JANUARY 18, 2019
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300784)
DWG #T1901090 THROUGH DWG #T1901092, INCLUSIVE, AND ALL DATED 1-15-19;
DWG #T1900060 AND DWG #T1900066 AND BOTH DATED 1-03-19;
DWG #T1900063 AND DWG #T1900064 AND BOTH DATED 1-03-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

JANUARY 18, 2019
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300799)
DWG #T1900044 THROUGH DWG #T1900046, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900114; DWG #T1900123; DWG #T1900127; AND ALL DATED 1-03-19;
DWG #T1900116; DWG #T1900117; DWG #T1900035; AND ALL DATED 1-03-19;
DWG #T1900062; DWG #T1900119; DWG #T1900120; AND ALL DATED 1-03-19;
DWG #T1900039; DWG #T1900040; DWG #T1900042; AND ALL DATED 1-03-19;
DWG #T1901093 AND DWG #T1901094 AND BOTH DATED 1-15-19;
DWG #T1900150 DATED 1-03-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

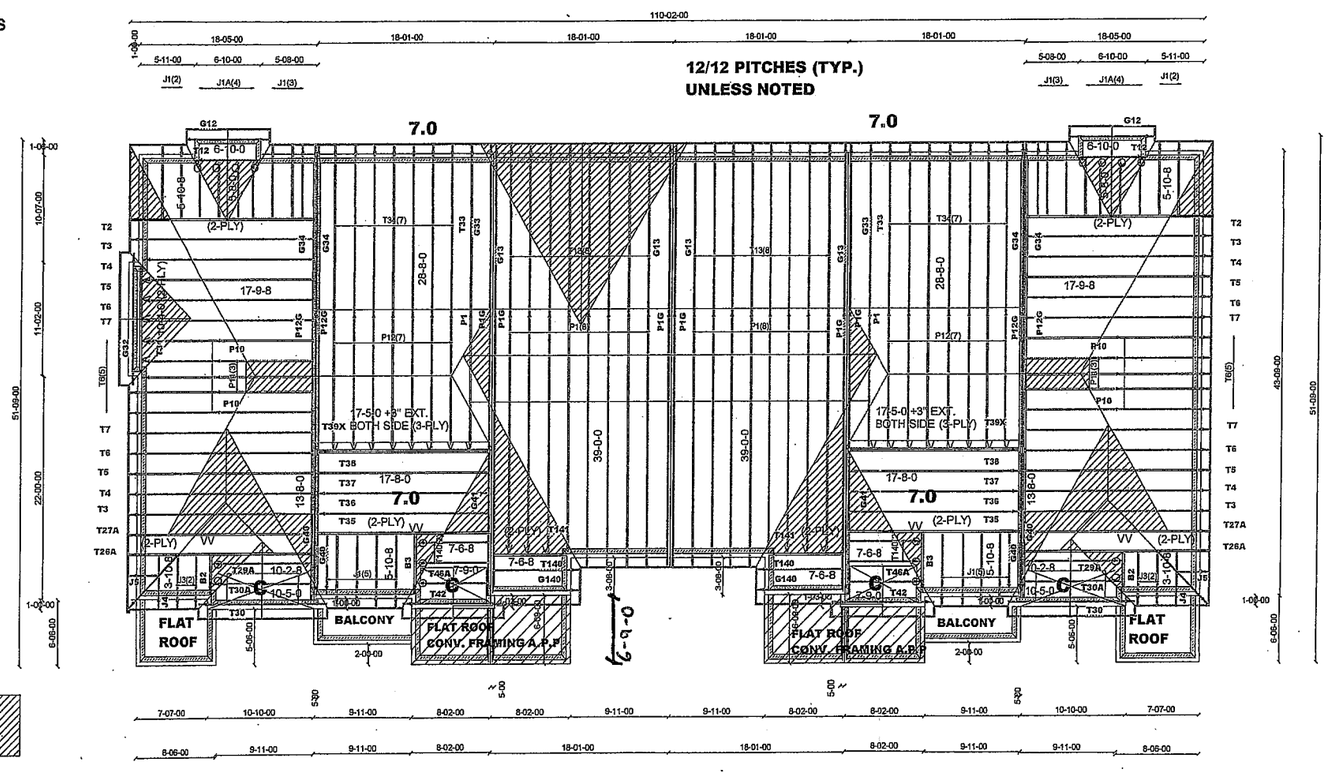
LUS26-2 -(VV)
LUS24 -(O)
LJS26DS -(V)

DENOTE:

C- 8/12 CATH. CEILING

UNIT1804 CORNER	UNIT1803	UNIT1802	UNIT1801	UNIT1803	UNIT1804 END
B -LOT1	B(REV) -LOT2	B -LOT3	B(REV) -LOT4	B -LOT5	B(REV) -LOT6
300868	300800	300784	300784	300800	300799

12/12 PITCHES (TYP.)
UNLESS NOTED



T-180485

DENOTES:
CONVENTIONAL FRAMING

DATE: 1-18-19
BCIN:26064; FIRM:BCIN #29991
ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER
ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO TRUSS INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT.
ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY THE BUILDING DESIGNER AND/OR
PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD
CAPACITY WITH RESPECT TO THE ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION. LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH
INDIVIDUAL LOT.

DWG #TAM..... THROUGH DWG #TAM..... , INCLUSIVE, DATED ;
(STRUCTURAL COMPONENTS ONLY); THIS LAYOUT MUST BE READ TOGETHER WITH REFERENCED TRUSS COMPONENT DRAWINGS AND SEALED HARDWARE DRAWINGS.

HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY.
REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING REQUIREMENTS:
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2 X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS SPECIFIED ON REFERENCED SEALED TRUSS COMPONENT
DRAWINGS USING 2 - 3-1/4" COMMON WIRE NAILS PER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF 'X-BRACING' ALONG THESE SAME WEB MEMBERS
AT LOCATIONS INDICATED (*) USING THE SAME NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
PROVIDE 2 X 4 #2 SPF T-BRACE ALONG FULL LENGTH OF LATERALLY BRACED WEB MEMBERS OF 1 PLY TRUSSES COMPONENTS REQUIRING 'T-BRACE'
USING 1 ROW OF 3-1/4" COMMON WIRE NAILS AT 6" O/C. SIMILARLY PROVIDE 2 X 6 BRACE WITH 2 ROWS OF NAILING FOR 2 PLY TRUSS COMPONENTS AND 2 X 8 BRACE WITH 3 ROWS OF NAILING FOR 3 PLY TRUSS COMPONENTS.

(1) 2 X 6 #2 SPF ON FLAT OVER PLYWOOD SHEATHING.
(2) 2 X 6 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS COMPONENTS AND AT LOCATIONS MARKED BY 'X'.
(3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O/C WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X 4 VERTICAL SUPPORTS TO TRUSS BASE BELOW WHERE NECESSARY.

ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE TRUSS COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH STUDS AT 24" O/C.
ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE 2012.

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN
WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION
3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED
AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES
AND/OR CATEGORIES.
REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.
DWG #TAM
BCIN: 26064 1-18-19
FIRM: 29991
SEALED STRUCTURAL
COMPONENTS ONLY

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 23.3 PSF
TC DEAD 15 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

JANUARY 18, 2019
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300868)
DWG #T1900044 THROUGH DWG #T1900046, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900035; DWG #T1900039; DWG #T1900040; AND ALL DATED 1-03-19;
DWG #T1900042; DWG #T1900062; DWG #T1900114; AND ALL DATED 1-03-19;
DWG #T1900116; DWG #T1900117; DWG #T1900123; AND ALL DATED 1-03-19;
DWG #T1900119; DWG #T1900120; DWG #T1900127; AND ALL DATED 1-03-19;
DWG #T1901093; DWG #T1901094; DWG #T1901098; AND ALL DATED 1-03-19;
DWG #T1900150 AND DWG #T1900151 AND BOTH DATED 1-03-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

JANUARY 18, 2019
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300800)
DWG #T1900126 THROUGH DWG #T1900136, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900114; DWG #T1900123; DWG #T1900124; AND ALL DATED 1-03-19;
DWG #T1900063; DWG #T1900064; DWG #T1900121; AND ALL DATED 1-03-19;
DWG #T1901091 DATED 1-15-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track: 41239	Builder / Location: ROYAL PINE HOMES/ BRAMPTON	Model / Elevation: BLOCK5 / LOTS 1TO6	Mitek ver 7.4.2 C:\MITEK\CA742A\J OBS\B13000000006
Layout ID: 300867	Project: FORESTSIDE PH.6	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.	
Plan Log: 98887	Date: 11/16/2018 Designer: SUNNY		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

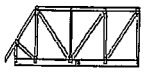
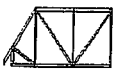
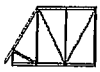
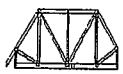
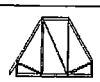
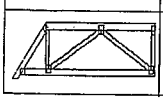
Delivery Shiplist

DATE	09/14/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300868 LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
MODEL: UNIT1804 CORNER ELEVATION: B LOT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	G34 GABLE	7.00 / 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 09-07-03	171.71 / 109.00		
	1	G40 GABLE	7.00 / 0.00	13-08-00	09-03-02	2 X 4	2 X 4	00-00-00 / 00-00-00	01-03-07 / 09-03-02	68.36 / 43.83		
	1 / 2 Ply	T2 HALF HIP	12.00 / 0.00	17-09-08	04-08-09	2 X 4	2 X 6	01-03-08 / 00-00-00	01-10-08 / 04-08-09	199.00 / 121.34		
	2	T3 HALF HIP	12.00 / 0.00	17-09-08	05-10-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 05-10-09	156.36 / 98.00		
	2	T4 HALF HIP	12.00 / 0.00	17-09-08	07-00-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 07-00-09	166.94 / 104.66		
	2	T5 HALF HIP	12.00 / 0.00	17-09-08	08-02-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 08-02-09	185.34 / 117.34		
	7	T6 HALF HIP	12.00 / 0.00	17-09-08	09-04-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 09-04-09	737.94 / 462.00		
	2	T7 HALF HIP	12.00 / 0.00	17-09-08	10-06-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 10-06-09	208.64 / 127.66		
	1 / 2 Ply	T26A HIP GIRDER	12.00 / 0.00	17-09-08	05-09-00	2 X 4	2 X 6	01-03-08 / 00-00-00	01-10-08 / 02-01-00	205.94 / 124.00		
	1	T27A HIP	12.00 / 0.00	17-09-08	07-09-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 02-01-00	80.84 / 51.67		
	1	T29A COMMON	12.00 / 0.00	10-02-08	07-01-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 02-01-00	47.71 / 31.00		
	1	T30 ROOF	12.00 / 8.00	10-05-00	07-01-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	55.38 / 36.33		
	1	T30A ROOF	12.00 / 8.00	10-02-08	07-01-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 02-01-00	53.04 / 35.00		
	1 / 2 Ply	T31 COMMON	12.00 / 0.00	10-04-00	07-00-08	2 X 4	2 X 6	00-00-00 / 00-00-00	01-10-08 / 01-10-08	118.10 / 76.00		
	1	G32 COMMON	12.00 / 0.00	10-04-00	07-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	55.75 / 36.83		
	1	T12 COMMON	12.00 / 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 / 00-10-08	01-05-08 / 01-05-08	33.29 / 23.17		
	1	G12 GABLE	12.00 / 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 / 00-10-08	01-05-08 / 01-05-08	34.17 / 24.17		
	2	P10 PIGGYBACK	12.00 / 0.00	09-08-12	02-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-05-03 / 02-02-08	67.46 / 44.00		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

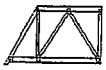
Delivery Shiplist

DATE	09/14/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300868 LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
MODEL: UNIT1804 CORNER ELEVATION: B LOT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	3	P11 PIGGYBACK	12.00 / 0.00	09-08-12	03-04-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-05-03 / 03-04-08	110.34 / 69.51		
	1	P12G GABLE	7.00 / 0.00	11-04-10	03-08-02	2 X 4	2 X 4	00-00-00 / 00-00-00	00-00-00 / 00-00-00	35.84 / 22.67		
	5	J1 JACK-OPEN	7.00 / 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 04-08-09	86.50 / 53.35		
	4	J1A JACK-OPEN	7.00 / 0.00	05-08-00	04-08-09	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-14 / 04-08-09	60.88 / 37.32		
	2	J3 JACK-OPEN	12.00 / 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 05-09-00	29.50 / 18.66		
	1	J4 JACK-OPEN	12.00 / 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 / 02-01-01	01-10-08 / 00-03-08	11.39 / 7.33		
	1	J5 JACK-OPEN	12.00 / 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 / 00-01-01	01-10-08 / 00-03-08	9.10 / 6.00		

TOTAL # TRUSS= **49.00** TOTAL BFT OF ALL TRUSSES= **1880.84** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2989.52** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
5	Hangers	LJS26DS	
6	Hangers	LUS24	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

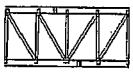
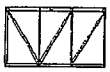
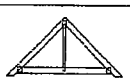
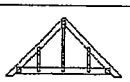
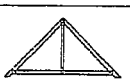
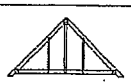
Delivery Shiplist

DATE	09/14/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300800 LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
MODEL: UNIT1803 ELEVATION: B LOT2 OR LOT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T33 PIGGYBACK	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 07-03-12	150.22 94.33		
	1	G33 GABLE	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 07-03-12	167.03 106.17		
	7	T34 PIGGYBACK	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 09-07-03	1055.25 659.19		
	1	G34 GABLE	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 09-07-03	171.71 109.00		
	1 2 Ply	T35 FLAT GIRDER	0.00 0.00	17-08-00	04-08-09	2 X 6	2 X 6	00-00-00 00-00-00	04-08-09 04-08-09	220.58 134.68		
	1	T36 FLAT	0.00 0.00	17-08-00	05-10-09	2 X 4	2 X 4	00-00-00 00-00-00	05-10-09 05-10-09	79.76 50.00		
	1	T37 FLAT	0.00 0.00	17-08-00	07-00-09	2 X 4	2 X 4	00-00-00 00-00-00	07-00-09 07-00-09	86.38 53.83		
	1	T38 FLAT	0.00 0.00	17-08-00	08-02-09	2 X 4	2 X 4	00-00-00 00-00-00	08-02-09 08-02-09	93.18 57.67		
	1 3 Ply	T39X FLAT GIRDER	0.00 0.00	17-05-00	09-04-09	2 X 6	2 X 6	00-00-00 00-00-00	00-05-08 00-05-08	498.06 311.01		
	1	T42 SCISSOR	12.00 8.00	07-09-00	05-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	41.86 26.83		
	1	T46A SCISSOR	12.00 8.00	07-06-08	09-05-00	2 X 4	2 X 4	00-00-00 00-00-00	01-10-08 09-05-00	46.44 29.50		
	2	T140 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	93.70 59.66		
	1	G40 GABLE	7.00 0.00	13-08-00	09-03-02	2 X 4	2 X 4	00-00-00 00-00-00	01-03-07 09-03-02	68.36 43.83		
	1	G41 GABLE	0.00 0.00	13-08-00	09-05-04	2 X 4	2 X 4	00-00-00 00-00-00	09-05-04 09-05-04	94.63 57.67		
	1	P1 PIGGYBACK	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	22.11 14.33		
	1	P1G GABLE	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	23.42 15.33		
	7	P12 PIGGYBACK	7.00 0.00	11-04-10	03-08-02	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	228.76 144.69		
	1	P12G GABLE	7.00 0.00	11-04-10	03-08-02	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	35.84 22.67		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/14/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300800 LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
MODEL: UNIT1803 ELEVATION: B LOT2 OR LOT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	86.50 53.35		

TOTAL # TRUSS= **39.00** TOTAL BFT OF ALL TRUSSES= **2043.74** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3263.79** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

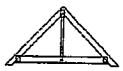
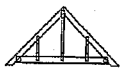
Delivery Shiplist

DATE	09/14/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300784 LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
MODEL: UNIT1801 OR UNIT1802 ELEVATION: B LOT3 OR LOT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	8	T13 PIGGYBACK	7.00 0.00	39-00-00	10-00-00	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	1640.08 1016.00		
	2	G13 GABLE	7.00 0.00	39-00-00	10-00-00	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	453.88 284.66		
	1	T140 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	46.85 29.83		
	1	G140 GABLE	12.00 0.00	07-06-08	09-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	48.08 29.83		
	1 2 Ply	T141 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4	2 X 6	00-00-00 00-00-00	01-10-08 09-05-00	109.56 66.66		
	8	P1 PIGGYBACK	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	176.88 114.64		
	2	P1G GABLE	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	46.84 30.66		

TOTAL # TRUSS= **24.00** TOTAL BFT OF ALL TRUSSES= **1572.28** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2522.17** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LJS26DS	

TOTAL # ITEMS= **3.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

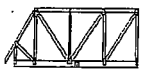
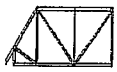
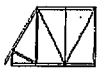
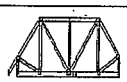
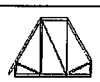
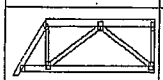
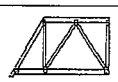
Delivery Shiplist

DATE	09/14/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300799 LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
MODEL: UNIT1804 END ELEVATION: B LOT6

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	G40 GABLE	7.00 0.00	13-08-00	09-03-02	2 X 4	2 X 4	00-00-00 00-00-00	01-03-07 09-03-02	68.36 43.83		
	1	G34 GABLE	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 09-07-03	171.71 109.00		
	1 2 Ply	T2 HALF HIP	12.00 0.00	17-09-08	04-08-09	2 X 4	2 X 6	01-03-08 00-00-00	01-10-08 04-08-09	199.00 121.34		
	2	T3 HALF HIP	12.00 0.00	17-09-08	05-10-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-10-09	156.36 98.00		
	2	T4 HALF HIP	12.00 0.00	17-09-08	07-00-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 07-00-09	166.94 104.66		
	2	T5 HALF HIP	12.00 0.00	17-09-08	08-02-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 08-02-09	185.34 117.34		
	7	T6 HALF HIP	12.00 0.00	17-09-08	09-04-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-04-09	737.94 462.00		
	2	T7 HALF HIP	12.00 0.00	17-09-08	10-06-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 10-06-09	208.64 127.66		
	1 2 Ply	T26A HIP GIRDER	12.00 0.00	17-09-08	05-09-00	2 X 4	2 X 6	01-03-08 00-00-00	01-10-08 02-01-00	205.94 124.00		
	1	T27A HIP	12.00 0.00	17-09-08	07-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	80.84 51.67		
	1	T29A COMMON	12.00 0.00	10-02-08	07-01-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	47.71 31.00		
	1	T30 ROOF	12.00 8.00	10-05-00	07-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	55.38 36.33		
	1	T30A ROOF	12.00 8.00	10-02-08	07-01-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	53.04 35.00		
	1	T12 COMMON	12.00 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	33.29 23.17		
	1	G12 GABLE	12.00 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	34.17 24.17		
	2	P10 PIGGYBACK	12.00 0.00	09-08-12	02-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 02-02-08	67.46 44.00		
	3	P11 PIGGYBACK	12.00 0.00	09-08-12	03-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 03-04-08	110.34 69.51		
	1	P12G PIGGYBACK	7.00 0.00	11-04-10	03-08-02	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	35.84 22.67		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/14/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300799 LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
MODEL: UNIT1804 END ELEVATION: B LOT6

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-04-15	86.50 53.35		
	4	J1A JACK-OPEN	7.00 0.00	05-08-00	04-08-09	2 X 4	2 X 4	00-00-00 00-00-00	01-04-14 04-08-09	60.88 37.32		
	2	J3 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	29.50 18.66		
	1	J4 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 02-01-01	01-10-08 00-03-08	11.39 7.33		
	1	J5 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 00-01-01	01-10-08 00-03-08	9.10 6.00		

TOTAL # TRUSS= **46.00** TOTAL BFT OF ALL TRUSSES= **1768.01** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2815.67** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
6	Hangers	LUS24	

TOTAL # ITEMS= **7.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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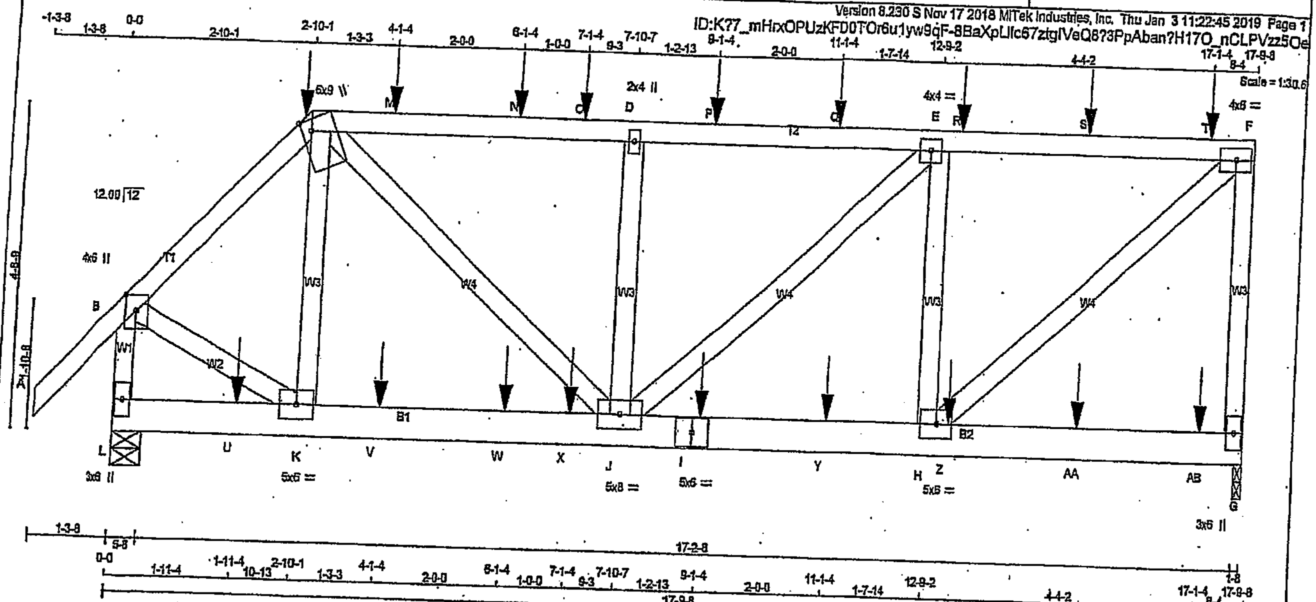

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x8	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-G	1	12	TOP
L-B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L-I	2	12	SIDE(0.0)
I-G	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	6.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2244	0	2244	0	0	1-8	1-8
L	2221	0	2221	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
G	1701	850 / 0	379 / 0	0 / 0	0 / 0	472 / 0	0 / 0
L	1676	862 / 0	355 / 0	0 / 0	0 / 0	459 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.07 (1)	10.00
B-C	-1854 / 0	-84.9	-84.9	0.08 (1)	6.25
C-M	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35
M-N	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35
N-O	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35
O-D	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35
D-P	-2381 / 0	-84.9	-84.9	0.36 (1)	5.35
P-Q	-2381 / 0	-84.9	-84.9	0.36 (1)	5.35
Q-E	-2381 / 0	-84.9	-84.9	0.36 (1)	5.35
E-R	-1958 / 0	-84.9	-84.9	0.32 (1)	5.8[illegible]
R-S	-1958 / 0	-84.9	-84.9	0.32 (1)	5.8[illegible]
S-T	-1958 / 0	-84.9	-84.9	0.32 (1)	5.8[illegible]
T-F	-1958 / 0	-84.9	-84.9	0.32 (1)	5.8[illegible]
G-F	-2065 / 0	0.0	0.0	0.35 (1)	7.6[illegible]
L-B	-2185 / 0	0.0	0.0	0.13 (1)	7.53
L-U	0 / 0	-38.5	-38.5	0.07 (3)	10.00
U-K	0 / 0	-38.5	-38.5	0.07 (3)	10.00
K-V	0 / 1294	-38.5	-38.5	0.14 (2)	10.00
V-W	0 / 1294	-38.5	-38.5	0.14 (2)	10.00
W-X	0 / 1294	-38.5	-38.5	0.14 (2)	10.00
X-J	0 / 1294	-38.5	-38.5	0.14 (2)	10.00
J-I	0 / 1958	-38.5	-38.5	0.19 (2)	10.00
I-Y	0 / 1958	-38.5	-38.5	0.19 (2)	10.00
Y-H	0 / 1958	-38.5	-38.5	0.19 (2)	10.00
H-Z	0 / 0	-38.5	-38.5	0.08 (3)	10.00
Z-AA	0 / 0	-38.5	-38.5	0.08 (3)	10.00
AA-AB	0 / 0	-38.5	-38.5	0.08 (3)	10.00
AB-G	0 / 0	-38.5	-38.5	0.08 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	-274 / 105	0.03 (1)
C-J	0 / 1431	0.13 (1)
J-D	[illegible]	[illegible]
J-E	[illegible]	[illegible]
H-E	[illegible]	[illegible]
[illegible]	0 / [illegible]	[illegible]
B-[illegible]	0 / 1447	0.13 (1)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1900039
STRUCTURAL
COMPONENT ONLY
1/2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-10-1	-35	-39	—	FRONT	VERT	DEAD	—	—
C	2-10-1	-162	-162	—	FRONT	VERT	SNOW	—	—
I	9-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
M	4-1-4	-164	-164	—	BACK	VERT	TOTAL	—	—
N	6-1-4	-156	-156	—	BACK	VERT	TOTAL	—	—
O	7-1-4	-156	-156	—	BACK	VERT	TOTAL	—	—
P	9-1-4	-156	-156	—	BACK	VERT	TOTAL	—	—
Q	11-1-4	-156	-156	—	BACK	VERT	TOTAL	—	—

TOTAL WEIGHT = 2 X 99 = 199 lb

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.19/1.00 (H-J:2) , WB=0.23/1.00 (F-H:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg. .

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T2	1	2	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	13-1-4	-164	-164	—	BACK	VERT	TOTAL	—	—
S	15-1-4	-164	-164	—	BACK	VERT	TOTAL	—	—
T	17-1-4	-178	-178	—	BACK	VERT	TOTAL	—	—
U	1-11-4	-75	-95	—	BACK	VERT	TOTAL	—	—
V	4-1-4	-75	-95	—	BACK	VERT	TOTAL	—	—
W	6-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
X	7-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
Y	11-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
Z	13-1-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AA	15-1-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AB	17-1-4	-81	-103	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/02/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1900039
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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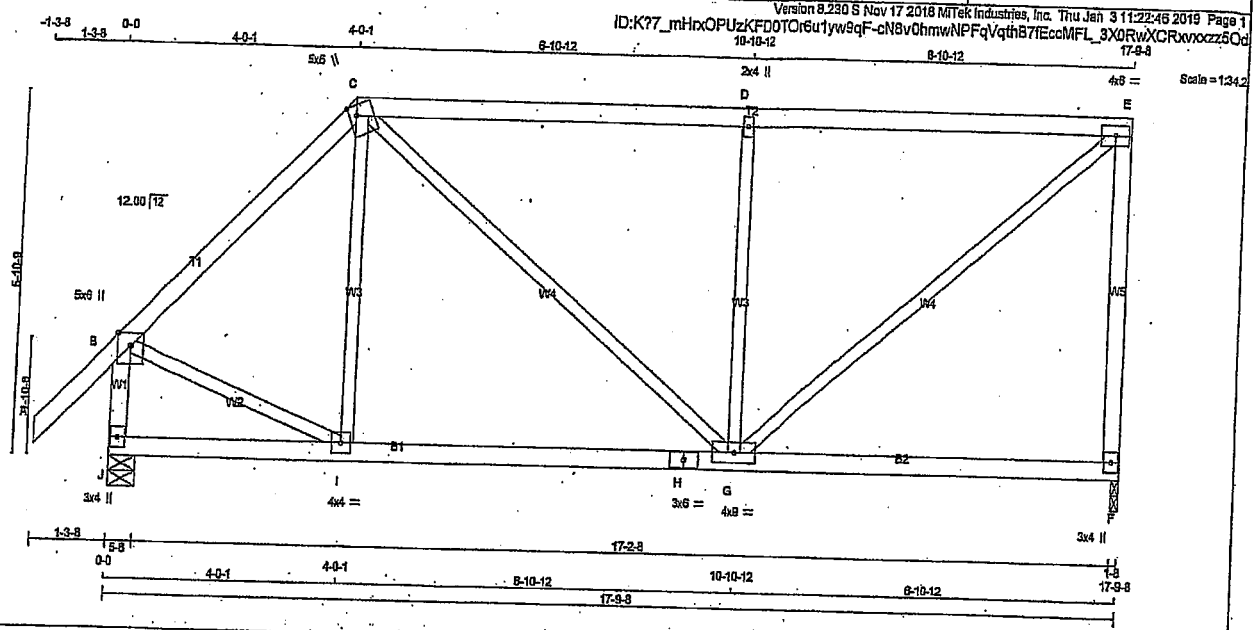


TOTAL WEIGHT = 78 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECRD BRG IN-SX
F	1097	0	1097	0	0	1-8	1-8
J	1216	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	832	414 / 0	167 / 0	0 / 0	0 / 0	231 / 0	0 / 0
J	914	479 / 0	167 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

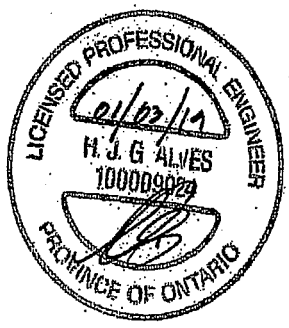
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT: LOAD LC1 FROM	TO	MAX (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	
B-C	-900 / 0	-84.9	-84.9	0.26 (1)	6.16	
C-D	-952 / 0	-84.9	-84.9	0.75 (1)	5.02	
D-E	-952 / 0	-84.9	-84.9	0.75 (1)	5.02	
F-E	-991 / 0	0.0	0.0	0.64 (1)	7.81	
J-B	-1162 / 0	0.0	0.0	0.13 (1)	7.39	
J-I	0 / 0	-38.5	-38.5	0.19 (3)	10.00	
I-H	0 / 634	-38.5	-38.5	0.42 (2)	10.00	
H-G	0 / 634	-38.5	-38.5	0.42 (2)	10.00	
G-F	0 / 0	-38.5	-38.5	0.36 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	-13 / 154	0.04 (3)
C-G	0 / -410	0.09 (1)
G-D	-723 / 0	0.39 (1)
G-E	0 / 1223	0.28 (1)
B-I	0 / 676	0.15 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9; NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.75/1.00 (D-E:1) , BC=0.42/1.00 (G-I:2) , WB=0.39/1.00 (D-G:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (I) (INPUT = 0.90)
JSI METAL= 0.44 (H) (INPUT = 1.00)

DWG NO. TAM T1900040
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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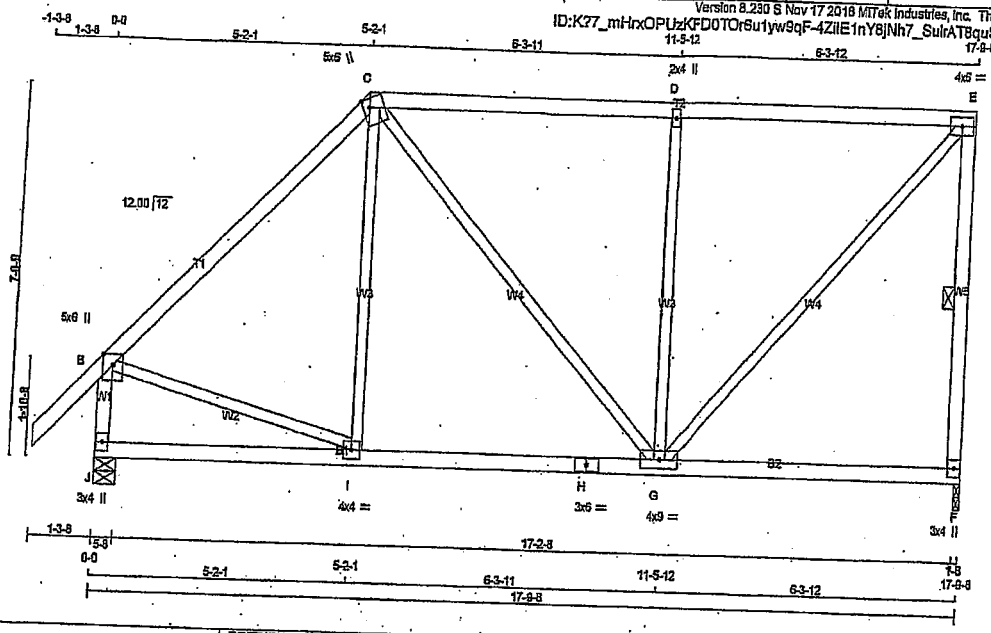

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTVW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	9.0		
H	BB-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 83 lb

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1097	0	1097	0	0	1-8	1-8
J	1215	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
J	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00
B-C	-875 / 0	-84.9	-84.9	0.44 (1)	5.94
C-D	-746 / 0	-84.9	-84.9	0.60 (1)	5.88
D-E	-746 / 0	-84.9	-84.9	0.60 (1)	5.88
F-E	-999 / 0	0.0	0.0	0.22 (1)	6.25
J-B	-1136 / 0	0.0	0.0	0.13 (1)	7.45
J-I	0 / 0	-36.5	-36.5	0.21 (3)	10.00
I-H	0 / 618	-36.5	-36.5	0.36 (2)	10.00
H-G	0 / 618	-36.5	-36.5	0.36 (2)	10.00
G-F	0 / 0	-36.5	-36.5	0.29 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	0 / 204	0.05 (3)
C-G	0 / 189	0.04 (1)
G-D	-662 / 0	0.57 (1)
G-E	0 / 1090	0.25 (1)
B-I	0 / 642	0.14 (1)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.60/1.00 (D-E:1) , BC=0.36/1.00 (G-I:2) , WB=0.57/1.00 (D-G:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (G) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)

DWG NO. TAM T1900042
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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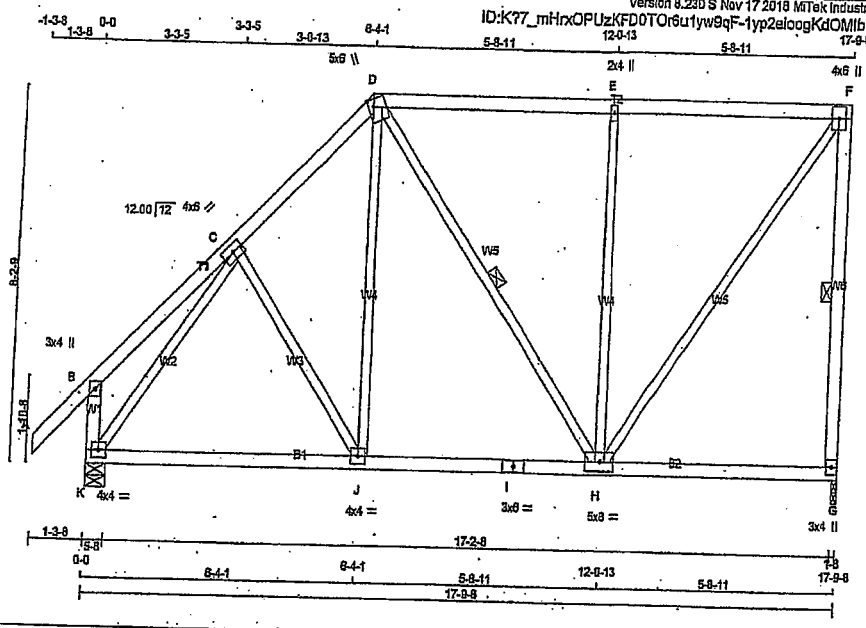


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

TOTAL WEIGHT = 93 lb

[M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	RECORD BRG
G	1097	0	1097	0	0	1-8	1-8
K	1216	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414/0	187/0	0/0	0/0	231/0	0/0
K	914	479/0	187/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

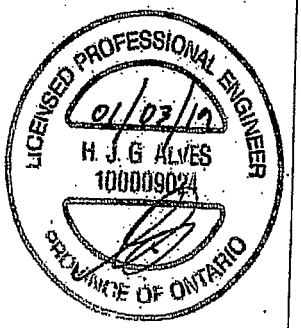
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0/42	-84.9	-84.9	0.12 (1)		10.00
B-C	0/21	-84.9	-84.9	0.13 (1)		10.00
C-D	-849/0	-84.9	-84.9	0.14 (1)		6.25
D-E	-596/0	-84.9	-84.9	0.48 (1)		6.25
E-F	-596/0	-84.9	-84.9	0.48 (1)		6.25
G-F	-1006/0	0.0	0.0	0.31 (1)		6.24
K-B	-221/0	0.0	0.0	0.02 (1)		7.81
K-J	0/600	-38.5	-38.5	0.34 (2)		10.00
J-I	0/597	-38.5	-38.5	0.34 (2)		10.00
I-H	0/597	-38.5	-38.5	0.34 (2)		10.00
H-G	0/0	-38.5	-38.5	0.21 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-29/57	0.02 (1)
J-D	0/322	0.07 (2)
D-H	-24/16	0.02 (3)
H-E	-600/0	0.79 (1)
H-F	0/1013	0.23 (1)
K-C	-1076/0	0.56 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.48/1.00 (E-F;1) , BC=0.34/1.00 (H-J;2) , WB=0.79/1.00 (E-H;1) , SSI=0.24/1.00 (E-F;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.27 (F) (INPUT = 1.00)

DWG NO. TAM T1900044
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T6	1	1	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

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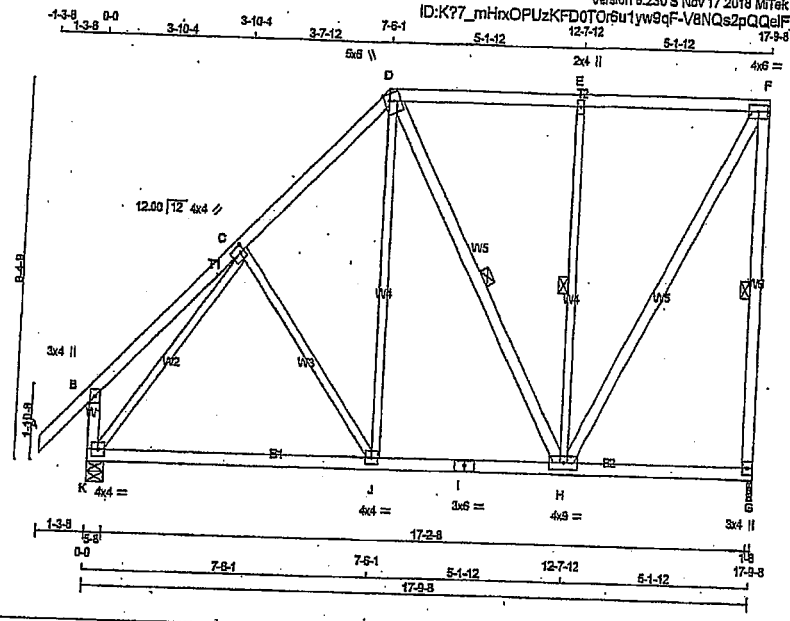

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 105 lb

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1097	0	1097	0	0	1-8	1-8
K	1216	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
K	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	
B-C	0 / 26	-84.9	-84.9	0.19 (1)	10.00	
C-D	-798 / 0	-84.9	-84.9	0.20 (1)	6.25	
D-E	-481 / 0	-84.9	-84.9	0.38 (1)	6.25	
E-F	-481 / 0	-84.9	-84.9	0.38 (1)	6.25	
G-F	-1011 / 0	0.0	0.0	0.42 (1)	6.23	
K-B	-239 / 0	0.0	0.0	0.03 (1)	7.81	
K-J	0 / 607	-38.5	-38.5	0.42 (2)	10.00	
J-I	0 / 549	-38.5	-38.5	0.42 (2)	10.00	
I-H	0 / 549	-38.5	-38.5	0.42 (2)	10.00	
H-G	0 / 0	-38.5	-38.5	0.17 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-105 / 32	0.08 (1)
J-D	0 / 413	0.09 (2)
D-H	-145 / 0	0.08 (2)
H-E	-539 / 0	0.31 (1)
H-F	0 / 971	0.16 (1)
K-C	-1049 / 0	0.76 (1)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/240 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 957 (0.22")

CSI: TC=0.42/1.00 (F-G:1) , BC=0.42/1.00 (J-K:2) , WB=0.76/1.00 (C-K:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.83 (C) (INPUT = 0.90)
JSI METAL= 0.99 (C)-(INPUT = 1.00)

DWG NO. TAM T1900045
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T7	1	1	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

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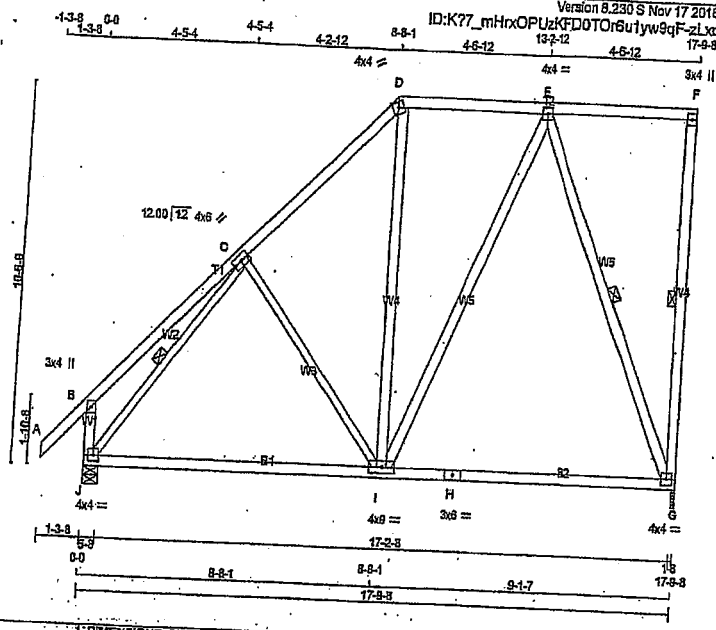


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - I	2x3	DRY	No.2	SPF
J - C	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BS-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	3.0	6.0		
J	BMWW1-t	MT20	4.0	9.0		
			4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 108 lb

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1097	0	1097	0	0	1-8	1-8
J	1216	0	1216	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
J	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G, C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

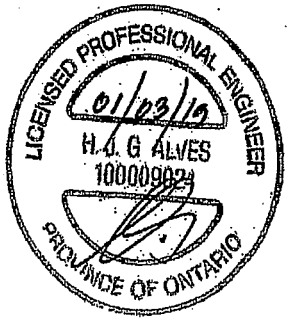
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)		10.00
B-C	0 / 31	-84.9	-84.9	0.26 (1)		10.00
C-D	-759 / 0	-84.9	-84.9	0.28 (1)		6.25
D-E	-512 / 0	-84.9	-84.9	0.30 (1)		6.25
E-F	0 / 0	-84.9	-84.9	0.29 (1)		10.00
G-F	-146 / 0	0.0	0.0	0.08 (1)		6.25
J-B	-256 / 0	0.0	0.0	0.03 (1)		7.81
J-I	0 / 614	-38.5	-38.5	0.74 (2)		10.00
I-H	0 / 365	-38.5	-38.5	0.72 (3)		10.00
H-G	0 / 365	-38.5	-38.5	0.72 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-I	-172 / 14	0.18 (1)
I-D	0 / 260	0.04 (2)
I-E	0 / 397	0.06 (2)
E-G	-898 / 0	0.62 (1)
J-C	-1034 / 0	0.36 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010; NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 693 (0.31")

CSI: TC=0.30/1.00 (D-E:1) , BC=0.74/1.00 (I-J:2) , WB=0.62/1.00 (E-G:1) , SSI=0.22/1.00 (G-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)

DWG NO. TAM T1900046
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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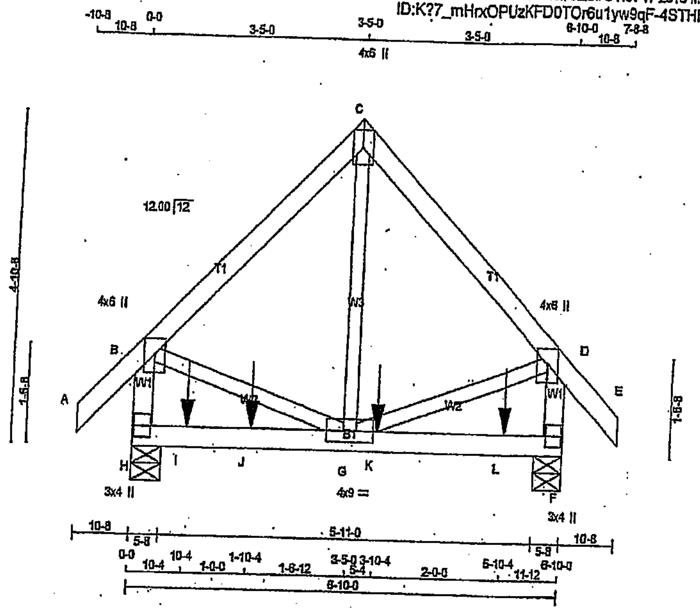

TOTAL WEIGHT = 33 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+p	MT20	4.0	6.0		
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1186	0	1186	0	0	5-8	5-8
F	1070	0	1070	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	882	493 / 0	157 / 0	0 / 0	0 / 0	232 / 0	0 / 0
F	796	444 / 0	142 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 29	-84.9	-84.9	0.07 (1)	10.00
B-C	-653 / 0	-84.9	-84.9	0.20 (1)	6.25
C-D	-653 / 0	-84.9	-84.9	0.20 (1)	6.25
D-E	0 / 29	-84.9	-84.9	0.07 (1)	10.00
H-B	-839 / 0	0.0	0.0	0.10 (1)	7.81
F-D	-839 / 0	0.0	0.0	0.10 (1)	7.81
H-I	0 / 0	-38.5	-38.5	0.49 (1)	10.00
I-J	0 / 0	-38.5	-38.5	0.49 (1)	10.00
J-G	0 / 0	-38.5	-38.5	0.49 (1)	10.00
G-K	0 / 0	-38.5	-38.5	0.49 (1)	10.00
K-L	0 / 0	-38.5	-38.5	0.49 (1)	10.00
L-F	0 / 0	-38.5	-38.5	0.49 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	0 / 633	0.16 (1)
B-G	0 / 485	0.12 (1)
G-D	0 / 485	0.12 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	10-4	-314	-314	—	FRONT	VERT.	TOTAL	—	—
J	1-10-4	-311	-311	—	FRONT	VERT	TOTAL	—	—
K	3-10-4	-311	-311	—	FRONT	VERT	TOTAL	—	—
L	5-10-4	-311	-311	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = **24.0** **IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.20/1.00 (C-D:1), BC=0.49/1.00 (G-H:1), WB=0.16/1.00 (C-G:1), SSI=0.45/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G ALVES
100008874
PROVINCE OF ONTARIO

DWG NO. TAM T19 00062
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	G12	1	1	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

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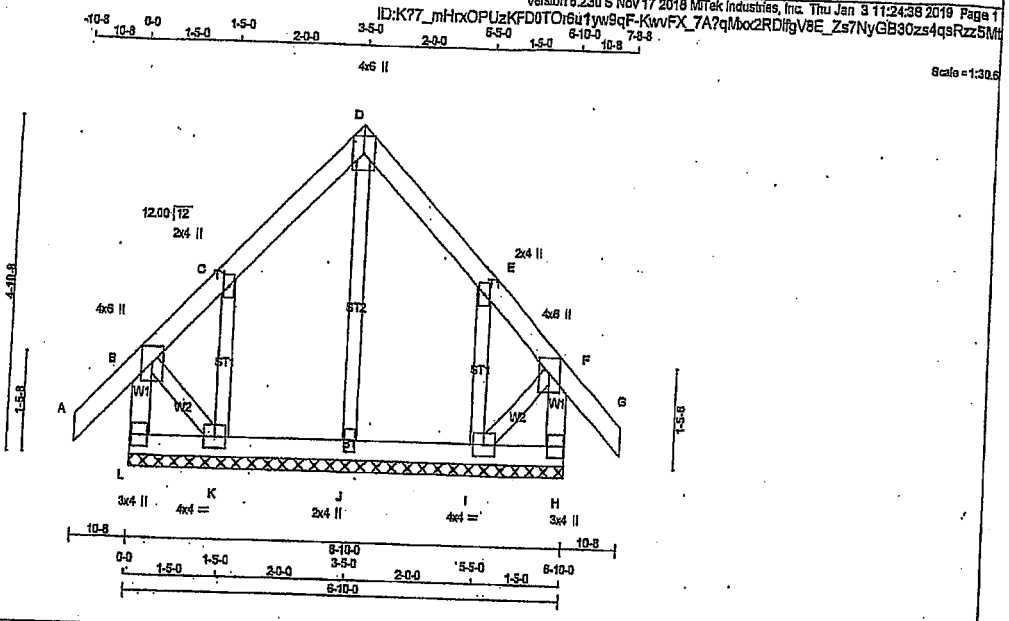

TOTAL WEIGHT = 34 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 29	-84.9	-84.9	0.06 (1)	10.00
B-C	-40 / 0	-84.9	-84.9	0.06 (1)	6.25
C-D	-33 / 0	-84.9	-84.9	0.05 (1)	6.25
D-E	-33 / 0	-84.9	-84.9	0.05 (1)	6.25
E-F	-40 / 0	-84.9	-84.9	0.06 (1)	6.25
F-G	0 / 29	-84.9	-84.9	0.06 (1)	10.00
L-B	-190 / 0	0.0	0.0	0.02 (1)	7.81
H-F	-190 / 0	0.0	0.0	0.02 (1)	7.81
L-K	0 / 0	-38.5	-38.5	0.02 (3)	10.00
K-J	0 / 17	-38.5	-38.5	0.03 (3)	10.00
J-I	0 / 17	-38.5	-38.5	0.03 (3)	10.00
I-H	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-D	-118 / 0	0.04 (1)
K-C	-142 / 0	0.02 (1)
I-E	-142 / 0	0.02 (1)
B-K	0 / 29	0.01 (1)
I-F	0 / 29	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B:1) , BC=0.03/1.00 (L-J:3) , WB=0.04/1.00 (D-J:1) , SSI=0.08/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (F) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T19000035
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	T13	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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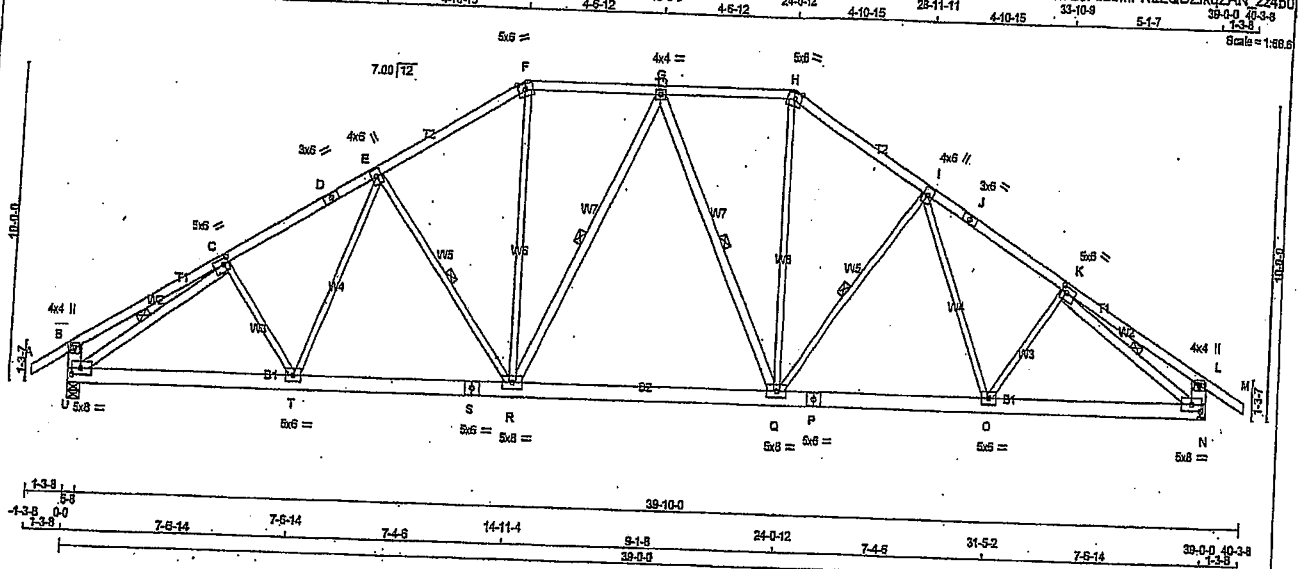

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
G - Q	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW-m	MT20	5.0	6.0	Edge	
G	TMWW-t	MT20	4.0	4.0		
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMWW+t	MT20	4.0	6.0		
J	TS-t	MT20	3.0	6.0		
K	TMWW-t	MT20	5.0	6.0	2.25	2.00
L	TMV+p	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	8.0	2.50	3.75
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	5.0	8.0	2.50	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 6 X 209 = 1252 lb

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2521	0	2521	0	0	5-8	5-8
N	2521	0	2521	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1905	972 / 0	410 / 0	0 / 0	0 / 0	523 / 0	0 / 0
N	1905	972 / 0	410 / 0	0 / 0	0 / 0	523 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, G-R, G-Q, I-Q, C-U, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 29	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 19	-84.9	-84.9	0.26 (1)	10.00
C-D	-3216 / 0	-84.9	-84.9	0.39 (1)	3.64
D-E	-3216 / 0	-84.9	-84.9	0.39 (1)	3.64
E-F	-2711 / 0	-84.9	-84.9	0.35 (1)	3.94
F-G	-2333 / 0	-84.9	-84.9	0.28 (1)	4.25
G-H	-2333 / 0	-84.9	-84.9	0.28 (1)	4.25
H-I	-2711 / 0	-84.9	-84.9	0.35 (1)	3.94
I-J,	-3216 / 0	-84.9	-84.9	0.39 (1)	3.64
J-K	-3216 / 0	-84.9	-84.9	0.39 (1)	3.64
K-L	0 / 19	-84.9	-84.9	0.26 (1)	10.00
L-M	0 / 29	-84.9	-84.9	0.11 (1)	10.00
U-B	-290 / 0	0.0	0.0	0.02 (1)	7.81
N-L	-290 / 0	0.0	0.0	0.02 (1)	7.81
U-T	0 / 2763	-38.5	-38.5	0.47 (1)	10.00
T-S	0 / 2681	-38.5	-38.5	0.47 (2)	10.00
S-R	0 / 2681	-38.5	-38.5	0.47 (2)	10.00
R-Q	0 / 2440	-38.5	-38.5	0.44 (2)	10.00
Q-P	0 / 2681	-38.5	-38.5	0.47 (2)	10.00
P-O	0 / 2681	-38.5	-38.5	0.47 (2)	10.00
O-N	0 / 2763	-38.5	-38.5	0.47 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-T	0 / 147	0.03 (3)
T-E	0 / 330	0.07 (2)
E-R	-598 / 0	0.28 (1)
R-F	0 / 1036	0.23 (1)
R-G	-255 / 0	0.15 (1)
G-Q	-256 / 0	0.15 (1)
Q-H	0 / 1036	0.23 (1)
Q-I	-598 / 0	0.28 (1)
I-O	0 / 330	0.07 (2)
O-K	0 / 147	0.03 (3)
U-C	-3472 / 0	0.64 (1)
K-N	-3472 / 0	[illegible]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = **24.0** **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.39/1.00 (I-K:1), BC=0.47/1.00 (O-Q:2), WB=0.64/1.00 (K-N:1), SSI=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.88 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100009074
PROVINCE OF ONTARIO

DWG NO. TAM T19 00066
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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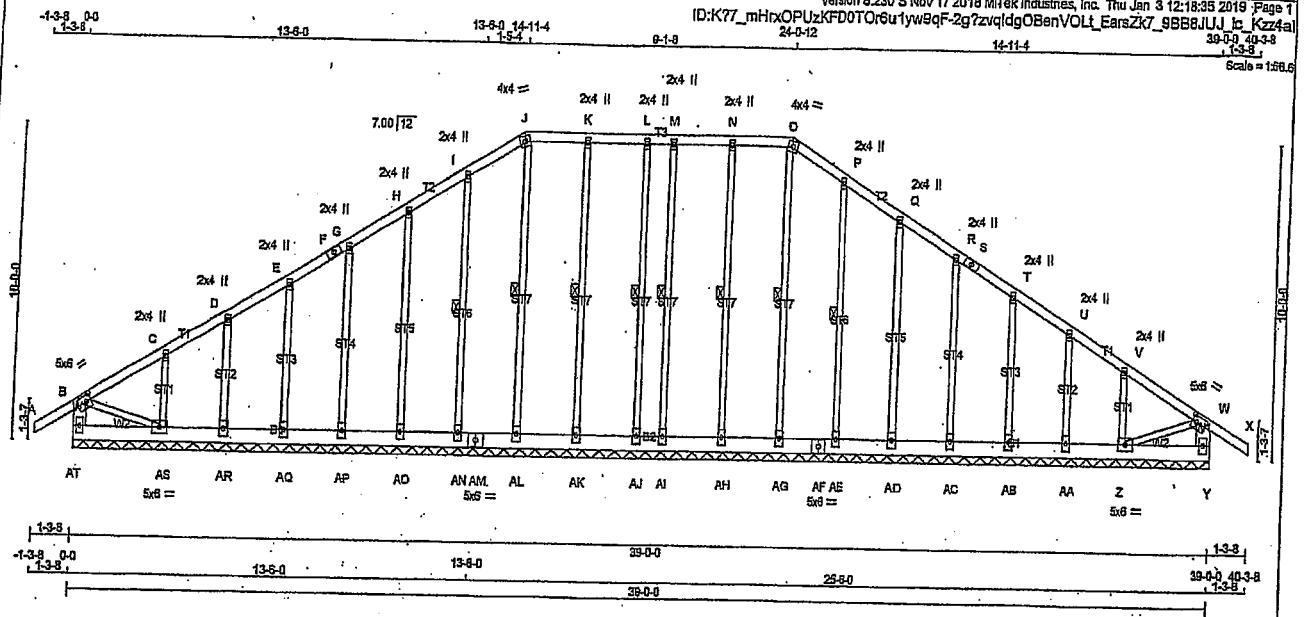

TOTAL WEIGHT = 2 X 227 = 454 lb

LUMBER
N.L.G.A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
O - S	2x4	DRY	No.2	SPF
S - X	2x4	DRY	No.2	SPF
AT- B	2x6	DRY	No.2	SPF
Y - W	2x6	DRY	No.2	SPF
AT- AM	2x6	DRY	No.2	SPF
AM- AF	2x6	DRY	No.2	SPF
AF- Y	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.00
C, D, E, G, H, I, K, L, M, N, P, Q, R, T, U, V						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
O	TTW-m	MT20	4.0	4.0		
S	TS-t	MT20	3.0	6.0		
W	TMVW-t	MT20	5.0	6.0	2.50	2.00
Y	BMV1+p	MT20	3.0	6.0		
Z	BMWW1-t	MT20	5.0	6.0		
AA, AB, AC, AD, AE, AG, AH, AI, AJ, AK, AL, AN, AO, AP, AQ, AR						
AA	BMW1+w	MT20	3.0	6.0		
AF	BS-t	MT20	5.0	6.0		
AM	BS-t	MT20	5.0	6.0		
AS	BMWW1-t	MT20	5.0	6.0		
AT	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-AJ, K-AK, J-AL, I-AN, N-AH, M-AI, O-AG, P-AE.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A-B	0 / 29	-84.9	-84.9	0.11 (1)	10.00	
B-C	-26 / 0	-84.9	-84.9	0.09 (1)	6.25	
C-D	-41 / 0	-84.9	-84.9	0.09 (1)	6.25	
D-E	-23 / 0	-84.9	-84.9	0.04 (1)	6.25	
E-F	-24 / 0	-84.9	-84.9	0.04 (1)	6.25	
F-G	-24 / 0	-84.9	-84.9	0.04 (1)	6.25	
G-H	-21 / 0	-84.9	-84.9	0.04 (1)	6.25	
H-I	-18 / 0	-84.9	-84.9	0.04 (1)	6.25	
I-J	-25 / 0	-84.9	-84.9	0.04 (1)	6.25	
J-K	-13 / 0	-84.9	-84.9	0.05 (1)	6.25	
K-L	-13 / 0	-84.9	-84.9	0.05 (1)	6.25	
L-M	-13 / 0	-84.9	-84.9	0.03 (1)	6.25	
M-N	-13 / 0	-84.9	-84.9	0.05 (1)	6.25	
N-O	-13 / 0	-84.9	-84.9	0.05 (1)	6.25	
O-P	-25 / 0	-84.9	-84.9	0.04 (1)	6.25	
P-Q	-18 / 0	-84.9	-84.9	0.04 (1)	6.25	
Q-R	-21 / 0	-84.9	-84.9	0.04 (1)	6.25	
R-S	-24 / 0	-84.9	-84.9	0.04 (1)	6.25	
S-T	-24 / 0	-84.9	-84.9	0.04 (1)	6.25	
T-U	-23 / 0	-84.9	-84.9	0.04 (1)	6.25	
U-V	-41 / 0	-84.9	-84.9	0.09 (1)	6.25	
V-W	-26 / 0	-84.9	-84.9	0.09 (1)	6.25	
W-X	0 / 29	-84.9	-84.9	0.11 (1)	10.00	
AT-B	-247 / 0	0.0	0.0	0.02 (1)	7.81	
Y-W	-247 / 0	0.0	0.0	0.02 (1)	7.81	
AT-AS	0 / 0	-38.5	-38.5	0.03 (3)	10.00	
AS-AR	0 / 25	-38.5	-38.5	0.03 (2)	10.00	
AR-AQ	0 / 22	-38.5	-38.5	0.01 (2)	10.00	
AQ-AP	0 / 20	-38.5	-38.5	0.01 (2)	10.00	
AP-AO	0 / 18	-38.5	-38.5	0.01 (2)	10.00	
AO-AN	0 / 16	-38.5	-38.5	0.01 (2)	10.00	
AN-AM	0 / 15	-38.5	-38.5	0.01 (2)	10.00	
AM-AL	0 / 15	-38.5	-38.5	0.01 (2)	10.00	
AL-AK	0 / 13	-38.5	-38.5	0.01 (2)	10.00	
AK-AJ	0 / 13	-38.5	-38.5	0.01 (2)	10.00	
AJ-AI	0 / 13	-38.5	-38.5	0.01 (2)	10.00	
AI-AH	0 / 13	-38.5	-38.5	0.01 (2)	10.00	
AH-AG	0 / 13	-38.5	-38.5	0.01 (2)	10.00	
AG-AF	0 / 15	-38.5	-38.5	0.01 (2)	10.00	
AF-AE	0 / 15	-38.5	-38.5	0.01 (2)	10.00	
AE-AD	0 / 16	-38.5	-38.5	0.01 (2)	10.00	
AD-AC	0 / 18	-38.5	-38.5	0.01 (2)	10.00	

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
AJ-L	-117 / 0	0.08 (1)
AK-K	-198 / 0	0.13 (1)
AL-J	-131 / 0	0.09 (1)
AN-I	-186 / 0	0.09 (1)
AO-H	-166 / 0	0.17 (1)
AP-G	-168 / 0	0.11 (1)
AQ-E	-174 / 0	0.07 (1)
AR-D	-139 / 0	0.04 (1)
AS-C	-254 / 0	0.04 (1)
AH-N	-198 / 0	0.13 (1)
AI-M	-117 / 0	0.08 (1)
AG-O	-131 / 0	0.09 (1)
AE-P	-186 / 0	0.09 (1)
AD-Q	-166 / 0	0.17 (1)
AC-R	-168 / 0	0.11 (1)
AB-T	-174 / 0	0.07 (1)
AA-U	-139 / 0	0.04 (1)
Z-V	-254 / 0	0.04 (1)
B-AS	0 / 34	0.01 (1)
Z-W	0 / 34	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (Z-AA:2) , WB=0.17/1.00 (Q-AD:1) , SSI=0.10/1.00 (V-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. B. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM T1900060
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
AC-AB	0 / 20	-38.5	-38.5	0.01 (2)	10.00
AB-AA	0 / 22	-38.5	-38.5	0.01 (2)	10.00
AA-Z	0 / 25	-38.5	-38.5	0.03 (2)	10.00
Z-Y	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100009624
PROVINCE OF ONTARIO

DRWG NO. TAM T1900060
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300783	T26A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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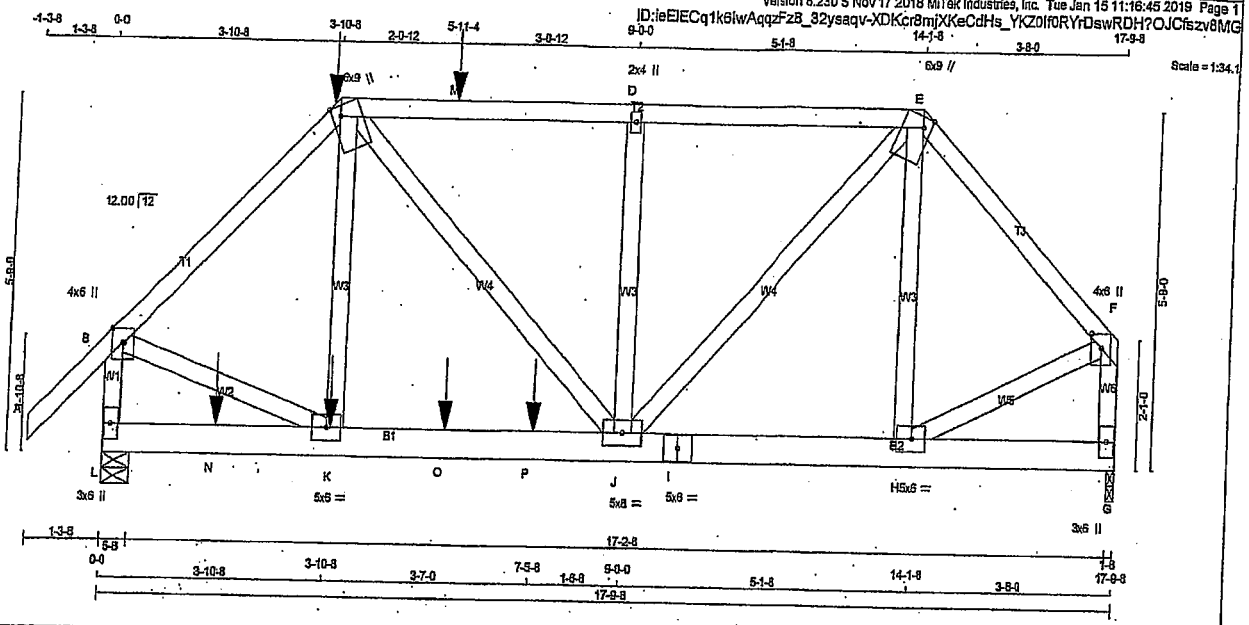

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-E	1	12	SIDE(61.0)
E-F	1	12	TOP
L-B	1	12	TOP
G-F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L-I	2	12	SIDE(183.1)
I-G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	9.0	Edge	1.75
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 103 = 206 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	2130	0	2130	0	0	5-8	5-8
G	1615	0	1615	0	0	1-8	1-8

UNFACTORED REACTIONS

1ST LCASE : MAX/MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1591	864 / 0	300 / 0	0 / 0	0 / 0	427 / 0	0 / 0
G	1214	638 / 0	248 / 0	0 / 0	0 / 0	331 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					FR-TO
A-B	0 / 42	-84.9	-84.9	0.07 (1)	10.00
B-C	-1911 / 0	-84.9	-84.9	0.14 (1)	6.17
C-M	-1887 / 0	-84.9	-84.9	0.25 (1)	6.04
M-D	-1887 / 0	-84.9	-84.9	0.25 (1)	6.04
D-E	-1887 / 0	-84.9	-84.9	0.25 (1)	6.04
E-F	-1359 / 0	-84.9	-84.9	0.12 (1)	6.25
L-B	-2121 / 0	0.0	0.0	0.12 (1)	7.61
G-F	-1535 / 0	0.0	0.0	0.09 (1)	7.81
L-N	0 / 0	-38.5	-38.5	0.13 (1)	10.00
N-K	0 / 0	-38.5	-38.5	0.13 (1)	10.00
K-O	0 / 1350	-38.5	-38.5	0.37 (1)	10.00
O-P	0 / 1350	-38.5	-38.5	0.37 (1)	10.00
P-J	0 / 1350	-38.5	-38.5	0.37 (1)	10.00
J-I	0 / 948	-38.5	-38.5	0.22 (1)	10.00
I-H	0 / 948	-38.5	-38.5	0.22 (1)	10.00
H-G	0 / 0	-38.5	-38.5	0.03 (2)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	0 / 267	0.03 (3)
C-J	0 / 777	0.07 (1)
J-D	-560 / 0	0.10 (1)
J-E	0 / 1358	0.12 (1)
H-E	-341 / 0	0.06 (1)
B-K	0 / 1482	0.13 (1)
H-E	-1045	0.09 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	REEL	CONC
C	3-10-8	-271	-271	—	FRONT	VERT	TOTAL	—	—
K	3-11-4	-36	-46	—	FRONT	VERT	TOTAL	—	—
M	5-11-4	-80	-80	—	FRONT	VERT	TOTAL	—	—
N	1-11-4	-36	-46	—	FRONT	VERT	TOTAL	—	—
O	5-11-4	-36	-46	—	FRONT	VERT	TOTAL	—	—
P	7-5-8	-972	-972	—	TOP	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.25/1.00 (C-D:1) , BC=0.37/1.00 (J-K:1) , WB=0.13/1.00 (B-K:1) , SSI=0.26/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90).
JSI METAL= 0.39 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T190193
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300783	T27A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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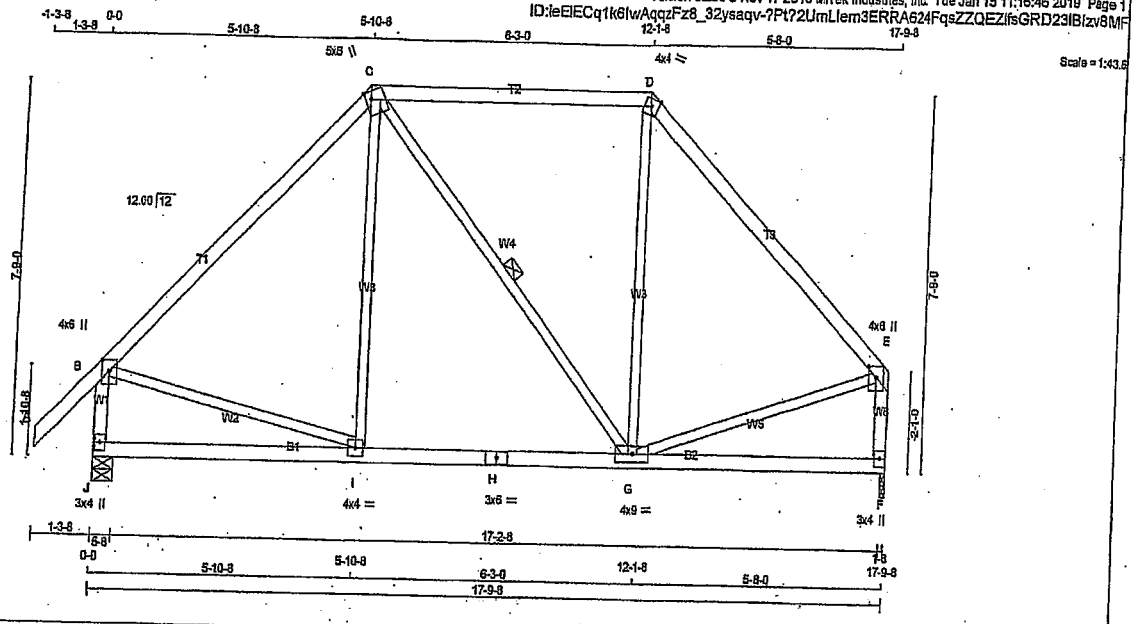

TOTAL WEIGHT =. 81 lb

MiTek

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1216	0	1216	0	0	5-8	5-8
F	1097	0	1097	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX/MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	914	479 / 0	167 / 0	0 / 0	0 / 0	248 / 0	0 / 0
F	892	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00
B-C	-851 / 0	-84.9	-84.9	0.40 (1)	6.09
C-D	-590 / 0	-84.9	-84.9	0.43 (1)	6.25
D-E	-834 / 0	-84.9	-84.9	0.37 (1)	6.18
J-B	-1123 / 0	0.0	0.0	0.13 (1)	7.46
F-E	-1009 / 0	0.0	0.0	0.12 (1)	7.79
J-I	0 / 0	-38.5	-38.5	0.25 (3)	10.00
I-H	0 / 601	-38.5	-38.5	0.32 (2)	10.00
H-G	0 / 601	-38.5	-38.5	0.32 (2)	10.00
G-F	0 / 0	-38.5	-38.5	0.24 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	0 / 243	0.06 (3)
G-G	-18 / 0	0.01 (1)
G-D	0 / 230	0.05 (3)
B-I	0 / 619	0.14 (1)
G-E	0 / 614	0.14 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.43/1.00 (C-D:1) , BC=0.32/1.00 (G-I:2) , WB=0.14/1.00 (B-I:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)

DWG NO. TAM T7401094
STRUCTURAL
COMPONENT SIM V

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300799	T29A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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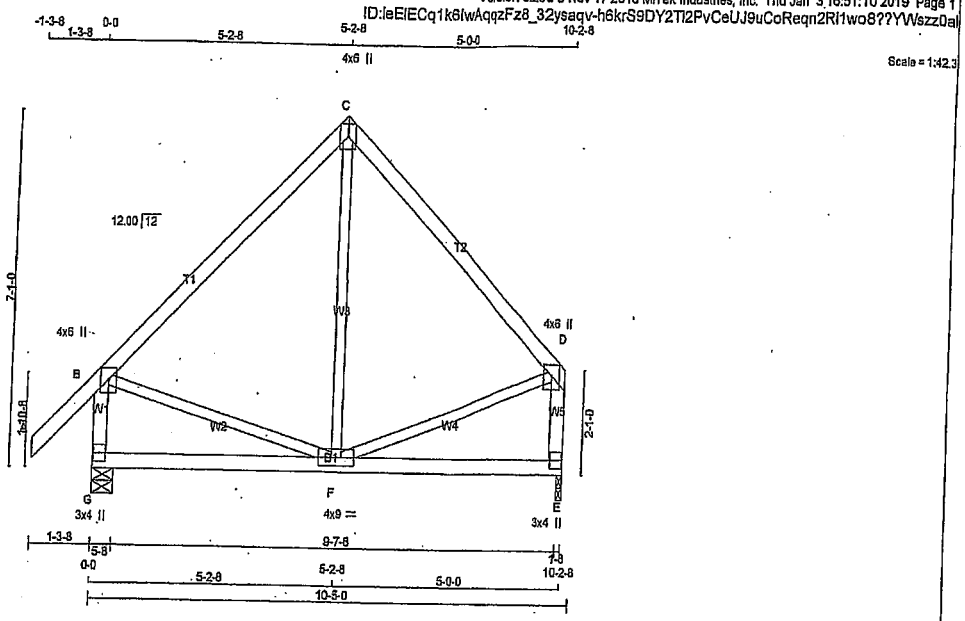


TOTAL WEIGHT = 46 lb

MiTek

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+p	MT20	4.0	6.0		
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	748	0	748	0	0	IN-SX	IN-SX
E	630	0	630	0	0	5-8	5-8
						1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	559	303 / 0	107 / 0	0 / 0	0 / 0	149 / 0	0 / 0
E	478	238 / 0	107 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

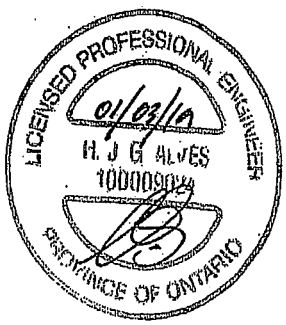
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)		10.00
B-C	-363 / 0	-84.9	-84.9	0.30 (1)		6.25
C-D	-363 / 0	-84.9	-84.9	0.27 (1)		6.25
G-B	-671 / 0	0.0	0.0	0.07 (1)		7.81
E-D	-558 / 0	0.0	0.0	0.07 (1)		7.81
G-F	0 / 0	-38.5	-38.5	0.23 (3)		10.00
F-E	0 / 0	-38.5	-38.5	0.23 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	0 / 217	0.05 (3)
B-F	0 / 267	0.06 (1)
F-D	0 / 271	0.06 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.30/1.00 (B-C:1) , BC=0.23/1.00 (E-F:3) , WB=0.06/1.00 (D-F:1) , SSI=0.13/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.50)
JSI METAL= 0.22 (B) (INPUT = 1.00)

DWG NO. TAM T1900119
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	T30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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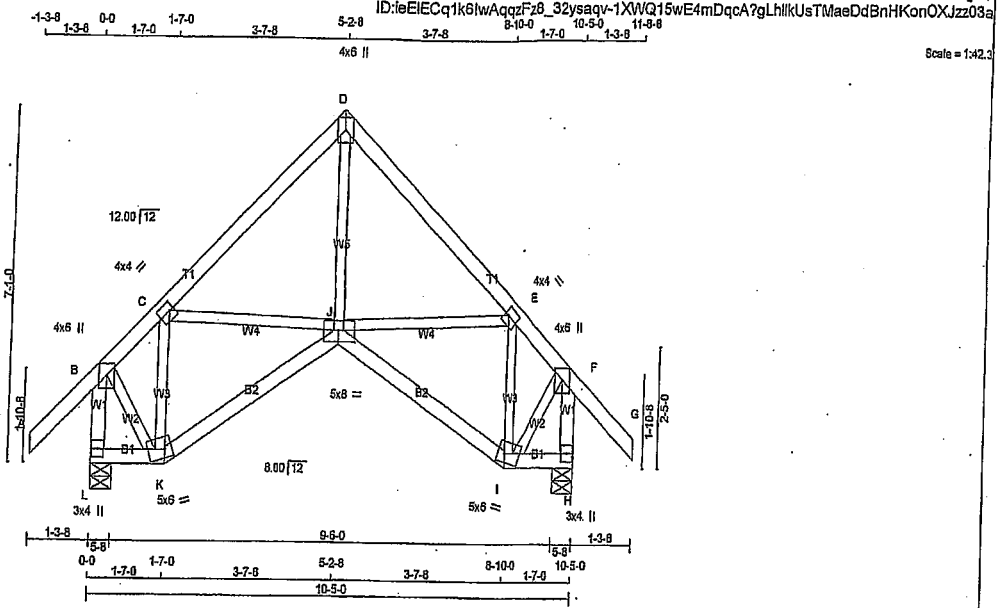

TOTAL WEIGHT = 55 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-m	MT20	5.0	6.0	2.00	1.75
J	BBWWW-p	MT20	5.0	8.0	2.75	4.00
K	BBWW-m	MT20	5.0	6.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	761	0	761	0	0	5-8	5-8
H	761	0	761	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	569	308 / 0	109 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	569	308 / 0	109 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	J-D	0 / 473	0.11 (2)
B-C	-401 / 0	-84.9	-84.9	0.11 (1)	6.25	J-E	0 / 107	0.02 (1)
C-D	-543 / 0	-84.9	-84.9	0.15 (1)	6.25	I-E	-354 / 0	0.07 (1)
D-E	-543 / 0	-84.9	-84.9	0.15 (1)	6.25	C-J	0 / 107	0.02 (1)
E-F	-401 / 0	-84.9	-84.9	0.11 (1)	6.25	K-C	-354 / 0	0.07 (1)
F-G	0 / 42	-84.9	-84.9	0.12 (1)	10.00	B-K	0 / 401	0.09 (1)
L-B	-730 / 0	0.0	0.0	0.08 (1)	7.81	I-F	0 / 401	0.09 (1)
H-F	-730 / 0	0.0	0.0	0.08 (1)	7.81			
L-K	0 / 0	-38.5	-38.5	0.02 (3)	10.00			
K-J	0 / 319	-38.5	-38.5	0.16 (2)	10.00			
J-I	0 / 319	-38.5	-38.5	0.15 (2)	10.00			
I-H	0 / 0	-38.5	-38.5	0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15/1.00 (C-D:1) , BC=0.16/1.00 (I-J:2) , WB=0.11/1.00 (D-J:2) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.67 (K) (INPUT = 0.90)
SSI METAL= 0.20 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100098824
PROVINCE OF ONTARIO

ENGINEERING T1900150
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300799	T30A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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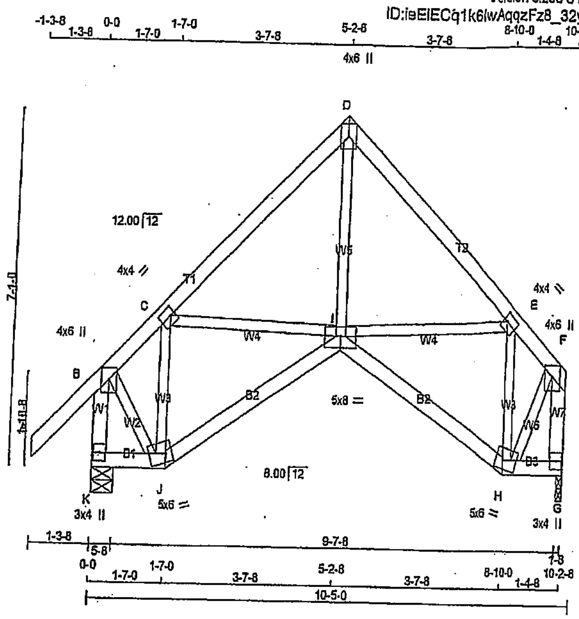

TOTAL WEIGHT = 53 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BBWW-m	MT20	5.0	6.0	2.00	2.00
I	BBWWW-p	MT20	5.0	8.0	2.75	4.00
J	BBWW-m	MT20	5.0	6.0	2.00	1.75
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	756	0	756	0	0	5-8	5-8
G	622	0	622	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	565	307 / 0	107 / 0	0 / 0	0 / 0	151 / 0	0 / 0
G	472	233 / 0	107 / 0	0 / 0	0 / 0	132 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-84.9	-84.9	0.12 (1)	10.00
B- C	-398 / 0	-84.9	-84.9	0.11 (1)	6.25
C- D	-535 / 0	-84.9	-84.9	0.15 (1)	6.25
D- E	-537 / 0	-84.9	-84.9	0.14 (1)	6.25
E- F	-336 / 0	-84.9	-84.9	0.12 (1)	6.25
K- B	-726 / 0	0.0	0.0	0.08 (1)	7.81
G- F	-595 / 0	0.0	0.0	0.07 (1)	7.81
K- J	0 / 0	-38.5	-38.5	0.02 (3)	10.00
J- I	0 / 316	-38.5	-38.5	0.16 (2)	10.00
I- H	0 / 306	-38.5	-38.5	0.15 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 462	0.10 (2)
I- E	0 / 112	0.03 (1)
H- E	-432 / 0	0.08 (1)
C- I	0 / 104	0.02 (1)
J- C	-351 / 0	0.07 (1)
B- J	0 / 397	0.09 (1)
H- F	0 / 456	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15/1.00 (C-D:1) , BC=0.16/1.00 (I-J:2) , WB=0.10/1.00 (D-I:2) , SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100009025
PROVINCE OF ONTARIO

DWG NO. TAM T1900120
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	T31	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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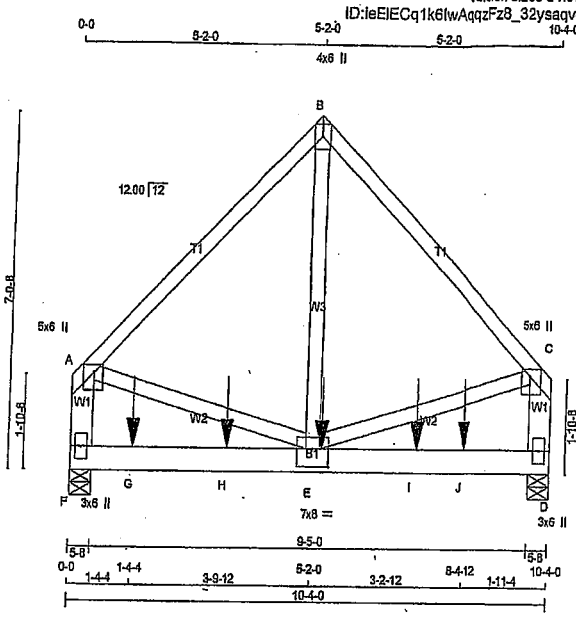


TOTAL WEIGHT = 2 X 59 = 118 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-B	1	12	TOP
B-C	1	12	TOP
F-A	2	12	TOP
D-C	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F-D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TTW+p	MT20	4.0	6.0		
C	TMVW+p	MT20	5.0	6.0	1.75	2.75
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	7.0	8.0	4.25	4.00
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	3604	0	3604	0	0	5-8	5-8
D	3557	0	3557	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2699	1449 / 0	526 / 0	0 / 0	0 / 0	724 / 0	0 / 0
D	2664	1429 / 0	520 / 0	0 / 0	0 / 0	715 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

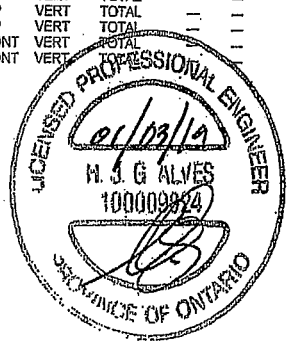
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-2643 / 0	-84.9	-84.9	0.29 (1)	5.31		E-B	0 / 3300	0.29 (1)
B-C	-2643 / 0	-84.9	-84.9	0.29 (1)	5.31		A-E	0 / 1940	0.17 (1)
F-A	-2609 / 0	0.0	0.0	0.10 (1)	7.81		E-C	-0 / 1940	0.17 (1)
D-C	-2609 / 0	0.0	0.0	0.10 (1)	7.81				
F-G	0 / 0	-38.5	-38.5	0.73 (1)	10.00				
G-H	0 / 0	-38.5	-38.5	0.73 (1)	10.00				
H-E	0 / 0	-38.5	-38.5	0.73 (1)	10.00				
E-I	0 / 0	-38.5	-38.5	0.73 (1)	10.00				
I-J	0 / 0	-38.5	-38.5	0.73 (1)	10.00				
J-D	0 / 0	-38.5	-38.5	0.73 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	5-4-4	-1177	-1177	—	TOP	VERT	TOTAL	—	—
G	1-4-4	-1177	-1177	—	TOP	VERT	TOTAL	—	—
H	3-4-4	-1177	-1177	—	TOP	VERT	TOTAL	—	—
I	7-4-4	-1177	-1177	—	FRONT	VERT	TOTAL	—	—
J	8-4-12	-1177	-1177	—	FRONT	VERT	TOTAL	—	—



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.73/1.00 (E-F:1) , WB=0.29/1.00 (B-E:1) , SSI=0.53/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (B) (INPUT = 0.90)
JSI METAL= 0.39 (A) (INPUT = 1.00)

ENG NO. TAM T1900157
STRUCTURAL
CALCULATIONS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300785	G32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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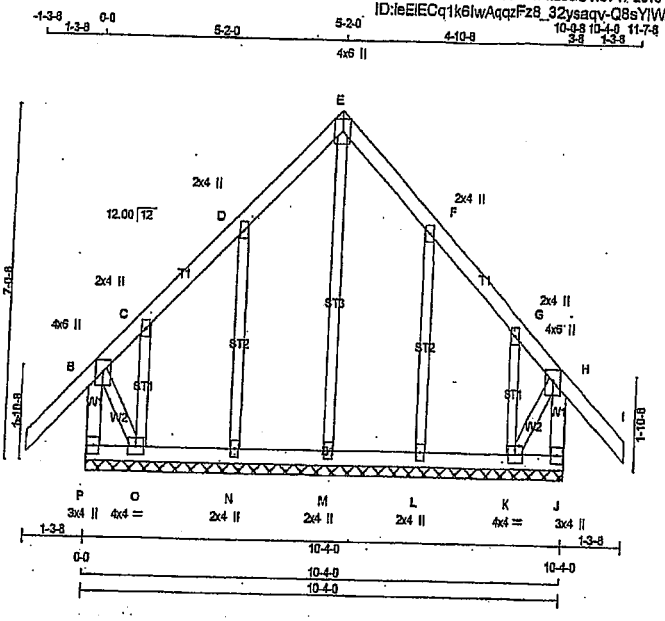


TOTAL WEIGHT = 56 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0		
H	TMVW+p	MT20	4.0	6.0	2.75	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

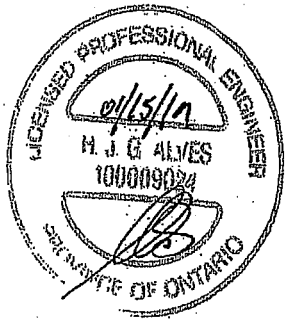
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-B	-258 / 0	0.0	0.0	0.03 (1)	7.81	
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	
B-C	-64 / 0	-84.9	-84.9	0.11 (1)	6.25	
C-D	-1 / 0	-84.9	-84.9	0.06 (1)	10.00	
D-E	-20 / 0	-84.9	-84.9	0.05 (1)	6.25	
E-F	-20 / 0	-84.9	-84.9	0.05 (1)	6.25	
F-G	-1 / 0	-84.9	-84.9	0.06 (1)	10.00	
G-H	-64 / 0	-84.9	-84.9	0.11 (1)	6.25	
H-I	0 / 42	-84.9	-84.9	0.12 (1)	10.00	
J-H	-258 / 0	0.0	0.0	0.03 (1)	7.81	
P-O	0 / 0	-38.5	-38.5	0.02 (3)	10.00	
O-N	0 / 9	-38.5	-38.5	0.03 (3)	10.00	
N-M	0 / 4	-38.5	-38.5	0.03 (3)	10.00	
M-L	0 / 4	-38.5	-38.5	0.03 (3)	10.00	
L-K	0 / 9	-38.5	-38.5	0.03 (3)	10.00	
K-J	0 / 0	-38.5	-38.5	0.02 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	-131 / 0	0.11 (1)
N-D	-205 / 0	0.08 (1)
O-C	-42 / 0	0.01 (1)
L-F	-205 / 0	0.08 (1)
K-G	-42 / 0	0.01 (1)
B-O	0 / 17	0.00 (1)
K-H	0 / 17	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1) , BC=0.03/1.00 (N-O:3) , WB=0.11/1.00 (E-M:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1957	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (H) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

DWG NO. TAM T1901099
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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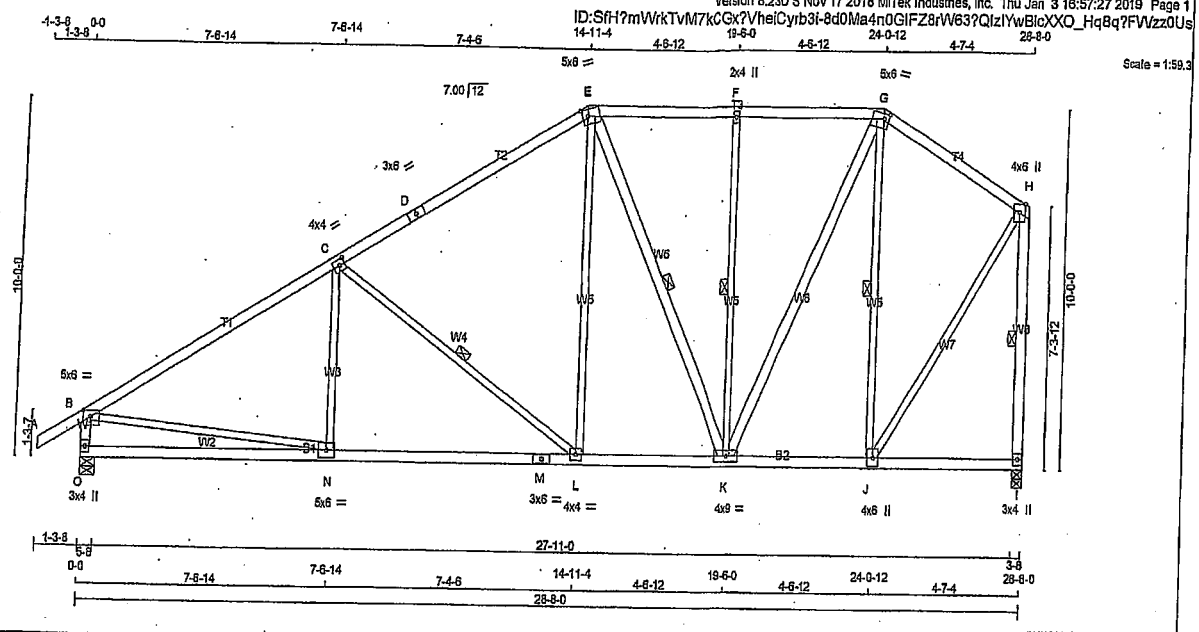

TOTAL WEIGHT = 150 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - K	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	1.75
H	TMVW+p	MT20	4.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW+t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1884	0	1884	0	0	5-8	5-8
I	1768	0	1768	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1421	731 / 0	301 / 0	0 / 0	0 / 0	369 / 0	0 / 0
I	1341	668 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-K, F-K, G-J, H-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 29	-84.9	-84.9	0.11 (1)	10.00
B-C	-2250 / 0	-84.9	-84.9	0.84 (1)	3.60
C-D	-1557 / 0	-84.9	-84.9	0.71 (1)	4.33
D-E	-1557 / 0	-84.9	-84.9	0.71 (1)	4.33
E-F	-1161 / 0	-84.9	-84.9	0.23 (1)	5.65
F-G	-1161 / 0	-84.9	-84.9	0.23 (1)	5.65
G-H	-840 / 0	-84.9	-84.9	0.24 (1)	6.25
O-B	-1768 / 0	0.0	0.0	0.16 (1)	6.27
I-H	-1697 / 0	0.0	0.0	0.42 (1)	5.10
O-N	0 / 0	-38.5	-38.5	0.43 (3)	10.00
N-M	0 / 1977	-38.5	-38.5	0.68 (2)	10.00
M-L	0 / 1977	-38.5	-38.5	0.68 (2)	10.00
L-K	0 / 1318	-38.5	-38.5	0.40 (2)	10.00
K-J	0 / 715	-38.5	-38.5	0.26 (2)	10.00
J-I	0 / 0	-38.5	-38.5	0.16 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	0 / 297	0.07 (3)
C-L	-822 / 0	0.49 (1)
L-E	0 / 723	0.16 (2)
E-K	-370 / 0	0.23 (1)
K-F	-472 / 0	0.32 (1)
K-G	0 / 1046	0.17 (1)
J-G	-868 / 0	0.59 (1)
B-N	0 / 1993	0.45 (1)
J-H	0 / 1298	0.29 (1)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100008[illegible]
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.84/1.00 (B-C:1) , BC=0.68/1.00 (L-N:2) , WB=0.59/1.00 (G-J:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.56 (M) (INPUT = 1.00)

INVOICE NO. TAM 71100128
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	G33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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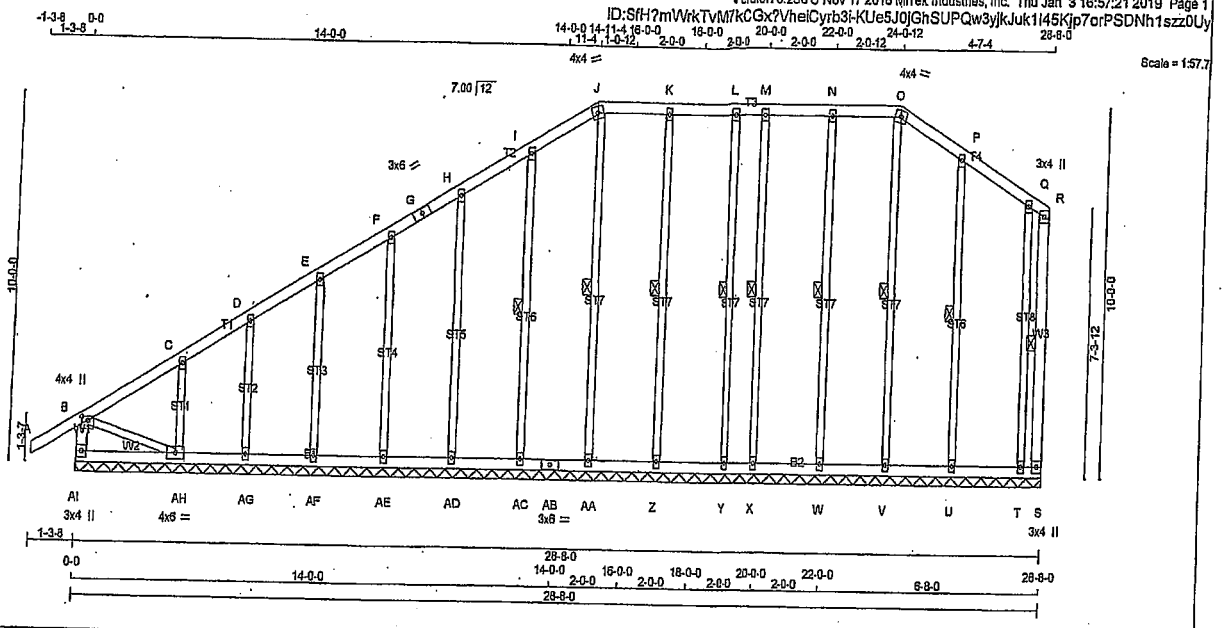


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AI - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
O - R	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
AI - AB	2x4	DRY	No.2	SPF
AB- S	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, F, H, I, K, L, M, N, P, Q						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
O	TTW-m	MT20	4.0	4.0		
R	TMV+p	MT20	3.0	4.0		
S	BMV1+p	MT20	3.0	4.0		
T, U, V, W, X, Y, Z, AA, AC, AD, AE, AF, AG						
T	BMW1+w	MT20	2.0	4.0		
AB	BS-t	MT20	3.0	6.0		
AH	BMWW1-t	MT20	4.0	6.0		
AI	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT S FOR 150 LBS FACTORED UPLIFT
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF R-S, I-AC, J-AA, K-Z, L-Y, M-X, N-W, O-V, P-U.

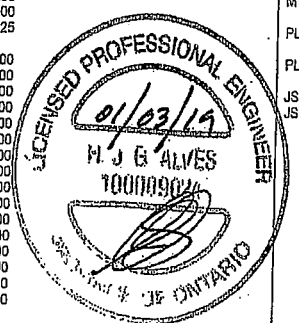
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
AI-B	-232 / 0	0.0	0.0	0.02 (1)	7.81	
A-B	0 / 29	-84.9	-84.9	0.11 (1)	10.00	
B-C	-5 / 0	-84.9	-84.9	0.09 (1)	10.00	
C-D	-21 / 0	-84.9	-84.9	0.09 (1)	6.25	
D-E	-3 / 0	-84.9	-84.9	0.04 (1)	10.00	
E-F	-3 / 0	-84.9	-84.9	0.04 (1)	10.00	
F-G	0 / 0	-84.9	-84.9	0.04 (1)	10.00	
G-H	0 / 0	-84.9	-84.9	0.04 (1)	10.00	
H-I	0 / 3	-84.9	-84.9	0.05 (1)	10.00	
I-J	-5 / 0	-84.9	-84.9	0.04 (1)	10.00	
J-K	0 / 5	-84.9	-84.9	0.05 (1)	10.00	
K-L	0 / 4	-84.9	-84.9	0.05 (1)	10.00	
L-M	0 / 4	-84.9	-84.9	0.03 (1)	10.00	
M-N	0 / 4	-84.9	-84.9	0.05 (1)	10.00	
N-O	0 / 5	-84.9	-84.9	0.05 (1)	10.00	
O-P	-6 / 0	-84.9	-84.9	0.05 (1)	10.00	
P-Q	0 / 8	-84.9	-84.9	0.05 (1)	10.00	
Q-R	0 / 11	-84.9	-84.9	0.02 (1)	10.00	
S-R	-2 / 0	0.0	0.0	0.00 (1)	6.25	
AI-AH	0 / 0	-38.5	-38.5	0.06 (3)	10.00	
AH-AG	0 / 8	-38.5	-38.5	0.06 (3)	10.00	
AG-AF	0 / 5	-38.5	-38.5	0.03 (3)	10.00	
AF-AE	0 / 2	-38.5	-38.5	0.03 (3)	10.00	
AE-AD	0 / 0	-38.5	-38.5	0.02 (3)	10.00	
AD-AC	-1 / 0	-38.5	-38.5	0.02 (3)	10.00	
AC-AB	-3 / 0	-38.5	-38.5	0.02 (3)	10.00	
AB-AA	-3 / 0	-38.5	-38.5	0.02 (3)	10.00	
AA-Z	-4 / 0	-38.5	-38.5	0.03 (3)	10.00	
Z-Y	-4 / 0	-38.5	-38.5	0.03 (3)	10.00	
Y-X	-4 / 0	-38.5	-38.5	0.02 (3)	10.00	
X-W	-4 / 0	-38.5	-38.5	0.03 (3)	10.00	
W-V	-4 / 0	-38.5	-38.5	0.03 (3)	10.00	
V-U	-3 / 0	-38.5	-38.5	0.03 (3)	10.00	
U-T	-1 / 0	-38.5	-38.5	0.03 (3)	10.00	
T-S	0 / 0	-38.5	-38.5	0.02 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AH-C	-254 / 0	0.05 (1)
AG-D	-139 / 0	0.04 (1)
AF-E	-174 / 0	0.06 (1)
AE-F	-168 / 0	0.12 (1)
AD-H	-166 / 0	0.18 (1)
AC-I	-187 / 0	0.10 (1)
AA-J	-141 / 0	0.10 (1)
Z-K	-198 / 0	0.13 (1)
Y-L	-117 / 0	0.08 (1)
X-M	-117 / 0	0.08 (1)
W-N	-198 / 0	0.13 (1)
V-O	-139 / 0	0.09 (1)
U-P	-203 / 0	0.10 (1)
T-Q	-119 / 0	0.13 (1)
B-AH	0 / 15	0.00 (1)



TOTAL WEIGHT = 167 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.06/1.00 (AG-AH:3) , WB=0.18/1.00 (H-AD:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

DWG NO. TAM 19 00128
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T34	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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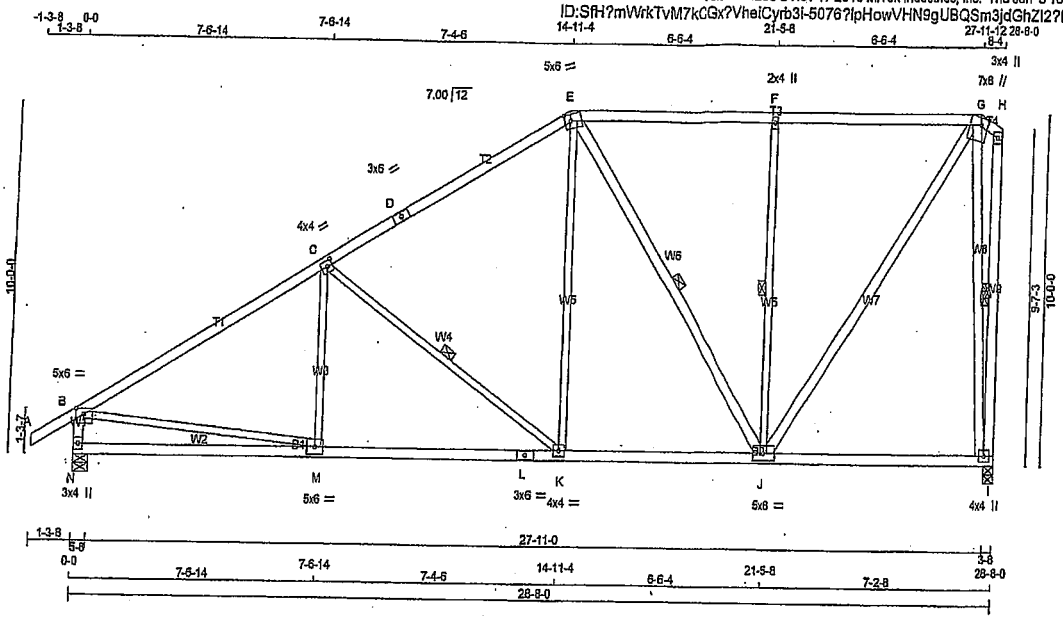


TOTAL WEIGHT = 7 X 151 = 1055 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - J	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	7.0	8.0	Edge	2.50
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	4.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1884	0	1884	0	0	5-8	5-8
I	1768	0	1768	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1421	731 / 0	301 / 0	0 / 0	0 / 0	369 / 0	0 / 0
I	1341	668 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-J, F-J, H-I, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 29	-84.9	-84.9	0.11 (1)	10.00
B-C	-2250 / 0	-84.9	-84.9	0.84 (1)	3.60
C-D	-1558 / 0	-84.9	-84.9	0.71 (1)	4.33
D-E	-1558 / 0	-84.9	-84.9	0.71 (1)	4.33
E-F	-1047 / 0	-84.9	-84.9	0.48 (1)	5.47
F-G	-1047 / 0	-84.9	-84.9	0.48 (1)	5.47
G-H	0 / 0	-84.9	-84.9	0.01 (1)	10.00
N-B	-1767 / 0	0.0	0.0	0.18 (1)	6.27
I-H	-29 / 0	0.0	0.0	0.01 (1)	6.25
N-M	0 / 0	-38.5	-38.5	0.42 (3)	10.00
M-L	0 / 1976	-38.5	-38.5	0.68 (2)	10.00
L-K	0 / 1976	-38.5	-38.5	0.68 (2)	10.00
K-J	0 / 1319	-38.5	-38.5	0.52 (2)	10.00
J-I	0 / 116	-38.5	-38.5	0.36 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	0 / 295	0.07 (3)
C-K	-820 / 0	0.49 (1)
K-E	0 / 736	0.17 (2)
E-J	-486 / 0	0.37 (1)
J-F	-665 / 0	0.47 (1)
B-M	0 / 1993	0.45 (1)
G-I	-1630 / 0	0.80 (1)
J-G	0 / 1670	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.84/1.00 (B-C:1) , BC=0.66/1.00 (K-M:2) , WB=0.80/1.00 (G-I:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

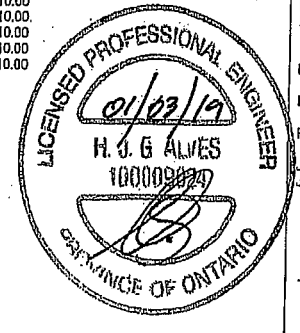
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.56 (L) (INPUT = 1.00)



DWG NO. TAM 11900129
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300799	G34	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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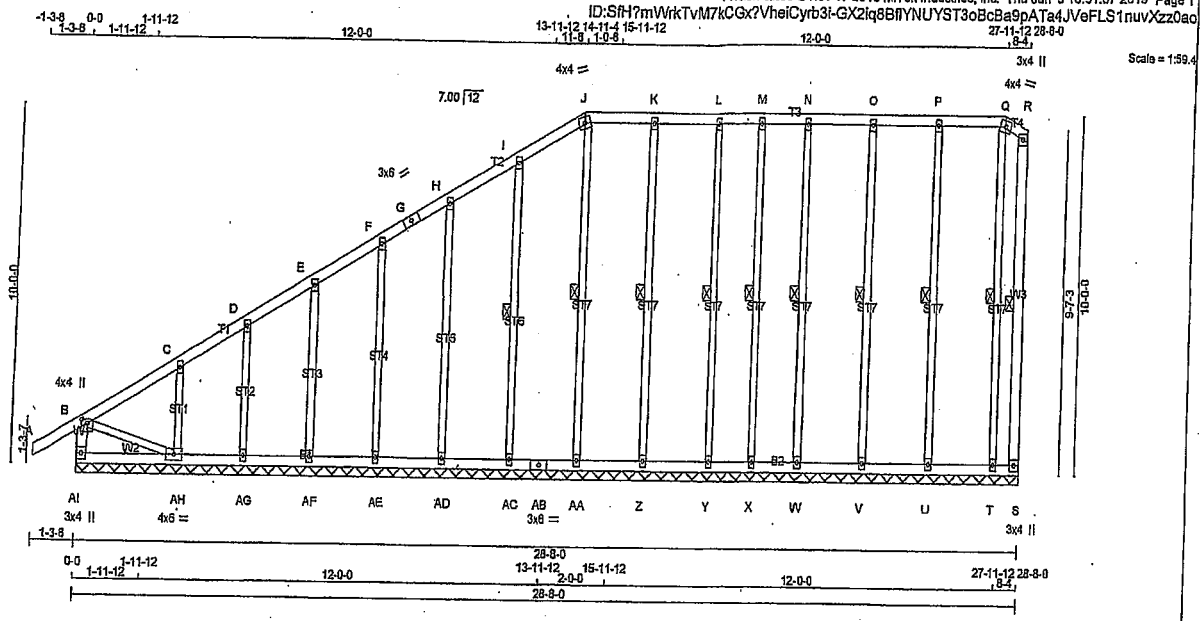


TOTAL WEIGHT = 172 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - Q	2x4	DRY	No.2	SPF
Q - R	2x4	DRY	No.2	SPF
AI - B	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
AI - AB	2x4	DRY	No.2	SPF
AB- S	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, F, H, I, K, L, M, N, O, P						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
Q	TTW-m	MT20	4.0	4.0		
R	TMV+p	MT20	3.0	4.0		
S	BMV1+p	MT20	3.0	4.0		
T, U, V, W, X, Y, Z, AA, AC, AD, AE, AF, AG						
T	BMW1+w	MT20	2.0	4.0		
AB	BS-t	MT20	3.0	6.0		
AH	BMWV1-t	MT20	4.0	6.0		
AI	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF R-S, Q-T, P-U, O-V, N-W, L-Y, K-Z, J-AA, I-AC, M-X.

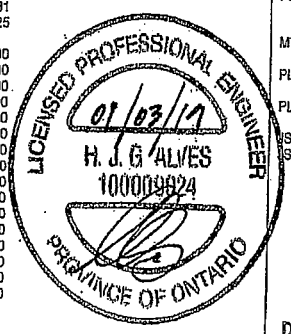
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 29	-84.9	-84.9	0.11 (1)	10.00
B-C	-9 / 0	-84.9	-84.9	0.09 (1)	10.00
C-D	-25 / 0	-84.9	-84.9	0.09 (1)	8.25
D-E	-7 / 0	-84.9	-84.9	0.04 (1)	10.00
E-F	-7 / 0	-84.9	-84.9	0.04 (1)	10.00
F-G	-4 / 0	-84.9	-84.9	0.04 (1)	10.00
G-H	-4 / 0	-84.9	-84.9	0.04 (1)	10.00
H-I	-1 / 0	-84.9	-84.9	0.04 (1)	10.00
I-J	-9 / 0	-84.9	-84.9	0.04 (1)	10.00
J-K	0 / 1	-84.9	-84.9	0.05 (1)	10.00
K-L	0 / 1	-84.9	-84.9	0.05 (1)	10.00
L-M	0 / 1	-84.9	-84.9	0.03 (1)	10.00
M-N	0 / 1	-84.9	-84.9	0.03 (1)	10.00
N-O	0 / 1	-84.9	-84.9	0.04 (1)	10.00
O-P	0 / 1	-84.9	-84.9	0.05 (1)	10.00
P-Q	0 / 1	-84.9	-84.9	0.05 (1)	10.00
Q-R	0 / 0	-84.9	-84.9	0.01 (1)	10.00
AI-B	-235 / 0	0.0	0.0	0.02 (1)	7.81
S-R	-29 / 0	0.0	0.0	0.01 (1)	6.25
AI-AH	0 / 0	-38.5	-38.5	0.06 (3)	10.00
AH-AG	0 / 11	-38.5	-38.5	0.06 (3)	10.00
AG-AF	0 / 8	-38.5	-38.5	0.03 (3)	10.00
AF-AE	0 / 6	-38.5	-38.5	0.03 (3)	10.00
AE-AD	0 / 4	-38.5	-38.5	0.02 (3)	10.00
AD-AC	0 / 2	-38.5	-38.5	0.02 (3)	10.00
AC-AB	0 / 0	-38.5	-38.5	0.02 (3)	10.00
AB-AA	0 / 0	-38.5	-38.5	0.02 (3)	10.00
AA-Z	-1 / 0	-38.5	-38.5	0.02 (3)	10.00
Z-Y	-1 / 0	-38.5	-38.5	0.02 (3)	10.00
Y-X	-1 / 0	-38.5	-38.5	0.02 (3)	10.00
X-W	-1 / 0	-38.5	-38.5	0.02 (3)	10.00
W-V	-1 / 0	-38.5	-38.5	0.02 (3)	10.00
V-U	-1 / 0	-38.5	-38.5	0.02 (3)	10.00
U-T	-1 / 0	-38.5	-38.5	0.02 (3)	10.00
T-S	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-Q	-102 / 0	0.07 (1)
U-P	-193 / 0	0.13 (1)
V-O	-171 / 0	0.12 (1)
W-N	-145 / 0	0.10 (1)
Y-L	-141 / 0	0.10 (1)
Z-K	-199 / 0	0.13 (1)
AA-J	-139 / 0	0.09 (1)
AC-I	-187 / 0	0.10 (1)
AD-H	-166 / 0	0.18 (1)
AE-F	-168 / 0	0.12 (1)
AF-E	-174 / 0	0.08 (1)
AG-D	-139 / 0	0.04 (1)
AH-C	-254 / 0	0.05 (1)
X-M	-112 / 0	0.08 (1)
B-AH	0 / 19	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.08/1.00 (AG-AH:3), WB=0.18/1.00 (H-AD:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.60 (J) (INPUT = 0.90)
SI METAL= 0.12 (C) (INPUT = 1.00)

DWG NO. TAM T19001/14
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T35	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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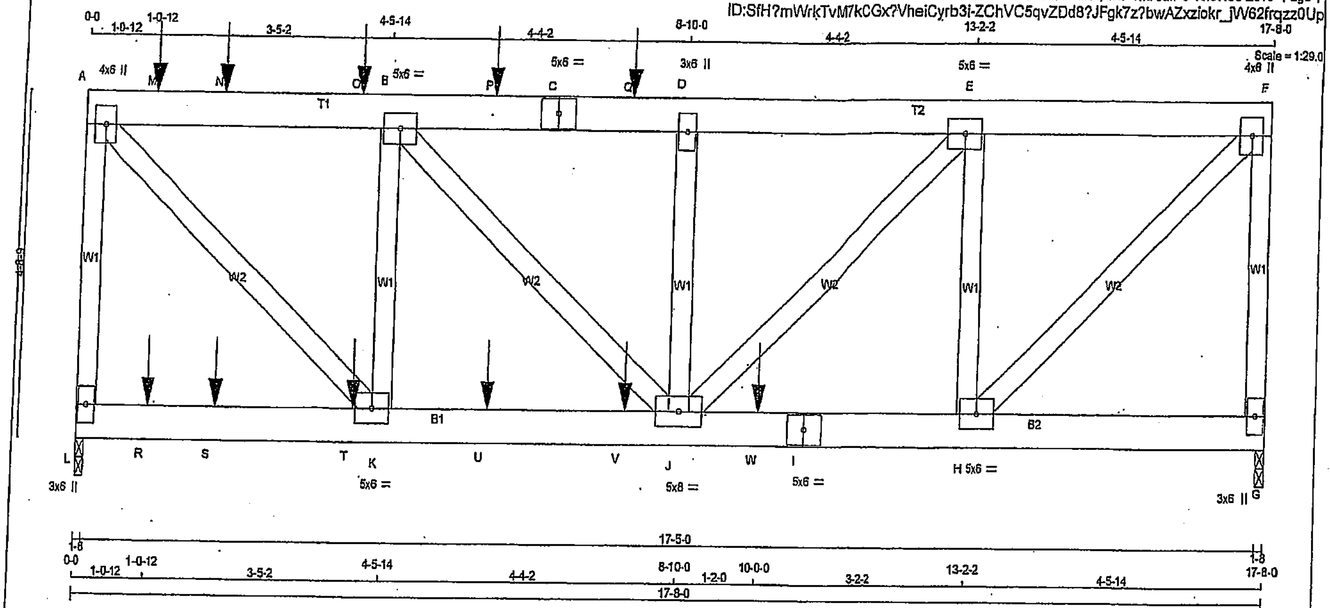

TOTAL WEIGHT = 2 X 110 = 221 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
L-A	1	12	TOP
F-G	1	12	TOP
A-C	2	12	SIDE(0.0)
C-F	2	12	SIDE(0.0)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L-I	2	12	SIDE(0.0)
I-G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
B	TMWW-t	MT20	5.0	6.0		
C	TS-t	MT20	5.0	6.0		
D	TMW+w	MT20	3.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMVW+p	MT20	4.0	8.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2499	0	2499	0	0	1-8	1-8
G	2034	0	2034	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
L	1884	971 / 0	398 / 0	0 / 0	0 / 0	515 / 0	0 / 0
G	1528	804 / 0	310 / 0	0 / 0	0 / 0	414 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
L-A	-2318 / 0	0.0	0.0	0.36 (1)	7.37
A-M	-2085 / 0	-84.9	-84.9	0.14 (1)	6.25
M-N	-2085 / 0	-84.9	-84.9	0.14 (1)	6.25
N-O	-2085 / 0	-84.9	-84.9	0.14 (1)	6.25
O-B	-2085 / 0	-84.9	-84.9	0.14 (1)	6.25
B-P	-2884 / 0	-84.9	-84.9	0.14 (1)	6.25
P-C	-2884 / 0	-84.9	-84.9	0.14 (1)	6.25
C-Q	-2884 / 0	-84.9	-84.9	0.14 (1)	6.25
Q-D	-2884 / 0	-84.9	-84.9	0.14 (1)	6.25
D-E	-2884 / 0	-84.9	-84.9	0.06 (1)	6.25
E-F	-1943 / 0	-84.9	-84.9	0.06 (1)	6.25
G-F	-2004 / 0	0.0	0.0	0.31 (1)	7.78
L-R	0 / 0	-38.5	-38.5	0.09 (2)	10.00
R-S	0 / 0	-38.5	-38.5	0.09 (2)	10.00
S-T	0 / 0	-38.5	-38.5	0.09 (2)	10.00
T-K	0 / 0	-38.5	-38.5	0.09 (2)	10.00
K-U	0 / 2085	-38.5	-38.5	0.28 (1)	10.00
U-V	0 / 2085	-38.5	-38.5	0.28 (1)	10.00
V-J	0 / 2085	-38.5	-38.5	0.28 (1)	10.00
J-W	0 / 1943	-38.5	-38.5	0.39 (1)	10.00
W-I	0 / 1943	-38.5	-38.5	0.39 (1)	10.00
I-H	0 / 1943	-38.5	-38.5	0.39 (1)	10.00
H-G	0 / 0	-38.5	-38.5	0.09 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-K	0 / 2873	0.25 (1)
K-B	-1693 / 0	0.18 (1)
B-J	0 / 1119	0.10 (1)
J-D	-496 / 0	0.05 (1)
J-E	0 / [illegible]	[illegible] (1)
H-E	[illegible]333 / 0	0.14 (1)
H-F	[illegible]	[illegible]

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J G ALVES
100009[illegible]
PROVINCE OF ONTARIO

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	1-0-12	-164	-164	—	FRONT	VERT	TOTAL	—	—
N	2-0-12	-164	-164	—	FRONT	VERT	TOTAL	—	—
O	4-0-12	-164	-164	—	FRONT	VERT	TOTAL	—	—
P	6-0-12	-164	-164	—	FRONT	VERT	TOTAL	—	—
Q	8-0-12	-164	-164	—	FRONT	VERT	TOTAL	—	—
R	1-0-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
S	2-0-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
T	4-0-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
U	6-0-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
V	8-0-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
W	10-0-0	-1158	-1158	—	TOP	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14.
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.36/1.00 (A-L:1) , BC=0.39/1.00 (H-J:1) , WB=0.25/1.00 (A-K:1) , SSI=0.32/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.47 (A) (INPUT = 1.00)

DWG NO. TAM 1960130
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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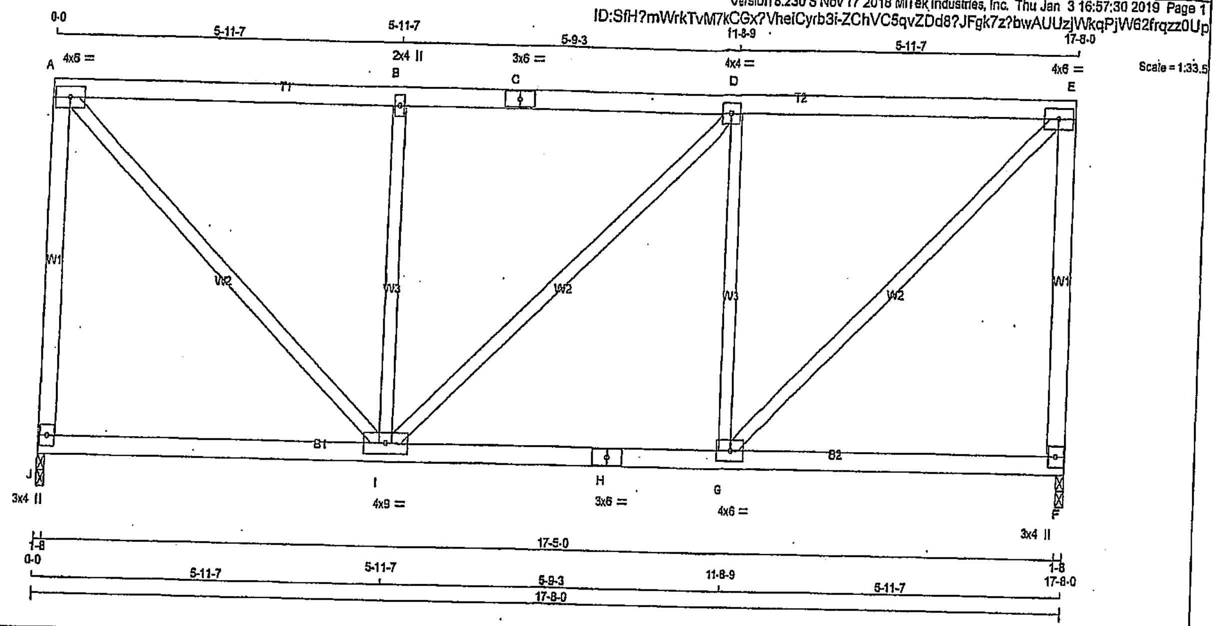

TOTAL WEIGHT = 80 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1090	0	1090	0	0	1-8	1-8
F	1090	0	1090	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
J	827	411 / 0	186 / 0	0 / 0	0 / 0	230 / 0	0 / 0
F	827	411 / 0	186 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J-A	-996 / 0	0.0	0.0	0.65 (1)	7.81
A-B	-842 / 0	-84.9	-84.9	0.42 (1)	6.03
B-C	-842 / 0	-84.9	-84.9	0.42 (1)	6.02
C-D	-842 / 0	-84.9	-84.9	0.42 (1)	6.02
D-E	-843 / 0	-84.9	-84.9	0.42 (1)	6.02
F-E	-996 / 0	0.0	0.0	0.65 (1)	7.81
J-I	0 / 0	-38.5	-38.5	0.24 (3)	10.00
I-H	0 / 843	-38.5	-38.5	0.34 (2)	10.00
H-G	0 / 843	-38.5	-38.5	0.34 (2)	10.00
G-F	0 / 0	-38.5	-38.5	0.24 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-E	0 / 1157	0.26 (1)
A-I	0 / 1155	0.26 (1)
G-D	-544 / 0	0.29 (1)
I-B	-544 / 0	0.29 (1)
I-D	-2 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.65/1.00 (E-F:1) , BC=0.34/1.00 (G-I:2) , WB=0.29/1.00 (D-G:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (G) (INPUT = 0.90)
JSI METAL= 0.29 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 119 00/31
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T37	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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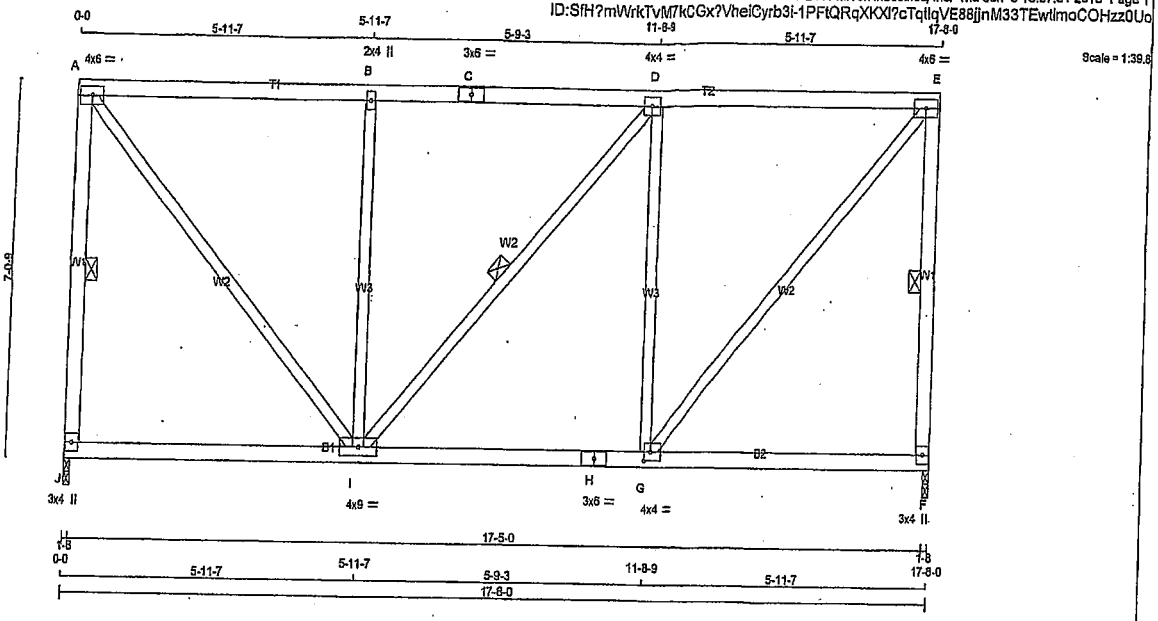


TOTAL WEIGHT = 86 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	4.0	2.00	1.50
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1090	0	1090	0	0	1-8	1-8
F	1090	0	1090	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	827	411 / 0	166 / 0	0 / 0	0 / 0	230 / 0	0 / 0
F	827	411 / 0	166 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-J, E-F, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

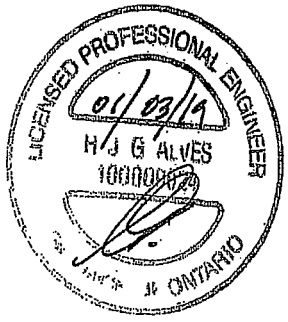
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
J-A	-996 / 0	0.0	0.0	0.22 (1)	6.25	
A-B	-697 / 0	-84.9	-84.9	0.41 (1)	6.25	
B-C	-697 / 0	-84.9	-84.9	0.42 (1)	6.25	
C-D	-697 / 0	-84.9	-84.9	0.42 (1)	6.25	
D-E	-698 / 0	-84.9	-84.9	0.42 (1)	6.25	
F-E	-996 / 0	0.0	0.0	0.22 (1)	6.25	
J-I	0 / 0	-38.5	-38.5	0.24 (3)	10.00	
I-H	0 / 698	-38.5	-38.5	0.32 (2)	10.00	
H-G	0 / 698	-38.5	-38.5	0.32 (2)	10.00	
G-F	0 / 0	-38.5	-38.5	0.24 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-E	0 / 1056	0.24 (1)
A-I	0 / 1054	0.24 (1)
G-D	-545 / 0	0.47 (1)
I-B	-544 / 0	0.46 (1)
I-D	-2 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.42/1.00 (D-E:1) , BC=0.32/1.00 (G-I:2) , WB=0.47/1.00 (D-G:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.29 (H) (INPUT = 1.00)

DWG NO. TAM 19000132
STRUCTURAL
ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T38	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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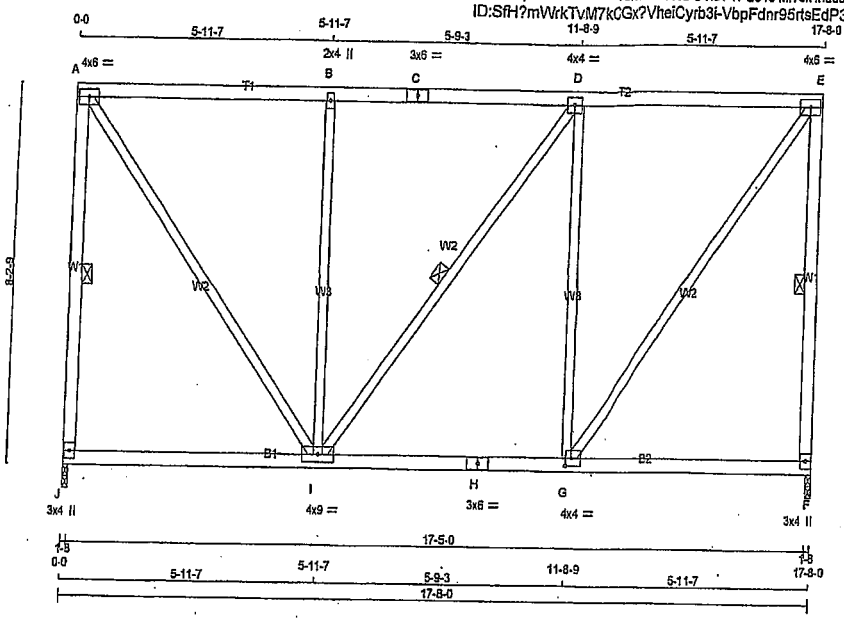


Scale = 1:46.1

TOTAL WEIGHT = 93 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	4.0	2.00	1.50
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1090	0	1090	0	0	1-8	1-8
F	1090	0	1090	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	827	411 / 0	166 / 0	0 / 0	0 / 0	230 / 0	0 / 0
F	827	411 / 0	166 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-J, E-F, D-I.

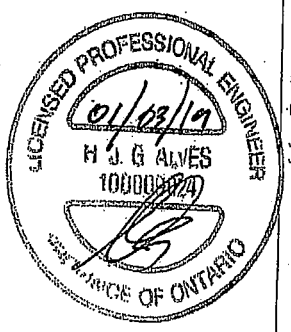
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
J-A	-996 / 0	0.0	0.0	0.30 (1)	6.25	
A-B	-594 / 0	-84.9	-84.9	0.41 (1)	6.25	
B-C	-594 / 0	-84.9	-84.9	0.41 (1)	6.25	
C-D	-594 / 0	-84.9	-84.9	0.41 (1)	6.25	
D-E	-595 / 0	-84.9	-84.9	0.41 (1)	6.25	
F-E	-997 / 0	0.0	0.0	0.30 (1)	6.25	
J-I	0 / 0	-38.5	-38.5	0.24 (3)	10.00	
I-H	0 / 595	-38.5	-38.5	0.31 (2)	10.00	
H-G	0 / 595	-38.5	-38.5	0.31 (2)	10.00	
G-F	0 / 0	-38.5	-38.5	0.24 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-E	0 / 991	0.22 (1)
A-I	0 / 989	0.22 (1)
G-D	-545 / 0	0.72 (1)
I-B	-544 / 0	0.72 (1)
I-D	-2 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.31/1.00 (G-I:2) , WB=0.72/1.00 (D-G:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.25 (G) (INPUT = 1.00)

DWG NO. TAM 1900133
STRUCTURAL
CONFIGURATION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T39X	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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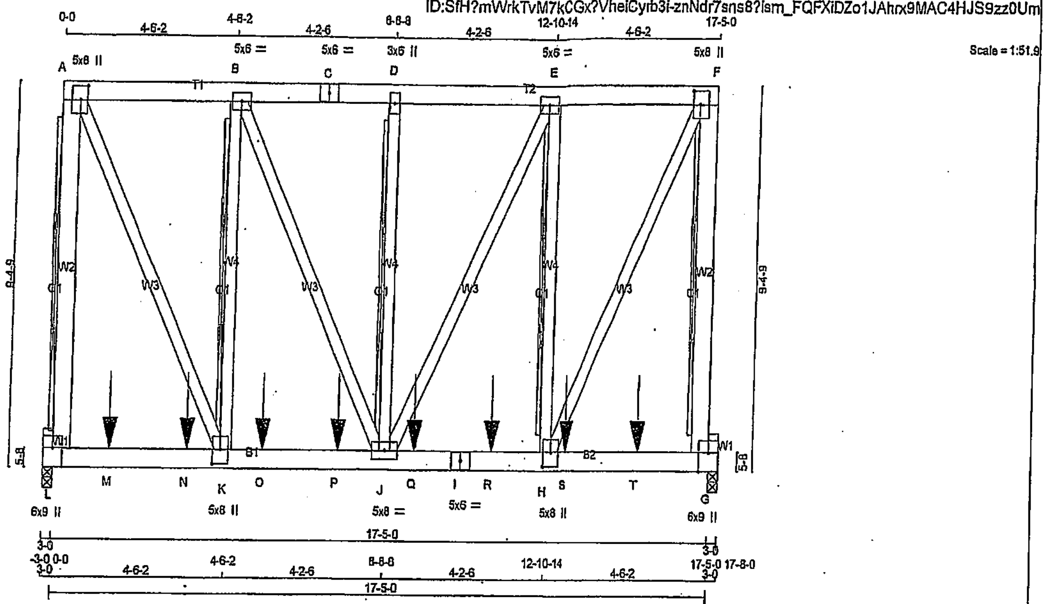

TOTAL WEIGHT = 3 X 166 = 498 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
L-A	2	12	TOP
A-C	2	12	TOP
C-F	2	12	TOP
F-G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L-I	2	9	SIDE(451.2)
I-G	2	9	SIDE(451.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	8.0	4.00	2.25
B	TMWW-t	MT20	5.0	6.0		
C	TS-t	MT20	5.0	6.0		
D	TMW+w	MT20	3.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMVW+p	MT20	5.0	8.0	4.00	2.25
G	BMVW1+t	MT20	6.0	9.0	Edge	3.00
H	BMWW+t	MT20	5.0	8.0	4.25	2.50
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW+t	MT20	5.0	8.0	4.25	2.50
L	BMVW1+t	MT20	6.0	9.0	Edge	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	8175	0	8175	0	0	3-0	3-0
G	7811	0	7811	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
L	6177	3146 / 0	1332 / 0	0 / 0	0 / 0	1699 / 0	0 / 0
G	5902	3006 / 0	1273 / 0	0 / 0	0 / 0	1623 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT A-L, F-G, E-H, B-K, D-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-A	-6951 / 0	0.0	0.0	0.54 (1)	7.81	H-E	-2276 / 0	0.22 (1)
A-B	-3419 / 0	-84.9	-84.9	0.05 (1)	6.25	K-B	-2227 / 0	0.21 (1)
B-C	-4294 / 0	-84.9	-84.9	0.05 (1)	6.25	J-D	-367 / 0	0.04 (1)
C-D	-4294 / 0	-84.9	-84.9	0.05 (1)	6.25	H-F	0 / 7527	0.40 (1)
D-E	-4294 / 0	-84.9	-84.9	0.05 (1)	6.25	A-K	0 / 7578	0.41 (1)
E-F	-3396 / 0	-84.9	-84.9	0.05 (1)	6.25	J-E	0 / 2109	0.11 (1)
G-F	-6906 / 0	0.0	0.0	0.54 (1)	7.81	B-J	0 / 2056	0.11 (1)
L-M	0 / 0	-38.5	-38.5	0.42 (1)	10.00			
M-N	0 / 0	-38.5	-38.5	0.42 (1)	10.00			
N-K	0 / 0	-38.5	-38.5	0.42 (1)	10.00			
K-O	0 / 3419	-38.5	-38.5	0.56 (1)	10.00			
O-P	0 / 3419	-38.5	-38.5	0.56 (1)	10.00			
P-J	0 / 3419	-38.5	-38.5	0.56 (1)	10.00			
J-Q	0 / 3396	-38.5	-38.5	0.52 (1)	10.00			
Q-I	0 / 3396	-38.5	-38.5	0.52 (1)	10.00			
I-R	0 / 3396	-38.5	-38.5	0.52 (1)	10.00			
R-H	0 / 3396	-38.5	-38.5	0.52 (1)	10.00			
H-S	0 / 0	-38.5	-38.5	0.39 (1)	10.00			
S-T	0 / 0	-38.5	-38.5	0.39 (1)	10.00			
T-G	0 / 0	-38.5	-38.5	0.39 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	1-5-12	-1730	-1730	—	BACK	VERT	TOTAL	—	—
N	3-5-12	-1730	-1730	—	BACK	VERT	TOTAL	—	—
O	5-5-12	-1730	-1730	—	BACK	VERT	TOTAL	—	—
P	7-5-12	-1730	-1730	—	BACK	VERT	TOTAL	—	—
Q	9-5-12	-1730	-1730	—	BACK	VERT	TOTAL	—	—
R	11-5-12	-1730	-1730	—	BACK	VERT	TOTAL	—	—
S	13-5-12	-1730	-1730	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G ALVES
100009824
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.54/1.00 (A-L:1) , BC=0.56/1.00 (J-K:1) , WB=0.41/1.00 (A-K:1) , SSI=0.55/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (A) (INPUT = 0.90)
JSI METAL= 0.47 (A) (INPUT = 1.00)

DWG NO. TAM 1900134
STRUCTURAL
COMPONENT ONLY

1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T39X	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	15-5-12	-1730	-1730	—	TOP	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/03/19
H.J.G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM T1900134
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	G40	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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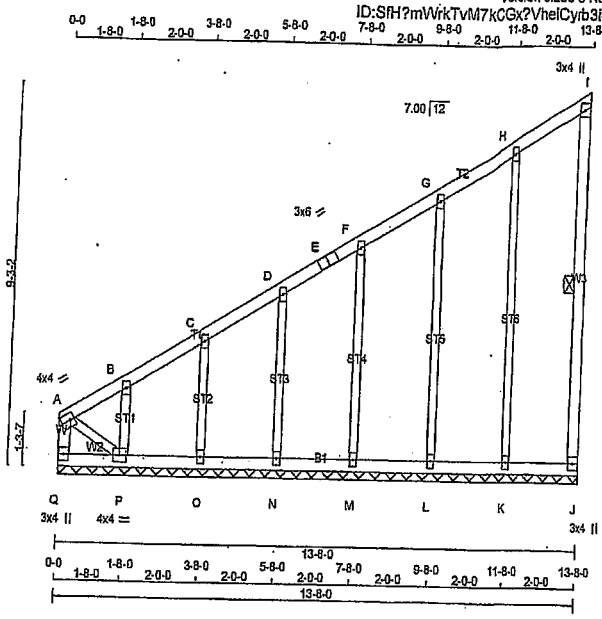


Scale = 1:51.6

TOTAL WEIGHT = 68 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
Q - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	Edge
B, C, D, F, G, H						
B	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	BMV1+p	MT20	3.0	4.0		
K, L, M, N, O						
K	BMW1+w	MT20	2.0	4.0		
P	BMWW1-t	MT20	4.0	4.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-J.

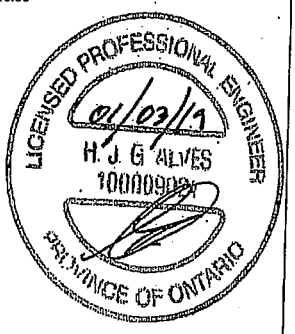
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	-14 / 0	-84.9	-84.9	0.04 (1)		6.25
B-C	-15 / 0	-84.9	-84.9	0.04 (1)		6.25
C-D	-10 / 0	-84.9	-84.9	0.04 (1)		6.25
D-E	-7 / 0	-84.9	-84.9	0.04 (1)		10.00
E-F	-7 / 0	-84.9	-84.9	0.04 (1)		10.00
F-G	-4 / 0	-84.9	-84.9	0.04 (1)		10.00
G-H	0 / 0	-84.9	-84.9	0.04 (1)		10.00
H-I	-8 / 0	-84.9	-84.9	0.04 (1)		10.00
J-I	-72 / 0	0.0	0.0	0.03 (1)		6.25
Q-A	-73 / 0	0.0	0.0	0.01 (1)		7.81
Q-P	0 / 0	-38.5	-38.5	0.02 (3)		10.00
P-O	0 / 13	-38.5	-38.5	0.02 (3)		10.00
O-N	0 / 9	-38.5	-38.5	0.02 (3)		10.00
N-M	0 / 6	-38.5	-38.5	0.02 (3)		10.00
M-L	0 / 4	-38.5	-38.5	0.02 (3)		10.00
L-K	0 / 2	-38.5	-38.5	0.03 (3)		10.00
K-J	0 / 0	-38.5	-38.5	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-H	-190 / 0	0.24 (1)
L-G	-166 / 0	0.14 (1)
M-F	-169 / 0	0.09 (1)
N-D	-168 / 0	0.05 (1)
O-C	-168 / 0	0.03 (1)
P-B	-165 / 0	0.03 (1)
A-P	0 / 22	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (H-I:1) , BC=0.03/1.00 (K-L:3) , WB=0.24/1.00 (H-K:1) , SSI=0.07/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (H) (INPUT = 0.90)
JSI METAL= 0.09 (H) (INPUT = 1.00)

DWG NO. TAM T1900123
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	G41	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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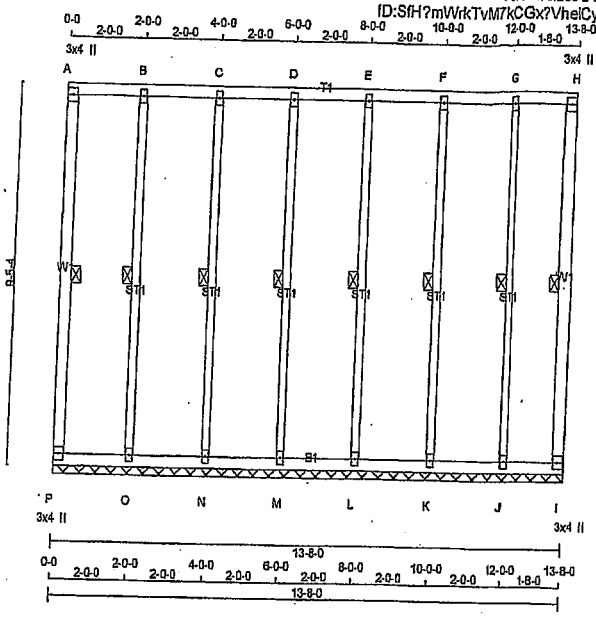

Scale = 1:52.7

TOTAL WEIGHT = 95 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - A	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B, C, D, E, F, G						
B	TMV+w	MT20	2.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		
J, K, L, M, N, O						
J	BMW1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-P, H-I, B-O, C-N, D-M, E-L, F-K, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-A	-74 / 0	0.0	0.0	0.01 (1)		6.25
A-B	-1 / 0	-84.9	-84.9	0.04 (1)		10.00
B-C	-1 / 0	-84.9	-84.9	0.04 (1)		10.00
C-D	-1 / 0	-84.9	-84.9	0.04 (1)		10.00
D-E	-1 / 0	-84.9	-84.9	0.04 (1)		10.00
E-F	-1 / 0	-84.9	-84.9	0.04 (1)		10.00
F-G	-1 / 0	-84.9	-84.9	0.04 (1)		10.00
G-H	-1 / 0	-84.9	-84.9	0.03 (1)		10.00
I-H	-62 / 0	0.0	0.0	0.01 (1)		6.25
P-O	0 / 1	-38.5	-38.5	0.03 (3)		10.00
O-N	0 / 1	-38.5	-38.5	0.03 (3)		10.00
N-M	0 / 1	-38.5	-38.5	0.02 (3)		10.00
M-L	0 / 1	-38.5	-38.5	0.02 (3)		10.00
L-K	0 / 1	-38.5	-38.5	0.02 (3)		10.00
K-J	0 / 1	-38.5	-38.5	0.02 (3)		10.00
J-I	0 / 1	-38.5	-38.5	0.02 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-B	-180 / 0	0.11 (1)
N-C	-170 / 0	0.10 (1)
M-D	-170 / 0	0.10 (1)
L-E	-169 / 0	0.10 (1)
K-F	-174 / 0	0.10 (1)
J-G	-161 / 0	0.09 (1)

LICENSED PROFESSIONAL ENGINEER
01/02/19
H. J. G ALVES
100099024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (E-F:1) , BC=0.03/1.00 (N-O:3) , WB=0.11/1.00 (B-O:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (O) (INPUT = 0.90)
JSI METAL= 0.04 (O) (INPUT = 1.00)

DWG NO. TAM 19 00124
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T42	1.	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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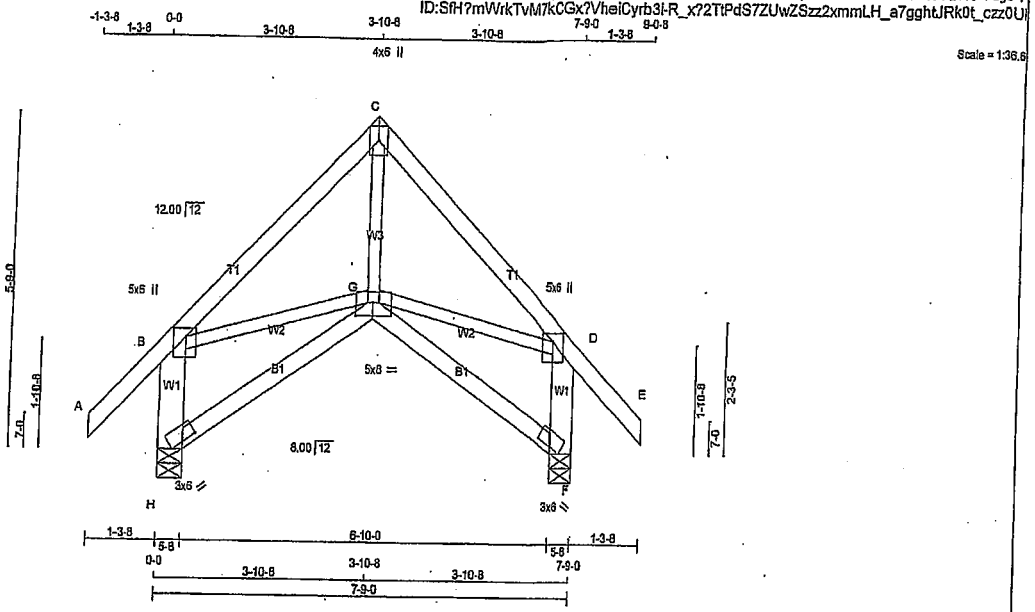

TOTAL WEIGHT = 42 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	1.75	2.75
C	TTW+p	MT20	4.0	6.0		
D	TMWW+p	MT20	5.0	6.0	1.75	2.75
F	BVM1-I	MT20	3.0	6.0	0.50	3.00
G	BBWWW-p	MT20	5.0	8.0	2.75	4.00
H	BVM1-I	MT20	3.0	6.0	0.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	596	0	596	0	0	5-8	5-8
F	596	0	596	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	444	245 / 0	81 / 0	0 / 0	0 / 0	117 / 0	0 / 0
F	444	245 / 0	81 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 42	-84.9	-84.9	0.12 (1)		10.00	G-C	0 / 299	0.07 (2)
B-C	-424 / 0	-84.9	-84.9	0.17 (1)		6.25	B-G	0 / 306	0.07 (1)
C-D	-424 / 0	-84.9	-84.9	0.17 (1)		6.25	G-D	0 / 306	0.07 (1)
D-E	0 / 42	-84.9	-84.9	0.12 (1)		10.00			
H-B	-522 / 0	0.0	0.0	0.04 (1)		7.81			
F-D	-522 / 0	0.0	0.0	0.04 (1)		7.81			
H-G	0 / 0	-38.5	-38.5	0.14 (3)		10.00			
G-F	0 / 0	-38.5	-38.5	0.14 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.17/1.00 (C-D:1) , BC=0.14/1.00 (F-G:3) , WB=0.07/1.00 (B-G:1) , SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (G) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G ALVES
100099024
PROVINCE OF ONTARIO

DWG NO. TAM 1900135
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T46A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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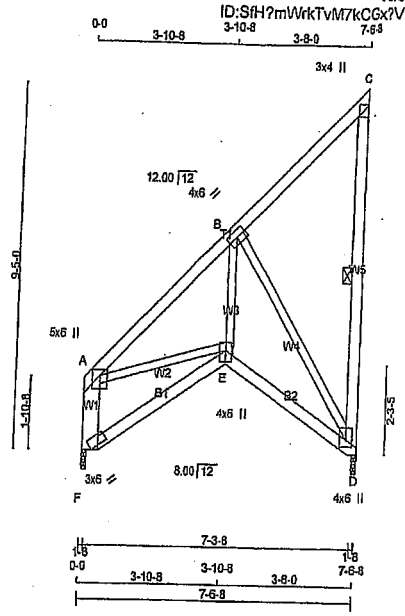

Scale = 1:54.6

TOTAL WEIGHT = 46 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES.	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0	2.25	2.00
E	BBWW+p	MT20	4.0	6.0	3.50	2.00
F	BVM1-I	MT20	3.0	6.0	0.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	465	0	465	0	0	1-8	1-8
F	465	0	465	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	353	176 / 0	79 / 0	0 / 0	0 / 0	98 / 0	0 / 0
F	353	176 / 0	79 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-431 / 0	-84.9	-84.9	0.16 (1)	6.25	E- B	0 / 421	0.09 (2)
B- C	-28 / 0	-84.9	-84.9	0.15 (1)	6.25	A- E	0 / 329	0.07 (1)
D- C	-111 / 0	0.0	0.0	0.05 (1)	6.25	B- D	-579 / 0	0.43 (1)
F- A	-391 / 0	0.0	0.0	0.03 (1)	7.81			
F- E	0 / 0	-38.5	-38.5	0.13 (3)	10.00			
E- D	0 / 372	-38.5	-38.5	0.16 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)=. L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.16/1.00 (D-E:2) , WB=0.43/1.00 (B-D:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (D) (INPUT = 0.90)
JSI METAL= 0.27 (D) (INPUT = 1.00)

DWG NO. TAM 1900136
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	T140	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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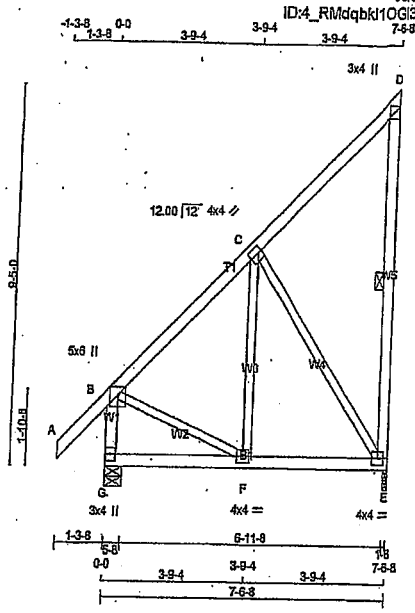

TOTAL WEIGHT = 2 X 47 = 94 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMVW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	465	0	465	0	0	1-8	1-8
G	584	0	584	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	353	176 / 0	79 / 0	0 / 0	0 / 0	98 / 0	0 / 0
G	435	241 / 0	79 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	F-C	0 / 186	0.04 (3)
B-C	-263 / 0	-84.9	-84.9	0.16 (1)	6.25	C-E	-354 / 0	0.26 (1)
C-D	-26 / 0	-84.9	-84.9	0.16 (1)	6.25	B-F	0 / 221	0.05 (1)
E-D	-121 / 0	0.0	0.0	0.05 (1)	6.25			
G-B	-527 / 0	0.0	0.0	0.06 (1)	7.81			
G-F	0 / 0	-38.5	-38.5	0.12 (3)	10.00			
F-E	0 / 205	-38.5	-38.5	0.14 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015.

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.14/1.00 (E-F:2) , WB=0.26/1.00 (C-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009924
PROVINCE OF ONTARIO

DWG NO. TAM T1901091
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	G140	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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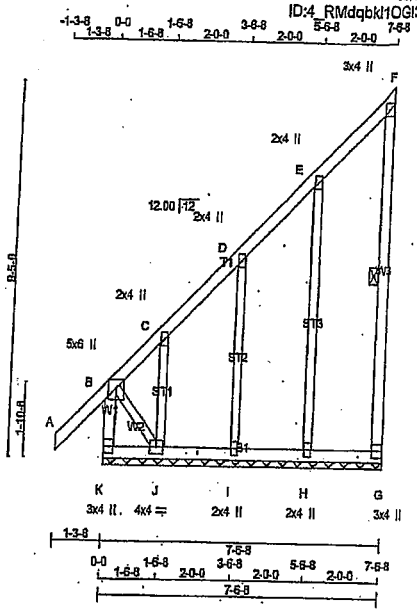


Scale = 1:53.2

TOTAL WEIGHT = 2 X 48 = 96 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C, D, E						
C	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMV1+w	MT20	2.0	4.0		
J	BMWW1-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

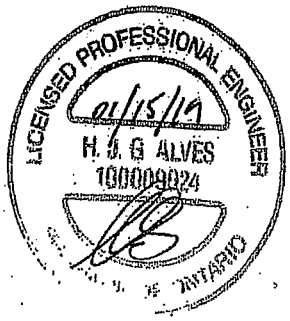
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	
B-C	-49 / 0	-84.9	-84.9	0.11 (1)	6.25	
C-D	-27 / 0	-84.9	-84.9	0.04 (1)	10.00	
D-E	-3 / 0	-84.9	-84.9	0.05 (1)	10.00	
E-F	-11 / 0	-84.9	-84.9	0.05 (1)	6.25	
G-F	-73 / 0	0.0	0.0	0.03 (1)	6.25	
K-B	-249 / 0	0.0	0.0	0.03 (1)	7.81	
K-J	0 / 0	-38.5	-38.5	0.02 (3)	10.00	
J-I	0 / 6	-38.5	-38.5	0.02 (3)	10.00	
I-H	0 / 3	-38.5	-38.5	0.03 (3)	10.00	
H-G	0 / 0	-38.5	-38.5	0.03 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-E	-189 / 0	0.18 (1)
I-D	-175 / 0	0.08 (1)
J-C	-82 / 0	0.02 (1)
B-J	0 / 13	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1) , BC=0.03/1.00 (H-I:3) , WB=0.18/1.00 (E-H:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX.	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

DWG NO. TAM T1901092
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	T141	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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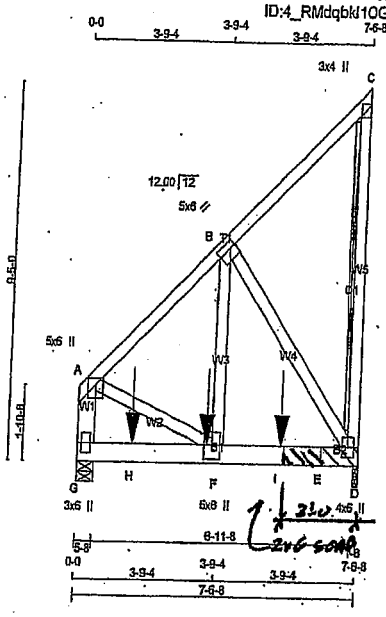

TOTAL WEIGHT = 4 X 57 = 226 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
G - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-D	1	12	TOP
G-A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G-D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B-F	1	8	SIDE(464.6)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TMVW+t	MT20	5.0	6.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
F	BMVW+t	MT20	5.0	8.0	4.25	2.50
G	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	3997	0	3997	0	0	1-8	1-8 & BLOCK
G	4382	0	4382	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	3013	1558 / 0	632 / 0	0 / 0	0 / 0	823 / 0	0 / 0
G	3302	1708 / 0	692 / 0	0 / 0	0 / 0	902 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G

2x6 DRY SPF No.2 BEARING BLOCK 24" LONG AT JT. D ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 3" O.C. 24 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-3103 / 0	-84.9	-84.9	0.15 (1)	5.13	F-B	0 / 4679	0.43 (1)
B-C	-24 / 0	-84.9	-84.9	0.10 (1)	6.25	B-D	-3772 / 0	0.97 (1)
D-C	-125 / 0	0.0	0.0	0.04 (1)	7.81	A-F	0 / 2367	0.21 (1)
G-A	-3181 / 0	0.0	0.0	0.12 (1)	7.75			
G-H	0 / 0	-38.5	-38.5	0.64 (1)	10.00			
H-F	0 / 0	-38.5	-38.5	0.64 (1)	10.00			
F-I	0 / 2212	-38.5	-38.5	0.73 (1)	10.00			
I-E	0 / 2212	-38.5	-38.5	0.73 (1)	10.00			
E-D	0 / 2212	-38.5	-38.5	0.73 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	3-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
H	1-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
I	5-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.73/1.00 (D-F:1) , WB=0.97/1.00 (B-D:1) , SSI=0.60/1.00 (D-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	769	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.53 (F) (INPUT = 1.00)

DWG NO. TAM T1901090
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	P1	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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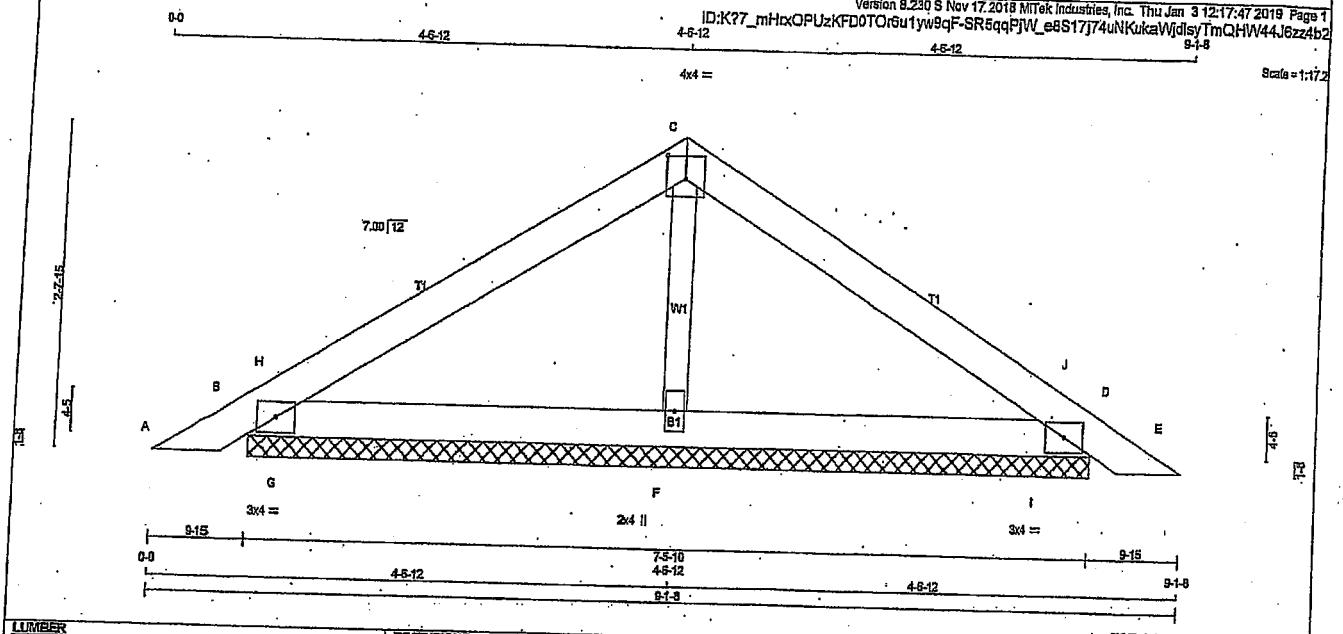

TOTAL WEIGHT = 6 X 22 = 133 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	322	0	322	0	0	7-5-10	7-5-10
D	322	0	322	0	0	7-5-10	7-5-10
F	404	0	404	0	0	7-5-10	7-5-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	237	141 / 0	36 / 0	0 / 0	0 / 0	60 / 0	0 / 0
D	237	141 / 0	36 / 0	0 / 0	0 / 0	60 / 0	0 / 0
F	313	139 / 0	85 / 0	0 / 0	0 / 0	92 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-84.9	-84.9	0.04 (1)	10.00
B-H	-75 / 0	-84.9	-84.9	0.04 (1)	6.25
H-C	-116 / 0	-84.9	-84.9	0.15 (1)	6.25
C-J	-116 / 0	-84.9	-84.9	0.15 (1)	6.25
J-D	-75 / 0	-84.9	-84.9	0.04 (1)	6.25
D-E	0 / 15	-84.9	-84.9	0.04 (1)	10.00
B-G	0 / 97	-38.5	-38.5	0.14 (1)	10.00
G-F	0 / 97	-38.5	-38.5	0.16 (1)	10.00
F-I	0 / 97	-38.5	-38.5	0.16 (1)	10.00
I-D	0 / 97	-38.5	-38.5	0.14 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-158 / 0	0.02 (1)
G-H	-246 / 24	0.00 (1)
I-J	-246 / 24	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-J:1) , BC=0.16/1.00 (F-I:1) , WB=0.02/1.00 (C-F:1) , SSI=0.21/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	766	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100009021
PROVINCE OF ONTARIO

DWG NO. TAM 1900063
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	P1G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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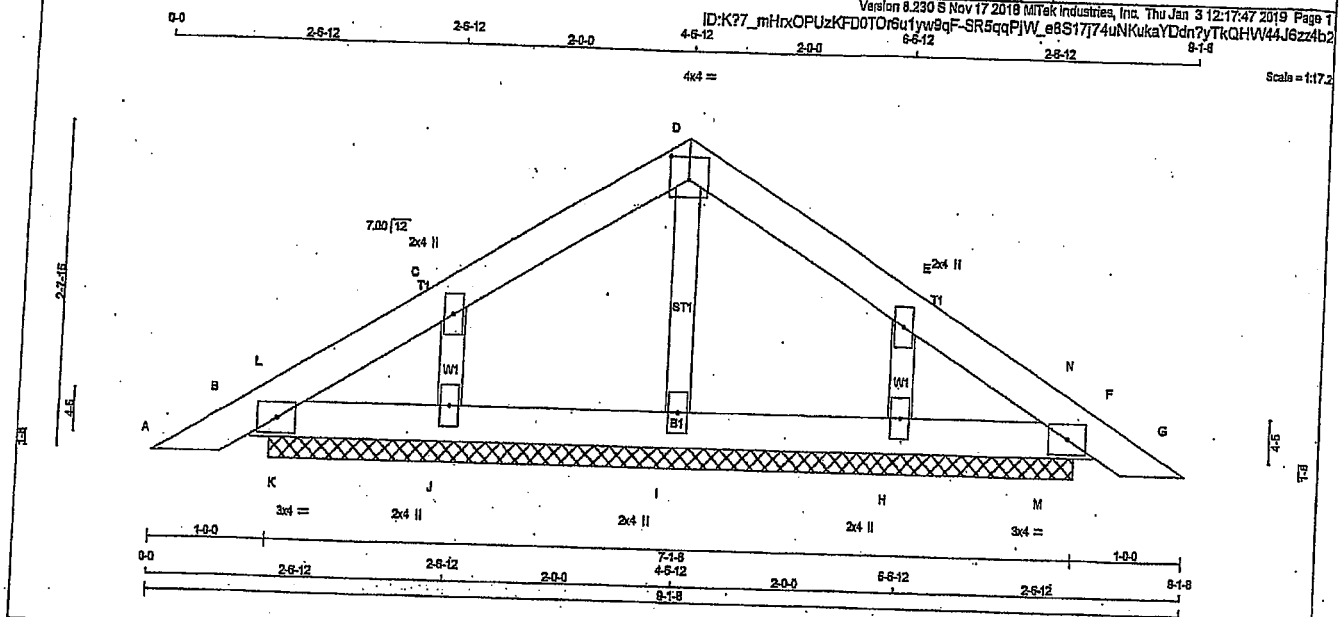


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 23 = 47 lb [M]

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-84.9	-84.9	0.04 (1)	10.00
B-L	-53 / 0	-84.9	-84.9	0.01 (2)	6.25
L-C	-21 / 0	-84.9	-84.9	0.05 (1)	6.25
C-D	-31 / 0	-84.9	-84.9	0.05 (1)	6.25
D-E	-31 / 0	-84.9	-84.9	0.05 (1)	6.25
E-N	-21 / 0	-84.9	-84.9	0.05 (1)	6.25
N-F	-53 / 0	-84.9	-84.9	0.01 (2)	6.25
F-G	0 / 15	-84.9	-84.9	0.04 (1)	10.00
B-K	0 / 33	-38.5	-38.5	0.02 (1)	10.00
K-J	0 / 33	-38.5	-38.5	0.03 (2)	10.00
J-I	0 / 19	-38.5	-38.5	0.03 (2)	10.00
I-H	0 / 19	-38.5	-38.5	0.03 (2)	10.00
H-M	0 / 33	-38.5	-38.5	0.03 (2)	10.00
M-F	0 / 33	-38.5	-38.5	0.02 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	-118 / 0	0.02 (1)
J-C	-188 / 0	0.03 (1)
H-E	-188 / 0	0.03 (1)
K-L	-11 / 17	0.00 (1)
M-N	-11 / 17	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-E:1) , BC=0.03/1.00 (J-K:2) , WB=0.03/1.00 (E-H:1) , SSI=0.07/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

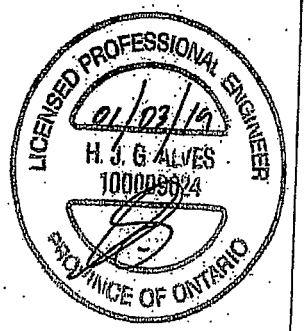
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1556

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.12 (C) (INPUT = 0.90)
JSI METAL = 0.09 (C) (INPUT = 1.00)



ENG NO. TAM T190064
STRUCTURAL
CERTIFICATE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300799	P10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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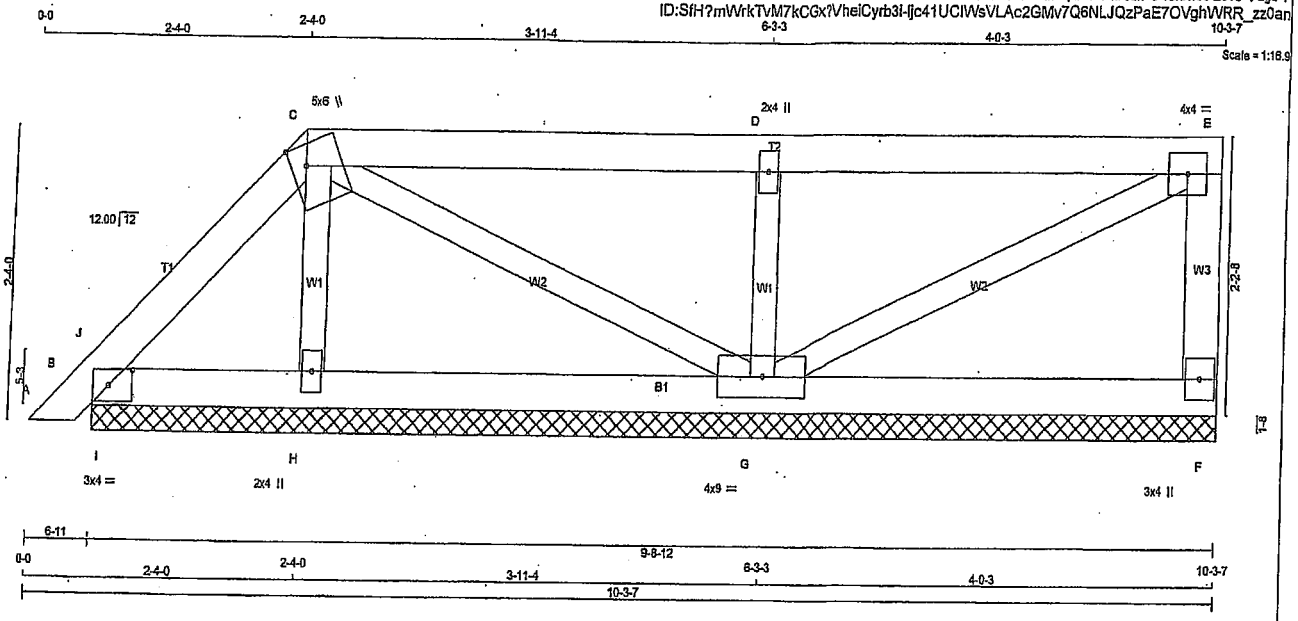


TOTAL WEIGHT = 2 X 35 = 70 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX.
F	189	0	189	0	0	9-8-12	9-8-12
B	173	0	173	0	0	9-8-12	9-8-12
H	263	0	263	0	0	9-8-12	9-8-12
G	613	0	613	0	0	9-8-12	9-8-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	144	70 / 0	33 / 0	0 / 0	0 / 0	40 / 0	0 / 0
B	124	64 / 0	11 / 0	0 / 0	0 / 0	29 / 0	0 / 0
H	207	81 / 0	63 / 0	0 / 0	0 / 0	63 / 0	0 / 0
G	462	239 / 0	97 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

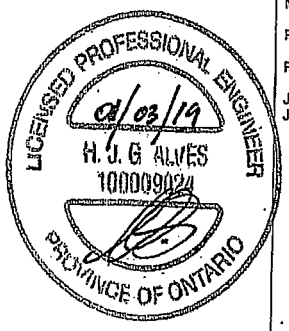
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 12	-84.9	-84.9	0.01 (1)	10.00
B-J	-30 / 0	-84.9	-84.9	0.01 (1)	6.25
J-C	-58 / 0	-84.9	-84.9	0.03 (1)	6.25
C-D	0 / 2	-84.9	-84.9	0.23 (1)	10.00
D-E	0 / 2	-84.9	-84.9	0.23 (1)	10.00
F-E	-128 / 0	0.0	0.0	0.01 (1)	7.81
B-I	0 / 38	-38.5	-38.5	0.04 (1)	10.00
I-H	0 / 38	-38.5	-38.5	0.06 (2)	10.00
H-G	0 / 31	-38.5	-38.5	0.12 (3)	10.00
G-F	0 / 0	-38.5	-38.5	0.12 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-132 / 0	0.02 (1)
G-E	-2 / 0	0.00 (1)
C-G	-36 / 0	0.01 (1)
G-D	-421 / 0	0.06 (1)
I-J	-110 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-D:1) , BC=0.12/1.00 (G-H:3) , WB=0.06/1.00 (D-G:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

DWG NO. TAM T1900116
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300799	P11	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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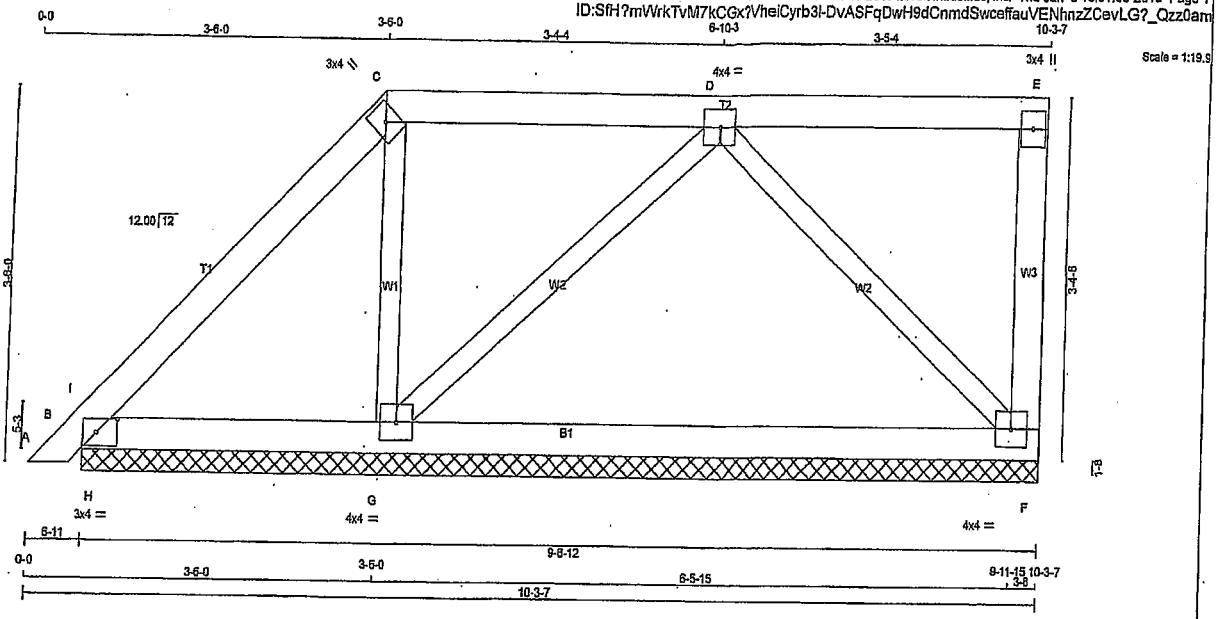


TOTAL WEIGHT = 3 X 37 = 110 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTW+h	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	395	0	395	0	0	9-8-12	9-8-12
B	190	0	190	0	0	9-8-12	9-8-12
G	654	0	654	0	0	9-8-12	9-8-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	295	162 / 0	54 / 0	0 / 0	0 / 0	78 / 0	0 / 0
B	130	107 / 0	0 / -2	0 / 0	0 / 0	26 / 0	0 / 0
G	512	206 / 0	152 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

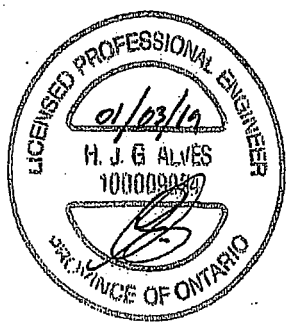
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 12	-84.9	-84.9	0.01 (1)	10.00
B-I	0 / 163	-84.9	-84.9	0.09 (1)	10.00
I-C	-46 / 1	-84.9	-84.9	0.05 (1)	6.25
C-D	-16 / 7	-84.9	-84.9	0.16 (1)	6.25
D-E	0 / 0	-84.9	-84.9	0.16 (1)	10.00
F-E	-112 / 0	0.0	0.0	0.02 (1)	7.81
B-H	-6 / 23	-38.5	-38.5	0.10 (1)	10.00
H-G	-6 / 23	-38.5	-38.5	0.30 (3)	10.00
G-F	0 / 202	-38.5	-38.5	0.32 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	-182 / 0	0.04 (1)
G-D	-255 / 0	0.09 (1)
D-F	-271 / 0	0.09 (1)
H-I	-363 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-D:1) , BC=0.32/1.00 (F-G:2) , WB=0.09/1.00 (D-F:1) , SSI=0.28/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (C) (INPUT = 0.80)
JSI METAL= 0.08 (F) (INPUT = 1.00)

DWG NO. TAM T1900117
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	P12	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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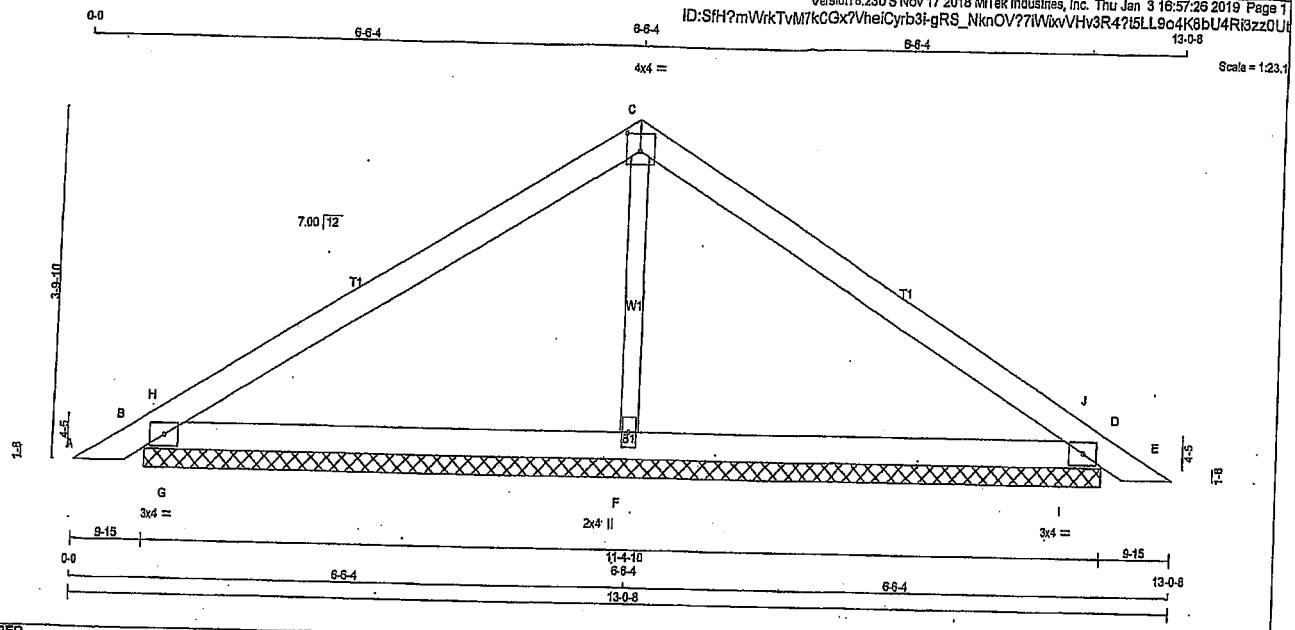

TOTAL WEIGHT = 7 X 33 = 229 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	486	0	486	0	0	11-4-10	11-4-10
D	486	0	486	0	0	11-4-10	11-4-10
F	560	0	560	0	0	11-4-10	11-4-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	359	209 / 0	58 / 0	0 / 0	0 / 0	92 / 0	0 / 0
D	359	209 / 0	58 / 0	0 / 0	0 / 0	92 / 0	0 / 0
F	436	183 / 0	124 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.04 (1)	10.00
B- H	-134 / 0	-84.9	-84.9	0.14 (1)	6.25
H- C	-235 / 0	-84.9	-84.9	0.35 (1)	6.25
C- J	-235 / 0	-84.9	-84.9	0.35 (1)	6.25
J- D	-134 / 0	-84.9	-84.9	0.14 (1)	6.25
D- E	0 / 15	-84.9	-84.9	0.04 (1)	10.00
B- G	0 / 194	-38.5	-38.5	0.30 (1)	10.00
G- F	0 / 194	-38.5	-38.5	0.37 (2)	10.00
F- I	0 / 194	-38.5	-38.5	0.37 (2)	10.00
I- D	0 / 194	-38.5	-38.5	0.30 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-188 / 0	0.04 (1)
G- H	-534 / 54	0.00 (1)
I- J	-534 / 54	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.35/1.00 (C-H:1) , BC=0.37/1.00 (F-G:2) , WB=0.04/1.00 (C-F:1) , SSI=0.44/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (D) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/03/19
H. J. G. ALVES
100009844
PROVINCE OF ONTARIO

DWG NO. TAM T19 00126
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	P12G	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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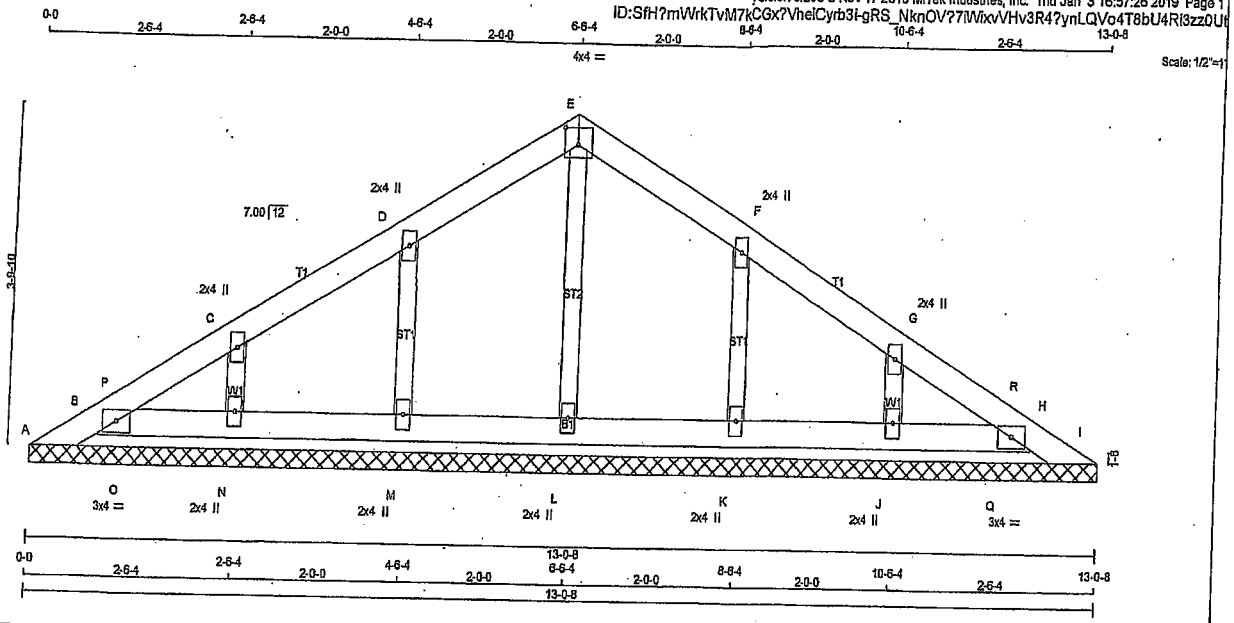


TOTAL WEIGHT = 37 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMB1-I	MT20	3.0	4.0		
J, K, L, M, N						
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): L, M, K, J, N
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 8	-102.4	-102.4	0.02 (1)	10.00
B-P	-67 / 0	-84.9	-84.9	0.02 (1)	6.25
P-C	-29 / 0	-84.9	-84.9	0.03 (1)	6.25
C-D	-22 / 0	-84.9	-84.9	0.05 (1)	6.25
D-E	-28 / 0	-84.9	-84.9	0.05 (1)	6.25
E-F	-28 / 0	-84.9	-84.9	0.05 (1)	6.25
F-G	-22 / 0	-84.9	-84.9	0.05 (1)	6.25
G-R	-29 / 0	-84.9	-84.9	0.03 (1)	6.25
R-H	-67 / 0	-84.9	-84.9	0.02 (1)	6.25
H-I	0 / 8	-102.4	-102.4	0.02 (1)	10.00
B-O	0 / 33	-38.5	-38.5	0.01 (1)	10.00
O-N	0 / 33	-38.5	-38.5	0.03 (2)	10.00
N-M	0 / 23	-38.5	-38.5	0.03 (3)	10.00
M-L	0 / 16	-38.5	-38.5	0.03 (3)	10.00
L-K	0 / 16	-38.5	-38.5	0.03 (3)	10.00
K-J	0 / 23	-38.5	-38.5	0.03 (3)	10.00
J-Q	0 / 33	-38.5	-38.5	0.03 (2)	10.00
Q-H	0 / 33	-38.5	-38.5	0.01 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-E	-121 / 0	0.03 (1)
M-D	-195 / 0	0.03 (1)
K-F	-195 / 0	0.03 (1)
J-G	-145 / 0	0.02 (1)
N-C	-145 / 0	0.02 (1)
O-P	0 / 25	0.00 (1)
Q-R	0 / 25	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-E:1) , BC=0.03/1.00 (J-K:3) , WB=0.03/1.00 (F-K:1) , SSI=0.07/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

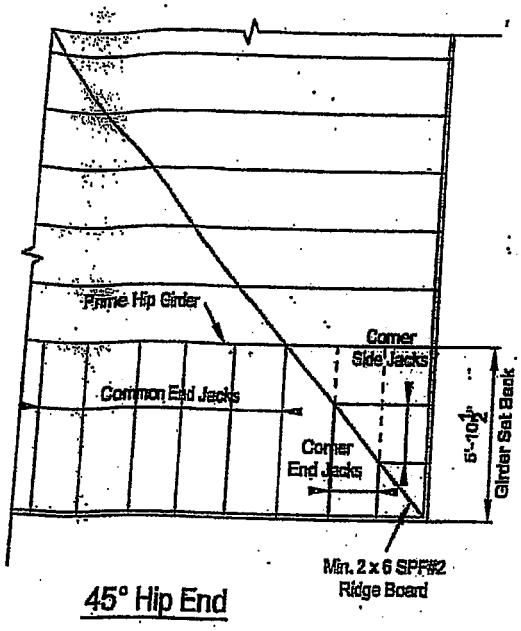
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

DWG NO. TAM 19 00127
STRUCTURAL
CONSENT ONLY



45° Hip End

LUMBER SPECIFICATION

TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

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06/11/18
H.J.G. ALVES
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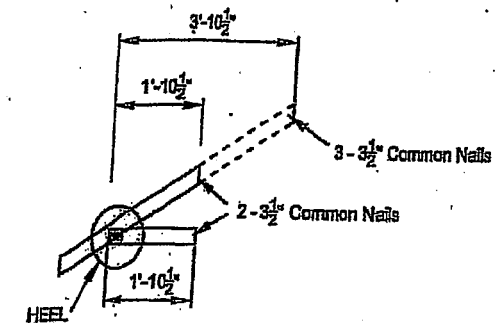
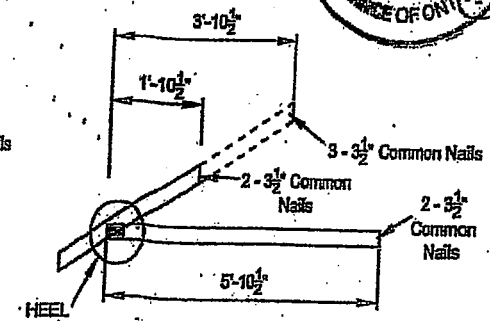
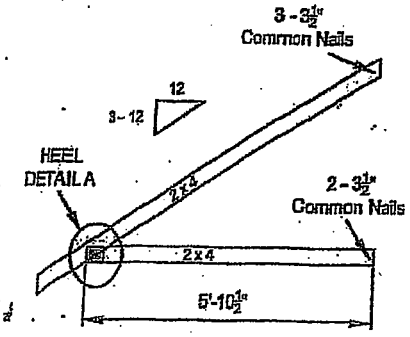


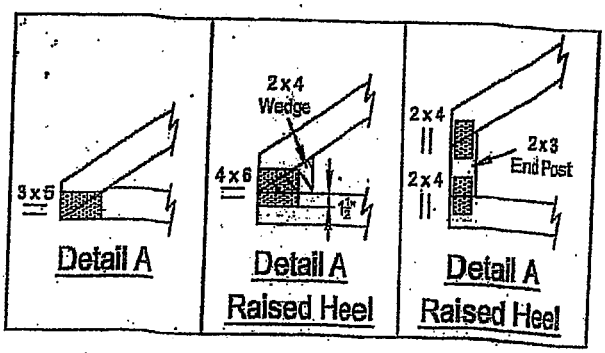
Corner Side Jacks



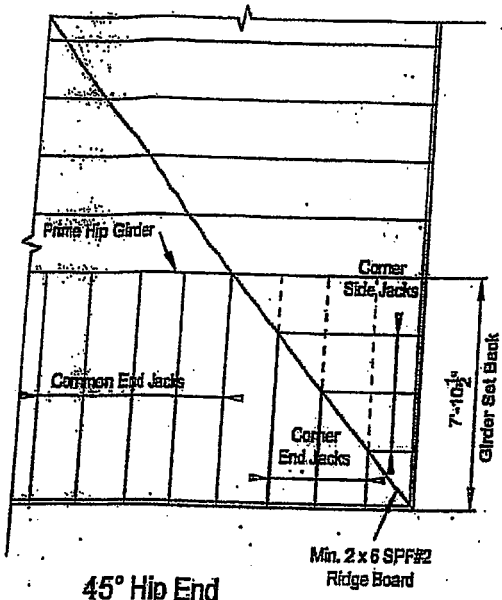
Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)



45° Hip End

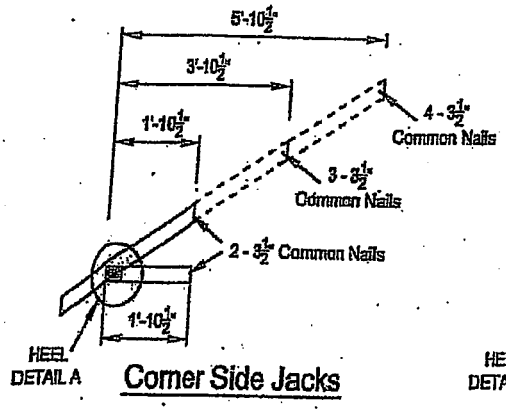
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

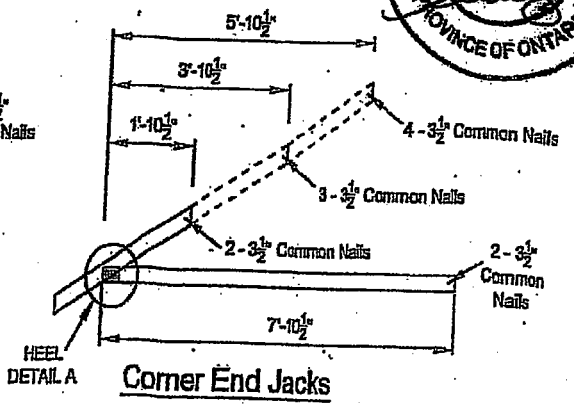
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F.

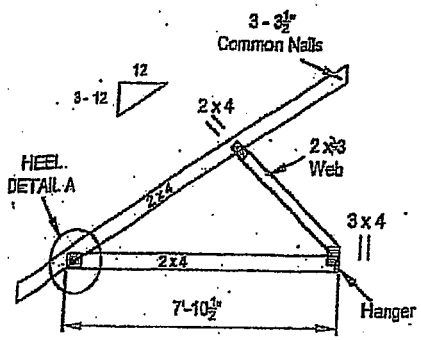
LICENSED PROFESSIONAL ENGINEER
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



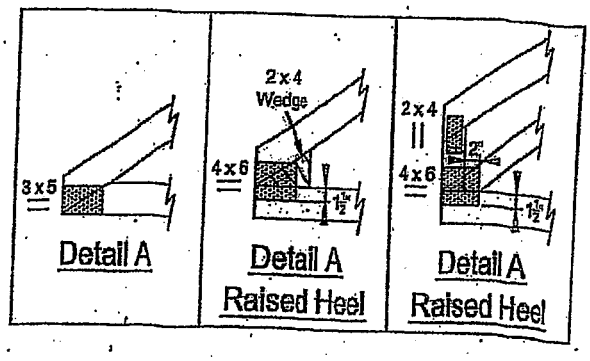
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A Raised Heel

Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

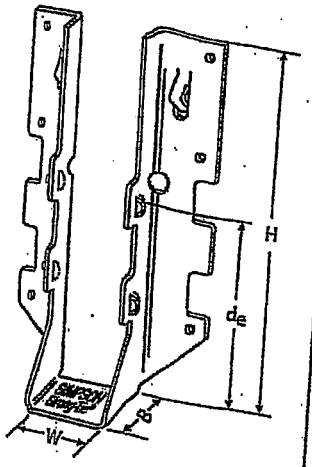
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

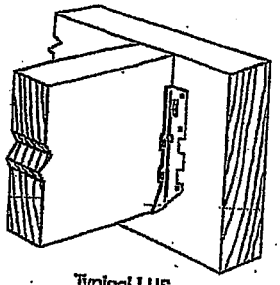
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

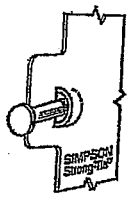


LUS28

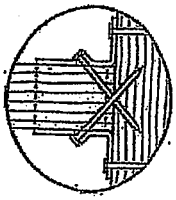

Typical LUS Installation

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⅜	3⅛	1¾	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⅜	4¾	1¾	3⅝	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4⅞	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⅜	7 13/16	1¾	3⅝	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8 9/16	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/5/2017
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

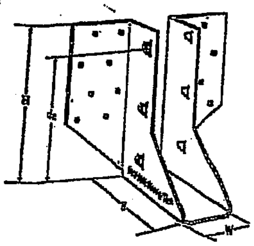
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

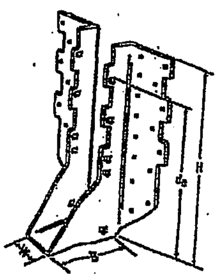
Installation:

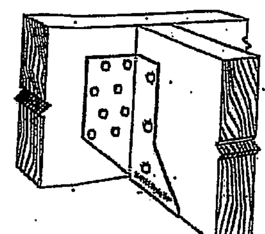
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

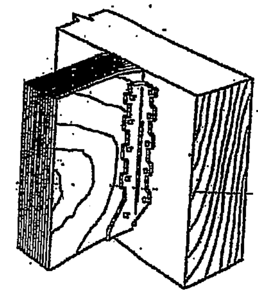
Options:

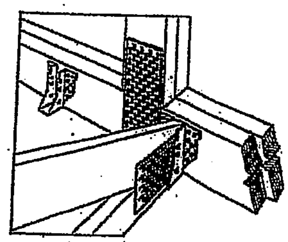
- See current catalogue for options


LJS26DS


HUS210
(HUS26, HUS28, similar)

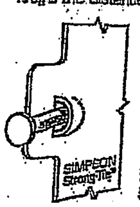

Typical LJS26DS Installation


Typical HUS Installation

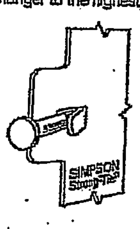

Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

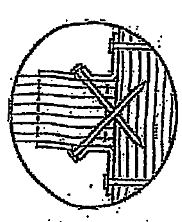
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7⁹⁄₃₂	3	6³⁄₃₂	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9⁵⁄₃₂	3	7³¹⁄₃₂	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/3/2017
B. D. BUNTING
PROVINCE OF ONTARIO

TECHNICAL BULLETIN

HGUS - Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

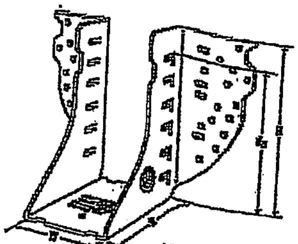
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

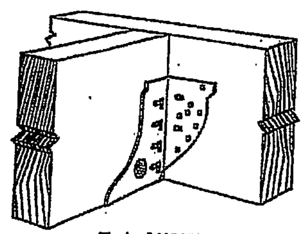
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

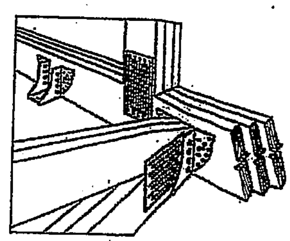
- See current catalogue for options



HGUS28-2



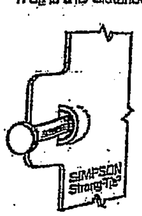
Typical HGUS Installation



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

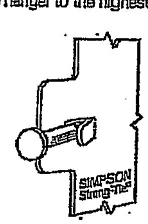
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_a[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8¾	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_a is the distance from the seat of the hanger to the highest joist nail.

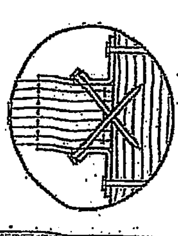


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

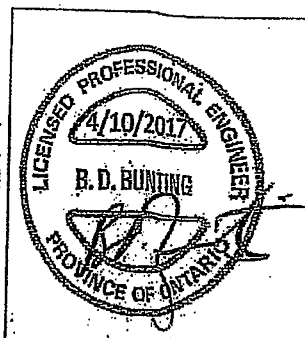
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.

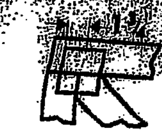


Double Shear Nailing Top View.

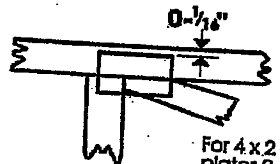


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4 — The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

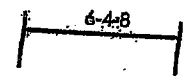
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

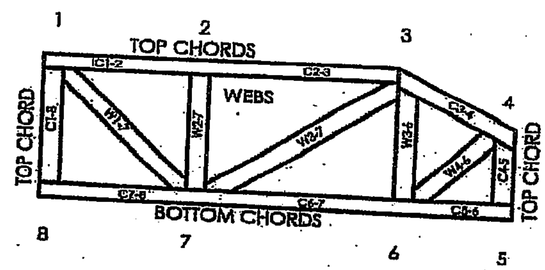
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek® POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

H – Seismic and Hurricane Ties

SIMPSON Strong-Tie

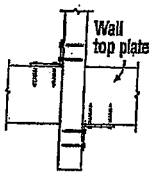
The H connector series provides wind and seismic ties for trusses and rafters.
Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

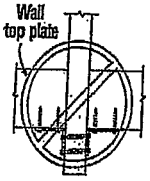
Installation: • Use all specified fasteners
• Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
• H1 can be installed with flanges facing outwards
• Hurricane ties do not replace solid blocking

Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

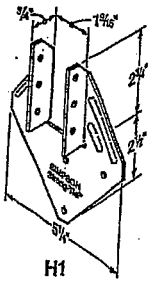
Hurricane Tie Installations to Achieve Twice the Load (Top View)



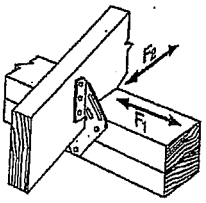
Install diagonally across from each other for minimum 2x truss.



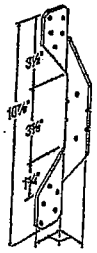
Nailing into both sides of a single ply 2x truss may cause the wood to split.



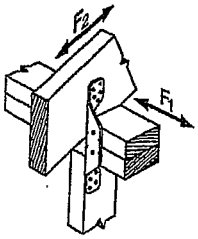
H1



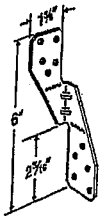
H1 Installation



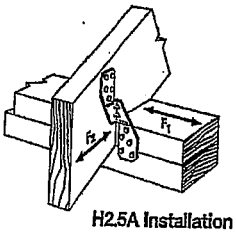
H2A



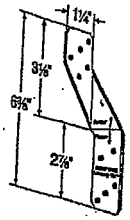
H2A Installation



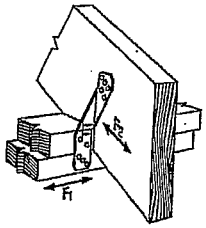
H2.5A



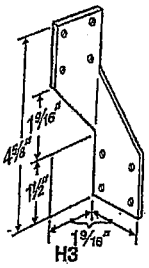
H2.5A Installation



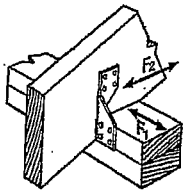
H2.5T



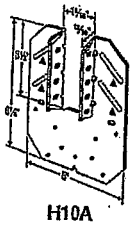
H2.5T Installation
(Nails into both top plates)



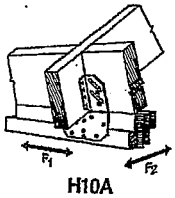
H3



H3 Installation



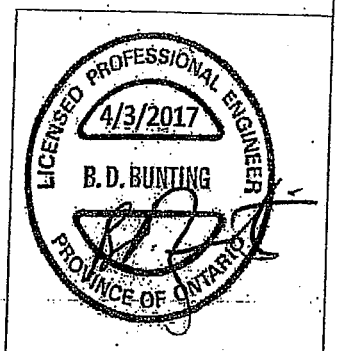
H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
		To Rafter	To Plates	To Studs		F_1	F_2		F_1	F_2
					(K_D=1.15)			(K_D=1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



TECHNICAL BULLETIN

CP Crush Plates – Bearing Enhancers

SIMPSON Strong-Tie®

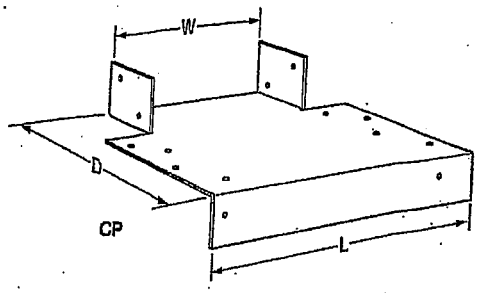
The CP transfers load from the truss or girder to plates for bearing limited conditions. Replaces nail-on scabs or in some cases, an additional ply when needed for bearing.

Material: See table

Finish: G90 galvanized

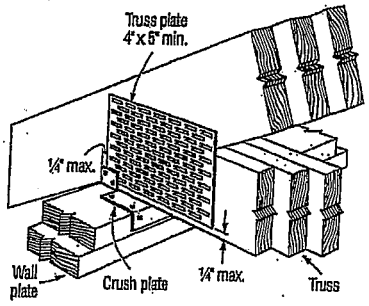
Design:

- Factored resistances are in accordance with CSA O86-14 assuming Q_r/A_b and Q'_r/A'_b = 812 psi for D.Fir-L and 615 psi for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 when compression loads are applied to both sides of truss chord members at bearing locations.

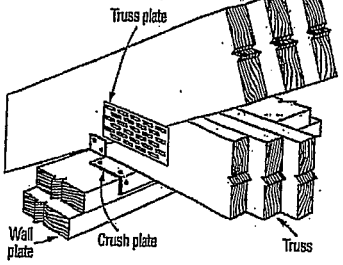


Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- For Case 1, truss plates must be located a maximum of ¼" from the underside of the truss chord and a maximum of ¼" from the edge of the wall plates in accordance with the reinforcing requirements of 7.5.9 TPIC 2014.



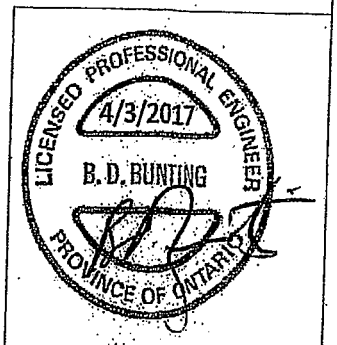
Case 1



Case 2

Model No.	Ga.	Wall Plate	Dimensions (in.) W	D	L	Fasteners Wall Plate	Truss	Factored Resistance (lb.) D.Fir-L Uplift (K_D=1.15)	D.Fir-L Bearing (K_D=1.00)	S-P-F Uplift (K_D=1.15)	S-P-F Bearing (K_D=1.00)
CASE 1 (Truss Plate Reinforcement)											
CP1-4	20	2x4	1⅝	3½	4½	(6) 10d	(4) 10d x 1½"	225	5965	225	4515
CP2-4	16		3¼	3½	5¾	(6) 10d	(4) 10d x 1½"	225	11390	225	9030
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	17895	225	13545
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	23860	225	18065
CP1-6	20	2x6	1⅝	5½	4½	(10) 10d	(4) 10d x 1½"	225	9370	225	7095
CP2-6	16		3¼	5½	5¾	(10) 10d	(4) 10d x 1½"	225	18740	225	14190
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	28110	225	21285
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	37495	225	28390
CASE 2 (No Reinforcement)											
CP1-4	20	2x4	1⅝	3½	4½	(6) 10d	(4) 10d x 1½"	225	4685	225	3550
CP2-4	16		3¼	3½	5¾	(6) 10d	(4) 10d x 1½"	225	9370	225	7100
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	14055	225	10650
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	18750	225	14195
CP1-6	20	2x6	1⅝	5½	4½	(10) 10d	(4) 10d x 1½"	225	7365	225	5575
CP2-6	16		3¼	5½	5¾	(10) 10d	(4) 10d x 1½"	225	14730	225	11150
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	22095	225	16725
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	29460	225	22305

1. Factored bearing resistances assume wall plate and truss are the same species. For a mixed species system use S-P-F values.



TS/LTS/MTS/HTS *Twist Straps*

SIMPSON Strong-Tie®

Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3" bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

MATERIAL: LTS–18 gauge; MTS–16 gauge; HTS–14 gauge; TS–16 gauge

FINISH: Galvanized. Some products available in stainless steel and ZMAX®; see Corrosion Information, pages 14-17.

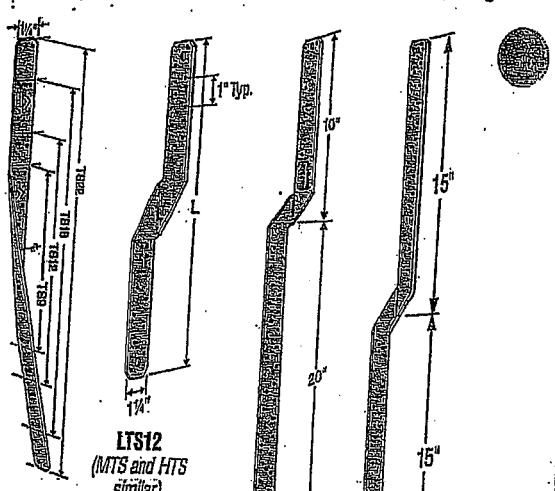
INSTALLATION: Use all specified fasteners. See General Notes.

- TS should be installed in pairs to reduce eccentricity.
- When LTS/MTS is installed as Truss-to-Top Plate tie, install 3–10dx1½" nails to the u/s of the plate and 3–10dx1½" nails into the edge of the double top plate.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Model No.	L (in)	Fasteners (Total)	Factored Resistance (Ka = 1.15) D.Fir-L lbs	Factored Resistance (Ka = 1.15) S-P-F lbs
			kN	kN
TS9	9	8-16d	1125	1040
			5.00	4.63
TS12	11¾	10-16d	1410	1300
			6.27	5.78
TS18	17¾	14-16d	1970	1820
			8.76	8.10
TS22	21¾	18-16d	2125	2125
			9.45	9.45
LTS12	12	12-10dx1½	1015	720
			4.52	3.20
LTS16	16	12-10dx1½	1015	720
			4.52	3.20
LTS18	18	12-10dx1½	1015	720
			4.52	3.20
LTS20	20	12-10dx1½	1015	720
			4.52	3.20
MTS12	12	14-10dx1½	1570	1180
			6.98	5.25
MTS16	16	14-10dx1½	1570	1180
			6.98	5.25
MTS18	18	14-10dx1½	1570	1180
			6.98	5.25
MTS20	20	14-10dx1½	1570	1180
			6.98	5.25
MTS30	30	14-10dx1½	1570	1180
			6.98	5.25
MTS24C	24	14-10dx1½	1570	1180
			6.98	5.25
MTS30C	30	14-10dx1½	1570	1180
			6.98	5.25
HTS16	16	16-10dx1½	2050	1455
			9.12	6.47
HTS20	20	24-10dx1½	2050	1455
			9.12	6.47
HTS24	24	24-10dx1½	2050	1455
			9.12	6.47
HTS28	28	24-10dx1½	2050	1455
			9.12	6.47

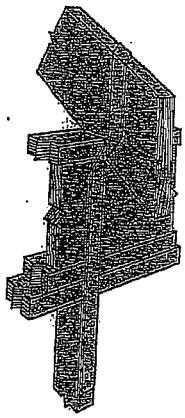


TS

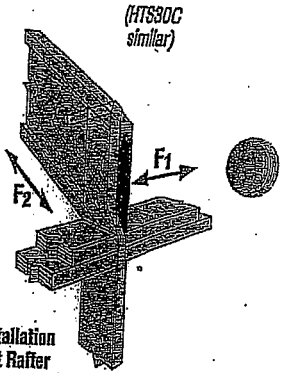
LTS12 *(MTS and HTS similar)*

MTS30 *(HTS30 similar)*

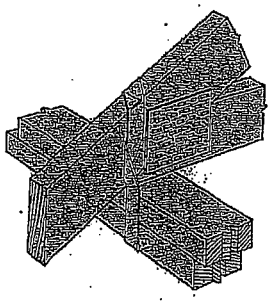
MTS30C *(HTS30C similar)*



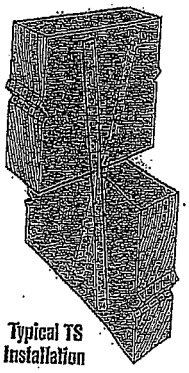
Typical MTS30 Installation



MTS30 Installation with I-Joist Rafter



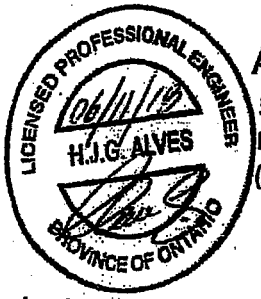
LTS/MTS Installation as a Truss-to-Top Plate Tie *(See note 7)*



Typical TS Installation

1. LTS12 thru LTS20, MTS16 through MTS30, HTS24 through HTS30C *(except HTS30)* have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30

Catalogue C-C-CAN2015 © 2015 SIMPSON STRONG-TIE COMPANY INC.
Catalogue C-C-CAM2015 © 2015 SIMPSON STRONG-TIE COMPANY INC.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information			Application number:
Building number, street name			Unit no. UNITS 1 – 6 / Lot/con. BLOCK 5
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description	

B. Individual who reviews and takes responsibility for design activities			
Name SAM KATSOULAKOS		Firm MICRO CITY ENGINEERING SERVICES INC.	
Street address R.R #1, PO BOX 61		Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750		Cell number

C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]

☐ House	☐ HVAC – House	☒ Building Structural
☐ Small Buildings	☐ Building Services	☐ Plumbing – House
☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing – All Buildings
☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems

Description of designer's work **ROYAL PINE HOMES – FORESTSIDE PH6 – BLOCK 5 – UNITS 1 - 6**
(SCHEDULE IS ISSUED AS LOT SPECIFIC)
REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM1143-19 DATED 1-18-19).
SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFED BY QUALIFIED BUILDING DESIGNER.

D. Declaration of Designer

I, **SAM KATSOULAKOS** declare that (choose one as appropriate):
(print name)

☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4.of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.

Individual BCIN: **26064**

Firm BCIN: **29991**

☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5.of Division C, of the Building Code.
Individual BCIN: _____

Basis for exemption from registration: _____

☐ The design work is exempt from the registration and qualification requirements of the Building Code.
Basis for exemption from registration and qualification: _____

I certify that:

1. The information contained in this schedule is true to the best of my knowledge.
2. I have submitted this application with the knowledge and consent of the firm.

Date 1-18-19 Signature of Designer

NOTE:

1. For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d).of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
2. Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG #TAM 1143 -19-S
DWG #TAM 1144 -19-S
1-18-19