

JANUARY 18, 2019
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT: 41239/300784)
DWG #T1901090 THROUGH DWG #T1901092, INCLUSIVE, AND ALL DATED 1-15-19;
DWG #T1900060 AND DWG #T1900066 AND BOTH DATED 1-03-19;
DWG #T1900063 AND DWG #T1900064 AND BOTH DATED 1-03-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

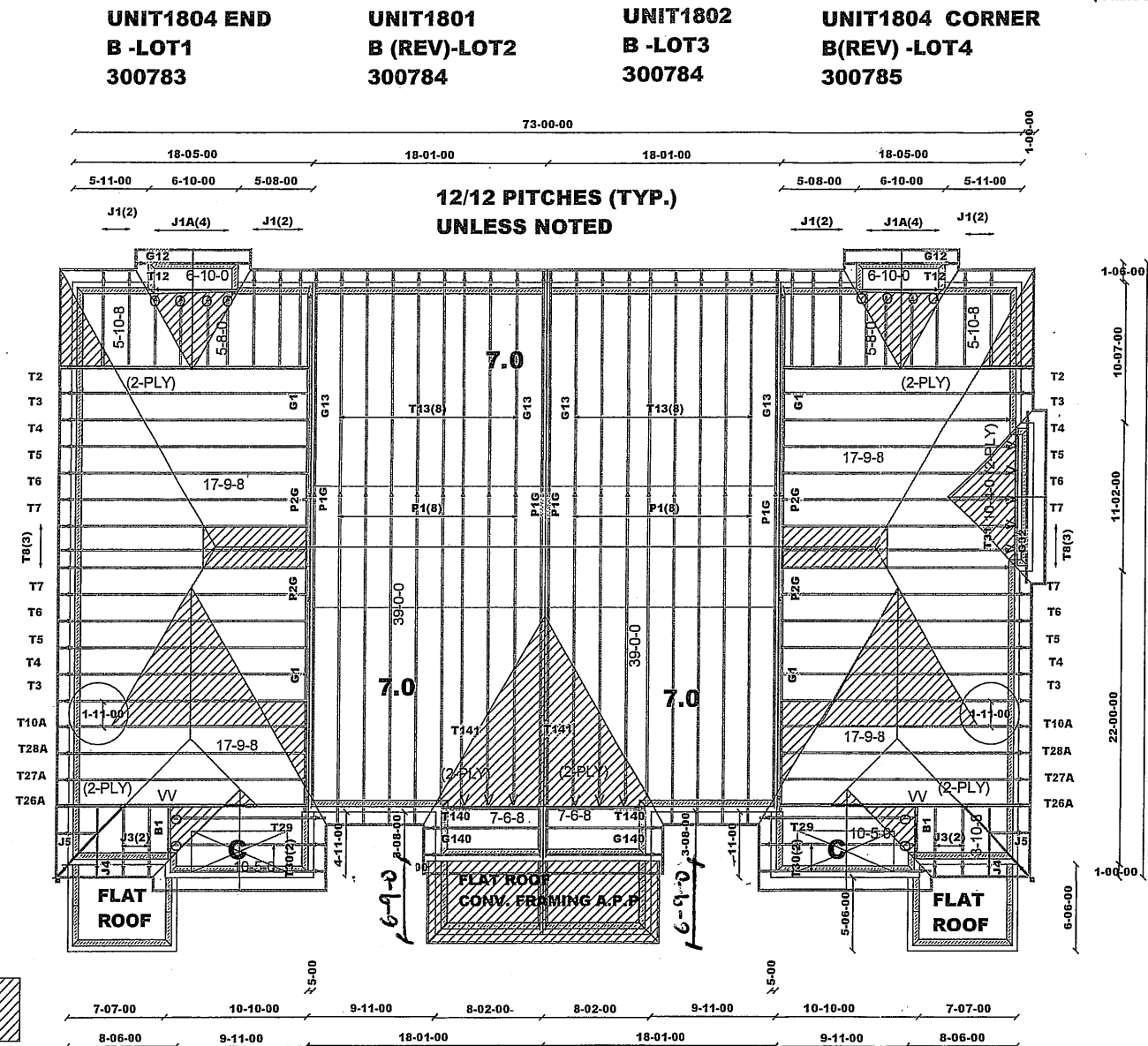
LUS26-2 -(VV)
LUS24 -(O)
LJS26DS -(V)

DENOTE:

C- 8/12 CATH. CEILING
ALL B - 2-2X10 FLUSH

T-180485

DENOTES:
CONVENTIONAL
FRAMING



JANUARY 18, 2019
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT: 41239/300783)
DWG #T1900044 THROUGH DWG #T1900047, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900093 THROUGH DWG #T1900097, INCLUSIVE, AND ALL DATED 1-15-19;
DWG #T1900039; DWG #T1900040; DWG #T1900042; AND ALL DATED 1-03-19;
DWG #T1900034; DWG #T1900035; DWG #T1900049; AND ALL DATED 1-03-19;
DWG #T1900052 AND DWG #T1900056 AND BOTH DATED 1-03-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

JANUARY 18, 2019
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT: 41239/300785)
DWG #T1901093 THROUGH DWG #T1901099, INCLUSIVE, AND ALL DATED 1-15-19;
DWG #T1900044 THROUGH DWG #T1900047, INCLUSIVE, AND ALL DATED 1-03-19;
DWG #T1900034; DWG #T1900035; DWG #T1900042; AND ALL DATED 1-03-19;
DWG #T1900039; DWG #T1900040; DWG #T1900056; AND ALL DATED 1-03-19;
DWG #T1900049 AND DWG #T1900052 AND BOTH DATED 1-03-19;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

DATE: 1-18-19
BCIN: 26064; FIRM: BCIN #29991
ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER
ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO TRUSS INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT.
ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY THE BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION. LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH INDIVIDUAL LOT.

DWG #TAM... THROUGH DWG #TAM... INCLUSIVE, DATED ;
(STRUCTURAL COMPONENTS ONLY); THIS LAYOUT MUST BE READ TOGETHER WITH REFERENCED TRUSS COMPONENT DRAWINGS AND SEALED HARDWARE DRAWINGS.

HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY.
REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING REQUIREMENTS:
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2 X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS SPECIFIED ON REFERENCED SEALED TRUSS COMPONENT DRAWINGS USING 2 - 3-1/4" COMMON WIRE NAILS PER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF 'X-BRACING' ALONG THESE SAME WEB MEMBERS AT LOCATIONS INDICATED (*) USING THE SAME NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
PROVIDE 2 X 4 #2 SPF T-BRACE ALONG FULL LENGTH OF LATERALLY BRACED WEB MEMBERS OF 1 PLY TRUSSES COMPONENTS REQUIRING T-BRACE.
USING 1 ROW OF 3-1/4" COMMON WIRE NAILS AT 6" O/C. SIMILARLY PROVIDE 2 X 6 BRACE WITH 2 ROWS OF NAILING FOR 2 PLY TRUSS COMPONENTS AND 2 X 8 BRACE WITH 3 ROWS OF NAILING FOR 3 PLY TRUSS COMPONENTS.

(1) 2 X 6 #2 SPF ON FLAT OVER PLYWOOD SHEATHING.
(2) 2 X 6 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS COMPONENTS AND AT LOCATIONS MARKED BY 'X'.
(3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O/C WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X 4 VERTICAL SUPPORTS TO TRUSS BASE BELOW WHERE NECESSARY).

ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE TRUSS COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH STUDS AT 24" O/C.
ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE 2012.

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION 3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES AND/OR CATEGORIES.
REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.
DWG #TAM
BCIN: 26064
FIRM: 29991
SEALED STRUCTURAL COMPONENTS ONLY

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC, LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 23.3 PSF
TC DEAD 15 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

M12143



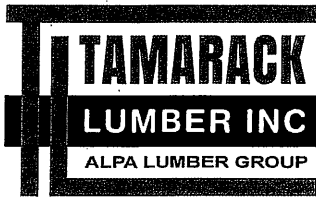
Job Track: 41239
Layout ID: 300782
Plan Log: 98887

Builder / Location:
ROYAL PINE HOMES/ BRAMPTON
Project: FORESTSIDE PH.6
Date: 11/15/2018 Designer: SUNNY

Model / Elevation:
BLOCK 9/ LOTS 1 TO 4

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THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK: 41239

LAYOUT ID: 300783

LOCATION:

BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT

SUB-BUILDER:

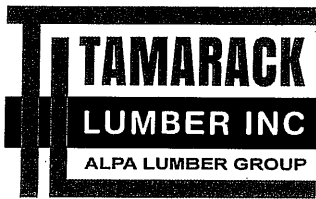
MODEL: UNIT1804

ELEVATION: B-LOT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	G1 GABLE	7.00 0.00	19-06-00	10-00-00	2 X 4	2 X 4	00-00-00 00-00-00	01-03-07 10-00-00	221.50 140.00		
	1 2 Ply	T2 HALF HIP	12.00 0.00	17-09-08	04-08-09	2 X 4	2 X 6	01-03-08 00-00-00	01-10-08 04-08-09	199.00 121.34		
	2	T3 HALF HIP	12.00 0.00	17-09-08	05-10-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-10-09	156.36 98.00		
	2	T4 HALF HIP	12.00 0.00	17-09-08	07-00-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 07-00-09	166.94 104.66		
	2	T5 HALF HIP	12.00 0.00	17-09-08	08-02-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 08-02-09	185.34 117.34		
	2	T6 HALF HIP	12.00 0.00	17-09-08	09-04-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-04-09	210.84 132.00		
	2	T7 HALF HIP	12.00 0.00	17-09-08	10-06-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 10-06-09	208.64 127.66		
	3	T8 HALF HIP	12.00 0.00	17-09-08	11-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 11-08-09	350.76 216.00		
	1 2 Ply	T26A HIP GIRDER	12.00 0.00	17-09-08	05-09-00	2 X 4	2 X 6	01-03-08 00-00-00	01-10-08 02-01-00	205.94 124.00		
	1	T27A HIP	12.00 0.00	17-09-08	07-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	80.84 51.67		
	1	T28A HIP	12.00 0.00	17-09-08	09-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	97.54 63.00		
	1	T10A COMMON	12.00 0.00	17-09-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	95.38 59.67		
	1	T29 COMMON	12.00 0.00	10-05-00	07-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	50.24 32.83		
	2	T30 ROOF	12.00 8.00	10-05-00	07-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	110.76 72.66		
	1	T12 COMMON	12.00 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	33.29 23.17		
	1	G12 GABLE	12.00 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	34.17 24.17		
	2	P2G GABLE	7.00 0.00	03-08-13	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-07-15	24.90 16.00		
	5	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	86.50 53.35		



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK:41239	LAYOUT ID: 300783	LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT	SUB-BUILDER:	
MODEL: UNIT1804	ELEVATION: B-LOT1	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	4	J1A JACK-OPEN	7.00 0.00	05-08-00	04-08-09	2 X 4	2 X 4	00-00-00 00-00-00	01-04-14 04-08-09	60.88 37.32		
	2	J3 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	29.50 18.66		
	1	J4 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 02-01-01	01-10-08 00-03-08	11.39 7.33		
	1	J5 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 00-01-01	01-10-08 00-03-08	9.10 6.00		

TOTAL # TRUSS= 42.00

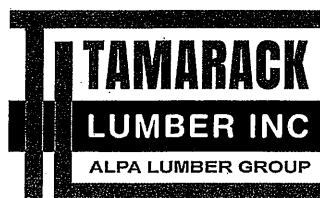
TOTAL BFT OF ALL TRUSSES=

1646.83 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2629.81 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
6	Hangers	LUS24	

TOTAL # ITEMS= 7.00



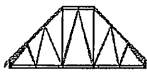
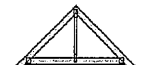
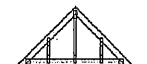
Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK:41239 LAYOUT ID: 300784 LOCATION:
 BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT SUB-BUILDER:
 MODEL: UNIT1801 OR UNIT1802 ELEVATION: B LOT2 ,LOT3

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	8	T13 PIGGYBACK	7.00 0.00	39-00-00	10-00-00	2 X 4 2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	1640.08 1016.00		
	2	G13 GABLE	7.00 0.00	39-00-00	10-00-00	2 X 4 2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	453.88 284.66		
	1	T140 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	46.85 29.83		
	1	G140 GABLE	12.00 0.00	07-06-08	09-05-00	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 09-05-00	48.08 29.83		
	1 2 Ply	T141 MONOPITCH	12.00 0.00	07-06-08	09-05-00	2 X 4 2 X 6	00-00-00 00-00-00	01-10-08 09-05-00	109.56 66.66		
	8	P1 PIGGYBACK	7.00 0.00	07-05-10	02-06-07	2 X 4 2 X 4	00-00-00 00-00-00	00-04-05 00-04-05	176.88 114.64		
	2	P1G GABLE	7.00 0.00	07-05-10	02-06-07	2 X 4 2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	46.84 30.66		

TOTAL # TRUSS= 24.00

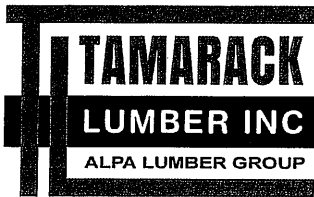
TOTAL BFT OF ALL TRUSSES=

1572.28 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2522.17 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LJS26DS	

TOTAL # ITEMS= 3.00



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK:41239

LAYOUT ID: 300785

LOCATION:

BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT

SUB-BUILDER:

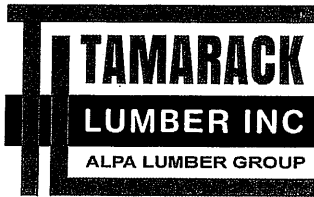
MODEL: UNIT1804 CORNER

ELEVATION: B-LOT4

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	G1 GABLE	7.00 0.00	19-06-00	10-00-00	2 X 4	2 X 4	00-00-00 00-00-00	01-03-07 10-00-00	221.50 140.00		
	1 2 Ply	T2 HALF HIP	12.00 0.00	17-09-08	04-08-09	2 X 4	2 X 6	01-03-08 00-00-00	01-10-08 04-08-09	199.00 121.34		
	2	T3 HALF HIP	12.00 0.00	17-09-08	05-10-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-10-09	156.36 98.00		
	2	T4 HALF HIP	12.00 0.00	17-09-08	07-00-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 07-00-09	166.94 104.66		
	2	T5 HALF HIP	12.00 0.00	17-09-08	08-02-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 08-02-09	185.34 117.34		
	2	T6 HALF HIP	12.00 0.00	17-09-08	09-04-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 09-04-09	210.84 132.00		
	2	T7 HALF HIP	12.00 0.00	17-09-08	10-06-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 10-06-09	208.64 127.66		
	3	T8 HALF HIP	12.00 0.00	17-09-08	11-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 11-08-09	350.76 216.00		
	1 2 Ply	T26A HIP GIRDER	12.00 0.00	17-09-08	05-09-00	2 X 4	2 X 6	01-03-08 00-00-00	01-10-08 02-01-00	205.94 124.00		
	1	T27A HIP	12.00 0.00	17-09-08	07-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	80.84 51.67		
	1	T28A HIP	12.00 0.00	17-09-08	09-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	97.54 63.00		
	1	T10A COMMON	12.00 0.00	17-09-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 02-01-00	95.38 59.67		
	1	T29 COMMON	12.00 0.00	10-05-00	07-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	50.24 32.83		
	2	T30 ROOF	12.00 8.00	10-05-00	07-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	110.76 72.66		
	1 2 Ply	T31 COMMON	12.00 0.00	10-04-00	07-00-08	2 X 4	2 X 6	00-00-00 00-00-00	01-10-08 01-10-08	118.10 76.00		
	1	G32 COMMON	12.00 0.00	10-04-00	07-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	55.75 36.83		
	1	T12 COMMON	12.00 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	33.29 23.17		
	1	G12 GABLE	12.00 0.00	06-10-00	04-10-08	2 X 4	2 X 4	00-10-08 00-10-08	01-05-08 01-05-08	34.17 24.17		



Delivery Shiplist

DATE	09/13/18
SALES REP	Mario

JOB TRACK:41239	LAYOUT ID: 300785	LOCATION:
BUILDER: ROYAL PINE - FORESTSIDE/BRAMPT	SUB-BUILDER:	
MODEL: UNIT1804 CORNER	ELEVATION: B-LOT4	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	P2G GABLE	7.00 0.00	03-08-13	02-06-07	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-07-15	24.90 16.00		
	5	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	86.50 53.35		
	4	J1A JACK-OPEN	7.00 0.00	05-08-00	04-08-09	2 X 4	2 X 4	00-00-00 00-00-00	01-04-14 04-08-09	60.88 37.32		
	2	J3 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	29.50 18.66		
	1	J4 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 02-01-01	01-10-08 00-03-08	11.39 7.33		
	1	J5 JACK-OPEN	12.00 0.00	01-09-07	03-07-15	2 X 4	2 X 4	01-03-08 00-01-01	01-10-08 00-03-08	9.10 6.00		

TOTAL # TRUSS= 45.00

TOTAL BFT OF ALL TRUSSES=

1759.66 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2803.66 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
6	Hangers	LUS24	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 12.00

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:24:25 2019 Page 1
ID:K??_mHrXOPUzKFD0TOr6u1yw9qF-QQdLoXz03qint2xjVQxS7VYKeuxMjGa8zLQewizz5N4



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

LOADING									
TOTAL LOAD CASES: (4).									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	-10 / 0	-84.9	-84.9	0.09 (1)	6.25	N-K	-121 / 0	0.08 (1)	
B-C	-28 / 0	-84.9	-84.9	0.09 (1)	6.25	W-B	-254 / 0	0.05 (1)	
C-D	-8 / 0	-84.9	-84.9	0.04 (1)	10.00	V-C	-139 / 0	0.04 (1)	
D-E	-8 / 0	-84.9	-84.9	0.04 (1)	10.00	U-D	-174 / 0	0.08 (1)	
E-F	-6 / 0	-84.9	-84.9	0.04 (1)	10.00	T-E	-168 / 0	0.12 (1)	
F-G	-6 / 0	-84.9	-84.9	0.04 (1)	10.00	S-G	-166 / 0	0.18 (1)	
G-H	-2 / 0	-84.9	-84.9	0.04 (1)	10.00	Q-H	-187 / 0	0.10 (1)	
H-I	-10 / 0	-84.9	-84.9	0.04 (1)	6.25	P-I	-136 / 0	0.09 (1)	
I-J	0 / 0	-84.9	-84.9	0.04 (1)	10.00	O-J	-125 / 0	0.08 (1)	
J-K	0 / 0	-84.9	-84.9	0.04 (1)	10.00	A-W	0 / 20	0.00 (1)	
K-L	0 / 0	-84.9	-84.9	0.04 (1)	10.00				
M-L	-72 / 0	0.0	0.0	0.03 (1)	6.25				
X-A	-120 / 0	0.0	0.0	0.01 (1)	7.81				
X-W	0 / 0	-38.5	-38.5	0.06 (3)	10.00				
W-V	0 / 12	-38.5	-38.5	0.08 (3)	10.00				
V-U	0 / 9	-38.5	-38.5	0.03 (3)	10.00				
U-T	0 / 7	-38.5	-38.5	0.03 (3)	10.00				
T-S	0 / 5	-38.5	-38.5	0.02 (3)	10.00				
S-R	0 / 3	-38.5	-38.5	0.02 (3)	10.00				
R-Q	0 / 3	-38.5	-38.5	0.02 (3)	10.00				
Q-P	0 / 1	-38.5	-38.5	0.02 (3)	10.00				
P-O	0 / 0	-38.5	-38.5	0.02 (3)	10.00				
O-N	0 / 0	-38.5	-38.5	0.03 (3)	10.00				
N-M	0 / 0	-38.5	-38.5	0.03 (3)	10.00				

CSI: TC=0.09/1.00 (B-C:1), BC=0.06/1.00 (V-W:3)
WB=0.18/1.00 (G-S:1), SSI=0.10/1.00 (A-B:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

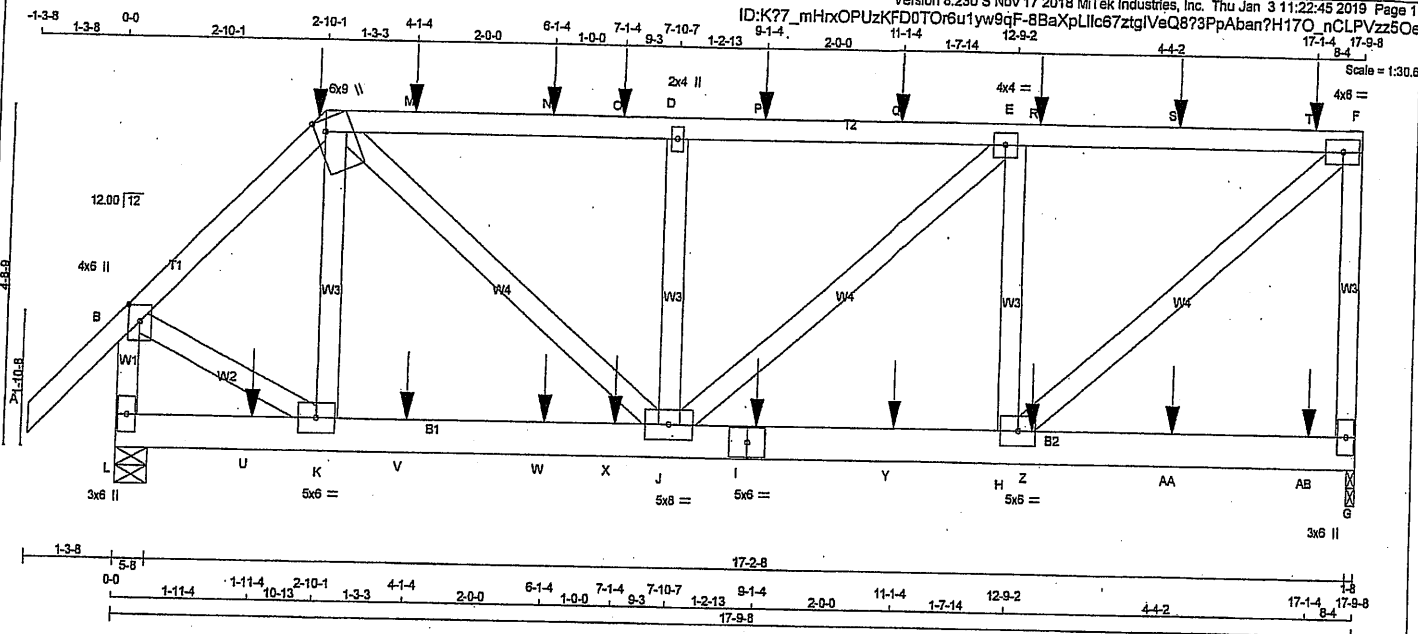
JSI GRIP= 0.60 (I) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHxOPUzKFD0TO6u1yw9gF-8BaXpLlc67ztglVeQ873PpAban?H17O_nCLPVzz5Oe



TOTAL WEIGHT = 2 X 99 = 199 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x6	DRY	No.2
I - G	2x6	DRY	No.2

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
A-C	12	SIDE(61.0)
C-F	12	SIDE(61.0)
F-G	12	TOP
L-B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-I	2	SIDE(0.0)
I-G	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	2.75	2.00
C TTWW+m	MT20	6.0	9.0		
D TMVW+w	MT20	2.0	4.0	Edge	1.75
E TMVW+t	MT20	4.0	4.0		
F TMVW-t	MT20	4.0	6.0		
G BMV1+p	MT20	3.0	6.0		
H BMVW+t	MT20	5.0	6.0		
I -BS-t	MT20	5.0	6.0		
J BMVWV-t	MT20	5.0	8.0		
K BMVW-t	MT20	5.0	6.0		
L BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
G	VERT	DOWN	UPLIFT	IN-SX
L	2244	0	1-8	1-8
L	2221	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1701	850 / 0	379 / 0	0 / 0	0 / 0	472 / 0	0 / 0
L	1676	852 / 0	355 / 0	0 / 0	0 / 0	459 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	UNBRAC LENGTH	FR-TO		
A-B	0 / 42	-84.9	-84.9	0.07 (1)	10.00	K-C	-274 / 105
B-C	-1854 / 0	-84.9	-84.9	0.08 (1)	8.25	C-J	0 / 1431
C-M	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35	J-D	-84.9 / 0
M-N	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35	D-E	-84.9 / 0
N-O	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35	E-H	-84.9 / 0
O-D	-2381 / 0	-84.9	-84.9	0.37 (1)	5.35	H-I	-84.9 / 0
D-P	-2381 / 0	-84.9	-84.9	0.36 (1)	5.35	P-Q	-2381 / 0
P-Q	-2381 / 0	-84.9	-84.9	0.36 (1)	5.35	Q-E	-2381 / 0
Q-E	-2381 / 0	-84.9	-84.9	0.36 (1)	5.35	E-R	-1958 / 0
E-R	-1958 / 0	-84.9	-84.9	0.32 (1)	5.35	R-S	-1958 / 0
R-S	-1958 / 0	-84.9	-84.9	0.32 (1)	5.35	S-T	-1958 / 0
S-T	-1958 / 0	-84.9	-84.9	0.32 (1)	5.35	T-F	-1958 / 0
T-F	-1958 / 0	-84.9	-84.9	0.32 (1)	5.35	F-G	-2085 / 0
F-G	-2085 / 0	0.0	0.0	0.35 (1)	7.63	G-L	-2185 / 0
G-L	-2185 / 0	0.0	0.0	0.13 (1)	7.53		

L-U	0 / 0	-38.5	-38.5	0.07 (3)	10.00		
U-K	0 / 0	-38.5	-38.5	0.07 (3)	10.00		
K-V	0 / 1294	-38.5	-38.5	0.14 (2)	10.00		
V-W	0 / 1294	-38.5	-38.5	0.14 (2)	10.00		
W-X	0 / 1294	-38.5	-38.5	0.14 (2)	10.00		
X-J	0 / 1294	-38.5	-38.5	0.14 (2)	10.00		
J-I	0 / 1958	-38.5	-38.5	0.19 (2)	10.00		
I-Y	0 / 1958	-38.5	-38.5	0.19 (2)	10.00		
Y-H	0 / 1958	-38.5	-38.5	0.19 (2)	10.00		
H-Z	0 / 0	-38.5	-38.5	0.08 (3)	10.00		
Z-AA	0 / 0	-38.5	-38.5	0.08 (3)	10.00		
AA-AB	0 / 0	-38.5	-38.5	0.08 (3)	10.00		
AB-G	0 / 0	-38.5	-38.5	0.08 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-10-1	-35	-39		FRONT	VERT	DEAD		
C	2-10-1	-162	-162		FRONT	VERT	SNOW		
I	9-1-4	-71	-90		BACK	VERT	TOTAL		
M	4-1-4	-164	-164		BACK	VERT	TOTAL		
N	6-1-4	-156	-156		BACK	VERT	TOTAL		
O	7-1-4	-156	-156		BACK	VERT	TOTAL		
P	9-1-4	-156	-156		BACK	VERT	TOTAL		
Q	11-1-4	-156	-156		BACK	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.37/1.00 (C-D-1), BC=0.19/1.00 (H-J-2),
WB=0.23/1.00 (F-H-1), SS=0.25/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
		MAX MIN	MAX MIN
MT20	618	354	1657

PLATE PLACEMENT TOL = 0.250 inches

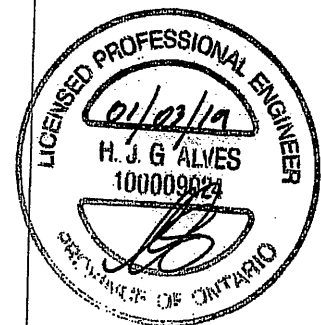
PLATE ROTATION TOL = 6.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T2	1	2	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 11:22:45 2019 Page 2
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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	13-1-4	-164	-164	---	BACK	VERT	TOTAL	---	---
S	15-1-4	-164	-164	---	BACK	VERT	TOTAL	---	---
T	17-1-4	-178	-178	---	BACK	VERT	TOTAL	---	---
U	1-11-4	-75	-95	---	BACK	VERT	TOTAL	---	---
V	4-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
W	6-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
X	7-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
Y	11-1-4	-71	-90	---	BACK	VERT	TOTAL	---	---
Z	13-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
AA	15-1-4	-75	-95	---	BACK	VERT	TOTAL	---	---
AB	17-1-4	-81	-103	---	BACK	VERT	TOTAL	---	---

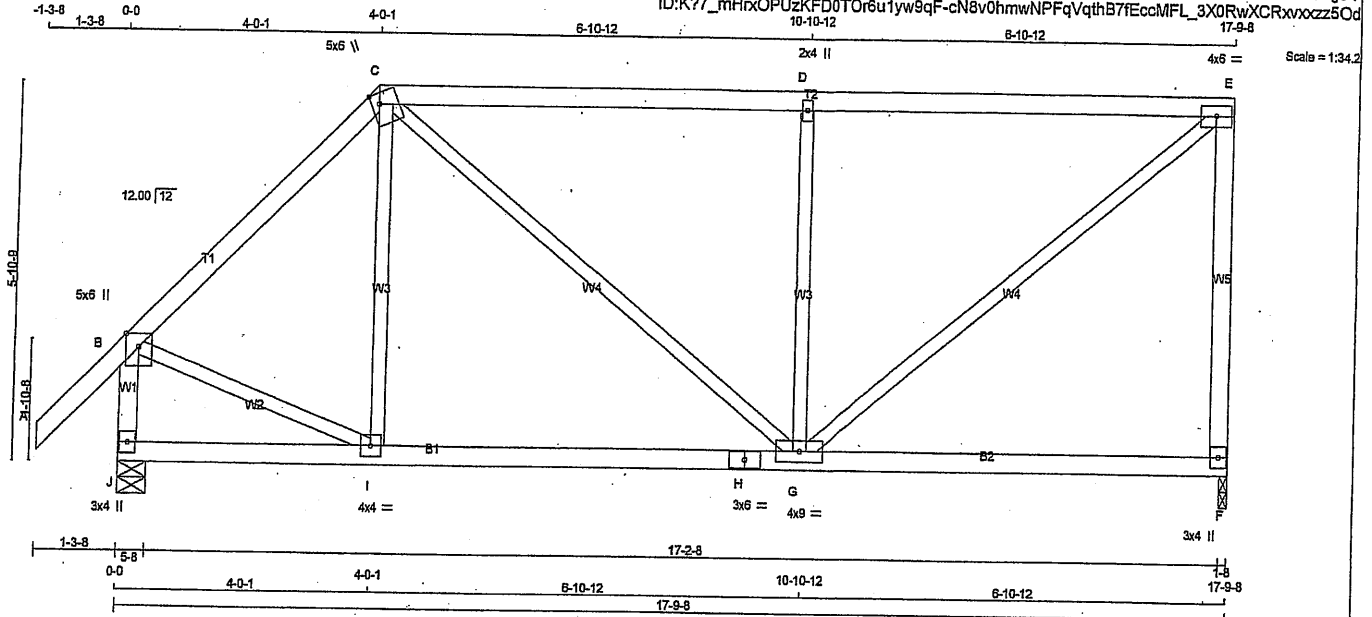


DWG NO. TAM **T1900039**
 STRUCTURAL
 COMPONENT ONLY **2/2**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MTek Industries, Inc. Thu Jan 3 11:22:46 2019 Page 1
ID:K77_mHrxOPUzKFD0TOr6u1yw9qF-cN8v0hmvNPFqVqthB7EccMFL_3X0RwXCRxvxxzz5Od



TOTAL WEIGHT = 78 lb

LUMBER

N L G A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - D	2x4	DRY No.2
D - E	2x4	DRY No.2
E - F	2x4	DRY No.2
F - G	2x4	DRY No.2
G - H	2x4	DRY No.2
H - I	2x4	DRY No.2
I - J	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMVWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1097	0	1097	0
J	1216	0	1216	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
F	832	414/0	187/0	0/0	0/0	231/0
J	914	479/0	187/0	0/0	0/0	248/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO		FROM	TO		FR-TO		
A-B	0/42	-84.9	-84.9	0.12 (1)	I-C	-13/154	0.04 (3)
B-C	-900/0	-84.9	-84.9	0.26 (1)	C-G	0/410	0.09 (1)
C-D	-952/0	-84.9	-84.9	0.75 (1)	G-D	-723/0	0.39 (1)
D-E	-952/0	-84.9	-84.9	0.75 (1)	G-E	0/1223	0.28 (1)
E-F	-991/0	0.0	0.0	0.64 (1)	B-I	0/676	0.15 (1)
F-G	-1162/0	0.0	0.0	0.13 (1)			
J-I	0/0	-38.5	-38.5	0.19 (3)			
I-H	0/634	-38.5	-38.5	0.42 (2)			
H-G	0/634	-38.5	-38.5	0.42 (2)			
G-F	0/0	-38.5	-38.5	0.36 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.75/1.00 (D-E:1), BC=0.42/1.00 (G-I:2), WB=0.39/1.00 (D-G:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

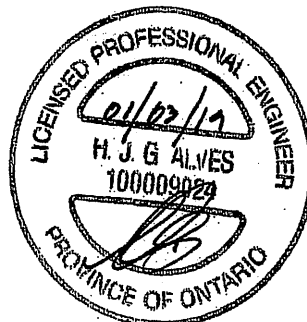
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI)
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1967 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.79 (I) (INPUT= 0.90)
JSI METAL= 0.44 (H) (INPUT= 1.00)

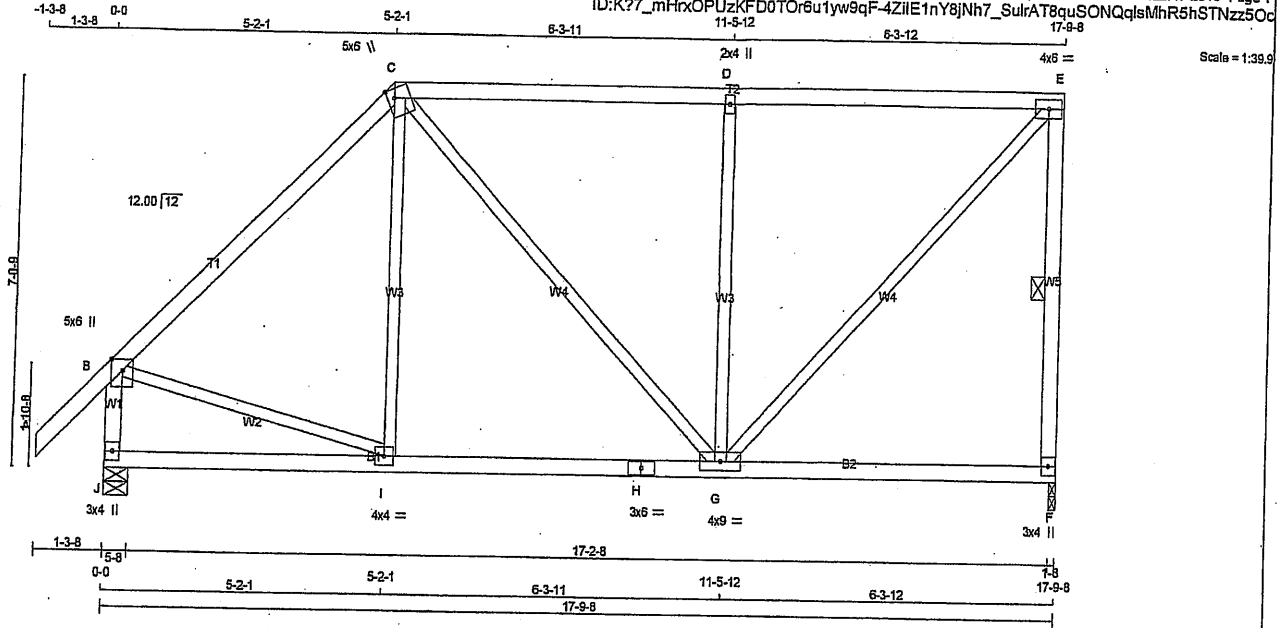


DRWG NO. TAM 11900040
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:39.9

TOTAL WEIGHT = 83 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C TTVW+m	MT20	5.0	6.0	2.00	1.50
D TMW+w	MT20	2.0	4.0		
E TMVW-t	MT20	4.0	6.0		
F BMV1+p	MT20	3.0	4.0		
G BMVWV-t	MT20	4.0	9.0		
H BS-t	MT20	3.0	6.0		
I BMVW-t	MT20	4.0	4.0		
J BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	1097	0	1097	0	0	1-8	1-8		
J	1216	0	1216	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	PERM	LIVE	WIND	DEAD	SOIL
F	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0	0 / 0
J	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 42	-84.9 -84.9	0.12 (1)	10.00	I-C	0 / 204	0.05 (3)
B-C	-875 / 0	-84.9 -84.9	0.44 (1)	5.94	C-G	0 / 189	0.04 (1)
C-D	-748 / 0	-84.9 -84.9	0.60 (1)	5.88	G-D	-652 / 0	0.57 (1)
D-E	-748 / 0	-84.9 -84.9	0.60 (1)	5.88	G-E	0 / 1090	0.25 (1)
F-E	-999 / 0	0.0 0.0	0.22 (1)	6.25	B-I	0 / 642	0.14 (1)
J-B	-1136 / 0	0.0 0.0	0.13 (1)	7.45			
J-I	0 / 0	-38.5 -38.5	0.21 (3)	10.00			
I-H	0 / 618	-38.5 -38.5	0.36 (2)	10.00			
H-G	0 / 618	-38.5 -38.5	0.36 (2)	10.00			
G-F	0 / 0	-38.5 -38.5	0.29 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.60/1.00 (D-E:1), BC=0.36/1.00 (G-I:2), WB=0.57/1.00 (D-G:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

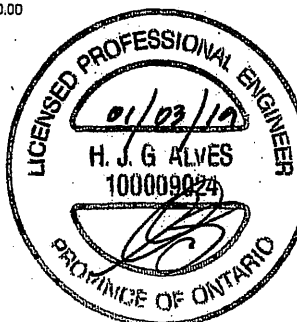
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (G) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)



DWG NO. TAM 71900042
STRUCTURAL
COMPONENT ONLY

amarack Roof Russ, Burlington

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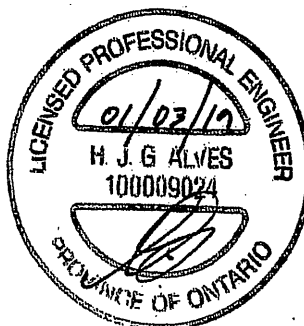
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12-0-13

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DWG NO. TAM 7190044
STRUCTURAL
COMPONENT ONLY

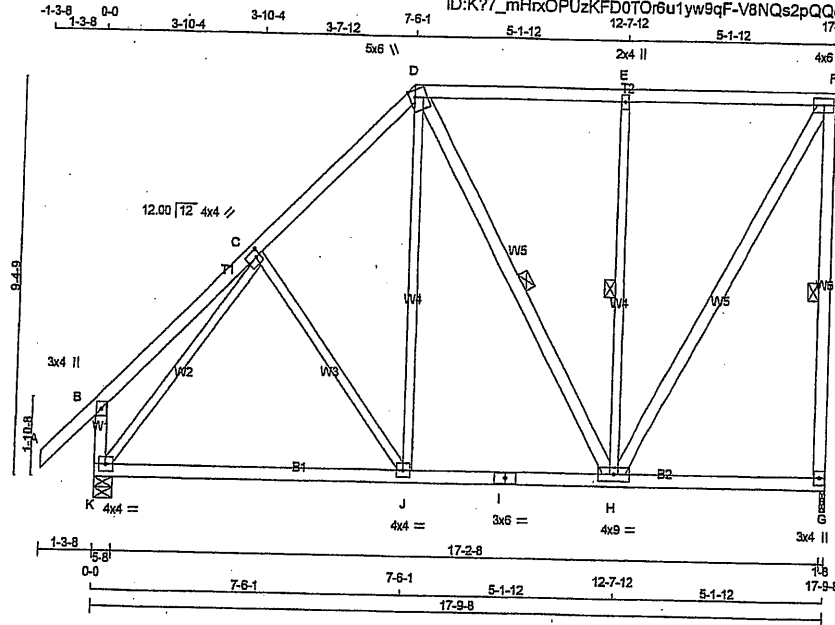


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:52.0

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS	SIZE	DRY	LUMBER
EXCEPT			
D - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMV-w	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
G	1097	0	1-8	1-8
K	1216	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
K	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-84.9 -84.9	0.12 (1)	10.00	C-J	-105 / 32	0.08 (1)
B-C	0 / 26	-84.9 -84.9	0.19 (1)	10.00	J-D	0 / 413	0.09 (2)
C-D	-798 / 0	-84.9 -84.9	0.20 (1)	6.25	D-H	-145 / 0	0.08 (2)
D-E	-481 / 0	-84.9 -84.9	0.38 (1)	6.25	H-E	-539 / 0	0.31 (1)
E-F	-481 / 0	-84.9 -84.9	0.38 (1)	6.25	F-F	0 / 971	0.16 (1)
G-F	-1011 / 0	0.0 0.0	0.42 (1)	6.25	K-C	-1049 / 0	0.76 (1)
K-B	-239 / 0	0.0 0.0	0.03 (1)	7.81			
K-J	0 / 607	-38.5 -38.5	0.42 (2)	10.00			
J-I	0 / 549	-38.5 -38.5	0.42 (2)	10.00			
I-H	0 / 549	-38.5 -38.5	0.42 (2)	10.00			
H-G	0 / 0	-38.5 -38.5	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")

CALCULATED VERT. DEFL.(LL) = L/999 (0.13")

ALLOWABLE DEFL.(TL) = L/360 (0.59")

CALCULATED VERT. DEFL.(TL) = L/957 (0.22")

CSI: TC=0.42/1.00 (F-G-1), BC=0.42/1.00 (J-K-2), WB=0.76/1.00 (C-K-1), SSI=0.21/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

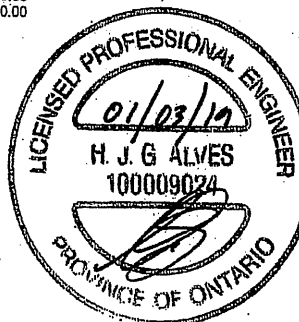
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
618	354	1667	788
1987	1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

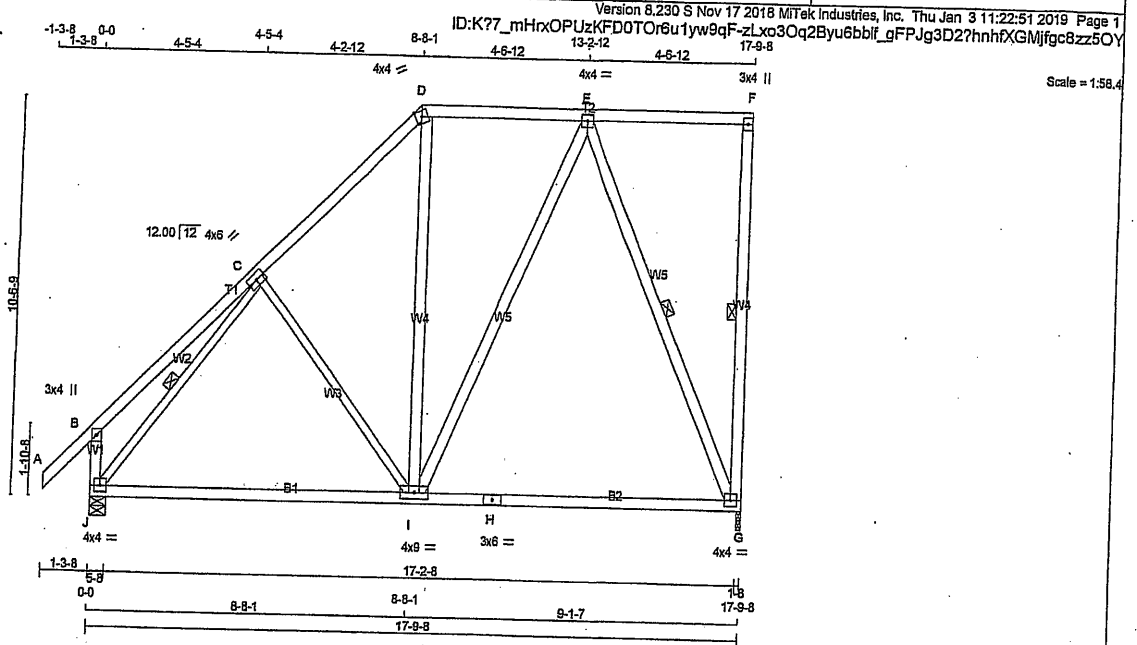
JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)



DRWG NO. TAM T1900045
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
C - I	2x3	DRY	No.2	SPF	
J - C	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMWW-t	MT20	4.0	6.0			
D	TTW-m	MT20	4.0	4.0			Edge
E	TMWW-t	MT20	4.0	4.0			
F	TMV+p	MT20	3.0	4.0			
G	BMWW1-t	MT20	4.0	4.0			
H	BS-t	MT20	3.0	6.0			
I	BMWWWW-t	MT20	4.0	9.0			
J	BMWW1-t	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT UPLIFT	REQD BRG IN-SX
JT	VERT	DOWN	UP	
G	1097	0	0	1-8
J	1216	0	0	5-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0
J	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G, C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	CHORDS	WEBS
	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)
	FROM	TO
FR-TO		
A-B	0 / 42	10.00
B-C	0 / 31	10.00
C-D	-753 / 0	8.25
D-E	-512 / 0	6.25
E-F	0 / 0	10.00
G-F	-146 / 0	6.25
J-B	-256 / 0	7.81
J-I	0 / 614	10.00
I-H	0 / 365	10.00
H-G	0 / 365	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/693 (0.31")

CSI: TC=0.30/1.00 (D-E-1), BC=0.74/1.00 (I-J-2), WB=0.62/1.00 (E-G-1), SS=0.22/1.00 (G-I-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

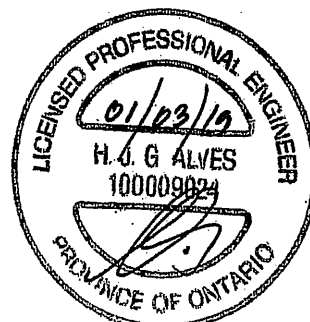
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)



DRWG NO. TAM T1900046
STRUCTURAL
COMPONENT ONLY

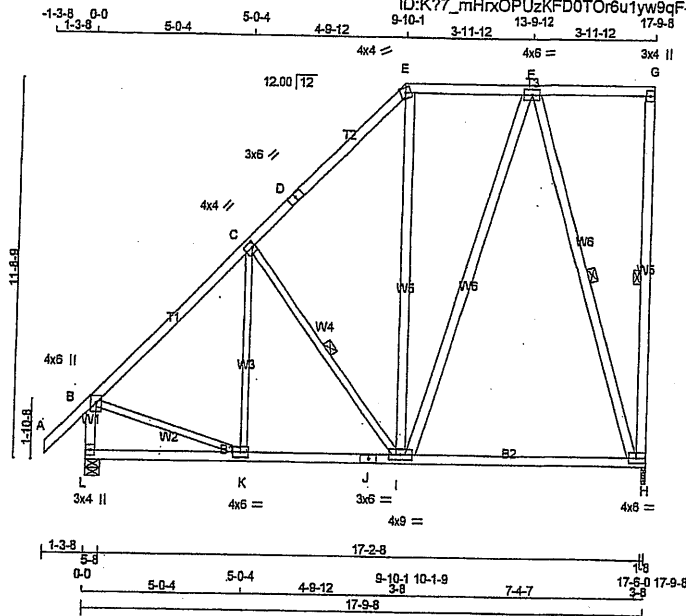
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:67.7



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS

EXCEPT	SIZE	DRY	LUMBER
K - C	2x3	DRY	No.2
C - I	2x3	DRY	No.2
B - K	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	6.0	2.75	2.00
C	TMW+t	MT20	4.0	4.0	2.00	1.00
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMW+t	MT20	4.0	6.0		
G	TMW+p	MT20	3.0	4.0		
H	BMW+1+t	MT20	4.0	6.0		
I	BMW+t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMW+t	MT20	4.0	6.0		
L	BMV+1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	1097	0	1097	0
L	1216	0	1216	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
H	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD
L	832	414 / 0	187 / 0	0 / 0	231 / 0	0 / 0
L	914	479 / 0	187 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	K-C	-24 / 122 0.03 (3)
B-C	-887 / 0	-84.9	-84.9	0.29 (1)	6.18	C-I	-356 / 0 0.16 (1)
C-D	-857 / 0	-84.9	-84.9	0.27 (1)	6.25	I-E	0 / 195 0.03 (3)
D-E	-857 / 0	-84.9	-84.9	0.27 (1)	6.25	B-K	0 / 681 0.15 (1)
E-F	-439 / 0	-84.9	-84.9	0.17 (1)	6.25	I-F	0 / 457 0.07 (2)
F-G	0 / 0	-84.9	-84.9	0.22 (1)	10.00	F-H	-896 / 0 0.76 (1)
H-G	-128 / 0	0.0	0.0	0.09 (1)	6.25		
L-B	-1131 / 0	0.0	0.0	0.13 (1)	7.46		
L-K	0 / 0	-38.5	-38.5	0.17 (3)	10.00		
K-J	0 / 652	-38.5	-38.5	0.46 (2)	10.00		
J-I	0 / 652	-38.5	-38.5	0.46 (2)	10.00		
I-H	0 / 295	-38.5	-38.5	0.41 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/896 (0.24")

CSI: TC=0.29/1.00 (B-C:1), BC=0.46/1.00 (I-K:2), WB=0.76/1.00 (F-H:1), SSI=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

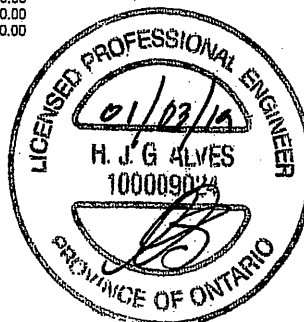
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (J) (INPUT = 0.90)
JSI METAL= 0.52 (J) (INPUT = 1.00)



DWG NO. TAM T1900047
STRUCTURAL
COMPONENT ONLY

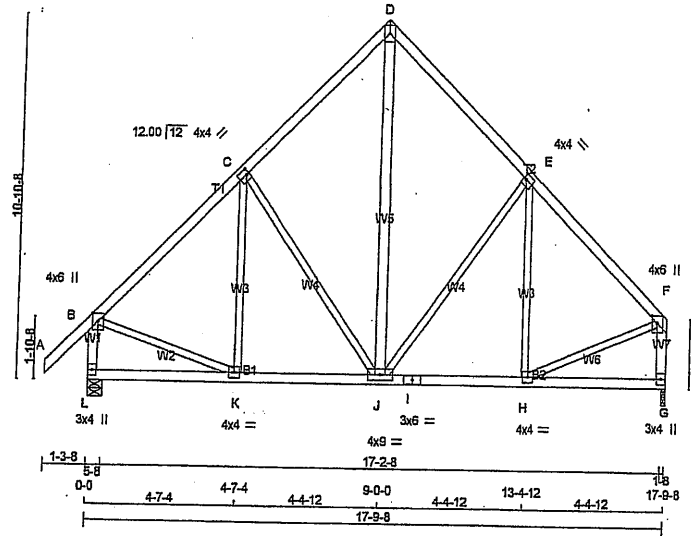
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	T10A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHrxOPUzKFD0TOr6u1yw9qF-vj3ZU4rJZ8qrvv265HtO58adoVC9cyZp18mh1zz5OW
 1-3-8 0-0 4-7-4 4-7-4 4-4-12 9-0-0 4-4-12 13-4-12 4-4-12 17-9-8
 4x6 II

Scale = 1:65.7



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
M - N	2x4	DRY	No.2	SPF
O - P	2x4	DRY	No.2	SPF
Q - R	2x4	DRY	No.2	SPF
S - T	2x4	DRY	No.2	SPF
U - V	2x4	DRY	No.2	SPF
W - X	2x4	DRY	No.2	SPF
Y - Z	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	9.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	1216	0	5-8	5-8
G	1097	0	1-8	1-8

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
L	914	479 / 0	187 / 0	0 / 0	248 / 0
G	832	414 / 0	187 / 0	0 / 0	231 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.22 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	CS1 (LC)
FR-TO								FR-TO				
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00			K-C	-22 / 138	0.03 (3)		
B-C	-698 / 0	-84.9	-84.9	0.23 (1)	6.22			C-J	-323 / 0	0.37 (1)		
C-D	-696 / 0	-84.9	-84.9	0.23 (1)	6.25			J-D	0 / 656	0.11 (1)		
D-E	-695 / 0	-84.9	-84.9	0.21 (1)	6.25			J-E	-288 / 0	0.33 (1)		
E-F	-869 / 0	-84.9	-84.9	0.22 (1)	6.25			H-E	-61 / 114	0.04 (1)		
L-B	-1144 / 0	0.0	0.0	0.13 (1)	7.43			B-K	0 / 692	0.16 (1)		
G-F	-1029 / 0	0.0	0.0	0.12 (1)	7.74			H-F	0 / 684	0.15 (1)		
L-K	0 / 0	-38.5	-38.5	0.15 (3)	10.00							
K-J	0 / 657	-38.5	-38.5	0.23 (2)	10.00							
J-I	0 / 637	-38.5	-38.5	0.22 (2)	10.00							
I-H	0 / 637	-38.5	-38.5	0.22 (2)	10.00							
H-G	0 / 0	-38.5	-38.5	0.14 (3)	10.00							

TOTAL WEIGHT = 3 X 95 = 286 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.59")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.23/1.00 (B-C:1), BC=0.23/1.00 (J-K:2), WB=0.37/1.00 (C-J:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

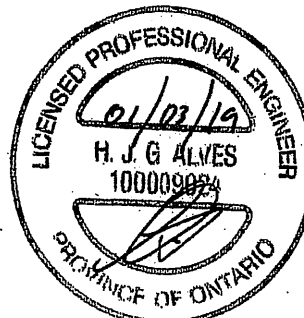
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
 JSI METAL= 0.42 (B) (INPUT = 1.00)



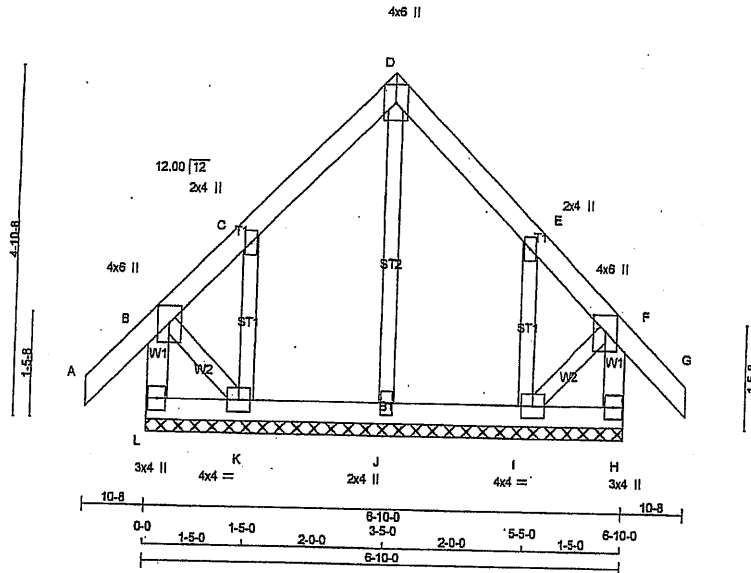
DRWG NO. TAM T19 000499
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300536	G12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHrxOPUzKFDOTOr6u1yw9qF-KwvFX_7A7qMxx2RDIfgV8E_Zs7NyGB30zs4qsRzz5Mf

Scale = 1:30.6



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0		
E	TMVW+p	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMW1+w	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMW1+w	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/29	-84.9 -84.9	0.06 (1)	10.00	J-D	-118/0	0.04 (1)
B-C	-40/0	-84.9 -84.9	0.06 (1)	6.25	K-C	-142/0	0.02 (1)
C-D	-33/0	-84.9 -84.9	0.05 (1)	6.25	I-E	-142/0	0.02 (1)
D-E	-33/0	-84.9 -84.9	0.05 (1)	6.25	B-K	0/29	0.01 (1)
E-F	-40/0	-84.9 -84.9	0.06 (1)	6.25	I-F	0/29	0.01 (1)
F-G	0/29	-84.9 -84.9	0.06 (1)	10.00			
L-B	-190/0	0.0 0.0	0.02 (1)	7.81			
H-F	-190/0	0.0 0.0	0.02 (1)	7.81			
L-K	0/0	-38.5 -38.5	0.02 (3)	10.00			
K-J	0/17	-38.5 -38.5	0.03 (3)	10.00			
J-I	0/17	-38.5 -38.5	0.03 (3)	10.00			
I-H	0/0	-38.5 -38.5	0.02 (3)	10.00			

TOTAL WEIGHT = 34 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.8	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B:1), BC=0.03/1.00 (I-J:3), WB=0.04/1.00 (D-J:1), SSI=0.06/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

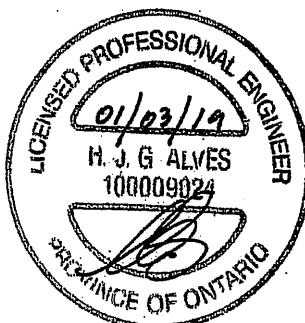
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1655

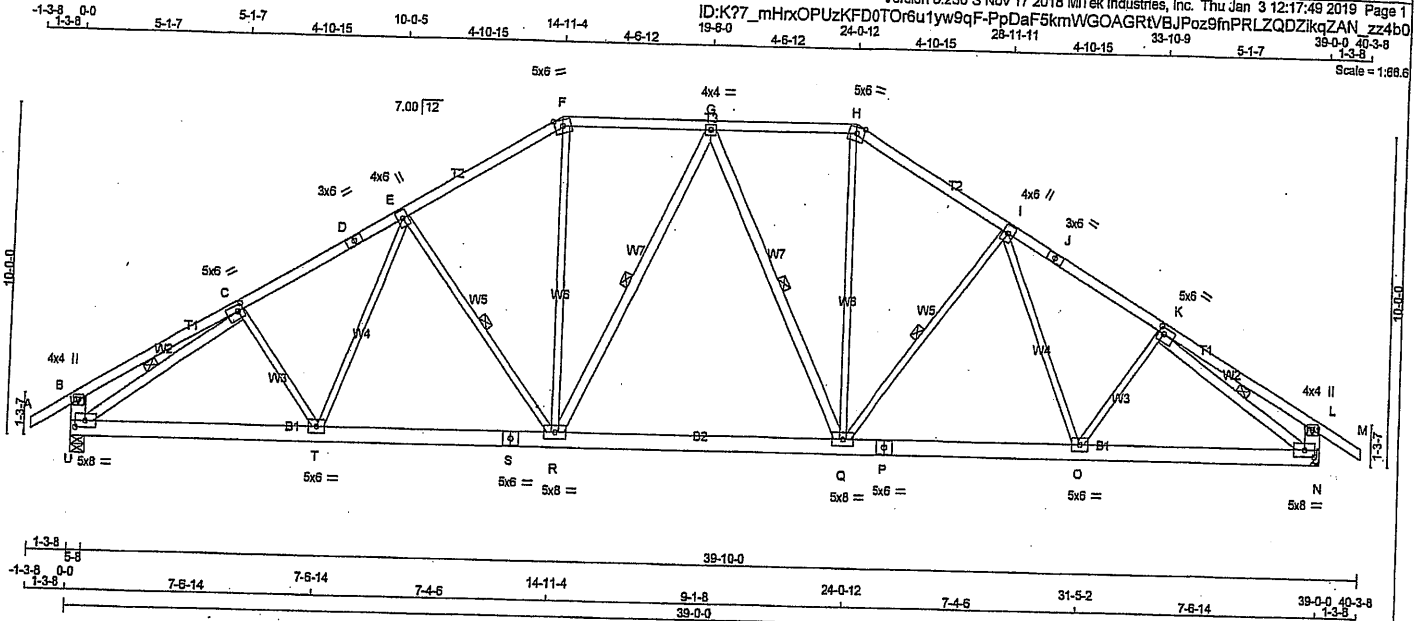
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (F) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)



DWG NO. TAM T1900035
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - M	2x4	DRY	No.2
U - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
U - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
R - G	2x4	DRY	No.2
G - Q	2x4	DRY	No.2
U - C	2x4	DRY	No.2
K - N	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.25	2.00
D	TS-t	MT20	3.0	6.0		
E	TMVW-t	MT20	4.0	6.0		
F	TTW-m	MT20	5.0	6.0	Edge	
G	TMVW-t	MT20	4.0	4.0		
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMVW-t	MT20	4.0	6.0		
J	TS-t	MT20	3.0	6.0		
K	TMVW-t	MT20	5.0	6.0	2.25	2.00
L	TMV+p	MT20	4.0	4.0		
N	BMVW-t	MT20	5.0	8.0	2.50	3.75
O	BMVW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	8.0		
R	BMVW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMVW-t	MT20	5.0	6.0		
U	BMVW-t	MT20	5.0	8.0	2.50	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT VERT 2521	DOWN 2521	0	5-8
U 2521	HORIZ 0	0	5-8
N 2521	UP/LIFT 0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM. LIVE WIND DEAD SOIL
U 1905	972/0	410/0	0/0 0/0 523/0 0/0
N 1905	972/0	410/0	0/0 0/0 523/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, G-R, G-Q, I-Q, C-U, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/29	-84.9 -84.9	0.11 (1)	10.00	C-T	0/147	0.03 (3)
B-C	0/19	-84.9 -84.9	0.26 (1)	10.00	T-E	0/330	0.07 (2)
C-D	-3216/0	-84.9 -84.9	0.39 (1)	3.64	E-R	-598/0	0.28 (1)
D-E	-3216/0	-84.9 -84.9	0.39 (1)	3.64	R-F	0/1036	0.23 (1)
E-F	-2711/0	-84.9 -84.9	0.35 (1)	3.94	G-Q	-255/0	0.15 (1)
F-G	-2333/0	-84.9 -84.9	0.28 (1)	4.25	Q-H	0/1036	0.15 (1)
G-H	-2333/0	-84.9 -84.9	0.28 (1)	4.25	H-I	-598/0	0.23 (1)
H-I	-2711/0	-84.9 -84.9	0.35 (1)	3.94	I-O	0/330	0.07 (2)
I-J	-3216/0	-84.9 -84.9	0.39 (1)	3.64	O-K	0/147	0.03 (3)
J-K	-3216/0	-84.9 -84.9	0.39 (1)	3.64	K-N	-3472/0	0.64 (1)
K-L	0/19	-84.9 -84.9	0.26 (1)	10.00			
L-M	0/29	-84.9 -84.9	0.11 (1)	10.00			
U-B	-290/0	0.0 0.0	0.02 (1)	7.81			
N-L	-290/0	0.0 0.0	0.02 (1)	7.81			
U-T	0/2763	-38.5 -38.5	0.47 (1)	10.00			
T-S	0/2681	-38.5 -38.5	0.47 (2)	10.00			
S-R	0/2681	-38.5 -38.5	0.47 (2)	10.00			
R-Q	0/2440	-38.5 -38.5	0.44 (2)	10.00			
Q-P	0/2681	-38.5 -38.5	0.47 (2)	10.00			
P-O	0/2681	-38.5 -38.5	0.47 (2)	10.00			
O-N	0/2763	-38.5 -38.5	0.47 (1)	10.00			

TOTAL WEIGHT = 6 X 209 = 1252 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.39/1.00 (I-K:1), BC=0.47/1.00 (O-Q:2), WB=0.64/1.00 (K-N:1), SSI=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

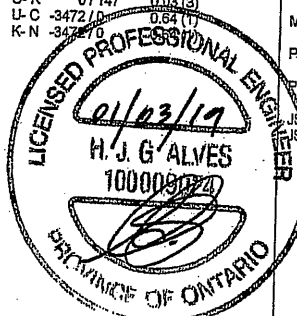
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JS GRIP= 0.90 (K) (INPUT = 0.90)
JS METAL= 0.86 (C) (INPUT = 1.00)



DWG NO. TAM T19 000 66
STRUCTURAL
COLLAPSE PREVENTION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

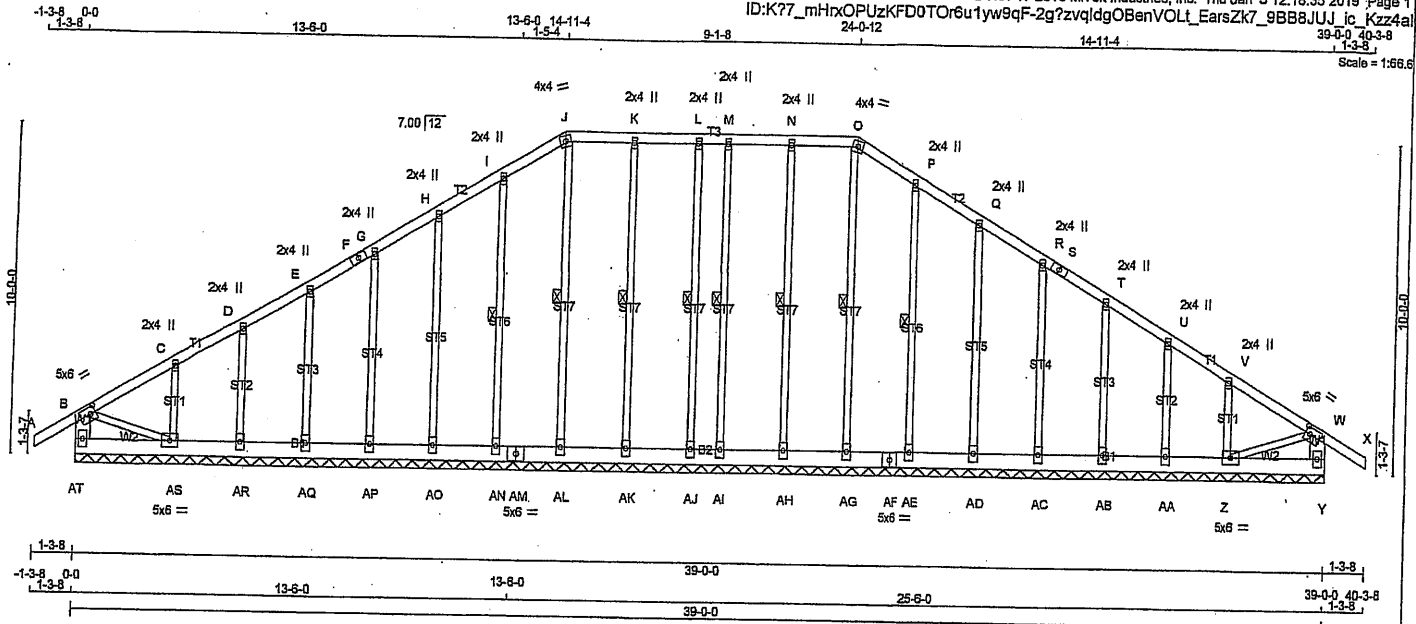
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Thu Jan 3 12:18:35 2019 Page 1

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39-0-0 40-3-8

1-3-8

Scale = 1:66.6



TOTAL WEIGHT = 2 X 227 = 454 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - F	2x4	DRY No.2	SPF
F - J	2x4	DRY No.2	SPF
J - O	2x4	DRY No.2	SPF
O - S	2x4	DRY No.2	SPF
S - X	2x4	DRY No.2	SPF
AT - B	2x6	DRY No.2	SPF
Y - W	2x6	DRY No.2	SPF
AT - AM	2x6	DRY No.2	SPF
AM - AF	2x6	DRY No.2	SPF
AF - Y	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	6.0	2.50	2.00
C D, E, G, H, I, K, L, M, N, P, Q, R, T, U, V	MT20	2.0	4.0		
F TS-t	MT20	3.0	6.0		
J TTW-m	MT20	4.0	4.0		
O TS-t	MT20	3.0	6.0		
W TMVW-t	MT20	5.0	6.0	2.50	2.00
Y BMVW-t	MT20	3.0	6.0		
Z BMVW-t	MT20	5.0	6.0		
AA, AB, AC, AD, AE, AG, AH, AI, AJ, AK, AL, AN, AO, AP, AQ, AR					
AA BMVW-t	MT20	3.0	6.0		
AF BS-t	MT20	5.0	6.0		
AM BS-t	MT20	5.0	6.0		
AS BMVW-t	MT20	5.0	6.0		
AT BMVW-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-AJ, K-AK, J-AL, I-AN, N-AH, M-AI, O-AG, P-AE.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CS1 (LC)	MAX UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/29	-84.9 -84.9	0.11 (1)	10.00	AJ-L	-117/0	0.08 (1)
B-C	-26/0	-84.9 -84.9	0.09 (1)	6.25	AK-K	-198/0	0.13 (1)
C-D	-41/0	-84.9 -84.9	0.09 (1)	6.25	AL-J	-131/0	0.09 (1)
D-E	-23/0	-84.9 -84.9	0.04 (1)	6.25	AN-I	-166/0	0.09 (1)
E-F	-24/0	-84.9 -84.9	0.04 (1)	6.25	AO-H	-166/0	0.17 (1)
F-G	-24/0	-84.9 -84.9	0.04 (1)	6.25	AP-G	-168/0	0.11 (1)
G-H	-21/0	-84.9 -84.9	0.04 (1)	6.25	AQ-E	-174/0	0.07 (1)
H-I	-18/0	-84.9 -84.9	0.04 (1)	6.25	AR-D	-139/0	0.04 (1)
I-J	-25/0	-84.9 -84.9	0.04 (1)	6.25	AS-C	-254/0	0.04 (1)
J-K	-13/0	-84.9 -84.9	0.05 (1)	6.25	AH-N	-198/0	0.13 (1)
K-L	-13/0	-84.9 -84.9	0.05 (1)	6.25	AI-M	-117/0	0.08 (1)
L-M	-13/0	-84.9 -84.9	0.03 (1)	6.25	AG-O	-131/0	0.09 (1)
M-N	-13/0	-84.9 -84.9	0.05 (1)	6.25	AE-P	-166/0	0.09 (1)
N-O	-13/0	-84.9 -84.9	0.05 (1)	6.25	AD-Q	-166/0	0.17 (1)
O-P	-25/0	-84.9 -84.9	0.04 (1)	6.25	AC-R	-168/0	0.11 (1)
P-Q	-18/0	-84.9 -84.9	0.04 (1)	6.25	AB-T	-174/0	0.07 (1)
Q-R	-21/0	-84.9 -84.9	0.04 (1)	6.25	AA-U	-139/0	0.04 (1)
R-S	-24/0	-84.9 -84.9	0.04 (1)	6.25	Z-V	-254/0	0.04 (1)
S-T	-24/0	-84.9 -84.9	0.04 (1)	6.25	B-AS	0/34	0.01 (1)
T-U	-23/0	-84.9 -84.9	0.04 (1)	6.25	Z-W	0/34	0.01 (1)
U-V	-41/0	-84.9 -84.9	0.09 (1)	6.25			
V-W	-26/0	-84.9 -84.9	0.09 (1)	6.25			
W-X	0/29	-84.9 -84.9	0.11 (1)	10.00			
AT-B	-247/0	0.0	0.02 (1)	7.81			
Y-W	-247/0	0.0	0.02 (1)	7.81			
AT-AS	0/0	-38.5 -38.5	0.03 (3)	10.00			
AS-AR	0/25	-38.5 -38.5	0.03 (2)	10.00			
AR-AQ	0/22	-38.5 -38.5	0.01 (2)	10.00			
AQ-AP	0/20	-38.5 -38.5	0.01 (2)	10.00			
AP-AO	0/18	-38.5 -38.5	0.01 (2)	10.00			
AO-AN	0/16	-38.5 -38.5	0.01 (2)	10.00			
AN-AM	0/15	-38.5 -38.5	0.01 (2)	10.00			
AM-AL	0/15	-38.5 -38.5	0.01 (2)	10.00			
AL-AK	0/13	-38.5 -38.5	0.01 (2)	10.00			
AK-AJ	0/13	-38.5 -38.5	0.01 (2)	10.00			
AJ-AI	0/13	-38.5 -38.5	0.01 (2)	10.00			
AI-AH	0/13	-38.5 -38.5	0.01 (2)	10.00			
AH-AG	0/13	-38.5 -38.5	0.01 (2)	10.00			
AG-AF	0/15	-38.5 -38.5	0.01 (2)	10.00			
AF-AE	0/15	-38.5 -38.5	0.01 (2)	10.00			
AE-AD	0/16	-38.5 -38.5	0.01 (2)	10.00			
AD-AC	0/18	-38.5 -38.5	0.01 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.11/1.00 (A-B-1), BC=0.03/1.00 (Z-AA-2), WB=0.17/1.00 (Q-AD-1), SI=0.10/1.00 (V-W-1)

DOL LUMBER-1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

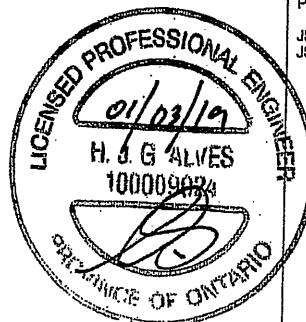
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)



DRWG NO. TAM T1900060
STRUCTURAL
COMPONENT ONLY 1/2

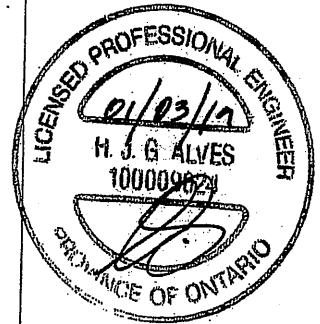
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	G13	2	1	TRUSS DESC.	
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Jan 3 12:18:35 2019 Page 2					
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LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED UNBRAC LENGTH	FR-TO MAX CSI (LC)
FR-TO		FROM TO					
AC-AB	0 / 20	-38.5 -38.5	0.01 (2)			10.00	
AB-AA	0 / 22	-38.5 -38.5	0.01 (2)			10.00	
AA-Z	0 / 25	-38.5 -38.5	0.03 (2)			10.00	
Z-Y	0 / 0	-38.5 -38.5	0.03 (3)			10.00	



DWG NO. TAM T1900060

STRUCTURAL

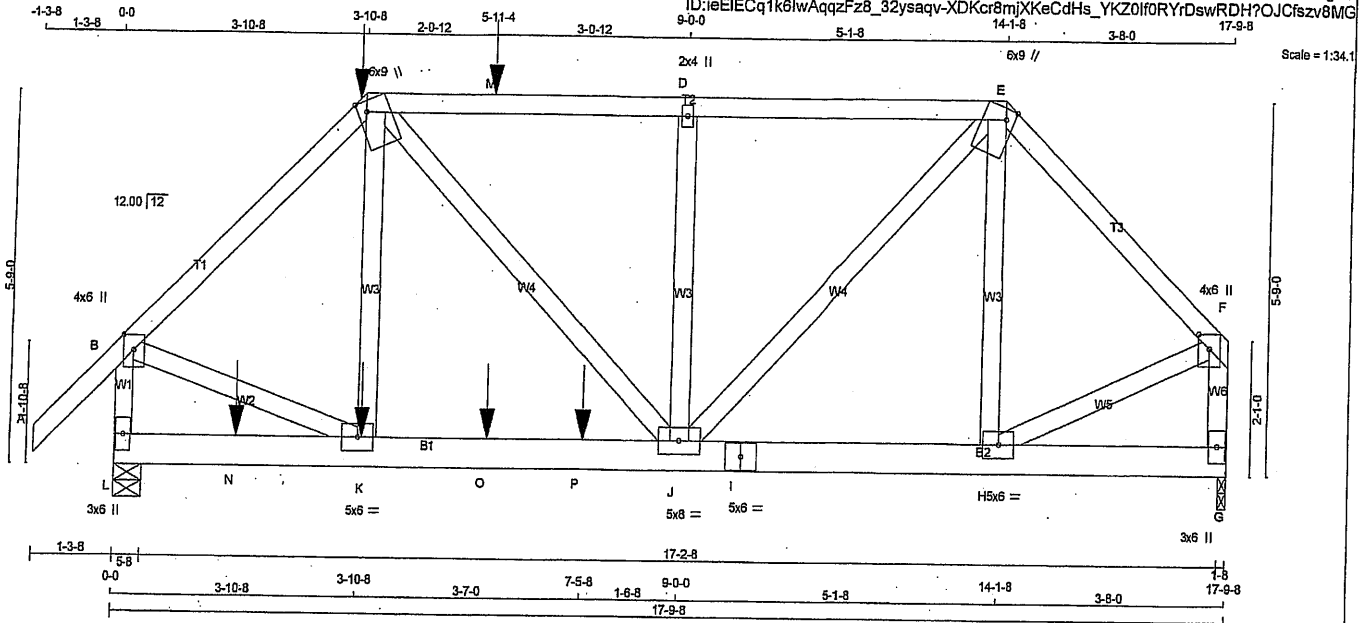
CONNECTIONS ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300783	T26A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:34.1

TOTAL WEIGHT = 2 X 103 = 206 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
L - I	2x6	DRY No.2	SPF
I - G	2x6	DRY No.2	SPF

ALL WEBS 2x4 DRY
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - E	12	SIDE(61.0)
E - F	12	TOP
L - B	12	TOP
G - F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L - I	12	SIDE(183.1)
I - G	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE
FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND
MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR
THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED
TO ONE SIDE THAT THE CORRESPONDING NAILING
PATTERN SHALL BE CAPABLE OF TRANSFERING.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE
SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	2.75	2.00
C TTWW+m	MT20	6.0	9.0	Edge	1.75
D TMVW+p	MT20	2.0	4.0		
E TTWW+m	MT20	6.0	9.0	Edge	1.75
F TMVW+p	MT20	4.0	6.0	2.75	2.00
G BMV1+p	MT20	3.0	6.0		
H BMVW-t	MT20	5.0	6.0		
I BS-t	MT20	5.0	6.0		
J BMVW-t	MT20	5.0	6.0		
K BMVW-t	MT20	5.0	6.0		
L BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP/LIFT
L	2130	0	2130	0
G	1615	0	1615	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1591	864/0	300/0	0/0	0/0	427/0	0/0
G	1214	636/0	248/0	0/0	0/0	331/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0/42	-84.9	-84.9	0.07 (1)	10.00	K-C	0/267	0.03 (3)
B-C	-1911/0	-84.9	-84.9	0.14 (1)	6.17	C-J	0/777	0.07 (1)
C-M	-1887/0	-84.9	-84.9	0.25 (1)	6.04	J-D	-560/0	0.10 (1)
M-D	-1887/0	-84.9	-84.9	0.25 (1)	6.04	J-E	0/1358	0.12 (1)
D-E	-1887/0	-84.9	-84.9	0.25 (1)	6.04	H-E	-341/0	0.06 (1)
E-F	-1359/0	-84.9	-84.9	0.12 (1)	6.25	B-K	0/1480	0.08 (1)
L-B	-2121/0	0.0	0.0	0.12 (1)	7.61	H-F	-341/0	0.06 (1)
G-F	-1535/0	0.0	0.0	0.09 (1)	7.81			
L-N	0/0	-38.5	-38.5	0.13 (1)	10.00			
N-K	0/0	-38.5	-38.5	0.13 (1)	10.00			
K-O	0/1350	-38.5	-38.5	0.37 (1)	10.00			
O-P	0/1350	-38.5	-38.5	0.37 (1)	10.00			
P-J	0/1350	-38.5	-38.5	0.37 (1)	10.00			
J-I	0/948	-38.5	-38.5	0.22 (1)	10.00			
I-H	0/948	-38.5	-38.5	0.22 (1)	10.00			
H-G	0/0	-38.5	-38.5	0.03 (2)	10.00			

9/15/19

H. J. G. ALVES

100009024

LICENSED PROFESSIONAL ENGINEER

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	CON.
C	3-10-8	-271	-271	—	FRONT	VERT	TOTAL	—
K	3-11-4	-36	-46	—	FRONT	VERT	TOTAL	—
M	5-11-4	-80	-80	—	FRONT	VERT	TOTAL	—
N	1-11-4	-36	-46	—	FRONT	VERT	TOTAL	—
O	5-11-4	-36	-46	—	FRONT	VERT	TOTAL	—
P	7-5-8	-972	-972	—	TOP	VERT	TOTAL	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.25/1.00 (C-D:1), BC=0.37/1.00 (J-K:1),
WB=0.13/1.00 (B-K:1), SSI=0.26/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

WAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

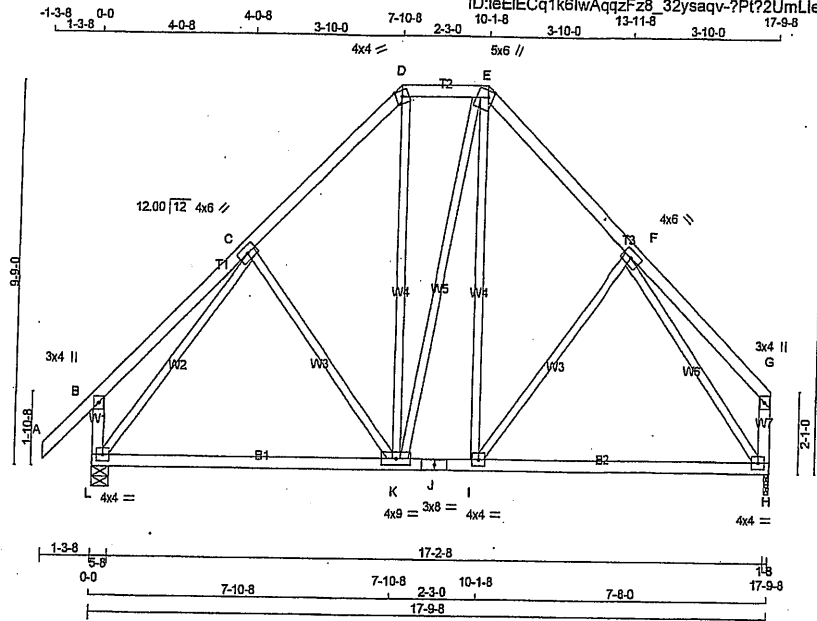
DRWG NO. TAM 1720/1093
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300783	T28A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:56.1

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4 DRY	No.2
D - E	2x4 DRY	No.2
E - G	2x4 DRY	No.2
L - B	2x4 DRY	No.2
H - G	2x4 DRY	No.2
L - J	2x4 DRY	No.2
J - H	2x4 DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	4.0	6.0		
D TTW-m	MT20	4.0	4.0	Edge	
E TTWW+m	MT20	5.0	6.0	2.00	1.50
F TMWW-t	MT20	4.0	6.0		
G TMV+p	MT20	3.0	4.0		
H BMVV1-t	MT20	4.0	4.0		
I BMWW-t	MT20	4.0	4.0		
J BS-t	MT20	3.0	8.0		
K BMWW-t	MT20	4.0	9.0		
L BMVV1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1216	0	1216	0
H	1097	0	1097	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	914	479 / 0	187 / 0	0 / 0	0 / 0	248 / 0	0 / 0	0 / 0
H	832	414 / 0	187 / 0	0 / 0	0 / 0	231 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-84.9 -84.9 0.12 (1)	10.00	C-K	-127 / 25	0.11 (1)	
B-C	0 / 28	-84.9 -84.9 0.21 (1)	10.00	K-D	0 / 325	0.07 (2)	
C-D	-782 / 0	-84.9 -84.9 0.17 (1)	6.25	K-E	0 / 26	0.01 (2)	
D-E	-535 / 0	-84.9 -84.9 0.06 (1)	6.25	E-F	0 / 292	0.07 (2)	
E-F	-773 / 0	-84.9 -84.9 0.16 (1)	6.25	F-G	-94 / 38	0.08 (1)	
F-G	0 / 28	-84.9 -84.9 0.20 (1)	10.00	L-C	-1042 / 0	0.84 (1)	
L-B	-244 / 0	0.0 0.0 0.03 (1)	7.81	F-H	-1036 / 0	0.80 (1)	
H-G	-117 / 0	0.0 0.0 0.01 (1)	7.81				
L-K	0 / 608	-38.5 -38.5 0.48 (2)	10.00				
K-J	0 / 529	-38.5 -38.5 0.47 (2)	10.00				
J-I	0 / 529	-38.5 -38.5 0.47 (2)	10.00				
I-H	0 / 583	-38.5 -38.5 0.43 (2)	10.00				

TOTAL WEIGHT = 98 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/836 (0.26")

CSI: TC=0.21/1.00 (B-C:1), BC=0.48/1.00 (K-L:2), WB=0.84/1.00 (C-L:1), SSI=0.19/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

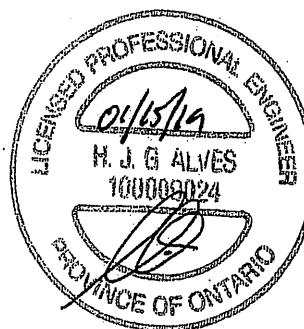
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618 354 1667 788 1987 1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.80 (J) (INPUT = 1.00)



DWG NO. TAM 190 1095
STRUCTURAL
COMPUTER FILE

Tamarack Roof Truss, Burlington

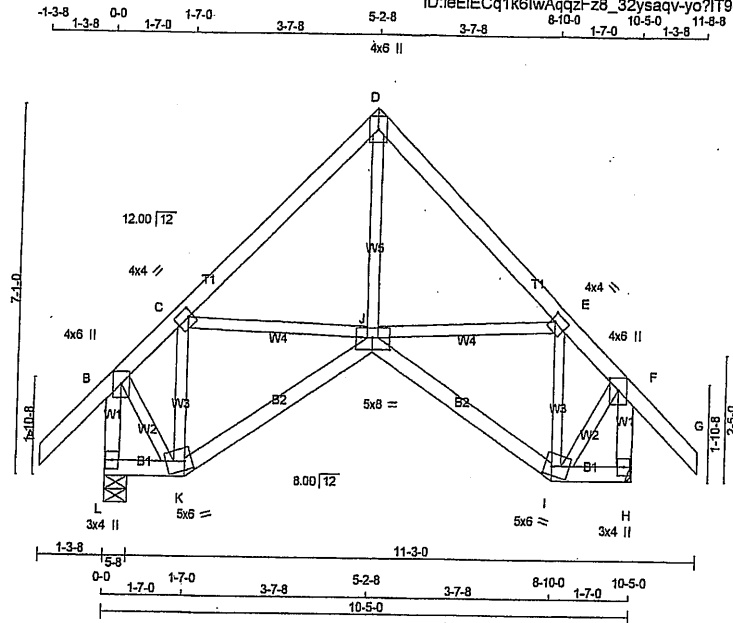


DWG NO. TAM 172.001096
STRUCTURAL
CONCRETE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300783	T30	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:42.3

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBVW-m	MT20	5.0	6.0	2.00	1.75
J	BBVW-p	MT20	5.0	8.0	2.75	4.00
K	BBVW-m	MT20	5.0	6.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
L	761	0	761	0	0
H	761	0	761	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	569	308 / 0	109 / 0	0 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	569	308 / 0	109 / 0	0 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		FACTORED		MEMB.		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	UNBRAC
FR-TO						FR-TO					
A-B	0 / 42	-84.9	-84.9	0.12 (1)	10.00	J-D	0 / 473	0.11 (2)			
B-C	-401 / 0	-84.9	-84.9	0.11 (1)	6.25	J-E	0 / 107	0.02 (1)			
C-D	-543 / 0	-84.9	-84.9	0.15 (1)	6.25	I-E	-354 / 0	0.07 (1)			
D-E	-543 / 0	-84.9	-84.9	0.15 (1)	6.25	C-J	0 / 107	0.02 (1)			
E-F	-401 / 0	-84.9	-84.9	0.11 (1)	6.25	K-C	-354 / 0	0.07 (1)			
F-G	0 / 42	-84.9	-84.9	0.12 (1)	10.00	B-K	0 / 401	0.09 (1)			
L-B	-730 / 0	0.0	0.0	0.08 (1)	7.81	I-F	0 / 401	0.09 (1)			
H-F	-730 / 0	0.0	0.0	0.08 (1)	7.81						
L-K	0 / 0	-38.5	-38.5	0.02 (3)	10.00						
K-J	0 / 319	-38.5	-38.5	0.16 (2)	10.00						
J-I	0 / 319	-38.5	-38.5	0.16 (2)	10.00						
I-H	0 / 0	-38.5	-38.5	0.02 (3)	10.00						

TOTAL WEIGHT = 2 X 55 = 111 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.15/1.00 (C-D:1), BC=0.16/1.00 (I-J:2), WB=0.11/1.00 (D-J:2), SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10, COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

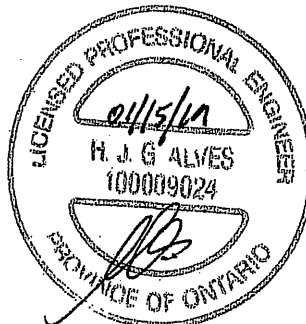
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.67 (K) (INPUT = 0.90)
JSI METAL= 0.20 (F) (INPUT = 1.00)

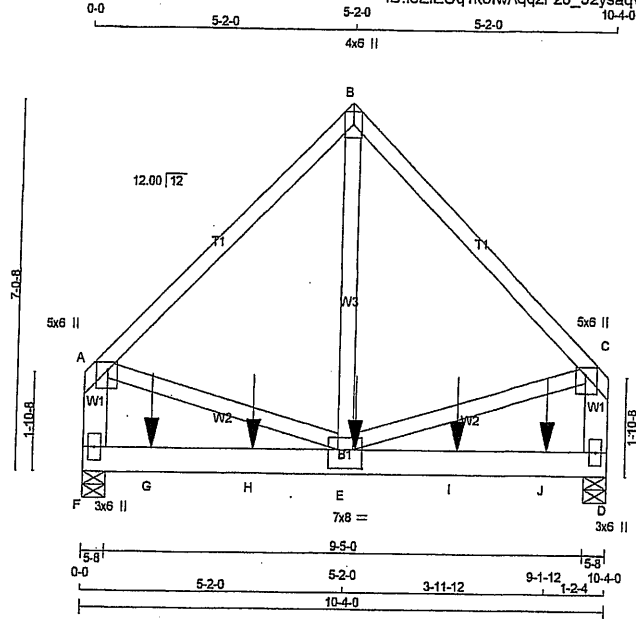


DWG NO. TAMT1901097
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300785	T31	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:42.1

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B 1	12	TOP
B - C 1	12	TOP
F - A 2	12	TOP
D - C 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
E - B 1	6	SIDE(124.0)
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75 2.75
B	TTW+p	MT20	4.0	6.0	
C	TMVW+p	MT20	5.0	6.0	1.75 2.75
D	BMV1+p	MT20	3.0	6.0	
E	BMVWW+1	MT20	7.0	8.0	4.25 4.00
F	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	3519	0	3519	0	5-8	5-8
D	3642	0	3642	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
F	2635	1414 / 0	514 / 0	0 / 0	0 / 0	707 / 0	0 / 0
D	2727	1464 / 0	532 / 0	0 / 0	0 / 0	732 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		WEBS	
	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED MEMB. FORCE (LBS)
A-B	-2516 / 0	-84.9	0.29 (1)	0 / 3120
B-C	-2516 / 0	-84.9	0.29 (1)	0 / 1847
F-A	-2494 / 0	0.0	0.10 (1)	0 / 1847
D-C	-2494 / 0	0.0	0.10 (1)	
F-G	0 / 0	-38.5	0.68 (1)	10.00
G-H	0 / 0	-38.5	0.68 (1)	10.00
H-E	0 / 0	-38.5	0.68 (1)	10.00
E-I	0 / 0	-38.5	0.68 (1)	10.00
I-J	0 / 0	-38.5	0.68 (1)	10.00
J-D	0 / 0	-38.5	0.68 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	5-4-4	-1177	-1177	—	FRONT	VERT	TOTAL	—	—
G	1-4-4	-1177	-1177	—	FRONT	VERT	TOTAL	—	—
H	3-4-4	-1177	-1177	—	FRONT	VERT	TOTAL	—	—
I	7-4-4	-1177	-1177	—	FRONT	VERT	TOTAL	—	—
J	9-1-12	-1177	-1177	—	FRONT	VERT	TOTAL	—	—

TOTAL WEIGHT = 2 X 59 = 118 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.29/1.00 (B-C:1), BC=0.68/1.00 (E-F:1), WB=0.28/1.00 (B-E:1), SSI=0.51/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

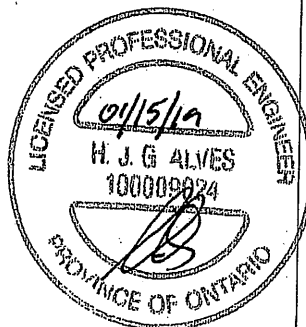
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (B) (INPUT = 0.90)
JSI METAL= 0.37 (A) (INPUT = 1.00)

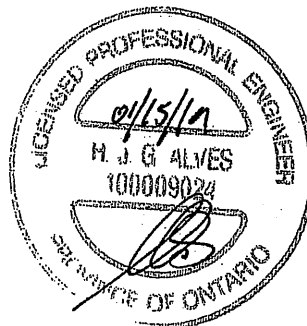


DWG NO. TAM 71901098
STRUCTURAL
COMPONENT ONLY

11amarack Roof Truss, Burlington



DWG NO. TAM T190109.9
STRUCTURAL
COMPONENT ONLY

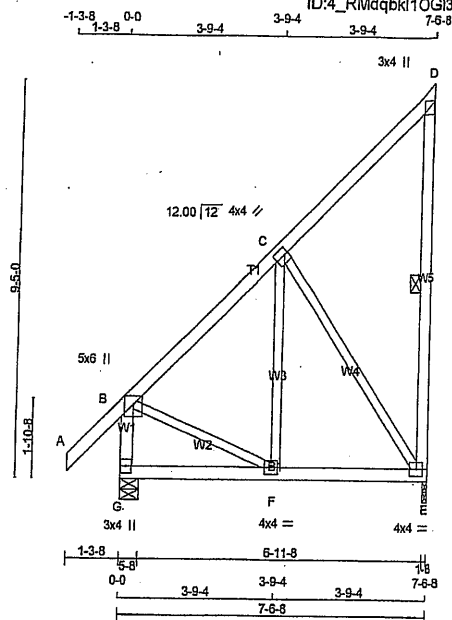


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	T140	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:52.5

TOTAL WEIGHT = 2 X 47 = 94 lb

MITF

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TMV+p	MT20	3.0	4.0		
E	BMVW-t	MT20	4.0	4.0		
F	BMVW-t	MT20	4.0	4.0		
G	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REGRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	465	0	465	0	0	1-8	1-8
G	584	0	584	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	353	176 / 0	79 / 0	0 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
G	435	241 / 0	79 / 0	0 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-84.9 -84.9	0.12 (1)	10.00	F-C	0 / 186	0.04 (3)
B-C	-263 / 0	-84.9 -84.9	0.16 (1)	6.25	C-E	-354 / 0	0.26 (1)
C-D	-28 / 0	-84.9 -84.9	0.16 (1)	6.25	B-F	0 / 221	0.05 (1)
E-D	-121 / 0	0.0 0.0	0.05 (1)	6.25			
G-B	-527 / 0	0.0 0.0	0.06 (1)	7.81			
G-F	0 / 0	-38.5 -38.5	0.12 (3)	10.00			
F-E	0 / 205	-38.5 -38.5	0.14 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015.

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.16/1.00 (B-C:1), BC=0.14/1.00 (E-F:2), WB=0.26/1.00 (C-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE-LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

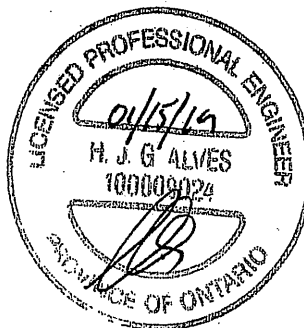
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

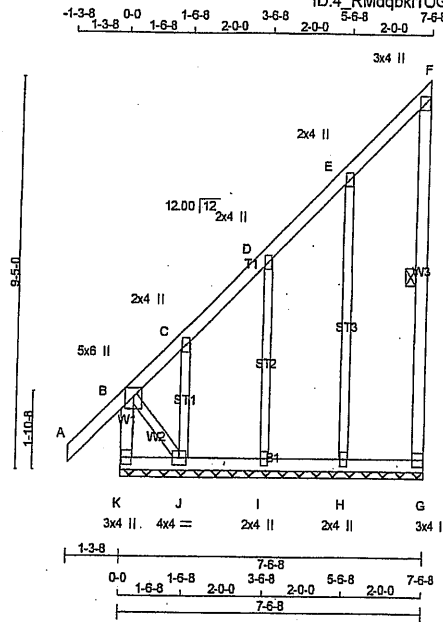


DWG NO. TAM 17901091
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	G140	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:4_RMDqbK1OGI3fXnfrj7uyvEbu-2kHHUe0510vpXirzDL0ePiMjP?XAc9jwZfGCgzv8pd



Scale = 1:53.2

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT	2-0-0	OC.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C, D, E						
C	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMW1+w	MT20	2.0	4.0		
J	BMWW1-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/42	-84.9 -84.9	0.12 (1)	10.00	H-E	-189/0	0.18 (1)
B-C	-49/0	-84.9 -84.9	0.11 (1)	6.25	I-D	-175/0	0.08 (1)
C-D	-2/0	-84.9 -84.9	0.04 (1)	10.00	J-C	-82/0	0.02 (1)
D-E	-3/0	-84.9 -84.9	0.05 (1)	10.00	B-J	0/13	0.00 (1)
E-F	-11/0	-84.9 -84.9	0.05 (1)	6.25			
G-F	-73/0	0.0 0.0	0.03 (1)	6.25			
K-B	-249/0	0.0 0.0	0.03 (1)	7.81			
K-J	0/0	-38.5 -38.5	0.02 (3)	10.00			
J-I	0/6	-38.5 -38.5	0.02 (3)	10.00			
I-H	0/3	-38.5 -38.5	0.03 (3)	10.00			
H-G	0/0	-38.5 -38.5	0.03 (3)	10.00			

TOTAL WEIGHT = 2 X 48 = 96 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B:1), BC=0.03/1.00 (H-I:3), WB=0.18/1.00 (E-H:1), SS=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

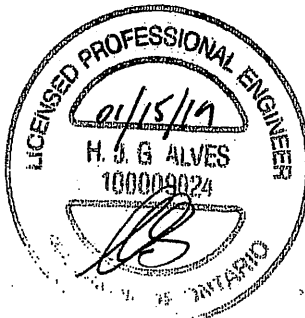
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

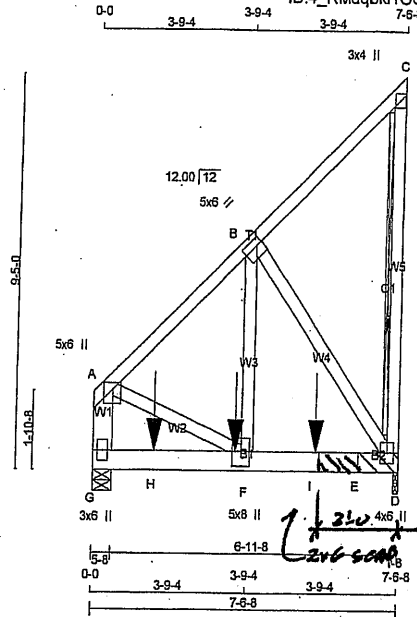


DWG NO. TAM 11901092
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300526	T141	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:4_RMdbk10G13fXnfrJ7uyvEbu-76P1vK2LZd9XxsE5dOsqnhICV0eiQ0Nt8NH9zv8pb



Scale = 1:52.8

TOTAL WEIGHT = 4 X 57 = 228 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
G - A	2x6	DRY	No.2	SPF	
G - D	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - D 1	12	TOP
G - A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - D 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
B - F 1	6	SIDE(464.6)
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TMVW-t	MT20	5.0	6.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
F	BMVW-t	MT20	5.0	8.0	4.25	2.50
G	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
D	3997	0	3997	0
G	4382	0	4382	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
JT	COMBINED						
D	3013	1558 / 0	632 / 0	0 / 0	0 / 0	823 / 0	0 / 0
G	3302	1708 / 0	692 / 0	0 / 0	0 / 0	902 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G

2x6 DRY SPF No.2 BEARING BLOCK 24" LONG AT JT. D ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 24" NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)
FR-TO				
A-B	-3103 / 0	-84.9	-84.9 0.15 (1)	5.13
B-C	-24 / 0	-84.9	-84.9 0.10 (1)	6.25
D-C	-125 / 0	0.0	0.0 0.04 (1)	7.81
G-A	-3181 / 0	0.0	0.0 0.12 (1)	7.75
G-H	0 / 0	-38.5	-38.5 0.84 (1)	10.00
H-F	0 / 0	-38.5	-38.5 0.84 (1)	10.00
F-I	0 / 2212	-38.5	-38.5 0.73 (1)	10.00
I-E	0 / 2212	-38.5	-38.5 0.73 (1)	10.00
E-D	0 / 2212	-38.5	-38.5 0.73 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	3-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
H	1-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—
I	5-5-12	-2483	-2483	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.15/1.00 (A-B:1), BC=0.73/1.00 (D-F:1), WB=0.97/1.00 (B-D:1), SSI=0.60/1.00 (D-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

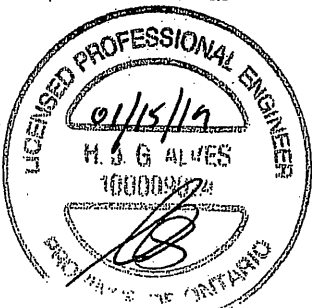
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)

JSI METAL= 0.53 (F) (INPUT = 1.00)



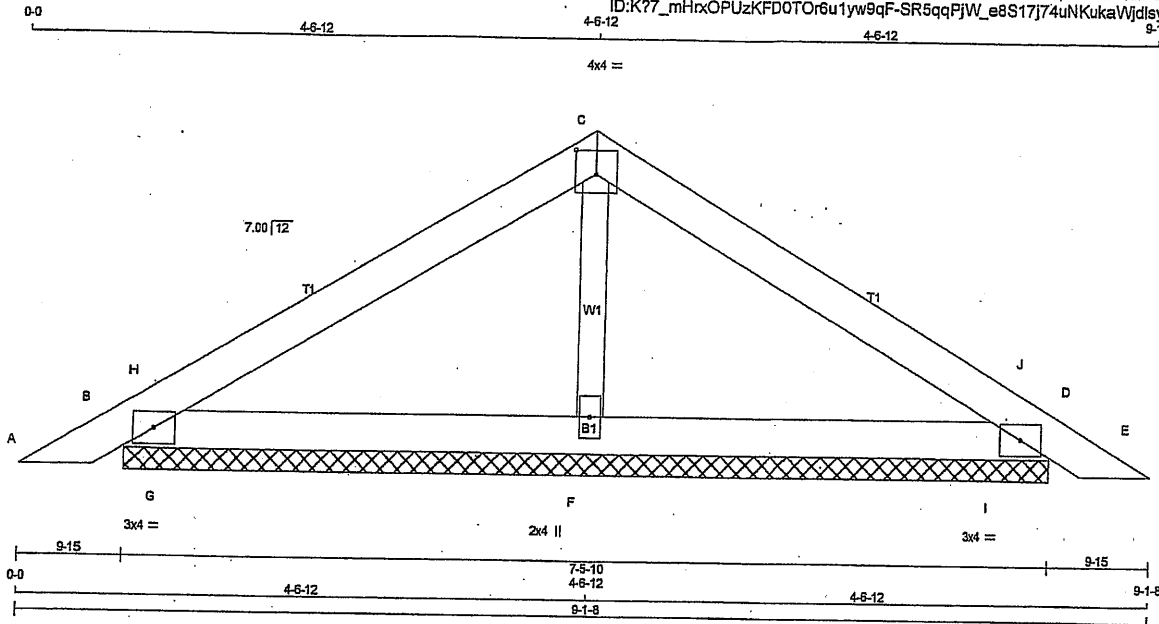
DRWG NO. TAM1901090
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	P1	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHxOPUzKFD0r6u1yw9qF-SR5qqPJW_e8S17j74uNKukaVjdlsyTmQHW44J6zz4b2



Scale = 1:17.2

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+V	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	JT VERT	IN-SX	IN-SX	
B	322	0	0	7-5-10
D	322	0	0	7-5-10
F	404	0	0	7-5-10

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
COMBINED	SNOW	LIVE	PERM. LIVE		
B	237	141 / 0	36 / 0	0 / 0	60 / 0
D	237	141 / 0	36 / 0	0 / 0	60 / 0
F	313	136 / 0	85 / 0	0 / 0	92 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 15	-84.9	-84.9	0.04 (1)	10.00	F-C	-158 / 0	0.02 (1)
B-H	-75 / 0	-84.9	-84.9	0.04 (1)	6.25	G-H	-246 / 24	0.00 (1)
H-C	-116 / 0	-84.9	-84.9	0.15 (1)	6.25	I-J	-246 / 24	0.00 (1)
C-J	-116 / 0	-84.9	-84.9	0.15 (1)	6.25			
J-D	-75 / 0	-84.9	-84.9	0.04 (1)	6.25			
D-E	0 / 15	-84.9	-84.9	0.04 (1)	10.00			
B-G	0 / 97	-38.5	-38.5	0.14 (1)	10.00			
G-F	0 / 97	-38.5	-38.5	0.16 (1)	10.00			
F-I	0 / 97	-38.5	-38.5	0.16 (1)	10.00			
I-D	0 / 97	-38.5	-38.5	0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-J:1), BC=0.16/1.00 (F-I:1),
WB=0.02/1.00 (C-F:1), SS=0.21/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

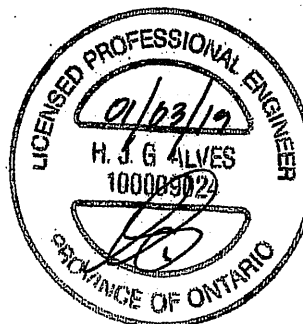
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1657 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)



ENGR NO. TAM 77900063
STRUCTURAL
COMPONENT ONLY

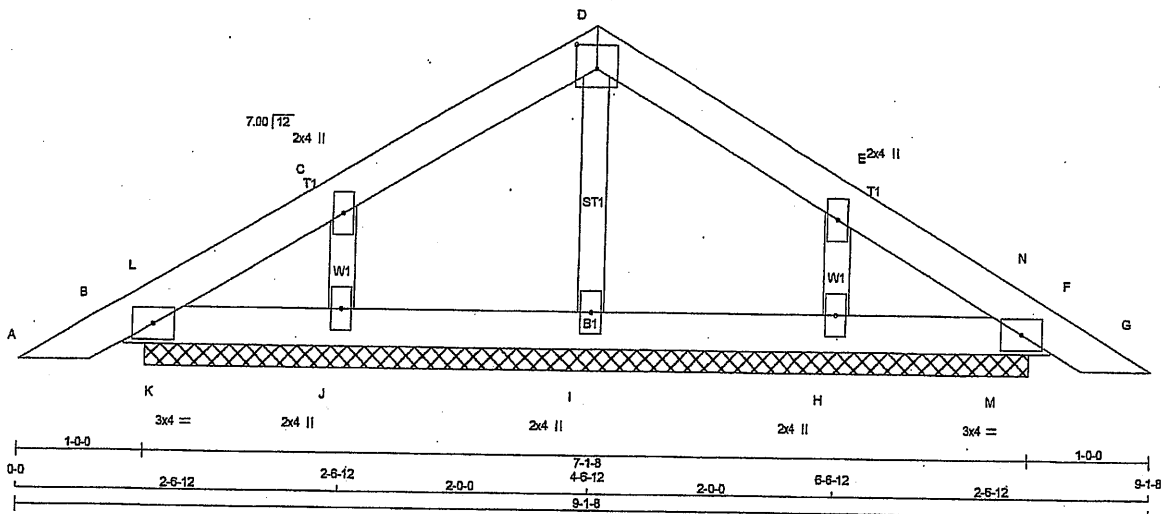
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300539	P1G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:K77_mHxOPUzKFD0TOr6u1yw9qF-SR5qqPJW_e8S17J74uNKukaYDdn?yTkQHW44J6zz4b2

Scale = 1:17.2



TOTAL WEIGHT = 2 X 23 = 47 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TM1-I	MT20	3.0	4.0		
H, I, J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	FACTORED FORCE (LBS)	MAX UNBRACED LENGTH (FR-TO)	MAX LC1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0/15	-84.9	-84.9 0.04 (1)	I-D	-118/0	10.00	0.02 (1)
B-L	-53/0	-84.9	-84.9 0.01 (2)	J-C	-188/0	6.25	0.03 (1)
L-C	-21/0	-84.9	-84.9 0.05 (1)	H-E	-188/0	6.25	0.03 (1)
C-D	-31/0	-84.9	-84.9 0.05 (1)	K-L	-11/17	6.25	0.00 (1)
D-E	-31/0	-84.9	-84.9 0.05 (1)	M-N	-11/17	6.25	0.00 (1)
E-N	-21/0	-84.9	-84.9 0.05 (1)				
N-F	-53/0	-84.9	-84.9 0.01 (2)				
F-G	0/15	-84.9	-84.9 0.04 (1)				
B-K	0/33	-38.5	-38.5 0.02 (1)				
K-J	0/33	-38.5	-38.5 0.03 (2)				
J-I	0/19	-38.5	-38.5 0.03 (2)				
I-H	0/19	-38.5	-38.5 0.03 (2)				
H-M	0/33	-38.5	-38.5 0.03 (2)				
M-F	0/33	-38.5	-38.5 0.02 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-E:1), BC=0.03/1.00 (J-K:2), WB=0.03/1.00 (E-H:1), SS=0.07/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

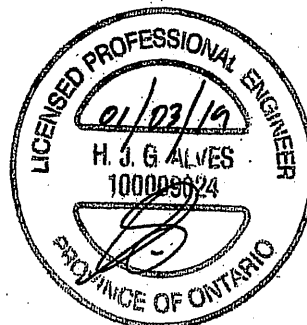
NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1657
	354	1657	788
			1987
			1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.12 (C) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



DRWG NO. TAM 11900064
STRUCTURAL
CHECKED BY ONLY

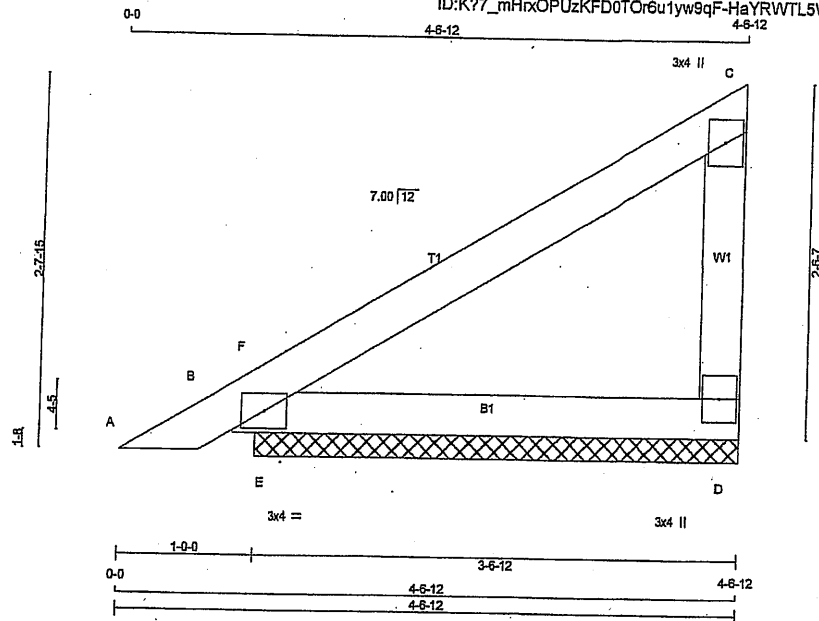
JOB NAME 300536	TRUSS NAME P2G	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MTek Industries, Inc. Thu Jan 3 11:24:57 2019 Page 1

ID:K77_mHxOPUzKFD0TOr6u1yw9qF-HaYRWTL5WfmEjzOtN8VyQFHnRnqIDnNpKJAK1rzz5Ma

Scale = 1:15.7



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2
D - C	2x4 DRY	No.2
B - D	2x4 DRY	No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-H	MT20	3.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

- CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	LC1 MAX (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
FR-TO					FR-TO		
A-B	0/15	-84.9	-84.9 0.04 (1)	10.00	E-F	-165/86	0.00 (1)
B-F	-60/4	-84.9	-84.9 0.05 (3)	6.25			
F-C	0/5	-84.9	-84.9 0.16 (1)	10.00			
D-C	-143/0	0.0	0.0 0.02 (1)	7.81			
B-E	0/0	-38.5	-38.5 0.10 (1)	10.00			
E-D	0/0	-38.5	-38.5 0.13 (1)	10.00			

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
FR-TO		
E-F	-165/86	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (C-F:1), BC=0.13/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.17/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

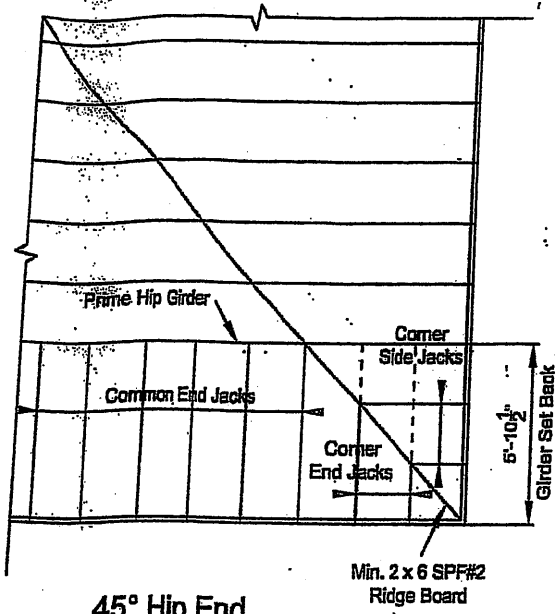
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.80)
JSI METAL= 0.04 (B) (INPUT = 1.00)



DRWG NO. TAM **T1900056**
STRUCTURAL
COMPONENT ONLY



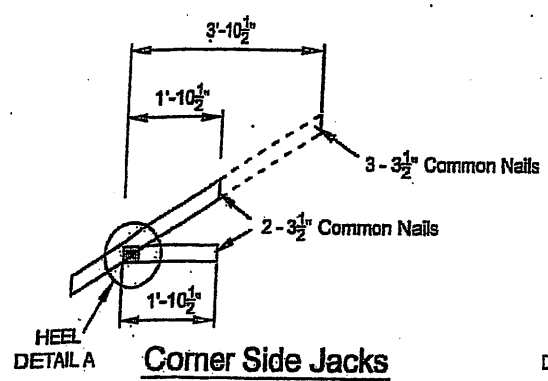
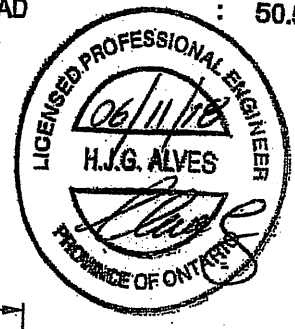
45° Hip End

LUMBER SPECIFICATION

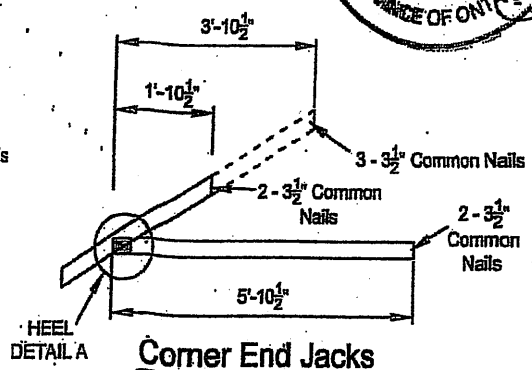
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

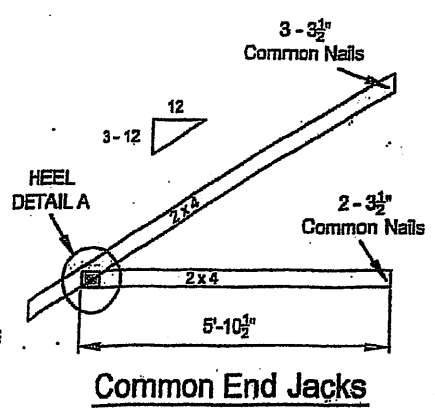
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 50.5 P.S.F.



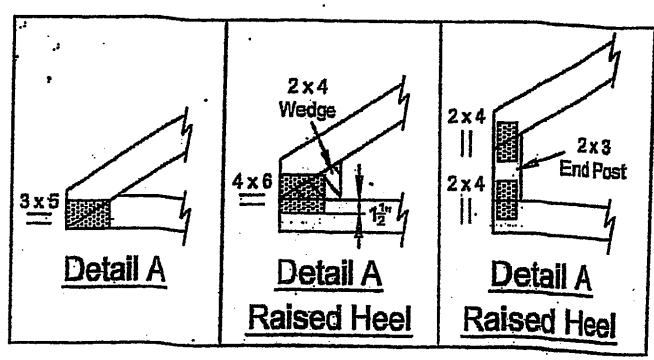
Corner Side Jacks



Corner End Jacks

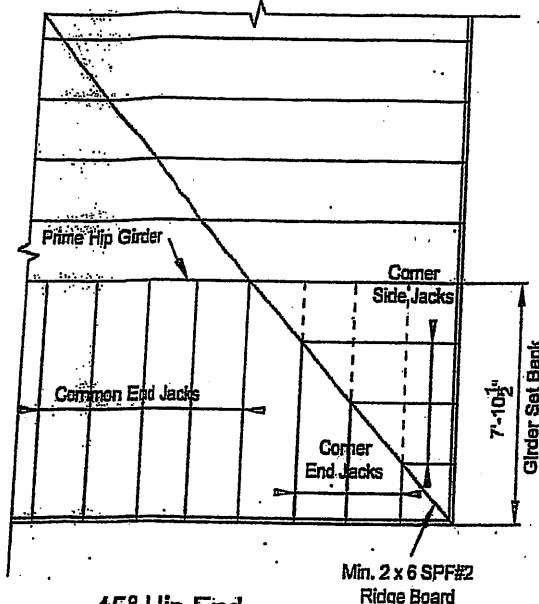


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

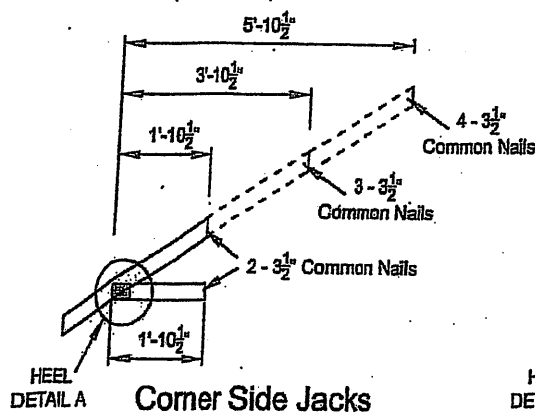
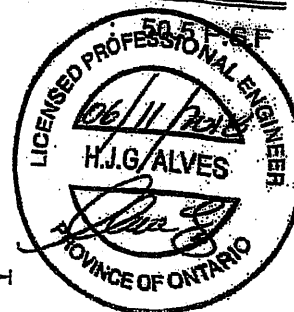
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

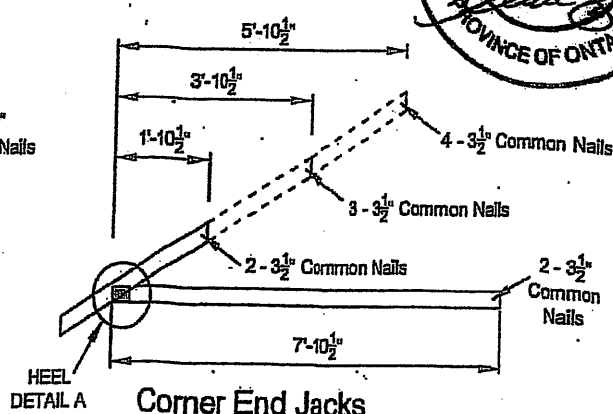
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

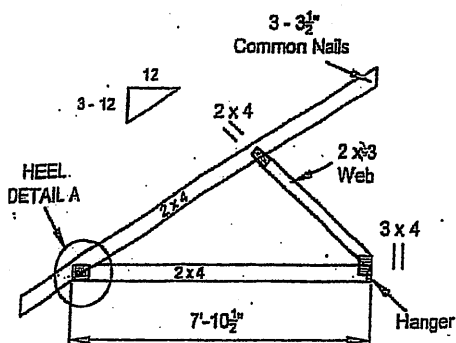
TOTAL LOAD



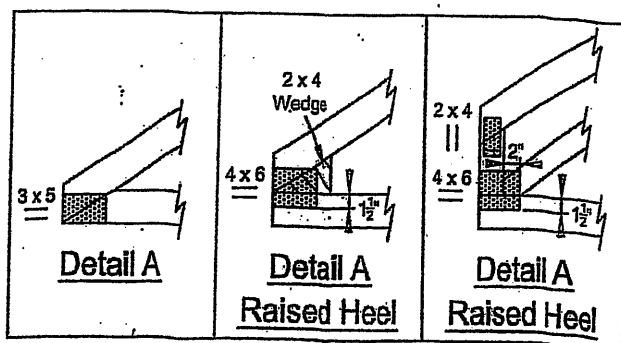
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

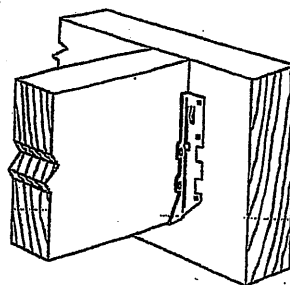
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

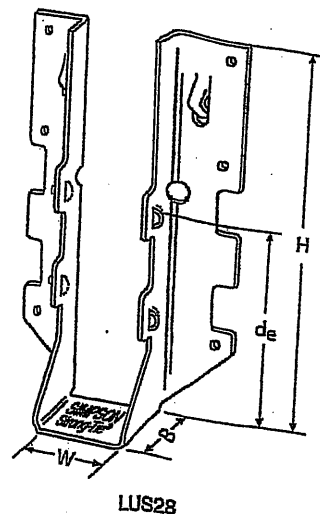
- Use all specified fasteners.
- Nails: $16d = 0.162" \text{ dia.} \times 3\frac{1}{2}" \text{ long common wire,}$
 $10d = 0.148" \times 3" \text{ long common wire.}$
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- * These hangers cannot be modified

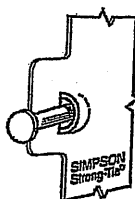


Typical LUS Installation



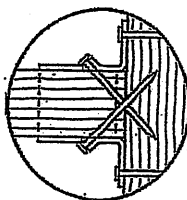
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		SP-F	
								Uplift (K ₂ =1.15)	Normal (K ₃ =1.00)	Uplift (K ₂ =1.15)	Normal (K ₃ =1.00)
LUS24	18	1 1/4	3 1/4	1 1/4	1 1/4	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/4	3 1/4	2	1 1/4	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/4	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/4	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/4	4 1/4	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/4	6 1/4	1 1/4	3 1/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/4	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/4	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/4	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/4	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/4	8 1/4	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_a is the distance from the seat of the hanger to the highest joist nail.

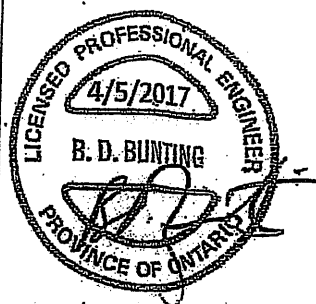


**Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).**

**U.S. Patent
5,603,580**



**Double
Shear
Nailing
Top View.**



EMPIRE STATES DESIGN

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SECRET

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

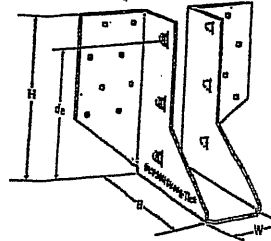
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

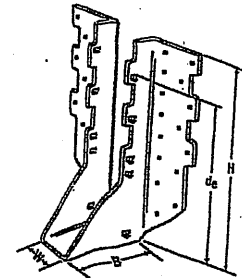
- Use all specified fasteners
- Nails: $16d = 0.162"$ dia. x $3\frac{1}{2}"$ long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

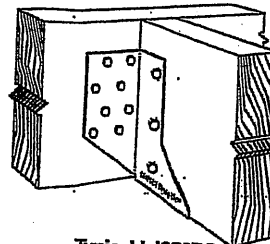
- See current catalogue for options



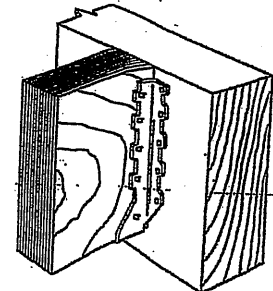
LJS26DS



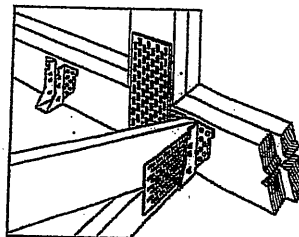
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



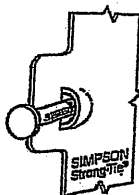
Typical HUS Installation



**Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)**

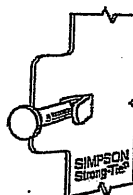
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _F =1.15) lb.	Normal (K _F =1.00) lb.	Uplift ² (K _F =1.15) lb.	Normal (K _F =1.00) lb.
LIS26DS	18	1 ¹⁵ / ₁₆	5	3½	4 ⁵ / ₈	(16) 16d	(8) 16d	2055	4265	1460	4115
HUS26	16	1 ⁵ / ₈	5½	3	3 ¹ / ₂ ¹⁵ / ₁₆	(14) 16d	(8) 16d	2705	4940	2065	3875
HUS28	16	1 ⁵ / ₈	7 ³ / ₃₂	3	6 ³ / ₃₂	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 ⁵ / ₈	9 ³ / ₃₂	3	7 ³ / ₃₂	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 ¹ / ₄ ¹⁵ / ₁₆	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_a is the distance from the seat of the hanger to the highest joist nail.

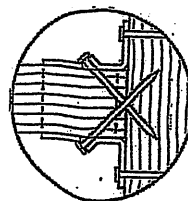


**Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).**

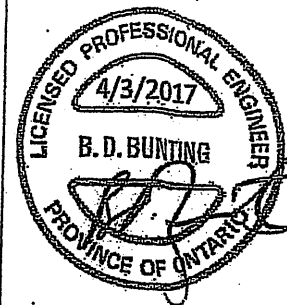
**U.S. Patent
5,603,580**

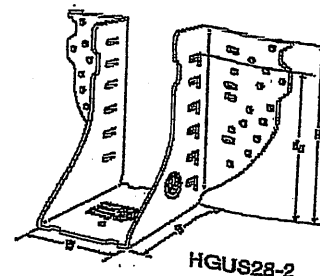


**Double
Shear
Nailing
Side View.
Do not
bend tab
back.**

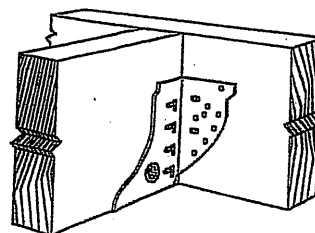


Double
Shear
Nailing
Top View.

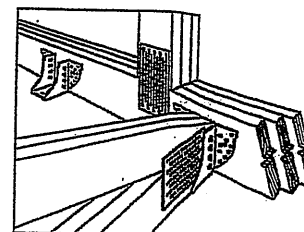




• See current catalogue for options



Typical HGUS Installation



**Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)**

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₂ =1.15)	Normal (K ₃ =1.00)	Uplift (K ₂ =1.15)	Normal (K ₃ =1.00)
HGUS26	12	1 $\frac{1}{8}$ "	5 $\frac{1}{8}$ "	5	4 $\frac{3}{32}$ "	(20) 16d	(8) 16d	2685	6625	2665	5700
HGUS26-2	12	3 $\frac{3}{16}$ "	5 $\frac{1}{8}$ "	4	4 $\frac{1}{8}$ "	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 $\frac{1}{8}$ "	5 $\frac{1}{8}$ "	4	4 $\frac{1}{8}$ "	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 $\frac{1}{8}$ "	5 $\frac{1}{8}$ "	4	4 $\frac{1}{8}$ "	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 $\frac{1}{8}$ "	7 $\frac{1}{8}$ "	5	6 $\frac{1}{8}$ "	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 $\frac{3}{16}$ "	7 $\frac{1}{8}$ "	4	6 $\frac{1}{8}$ "	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 $\frac{1}{8}$ "	7 $\frac{1}{8}$ "	4	6 $\frac{1}{8}$ "	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 $\frac{1}{8}$ "	7 $\frac{1}{8}$ "	4	6 $\frac{1}{8}$ "	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 $\frac{3}{16}$ "	9 $\frac{3}{16}$ "	4	8 $\frac{1}{8}$ "	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 $\frac{1}{8}$ "	9 $\frac{1}{4}$ "	4	8 $\frac{3}{8}$ "	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 $\frac{1}{8}$ "	9 $\frac{3}{16}$ "	4	8 $\frac{3}{8}$ "	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 $\frac{1}{8}$ "	10 $\frac{3}{8}$ "	4	10 $\frac{1}{8}$ "	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 $\frac{1}{8}$ "	12 $\frac{1}{2}$ "	4	11 $\frac{1}{8}$ "	(66) 16d	(22) 16d	10130	16400	7195	11645

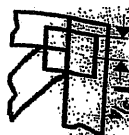


Double
Shear
Nailing
Top View.

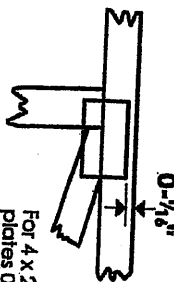


Symbols

PLATE LOCATION AND ORIENTATION



General plate on joint. Unless x, y, z, or dimensions are in 1/16, 1/8, 1/4, 1/2, 3/4, 1, or 2, apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

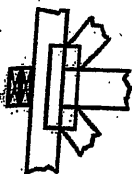
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

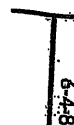


Indicates location where bearings (supports) occur. Icons vary but number where bearings occur.

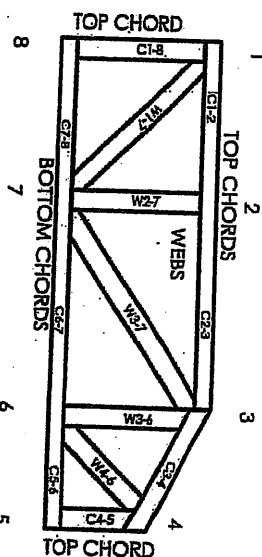
Industry Standard:

Truss Design Practices and Specifications for Light Metal Plate Connected Wood Trusses BCS1: Building Component Safety Information, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in 1/16, 1/8, 1/4, 1/2, 3/4, 1, or 2, apply plates to both sides of truss and fully embed teeth.



JOINTS ARE GENERALLY NUMBERED/CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCS1.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral bracing themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing of 10' ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MITEK
POWER TO PERFORMANCE[™]
Mitek Engineering Reference Sheet: Miti-2473C rev. 10-08

H - Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA 086-14

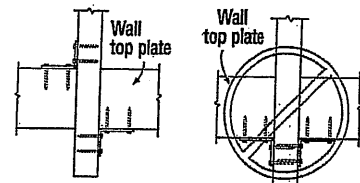
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

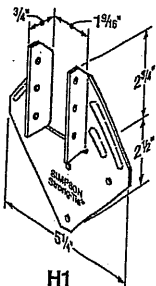
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

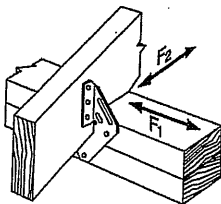


Install diagonally across from each other for minimum 2x truss.

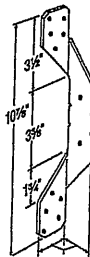
Nailing into both sides of a single ply 2x truss may cause the wood to split.



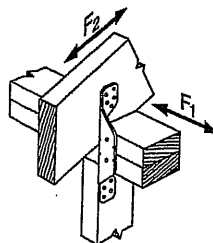
H1



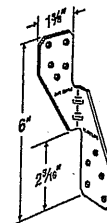
H1 Installation



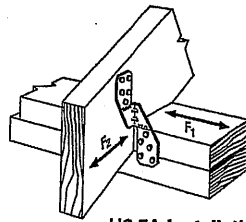
H2A



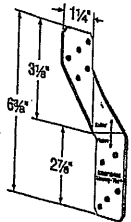
H2A Installation



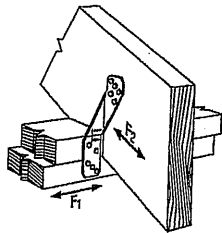
H2.5A



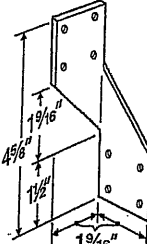
H2.5A Installation



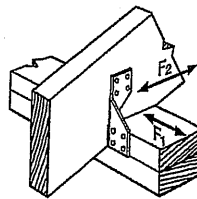
H2.5T



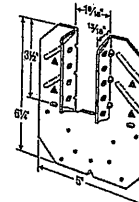
H2.5T Installation
(Nails into both top plates)



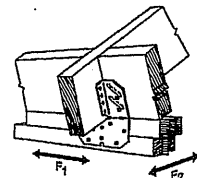
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



SIMPSON
STRONG-TIE
DESIGN

© 2014 Simpson Strong-Tie Company Inc.
SIMPSON STRONG-TIE COMPANY INC.
1000 W. 10th St., Suite 100, Portland, OR 97204
800.999.5099
strongtie.com

(800) 999-5099
strongtie.com

CP Crush Plates – Bearing Enhancers

SIMPSON
Strong-Tie®

The CP transfers load from the truss or girder to plates for bearing limited conditions. Replaces nail-on scabs or in some cases, an additional ply when needed for bearing.

Material: See table

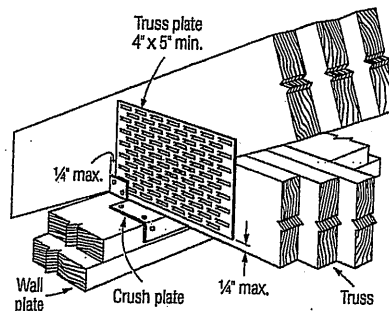
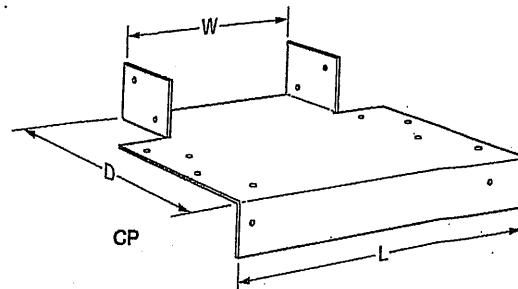
Finish: G90 galvanized

Design:

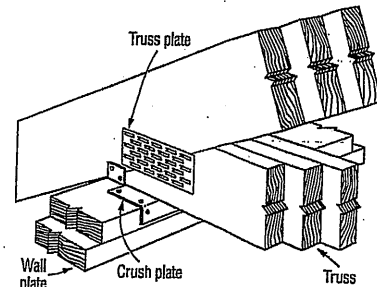
- Factored resistances are in accordance with CSA O86-14 assuming Q_r/A_b and $Q_r/A'_b = 812$ psi for D.Fir-L and 615 psi for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 when compression loads are applied to both sides of truss chord members at bearing locations.

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- For Case 1, truss plates must be located a maximum of ¼" from the underside of the truss chord and a maximum of ¼" from the edge of the wall plates in accordance with the reinforcing requirements of 7.5.9 TPIC 2014.



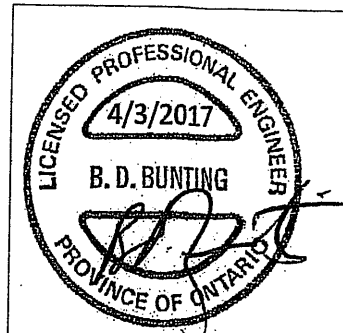
Case 1



Case 2

Model No.	Ga.	Wall Plate	Dimensions (in.)			Fasteners		Factored Resistance (lb.)				
			W	D	L	Wall Plate	Truss	D.Fir-L		S-P-F		
								Uplift	Bearing	Uplift	Bearing	
												$(K_u=1.15)$
CASE 1 (Truss Plate Reinforcement)												
CP1-4	20	2x4	1%	3½	4½	(6) 10d	(4) 10d x 1½"	225	5985	225	4515	
CP2-4	16		3¾	3½	5¾	(6) 10d	(4) 10d x 1½"	225	11390	225	9030	
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	17895	225	13545	
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	23860	225	18065	
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	9370	225	7095	
CP2-6	16		3¾	5½	5¾	(10) 10d	(4) 10d x 1½"	225	18740	225	14190	
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	28110	225	21285	
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	37495	225	28390	
CASE 2 (No Reinforcement)												
CP1-4	20	2x4	1%	3½	4½	(6) 10d	(4) 10d x 1½"	225	4685	225	3550	
CP2-4	16		3¾	3½	5¾	(6) 10d	(4) 10d x 1½"	225	9370	225	7100	
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	14055	225	10650	
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	18750	225	14195	
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	7365	225	5575	
CP2-6	16		3¾	5½	5¾	(10) 10d	(4) 10d x 1½"	225	14730	225	11150	
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	22095	225	16725	
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	29460	225	22305	

1. Factored bearing resistances assume wall plate and truss are the same species. For a mixed species system use S-P-F values.


**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC-CP17-3/17 exp. 6/19

(800) 999-5099
strongtie.com

TS/LTS/MTS/HTS Twist Straps

SIMPSON
Strong-Tie

Twist straps provide a tension connection between two wood members. They resist uplift at the heel of a truss economically. The 3" bend section eliminates interference at the transition points between wood members. TS twist straps come with an equal number of left and right hand units in each carton.

MATERIAL: LTS-18 gauge; MTS-16 gauge; HTS-14 gauge; TS-16 gauge
FINISH: Galvanized. Some products available in stainless steel and ZMAX®; see Corrosion Information, pages 14-17.

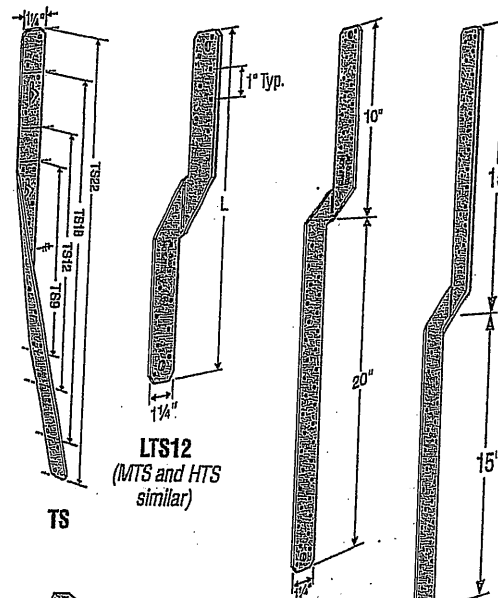
INSTALLATION: Use all specified fasteners. See General Notes.

- TS should be installed in pairs to reduce eccentricity.
- When LTS/MTS is installed as Truss-to-Top Plate tie, install 3-10d $\times$ 1½" nails to the u/s of the plate and 3-10d $\times$ 1½" nails into the edge of the double top plate.

These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.

These products are approved for installation with the Strong-Drive® SD Connector screw. See page 24 for more information.

Model No.	L (in)	Fasteners (Total)	Factored Resistance (K _n = 1.15)	
			D-Fir-L	S-P-F
			lbs	lbs
TS9	9	8-16d	1125	1040
TS12	11½	10-16d	1410	1300
TS18	17½	14-16d	1970	1820
TS22	21½	18-16d	2125	2125
LTS12	12	12-10d $\times$ 1½	1015	720
LTS16	16	12-10d $\times$ 1½	1015	720
LTS18	18	12-10d $\times$ 1½	1015	720
LTS20	20	12-10d $\times$ 1½	1015	720
MTS12	12	14-10d $\times$ 1½	1570	1180
MTS16	16	14-10d $\times$ 1½	1570	1180
MTS18	18	14-10d $\times$ 1½	1570	1180
MTS20	20	14-10d $\times$ 1½	1570	1180
MTS30	30	14-10d $\times$ 1½	1570	1180
MTS24C	24	14-10d $\times$ 1½	1570	1180
MTS30C	30	14-10d $\times$ 1½	1570	1180
HTS16	16	16-10d $\times$ 1½	2050	1455
HTS20	20	24-10d $\times$ 1½	2050	1455
HTS24	24	24-10d $\times$ 1½	2050	1455
HTS28	28	24-10d $\times$ 1½	2050	1455





Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

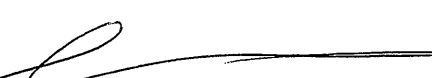
- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900218

Feb 09, 2018

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information		Application number:	
Building number, street name		Unit no. UNITS 1 – 4	Lot/con. BLOCK 9
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description	
B. Individual who reviews and takes responsibility for design activities			
Name SAM KATSOULAKOS		Firm MICRO CITY ENGINEERING SERVICES INC.	
Street address R.R #1, PO BOX 61		Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750	Cell number	
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]			
☐ House	☐ HVAC – House	☒ Building Structural	
☐ Small Buildings	☐ Building Services	☐ Plumbing – House	
☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing – All Buildings	
☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems	
Description of designer's work ROYAL PINE HOMES – FORESTSIDE PH6 – BLOCK 9 – UNITS 1 - 4 (SCHEDULE IS ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM1149-19 DATED 1-18-19). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.			
D. Declaration of Designer			
I, <u>SAM KATSOULAKOS</u> declare that (choose one as appropriate): (print name)			
☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.			
Individual BCIN: <u>26064</u>			
Firm BCIN: <u>29991</u>			
☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code.			
Individual BCIN: _____			
Basis for exemption from registration: _____			
☐ The design work is exempt from the registration and qualification requirements of the Building Code.			
Basis for exemption from registration and qualification: _____			
I certify that:			
1. The information contained in this schedule is true to the best of my knowledge.			
2. I have submitted this application with the knowledge and consent of the firm.			
Date	Signature of Designer 		

NOTE:

1. For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d). of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
2. Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

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