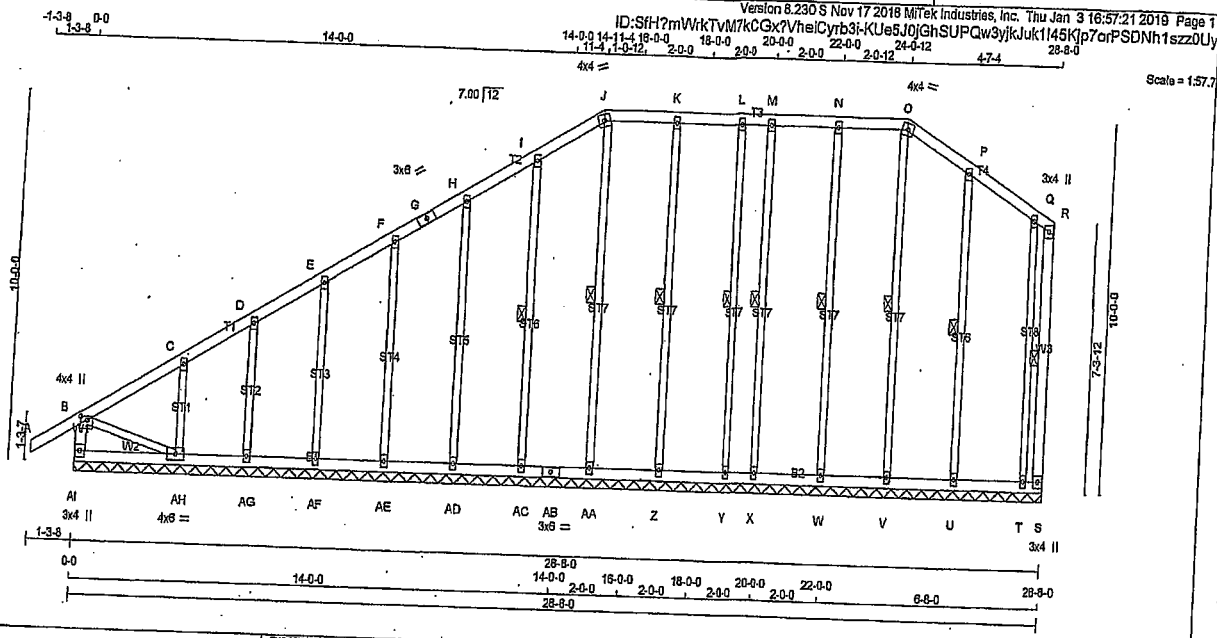
JSI METAL=0.65 (M) (INPUT=1.00)



DRWG NO. TAM 77900128
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	G33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



Scale = 1:57.7

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
AI - B	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - O	2x4	DRY	No.2
O - R	2x4	DRY	No.2
S - R	2x4	DRY	No.2
AI - AB	2x4	DRY	No.2
AB - S	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0 4.0 1.25 2.00
C	TMVW+p	MT20	2.0 4.0
G	TS-t	MT20	3.0 6.0
J	TTW-m	MT20	4.0 4.0
O	TTW-m	MT20	4.0 4.0
R	TMV+p	MT20	3.0 4.0
S	BMV1+p	MT20	3.0 4.0
T	U, V, W, X, Y, Z, AA, AC, AD, AE, AF, AG	MT20	2.0 4.0
AB	BS-t	MT20	3.0 6.0
AI	BMVWV1-t	MT20	4.0 6.0
AI	BMV1+p	MT20	3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINTS FOR 150 LBS. FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF R-S, I-AQ, J-AA, K-Z, L-Y, M-X, N-W, O-V, P-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED		WEBS MAX. FACTORED FORCE (LBS)
		VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	
FR-TO				
AI-B	-232/10	0.0	0.02 (1)	7.81
A-B	0/29	-84.9	-84.9 0.11 (1)	10.00
B-C	-5/10	-84.9	-84.9 0.09 (1)	10.00
C-D	-21/10	-84.9	-84.9 0.09 (1)	6.25
D-E	-3/10	-84.9	-84.9 0.04 (1)	10.00
E-F	-3/10	-84.9	-84.9 0.04 (1)	10.00
F-G	0/10	-84.9	-84.9 0.04 (1)	10.00
G-H	0/10	-84.9	-84.9 0.04 (1)	10.00
H-I	0/10	-84.9	-84.9 0.05 (1)	10.00
I-J	-5/10	-84.9	-84.9 0.05 (1)	10.00
J-K	0/5	-84.9	-84.9 0.05 (1)	10.00
K-L	0/4	-84.9	-84.9 0.05 (1)	10.00
L-M	0/4	-84.9	-84.9 0.05 (1)	10.00
M-N	0/4	-84.9	-84.9 0.03 (1)	10.00
N-O	0/5	-84.9	-84.9 0.05 (1)	10.00
O-P	-6/10	-84.9	-84.9 0.05 (1)	10.00
P-Q	0/8	-84.9	-84.9 0.05 (1)	10.00
Q-R	0/11	-84.9	-84.9 0.02 (1)	10.00
S-R	-2/10	0.0	0.0 0.00 (1)	6.25
AI-AH	0/0	-38.5	-38.5 0.08 (3)	10.00
AH-AG	0/8	-38.5	-38.5 0.08 (3)	10.00
AG-AF	0/5	-38.5	-38.5 0.03 (3)	10.00
AF-AE	0/2	-38.5	-38.5 0.03 (3)	10.00
AE-AD	0/0	-38.5	-38.5 0.03 (3)	10.00
AD-AC	-1/10	-38.5	-38.5 0.02 (3)	10.00
AC-AB	-3/10	-38.5	-38.5 0.02 (3)	10.00
AB-AA	-3/10	-38.5	-38.5 0.02 (3)	10.00
AA-Z	-4/10	-38.5	-38.5 0.03 (3)	10.00
Z-Y	-4/10	-38.5	-38.5 0.03 (3)	10.00
Y-X	-4/10	-38.5	-38.5 0.03 (3)	10.00
X-W	-4/10	-38.5	-38.5 0.03 (3)	10.00
W-V	-4/10	-38.5	-38.5 0.03 (3)	10.00
V-U	-3/10	-38.5	-38.5 0.03 (3)	10.00
U-T	-1/10	-38.5	-38.5 0.03 (3)	10.00
T-S	0/0	-38.5	-38.5 0.02 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. - G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1), BC=0.08/1.00 (AG-AH:3), WB=0.18/1.00 (H-AD:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

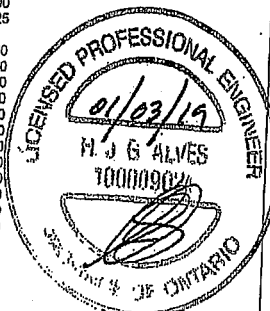
MT20 619 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.80)

JSI METAL= 0.12 (C) (INPUT = 1.00)



DRWG NO. TAM 19A0012A

STRUCTURAL

COMPLEMENT ONLY

[illegible]

CHORDS						WEBS					
MAX. FACTORED			FACTORED			MEMB.			MAX. FACTORED		
MEMB.	FORCE		VERT. LOAD	LC1	MAX	MAX.	UNBRAC	MEMB.	FORCE	MAX	
	(LBS)		(PLF)	CSI	(LC)				(LBS)	CSI	(LC)
FR-TO			FROM TO			LENGTH	FR-TO				
A-B	0/29		-84.9	-84.9	0.11 (I)	10.00	T-Q	-102 / 0		0.07 (I)	
B-C	-9/0		-84.9	-84.9	0.09 (I)	10.00	U-P	-193 / 0		0.13 (I)	
C-D	-25/0		-84.9	-84.9	0.09 (I)	6.25	V-O	-171 / 0		0.12 (I)	
D-E	-7/0		-84.9	-84.9	0.04 (I)	10.00	W-N	-145 / 0		0.16 (I)	
E-F	-7/0		-84.9	-84.9	0.04 (I)	10.00	X-L	-141 / 0		0.10 (I)	
F-G	-4/0		-84.9	-84.9	0.04 (I)	10.00	Z-K	-199 / 0		0.13 (I)	
G-H	-4/0		-84.9	-84.9	0.04 (I)	10.00	AA-J	-139 / 0		0.09 (I)	
H-I	-1/0		-84.9	-84.9	0.04 (I)	10.00	AC-I	-187 / 0		0.10 (I)	
I-J	-9/0		-84.9	-84.9	0.04 (I)	10.00	AD-H	-166 / 0		0.12 (I)	
J-K	0/1		-84.9	-84.9	0.05 (I)	10.00	AE-F	-168 / 0		0.18 (I)	
K-L	0/1		-84.9	-84.9	0.05 (I)	10.00	AF-E	-174 / 0		0.08 (I)	
L-M	0/1		-84.9	-84.9	0.03 (I)	10.00	AG-D	-139 / 0		0.04 (I)	
M-N	0/1		-84.9	-84.9	0.03 (I)	10.00	AH-C	-254 / 0		0.05 (I)	
N-O	0/1		-84.9	-84.9	0.04 (I)	10.00	X-M	-112 / 0		0.08 (I)	
O-P	0/1		-84.9	-84.9	0.05 (I)	10.00	B-AH	0 / 19		0.00 (I)	
P-Q	0/1		-84.9	-84.9	0.05 (I)	10.00					
Q-R	0/0		-84.9	-84.9	0.05 (I)	10.00					
R-S	-235/0	0.0	-84.9	-84.9	0.01 (I)	10.00					
B-B	-29/0	0.0	0.0	0.0	0.02 (I)	7.61					
		0.0	0.0	0.0	0.01 (I)	6.25					
A-AH	0/0	-38.5	-38.5	0.08 (3)	10.00						
H-AG	0/11	-38.5	-38.5	0.08 (3)	10.00						
G-AF	0/8	-38.5	-38.5	0.03 (3)	10.00						
F-AE	0/6	-38.5	-38.5	0.03 (3)	10.00						
E-AD	0/4	-38.5	-38.5	0.02 (3)	10.00						
D-AC	0/2	-38.5	-38.5	0.02 (3)	10.00						
C-AB	0/0	-38.5	-38.5	0.02 (3)	10.00						
B-AZ	0/0	-38.5	-38.5	0.02 (3)	10.00						
A-Z	-1/0	-38.5	-38.5	0.02 (3)	10.00						
X-Y	-1/0	-38.5	-38.5	0.02 (3)	10.00						
Y-X	-1/0	-38.5	-38.5	0.02 (3)	10.00						
W-V	-1/0	-38.5	-38.5	0.02 (3)	10.00						
V-W	-1/0	-38.5	-38.5	0.02 (3)	10.00						
U-U	-1/0	-38.5	-38.5	0.02 (3)	10.00						
T-T	-1/0	-38.5	-38.5	0.02 (3)	10.00						
S	0/0	-38.5	-38.5	0.02 (3)	10.00						

LICENSED PROFESSIONAL

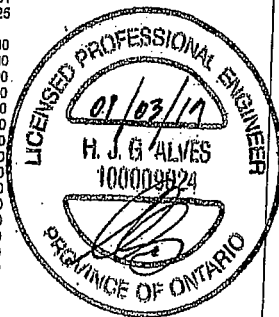
01/03/17

H. J. G. ALVES

100009824

PROVINCE OF ONTARIO

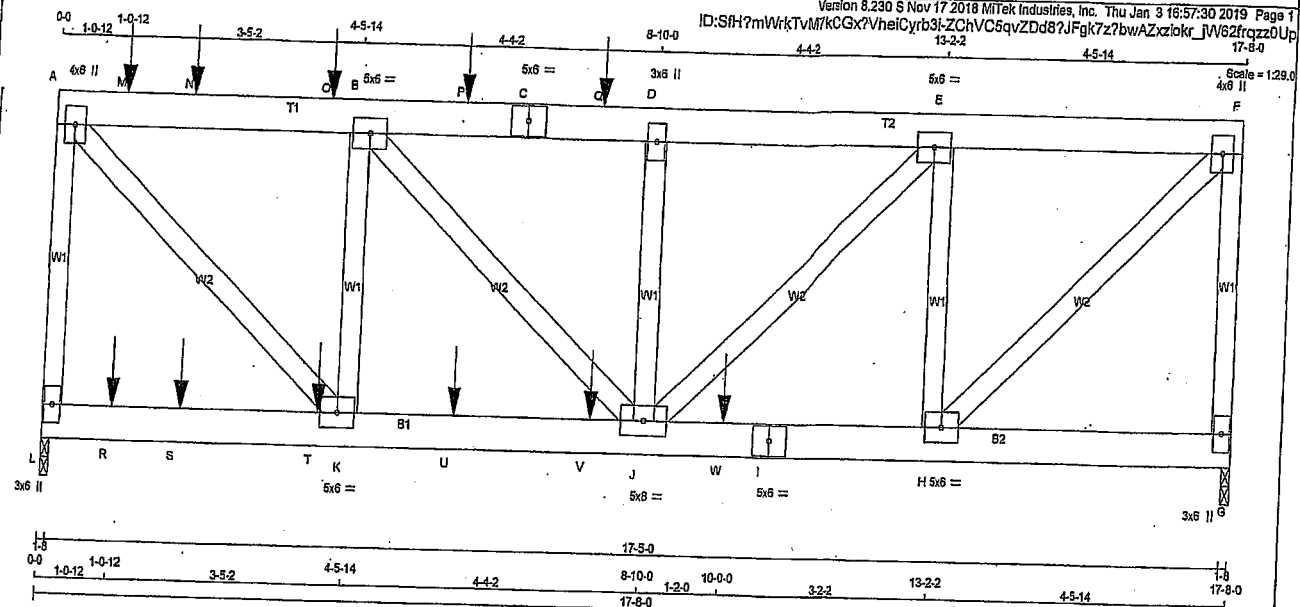
WG NO. TAM T1900114
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T35	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:57:30 2019 Page 1
ID: Sfh2mWrkTVM7kCGx7VheICyrb3t-ZChVCsqvZD8d7JFgk7z7bwAZxzkrc_JV62frqzz0Up



TOTAL WEIGHT = 2 X 110 = 221 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF	
A - C	2x6	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - I	2x6	DRY	No.2	SPF	
I - G	2x6	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
L-A	1	12	TOP
F-G	1	12	TOP
A-C	2	12	SIDE(0.0)
C-F	2	12	SIDE(0.0)
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
L-I	2	12	SIDE(0.0)
I-G	2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV+p	MT20	4.0	6.0		
B	TMWV-l	MT20	5.0	6.0		
C	TS-t	MT20	5.0	6.0		
D	TMWV+w	MT20	3.0	6.0		
E	TMWV-l	MT20	5.0	6.0		
F	TMWV+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWV-l	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWV+l	MT20	5.0	6.0		
K	BMWV-l	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	JT	BRG	JT	BRG
L	2499	0	2499	0	1-8	1-8	1-8
G	2034	0	2034	0	1-8	1-8	1-8

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
L	1884 971/0 398/0 0/0 0/0 515/0 0/0
G	1528 804/0 310/0 0/0 0/0 414/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX (PLF)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
L-A	-2318/0	0.0	0.0 0.38 (1)	7.37	A-K	0/2873	0.25 (1)		
A-M	-2085/0	-84.9	-84.9 0.14 (1)	6.25	K-B	-1693/0	0.18 (1)		
M-N	-2085/0	-84.9	-84.9 0.14 (1)	6.25	B-J	0/1119	0.10 (1)		
N-O	-2085/0	-84.9	-84.9 0.14 (1)	6.25	J-D	-498/0	0.05 (1)		
O-B	-2085/0	-84.9	-84.9 0.14 (1)	6.25	D-E	-498/0	0.05 (1)		
B-P	-2884/0	-84.9	-84.9 0.14 (1)	6.25	E-H	-498/0	0.05 (1)		
P-C	-2884/0	-84.9	-84.9 0.14 (1)	6.25	H-I	-498/0	0.05 (1)		
C-Q	-2884/0	-84.9	-84.9 0.14 (1)	6.25	I-J	-498/0	0.05 (1)		
Q-D	-2884/0	-84.9	-84.9 0.14 (1)	6.25	J-K	-498/0	0.05 (1)		
D-E	-2884/0	-84.9	-84.9 0.14 (1)	6.25	K-L	-498/0	0.05 (1)		
E-F	-1943/0	-84.9	-84.9 0.08 (1)	6.25	L-M	-498/0	0.05 (1)		
F-G	-2004/0	-84.9	-84.9 0.08 (1)	6.25	M-N	-498/0	0.05 (1)		
		0.0	0.0 0.31 (1)	7.75	N-O	-498/0	0.05 (1)		
L-R	0/0	-38.5	-38.5 0.09 (2)	10.00					
R-S	0/0	-38.5	-38.5 0.09 (2)	10.00					
S-T	0/0	-38.5	-38.5 0.09 (2)	10.00					
T-K	0/0	-38.5	-38.5 0.09 (2)	10.00					
K-U	0/2085	-38.5	-38.5 0.28 (1)	10.00					
U-V	0/2085	-38.5	-38.5 0.28 (1)	10.00					
V-J	0/2085	-38.5	-38.5 0.28 (1)	10.00					
J-W	0/1943	-38.5	-38.5 0.28 (1)	10.00					
W-I	0/1943	-38.5	-38.5 0.28 (1)	10.00					
I-H	0/1943	-38.5	-38.5 0.28 (1)	10.00					
H-G	0/0	-38.5	-38.5 0.09 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
M	1-0-12	-164	-164	FRONT	VERT	TOTAL	-	-
N	2-0-12	-164	-164	FRONT	VERT	TOTAL	-	-
O	4-0-12	-164	-164	FRONT	VERT	TOTAL	-	-
P	6-0-12	-164	-164	FRONT	VERT	TOTAL	-	-
Q	8-0-12	-164	-164	FRONT	VERT	TOTAL	-	-
R	1-0-12	-75	-75	FRONT	VERT	TOTAL	-	-
S	2-0-12	-75	-75	FRONT	VERT	TOTAL	-	-
T	4-0-12	-75	-75	FRONT	VERT	TOTAL	-	-
U	6-0-12	-75	-75	FRONT	VERT	TOTAL	-	-
V	8-0-12	-75	-75	FRONT	VERT	TOTAL	-	-
W	10-0-0	-1158	-1158	TOP	VERT	TOTAL	-	-

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14.
- TPC 2011, TPC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.36/1.00 (A-L-1), BC=0.39/1.00 (H-J-1), WB=0.25/1.00 (A-K-1), SSI=0.32/1.00 (H-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PS) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1997 1668

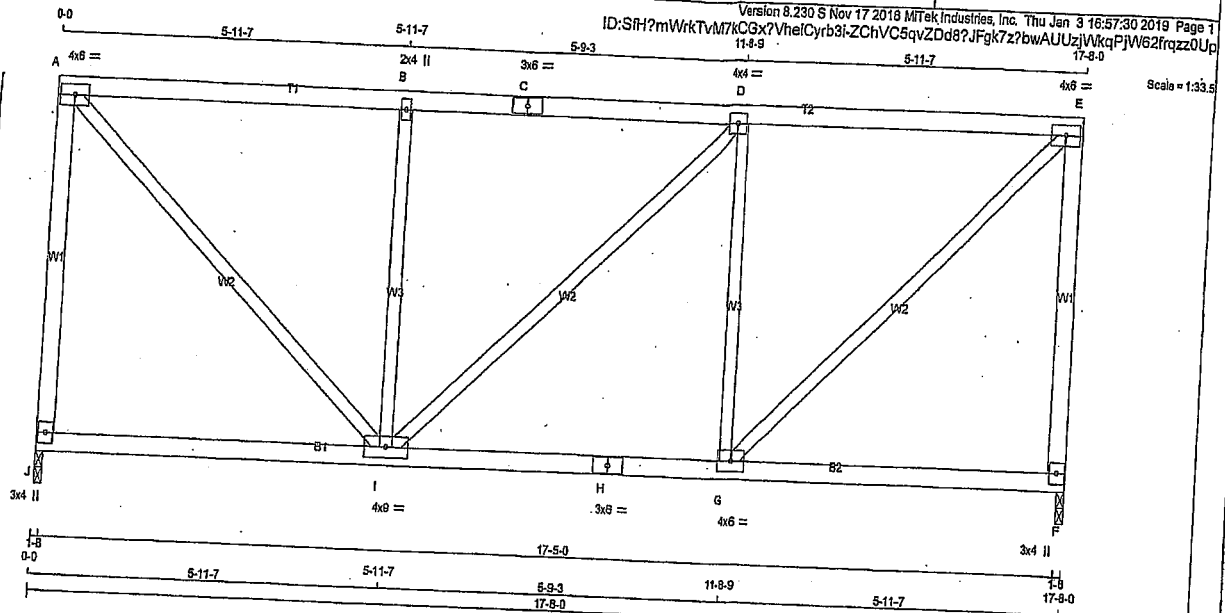
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.47 (A) (INPUT = 1.00)

ENG. NO. TAM 7960130
STRUCTURAL
COMPONENT ONLY

JOB NAME 300800	TRUSS NAME T36	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N.L.G.A. RULES	SIZE	DRY	LUMBER
J - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS 2x3 DRY, SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	UP	IN-SX	IN-SX	
J	1080	0	1080	0	1-8	1-8	
F	1080	0	1080	0	1-8	1-8	

UNFACTORED REACTIONS							
1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	827	411/0	188/0	0/0	0/0	230/0	0/0
F	827	411/0	188/0	0/0	0/0	230/0	0/0

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-t	MT20	4.0	6.0
B	TMVW-w	MT20	2.0	4.0
C	TS-t	MT20	3.0	6.0
D	TMVW-t	MT20	4.0	4.0
E	TMVW-t	MT20	4.0	6.0
F	BMV1+p	MT20	3.0	4.0
G	BMVW-t	MT20	4.0	6.0
H	BS-t	MT20	3.0	6.0
I	BMVW-t	MT20	4.0	9.0
J	BMV1+p	MT20	3.0	4.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
J-A	-996/0	0.0	0.0 0.65 (1)	G-E	0/1157	0.25 (1)	
A-B	-842/0	-84.9	-84.9 0.42 (1)	A-I	0/1155	0.25 (1)	
B-C	-842/0	-84.9	-84.9 0.42 (1)	G-D	-544/0	0.29 (1)	
C-D	-842/0	-84.9	-84.9 0.42 (1)	I-B	-544/0	0.29 (1)	
D-E	-843/0	-84.9	-84.9 0.42 (1)	I-D	-2/0	0.00 (1)	
F-E	-996/0	0.0	0.0 0.65 (1)				
J-I	0/0	-38.5	-38.5 0.24 (3)				
I-H	0/843	-38.5	-38.5 0.34 (2)				
H-G	0/843	-38.5	-38.5 0.34 (2)				
G-F	0/0	-38.5	-38.5 0.24 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.59")
CALCULATED VERT. DEFL. (LL) = L/999 (0.06")
ALLOWABLE DEFL. (TL) = L/360 (0.59")
CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.65/1.00 (E-F), BC=0.34/1.00 (G-I), WB=0.23/1.00 (D-G), SI=0.23/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

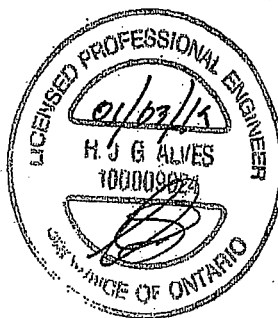
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1867

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.79 (3) (INPUT = 0.90)
JSI METAL= 0.29 (4) (INPUT = 1.00)

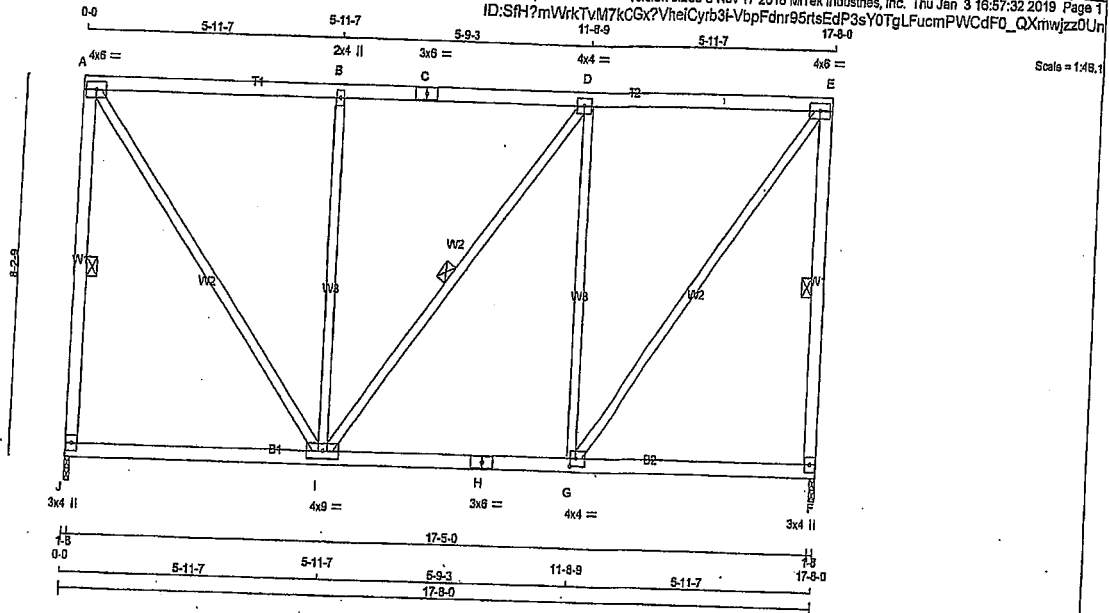


DRWG NO. TAM 11900131
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T38	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:57:32 2019 Page 1
ID:SM?mWrkTvm7kCGx?VheICyrb3FVbpFdnr95rfsEdP3sY0TgLFucmPWCdFO_QXmwjzz0Un



Scale = 1/4\"

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER
J - A	2x4 DRY	No.2		
A - C	2x4 DRY	No.2		
C - E	2x4 DRY	No.2		
E - H	2x4 DRY	No.2		
H - F	2x4 DRY	No.2		

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMVW-w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-t	MT20	4.0	4.0	2.00	1.50
H	BS-t	MT20	3.0	6.0		
I	BMVW-t	MT20	4.0	9.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
J	1080	0	1-8	1-8
F	1080	0	1-8	1-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	827	411/0	168/0	0/0	0/0	230/0	0/0
F	827	411/0	166/0	0/0	0/0	230/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-J, E-F, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	UNBRACED LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
J-A		-996/0	0.0	0.0	0.30 (1)	6.25	G-E		0/991	0.22 (1)
A-B		-594/0	-84.9	-84.9	0.41 (1)	6.25	A-I		0/889	0.22 (1)
B-C		-594/0	-84.9	-84.9	0.41 (1)	6.25	G-D		-545/0	0.72 (1)
C-D		-594/0	-84.9	-84.9	0.41 (1)	6.25	I-B		-544/0	0.72 (1)
D-E		-595/0	-84.9	-84.9	0.41 (1)	6.25	I-D		-2/0	0.00 (1)
E-F		-997/0	0.0	0.0	0.30 (1)	6.25				
J-I		0/0	-38.5	-38.5	0.24 (3)	10.00				
I-H		0/595	-38.5	-38.5	0.31 (2)	10.00				
H-G		0/595	-38.5	-38.5	0.31 (2)	10.00				
G-F		0/0	-38.5	-38.5	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.59")
CALCULATED VERT. DEFL.(LL) = $L/699$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.59")
CALCULATED VERT. DEFL.(TL) = $L/699$ (0.09")

CSI: TC=0.41/1.00 (D-E:1), BC=0.31/1.00 (G-I:2), WB=0.72/1.00 (D-G:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

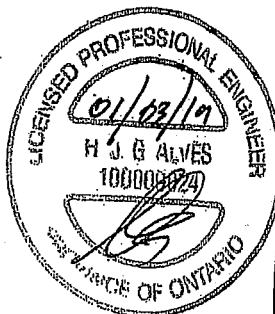
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (G) (INPUT = 0.90)
JSI METAL=0.25 (G) (INPUT = 1.00)

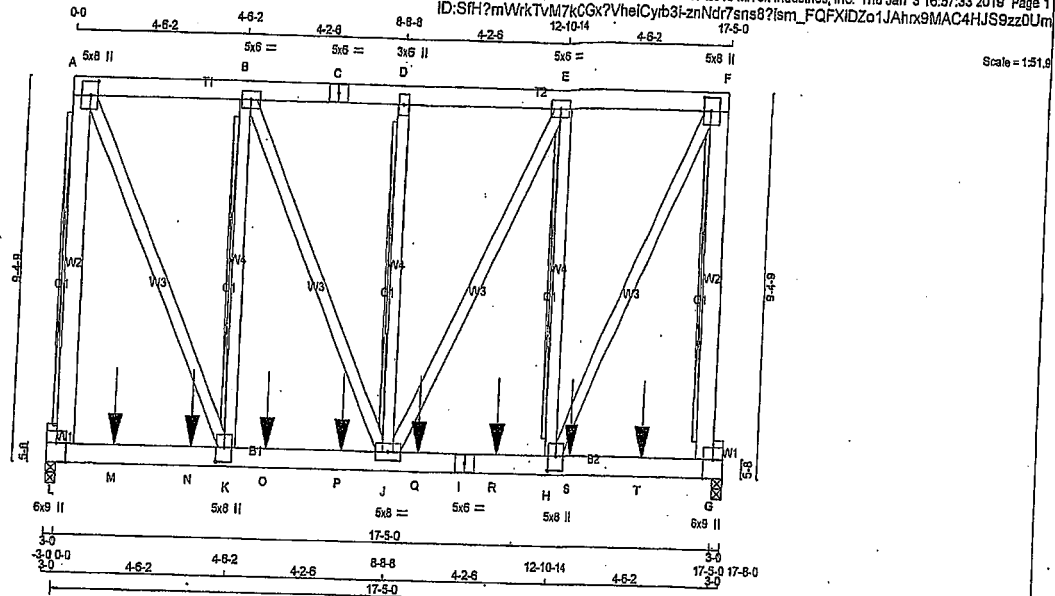


DWG NO. TAM 71900133
STRUCTURAL
COMMENTS ONLY

JOB NAME 300800	TRUSS NAME T39X	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
---------------------------	---------------------------	----------------------	-----------------	---------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Thu Jan 3 16:57:33 2018 Page 1
ID:SFH?mVrkTVM7kGx?VheICyrb3i-znNdr7sns8?ism_FQFXIDzo1JAhrx9MAC4HJS9zz0Um



TOTAL WEIGHT = 3 X 165 = 495 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - A	2x6	DRY	No.2
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
G - F	2x6	DRY	No.2
L - I	2x6	DRY	No.2
I - G	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
DRY: SEASONED LUMBER			

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
L-A	2	12
A-C	2	12
C-F	2	12
F-G	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-I	2	8
I-G	2	9
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	8

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TMVW+p	MT20	5.0	8.0	4.00 2.25
B TMVW+H	MT20	5.0	8.0	
C TS-I	MT20	5.0	8.0	
D TMVW+H	MT20	5.0	8.0	
E TMVW+H	MT20	5.0	8.0	
F TMVW+p	MT20	5.0	8.0	4.00 2.25
G BMVW+H	MT20	5.0	8.0	Edge 3.00
H BMVW+H	MT20	5.0	8.0	4.25 2.50
I BS-I	MT20	5.0	8.0	
J BMVW+H	MT20	5.0	8.0	
K BMVW+H	MT20	5.0	8.0	4.25 2.50
L BMVW+H	MT20	5.0	8.0	Edge 3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	VERT	DOWN	IN-SX	IN-SX
G	8175	0	3-0	3-0
G	7811	0	3-0	3-0

UNFACTORED REACTIONS					
1ST CASE	MAX	MIN	COMPONENT	REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
L	8177	3148 / 0	1332 / 0	0 / 0	0 / 0
G	5902	3008 / 0	1273 / 0	0 / 0	1699 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF NO.2 T-BRACE AT A-L, F-G, E-H, B-K, D-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX.	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE
FR-TO	(LBS)	FROM TO	CSI (LC)	FR-TO	(LBS)
L-A	-6951 / 0	0.0	0.0 0.54 (1)	H-E	-2278 / 0
A-B	-3419 / 0	-84.9	-84.9 0.05 (1)	K-B	-2227 / 0
B-C	-4294 / 0	-84.9	-84.9 0.05 (1)	J-D	-387 / 0
C-D	-4294 / 0	-84.9	-84.9 0.05 (1)	H-F	0 / 7527
D-E	-4294 / 0	-84.9	-84.9 0.05 (1)	A-K	0 / 7527
E-F	-3398 / 0	-84.9	-84.9 0.05 (1)	J-E	0 / 7527
G-F	-6908 / 0	0.0	0.0 0.54 (1)	B-J	0 / 7527
L-M	0 / 0	-38.5	-38.5 0.42 (1)		
M-N	0 / 0	-38.5	-38.5 0.42 (1)		
N-K	0 / 0	-38.5	-38.5 0.42 (1)		
K-O	0 / 3419	-38.5	-38.5 0.56 (1)		
O-P	0 / 3419	-38.5	-38.5 0.56 (1)		
P-J	0 / 3419	-38.5	-38.5 0.56 (1)		
J-Q	0 / 3398	-38.5	-38.5 0.52 (1)		
Q-I	0 / 3398	-38.5	-38.5 0.52 (1)		
I-R	0 / 3398	-38.5	-38.5 0.52 (1)		
R-H	0 / 3398	-38.5	-38.5 0.52 (1)		
H-S	0 / 0	-38.5	-38.5 0.39 (1)		
S-T	0 / 0	-38.5	-38.5 0.39 (1)		
T-G	0 / 0	-38.5	-38.5 0.39 (1)		

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX.	FACE	DIR.
M	1-5-12	-1730	-1730	BACK	VERT
N	3-5-12	-1730	-1730	BACK	VERT
O	5-5-12	-1730	-1730	BACK	VERT
P	7-5-12	-1730	-1730	BACK	VERT
Q	9-5-12	-1730	-1730	BACK	VERT
R	11-5-12	-1730	-1730	BACK	VERT
S	13-5-12	-1730	-1730	BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.58")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.05")
ALLOWABLE DEFL. (TL) = 1/360 (0.58")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.09")

CSI: TC=0.54/1.00 (A-L1), BC=0.58/1.00 (J-K1), WB=0.41/1.00 (A-K1), SS=0.55/1.00 (G-H1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

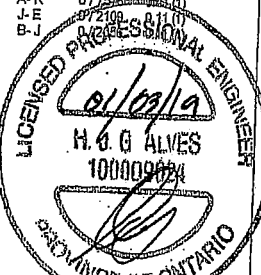
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP = 0.84 (A) (INPUT = 0.90)
SI METAL = 0.47 (A) (INPUT = 1.00)



DRWG NO. TAM 1900134
STRUCTURAL
CONNECTION ONLY

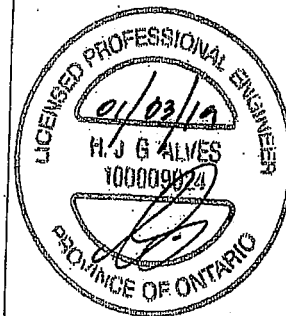
1/2

JOB NAME 300800	TRUSS NAME T39X	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
--------------------	--------------------	---------------	----------	--------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.230 8 Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 16:57:33 2019 Page 2
ID:Sfh7mWrkTvM7kCGx7VheICyrb3I-znNd7sns87ism FQFXIDZo1JAhrx9MAC4HJS9z2oUmy

FACTORED CONCENTRATED LOADS (LBS)									
	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	15-5-12	-1730	-1730	—	TOP	VERT	TOTAL	—	—
T									



DRWG NO. TAM 71900134
STRUCTURAL
CORRECTION ONLY 3/2

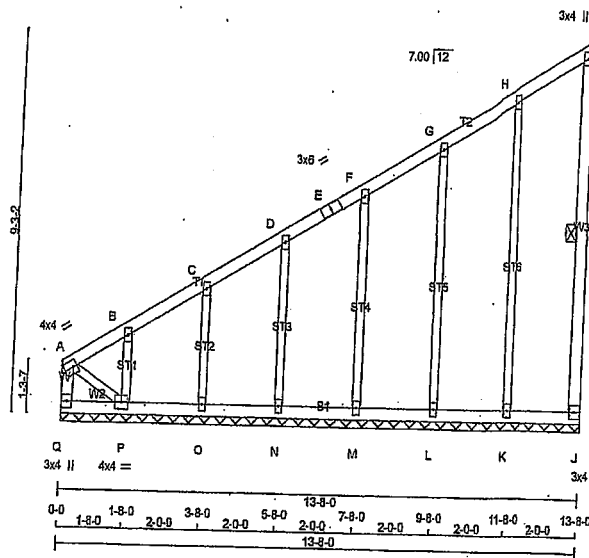
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	G40	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:57:23 2019 Page 1

ID:SH?mWkTvm7kCGX?VheICyb3t-GsmrkWC4t7EDKq9LmpSNRfPmbhRhvXsn6kzz0Uw

Scale = 1:51.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
A - E	2x4	DRY	No.2		
E - I	2x4	DRY	No.2		
J - I	2x4	DRY	No.2		
Q - A	2x4	DRY	No.2		
Q - J	2x4	DRY	No.2		

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - TMVW-t	MT20	4.0	4.0	2.00	Edge	
B, C, D, F, G, H	MT20	2.0	4.0			
E - TS-t	MT20	3.0	6.0			
I - TMV-t	MT20	3.0	4.0			
J - BMV-t	MT20	3.0	4.0			
K, L, M, N, O	MT20	2.0	4.0			
P - BMVW-t	MT20	4.0	4.0			
Q - BMV-t	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LOAD (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED LOAD (LC)	
FR-TO		FROM TO			FR-TO				
A-B	-14/0	-84.9	-84.9	0.04 (1)	6.25	K-H	-190/0	0.24 (1)	
B-C	-15/0	-84.9	-84.9	0.04 (1)	6.25	L-G	-166/0	0.14 (1)	
C-D	-10/0	-84.9	-84.9	0.04 (1)	6.25	M-F	-169/0	0.09 (1)	
D-E	-7/0	-84.9	-84.9	0.04 (1)	10.00	N-D	-168/0	0.05 (1)	
E-F	-7/0	-84.9	-84.9	0.04 (1)	10.00	O-C	-168/0	0.03 (1)	
F-G	-4/0	-84.9	-84.9	0.04 (1)	10.00	P-B	-165/0	0.03 (1)	
G-H	0/0	-84.9	-84.9	0.04 (1)	10.00	A-P	0/22	0.01 (1)	
H-I	-8/0	-84.9	-84.9	0.04 (1)	10.00				
I-J	-72/0	0.0	0.0	0.03 (1)	6.25				
Q-A	-73/0	0.0	0.0	0.01 (1)	7.81				
Q-P	0/0	-38.5	-38.5	0.02 (3)	10.00				
P-O	0/13	-38.5	-38.5	0.02 (3)	10.00				
O-N	0/9	-38.5	-38.5	0.02 (3)	10.00				
N-M	0/6	-38.5	-38.5	0.02 (3)	10.00				
M-L	0/4	-38.5	-38.5	0.02 (3)	10.00				
L-K	0/2	-38.5	-38.5	0.03 (3)	10.00				
K-J	0/0	-38.5	-38.5	0.03 (3)	10.00				

TOTAL WEIGHT = 68 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSH TO=0.04/1.00 (H-I), BC=0.03/1.00 (K-L), WB=0.24/1.00 (H-K), SS=0.07/1.00 (H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

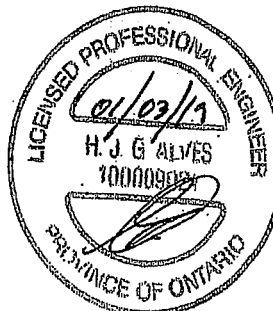
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

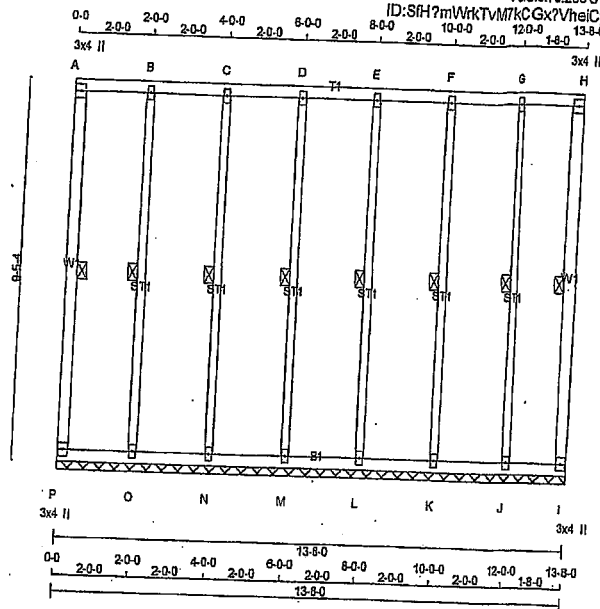
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (H) (INPUT = 0.90)
JSI METAL= 0.09 (H) (INPUT = 1.00)



DRWG NO. TAM 11900123
STRUCTURAL
COMPONENT ONLY

JOB NAME 300800 Tamarack Roof Truss, Burlington	TRUSS NAME G41	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
---	-------------------	---------------	----------	--------------------------	----------



<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	
P - A	2x4	DRY	No.2	
A - H	2x4	DRY	No.2	
I - H	2x4	DRY	No.2	
P - I	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
ALL GABLE WEBS	2x3	DRY	No.2	
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B, C, D, E, F, G	TMV+w	MT20	2.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		
J, K, L, M, N, O	BMV1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, C-N, D-M, E-L, F-K, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	MAX CS (LC)	MAX UNBRACED LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS (LC)
FR-TO							
P-A	-74/0	0.0	0.01 (1)	6.25	O-B	-180/0	0.11 (1)
A-B	-1/0	-84.9	-84.9 0.04 (1)	10.00	N-C	-170/0	0.10 (1)
B-C	-1/0	-84.9	-84.9 0.04 (1)	10.00	M-D	-170/0	0.10 (1)
C-D	-1/0	-84.9	-84.9 0.04 (1)	10.00	L-E	-169/0	0.10 (1)
D-E	-1/0	-84.9	-84.9 0.04 (1)	10.00	K-F	-174/0	0.10 (1)
E-F	-1/0	-84.9	-84.9 0.04 (1)	10.00	J-G	-161/0	0.09 (1)
F-G	-1/0	-84.9	-84.9 0.04 (1)	10.00			
G-H	-1/0	-84.9	-84.9 0.03 (1)	10.00			
H-I	-62/0	0.0	0.0 0.01 (1)	6.25			
P-O	0/1	-38.5	-38.5 0.03 (3)	10.00			
O-N	0/1	-38.5	-38.5 0.03 (3)	10.00			
N-M	0/1	-38.5	-38.5 0.02 (3)	10.00			
M-L	0/1	-38.5	-38.5 0.02 (3)	10.00			
L-K	0/1	-38.5	-38.5 0.02 (3)	10.00			
K-J	0/1	-38.5	-38.5 0.02 (3)	10.00			
J-I	0/1	-38.5	-38.5 0.02 (3)	10.00			

TOTAL WEIGHT = 95 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.04/1.00 (E-F:1), BC=0.03/1.00 (N-O:3), WB=0.11/1.00 (B-O:1), SS=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

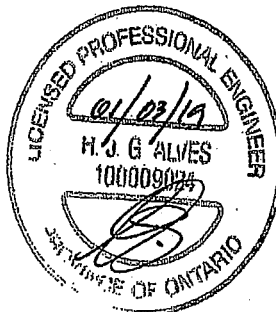
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (O) (INPUT = 0.80)
JSI METAL= 0.04 (O) (INPUT = 1.00)

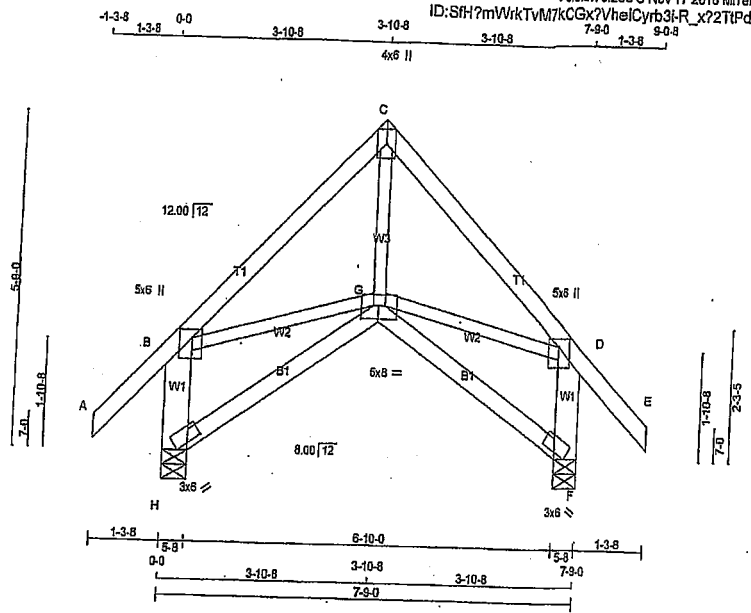


DRWG NO. TAM T79 00124
STRUCTURAL
COMPONENT ONLY

JOB NAME 300800 Tamarack Roof Truss, Burlington	TRUSS NAME T42	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
---	-------------------	---------------	----------	--------------------------	----------

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:57:34 2019 Page 1
ID: Sfh7mWkTVM7kCGx7VheICyrb3i-R_x72TPdS7ZUwZSzz2xmmLH_a7gght/Rk0t_czz0U

Scale = 1:36.6



TOTAL WEIGHT = 42 lb [MIF]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x6	DRY	No.2
F - D	2x6	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	5.0	6.0	1.75 2.75
C	TTW+p	MT20	4.0	6.0	
D	TMW+p	MT20	5.0	6.0	1.75 2.75
F	BVM1-I	MT20	3.0	6.0	0.50 3.00
G	BMWW-p	MT20	5.0	8.0	2.75 4.00
H	BVM1-I	MT20	3.0	6.0	0.50 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	DOWN	GROSS REACTION	BRG	BRG
H	596	0	596	0	5-8
F	596	0	596	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	444	245/0	81/0
F	444	245/0	81/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0/42	A-B	0/299
B-C	-424/0	B-C	0/308
C-D	-424/0	C-D	0/308
D-E	0/42		
H-B	-522/0		
F-D	-522/0		
H-G	0/0		
G-F	0/0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD.

ALLOWABLE DEFL. (LL) = L/360 (0.26")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.26")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.171/0.0 (C-D:1), BC=0.14/1.00 (F-G:3), WB=0.07/1.00 (B-G:1), SS=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

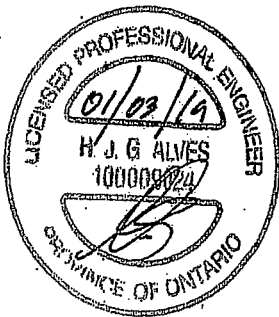
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE GRIP(DRY) SHEAR (PSI)	SECTION (PLI)
MAX MIN MAX MIN MAX MIN	
MT20	618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.22 (G) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)



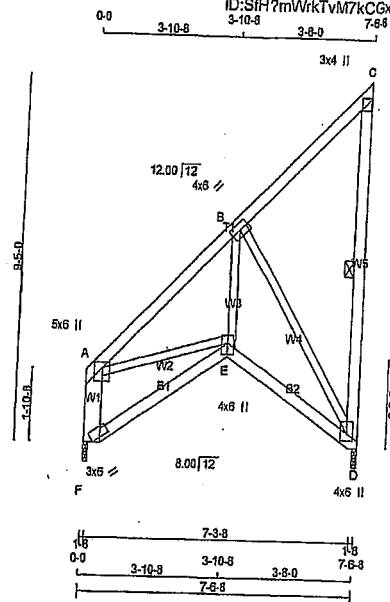
DRWG NO. TAM 17940135
STRUCTURAL
CHECKED ONLY

JOB NAME 300800	TRUSS NAME T46A	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
--------------------	--------------------	---------------	----------	--------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:57:35 2019 Page 1

ID: Sfh7mWvKtVIM7kCGx7VheiCyrb3l-vAVOFpu2OmFQ547eXgZAI_sQu_TUP3TSgOmOX2z0Uk



Scale = 1/4" = 1'-0"

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x6	DRY	No.2
F - E	2x4	DRY	No.2
E - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TMVW+1	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW+p	MT20	4.0	6.0	2.25	2.00
E	BMVW+p	MT20	4.0	6.0	3.50	2.00
F	BVM+1	MT20	3.0	6.0	0.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
D	465	0	465	0
F	465	0	465	0

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
D	353	176/0	79/0
F	353	176/0	79/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 (FLP)	MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)			
FR-TO	FROM	TO	FROM	TO	FR-TO	FROM	TO	FROM	TO		
A-B	-431 / 0	-84.9	-84.9	0.16 (1)	6.25	E-B	0 / 421	0.09 (2)			
B-C	-28 / 0	-84.9	-84.9	0.15 (1)	6.25	A-E	0 / 329	0.07 (1)			
D-C	-111 / 0	0.0	0.0	0.05 (1)	6.25	B-D	-579 / 0	0.43 (1)			
F-A	-391 / 0	0.0	0.0	0.03 (1)	7.81						
F-E	0 / 0	-38.5	-38.5	0.13 (3)	10.00						
E-D	0 / 372	-38.5	-38.5	0.16 (2)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 6.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.25")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.25")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CS: TC=0.16/1.00 (A-B:1), BC=0.16/1.00 (D-E:2), WB=0.43/1.00 (B-D:1), SS=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.54 (D) (INPUT = 0.90)

JSI METAL= 0.27 (D) (INPUT = 1.00)

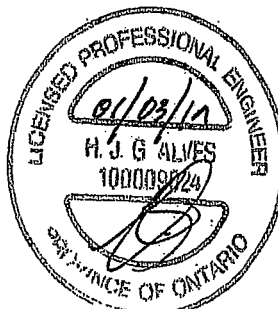
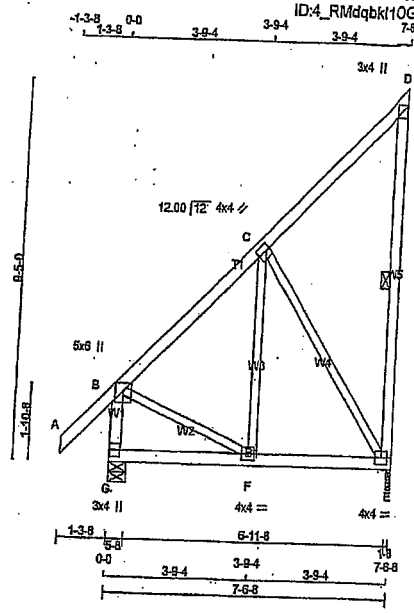


FIG NO. TAM 71900136
STRUCTURAL
FOR OFFICE ONLY

JOB NAME 300526	TRUSS NAME T140	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
--------------------	--------------------	---------------	----------	--------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Jan 15 10:45:27 2019 Page 1
ID:4_RMdqbk10G133Xnfrj7uyvEbu-Xwrfh_1joJ1gJhH2XWsdAdEVUoJ2v0Bs8D0pk2v8pc



Scale = 1/2" = 1'-0"

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
E - D	2x4	DRY No.2
G - B	2x4	DRY No.2
G - E	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TMV+p	MT20	3.0	4.0		
E	BMVW-t	MT20	4.0	4.0		
F	BMVW-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		

Edges - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	465	0	465	0	1-8	1-8
G	584	0	584	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	353	178 / 0	79 / 0	0 / 0	0 / 0	98 / 0	0 / 0
G	435	241 / 0	79 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)				MAX. UNBRACED LENGTH FR-TO	WEBS MEMB. MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
		FROM		TO				
		FR-TO	FR-TO	FR-TO	FR-TO			
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	F-C	0 / 186	0.04 (3)
B-C	-263 / 0	-84.9	-84.9	0.18 (1)	8.25	C-E	-354 / 0	0.26 (1)
C-D	-28 / 0	-84.9	-84.9	0.18 (1)	6.25	B-F	0 / 221	0.05 (1)
E-D	-121 / 0	0.0	0.0	0.03 (1)	6.25			
G-B	-527 / 0	0.0	0.0	0.03 (1)	7.81			
G-F	0 / 0	-38.5	-38.5	0.12 (3)	10.00			
F-E	0 / 205	-38.5	-38.5	0.14 (2)	10.00			

TOTAL WEIGHT = 2 X 47 = 94 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24" O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015.

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAINLOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.25")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.25")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI TO = 0.16 / 1.00 (B-C:1), 8C = 0.14 / 1.00 (E-F:2),
WB = 0.26 / 1.00 (C-E:1), 6S = 0.11 / 1.00 (B-C:1)

DOL LUMBER = 1.00 NAIL = 1.00 LS BEND = 1.10
COMP = 1.10 SHEAR = 1.10 TENS = 1.10

COMPANION LIVE LOAD FACTOR = 1.00

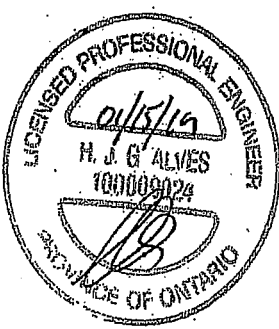
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1656

PLATE PLACEMENT TOL = 0.250 inches

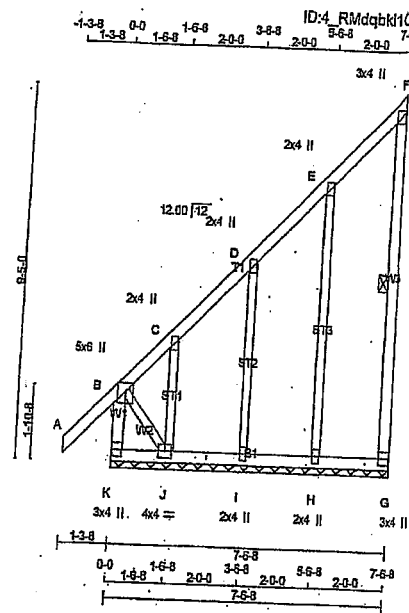
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.27 (B) (INPUT = 0.90)
JSI METAL = 0.14 (B) (INPUT = 1.00)



DWG NO. TAM 11901091
STRUCTURAL
EQUIPMENT ONLY

JOB NAME 300526 Tamarack Roof Truss, Burlington	TRUSS NAME G140	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
---	--------------------	---------------	----------	--------------------------	----------



Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Jan 15 10:45:26 2019 Page 1
ID:4_RMDqbk10G13PxfRj7uyvEbu-2kHHUe0510vpXrzDL0ePmJfP?XAc9jwZfGCGzv8pd

Scale = 1/8" = 1'-0"

TOTAL WEIGHT = 2 X 48 = 96 lb

LUMBER
N.L.G.A. RULES

CHORDS	SIZE	LUMBER
A - F	2x4	DRY No.2
G - H	2x4	DRY No.2
I - J	2x4	DRY No.2
K - L	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)
FR-TO				
A-B	0/42	-84.9	10.00	H-E -189/0
B-C	-49/0	-84.9	6.25	I-D -175/0
C-D	-2/0	-84.9	10.00	J-C -82/0
D-E	-3/0	-84.9	10.00	B-J 0/13
E-F	-11/0	-84.9	6.25	
F-G	-73/0	0.0	6.25	
G-H	-249/0	0.0	7.81	
K-J	0/0	-38.5	10.00	
J-I	0/6	-38.5	10.00	
I-H	0/3	-38.5	10.00	
H-G	0/0	-38.5	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.8 PSF
SPACING = 24" O.C.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TRC 2011, TPIC 2014
DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TO=0.12/1.00 (A-B:1), BC=0.03/1.00 (H-I:3), WB=0.18/1.00 (E-H:1), GS1=0.07/1.00 (A-B:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00

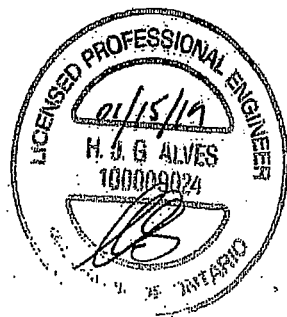
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (FBI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.15 (B) (INPUT = 0.90)
JSI METAL = 0.10 (E) (INPUT = 1.00)

DWG NO. TAM 11901092
STRUCTURAL
COMPONENT ONLY



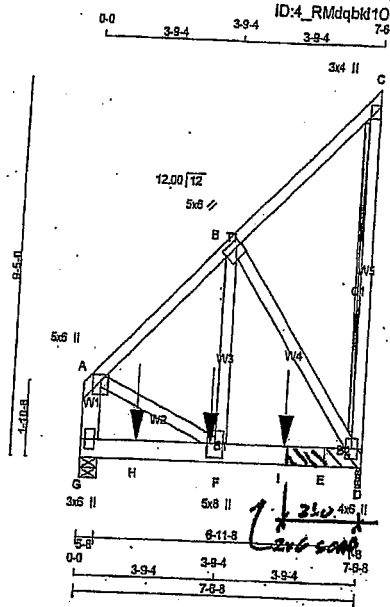
PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C, D, E					
C TMVW+w	MT20	2.0	4.0		
F TMVW+p	MT20	3.0	4.0		
G BMVW+p	MT20	3.0	4.0		
H BMVW+w	MT20	2.0	4.0		
I BMVW+w	MT20	2.0	4.0		
J BMVW+w	MT20	4.0	4.0		
K BMVW+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

JOB NAME 300526 Tamarack Roof Truss, Burlington	TRUSS NAME T141	QUANTITY 2	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
---	--------------------	---------------	----------	--------------------------	----------

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Jan 15 10:45:28 2019 Page 1
ID: 4_RMDqbk10G13fXnRj7uyvEbu-76P1vK2LZd9Xq5E5dOsjqnhiCV0elQ0N18NH9zv8pb
Scale = 1:52.8



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
G - A	2x4	DRY	No.2	SPF	
G - D	2x4	DRY	No.2	SPF	

ALL WEBS 2x4 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'x3') SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	TOP
G-A 2	12	TOP
BOTTOM CHORDS: (0.122'x3') SPIRAL NAILS		
G-D 2	12	SIDE (183.1)
WEBS: (0.122'x3') SPIRAL NAILS		
B-F 1	6	SIDE (464.6)
2x4 1	6	

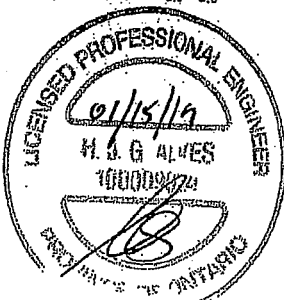
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TMWV+p	MT20	5.0	6.0	1.75 2.75
B TMWV+t	MT20	5.0	6.0	2.00 1.50
C TMV+p	MT20	3.0	4.0	
D BMWV+p	MT20	4.0	6.0	
F BMWV+t	MT20	5.0	6.0	4.25 2.50
G BMV+p	MT20	3.0	6.0	



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP/LIFT	IN-SX
D	3897	0	0	1-8
G	4382	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
D	3013 1658/0 632/0 0/0 0/0 823/0 0/0
G	3302 1708/0 692/0 0/0 0/0 902/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G

2x6 DRY SPF No.2 BEARING BLOCK 2' LONG AT JT. D ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122'x3') SPIRAL NAILS SPACED 3" C.C. 24 NAILS TOTAL

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 5.13 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO					FR-TO		
A-B	-3103/0	-84.9	-84.9	0.15 (1)	5.13	F-B	0/4878
B-C	-24/0	-84.9	-84.9	0.10 (1)	6.25	E-D	-3772/0
D-C	-125/0	0.0	0.0	0.04 (1)	7.81	A-F	0/2367
G-A	-3181/0	0.0	0.0	0.12 (1)	7.75		
G-H	0/0	-38.5	-38.5	0.64 (1)	10.00		
H-F	0/0	-38.5	-38.5	0.64 (1)	10.00		
F-I	0/2212	-38.5	-38.5	0.73 (1)	10.00		
I-E	0/2212	-38.5	-38.5	0.73 (1)	10.00		
E-D	0/2212	-38.5	-38.5	0.73 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	3-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—
H	1-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—
I	5-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—

TOTAL WEIGHT = 4 X 57 = 228 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/860 (0.25")
CALCULATED VERT. DEFL. (LL) = L/899 (0.03")
ALLOWABLE DEFL. (TL) = L/860 (0.25")
CALCULATED VERT. DEFL. (TL) = L/999 (0.06")

CSI: TC=0.15/1.00 (A-B:1), BC=0.73/1.00 (D-F:1), WB=0.97/1.00 (B-D:1), SSI=0.60/1.00 (D-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1686

PLATE PLACEMENT TOL. = 0.250 inches

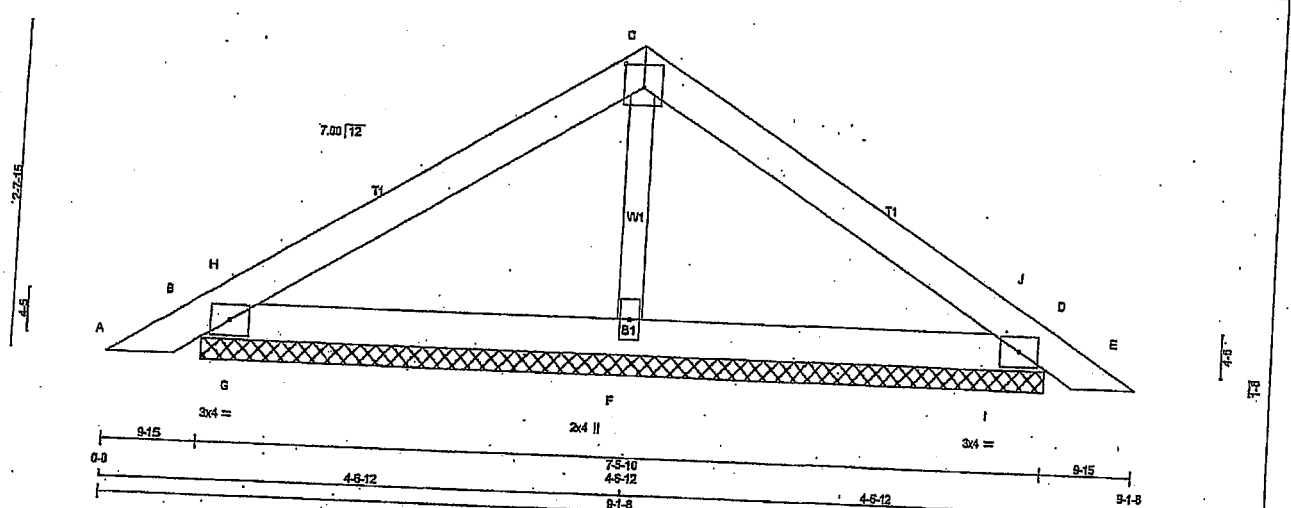
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)
JSI METAL = 0.53 (F) (INPUT = 1.00)

DWG NO. TAM 17901090
STRUCTURAL
COMPANION ONLY

JOB NAME 300539	TRUSS NAME P1	QUANTITY 6	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.290 S Nov 17 2018 MITek Industries, Inc. Thu Jan 3 12:17:47 2019 Page 1
 ID:K77_mHxOPUzKF0D0T0r6u1yw9qF-SR5qqPJW_e8S17J74uNKukaVjdleyTmQHW44J6zz4b2
 9-1-8
 Scale = 1:17.2



LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	3.0	4.0		
C	TTW-p	4.0	4.0	2.25	2.00
D	TMB1-I	3.0	4.0		
F	BMV1+H	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	322	0	322	0	0
D	322	0	322	0	0
F	404	0	404	0	0

UNFACTORED REACTIONS						
JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS				
B	237	141/0	35/0	0/0	0/0	0/0
D	237	141/0	35/0	0/0	0/0	0/0
F	313	138/0	35/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LCH: MAX CS (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCH: MAX CS (L/C)	
FR-TO				FR-TO			
A-B	0/15	-84.9	-84.9 0.04 (1)	10.00	F-C	-155/0	0.02 (1)
B-H	-75/0	-84.9	-84.9 0.04 (1)	6.25	G-H	-248/24	0.00 (1)
H-C	-115/0	-84.9	-84.9 0.15 (1)	6.25	I-J	-248/24	0.00 (1)
C-J	-115/0	-84.9	-84.9 0.15 (1)	6.25			
J-D	-75/0	-84.9	-84.9 0.04 (1)	6.25			
D-E	0/15	-84.9	-84.9 0.04 (1)	10.00			
B-G	0/97	-38.5	-38.5 0.14 (1)	10.00			
G-F	0/97	-38.5	-38.5 0.16 (1)	10.00			
F-I	0/97	-38.5	-38.5 0.16 (1)	10.00			
I-D	0/97	-38.5	-38.5 0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF ECBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-M), BC=0.16/1.00 (F-M), WS=0.02/1.00 (C-F:1), SS=0.21/1.00 (B-G:1)

DCL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

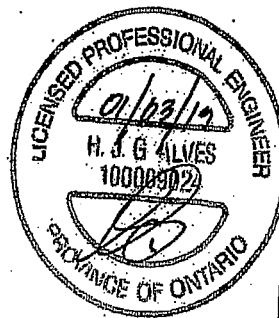
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

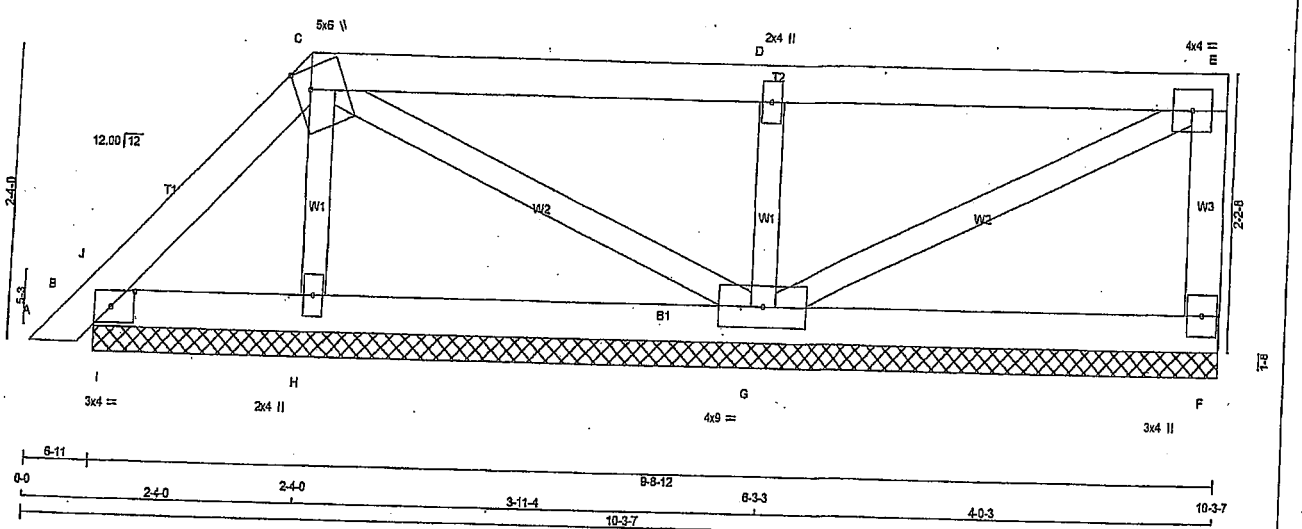
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.50)
JSI METAL= 0.05 (D) (INPUT = 1.00)



ENGR. NO. TAM 77900063
STRUCTURAL
COMPANION ONLY



LUMBER
N.L.G.A. RULES
CHORDS SIZE
A - C 2x4 DRY No.2
C - E 2x4 DRY No.2
F - H 2x4 DRY No.2
B - F 2x4 DRY No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMW-t	MT20	4.0	4.0		
F	BMW1+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	189	0	189	0	9-8-12	9-8-12
B	173	0	173	0	9-8-12	9-8-12
H	263	0	263	0	9-8-12	9-8-12
G	613	0	613	0	9-8-12	9-8-12

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN COMPONENT REACTIONS					SOIL
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	
F	144	70/0	33/0	0/0	0/0	40/0	0/0
B	124	84/0	11/0	0/0	0/0	29/0	0/0
H	207	81/0	63/0	0/0	0/0	63/0	0/0
G	482	239/0	97/0	0/0	0/0	128/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. CSI (C)
A-B	0/12	-84.9	-84.9 0.01 (1)	10.00	H-C	-132/0	0.02 (1)
B-J	-30/0	-84.9	-84.9 0.01 (1)	6.25	G-E	-2/0	0.00 (1)
J-C	-59/0	-84.9	-84.9 0.03 (1)	6.25	C-G	-38/0	0.01 (1)
C-D	0/2	-84.9	-84.9 0.23 (1)	10.00	G-D	-421/0	0.08 (1)
D-E	0/2	-84.9	-84.9 0.23 (1)	10.00	I-J	-110/0	0.00 (1)
E-F	-128/0	0.0	0.0 0.01 (1)	7.81			
B-I	0/38	-38.5	-38.5 0.04 (1)	10.00			
I-H	0/38	-38.5	-38.5 0.06 (2)	10.00			
H-G	0/31	-38.5	-38.5 0.12 (3)	10.00			
G-F	0/0	-38.5	-38.5 0.12 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(85% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-D:1), BC=0.12/1.00 (G-H:3), WB=0.09/1.00 (D-G:1), SE=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LG BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

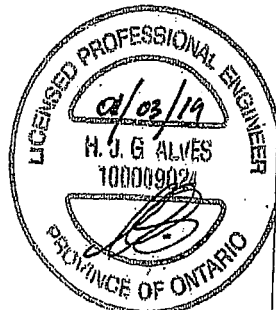
NAIL VALUES
PLATE, GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1667 1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)

JSI METAL= 0.09 (D) (INPUT = 1.00)

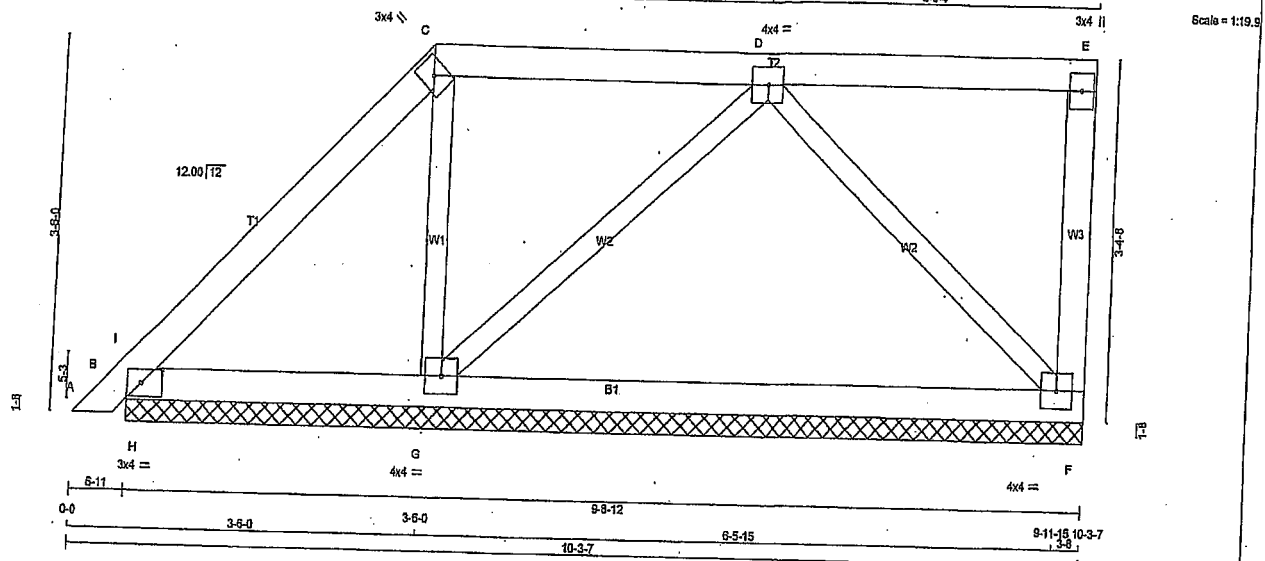


DRWG NO. TAM 7190016
STRUCTURAL
COMPONENT ONLY

JOB NAME 300799	TRUSS NAME P11	QUANTITY 3	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
--------------------	-------------------	---------------	----------	--------------------------	----------

Tamarack Roof Truss, Burlington

Version 6.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:51:09 2019 Page 1
ID: Sfh7mWrkTvm7kCGk7VheICyrb3l-DvASFqDwHedCnmdSwceffauVENhznzZCevLG?_Qzz0am
6-103 6-103 10-3-7 10-3-7
Scale = 1:19.9



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
A - C 2x4 DRY No.2
C - E 2x4 DRY No.2
F - E 2x4 DRY No.2
B - F 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0	1.50	2.50
C TTW+h	MT20	3.0	4.0		
D TMWW-I	MT20	4.0	4.0		
E TMW+p	MT20	3.0	4.0		
F BMW1-I	MT20	4.0	4.0		
G BMWW1-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ UPLIFT		
F	395	0	395	0	9-8-12	9-8-12
B	190	0	190	0	9-8-12	9-8-12
G	654	0	654	0	9-8-12	9-8-12

UNFACTORED REACTIONS

JT	COMBINED	1ST LOASE MAX/MIN. COMPONENT REACTIONS		PERM.LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
F	235	162/0	54/0	0/0	0/0	78/0	0/0
B	130	107/0	0/-2	0/0	0/0	26/0	0/0
G	612	206/0	152/0	0/0	0/0	154/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0/12	-84.9	-84.9	0.01 (1)
B-I	0/163	-84.9	-84.9	0.09 (1)
C-D	-46/1	-84.9	-84.9	0.08 (1)
D-E	-16/7	-84.9	-84.9	0.16 (1)
E-F	0/0	-84.9	-84.9	0.16 (1)
F-E	-112/0	0.0	0.0	0.02 (1)
B-H	-6/23	-38.5	-38.5	0.10 (1)
H-G	-6/23	-38.5	-38.5	0.30 (3)
G-F	0/202	-38.5	-38.5	0.32 (2)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
CL = 6.0 PSF
BOT CH. LL = 10.5 PSF
CL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (C-D:1), BC=0.32/1.00 (F-G:2), WB=0.09/1.00 (D-F:1), SS=0.28/1.00 (B-H:1)

DOL LUMBER=1.00, NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

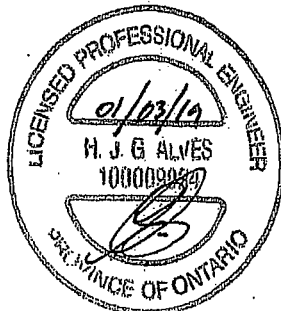
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1659

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.31 (C) (INPUT = 0.80)
JSI METAL= 0.08 (F) (INPUT = 1.00)

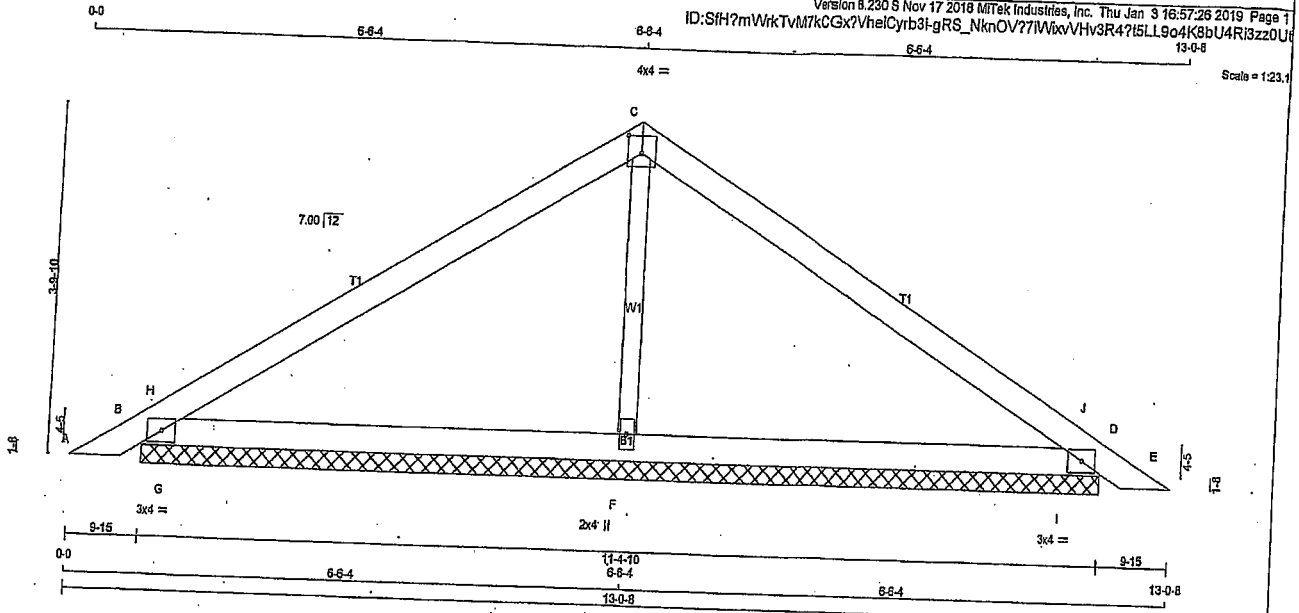


DWG NO. TAM 17900117
STRUCTURAL
COMPONENT ONLY

JOB NAME 300800	TRUSS NAME P12	QUANTITY 7	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 6.230.6 Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:57:26 2019 Page 1
ID: Sfh7mWkTvm7kCGx?VhelCyrb3t-gRS_NknOV77IVWxVHV3R4?i5LL9o4K8bU4R3zz0U

Scale = 1/23.1



TOTAL WEIGHT = 7 X 33 = 229 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	NO.2
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
B	486	486	11-4-10	11-4-10
D	486	486	11-4-10	11-4-10
F	560	560	11-4-10	11-4-10

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
B	359	209 / 0	58 / 0	0 / 0	92 / 0	0 / 0
D	359	209 / 0	58 / 0	0 / 0	92 / 0	0 / 0
F	436	183 / 0	124 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO	UNBRAC LENGTH	FR-TO			
A-B	0 / 15	-84.9 -84.9 0.04 (1)	10.00	F-C	-188 / 0	0.04 (1)	
B-H	-134 / 0	-84.9 -84.9 0.14 (1)	6.25	G-H	-534 / 54	0.00 (1)	
H-C	-235 / 0	-84.9 -84.9 0.35 (1)	6.25	I-J	-534 / 54	0.00 (1)	
C-J	-235 / 0	-84.9 -84.9 0.35 (1)	6.25				
J-D	-134 / 0	-84.9 -84.9 0.14 (1)	6.25				
D-E	0 / 15	-84.9 -84.9 0.04 (1)	10.00				
B-G	0 / 194	-38.5 -38.5 0.30 (1)	10.00				
G-F	0 / 194	-38.5 -38.5 0.37 (2)	10.00				
F-I	0 / 194	-38.5 -38.5 0.37 (2)	10.00				
I-D	0 / 194	-38.5 -38.5 0.30 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.35/1.00 (C-H:1), BC=0.37/1.00 (F-G:2), WB=0.04/1.00 (G-F:1), SSI=0.44/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

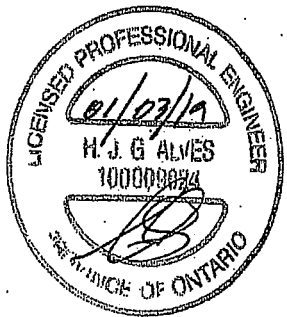
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.39 (D) (INPUT = 0.90)
JSI METAL = 0.11 (B) (INPUT = 1.00)



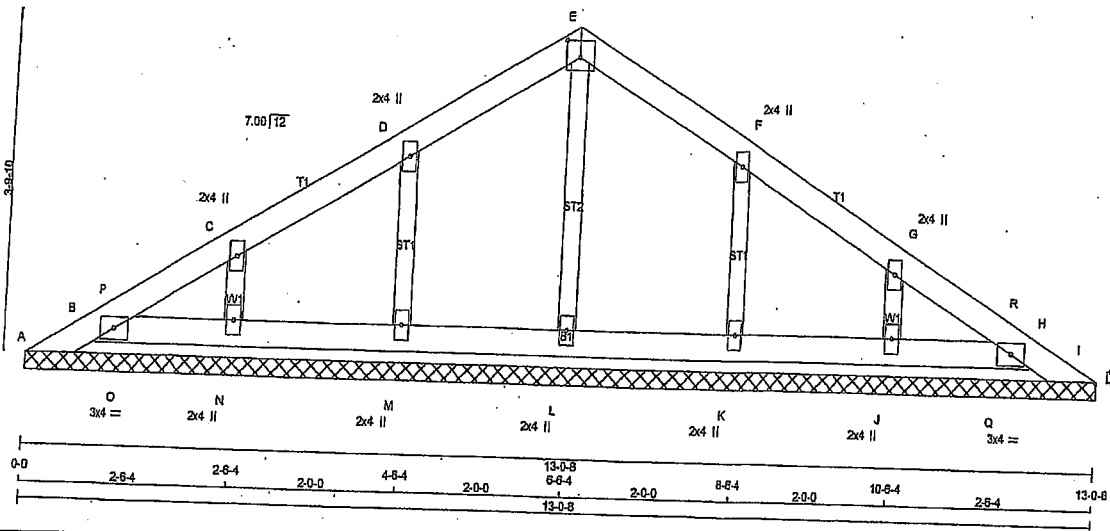
DRWG NO. 1AM 719 00126
STRUCTURAL
COMPONENT ONLY

JOB NAME 300800	TRUSS NAME P12G	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 Mitak Industries, Inc. Thu Jan 3 16:57:26 2019 Page 1
ID: Sfh-H7mWrkTvM7kCGx?VheICyrb3t-gRS_NknOV77WVxVHV3R4?ynLQVo4T8bU4Rl3zz20U

0-0 2-6-4 2-6-4 2-0-0 4-6-4 2-0-0 6-6-4 2-0-0 8-6-4 2-0-0 10-6-4 2-6-4 13-0-8
4x4 =

Scale: 1/2"=1'



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
B - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C, D, F, G				
C	TMW+w	MT20	2.0	4.0
E	TTW-p	MT20	4.0	4.0
H	TMB1-I	MT20	3.0	4.0
J, K, L, M, N				
J	BMW1+w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): L, M, K, J, N
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0/8	-102.4 -102.4	0.02 (1)	10.00	L-E	-121/0	0.03 (1)
B-P	-67/0	-84.9 -84.9	0.02 (1)	6.25	M-D	-195/0	0.03 (1)
P-C	-29/0	-84.9 -84.9	0.03 (1)	6.25	K-F	-195/0	0.03 (1)
C-D	-22/0	-84.9 -84.9	0.05 (1)	6.25	J-G	-145/0	0.02 (1)
D-E	-28/0	-84.9 -84.9	0.05 (1)	6.25	N-C	-145/0	0.02 (1)
E-F	-28/0	-84.9 -84.9	0.05 (1)	6.25	Q-P	0/25	0.00 (1)
F-G	-22/0	-84.9 -84.9	0.05 (1)	6.25	Q-R	0/25	0.00 (1)
G-R	-29/0	-84.9 -84.9	0.03 (1)	6.25			
R-H	-67/0	-84.9 -84.9	0.02 (1)	6.25			
H-I	0/8	-102.4 -102.4	0.02 (1)	10.00			
B-O	0/33	-38.5 -38.5	0.01 (1)	10.00			
O-N	0/33	-38.5 -38.5	0.03 (2)	10.00			
N-M	0/23	-38.5 -38.5	0.03 (3)	10.00			
M-L	0/16	-38.5 -38.5	0.03 (3)	10.00			
L-K	0/16	-38.5 -38.5	0.03 (3)	10.00			
K-J	0/23	-38.5 -38.5	0.03 (3)	10.00			
J-Q	0/33	-38.5 -38.5	0.03 (2)	10.00			
Q-H	0/33	-38.5 -38.5	0.01 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-E), BC=0.03/1.00 (J-K), WB=0.03/1.00 (F-K), SS=0.07/1.00 (E-F)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

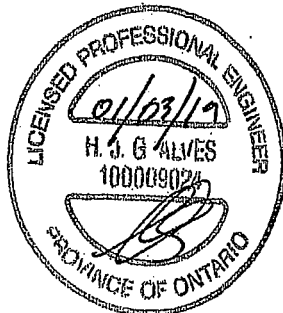
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1897 1656

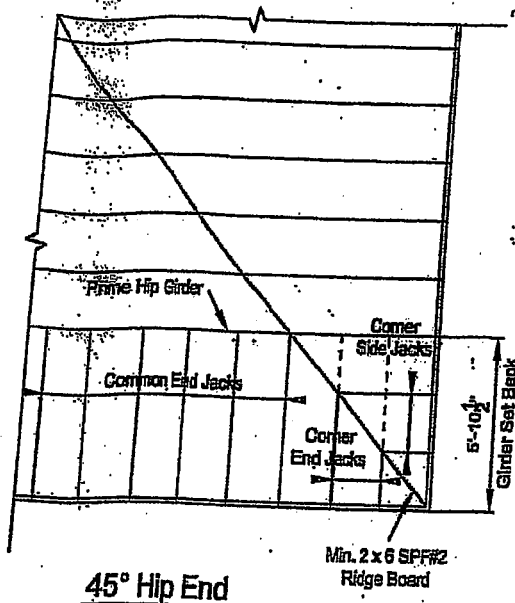
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.13 (D) (INPUT = 0.80)
JSI METAL= 0.09 (D) (INPUT = 1.00)



DRWG NO. TAM 11900127
STRUCTURAL
CONSTRUCTION ONLY



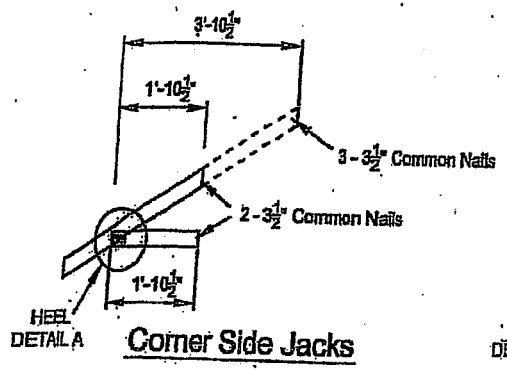
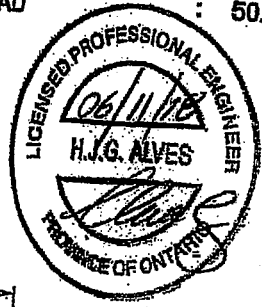
45° Hip End

LUMBER SPECIFICATION

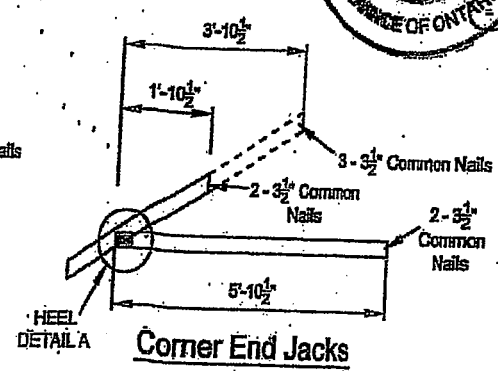
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

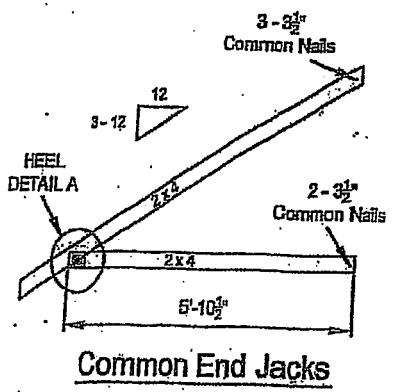
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 50.5 P.S.F.



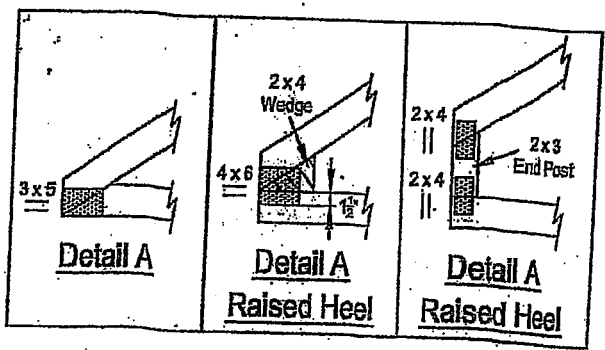
Corner Side Jacks



Corner End Jacks

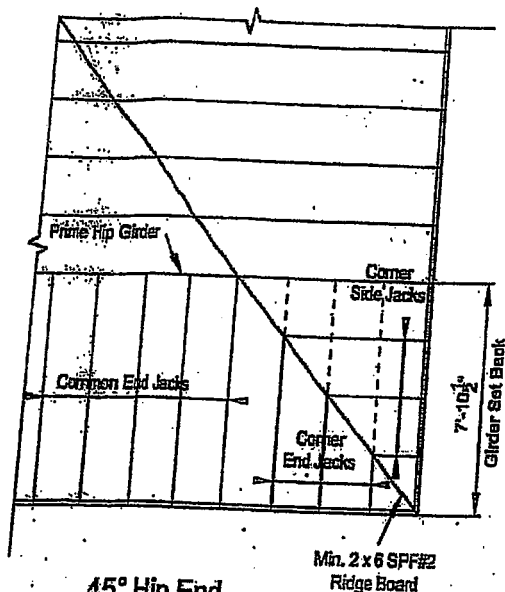


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 // S.D. DESIGN

T-1800211



45° Hip End

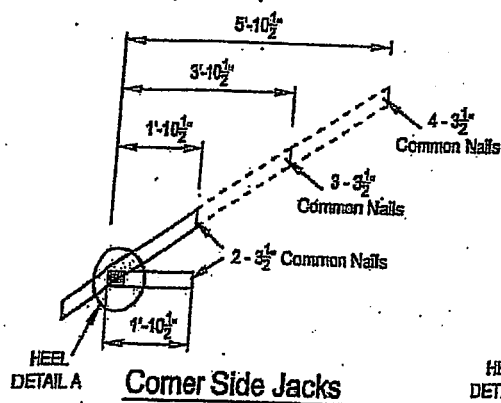
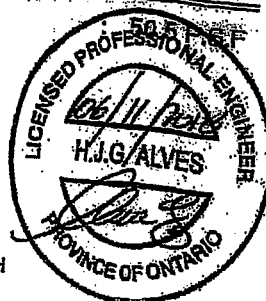
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

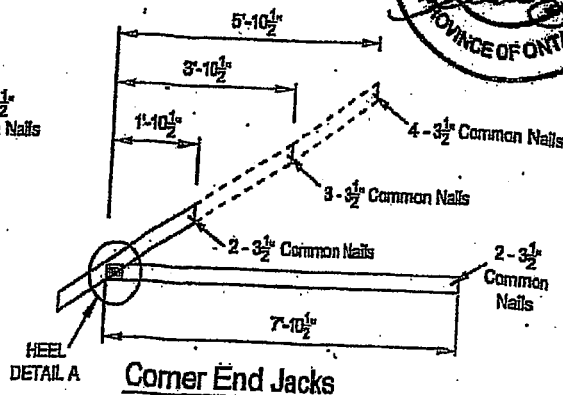
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

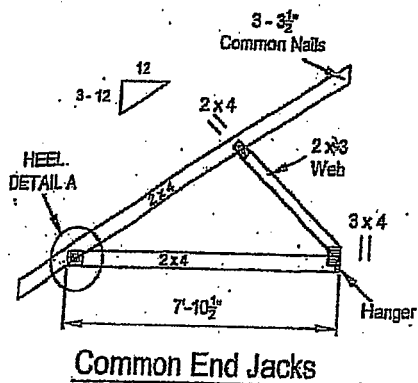
TOTAL LOAD



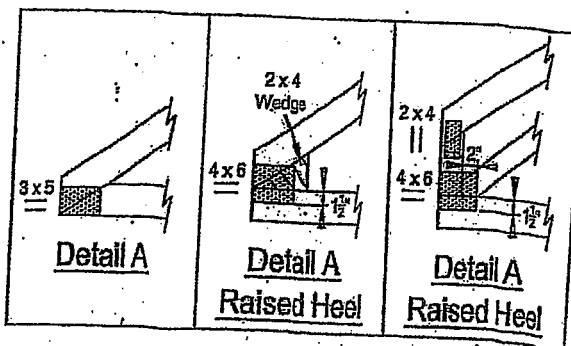
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

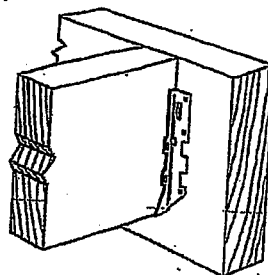
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

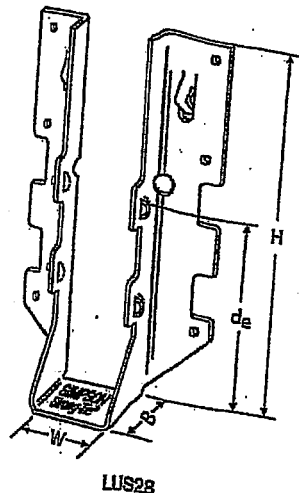
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 8 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

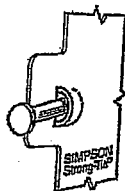


Typical LUS Installation



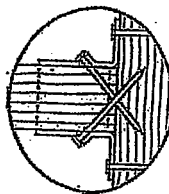
Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist		D.Fir-L		SPF	
									Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LUS24	18	1 1/8	3 1/8	1 1/4	1 3/8	(4) 10d	(2) 10d		710	1630	645	1155
LUS24-2	18	3 3/8	3 1/8	2	1 3/8	(4) 16d	(2) 16d		835	2020	590	1435
LUS26	18	1 1/8	4 3/8	1 1/4	3 3/8	(4) 10d	(4) 10d		1420	2170	1290	1630
LUS26-2	18	3 3/8	4 3/8	2	4	(4) 16d	(4) 16d		1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 3/4	(4) 16d	(4) 16d		1720	2595	1545	2340
LUS28	18	1 1/8	6 3/8	1 1/4	3 3/4	(6) 10d	(6) 10d		1420	2520	1290	1790
LUS28-2	18	3 3/8	7	2	4	(6) 16d	(4) 16d		1720	3325	1545	2675
LUS28-3	18	4 3/8	6 3/4	2	3 3/4	(6) 16d	(4) 16d		1720	3325	1545	2375
LUS210	18	1 1/8	7 3/8	1 1/4	3 3/8	(6) 10d	(4) 10d		1420	2785	1290	2210
LUS210-2	18	3 3/8	9	2	6	(6) 16d	(6) 16d		2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d		2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

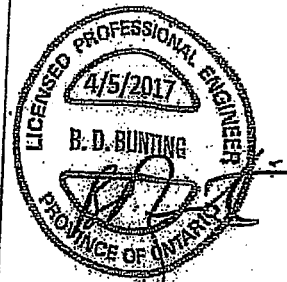


Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 6,603,590



Double Shear Nailing Top View.



DMH
STATS
DESIGN

1000 Highway 100, Unit 100, Mississauga, ON L4W 5G1
Tel: (905) 276-1111 Fax: (905) 276-1112
Email: info@dmhstatsdesign.com Website: www.dmhstatsdesign.com

(800) 999-3099
strongtie.com

HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

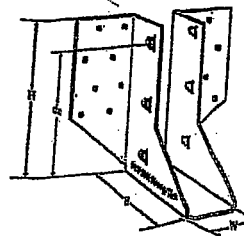
- Factored resistances are in accordance with CSA 086-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

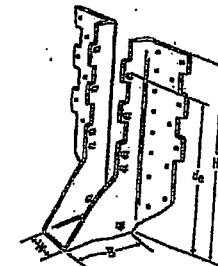
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

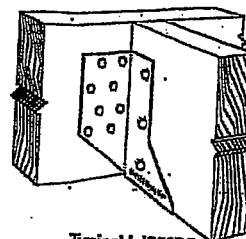
- See current catalogue for options



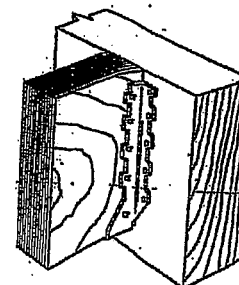
LJS26DS



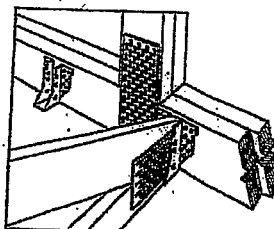
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



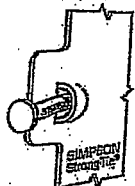
Typical HUS
Installation



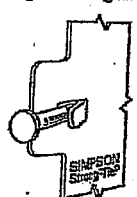
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist		D.F.P.		S.P.F.	
									Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/2	6	3 1/2	4 1/2	(16) 16d	(8) 16d		2055	4265	1480	4115
HUS26	16	1 1/2	5 1/2	3	3 1/2	(14) 16d	(8) 16d		2705	4940	2085	3875
HUS28	16	1 1/2	7 1/2	3	6 1/2	(22) 16d	(8) 16d		3605	5965	2675	4345
HUS210	16	1 1/2	9 1/2	3	7 1/2	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS181/10	16	1 1/2	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

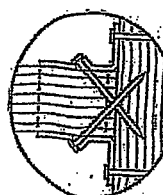
1. d_e is the distance from the seat of the hanger to the highest joist nail.



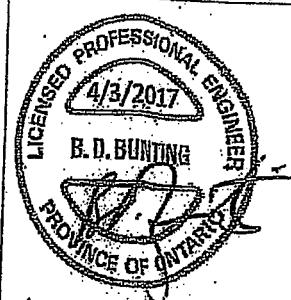
Double Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,590



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



For more information, visit us online at www.simpsonstrongtie.com or call 1-800-999-5099. We are your one-stop source for all your construction needs. Our products are made in the USA and are available in all 50 states and Canada. We are committed to providing you with the highest quality products and service. Please contact us today for more information.

(800) 999-5099
[simpsonstrongtie.com](http://www.simpsonstrongtie.com)

HGUS - Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

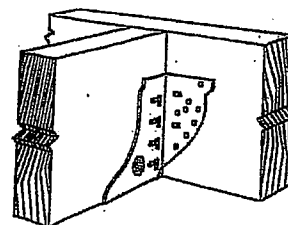
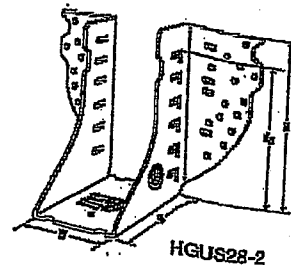
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

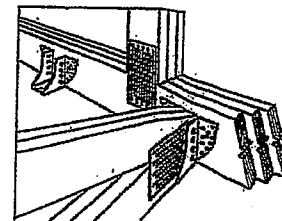
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



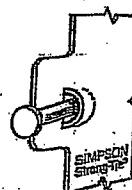
Typical HGUS Installation



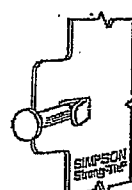
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁		Fane	Joist	D-Fit-L		S-P-F	
									Uplift (K _c =1.15)	Normal (K _c =1.00)	Uplift (K _c =1.15)	Normal (K _c =1.00)
HGUS26	12	1 1/2	5 1/2	5	4 1/2	(20) 16d	(8) 16d		2685	6625	2685	5700
HGUS26-2	12	3 3/4	5 1/2	4	4 1/2	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-3	12	4 3/4	5 1/2	4	4 1/2	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-4	12	6 3/4	5 1/2	4	4 1/2	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28	12	1 1/2	7 1/2	5	6 1/2	(36) 16d	(12) 16d		3310	7675	3100	6900
HGUS28-2	12	3 3/4	7 1/2	4	6 1/2	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-3	12	4 3/4	7 1/2	4	6 1/2	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-4	12	6 3/4	7 1/2	4	6 1/2	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS210-2	12	3 3/4	8 3/4	4	8 1/2	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-3	12	4 3/4	8 3/4	4	8 1/2	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-4	12	6 3/4	8 3/4	4	8 1/2	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS212-4	12	6 3/4	10 3/4	4	10 1/2	(66) 16d	(20) 16d		7840	14995	5425	10645
HGUS214-4	12	6 3/4	12 3/4	4	11 1/2	(66) 16d	(22) 16d		10130	16400	7185	11645

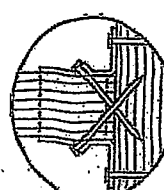
1. d₂ is the distance from the seat of the hanger to the highest joist nail.



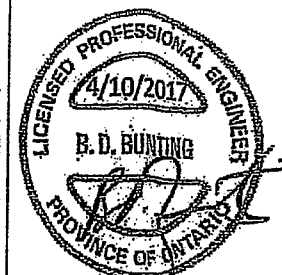
Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



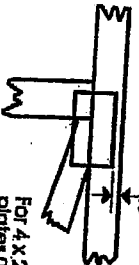
(800) 993-5093
strongtie.com

Symbols

PLATE ORIENTATION AND ORIENTATION



Symbol indicates orientation of plates. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, 1 or Eliminator bracing if indicated.



Indicates location where bearings (supports) occur. Icons vary but number where bearings occur.

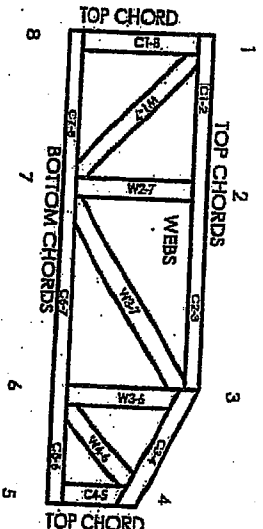
BEARING

DSB-99: Design Standard for Designing Building Component Safety Information, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in feet-inches or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED COUNTERCLOCKWISE AROUND THE JOINTS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11994-L, 10319-L, 13270-L, 12491-R

© 2007 Mitek® All Rights Reserved

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, indicated bracing for trusses may require bracing, or alternative 1, 1, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack material on inadequately braced trusses.
4. Provide copies of this truss design to the building department, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Slots and waste at joint locations are regulated by TPIC.
7. Design trusses trusses will be subject to protection from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 15% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire-retardant, preservative treated, or green lumber.
10. Compliance is a non-structural consideration and is the responsibility of the applicator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum/piling requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be braced or purlin provided at spacing indicated on design.
14. Bottom chords require lateral bracing of 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Mitek
FAILURE TO PERFORM™

Mitek Engineering Reference Sheet: MTK-74736 rev. 10-08

H - Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14

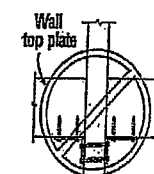
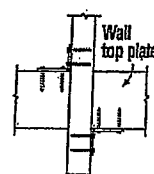
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

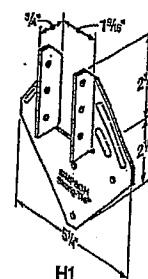
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

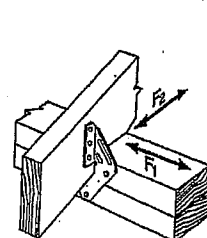


Install diagonally across from each other for minimum 2x truss.

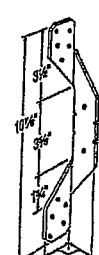
Nailing into both sides of a single ply 2x truss may cause the wood to split.



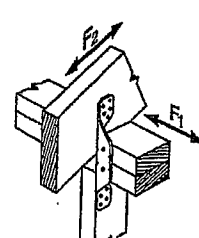
H1



H1 Installation



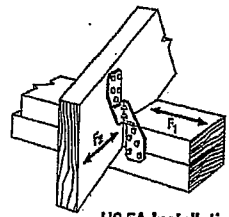
H2A



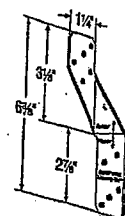
H2A Installation



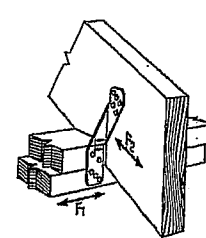
H2.5A



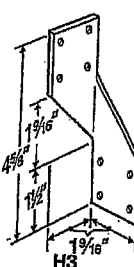
H2.5A Installation



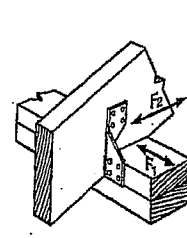
H2.5T



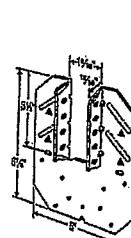
H2.5T Installation
(Nails into both top plates)



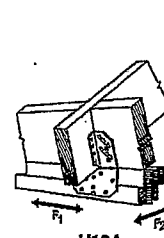
H3



H3 Installation



H10A



H10A Installation

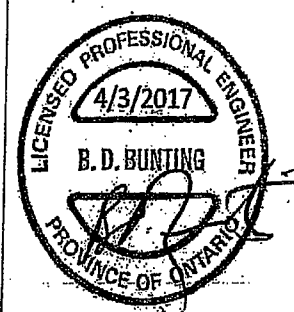
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	285	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



(800) 999-5099
strongtie.com

CP Crush Plates – Bearing Enhancers

SIMPSON
Strong-Tie

The CP transfers load from the truss or girder to plates for bearing limited conditions. Replaces nail-on scabs or in some cases, an additional ply when needed for bearing.

Material: See table

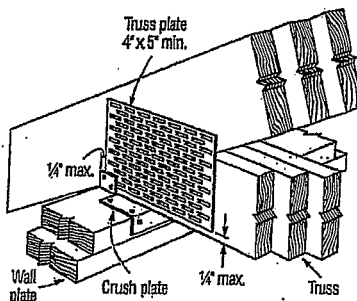
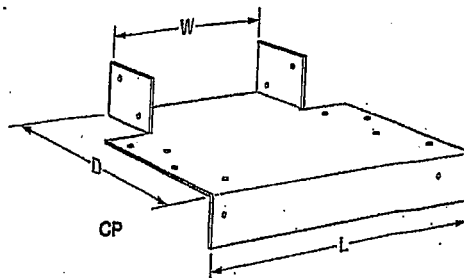
Finish: G90 galvanized

Design:

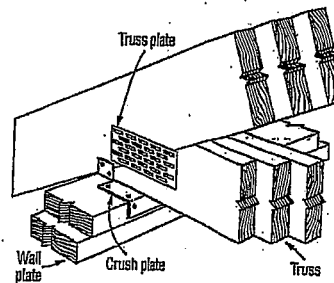
- Factored resistances are in accordance with CSA O86-14 assuming Q_r/A_b and $Q_r'/A_b' = 812$ psi for D.Fir-L and 615 psi for S-P-F. See clauses 6.5.4 and 7.5.9 TPIG 2014 when compression loads are applied to both sides of truss chord members at bearing locations.

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- For Case 1, truss plates must be located a maximum of ¼" from the underside of the truss chord and a maximum of ¼" from the edge of the wall plates in accordance with the reinforcing requirements of 7.5.9 TPIG 2014.



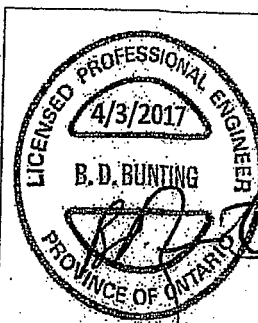
Case 1



Case 2

Model No.	Ga.	Wall Plate	Dimensions (in.)			Fasteners		Factored Resistance (lb.)				
			W	D	L	Wall Plate	Truss	D.Fir-L		S-P-F		
								Uplift	Bearing	Uplift	Bearing	
												(K _u =1.15)
CASE 1 (Truss Plate Reinforcement)												
CP1-4	20	2x4	1½	3½	4½	(6) 10d	(4) 10d x 1½"	225	5965	225	4515	
CP2-4	18		3¼	3½	5¼	(6) 10d	(4) 10d x 1½"	225	11390	225	9030	
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	17895	225	13545	
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	23860	225	18065	
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	9370	225	7095	
CP2-6	18		3¼	5½	5¼	(10) 10d	(4) 10d x 1½"	225	18740	225	14190	
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	28110	225	21285	
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	37495	225	28390	
CASE 2 (No Reinforcement)												
CP1-4	20	2x4	1½	3½	4½	(6) 10d	(4) 10d x 1½"	225	4685	225	3550	
CP2-4	18		3¼	3½	5¼	(6) 10d	(4) 10d x 1½"	225	9370	225	7100	
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	14055	225	10650	
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	18750	225	14195	
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	7365	225	5575	
CP2-6	18		3¼	5½	5¼	(10) 10d	(4) 10d x 1½"	225	14730	225	11150	
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	22095	225	16725	
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	29460	225	22305	

1. Factored bearing resistances assume wall plate and truss are the same species. For a mixed species system use S-P-F values.



This technical bulletin is effective until June 30, 2019, and reflects information available on March 1, 2019. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

TS/LTS/MTS/HTS *Twist Straps*

SIMPSON
Strong-Tie

Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3" bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

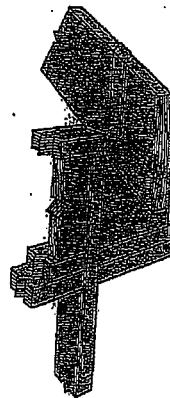
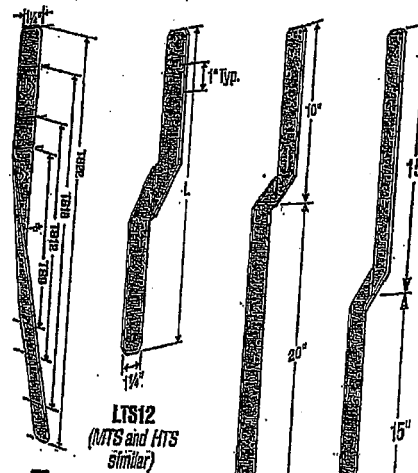
MATERIAL: LTS-18 gauge; MTS-18 gauge; HTS-14 gauge; TS-16 gauge
FINISH: Galvanized. Some products available in stainless steel and ZMAX®; see Corrosion Information, pages 14-17.

INSTALLATION: Use all specified fasteners. See General Notes.

- TS should be installed in pairs to reduce eccentricity.
- When LTS/MTS is installed as Truss-to-Top Plate Tie, install 3-10d $\times$ 1½" nails to the w/s of the plate and 3-10d $\times$ 1½" nails into the edge of the double top plate.

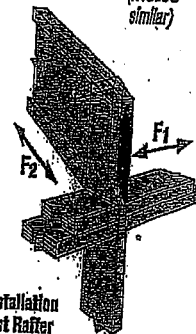
These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

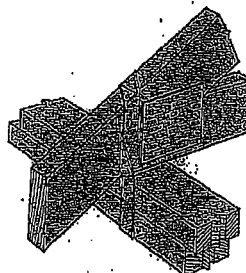


Typical MTS30 installation

MTS30 installation with I-Joist Rafter



Typical TS installation



LTS/MTS installation as a Truss-to-Top Plate Tie (See note 7)



Model	Length (in)	Fasteners (10d)	Factored Resistance (k = 1.16)	
			lbs	lbs
TS9	9	8-16d	1125	1040
TS12	12	10-16d	1410	1300
TS18	18	14-16d	1870	1820
TS22	22	18-16d	2125	2125
MTS12	12	14-10d $\times$ 1½	1015	720
MTS16	16	14-10d $\times$ 1½	1015	720
MTS18	18	14-10d $\times$ 1½	1015	720
MTS20	20	14-10d $\times$ 1½	1015	720
MTS30	30	14-10d $\times$ 1½	1570	1180
MTS30C	30	14-10d $\times$ 1½	1570	1180
HTS16	16	16-10d $\times$ 1½	2050	1455
HTS20	20	24-10d $\times$ 1½	2050	1455
HTS24	24	24-10d $\times$ 1½	2050	1455
HTS28	28	24-10d $\times$ 1½	2050	1455

1. LTS12 thru LTS20, MTS18 through MTS30, HTS24 through HTS30C (except HTS30C) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 16% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24' c/c for (part 9) and not exceeding 48' for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

Feb 09, 2018

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information		Application number:													
Building number, street name		Unit no. UNITS 1 - 6	Lot/con. BLOCK 2												
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description													
B. Individual who reviews and takes responsibility for design activities															
Name SAM KATSOULAKOS		Firm MICRO CITY ENGINEERING SERVICES INC.													
Street address R.R #1, PO BOX 61		Unit no.	Lot/con.												
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail												
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750	Cell number													
C. Design activities undertaken by individual identified in Section B. (Building Code Table 3.5.2.1. of Division C)															
<table style="width: 100%; border: none;"> <tr> <td>☐ House</td> <td>☐ HVAC - House</td> <td>☒ Building Structural</td> </tr> <tr> <td>☐ Small Buildings</td> <td>☐ Building Services</td> <td>☐ Plumbing - House</td> </tr> <tr> <td>☐ Large Buildings</td> <td>☐ Detection, Lighting and Power</td> <td>☐ Plumbing - All Buildings</td> </tr> <tr> <td>☐ Complex Buildings</td> <td>☐ Fire Protection</td> <td>☐ On-site Sewage Systems</td> </tr> </table>				☐ House	☐ HVAC - House	☒ Building Structural	☐ Small Buildings	☐ Building Services	☐ Plumbing - House	☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing - All Buildings	☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems
☐ House	☐ HVAC - House	☒ Building Structural													
☐ Small Buildings	☐ Building Services	☐ Plumbing - House													
☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing - All Buildings													
☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems													
Description of designer's work ROYAL PINE HOMES - FORESTSIDE PH6 - BLOCK 2 - UNITS 1 - 6 (SCHEDULE IS ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM1141-19 DATED 1-18-19). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.															
D. Declaration of Designer															
I, SAM KATSOULAKOS declare that (choose one as appropriate): (print name)															
☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.															
Individual BCIN: <u>26064</u>															
Firm BCIN: <u>29991</u>															
☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code.															
Individual BCIN: _____															
Basis for exemption from registration: _____															
☐ The design work is exempt from the registration and qualification requirements of the Building Code.															
Basis for exemption from registration and qualification: _____															
I certify that:															
1. The information contained in this schedule is true to the best of my knowledge.															
2. I have submitted this application with the knowledge and consent of the firm.															
Date <u>1-18-19</u>		Signature of Designer 													

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d). of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG #TAM 1141-19S
 DWG #TAM 1142-19S