

JANUARY 18, 2019

THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300805)
DWG #T1900144 THROUGH DWG #T1900156, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900044 THROUGH DWG #T1900047, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900039 AND DWG #T1900040 AND BOTH DATED 1-03-19;
DWG #T1900035 AND DWG #T1900042 AND BOTH DATED 1-03-19;
DWG #T1901099 DATED 1-15-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS26-2 (VV)
LUS24 (O)
LJS26DS (V)

DENOTE:

C- 8/12 CATH. CEILING

T-180485

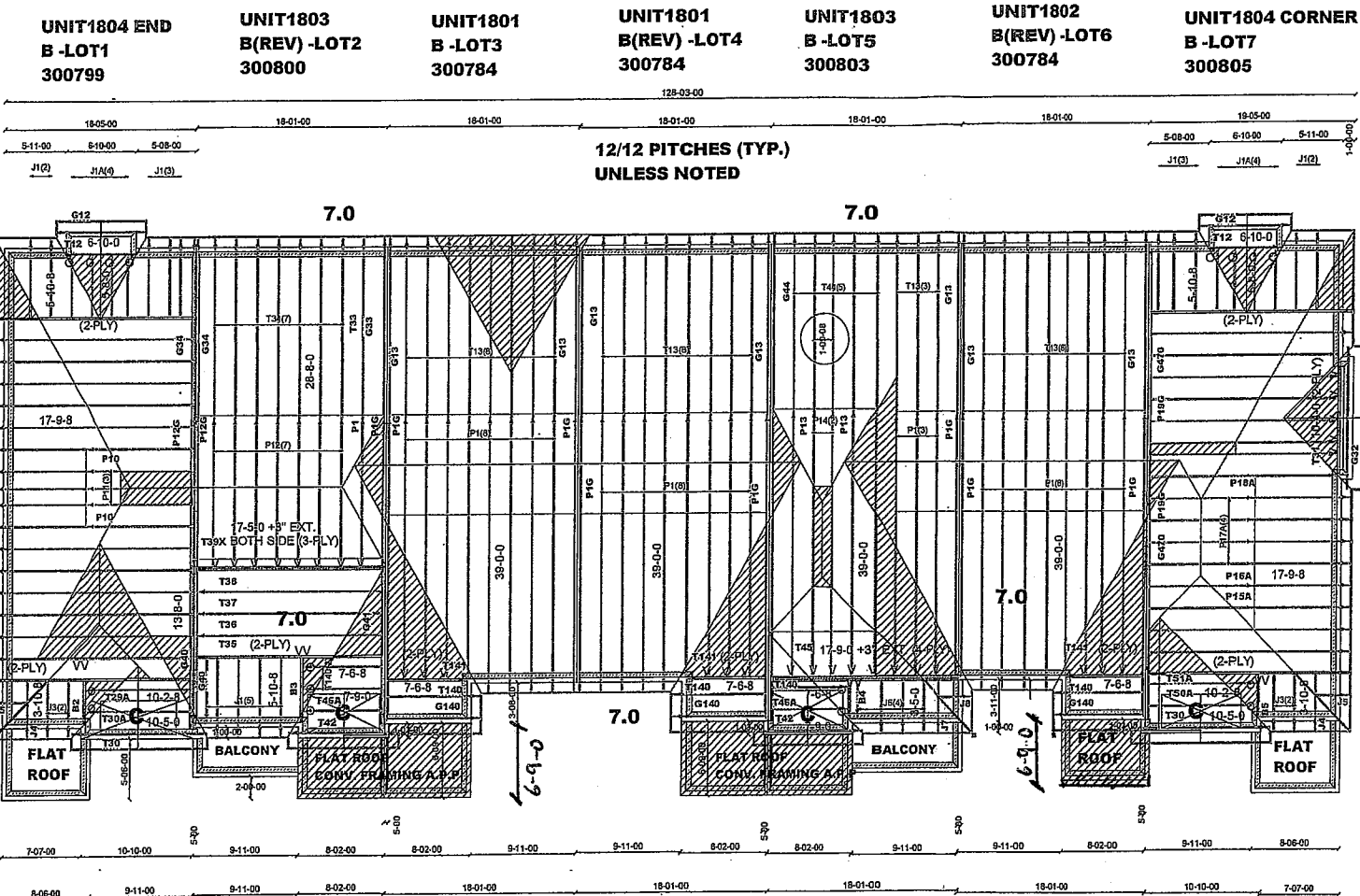
DENOTES:
CONVENTIONAL
FRAMING

JANUARY 18, 2019

THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300799)
DWG #T1900044 THROUGH DWG #T1900046, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900035; DWG #T1900039; DWG #T1900040; AND ALL DATED 1-03-19;
DWG #T1900042; DWG #T1900052; DWG #T1900114; AND ALL DATED 1-03-19;
DWG #T1900116; DWG #T1900117; DWG #T1900119; AND ALL DATED 1-03-19;
DWG #T1900120; DWG #T1900123; DWG #T1900127; AND ALL DATED 1-03-19;
DWG #T1901093 AND DWG #T1901094 AND BOTH DATED 1-15-19;
DWG #T1900150 DATED 1-03-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

JANUARY 18, 2019

THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300800)
DWG #T1900123 THROUGH DWG #T1900127, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900131 THROUGH DWG #T1900136, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900063; DWG #T1900064; DWG #T1900129; AND ALL DATED 1-03-19;
DWG #T1900120; DWG #T1900121; DWG #T1900114; AND ALL DATED 1-03-19;
DWG #T1901091 DATED 1-15-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)



DATE: 1-18-19
BCIN:28064; FIRM:BCIN #29991
ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER
ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO TRUSS
INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS
WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT.
ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY THE BUILDING DESIGNER
AND/OR
PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS PRIOR TO FABRICATION TO ENSURE
ADEQUATE LOAD
CAPACITY WITH RESPECT TO THE ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION. LOT SPECIFIC SCHEDULE 1 REQUIRED FOR
EACH
INDIVIDUAL LOT.

DWG #TAM... THROUGH DWG #TAM... INCLUSIVE, DATED:
(STRUCTURAL COMPONENTS ONLY); THIS LAYOUT MUST BE READ TOGETHER WITH REFERENCED TRUSS COMPONENT DRAWINGS AND
SEALED HARDWARE DRAWINGS.

HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY.
REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING REQUIREMENTS:
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2 X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS SPECIFIED ON REFERENCED SEALED TRUSS
COMPONENT
DRAWINGS USING 2 - 3-1/4" COMMON WIRE NAILS PER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF 'X'-BRACING ALONG THESE SAME WEB
MEMBERS
AT LOCATIONS INDICATED (*) USING THE SAME NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
PROVIDE 2 X 4 #2 SPF T-BRACE ALONG FULL LENGTH OF LATERALLY BRACED WEB MEMBERS OF 1 PLY TRUSSES COMPONENTS
REQUIRING 'T-BRACE'
USING 1 ROW OF 3-1/4" COMMON WIRE NAILS AT 6" O.C. SIMILARLY PROVIDE 2 X 6 BRACE WITH 2 ROWS OF NAILING FOR 2 PLY TRUSS
COMPONENTS AND 2 X 8 BRACE WITH 3 ROWS OF NAILING FOR 3 PLY TRUSS COMPONENTS.

(1) 2 X 6 #2 SPF ON FLAT OVER PLYWOOD SHEATHING.
(2) 2 X 8 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS COMPONENTS AND AT LOCATIONS MARKED BY 'X'.
(3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O.C. WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X 4 VERTICAL SUPPORTS TO
TRUSS BASE BELOW WHERE NECESSARY).

ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE TRUSS
COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH STUDS AT
24" O.C.
ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE 2012.

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN
WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION
3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED
AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES
AND/OR CATEGORIES.
REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.
DWG #TAM
BCIN: 28064
FIRM: 29991
SEALED STRUCTURAL
COMPONENTS ONLY

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PARTS OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:
SNOW LOAD 23.3 PSF
TC DEAD 15 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

JANUARY 18, 2019

THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300784)
DWG #T1901090 THROUGH DWG #T1901092, INCLUSIVE, AND ALL DATED 1-15-19;
DWG #T1900060 AND DWG #T1900066 AND BOTH DATED 1-03-19;
DWG #T1900063 AND DWG #T1900064 AND BOTH DATED 1-03-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

JANUARY 18, 2019

THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300803)
DWG #T1900138 THROUGH DWG #T1900142, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900060; DWG #T1900066; DWG #T1900063; AND ALL DATED 1-03-19;
DWG #T1900064; DWG #T1900135; DWG #T1900136; AND ALL DATED 1-03-19;
DWG #T1901091 DATED 1-15-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)



Job Track: 41239

Layout ID: 300798

Plan Log: 98887

Builder / Location:

ROYAL PINE HOMES/ BRAMPTON

Project:

FORESTSIDE PH.6

Date: 11/15/2018

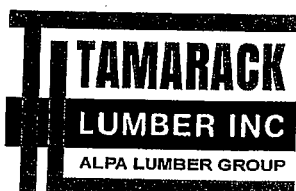
Designer: SUNNY

Model / Elevation:

BLOCK7 / LOTS1 TO7

Milek ver 7.4.2
C:\MITEK\CA742AJ
OBS\B13000000006

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR
REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK:41239

LAYOUT ID: 300799

LOCATION:

BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT

SUB-BUILDER:

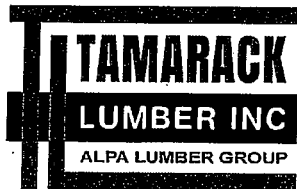
MODEL: UNIT1804 END

ELEVATION: B LOT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	G40 GABLE	7.00 0.00	13-08-00	09-03-02	2 X 4	2 X 4	00-00-00 00-00-00	01-03-07 09-03-02	68.36 43.83		
	1	G34 GABLE	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 09-07-03	171.71 109.00		
	1 2 Ply	T2 HALF HIP	12.00 0.00	17-09-08	04-08-09	2 X 4	2 X 6	01-03-08 00-00-00	01-10-08 04-08-09	199.00 121.34		
	2	T3 HALF HIP	12.00 0.00	17-09-08	05-10-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-10-09	156.36 98.00		
	2	T4 HALF HIP	12.00 0.00	17-09-08	07-00-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 07-00-09	166.94 104.66		
	2	T5 HALF HIP	12.00 0.00	17-09-08	08-02-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 08-02-09	185.34 117.34		
	7	T6 HALF HIP	12.00 0.00	17-09-08	09-04-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-04-09	737.94 462.00		
	2	T7 HALF HIP	12.00 0.00	17-09-08	10-06-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 10-06-09	208.64 127.66		
	1 2 Ply	T26A HIP GIRDER	12.00 0.00	17-09-08	05-09-00	2 X 4	2 X 6	01-03-08 00-00-00	01-10-08 02-01-00	205.94 124.00		
	1	T27A HIP	12.00 0.00	17-09-08	07-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	80.84 51.67		
	1	T29A COMMON	12.00 0.00	10-02-08	07-01-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	47.71 31.00		
	1	T30 ROOF	12.00 8.00	10-05-00	07-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	55.38 36.33		
	1	T30A ROOF	12.00 8.00	10-02-08	07-01-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	53.04 35.00		
	1	T12 COMMON	12.00 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	33.29 23.17		
	1	G12 GABLE	12.00 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	34.17 24.17		
	2	P10 PIGGYBACK	12.00 0.00	09-08-12	02-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 02-02-08	67.46 44.00		
	3	P11 PIGGYBACK	12.00 0.00	09-08-12	03-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 03-04-08	110.34 69.51		
	1	P12G PIGGYBACK	7.00 0.00	11-04-10	03-08-02	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	35.84 22.67		



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK:41239 LAYOUT ID: 300799 LOCATION:
 BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
 MODEL: UNIT1804 END ELEVATION: B LOT1

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	5	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-04-15	86.50 53.35		
	4	J1A JACK-OPEN	7.00 0.00	05-08-00	04-08-09	2 X 4	2 X 4	00-00-00 00-00-00	01-04-14 04-08-09	60.88 37.32		
	2	J3 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	29.50 18.66		
	1	J4 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 02-01-01	01-10-08 00-03-08	11.39 7.33		
	1	J5 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 00-01-01	01-10-08 00-03-08	9.10 6.00		

TOTAL # TRUSS= 46.00

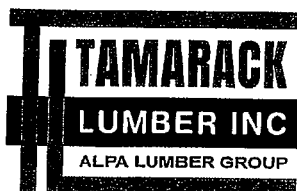
TOTAL BFT OF ALL TRUSSES=

1768.01 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2815.67 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
6	Hangers	LUS24	

TOTAL # ITEMS= 7.00



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK:41239

LAYOUT ID: 300800

LOCATION:

BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT

SUB-BUILDER:

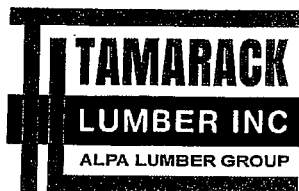
MODEL: UNIT1803

ELEVATION: B LOT2

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T33 PIGGYBACK	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 07-03-12	150.22 94.33		
	1	G33 GABLE	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 07-03-12	167.03 106.17		
	7	T34 PIGGYBACK	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 09-07-03	1055.25 659.19		
	1	G34 GABLE	7.00 0.00	28-08-00	10-00-00	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 09-07-03	171.71 109.00		
	1 2 Ply	T35 FLAT GIRDER	0.00 0.00	17-08-00	04-08-09	2 X 6	2 X 6	00-00-00 00-00-00	04-08-09 04-08-09	220.58 134.68		
	1	T36 FLAT	0.00 0.00	17-08-00	05-10-09	2 X 4	2 X 4	00-00-00 00-00-00	05-10-09 05-10-09	79.76 50.00		
	1	T37 FLAT	0.00 0.00	17-08-00	07-00-09	2 X 4	2 X 4	00-00-00 00-00-00	07-00-09 07-00-09	86.38 53.83		
	1	T38 FLAT	0.00 0.00	17-08-00	08-02-09	2 X 4	2 X 4	00-00-00 00-00-00	08-02-09 08-02-09	93.18 57.67		
	1 3 Ply	T39X FLAT GIRDER	0.00 0.00	17-05-00	09-04-09	2 X 6	2 X 6	00-00-00 00-00-00	00-05-08 00-05-08	498.06 311.01		
	1	T42 SCISSOR	12.00 8.00	07-09-00	05-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	41.86 26.83		
	1	T46A SCISSOR	12.00 8.00	07-06-08	09-05-00	2 X 4	2 X 4	00-00-00 00-00-00	01-10-08 09-05-00	46.44 29.50		
	2	T140 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	93.70 59.66		
	1	G40 GABLE	7.00 0.00	13-08-00	09-03-02	2 X 4	2 X 4	00-00-00 00-00-00	01-03-07 09-03-02	68.36 43.83		
	1	G41 GABLE	0.00 0.00	13-08-00	09-05-04	2 X 4	2 X 4	00-00-00 00-00-00	09-05-04 09-05-04	94.63 57.67		
	1	P1 PIGGYBACK	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	22.11 14.33		
	1	P1G GABLE	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	23.42 15.33		
	7	P12 PIGGYBACK	7.00 0.00	11-04-10	03-08-02	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	228.76 144.69		
	1	P12G GABLE	7.00 0.00	11-04-10	03-08-02	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	35.84 22.67		



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300800 LOCATION:
 BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
 MODEL: UNIT1803 ELEVATION: B LOT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	5	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4 2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	86.50 53.35		

TOTAL # TRUSS= 39.00

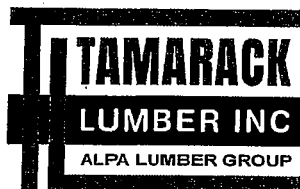
TOTAL BFT OF ALL TRUSSES=

2043.74 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3263.79 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	
8	Hangers	LJS26DS	

TOTAL # ITEMS= 12.00



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300784 LOCATION:
 BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
 MODEL: UNIT1801 OR UNIT1802 ELEVATION: B LOT3, LOT4 OR LOT6

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	8	T13 PIGGYBACK	7.00 0.00	39-00-00	10-00-00	2 X 4 2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	1640.08 1016.00		
	2	G13 GABLE	7.00 0.00	39-00-00	10-00-00	2 X 4 2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	453.88 284.66		
	1	T140 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	46.85 29.83		
	1	G140 GABLE	12.00 0.00	07-06-08	09-05-00	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	48.08 29.83		
	1 2 Ply	T141 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4 2 X 6	00-00-00 00-00-00	01-10-08 09-05-00	109.56 66.66		
	8	P1 PIGGYBACK	7.00 0.00	07-05-10	02-06-07	2 X 4 2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	176.88 114.64		
	2	P1G GABLE	7.00 0.00	07-05-10	02-06-07	2 X 4 2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	46.84 30.66		

TOTAL # TRUSS= 24.00

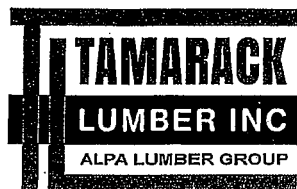
TOTAL BFT OF ALL TRUSSES=

1572.28 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2522.17 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LJS26DS	

TOTAL # ITEMS= 3.00



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300803 LOCATION:
 BUILDER: ROYAL PINE - FORESTSIDE/BRAMPTON SUB-BUILDER:
 MODEL: UNIT1803 ELEVATION: B LOT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	5	T44 PIGGYBACK	7.00 0.00	39-00-00	09-10-00	2 X 4	2 X 6	01-03-08 00-00-00	01-03-07 05-09-08	1125.15 694.15		
	1	G44 GABLE	7.00 0.00	39-00-00	09-10-00	2 X 4	2 X 6	01-03-08 00-00-00	01-03-07 05-09-08	245.57 155.00		
	3	T13 PIGGYBACK	7.00 0.00	39-00-00	10-00-00	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	615.03 381.00		
	1	G13 GABLE	7.00 0.00	39-00-00	10-00-00	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	226.94 142.33		
	1 4 Ply	T45 HALF HIP	12.00 0.00	17-09-00	05-03-08	2 X 6	2 X 6	00-00-00 00-00-00	01-10-08 00-05-08	459.52 296.00		
	1	T42 SCISSOR	12.00 8.00	07-09-00	05-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	41.86 26.83		
	1	T46A SCISSOR	12.00 8.00	07-06-08	09-05-00	2 X 4	2 X 4	00-00-00 00-00-00	01-10-08 09-05-00	46.44 29.50		
	1	T140 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	46.85 29.83		
	1	P1G GABLE	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	23.42 15.33		
	3	P1 PIGGYBACK	7.00 0.00	07-05-10	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	66.33 42.99		
	2	P13 PIGGYBACK	7.00 0.00	18-11-01	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-05-03	122.48 76.66		
	2	P14 PIGGYBACK	7.00 0.00	18-11-01	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-05 00-05-03	129.72 81.34		
	4	J6 JACK-OPEN	12.00 0.00	03-05-00	05-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-03-08	53.96 34.68		
	1	J7 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 01-07-09	01-10-08 00-03-08	10.86 7.33		
	1	J8 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 00-01-01	01-10-08 00-03-08	9.10 6.00		

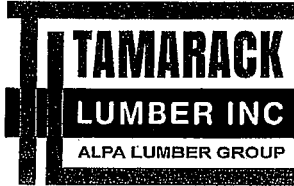
TOTAL # TRUSS= 31.00

TOTAL BFT OF ALL TRUSSES=

2018.97 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3223.23 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
8	Hangers	LJS26DS	



Delivery Shiplist

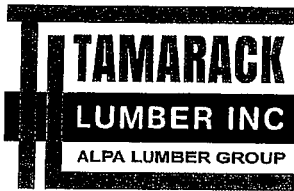
DATE	09/13/18
SALES REP	Mario

JOB TRACK: 41239	LAYOUT ID: 300803	LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT	SUB-BUILDER:	
MODEL: UNIT1803	ELEVATION: B LOT5	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	LUS24	

TOTAL # ITEMS= 11.00



Delivery Shiplist

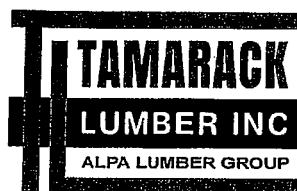
DATE	09/13/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300805 LOCATION:
 BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
 MODEL: UNIT1804 CORNER ELEVATION: B LOT7

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	G470 GABLE	7.00 0.00	19-06-00	10-00-00	2 X 4	2 X 4	00-00-00 00-00-00	01-03-07 10-00-00	221.50 140.00		
	1 2 Ply	T47A HALF HIP	12.00 0.00	17-09-08	05-09-00	2 X 4	2 X 6	01-03-08 00-00-00	01-10-08 05-09-00	215.00 128.00		
	1	T48A HALF HIP	12.00 0.00	17-09-08	07-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 07-09-00	86.80 55.00		
	8	T49A PIGGYBACK	12.00 0.00	17-09-08	09-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-09-00	860.72 541.36		
	1 2 Ply	T2 HALF HIP	12.00 0.00	17-09-08	04-08-09	2 X 4	2 X 6	01-03-08 00-00-00	01-10-08 04-08-09	199.00 121.34		
	1	T3 HALF HIP	12.00 0.00	17-09-08	05-10-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-10-09	78.18 49.00		
	1	T4 HALF HIP	12.00 0.00	17-09-08	07-00-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 07-00-09	83.47 52.33		
	1	T5 HALF HIP	12.00 0.00	17-09-08	08-02-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 08-02-09	92.67 58.67		
	1	T6 HALF HIP	12.00 0.00	17-09-08	09-04-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-04-09	105.42 66.00		
	1	T7 HALF HIP	12.00 0.00	17-09-08	10-06-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 10-06-09	104.32 63.83		
	2	T8 HALF HIP	12.00 0.00	17-09-08	11-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 11-08-09	233.84 144.00		
	1 2 Ply	T31 COMMON	12.00 0.00	10-04-00	07-00-08	2 X 4	2 X 6	00-00-00 00-00-00	01-10-08 01-10-08	118.10 76.00		
	1	G32 COMMON	12.00 0.00	10-04-00	07-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	55.75 36.83		
	1	T30 ROOF	12.00 8.00	10-05-00	07-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	55.38 36.33		
	1	T50A SCISSOR	12.00 8.00	10-02-08	10-11-08	2 X 4	2 X 4	00-00-00 00-00-00	01-10-08 09-10-00	79.07 51.33		
	1	T51A MONOPITCH	12.00 0.00	10-02-08	10-11-08	2 X 4	2 X 4	00-00-00 00-00-00	01-10-08 09-10-00	74.08 46.83		
	1	T12 COMMON	12.00 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	33.29 23.17		
	1	G12 GABLE	12.00 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	34.17 24.17		



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300805 LOCATION:
 BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
 MODEL: UNIT1804 CORNER ELEVATION: B LOT7

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	P15A PIGGYBACK	12.00 0.00	08-10-10	01-10-08	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	30.13 19.00		
	1	P16A PIGGYBACK	12.00 0.00	08-10-10	03-10-08	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	33.58 21.33		
	4	P17A PIGGYBACK	12.00 0.00	08-10-10	04-10-08	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	118.36 76.68		
	1	P18A PIGGYBACK	12.00 0.00	08-10-10	03-07-06	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	32.95 22.00		
	2	P19G GABLE	7.00 0.00	03-08-13	02-06-07	2 X 4 2 X 4	00-00-00 00-00-00	00-00-00 02-07-15	24.90 16.00		
	5	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4 2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	86.50 53.35		
	4	J1A JACK-OPEN	7.00 0.00	05-08-00	04-08-09	2 X 4 2 X 4	00-00-00 00-00-00	01-04-14 04-08-09	60.88 37.32		
	2	J3 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	29.50 18.66		
	1	J4 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4 2 X 4	01-03-08 02-01-01	01-10-08 00-03-08	11.39 7.33		
	1	J5 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4 2 X 4	01-03-08 00-01-01	01-10-08 00-03-08	9.10 6.00		

TOTAL # TRUSS= 52.00

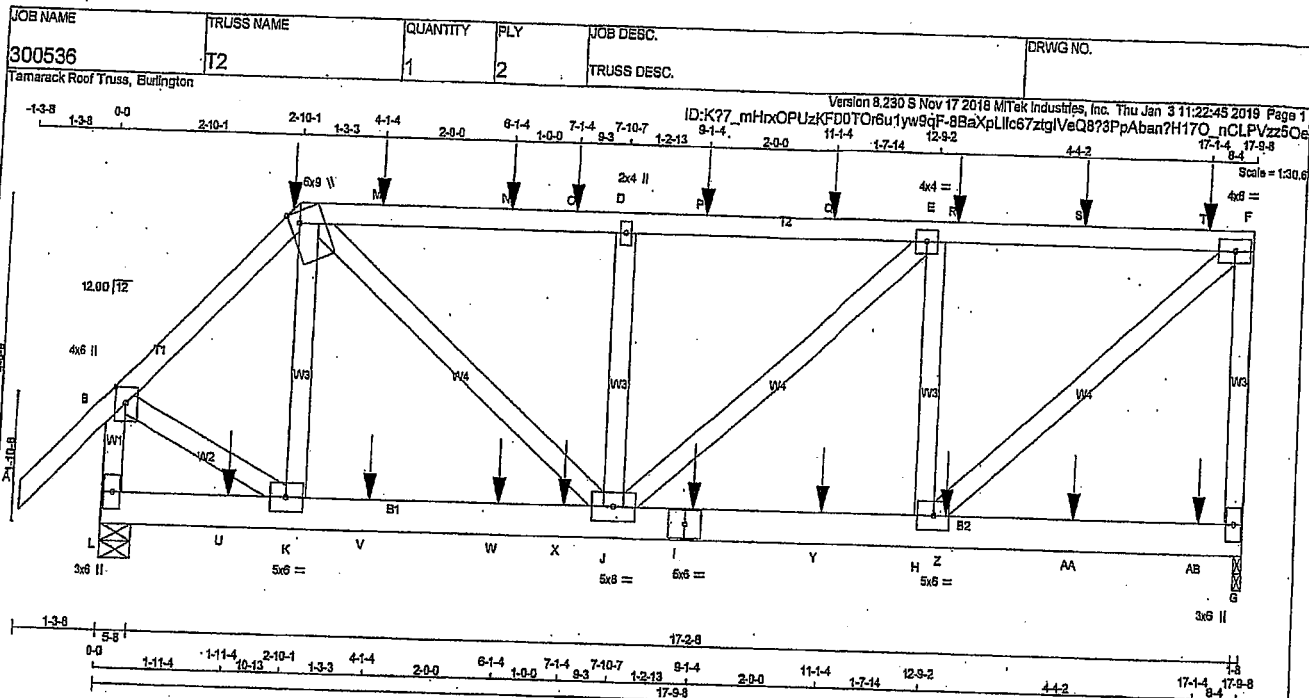
TOTAL BFT OF ALL TRUSSES=

1991.86 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3168.05 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 6.00



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - I	2x6	DRY	No.2	SPF	
I - G	2x6	DRY	No.2	SPF	

ALL WEBS 2x4 DRY SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (in)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C 1 12		SIDE (81.0)
C-F 1 12		SIDE (81.0)
F-G 1 12		TOP
L-B 1 12		TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
L-I 2 12		SIDE (0.0)
I-G 2 12		SIDE (163.1)
WEBS: (0.122"x3") SPIRAL NAILS		
2x4 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+P	MT20	4.0	6.0	2.75	2.00
C	TTVW+m	MT20	6.0	9.0	Edge	1.75
D	TMVW+	MT20	2.0	4.0		
F	TMVW+	MT20	4.0	4.0		
H	BMVW+	MT20	3.0	6.0		
I	BS+	MT20	5.0	6.0		
J	BMVW+	MT20	5.0	6.0		
K	BMVW+	MT20	5.0	6.0		
L	BMVW+	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
G	2244	0	0	1-8
L	2221	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
G	1701	850/0	379/0	0/0	0/0	472/0
L	1676	862/0	365/0	0/0	0/0	459/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. (PLF)	FACTORED MAX. CSI (LC)	UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0/142					K-C	-274/105	0.03 (1)	
B-C	-1854/0	-84.9	-84.9	0.07 (1)	10.00	C-J	0/1431	0.13 (1)	
C-M	-2381/0	-84.9	-84.9	0.08 (1)	8.25	J-D	0/1431	0.13 (1)	
M-N	-2381/0	-84.9	-84.9	0.37 (1)	5.35	E-E	0/1431	0.13 (1)	
N-O	-2381/0	-84.9	-84.9	0.37 (1)	5.35	H-H	0/1431	0.13 (1)	
O-D	-2381/0	-84.9	-84.9	0.37 (1)	5.35	F-F	0/1431	0.13 (1)	
D-P	-2381/0	-84.9	-84.9	0.38 (1)	5.35	G-G	0/1431	0.13 (1)	
P-Q	-2381/0	-84.9	-84.9	0.38 (1)	5.35	B-B	0/1431	0.13 (1)	
Q-E	-2381/0	-84.9	-84.9	0.38 (1)	5.35				
E-R	-1959/0	-84.9	-84.9	0.38 (1)	5.35				
R-S	-1959/0	-84.9	-84.9	0.32 (1)	5.35				
S-T	-1959/0	-84.9	-84.9	0.32 (1)	5.35				
T-F	-1959/0	-84.9	-84.9	0.32 (1)	5.35				
G-F	-2065/0	-84.9	-84.9	0.32 (1)	5.35				
L-B	-2185/0	0.0	0.0	0.35 (1)	7.53				
		0.0	0.0	0.13 (1)	7.53				
L-U	0/0	-38.5	-38.5	0.07 (3)	10.00				
U-K	0/0	-38.5	-38.5	0.07 (3)	10.00				
K-V	0/1294	-38.5	-38.5	0.14 (2)	10.00				
V-W	0/1294	-38.5	-38.5	0.14 (2)	10.00				
W-X	0/1294	-38.5	-38.5	0.14 (2)	10.00				
X-J	0/1294	-38.5	-38.5	0.14 (2)	10.00				
J-I	0/1958	-38.5	-38.5	0.19 (2)	10.00				
I-Y	0/1958	-38.5	-38.5	0.19 (2)	10.00				
Y-H	0/1958	-38.5	-38.5	0.19 (2)	10.00				
H-Z	0/0	-38.5	-38.5	0.08 (3)	10.00				
Z-AA	0/0	-38.5	-38.5	0.08 (3)	10.00				
AA-AB	0/0	-38.5	-38.5	0.08 (3)	10.00				
AB-G	0/0	-38.5	-38.5	0.08 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	2-10-1	-35	-39	FRONT	VERT	DEAD		
C	2-10-1	-182	-162	FRONT	VERT	SNOW		
I	9-1-4	-71	-80	BACK	VERT	TOTAL		
M	4-1-4	-164	-164	BACK	VERT	TOTAL		
N	6-1-4	-156	-156	BACK	VERT	TOTAL		
O	7-1-4	-156	-156	BACK	VERT	TOTAL		
P	9-1-4	-158	-158	BACK	VERT	TOTAL		
Q	11-1-4	-156	-156	BACK	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.59")
CALCULATED VERT. DEF. (LL) = L/999 (0.03")
ALLOWABLE DEF. (TL) = L/360 (0.59")
CALCULATED VERT. DEF. (TL) = L/999 (0.05")

CSI: TC=0.37/1.00 (C-D:1), 8C=0.19/1.00 (H-J:2),
WB=0.23/1.00 (F-H:1), 6SI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1667 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

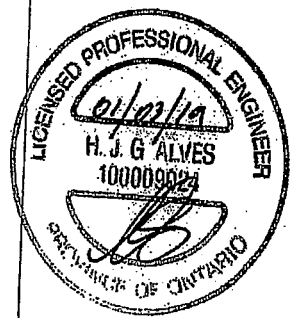
JSI GRIP = 0.77 (B) (INPUT = 0.90)
JSI METAL = 0.38 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T2	1	2	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LCI	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	13-1-4	-164	-164	---	BACK	VERT	TOTAL	---	---
S	15-1-4	-164	-164	---	BACK	VERT	TOTAL	---	---
T	17-1-4	-178	-178	---	BACK	VERT	TOTAL	---	---
U	1-11-4	-75	-95	---	BACK	VERT	TOTAL	---	---
V	4-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
W	6-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
X	7-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
Y	11-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
Z	13-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
AA	15-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
AB	17-1-4	-81	-103	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM T1900039
 STRUCTURAL
 COMPONENT ONLY 2/2

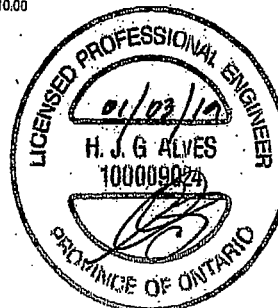
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Version 8.230 S Nov 17 2018 Mitek Industries Inc Thu Jan 3 11:22:43 2019

11-5-12

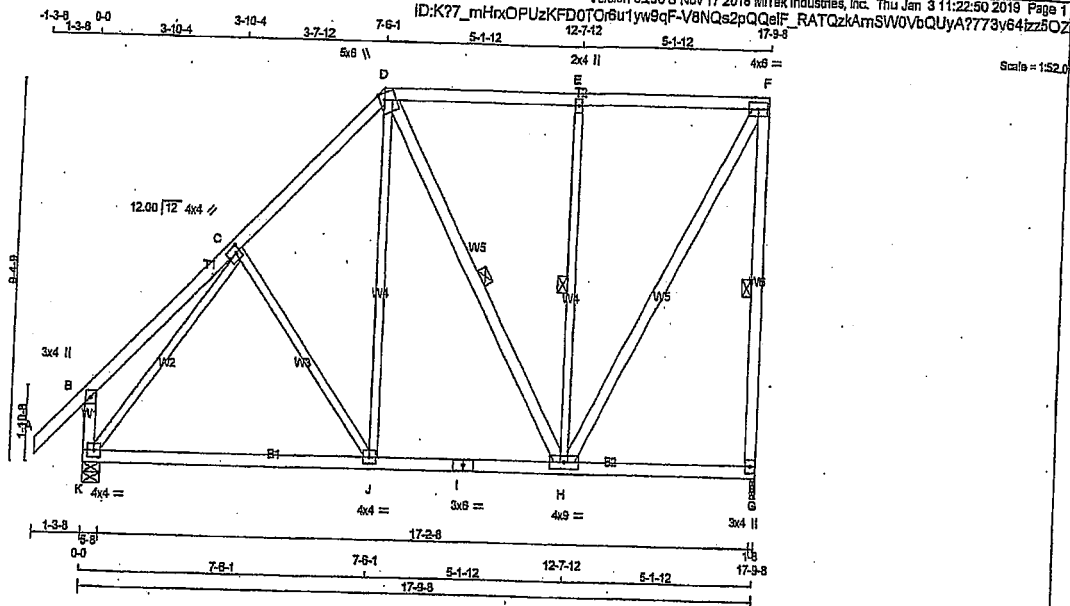


DWG NO. TAM 7190042
STRUCTURAL
COMPONENT ONLY



[illegible]

JOB NAME 300536	TRUSS NAME T6	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N L G A: RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW+t	MT20	2.0	4.0		
F	TMVW+t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW+t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW+t	MT20	4.0	4.0		
K	BMVW+t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
G	1097	0	1097	0	0
K	1216	0	1216	0	0

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
K	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM TO	CS1 (LC)	FR-TO	
A-B	0 / 42	-84.9	-84.9 0.12 (1)	C-J	-105 / 32
B-C	0 / 26	-84.9	-84.9 0.19 (1)	D-H	0 / 413
C-D	-798 / 0	-84.9	-84.9 0.20 (1)	E-H	-145 / 0
D-E	-481 / 0	-84.9	-84.9 0.38 (1)	H-F	-539 / 0
E-F	-481 / 0	-84.9	-84.9 0.38 (1)	H-F	0 / 971
G-F	-1011 / 0	0.0	0.0 0.42 (1)	K-C	-1049 / 0
K-B	-239 / 0	0.0	0.0 0.03 (1)		
K-J	0 / 607	-38.5	-38.5 0.42 (2)		
J-I	0 / 549	-38.5	-38.5 0.42 (2)		
I-H	0 / 549	-38.5	-38.5 0.42 (2)		
H-G	0 / 0	-38.5	-38.5 0.17 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/989 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/957 (0.22")

CSK TC=0.42/1.00 (F-G:1), BC=0.42/1.00 (H-K:2),
WB=0.76/1.00 (C-K:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

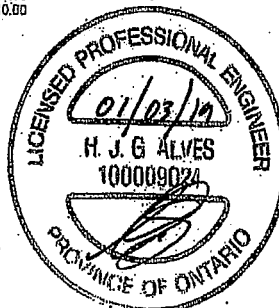
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

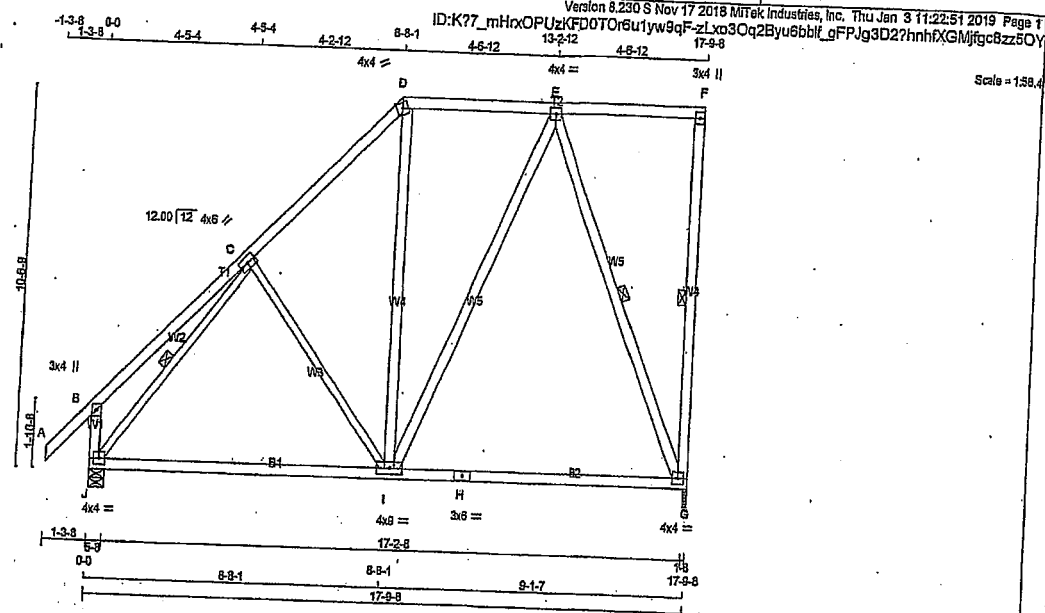
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.83 (C) (INPUT = 0.90)
JSI METAL=0.36 (C) (INPUT = 1.00)



DWG NO. TAM T1900045
STRUCTURAL
COMPONENT ONLY

JOB NAME 300536 Tamarack Roof Truss, Burlington	TRUSS NAME T7	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2

ALL WEBS 2x4 DRY No.2

EXCEPT

G - I 2x3 DRY No.2

J - C 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	4.0	6.0		
D	TMV+t	MT20	4.0	4.0		
E	TMVW+t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW+t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMVW+t	MT20	4.0	9.0		
J	BMVW+t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	1097	0	1097	0	1-8	1-8
J	1216	0	1216	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL
G	632	414/0	187/0	0/0	0/0	231/0	0/0
J	914	479/0	187/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G, C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
			FROM	TO						
A-B	0/42	-84.9	-84.9	0.12 (1)	18.00	C-I	-172/14	0.18 (1)		
B-C	0/31	-84.9	-84.9	0.28 (1)	10.00	I-D	0/280	0.04 (2)		
C-D	-753/0	-84.9	-84.9	0.28 (1)	9.25	E-G	0/397	0.06 (2)		
D-E	-512/0	-84.9	-84.9	0.30 (1)	8.25	E-G	-898/0	0.62 (1)		
E-F	0/0	-84.9	-84.9	0.29 (1)	10.00	J-C	-1034/0	0.38 (1)		
F-G	-148/0	0.0	0.0	0.08 (1)	6.25					
G-H	-255/0	0.0	0.0	0.03 (1)	7.51					
J-I	0/614	-38.5	-38.5	0.74 (2)	10.00					
I-H	0/365	-38.5	-38.5	0.72 (3)	10.00					
H-G	0/365	-38.5	-38.5	0.72 (3)	10.00					

TOTAL WEIGHT = 109 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/899 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/693 (0.31")

CS1: TC=0.30/1.00 (C-E1), SC=0.74/1.00 (I-J2), WB=0.62/1.00 (E-G1), SS1=0.22/1.00 (G-I3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

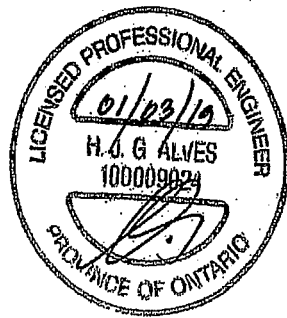
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1897 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.80 (J) (INPUT = 0.90)
JSI METAL = 0.25 (C) (INPUT = 1.00)

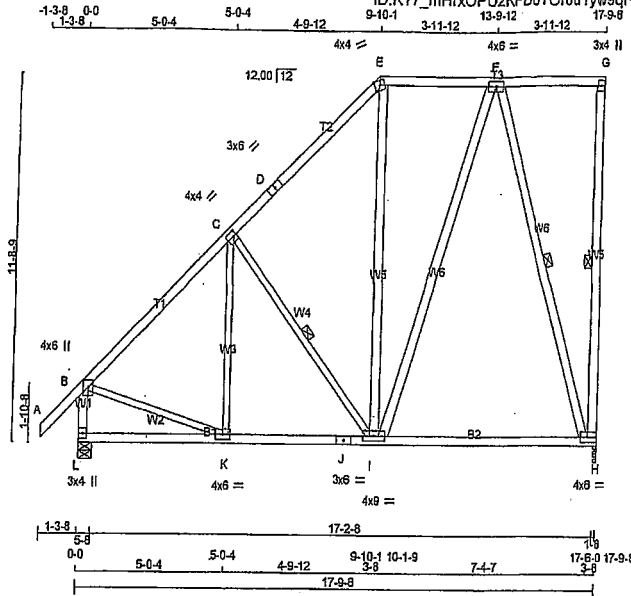


DRWG NO. TAM **T1400046**
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:67.7

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS			
EXCEPT	SIZE	LUMBER	DESCR.
K - C	2x3	DRY	No.2
C - I	2x3	DRY	No.2
B - K	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVWV-t	MT20	4.0	6.0		
I	BMVWV-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMVWV-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	1097	0	1097	0
L	1216	0	1216	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	832	414/0	187/0	0/0	0/0	231/0	0/0
L	914	479/0	187/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO				FR-TO			
A-B	0/42	-84.9	-84.9 0.12 (1)	10.00	K-C	-24/122	0.03 (3)
B-C	-887/0	-84.9	-84.9 0.29 (1)	6.18	C-I	-356/0	0.16 (1)
C-D	-857/0	-84.9	-84.9 0.27 (1)	6.25	I-E	0/195	0.03 (3)
D-E	-857/0	-84.9	-84.9 0.27 (1)	6.25	B-K	0/681	0.15 (1)
E-F	-438/0	-84.9	-84.9 0.17 (1)	6.25	I-F	0/457	0.07 (2)
F-G	0/0	-84.9	-84.9 0.22 (1)	10.00	F-H	-898/0	0.78 (1)
H-G	-128/0	0.0	0.0 0.09 (1)	6.25			
L-B	-1131/0	0.0	0.0 0.13 (1)	7.48			
L-K	0/0	-38.5	-38.5 0.17 (3)	10.00			
K-J	0/852	-38.5	-38.5 0.46 (2)	10.00			
J-I	0/852	-38.5	-38.5 0.46 (2)	10.00			
I-H	0/295	-38.5	-38.5 0.41 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF B08C 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.59")
CALCULATED VERT. DEFL. (LL) = L/898 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (0.59")
CALCULATED VERT. DEFL. (TL) = L/898 (0.24")

CSI: TC=0.29/1.00 (B-C-1), BC=0.46/1.00 (I-K-2), WB=0.76/1.00 (F-H-1), SSI=0.19/1.00 (H-I-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

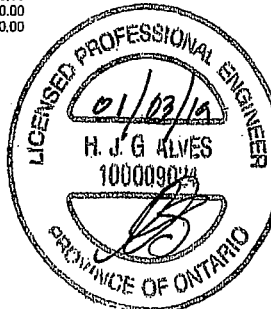
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

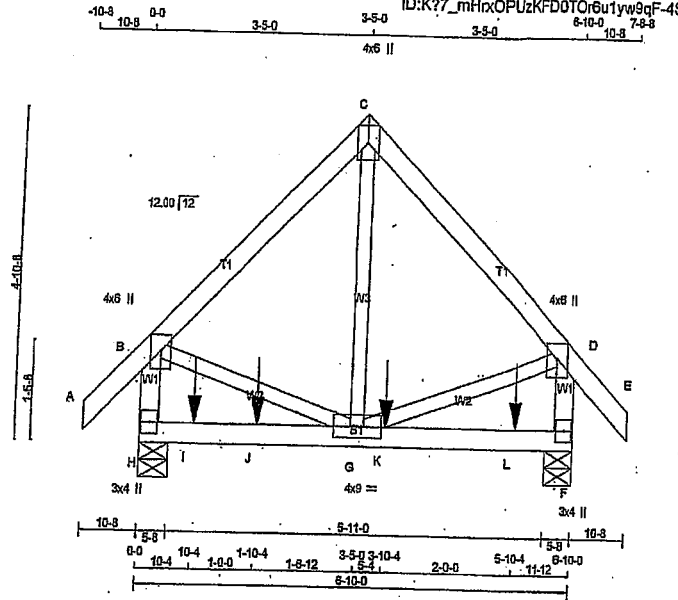
JSI GRIP= 0.81 (J) (INPUT= 0.90)
JSI METAL= 0.52 (J) (INPUT= 1.00)



DRWG NO. TAM T1900047
STRUCTURAL
COMP. DWG. ONLY

JOB NAME 300536	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 33 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+p	MT20	4.0	6.0	2.75	2.00
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW+1	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	RECORD BRG IN-SX
JT	VERT	HORZ	DOWN	HORZ
H	1188	0	1188	0
F	1070	0	1070	0

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	882	493/0	157/0	0/0	0/0	232/0	0/0
F	795	444/0	142/0	0/0	0/0	209/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/29	-84.9	-84.9 0.07 (1)	10.00	G-C	0/633	0.16 (1)
B-C	-653/0	-84.9	-84.9 0.20 (1)	6.25	B-G	0/485	0.12 (1)
C-D	-653/0	-84.9	-84.9 0.20 (1)	6.25	G-D	0/485	0.12 (1)
D-E	0/29	-84.9	-84.9 0.07 (1)	10.00			
H-B	-839/0	0.0	0.0 0.10 (1)	7.81			
F-D	-839/0	0.0	0.0 0.10 (1)	7.81			
H-I	0/0	-38.5	-38.5 0.49 (1)	10.00			
I-J	0/0	-38.5	-38.5 0.49 (1)	10.00			
J-G	0/0	-38.5	-38.5 0.49 (1)	10.00			
G-K	0/0	-38.5	-38.5 0.49 (1)	10.00			
K-L	0/0	-38.5	-38.5 0.49 (1)	10.00			
L-F	0/0	-38.5	-38.5 0.49 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	10-4	-314	-314	---	FRONT	VERT.	TOTAL	---	---
J	1-10-4	-311	-311	---	FRONT	VERT.	TOTAL	---	---
K	3-10-4	-311	-311	---	FRONT	VERT.	TOTAL	---	---
L	5-10-4	-311	-311	---	FRONT	VERT.	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.23")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.06")

CSI: TC=0.20/1.00 (C-D:1), BC=0.49/1.00 (G-H:1), WB=0.16/1.00 (C-G:1), SS=0.45/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

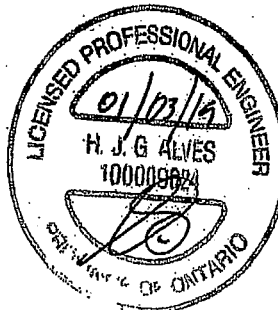
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.50)
JSI METAL= 0.30 (D) (INPUT = 1.00)



DRWG NO. TAM T1900052
STRUCTURAL
COMPONENT ONLY

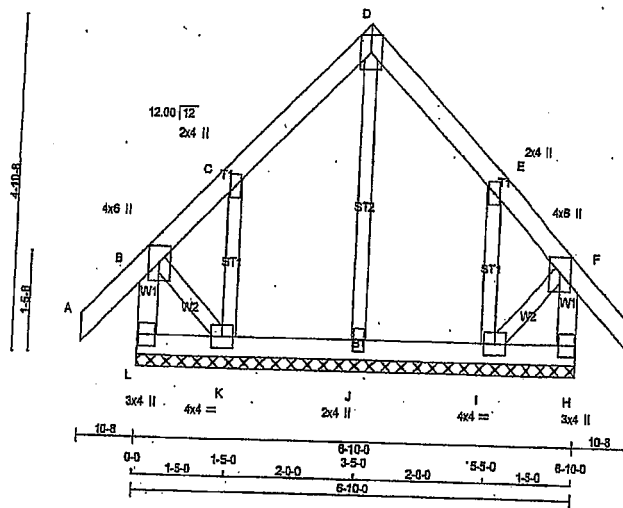
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	G12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHxOPuzKFD0T0r9u1yw9qF-KwvFX_7A?Moc2RDfigV8E_Zs7NyGB30zs4qsRzz5M

Scale = 1:30.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW+p	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0		
E	TMVW+p	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMVW1-w	MT20	2.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED	
		VERT. LOAD LC1 (PLF)	MAX. CSI (LC)			FORCE (LBS)	MAX CSI (LC)
FR-TO		FR-TO			FR-TO		
A-B	0/29	-84.9	-84.9 0.06 (1)	10.00	J-D	-118/0	0.04 (1)
B-C	-40/0	-84.9	-84.9 0.06 (1)	6.25	K-C	-142/0	0.02 (1)
C-D	-33/0	-84.9	-84.9 0.05 (1)	8.25	I-E	-142/0	0.02 (1)
D-E	-33/0	-84.9	-84.9 0.05 (1)	8.25	B-K	0/29	0.01 (1)
E-F	-40/0	-84.9	-84.9 0.06 (1)	8.25	I-F	0/29	0.01 (1)
F-G	0/29	-84.9	-84.9 0.06 (1)	10.00			
L-B	-190/0	0.0	0.0 0.02 (1)	7.81			
H-F	-190/0	0.0	0.0 0.02 (1)	7.81			
L-K	0/0	-38.5	-38.5 0.02 (3)	10.00			
K-J	0/17	-38.5	-38.5 0.03 (3)	10.00			
J-I	0/17	-38.5	-38.5 0.03 (3)	10.00			
L-H	0/0	-38.5	-38.5 0.02 (3)	10.00			

TOTAL WEIGHT = 24 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 46.8	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B:1), BC=0.03/1.00 (I-J:3), WB=0.04/1.00 (D-I:1), SE=0.08/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

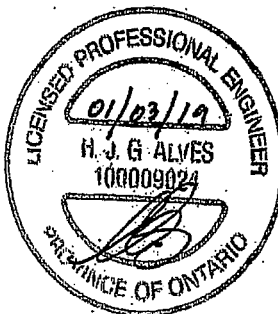
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1967 1658

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.13 (F) (INPUT = 0.90)

JSI METAL= 0.07 (C) (INPUT = 1.00)



DWG NO. TAM 71900035
STRUCTURAL
COMPONENT ONLY

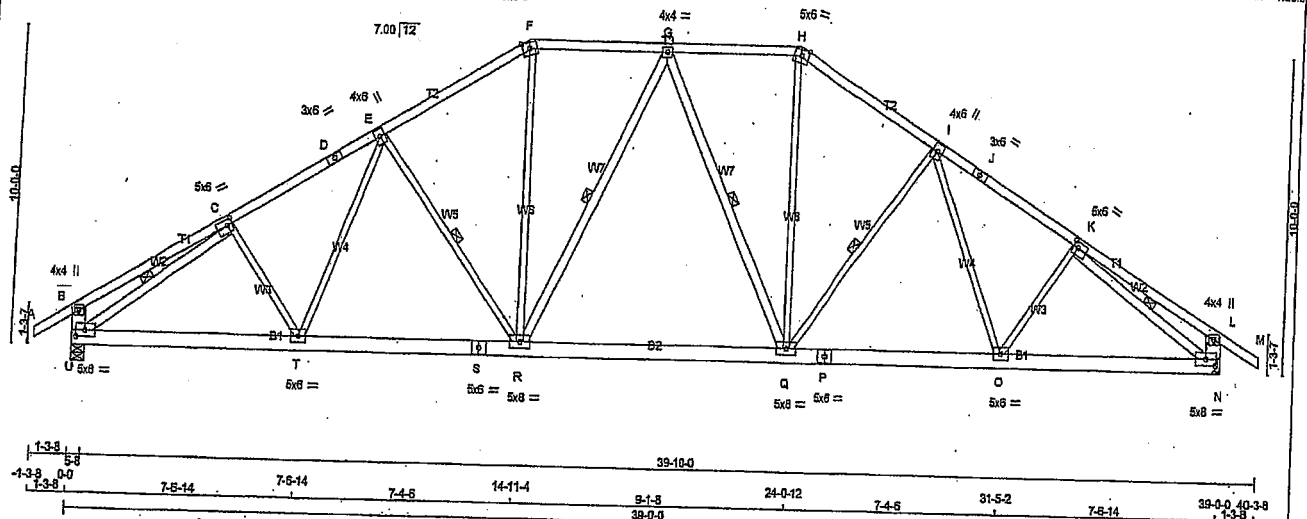
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	T13	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHxOPUzKFD0T0r6u1yw9qF-PpDaF5kmWGOAGRNBjPoz9mPRLZQDZikqZAN_zz4b0

1-3-8 0-0 5-1-7 5-1-7 4-10-15 10-0-5 4-10-15 14-11-4 4-6-12 4-5-12 24-0-12 4-10-15 25-11-11 4-10-15 33-10-9 5-1-7 39-0-0 40-3-8 1-3-8



TOTAL WEIGHT = 6 X 209 = 1252 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - H	2x4	DRY No.2
H - J	2x4	DRY No.2
J - M	2x4	DRY No.2
U - B	2x6	DRY No.2
N - L	2x6	DRY No.2
U - S	2x6	DRY No.2
S - P	2x6	DRY No.2
P - N	2x6	DRY No.2

ALL WEBS EXCEPT	SIZE	LUMBER
R - G	2x4	DRY No.2
G - Q	2x4	DRY No.2
U - C	2x4	DRY No.2
K - N	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0	
C	TMVW+1	MT20	5.0	8.0	2.25 2.00
D	TS+1	MT20	3.0	6.0	
E	TMVW+1	MT20	4.0	6.0	
F	TTW+n	MT20	5.0	6.0	Edge
G	TTW+n	MT20	4.0	4.0	
H	TTW+n	MT20	5.0	6.0	Edge
I	TS+1	MT20	4.0	6.0	
J	TMVW+1	MT20	5.0	8.0	2.25 2.00
K	TMV+p	MT20	4.0	4.0	
L	BMVW+1	MT20	5.0	8.0	2.50 3.75
O	BMVW+1	MT20	5.0	6.0	
P	BS+1	MT20	5.0	6.0	
Q	BMVW+1	MT20	5.0	8.0	
R	BMVW+1	MT20	5.0	8.0	
S	BS+1	MT20	5.0	6.0	
T	BMVW+1	MT20	5.0	6.0	
U	BMVW+1	MT20	5.0	8.0	2.50 3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT 2521	DOWN 2521	0	0
U	HORIZ 0	HORIZ 0	5-8	5-8
N	2521	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS	1ST LOASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
U	1905	972/0 410/0 0/0 0/0 523/0 0/0
N	1905	972/0 410/0 0/0 0/0 523/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, G-Q, I-Q, C-U, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED LO1 MAX (LBS)	MAX. UNBRACED LENGTH (FT)	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX (LBS)
A-B	0/129	-84.9	-84.9	0.11 (1)	10.00	C-T	0/147	0.03 (3)		
B-C	0/119	-84.9	-84.9	0.26 (1)	10.00	T-E	0/330	0.07 (2)		
C-D	-3216/0	-84.9	-84.9	0.39 (1)	3.64	E-R	-598/0	0.28 (1)		
D-E	-3216/0	-84.9	-84.9	0.39 (1)	3.64	R-F	0/1038	0.23 (1)		
E-F	-2711/0	-84.9	-84.9	0.35 (1)	3.94	G-Q	-255/0	0.15 (1)		
F-G	-2333/0	-84.9	-84.9	0.28 (1)	4.25	Q-H	-255/0	0.15 (1)		
G-H	-2333/0	-84.9	-84.9	0.28 (1)	4.25	H-I	0/1038	0.23 (1)		
H-I	-2711/0	-84.9	-84.9	0.35 (1)	3.94	I-J	-598/0	0.28 (1)		
I-J	-3216/0	-84.9	-84.9	0.39 (1)	3.64	J-K	0/330	0.07 (2)		
J-K	-3216/0	-84.9	-84.9	0.39 (1)	3.64	K-L	0/147	0.03 (3)		
K-L	0/119	-84.9	-84.9	0.26 (1)	10.00	L-M	-3472/0	0.84 (1)		
L-M	0/129	-84.9	-84.9	0.11 (1)	10.00	M-N	-3472/0	0.84 (1)		
M-N	-290/0	0.0	0.0	0.02 (1)	7.81					
N-L	-290/0	0.0	0.0	0.02 (1)	7.81					
U-T	0/2763	-38.5	-38.5	0.47 (1)	10.00					
T-S	0/2681	-38.5	-38.5	0.47 (2)	10.00					
S-R	0/2681	-38.5	-38.5	0.47 (2)	10.00					
R-Q	0/2440	-38.5	-38.5	0.44 (2)	10.00					
Q-P	0/2681	-38.5	-38.5	0.47 (2)	10.00					
P-O	0/2681	-38.5	-38.5	0.47 (2)	10.00					
O-N	0/2763	-38.5	-38.5	0.47 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.30")
CALCULATED VERT. DEFL. (LL) = L/959 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (1.30")
CALCULATED VERT. DEFL. (TL) = L/959 (0.25")

CSI: TC=0.39/1.00 (K=1), BC=0.47/1.00 (Q=2), WB=0.64/1.00 (K=1), SB=0.19/1.00 (G=1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

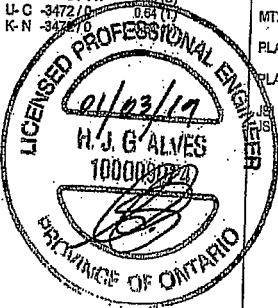
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1665

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.90 (K) (INPUT = 0.90)
TENS = 0.88 (C) (INPUT = 1.00)

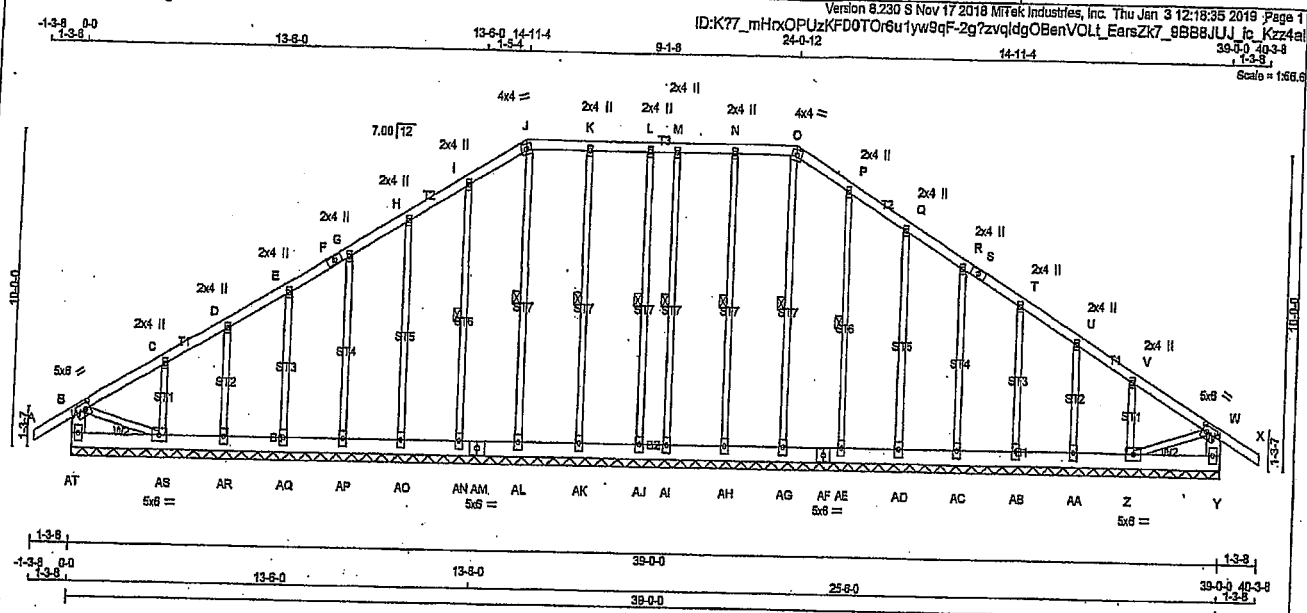


DRWG NO. TAM T1900066
STRUCTURAL
COLUMBIA, MARYLAND

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHxOPUzKF0T0r6u1yw9qF-2g7zvqlgObenVOLT_EarsZk7_8B88JUUJc_Kzz4al
39-0-0 40-3-8
1-3-8
Scale = 1/8" = 1'-0"



TOTAL WEIGHT = 2 X 227 = 454 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
J - O	2x4	DRY	No.2
O - S	2x4	DRY	No.2
S - X	2x4	DRY	No.2
AT - B	2x6	DRY	No.2
Y - W	2x6	DRY	No.2
AT - AM	2x6	DRY	No.2
AM - AF	2x6	DRY	No.2
AF - Y	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y
B TMVW-1	MT20	5.0	6.0	2.50 2.00
C, D, E, G, H, I, K, L, M, N, P, Q, R, T, U, V	MT20	2.0	4.0	
F TMVW+	MT20	3.0	6.0	
J TTW-m	MT20	4.0	4.0	
O TTW-m	MT20	3.0	6.0	
S TS-1	MT20	5.0	6.0	2.50 2.00
W TMVW-1	MT20	3.0	6.0	
Y BMV1+p	MT20	5.0	6.0	
Z BMVW-1	MT20	3.0	6.0	
AA, AB, AC, AD, AE, AG, AH, AI, AJ, AK, AL, AN, AO, AP, AQ, AR	MT20	3.0	6.0	
AA BMV1+w	MT20	5.0	6.0	
AF BS-1	MT20	5.0	6.0	
AM BS-1	MT20	5.0	6.0	
AS BMVW-1	MT20	3.0	6.0	
AT BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-AJ, K-AK, J-AL, I-AN, N-AH, M-AI, O-AG, P-AE.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS		WEBS	
MEMB.	FORCE FACTORED (LBS)	MAX. FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED UNBRAC LENGTH FR-TO (LC)
FR-TO			
A-B	0/29	-84.9 -84.9 0.11 (1)	10.00 A-L -117 / 0
B-C	-28 / 0	-84.9 -84.9 0.09 (1)	6.25 AK-K -198 / 0
C-D	-41 / 0	-84.9 -84.9 0.09 (1)	6.25 AL-J -131 / 0
D-E	-23 / 0	-84.9 -84.9 0.04 (1)	6.25 AN-I -186 / 0
E-F	-24 / 0	-84.9 -84.9 0.04 (1)	6.25 AO-H -165 / 0
F-G	-24 / 0	-84.9 -84.9 0.04 (1)	6.25 AP-G -168 / 0
G-H	-21 / 0	-84.9 -84.9 0.04 (1)	6.25 AS-C -254 / 0
H-I	-18 / 0	-84.9 -84.9 0.04 (1)	6.25 AR-D -139 / 0
I-J	-25 / 0	-84.9 -84.9 0.04 (1)	6.25 AS-C -254 / 0
J-K	-13 / 0	-84.9 -84.9 0.05 (1)	6.25 AH-N -198 / 0
K-L	-13 / 0	-84.9 -84.9 0.05 (1)	6.25 AI-M -117 / 0
L-M	-13 / 0	-84.9 -84.9 0.03 (1)	6.25 AG-O -131 / 0
M-N	-13 / 0	-84.9 -84.9 0.05 (1)	6.25 AE-P -186 / 0
N-O	-13 / 0	-84.9 -84.9 0.05 (1)	6.25 AD-Q -168 / 0
O-P	-25 / 0	-84.9 -84.9 0.04 (1)	6.25 AC-R -168 / 0
P-Q	-18 / 0	-84.9 -84.9 0.04 (1)	6.25 AB-T -174 / 0
Q-R	-21 / 0	-84.9 -84.9 0.04 (1)	6.25 AA-U -139 / 0
R-S	-24 / 0	-84.9 -84.9 0.04 (1)	6.25 Z-V -254 / 0
S-T	-24 / 0	-84.9 -84.9 0.04 (1)	6.25 B-AS 0 / 34
T-U	-23 / 0	-84.9 -84.9 0.04 (1)	6.25 Z-W 0 / 34
U-V	-41 / 0	-84.9 -84.9 0.09 (1)	
V-W	-28 / 0	-84.9 -84.9 0.09 (1)	
W-X	0 / 29	-84.9 -84.9 0.11 (1)	10.00
AT-B	-247 / 0	0.0 0.0 0.02 (1)	7.81
Y-W	-247 / 0	0.0 0.0 0.02 (1)	7.81
AT-AS	0 / 0	-38.5 -38.5 0.03 (3)	10.00
AS-AR	0 / 25	-38.5 -38.5 0.03 (2)	10.00
AR-AQ	0 / 22	-38.5 -38.5 0.01 (2)	10.00
AQ-AP	0 / 20	-38.5 -38.5 0.01 (2)	10.00
AP-AO	0 / 18	-38.5 -38.5 0.01 (2)	10.00
AO-AN	0 / 16	-38.5 -38.5 0.01 (2)	10.00
AN-AM	0 / 15	-38.5 -38.5 0.01 (2)	10.00
AM-AL	0 / 15	-38.5 -38.5 0.01 (2)	10.00
AL-AK	0 / 13	-38.5 -38.5 0.01 (2)	10.00
AK-AJ	0 / 13	-38.5 -38.5 0.01 (2)	10.00
AJ-AI	0 / 13	-38.5 -38.5 0.01 (2)	10.00
AI-AH	0 / 13	-38.5 -38.5 0.01 (2)	10.00
AH-AG	0 / 13	-38.5 -38.5 0.01 (2)	10.00
AG-AF	0 / 15	-38.5 -38.5 0.01 (2)	10.00
AF-AE	0 / 15	-38.5 -38.5 0.01 (2)	10.00
AE-AD	0 / 16	-38.5 -38.5 0.01 (2)	10.00
AD-AC	0 / 18	-38.5 -38.5 0.01 (2)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL TC=0.11/1.00 (A-B:1), BC=0.03/1.00 (Z-AA:2), WB=0.17/1.00 (Q-AD:1), SB=0.10/1.00 (V-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)

MAX MIN MAX MIN MAX MIN

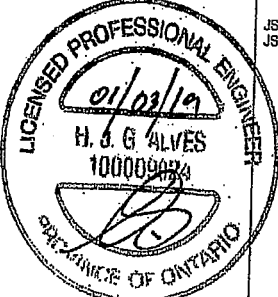
MT20 618 354 1657 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)

JSI METAL= 0.12 (C) (INPUT = 1.00)



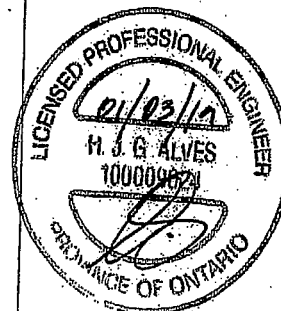
DRWG NO. TAM T1400060
STRUCTURAL
COMPONENT 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	

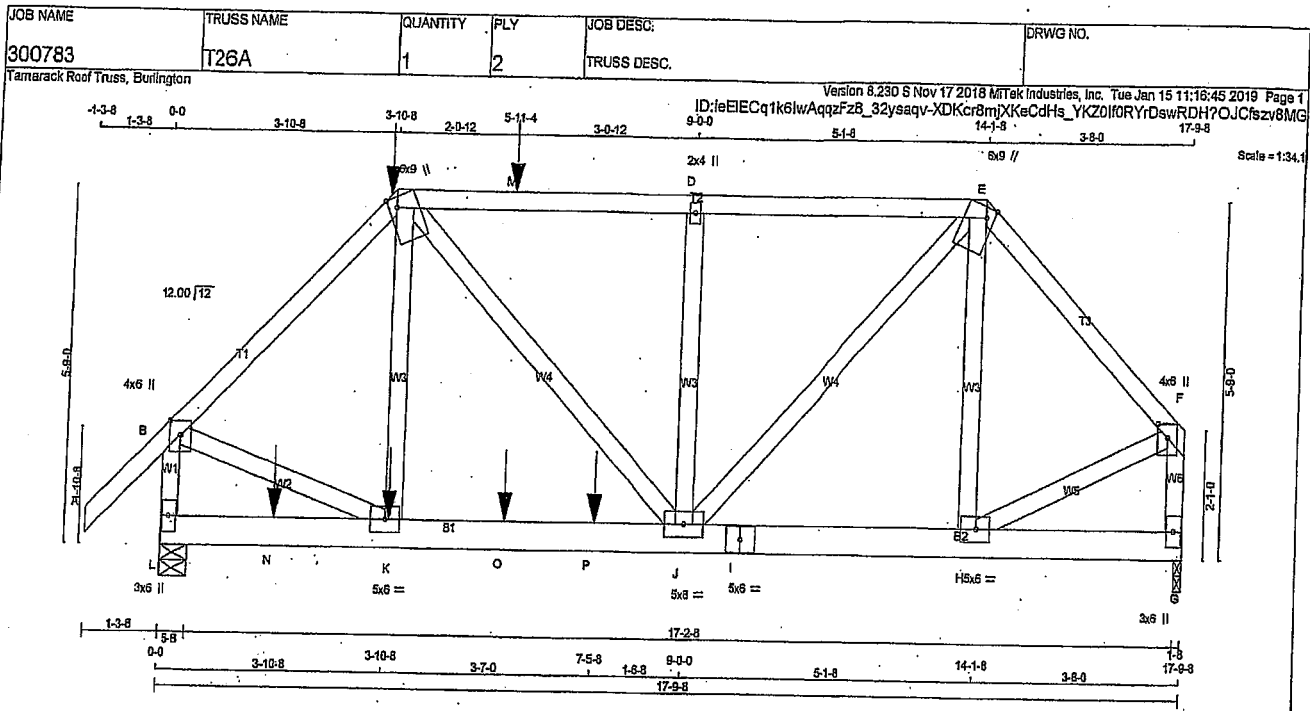
Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 12:18:35 2019 Page 2
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LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX MEMB. UNBRAC LENGTH FR-TO	MEMB.	MAX FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO					
AG-AB	0/20	-38.5 -38.5	0.01 (2)	10.00			
AB-AA	0/22	-38.5 -38.5	0.01 (2)	10.00			
AA-Z	0/25	-38.5 -38.5	0.03 (2)	10.00			
Z-Y	0/0	-38.5 -38.5	0.03 (3)	10.00			



DESIGN NO. TAM T1900060
STRUCTURAL
COMPUTATION ONLY 4/2



TOTAL WEIGHT = 2 X 103 = 206 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - I	2x6	DRY	No.2
I - G	2x6	DRY	No.2

ALL WEBS 2x4 DRY No.2 SPF

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE (61.0)
C-E	12	SIDE (61.0)
E-F	12	TOP
L-B	12	TOP
G-F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-I	12	SIDE (183.1)
I-G	12	TOP

WEBS : (0.122"x3") SPIRAL NAILS

2x4 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV+p	MT20	4.0	6.0	2.75	2.00
C TTWW+m	MT20	6.0	9.0	Edge	1.75
D TMW+w	MT20	2.0	4.0		
E TTWW+m	MT20	6.0	9.0	Edge	1.75
F TMW+p	MT20	4.0	6.0	2.75	2.00
G BMV+p	MT20	3.0	6.0		
H BMWV+t	MT20	5.0	6.0		
I BS+t	MT20	5.0	6.0		
J BMWV+t	MT20	5.0	6.0		
K BMWV+t	MT20	5.0	6.0		
L BMV+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT VERT	2130	2130	0	5-8
L VERT	1615	1615	0	5-8
G VERT	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LOOSE	MAX/MIN. COMPONENT REACTIONS					
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
L	1591	864/0	300/0	0/0	427/0	0/0	
G	1214	636/0	249/0	0/0	331/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0/42	-84.9 -84.9 0.07 (1)	K-C	0/267
B-C	-1911/0	-84.9 -84.9 0.14 (1)	C-J	0/777
C-M	-1887/0	-84.9 -84.9 0.25 (1)	J-D	-560/0
M-D	-1887/0	-84.9 -84.9 0.25 (1)	D-E	0/1358
D-E	-1887/0	-84.9 -84.9 0.25 (1)	E-F	0/1358
E-F	-1359/0	-84.9 -84.9 0.12 (1)	F-G	0/1358
L-B	-2121/0	0.0 0.0 0.12 (1)	G-H	0/1358
G-F	-1835/0	0.0 0.0 0.09 (1)	H-I	0/1358
L-N	0/0	-38.5 -38.5 0.13 (1)	I-J	0/1358
N-K	0/0	-38.5 -38.5 0.13 (1)	J-K	0/1358
K-O	0/1350	-38.5 -38.5 0.37 (1)	K-L	0/1358
O-P	0/1350	-38.5 -38.5 0.37 (1)	L-M	0/1358
P-J	0/1350	-38.5 -38.5 0.37 (1)	M-N	0/1358
J-I	0/948	-38.5 -38.5 0.22 (1)	N-O	0/1358
I-H	0/948	-38.5 -38.5 0.22 (1)	O-P	0/1358
H-G	0/0	-38.5 -38.5 0.03 (2)	P-Q	0/1358

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
C	3-10-8	-271	-271		FRONT	VERT
K	3-11-4	-36	-36		FRONT	VERT
M	5-11-4	-80	-80		FRONT	VERT
N	1-11-4	-36	-36		FRONT	VERT
O	5-11-4	-36	-36		FRONT	VERT
P	7-5-8	-972	-972		TOP	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3	PSF
DL	= 6.0	PSF
BOT CH. LL	= 10.5	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 46.8	PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.59")
CALCULATED VERT. DEFL. (LL) = L/699 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.59")
CALCULATED VERT. DEFL. (TL) = L/699 (0.05")

CSI: TC=0.25/1.00 (C-D-1), BC=0.37/1.00 (J-K-1),
WB=0.13/1.00 (B-K-1), SSI=0.26/1.00 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

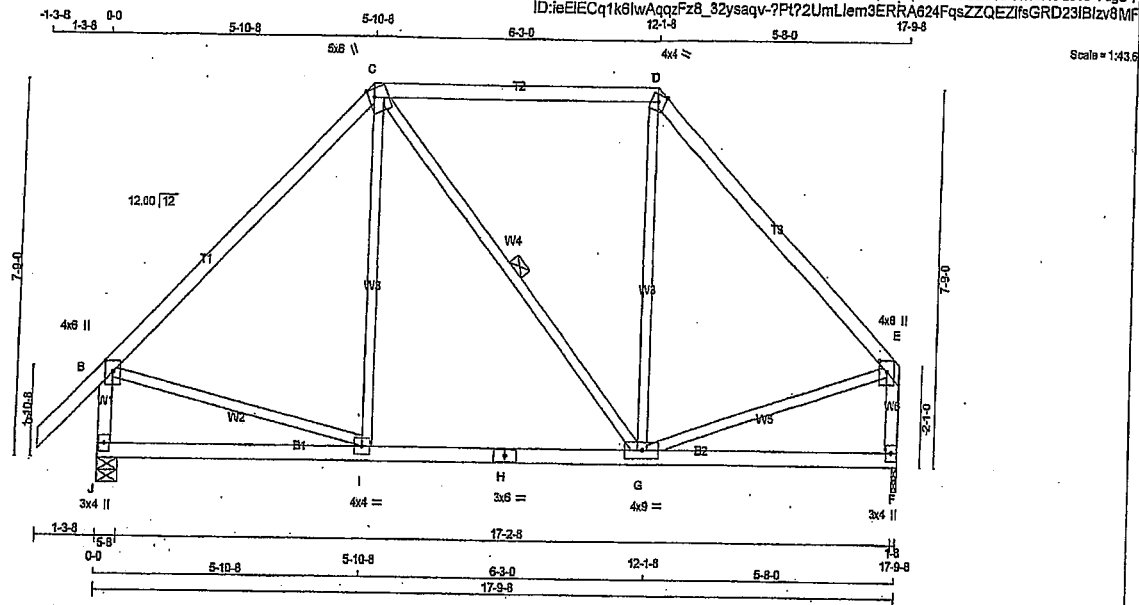
JSI GRIP= 0.74 (B) (INPUT = 0.90)
JSI METAL = 0.39 (B) (INPUT = 1.00)

DWG NO. TAM 17201093
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300783	T27A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1/4" = 1'-0"

TOTAL WEIGHT = 84.1b

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2		
C - D	2x4 DRY	No.2		
D - E	2x4 DRY	No.2		
J - B	2x4 DRY	No.2		
F - E	2x4 DRY	No.2		
J - H	2x4 DRY	No.2		
H - F	2x4 DRY	No.2		

ALL WEBS 2x3 DRY No.2

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMW+p	MT20	4.0	5.0	2.75	2.00
F	BMV+p	MT20	3.0	4.0		
G	BMV+m	MT20	4.0	9.0		
H	BS-1	MT20	3.0	6.0		
I	BMV+m	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
J	1216	0	1216	0	0	5-8	5-8	1-8	1-8
F	1097	0	1097	0	0	1-8	1-8		

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS				
J	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
F	814	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0	
F	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-84.9 -84.9	0.12 (1)	10.00	I-C	0 / 243	0.06 (3)
B-C	-851 / 0	-84.9 -84.9	0.40 (1)	6.09	C-G	-18 / 0	0.01 (1)
C-D	-590 / 0	-84.9 -84.9	0.43 (1)	6.25	G-D	0 / 230	0.05 (3)
D-E	-834 / 0	-84.9 -84.9	0.37 (1)	6.18	B-I	0 / 619	0.14 (1)
J-B	-1123 / 0	0.0 0.0	0.13 (1)	7.48	G-E	0 / 614	0.14 (1)
F-E	-1009 / 0	0.0 0.0	0.12 (1)	7.79			
J-I	0 / 0	-38.5 -38.5	0.25 (3)	10.00			
I-H	0 / 601	-38.5 -38.5	0.32 (2)	10.00			
H-G	0 / 601	-38.5 -38.5	0.32 (2)	10.00			
G-F	0 / 0	-38.5 -38.5	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 5.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/599 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/599 (0.08")

CSI: TC=0.43/1.00 (C-D:1), BC=0.32/1.00 (G-I:2), WB=0.14/1.00 (B-I:1), GS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

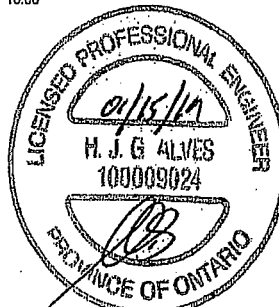
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)



DRWG NO. TAM 77401094
STRUCTURAL
COMPLEMENT FINAL

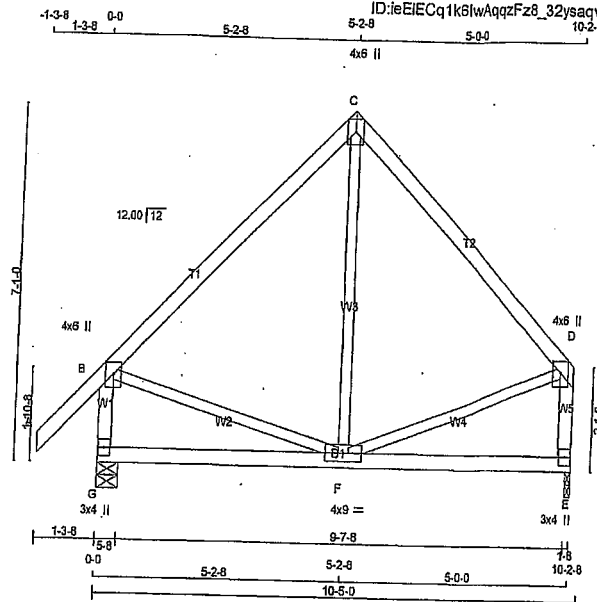
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300799	T29A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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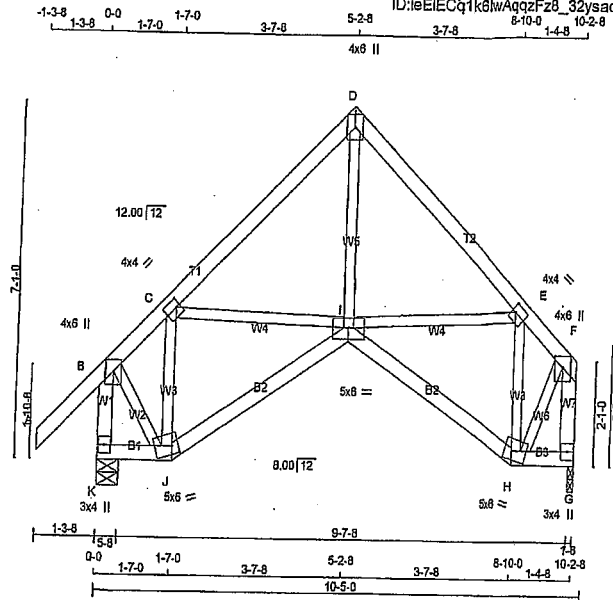


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300799	T30A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - B	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW+1	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0		
E	TMVW+1	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BBVW+m	MT20	5.0	6.0	2.00	2.00
I	BBVW+1	MT20	5.0	8.0	2.75	4.00
J	BBVW+m	MT20	5.0	6.0	2.00	1.75
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
K	756	0	756	0
G	622	0	622	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	565	307 / 0	107 / 0	0 / 0	0 / 0	151 / 0	0 / 0
G	472	233 / 0	107 / 0	0 / 0	0 / 0	132 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX (LBS)	CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	I-D	0 / 462	0.10 (2)
B-C	-398 / 0	-84.9	-84.9	0.11 (1)	6.25	I-E	0 / 112	0.08 (1)
C-D	-535 / 0	-84.9	-84.9	0.15 (1)	6.25	H-E	-432 / 0	0.08 (1)
D-E	-537 / 0	-84.9	-84.9	0.14 (1)	6.25	C-I	0 / 104	0.02 (1)
E-F	-336 / 0	-84.9	-84.9	0.12 (1)	6.25	J-C	-351 / 0	0.07 (1)
K-B	-726 / 0	0.0	0.0	0.08 (1)	7.81	B-J	0 / 397	0.09 (1)
G-F	-595 / 0	0.0	0.0	0.07 (1)	7.81	H-F	0 / 456	0.10 (1)
K-J	0 / 0	-38.5	-38.5	0.02 (3)	10.00			
J-I	0 / 316	-38.5	-38.5	0.16 (2)	10.00			
I-H	0 / 306	-38.5	-38.5	0.15 (2)	10.00			
H-G	0 / 0	-38.5	-38.5	0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.34")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.34")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CS1: TC=0.15/1.00 (C-D-1), BC=0.16/1.00 (I-L2),
WB=0.10/1.00 (D-I-2), SS=0.11/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

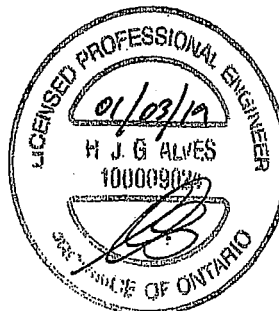
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (H) (INPUT = 0.90)
JSI METAL = 0.19 (B) (INPUT = 1.00)



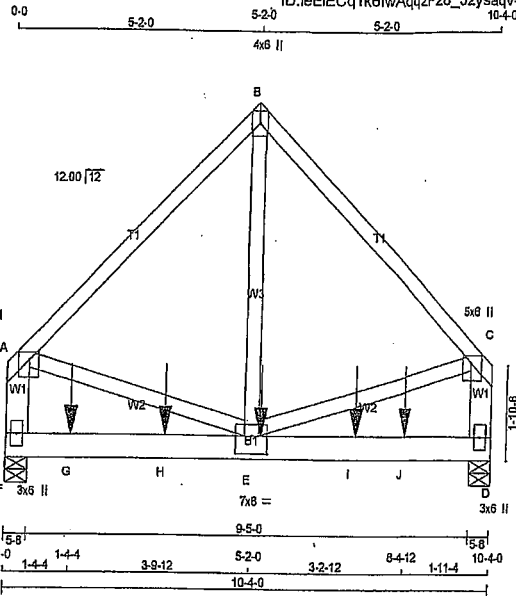
DRWG NO. TAM T1900120
STRUCTURAL
COMMENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	T31	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:ieIEECq1k6lwAqzFz8_32ysaqv-VJ4oERw3LhDKasvOG_HiPct_QMb8QYRWy3lzz03Z



Scale = 1/4" = 1'-0"

TOTAL WEIGHT = 2 X 59 = 118 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2 SPF
B - C	2x4	DRY	No.2 SPF
F - A	2x6	DRY	No.2 SPF
F - C	2x6	DRY	No.2 SPF
F - D	2x6	DRY	No.2 SPF

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
A-B	1	TOP
B-C	1	TOP
F-A	2	TOP
D-C	2	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F-D	2	SIDE (0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TTW+p	MT20	4.0	6.0		
C	TMVW+p	MT20	5.0	6.0	1.75	2.75
D	BMV1+p	MT20	3.0	6.0		
E	BMVWV+t	MT20	7.0	8.0	4.25	4.00
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	HORZ	UPLIFT
F	3604	0	0	5-8
D	3557	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE
F	2699	1449 / 0	526 / 0	0 / 0
D	2664	1429 / 0	520 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

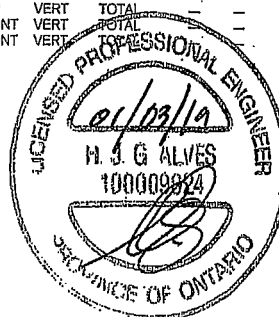
LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
A-B	-2643 / 0	-84.9	-84.9	0.29 (1)	5.31	E-B	0 / 3300	0.29 (1)
B-C	-2643 / 0	-84.9	-84.9	0.29 (1)	5.31	A-E	0 / 1940	0.17 (1)
F-A	-2609 / 0	0.0	0.0	0.10 (1)	7.81	E-C	-0 / 1940	0.17 (1)
D-C	-2609 / 0	0.0	0.0	0.10 (1)	7.81			
F-G	0 / 0	-38.5	-38.5	0.73 (1)	10.00			
G-H	0 / 0	-38.5	-38.5	0.73 (1)	10.00			
H-E	0 / 0	-38.5	-38.5	0.73 (1)	10.00			
E-I	0 / 0	-38.5	-38.5	0.73 (1)	10.00			
I-J	0 / 0	-38.5	-38.5	0.73 (1)	10.00			
J-D	0 / 0	-38.5	-38.5	0.73 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
E	5-4.4	-1177	-1177	---	TOP	VERT	TOTAL	---
G	1-4.4	-1177	-1177	---	TOP	VERT	TOTAL	---
H	3-4.4	-1177	-1177	---	TOP	VERT	TOTAL	---
I	7-4.4	-1177	-1177	---	FRONT	VERT	TOTAL	---
J	8-4.12	-1177	-1177	---	FRONT	VERT	TOTAL	---



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CS1: TC=0.29/1.00 (B-C:1), BC=0.73/1.00 (E-F:1), WB=0.29/1.00 (B-E:1), SS1=0.53/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
			788
			1987
			1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (B) (INPUT = 0.90)
JSI METAL= 0.39 (A) (INPUT = 1.00)

ENGR. NO. TAM T1900151
STRUCTURAL
CALC. FOR TRUSS ONLY

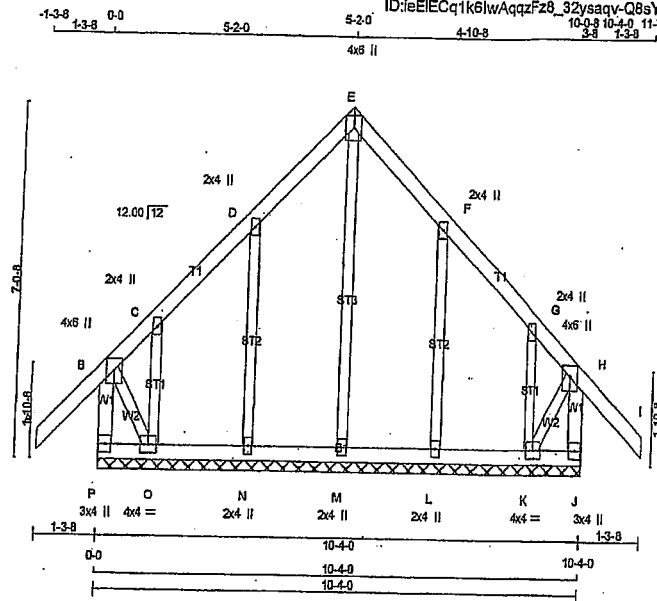
JOB NAME 300785	TRUSS NAME G32	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:leIEECq1k6lwAqzFz8_32ysaqv-Q8sYIWz31ly2qXJpIMo5GYN7laO1PMDk5yDVHNzv8FX
10-9-8 10-4-0 11-7-8

Scale = 1/4" = 1'-0"



TOTAL WEIGHT = 56 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
E - I	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
P - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWt+p	MT20	4.0	6.0	2.75	2.00
C, D, F, G					
C TMWt+w	MT20	2.0	4.0		
E TMWt+p	MT20	4.0	6.0		
H TMWt+p	MT20	4.0	6.0	2.75	2.00
J BMWt+p	MT20	3.0	4.0		
K BMWt-t	MT20	4.0	4.0		
L, M, N					
L BMWt+w	MT20	2.0	4.0		
O BMWt-t	MT20	4.0	4.0		
R BMWt+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	L1 CS (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO					FR-TO		
P-B	-258 / 0	0.0	0.0	0.03 (1)	M-E	-131 / 0	0.11 (1)
A-B	0 / 42	-84.9	-84.9	0.12 (1)	N-D	-205 / 0	0.08 (1)
B-C	-84 / 0	-84.9	-84.9	0.11 (1)	C-C	-42 / 0	0.01 (1)
C-D	-1 / 0	-84.9	-84.9	0.05 (1)	L-F	-205 / 0	0.08 (1)
D-E	-20 / 0	-84.9	-84.9	0.05 (1)	K-G	-42 / 0	0.01 (1)
E-F	-20 / 0	-84.9	-84.9	0.05 (1)	B-O	0 / 17	0.00 (1)
F-G	-1 / 0	-84.9	-84.9	0.11 (1)	K-H	0 / 17	0.00 (1)
G-H	-84 / 0	-84.9	-84.9	0.12 (1)			
H-I	0 / 42	-84.9	-84.9	0.03 (1)			
J-H	-258 / 0	0.0	0.0	0.03 (1)			
P-O	0 / 0	-38.5	-38.5	0.02 (3)			
O-N	0 / 9	-38.5	-38.5	0.03 (3)			
N-M	0 / 4	-38.5	-38.5	0.03 (3)			
M-L	0 / 4	-38.5	-38.5	0.03 (3)			
L-K	0 / 9	-38.5	-38.5	0.03 (3)			
K-J	0 / 0	-38.5	-38.5	0.02 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.12/1.00 (A-B:1), BC=0.03/1.00 (N-O:3), WB=0.11/1.00 (E-M:1), SS=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

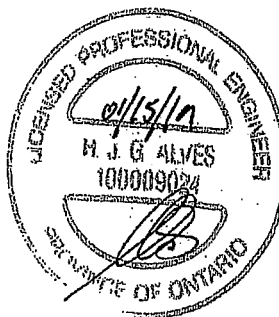
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (H) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

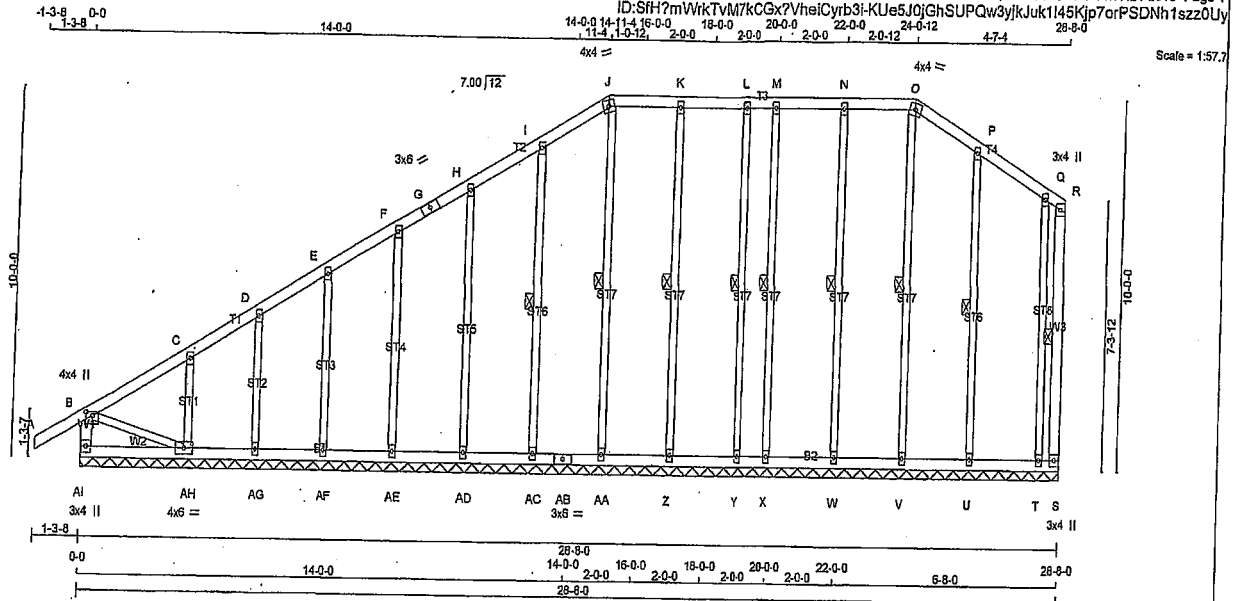


DRWG NO. TAM 119010990
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	G33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:57.7

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
AI - B	2x4	DRY	No.2	SPF	
A - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
J - O	2x4	DRY	No.2	SPF	
O - R	2x4	DRY	No.2	SPF	
S - R	2x4	DRY	No.2	SPF	
AI - AB	2x4	DRY	No.2	SPF	
AB - S	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00
C D, E, F, H, I, K, L, M, N, P, Q					
C TMVW+w	MT20	2.0	4.0		
G TS-t	MT20	3.0	6.0		
J TTW-m	MT20	4.0	4.0		
O TTW-m	MT20	4.0	4.0		
R TMV+p	MT20	3.0	4.0		
S BMV1+p	MT20	3.0	4.0		
T, U, V, W, X, Y, Z, AA, AC, AD, AE, AF, AG					
T BMV1+w	MT20	2.0	4.0		
AB BS-t	MT20	3.0	6.0		
AH BMVW1-t	MT20	4.0	6.0		
AI BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINTS FOR 150 LBS. FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF R-S, I-AC, J-AA, K-Z, L-Y, M-X, N-W, O-V, P-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. CS1 (LC)	UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO						FR-TO		
AI-B	-232/0	0.0	0.0	0.02 (1)	7.81	AH-C	-254/0	0.05 (1)
A-B	0/29	-84.9	-84.9	0.11 (1)	10.00	AG-D	-139/0	0.04 (1)
B-C	-5/0	-84.9	-84.9	0.09 (1)	10.00	AF-E	-174/0	0.08 (1)
C-D	-21/0	-84.9	-84.9	0.09 (1)	6.25	AE-F	-168/0	0.12 (1)
D-E	-3/0	-84.9	-84.9	0.04 (1)	10.00	AD-H	-166/0	0.18 (1)
E-F	-3/0	-84.9	-84.9	0.04 (1)	10.00	AC-I	-167/0	0.10 (1)
F-G	0/0	-84.9	-84.9	0.04 (1)	10.00	AA-J	-141/0	0.10 (1)
G-H	0/0	-84.9	-84.9	0.04 (1)	10.00	Z-K	-198/0	0.13 (1)
H-I	0/3	-84.9	-84.9	0.05 (1)	10.00	Y-L	-117/0	0.08 (1)
I-J	-5/0	-84.9	-84.9	0.04 (1)	10.00	X-M	-117/0	0.08 (1)
J-K	0/5	-84.9	-84.9	0.05 (1)	10.00	W-N	-198/0	0.13 (1)
K-L	0/4	-84.9	-84.9	0.05 (1)	10.00	V-O	-139/0	0.10 (1)
L-M	0/4	-84.9	-84.9	0.03 (1)	10.00	U-P	-203/0	0.09 (1)
M-N	0/4	-84.9	-84.9	0.05 (1)	10.00	T-Q	-119/0	0.13 (1)
N-O	0/5	-84.9	-84.9	0.05 (1)	10.00	B-AH	0/15	0.00 (1)
O-P	-6/0	-84.9	-84.9	0.05 (1)	10.00			
P-Q	0/8	-84.9	-84.9	0.05 (1)	10.00			
Q-R	0/11	-84.9	-84.9	0.02 (1)	10.00			
S-R	-2/0	0.0	0.0	0.00 (1)	6.25			
AI-AH	0/0	-38.5	-38.5	0.06 (3)	10.00			
AH-AG	0/8	-38.5	-38.5	0.06 (3)	10.00			
AG-AF	0/5	-38.5	-38.5	0.03 (3)	10.00			
AF-AE	0/2	-38.5	-38.5	0.03 (3)	10.00			
AE-AD	0/0	-38.5	-38.5	0.02 (3)	10.00			
AD-AC	-1/0	-38.5	-38.5	0.02 (3)	10.00			
AC-AB	-3/0	-38.5	-38.5	0.02 (3)	10.00			
AB-AA	-3/0	-38.5	-38.5	0.02 (3)	10.00			
AA-Z	-4/0	-38.5	-38.5	0.03 (3)	10.00			
Z-Y	-4/0	-38.5	-38.5	0.03 (3)	10.00			
Y-X	-4/0	-38.5	-38.5	0.02 (3)	10.00			
X-W	-4/0	-38.5	-38.5	0.03 (3)	10.00			
W-V	-4/0	-38.5	-38.5	0.03 (3)	10.00			
V-U	-3/0	-38.5	-38.5	0.03 (3)	10.00			
U-T	-1/0	-38.5	-38.5	0.03 (3)	10.00			
T-S	0/0	-38.5	-38.5	0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 27.2 P.S.F. - G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.11/1.00 (A-B:1), BC=0.08/1.00 (AG-AH:3), WB=0.18/1.00 (H-AD:1), SS=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

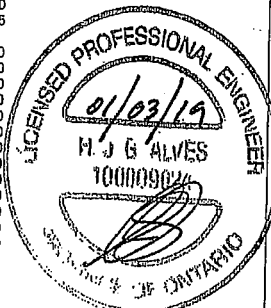
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1658

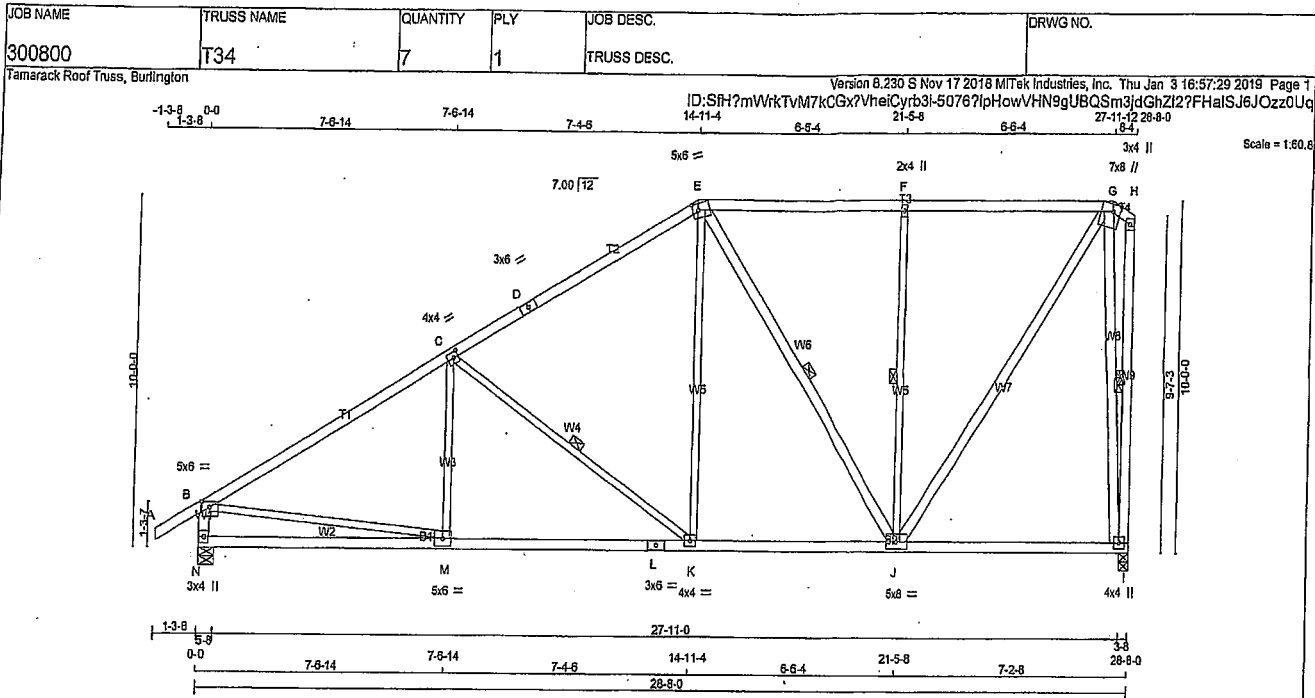
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)
JSI METAL = 0.12 (C) (INPUT = 1.00)

DWG NO. TAM 17A00124
STRUCTURAL
COMPONENT ONLY





TOTAL WEIGHT = 7 X 151 = 1056 lb
[M/F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - J	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTVW-m	MT20	5.0	6.0	2.25	2.00
F	TTVW-w	MT20	2.0	4.0		
G	TTVW-m	MT20	7.0	8.0	Edge 2.50	
H	TMVW-p	MT20	3.0	4.0		
I	BMVW1-p	MT20	4.0	4.0		
J	BMVW-t	MT20	5.0	8.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	6.0		
N	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
N	1884	0	1884	0	0	5-8	5-8	5-8	5-8
I	1768	0	1768	0	0	3-8	3-8	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	BRG	IN-SX	BRG	IN-SX
N	1421	731 / 0	301 / 0	0 / 0	0 / 0	389 / 0	0 / 0	5-8	5-8	5-8	5-8
I	1341	688 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0	3-8	3-8	3-8	3-8

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-J, F-J, H-I, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO		
A-B	0 / 29	-84.9	-84.9 0.11 (1)	10.00	M-C	0 / 295	0.07 (3)
B-C	-2250 / 0	-84.9	-84.9 0.84 (1)	3.60	C-K	-820 / 0	0.49 (1)
C-D	-1558 / 0	-84.9	-84.9 0.71 (1)	4.33	K-E	0 / 736	0.17 (2)
D-E	-1558 / 0	-84.9	-84.9 0.71 (1)	4.33	E-J	-488 / 0	0.37 (1)
E-F	-1047 / 0	-84.9	-84.9 0.48 (1)	5.47	J-F	-685 / 0	0.47 (1)
F-G	-1047 / 0	-84.9	-84.9 0.48 (1)	5.47	B-M	0 / 1993	0.45 (1)
G-H	0 / 0	-84.9	-84.9 0.01 (1)	10.00	G-I	-1630 / 0	0.80 (1)
N-B	-1767 / 0	0.0	0.0 0.18 (1)	6.27	J-G	0 / 1670	0.27 (1)
I-H	-29 / 0	0.0	0.0 0.01 (1)	6.25			
N-M	0 / 0	-38.5	-38.5 0.42 (3)	10.00			
M-L	0 / 1976	-38.5	-38.5 0.68 (2)	10.00			
L-K	0 / 1976	-38.5	-38.5 0.68 (2)	10.00			
K-J	0 / 1319	-38.5	-38.5 0.52 (2)	10.00			
J-I	0 / 116	-38.5	-38.5 0.36 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.84/1.00 (B-C:1), BC=0.68/1.00 (K-M:2), WB=0.80/1.00 (G-I:1), SS=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

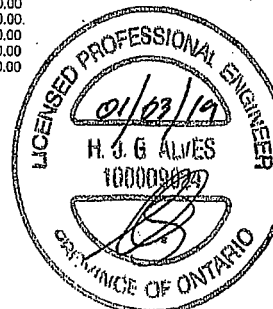
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1668

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.56 (L) (INPUT = 1.00)

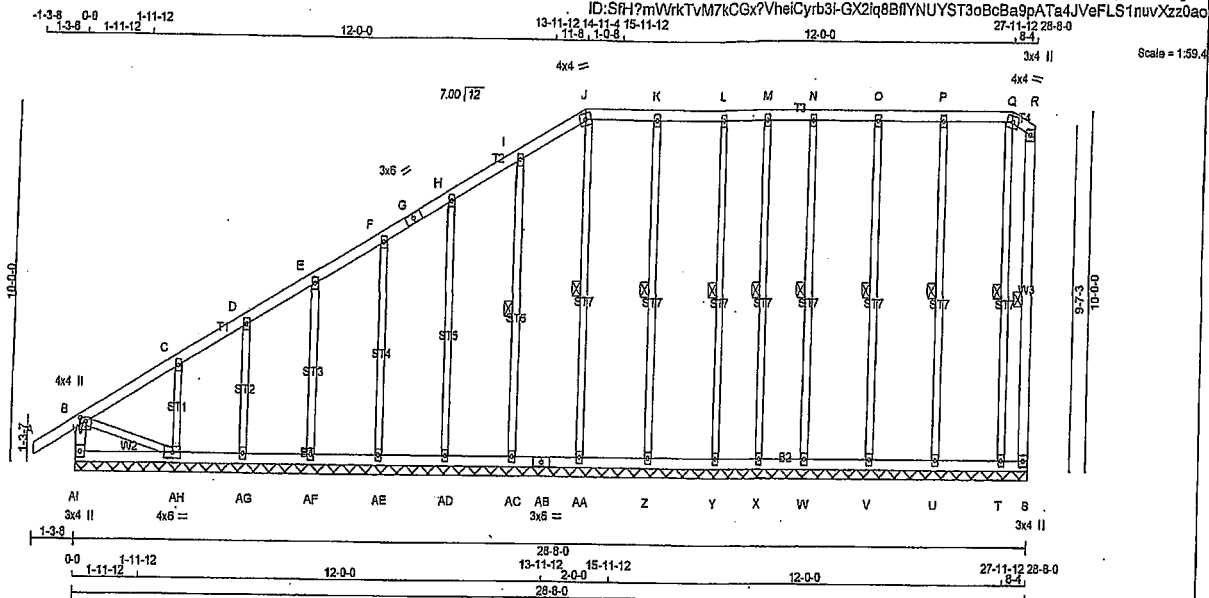


DRWG NO. TAM 71900129
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300799	G34	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 172 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - Q	2x4	DRY	No.2	SPF
Q - R	2x4	DRY	No.2	SPF
AI - B	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
AI - AB	2x4	DRY	No.2	SPF
AB - S	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	4.0	4.0	1.25	2.00
C D, E, F, H, I, K, L, M, N, O, P					
C TMW+w	MT20	2.0	4.0		
G TS-t	MT20	3.0	6.0		
J TTW-m	MT20	4.0	4.0		
Q TTW-m	MT20	4.0	4.0		
R TMV+p	MT20	3.0	4.0		
S BMV1+p	MT20	3.0	4.0		
T, U, V, W, X, Y, Z, AA, AC, AD, AE, AF, AG					
T BMW1+w	MT20	2.0	4.0		
AB BS-t	MT20	3.0	6.0		
AF BMW1-t	MT20	4.0	6.0		
AF BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF R-S, Q-T, P-U, O-V, N-W, L-Y, K-Z, J-AA, I-AC, M-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO		
A-B	0/29	-84.9	-84.9	0.11 (1)	T-Q	-102/0	0.07 (1)
B-C	-9/0	-84.9	-84.9	0.09 (1)	U-P	-193/0	0.13 (1)
C-D	-25/0	-84.9	-84.9	0.09 (1)	V-O	-171/0	0.12 (1)
D-E	-7/0	-84.9	-84.9	0.04 (1)	W-N	-145/0	0.10 (1)
E-F	-7/0	-84.9	-84.9	0.04 (1)	Y-L	-141/0	0.10 (1)
F-G	-4/0	-84.9	-84.9	0.04 (1)	Z-K	-199/0	0.13 (1)
G-H	-4/0	-84.9	-84.9	0.04 (1)	AA-J	-139/0	0.09 (1)
H-I	-1/0	-84.9	-84.9	0.04 (1)	AC-I	-187/0	0.10 (1)
I-J	-9/0	-84.9	-84.9	0.04 (1)	AD-H	-166/0	0.18 (1)
J-K	0/1	-84.9	-84.9	0.05 (1)	AE-F	-168/0	0.12 (1)
K-L	0/1	-84.9	-84.9	0.05 (1)	AF-E	-174/0	0.08 (1)
L-M	0/1	-84.9	-84.9	0.03 (1)	AG-D	-139/0	0.04 (1)
M-N	0/1	-84.9	-84.9	0.03 (1)	AH-C	-254/0	0.05 (1)
N-O	0/1	-84.9	-84.9	0.04 (1)	X-M	-112/0	0.08 (1)
O-P	0/1	-84.9	-84.9	0.05 (1)	B-AH	0/19	0.00 (1)
P-Q	0/1	-84.9	-84.9	0.05 (1)			
Q-R	0/0	-84.9	-84.9	0.01 (1)			
AI-B	-235/0	0.0	0.0	0.02 (1)			
S-R	-29/0	0.0	0.0	0.01 (1)			
AI-AH	0/0	-38.5	-38.5	0.06 (3)			
AH-AG	0/11	-38.5	-38.5	0.06 (3)			
AG-AF	0/8	-38.5	-38.5	0.03 (3)			
AF-AE	0/6	-38.5	-38.5	0.03 (3)			
AE-AD	0/4	-38.5	-38.5	0.02 (3)			
AD-AC	0/2	-38.5	-38.5	0.02 (3)			
AC-AB	0/0	-38.5	-38.5	0.02 (3)			
AB-AA	0/0	-38.5	-38.5	0.02 (3)			
AA-Z	-1/0	-38.5	-38.5	0.02 (3)			
Z-Y	-1/0	-38.5	-38.5	0.02 (3)			
Y-X	-1/0	-38.5	-38.5	0.02 (3)			
X-W	-1/0	-38.5	-38.5	0.02 (3)			
W-V	-1/0	-38.5	-38.5	0.02 (3)			
V-U	-1/0	-38.5	-38.5	0.02 (3)			
U-T	-1/0	-38.5	-38.5	0.02 (3)			
T-S	0/0	-38.5	-38.5	0.02 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1), BC=0.08/1.00

(AG-AH:3), WB=0.18/1.00 (H-AD:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

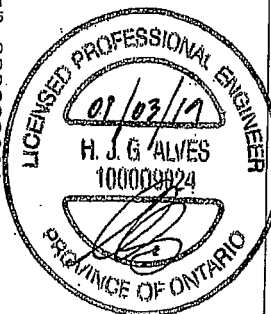
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 788 1987 1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP=0.60 (J) (INPUT = 0.90)
SI METAL=0.12 (C) (INPUT = 1.00)



DRWG NO. TAM T1900114
STRUCTURAL
COMPONENT ONLY

JOB NAME Tamarack Roof Truss, Burlington
TRUSS NAME T35
QUANTITY 1
PLY 2
JOB DESC. TRUSS DESC.
DRWG NO.

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ID: Sfh7mWrkTvM7KcGx?VheiCyrb3l-ZChVC5qvZDd8?JFgk7z?bwAZxiokr_jwB2frqzz0Up

Scale = 1/2"=1'-0"

TOTAL WEIGHT = 2 X 110 = 221 lb

LUMBER
L.G.A. RULES
CHORDS SIZE DRY LUMBER No.2 DESCR. SPF
- A 2x4 DRY No.2 SPF
- C 2x6 DRY No.2 SPF
- F 2x4 DRY No.2 SPF
- I 2x4 DRY No.2 SPF
- G 2x4 DRY No.2 SPF
ALL WEBS 2x4 DRY No.2 SPF
RY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS
A 1 12 TOP
C 1 12 TOP
G 2 12 SIDE(0.0)
F 2 12 SIDE(0.0)
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS
I 2 12 SIDE(0.0)
U 2 12 TOP
EBS : (0.122"x3") SPIRAL NAILS
1 6 TOP

ALLS TO BE DRIVEN FROM ONE SIDE ONLY.
ORDER NAILING ASSUMES NAILED HANGERS ARE STRENGTHENED WITH MIN. 3-0 INCH NAILS.
P - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR E LOAD TO BE TRANSFERRED TO EACH PLY.
E - PLF SHOWN IS THE EQUIVALENT UDL APPLIED ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. MAIN PLY MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

TES (table is in inches)
TYPE PLATES W LEN Y X
TMWV+p MT20 4.0 6.0
TMWW-t MT20 5.0 6.0
TS-t MT20 5.0 6.0
TMW+w MT20 3.0 6.0
TMWW-t MT20 5.0 6.0
BMV1+p MT20 4.0 6.0
BMW-t MT20 3.0 6.0
BMWW-t MT20 5.0 6.0
BMW-t MT20 5.0 6.0
BMWW-t MT20 5.0 6.0
BMW1+p MT20 3.0 6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
FACTORED MAXIMUM FACTORED INPUT REQ'D
GROSS REACTION GROSS REACTION BRG BRG
JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX
L 2499 0 2499 0 0 1-8 1-8
G 2034 0 2034 0 0 1-8 1-8

UNFACTORED REACTIONS
1ST CASE MAX/MIN. COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
L 1884 971/0 389/0 0/0 0/0 515/0 0/0
G 1528 804/0 310/0 0/0 0/0 414/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS
MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. UNBRACED LENGTH FR-TO CS1 (LC)
FR-TO L-A -2318/0 0.0 0.0 0.36 (1) 7.37 A-K 0/2873 0.25 (1)
A-M -2085/0 -84.9 -84.9 0.14 (1) 6.25 K-B -1693/0 0.18 (1)
M-N -2085/0 -84.9 -84.9 0.14 (1) 6.25 B-J 0/1119 0.10 (1)
N-O -2085/0 -84.9 -84.9 0.14 (1) 6.25 J-D -496/0 0.05 (1)
O-B -2085/0 -84.9 -84.9 0.14 (1) 6.25 E-E 0/2873 0.25 (1)
B-P -2884/0 -84.9 -84.9 0.14 (1) 6.25 H-E -333/0 0.14 (1)
P-C -2884/0 -84.9 -84.9 0.14 (1) 6.25 H-F -333/0 0.14 (1)
C-Q -2884/0 -84.9 -84.9 0.14 (1) 6.25 Q-D -2884/0 -84.9 -84.9 0.14 (1) 6.25
Q-D -2884/0 -84.9 -84.9 0.14 (1) 6.25 D-E -2884/0 -84.9 -84.9 0.06 (1) 6.25
D-E -2884/0 -84.9 -84.9 0.06 (1) 6.25 E-F -1943/0 -84.9 -84.9 0.06 (1) 6.25
E-F -1943/0 -84.9 -84.9 0.06 (1) 6.25 F-G -2004/0 0.0 0.0 0.31 (1) 7.73

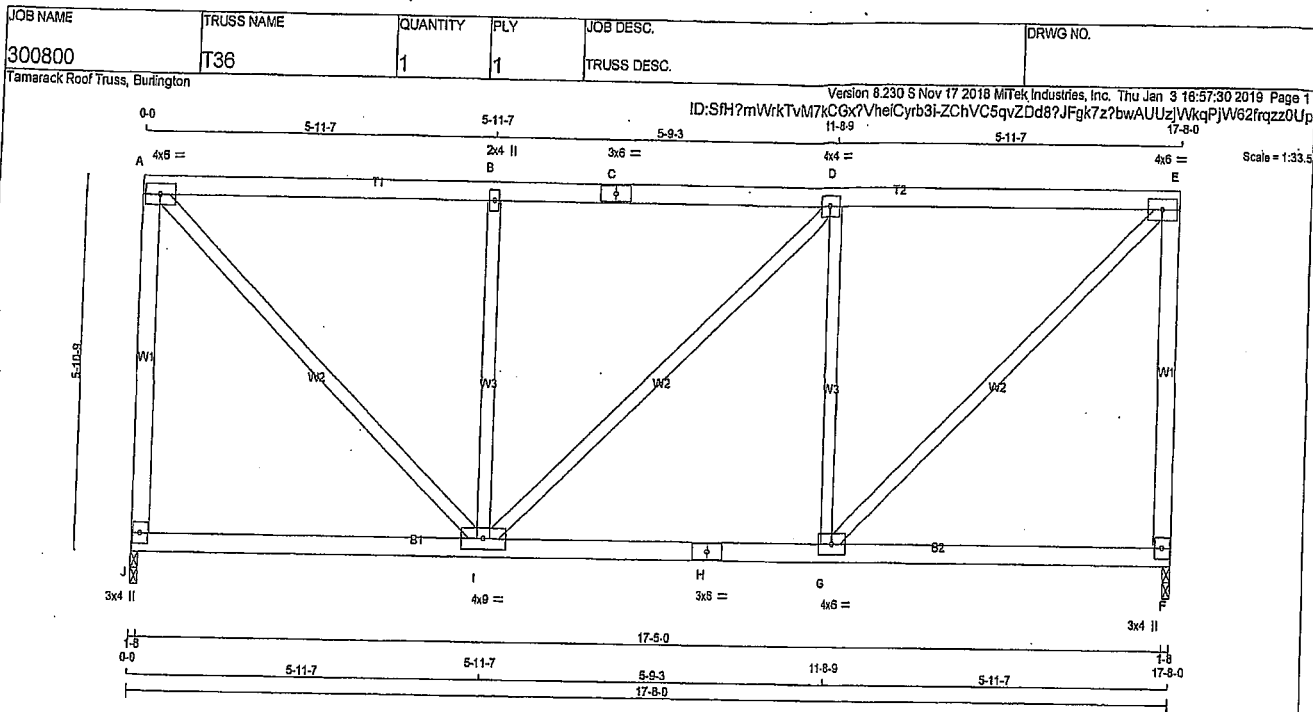
WEBS
MEMB. MAX. FACTORED FORCE (LBS) CS1 (LC)
L-R 0/0 -38.5 -38.5 0.09 (2) 10.00
R-S 0/0 -38.5 -38.5 0.09 (2) 10.00
S-T 0/0 -38.5 -38.5 0.09 (2) 10.00
T-K 0/0 -38.5 -38.5 0.09 (2) 10.00
K-U 0/2085 -38.5 -38.5 0.28 (1) 10.00
U-V 0/2085 -38.5 -38.5 0.28 (1) 10.00
V-J 0/2085 -38.5 -38.5 0.28 (1) 10.00
J-W 0/1943 -38.5 -38.5 0.39 (1) 10.00
W-I 0/1943 -38.5 -38.5 0.39 (1) 10.00
I-H 0/1943 -38.5 -38.5 0.39 (1) 10.00
H-G 0/0 -38.5 -38.5 0.09 (1) 10.00

FACTORED CONCENTRATED LOADS (LBS)
JT LOC. LC1 MAX+ MAX- FACE DIR. TYPE HEEL CONN.
M 1-0-12 -164 -164 FRONT VERT TOTAL
N 2-0-12 -164 -164 FRONT VERT TOTAL
O 4-0-12 -164 -164 FRONT VERT TOTAL
P 6-0-12 -164 -164 FRONT VERT TOTAL
Q 8-0-12 -164 -164 FRONT VERT TOTAL
R 1-0-12 -75 -95 FRONT VERT TOTAL
S 2-0-12 -75 -95 FRONT VERT TOTAL
T 4-0-12 -75 -95 FRONT VERT TOTAL
U 6-0-12 -75 -95 FRONT VERT TOTAL
V 8-0-12 -75 -95 FRONT VERT TOTAL
W 10-0-0 -1158 -1158 TOP VERT TOTAL

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
BOT CH. DL = 6.0 PSF
LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF
SPACING = 24.0 IN./C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014
(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")
CSI: TC=0.36/1.00 (A-L), BC=0.39/1.00 (H-I), WB=0.25/1.00 (A-K), SS=0.32/1.00 (H-I),
DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 1.00
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THIS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (FL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 619 354 1657 788 1987 1656
PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.
JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.47 (A) (INPUT = 1.00)

Stamp: LICENSED PROFESSIONAL ENGINEER H. J. G. ALVES 100009024 PROVINCE OF ONTARIO

Stamp: DWG NO. TAM 7900130 STRUCTURAL CALCULATIONS ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
J - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVVV-I	MT20	4.0	6.0		
B	TMVVV-w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMVVV-I	MT20	4.0	4.0		
E	TMVVV-I	MT20	4.0	6.0		
F	BMVVV-I	MT20	3.0	4.0		
G	BMVVV-I	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMVVV-I	MT20	4.0	9.0		
J	BMVVV-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
J	1090	0	1090	0	1-8	1-8
F	1090	0	1090	0	1-8	1-8

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
		1ST LOSE	2ND LOSE				
J	827	411/0	188/0	0/0	0/0	230/0	0/0
F	827	411/0	188/0	0/0	0/0	230/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
J-A	-996/0	0.0	0.0 0.65 (1)	G-E	0/1157	0.26 (1)	0.26 (1)
A-B	-842/0	-84.9	-84.9 0.42 (1)	A-I	0/1155	0.26 (1)	0.26 (1)
B-C	-842/0	-84.9	-84.9 0.42 (1)	G-D	-544/0	0.29 (1)	0.29 (1)
C-D	-842/0	-84.9	-84.9 0.42 (1)	I-B	-544/0	0.29 (1)	0.29 (1)
D-E	-843/0	-84.9	-84.9 0.42 (1)	I-D	-2/0	0.00 (1)	0.00 (1)
E-F	-996/0	0.0	0.0 0.65 (1)				
J-I	0/0	-38.5	-38.5 0.24 (3)				
I-H	0/843	-38.5	-38.5 0.34 (2)				
H-G	0/843	-38.5	-38.5 0.34 (2)				
G-F	0/0	-38.5	-38.5 0.24 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.65/1.00 (E-F:1), BC=0.34/1.00 (G-I:2), WB=0.29/1.00 (D-G:1), SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

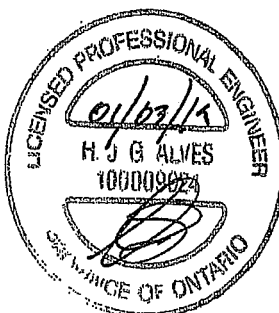
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (G) (INPUT = 0.90)
JSI METAL= 0.29 (H) (INPUT = 1.00)

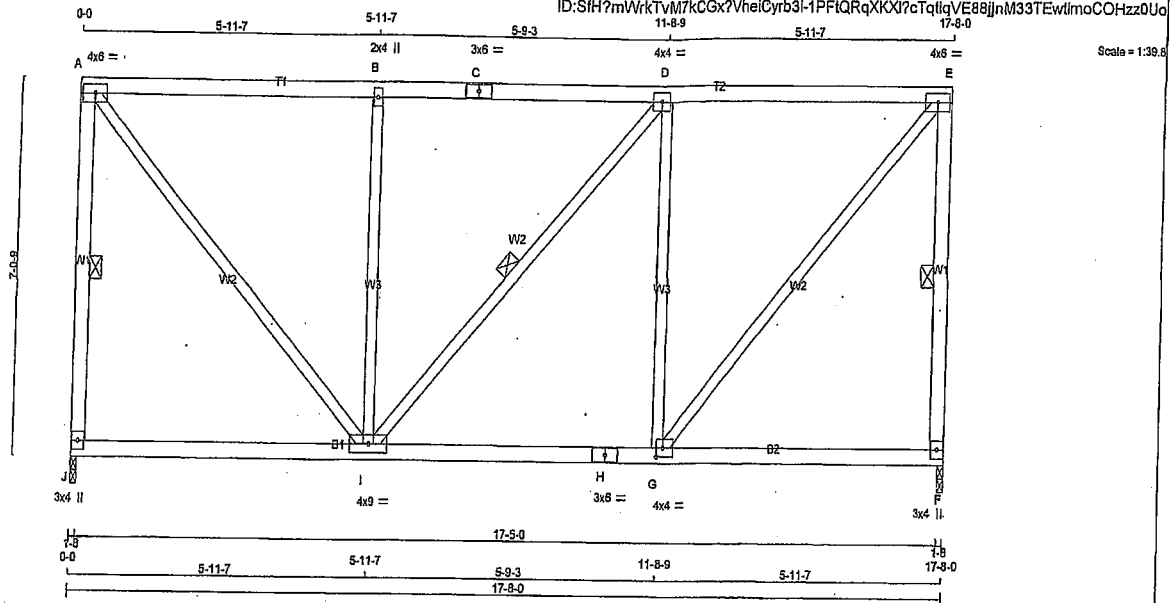


DRWG NO. TAM 71900/31
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T37	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:39.6

TOTAL WEIGHT = 86 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	J - A	2x4 DRY	No.2	SPF
	A - C	2x4 DRY	No.2	SPF
	C - E	2x4 DRY	No.2	SPF
	E - G	2x4 DRY	No.2	SPF
	G - H	2x4 DRY	No.2	SPF
	H - F	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF	
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	6.0			
B	TMVW-1	MT20	2.0	4.0			
C	TS-1	MT20	3.0	6.0			
D	TMVW-1	MT20	4.0	4.0			
E	TMVW-1	MT20	4.0	6.0			
F	BMV1-p	MT20	3.0	4.0			
G	BMVW-1	MT20	4.0	4.0	2.00	1.50	
H	BS-1	MT20	3.0	6.0			
I	BMVW-1	MT20	4.0	4.0			
J	BMV1-p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REORD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1090	0	1090	0	0
F	1090	0	1090	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
J	827	411/0	186/0
F	827	411/0	186/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-J, E-F, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED	VERT. LOAD	LC1 MAX	MEMB.	MAX. FACTORED
	(LBS)	(PLF)	CSI (LC)		(LBS)
FR-TO		FROM	TO	FR-TO	
J-A	-998/0	0.0	0.0 0.22 (1)	G-E	0/1056
A-B	-687/0	-84.9	-84.9 0.41 (1)	A-I	0/1054
B-C	-687/0	-84.9	-84.9 0.42 (1)	G-D	-545/0
C-D	-687/0	-84.9	-84.9 0.42 (1)	I-B	-544/0
D-E	-688/0	-84.9	-84.9 0.42 (1)	I-D	-2/0
E-F	-998/0	0.0	0.0 0.22 (1)		
J-I	0/0	-38.5	-38.5 0.24 (3)		
I-H	0/698	-38.5	-38.5 0.32 (2)		
H-G	0/698	-38.5	-38.5 0.32 (2)		
G-F	0/0	-38.5	-38.5 0.24 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.42/1.00 (D-E:1), BC=0.32/1.00 (G-I:2), WB=0.47/1.00 (D-G:1), SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

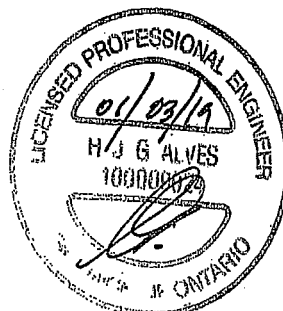
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (G) (INPUT = 0.90)
JSI METAL = 0.29 (H) (INPUT = 1.00)



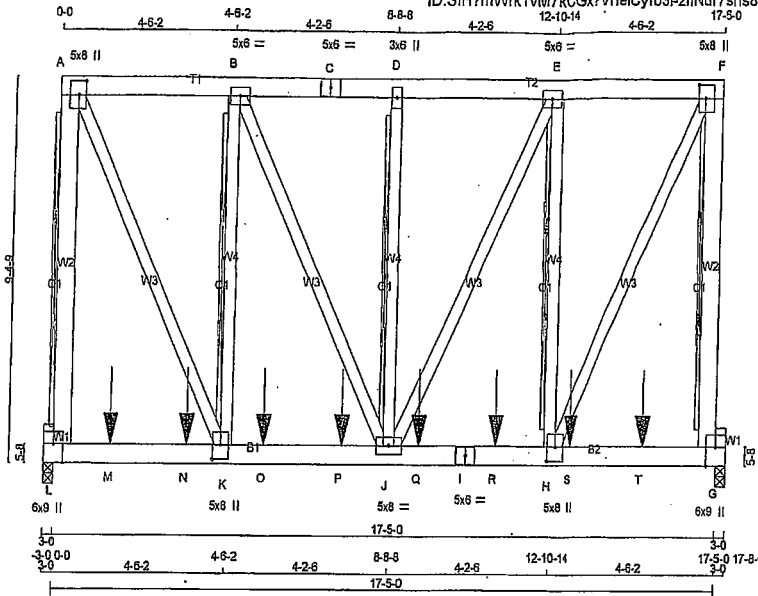
DWG NO. TAM 11900132
STRUCTURAL
CHECKED ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T39X	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:51.9

TOTAL WEIGHT = 3 X 166 = 498 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF	
A - C	2x6	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
G - F	2x6	DRY	No.2	SPF	
L - I	2x8	DRY	No.2	SPF	
I - G	2x8	DRY	No.2	SPF	

ALL WEBS 2x4 DRY No.2 SPF
DRY; SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
L-A 2	12	TOP
A-C 2	12	TOP
C-F 2	12	TOP
F-G 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
L-I 2	9	SIDE (451.2)
I-G 2	9	SIDE (451.2)
WEBS: (0.122"x3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	8.0	4.00	2.25
B	TMVW-i	MT20	5.0	6.0		
C	TS-I	MT20	5.0	6.0		
D	TMVW+w	MT20	3.0	6.0		
E	TMVW-i	MT20	5.0	6.0		
F	TMVW+p	MT20	5.0	8.0	4.00	2.25
G	BMVW+i	MT20	6.0	9.0	Edge	3.00
H	BMVW+i	MT20	5.0	8.0	4.25	2.50
I	CS-I	MT20	5.0	6.0		
J	BMVW+w	MT20	5.0	8.0		
K	BMVW+i	MT20	5.0	8.0	4.25	2.50
L	BMVW+i	MT20	6.0	9.0	Edge	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
L	8175	0	8175	0
G	7811	0	7811	0

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
L	6177	3146 / 0 1332 / 0 0 / 0 1699 / 0 0 / 0
G	5902	3005 / 0 1273 / 0 0 / 0 1623 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT A-L, F-G, E-H, B-K, D-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE
(LBS)	(PLF)	CS1 (LC)	UNBRAC	(LBS)	CS1 (LC)
FR-TO	FROM TO	LENGTH	FR-TO		
L-A	-8951 / 0	0.0	0.0 0.54 (1)	H-E	-2276 / 0
A-B	-3419 / 0	-84.9	-84.9 0.05 (1)	K-B	-2227 / 0
B-C	-4294 / 0	-84.9	-84.9 0.05 (1)	J-D	-367 / 0
C-D	-4294 / 0	-84.9	-84.9 0.05 (1)	H-F	0 / 7527
D-E	-4294 / 0	-84.9	-84.9 0.05 (1)	A-K	0 / 7527
E-F	-3396 / 0	-84.9	-84.9 0.05 (1)	J-E	0 / 2109
G-F	-6906 / 0	0.0	0.0 0.54 (1)	B-J	0 / 2109
L-M	0 / 0	-38.5	-38.5 0.42 (1)		
M-N	0 / 0	-38.5	-38.5 0.42 (1)		
N-K	0 / 0	-38.5	-38.5 0.42 (1)		
K-O	0 / 3419	-38.5	-38.5 0.56 (1)		
O-P	0 / 3419	-38.5	-38.5 0.56 (1)		
P-J	0 / 3419	-38.5	-38.5 0.56 (1)		
J-Q	0 / 3396	-38.5	-38.5 0.52 (1)		
Q-I	0 / 3396	-38.5	-38.5 0.52 (1)		
I-R	0 / 3396	-38.5	-38.5 0.52 (1)		
R-H	0 / 3396	-38.5	-38.5 0.52 (1)		
H-S	0 / 0	-38.5	-38.5 0.39 (1)		
S-T	0 / 0	-38.5	-38.5 0.39 (1)		
T-G	0 / 0	-38.5	-38.5 0.39 (1)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEB.	CONN.
M	1-5-12	-1730	-1730		BACK	VERT	TOTAL		
N	3-5-12	-1730	-1730		BACK	VERT	TOTAL		
O	5-5-12	-1730	-1730		BACK	VERT	TOTAL		
P	7-5-12	-1730	-1730		BACK	VERT	TOTAL		
Q	9-5-12	-1730	-1730		BACK	VERT	TOTAL		
R	11-5-12	-1730	-1730		BACK	VERT	TOTAL		
S	13-5-12	-1730	-1730		BACK	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	6.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.56")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.58")
CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.54/1.00 (A-L:1), BC=0.56/1.00 (J-K:1), WB=0.41/1.00 (A-K:1), SSI=0.55/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP=0.84 (A) (INPUT = 0.90)
SI METAL=0.47 (A) (INPUT = 1.00)

DRWG NO. TAM 1900134
STRUCTURAL
EXPERIMENTAL ONLY

1/2

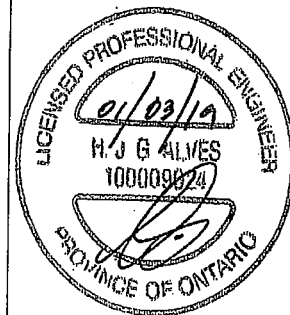
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T39X	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 16:57:33 2019 Page 2
ID:Sfh7mWkTvM7kCGx7VhelCvrb3fznNdr7sns8?ism FQFXIDZo1JAhrx9MAC4HJS9zz0Ujm

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	15-5-12	-1730	-1730	—	TOP	VERT	TOTAL	—	—



DRWG NO. TAM 71900134
STRUCTURAL
CORPORATION ONLY 7/2

JOB NAME 300800	TRUSS NAME G40	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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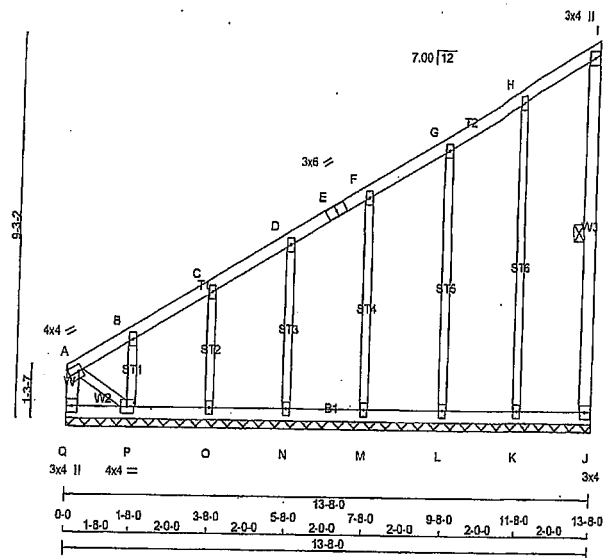
Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Thu Jan 3 16:57:23 2019 Page 1

ID: Sfh?mWrkTvM7kCGx?VheICyrb3f-GsmrkikWC4i7fEDKq9LmPSNRf8PmbhRhvXsn6kzz0Uw

0-0 1-8-0 1-8-0 2-0-0 3-8-0 2-0-0 5-8-0 2-0-0 7-8-0 2-0-0 9-8-0 2-0-0 11-8-0 2-0-0 13-8-0

Scale = 1:51.6



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - I	2x4	DRY	No.2
Q - A	2x4	DRY	No.2
Q - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV-t	MT20	4.0	4.0	2.00	Edge
B, C, D, F, G, H						
B	TMV+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	BMV1+p	MT20	3.0	4.0		
K, L, M, N, O						
K	BMV1+w	MT20	2.0	4.0		
P	BMVV1-t	MT20	4.0	4.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-14/0	-84.9 -84.9	0.04 (1)	6.25	K-H	-190/0	0.24 (1)
B-C	-15/0	-84.9 -84.9	0.04 (1)	6.25	L-G	-168/0	0.14 (1)
C-D	-10/0	-84.9 -84.9	0.04 (1)	6.25	M-F	-169/0	0.09 (1)
D-E	-7/0	-84.9 -84.9	0.04 (1)	10.00	N-D	-168/0	0.05 (1)
E-F	-7/0	-84.9 -84.9	0.04 (1)	10.00	O-C	-168/0	0.03 (1)
F-G	-4/0	-84.9 -84.9	0.04 (1)	10.00	P-B	-165/0	0.03 (1)
G-H	0/0	-84.9 -84.9	0.04 (1)	10.00	A-P	0/22	0.01 (1)
H-I	-8/0	-84.9 -84.9	0.04 (1)	10.00			
J-I	-72/0	0.0 0.0	0.03 (1)	6.25			
Q-A	-73/0	0.0 0.0	0.01 (1)	7.81			
Q-P	0/0	-38.5 -38.5	0.02 (3)	10.00			
P-O	0/13	-38.5 -38.5	0.02 (3)	10.00			
O-N	0/9	-38.5 -38.5	0.02 (3)	10.00			
N-M	0/6	-38.5 -38.5	0.02 (3)	10.00			
M-L	0/4	-38.5 -38.5	0.02 (3)	10.00			
L-K	0/2	-38.5 -38.5	0.03 (3)	10.00			
K-J	0/0	-38.5 -38.5	0.03 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3	PSF
DL	= 6.0	PSF
BOT CH. LL	= 10.5	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 46.8	PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 085-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1 TC=0.04/1.00 (H-K-1), BC=0.03/1.00 (K-L-3), WB=0.24/1.00 (H-K-1), SS1=0.07/1.00 (H-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

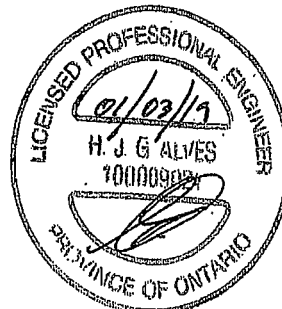
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX	MIN	MAX MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (H) (INPUT = 0.90)
JSI METAL= 0.09 (H) (INPUT = 1.00)



DRWG NO. TAM 71900123
STRUCTURAL
COMPANY ONLY

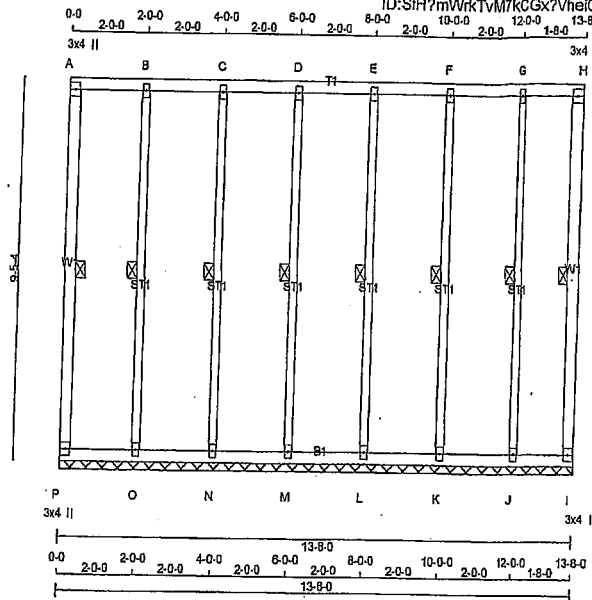
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	G41	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:57:24 2019 Page 1

ID: S/H?mWkTvm7KCGx7VheICyrb3i-k2KDy2l8zNt_HoXOstbMfwcTYi0KAor8AbLeBzz0Uv

Scale = 1:52.7



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
P - A	2x4	DRY	No.2
A - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - I	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMV+p	MT20	3.0	4.0		
B, C, D, E, F, G					
B TMV+w	MT20	2.0	4.0		
H TMV+p	MT20	3.0	4.0		
I BMV1+p	MT20	3.0	4.0		
J, K, L, M, N, O					
J BMV1+w	MT20	2.0	4.0		
P BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, H-I, B-O, C-N, D-M, E-L, F-K, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS			WEBS		
	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
P-A		-74 / 0	0.0 0.0	0.01 (1)	C-B	-180 / 0
A-B		-1 / 0	-84.9 -84.9	0.04 (1)	N-C	-170 / 0
B-C		-1 / 0	-84.9 -84.9	0.04 (1)	M-D	-170 / 0
C-D		-1 / 0	-84.9 -84.9	0.04 (1)	L-E	-169 / 0
D-E		-1 / 0	-84.9 -84.9	0.04 (1)	K-F	-174 / 0
E-F		-1 / 0	-84.9 -84.9	0.04 (1)	J-G	-161 / 0
F-G		-1 / 0	-84.9 -84.9	0.04 (1)		
G-H		-1 / 0	-84.9 -84.9	0.03 (1)		
H-I		-62 / 0	0.0 0.0	0.01 (1)		
P-O		0 / 1	-38.5 -38.5	0.03 (3)		
O-N		0 / 1	-38.5 -38.5	0.03 (3)		
N-M		0 / 1	-38.5 -38.5	0.02 (3)		
M-L		0 / 1	-38.5 -38.5	0.02 (3)		
L-K		0 / 1	-38.5 -38.5	0.02 (3)		
K-J		0 / 1	-38.5 -38.5	0.02 (3)		
J-I		0 / 1	-38.5 -38.5	0.02 (3)		

TOTAL WEIGHT = 95 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (E-F:1), BC=0.03/1.00 (N-O:3), WB=0.11/1.00 (B-O:1), SS=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

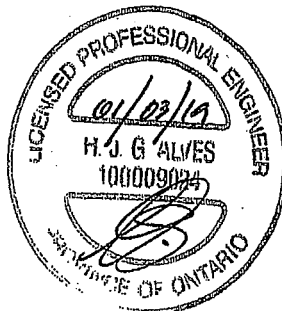
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1687 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (O) (INPUT = 0.90)
JSI METAL= 0.04 (O) (INPUT = 1.00)



DRWG NO. TAM T79 00124
STRUCTURAL
CONTRACT ONLY

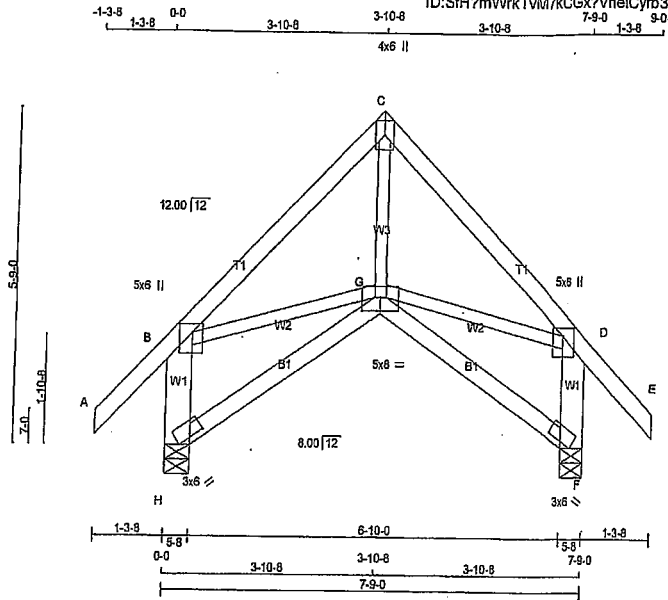
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	T42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MTek Industries, Inc. Thu Jan 3 16:57:34 2019 Page 1

ID: Sfh7mWrkTvm7kCGx?VheiCyrb3i-R_x?2TTPdS7ZUwZSzz2xmmLH_a7gghtJRk0t_czz0U

Scale = 1:36.6



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
H - B	2x6 DRY	No.2	SPF
F - D	2x6 DRY	No.2	SPF
H - G	2x4 DRY	No.2	SPF
G - F	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TTW+p	MT20	4.0	6.0		
D	TMVW+p	MT20	5.0	6.0	1.75	2.75
F	BVM1-I	MT20	3.0	6.0	0.50	3.00
G	BBWW+p	MT20	5.0	8.0	2.75	4.00
H	BVM1-I	MT20	3.0	6.0	0.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	596	0	596	0
F	596	0	596	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	444	245/0	81/0	0/0	0/0	117/0	0/0
F	444	245/0	81/0	0/0	0/0	117/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	
FR-TO		FROM TO		FR-TO			
A-B	0/42	-84.9 -84.9 0.12 (1)	10.00	G-C	0/299	0.07 (2)	
B-C	-424/0	-84.9 -84.9 0.17 (1)	6.25	B-G	0/306	0.07 (1)	
C-D	-424/0	-84.9 -84.9 0.17 (1)	6.25	G-D	0/306	0.07 (1)	
D-E	0/42	-84.9 -84.9 0.12 (1)	10.00				
H-B	-522/0	0.0 0.0 0.04 (1)	7.81				
F-D	-522/0	0.0 0.0 0.04 (1)	7.81				
H-G	0/0	-38.5 -38.5 0.14 (3)	10.00				
G-F	0/0	-38.5 -38.5 0.14 (3)	10.00				

TOTAL WEIGHT = 42 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.26")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.26")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.17/1.00 (C-D:1), BC=0.14/1.00 (F-G:3), WB=0.07/1.00 (B-G:1), SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

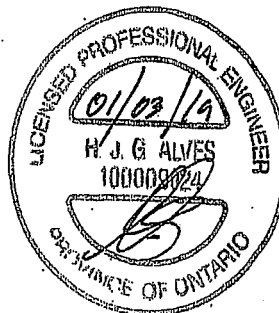
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (G) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)



ENGINEER TAM 11900135
STRUCTURAL
QUALITY CONTROL

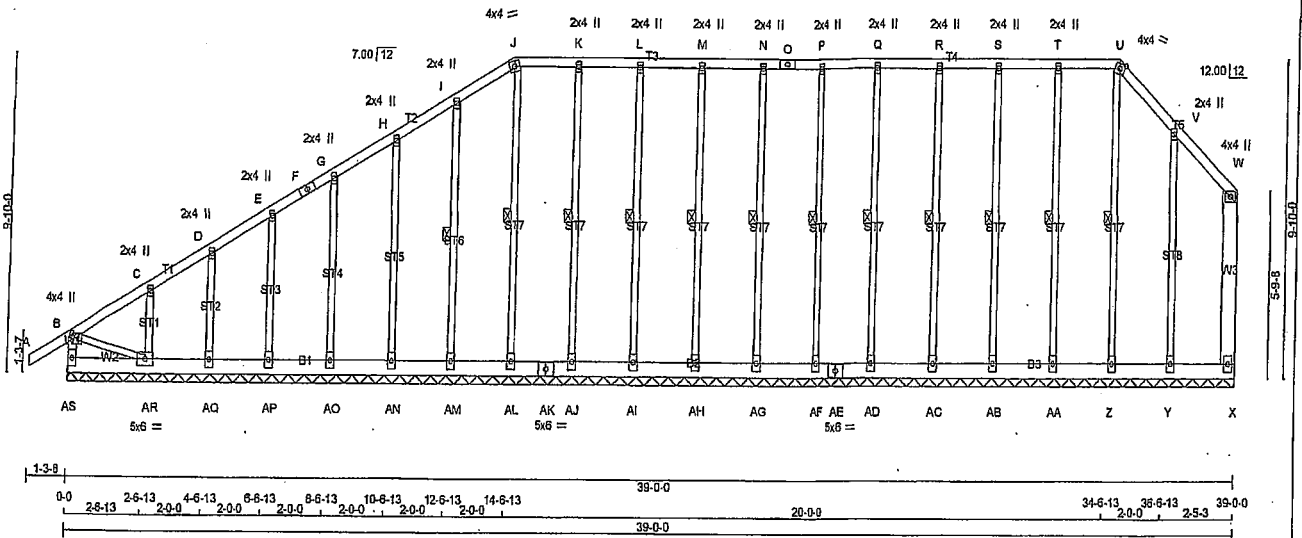
JOB NAME 300803	TRUSS NAME G44	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 17:08:29 2019 Page 1
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1-3-8 0-0 2-6-13 2-0-0 4-6-13 2-0-0 6-6-13 2-0-0 8-6-13 2-0-0 10-6-13 2-0-0 12-6-13 2-0-0 14-6-13 2-0-0 16-6-13 2-0-0 18-11-0 34-6-13 34-11-8 36-6-13 39-0-0 4-11 1-7-5 2-5-3

Scale = 1:65.0



TOTAL WEIGHT = 246 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - F	2x4	DRY No.2	SPF
F - J	2x4	DRY No.2	SPF
J - O	2x4	DRY No.2	SPF
O - U	2x4	DRY No.2	SPF
U - W	2x4	DRY No.2	SPF
AS - B	2x4	DRY No.2	SPF
X - W	2x6	DRY No.2	SPF
AK - AK	2x6	DRY No.2	SPF
AK - AE	2x6	DRY No.2	SPF
AE - X	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I, K, L, M, N, P, Q, R, S, T, V						
C	TMVW+p	MT20	2.0	4.0		
F	TS-1	MT20	3.0	6.0		
J	TS-1	MT20	4.0	4.0		
O	TS-1	MT20	3.0	6.0		
U	TS-1	MT20	4.0	4.0	Edge	
W	TMVW+p	MT20	4.0	4.0		
X	BMV1+p	MT20	3.0	6.0		
Y, Z, AA, AB, AC, AD, AF, AG, AH, AI, AJ, AL, AM, AN, AO, AP, AQ						
Y	BMV1+w	MT20	3.0	6.0		
AE	BS-1	MT20	5.0	6.0		
AK	BS-1	MT20	5.0	6.0		
AR	BMVW1-t	MT20	5.0	6.0		
AS	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF P-AF, N-AG, M-AH, L-AI, K-AJ, J-AL, I-AM, Q-AD, R-AC, S-AB, T-AA, U-Z.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO								
A-B	0/29	-84.9	-84.9	0.11 (1)	10.00	AF-P	-169/0	0.11 (1)
B-C	-7/10	-84.9	-84.9	0.08 (1)	10.00	AG-N	-170/0	0.11 (1)
C-D	-18/0	-84.9	-84.9	0.08 (1)	6.25	AH-M	-170/0	0.11 (1)
D-E	-4/0	-84.9	-84.9	0.04 (1)	10.00	AI-L	-166/0	0.10 (1)
E-F	-3/0	-84.9	-84.9	0.04 (1)	10.00	AJ-K	-195/0	0.12 (1)
F-G	-3/0	-84.9	-84.9	0.04 (1)	10.00	AL-J	-141/0	0.09 (1)
G-H	0/0	-84.9	-84.9	0.04 (1)	10.00	AM-I	-167/0	0.09 (1)
H-I	0/3	-84.9	-84.9	0.05 (1)	10.00	AN-H	-165/0	0.16 (1)
I-J	-5/0	-84.9	-84.9	0.04 (1)	10.00	AO-G	-168/0	0.10 (1)
J-K	0/5	-84.9	-84.9	0.05 (1)	10.00	AP-E	-173/0	0.07 (1)
K-L	0/4	-84.9	-84.9	0.05 (1)	10.00	AQ-D	-147/0	0.03 (1)
L-M	0/4	-84.9	-84.9	0.04 (1)	10.00	AR-C	-233/0	0.04 (1)
M-N	0/4	-84.9	-84.9	0.04 (1)	10.00	AD-Q	-175/0	0.11 (1)
N-O	0/4	-84.9	-84.9	0.04 (1)	10.00	AC-R	-175/0	0.11 (1)
O-P	0/4	-84.9	-84.9	0.04 (1)	10.00	AB-S	-165/0	0.10 (1)
P-Q	0/4	-84.9	-84.9	0.04 (1)	10.00	AA-T	-195/0	0.12 (1)
Q-R	0/4	-84.9	-84.9	0.04 (1)	10.00	Z-U	-141/0	0.09 (1)
R-S	0/4	-84.9	-84.9	0.04 (1)	10.00	Y-V	-207/0	0.23 (1)
S-T	0/4	-84.9	-84.9	0.05 (1)	10.00	B-A	0/16	0.00 (1)
T-U	0/5	-84.9	-84.9	0.05 (1)	10.00			
U-V	-10/0	-84.9	-84.9	0.05 (1)	10.00			
V-W	0/14	-84.9	-84.9	0.06 (1)	10.00			
AS-B	-222/0	0.0	0.0	0.02 (1)	7.81			
X-W	-57/0	0.0	0.0	0.03 (1)	7.81			
AS-AR	0/0	-38.5	-38.5	0.02 (3)	10.00			
AR-AQ	0/8	-38.5	-38.5	0.02 (3)	10.00			
AQ-AP	0/5	-38.5	-38.5	0.01 (3)	10.00			
AP-AO	0/3	-38.5	-38.5	0.01 (3)	10.00			
AO-AN	0/0	-38.5	-38.5	0.01 (3)	10.00			
AN-AM	-1/0	-38.5	-38.5	0.01 (3)	10.00			
AM-AL	-3/0	-38.5	-38.5	0.01 (3)	10.00			
AL-AK	-4/0	-38.5	-38.5	0.01 (3)	10.00			
AK-AJ	-4/0	-38.5	-38.5	0.01 (3)	10.00			
AJ-AI	-4/0	-38.5	-38.5	0.01 (3)	10.00			
AI-AH	-4/0	-38.5	-38.5	0.01 (3)	10.00			
AH-AG	-4/0	-38.5	-38.5	0.01 (3)	10.00			
AG-AF	-4/0	-38.5	-38.5	0.01 (3)	10.00			
AF-AE	-4/0	-38.5	-38.5	0.01 (3)	10.00			
AE-AD	-4/0	-38.5	-38.5	0.01 (3)	10.00			
AD-AC	-4/0	-38.5	-38.5	0.01 (3)	10.00			
AC-AB	-4/0	-38.5	-38.5	0.01 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1), BC=0.02/1.00 (A-Q:3), WB=0.23/1.00 (V-Y:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

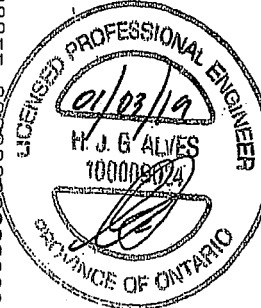
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (U) (INPUT = 0.90)

JSI METAL= 0.11 (V) (INPUT = 1.00)



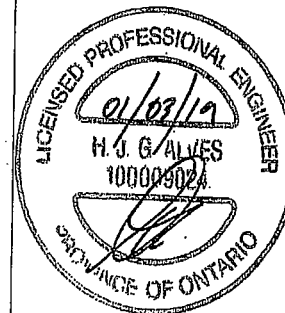
DWG NO. TAM T19 00 138
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	FR-TO	MAX. CSI (LC)
FR-TO		FROM	TO						
AB-AA	-4/0	-38.5	-38.5	0.01 (3)	10.00				
AA-Z	-4/0	-38.5	-38.5	0.01 (3)	10.00				
Z-Y	-3/0	-38.5	-38.5	0.02 (3)	10.00				
Y-X	0/0	-38.5	-38.5	0.02 (3)	10.00				



DWG NO. TAM T1900138
STRUCTURAL
COMPONENT ONLY

4/2

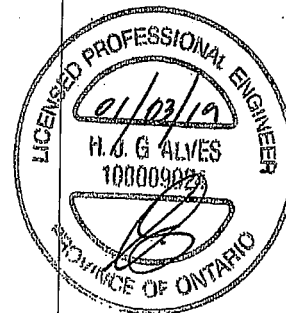
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300803	T45	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID: Sfh?mWrkTvM7kCGx?VheICyrb3f-emF8MmQw931CR64INUzk7u2DscaefJzm9Edvzz0KS

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	8-0-4	-2367	-2367	—	FRONT	VERT	TOTAL	—	—
J	3-5-12	-27	-35	—	BACK	VERT	TOTAL	—	—
L	5-5-12	-80	-80	—	BACK	VERT	TOTAL	—	—
M	7-5-12	-80	-80	—	BACK	VERT	TOTAL	—	—
N	9-5-12	-80	-80	—	BACK	VERT	TOTAL	—	—
O	1-11-4	-24	-35	—	BACK	VERT	TOTAL	—	—
P	2-3-4	-2483	-2483	—	FRONT	VERT	TOTAL	—	—
R	4-3-4	-2483	-2483	—	FRONT	VERT	TOTAL	—	—
S	5-5-12	-27	-35	—	BACK	VERT	TOTAL	—	—
T	6-3-4	-2483	-2483	—	FRONT	VERT	TOTAL	—	—
U	7-5-12	-27	-35	—	BACK	VERT	TOTAL	—	—
V	9-5-12	-27	-35	—	BACK	VERT	TOTAL	—	—
W	12-0-4	-2367	-2367	—	FRONT	VERT	TOTAL	—	—
X	13-9-12	-2367	-2367	—	FRONT	VERT	TOTAL	—	—
	15-9-12	-2367	-2367	—	FRONT	VERT	TOTAL	—	—



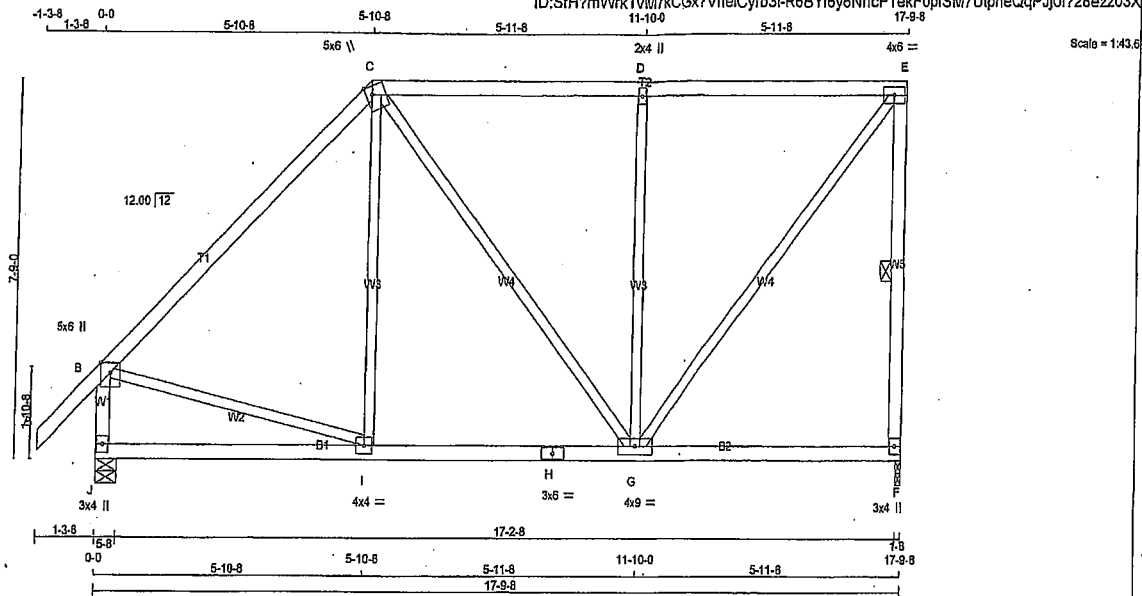
DRWG NO. TAM T1900142
STRUCTURAL
COMPONENT 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	T48A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 87 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMW+P	MT20	5.0	6.0	Edge
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TMW+w	MT20	2.0	4.0	
E	TMW+V	MT20	4.0	6.0	
F	BMV1+P	MT20	3.0	4.0	
G	BMW+V	MT20	4.0	9.0	
H	BS-t	MT20	3.0	6.0	
I	BMW+V	MT20	4.0	4.0	
J	BMV1+P	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	1097	0	1097	0	0	1-8	1-8		
J	1216	0	1216	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	COMBINED	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
J	COMBINED	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD (PLF)	LC1	MAX.	MEMB.	FORCE (LBS)	MAX.	CS1 (LC)
FR-TO		FROM	TO			FR-TO			
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	I-C	0 / 235	0.06 (3)	
B-C	-850 / 0	-84.9	-84.9	0.58 (1)	5.75	C-G	0 / 81	0.02 (1)	
C-D	-651 / 0	-84.9	-84.9	0.53 (1)	6.25	G-D	-525 / 0	0.70 (1)	
D-E	-651 / 0	-84.9	-84.9	0.53 (1)	6.25	G-E	0 / 1039	0.23 (1)	
F-E	-1003 / 0	0.0	0.0	0.27 (1)	6.25	B-I	0 / 618	0.14 (1)	
J-B	-1123 / 0	0.0	0.0	0.13 (1)	7.48				
J-I	0 / 0	-38.5	-38.5	0.24 (3)	10.00				
I-H	0 / 801	-38.5	-38.5	0.32 (2)	10.00				
H-G	0 / 801	-38.5	-38.5	0.32 (2)	10.00				
G-F	0 / 0	-38.5	-38.5	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, NBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.59")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.05")
ALLOWABLE DEFL. (TL) = $L/360$ (0.59")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.09")

CS1: TC=0.58/1.00 (B-C-1), BC=0.32/1.00 (G-I-2), WB=0.70/1.00 (D-G-1), SS=0.25/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

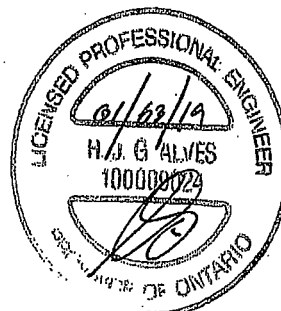
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (E) (INPUT = 0.90)
JSI METAL = 0.33 (B) (INPUT = 1.00)

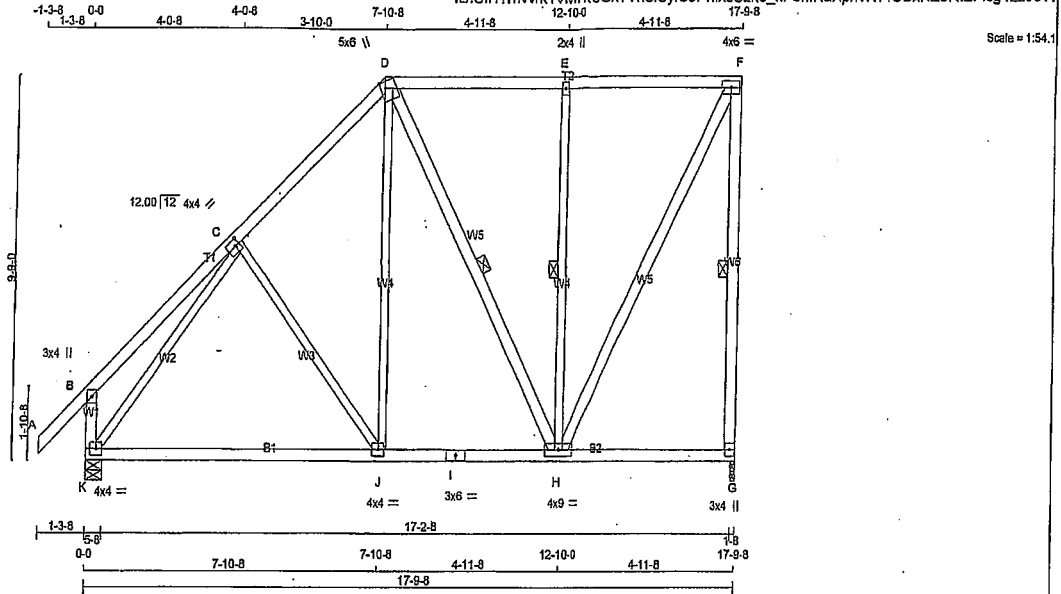


STAMPED T400153
STAMPED
01/22/2019

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	T49A	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:SH7mWkTVM7kCGx7VheICyrb3i-vlksSzK8_kF5nlRaXphvK14UBxRZoKIEPlcg4zz03VV



TOTAL WEIGHT = 8 X 108 = 861 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
G - F	2x4 DRY	No.2	SPF
K - B	2x4 DRY	No.2	SPF
K - I	2x4 DRY	No.2	SPF
I - G	2x4 DRY	No.2	SPF

ALL WEBS EXCEPT	2x3 DRY	No.2	SPF
D - H	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	2.00 1.25
D	TTWW+m	MT20	5.0	6.0	2.00 1.50
E	TMV+w	MT20	2.0	4.0	
F	TMV-w	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMWWW-t	MT20	4.0	9.0	
I	BS-t	MT20	3.0	6.0	
J	BMWW-t	MT20	4.0	4.0	
K	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	IN-SX
G	1097	0	1097	0	0	0	1-8	1-8	
K	1216	0	1216	0	0	0	5-8	5-8	

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	832	414/0	187/0	0/0	0/0	231/0	0/0
K	914	479/0	187/0	0/0	0/0	248/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	C-J	-127 / 25	0.11 (1)
B-C	0 / 28	-84.9	-84.9	0.21 (1)	10.00	J-D	0 / 444	0.10 (2)
C-D	-780 / 0	-84.9	-84.9	0.23 (1)	6.25	D-H	-190 / 0	0.11 (2)
D-E	-449 / 0	-84.9	-84.9	0.35 (1)	6.25	H-E	-519 / 0	0.33 (1)
E-F	-450 / 0	-84.9	-84.9	0.35 (1)	6.25	H-F	0 / 862	0.15 (1)
G-F	-1012 / 0	0.0	0.0	0.46 (1)	6.23	K-C	-1040 / 0	0.84 (1)
K-B	-244 / 0	0.0	0.0	0.03 (1)	7.81			
K-J	0 / 607	-38.5	-38.5	0.46 (2)	10.00			
J-I	0 / 535	-38.5	-38.5	0.45 (2)	10.00			
I-H	0 / 535	-38.5	-38.5	0.45 (2)	10.00			
H-G	0 / 0	-38.5	-38.5	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. G/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/800 (0.27")

CSI: TC=0.46/1.00 (F-G:1), BC=0.46/1.00 (J-K:2),
WB=0.84/1.00 (C-K:1), SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

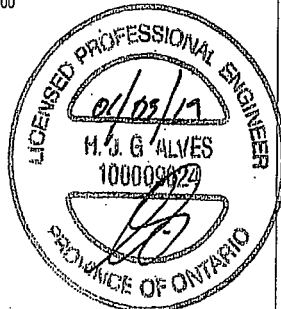
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)



ENGINEER T4900159
STRUCTURAL
ALL IN CONFORM

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	T50A	1	1	TRUSS DESC.	

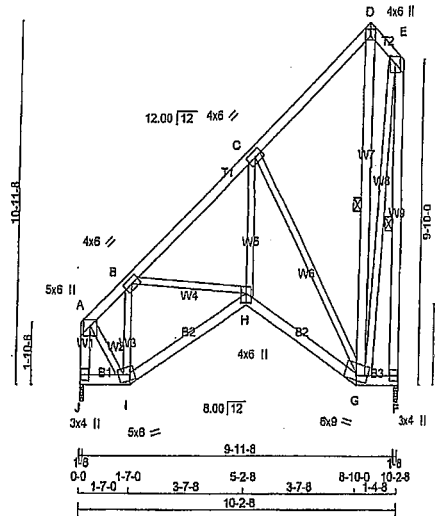
Tamarack Roof Truss, Burlington

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ID: Sfh7mWkTvm7kCGx?VheICyrb3i-vlxSszk8_kf5nRaXphvK176B0rZoBIEPlcg4zz03VM

0-0 1-7-0 3-7-8 5-2-8 3-10-8 8-1-0 10-2-8
1-7-0 3-7-8 5-2-8 3-10-8 8-1-0 10-2-8
4x4 =

Scale: 3/16"=1'



TOTAL WEIGHT = 79 lb

[M/JF]

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
G - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-i	MT20	4.0	6.0		
C	TMWW-i	MT20	4.0	6.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMWW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BBWWWW-m	MT20	6.0	9.0	Edge	
H	BBWW+p	MT20	4.0	6.0	3.50	2.00
I	BBWW-m	MT20	5.0	6.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	630	0	630	0
F	630	0	630	0

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	478	238 / 0	107 / 0	0 / 0	0 / 0	133 / 0	0 / 0
F	478	238 / 0	107 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-G, E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	FORCE (LBS)
FR-TO		FROM TO		
A-B	-389 / 0	-84.9 -84.9 0.07 (1)	6.25	I-B -373 / 0 0.07 (1)
B-C	-554 / 0	-84.9 -84.9 0.14 (1)	6.25	G-D -34 / 42 0.02 (1)
C-D	-122 / 0	-84.9 -84.9 0.14 (1)	6.25	A-I 0 / 410 0.09 (1)
D-E	-100 / 0	-84.9 -84.9 0.01 (1)	6.25	G-E 0 / 490 0.11 (1)
J-A	-599 / 0	0.0 0.0 0.07 (1)	7.81	H-C 0 / 572 0.13 (1)
F-E	-603 / 0	0.0 0.0 0.29 (1)	6.25	B-H 0 / 118 0.03 (1)
				C-G -691 / 0 0.85 (1)
J-I	0 / 0	-38.5 -38.5 0.02 (3)	10.00	
I-H	0 / 342	-38.5 -38.5 0.15 (2)	10.00	
H-G	0 / 466	-38.5 -38.5 0.18 (2)	10.00	
G-F	0 / 0	-38.5 -38.5 0.02 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.29/1.00 (E-F:1), BC=0.18/1.00 (G-H:2), WB=0.85/1.00 (C-G:1), SI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

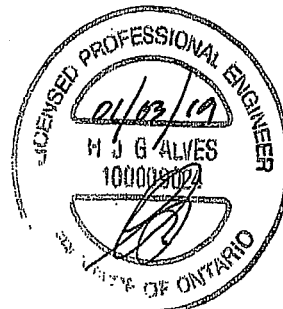
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818 354 1667 788 1987 1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.68 (I) (INPUT = 0.90)
JSI METAL= 0.18 (G) (INPUT = 1.00)



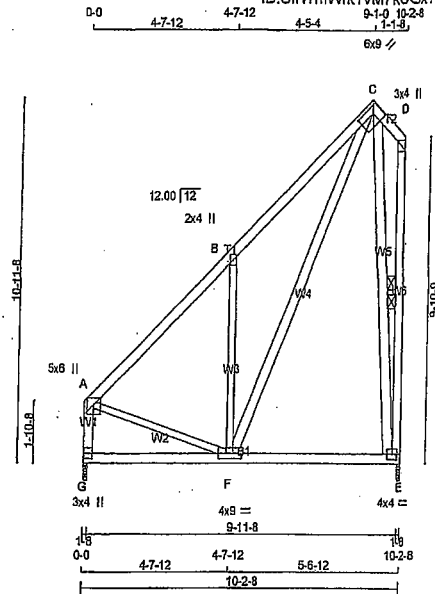
ENGR. 1900/155
SPECIAL
CLERK 1900/155

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	T51A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID: Sfh7mVrkTvM7kCGx7VheiCyrb3i-NVJJ4o_MvIs6lxd8ELwRYZJ_bKEIND0T3U9CWz03V



Scale: 3/16"=1'

TOTAL WEIGHT = 74 lb

LUMBER

N L G A RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
F - B	2x3	DRY	No.2	SPF
A - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMVW+w	MT20	2.0	4.0		
C	TTWW-i	MT20	6.0	9.0	2.25	6.00
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-i	MT20	4.0	4.0		
F	BMVWW-i	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
G	630	0	630	0	0	1-8	1-8		
E	630	0	630	0	0	1-8	1-8		

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	478	238 / 0	107 / 0	0 / 0	0 / 0	133 / 0	0 / 0
E	478	238 / 0	107 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E, C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)
FR-TO		FROM	TO	FR-TO			
A-B	-413 / 0	-84.9	-84.9	0.22 (1)	6.25	F-B	-469 / 0 0.32 (1)
B-C	-467 / 0	-84.9	-84.9	0.22 (1)	6.25	F-C	0 / 659 0.11 (1)
C-D	0 / 0	-84.9	-84.9	0.02 (1)	10.00	A-F	0 / 332 0.07 (1)
G-A	-565 / 0	0.0	0.0	0.06 (1)	7.81	C-E	-499 / 0 0.30 (1)
E-D	-48 / 0	0.0	0.0	0.02 (1)	6.25		
G-F	0 / 0	-38.5	-38.5	0.23 (3)	10.00		
F-E	0 / 53	-38.5	-38.5	0.23 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.22/1.00 (B-C:1), BC=0.23/1.00 (E-F:3),
WB=0.32/1.00 (B-F:1), SSI=0.14/1.00 (E-F:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

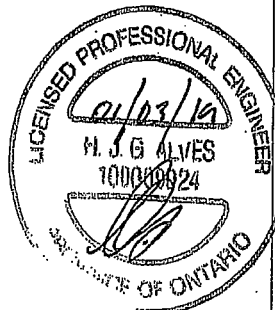
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	798	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.59 (F) (INPUT = 0.80)
JSI METAL= 0.25 (B) (INPUT = 1.00)

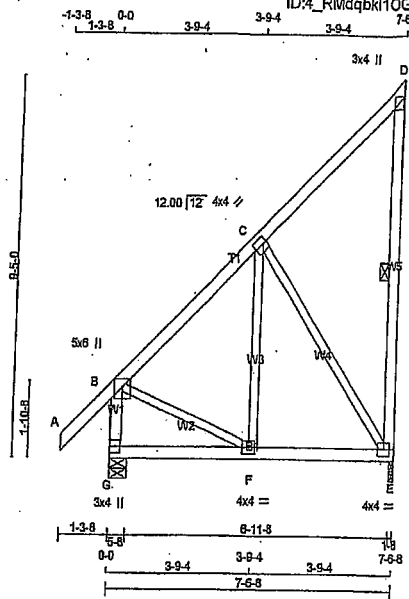


DRG NO. T1900156
SEC. 100019924
01/03/19

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	T140	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:4_RMdqbk1OG38XnfRj7uyvEbu-Xwrth_1joJ1gJhH2XsdAdEWu0J2v0Bs8D0pkizv8pc



Scale = 1:52.5

TOTAL WEIGHT = 2 X 47 = 94 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+1	MT20	4.0	4.0	2.00	1.00
D	TMVW+p	MT20	3.0	4.0		
E	BMVW+1	MT20	4.0	4.0		
F	BMVW+1	MT20	4.0	4.0		
G	BMV+1	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	465	0	465	0
G	584	0	584	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	353	176 / 0	79 / 0	0 / 0	0 / 0	88 / 0	0 / 0
G	435	241 / 0	79 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-84.9	-84.9 0.12 (1)	10.00	F-C	0 / 186	0.04 (3)
B-C	-263 / 0	-84.9	-84.9 0.16 (1)	8.25	C-E	-354 / 0	0.26 (1)
C-D	-28 / 0	-84.9	-84.9 0.16 (1)	6.25	B-F	0 / 221	0.05 (1)
E-D	-121 / 0	0.0	0.0 0.05 (1)	8.25			
G-B	-527 / 0	0.0	0.0 0.06 (1)	7.81			
G-F	0 / 0	-38.5	-38.5 0.12 (3)	10.00			
F-E	0 / 205	-38.5	-38.5 0.14 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 085-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAINLOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.25")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.25")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TO=0.16/1.00 (B-C:1), BC=0.14/1.00 (E-F:2), WB=0.26/1.00 (C-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

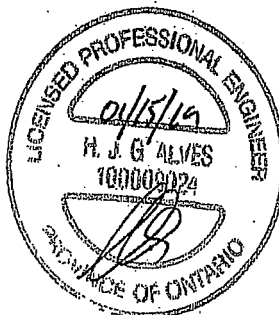
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

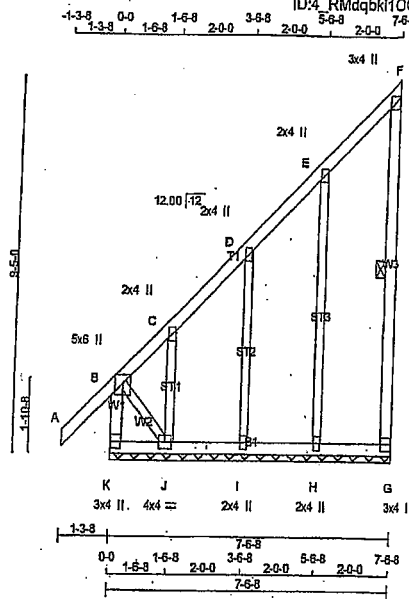


DRWG NO. TAM 11901091
STRUCTURAL
MANUFACTURING ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	G140	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:4_RMdqbkh1OG3fXnfR7uyvEbu-2kHHUe0510vpXirzDLDePMfP?XAcSjwZfGCGzv8pd



Scale = 1:53.2

TOTAL WEIGHT = 2 X 48 = 96 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - F	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
K - B	2x4	DRY No.2	SPF
K - G	2x4	DRY No.2	SPF

ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 24-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	5.0	6.0	Edge	
C, D, E					
C TMW+w	MT20	2.0	4.0		
F TMW+p	MT20	3.0	4.0		
G BMW1+p	MT20	3.0	4.0		
H BMW1+w	MT20	2.0	4.0		
I BMW1+w	MT20	2.0	4.0		
J BMW1+t	MT20	4.0	4.0		
K BMW1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/42	-84.9 -84.9	0.12 (1)	10.00	H-E	-189/0	0.18 (1)
B-C	-49/0	-84.9 -84.9	0.11 (1)	8.25	I-D	-175/0	0.08 (1)
C-D	-2/0	-84.9 -84.9	0.04 (1)	10.00	J-C	-82/0	0.02 (1)
D-E	-3/0	-84.9 -84.9	0.05 (1)	10.00	B-J	0/13	0.00 (1)
E-F	-11/0	-84.9 -84.9	0.05 (1)	6.25			
G-F	-73/0	0.0 0.0	0.03 (1)	6.25			
K-B	-249/0	0.0 0.0	0.03 (1)	7.81			
K-J	0/0	-38.5 -38.5	0.02 (3)	10.00			
J-I	0/6	-38.5 -38.5	0.02 (3)	10.00			
I-H	0/3	-38.5 -38.5	0.03 (3)	10.00			
H-G	0/0	-38.5 -38.5	0.03 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 8.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.03/1.00 (H-I:3), WB=0.18/1.00 (E-H:1), SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

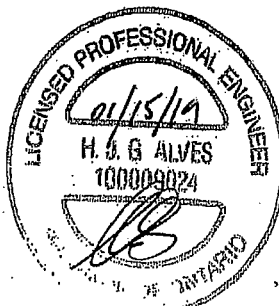
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 854	1687 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)



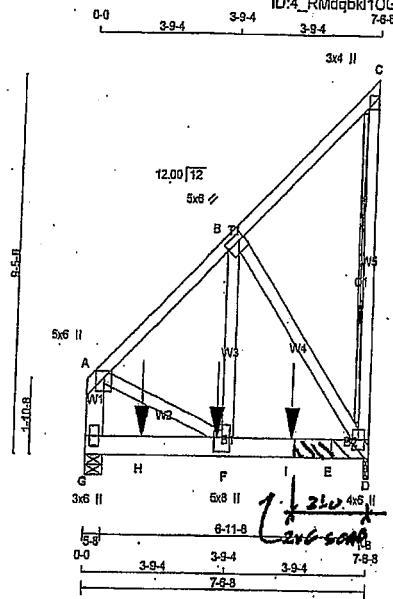
DRWG NO. TAM 11901092
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	T141	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID: 4_RMDqbk10G3fXnRj7uyvEbu-76P1vK2LZd9XxsE5dOsJqnhCVDeIQON18NH9zv8pb

Scale = 1/52'



TOTAL WEIGHT = 4 X 57 = 228 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
G - A	2x6 DRY	No.2	SPF		
G - D	2x6 DRY	No.2	SPF		
ALL WEBS	2x4 DRY	No.2	SPF		

EXCEPT

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
A-C	12	TOP
C-D	12	TOP
G-A	12	TOP
G-D	12	TOP
WEBS (0.122"x3") SPIRAL NAILS		SIDE (183.1)
B-F	6	SIDE (464.6)
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TMVW-t	MT20	5.0	6.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW-t+p	MT20	4.0	6.0		
F	BMVW+t	MT20	5.0	8.0	4.25	2.50
G	BMV+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
FACTORED			MAXIMUM FACTORED			INPUT	REQD	
GROSS REACTION			GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
D	3997	0	3997	0	0	1-8	1-8	1-8 & BLOCK
G	4382	0	4382	0	0	5-8	5-8	

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
D	3013	1558 / 0	632 / 0	0 / 0	0 / 0	823 / 0	0 / 0		
G	3302	1708 / 0	692 / 0	0 / 0	0 / 0	902 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G
2x6 DRY SPF No.2 BEARING BLOCK 24" LONG AT JT. D ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 24" NAILS TOTAL

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-D
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (L)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH FR-TO	MAX. UNBRAC LENGTH FR-TO	
H	0 / 0	-38.5	-38.5	0.64 (1)	10.00				
F	0 / 0	-38.5	-38.5	0.64 (1)	10.00				
I	0 / 2212	-38.5	-38.5	0.73 (1)	10.00				
E	0 / 2212	-38.5	-38.5	0.73 (1)	10.00				
D	0 / 2212	-38.5	-38.5	0.73 (1)	10.00				
A	-3181 / 0	0.0	0.0	0.12 (1)	7.75				
C	-125 / 0	0.0	0.0	0.04 (1)	7.81				
B	-24 / 0	-84.9	-84.9	0.10 (1)	6.25	B-D	-3772 / 0	0.97 (1)	
B	-3103 / 0	-84.8	-84.9	0.15 (1)	5.13	F-B	0 / 4879	0.43 (1)	
A-F						A-F	0 / 2367	0.21 (1)	

FACTORED CONCENTRATED LOADS (LBS)									
LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	
3-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—	
1-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—	
5-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.25")
CALCULATED VERT. DEFL. (LL) = $L/699$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.25")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.06")

CSI: TC=0.15/1.00 (A-B-1), BC=0.73/1.00 (D-F-1), WB=0.97/1.00 (B-D-1), SS=0.60/1.00 (D-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP (DRY)		SHEAR		SECTION	
(FSI)	(PLI)	(FSI)	(PLI)	(FSI)	(PLI)	(FSI)	(PLI)
MT20	618	354	1687	788	1987	1658	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (B) (INPUT = 0.90)
JSI METAL=0.53 (F) (INPUT = 1.00)

DWG NO. TAM 17901090
STRUCTURAL
COMPONENT ONLY

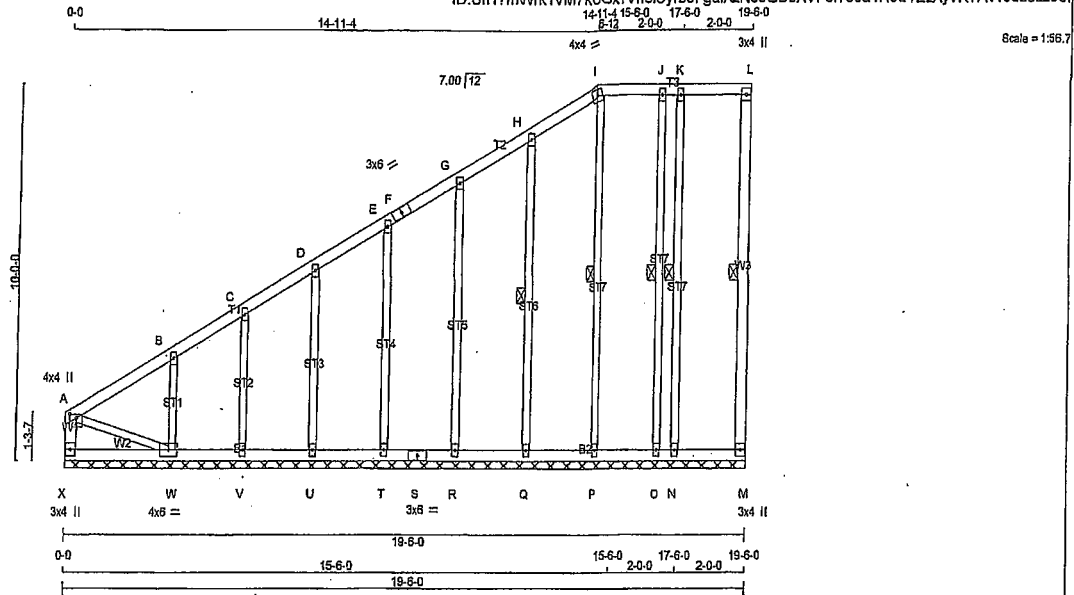


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	G470	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID: Sfh7mWkTvm7kCGx7VheICyrb3t-galXzNs5GD6bXVP8iY89a1R9dYzzXyYvKYAW3ds5z03f



TOTAL WEIGHT = 2 X 111 = 221 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCOR.
CHORDS			
A - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
M - L	2x4	DRY	No.2
X - A	2x4	DRY	No.2
X - S	2x4	DRY	No.2
S - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2'-0" OC.		

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0 1.25 2.00
B, C, D, E, G, H, J, K				
B	TMVW+p	MT20	2.0	4.0
F	TS-1	MT20	3.0	6.0
I	TTW-m	MT20	4.0	4.0
L	TMV+p	MT20	3.0	4.0
M	BMV1+p	MT20	3.0	4.0
N, O, P, Q, R, T, U, V				
N	BMV1+w	MT20	2.0	4.0
S	BS-1	MT20	3.0	6.0
W	BMVW1-t	MT20	4.0	6.0
X	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-M, K-N, I-P, J-Q, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (7)

MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-10/0	-84.9	-84.9 0.09 (1)	6.25	N-K	-121/0	0.08 (1)
B-C	-26/0	-84.9	-84.9 0.09 (1)	6.25	P-I	-138/0	0.09 (1)
C-D	-8/0	-84.9	-84.9 0.04 (1)	10.00	Q-J	-125/0	0.08 (1)
D-E	-8/0	-84.9	-84.9 0.04 (1)	10.00	Q-H	-187/0	0.10 (1)
E-F	-8/0	-84.9	-84.9 0.04 (1)	10.00	R-G	-166/0	0.18 (1)
F-G	-8/0	-84.9	-84.9 0.04 (1)	10.00	T-E	-168/0	0.12 (1)
G-H	-2/0	-84.9	-84.9 0.04 (1)	10.00	U-D	-174/0	0.08 (1)
H-I	-10/0	-84.9	-84.9 0.04 (1)	6.25	V-C	-139/0	0.04 (1)
I-J	0/0	-84.9	-84.9 0.04 (1)	10.00	W-B	-254/0	0.05 (1)
J-K	0/0	-84.9	-84.9 0.04 (1)	10.00	A-W	0/20	0.00 (1)
K-L	0/0	-84.9	-84.9 0.04 (1)	10.00			
M-L	-72/0	0.0	0.0 0.03 (1)	6.25			
X-A	-120/0	0.0	0.0 0.01 (1)	7.81			
X-W	0/0	-38.5	-38.5 0.06 (3)	10.00			
W-V	0/12	-38.5	-38.5 0.06 (3)	10.00			
V-U	0/9	-38.5	-38.5 0.03 (3)	10.00			
U-T	0/7	-38.5	-38.5 0.03 (3)	10.00			
T-S	0/5	-38.5	-38.5 0.02 (3)	10.00			
S-R	0/5	-38.5	-38.5 0.02 (3)	10.00			
R-Q	0/3	-38.5	-38.5 0.02 (3)	10.00			
Q-P	0/1	-38.5	-38.5 0.02 (3)	10.00			
P-O	0/0	-38.5	-38.5 0.02 (3)	10.00			
O-N	0/0	-38.5	-38.5 0.03 (3)	10.00			
N-M	0/0	-38.5	-38.5 0.03 (3)	10.00			

A circular seal for a Professional Engineer. The outer ring contains the text "PROFESSIONAL ENGINEER" at the top and "STATE OF TEXAS" at the bottom. The center of the seal features a five-pointed star.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, OBC 2012

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.09/1.00 (B-C:1), BC=0.06/1.00 (V-W:3), WB=0.18/1.00 (G-R:1), SS=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE - GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

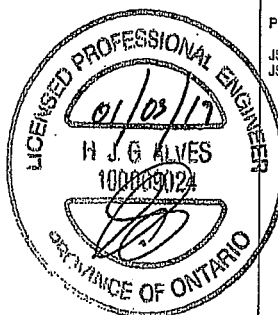
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (I) (INPUT = 0.90)

JSI METAL= 0.12 (B) (INPUT = 1.00)



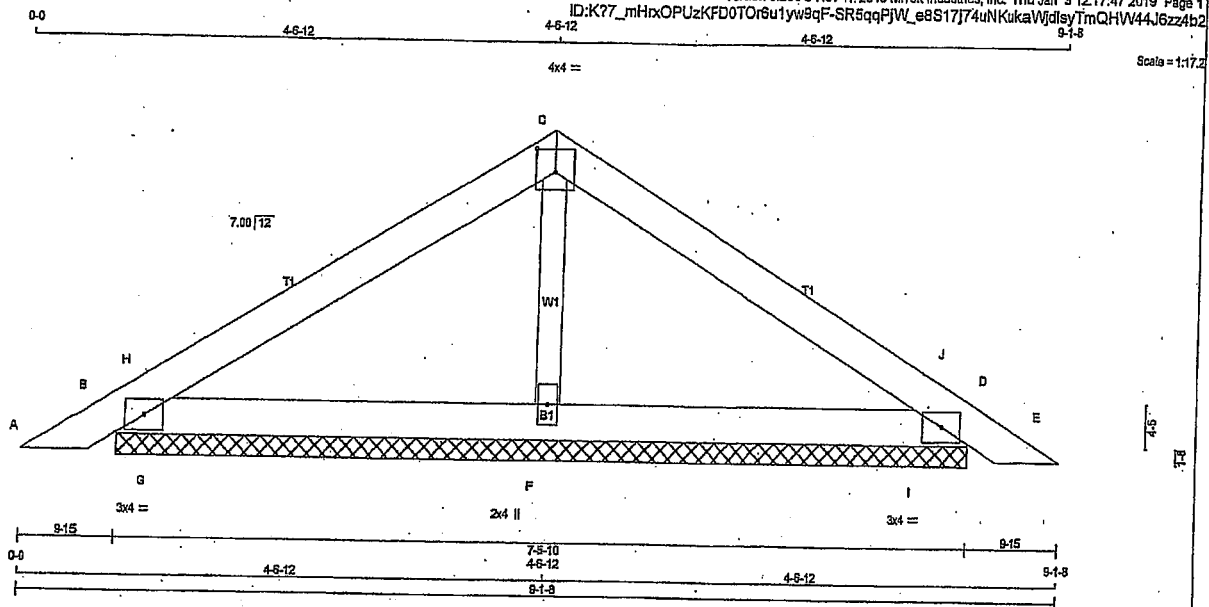
ENG. NO. TAM 119 00144
STRUCTURAL
CALCULATION ONLY

JOB NAME 300539	TRUSS NAME P1	QUANTITY 6	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:K77_mHxOPUzKFD0TOsu1yw9qF-SR5qqPJW_e8S17J74uNKukaVjdlsyTmQHV44J6zz4b2



Scale = 1:17.2

TOTAL WEIGHT = 6 X 22 = 133 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	HORZ	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG
B	322	0	322	0	0	0	0	7-5-10	7-5-10
D	322	0	322	0	0	0	0	7-5-10	7-5-10
F	404	0	404	0	0	0	0	7-5-10	7-5-10

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	
B	237 141/0 36/0 0/0 0/0 60/0 0/0
D	237 141/0 36/0 0/0 0/0 60/0 0/0
F	313 138/0 85/0 0/0 0/0 92/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0/15	-84.9	-84.9 0.04 (1)	10.00	F-C -158/0 0.02 (1)
B-H	-75/0	-84.9	-84.9 0.04 (1)	6.25	G-H -248/24 0.00 (1)
H-C	-118/0	-84.9	-84.9 0.15 (1)	6.25	I-J -248/24 0.00 (1)
C-J	-118/0	-84.9	-84.9 0.15 (1)	6.25	
J-D	-75/0	-84.9	-84.9 0.04 (1)	6.25	
D-E	0/15	-84.9	-84.9 0.04 (1)	10.00	
B-G	0/97	-38.5	-38.5 0.14 (1)	10.00	
G-F	0/97	-38.5	-38.5 0.16 (1)	10.00	
F-I	0/97	-38.5	-38.5 0.16 (1)	10.00	
I-D	0/97	-38.5	-38.5 0.14 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TO=0.15/1.00 (C-J:1), BO=0.16/1.00 (F-I:1), WB=0.02/1.00 (C-F:1), SSI=0.21/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

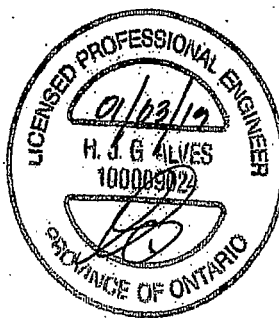
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)



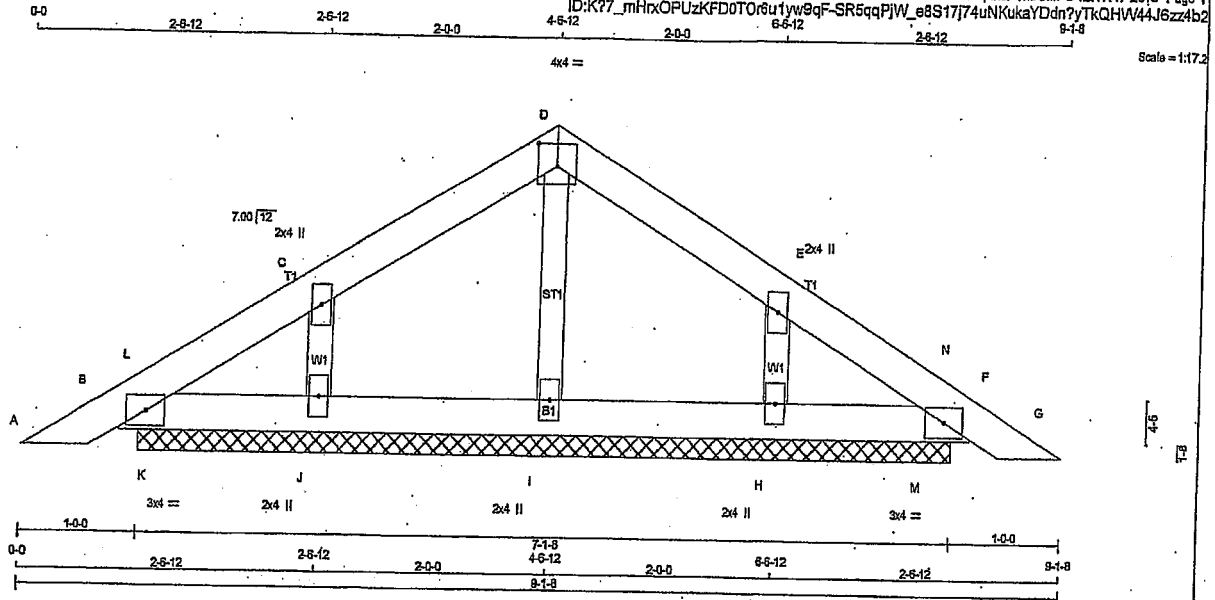
ENGR NO. TAM 17900063
STRUCTURAL
COMP. AGENT ONLY

JOB NAME 300539	TRUSS NAME P1G	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 6.230 S Nov 17 2018 MITek Industries, Inc. Thu Jan 3 12:17:47 2019 Page 1

ID:K77_mHxOPUzKFD0T0r6u1yv9qF-SR5qqPJW_e8S17j74uNKukaYDdn?yTkQHW44J6zz4b2



TOTAL WEIGHT = 2 X 23 = 47 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT	2-0-0	OC.			

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-H	MT20	3.0	4.0		
C	TMB1+H	MT20	2.0	4.0		
D	TTW-P	MT20	4.0	4.0	2.25	2.00
E	TMB1+H	MT20	2.0	4.0		
F	TMB1-H	MT20	3.0	4.0		
H, I, J	BMW1+H	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING				TOTAL LOAD CASES: (4)			
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0/15	-84.9 -84.9 0.04 (1)	10.00	I-D	-118/0	0.02 (1)	
B-L	-53/0	-84.9 -84.9 0.01 (2)	6.25	J-C	-188/0	0.03 (1)	
L-C	-21/0	-84.9 -84.9 0.05 (1)	6.25	H-E	-188/0	0.03 (1)	
C-D	-31/0	-84.9 -84.9 0.05 (1)	6.25	K-L	-11/17	0.00 (1)	
D-E	-31/0	-84.9 -84.9 0.05 (1)	6.25	M-N	-11/17	0.00 (1)	
E-N	-21/0	-84.9 -84.9 0.05 (1)	6.25				
N-F	-53/0	-84.9 -84.9 0.01 (2)	6.25				
F-G	0/15	-84.9 -84.9 0.04 (1)	10.00				
B-K	0/33	-38.5 -38.5 0.02 (1)	10.00				
K-J	0/33	-38.5 -38.5 0.03 (2)	10.00				
J-I	0/19	-38.5 -38.5 0.03 (2)	10.00				
I-H	0/19	-38.5 -38.5 0.03 (2)	10.00				
H-M	0/33	-38.5 -38.5 0.03 (2)	10.00				
M-F	0/33	-38.5 -38.5 0.02 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-E:1), BC=0.03/1.00 (J-K:2), WS=0.03/1.00 (E-H:1), SSI=0.07/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

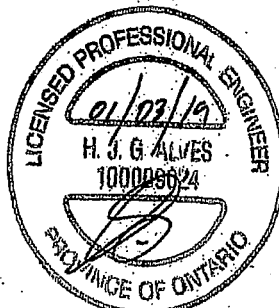
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1666

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.12 (C) (INPUT = 0.80)

JSI METAL = 0.09 (C) (INPUT = 1.00)



DRWG NO. TAM 11900064

STRUCTURAL

DESIGNED BY ONLY

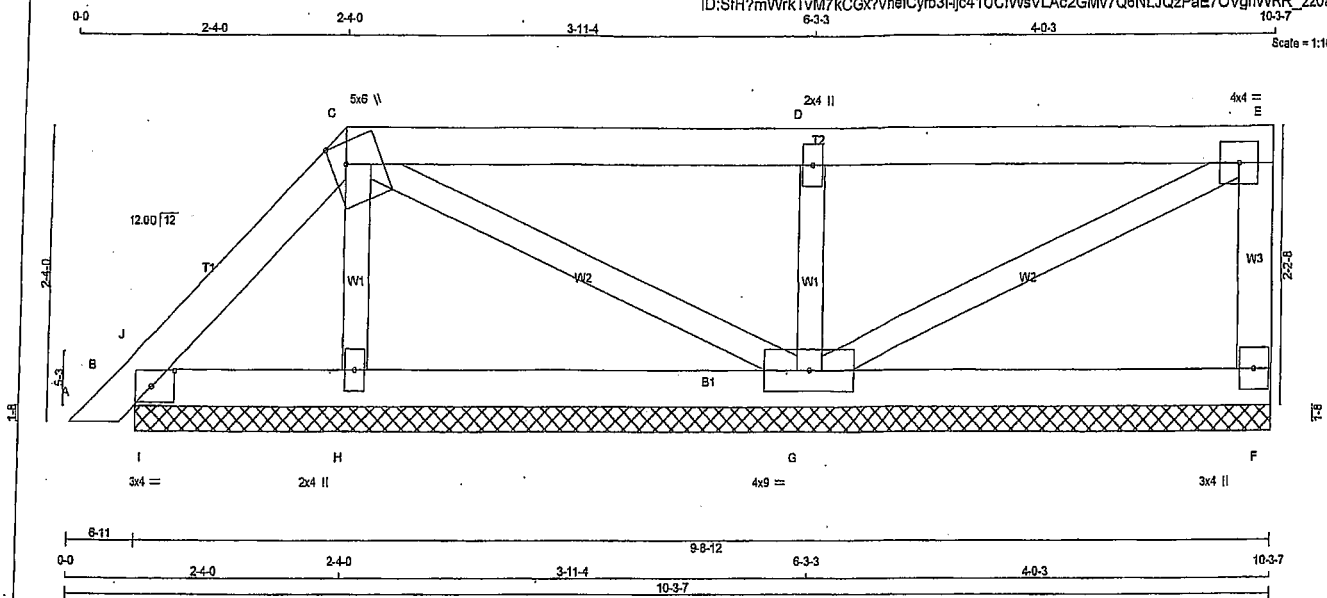
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300799	P10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 8 Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 16:51:08 2019 Page 1

ID:SFH7mWrkTVM7kCGx7VheICyrb3Hjc41UCIWsvLAc2GMv7Q6NLJQZPaE7OVghWRR_zz0an

Scale = 1:16.9



TOTAL WEIGHT = 2 X 35 = 70 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	189	0	189	0	9-8-12	9-8-12
B	173	0	173	0	9-8-12	9-8-12
H	263	0	263	0	9-8-12	9-8-12
G	613	0	613	0	9-8-12	9-8-12

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	144	70/0	33/0	0/0	0/0	40/0	0/0
B	124	84/0	11/0	0/0	0/0	29/0	0/0
H	207	81/0	63/0	0/0	0/0	63/0	0/0
G	462	239/0	97/0	0/0	0/0	128/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED LC1 MAX (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LBS)	MEMB.
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/12	-84.9	-84.9 0.01 (1)	10.00	H-C	-132/0	0.02 (1)
B-J	-30/0	-84.9	-84.9 0.01 (1)	6.25	G-E	-2/0	0.00 (1)
J-C	-58/0	-84.9	-84.9 0.03 (1)	6.25	C-G	-38/0	0.01 (1)
C-D	0/2	-84.9	-84.9 0.23 (1)	10.00	G-D	-421/0	0.06 (1)
D-E	0/2	-84.9	-84.9 0.23 (1)	10.00	I-J	-110/0	0.00 (1)
F-E	-128/0	0.0	0.0 0.01 (1)	7.81			
B-I	0/38	-38.5	-38.5 0.04 (1)	10.00			
I-H	0/38	-38.5	-38.5 0.06 (2)	10.00			
H-G	0/31	-38.5	-38.5 0.12 (3)	10.00			
G-F	0/0	-38.5	-38.5 0.12 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-D:1), BC=0.12/1.00 (G-H:3), WB=0.08/1.00 (D-G:1), SS=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

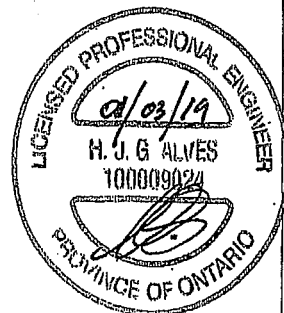
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

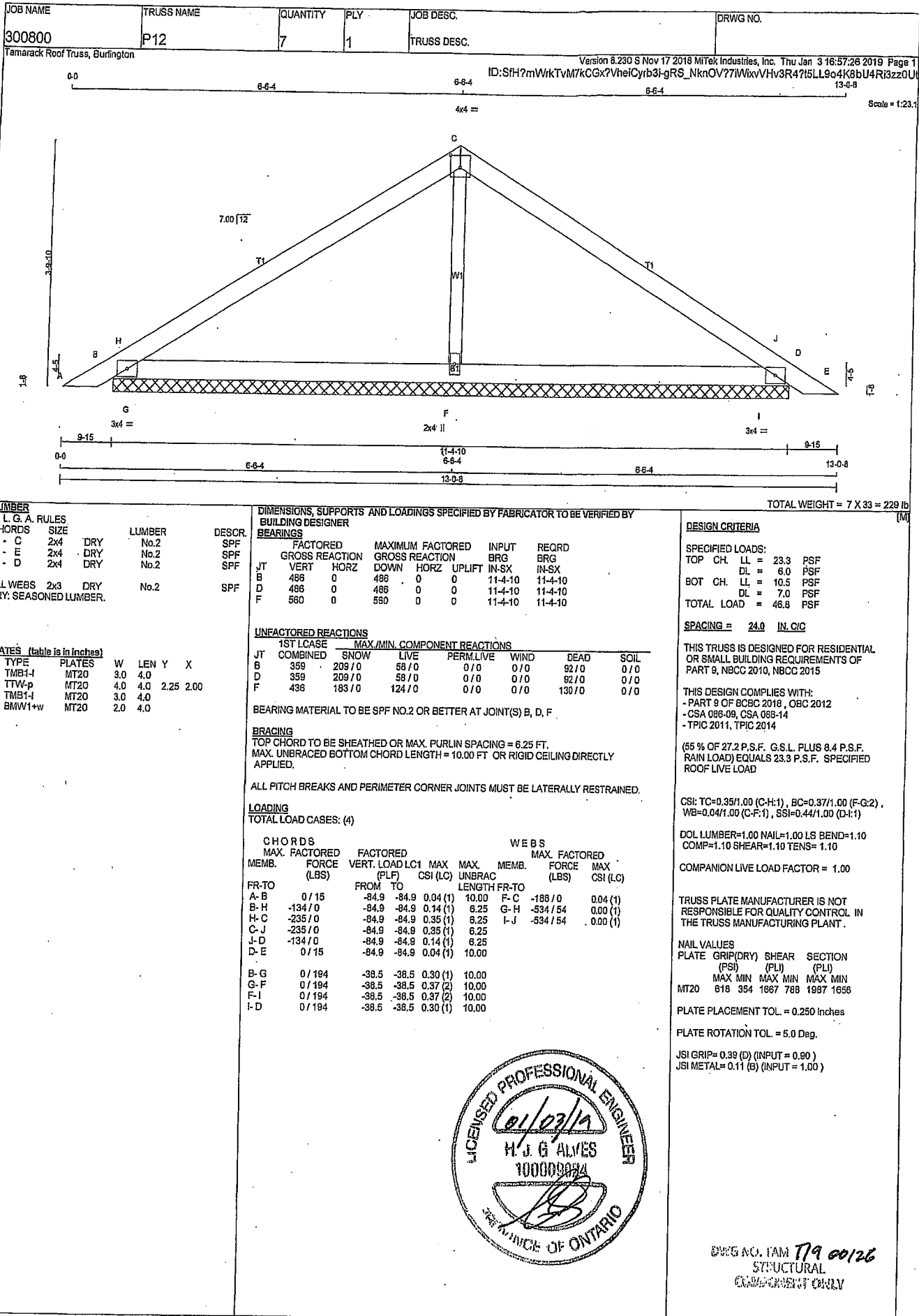
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)

JSI METAL= 0.09 (D) (INPUT = 1.00)



DRWG NO. TAM 17900116
STRUCTURAL
COMPONENT ONLY

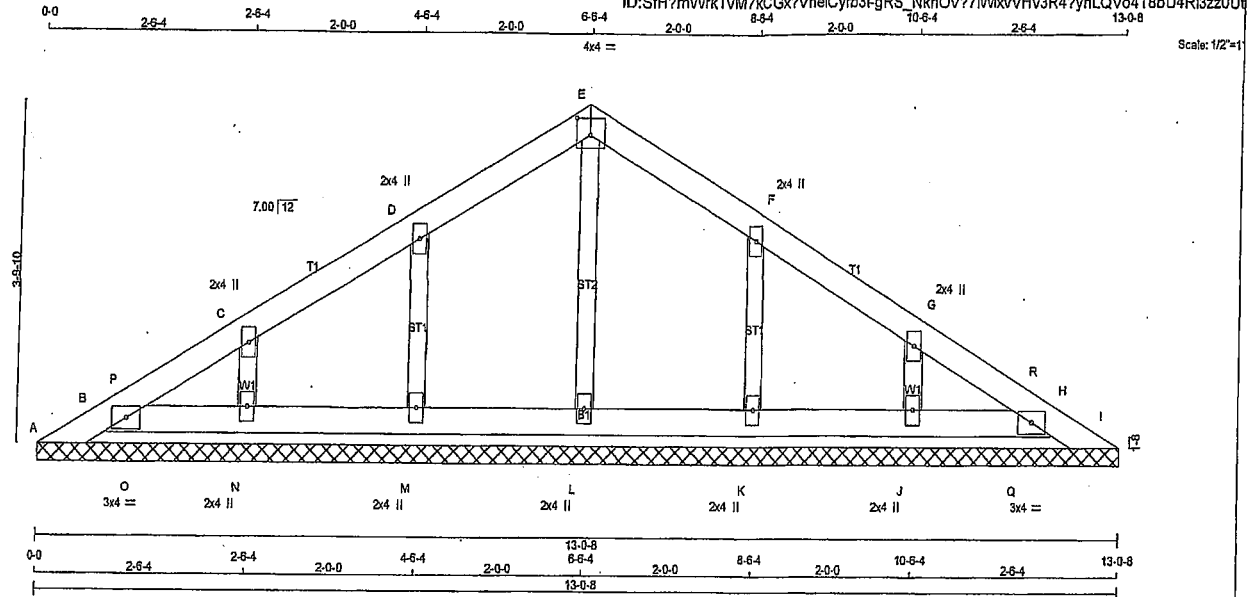


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300800	P12G	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 16:57:26 2019 Page 1

ID:SFH?mVrkTvM7kCGx?VheICyrb3I-gRS_NknOV?7IMxvVhv3R4?ynLQVo4T8bU4Ri3zz0U



TOTAL WEIGHT = 37 lb (M)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C, D, F, G				
C	TMW+H	MT20	2.0	4.0
E	TTW+P	MT20	4.0	4.0 2.25 2.00
H	TMB1-I	MT20	3.0	4.0
J, K, L, M, N				
J	BMW1+H	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): L, M, K, J, N

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH
FR-TO		FROM TO			FR-TO		FROM TO		
A-B	0/8	-102.4 -102.4	0.02 (1)	10.00	L-E	-121/0	0.03 (1)		
B-P	-67/0	-84.9 -84.9	0.02 (1)	6.25	M-D	-195/0	0.03 (1)		
P-C	-29/0	-84.9 -84.9	0.03 (1)	6.25	K-F	-195/0	0.03 (1)		
C-D	-22/0	-84.9 -84.9	0.05 (1)	6.25	J-G	-145/0	0.02 (1)		
D-E	-28/0	-84.9 -84.9	0.05 (1)	6.25	N-C	-145/0	0.02 (1)		
E-F	-28/0	-84.9 -84.9	0.05 (1)	6.25	O-P	0/25	0.03 (1)		
F-G	-22/0	-84.9 -84.9	0.05 (1)	6.25	Q-R	0/25	0.00 (1)		
G-R	-29/0	-84.9 -84.9	0.03 (1)	6.25					
R-H	-67/0	-84.9 -84.9	0.02 (1)	6.25					
H-I	0/8	-102.4 -102.4	0.02 (1)	10.00					
B-O	0/33	-38.5 -38.5	0.01 (1)	10.00					
O-N	0/33	-38.5 -38.5	0.03 (2)	10.00					
N-M	0/23	-38.5 -38.5	0.03 (3)	10.00					
M-L	0/16	-38.5 -38.5	0.03 (3)	10.00					
L-K	0/16	-38.5 -38.5	0.03 (3)	10.00					
K-J	0/23	-38.5 -38.5	0.03 (3)	10.00					
J-Q	0/33	-38.5 -38.5	0.03 (2)	10.00					
Q-H	0/33	-38.5 -38.5	0.01 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-E-1), BC=0.03/1.00 (J-K-3), WB=0.03/1.00 (F-K-1), SSI=0.07/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (FL) (FL)

MAX MIN MAX MIN MAX MIN

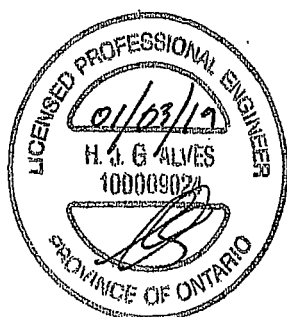
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.13 (D) (INPUT = 0.90)

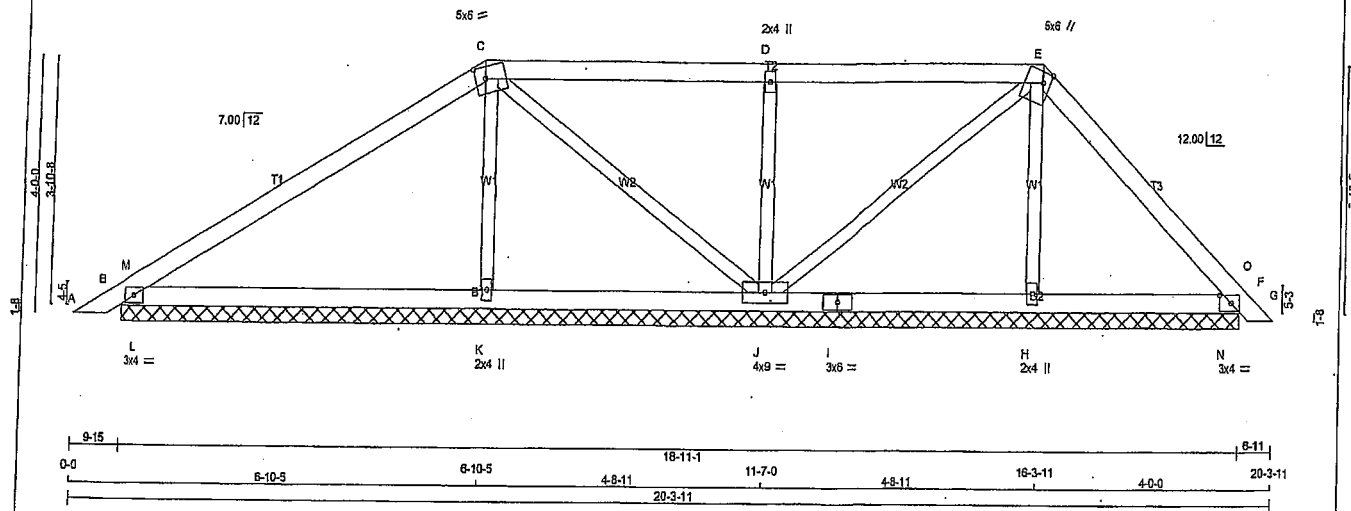
JSI METAL= 0.09 (D) (INPUT = 1.00)



DRWG NO. TAM 119 00127

STRUCTURAL COMPONENT ONLY

JOB NAME 300803	TRUSS NAME P14	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 17:06:31 2019 Page 1 ID: S/H?mWkTvm7kCGx?VhelCyrb3H-N7Nx7qXuJvLouHjzSK0uJ2VLQJM8qIOWSg8Y0z20KU	
Scale = 1:33.4					



LUMBER			LUMBER			DESCR.		
N. L. G. A. RULES	CHORDS	SIZE				SPF		
A - C	2x4	DRY	No.2			SPF		
C - E	2x4	DRY	No.2			SPF		
E - G	2x4	DRY	No.2			SPF		
B - I	2x4	DRY	No.2			SPF		
I - F	2x4	DRY	No.2			SPF		
ALL WEBS	2x3	DRY	No.2			SPF		
DRY, SEASONED LUMBER.								

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW-m	MT20	5.0	6.0	2.25 2.00
D	TMW-w	MT20	2.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.00 1.50
F	TMB1-I	MT20	3.0	4.0	1.50 2.50
H	BMW1+w	MT20	2.0	4.0	
I	BS-t	MT20	3.0	6.0	
J	BMWWW1-t	MT20	4.0	9.0	
K	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	IN-SX	BRG	IN-SX
B	501	0	0	501	0	0	18-11-1 (6-11-1)	11-1	11-1
K	512	0	0	512	0	0	18-11-1 (6-11-1)	11-1	11-1
J	735	0	0	735	0	0	18-11-1 (6-11-1)	11-1	11-1
H	359	0	0	359	0	0	18-11-1 (6-11-1)	11-1	11-1
F	330	0	0	330	0	0	18-11-1 (6-11-1)	11-1	11-1

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LOSE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	371	212/0	63/0	0/0	0/0	99/0	0/0
K	400	193/0	117/0	0/0	0/0	120/0	0/0
J	544	311/0	91/0	0/0	0/0	141/0	0/0
H	285	104/0	92/0	0/0	0/0	88/0	0/0
F	241	147/0	34/0	0/0	0/0	60/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (CSI) (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (CSI) (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/15	-84.9	-84.9 0.04 (1)	10.00	K-C	-209/0	0.05 (1)
B-M	-151/0	-84.9	-84.9 0.13 (1)	6.25	C-J	-122/0	0.07 (1)
M-C	-191/0	-84.9	-84.9 0.41 (1)	6.25	J-D	-500/0	0.12 (1)
C-D	-54/0	-84.9	-84.9 0.32 (1)	6.25	J-E	-50/0	0.03 (1)
D-E	-54/0	-84.9	-84.9 0.32 (1)	6.25	H-E	-145/0	0.03 (1)
E-O	-150/0	-84.9	-84.9 0.12 (1)	6.25	L-M	-519/110	0.00 (1)
O-F	-71/5	-84.9	-84.9 0.06 (1)	6.25	N-O	-336/11	0.00 (1)
F-G	0/12	-84.9	-84.9 0.01 (1)	10.00			
B-L	0/158	-38.5	-38.5 0.30 (1)	10.00			
L-K	0/158	-38.5	-38.5 0.33 (1)	10.00			
K-J	0/152	-38.5	-38.5 0.30 (2)	10.00			
J-I	0/95	-38.5	-38.5 0.15 (2)	10.00			
I-H	0/95	-38.5	-38.5 0.15 (2)	10.00			
H-N	0/99	-38.5	-38.5 0.15 (2)	10.00			
N-F	0/99	-38.5	-38.5 0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.41/1.00 (C-M:1), BC=0.33/1.00 (K-L:1), WB=0.12/1.00 (D-J:1), SSI=0.45/1.00 (B-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

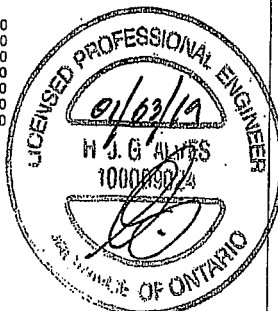
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1637 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.80)
JSI METAL= 0.11 (I) (INPUT = 1.00)

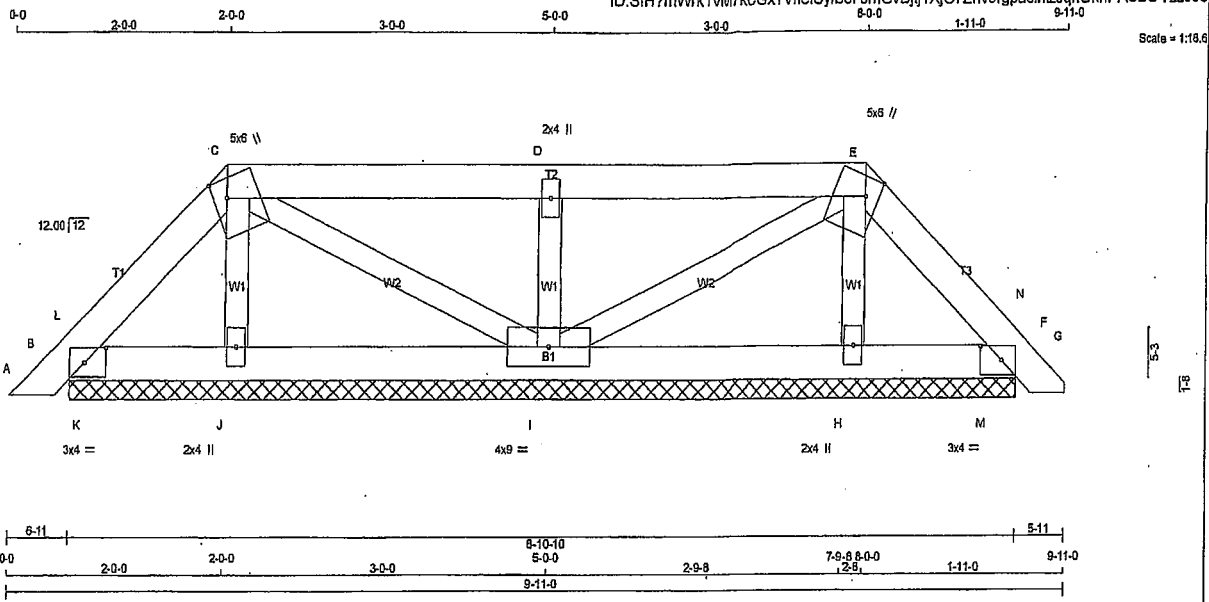


DRWG NO. TAM T1900140
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	P15A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 17:28:29 2019 Page 1
ID:SIH?mWrkTVM7kCGx?VheICyrb3I-8mGvBjI1Xj07Zhv6rgpaelnizJqhOkhPAoBOYzz03e



TOTAL WEIGHT = 31 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMW1+w	MT20	2.0	4.0		
I	BMW1+w	MT20	4.0	9.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS									
FACTORED		MAXIMUM FACTORED		INPUT		REQRD			
GROSS REACTION	GROSS REACTION	BRG	BRG	IN-SX	IN-SX				
JT	VERT	HORZ	DOWN	HORZ	UPLIFT				
B	151	0	151	0	0	8-10-10	8-10-10		
F	147	0	147	0	0	8-10-10	8-10-10		
J	209	0	209	0	0	8-10-10	8-10-10		
H	209	0	209	0	0	8-10-10	8-10-10		
I	453	0	453	0	0	8-10-10	8-10-10		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	108	73/0	9/0	0/0	0/0	25/0	0/0
F	105	71/0	9/0	0/0	0/0	25/0	0/0
J	165	63/0	52/0	0/0	0/0	51/0	0/0
H	165	63/0	52/0	0/0	0/0	51/0	0/0
I	339	184/0	65/0	0/0	0/0	90/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/12	-84.9 -84.9 0.01 (1)	10.00	J-C	-100/0	0.01 (1)	
B-L	-38/0	-84.9 -84.9 0.01 (1)	6.25	H-E	-100/0	0.01 (1)	
L-C	-60/0	-84.9 -84.9 0.02 (1)	6.25	I-E	-23/0	0.00 (1)	
C-D	-6/0	-84.9 -84.9 0.13 (1)	10.00	C-I	-23/0	0.00 (1)	
D-E	-6/0	-84.9 -84.9 0.13 (1)	10.00	I-D	-37/0	0.05 (1)	
E-N	-50/0	-84.9 -84.9 0.02 (1)	6.25	K-L	-78/0	0.00 (1)	
N-F	-38/0	-84.9 -84.9 0.01 (1)	6.25	M-N	-78/0	0.00 (1)	
F-G	0/11	-84.9 -84.9 0.01 (1)	10.00				
B-K	0/33	-38.5 -38.5 0.03 (1)	10.00				
K-J	0/33	-38.5 -38.5 0.04 (2)	10.00				
J-I	0/26	-38.5 -38.5 0.06 (3)	10.00				
I-H	0/26	-38.5 -38.5 0.06 (3)	10.00				
H-M	0/33	-38.5 -38.5 0.04 (2)	10.00				
M-F	0/33	-38.5 -38.5 0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-D:1), BC=0.08/1.00 (I-J:3), WB=0.05/1.00 (D-I:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMPE=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

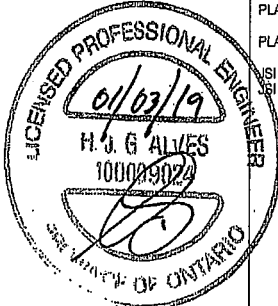
NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)	(PL)
MT20	618	354	1667	788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.17 (D) (INPUT = 0.90)

ISI METAL= 0.07 (D) (INPUT = 1.00)

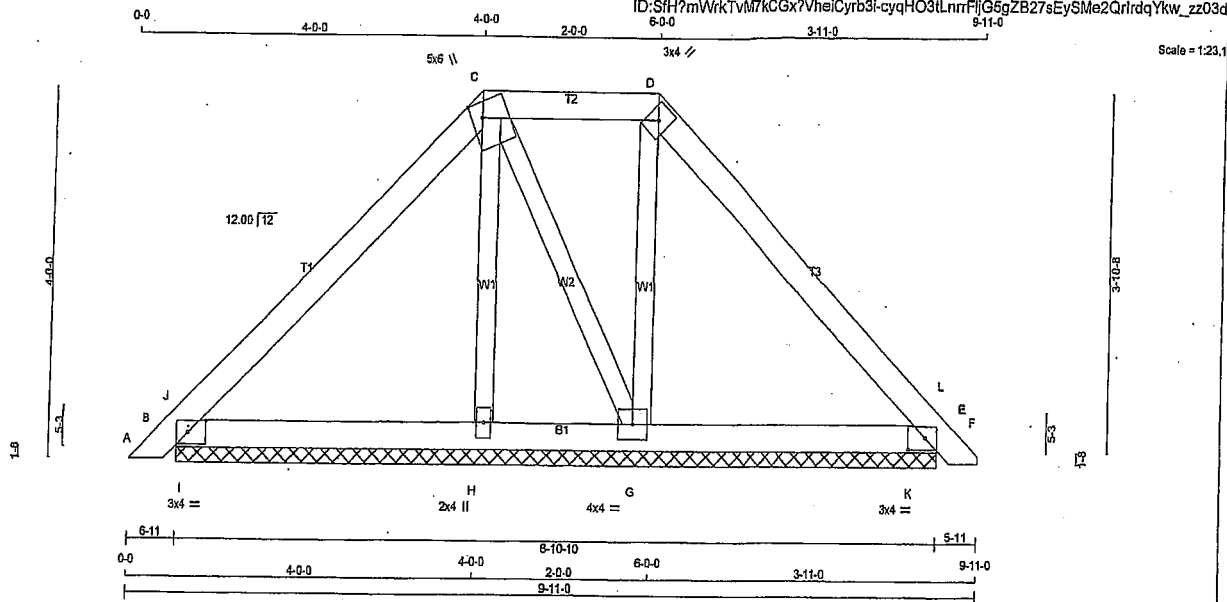


ENG. NO. TAM 71900 145
STRUCTURAL
COST ONLY ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	P16A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MitTek Industries, Inc. Thu Jan 3 17:26:30 2019 Page 1
ID:StH?mWrkTvm7kCGx7VheiCyrb3l-cyqHO3tLnrrFjG5gZB27sEySMe2QlrldqYkw_zz03d



TOTAL WEIGHT = 34 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

C - D 2x4 DRY

D - F 2x4 DRY

B - E 2x4 DRY

ALL WEBS 2x3 DRY

DRY: SEASONED LUMBER.

LUMBER

No.2

No.2

No.2

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	354	0	354	0	8-10-10	8-10-10
E	354	0	354	0	8-10-10	8-10-10
H	202	0	202	0	8-10-10	8-10-10
G	279	0	279	0	8-10-10	8-10-10

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
B	261	163/0	41/0	0/0	0/0	67/0	0/0
E	248	142/0	42/0	0/0	0/0	64/0	0/0
H	161	57/0	54/0	0/0	0/0	51/0	0/0
G	212	103/0	50/0	0/0	0/0	60/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0/12	-84.9 -84.9 0.01 (1)	10.00	H-C -67/0 0.02 (1)
B-J	-118/0	-84.9 -84.9 0.05 (1)	6.25	C-G -34/0 0.01 (1)
J-C	-168/0	-84.9 -84.9 0.13 (1)	6.25	G-D -113/0 0.03 (1)
C-D	-94/0	-84.9 -84.9 0.06 (1)	6.25	I-J -294/59 0.00 (1)
D-L	-147/0	-84.9 -84.9 0.13 (1)	6.25	K-L -298/59 0.00 (1)
L-E	-116/0	-84.9 -84.9 0.05 (1)	6.25	
E-F	0/11	-84.9 -84.9 0.01 (1)	10.00	
B-I	0/112	-38.5 -38.5 0.11 (1)	10.00	
I-H	0/112	-38.5 -38.5 0.12 (1)	10.00	
H-G	0/110	-38.5 -38.5 0.09 (2)	10.00	
G-K	0/98	-38.5 -38.5 0.12 (1)	10.00	
K-E	0/98	-38.5 -38.5 0.11 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012

- CSA 088-09, CSA 088-14

- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-J:1), EC=0.12/1.00 (H-I:1), WB=0.03/1.00 (D-G:1), SSI=0.28/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

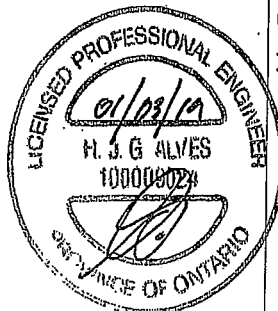
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT= 0.90)

JSI METAL= 0.08 (B) (INPUT= 1.00)

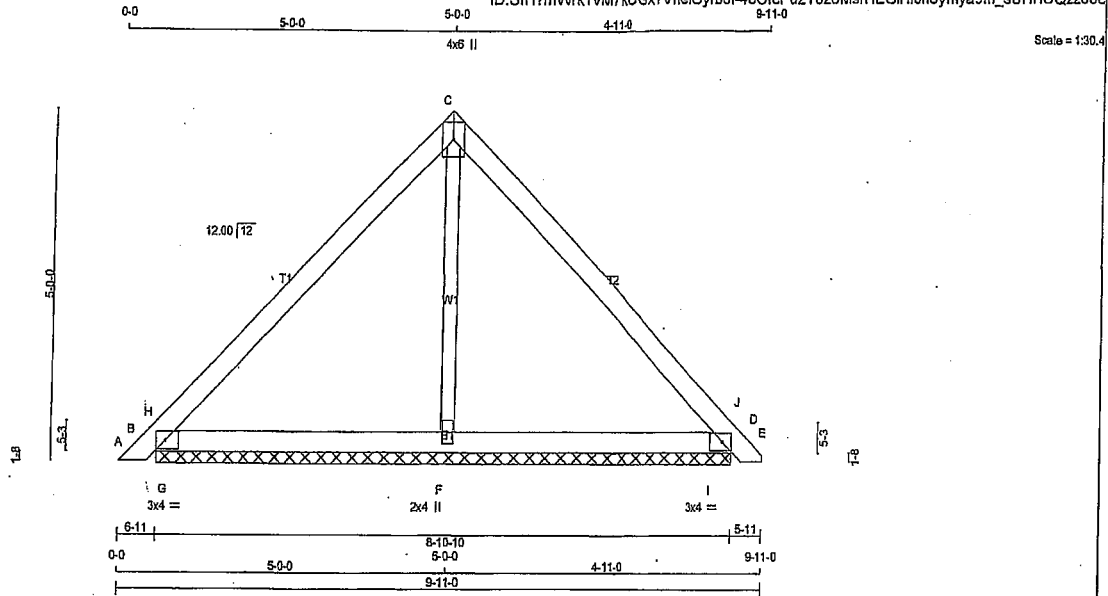


SAUGMONT 11900196
STRUCTURAL
COLUMBIA

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	P17A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:SFH7mWVrkTVm7kC6x7VheICyrb3i48OfcPuzY8z6MsrHEGfHf3n6ymya9In_sUHHSQzz03c



TOTAL WEIGHT = 4 X 30 = 118 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
B	423	0	423	0
D	419	0	419	0
F	328	0	331	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
B	311	185/0	47/0	0/0	0/0	78/0
D	306	183/0	47/0	0/0	0/0	78/0
F	264	86/0	93/0	0/0	0/0	84/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	FACTORED	
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/12	-84.9	-84.9 0.01 (1)	10.00	F-C	-36/23	0.01 (1)
B-H	-113/97	-84.9	-84.9 0.13 (1)	6.25	G-H	-572/31	0.00 (1)
H-C	-215/0	-84.9	-84.9 0.21 (1)	6.25	I-J	-572/31	0.00 (1)
C-J	-215/0	-84.9	-84.9 0.21 (1)	6.25			
J-D	-113/97	-84.9	-84.9 0.13 (1)	6.25			
D-E	0/11	-84.9	-84.9 0.01 (1)	10.00			
B-G	0/142	-38.5	-38.5 0.19 (1)	10.00			
G-F	0/142	-38.5	-38.5 0.23 (1)	10.00			
F-I	0/142	-38.5	-38.5 0.23 (1)	10.00			
I-D	0/142	-38.5	-38.5 0.19 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-H:1), BC=0.23/1.00 (F-I:1), WB=0.01/1.00 (C-F:1), SSI=0.47/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

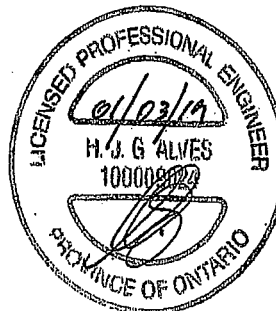
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1657
	788	1987	1656

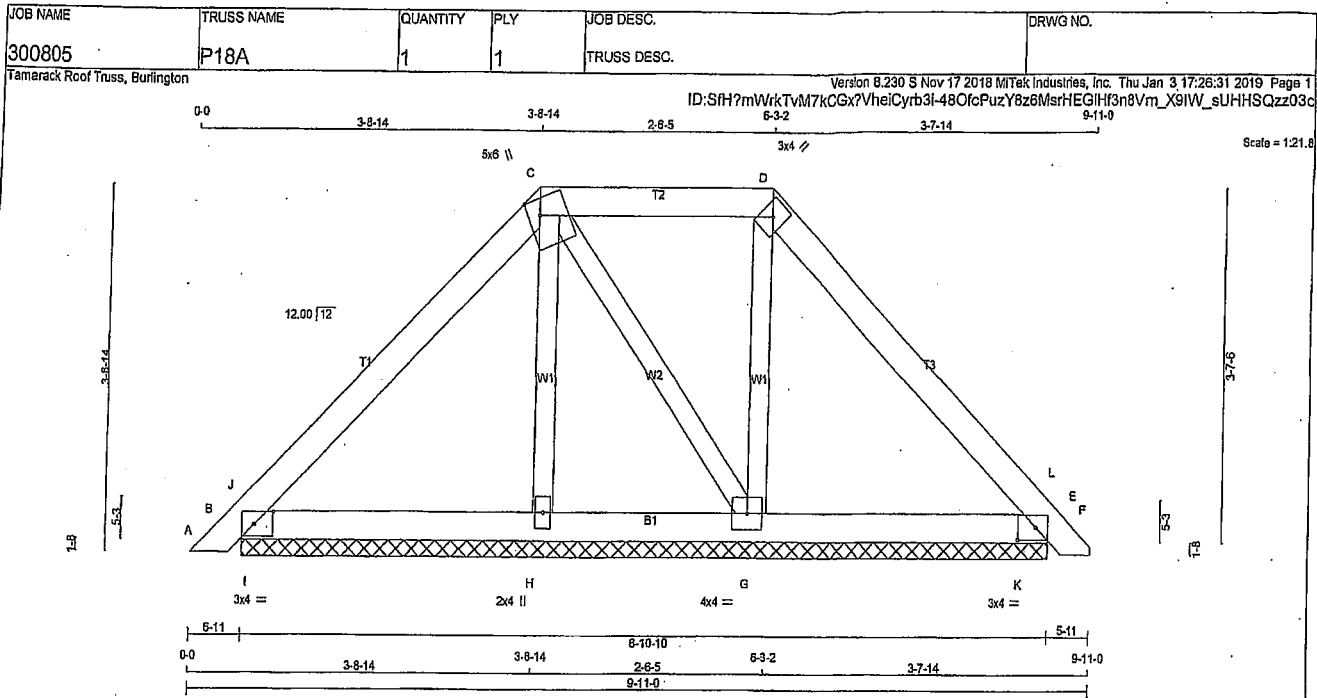
PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



DESIGNED BY TAM 11900147
STRUCTURAL
COMPUTATIONS ONLY



TOTAL WEIGHT = 33 lb (M)

LUMBER

N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - C 2x4 DRY No.2 SPF
C - D 2x4 DRY No.2 SPF
D - F 2x4 DRY No.2 SPF
B - E 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-J	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTWW+h	MT20	3.0	4.0		
E	TMB1-J	MT20	3.0	4.0	1.50	2.50
G	BMWW1-t	MT20	4.0	4.0		
H	BMWW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	333	0	333	0	8-10-10	8-10-10
E	310	0	310	0	8-10-10	8-10-10
H	227	0	227	0	8-10-10	8-10-10
G	300	0	300	0	8-10-10	8-10-10

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	244	146 / 0	37 / 0	0 / 0	0 / 0	62 / 0	0 / 0
E	229	133 / 0	37 / 0	0 / 0	0 / 0	59 / 0	0 / 0
H	180	66 / 0	58 / 0	0 / 0	0 / 0	56 / 0	0 / 0
G	229	108 / 0	55 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 12	-84.9	-84.9	0.01 (1)	10.00	H-C	-89 / 0	0.02 (1)
B-J	-102 / 0	-84.9	-84.9	0.04 (1)	6.25	C-G	-34 / 0	0.01 (1)
J-C	-158 / 0	-84.9	-84.9	0.11 (1)	6.25	G-D	-135 / 0	0.03 (1)
C-D	-83 / 0	-84.9	-84.9	0.09 (1)	6.25	I-J	-249 / 50	0.00 (1)
D-L	-131 / 0	-84.9	-84.9	0.11 (1)	6.25	K-L	-254 / 49	0.00 (1)
L-E	-99 / 0	-84.9	-84.9	0.04 (1)	6.25			
E-F	0 / 11	-84.9	-84.9	0.01 (1)	10.00			
B-I	0 / 106	-38.5	-38.5	0.09 (1)	10.00			
I-H	0 / 106	-38.5	-38.5	0.11 (1)	10.00			
H-G	0 / 103	-38.5	-38.5	0.08 (2)	10.00			
G-K	0 / 88	-38.5	-38.5	0.10 (1)	10.00			
K-E	0 / 88	-38.5	-38.5	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (D-L1), BC=0.11/1.00 (H-K1), WB=0.03/1.00 (D-G1), SSI=0.22/1.00 (E-K1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

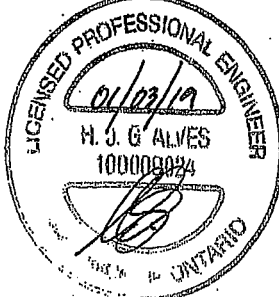
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (D) (INPUT = 0.90)

JSI METAL= 0.07 (B) (INPUT = 1.00)



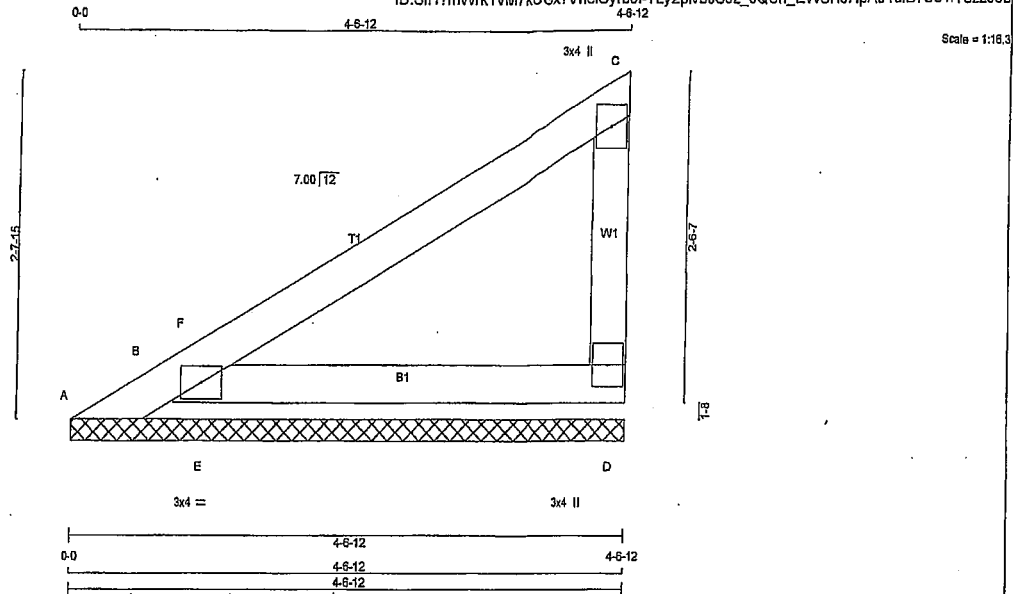
ENGINEER T1900148
STRUCTURAL
CONCRETE

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300805	P19G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 17:26:32 2019 Page 1

ID: Sfh?mWrkTvM7kCGx?VheiCyrb3i-YLy2plvbJS5z_0QU_n_EVCHJHPAJ4ulB7581r?sz203b



TOTAL WEIGHT = 2 X 12 = 25 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMV+p	MT20	3.0	4.0	
D	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS. FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0/85	-102.4 -102.4	0.20 (1)	E-F	0/204	0.00 (1)	
B-F	-152/0	-84.9 -84.9	0.19 (1)				
F-C	-8/1	-84.9 -84.9	0.13 (1)				
D-C	-127/0	0.0 0.0	0.02 (1)				
B-E	0/0	-38.5 -38.5	0.05 (3)				
E-D	0/0	-38.5 -38.5	0.09 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL =	23.3 PSF
DL =	6.0 PSF
BOT CH. LL =	10.5 PSF
DL =	7.0 PSF
TOTAL LOAD =	46.8 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (A-B:1), BC=0.09/1.00 (D-E:2), WB=0.00/1.00 (E-F:1), SS=0.17/1.00 (B-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

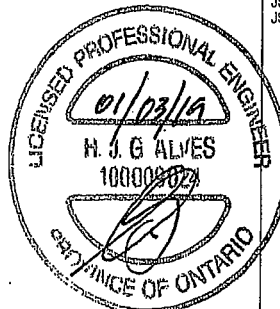
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
		768	1987
			1656

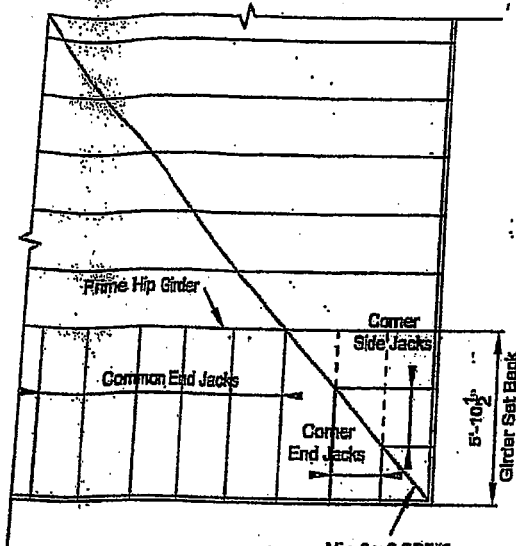
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.04 (C) (INPUT = 1.00)



DESIGNER: TAM 11100149
STRUCTURAL
QUALITY CONTROL



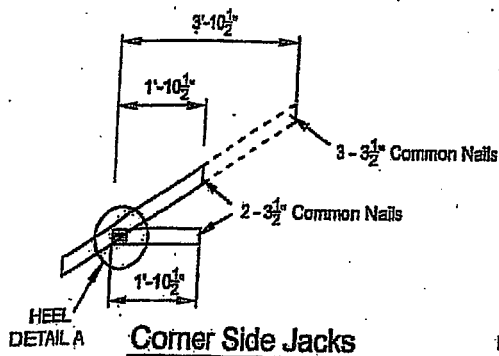
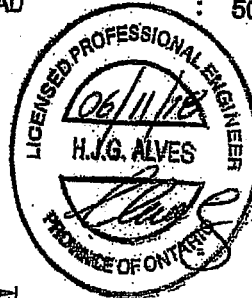
45° Hip End

LUMBER SPECIFICATION

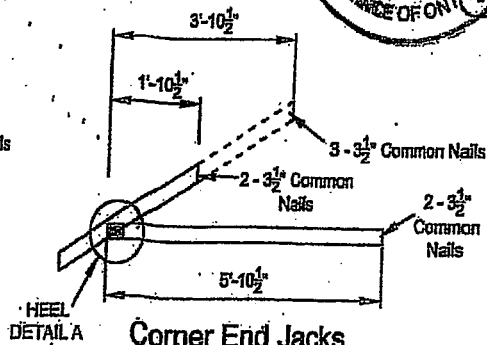
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

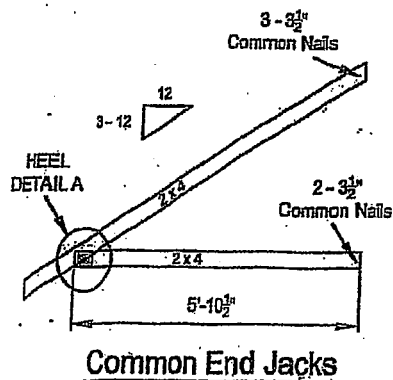
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 50.5 P.S.F.



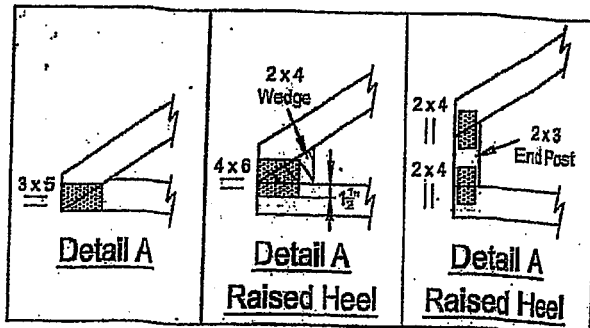
Corner Side Jacks

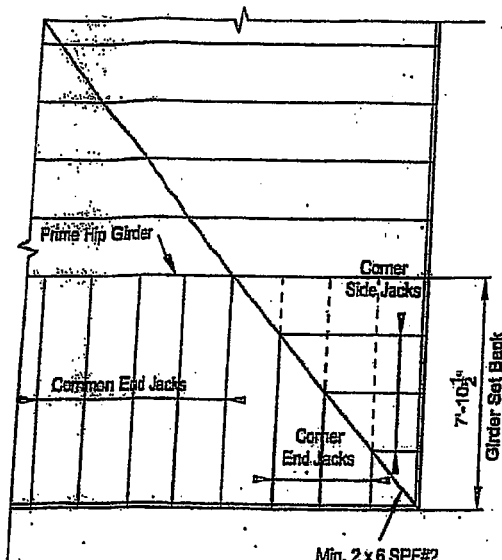


Corner End Jacks



Common End Jacks





45° Hip End

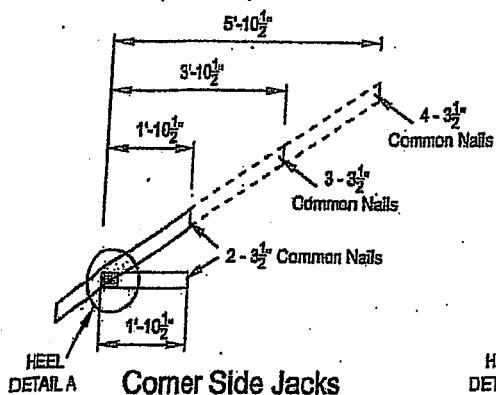
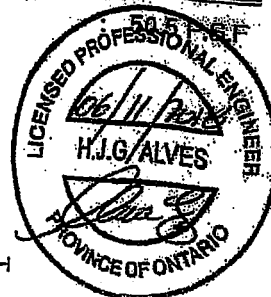
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

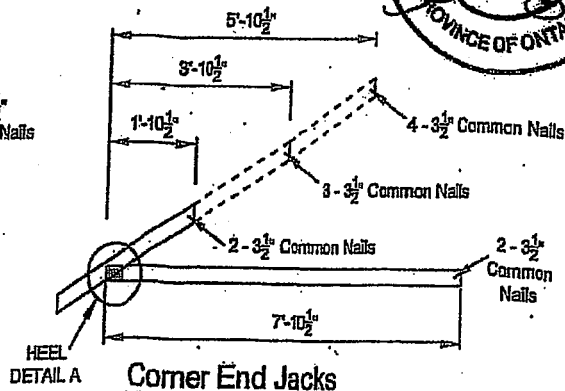
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

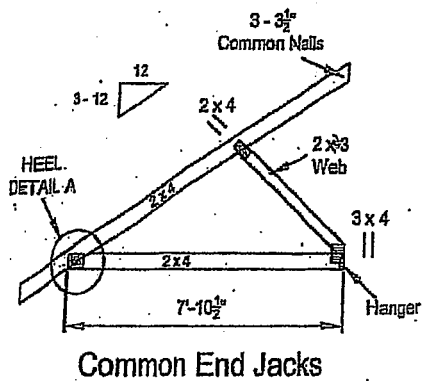
TOTAL LOAD



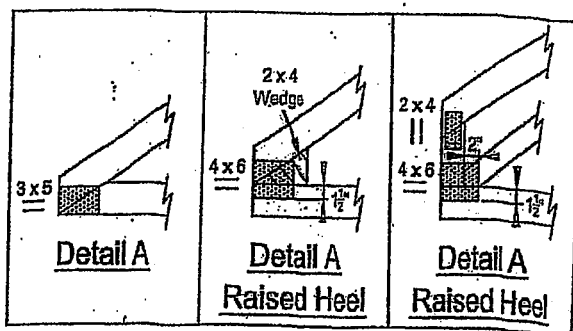
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

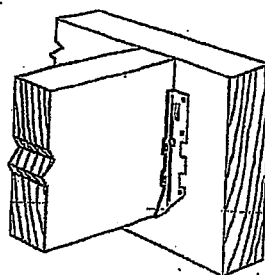
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

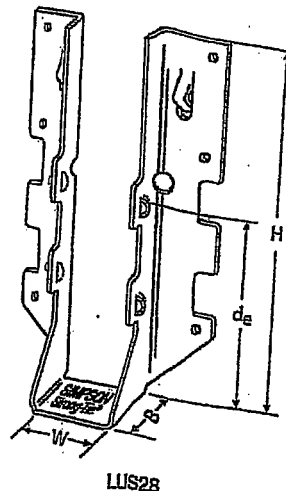
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.146" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



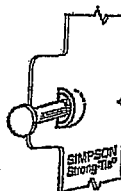
Typical LUS Installation



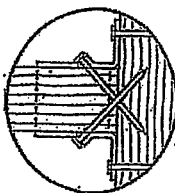
LUS28

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹		Face	Joist	D.Fir-L		SP-F	
									Uplift	Normal	Uplift	Normal
LUS24	18	1 1/8	3 1/4	1 1/4	1 1/8		(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/4	2	1 1/8		(4) 16d	(2) 16d	835	2020	690	1435
LUS26	18	1 1/8	4 1/4	1 1/4	3/8		(4) 10d	(4) 10d	1420	2170	1290	1680
LUS26-2	18	3 1/8	4 1/4	2	4		(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3/4		(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/4	1 1/4	3/8		(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4		(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3/4		(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/4	1 1/4	3/8		(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	8	2	6		(8) 16d	(6) 16d	2580	4500	2320	3185
LUS210-3	18	4 1/8	8 1/4	2	5/4		(8) 16d	(6) 16d	2580	3345	2320	2375

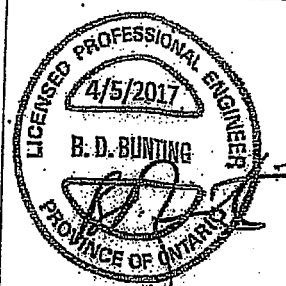
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

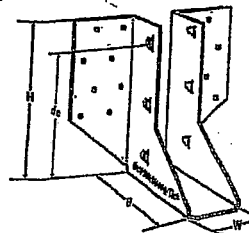
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

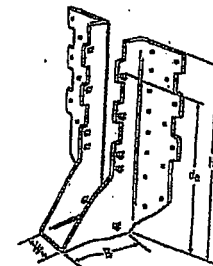
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

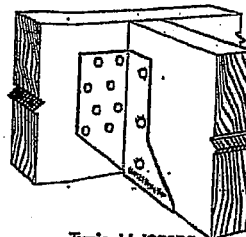
- See current catalogue for options



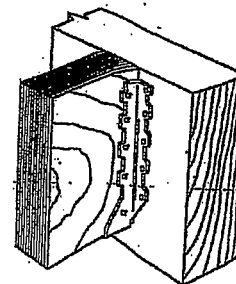
LJS26DS



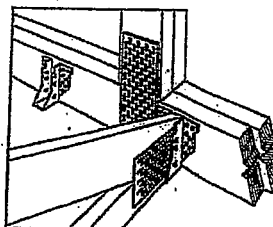
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



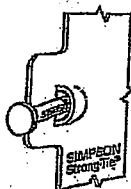
Typical HUS
Installation



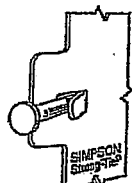
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	d ₂	Face	Joist	D.Fr-L		S-P-F	
									Uplift (K ₁ =1.15) lb.	Normal (K ₂ =1.00) lb.	Uplift (K ₁ =1.15) lb.	Normal (K ₂ =1.00) lb.
LJS26DS	18	1 1/4	5	3 1/2	4 3/4	(16) 16d	(6) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/4	5 1/2	3	3 3/4	(14) 16d	(6) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/4	7 1/2	3	6 1/2	(22) 16d	(8) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/4	9 1/2	3	7 3/4	(30) 16d	(10) 16d	(10) 16d	4505	5785	4010	4740
HUS1.81/10	16	1 1/4	9	3	8	(30) 16d	(10) 16d	(10) 16d	4505	6450	4010	5200

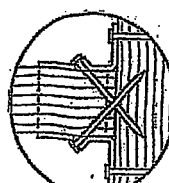
1. d₂ is the distance from the seat of the hanger to the highest joist nail.



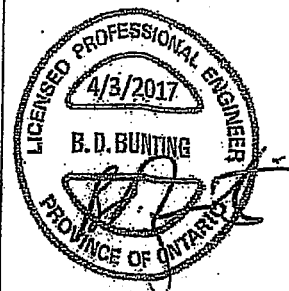
Double
Shear
Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



HGUS - Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

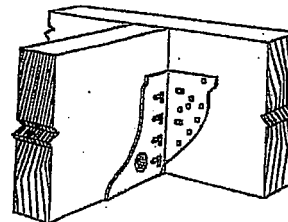
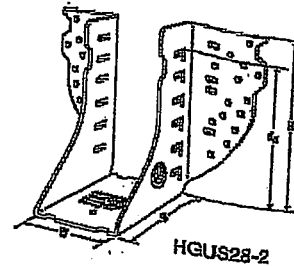
- Factored resistances are in accordance with CSA C86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

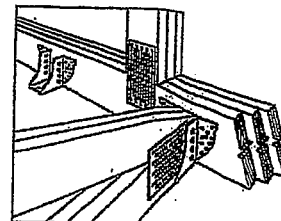
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



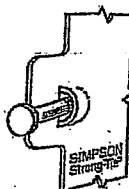
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

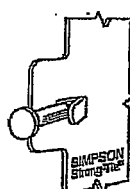
Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1 1/4	5 1/4	5	4 1/2	(20) 16d	(8) 16d		2885	6625	2885	5700
HGUS26-2	12	3 3/8	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28	12	1 1/4	7 1/4	5	6 1/4	(36) 16d	(12) 16d		3810	7675	3100	6900
HGUS28-2	12	3 3/8	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS210-2	12	3 3/8	9 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS212-4	12	6 3/8	10 1/4	4	10 1/4	(66) 16d	(20) 16d		7640	14995	5425	10645
HGUS214-4	12	6 3/8	12 1/4	4	11 1/4	(66) 16d	(22) 16d		10130	16400	7195	11645

1. d_s is the distance from the seat of the hanger to the highest joist nail.

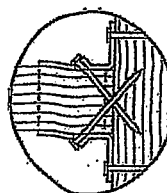


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

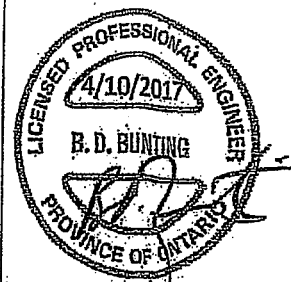
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



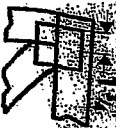
DESIGN

For more information, visit www.simpsonstrongtie.com or call 1-800-999-5099. © 2017 Simpson Strong-Tie Company. All rights reserved.

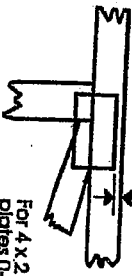
(800) 999-5099
strongtie.com

Symbols

PLATE LOCATION AND ORIENTATION



Apply plate on joint trusses only. Dimensions are in ft-in, kiloevents or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/2" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

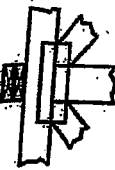
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

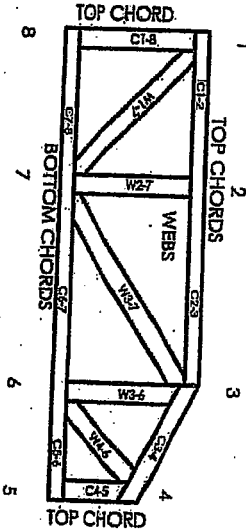


Indicates location where bearings (supports) occur. Icons vary but number where bearings occur.

Design Standards:
For Light Metal Plate Connected Wood Trusses
Building Standard for Design
Guide to Good Practice for Handling,
Installing & Ridding of Metal Plate
Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in, kiloevents or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10819-L, 13270-L, 12691-R

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSL.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral bracing members may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by TFC.
7. Design assumes trusses will be suitably protected from the environment in accord with TFC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 15% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire-retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration, and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or pulins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, waste and plates) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TFC Quality Criteria.

MITEK
POWER TO PERFORM.

Mitek Engineering Reference Sheet MTK-7473G rev. 10-08

H - Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14

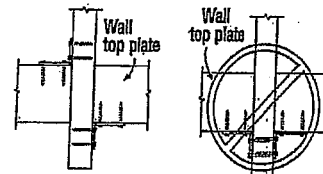
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

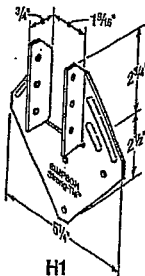
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

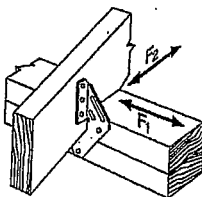


Install diagonally across from each other for minimum 2x truss.

Nailing into both sides of a single ply 2x truss may cause the wood to split.



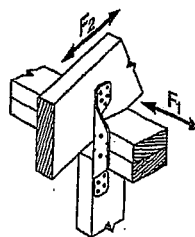
H1



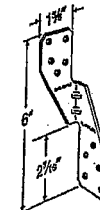
H1 Installation



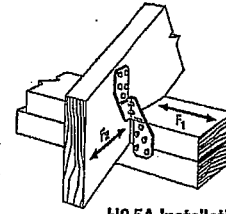
H2A



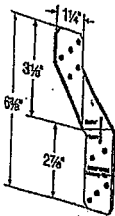
H2A Installation



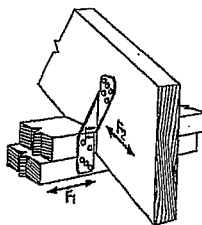
H2.5A



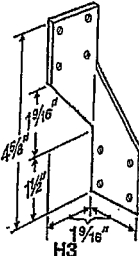
H2.5A Installation



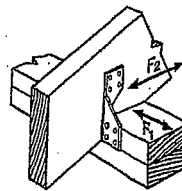
H2.5T



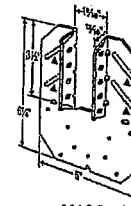
H2.5T Installation
(Nails into both top plates)



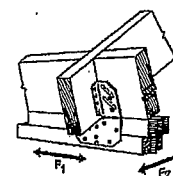
H3



H3 Installation



H10A



H10A Installation

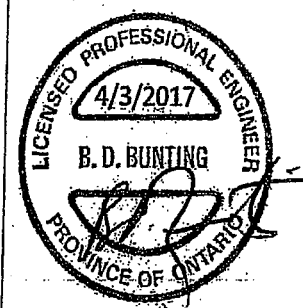
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
								(K ₀ =1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



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CP Crush Plates – Bearing Enhancers

SIMPSON
Strong-Tie

The CP transfers load from the truss or girder to plates for bearing limited conditions. Replaces nail-on scabs or in some cases, an additional ply when needed for bearing.

Material: See table

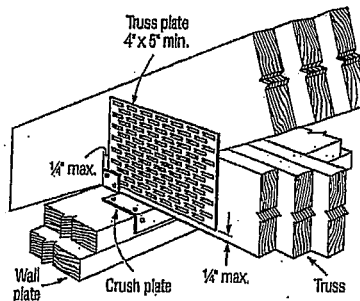
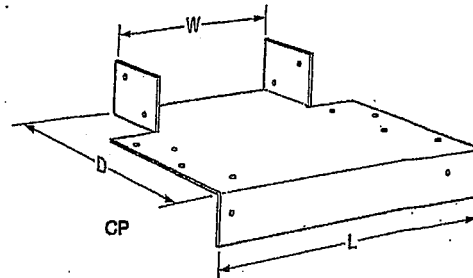
Finish: G90 galvanized

Design:

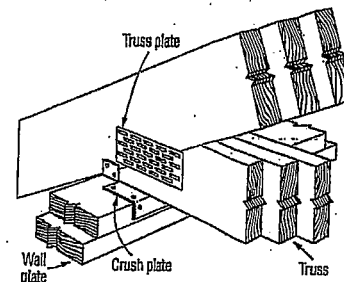
- Factored resistances are in accordance with CSA O86-14 assuming Q_r/A_b and $Q_r/A'_b = 812$ psi for D.Fir-L and 615 psi for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 when compression loads are applied to both sides of truss chord members at bearing locations.

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- For Case 1, truss plates must be located a maximum of ¼" from the underside of the truss chord and a maximum of ¼" from the edge of the wall plates in accordance with the reinforcing requirements of 7.5.9 TPIC 2014.



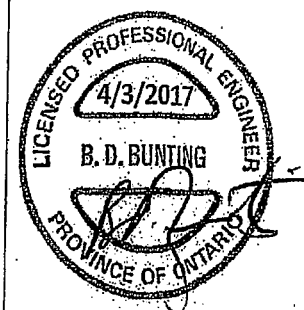
Case 1



Case 2

Model No.	Ga.	Wall Plate	Dimensions (in.)			Fasteners		Factored Resistance (lb.)				
			W	D	L	Wall Plate	Truss	D.Fir-L		S-P-F		
								Uplift	Bearing	Uplift	Bearing	
												(K _b =1.15)
CASE 1 (Truss Plate Reinforcement)												
CP1-4	20	2x4	1½	3½	4½	(6) 10d	(4) 10d x 1½"	225	5965	225	4515	
CP2-4	16		3¼	3½	5¾	(6) 10d	(4) 10d x 1½"	225	11390	225	9030	
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	17895	225	13545	
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	23860	225	18065	
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	9370	225	7095	
CP2-6	16		3¼	5½	5¾	(10) 10d	(4) 10d x 1½"	225	18740	225	14190	
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	28110	225	21285	
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	37495	225	28390	
CASE 2 (No Reinforcement)												
CP1-4	20	2x4	1½	3½	4½	(6) 10d	(4) 10d x 1½"	225	4885	225	3550	
CP2-4	16		3¼	3½	5¾	(6) 10d	(4) 10d x 1½"	225	9370	225	7100	
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	14055	225	10650	
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	18750	225	14195	
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	7365	225	5575	
CP2-6	16		3¼	5½	5¾	(10) 10d	(4) 10d x 1½"	225	14730	225	11150	
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	22095	225	16725	
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	29460	225	22305	

1. Factored bearing resistances assume wall plate and truss are the same species. For a mixed species system use S-P-F values.



This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty of use: www.simpsonstrongtie.com. © 2017 Simpson Strong-Tie Company Inc. T-SPEC0017-3/17 exp: 6/19

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strongtie.com

TS/LTS/MTS/HTS *Twist Straps*

Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3" bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

MATERIAL: LTS-18 gauge; MTS-16 gauge; HTS-14 gauge; TS-16 gauge
FINISH: Galvanized. Some products available in stainless steel and ZMAX®; see Corrosion Information, pages 14-17.

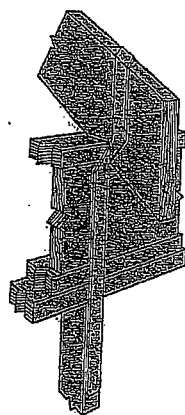
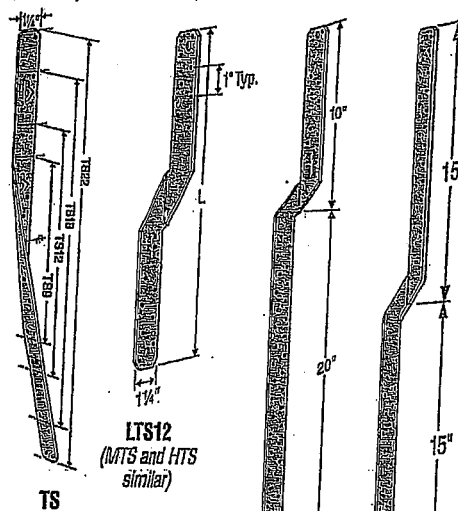
INSTALLATION: Use all specified fasteners. See General Notes.

- When LTS/MTS is installed as Truss-to-Top Plate tie, install 3-10d $\times$ 1½" nails to the w/s of the plate and 3-10d $\times$ 1½" nails into the edge of the double top plate.

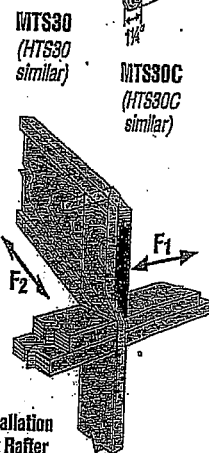
 These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

 These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

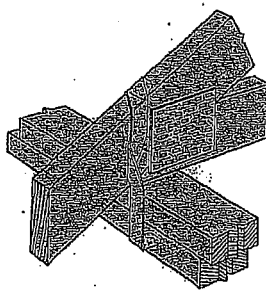
Model No.	L (in)	Fasteners (Total)	Factored Resistance (φ _p = 1.15)	
			D-Hi lbs kN	S-P lbs kN
TS9	9	8-16d	1125 500	1040 463
TS12	11½	10-16d	1410 627	1300 578
TS18	17¾	14-16d	1970 876	1820 810
TS22	21¾	18-16d	2125 946	2125 946
MTS12	12	12-10dx1½	1015 452	720 320
MTS16	16	12-10dx1½	1015 452	720 320
MTS18	18	12-10dx1½	1015 452	720 320
MTS20	20	12-10dx1½	1015 452	720 320
MTS12	12	14-10dx1½	1570 699	1180 525
MTS16	16	14-10dx1½	1570 699	1180 525
MTS18	18	14-10dx1½	1570 699	1180 525
MTS20	20	14-10dx1½	1570 699	1180 525
MTS30	30	14-10dx1½	1570 699	1180 525
MTS24C	24	14-10dx1½	1570 699	1180 525
MTS300	30	14-10dx1½	1570 699	1180 525
HTS16	16	16-10dx1½	2050 912	1455 647
HTS20	20	24-10dx1½	2050 912	1455 647
HTS24	24	24-10dx1½	2050 912	1455 647
HTS28	28	24-10dx1½	2050 912	1455 647



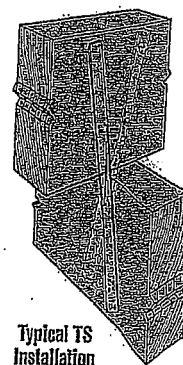
Typical MTS30 Installation



MTS30 Installation with I-joist Rafter

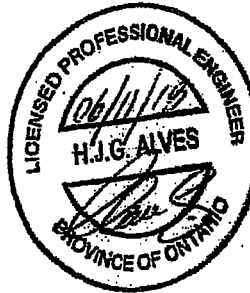


**LTS/MTS Installation
as a Truss-to-Top Plate Tie**
(See note 7)



Typical TS Installation

1. L1S12 thru L1S20, M1S16 through M1S30, H1S24 through H1S30G (except H1S30) have additional nail holes.
2. Install half of the fasteners on each end of strap to achieve maximum factored resistance.
3. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
4. All straps except the M1S30 and H1S30 have the twist in the centre of the strap.
5. Twist of straps shall be as follows:



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1- Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2- It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1- Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8- Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

F-1800218

Feb 09, 2018

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information		Application number:	
Building number, street name		Unit no. UNITS 1 - 7	Lot/con. BLOCK 7
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description	
B. Individual who reviews and takes responsibility for design activities			
Name SAM KATSOULAKOS		Firm MICRO CITY ENGINEERING SERVICES INC.	
Street address R.R #1, PO BOX 61		Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750	Cell number	
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]			
☐ House ☐ Small Buildings ☐ Large Buildings ☐ Complex Buildings ☐ HVAC – House ☐ Building Services ☐ Detection, Lighting and Power ☐ Fire Protection ☒ Building Structural ☐ Plumbing – House ☐ Plumbing – All Buildings ☐ On-site Sewage Systems			
Description of designer's work <u>ROYAL PINE HOMES – FORESTSIDE PH6 – BLOCK 7 – UNITS 1 - 7</u> (SCHEDULE IS ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM1147-19 DATED 1-18-19). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.			
D. Declaration of Designer			
I, <u>SAM KATSOULAKOS</u> declare that (choose one as appropriate): (print name)			
☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.			
Individual BCIN: <u>26064</u>			
Firm BCIN: <u>29991</u>			
☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code.			
Individual BCIN: _____			
Basis for exemption from registration: _____			
☐ The design work is exempt from the registration and qualification requirements of the Building Code.			
Basis for exemption from registration and qualification: _____			
I certify that:			
1. The information contained in this schedule is true to the best of my knowledge.			
2. I have submitted this application with the knowledge and consent of the firm.			
Date <u>1-18-19</u>		Signature of Designer 	

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d). of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG #TAM 1147 - 18S
 DWG #TAM 1148 - 17S

1-18-19
