

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS24 -(O)
LJS26DS -(V)

H2.5A (2) -(@)

TS22 (2) -(SS)
CP4-6 -(C4)
H8 (2) -(HH)

DENOTE:

C- 10/12 CATH. CEILING

T-180485

DENOTES:
CONVENTIONAL
FRAMING

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OR PART4 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

GROUND SNOW LOAD $S_s = 1.3kPa$
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

UNIT1804 END
A -LOT1
402590

UNIT1801
A (REV)-LOT2
402591

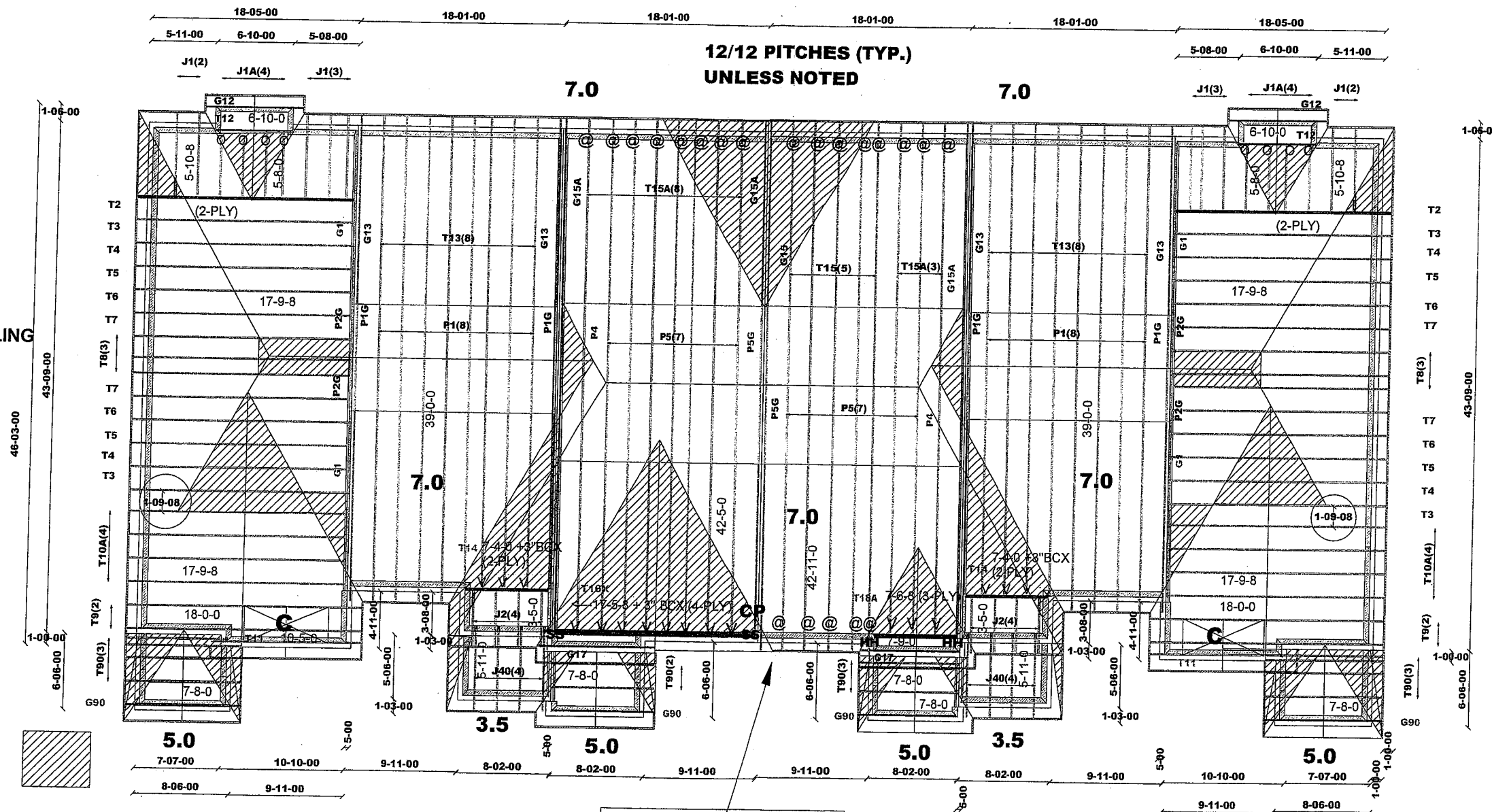
UNIT1803
A1-LOT3
402592

UNIT1803
A (REV)-LOT4
402593

UNIT1802
A -LOT5
402591

UNIT1804 END
A(REV) -LOT6
402590

109-02-00





DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE PH. 6
 Location: BRAMPTON
 Model: UNIT 1804 END
 Lot #:
 Elevation: EL:A LOT1 OR LOT6

Job Track: 50530
 PlanLog: 200908
 Layout ID: 402590
 Ref #
 Page: 1 of 2
 Date: 04/22/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	G1 GABLE	7 /12	19-06-00	10-00-00	2 x 4		1-03-07 10-00-00	221.47 140.00		
	1 2-ply	T2 Half Hip Girder	12 /12	17-09-08	4-08-09	2 x 4 2 x 6	1-03-08	1-10-08 4-08-09	199 125.33		
	2	T3 Half Hip	12 /12	17-09-08	5-10-09	2 x 4	1-03-08	1-10-08 5-10-09	156.36 98.00		
	2	T4 Half Hip	12 /12	17-09-08	7-00-09	2 x 4	1-03-08	1-10-08 7-00-09	166.94 104.67		
	2	T5 Half Hip	12 /12	17-09-08	8-02-09	2 x 4	1-03-08	1-10-08 8-02-09	185.34 117.33		
	2	T6 Half Hip	12 /12	17-09-08	9-04-09	2 x 4	1-03-08	1-10-08 9-04-09	210.83 132.00		
	2	T7 Half Hip	12 /12	17-09-08	10-06-09	2 x 4	1-03-08	1-10-08 10-06-09	208.65 127.67		
	3	T8 Half Hip	12 /12	17-09-08	11-08-09	2 x 4	1-03-08	1-10-08 11-08-09	350.76 216.00		
	2	T9 Roof Special	12 /12 10 /12	18-00-00	10-10-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	204.82 129.33		
	4	T10A Common	12 /12	17-09-08	10-10-08	2 x 4	1-03-08	1-10-08 2-01-00	381.53 238.67		
	1	T11 Roof Special	12 /12 10 /12	10-05-00	7-01-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	55.81 36.33		
	1	T12 Common Girder	12 /12	6-10-00	4-10-08	2 x 4	10-08 10-08	1-05-08 1-05-08	33.29 23.17		
	1	G12 GABLE	12 /12	6-10-00	4-10-08	2 x 4	10-08 10-08	1-05-08 1-05-08	34.17 24.17		
	3	T90 Common	5 /12	7-08-00	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	73.47 47.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE PH.6
 Location: BRAMPTON
 Model: UNIT 1804 END
 Lot #:
 Elevation: EL:A LOT1 OR LOT6

Job Track: 50530
 PlanLog: 200908
 Layout ID: 402590
 Ref #
 Page: 2 of 2
 Date: 04/22/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G90 GABLE	5 /12	7-08-00	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	25.63 16.67		
	2	P2G GABLE	7 /12	4-06-12	2-07-15	2 x 4		2-07-15	24.91 16.00		
	5	J1 Jack-Open	7 /12	5-10-08	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	92.39 58.33		
	4	J1A Jack-Open	7 /12	5-08-00	4-08-09	2 x 4		1-04-14 4-08-09	65.83 41.33		

TOTAL # TRUSS= 41

TOTAL BFT OF ALL TRUSSES= 1692

BFT. TOTAL WEIGHT OF ALL TRSSES 2691.2 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 4

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE PH.6
 Location: BRAMPTON
 Model: UNIT1801
 Lot #:
 Elevation: EL:A- LOT2 OR LOT5

Job Track: 50530
 PlanLog: 200908
 Layout ID: 402591
 Ref #
 Page: 1 of 1
 Date: 04/22/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	8	T13 Piggyback Base	7 /12	39-00-00	10-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	1640.08 1016.00		
	2	G13 GABLE	7 /12	39-00-00	10-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-03-07 1-03-07	453.81 284.67		
	1 2-ply	T14 Half Hip Girder	12 /12	7-04-00	3-03-06	2 x 4 2 x 6		1-10-08 3-03-06	89.69 59.33		
	8	P1 Piggyback	7 /12	9-01-08	2-07-15	2 x 4			176.91 114.67		
	2	P1G GABLE	7 /12	9-01-08	2-07-15	2 x 4			46.84 30.67		
	4	J2 Jack-Open	7 /12	3-05-00	3-03-06	2 x 4	1-03-08	1-03-07 3-03-06	49.72 33.33		
	4	J40 Monopitch	3.5 /12	5-11-00	2-04-14	2 x 4	1-03-08	3-14 2-03-09	61.62 37.33		

TOTAL # TRUSS= 30

TOTAL BFT OF ALL TRUSSES= 1576

BFT. TOTAL WEIGHT OF ALL TRSSES 2518.67 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 3

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FOREST SIDE PH.6
 Location: BRAMPTON
 Model: UNIT1803
 Lot #:
 Elevation: EL:A1 LOT3

Job Track: 50530
 PlanLog: 200908
 Layout ID: 402592
 Ref #
 Page: 1 of 1
 Date: 04/22/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	8	T15A Piggyback Base	7 /12	42-05-00	9-09-08	2 x 4 2 x 6	1-03-08	1-03-07 1-01-11	1930.78 1173.33		
	2	G15A GABLE	7 /12	42-05-00	9-09-08	2 x 4 2 x 6	1-03-08	1-03-07 1-01-11	516.53 315.00		
	1 4-ply	T16X Common Girder	12 /12	17-05-08	10-11-00	2 x 6	3-00	2-03-08 2-01-00	543.63 340.00		
	2	T90 Common	5 /12	7-08-00	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	48.98 31.33		
	1	G90 GABLE	5 /12	7-08-00	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	25.63 16.67		
	1	P4 Piggyback	7 /12	13-06-01	2-00-00	2 x 4			37.43 23.50		
	7	P5 Piggyback	7 /12	13-06-01	3-11-04	2 x 4			237.53 144.67		
	1	P5G GABLE	7 /12	13-06-01	3-11-04	2 x 4			38.81 24.67		

TOTAL # TRUSS= 26

TOTAL BFT OF ALL TRUSSES= 2069.17

BFT.

TOTAL WEIGHT OF ALL TRSSES 3379.32 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	TS22	
8	Hardware	LJS26DS	
16	Hardware	H2.5A	
1	Hardware	CP4-6	

TOTAL NUMBER OF ITEMS= 29

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: ROYAL PINE HOMES
 Project: FORESTSIDE PH. 6
 Location: BRAMPTON
 Model: UNIT1803
 Lot #:
 Elevation: EL:A LOT4

Job Track: 50530
 PlanLog: 200908
 Layout ID: 402593
 Ref #
 Page: 1 of 1
 Date: 04/22/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T15 Piggyback Base	7 /12	42-11-00	9-09-08	2 x 4 2 x 6	1-03-08 1-00-08	1-03-07 1-01-11	1221.12 743.33		
	1	G15 GABLE	7 /12	42-11-00	9-09-08	2 x 4 2 x 6	1-03-08 1-00-08	1-03-07 1-01-11	260.96 159.33		
	3	T15A Piggyback Base	7 /12	42-05-00	9-09-08	2 x 4 2 x 6	1-03-08	1-03-07 1-01-11	724.04 440.00		
	1	G15A GABLE	7 /12	42-05-00	9-09-08	2 x 4 2 x 6	1-03-08	1-03-07 1-01-11	258.26 157.50		
	1	G17 GABLE	12 /12	7-09-00	5-09-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	42.13 28.67		
	1 3-ply	T18A Common Girder	12 /12	7-06-08	5-09-00	2 x 4 2 x 6	1-03-08	1-10-08 2-01-00	143 90.00		
	3	T90 Common	5 /12	7-08-00	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	73.47 47.00		
	1	G90 GABLE	5 /12	7-08-00	2-05-07	2 x 4	1-03-08 1-03-08	6-00 6-00	25.63 16.67		
	1	P4 Piggyback	7 /12	13-06-01	2-00-00	2 x 4			37.43 23.50		
	7	P5 Piggyback	7 /12	13-06-01	3-11-04	2 x 4			237.53 144.67		
	1	P5G GABLE	7 /12	13-06-01	3-11-04	2 x 4			38.81 24.67		

TOTAL # TRUSS= 27

TOTAL BFT OF ALL TRUSSES= 1875.34

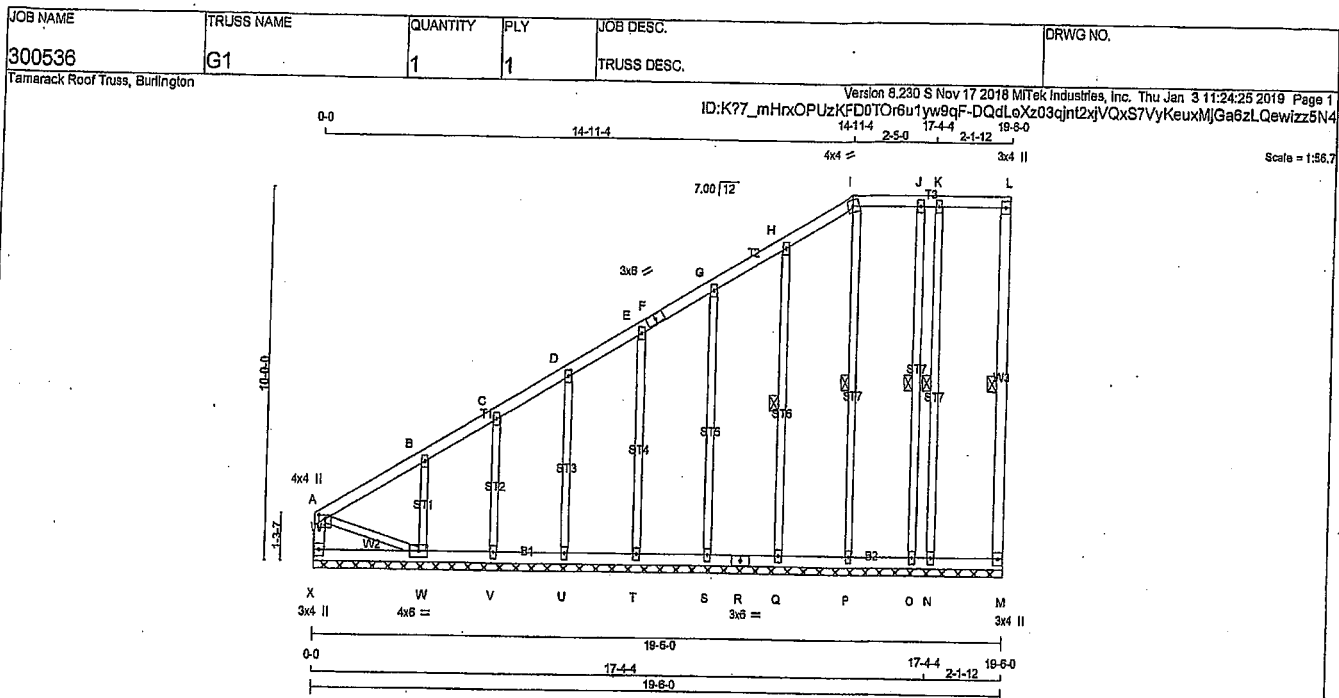
BFT.

TOTAL WEIGHT OF ALL TRSSES 3062.38 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
4	Hardware	H8	
26	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 33



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
M - L	2x4	DRY	No.2
X - A	2x4	DRY	No.2
R - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMW+p	MT20	4.0	4.0	1.25	2.00
B, C, D, E, G, H, J, K					
B TMW+w	MT20	2.0	4.0		
F TS-t	MT20	3.0	6.0		
I TTW-m	MT20	4.0	4.0		
L TMV+p	MT20	3.0	4.0		
M BMV1+p	MT20	3.0	4.0		
N, O, P, Q, S, T, U, V					
N BMV1+w	MT20	2.0	4.0		
R BS-t	MT20	3.0	6.0		
W BMVW1-t	MT20	4.0	6.0		
X BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-M, K-N, H-Q, I-P, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LENGTH (LC)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LENGTH (LC)	MAX. FACTORED LENGTH (LC)
FR-TO						
A-B	-10/0	-84.9	0.09 (1)	6.25	N-K	-121/0
B-C	-26/0	-84.9	0.09 (1)	6.25	W-B	-254/0
C-D	-8/0	-84.9	0.04 (1)	10.00	V-C	-139/0
D-E	-8/0	-84.9	0.04 (1)	10.00	U-D	-174/0
E-F	-6/0	-84.9	0.04 (1)	10.00	T-E	-168/0
F-G	-6/0	-84.9	0.04 (1)	10.00	S-G	-168/0
G-H	-2/0	-84.9	0.04 (1)	10.00	Q-H	-187/0
H-I	-10/0	-84.9	0.04 (1)	6.25	P-I	-136/0
I-J	0/0	-84.9	0.04 (1)	10.00	O-J	-125/0
J-K	0/0	-84.9	0.04 (1)	10.00	A-W	0/20
K-L	0/0	-84.9	0.04 (1)	10.00		
M-L	-72/0	0.0	0.03 (1)	6.25		
X-A	-120/0	0.0	0.01 (1)	7.81		
X-W	0/0	-38.5	0.06 (3)	10.00		
W-V	0/12	-38.5	0.06 (3)	10.00		
V-U	0/9	-38.5	0.03 (3)	10.00		
U-T	0/7	-38.5	0.03 (3)	10.00		
T-S	0/5	-38.5	0.02 (3)	10.00		
S-R	0/3	-38.5	0.02 (3)	10.00		
R-Q	0/3	-38.5	0.02 (3)	10.00		
Q-P	0/1	-38.5	0.02 (3)	10.00		
P-O	0/0	-38.5	0.02 (3)	10.00		
O-N	0/0	-38.5	0.03 (3)	10.00		
N-M	0/0	-38.5	0.03 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (B-C:1), BC=0.06/1.00 (V-W:3), WB=0.18/1.00 (G-S:1), SI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (I) (INPUT = 0.90)

JSI METAL= 0.12 (B) (INPUT = 1.00)

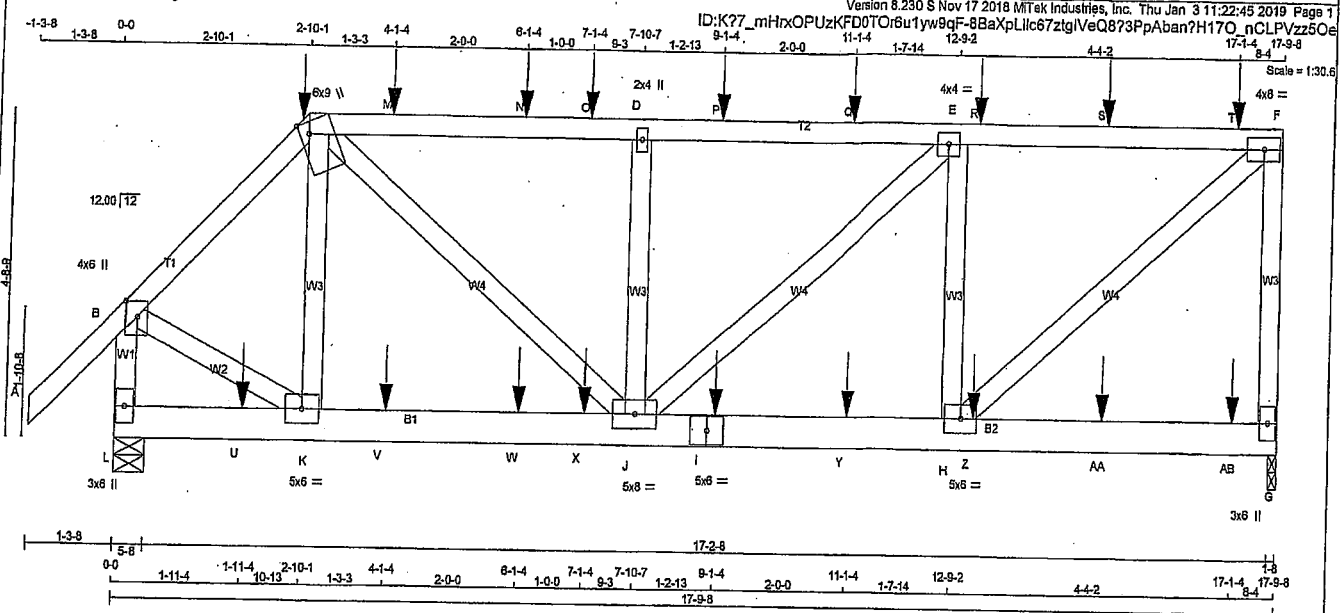
DRWG NO. TAM T1900034

STRUCTURAL

COMPONENT ONLY



JOB NAME 300536	TRUSS NAME T2	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
L - U	2x4	DRY	No.2
U - K	2x4	DRY	No.2
K - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - Y	2x4	DRY	No.2
Y - H	2x4	DRY	No.2
H - Z	2x4	DRY	No.2
Z - AA	2x4	DRY	No.2
AA - BB	2x4	DRY	No.2
BB - G	2x4	DRY	No.2

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE (61.0)
C-F	12	SIDE (61.0)
F-G	12	TOP
G-H	12	TOP
H-I	12	TOP
I-J	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-I	2	SIDE (0.0)
I-G	2	SIDE (163.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMW/p	MT20	4.0	6.0	2.75 2.00
C	TMW/m	MT20	6.0	9.0	Edge 1.75
D	TMW/h	MT20	2.0	4.0	
E	TMW/h	MT20	4.0	4.0	
F	TMW/h	MT20	4.0	6.0	
G	BMW/p	MT20	3.0	6.0	
H	BMW/h	MT20	5.0	6.0	
I	BS-t	MT20	5.0	6.0	
J	BMW/h	MT20	5.0	6.0	
K	BMW/h	MT20	5.0	6.0	
L	BMW/p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS	REACTION	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
G	2244	0	2244	0	0	1-8	1-8	1-8	1-8
L	2221	0	2221	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
G	1701	850 / 0	379 / 0	0 / 0	0 / 0
L	1676	852 / 0	355 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS (S), G, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	UNBRACED LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX	CSI (LC)
FR-TO						FR-TO			
A-B	0 / 42	-84.9	-84.9 0.07 (1)	10.00	K-C	-274 / 105	0.03 (1)		
B-C	-1854 / 0	-84.9	-84.9 0.08 (1)	6.25	C-J	0 / 1431	0.13 (1)		
C-M	-2381 / 0	-84.9	-84.9 0.37 (1)	5.35	J-D	-244 / 105	0.03 (1)		
M-N	-2381 / 0	-84.9	-84.9 0.37 (1)	5.35	E-F	0 / 1431	0.13 (1)		
N-O	-2381 / 0	-84.9	-84.9 0.37 (1)	5.35	H-I	-244 / 105	0.03 (1)		
O-D	-2381 / 0	-84.9	-84.9 0.37 (1)	5.35	F-E	0 / 1431	0.13 (1)		
D-P	-2381 / 0	-84.9	-84.9 0.36 (1)	5.35	I-H	-244 / 105	0.03 (1)		
P-Q	-2381 / 0	-84.9	-84.9 0.36 (1)	5.35	G-F	0 / 1431	0.13 (1)		
Q-E	-2381 / 0	-84.9	-84.9 0.36 (1)	5.35	J-I	-244 / 105	0.03 (1)		
E-R	-1958 / 0	-84.9	-84.9 0.32 (1)	5.8	H-G	0 / 1431	0.13 (1)		
R-S	-1958 / 0	-84.9	-84.9 0.32 (1)	5.8	I-H	-244 / 105	0.03 (1)		
S-T	-1958 / 0	-84.9	-84.9 0.32 (1)	5.8	G-F	0 / 1431	0.13 (1)		
T-F	-1958 / 0	-84.9	-84.9 0.32 (1)	5.8	J-I	-244 / 105	0.03 (1)		
G-F	-2065 / 0	0.0	0.0 0.35 (1)	7.6	L-B	-2185 / 0	0.0	0.0 0.13 (1)	7.53
L-B	-2185 / 0	0.0	0.0 0.13 (1)	7.53					

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	UNBRACED LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX	CSI (LC)
FR-TO						FR-TO			
L-U	0 / 0	-38.5	-38.5 0.07 (3)	10.00					
U-K	0 / 0	-38.5	-38.5 0.07 (3)	10.00					
K-V	0 / 1294	-38.5	-38.5 0.14 (2)	10.00					
V-W	0 / 1294	-38.5	-38.5 0.14 (2)	10.00					
W-X	0 / 1294	-38.5	-38.5 0.14 (2)	10.00					
X-J	0 / 1294	-38.5	-38.5 0.14 (2)	10.00					
J-I	0 / 1958	-38.5	-38.5 0.19 (2)	10.00					
I-Y	0 / 1958	-38.5	-38.5 0.19 (2)	10.00					
Y-H	0 / 1958	-38.5	-38.5 0.19 (2)	10.00					
H-Z	0 / 0	-38.5	-38.5 0.08 (3)	10.00					
Z-AA	0 / 0	-38.5	-38.5 0.08 (3)	10.00					
AA-BB	0 / 0	-38.5	-38.5 0.08 (3)	10.00					
BB-G	0 / 0	-38.5	-38.5 0.08 (3)	10.00					

FACTORED CONCENTRATED LOADS (LBS)									
LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	
2-10-1	-35	-39	---	FRONT	VERT	DEAD	---	---	---
2-10-1	-162	-162	---	FRONT	VERT	SNOW	---	---	---
9-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---	---
4-1-4	-164	-164	---	BACK	VERT	TOTAL	---	---	---
6-1-4	-156	-156	---	BACK	VERT	TOTAL	---	---	---
7-1-4	-156	-156	---	BACK	VERT	TOTAL	---	---	---
9-1-4	-156	-156	---	BACK	VERT	TOTAL	---	---	---
11-1-4	-156	-156	---	BACK	VERT	TOTAL	---	---	---

TOTAL WEIGHT = 2 X 99 = 198 lb

DESIGN CRITERIA
 *** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL) = L/360 (0.59")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.37/1.00 (C-D:1), BC=0.19/1.00 (H-I:2), WB=0.23/1.00 (F-H:1), SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP (DRY)	SHEAR	SECTION	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

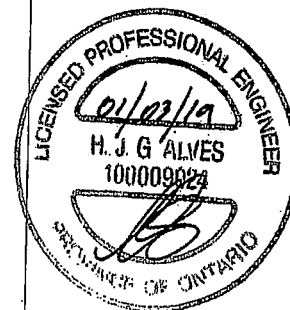
JSI GRIP= 0.77 (B) (INPUT = 0.90)
 JSI METAL= 0.35 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77 mHxOPUzKFD0TOR6u1yw9qF-8BaXpLlIc87ztglVeQ873PpAban7H170 nCLPVzz50e

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	13-1-4	-184	-184	—	BACK	VERT	TOTAL	—	—
S	15-1-4	-184	-184	—	BACK	VERT	TOTAL	—	—
T	17-1-4	-178	-178	—	BACK	VERT	TOTAL	—	—
U	1-11-4	-75	-95	—	BACK	VERT	TOTAL	—	—
V	4-1-4	-75	-95	—	BACK	VERT	TOTAL	—	—
W	6-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
X	7-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
Y	11-1-4	-71	-90	—	BACK	VERT	TOTAL	—	—
Z	13-1-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AA	15-1-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AB	17-1-4	-81	-103	—	BACK	VERT	TOTAL	—	—



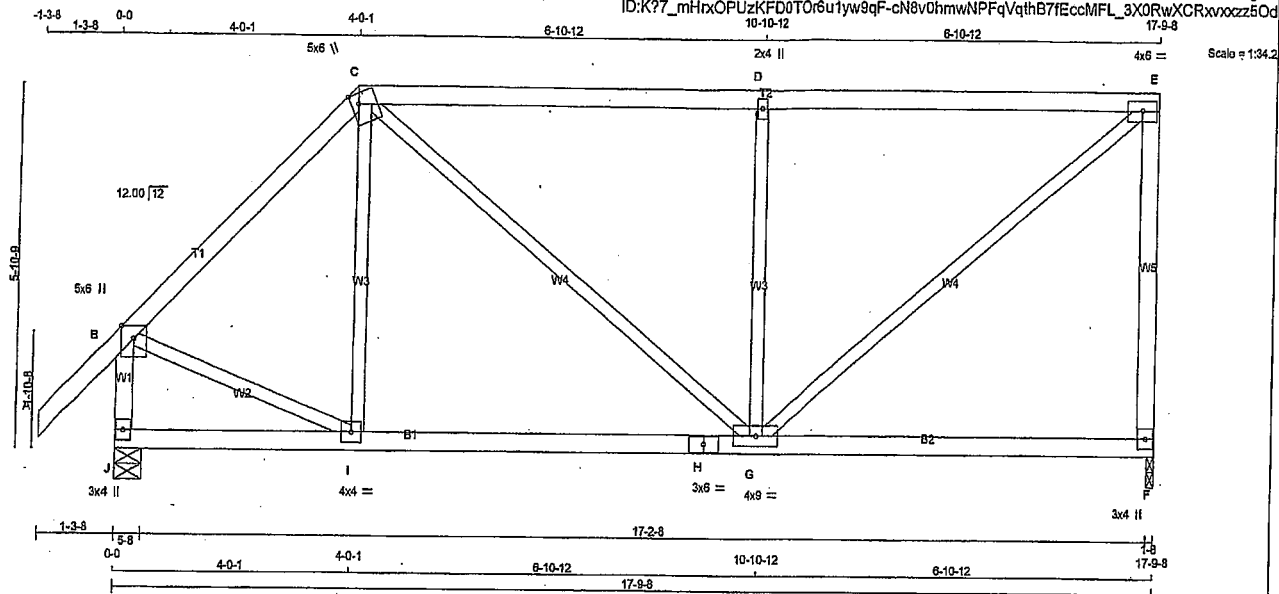
DWG NO. TAM T1900039
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHxOPUzKFD0T0r6u1yw9qF-cN8v0hmwNPFqVqthB7fEccMFL_3X0RwXCRxvxxzz5Od



TOTAL WEIGHT = 78 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
J - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	1.50
C TTVW+m	MT20	5.0	6.0	2.00	1.50
D TMVW+w	MT20	2.0	4.0		
E TMVW-t	MT20	4.0	6.0		
F BMV1+p	MT20	3.0	4.0		
G BMVWW-t	MT20	4.0	9.0		
H BS-t	MT20	3.0	6.0		
I BMVW-t	MT20	4.0	4.0		
J BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	1097	0	1097	0	1-8	1-8
J	1216	0	1216	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	LIVE			
F	832	414/0	187/0	0/0	0/0	231/0	0/0
J	914	479/0	187/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0/42	-84.9	-84.9	0.12 (1)	10.00	I-C	-13 / 154	0.04 (3)	
B-C	-800/0	-84.9	-84.9	0.28 (1)	8.18	C-G	0 / 410	0.09 (1)	
C-D	-852/0	-84.9	-84.9	0.75 (1)	5.02	G-D	-723 / 0	0.39 (1)	
D-E	-852/0	-84.9	-84.9	0.75 (1)	5.02	G-E	0 / 1223	0.28 (1)	
F-E	-891/0	0.0	0.0	0.64 (1)	7.81	B-I	0 / 678	0.15 (1)	
J-B	-1162/0	0.0	0.0	0.13 (1)	7.39				
J-I	0/0	-38.5	-38.5	0.19 (3)	10.00				
I-H	0/634	-38.5	-38.5	0.42 (2)	10.00				
H-G	0/634	-38.5	-38.5	0.42 (2)	10.00				
G-F	0/0	-38.5	-38.5	0.38 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 6.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.59")
CALCULATED VERT. DEFL. (LL) = $L/899$ (0.09")
ALLOWABLE DEFL. (TL) = $L/360$ (0.59")
CALCULATED VERT. DEFL. (TL) = $L/899$ (0.14")

CSI: TC=0.75/1.00 (D-E:1), BC=0.42/1.00 (G-I:2),
WB=0.39/1.00 (D-G:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

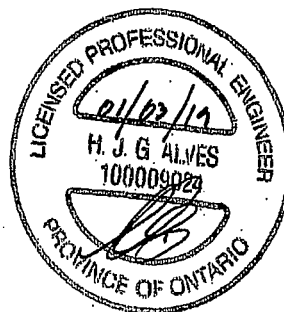
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (I) (INPUT = 0.90)
JSI METAL = 0.44 (I) (INPUT = 1.00)



DWG NO. TAM 71900040
STRUCTURAL
COMPONENT ONLY

amarack Roof Truss, Burlington

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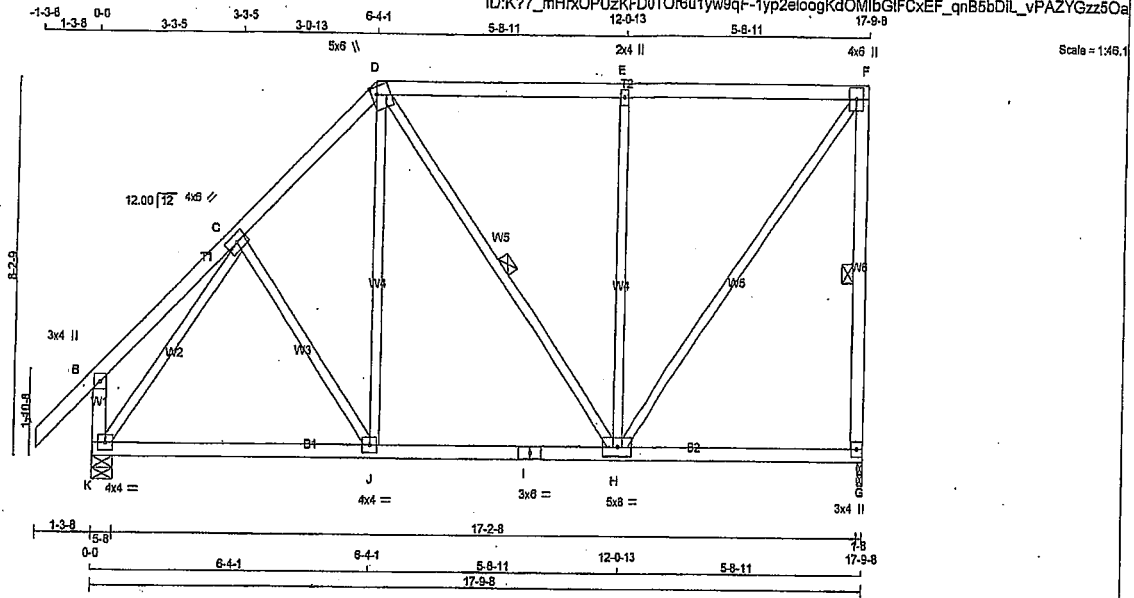


DWG NO. TAM 71900042
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T5	1	1	TRUSS DESC.	

Temarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 Mitek Industries, Inc. Thu Jan 3 11:22:49 2019 Page 1
ID:K77_mHrxOPuzKFD0r6u1yw9qF-1yp2eloogKdOMibGFCxEF_qnB5bDil_vPAZYGzz5Oa



Scale = 1/4"=1'-0"

TOTAL WEIGHT = 93 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
G - B	2x4	DRY No.2
K - I	2x4	DRY No.2
I - G	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	4.0	6.0		
D	TMVW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV+t+p	MT20	3.0	4.0		
H	BMVWVW+t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVWVW-t	MT20	4.0	4.0		
K	BMVWVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1087	0	1087	0
K	1216	0	1216	0

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
K	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO						FR-TO			
A-B	0 / 142	-84.9	-84.9	0.12 (1)	10.00	C-J	-29 / 57	0.02 (1)	
B-C	0 / 121	-84.9	-84.9	0.13 (1)	10.00	J-D	0 / 322	0.07 (2)	
C-D	-849 / 0	-84.9	-84.9	0.14 (1)	6.25	D-H	-24 / 16	0.02 (3)	
D-E	-598 / 0	-84.9	-84.9	0.48 (1)	6.25	H-E	-500 / 0	0.79 (1)	
E-F	-598 / 0	-84.9	-84.9	0.48 (1)	6.25	H-F	0 / 1013	0.23 (1)	
G-F	-1006 / 0	0.0	0.0	0.31 (1)	6.24	K-C	-1076 / 0	0.56 (1)	
K-B	-221 / 0	0.0	0.0	0.02 (1)	7.81				
K-J	0 / 600	-38.5	-38.5	0.34 (2)	10.00				
J-I	0 / 587	-38.5	-38.5	0.34 (2)	10.00				
I-H	0 / 587	-38.5	-38.5	0.34 (2)	10.00				
H-G	0 / 0	-38.5	-38.5	0.21 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.48/1.00 (E-F:1), BC=0.34/1.00 (H-J:2),
WB=0.79/1.00 (E-H:1), SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

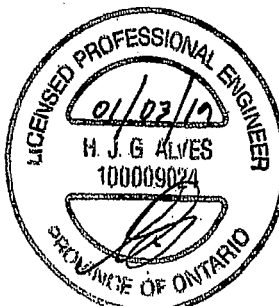
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	788	1937	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Degrees

JSI GRIP= 0.82 (K) (INPUT = 0.80)
JSI METAL= 0.27 (F) (INPUT = 1.00)



DRWG NO. TAM 11900044
STRUCTURAL
COMPONENT ONLY

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:50 2019 Page 1
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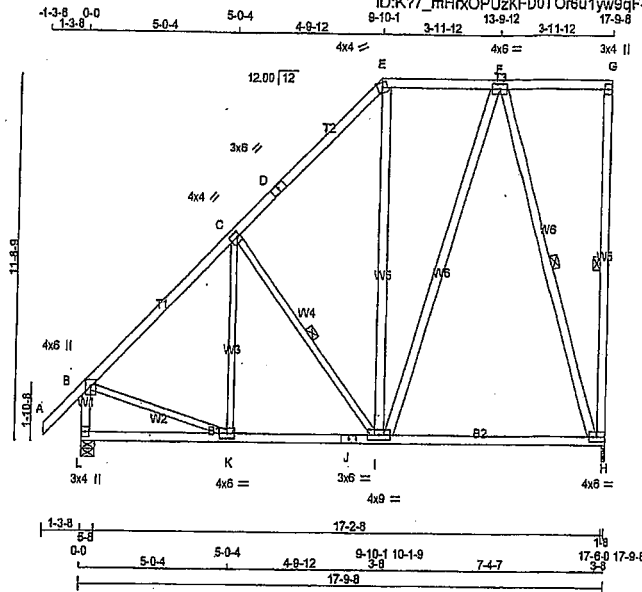
DRY: SEASONED LUMBER.

DWG NO. TAM **T1900045**
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:22:51 2019 Page 1
ID:K77_mHxOPuzKFD0TOr6u1yw9gF-zLxo3Qq2Byu6bbf_gFPJg3DJ7IAhdNGMjfgc8zz5OY



Scale = 1:87.7

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
K - C	2x3	DRY	No.2
C - I	2x3	DRY	No.2
B - K	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	6.0 2.75 2.00
C	TMVW+4	MT20	4.0	4.0 2.00 1.00
D	TS-t	MT20	3.0	6.0
E	TTW-m	MT20	4.0	4.0 Edge
F	TMVW+4	MT20	4.0	6.0
G	TMV+p	MT20	3.0	4.0
H	BMVW+1-t	MT20	4.0	6.0
I	BMVW+4-t	MT20	4.0	9.0
J	BS-t	MT20	3.0	6.0
K	BMVW+4-t	MT20	4.0	6.0
L	BMV+1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
H	1097	0	1097	0	1-8
L	1216	0	1216	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
L	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 42	K-C	-24 / 122
B-C	-887 / 0	C-I	-358 / 0
C-D	-657 / 0	I-E	0 / 195
D-E	-657 / 0	E-K	0 / 681
E-F	-439 / 0	F-H	0 / 457
F-G	0 / 0		
G-H	-128 / 0		
H-B	-1131 / 0		
L-K	0 / 0		
K-J	0 / 652		
J-I	0 / 652		
I-H	0 / 285		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/886 (0.24")

CSI: TC=0.29/1.00 (B-C:1), BC=0.46/1.00 (I-K:2), WB=0.76/1.00 (F-H:1), SSI=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

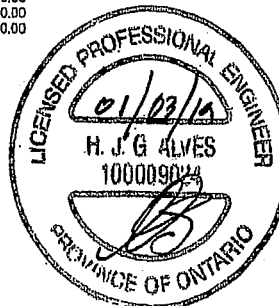
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (J) (INPUT = 0.90)
JSI METAL= 0.52 (J) (INPUT = 1.00)

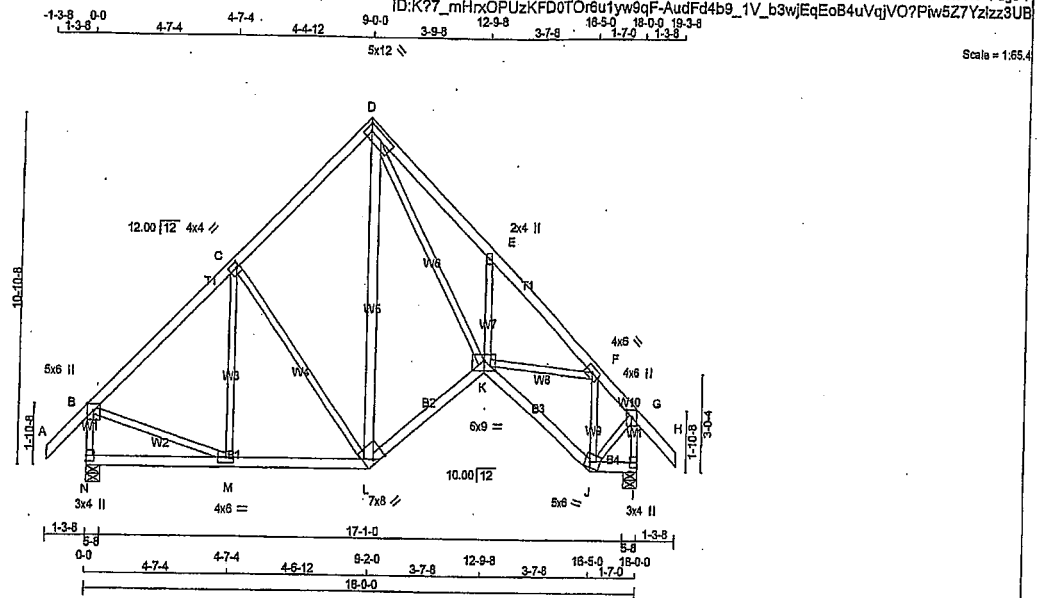


DRWG NO. TAM T1900047
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
259671	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 101 = 203 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - H	2x4 DRY	No.2	SPF		
N - B	2x3 DRY	No.2	SPF		
I - G	2x3 DRY	No.2	SPF		
N - L	2x4 DRY	No.2	SPF		
L - K	2x4 DRY	No.2	SPF		
K - J	2x4 DRY	No.2	SPF		
J - I	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT	L - D	2x4 DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-i	MT20	4.0	4.0	2.00	1.00
D	TTVW-i	MT20	5.0	12.0	2.25	2.00
E	TMVW+w	MT20	2.0	4.0		
F	TMVW-i	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	6.0	2.75	2.00
I	BMV1+p	MT20	3.0	4.0	2.00	Edge
J	BBVW-m	MT20	5.0	6.0	2.00	2.00
K	BBVW-w	MT20	6.0	9.0	Edge	4.50
L	BMVW-h	MT20	7.0	8.0	Edge	4.50
M	BMVW-i	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
N	1224	0	1224	0	0	5-8	5-8		
I	1233	0	1233	0	0	5-8	5-8		

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
N	921	482/0	189/0	0/0	0/0	250/0	0/0		
I	927	487/0	188/0	0/0	0/0	251/0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0/42	-84.9	-84.9 0.12 (1)	10.00	M-C	-51/163	0.04 (3)
B-C	-910/0	-84.9	-84.9 0.24 (1)	6.19	C-L	-338/0	0.39 (1)
C-D	-693/0	-84.9	-84.9 0.23 (1)	6.25	L-D	0/110	0.02 (3)
D-E	-1609/0	-84.9	-84.9 0.20 (1)	5.04	D-K	0/1451	0.33 (1)
E-F	-1608/0	-84.9	-84.9 0.15 (1)	5.10	K-E	-348/0	0.09 (1)
F-G	-744/0	-84.9	-84.9 0.08 (1)	6.25	K-F	0/689	0.15 (1)
G-H	0/42	-84.9	-84.9 0.12 (1)	10.00	J-F	-771/0	0.15 (1)
N-B	-1155/0	0.0	0.0 0.18 (1)	6.50	B-M	0/702	0.18 (1)
I-G	-1203/0	0.0	0.0 0.18 (1)	6.40	J-G	0/708	0.16 (1)
N-M	0/0	-38.5	-38.5 0.18 (3)	10.00			
M-L	0/666	-38.5	-38.5 0.28 (2)	10.00			
L-K	0/609	-38.5	-38.5 0.20 (2)	10.00			
K-J	0/598	-38.5	-38.5 0.20 (2)	10.00			
J-I	0/0	-38.5	-38.5 0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.60")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.60")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CSI: TC=0.24/1.00 (B-C:1), BC=0.28/1.00 (L-M:2), WB=0.39/1.00 (C-L:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

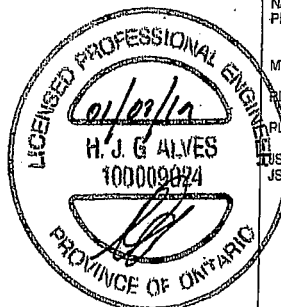
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.88 (D) (INPUT = 0.90)
JS METAL= 0.34 (B) (INPUT = 1.00)

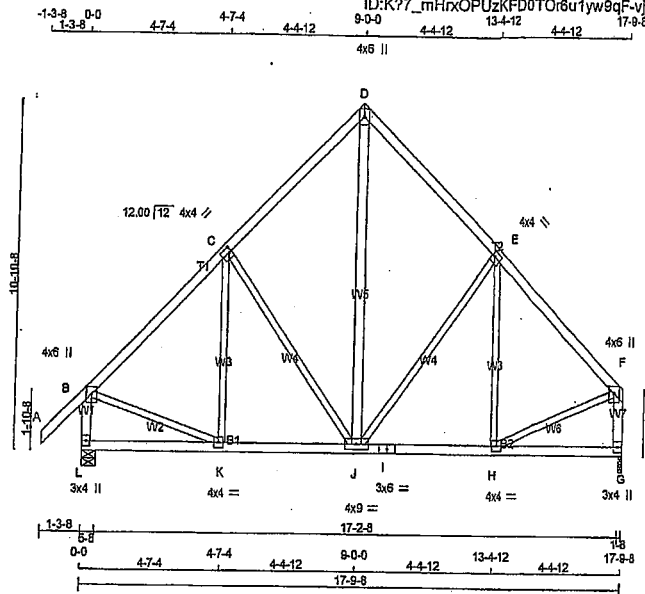


DRWG NO. TAM 17900082
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T10A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:85.7

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY No.2	SPF			
D - F	2x4 DRY No.2	SPF			
L - B	2x4 DRY No.2	SPF			
G - F	2x4 DRY No.2	SPF			
L - I	2x4 DRY No.2	SPF			
I - G	2x4 DRY No.2	SPF			

ALL WEBS	2x3 DRY No.2	SPF
EXCEPT		
J - D	2x4 DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW+p	MT20	4.0	6.0	2.75	2.00	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00	
D	TTW+p	MT20	4.0	6.0			
E	TMVW-t	MT20	4.0	4.0	2.00	1.00	
F	TMVW+p	MT20	4.0	6.0	2.75	2.00	
G	BMV1+p	MT20	3.0	4.0			
H	BMVW-t	MT20	4.0	4.0			
I	BS-t	MT20	3.0	6.0			
J	BMVWVW-t	MT20	4.0	9.0			
K	BMVW-t	MT20	4.0	4.0			
L	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	1216	0	1216	0
G	1097	0	1097	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE
L	914	479 / 0	187 / 0	0 / 0
G	832	414 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING	TOTAL LOAD CASES: (4)								
CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED					
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE	MAX.				
(LBS)	(PLF)	LC1	(LBS)	(LBS)	CSI (LC)				
FR-TO	FROM	TO	FR-TO	FROM	TO				
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	K-C	-22 / 138	-0.03 (3)	
B-C	-898 / 0	-84.9	-84.9	0.23 (1)	6.22	C-J	-323 / 0	0.37 (1)	
C-D	-698 / 0	-84.9	-84.9	0.23 (1)	6.25	J-D	0 / 656	0.11 (1)	
D-E	-695 / 0	-84.9	-84.9	0.21 (1)	6.25	J-E	-288 / 0	0.33 (1)	
E-F	-869 / 0	-84.9	-84.9	0.22 (1)	6.25	H-E	-61 / 114	0.04 (1)	
L-B	-1144 / 0	0.0	0.0	0.13 (1)	7.43	E-K	0 / 692	0.16 (1)	
G-F	-1029 / 0	0.0	0.0	0.12 (1)	7.74	H-F	0 / 684	0.15 (1)	
L-K	0 / 0	-38.5	-38.5	0.15 (3)	10.00				
K-J	0 / 657	-38.5	-38.5	0.23 (2)	10.00				
J-I	0 / 637	-38.5	-38.5	0.22 (2)	10.00				
I-H	0 / 637	-38.5	-38.5	0.22 (2)	10.00				
H-G	0 / 0	-38.5	-38.5	0.14 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.59")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.59")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.23/1.00 (B-C:1), BC=0.23/1.00 (J-K:2), WB=0.37/1.00 (C-J:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

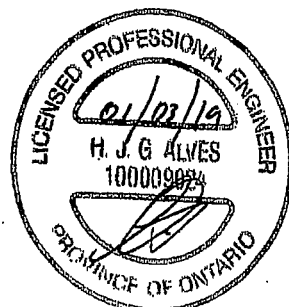
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)

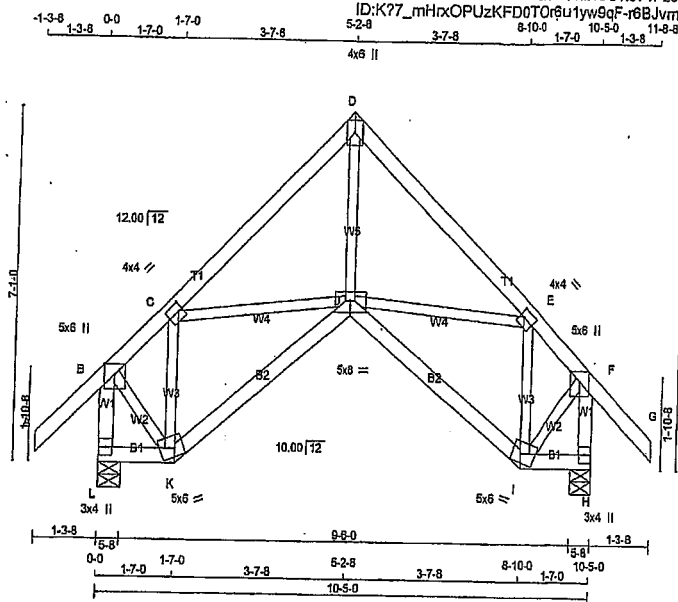


DRWG NO. TAM T19 00049
STRUCTURAL
COMPONENT ONLY

JOB NAME 300536	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 11:22:55 2019 Page 1
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Scale = 1/4\"/>

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C TMWW-t	MT20	4.0	4.0	2.00	1.00
D TTW+p	MT20	4.0	6.0		
E TMWW-t	MT20	4.0	4.0	2.00	1.00
F TMVW+p	MT20	5.0	6.0	Edge	
H BMV1+p	MT20	3.0	4.0		
I BBWW-m	MT20	5.0	6.0	2.00	2.25
J BBWW-p	MT20	5.0	6.0	2.75	4.00
K BBWW-m	MT20	5.0	6.0	2.00	2.25
L BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
L	761	0	761	0
H	761	0	761	0

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
L	569	308 / 0	109 / 0	0 / 0	152 / 0	0 / 0
H	569	308 / 0	109 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LGT MAX (LC)	MAX. UNBRACED LENGTH	FR-TO
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00
B-C	-402 / 0	-84.9	-84.9	0.11 (1)	8.25
C-D	-632 / 0	-84.9	-84.9	0.15 (1)	8.25
D-E	-632 / 0	-84.9	-84.9	0.15 (1)	8.25
E-F	-402 / 0	-84.9	-84.9	0.11 (1)	8.25
F-G	0 / 42	-84.9	-84.9	0.12 (1)	10.00
L-B	-730 / 0	0.0	0.0	0.08 (1)	7.81
H-F	-730 / 0	0.0	0.0	0.08 (1)	7.81
L-K	0 / 0	-38.5	-38.5	0.02 (3)	10.00
K-J	0 / 342	-38.5	-38.5	0.16 (2)	10.00
J-I	0 / 342	-38.5	-38.5	0.16 (2)	10.00
I-H	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	FR-TO
J-D	0 / 594	0.13 (1)	0.04 (1)
J-E	0 / 172	0.04 (1)	0.04 (1)
I-E	-398 / 0	0.04 (1)	0.04 (1)
C-J	0 / 172	0.04 (1)	0.04 (1)
K-C	-398 / 0	0.04 (1)	0.04 (1)
B-K	0 / 400	0.09 (1)	0.09 (1)
I-F	0 / 400	0.09 (1)	0.09 (1)

TOTAL WEIGHT = 56 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	23.3	PSF
BOT CH.	LL	PSF
LL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.15/1.00 (D-E:1), BC=0.16/1.00 (J-K:2), WB=0.13/1.00 (D-J:1), SS=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

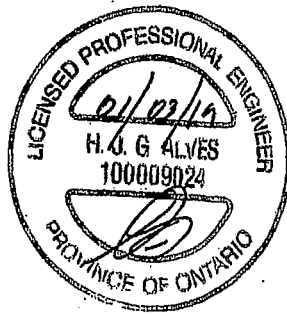
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	616	354	1697

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (I) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



DRWG NO. 1AM T1900057
STRUCTURAL
COMPONENT ONLY

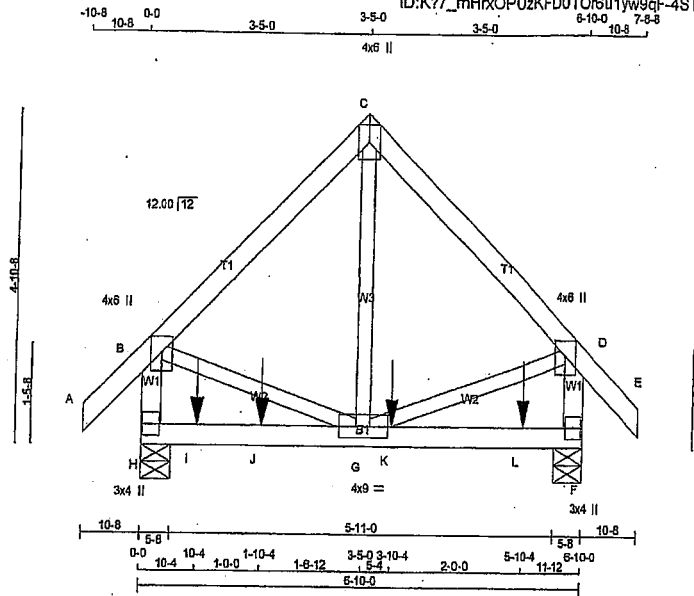
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHxOPUzKFD0T06u1yw9qF-4STHHDkjh1VJ_ohmxXcmHtCFJcpNF1jt79tzz51C

Scale = 1/31.0



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+p	MT20	4.0	6.0		
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWV+t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
H	VERT 1185	DOWN 1185	0	5-8
F	HORIZ 0	HORIZ 0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	DEAD	SOIL
H	COMBINED	SNOW 493/0	157/0	0/0
F		LIVE 444/0	142/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIM SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0/29	-84.9	-84.9	0.07 (1)	10.00	G-C	0/533 0.16 (1)
B-C	-653/0	-84.9	-84.9	0.20 (1)	6.25	B-G	0/485 0.12 (1)
C-D	-653/0	-84.9	-84.9	0.20 (1)	6.25	G-D	0/485 0.12 (1)
D-E	0/29	-84.9	-84.9	0.07 (1)	10.00		
H-B	-839/0	0.0	0.0	0.10 (1)	7.81		
F-D	-839/0	0.0	0.0	0.10 (1)	7.81		
H-I	0/0	-38.5	-38.5	0.49 (1)	10.00		
I-J	0/0	-38.5	-38.5	0.49 (1)	10.00		
J-K	0/0	-38.5	-38.5	0.49 (1)	10.00		
G-K	0/0	-38.5	-38.5	0.49 (1)	10.00		
K-L	0/0	-38.5	-38.5	0.49 (1)	10.00		
L-F	0/0	-38.5	-38.5	0.49 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	10-4	-311	-311	---	FRONT	VERT	TOTAL	---	---
J	1-10-4	-311	-311	---	FRONT	VERT	TOTAL	---	---
K	3-10-4	-311	-311	---	FRONT	VERT	TOTAL	---	---
L	5-10-4	-311	-311	---	FRONT	VERT	TOTAL	---	---

TOTAL WEIGHT = 33 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.23")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.06")

CSI: TC=0.20/1.00 (C-D:1), BC=0.49/1.00 (G-H:1), WB=0.18/1.00 (C-G:1), SS=0.45/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

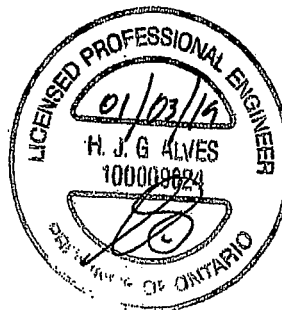
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)

JSI METAL= 0.30 (D) (INPUT = 1.00)

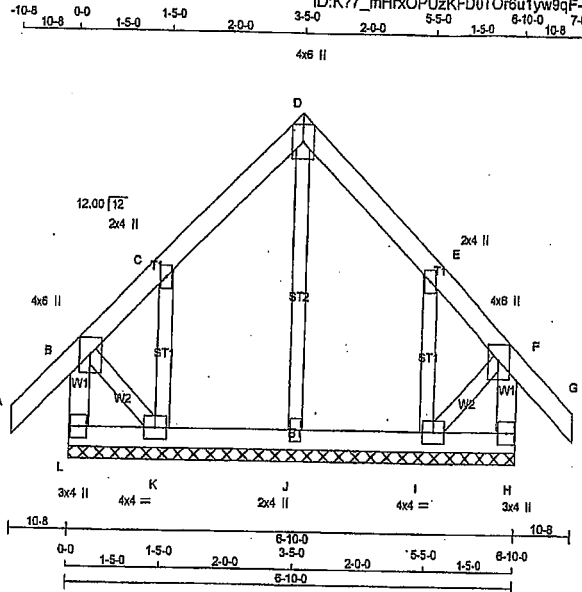


DRWG NO. TAM T1900052
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	G12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHrxOPuzKFD0r6u1yw9qF-KwvFX_7A?Mx2RDlfgV8E_Zs7NyGB30zs4qsRzz5M



Scale = 1:30.6

<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESC
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0		
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW1+t	MT20	4.0	4.0		
J	BMV1+w	MT20	2.0	4.0		
K	BMVW1+t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED (LC)	MEMB.	FORCE (LBS)	FACTORED (LC)	MAX. FACTORED (LC)
FR-TO		FROM TO		FR-TO			
A-B	0/29	-84.9	-84.9 0.08 (1)	J-D	-118/0	0.04 (1)	
B-C	-40/0	-84.9	-84.9 0.08 (1)	K-C	-142/0	0.02 (1)	
C-D	-33/0	-84.9	-84.9 0.05 (1)	I-E	-142/0	0.02 (1)	
D-E	-33/0	-84.9	-84.9 0.05 (1)	B-K	0/29	0.01 (1)	
E-F	-40/0	-84.9	-84.9 0.08 (1)	I-F	0/29	0.01 (1)	
F-G	0/29	-84.9	-84.9 0.08 (1)				
L-B	-190/0	0.0	0.0 0.02 (1)				
H-F	-190/0	0.0	0.0 0.02 (1)				
L-K	0/0	-38.5	-38.5 0.02 (3)				
K-J	0/17	-38.5	-38.5 0.03 (3)				
J-I	0/17	-38.5	-38.5 0.03 (3)				
I-H	0/0	-38.5	-38.5 0.02 (3)				

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B:1), BC=0.03/1.00 (I-J:3), WB=0.04/1.00 (D-J:1), SS=0.08/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

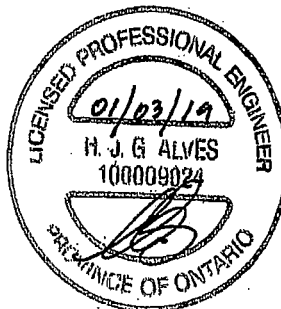
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1697

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.13 (F) (INPUT = 0.80)
JSI METAL= 0.07 (C) (INPUT = 1.00)

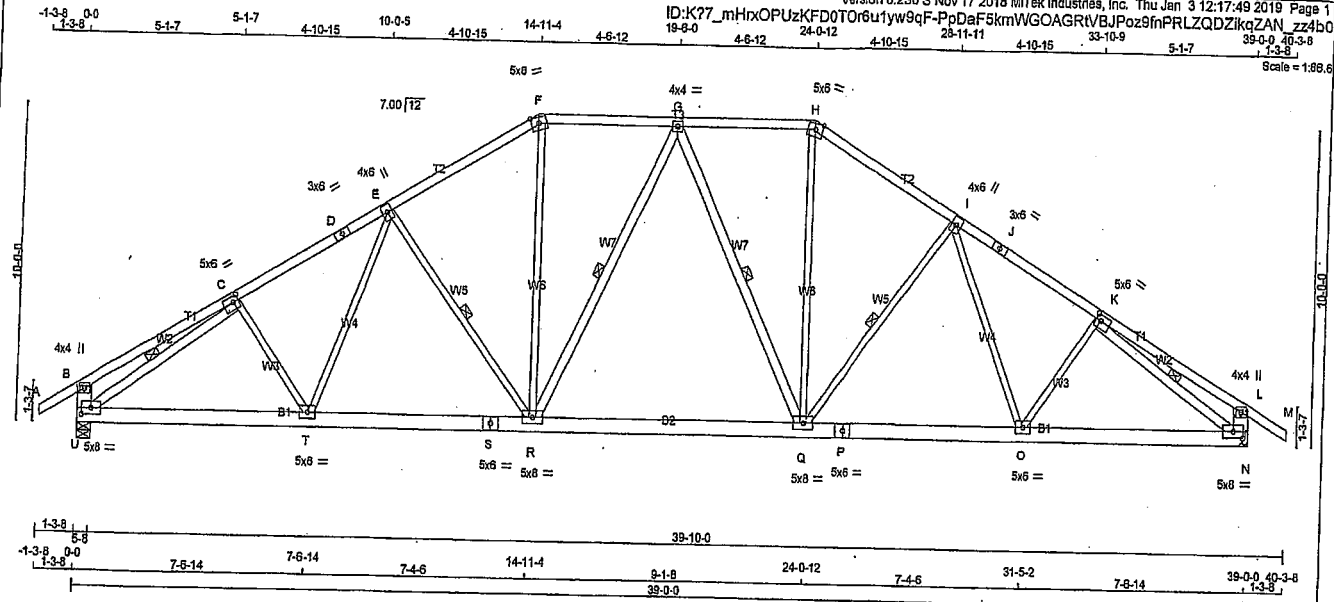


DRWG NO. TAM T1900035
STRUCTURAL
COMPONENT ONLY

JOB NAME 300539	TRUSS NAME T13	QUANTITY 6	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:K77_mHixOPUzKFD0T0r6u1yw9qF-PpDaF5kmVGOAGRTVBJPoz9fnpRLZQDZikqZAN_zz4b0



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - M	2x4	DRY	No.2
U - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
U - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2

ALL WEBS EXCEPT			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
R - G	2x4	DRY	No.2
G - Q	2x4	DRY	No.2
U - C	2x4	DRY	No.2
K - N	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	4.0	4.0	
C TMWW-t	MT20	5.0	6.0	2.25 2.00
D TS-t	MT20	3.0	6.0	
E TMWW-t	MT20	4.0	6.0	
F TTV-m	MT20	5.0	6.0	Edge
G TMWW-t	MT20	4.0	4.0	
H TTV-m	MT20	5.0	6.0	Edge
I TMWW-t	MT20	4.0	6.0	
J TS-t	MT20	3.0	6.0	
K TMWW-t	MT20	5.0	6.0	2.25 2.00
L TMV+p	MT20	4.0	4.0	
N BMVW-t	MT20	5.0	8.0	2.50 3.75
O BMVW-t	MT20	5.0	6.0	
P BS-t	MT20	5.0	8.0	
R BMVW-t	MT20	5.0	8.0	
S BS-t	MT20	5.0	6.0	
T BMVW-t	MT20	5.0	6.0	
U BMVW-t	MT20	5.0	8.0	2.50 3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS	REACTION	DOWN	HORIZ	BRG	BRG	IN-SX	IN-SX
U	2521	0	2521	0	0	5-8	5-8		
N	2521	0	2521	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1905	972 / 0	410 / 0	0 / 0	0 / 0	523 / 0	0 / 0
N	1905	972 / 0	410 / 0	0 / 0	0 / 0	523 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, G-R, G-Q, I-Q, C-U, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO				FR-TO			
A-B	0 / 29	-84.9	-84.9 0.11 (1)	10.00	C-T	0 / 147	0.03 (3)
B-C	0 / 19	-84.9	-84.9 0.26 (1)	10.00	T-E	0 / 330	0.07 (2)
C-D	-3216 / 0	-84.9	-84.9 0.39 (1)	3.64	E-R	-598 / 0	0.28 (1)
D-E	-3216 / 0	-84.9	-84.9 0.39 (1)	3.64	R-F	0 / 1038	0.23 (1)
E-F	-2711 / 0	-84.9	-84.9 0.35 (1)	3.94	F-G	-255 / 0	0.15 (1)
F-G	-2333 / 0	-84.9	-84.9 0.28 (1)	4.25	G-Q	-255 / 0	0.15 (1)
G-H	-2333 / 0	-84.9	-84.9 0.28 (1)	4.25	Q-I	0 / 1038	0.23 (1)
H-I	-2711 / 0	-84.9	-84.9 0.35 (1)	3.94	I-O	-598 / 0	0.28 (1)
I-J	-3216 / 0	-84.9	-84.9 0.39 (1)	3.64	O-K	0 / 330	0.07 (2)
J-K	-3216 / 0	-84.9	-84.9 0.39 (1)	3.64	K-N	0 / 147	0.03 (3)
K-L	0 / 19	-84.9	-84.9 0.26 (1)	10.00	U-C	-3472 / 0	0.64 (1)
L-M	0 / 29	-84.9	-84.9 0.11 (1)	10.00	C-N	-3472 / 0	0.64 (1)
U-B	-290 / 0	0.0	0.0 0.02 (1)	7.81			
N-L	-290 / 0	0.0	0.0 0.02 (1)	7.81			
U-T	0 / 2763	-38.5	-38.5 0.47 (1)	10.00			
T-S	0 / 2681	-38.5	-38.5 0.47 (2)	10.00			
S-R	0 / 2681	-38.5	-38.5 0.47 (2)	10.00			
R-Q	0 / 2440	-38.5	-38.5 0.44 (2)	10.00			
Q-P	0 / 2681	-38.5	-38.5 0.47 (2)	10.00			
P-O	0 / 2681	-38.5	-38.5 0.47 (2)	10.00			
O-N	0 / 2763	-38.5	-38.5 0.47 (1)	10.00			

TOTAL WEIGHT = 6 X 209 = 1252 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TO=0.39/1.00 (K-K1), BC=0.47/1.00 (Q-Q2), WB=0.64/1.00 (K-N1), SSI=0.19/1.00 (G-H1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

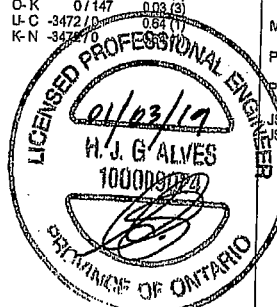
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687
	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

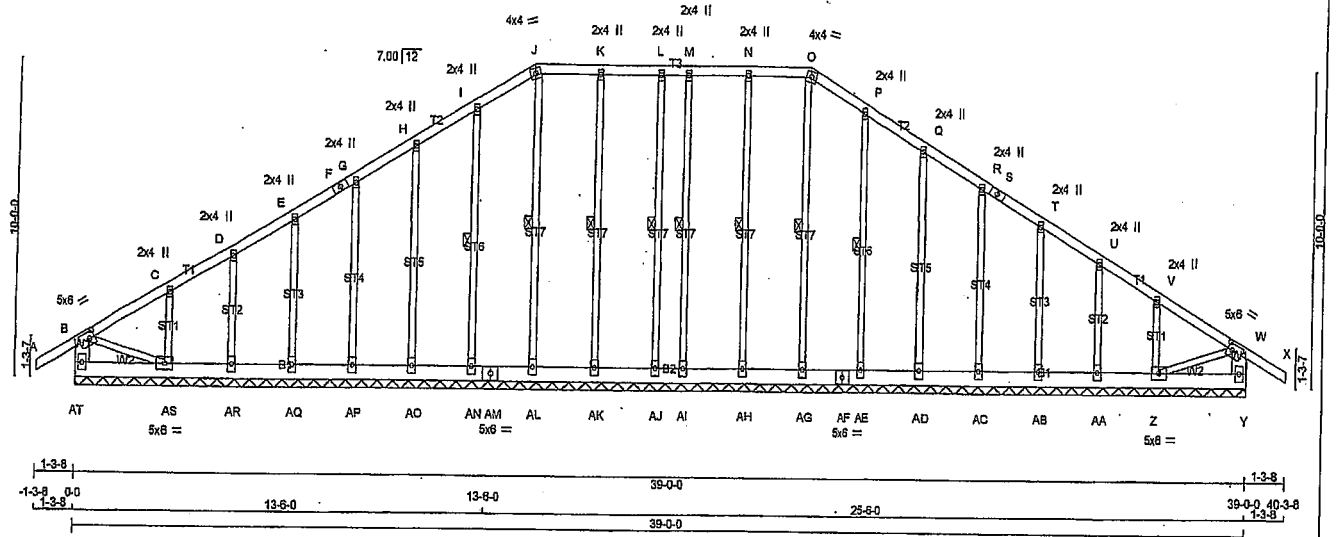
PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP=0.90 (K) (INPUT = 0.90)
JS METAL=0.85 (C) (INPUT = 1.00)



DWS NO. TAM T19 000 66
STRUCTURAL
CALCULATED BY

JOB NAME 300539	TRUSS NAME G13	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Thu Jan 3 12:18:35 2019 Page 1 ID:K77_mHxOPUzKFD0TOr6u1yw9qF-2g7zvqldgOBenVOLT_EarsZk7_9B88JUUJ_c_Kzz4a1 39-0-0 40-3-8 1-3-8 Scale = 1/8"=1'-0"	



TOTAL WEIGHT = 2 X 227 = 454 lb

LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - F	2x4 DRY	No.2	SPF
F - J	2x4 DRY	No.2	SPF
J - O	2x4 DRY	No.2	SPF
O - S	2x4 DRY	No.2	SPF
S - X	2x4 DRY	No.2	SPF
AT - Y	2x6 DRY	No.2	SPF
Y - W	2x6 DRY	No.2	SPF
AT - AM	2x6 DRY	No.2	SPF
AM - AF	2x6 DRY	No.2	SPF
AF - Y	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-AJ, K-AK, J-AL, I-AN, N-AH, M-AI, O-AG, P-AE.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMW-I	MT20	5.0	6.0	2.50 2.00
C, D, E, G, H, I, K, L, M, N, P, Q, R, T, U, V				
C TMW+u	MT20	2.0	4.0	
F TS-I	MT20	3.0	6.0	
J TTW-m	MT20	4.0	4.0	
O TTW-m	MT20	4.0	4.0	
S TS-I	MT20	3.0	6.0	
W TMW+u	MT20	5.0	6.0	2.50 2.00
Y BMV1+p	MT20	3.0	6.0	
Z BMW1-t	MT20	5.0	6.0	
AA, AB, AC, AD, AE, AG, AH, AI, AJ, AK, AL, AN, AO, AP, AQ, AR				
AA BMV1+w	MT20	3.0	6.0	
AF BS-I	MT20	5.0	6.0	
AM BS-I	MT20	5.0	6.0	
AS BMW1-t	MT20	5.0	6.0	
AT BMV1+p	MT20	3.0	6.0	

MEMB.	FORCE (LBS)	FACTORED (PLF)	VERT. LOAD LC1	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)
FR-TO							
A-B	0/29	-84.9	-84.9	0.11 (1)	10.00	AJ-L	-117/0
B-C	-26/0	-84.9	-84.9	0.09 (1)	6.25	AK-K	-198/0
C-D	-41/0	-84.9	-84.9	0.09 (1)	6.25	AL-J	-131/0
D-E	-23/0	-84.9	-84.9	0.04 (1)	6.25	AN-I	-185/0
E-F	-24/0	-84.9	-84.9	0.04 (1)	6.25	AO-H	-185/0
F-G	-24/0	-84.9	-84.9	0.04 (1)	6.25	AP-G	-185/0
G-H	-21/0	-84.9	-84.9	0.04 (1)	6.25	AQ-E	-174/0
H-I	-18/0	-84.9	-84.9	0.04 (1)	6.25	AR-D	-139/0
I-J	-25/0	-84.9	-84.9	0.04 (1)	6.25	AS-C	-254/0
J-K	-13/0	-84.9	-84.9	0.05 (1)	6.25	AH-N	-198/0
K-L	-13/0	-84.9	-84.9	0.05 (1)	6.25	AI-M	-117/0
L-M	-13/0	-84.9	-84.9	0.03 (1)	6.25	AG-O	-131/0
M-N	-13/0	-84.9	-84.9	0.05 (1)	6.25	AE-P	-185/0
N-O	-13/0	-84.9	-84.9	0.05 (1)	6.25	AD-Q	-165/0
O-P	-25/0	-84.9	-84.9	0.04 (1)	6.25	AC-R	-168/0
P-Q	-18/0	-84.9	-84.9	0.04 (1)	6.25	AB-T	-174/0
Q-R	-21/0	-84.9	-84.9	0.04 (1)	6.25	AA-U	-139/0
R-S	-24/0	-84.9	-84.9	0.04 (1)	6.25	Z-V	-254/0
S-T	-24/0	-84.9	-84.9	0.04 (1)	6.25	BA-S	0/34
T-U	-23/0	-84.9	-84.9	0.04 (1)	6.25	Z-W	0/34
U-V	-41/0	-84.9	-84.9	0.09 (1)	6.25		
V-W	-26/0	-84.9	-84.9	0.09 (1)	6.25		
W-X	0/29	-84.9	-84.9	0.11 (1)	10.00		
AT-B	-247/0	0.0	0.0	0.02 (1)	7.81		
Y-W	-247/0	0.0	0.0	0.02 (1)	7.81		
AT-AS	0/0	-38.5	-38.5	0.03 (3)	10.00		
AS-AR	0/25	-38.5	-38.5	0.03 (2)	10.00		
AR-AQ	0/22	-38.5	-38.5	0.01 (2)	10.00		
AQ-AP	0/20	-38.5	-38.5	0.01 (2)	10.00		
AP-AO	0/18	-38.5	-38.5	0.01 (2)	10.00		
AO-AN	0/16	-38.5	-38.5	0.01 (2)	10.00		
AN-AM	0/15	-38.5	-38.5	0.01 (2)	10.00		
AM-AL	0/15	-38.5	-38.5	0.01 (2)	10.00		
AL-AK	0/13	-38.5	-38.5	0.01 (2)	10.00		
AK-AJ	0/13	-38.5	-38.5	0.01 (2)	10.00		
AJ-AI	0/13	-38.5	-38.5	0.01 (2)	10.00		
AI-AH	0/13	-38.5	-38.5	0.01 (2)	10.00		
AH-AG	0/13	-38.5	-38.5	0.01 (2)	10.00		
AG-AF	0/15	-38.5	-38.5	0.01 (2)	10.00		
AF-AE	0/15	-38.5	-38.5	0.01 (2)	10.00		
AE-AD	0/16	-38.5	-38.5	0.01 (2)	10.00		
AD-AC	0/18	-38.5	-38.5	0.01 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1), BC=0.03/1.00 (Z-AA:2)

WB=0.17/1.00 (Q-AD:1), SS=0.10/1.00 (V-W:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (FLI) (FLI)

MAX MIN MAX MIN MAX MIN

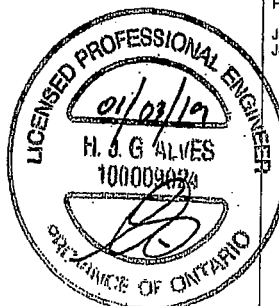
MT20 618 354 1657 788 1657 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)

JSI METAL= 0.12 (C) (INPUT = 1.00)



DRWG NO. TAM T1900060

STRUCTURAL

COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	

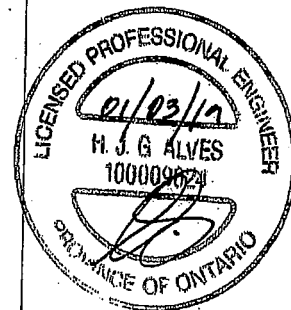
Tamarack Roof Truss, Burlington

Version 8.230 6 Nov 17 2018 Mitak Industries, Inc. Thu Jan 3 12:18:35 2019 Page 2
ID:K77_mHrxOPUzkFDOTOr6u1yw9qF-2a7zvaldgOBenVOLT EarsZK7 9BB6JUU to Kzz4a

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC. LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
AC-AB	0 / 20	-38.5 -38.5	0.01 (2)	10.00			
AB-AA	0 / 22	-38.5 -38.5	0.01 (2)	10.00			
AA-Z	0 / 25	-38.5 -38.5	0.03 (2)	10.00			
Z-Y	0 / 0	-38.5 -38.5	0.03 (3)	10.00			



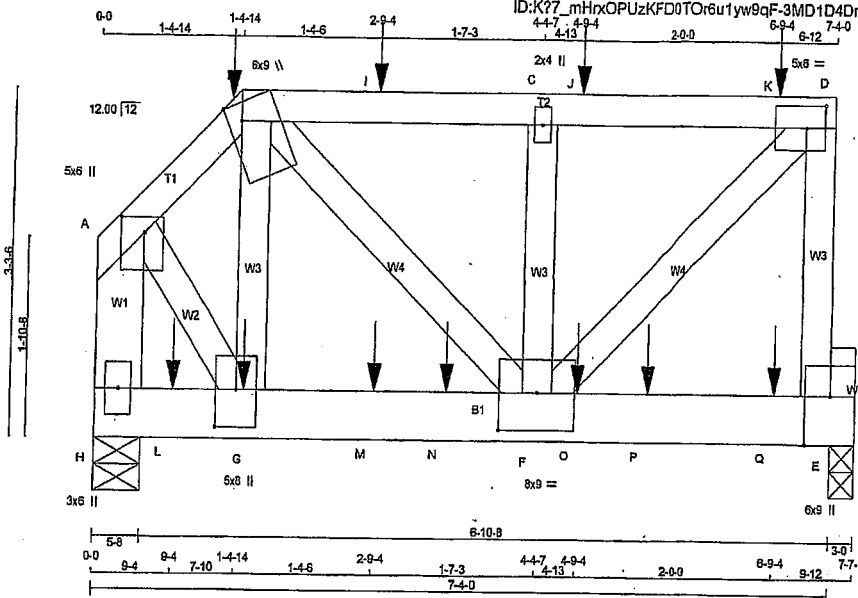
ENGINEER TAM T1900060
STRUCTURAL
CONFORMITY ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
259672	T14	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 8 Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 13:41:07 2019 Page 1
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Scale = 1:19.6



TOTAL WEIGHT = 2 X 45 = 90 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - A	2x6	DRY	No.2
H - E	2x6	DRY	No.2

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	SIDE(61.0)
B-D 1	12	SIDE(61.0)
D-E 1	12	TOP
H-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H-E 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
G-B 1	3	SIDE(464.6)
2x4 1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMW+p	MT20	5.0	6.0	1.75	2.75
B TTWW+m	MT20	6.0	9.0	Edge	1.75
C TMW+w	MT20	2.0	4.0		
D TMW-i	MT20	5.0	6.0	2.50	2.25
E BMW+i	MT20	6.0	9.0	Edge	3.00
F BMW+i	MT20	8.0	9.0	4.25	4.50
G BMW+i	MT20	5.0	8.0	4.25	2.50
H BMW+i	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORIZ	DOWN	HORIZ
E	4218	0	4218	0
H	4505	0	4505	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE		
E	3180	1642/0	669/0	0/0	0/0	869/0
H	3397	1751/0	717/0	0/0	0/0	929/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 4.94 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	CS1	LC2	MAX	UNBRAC	FR-TO	WEBS	MEMB.	MAX. FACTORED	FORCE	MAX	CS1	LC2
			(LBS)															
FR-TO																		
A-B	-3307/0			-84.9	-84.9	0.05	(1)	5.10				G-B	0/1040	0.09	(1)			
B-I	-3440/0			-84.9	-84.9	0.12	(1)	4.94				B-F	0/1496	0.12	(1)			
I-C	-3440/0			-84.9	-84.9	0.12	(1)	4.94				F-C	-3507/0	0.02	(1)			
C-J	-3440/0			-84.9	-84.9	0.12	(1)	4.95				F-D	0/1496	0.12	(1)			
J-K	-3440/0			-84.9	-84.9	0.12	(1)	4.95				A-G	0/1496	0.12	(1)			
K-D	-3440/0			-84.9	-84.9	0.12	(1)	4.95										
E-D	-3549/0			0.0	0.0	0.30	(1)	6.23										
H-A	-4570/0			0.0	0.0	0.18	(1)	6.74										
H-L	0/0			-38.5	-38.5	0.08	(1)	10.00										
L-G	0/0			-38.5	-38.5	0.08	(1)	10.00										
G-M	0/2366			-38.5	-38.5	0.54	(1)	10.00										
M-N	0/2366			-38.5	-38.5	0.54	(1)	10.00										
N-F	0/2366			-38.5	-38.5	0.54	(1)	10.00										
F-O	0/0			-38.5	-38.5	0.42	(1)	10.00										
O-P	0/0			-38.5	-38.5	0.42	(1)	10.00										
P-Q	0/0			-38.5	-38.5	0.42	(1)	10.00										
Q-E	0/0			-38.5	-38.5	0.42	(1)	10.00										

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.
B	1-4-14	-9	-10	FRONT	VERT	DEAD		
B	1-4-14	-41	-41	FRONT	VERT	SNOW		
G	1-8-12	-2483	-2483	BACK	VERT	TOTAL		
I	2-9-4	-60	-60	FRONT	VERT	TOTAL		
J	4-9-4	-60	-60	FRONT	VERT	TOTAL		
K	6-9-4	-79	-79	FRONT	VERT	TOTAL		
L	9-4	-32	-40	FRONT	VERT	TOTAL		
M	2-9-4	-27	-35	FRONT	VERT	TOTAL		
N	3-5-12	-2483	-2483	BACK	VERT	TOTAL		
O	4-9-4	-27	-35	FRONT	VERT	TOTAL		
P	5-5-12	-2483	-2483	BACK	VERT	TOTAL		
Q	6-9-4	-36	-45	FRONT	VERT	TOTAL		

DRWG NO. TAM T1900089
STRUCTURAL
COMPOUND

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	46.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.24")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.02")
ALLOWABLE DEFL. (TL) = 1/360 (0.24")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.03")

CS1: TC=0.30/1.00 (D-E:1), BC=0.54/1.00 (F-G:1),
WB=0.43/1.00 (D-F:1), SS1=0.68/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMF=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	
MT20	618	354	1687
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION = 0.000

JSI GRIP = 0.89 (D) (INPUT) JSI METAL = 0.47 (A) (INPUT)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES-BRAMPTON	DRWG NO.
402585	T15A	11	1	TRUSS DESC.		

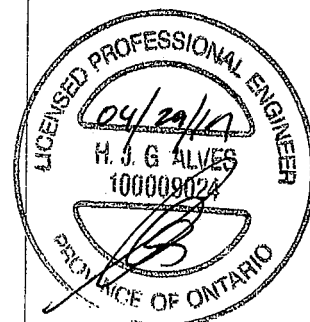
Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Apr 29 16:09:37 2019 Page 2

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TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {9.2} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



OWG NO. TAM 17909774
STRUCTURAL
COMPONENT ONLY 2/2

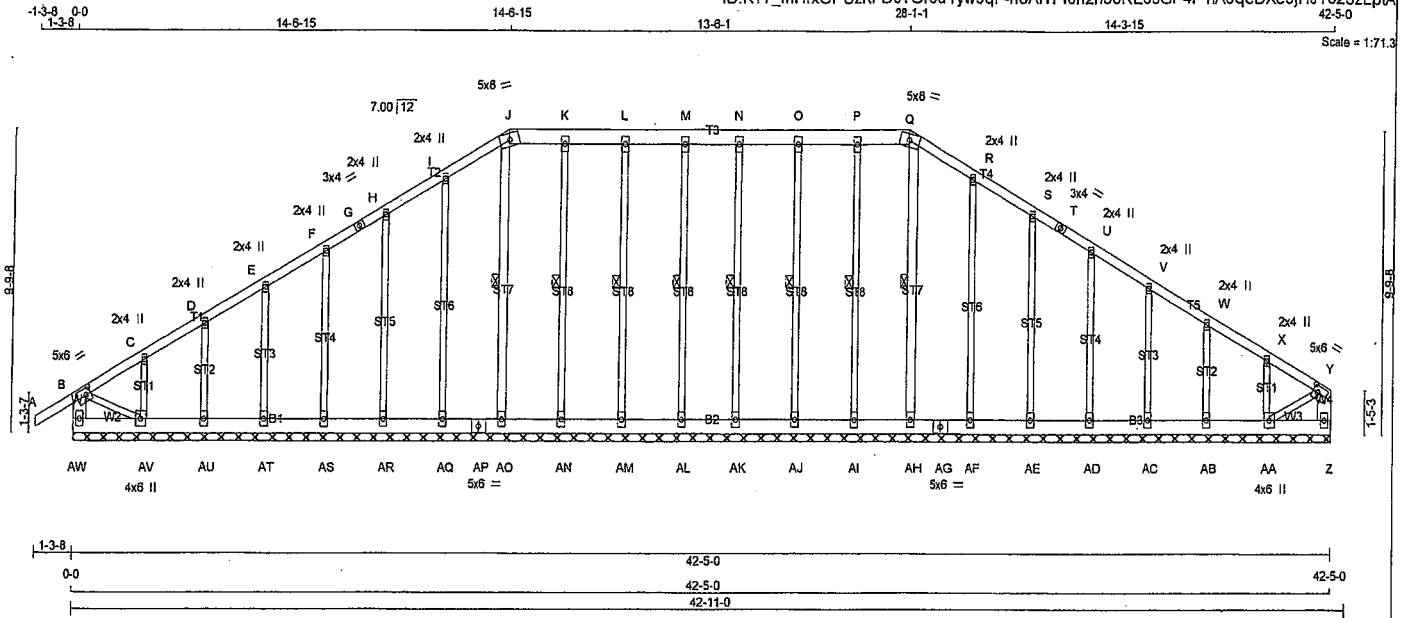
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES-BRAMPTON	DRWG NO.
402585	G15A	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Mon Apr 29 16:12:35 2019 Page 1

ID:K77_mHrxOPUzKFD0TOr6u1yw9qF-h8A1TPf0n2n36RE65GF4P1iA0qeDXe9jHJY823zLpTA

Scale = 1:71.3



TOTAL WEIGHT = 3 X 258 = 775 lb

LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA											
N. L. G. A. RULES										BEARINGS										SPECIFIED LOADS:											
CHORDS		SIZE		LUMBER		DESCR.				THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.										TOP CH. LL = 23.3 PSF											
A - G		2x4		DRY		No.2		SPF												DL = 6.0 PSF											
G - J		2x4		DRY		No.2		SPF												BOT CH. LL = 10.5 PSF											
J - Q		2x6		DRY		No.2		SPF												DL = 7.0 PSF											
Q - T		2x4		DRY		No.2		SPF		THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.										TOTAL LOAD = 46.8 PSF											
T - Y		2x4		DRY		No.2		SPF																							
AW-B		2x6		DRY		No.2		SPF																							
Z - Y		2x6		DRY		No.2		SPF																							
AW-AP		2x6		DRY		No.2		SPF																							
AP-AG		2x6		DRY		No.2		SPF																							
AG-Z		2x6		DRY		No.2		SPF																							
ALL WEBS EXCEPT										2x3 DRY No.2 SPF										LOADING IN FLAT SECTION BASED ON A SLOPE OF 7.00/12											
AO-Q										2x4 DRY No.2 SPF										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015											
AH-Q										2x4 DRY No.2 SPF										THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, CBC 2012 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014											
ALL GABLE WEBS EXCEPT										2x3 DRY No.2 SPF																					
ST5										2x4 DRY No.2 SPF																					
ST15										2x4 DRY No.2 SPF																					
DRY: SEASONED LUMBER.																															
GABLE STUDS SPACED AT 2-0-0 OC.																															
PLATES (table is in inches)																															
JT TYPE		PLATES		W		LEN		Y		X																					
B TMVW-t		MT20		5.0		6.0		2.50		2.00																					
C, D, E, F, H, I, R, S, U, V, W, X				2.0		4.0																									
C TMW+w		MT20		3.0		4.0																									
G TS-t		MT20		3.0		4.0																									
J TTW-m		MT20		5.0		8.0																									
K, L, M, N, O, P				3.0		6.0																									
K TMW+w		MT20		3.0		6.0																									
Q TTW-m		MT20		5.0		8.0																									
T TS-t		MT20		3.0		4.0																									
Y TMVW-t		MT20		5.0		6.0		2.50		2.00																					
Z BMV1+p		MT20		3.0		6.0																									
AA BMWw1+t		MT20		4.0		6.0																									
AB, AC, AD, AE, AF, AH, AI, AJ, AK, AL, AM, AN, AO, AQ, AR, AS, AT, AU				3.0		6.0																									
AB BMW1+w		MT20		3.0		6.0																									
AG BS-t		MT20		5.0		6.0																									
AP BS-t		MT20		5.0		6.0																									
AV BMWw1+t		MT20		4.0		6.0																									
AW BMV1+p		MT20		3.0		6.0																									
AW-AV										0/0 -38.5 -38.5 0.02 (3) 10.00																					
AV-AU										0/20 -38.5 -38.5 0.02 (2) 10.00																					
AU-AT										0/17 -38.5 -38.5 0.01 (2) 10.00																					
AT-AS										0/14 -38.5 -38.5 0.01 (2) 10.00																					
AS-AR										0/12 -38.5 -38.5 0.01 (2) 10.00																					
AR-AQ										0/10 -38.5 -38.5 0.01 (2) 10.00																					
AQ-AP										0/9 -38.5 -38.5 0.01 (3) 10.00																					
AP-AO										0/9 -38.5 -38.5 0.01 (3) 10.00																					
AO-AN										0/11 -38.5 -38.5 0.01 (2) 10.00																					
AN-AM										0/11 -38.5 -38.5 0.01 (2) 10.00																					
AM-AL										0/11 -38.5 -38.5 0.01 (2) 10.00																					
AL-AK										0/11 -38.5 -38.5 0.01 (2) 10.00																					
AK-AJ										0/11 -38.5 -38.5 0.01 (2) 10.00																					
AJ-AI										0/11 -38.5 -38.5 0.01 (2) 10.00																					
AI-AH										0/11 -38.5 -38.5 0.01 (2) 10.00																					
AH-AG										0/9 -38.5 -38.5 0.01 (3) 10.00																					

CHORDS										WEBS									
MEMB.		MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF)		MAX. UNBRACED LENGTH FR-TO		MEMB.		MAX. FACTORED FORCE (LBS)		MAX. UNBRACED LENGTH FR-TO							
A-B		0/29		-84.9 -84.9 0.11 (1)		10.00		AL-M		-180/0		0.09 (1)							
B-C		-22/0		-84.9 -84.9 0.08 (1)		8.25		AM-L		-174/0		0.10 (1)							
C-D		-29/0		-84.9 -84.9 0.08 (1)		8.25		AN-K		-172/0		0.10 (1)							
D-E		-19/0		-84.9 -84.9 0.04 (1)		8.25		AO-J		-139/0		0.06 (1)							
E-F		-17/0		-84.9 -84.9 0.04 (1)		6.25		AQ-I		-200/0		0.28 (1)							
F-G		-14/0		-84.9 -84.9 0.04 (1)		6.25		AR-H		-163/0		0.15 (1)							
G-H		-14/0		-84.9 -84.9 0.04 (1)		6.25		AS-F		-169/0		0.10 (1)							
H-I		-9/0		-84.9 -84.9 0.05 (1)		10.00		AT-E		-171/0		0.08 (1)							
I-J		-18/0		-84.9 -84.9 0.05 (1)		6.25		AU-D		-155/0		0.03 (1)							
J-K		-11/0		-84.9 -84.9 0.02 (1)		6.25		AV-C		-210/0		0.03 (1)							
K-L		-11/0		-84.9 -84.9 0.02 (1)		6.25		AK-N		-160/0		0.09 (1)							
L-M		-11/0		-84.9 -84.9 0.02 (1)		6.25		AJ-O		-174/0		0.10 (1)							
M-N		-11/0		-84.9 -84.9 0.02 (1)		6.25		AI-P		-172/0		0.10 (1)							
N-O		-11/0		-84.9 -84.9 0.02 (1)		6.25		AH-Q		-139/0		0.06 (1)							
O-P		-11/0		-84.9 -84.9 0.02 (1)		6.25		AF-R		-200/0		0.28 (1)							
P-Q		-11/0		-84.9 -84.9 0.02 (1)		6.25		AE-S		-163/0		0.15 (1)							
Q-R		-18/0		-84.9 -84.9 0.05 (1)		6.25		AD-U		-169/0		0.10 (1)							
R-S		-9/0		-84.9 -84.9 0.05 (1)		10.00		AC-V		-170/0		0.06 (1)							
S-T		-14/0		-84.9 -84.9 0.04 (1)		6.25		AB-W		-161/0		0.04 (1)							
T-U		-14/0		-84.9 -84.9 0.04 (1)		6.25		AA-X		-194/0		0.03 (1)							
U-V		-17/0		-84.9 -84.9 0.04 (1)		6.25		AA-Y		0/30		0.01 (1)							
V-W		-19/0		-84.9 -84.9 0.04 (1)		6.25		B-AV		0/29		0.01 (1)							
W-X		-27/0		-84.9 -84.9 0.05 (1)		6.25													
X-Y		-22/0		-84.9 -84.9 0.05 (1)		6.25													
AW-B		-223/0		0.0		0.0		0.01 (1)		7.81									
Z-Y		-100/0		0.0		0.0		0.01 (1)		7.81									

04/24/10

H. J. G. ALVES

100089024

LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

DWG NO. TAM 1009775

STRUCTURAL

COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES-BRAMPTON	DRWG NO.
402585	G15A	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

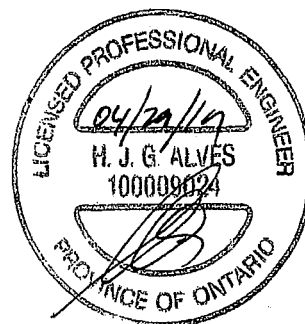
Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Apr 29 16:12:35 2019 Page 2

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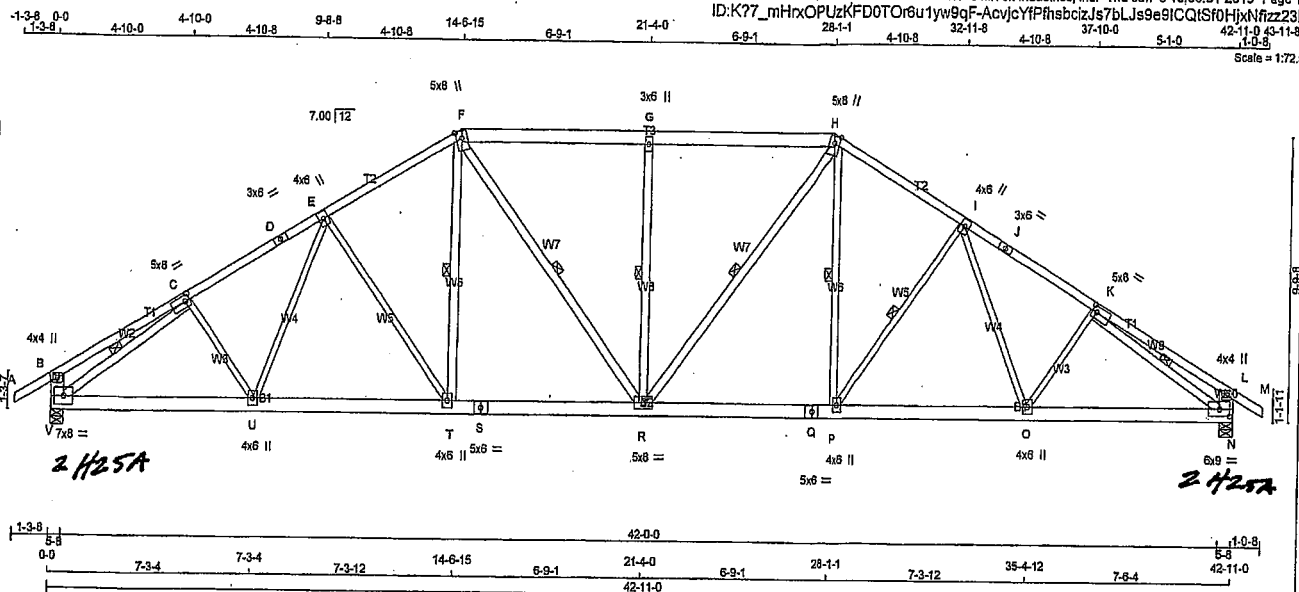
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
AG-AF	0/9	-38.5	-38.5 0.01 (3)	10.00			
AF-AE	0/10	-38.5	-38.5 0.01 (2)	10.00			
AE-AD	0/12	-38.5	-38.5 0.01 (2)	10.00			
AD-AC	0/14	-38.5	-38.5 0.01 (2)	10.00			
AC-AB	0/17	-38.5	-38.5 0.01 (2)	10.00			
AB-AA	0/20	-38.5	-38.5 0.02 (2)	10.00			
AA-Z	0/0	-38.5	-38.5 0.02 (3)	10.00			



DRWG NO. TAM 17409775
STRUCTURAL
COMPONENT ONLY 2/2

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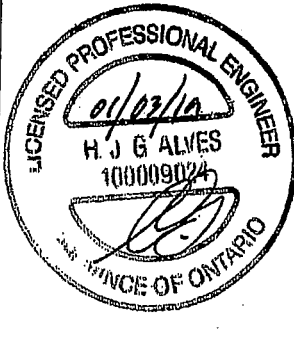


LUMBER		
N. L. G. A. RULES		
CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - H	2x6	DRY No.2
H - J	2x4	DRY No.2
J - M	2x4	DRY No.2
V - B	2x6	DRY No.2
N - L	2x6	DRY No.2
V - S	2x6	DRY No.2
S - Q	2x6	DRY No.2
Q - N	2x6	DRY No.2
ALL WEBS EXCEPT		
C - U	2x3	DRY No.2
U - E	2x3	DRY No.2
E - T	2x3	DRY No.2
P - I	2x3	DRY No.2
I - O	2x3	DRY No.2
O - K	2x3	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
J	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMVWW-t	MT20	5.0	8.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMVWW-t	MT20	4.0	6.0		
F	TTWW+m	MT20	5.0	8.0	Edge	
G	TMV+w	MT20	3.0	6.0		
H	TTWW+m	MT20	5.0	8.0	Edge	
I	TMVWW-t	MT20	4.0	8.0		
J	TS-t	MT20	3.0	6.0		
K	TMVWW-t	MT20	5.0	8.0	2.50	2.00
L	TMV+p	MT20	4.0	4.0		
N	BMVWW-t	MT20	6.0	9.0	3.00	4.25
O, P, T, U						
Q	BMVWW-t	MT20	4.0	6.0		
R	BS-t	MT20	5.0	6.0		
Q	BMVWWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
V	BMVWW-t	MT20	7.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
GROSS REACTION		GROSS REACTION				BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
V	3228	0	3327	-421	-954	5-8	5-8
N	3201	0	3305	0	-946	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT V FOR 954 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 946 LBS FACTORED UPLIFT

PROVIDE FOR 421 LBS FACTORED HORIZONTAL REACTION AT JOINT V

UNFACTORED REACTIONS							
1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2398	1439 / 0	451 / 0	0 / 0	484 / -1050	574 / 0	0 / 0
N	2380	1427 / 0	451 / 0	0 / 0	292 / -1043	571 / 0	0 / 0

HORIZONTAL REACTIONS

V	—	0/0	0/0	0/0	300/-301	0/0	0/0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V. N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-T, F-R, G-R, H-R, H-P, I-P, C-V, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LBS)	
FR-TO		FROM - TO		FR-TO			
A-B	0/42	-105.2	-105.2	10.00	C-U	-25/361	0.08 (3)
B-C	-24/161	-105.2	-105.2	0.34 (2)	E-E	-80/295	0.07 (10)
C-D	-4227/1361	-105.2	-105.2	0.84 (2)	D-F	-733/431	0.98 (2)
D-E	-4227/1361	-105.2	-105.2	0.84 (2)	F-F	-272/875	0.14 (2)
E-F	-3718/1297	-105.2	-105.2	0.73 (2)	R-R	-373/1148	0.27 (4)
F-G	-3618/1278	-105.2	-105.2	0.52 (3)	R-G	-1127/501	0.46 (10)
G-H	-3618/1278	-105.2	-105.2	0.52 (3)	R-H	-352/1110	0.25 (13)
H-I	-3748/1301	-105.2	-105.2	0.73 (3)	P-H	-286/826	0.15 (3)
I-J	-4358/1397	-105.2	-105.2	0.86 (3)	P-I	-798/447	0.34 (3)
J-K	-4358/1397	-105.2	-105.2	0.86 (3)	I-O	-100/388	0.09 (14)
K-L	-23/154	-105.2	-105.2	0.36 (3)	O-K	-126/285	0.05 (2)
L-M	0/34	-105.2	-105.2	0.11 (3)	V-C	-4583/1203	0.78 (2)
M-N	-400/250	0.0	0.0	0.03 (10)	K-N	-4721/1246	0.85 (3)
N-L	-384/242	0.0	0.0	0.03 (14)			
U	-1186/3628	-38.5	-38.5	0.57 (2)			
T	-1035/3565	-38.5	-38.5	0.52 (1)			
S	-778/3185	-38.5	-38.5	0.48 (1)			
R	-778/3185	-38.5	-38.5	0.48 (1)			
Q	-648/3211	-38.5	-38.5	0.48 (1)			
P	-648/3211	-38.5	-38.5	0.48 (1)			
O	-805/3639	-38.5	-38.5	0.53 (3)			
N	-990/3808	-38.5	-38.5	0.60 (3)			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

EX. NO. TAM T71 00102
ST. FEDERAL

CONFIDENTIAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 30.1 PSF

		DL =	6.0	PSF
POT	CH	U =	10.5	PSF

BOI	CH.	LL	=	10.5	PSF
LOB		DL	=	7.0	PSF

TOTAL LOAD	=	53.6	PSF
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SPACING. 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 7.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL
OR INDUSTRIAL BUILDING REQUIREMENTS OF
PART 4, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.43")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.18")
ALLOWABLE DEFL.(TL)= $L/180$ (2.86")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.26")

CSI: TC=0.86/1.00 (I-K:3), BC=0.60/1.00 (N-O:3),
WB=0.98/1.00 (E-T:2), SSI=0.30/1.00 (G-H:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

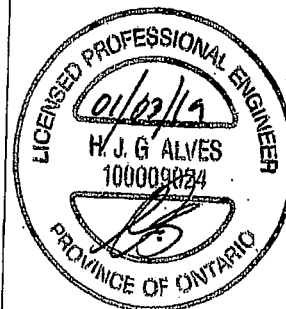
JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.88 (K) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME 259674	TRUSS NAME T15	QUANTITY 5	PLY 1	JOB DESC. Tamarack Roof Truss, Burlington	TRUSS DESC.	DRWG NO.
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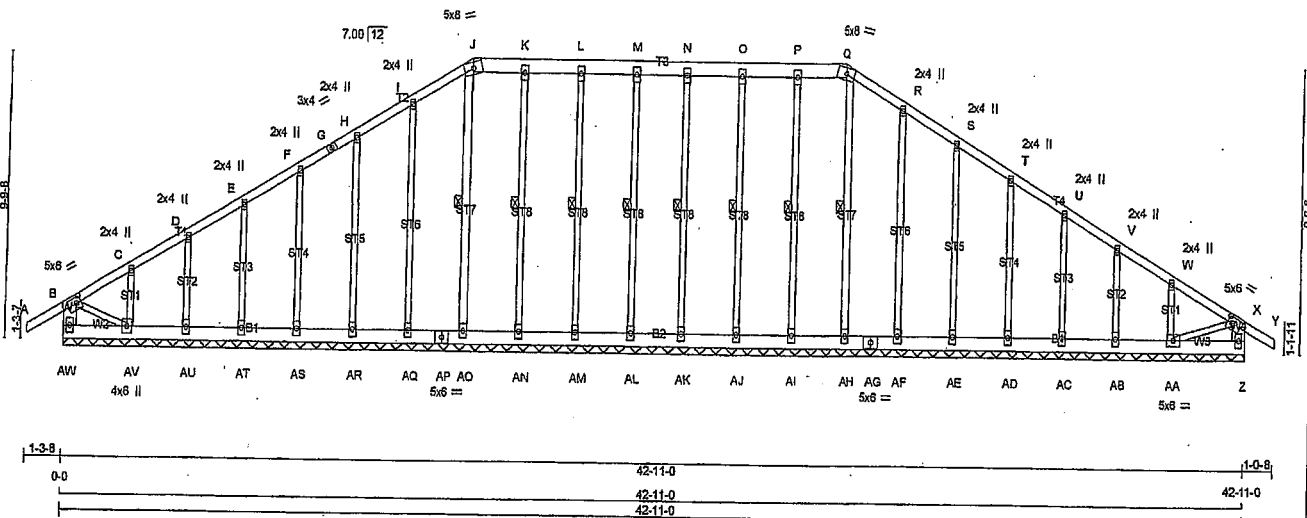
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 15:09:51 2019 Page 2
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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_p , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



ENGINEER TAM 1900102
STRUCTURAL
COMPONENT ONLY 7/2

JOB NAME 259674	TRUSS NAME G15	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 16:33:22 2019 Page 1					
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42-11-0 43-11-8 1-0-8 Scale = 1:72.5					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - G	2x4 DRY	No.2	SPF
G - J	2x4 DRY	No.2	SPF
J - Q	2x6 DRY	No.2	SPF
Q - Y	2x4 DRY	No.2	SPF
AW - B	2x6 DRY	No.2	SPF
Z - X	2x6 DRY	No.2	SPF
AW - AP	2x6 DRY	No.2	SPF
AP - AG	2x6 DRY	No.2	SPF
AG - Z	2x6 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF
AO - J	2x4 DRY	No.2	SPF
AH - Q	2x4 DRY	No.2	SPF
ALL GABLE WEBS EXCEPT	2x3 DRY	No.2	SPF
ST5	2x4 DRY	No.2	SPF
ST15	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-4	MT20	5.0	6.0	2.50 2.00
C	TMVW-w	MT20	2.0	4.0	
G	TS-4	MT20	3.0	4.0	
J	TTW-m	MT20	5.0	6.0	
K, L, M, N, O, P					
K	TMVW-w	MT20	3.0	6.0	
Q	TTW-m	MT20	5.0	6.0	
X	TMVW-4	MT20	5.0	6.0	2.50 2.00
Z	BMV1-p	MT20	3.0	6.0	
AA	BMVW-4	MT20	5.0	6.0	
AB, AC, AD, AE, AF, AH, AI, AJ, AK, AL, AM, AN, AO, AQ, AR, AS, AT, AU					
AB	BMV1-w	MT20	3.0	6.0	
AG	BS-4	MT20	5.0	6.0	
AP	BS-4	MT20	5.0	6.0	
AV	BMVW-4	MT20	4.0	6.0	
AW	BMV1-p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF M-A-L, L-A-M, K-A-N, J-A-O, N-A-K, O-A-J, P-A-I, Q-A-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED FORCE (LBS)	MEMB.	FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0/29	AL-M	-160/0
B-C	-21/0	AM-L	-174/0
C-D	-28/0	AN-K	-172/0
D-E	-18/0	AO-J	-139/0
E-F	-16/0	AQ-I	-200/0
F-G	-14/0	AR-H	-163/0
G-H	-14/0	AS-F	-169/0
H-I	-9/0	AT-E	-171/0
I-J	-18/0	AU-D	-155/0
J-K	-11/0	AV-C	-210/0
K-L	-10/0	AK-N	-160/0
L-M	-10/0	AJ-O	-174/0
M-N	-10/0	AI-P	-172/0
N-O	-10/0	AH-Q	-139/0
O-P	-10/0	AF-R	-200/0
P-Q	-11/0	AE-S	-163/0
Q-R	-18/0	AD-T	-169/0
R-S	-9/0	AC-U	-172/0
S-T	-14/0	AB-V	-149/0
T-U	-16/0	AA-W	-227/0
U-V	-16/0	AA-X	0/28
V-W	-31/0	BAV	0/28
W-X	-21/0		
X-Y	0/24		
AW-B	-222/0		
Z-X	-208/0		
AW-AV	0/0		
AV-AU	0/20		
AU-AT	0/17		
AT-AS	0/14		
AS-AR	0/12		
AR-AQ	0/10		
AQ-AP	0/8		
AP-AO	0/8		
AO-AN	0/10		
AN-AM	0/10		
AM-AL	0/10		
AL-AK	0/10		
AK-AJ	0/10		
AJ-AI	0/10		
AI-AH	0/10		
AH-AG	0/8		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 7.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1), BC=0.02/1.00 (AA-AB:2), WB=0.28/1.00 (I-AQ:1), SSI=0.09/1.00 (W-X:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

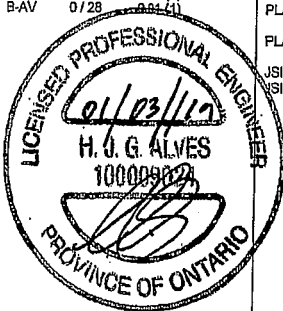
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 788 1967 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (AV) (INPUT = 0.90)
JSI METAL= 0.11 (W) (INPUT = 1.00)



DRWG NO. TAM 11900101
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
259674	G15	1	1	TRUSS DESC.	

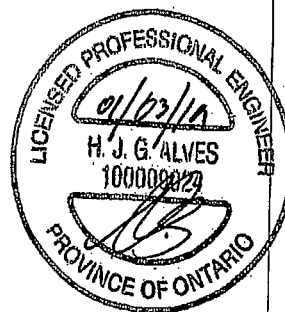
Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:33:22 2019 Page 2
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LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO		FROM	TO		FR-TO		FROM	TO	
AG-AF	0 / 8	-38.5	-38.5	0.01 (3)	10.00				
AF-AE	0 / 10	-38.5	-38.5	0.01 (2)	10.00				
AE-AD	0 / 12	-38.5	-38.5	0.01 (2)	10.00				
AD-AC	0 / 14	-38.5	-38.5	0.01 (2)	10.00				
AC-AB	0 / 17	-38.5	-38.5	0.01 (2)	10.00				
AB-AA	0 / 20	-38.5	-38.5	0.02 (2)	10.00				
AA-Z	0 / 0	-38.5	-38.5	0.02 (3)	10.00				



DRWG NO. TAM 7700101
STRUCTURAL
COMPLEMENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
259673	T16X	1	4	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

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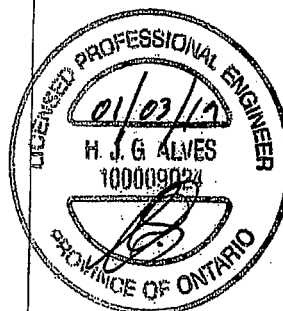
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	6-0-12	-3099	-3099	902	BACK	VERT	TOTAL	—	—
M	8-0-12	-3099	-3099	902	BACK	VERT	TOTAL	—	—
N	10-0-12	-3099	-3099	902	BACK	VERT	TOTAL	—	—
O	11-7-12	-3099	-3099	902	BACK	VERT	TOTAL	—	—
P	13-7-12	-3099	-3099	902	BACK	VERT	TOTAL	—	—
Q	15-7-12	-3099	-3099	902	BACK	VERT	TOTAL	—	—

JSI GRIP= 0.67 (D) (INPUT = 0.90)
JSI METAL= 0.68 (G) (INPUT = 1.00)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2 (B)

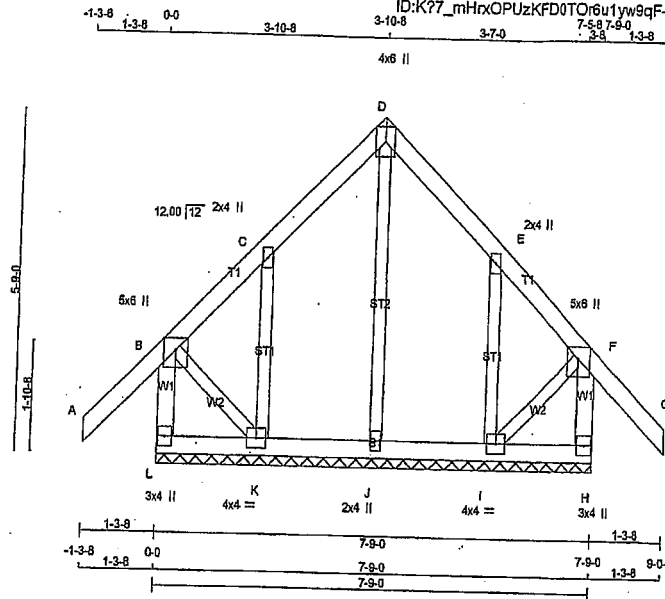
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (0.2) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{p0} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



ENGINEER TAM 11900099
STRUCTURAL
CHECK ONLY 2/2

JOB NAME 300539	TRUSS NAME G17	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:35.6



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	5.0 Edge
C	TMVW+w	MT20	2.0	4.0
D	TTW+p	MT20	4.0	6.0
E	TMVW+w	MT20	2.0	4.0
F	TMVW+p	MT20	5.0	6.0 Edge
H	BMV1+p	MT20	3.0	4.0
I	BMVW1-l	MT20	4.0	4.0
J	BMVW1+w	MT20	2.0	4.0
K	BMVW1-l	MT20	4.0	4.0
L	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
L-B	-253 / 0	0.0	0.0 0.03 (1)	J-D	-138 / 0	0.07 (1)	0.07 (1)
A-B	0 / 42	-84.9	-84.9 0.12 (1)	K-C	-137 / 0	0.03 (1)	0.03 (1)
B-C	-44 / 0	-84.9	-84.9 0.11 (1)	I-E	-137 / 0	0.03 (1)	0.03 (1)
C-D	-21 / 0	-84.9	-84.9 0.05 (1)	B-K	0 / 18	0.00 (1)	0.00 (1)
D-E	-21 / 0	-84.9	-84.9 0.05 (1)	I-F	0 / 18	0.00 (1)	0.00 (1)
E-F	-44 / 0	-84.9	-84.9 0.11 (1)				
F-G	0 / 42	-84.9	-84.9 0.12 (1)				
H-F	-253 / 0	0.0	0.0 0.03 (1)				
L-K	0 / 0	-38.5	-38.5 0.03 (3)				
K-J	0 / 10	-38.5	-38.5 0.03 (3)				
J-I	0 / 10	-38.5	-38.5 0.03 (3)				
I-H	0 / 0	-38.5	-38.5 0.03 (3)				

TOTAL WEIGHT = 42 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.8	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (F-G:1), BC=0.03/1.00 (J-K:3), WB=0.07/1.00 (D-J:1), SS=0.07/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

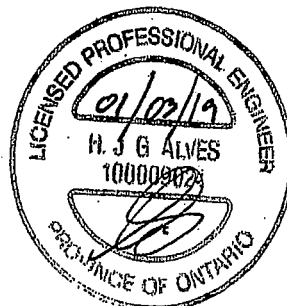
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1687
		788	1987
		1656	

PLATE PLACEMENT TOL. = 0.250 Inches

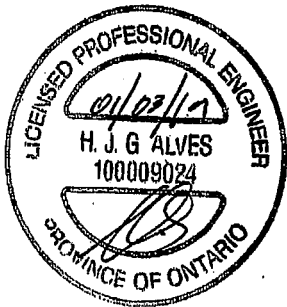
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.80)
JSI METAL= 0.07 (C) (INPUT = 1.00)

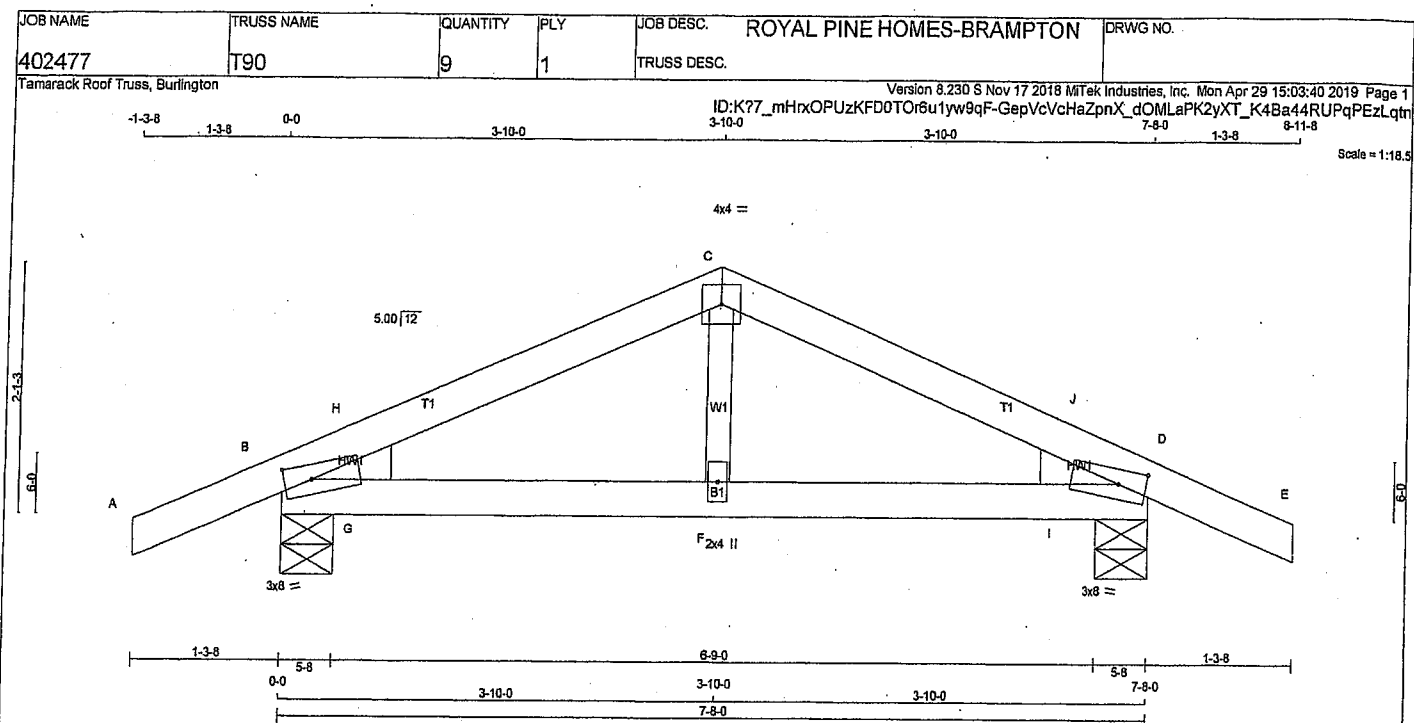


DRWG NO. TAM 77900061
STRUCTURAL
COMPANY

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 15:09:52 2019 Page 1
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-1-3-8 0-0



ENGINE NO. TAM 71900103
STRUCTURAL
ECONOMY PLAN



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0	1.50	2.75
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-m	MT20	3.0	8.0	1.50	2.75
F	BMW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	HEEL
B	587	0	587	0	0	5-8	5-8	2x4 L
D	587	0	587	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	438	241/0	81/0	0/0	0/0	116/0	0/0
D	438	241/0	81/0	0/0	0/0	116/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MAX. FACTORED UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO					FR-TO		
A-B	0/15	-84.9	-84.9 0.11 (1)	10.00	F-C	0/229	0.05 (2)
B-H	-849/0	-84.9	-84.9 0.03 (1)	6.25	G-H	-53/70	0.00 (1)
H-C	-598/0	-84.9	-84.9 0.12 (1)	6.25	I-J	-53/70	0.00 (1)
C-J	-598/0	-84.9	-84.9 0.12 (1)	6.25			
J-D	-849/0	-84.9	-84.9 0.03 (1)	6.25			
D-E	0/15	-84.9	-84.9 0.11 (1)	10.00			
B-G	0/549	-38.5	-38.5 0.17 (1)	10.00			
G-F	0/549	-38.5	-38.5 0.20 (1)	10.00			
F-I	0/549	-38.5	-38.5 0.20 (1)	10.00			
I-D	0/549	-38.5	-38.5 0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.12/1.00 (C-H:1), BC=0.20/1.00 (F-H:1), WB=0.05/1.00 (C-F:2), SS=0.10/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

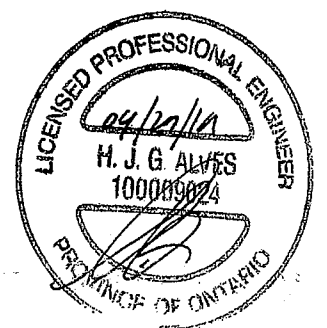
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.80)
JSI METAL= 0.12 (B) (INPUT = 1.00)

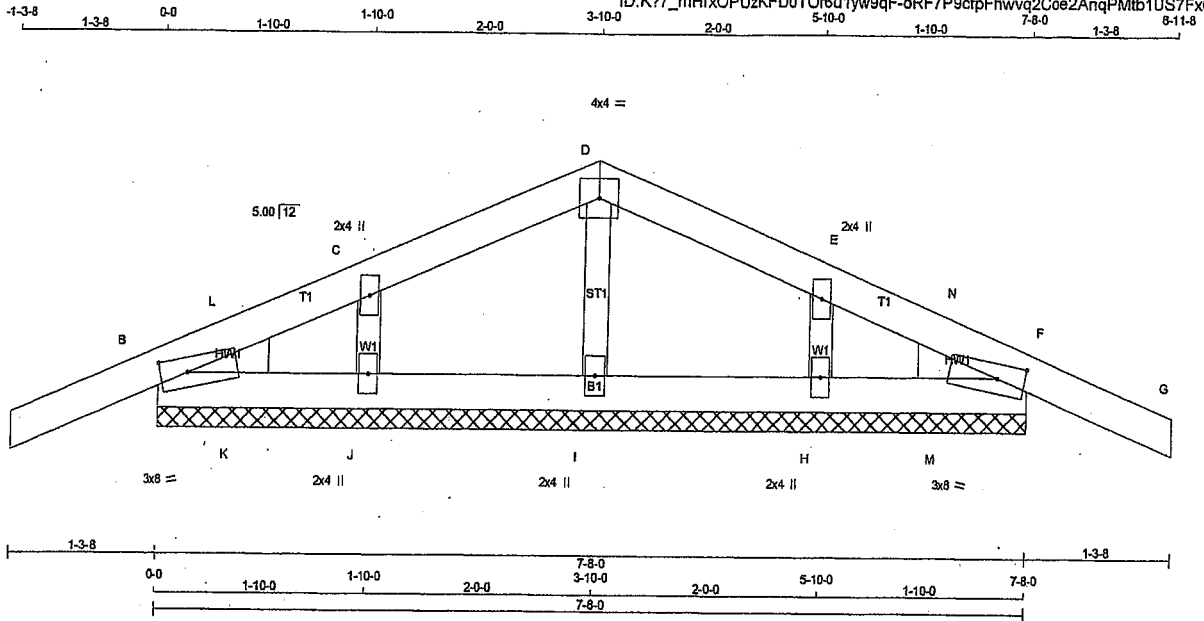


DWG NO. TAM 19029772
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ROYAL PINE HOMES-BRAMPTON	DRWG NO.
402477	G90	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Apr 29 15:03:39 2019 Page 1
ID: K77_mHxOPUzKF00TOr6u1yw9qF-oRF7P9cFpFhwvq2Coe2AnqPMib1US7FxCqfHtozLqto



Scale = 1:18.5

TOTAL WEIGHT = 3 X 26 = 77 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - D 2x4 DRY

D - G 2x4 DRY

B - F 2x4 DRY

ALL WEBS 2x3 DRY

ALL GABLE WEBS 2x3 DRY

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0	1.50	2.75
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-m	MT20	3.0	8.0	1.50	2.75
H, I, J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL

WEDGE

2x4 L

2x4 R

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	UNBRACED LENGTH FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							
A-B	0/15	-84.9	-84.9	0.11 (1)	10.00	I-D	-152/0
B-L	-65/0	-84.9	-84.9	0.10 (1)	6.25	J-C	-173/0
L-C	-1/10	-84.9	-84.9	0.04 (1)	10.00	H-E	-173/0
C-D	-5/4	-84.9	-84.9	0.04 (1)	10.00	K-L	0/66
D-E	-5/4	-84.9	-84.9	0.04 (1)	10.00	M-N	0/66
E-N	-1/10	-84.9	-84.9	0.04 (1)	10.00		
N-F	-65/0	-84.9	-84.9	0.10 (1)	6.25		
F-G	0/15	-84.9	-84.9	0.11 (1)	10.00		
B-K	0/6	-38.5	-38.5	0.02 (1)	10.00		
K-J	0/6	-38.5	-38.5	0.02 (3)	10.00		
J-I	-8/3	-38.5	-38.5	0.03 (2)	10.00		
I-H	-8/3	-38.5	-38.5	0.03 (2)	10.00		
H-M	0/6	-38.5	-38.5	0.02 (3)	10.00		
M-F	0/6	-38.5	-38.5	0.02 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012

- CSA 088-09, CSA 086-14

- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1), BC=0.03/1.00 (H-I:2), WB=0.02/1.00 (C-J:1), SS=0.09/1.00 (B-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (E) (INPUT = 1.00)



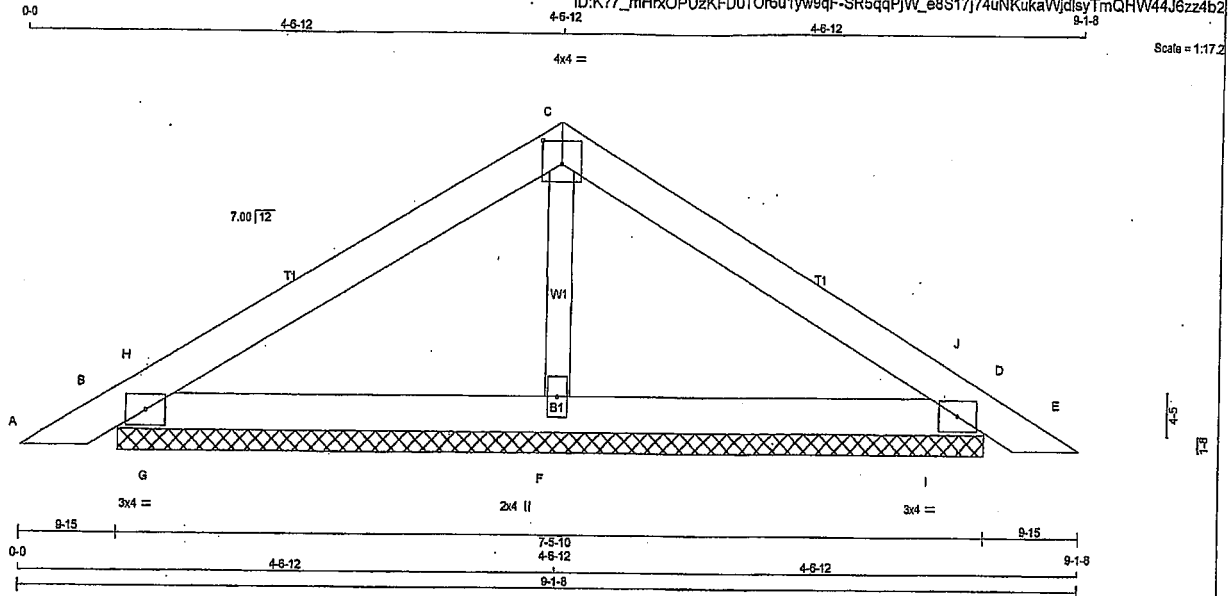
DWG NO. 1AM 1900972
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	P1	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHrxOPUzKFD0rGu1yw9qF-SR5qqPjW_e8S17174uNKuKaVjdlsyTmQHW44J6zz4b2



TOTAL WEIGHT = 6 X 22 = 132 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - E 2x4 DRY No.2

B - D 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY, SEASONED LUMBER.

LUMBER

No.2

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
B	322	0	322	0
D	322	0	322	0
F	404	0	404	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
B	237
D	237
F	313

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0/15	-84.9 -84.9 0.04 (1)	F-C	-158/0
B-H	-75/0	-84.9 -84.9 0.04 (1)	G-H	-246/24
H-C	-116/0	-84.9 -84.9 0.15 (1)	I-J	-246/24
C-J	-116/0	-84.9 -84.9 0.15 (1)		
J-D	-75/0	-84.9 -84.9 0.04 (1)		
D-E	0/15	-84.9 -84.9 0.04 (1)		
B-G	0/97	-38.5 -38.5 0.14 (1)		
G-F	0/97	-38.5 -38.5 0.16 (1)		
F-I	0/97	-38.5 -38.5 0.16 (1)		
I-D	0/97	-38.5 -38.5 0.14 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-J:1), BC=0.16/1.00 (F-I:1),
WB=0.02/1.00 (C-F:1), SS=0.21/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

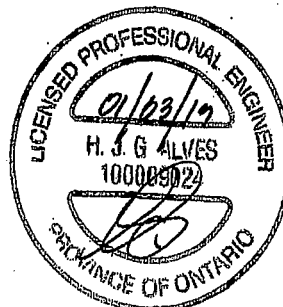
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

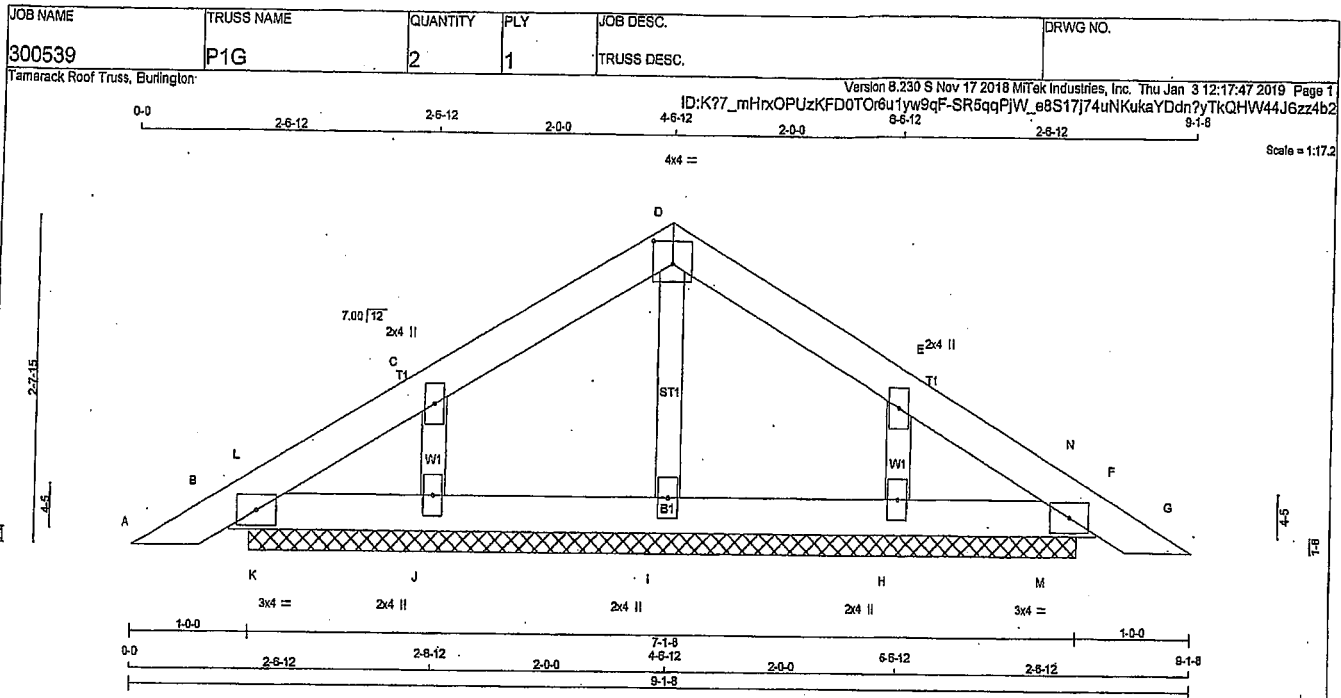
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)

JSI METAL= 0.05 (D) (INPUT = 1.00)



ENR NO. TAM 77900063
STRUCTURAL
CONSTRUCTION ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
B - F	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

ALL GABLE WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMBW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMBW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H, I, J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	0/15	-84.9	-84.9	I-D	-118/0	0.02	(1)
B-L	-53/0	-84.9	-84.9	J-C	-188/0	0.03	(1)
L-C	-21/0	-84.9	-84.9	H-E	-188/0	0.03	(1)
C-D	-31/0	-84.9	-84.9	K-L	-11/17	0.00	(1)
D-E	-31/0	-84.9	-84.9	M-N	-11/17	0.00	(1)
E-N	-21/0	-84.9	-84.9				
N-F	-53/0	-84.9	-84.9				
F-G	0/15	-84.9	-84.9				
B-K	0/33	-38.5	-38.5				
K-J	0/33	-38.5	-38.5				
J-I	0/19	-38.5	-38.5				
I-H	0/19	-38.5	-38.5				
H-M	0/33	-38.5	-38.5				
M-F	0/33	-38.5	-38.5				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.05/1.00 (D-E), BC=0.03/1.00 (J-K), WB=0.03/1.00 (E-H), SS=0.07/1.00 (D-E)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

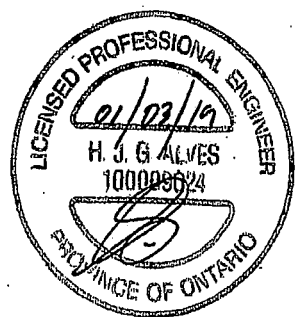
MT20 618 354 1667 786 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.12 (C) (INPUT = 0.90)

JSI METAL= 0.09 (C) (INPUT = 1.00)



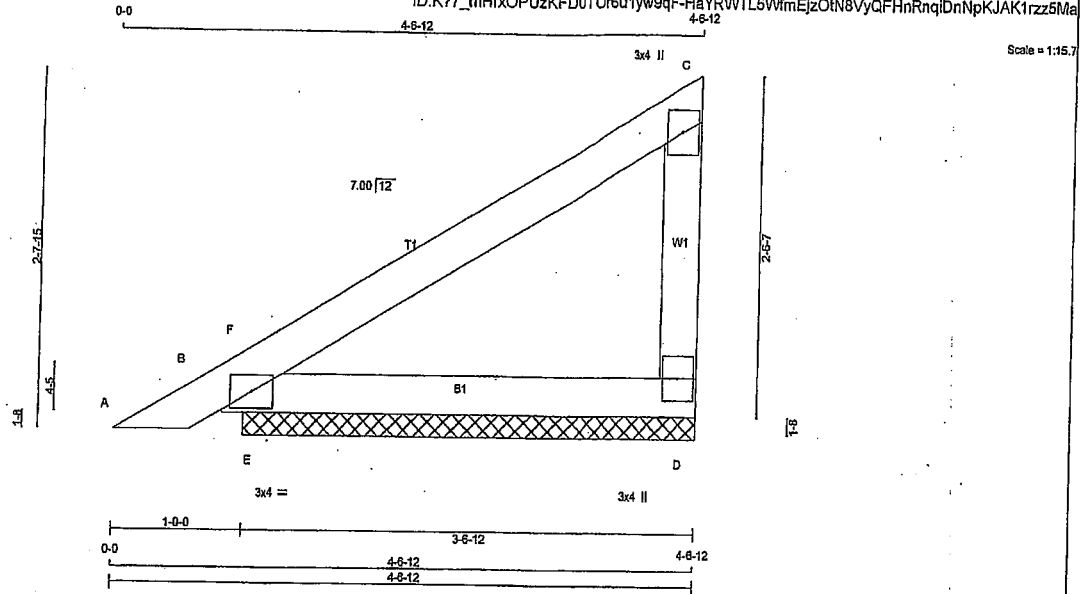
DRWG NO. TAM 11900064
STRUCTURAL
DESIGNED BY ONLY

JOB NAME 300536	TRUSS NAME P2G	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
A - C 2x4 DRY No.2
D - C 2x4 DRY No.2
B - D 2x4 DRY No.2
DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 24-0 OC.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMB1-J	MT20	3.0	4.0		
C TMB1-p	MT20	3.0	4.0		
D BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/15	-84.9 -84.9	0.04 (1)	10.00	E-F	-165/86	0.00 (1)
B-F	-60/4	-84.9 -84.9	0.05 (3)	6.25			
F-C	0/5	-84.9 -84.9	0.16 (1)	10.00			
D-C	-143/0	0.0 0.0	0.02 (1)	7.81			
B-E	0/0	-38.5 -38.5	0.10 (1)	10.00			
E-D	0/0	-38.5 -38.5	0.13 (1)	10.00			

TOTAL WEIGHT = 12 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (C-F:1), BC=0.13/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.17/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

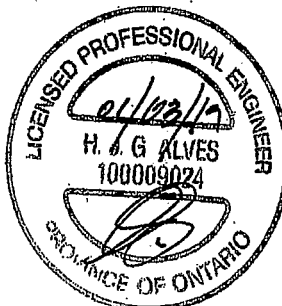
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



DRWG NO. TAM T1900056
STRUCTURAL
CONSENT ONLY

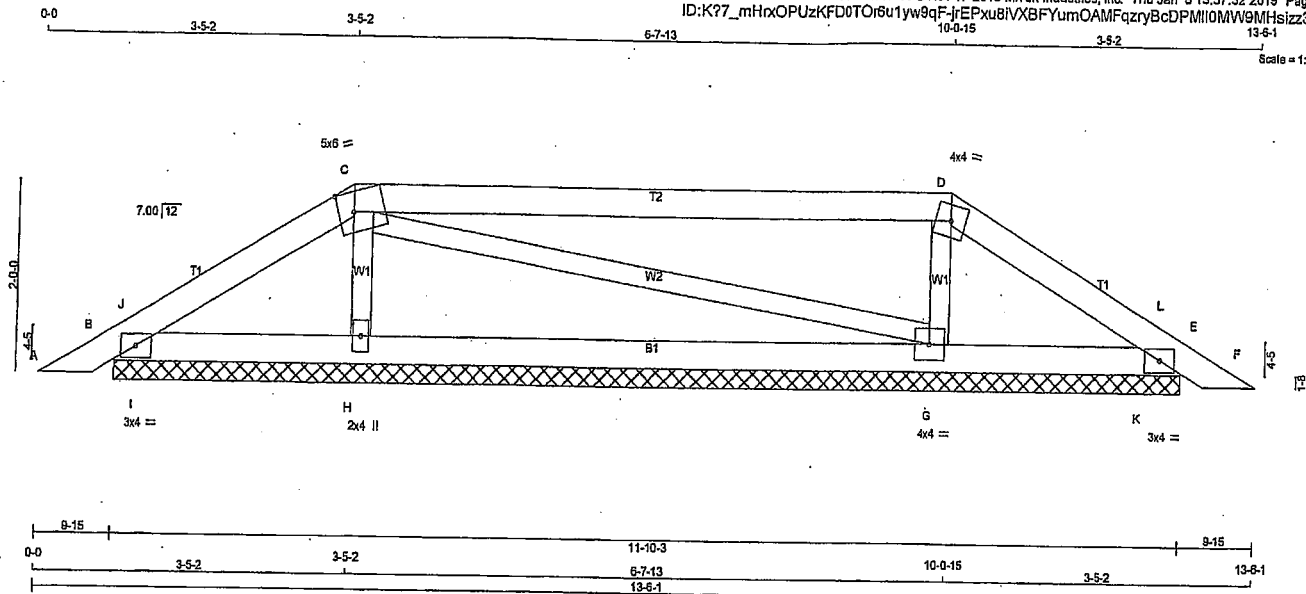
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
259673	P4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHxOPUzKFD0TO6u1yw9qF-JrEPxu8VXBFYumOAMFqzryBcDPMI0MW9MHsizz37X

Scale = 1:22.2



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - D 2x4 DRY No.2

D - F 2x4 DRY No.2

B - E 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

LUMBER

SIZE

2x4

2x4

2x4

2x4

2x3

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-H	MT20	3.0	4.0		
C	TTVV-m	MT20	5.0	6.0	2.50	2.00
D	TTVV-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWV1-t	MT20	4.0	4.0		
H	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
B	219	219	0	0
E	213	213	0	0
H	574	574	0	0
G	585	585	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
B	153	116/0	4/0	0/0	33/0	0/0
E	148	112/0	4/0	0/0	32/0	0/0
H	444	194/0	120/0	0/0	130/0	0/0
G	452	200/0	120/0	0/0	132/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LC1)	MAX. FACTORED UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LC)
FR-TO									
A-B	0/15	-84.9	-84.9	0.04 (1)	10.00	H-C	-331/0	0.05 (1)	
B-C	-32/21	-84.9	-84.9	0.03 (1)	6.25	C-G	-11/0	0.01 (1)	
C-D	-72/0	-84.9	-84.9	0.08 (1)	6.25	G-D	-340/0	0.05 (1)	
D-E	-26/0	-84.9	-84.9	0.04 (1)	6.25	I-J	-180/0	0.00 (1)	
E-F	-61/0	-84.9	-84.9	0.08 (1)	6.25	K-L	-181/0	0.00 (1)	
F-G	-20/23	-84.9	-84.9	0.03 (1)	6.25				
G-H	0/15	-84.9	-84.9	0.04 (1)	10.00				
H-I	0/58	-38.5	-38.5	0.08 (1)	10.00				
I-H	0/58	-38.5	-38.5	0.21 (3)	10.00				
H-G	0/36	-38.5	-38.5	0.21 (3)	10.00				
G-K	0/48	-38.5	-38.5	0.21 (3)	10.00				
K-E	0/48	-38.5	-38.5	0.08 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 7.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.64/1.00 (C-D:1), BC=0.21/1.00 (H-I:3), WB=0.05/1.00 (D-G:1), SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

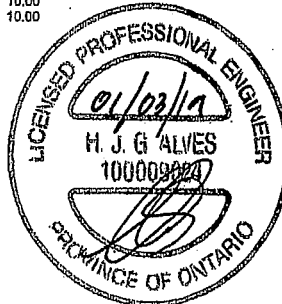
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	788	1987	1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.23 (D) (INPUT = 0.80)

JSI METAL= 0.07 (H) (INPUT = 1.00)



DRWG NO. TAM 11900095
STRUCTURAL
CONS. ONLY

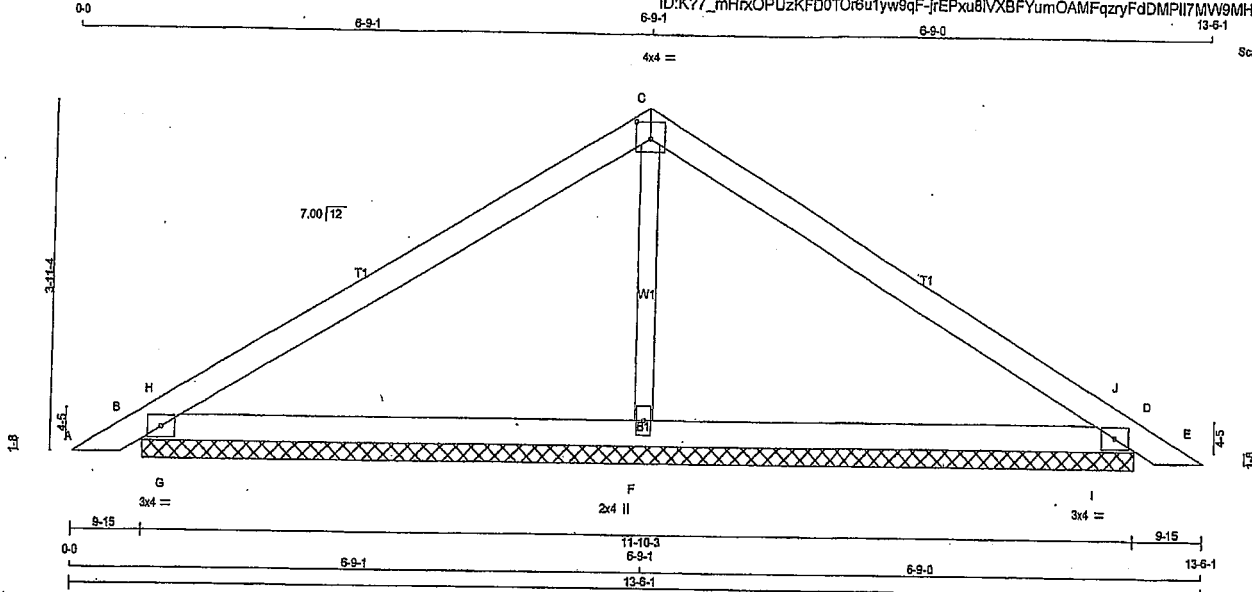
JOB NAME 259673	TRUSS NAME P5	QUANTITY 7	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:K77_mHxOPUzKFD0TOr8u1yw9qF-jrEPx8IVXBFYumOAMFqzyFdDMPiI7MW9MHsizz37X

Scale: 1/2"=1'



TOTAL WEIGHT = 7 X 34 = 238 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMB1-I	MT20	3.0	4.0	
C TTW-p	MT20	4.0	4.0	2.25 2.00
D TMB1-I	MT20	3.0	4.0	
F BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	507	0	507	0	11-10-3	11-10-3
D	507	0	507	0	11-10-3	11-10-3
F	576	0	576	0	11-10-3	11-10-3

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	374	217 / 0	60 / 0	0 / 0	0 / 0	96 / 0	0 / 0
D	374	217 / 0	60 / 0	0 / 0	0 / 0	96 / 0	0 / 0
F	449	187 / 0	128 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
		VERT. LOAD	LC1	MAX CSI (LC)					
FR-TO		FROM	TO			FR-TO			
A-B	0/15	-84.9	-84.9	0.04 (1)	10.00	F-C	-189/0	0.04 (1)	
B-H	-143/0	-84.9	-84.9	0.18 (1)	6.25	G-H	-576/58	0.00 (1)	
H-C	-251/0	-84.9	-84.9	0.38 (1)	6.25	I-J	-576/58	0.00 (1)	
C-J	-251/0	-84.9	-84.9	0.38 (1)	6.25				
J-D	-143/0	-84.9	-84.9	0.16 (1)	6.25				
D-E	0/15	-84.9	-84.9	0.04 (1)	10.00				
B-G	0/208	-38.5	-38.5	0.32 (1)	10.00				
G-F	0/208	-38.5	-38.5	0.40 (2)	10.00				
F-I	0/208	-38.5	-38.5	0.40 (2)	10.00				
I-D	0/208	-38.5	-38.5	0.32 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.38/1.00 (C-H:1), BC=0.40/1.00 (F-G:2), WB=0.04/1.00 (C-F:1), SSI=0.48/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

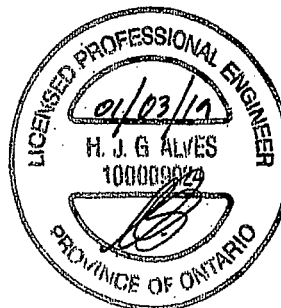
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (FL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.40 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



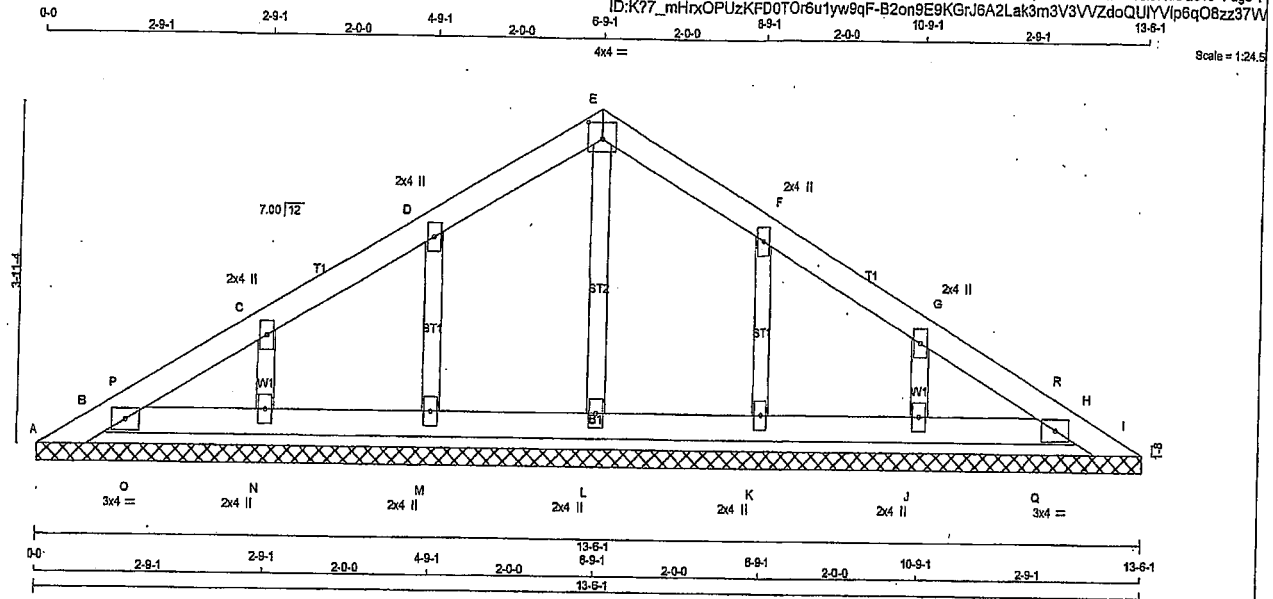
DESIGN NO. TAM 11900096
STRUCTURAL
FOR PERMIT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
259673	P5G	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Thu Jan 3 13:57:33 2019 Page 1

ID:K77_mHrxOPUzKFD0TOr6u1yw9qF-B2on9E9KGrJ6A2Lak3m3V3VVZdoQUIVip6qO8zz37W



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - E 2x4 DRY

E - I 2x4 DRY

B - H 2x4 DRY

ALL WEBS 2x3 DRY

ALL GABLE WEBS 2x3 DRY

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD

BEARING AND FIRST DIAGONAL OR VERTICAL WEB

SHALL NOT EXCEED 0.5 INCHES.

GABLE STUDS SPACED AT 2-0-0 OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS

CHORD AT JT(S): L, M, K, N, J

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. FACTORED (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)
FR-TO						FR-TO			
A-B	0/11	-102.4	-102.4	0.03 (1)	10.00	L-E	-121/0	0.03 (1)	
B-P	-73/0	-84.9	-84.9	0.03 (1)	6.25	M-D	-193/0	0.03 (1)	
P-C	-29/0	-84.9	-84.9	0.03 (1)	6.25	K-F	-193/0	0.03 (1)	
C-D	-24/0	-84.9	-84.9	0.05 (1)	6.25	N-C	-155/0	0.02 (1)	
D-E	-28/0	-84.9	-84.9	0.05 (1)	6.25	J-G	-155/0	0.02 (1)	
E-F	-28/0	-84.9	-84.9	0.05 (1)	6.25	O-P	0/35	0.00 (1)	
F-G	-24/0	-84.9	-84.9	0.05 (1)	6.25	Q-R	0/35	0.00 (1)	
G-H	-29/0	-84.9	-84.9	0.03 (1)	6.25				
H-I	-73/0	-84.9	-84.9	0.03 (1)	6.25				
	0/11	-102.4	-102.4	0.03 (1)	10.00				
B-O	0/33	-38.5	-38.5	0.02 (1)	10.00				
O-N	0/33	-38.5	-38.5	0.03 (2)	10.00				
N-M	0/23	-38.5	-38.5	0.03 (3)	10.00				
M-L	0/17	-38.5	-38.5	0.03 (3)	10.00				
L-K	0/17	-38.5	-38.5	0.03 (3)	10.00				
K-J	0/23	-38.5	-38.5	0.03 (3)	10.00				
J-Q	0/33	-38.5	-38.5	0.03 (2)	10.00				
Q-H	0/33	-38.5	-38.5	0.02 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL

OR SMALL BUILDING REQUIREMENTS OF

PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012

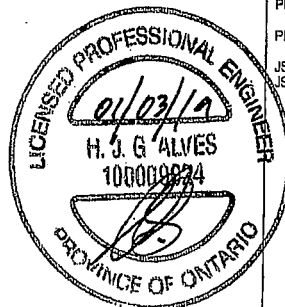
- CSA 086-08, CSA 086-14

- TPIC 2011, TPIC 2014

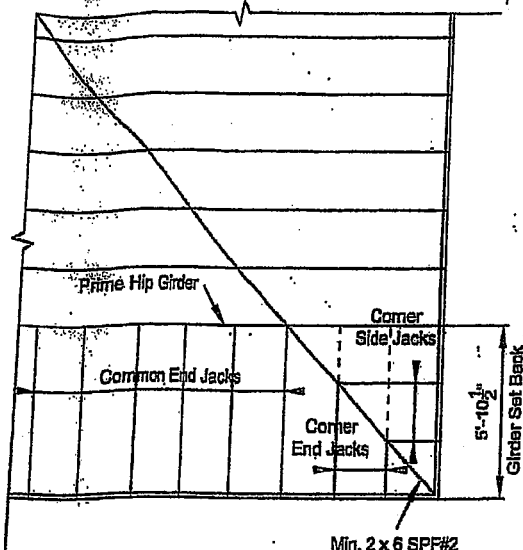
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.

RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED

ROOF LIVE LOAD



DWG NO. TAM 71900097
STRUCTURAL
COMPLETION ONLY



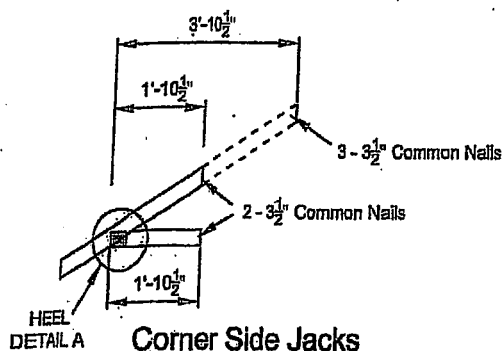
45° Hip End

LUMBER SPECIFICATION

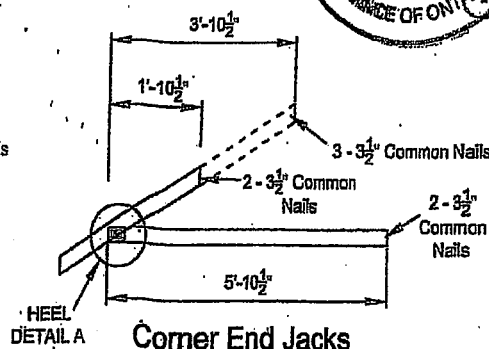
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

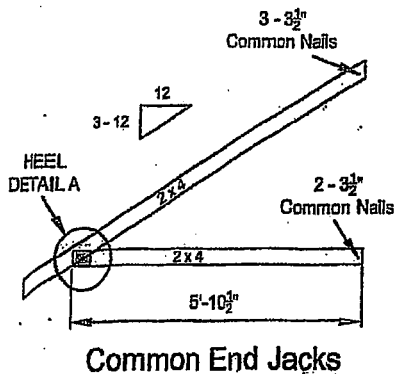
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 50.5 P.S.F.



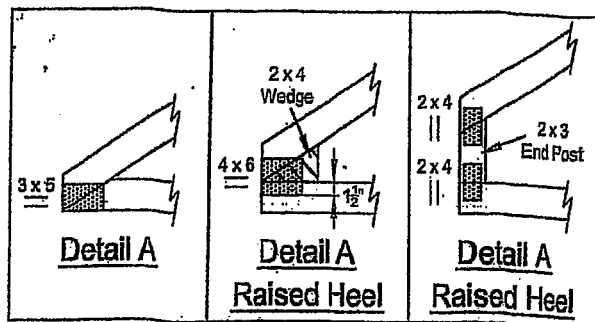
Corner Side Jacks



Corner End Jacks

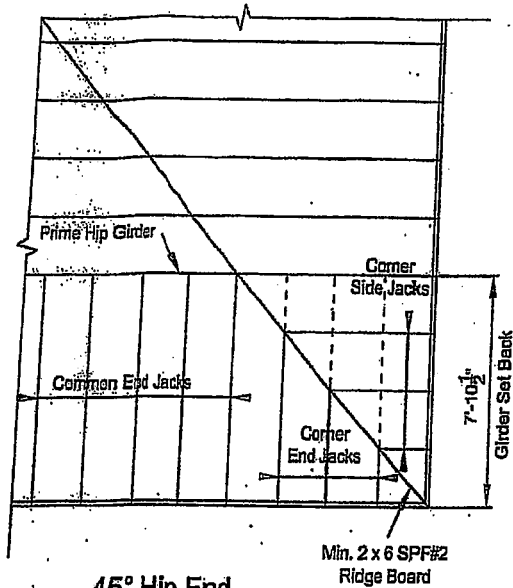


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



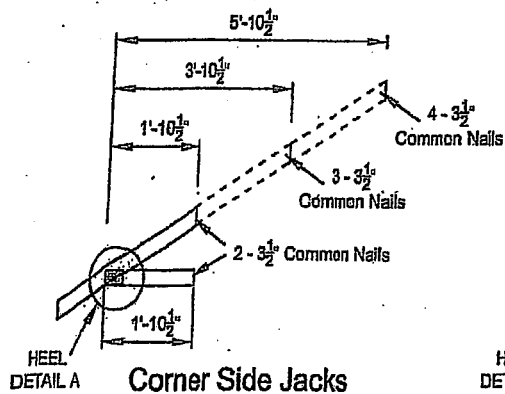
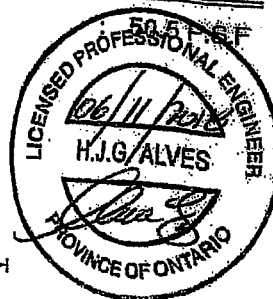
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

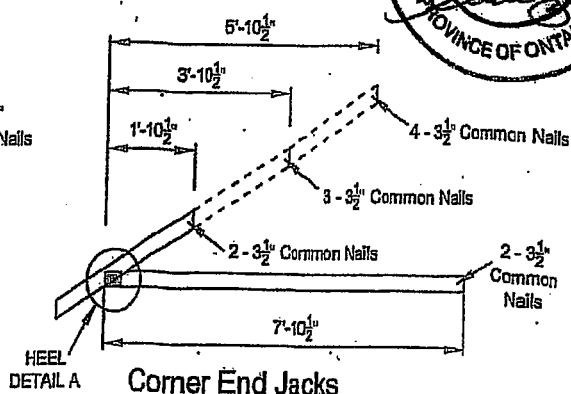
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

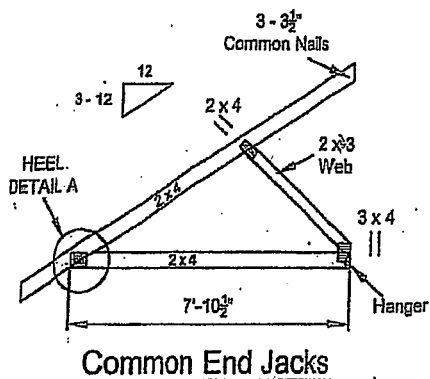
TOTAL LOAD



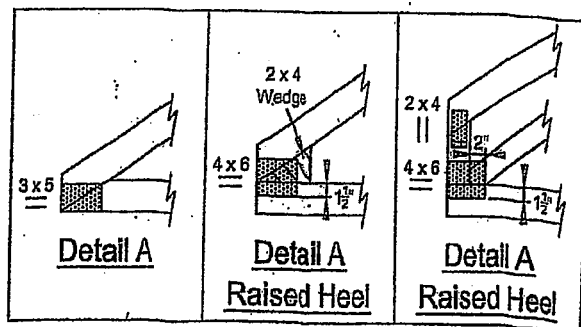
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

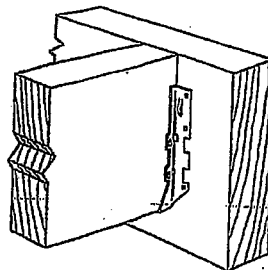
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

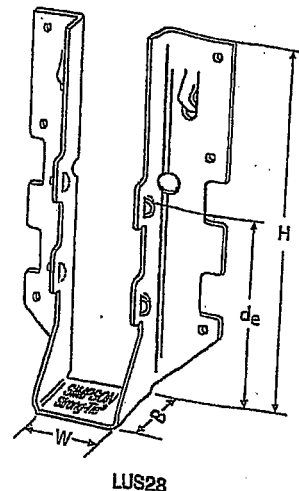
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

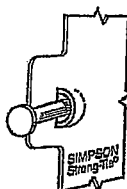


Typical LUS Installation

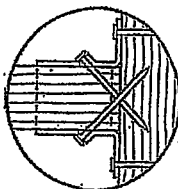


Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D.Fir-L		S.P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LUS24	18	1 1/2	3 1/2	1 1/2	1 1/2	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/2	3 1/2	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/2	4 1/2	1 1/2	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/2	4 1/2	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/2	4 1/2	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/2	6 1/2	1 1/2	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/2	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/2	6 1/2	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/2	7 1/2	1 1/2	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/2	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/2	8 1/2	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

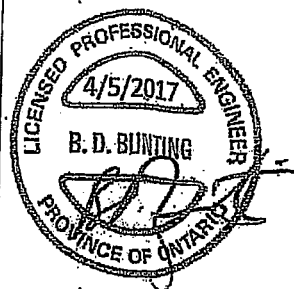
1. d_s is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,609,580



Double Shear Nailing Top View.



UNITED STATES DESIGN

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(800) 999-5099
strongtie.com

HUS/LJS -- Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

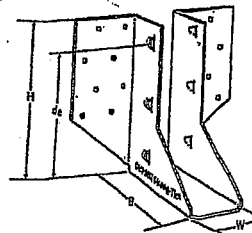
- Factored resistances are in accordance with CSA 086-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

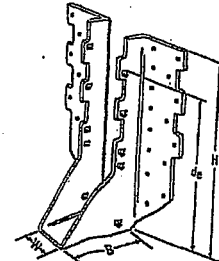
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

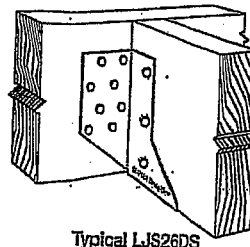
- See current catalogue for options



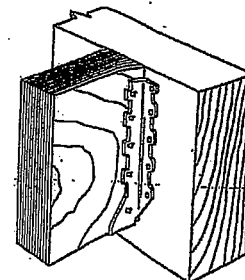
LJS26DS



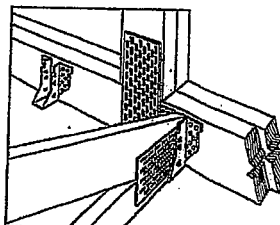
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



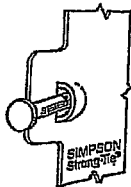
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

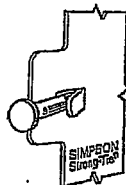
Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	d ₂	Face	Joist	D.Fr-L		S-P-F	
									Uplift (K _c =1.15)	Normal (K _c =1.00)	Uplift (K _c =1.15)	Normal (K _c =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	(6) 16d	2055	4285	1460	4115
HUS26	18	1 1/8	5 1/2	3	3 15/16	(14) 16d	(6) 16d	(6) 16d	2705	4940	2085	3875
HUS28	18	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	(8) 16d	3605	5365	2675	4345
HUS210	18	1 1/8	9 1/2	3	7 3/4	(30) 16d	(10) 16d	(10) 16d	4505	6785	4010	4740
HUS1.81/10	18	1 13/16	9	3	8	(30) 16d	(10) 16d	(10) 16d	4505	6450	4010	5200

1. d₂ is the distance from the seat of the hanger to the highest joist nail.

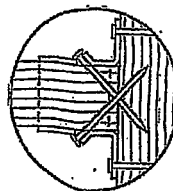


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

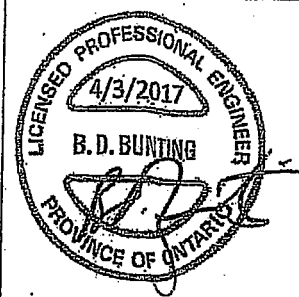
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



IMP
STATES
DESIGN

The technical information contained herein is for informational purposes only and does not constitute a contract. The user of this information assumes all liability for its use. Simpson Strong-Tie Company, Inc. is not responsible for any errors or omissions in this information. Simpson Strong-Tie Company, Inc. is a registered provider of continuing education for the International Board of Standards and Practices for Certified Engineers (IBSE).

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HGUS - Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

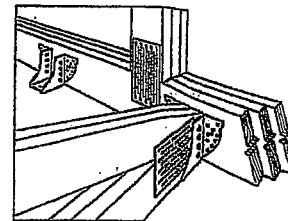
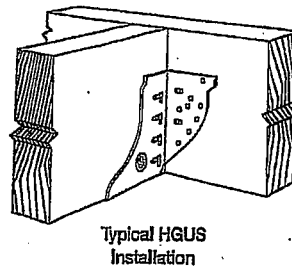
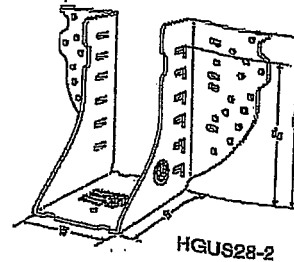
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

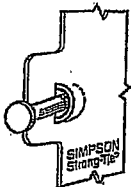
Options:

- See current catalogue for options

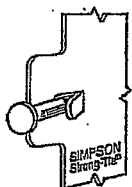


Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹		Face	Joist	D.Fir-L		S-P-F	
									Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1 5/8	5 3/4	5	4 1/2		(20) 16d	(8) 16d	2885	6625	2885	5700
HGUS26-2	12	3 1/8	5 7/8	4	4 1/8		(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8		(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 3/8	5 7/8	4	4 1/8		(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8		(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8		(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8		(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 3/8	7 1/8	4	6 1/8		(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8		(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8		(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 3/8	9 1/8	4	8 1/8		(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	8 3/8	10 3/8	4	10 3/8		(56) 16d	(20) 16d	7840	14995	5425	10645
HGUS214-4	12	6 3/8	12 3/8	4	11 3/8		(66) 16d	(22) 16d	10130	16400	7195	11645

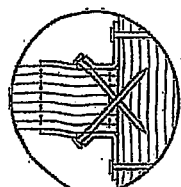
1. d_g is the distance from the seat of the hanger to the highest joist nail.



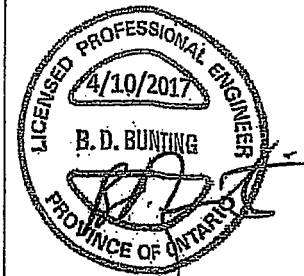
U.S. Patent
5,603,580



Double Shear Nailing Top View.



Double Shear Nailing Top View.



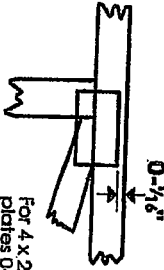
U.S. PATENT
5,603,580

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Symbols

PLATE LOCATION AND ORIENTATION



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



*Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

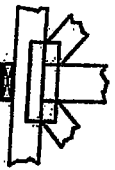
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use L, I or Eliminator bracing if indicated.

BEARING

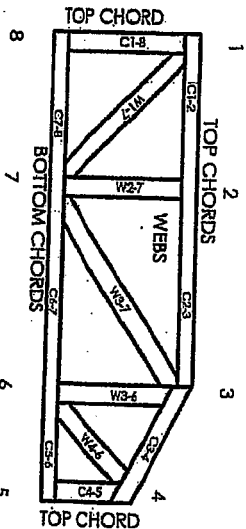


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

For Design Procedures and Specifications DSB-89:
For Light Metal Plate Connected Wood Trusses Design Standard for Bracing
Building Component Safety Information, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

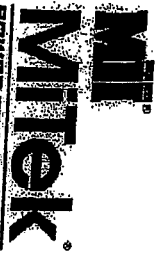
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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For many truss spacing, individual lateral bracing members may require bracing, or alternative L, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TFC.
7. Design assumes trusses will be suitably protected from the environment in accord with TFC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (text, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TFC Quality Criteria.

Mitek Engineering Reference Sheet: MIL-7470C rev. 10-08



H - Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14

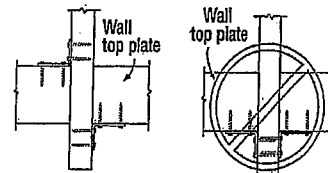
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

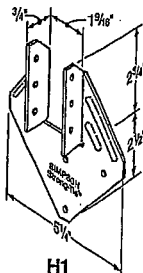
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

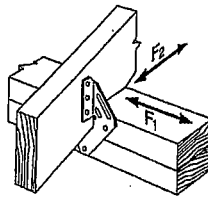


Install diagonally across from each other for minimum 2x truss.

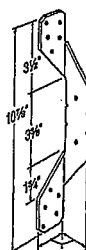
Nailing into both sides of a single ply 2x truss may cause the wood to split.



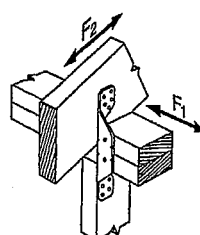
H1



H1 Installation



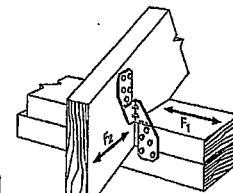
H2A



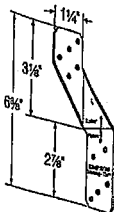
H2A Installation



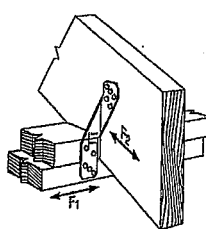
H2.5A



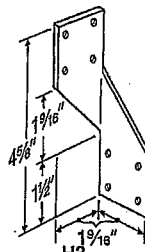
H2.5A Installation



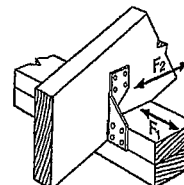
H2.5T



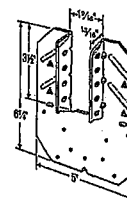
H2.5T Installation
(Nails into both top plates)



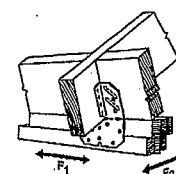
H3



H3 Installation



H10A



H10A Installation

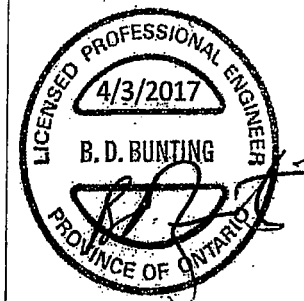
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
						(K _u =1.15)			(K _u =1.15)	
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



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CP Crush Plates – Bearing Enhancers



The CP transfers load from the truss or girder to plates for bearing limited conditions. Replaces nail-on scabs or in some cases, an additional ply when needed for bearing.

Material: See table

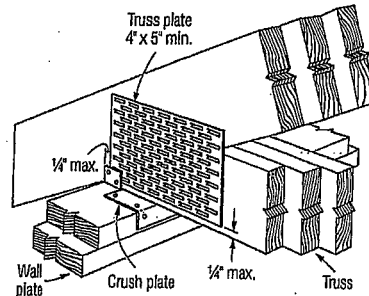
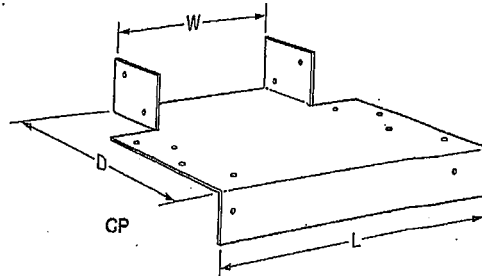
Finish: G90 galvanized

Design:

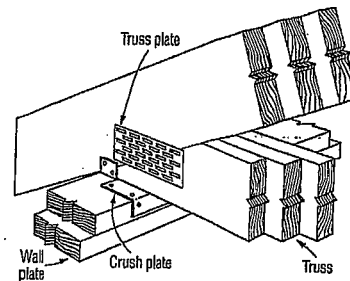
- Factored resistances are in accordance with CSA O86-14 assuming Q_r/A_b and $Q_r'/A_b' = 812$ psi for D.Fir-L and 615 psi for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 when compression loads are applied to both sides of truss chord members at bearing locations.

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- For Case 1, truss plates must be located a maximum of 1/4" from the underside of the truss chord and a maximum of 1/4" from the edge of the wall plates in accordance with the reinforcing requirements of 7.5.9 TPIC 2014.



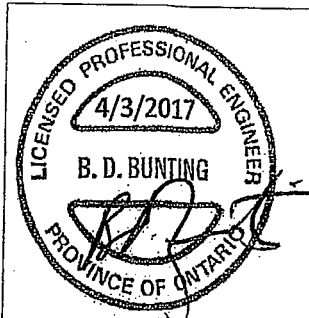
Case 1



Case 2

Model No.	Ga.	Wall Plate	Dimensions (in.)			Fasteners		Factored Resistance (lb.)			
			W	D	L	Wall Plate	Truss	D.Fir-L		S-P-F	
								Uplift	Bearing	Uplift	Bearing
CASE 1 (Truss Plate Reinforcement)											
CP1-4	20	2x4	1½	3½	4½	(6) 10d	(4) 10d x 1½"	225	5965	225	4515
CP2-4	16		¾	3½	5¾	(6) 10d	(4) 10d x 1½"	225	11390	225	9030
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	17895	225	13545
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	23860	225	18065
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	9370	225	7095
CP2-6	16		¾	5½	5¾	(10) 10d	(4) 10d x 1½"	225	18740	225	14190
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	28110	225	21285
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	37495	225	28390
CASE 2 (No Reinforcement)											
CP1-4	20	2x4	1½	3½	4½	(6) 10d	(4) 10d x 1½"	225	4685	225	3550
CP2-4	16		¾	3½	5¾	(6) 10d	(4) 10d x 1½"	225	9370	225	7100
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	14055	225	10850
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	18750	225	14195
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	7365	225	5575
CP2-6	16		¾	5½	5¾	(10) 10d	(4) 10d x 1½"	225	14730	225	11150
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	22095	225	16725
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	29460	225	22305

1. Factored bearing resistances assume wall plate and truss are the same species. For a mixed species system use S-P-F values.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.
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TS/LTS/MTS/HTS Twist Straps

SIMPSON
Strong-Tie

Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3" bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

MATERIAL: LTS-18 gauge; MTS-16 gauge; HTS-14 gauge; TS-16 gauge
FINISH: Galvanized. Some products available in stainless steel and ZMAX®; see Corrosion Information, pages 14-17.

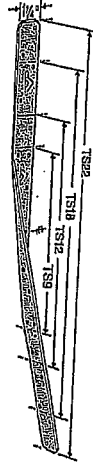
INSTALLATION: Use all specified fasteners. See General Notes.

- TS should be installed in pairs to reduce eccentricity.
- When LTS/MTS is installed as Truss-to-Top Plate tie, install 3-10d x 1½" nails to the u/s of the plate and 3-10d x 1½" nails into the edge of the double top plate.

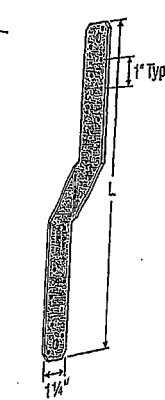
These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Model No.	L (in)	Fasteners (Total)	Factored Resistance (K _n = 1.15)	
			D-Fir lbs	S-P-F lbs
TS9	9	8-16d	1125	1040
TS12	11½	10-16d	1410	1300
TS18	17½	14-16d	1870	1820
TS22	21½	18-16d	2125	2125
MTS12	12	12-10d x 1½"	1015	720
MTS16	16	12-10d x 1½"	1015	720
MTS18	18	12-10d x 1½"	1015	720
MTS20	20	12-10d x 1½"	1015	720
MTS30	30	14-10d x 1½"	1570	1180
HTS16	16	16-10d x 1½"	2050	1455
HTS20	20	24-10d x 1½"	2050	1455
HTS24	24	24-10d x 1½"	2050	1455
HTS28	28	24-10d x 1½"	2050	1455
HTS30	30	24-10d x 1½"	2050	1455



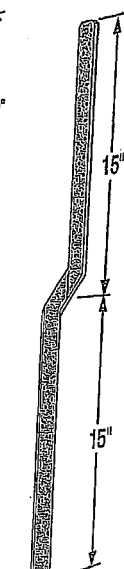
TS



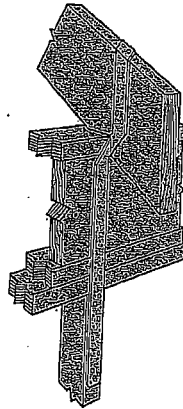
LTS12
(MTS and HTS similar)



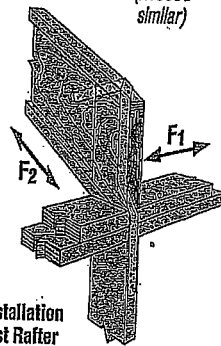
MTS30
(HTS30 similar)



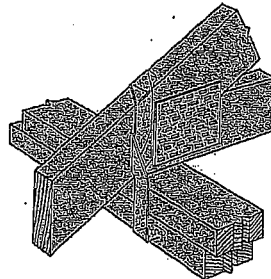
MTS30C
(HTS30C similar)



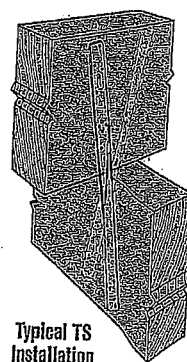
Typical MTS30 Installation



MTS30 Installation with I-joist Rafter



LTS/MTS Installation as a Truss-to-Top Plate Tie (See note 7)



Typical TS Installation

1. LTS12 thru LTS20, MTS16 through MTS30, HTS24 through HTS30C (except HTS30) have additional nail holes.
2. Install half of tie fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
4. All straps except the MTS30 and HTS30 have the twist in the center of the strap.
5. Twist straps do not have to be wrapped over the truss to achieve the load.
6. Optional nail holes are provided.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

Feb 09, 2018

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information		Application number:	
Building number, street name		Unit no. UNITS 1 - 6	Lot/con. BLOCK 6
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description	
B. Individual who reviews and takes responsibility for design activities			
Name SAM KATSOULAKOS		Firm MICRO CITY ENGINEERING SERVICES INC.	
Street address R.R #1, PO BOX 61		Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750	Cell number	
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]			
☐ House ☐ Small Buildings ☐ Large Buildings ☐ Complex Buildings ☐ HVAC - House ☐ Building Services ☐ Detection, Lighting and Power ☐ Fire Protection ☒ Building Structural ☐ Plumbing - House ☐ Plumbing - All Buildings ☐ On-site Sewage Systems			
Description of designer's work <u>ROYAL PINE HOMES - FORESTSIDE PH6 - BLOCK 6 - UNITS 1 - 6</u> (SCHEDULE IS ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM1145-19 DATED 1-18-19). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.			
D. Declaration of Designer			
I, <u>SAM KATSOULAKOS</u> declare that (choose one as appropriate): (print name) ☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories. Individual BCIN: <u>26064</u> Firm BCIN: <u>29991</u> ☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code. Individual BCIN: _____ Basis for exemption from registration: _____ ☐ The design work is exempt from the registration and qualification requirements of the Building Code. Basis for exemption from registration and qualification: _____			
I certify that: 1. The information contained in this schedule is true to the best of my knowledge. 2. I have submitted this application with the knowledge and consent of the firm.			
Date <u>1-18-19</u>		Signature of Designer	

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d) of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG #TAM 1145
 DWG #TAM 1146

1-18-19
 1-18-19