ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
HGUS28-2 -(##)
LUS26-2 -(VV)
LUS24 -(O)
LJS26DS -(V)

ALL B-2-2X10
(FLUSH)

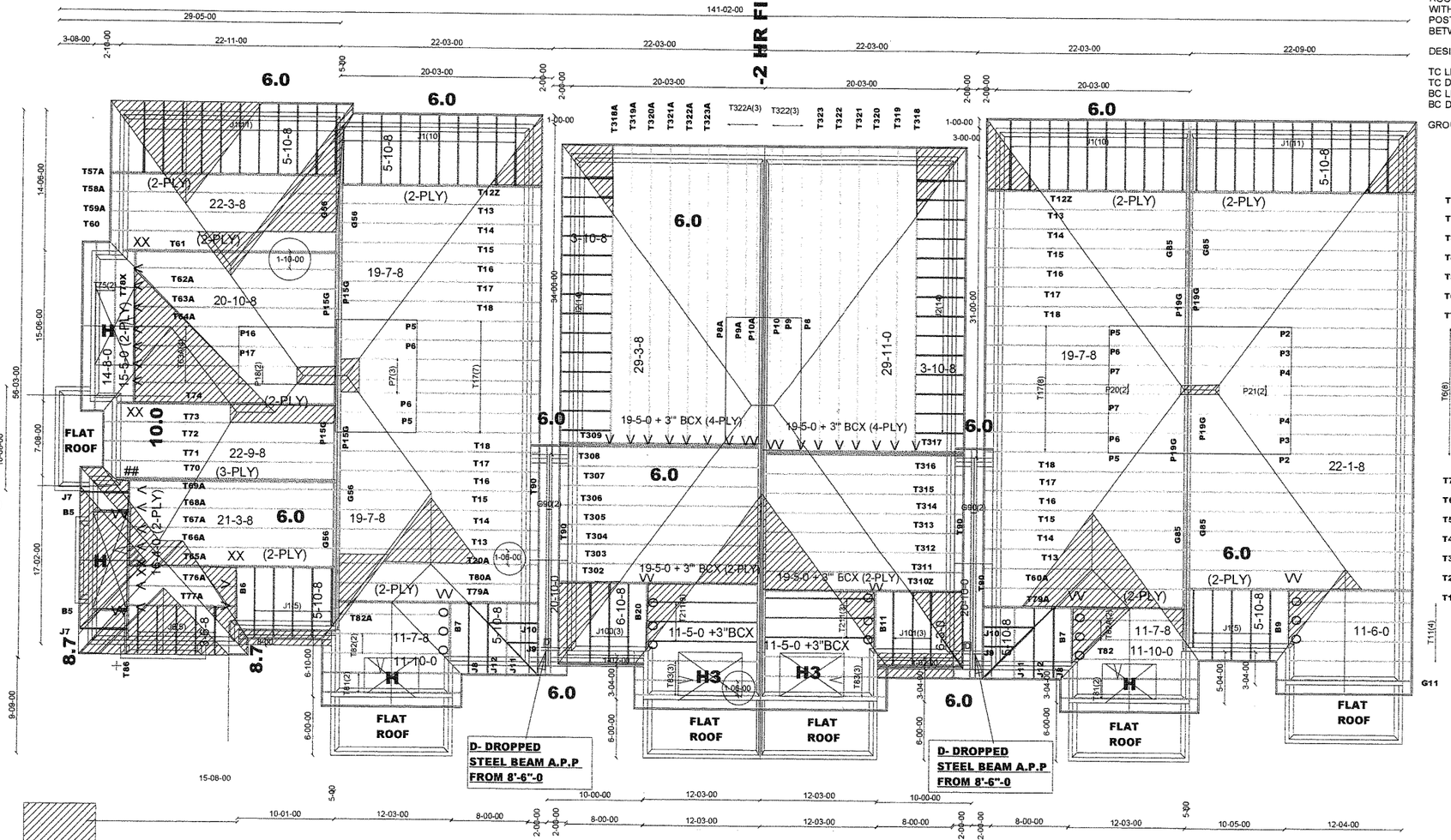
DENOTE:

H- 12" RAISED
CEILING

H3- 24" RAISED
CEILING

8/12 PITCHES (TYP.)
UNLESS NOTED

UNIT2204 CORNER B -LOT6 300399 | **UNIT2202 B -LOT5 300400** | **UNIT2201 B(REV) -LOT4 300401** | **2 HR FIREWALL** | **UNIT2201 B -LOT3 300582** | **UNIT2202 B(REV) -LOT2 300402** | **UNIT2203 B(REV) -LOT1 300403**



T-180485

DENOTES:
CONVENTIONAL
FRAMING

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

TC LIVE	23.3 PSF
TC DEAD	6 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

GROUND SNOW LOAD 27.2 PSF

Unit 2, DWG# T-1901128 to T-1901133, T-1901138 to T-1901140 , T-1901156, to T-1901161, T-1901171 to T-1901172 , T-1901215 to T-1901216 , T-1901224, T-1800216, T1800218

Unit 1, DWG# T-1901138 to T-1901139, T-1901141 to T-1901150, T-1901154, T-1901212 , T-1901214 , T-1800216, T1800218

LICENSED PROFESSIONAL ENGINEER
04/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

C of A NO. TAM T19083347
STRUCTURAL
COMPONENT ONLY

Unit 6, DWG# T-1901100 to T-1901127, T-1800216, T1800218

Unit 5, DWG# T-1901123, T-1901127 to T-1901133, T-1901156 to T-1901161 , T-1901165, T-1901171 to T-1901172 , T-1901215 to T-1901216 , T-1901224, T-1800216, T1800218

Unit 4, DWG# T-1901134 to T-1901136, T-1901171 to T-1901172, T-1901175 to T-1901181 , T-1901191, T-1901193, T-1901195, T-1901197, T-1901199, T-1901200, T-1901218, T-1901220, T-1901222, T-1800216, T1800218

Unit 3, DWG# T-1901134 to T-1901135, T-1901137, T-1901171 to T-1901172, T-1901183 to T-1901190 , T-1901192, T-1901194, T-1901196, T-1901198, T-1901200, T-1901217, T-1901219, T-1901221, T-1800216, T1800218

M-12126

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Job Track:	41239
Layout ID:	300398
Plan Log:	98887

Builder / Location: ROYAL PINE HOMES / BRAMPTON
Project: FORESTSIDE PH.6
Date: 4/10/2019 | Designer: AC

Model / Elevation: BLOCK 3 - LOTS 1 TO 6
Mitek ver 8.2.3.229

TAMARACK LUMBER INC
ALPA LUMBER GROUP

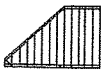
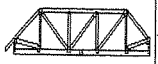
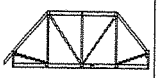
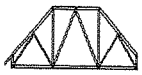
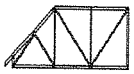
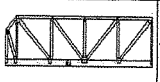
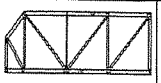
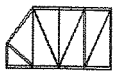
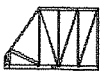
Delivery Shiplist

DATE	09/10/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300399 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2204 CORNER ELEVATION: B LT 6

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	G56 GABLE	6.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-02-00 09-01-04	267.38 167.34		
	1 2 Ply	T57A HIP GIRDER	8.00 0.00	22-03-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	228.32 142.68		
	1	T58A HIP	8.00 0.00	22-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	88.96 57.50		
	1	T59A HIP	8.00 0.00	22-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	93.56 59.67		
	1	T60 HALF HIP	8.00 0.00	22-03-08	06-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-07-04	95.54 61.17		
	1 2 Ply	T61 HALF HIP	8.00 0.00	22-03-08	07-07-04	2 X 4	2 X 6	00-00-00 00-00-00	01-04-13 07-07-04	291.60 176.66		
	1	T62A HALF HIP	8.00 0.00	20-10-08	08-06-04	2 X 4	2 X 4	00-00-00 00-00-00	02-04-02 08-06-04	103.07 63.17		
	5	T63A PIGGYBACK	8.00 0.00	20-10-08	09-06-04	2 X 4	2 X 4	00-00-00 00-00-00	02-04-02 09-06-04	576.05 358.35		
	1	T64A HALF HIP	8.00 0.00	20-10-08	10-06-04	2 X 4	2 X 4	00-00-00 00-00-00	02-04-02 10-06-04	113.38 68.67		
	1 2 Ply	T65A HALF HIP	10.00 0.00	21-03-08	04-01-04	2 X 4	2 X 6	00-00-00 00-00-00	02-10-11 04-01-04	227.74 143.34		
	1	T66A HALF HIP	10.00 0.00	21-03-08	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-10-11 05-01-04	89.71 56.83		
	1	T67A HALF HIP	10.00 0.00	21-03-08	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-10-11 06-01-04	95.34 60.17		
	1	T68A HALF HIP	10.00 0.00	21-03-08	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-10-11 07-01-04	101.35 64.33		
	1	T69A HALF HIP	10.00 0.00	21-03-08	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-10-11 08-01-04	107.60 67.33		
	1 3 Ply	T70 HALF HIP	10.00 0.00	22-09-08	08-06-00	2 X 4	2 X 8	00-00-00 00-00-00	01-07-11 08-06-00	483.84 294.00		
	1	T71 HALF HIP	10.00 0.00	22-09-08	09-06-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-06-00	121.82 74.67		
	1	T72 HALF HIP	10.00 0.00	22-09-08	10-06-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-06-00	127.70 78.67		
	1	T73 HALF HIP	10.00 0.00	22-09-08	11-06-00	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 11-06-00	140.81 86.33		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

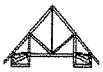
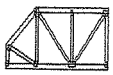
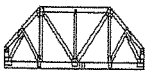
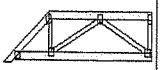
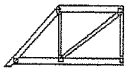
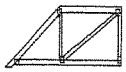
Delivery Shiplist

DATE	09/10/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300399 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2204 CORNER ELEVATION: B LT6

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T74 HALF HIP	10.00 / 0.00	22-09-08	11-05-02	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 11-05-02	328.46 / 199.34		
	2	T75 ROOF	8.00 / 0.00	14-08-00	06-03-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	143.68 / 90.34		
	1 / 2 Ply	T76X COMMON	8.00 / 0.00	15-05-00	06-03-08	2 X 4	2 X 6	00-00-00 / 00-00-00	01-01-13 / 01-01-13	166.50 / 105.34		
	1	T77A HALF HIP	8.70 / 0.00	11-00-00	03-09-02	2 X 4	2 X 4	00-00-00 / 00-00-00	01-05-13 / 03-09-02	46.42 / 31.00		
	1	T78A HIP	8.70 / 0.00	11-00-00	05-01-02	2 X 4	2 X 4	00-00-00 / 00-00-00	01-05-13 / 04-03-03	52.45 / 34.17		
	1 / 2 Ply	T86 HIP GIRDER	8.00 / 0.00	16-04-00	04-01-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-13 / 01-04-13	176.42 / 111.34		
	2	P15G GABLE	6.00 / 0.00	08-10-03	04-09-04	2 X 4	2 X 4	00-00-00 / 00-00-00	00-00-00 / 04-09-04	65.38 / 42.34		
	1	P16 PIGGYBACK	8.00 / 0.00	09-04-07	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 01-10-08	31.21 / 21.00		
	1	P17 PIGGYBACK	8.00 / 0.00	09-04-07	02-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 02-10-08	32.30 / 21.17		
	2	P18 PIGGYBACK	8.00 / 0.00	09-04-07	02-11-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 02-11-08	64.84 / 42.34		
	16	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	268.64 / 170.72		
	5	J6 JACK-OPEN	8.00 / 0.00	03-06-08	03-09-02	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-09-02	59.85 / 40.00		
	2	J7 JACK-OPEN	8.70 / 0.00	03-07-00	04-01-00	2 X 4	2 X 4	01-03-08 / 00-00-00	01-05-13 / 04-01-00	24.76 / 17.34		

TOTAL # TRUSS= **67.00** TOTAL BFT OF ALL TRUSSES= **3007.32** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4814.68** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS28-2	
4	Hangers	HGUS26-2	
2	Hangers	LUS26-2	
15	Hangers	LJS26DS	

TOTAL # ITEMS= **22.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

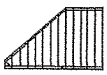
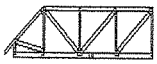
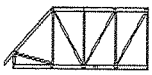
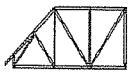
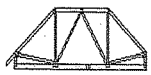
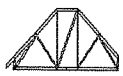
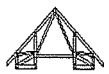
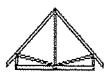
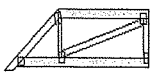
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300400 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2202 ELEVATION: B LT. 5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	G56 GABLE	6.00 0.00	25-08-00	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-02-00 09-01-04	267.38 167.34		
	1 2 Ply	T12Z HALF HIP	8.00 0.00	19-07-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	202.20 124.68		
	2	T13 HALF HIP	8.00 0.00	19-07-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	170.12 108.66		
	2	T14 HALF HIP	8.00 0.00	19-07-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	170.92 108.00		
	2	T15 HALF HIP	8.00 0.00	19-07-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	180.80 114.66		
	2	T16 HALF HIP	8.00 0.00	19-07-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	195.66 121.66		
	9	T17 PIGGYBACK	8.00 0.00	19-07-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	870.39 546.03		
	2	T18 HALF HIP	8.00 0.00	19-07-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	196.86 122.66		
	1 2 Ply	T79A HIP GIRDER	8.00 0.00	19-07-08	05-03-13	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	181.12 113.34		
	1	T80A HIP	8.00 0.00	19-07-08	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	85.03 55.83		
	1	T20A COMMON	8.00 0.00	19-07-08	08-00-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	84.19 53.67		
	2	T81 ROOF	8.00 0.00	11-10-00	05-04-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	120.26 77.34		
	2	T82 COMMON	8.00 0.00	11-10-00	05-04-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	97.64 59.66		
	1	T82A COMMON	8.00 0.00	11-07-08	05-04-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	46.55 29.83		
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	85.23 54.00		
	2	P15G GABLE	6.00 0.00	08-10-03	04-09-04	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 04-09-04	65.38 42.34		
	2	P5 PIGGYBACK	8.00 0.00	07-03-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	49.28 33.66		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

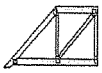
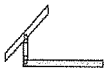
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300400 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2202 ELEVATION: B LT 5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	P6 PIGGYBACK	8.00 0.00	07-03-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	52.40 35.00		
	3	P7 PIGGYBACK	8.00 0.00	07-03-15	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 03-10-08	85.05 57.99		
	10	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	167.90 106.70		
	1	J8 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	17.83 10.67		
	1	J9 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 00-01-01	01-04-13 00-03-08	7.67 4.67		
	1	J10 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	10.41 6.67		
	1	J11 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 04-01-01	01-04-13 00-03-08	12.23 7.33		
	1	J12 JACK-OPEN	8.00 0.00	03-09-07	03-11-02	2 X 4	2 X 4	01-03-08 02-01-01	01-04-13 00-03-08	14.97 9.33		

TOTAL # TRUSS= **58.00** TOTAL BFT OF ALL TRUSSES= **2222.72** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3519.33** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

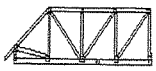
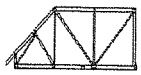
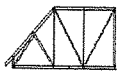
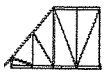
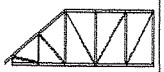
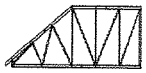
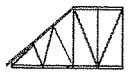
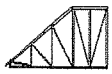
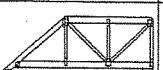
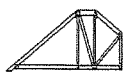
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300401 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2201 ELEVATION: B LT 4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T302 HALF HIP	8.00 / 0.00	19-05-00	04-07-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-04-13 / 00-00-04	209.54 / 126.00		
	1	T303 HALF HIP	8.00 / 0.00	19-05-00	05-07-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	83.02 / 53.67		
	1	T304 HALF HIP	8.00 / 0.00	19-05-00	06-07-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	87.82 / 56.83		
	1	T305 HALF HIP	8.00 / 0.00	19-05-00	07-07-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	94.99 / 60.50		
	1	T306 HALF HIP	8.00 / 0.00	19-05-00	08-07-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	100.65 / 63.67		
	1	T307 HALF HIP	8.00 / 0.00	19-05-00	09-07-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	99.23 / 62.50		
	1	T308 HALF HIP	8.00 / 0.00	19-05-00	10-07-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	103.68 / 65.00		
	1 / 4 Ply	T309 HALF HIP	8.00 / 0.00	19-05-00	11-07-04	2 X 4	2 X 6	01-03-08 / 00-00-00	00-00-00 / 00-00-00	571.80 / 358.68		
	1	T318A HALF HIP	6.00 / 0.00	29-03-08	03-11-13	2 X 4	2 X 6	01-03-08 / 00-00-00	00-00-00 / 00-00-00	157.44 / 95.00		
	1	T319A HALF HIP	6.00 / 0.00	29-03-08	05-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	121.58 / 75.00		
	1	T320A HALF HIP	6.00 / 0.00	29-03-08	06-07-13	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	125.30 / 79.83		
	1	T321A HALF HIP	6.00 / 0.00	29-03-08	07-11-13	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	136.79 / 85.00		
	4	T322A HALF HIP	6.00 / 0.00	29-03-08	09-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	562.40 / 345.32		
	1	T323A HALF HIP	6.00 / 0.00	29-03-08	10-07-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 10-07-13	158.91 / 99.33		
	3	T83 MONOPITCH	8.00 / 0.00	11-05-00	09-00-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-00-00 / 00-00-00	219.33 / 141.51		
	3	T211 JACK-CLOSED	8.00 / 0.00	11-05-00	09-00-02	2 X 4	2 X 4	00-00-00 / 00-00-00	00-00-00 / 00-00-00	161.88 / 102.51		
	1	P8A PIGGYBACK	6.00 / 0.00	12-00-09	02-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-00-00 / 00-00-00	41.19 / 25.33		
	1	P9A PIGGYBACK	6.00 / 0.00	12-00-09	03-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-00-00 / 00-00-00	43.42 / 28.67		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

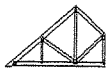
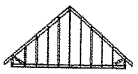
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 **LAYOUT ID:** 300401 **LOCATION:**
BUILDER: ROYAL PINE-FORESTSIDE PH.6 **SUB-BUILDER:**
MODEL: UNIT2201 **ELEVATION:** B LT 4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	P10A PIGGYBACK	6.00 0.00	12-00-09	04-04-11	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	44.04 28.67		
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	00-00-00 00-00-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	00-00-00 00-00-00	85.23 54.00		
	14	J2 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	179.34 112.00		
	3	J100 JACK-OPEN	6.00 0.00	06-10-08	04-07-04	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	57.63 36.00		

TOTAL # TRUSS= **49.00** TOTAL BFT OF ALL TRUSSES= **2206.02** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3527.07** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	
9	Hangers	LJS26DS	

TOTAL # ITEMS= **13.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

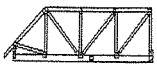
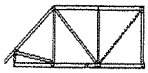
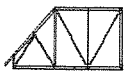
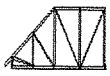
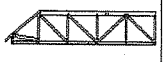
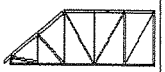
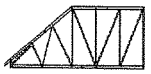
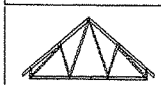
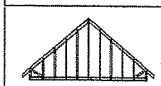
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300582 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2201 ELEVATION: B LT 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T310Z HALF HIP	8.00 0.00	19-05-00	04-03-08	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 00-00-04	204.90 128.00		
	1	T311 HALF HIP	8.00 0.00	19-05-00	05-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	79.84 49.33		
	1	T312 HALF HIP	8.00 0.00	19-05-00	06-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	86.29 54.67		
	1	T313 HALF HIP	8.00 0.00	19-05-00	07-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	93.28 58.83		
	1	T314 HALF HIP	8.00 0.00	19-05-00	08-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	98.85 62.17		
	1	T315 HALF HIP	8.00 0.00	19-05-00	09-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	97.64 60.67		
	1	T316 HALF HIP	8.00 0.00	19-05-00	10-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	99.14 62.00		
	1 4 Ply	T317 HALF HIP	8.00 0.00	19-05-00	11-03-08	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 00-00-04	564.24 350.68		
	1	T318 HALF HIP	6.00 0.00	29-11-00	03-11-13	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 03-11-13	160.30 97.67		
	1	T319 HALF HIP	6.00 0.00	29-11-00	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-03-13	123.36 75.00		
	1	T320 HALF HIP	6.00 0.00	29-11-00	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 06-07-13	127.07 79.83		
	1	T321 HALF HIP	6.00 0.00	29-11-00	07-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-11-13	138.49 85.67		
	4	T322 HALF HIP	6.00 0.00	29-11-00	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-03-13	569.76 348.00		
	1	T323 HALF HIP	6.00 0.00	29-11-00	10-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-07-13	146.82 91.67		
	3	T211 JACK-CLOSED	8.00 0.00	11-05-00	09-00-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 00-00-04	161.88 102.51		
	3	T83 MONOPITCH	8.00 0.00	11-05-00	09-00-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-00-04	219.33 141.51		
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	85.23 54.00		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

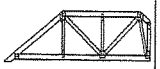
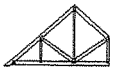
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 **LAYOUT ID:** 300582 **LOCATION:**
BUILDER: ROYAL PINE-FORESTSIDE PH.6 **SUB-BUILDER:**
MODEL: UNIT2201 **ELEVATION:** B- LT 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	P8 PIGGYBACK	6.00 / 0.00	12-08-01	02-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 02-01-03	44.19 / 28.17		
	1	P9 PIGGYBACK	6.00 / 0.00	12-08-01	03-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 02-01-03	44.88 / 29.33		
	1	P10 PIGGYBACK	6.00 / 0.00	12-08-01	04-04-11	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 02-01-03	45.55 / 29.33		
	14	J2 JACK-OPEN	8.00 / 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-11-13	179.34 / 112.00		
	3	J101 JACK-OPEN	6.00 / 0.00	06-03-00	04-03-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 00-04-11	53.10 / 33.99		

TOTAL # TRUSS= **49.00** TOTAL BFT OF ALL TRUSSES= **2186.03** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3505.34** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	
9	Hangers	LJS26DS	

TOTAL # ITEMS= **13.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

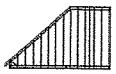
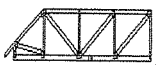
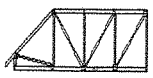
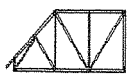
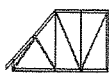
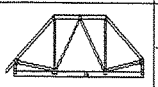
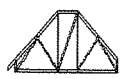
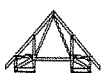
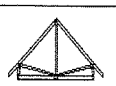
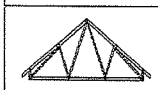
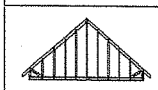
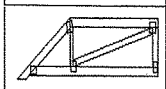
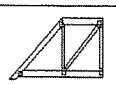
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300402 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2202 ELEVATION: B LT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	G85 GABLE	6.00 0.00	26-05-00	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-01-04	287.98 181.66		
	1 2 Ply	T12Z HALF HIP	8.00 0.00	19-07-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	202.20 124.68		
	2	T13 HALF HIP	8.00 0.00	19-07-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	170.12 108.66		
	2	T14 HALF HIP	8.00 0.00	19-07-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	170.92 108.00		
	2	T15 HALF HIP	8.00 0.00	19-07-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	180.80 114.66		
	2	T16 HALF HIP	8.00 0.00	19-07-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	195.66 121.66		
	10	T17 HALF HIP	8.00 0.00	19-07-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	967.10 606.70		
	2	T18 HALF HIP	8.00 0.00	19-07-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	196.86 122.66		
	1 2 Ply	T79A HIP GIRDER	8.00 0.00	19-07-08	05-03-13	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	181.12 113.34		
	1	T80A HIP	8.00 0.00	19-07-08	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	85.03 55.83		
	2	T81 ROOF	8.00 0.00	11-10-00	05-04-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	120.26 77.34		
	1	T82 COMMON	8.00 0.00	11-10-00	05-04-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	48.83 29.83		
	2	T82A COMMON	8.00 0.00	11-07-08	05-04-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	93.10 59.66		
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	85.23 54.00		
	2	P19G GABLE	6.00 0.00	09-07-03	05-01-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 05-01-12	71.70 45.00		
	2	P5 PIGGYBACK	8.00 0.00	07-03-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	49.28 33.66		
	2	P6 PIGGYBACK	8.00 0.00	07-03-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	52.40 35.00		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

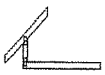
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 **LAYOUT ID:** 300402 **LOCATION:**
BUILDER: ROYAL PINE-FORESTSIDE PH.6 **SUB-BUILDER:**
MODEL: UNIT2202 **ELEVATION:** B LT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	P7 PIGGYBACK	8.00 0.00	07-03-15	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 03-10-08	56.70 38.66		
	2	P20 PIGGYBACK	8.00 0.00	07-03-15	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 04-10-08	60.84 39.66		
	10	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	167.90 106.70		
	1	J8 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	17.83 10.67		
	1	J9 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 00-01-01	01-04-13 00-03-08	7.67 4.67		
	1	J10 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	10.41 6.67		
	1	J11 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 04-01-01	01-04-13 00-03-08	12.23 7.33		
	1	J12 JACK-OPEN	8.00 0.00	03-09-07	03-11-02	2 X 4	2 X 4	01-03-08 02-01-01	01-04-13 00-03-08	14.97 9.33		

TOTAL # TRUSS= **59.00** TOTAL BFT OF ALL TRUSSES= **2267.03** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3589.00** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

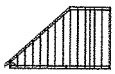
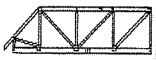
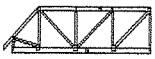
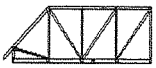
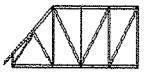
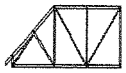
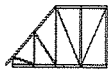
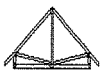
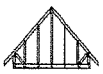
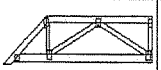
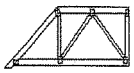
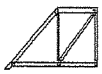
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300403 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2203 ELEVATION: B' LT.1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	G85 GABLE	6.00 0.00	26-05-00	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-01-04	287.98 181.66		
	1 2 Ply	T1 HALF HIP	8.00 0.00	22-01-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	219.12 137.34		
	1 2 Ply	T1Z HALF HIP	8.00 0.00	22-01-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	219.12 137.34		
	2	T2 HALF HIP	8.00 0.00	22-01-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	184.50 115.34		
	2	T3 HALF HIP	8.00 0.00	22-01-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	200.40 128.00		
	2	T4 HALF HIP	8.00 0.00	22-01-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	194.90 123.34		
	2	T5 HALF HIP	8.00 0.00	22-01-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	209.34 131.66		
	10	T6 PIGGYBACK	8.00 0.00	22-01-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	1102.00 685.00		
	2	T7 HALF HIP	8.00 0.00	22-01-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	230.16 144.00		
	4	T11 COMMON	8.00 0.00	11-06-00	05-02-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	190.52 119.32		
	1	G11 GABLE	8.00 0.00	11-06-00	05-02-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	50.14 31.83		
	2	P19G GABLE	6.00 0.00	09-07-03	05-01-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 05-01-12	71.70 45.00		
	2	P2 PIGGYBACK	8.00 0.00	09-09-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	65.22 42.00		
	2	P3 PIGGYBACK	8.00 0.00	09-09-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	69.32 45.66		
	2	P4 PIGGYBACK	8.00 0.00	09-09-15	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 03-10-08	70.70 46.00		
	2	P21 PIGGYBACK	8.00 0.00	09-09-15	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 04-10-08	74.90 48.66		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		

TOTAL # TRUSS= **57.00** TOTAL BFT OF ALL TRUSSES= **2332.87** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3708.66** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 **LAYOUT ID:** 300403 **LOCATION:**
BUILDER: ROYAL PINE-FORESTSIDE PH.6 **SUB-BUILDER:**
MODEL: UNIT2203 **ELEVATION:** B LT.1

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	

TOTAL # ITEMS= **4.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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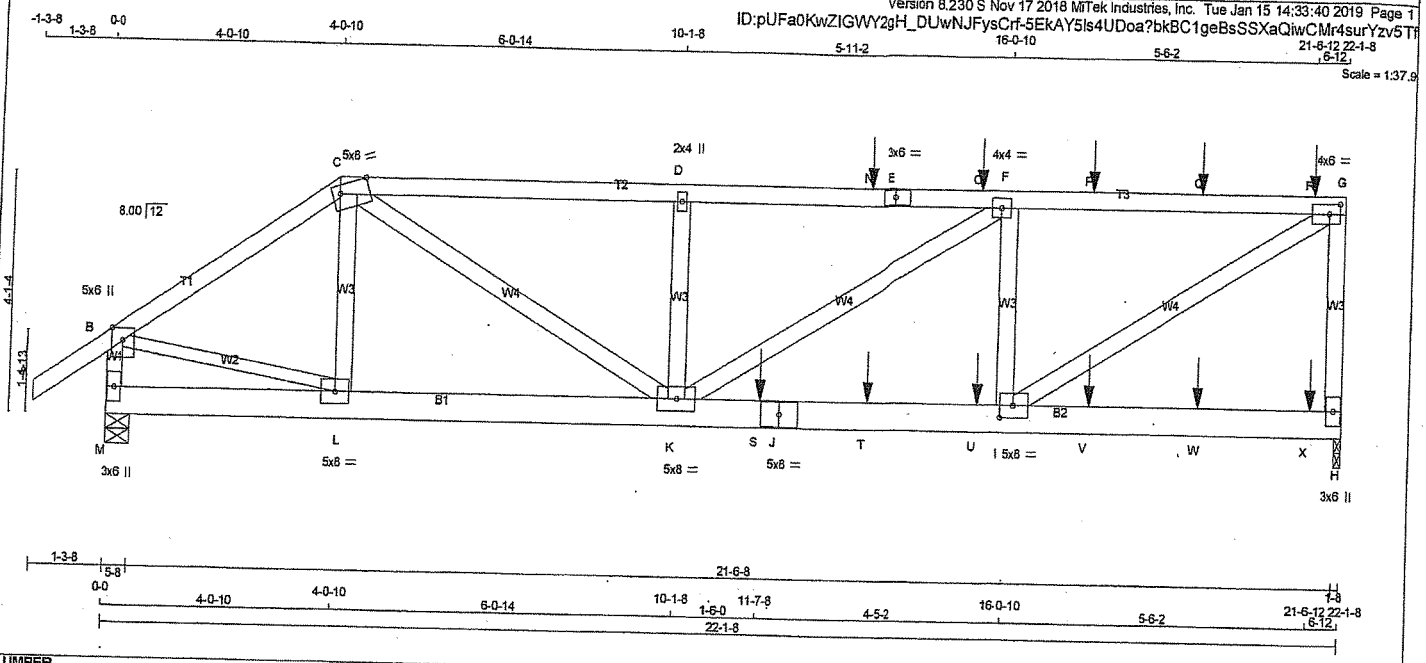


TOTAL WEIGHT = 2 X 110 = 219 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	ORY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	ORY	No.2	SPF
ALL WEBS EXCEPT	2x4	ORY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	SIDE(0.0)
E-G	1	12	SIDE(0.0)
G-H	1	12	TOP
M-B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-J	2	12	TOP
J-H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIROER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UOL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	6.25
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0	2.00	2.25
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	5.0	8.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2749	0	2749	0	0	1-8	1-8
M	2307	0	2307	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2078	1057 / 0	449 / 0	0 / 0	0 / 0	572 / 0	0 / 0
M	1732	917 / 0	347 / 0	0 / 0	0 / 0	468 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B-C	-2548 / 0	-84.9	-84.9	0.16 (1)	5.51
C-D	-4494 / 0	-84.9	-84.9	0.36 (1)	4.23
D-N	-4494 / 0	-84.9	-84.9	0.53 (1)	4.00
N-E	-4494 / 0	-84.9	-84.9	0.53 (1)	4.00
E-O	-4494 / 0	-84.9	-84.9	0.53 (1)	4.00
O-F	-4494 / 0	-84.9	-84.9	0.53 (1)	4.00
F-P	-3645 / 0	-84.9	-84.9	0.48 (1)	4.41
P-O	-3645 / 0	-84.9	-84.9	0.48 (1)	4.41
Q-R	-3645 / 0	-84.9	-84.9	0.48 (1)	4.41
R-G	-3645 / 0	-84.9	-84.9	0.48 (1)	4.41
H-G	-2611 / 0	0.0	0.0	0.33 (1)	7.03
M-B	-2210 / 0	0.0	0.0	0.12 (1)	7.50
M-L	0 / 0	-38.5	-38.5	0.05 (1)	10.00
L-K	0 / 2100	-38.5	-38.5	0.36 (1)	10.00
K-S	0 / 3645	-38.5	-38.5	0.66 (1)	10.00
S-J	0 / 3645	-38.5	-38.5	0.66 (1)	10.00
J-T	0 / 3645	-38.5	-38.5	0.68 (1)	10.00
T-U	0 / 3645	-38.5	-38.5	0.66 (1)	10.00
U-I	0 / 3645	-38.5	-38.5	0.68 (1)	10.00
I-V	0 / 0	-38.5	-38.5	0.19 (2)	10.00
V-W	0 / 0	-38.5	-38.5	0.19 (2)	10.00
W-X	0 / 0	-38.5	-38.5	0.19 (2)	10.00
X-H	0 / 0	-38.5	-38.5	0.19 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-440 / 0	0.04 (1)
K-C	0 / 2807	0.25 (1)
K-O	-563 / 0	0.05 (1)
K-F	0 / 1004	0.09 (1)
I-F	-1440 / 0	0.13 (1)
I-G	0 / 4277	0.38 (1)
B-L	0 / 2476	0.21 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	13-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
O	15-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
P	17-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
Q	19-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
R	21-6-12	-121	-121	—	FRONT	VERT	TOTAL	—	—
S	11-7-8	-1398	-1398	—	TOP	VERT	TOTAL	—	—
T	13-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
U	15-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
V	17-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
W	19-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
X	21-6-12	-63	-81	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 .

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.53/1.00 (D-F:1) , BC=0.66/1.00 (I-K:1) , WB=0.38/1.00 (G-I:1) , SSI=0.40/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

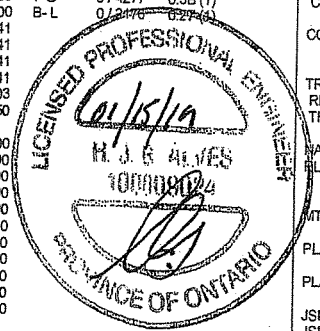
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.82 (J) (INPUT = 1.00)



DWG NO. TAM T1901144
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300403	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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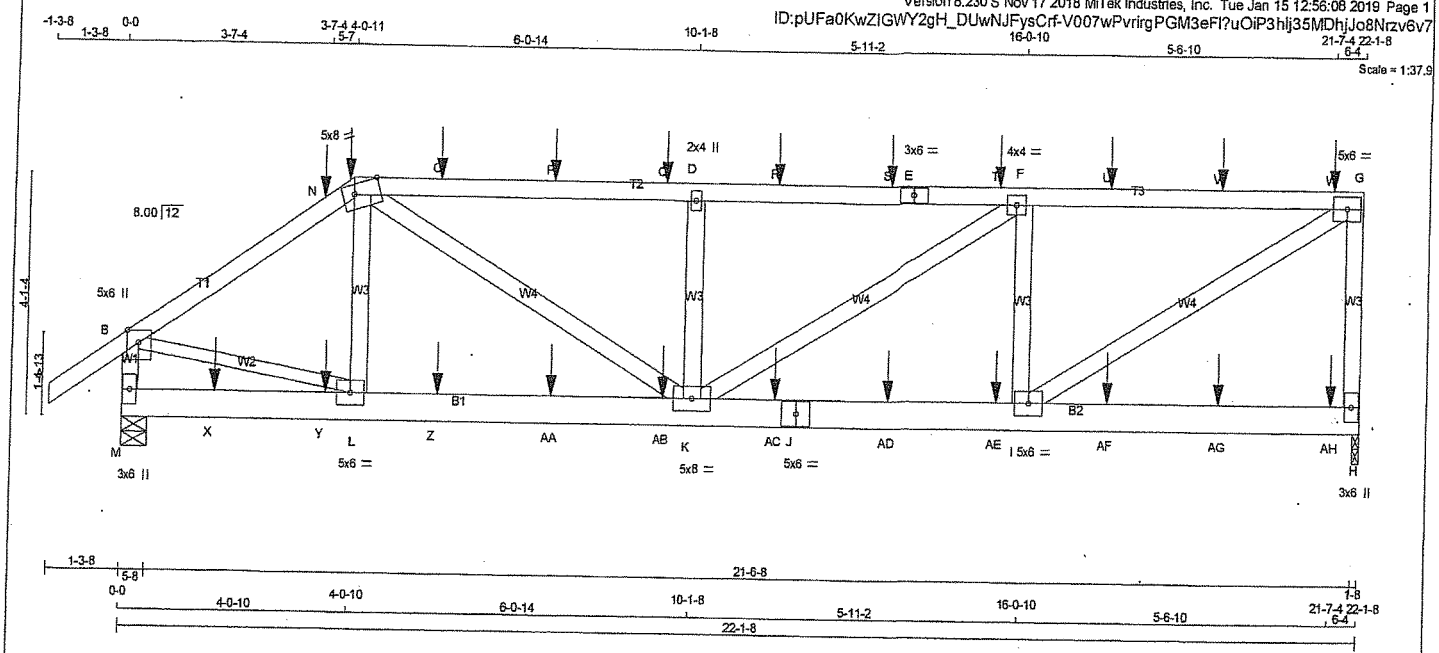

TOTAL WEIGHT = 2 X 110 = 219 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-E	1	12	SIDE(61.0)
E-G	1	12	SIDE(61.0)
G-H	1	12	TOP
M-B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-J	2	12	SIDE(0.0)
J-H	2	12	SIDE(122.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	5.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	8.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
H	2348	0	2348	0	0	1-8	1-8
M	2458	0	2458	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1785	876 / 0	410 / 0	0 / 0	0 / 0	499 / 0	0 / 0
M	1853	957 / 0	389 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B-N	-2716 / 0	-84.9	-84.9	0.19 (1)	5.34
N-C	-2716 / 0	-84.9	-84.9	0.19 (1)	5.34
C-O	-3592 / 0	-84.9	-84.9	0.47 (1)	4.46
O-P	-3592 / 0	-84.9	-84.9	0.47 (1)	4.46
P-O	-3592 / 0	-84.9	-84.9	0.47 (1)	4.46
O-D	-3592 / 0	-84.9	-84.9	0.47 (1)	4.46
D-R	-3592 / 0	-84.9	-84.9	0.47 (1)	4.44
R-S	-3592 / 0	-84.9	-84.9	0.47 (1)	4.44
S-E	-3592 / 0	-84.9	-84.9	0.47 (1)	4.44
E-T	-3592 / 0	-84.9	-84.9	0.47 (1)	4.44
T-F	-3592 / 0	-84.9	-84.9	0.47 (1)	4.44
F-U	-2897 / 0	-84.9	-84.9	0.44 (1)	4.88
U-V	-2897 / 0	-84.9	-84.9	0.44 (1)	4.88
V-W	-2897 / 0	-84.9	-84.9	0.44 (1)	4.88
W-G	-2897 / 0	-84.9	-84.9	0.44 (1)	4.88
H-G	-2156 / 0	0.0	0.0	0.27 (1)	7.57
M-B	-2371 / 0	0.0	0.0	0.13 (1)	7.30
M-X	0 / 0	-38.5	-38.5	0.08 (3)	10.00
X-Y	0 / 0	-38.5	-38.5	0.08 (3)	10.00
Y-L	0 / 0	-38.5	-38.5	0.08 (3)	10.00
L-Z	0 / 2272	-38.5	-38.5	0.21 (1)	10.00
Z-AA	0 / 2272	-38.5	-38.5	0.21 (1)	10.00
AA-AB	0 / 2272	-38.5	-38.5	0.21 (1)	10.00
AB-K	0 / 2272	-38.5	-38.5	0.21 (1)	10.00
K-AC	0 / 2897	-38.5	-38.5	0.26 (2)	10.00
AC-J	0 / 2897	-38.5	-38.5	0.26 (2)	10.00
J-AD	0 / 2897	-38.5	-38.5	0.26 (2)	10.00
AD-AE	0 / 2897	-38.5	-38.5	0.26 (2)	10.00
AE-I	0 / 2897	-38.5	-38.5	0.28 (2)	10.00
I-AF	0 / 0	-38.5	-38.5	0.11 (3)	10.00
AF-AG	0 / 0	-38.5	-38.5	0.11 (3)	10.00
AG-AH	0 / 0	-38.5	-38.5	0.11 (3)	10.00
AH-H	0 / 0	-38.5	-38.5	0.11 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
L-C	-161 / 201	0.02 (3)
C-K	0 / 1547	0.14 (1)
K-D	-881 / 0	0.08 (1)
K-F	0 / 821	0.07 (1)
I-F	-1336 / 0	0.12 (1)
I-G	0 / 3400	0.30 (1)
B-L	0 / 2342	0.29 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-50	-56	—	FRONT	VERT	DEAD	—	—
C	4-0-11	-231	-231	—	FRONT	VERT	SNOW	—	—

LICENSED PROFESSIONAL ENGINEER 01/15/19 H. J. G. ALVES 100090 PROVINCE OF ONTARIO

DWG NO. T1N 7901143
STRUCTURAL COMPONENT ONLY
4/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.47/1.00 (D-F:1) , BC=0.26/1.00 (I-K:2) , WB=0 30/1.00 (G-I:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1 00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300403	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	3-7-4	-125	-125	—	FRONT	VERT	TOTAL	—	—
O	5-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
P	7-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
Q	9-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
R	11-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
S	13-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
T	15-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
U	17-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
V	19-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
W	21-7-4	-122	-122	—	FRONT	VERT	TOTAL	—	—
X	1-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Y	3-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Z	5-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AA	7-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AB	9-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AC	11-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AD	13-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AE	15-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AF	17-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AG	19-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AH	21-7-4	-64	-82	—	FRONT	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100003024
PROVINCE OF ONTARIO

DWG NO. TAM 1790143
STRUCTURAL
FOR CONSENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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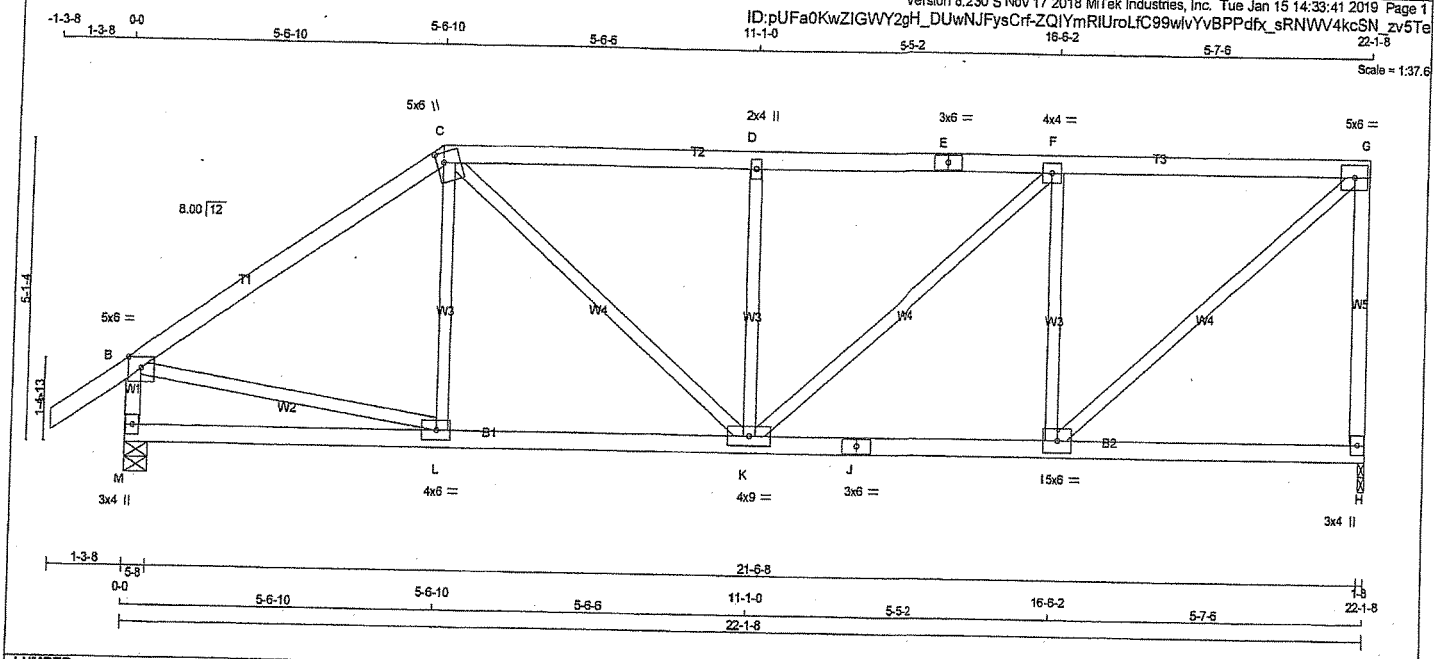


TOTAL WEIGHT = 2 X 92 = 184 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1365	0	1365	0	0	1-8	1-8
M	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
M	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

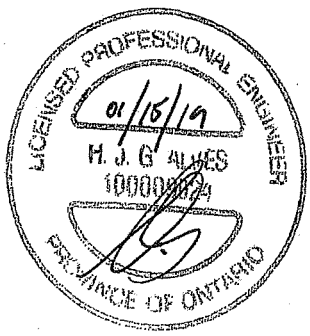
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1457 / 0	-84.9	-84.9	0.55 (1)	4.74
C- D	-1632 / 0	-84.9	-84.9	0.40 (1)	4.73
D- E	-1632 / 0	-84.9	-84.9	0.42 (1)	4.71
E- F	-1632 / 0	-84.9	-84.9	0.42 (1)	4.71
F- G	-1265 / 0	-84.9	-84.9	0.40 (1)	5.22
H- G	-1278 / 0	0.0	0.0	0.56 (1)	7.13
M- B	-1394 / 0	0.0	0.0	0.14 (1)	6.90
M- L	0 / 0	-38.5	-38.5	0.23 (3)	10.00
L- K	0 / 1211	-38.5	-38.5	0.39 (2)	10.00
K- J	0 / 1265	-38.5	-38.5	0.39 (2)	10.00
J- I	0 / 1265	-38.5	-38.5	0.39 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.22 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 188	0.04 (3)
B- L	0 / 1233	0.28 (1)
I- G	0 / 1666	0.37 (1)
C- K	0 / 557	0.13 (1)
I- F	-841 / 0	0.32 (1)
K- D	-502 / 0	0.19 (1)
K- F	0 / 491	0.11 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.56/1.00 (G-H:1) , BC=0.39/1.00 (I-K:2) , WB=0.37/1.00 (G-I:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.47 (J) (INPUT = 1.00)

DWG NO. TAM T1901145
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG ND.
300330	T3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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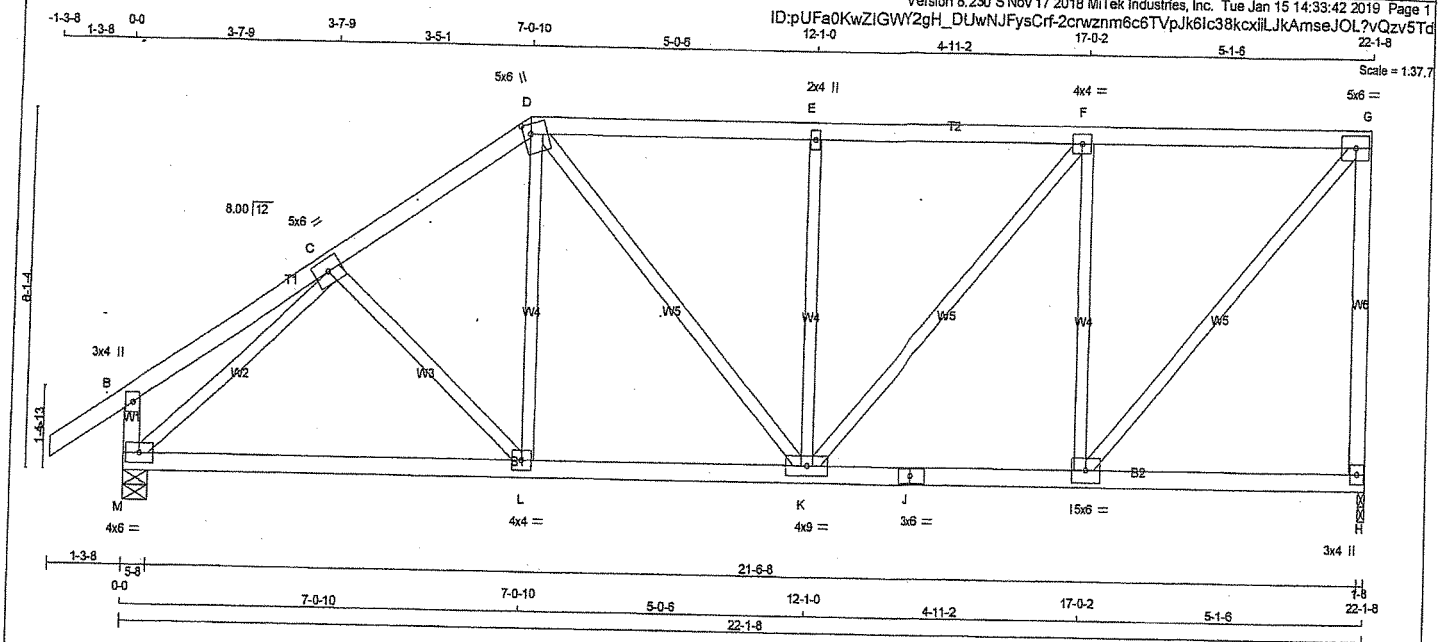


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - O	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
M - J	2x4	ORY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTOREO GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1365	0	1365	0	0	1-8	1-8
M	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
M	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

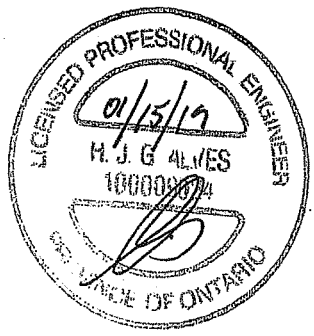
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C- L	-8 / 92	0.02 (3)
B- C	0 / 17	-84.9	-84.9	0.15 (1)	10.00	L- D	0 / 336	0.08 (2)
C- D	-1412 / 0	-84.9	-84.9	0.18 (1)	5.29	M- C	-1625 / 0	0.62 (1)
D- E	-1328 / 0	-84.9	-84.9	0.31 (1)	5.24	I- G	0 / 1479	0.33 (1)
E- F	-1328 / 0	-84.9	-84.9	0.33 (1)	5.22	D- K	0 / 250	0.06 (1)
F- G	-977 / 0	-84.9	-84.9	0.31 (1)	5.88	I- F	-883 / 0	0.52 (1)
H- G	-1287 / 0	0.0	0.0	0.93 (1)	7.11	K- E	-456 / 0	0.27 (1)
M- B	-234 / 0	0.0	0.0	0.02 (1)	7.81	K- F	0 / 543	0.12 (1)
M- L	0 / 1164	-38.5	-38.5	0.47 (2)	10.00			
L- K	0 / 1164	-38.5	-38.5	0.48 (2)	10.00			
K- J	0 / 977	-38.5	-38.5	0.32 (2)	10.00			
J- I	0 / 977	-38.5	-38.5	0.32 (2)	10.00			
I- H	0 / 0	-38.5	-38.5	0.20 (3)	10.00			



TOTAL WEIGHT = 2 X 100 = 200 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.93/1.00 (G-H:1) , BC=0.48/1.00 (K-L:2) , WB=0.62/1.00 (C-M:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.41 (J) (INPUT = 1.00)

DWG NO. TAM T1901146
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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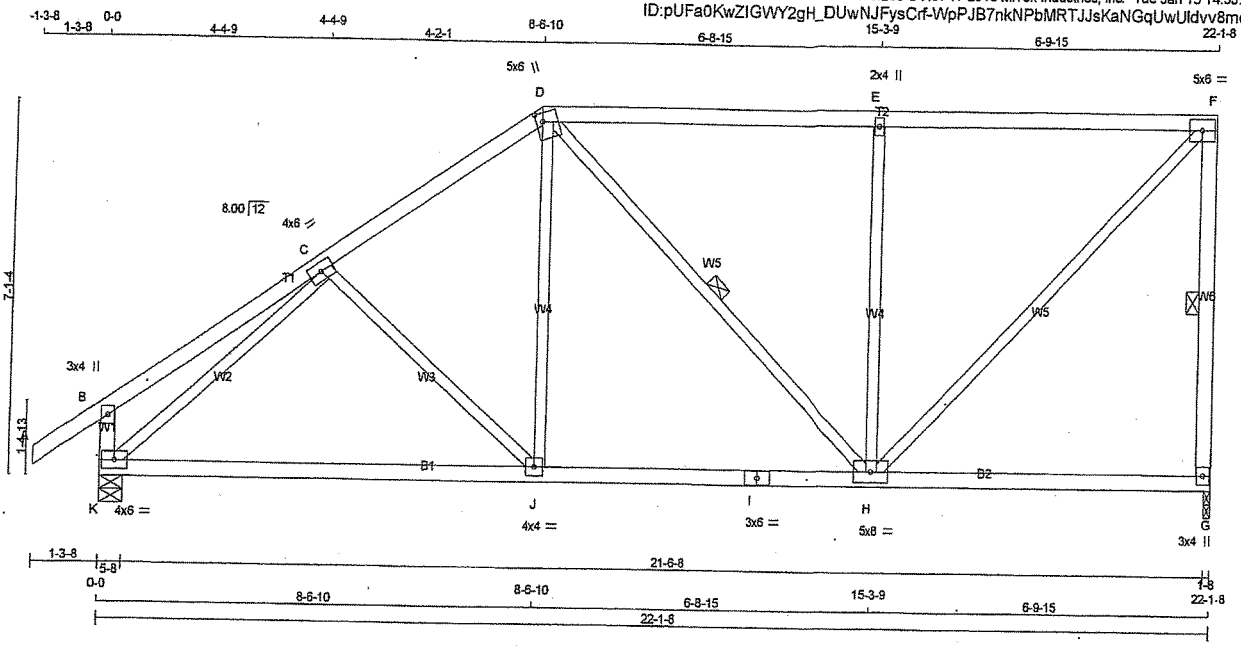

TOTAL WEIGHT = 2 X 97 = 195 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	OESCR
A - D	2x4	DRY	No.2	SPF
O - F	2x4	ORY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1365	0	1365	0	0	1-8	1-8
K	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
K	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00		C- J	-128 / 53	0.08 (1)
B- C	0 / 23	-84.9	-84.9	0.24 (1)	10.00		J- D	0 / 470	0.11 (2)
C- D	-1331 / 0	-84.9	-84.9	0.28 (1)	5.29		K- C	-1606 / 0	0.90 (1)
D- E	-1037 / 0	-84.9	-84.9	0.73 (1)	4.91		H- F	0 / 1465	0.33 (1)
E- F	-1037 / 0	-84.9	-84.9	0.73 (1)	4.91		D- H	-99 / 0	0.06 (2)
G- F	-1255 / 0	0.0	0.0	0.29 (1)	5.74		H- E	-714 / 0	0.62 (1)
K- B	-257 / 0	0.0	0.0	0.03 (1)	7.81				
K- J	0 / 1182	-38.5	-38.5	0.61 (2)	10.00				
J- I	0 / 1093	-38.5	-38.5	0.61 (2)	10.00				
I- H	0 / 1093	-38.5	-38.5	0.61 (2)	10.00				
H- G	0 / 0	-38.5	-38.5	0.28 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 710 (0.37")

CSI: TC=0.73/1.00 (E-F:1) , BC=0.61/1.00 (J-K:2) , WB=0.90/1.00 (C-K:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

DWG NO. TAM T7901147
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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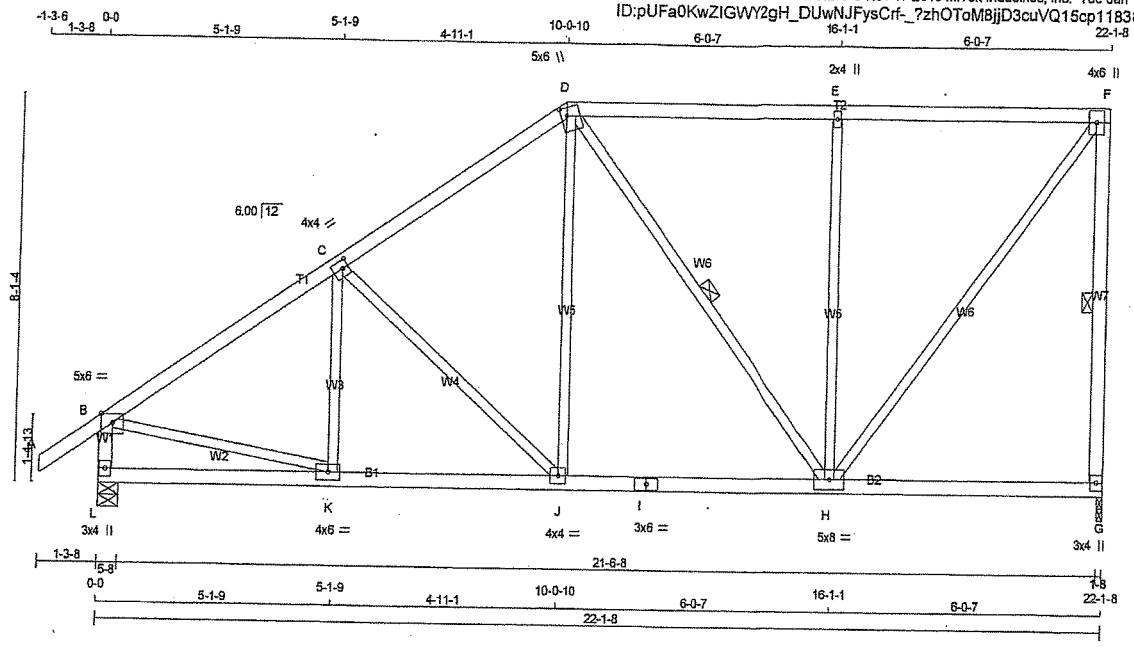


TOTAL WEIGHT = 2 X 105 = 209 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1365	0	1365	0	0	1-8	1-8
L	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
L	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

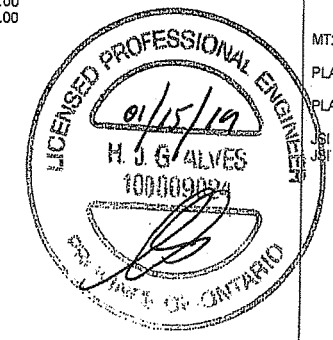
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	K- C	-53 / 139	0.03 (3)
B- C	-1500 / 0	-84.9	-84.9	0.43 (1)	4.86	C- J	-410 / 0	0.35 (1)
C- D	-1191 / 0	-84.9	-84.9	0.40 (1)	5.34	J- D	0 / 491	0.11 (2)
D- E	-836 / 0	-84.9	-84.9	0.55 (1)	5.76	D- H	-222 / 0	0.15 (1)
E- F	-836 / 0	-84.9	-84.9	0.55 (1)	5.76	H- E	-634 / 0	0.80 (1)
G- F	-1271 / 0	0.0	0.0	0.38 (1)	5.71	H- F	0 / 1362	0.31 (1)
L- B	-1400 / 0	0.0	0.0	0.14 (1)	6.88	B- K	0 / 1299	0.29 (1)
L- K	0 / 0	-38.5	-38.5	0.18 (3)	10.00			
K- J	0 / 1271	-38.5	-38.5	0.34 (2)	10.00			
J- I	0 / 972	-38.5	-38.5	0.39 (2)	10.00			
I- H	0 / 972	-38.5	-38.5	0.39 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.27 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.55/1.00 (E-F:1) , BC=0.39/1.00 (H-J:2) , WB=0.80/1.00 (E-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.39 (I) (INPUT = 1.00)

DWG NO. TAM T1901148
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T6	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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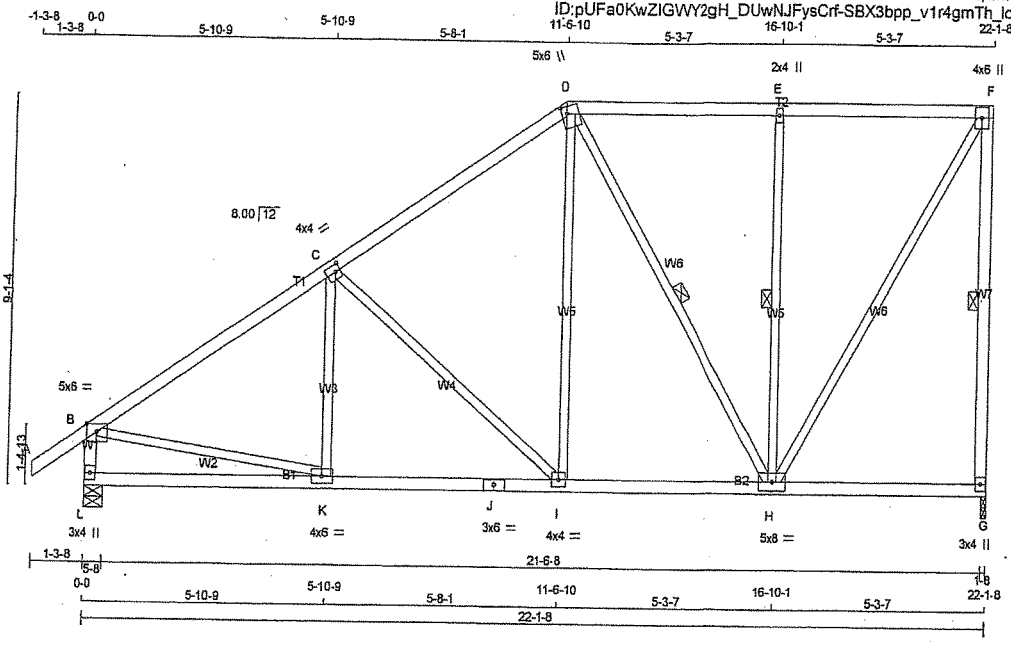


TOTAL WEIGHT = 9 X 110 = 992 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - O	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1365	0	1365	0	0	1-8	1-8
L	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
L	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

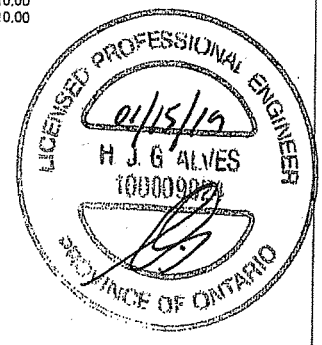
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	K- C	0 / 207	0.05 (3)	
B- C	-1497 / 0	-84.9	-84.9	0.42 (1)	4.91	C- I	-546 / 0	0.64 (1)	
C- D	-1068 / 0	-84.9	-84.9	0.38 (1)	5.62	I- D	0 / 565	0.13 (2)	
D- E	-670 / 0	-84.9	-84.9	0.30 (1)	6.25	D- H	-383 / 0	0.29 (1)	
E- F	-670 / 0	-84.9	-84.9	0.30 (1)	6.25	H- E	-554 / 0	0.30 (1)	
G- F	-1282 / 0	0.0	0.0	0.49 (1)	5.69	H- F	0 / 1296	0.29 (1)	
L- B	-1390 / 0	0.0	0.0	0.14 (1)	6.90	B- K	0 / 1294	0.29 (1)	
L- K	0 / 0	-38.5	-38.5	0.25 (3)	10.00				
K- J	0 / 1272	-38.5	-38.5	0.41 (2)	10.00				
J- I	0 / 1272	-38.5	-38.5	0.41 (2)	10.00				
I- H	0 / 866	-38.5	-38.5	0.31 (2)	10.00				
H- G	0 / 0	-38.5	-38.5	0.20 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	46.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.49/1.00 (F-G:1) , BC=0.41/1.00 (I-K:2) , WB=0.64/1.00 (C-I:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.46 (J) (INPUT = 1.00)

DWG NO. TAM T1901149
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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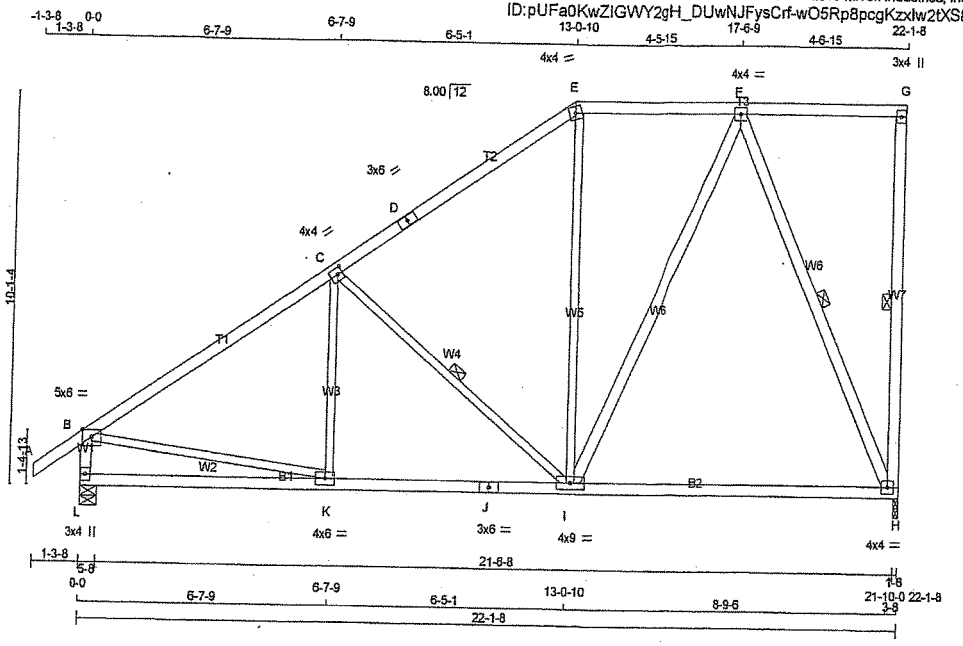


TOTAL WEIGHT = 2 X 115 = 230 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION 0OWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
H	1365	0	1365	0	0	1-8	1-8
L	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	0EAD	SOIL
H	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
L	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING 0IRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

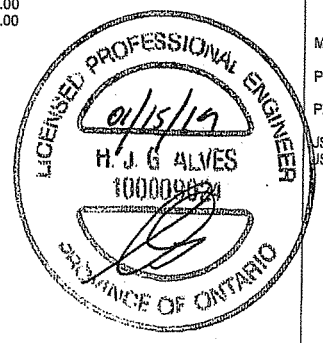
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1472 / 0	-84.9	-84.9	0.55 (1)	4.74
C- D	-982 / 0	-84.9	-84.9	0.49 (1)	5.64
D- E	-962 / 0	-84.9	-84.9	0.49 (1)	5.64
E- F	-770 / 0	-84.9	-84.9	0.23 (1)	6.25
F- G	0 / 0	-84.9	-84.9	0.29 (1)	10.00
H- G	-148 / 0	0.0	0.0	0.07 (1)	6.25
L- B	-1372 / 0	0.0	0.0	0.14 (1)	6.94
L- K	0 / 0	-38.5	-38.5	0.27 (3)	10.00
K- J	0 / 1255	-38.5	-38.5	0.67 (2)	10.00
J- I	0 / 1255	-38.5	-38.5	0.67 (2)	10.00
I- H	0 / 502	-38.5	-38.5	0.56 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 215	0.05 (3)
C- I	-637 / 0	0.32 (1)
I- E	0 / 217	0.05 (3)
B- K	0 / 1272	0.29 (1)
I- F	0 / 656	0.11 (1)
F- H	-1187 / 0	0.74 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNE0 FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 642 (0.41")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.67/1.00 (I-K:2) , WB=0.74/1.00 (F-H:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.48 (J) (INPUT = 1.00)

DWG NO. TAM T90 1/50
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T11	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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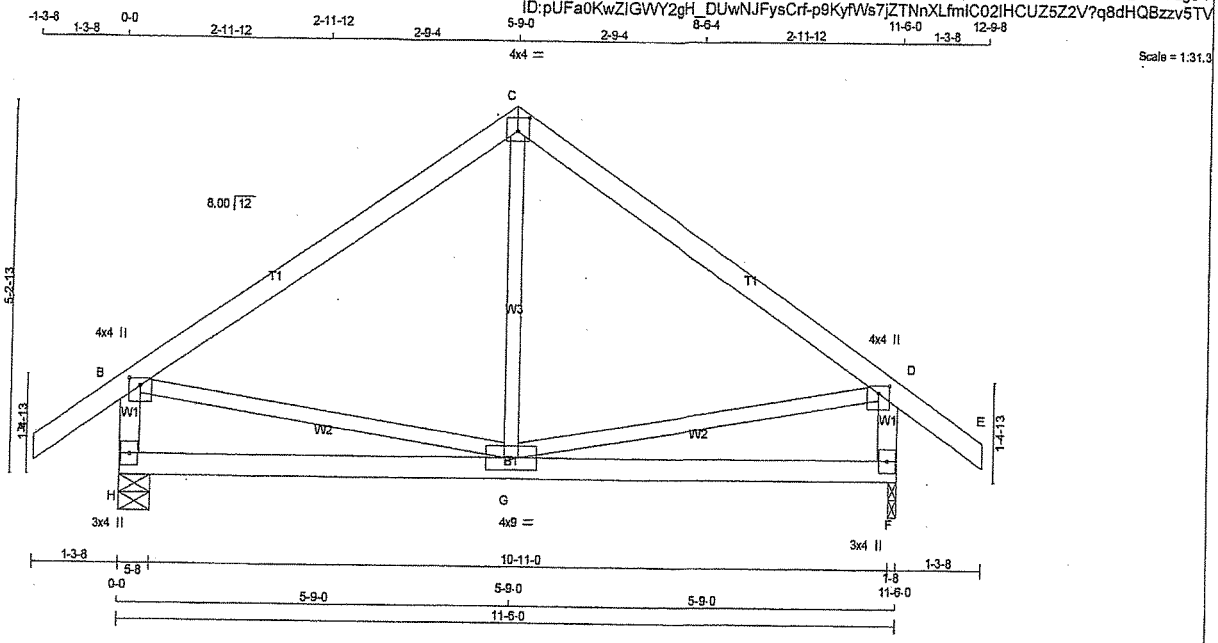


TOTAL WEIGHT = 3 X 48 = 143 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
H	826	0	826	0	0	5-8	5-8
F	826	0	826	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	618	332 / 0	121 / 0	0 / 0	0 / 0	166 / 0	0 / 0
F	618	332 / 0	121 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

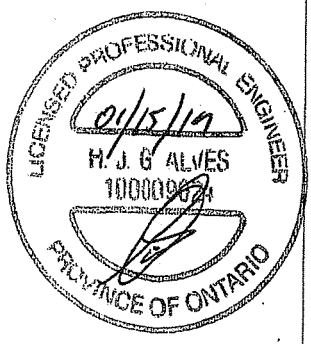
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00	G- C	0 / 255	0.06 (3)
B- C	-536 / 0	-84.9	-84.9	0.36 (1)		6.25	B- G	0 / 454	0.10 (1)
C- D	-536 / 0	-84.9	-84.9	0.36 (1)		6.25	G- D	0 / 454	0.10 (1)
D- E	0 / 32	-84.9	-84.9	0.11 (1)		10.00			
H- B	-742 / 0	0.0	0.0	0.08 (1)		7.81			
F- D	-742 / 0	0.0	0.0	0.08 (1)		7.81			
H- G	0 / 0	-38.5	-38.5	0.29 (3)		10.00			
G- F	0 / 0	-38.5	-38.5	0.29 (3)		10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.29/1.00 (G-H:3) , WB=0.10/1.00 (B-G:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

DWG NO. TAM T1901454
STRUCTURAL
COMPONENT ONLY

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300403	G11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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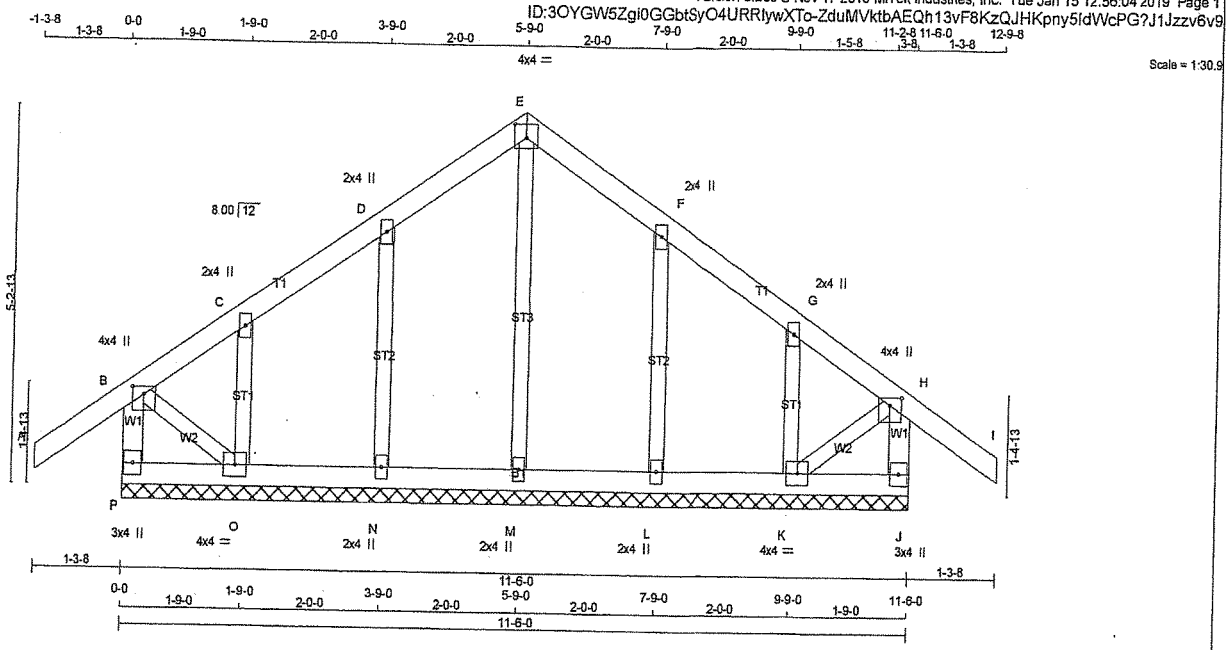


TOTAL WEIGHT = 50 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMWW+p	MT20	4.0	4.0	1.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

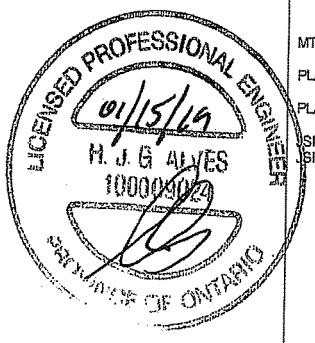
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	-41 / 0	-84.9	-84.9	0.11 (1)		6.25
C- D	-4 / 0	-84.9	-84.9	0.05 (1)		10.00
D- E	-18 / 0	-84.9	-84.9	0.05 (1)		6.25
E- F	-18 / 0	-84.9	-84.9	0.05 (1)		6.25
F- G	-4 / 0	-84.9	-84.9	0.05 (1)		10.00
G- H	-41 / 0	-84.9	-84.9	0.11 (1)		6.25
H- I	0 / 32	-84.9	-84.9	0.11 (1)		10.00
P- B	-248 / 0	0.0	0.0	0.03 (1)		7.81
J- H	-248 / 0	0.0	0.0	0.03 (1)		7.81
P- O	0 / 0	-38.5	-38.5	0.03 (3)		10.00
O- N	0 / 11	-38.5	-38.5	0.03 (3)		10.00
N- M	0 / 6	-38.5	-38.5	0.02 (3)		10.00
M- L	0 / 8	-38.5	-38.5	0.02 (3)		10.00
L- K	0 / 11	-38.5	-38.5	0.03 (3)		10.00
K- J	0 / 0	-38.5	-38.5	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-130 / 0	0.05 (1)
N- D	-205 / 0	0.05 (1)
O- C	-95 / 0	0.02 (1)
L- F	-205 / 0	0.05 (1)
K- G	-95 / 0	0.02 (1)
B- O	0 / 17	0.00 (1)
K- H	0 / 17	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (K-L:3) , WB=0.05/1.00 (E-M:1) , SSI=0.08/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.17 (B) (INPUT = 0.90)
SI METAL= 0.10 (D) (INPUT = 1.00)

AHJ NO. TAM 11901141
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T12Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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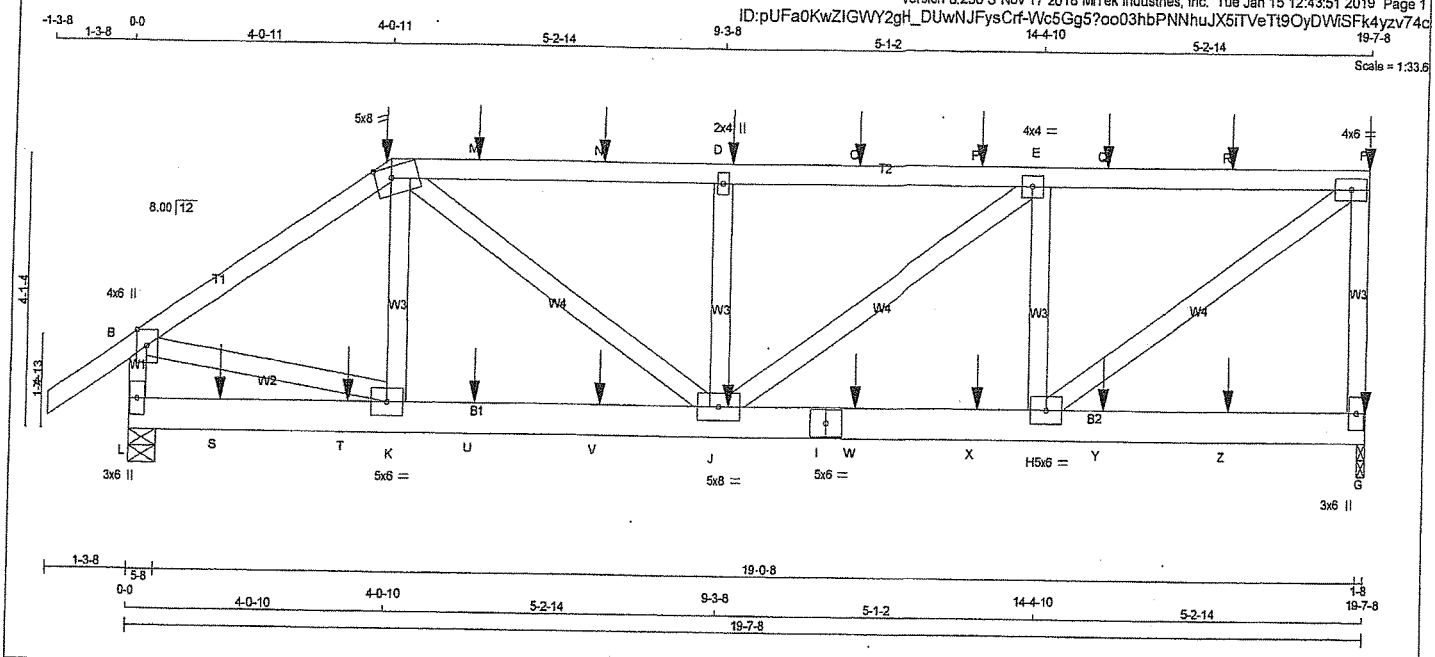

TOTAL WEIGHT = 2 X 101 = 202 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	ORY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x6	ORY	No.2	SPF
I - G	2x6	ORY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-G	1	12	TOP
L-B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L-I	2	12	SIDE(183.1)
I-G	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
O	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DWG NO. TAM T1901128
STRUCTURAL
COMPONENT ONLY
1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2129	0	2129	0	0	1-8	1-8
L	2106	0	2106	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1620	792 / 0	374 / 0	0 / 0	0 / 0	453 / 0	0 / 0
L	1593	807 / 0	346 / 0	0 / 0	0 / 0	439 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.06 (1)		10.00
B-C	-2282 / 0	-84.9	-84.9	0.16 (1)		5.75
C-M	-2791 / 0	-84.9	-84.9	0.30 (1)		5.15
M-N	-2791 / 0	-84.9	-84.9	0.30 (1)		5.15
N-D	-2791 / 0	-84.9	-84.9	0.30 (1)		5.15
D-O	-2791 / 0	-84.9	-84.9	0.31 (1)		5.12
O-P	-2791 / 0	-84.9	-84.9	0.31 (1)		5.12
P-E	-2791 / 0	-84.9	-84.9	0.31 (1)		5.12
E-Q	-2201 / 0	-84.9	-84.9	0.30 (1)		5.63
Q-R	-2201 / 0	-84.9	-84.9	0.30 (1)		5.63
R-F	-2201 / 0	-84.9	-84.9	0.30 (1)		5.63
G-F	-1939 / 0	0.0	0.0	0.24 (1)		7.81
L-B	-2009 / 0	0.0	0.0	0.11 (1)		7.77
L-S	0 / 0	-38.5	-38.5	0.07 (3)		10.00
S-T	0 / 0	-38.5	-38.5	0.07 (3)		10.00
T-K	0 / 0	-38.5	-38.5	0.07 (3)		10.00
K-U	0 / 1892	-38.5	-38.5	0.17 (2)		10.00
U-V	0 / 1892	-38.5	-38.5	0.17 (2)		10.00
V-J	0 / 1892	-38.5	-38.5	0.17 (2)		10.00
J-I	0 / 2201	-38.5	-38.5	0.20 (2)		10.00
I-W	0 / 2201	-38.5	-38.5	0.20 (2)		10.00
W-X	0 / 2201	-38.5	-38.5	0.20 (2)		10.00
X-H	0 / 2201	-38.5	-38.5	0.20 (2)		10.00
H-Y	0 / 0	-38.5	-38.5	0.08 (3)		10.00
Y-Z	0 / 0	-38.5	-38.5	0.08 (3)		10.00
Z-G	0 / 0	-38.5	-38.5	0.08 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	-99 / 206	0.02 (3)
J-O	0 / 1102	0.10 (1)
J-E	-762 / 0	0.07 (1)
H-E	0 / 731	0.06 (1)
H-F	-1191 / 0	0.11 (1)
B-K	0 / 2702	0.24 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-50	-56	—	FRONT	VERT	DEAD	—	—
C	4-0-11	-231	-231	—	FRONT	VERT	SNOW	—	—
O	9-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
F	19-7-8	-137	-137	—	FRONT	VERT	TOTAL	—	—
G	19-7-8	-71	-90	—	FRONT	VERT	TOTAL	—	—
J	9-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
M	5-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
N	7-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
O	11-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
P	13-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100090874
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	OL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.31/1.00 (D-E:1) , BC=0.20/1.00 (H-J:2) , WB=0.24/1.00 (F-H:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T12Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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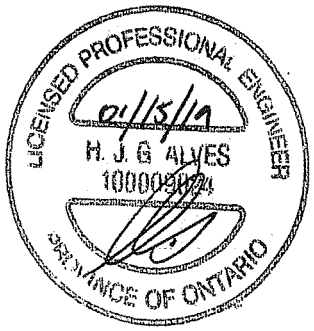
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	15-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
R	17-5-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
S	1-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
T	3-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
U	5-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
V	7-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
W	11-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
X	13-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Y	15-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Z	17-5-4	-55	-70	—	FRONT	VERT	TOTAL	—	—



DWG NO. TAM T1901128
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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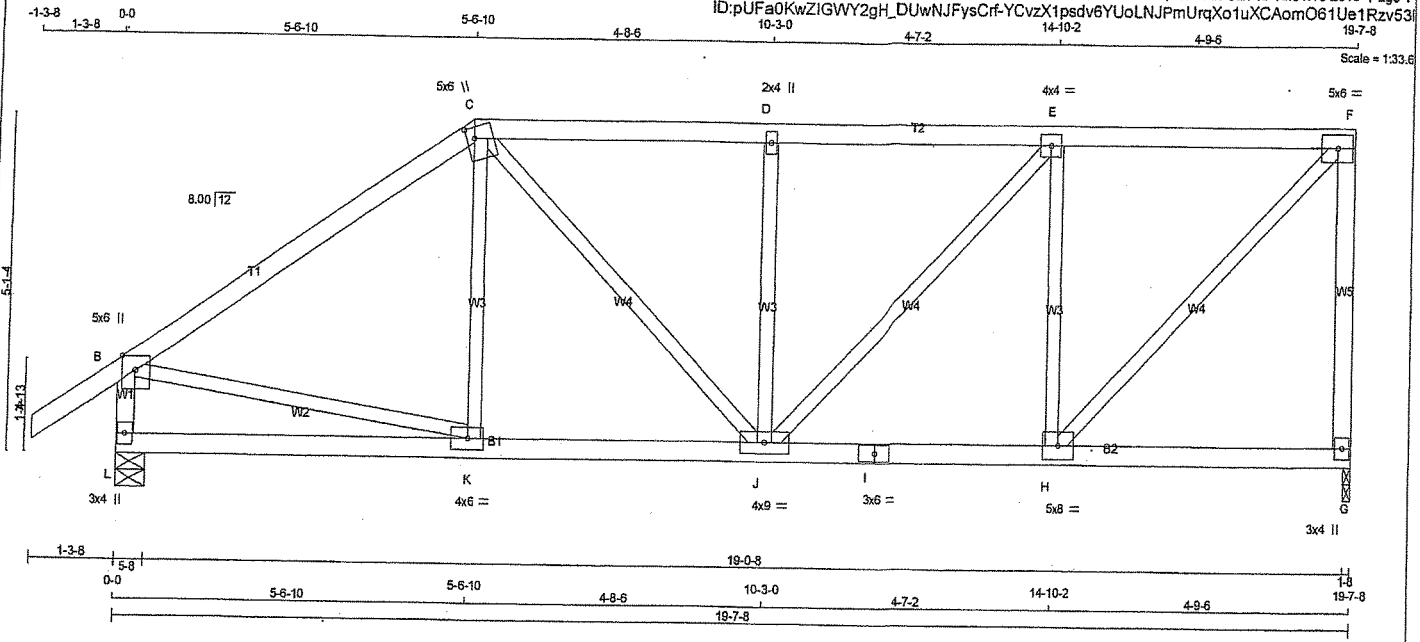


TOTAL WEIGHT = 2 X 85 = 170 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	ORY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
L	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

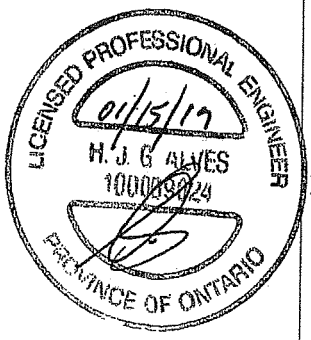
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	-1239 / 0	-84.9	-84.9	0.53 (1)		5.07
C- D	-1275 / 0	-84.9	-84.9	0.27 (1)		5.39
D- E	-1275 / 0	-84.9	-84.9	0.28 (1)		5.36
E- F	-965 / 0	-84.9	-84.9	0.27 (1)		5.98
G- F	-1137 / 0	0.0	0.0	0.50 (1)		7.45
L- B	-1238 / 0	0.0	0.0	0.13 (1)		7.22
L- K	0 / 0	-38.5	-38.5	0.21 (3)		10.00
K- J	0 / 1031	-38.5	-38.5	0.35 (2)		10.00
J- I	0 / 965	-38.5	-38.5	0.29 (2)		10.00
I- H	0 / 965	-38.5	-38.5	0.29 (2)		10.00
H- G	0 / 0	-38.5	-38.5	0.16 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 192	0.05 (3)
B- K	0 / 1049	0.24 (1)
H- F	0 / 1369	0.31 (1)
C- J	0 / 349	0.08 (1)
H- E	-762 / 0	0.29 (1)
J- O	-424 / 0	0.16 (1)
J- E	0 / 449	0.10 (1)



DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.53/1.00 (B-C:1) , BC=0.35/1.00 (J-K:2) , WB=0.31/1.00 (F-H:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

PERMIT NO. TAM 19011156
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T14	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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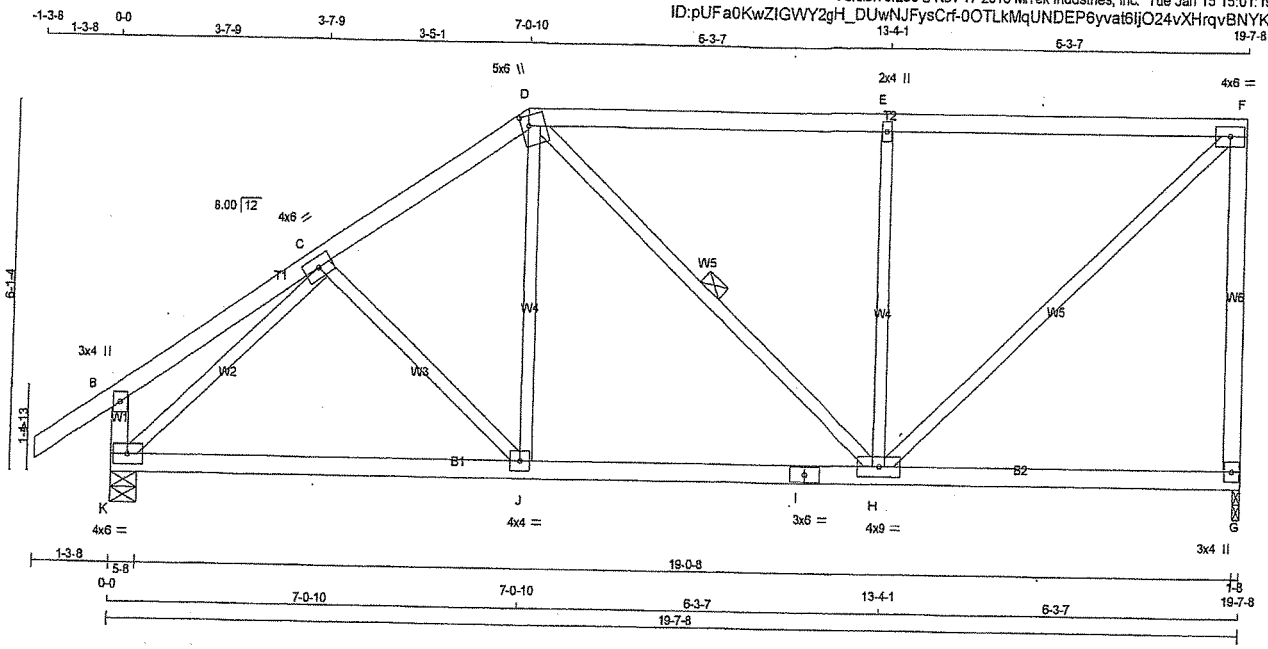


TOTAL WEIGHT = 2 X 85 = 171 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT2D	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT2D	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

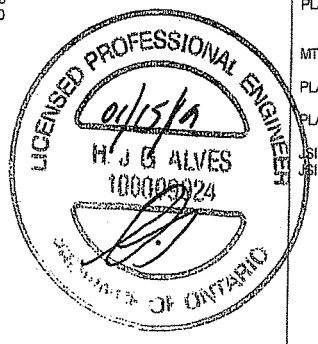
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 18	-84.9	-84.9	0.16 (1)	10.00
C- D	-1186 / 0	-84.9	-84.9	0.18 (1)	5.66
D- E	-985 / 0	-84.9	-84.9	0.61 (1)	5.29
E- F	-985 / 0	-84.9	-84.9	0.61 (1)	5.29
G- F	-1110 / 0	0.0	0.0	0.80 (1)	7.52
K- B	-234 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 1007	-38.5	-38.5	0.45 (2)	10.00
J- I	0 / 976	-38.5	-38.5	0.45 (2)	10.00
I- H	0 / 976	-38.5	-38.5	0.45 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.25 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-50 / 71	0.02 (1)
J- D	0 / 364	0.08 (2)
D- H	-36 / 13	0.02 (3)
H- E	-660 / 0	0.39 (1)
H- F	0 / 1338	0.30 (1)
K- C	-1405 / 0	0.54 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.80/1.00 (F-G:1) , BC=0.45/1.00 (H-J:2) , WB=0.54/1.00 (C-K:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (F) (INPUT = 0.90)
SI METAL= 0.34 (C) (INPUT = 1.00)

DWG NO. TAM T1901157
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T15	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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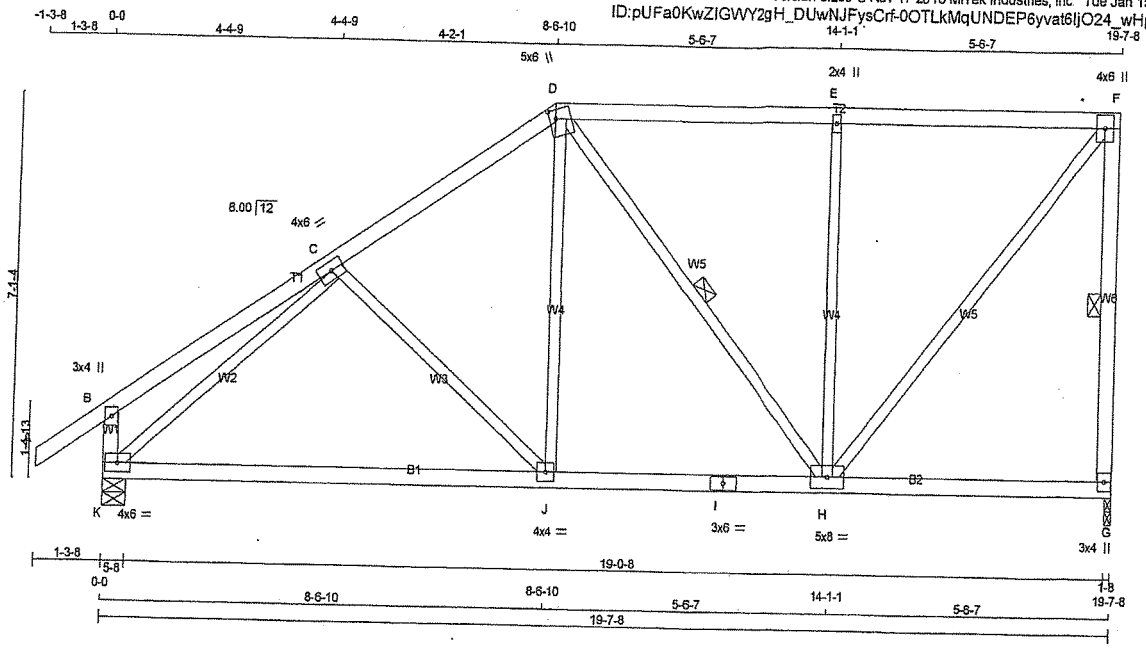

TOTAL WEIGHT = 2 X 90 = 181 lb [M]

LUMBER

N. L. G. A. RULES

CHOROS	SIZE		LUMBER	DESCR.
A - O	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 23	-84.9	-84.9	0.24 (1)	10.00
C- D	-1096 / 0	-84.9	-84.9	0.27 (1)	5.70
D- E	-766 / 0	-84.9	-84.9	0.45 (1)	6.15
E- F	-766 / 0	-84.9	-84.9	0.46 (1)	6.15
G- F	-1116 / 0	0.0	0.0	0.25 (1)	6.00
K- B	-256 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 1015	-38.5	-38.5	0.58 (2)	10.00
J- I	0 / 899	-38.5	-38.5	0.57 (2)	10.00
I- H	0 / 899	-38.5	-38.5	0.57 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-165 / 34	0.10 (1)
J- D	0 / 482	0.11 (2)
D- H	-218 / 0	0.11 (2)
H- E	-581 / 0	0.51 (1)
H- F	0 / 1210	0.27 (1)
K- C	-1379 / 0	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 635 (0.37")

CSI: TC=0.46/1.00 (E-F:1) , BC=0.58/1.00 (J-K:2) , WB=0.77/1.00 (C-K:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.75 (F) (INPUT = 0.90)
USI METAL= 0.35 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H J G ALVES
100[illegible]024
PROVINCE OF ONTARIO

DWG NO. TAM 190 1158
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T16	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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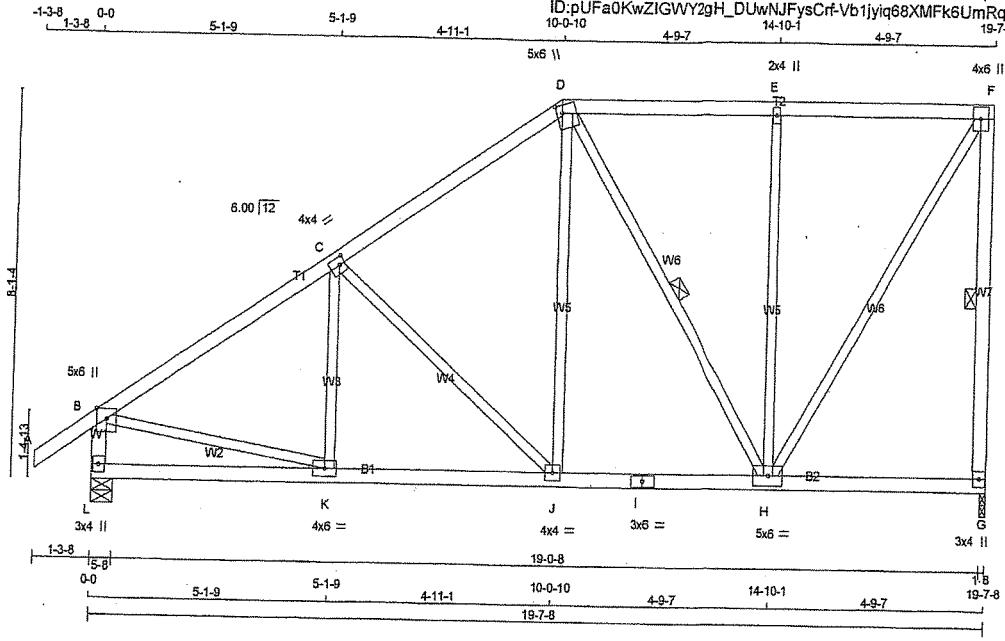


TOTAL WEIGHT = 2 X 98 = 196 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
L	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

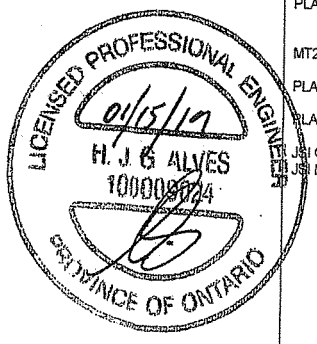
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1291 / 0	-84.9	-84.9	0.30 (1)	5.35
C- D	-948 / 0	-84.9	-84.9	0.28 (1)	6.02
D- E	-604 / 0	-84.9	-84.9	0.25 (1)	6.25
E- F	-604 / 0	-84.9	-84.9	0.25 (1)	6.25
G- F	-1135 / 0	0.0	0.0	0.34 (1)	5.96
L- B	-1247 / 0	0.0	0.0	0.13 (1)	7.19
L- K	0 / 0	-38.5	-38.5	0.19 (3)	10.00
K- J	0 / 1097	-38.5	-38.5	0.33 (2)	10.00
J- I	0 / 770	-38.5	-38.5	0.26 (2)	10.00
I- H	0 / 770	-38.5	-38.5	0.26 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-10 / 166	0.04 (3)
C- J	-447 / 0	0.38 (1)
J- D	0 / 484	0.11 (2)
D- H	-319 / 0	0.18 (1)
H- E	-501 / 0	0.53 (1)
H- F	0 / 1149	0.26 (1)
B- K	0 / 1121	0.25 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.34/1.00 (F-G:1) , BC=0.33/1.00 (J-K:2) , WB=0.63/1.00 (E-H:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1901159
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T17	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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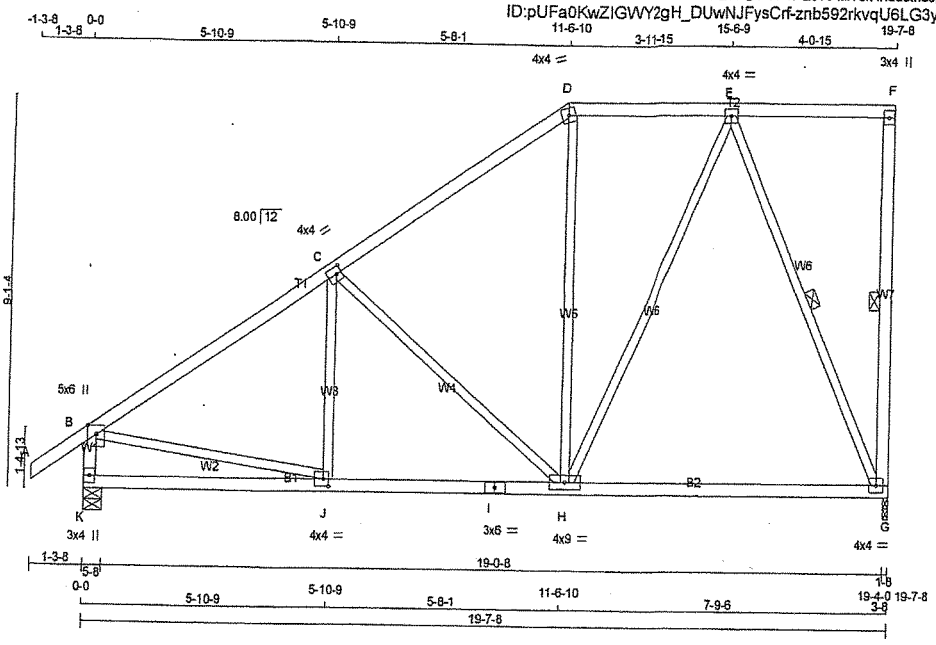


TOTAL WEIGHT = 9 X 97 = 870 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

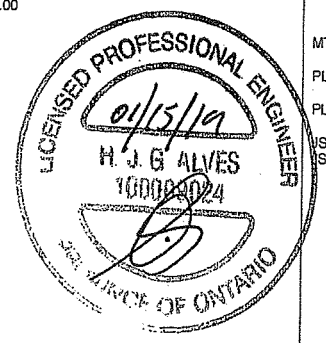
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	-1272 / 0	-84.9	-84.9	0.40 (1)		5.24
C- D	-843 / 0	-84.9	-84.9	0.38 (1)		6.14
D- E	-674 / 0	-84.9	-84.9	0.18 (1)		6.25
E- F	0 / 0	-84.9	-84.9	0.23 (1)		10.00
G- F	-132 / 0	0.0	0.0	0.05 (1)		6.25
K- B	-1230 / 0	0.0	0.0	0.13 (1)		7.23
K- J	0 / 0	-38.5	-38.5	0.22 (3)		10.00
J- I	0 / 1085	-38.5	-38.5	0.54 (2)		10.00
I- H	0 / 1085	-38.5	-38.5	0.54 (2)		10.00
H- G	0 / 441	-38.5	-38.5	0.44 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 180	0.04 (3)
C- H	-545 / 0	0.64 (1)
H- D	0 / 189	0.04 (3)
B- J	0 / 1103	0.25 (1)
H- E	0 / 579	0.13 (1)
E- G	-1050 / 0	0.70 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 893 (0.26")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.54/1.00 (H-J:2) , WB=0.70/1.00 (E-G:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (J) (INPUT = 0.90)
SI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1901160
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T18	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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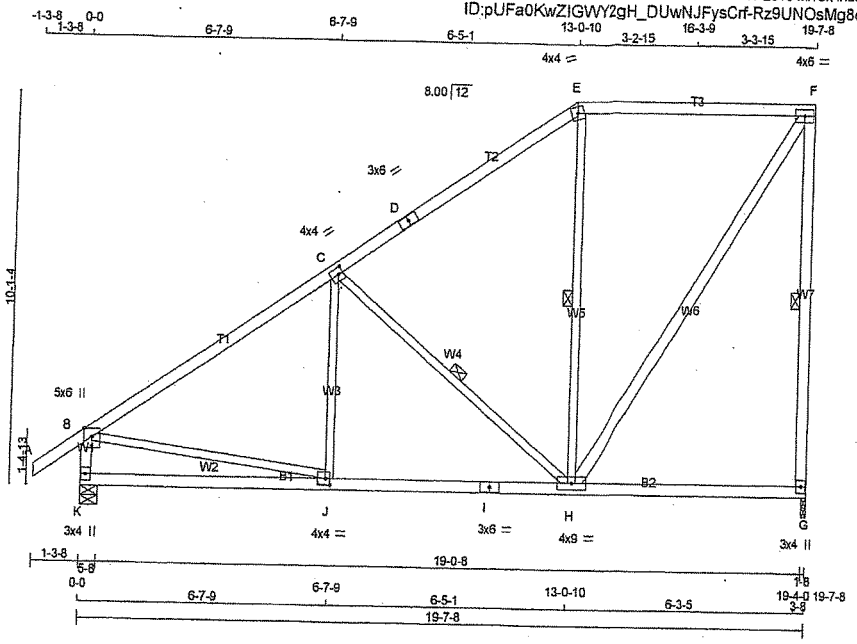


TOTAL WEIGHT = 2 X 98 = 197 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	8.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1 LATERAL BRACE(S) AT 1/ 2 LENGTH DF F-G, C-H, E-H.

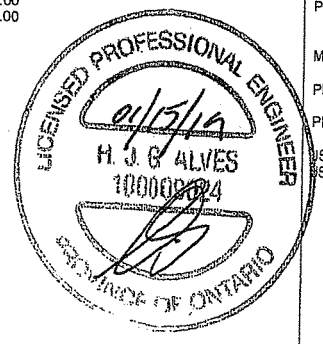
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	-1255 / 0	-84.9	-84.9	0.53 (1)		5.06
C- D	-700 / 0	-84.9	-84.9	0.48 (1)		6.25
D- E	-700 / 0	-84.9	-84.9	0.48 (1)		6.25
E- F	-549 / 0	-84.9	-84.9	0.47 (1)		6.25
G- F	-1110 / 0	0.0	0.0	0.55 (1)		6.01
K- B	-1223 / 0	0.0	0.0	0.13 (1)		7.25
K- J	0 / 0	-38.5	-38.5	0.30 (3)		10.00
J- I	0 / 1075	-38.5	-38.5	0.43 (2)		10.00
I- H	0 / 1075	-38.5	-38.5	0.43 (2)		10.00
H- G	0 / 0	-38.5	-38.5	0.29 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 264	0.06 (3)
C- H	-687 / 0	0.34 (1)
H- E	-115 / 112	0.08 (1)
B- J	0 / 1090	0.25 (1)
H- F	0 / 997	0.16 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.55/1.00 (F-G:1) , BC=0.43/1.00 (H-J:2) , WB=0.34/1.00 (C-H:1) , SSI=0.23/1.00 (B-C:1)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.87 (J) (INPUT = 0.90)
(SI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM 1901161
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T20A	4	1	TRUSS DESC.	

Temarack Roof Truss, Burlington

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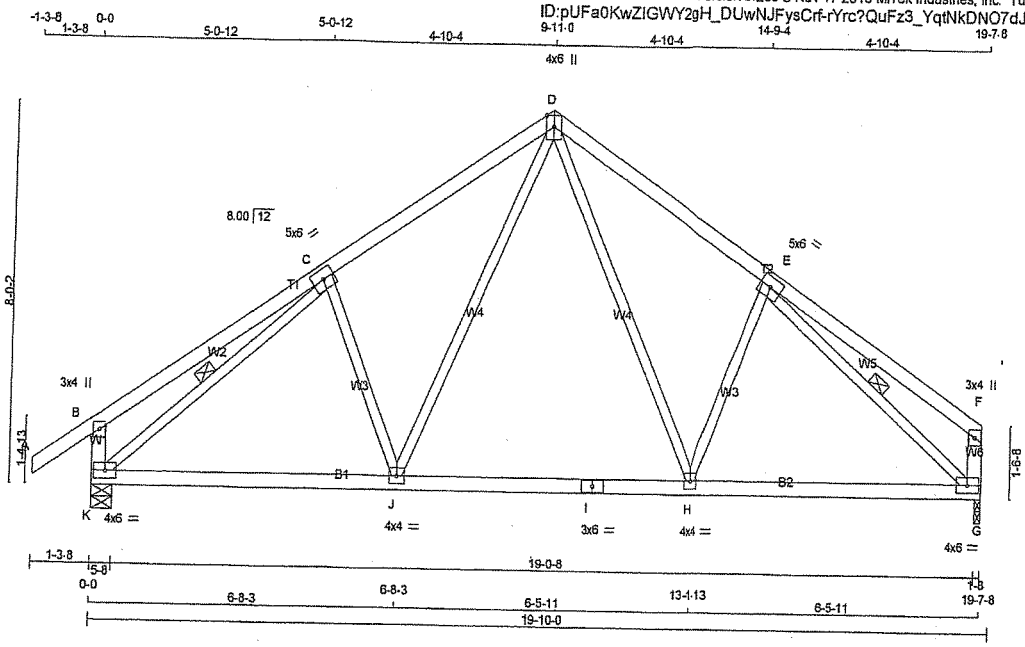


TOTAL WEIGHT = 4 X 84 = 337 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG	REQRD BRG
K	1327	0	1327	0	0	5-8	5-8
G	1210	0	1210	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

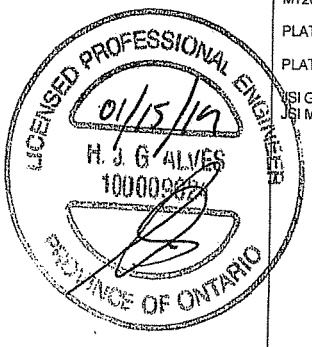
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 27	-84.9	-84.9	0.34 (1)	10.00
C-D	-1203 / 0	-84.9	-84.9	0.28 (1)	5.52
D-E	-1180 / 0	-84.9	-84.9	0.27 (1)	5.57
E-F	0 / 27	-84.9	-84.9	0.32 (1)	10.00
K-B	-277 / 0	0.0	0.0	0.03 (1)	7.81
G-F	-151 / 0	0.0	0.0	0.02 (1)	7.81
K-J	0 / 1068	-38.5	-38.5	0.43 (2)	10.00
J-I	0 / 766	-38.5	-38.5	0.39 (2)	10.00
I-H	0 / 766	-38.5	-38.5	0.39 (2)	10.00
H-G	0 / 1036	-38.5	-38.5	0.41 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-H	0 / 500	0.11 (1)
H-E	-205 / 38	0.08 (1)
J-D	0 / 546	0.12 (1)
C-J	-242 / 22	0.09 (1)
K-C	-1427 / 0	0.42 (1)
E-G	-1411 / 0	0.40 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.34/1.00 (B-C:1) , BC=0.43/1.00 (J-K:2) , WB=0.42/1.00 (C-K:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

SI GRIP= 0.77 (K) (INPUT = 0.90)
SI METAL= 0.37 (C) (INPUT = 1.00)

DWG NO. TAM T1901165
STRUCTURAL COMPONENT ONLY

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG ND.
300399	G56	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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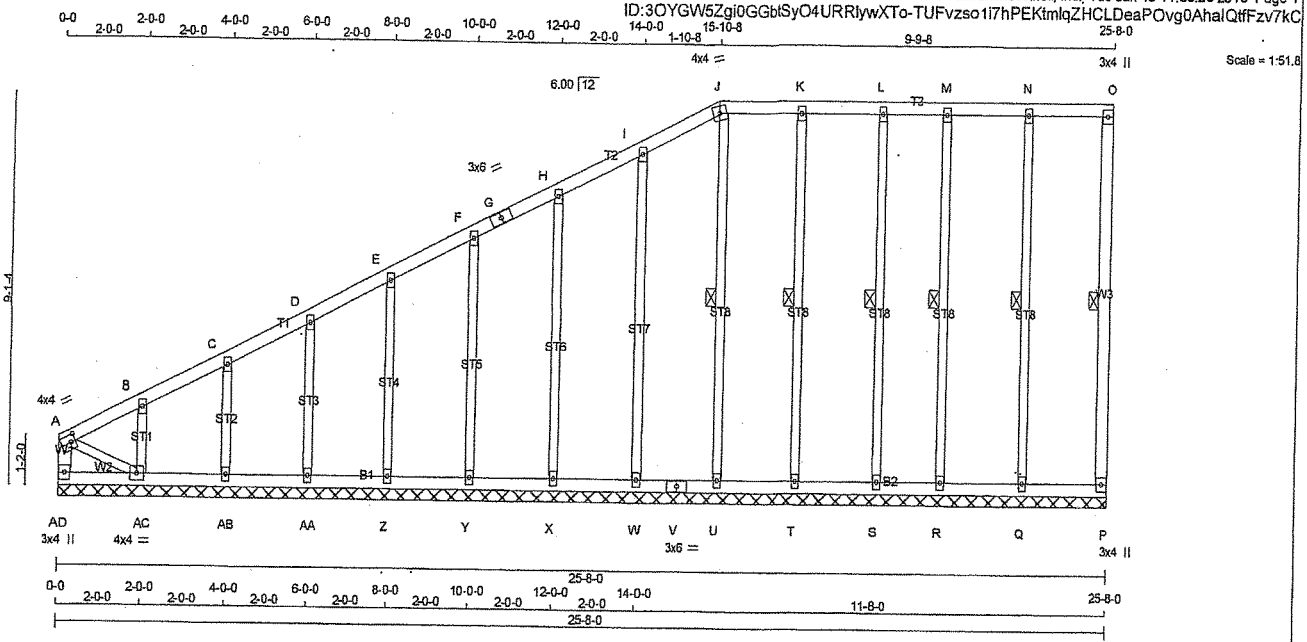


TOTAL WEIGHT = 2 X 134 = 267 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
AD- A	2x4	DRY	No.2	SPF
AD- V	2x4	DRY	No.2	SPF
V - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B, C, D, E, F, H, I, K, L, M, N						
B	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
O	TMV+p	MT20	3.0	4.0		
P	BMV1+p	MT20	3.0	4.0		
Q, R, S, T, U, W, X, Y, Z, AA, AB						
O	BMW1+w	MT20	2.0	4.0		
V	BS-t	MT20	3.0	6.0		
AC	BMWW1-t	MT20	4.0	4.0		
AD	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF O-P, J-U, K-T, L-S, M-R, N-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

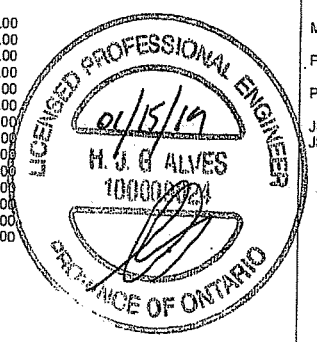
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	-17 / 0	-84.9	-84.9	0.05 (1)	6.25
B- C	-19 / 0	-84.9	-84.9	0.05 (1)	6.25
C- D	-13 / 0	-84.9	-84.9	0.04 (1)	6.25
D- E	-10 / 0	-84.9	-84.9	0.04 (1)	6.25
E- F	-7 / 0	-84.9	-84.9	0.04 (1)	10.00
F- G	-5 / 0	-84.9	-84.9	0.04 (1)	10.00
G- H	-5 / 0	-84.9	-84.9	0.04 (1)	10.00
H- I	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
I- J	-9 / 0	-84.9	-84.9	0.04 (1)	10.00
J- K	0 / 0	-84.9	-84.9	0.05 (1)	10.00
K- L	0 / 0	-84.9	-84.9	0.05 (1)	10.00
L- M	0 / 0	-84.9	-84.9	0.03 (1)	10.00
M- N	0 / 0	-84.9	-84.9	0.05 (1)	10.00
N- O	0 / 0	-84.9	-84.9	0.05 (1)	10.00
P- O	-75 / 0	0.0	0.0	0.03 (1)	6.25
AD- A	-85 / 0	0.0	0.0	0.01 (1)	7.81
AD-AC	0 / 0	-38.5	-38.5	0.03 (3)	10.00
AC-AB	0 / 16	-38.5	-38.5	0.03 (3)	10.00
AB-AA	0 / 12	-38.5	-38.5	0.02 (3)	10.00
AA- Z	0 / 9	-38.5	-38.5	0.02 (3)	10.00
Z- Y	0 / 7	-38.5	-38.5	0.02 (3)	10.00
Y- X	0 / 5	-38.5	-38.5	0.02 (3)	10.00
X- W	0 / 3	-38.5	-38.5	0.02 (3)	10.00
W- V	0 / 2	-38.5	-38.5	0.02 (3)	10.00
V- U	0 / 2	-38.5	-38.5	0.02 (3)	10.00
U- T	0 / 0	-38.5	-38.5	0.02 (3)	10.00
T- S	0 / 0	-38.5	-38.5	0.02 (3)	10.00
S- R	0 / 0	-38.5	-38.5	0.02 (3)	10.00
R- O	0 / 0	-38.5	-38.5	0.03 (3)	10.00
Q- P	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
AC- B	-184 / 0	0.03 (1)
AB- C	-164 / 0	0.03 (1)
AA- D	-169 / 0	0.04 (1)
Z- E	-168 / 0	0.07 (1)
Y- F	-169 / 0	0.10 (1)
X- H	-166 / 0	0.15 (1)
W- I	-185 / 0	0.24 (1)
U- J	-137 / 0	0.07 (1)
T- K	-198 / 0	0.11 (1)
S- L	-143 / 0	0.08 (1)
R- M	-144 / 0	0.08 (1)
Q- N	-198 / 0	0.11 (1)
A-AC	0 / 24	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (J-K:1) , BC=0.03/1.00 (Q-R:3) , WB=0.24/1.00 (I-W:1) , SSI=0.08/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NDT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00) .

DWG NO. A1190427
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T57A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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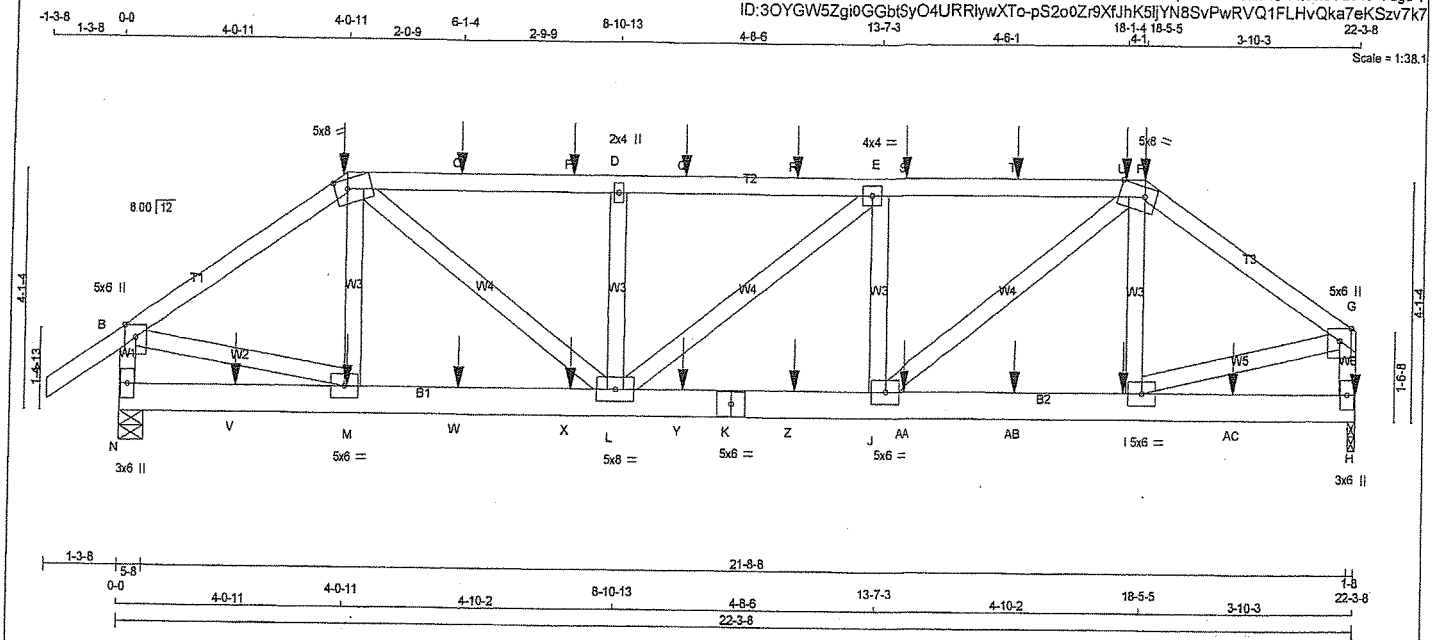

TOTAL WEIGHT = 2 X 114 = 228 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	ORY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	ORY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-G	1	12	SIDE(61.0)
N-B	1	12	TOP
H-G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N-K	2	12	SIDE(183.1)
K-H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FRDM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED DN TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	2.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	5.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I, J, M						
I	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NO. TAM T1901100
STRUCTURAL
COMPONENT ONLY
1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2464	0	2464	0	0	5-8	5-8
H	2422	0	2422	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1855	962 / 0	384 / 0	0 / 0	0 / 0	509 / 0	0 / 0
H	1842	904 / 0	422 / 0	0 / 0	0 / 0	516 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B-C	-2771 / 0	-84.9	-84.9	0.17 (1)	5.33
C-O	-3549 / 0	-84.9	-84.9	0.28 (1)	4.70
O-P	-3549 / 0	-84.9	-84.9	0.28 (1)	4.70
P-D	-3549 / 0	-84.9	-84.9	0.28 (1)	4.70
D-Q	-3549 / 0	-84.9	-84.9	0.29 (1)	4.69
Q-R	-3549 / 0	-84.9	-84.9	0.29 (1)	4.69
R-E	-3549 / 0	-84.9	-84.9	0.29 (1)	4.69
E-S	-3532 / 0	-84.9	-84.9	0.29 (1)	4.69
S-T	-3532 / 0	-84.9	-84.9	0.29 (1)	4.69
T-U	-3532 / 0	-84.9	-84.9	0.29 (1)	4.69
U-F	-3532 / 0	-84.9	-84.9	0.29 (1)	4.69
F-G	-2658 / 0	-84.9	-84.9	0.15 (1)	5.44
N-B	-2377 / 0	0.0	0.0	0.13 (1)	7.29
H-G	-2271 / 0	0.0	0.0	0.13 (1)	7.43
N-V	0 / 0	-38.5	-38.5	0.05 (3)	10.00
V-M	0 / 0	-38.5	-38.5	0.05 (3)	10.00
M-W	0 / 2293	-38.5	-38.5	0.19 (1)	10.00
W-X	0 / 2293	-38.5	-38.5	0.19 (1)	10.00
X-L	0 / 2293	-38.5	-38.5	0.19 (1)	10.00
L-Y	0 / 3532	-38.5	-38.5	0.28 (1)	10.00
Y-K	0 / 3532	-38.5	-38.5	0.28 (1)	10.00
K-Z	0 / 3532	-38.5	-38.5	0.28 (1)	10.00
Z-J	0 / 3532	-38.5	-38.5	0.28 (1)	10.00
J-AA	0 / 2195	-38.5	-38.5	0.18 (1)	10.00
AA-AB	0 / 2195	-38.5	-38.5	0.18 (1)	10.00
AB-I	0 / 2195	-38.5	-38.5	0.18 (1)	10.00
I-AC	0 / 0	-38.5	-38.5	0.05 (3)	10.00
AC-H	0 / 0	-38.5	-38.5	0.05 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-216 / 137	0.02 (1)
C-L	0 / 1583	0.14 (1)
L-D	-688 / 0	0.06 (1)
L-E	0 / 22	0.00 (2)
J-E	-718 / 0	0.06 (1)
J-F	0 / 1685	0.15 (1)
I-F	-314 / 82	0.03 (1)
B-M	0 / 2365	0.21 (1)
I-G	0 / 2295	0.20 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	CONN.
C	4-0-11	-50	-56	—	FRONT	VERT	DEAD	—	—
C	4-0-11	-109	-109	—	BACK	VERT	TOTAL	—	—
C	4-0-11	-231	-231	—	FRONT	VERT	SNOW	—	—
F	18-5-5	-48	-53	—	FRONT	VERT	DEAD	—	—
F	18-5-5	-219	-219	—	FRONT	VERT	SNOW	—	—
H	22-3-8	-71	-90	—	BACK	VERT	TOTAL	—	—
I	18-1-4	-55	-70	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/15/19
R. J. G ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.29/1.00 (E-F:1) , BC=0.28/1.00 (J-L:1) , WB=0.21/1.00 (B-M:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T57A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	4-1-4	-55	-70	—	BACK	VERT	TOTAL	—	—
O	6-1-4	-102	-102	—	BACK	VERT	TOTAL	—	—
P	8-1-4	-102	-102	—	BACK	VERT	TOTAL	—	—
Q	10-1-4	-102	-102	—	BACK	VERT	TOTAL	—	—
R	12-1-4	-102	-102	—	BACK	VERT	TOTAL	—	—
S	14-1-4	-102	-102	—	BACK	VERT	TOTAL	—	—
T	16-1-4	-102	-102	—	BACK	VERT	TOTAL	—	—
U	18-1-4	-130	-130	—	BACK	VERT	TOTAL	—	—
V	2-1-4	-55	-70	—	BACK	VERT	TOTAL	—	—
W	6-1-4	-55	-70	—	BACK	VERT	TOTAL	—	—
X	8-1-4	-55	-70	—	BACK	VERT	TOTAL	—	—
Y	10-1-4	-55	-70	—	BACK	VERT	TOTAL	—	—
Z	12-1-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AA	14-1-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AB	16-1-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AC	20-1-4	-55	-70	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1901100
STRUCTURAL
COMPONENT ONLY

2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T58A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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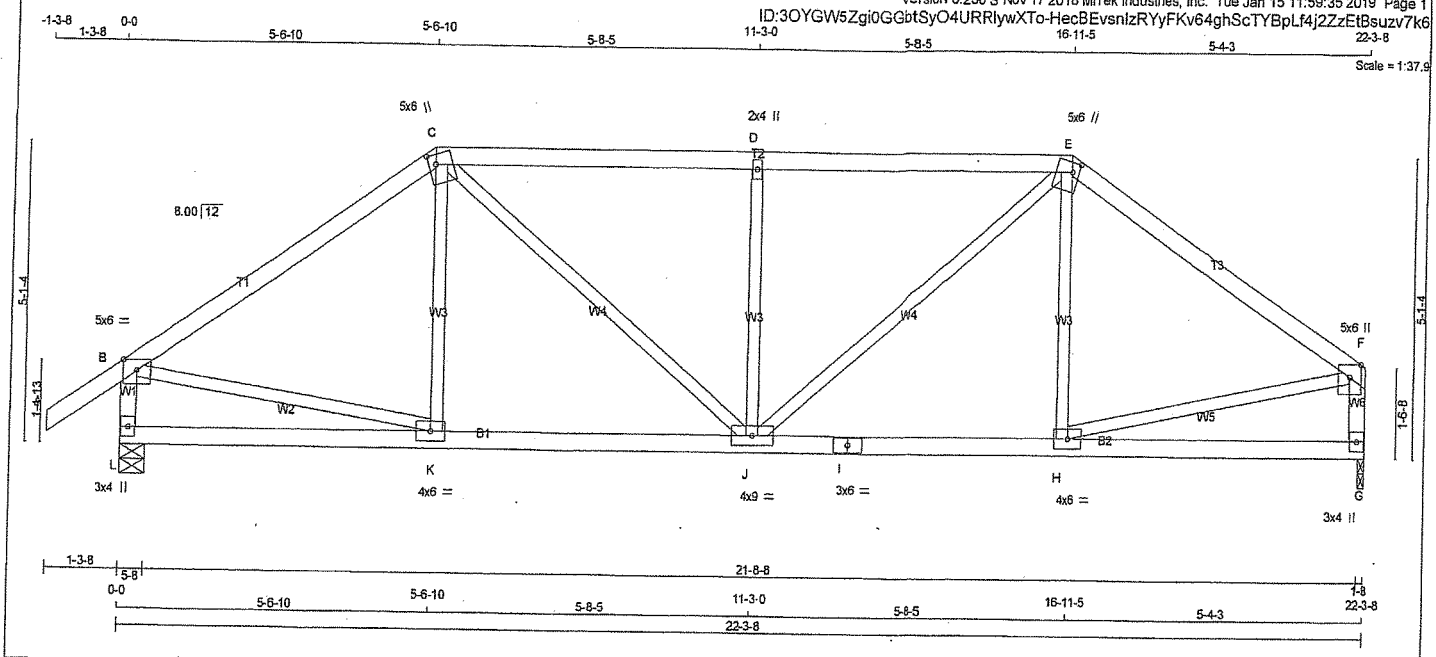


TOTAL WEIGHT = 89 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	ORY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1491	0	1491	0	0	5-8	5-8
G	1375	0	1375	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1123	583 / 0	234 / 0	0 / 0	0 / 0	306 / 0	0 / 0
G	1043	519 / 0	234 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

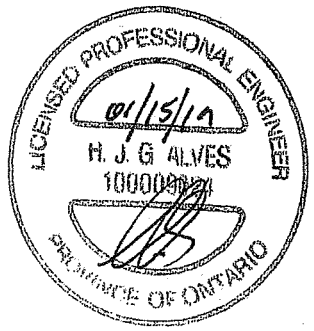
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00	K- C	0 / 190	0.04 (3)
B- C	-1472 / 0	-84.9	-84.9	1	0.55 (1)	4.72	C- J	0 / 591	0.13 (1)
C- D	-1676 / 0	-84.9	-84.9	1	0.54 (1)	4.46	J- O	-590 / 0	0.23 (1)
D- E	-1676 / 0	-84.9	-84.9	1	0.54 (1)	4.46	J- E	0 / 632	0.14 (1)
E- F	-1435 / 0	-84.9	-84.9	1	0.50 (1)	4.83	H- E	-24 / 165	0.04 (3)
L- B	-1405 / 0	0.0	0.0	1	0.14 (1)	6.87	B- K	0 / 1246	0.28 (1)
G- F	-1292 / 0	0.0	0.0	1	0.14 (1)	7.10	H- F	0 / 1222	0.28 (1)
L- K	0 / 0	-38.5	-38.5	1	0.23 (3)	10.00			
K- J	0 / 1224	-38.5	-38.5	1	0.40 (2)	10.00			
J- I	0 / 1192	-38.5	-38.5	1	0.38 (2)	10.00			
I- H	0 / 1192	-38.5	-38.5	1	0.38 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	1	0.22 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.40/1.00 (J-K:2) , WB=0.28/1.00 (B-K:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.57 (F) (INPUT = 1.00)

DWG NO. TAM T190101
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T59A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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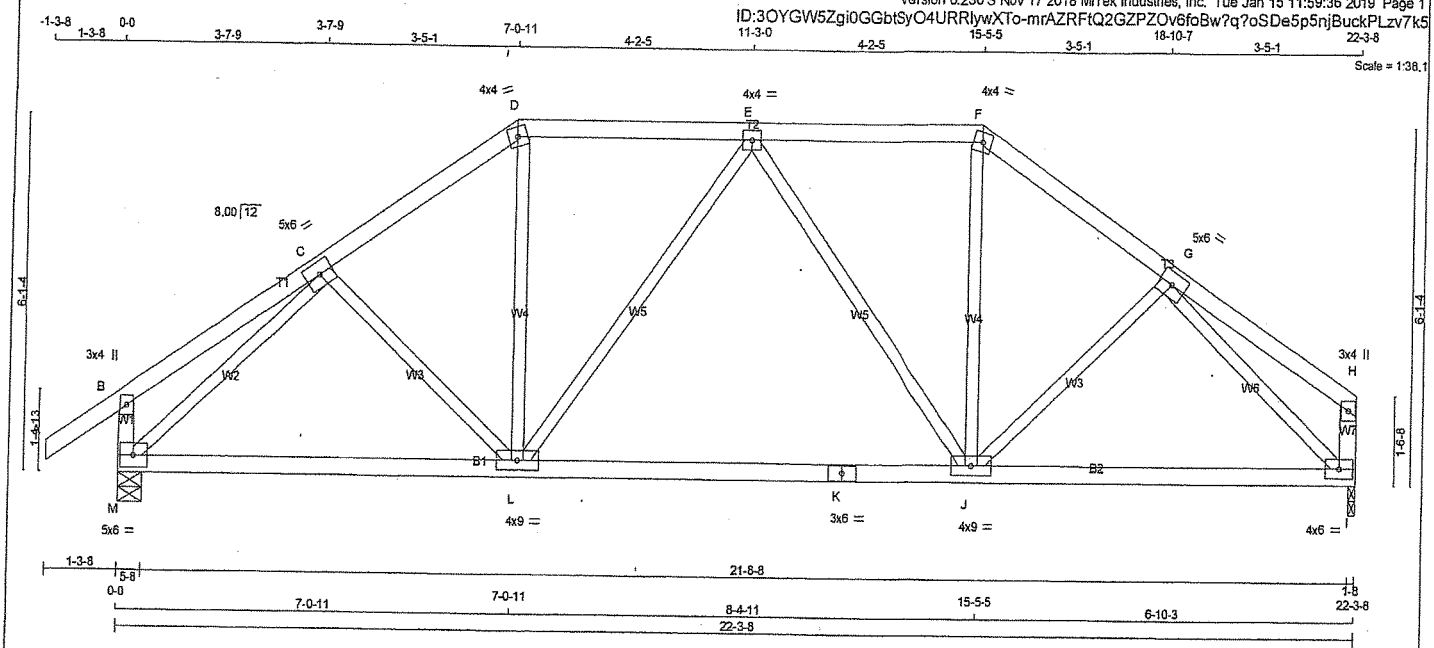


TOTAL WEIGHT = 94 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
M	1491	0	1491	0	0	5-8	5-8
I	1375	0	1375	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1123	583 / 0	234 / 0	0 / 0	0 / 0	306 / 0	0 / 0
I	1043	519 / 0	234 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX..PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

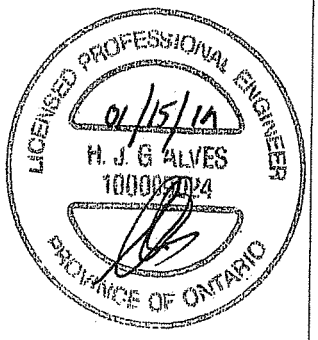
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00
B- C	0 / 17	-84.9	-84.9	1	0.15 (1)	10.00
C- D	-1436 / 0	-84.9	-84.9	1	0.14 (1)	5.32
D- E	-1188 / 0	-84.9	-84.9	1	0.20 (1)	5.65
E- F	-1168 / 0	-84.9	-84.9	1	0.20 (1)	5.68
F- G	-1412 / 0	-84.9	-84.9	1	0.14 (1)	5.36
G- H	0 / 17	-84.9	-84.9	1	0.14 (1)	10.00
M- B	-235 / 0	0.0	0.0	1	0.02 (1)	7.81
I- H	-110 / 0	0.0	0.0	1	0.01 (1)	7.81
M- L	0 / 1181	-38.5	-38.5	2	0.55 (2)	10.00
L- K	0 / 1330	-38.5	-38.5	2	0.58 (2)	10.00
K- J	0 / 1330	-38.5	-38.5	2	0.58 (2)	10.00
J- I	0 / 1123	-38.5	-38.5	2	0.53 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-3 / 96	0.02 (3)
L- D	0 / 530	0.12 (1)
L- E	-246 / 0	0.24 (1)
E- J	-280 / 0	0.27 (1)
J- F	0 / 516	0.12 (1)
J- G	0 / 119	0.03 (3)
M- C	-1648 / 0	0.63 (1)
G- I	-1616 / 0	0.58 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.20/1.00 (D-E:1) , BC=0.58/1.00 (J-L:2) , WB=0.63/1.00 (C-M:1) , SSI=0.18/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.51 (K) (INPUT = 1.00)

DWG NO. TAM T1901102
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T60	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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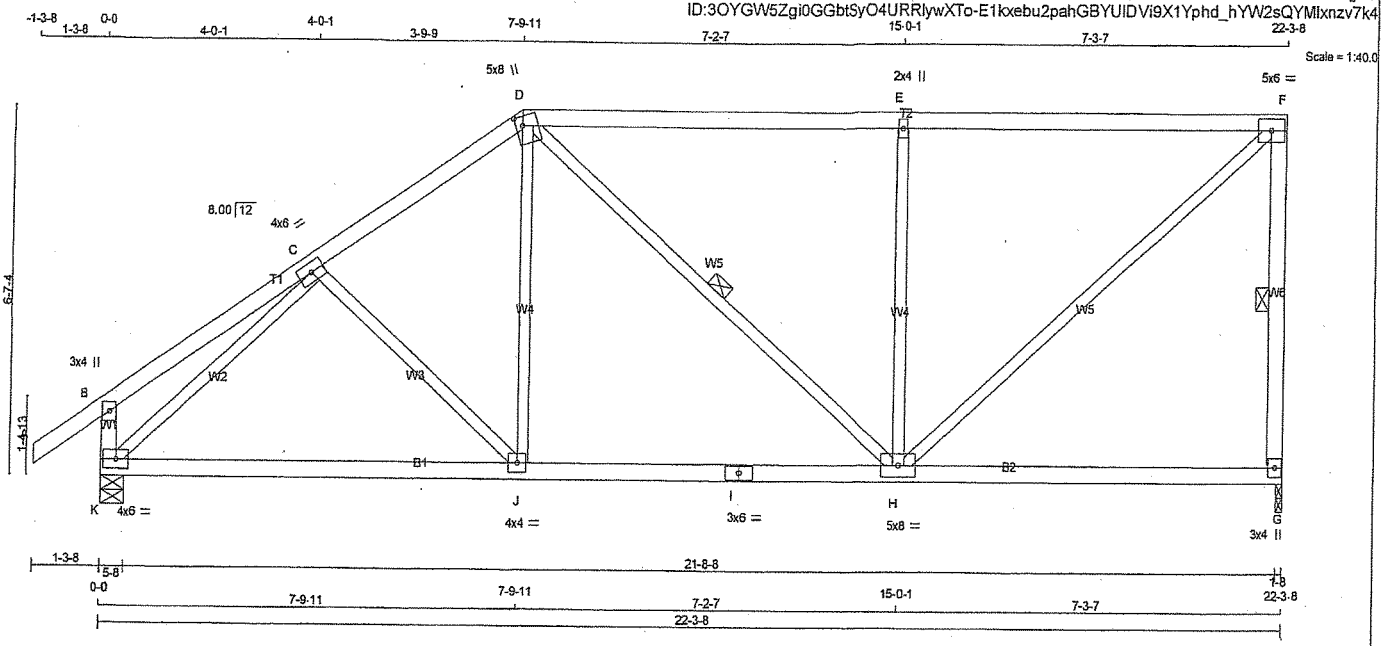


TOTAL WEIGHT = 96 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTDRED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1375	0	1375	0	0	1-8	1-8
K	1491	0	1491	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1043	519 / 0	234 / 0	0 / 0	0 / 0	290 / 0	0 / 0
K	1123	583 / 0	234 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

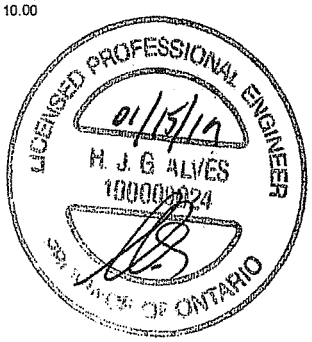
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FRDM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 20	-84.9	-84.9	0.20 (1)	10.00
C- D	-1391 / 0	-84.9	-84.9	0.23 (1)	5.27
D- E	-1183 / 0	-84.9	-84.9	0.87 (1)	4.30
E- F	-1183 / 0	-84.9	-84.9	0.87 (1)	4.30
G- F	-1260 / 0	0.0	0.0	0.25 (1)	5.74
K- B	-245 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 1188	-38.5	-38.5	0.55 (2)	10.00
J- I	0 / 1145	-38.5	-38.5	0.55 (2)	10.00
I- H	0 / 1145	-38.5	-38.5	0.55 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.35 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-67 / 73	0.03 (1)
J- D	0 / 414	0.09 (2)
K- C	-1634 / 0	0.76 (1)
H- F	0 / 1566	0.35 (1)
D- H	-21 / 51	0.01 (3)
H- E	-782 / 0	0.55 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.87/1.00 (E-F:1) , BC=0.55/1.00 (H-J:2) , WB=0.76/1.00 (C-K:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.42 (I) (INPUT = 1.00)

DWG NO. TAM T1901103
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T61	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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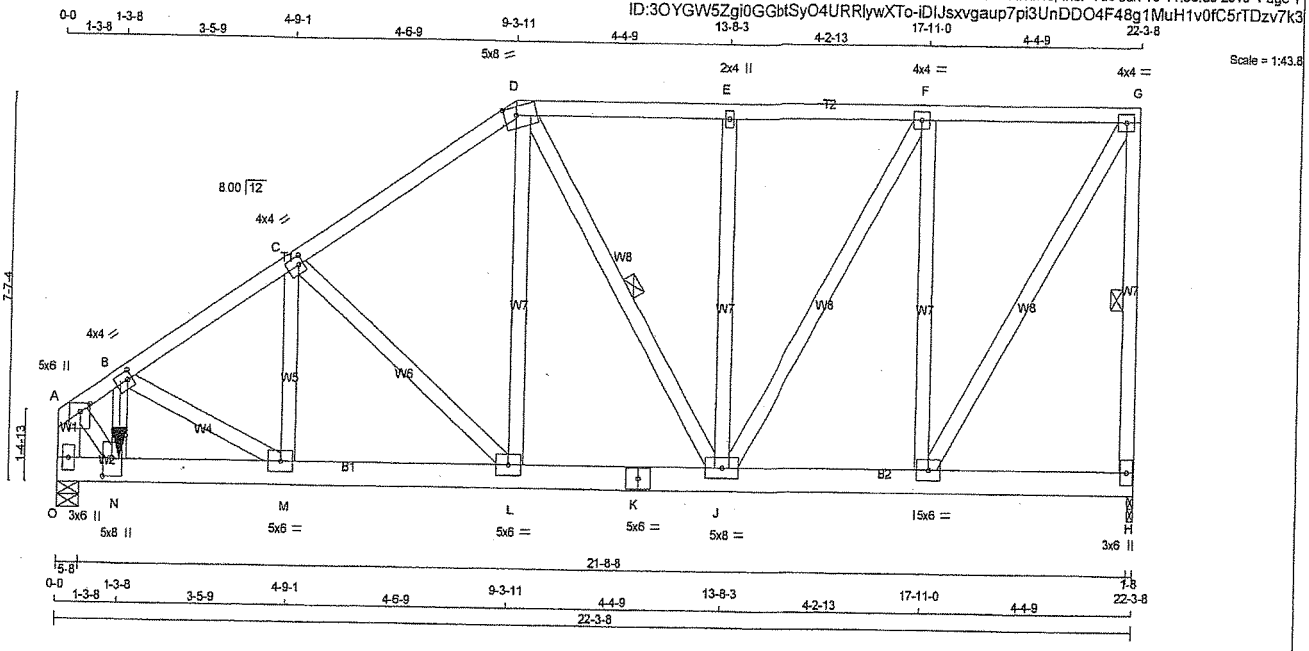


TOTAL WEIGHT = 2 X 146 = 292 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
G- H	1	12	TOP
O- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- K	2	12	TOP
K- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FRDM THE TOP AND MUST BE PLACED DN TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	8.0	2.00	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-m	MT20	5.0	8.0	2.00	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	8.0		
I, L, M						
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
N	BMWW+t	MT20	5.0	8.0	4.25	2.50
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
H	1885	0	1885	0	0	1-8	1-8
O	6415	0	6415	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1277	640 / 0	283 / 0	0 / 0	0 / 0	354 / 0	0 / 0
O	4841	2484 / 0	1030 / 0	0 / 0	0 / 0	1327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-J.

END VERTICAL(S) MUST BE SHEATHEO OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES. (4)

CHORDS

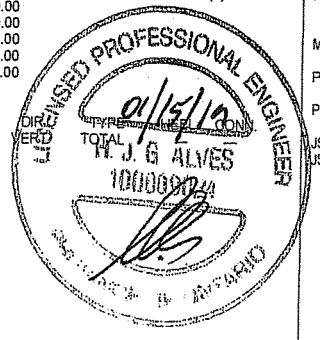
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH
A- B	-5012 / 0	-84.9	-84.9	0.11 (1)	4.25
B- C	-3094 / 0	-84.9	-84.9	0.17 (1)	5.10
C- D	-1918 / 0	-84.9	-84.9	0.15 (1)	6.15
D- E	-1399 / 0	-84.9	-84.9	0.12 (1)	6.25
E- F	-1399 / 0	-84.9	-84.9	0.12 (1)	6.25
F- G	-885 / 0	-84.9	-84.9	0.12 (1)	6.25
H- G	-1613 / 0	0.0	0.0	0.21 (1)	6.25
O- A	-6081 / 0	0.0	0.0	0.22 (1)	6.01
O- N	0 / 0	-38.5	-38.5	0.16 (1)	10.00
N- M	0 / 4157	-38.5	-38.5	0.43 (1)	10.00
M- L	0 / 2602	-38.5	-38.5	0.20 (1)	10.00
L- K	0 / 1598	-38.5	-38.5	0.12 (1)	10.00
K- J	0 / 1598	-38.5	-38.5	0.12 (1)	10.00
J- I	0 / 885	-38.5	-38.5	0.08 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	0 / 1152	0.10 (1)
C- L	-1372 / 0	0.34 (1)
L- D	0 / 1072	0.09 (1)
N- B	0 / 1756	0.16 (1)
A- N	0 / 5251	0.46 (1)
B- M	-1765 / 0	0.17 (1)
I- G	0 / 1709	0.15 (1)
D- J	-383 / 0	0.06 (1)
I- F	-1278 / 0	0.46 (1)
J- E	-394 / 0	0.14 (1)
J- F	0 / 1017	0.09 (1)

FACTOREO CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	TOTAL
N	1-3-8	-5350	-5350	—	TOP	VERT	FLOOR LOAD	2



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.22/1.00 (A-O:1) , BC=0.43/1.00 (M-N:1) , WB=0.46/1.00 (A-N:1) , SSI=0.12/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.75 (N) (INPUT = 1.00)

DWG NO. TAM T1901104
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T62A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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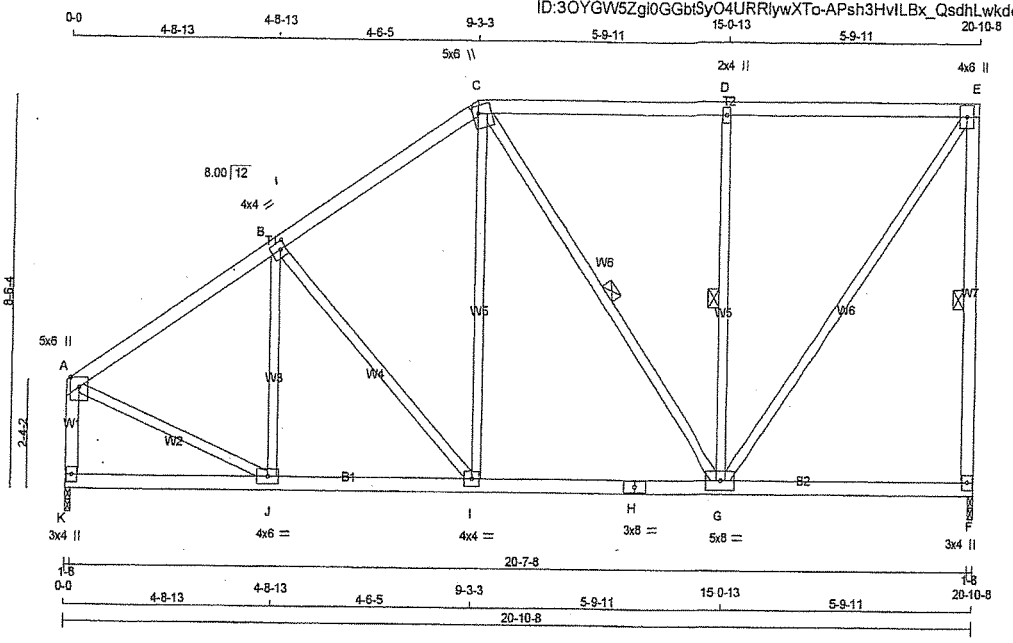

TOTAL WEIGHT = 103 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
O	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
F	1288	0	1288	0	0	1-8	1-8
K	1288	0	1288	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0
K	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-1136 / 0	-84.9	-84.9	0.34 (1)		5.53
B- C	-1005 / 0	-84.9	-84.9	0.33 (1)		5.80
C- D	-717 / 0	-84.9	-84.9	0.50 (1)		6.21
D- E	-717 / 0	-84.9	-84.9	0.50 (1)		6.20
F- E	-1198 / 0	0.0	0.0	0.40 (1)		5.85
K- A	-1213 / 0	0.0	0.0	0.15 (1)		7.27
K- J	0 / 0	-38.5	-38.5	0.15 (3)		10.00
J- I	0 / 967	-38.5	-38.5	0.28 (2)		10.00
I- H	0 / 818	-38.5	-38.5	0.35 (2)		10.00
H- G	0 / 818	-38.5	-38.5	0.35 (2)		10.00
G- F	0 / 0	-38.5	-38.5	0.25 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
J- B	-215 / 42	0.10 (1)
B- I	-231 / 0	0.21 (1)
I- C	0 / 385	0.09 (2)
C- G	-176 / 0	0.13 (1)
G- D	-610 / 0	0.28 (1)
G- E	0 / 1237	0.28 (1)
A- J	0 / 1051	0.24 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.35/1.00 (G-I:2) , WB=0.28/1.00 (D-G:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (G) (INPUT = 0.90)
JSI METAL= 0.46 (A) (INPUT = 1.00)

DWG NO. TAM T1901105
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T63A	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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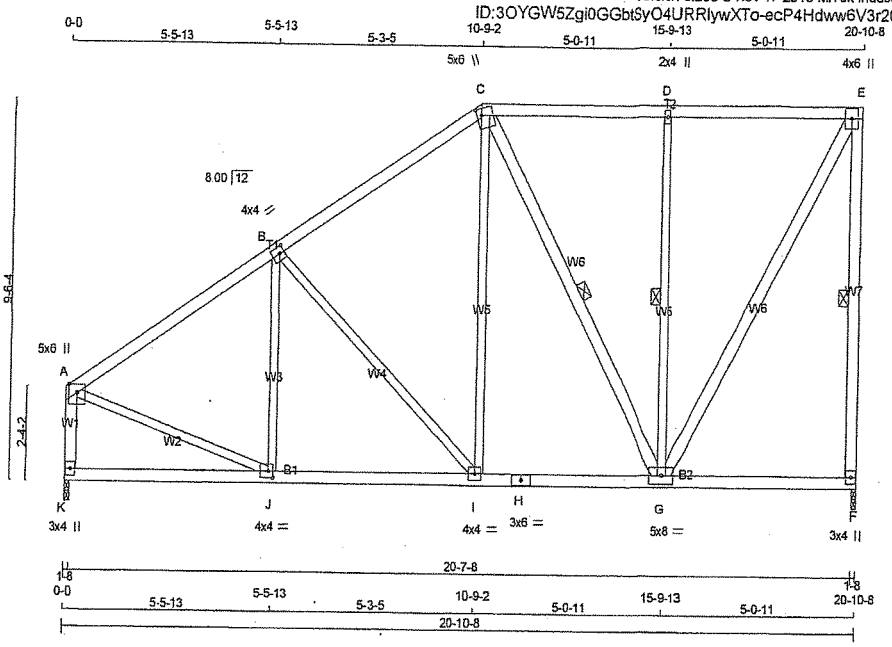


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
F	1288	0	1288	0	0	1-8	1-8
K	1288	0	1288	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0
K	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

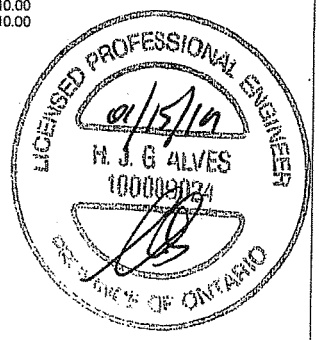
TOTAL LOAD CASES (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1159 / 0	-84.9	-84.9	0.34 (1)	5.51
B-C	-912 / 0	-84.9	-84.9	0.33 (1)	6.04
C-D	-576 / 0	-84.9	-84.9	0.28 (1)	6.25
D-E	-576 / 0	-84.9	-84.9	0.28 (1)	6.25
F-E	-1208 / 0	0.0	0.0	0.52 (1)	5.82
K-A	-1203 / 0	0.0	0.0	0.15 (1)	7.29
K-J	0 / 0	-38.5	-38.5	0.22 (3)	10.00
J-I	0 / 989	-38.5	-38.5	0.35 (2)	10.00
I-H	0 / 737	-38.5	-38.5	0.28 (2)	10.00
H-G	0 / 737	-38.5	-38.5	0.28 (2)	10.00
G-F	0 / 0	-38.5	-38.5	0.18 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
J-B	-126 / 117	0.07 (1)
B-I	-374 / 0	0.47 (1)
I-C	0 / 473	0.11 (2)
C-G	-337 / 0	0.19 (1)
G-D	-530 / 0	0.32 (1)
G-E	0 / 1192	0.19 (1)
A-J	0 / 1054	0.24 (1)



TOTAL WEIGHT = 5 X 115 = 576 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/380 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.52/1.00 (E-F:1) , BC=0.35/1.00 (I-J:2) , WB=0.47/1.00 (B-I:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.84 (J) (INPUT = 0.90)
JSI METAL= 0.47 (A) (INPUT = 1.00)

DWG NO. TAM T1901106
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T64A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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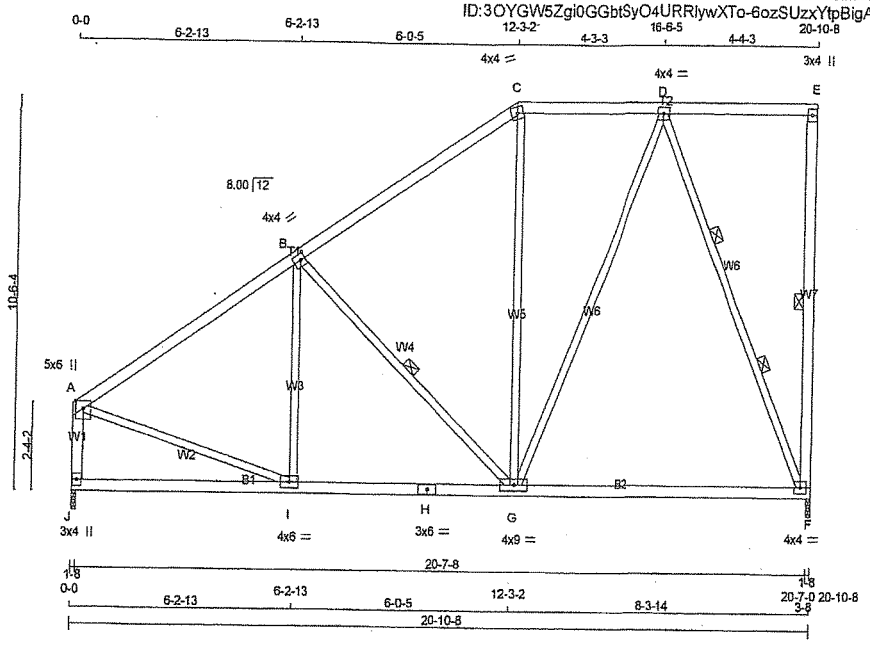

Scale = 1:59.6

TOTAL WEIGHT = 106 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
F	1288	0	1288	0	0	1-8	1-8
J	1288	0	1288	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0
J	977	486 / 0	219 / 0	0 / 0	0 / 0	271 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, B-G.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1158 / 0	-84.9	-84.9	0.45 (1)	5.34
B- C	-827 / 0	-84.9	-84.9	0.43 (1)	6.09
C- D	-658 / 0	-84.9	-84.9	0.21 (1)	6.25
D- E	0 / 0	-84.9	-84.9	0.26 (1)	10.00
F- E	-141 / 0	0.0	0.0	0.08 (1)	6.25
J- A	-1186 / 0	0.0	0.0	0.15 (1)	7.33
J- I	0 / 0	-38.5	-38.5	0.24 (3)	10.00
I- H	0 / 992	-38.5	-38.5	0.58 (2)	10.00
H- G	0 / 992	-38.5	-38.5	0.58 (2)	10.00
G- F	0 / 430	-38.5	-38.5	0.49 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-88 / 134	0.06 (1)
B- G	-476 / 0	0.25 (1)
G- C	0 / 180	0.04 (3)
A- I	0 / 1043	0.23 (1)
G- D	0 / 605	0.14 (1)
D- F	-1100 / 0	0.93 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/ 740 (0.34")

CSI: TC=0.45/1.00 (A-B:1) , BC=0.58/1.00 (G-I:2) , WB=0.93/1.00 (D-F:1) , SSI=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.48 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM 19011107
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO
300399	T65A	1	2	TRUSS OESC.	

Tamarack Roof Truss, Burlington

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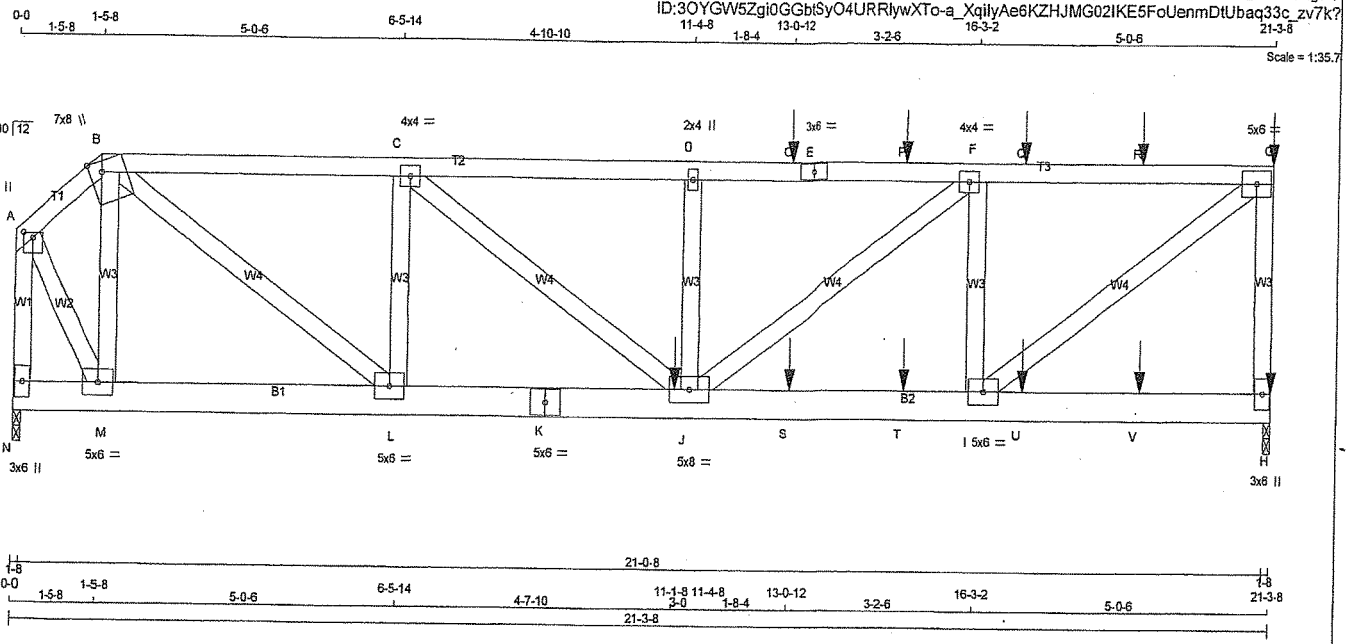

TOTAL WEIGHT = 2 X 114 = 228 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - G	2x4	ORY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
N - K	2x6	ORY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TDP CHORDS : (0.122"X3") SPIRAL NAILS			
A-B	1	12	TDP
B-E	1	12	SIDE(0.0)
E-G	1	12	SIDE(61.0)
G-H	1	12	TOP
N-A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N-K	2	12	TOP
K-H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED DN TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIOE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIOE OR ON THE TDP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	7.0	8.0	Edge	2.25
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I, L, M						
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE DF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2550	0	2550	0	0	1-8	1-8
N	1954	0	1954	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1933	966 / 0	431 / 0	0 / 0	0 / 0	536 / 0	0 / 0
N	1478	749 / 0	322 / 0	0 / 0	0 / 0	407 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JDINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1050 / 0	-84.9	-84.9	0.02 (1)	6.25
B-C	-2766 / 0	-84.9	-84.9	0.20 (1)	5.30
C-D	-3851 / 0	-84.9	-84.9	0.23 (1)	4.62
D-O	-3851 / 0	-84.9	-84.9	0.36 (1)	4.46
O-E	-3851 / 0	-84.9	-84.9	0.36 (1)	4.46
E-P	-3851 / 0	-84.9	-84.9	0.36 (1)	4.46
P-F	-3851 / 0	-84.9	-84.9	0.36 (1)	4.46
F-Q	-2722 / 0	-84.9	-84.9	0.32 (1)	5.16
Q-R	-2722 / 0	-84.9	-84.9	0.32 (1)	5.16
R-G	-2722 / 0	-84.9	-84.9	0.32 (1)	5.16
H-G	-2368 / 0	0.0	0.0	0.30 (1)	7.31
N-A	-1976 / 0	0.0	0.0	0.15 (1)	7.81
N-M	0 / 0	-38.5	-38.5	0.04 (2)	10.00
M-L	0 / 759	-38.5	-38.5	0.08 (1)	10.00
L-K	0 / 2767	-38.5	-38.5	0.22 (1)	10.00
K-J	0 / 2767	-38.5	-38.5	0.22 (1)	10.00
J-S	0 / 2722	-38.5	-38.5	0.24 (1)	10.00
S-T	0 / 2722	-38.5	-38.5	0.24 (1)	10.00
T-I	0 / 2722	-38.5	-38.5	0.24 (1)	10.00
I-U	0 / 0	-38.5	-38.5	0.08 (3)	10.00
U-V	0 / 0	-38.5	-38.5	0.08 (3)	10.00
V-H	0 / 0	-38.5	-38.5	0.08 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTDRED FORCE (LBS)	MAX (LC)
M-B	-1047 / 0	0.09 (1)
B-L	0 / 2493	0.22 (1)
L-C	-1287 / 0	0.12 (1)
C-J	0 / 1364	0.12 (1)
J-D	-454 / 0	0.04 (1)
J-F	0 / 1420	0.13 (1)
I-F	-1636 / 0	0.15 (1)
I-G	0 / 1636	0.20 (1)
A-M	0 / 1472	0.13 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT.	LDC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	21-3-8	-135	-135	—	FRONT	VERT	TOTAL	—	—
H	21-3-8	-70	-89	—	FRONT	VERT	TOTAL	—	—
J	11-1-8	-1044	-1044	—	TOP	VERT	TOTAL	—	—
O	13-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
P	15-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
Q	17-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
R	19-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
S	13-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
T	15-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
U	17-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
V	19-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TDP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.36/1.00 (D-F:1) , BC=0.24/1.00 (I-J:1) , WB=0.30/1.00 (G-I:1) , SSI=0.18/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTDR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FDR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATIDN TOL. = 5.0 Deg.

JSI GRIP= 0.72 (I) (INPUT = 0.90)
JSI METAL= 0.35 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 1900106
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	DRWG NO.
300399	T66A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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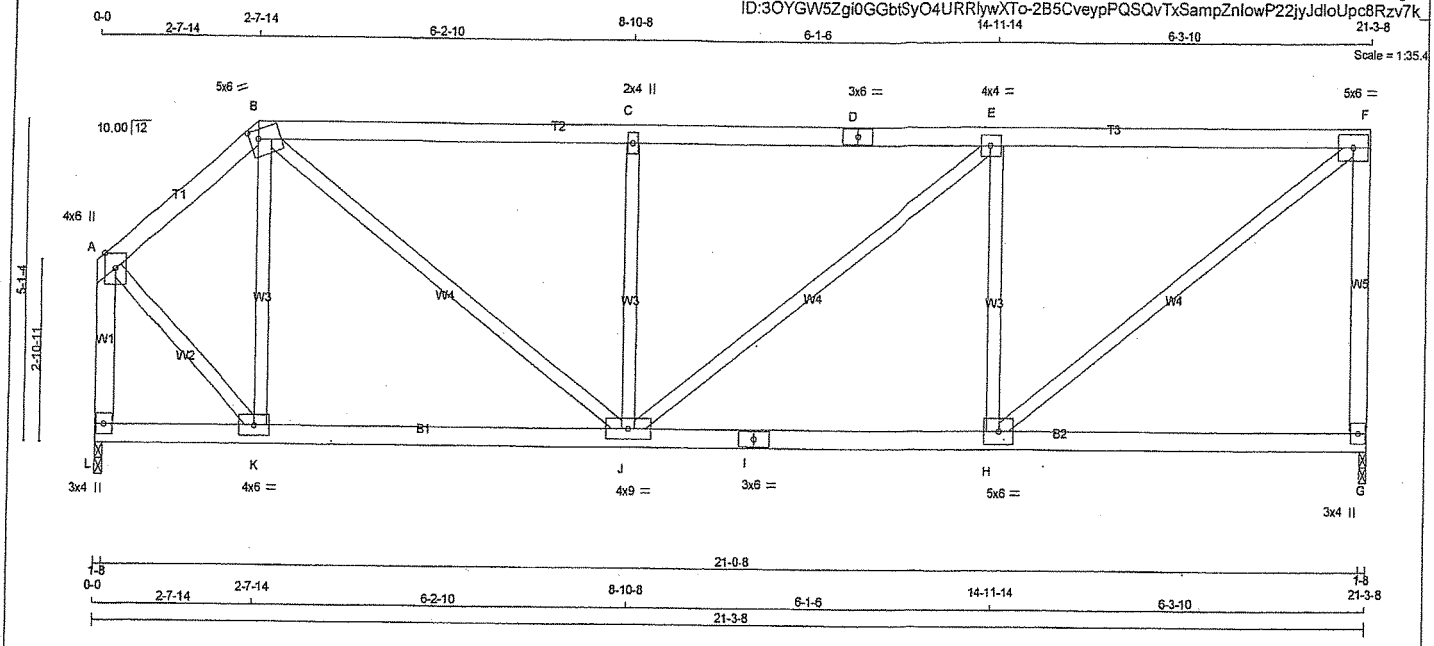


TOTAL WEIGHT = 90 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	1.75	1.75
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1313	0	1313	0	0	1-8	1-8
L	1313	0	1313	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	996	496 / 0	224 / 0	0 / 0	0 / 0	277 / 0	0 / 0
L	998	496 / 0	224 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

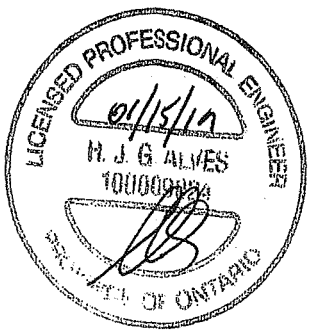
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-875 / 0	-84.9	-84.9	0	11 (1)	6.25	K- B	-414 / 0	0.16 (1)
B- C	-1498 / 0	-84.9	-84.9	0	51 (1)	4.73	A- K	0 / 913	0.21 (1)
C- D	-1498 / 0	-84.9	-84.9	0	53 (1)	4.70	H- F	0 / 1646	0.37 (1)
D- E	-1498 / 0	-84.9	-84.9	0	53 (1)	4.70	B- J	0 / 1057	0.24 (1)
E- F	-1308 / 0	-84.9	-84.9	0	52 (1)	4.97	H- E	-729 / 0	0.28 (1)
G- F	-1215 / 0	0.0	0.0	0	54 (1)	7.28	J- C	-566 / 0	0.22 (1)
L- A	-1296 / 0	0.0	0.0	0	20 (1)	7.09	J- E	0 / 242	0.05 (1)
L- K	0 / 0	-38.5	-38.5	0	17 (3)	10.00			
K- J	0 / 661	-38.5	-38.5	0	28 (2)	10.00			
J- I	0 / 1308	-38.5	-38.5	0	45 (2)	10.00			
I- H	0 / 1308	-38.5	-38.5	0	45 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0	28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.54/1.00 (F-G:1) , BC=0.45/1.00 (H-J:2) , WB=0.37/1.00 (F-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.49 (I) (INPUT = 1.00)

DWG NO. TAM 1140 1109
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T67A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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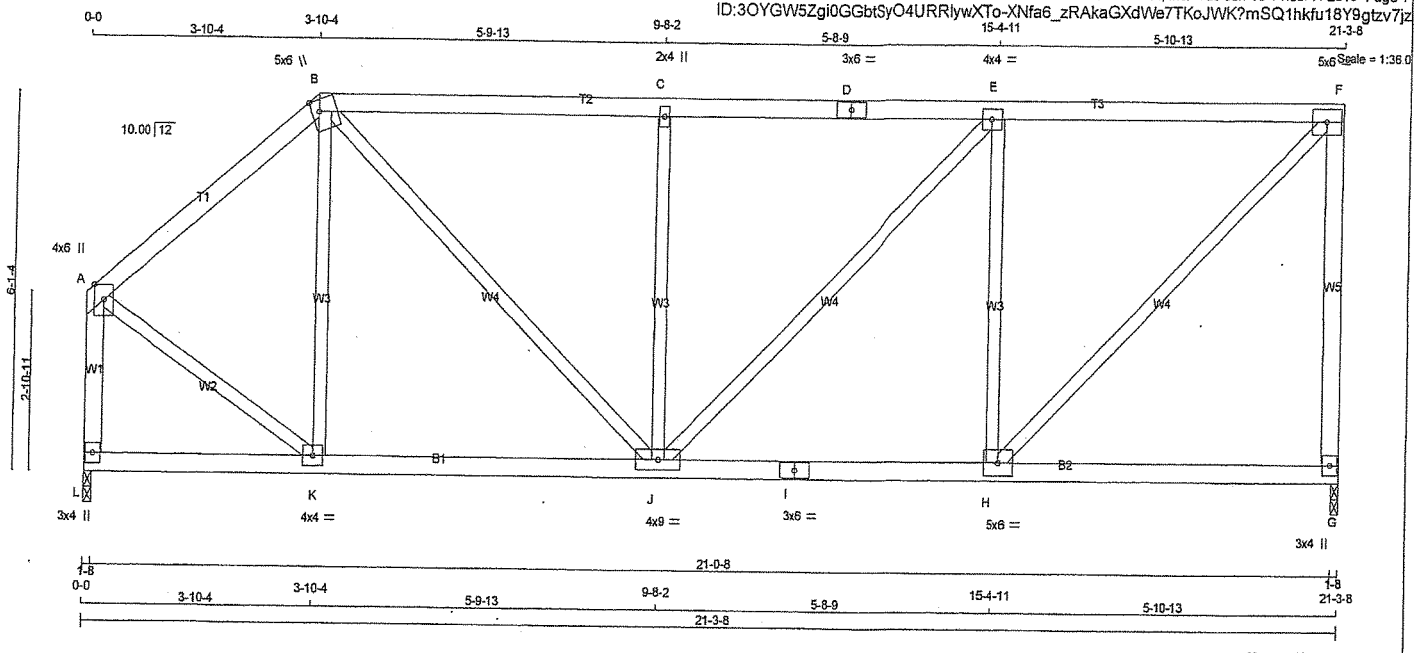


TOTAL WEIGHT = 95 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHDRD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1313	0	1313	0	0	1-8	1-8
L	1313	0	1313	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	996	496 / 0	224 / 0	0 / 0	0 / 0	277 / 0	0 / 0
L	996	496 / 0	224 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

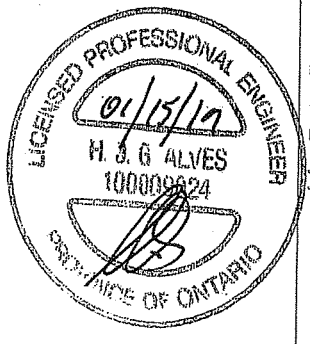
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-965 / 0	-84.9	-84.9	0.24 (1)		6.02
B-C	-1254 / 0	-84.9	-84.9	0.43 (1)		5.19
C-D	-1254 / 0	-84.9	-84.9	0.44 (1)		5.16
D-E	-1254 / 0	-84.9	-84.9	0.44 (1)		5.16
E-F	-1036 / 0	-84.9	-84.9	0.43 (1)		5.57
G-F	-1222 / 0	0.0	0.0	0.88 (1)		7.25
L-A	-1261 / 0	0.0	0.0	0.20 (1)		7.16
L-K	0 / 0	-38.5	-38.5	0.17 (3)		10.00
K-J	0 / 735	-38.5	-38.5	0.27 (2)		10.00
J-I	0 / 1035	-38.5	-38.5	0.38 (2)		10.00
I-H	0 / 1035	-38.5	-38.5	0.38 (2)		10.00
H-G	0 / 0	-38.5	-38.5	0.25 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-B	-265 / 12	0.16 (1)
A-K	0 / 880	0.20 (1)
H-F	0 / 1452	0.33 (1)
B-J	0 / 733	0.16 (1)
H-E	-766 / 0	0.45 (1)
J-C	-529 / 0	0.31 (1)
J-E	0 / 313	0.07 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.88/1.00 (F-G:1) , BC=0.38/1.00 (H-J:2) , WB=0.45/1.00 (E-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.45 (A) (INPUT = 1.00)

DWG NO. TAM T901110
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T68A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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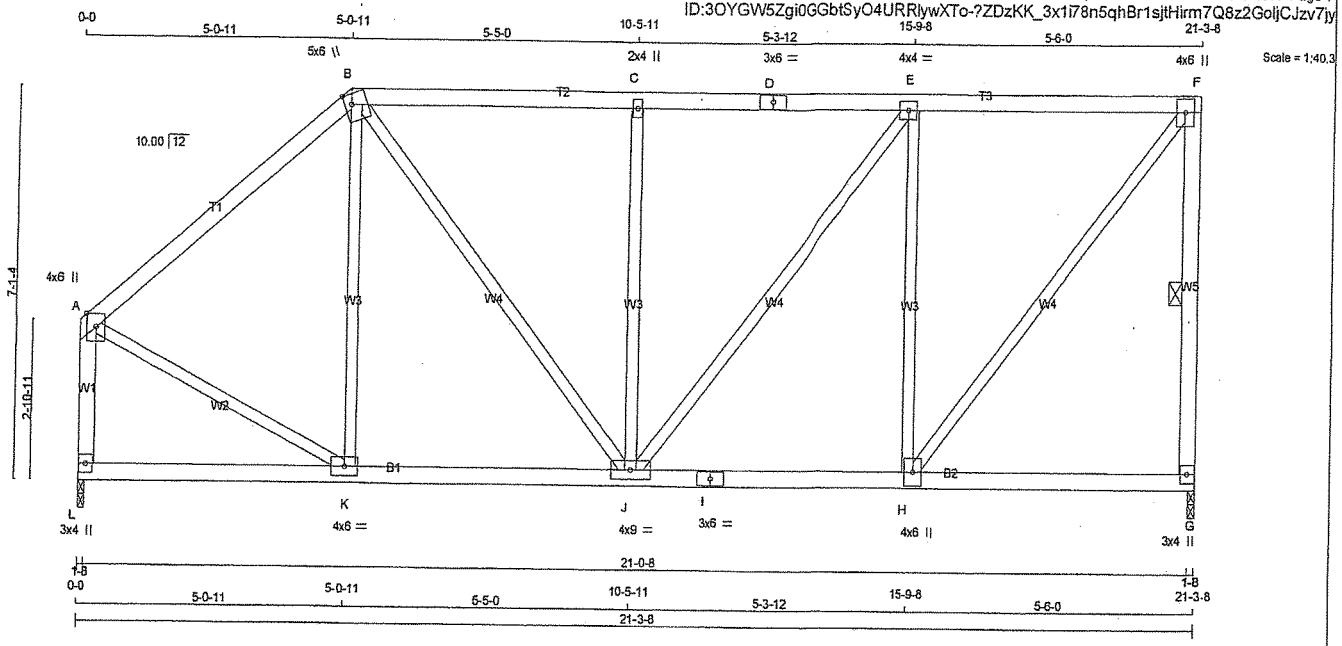


TOTAL WEIGHT = 101 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.D		
D	TS-t	MT20	3.D	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT2D	4.0	6.D		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	8.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
G	1313	0	1313	0	0	1-8	1-8
L	1313	0	1313	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	996	496 / 0	224 / 0	0 / 0	0 / 0	277 / 0	0 / 0
L	996	496 / 0	224 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

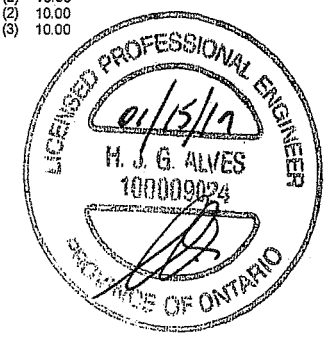
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-999 / 0	-84.9	-84.9	0.42 (1)		5.68
B- C	-1070 / 0	-84.9	-84.9	0.36 (1)		5.62
C- D	-1070 / 0	-84.9	-84.9	0.37 (1)		5.60
D- E	-1070 / 0	-84.9	-84.9	0.37 (1)		5.60
E- F	-839 / 0	-84.9	-84.9	0.36 (1)		6.14
G- F	-1228 / 0	0.0	0.0	0.28 (1)		5.79
L- A	-1236 / 0	0.0	0.0	0.19 (1)		7.22
L- K	0 / 0	-38.5	-38.5	0.20 (3)		10.00
K- J	0 / 764	-38.5	-38.5	0.30 (2)		10.00
J- I	0 / 839	-38.5	-38.5	0.32 (2)		10.00
I- H	0 / 839	-38.5	-38.5	0.32 (2)		10.00
H- G	0 / 0	-38.5	-38.5	0.22 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-148 / 90	0.13 (1)
A- K	0 / 855	0.19 (1)
H- F	0 / 1336	0.30 (1)
B- J	0 / 491	0.11 (1)
H- E	-801 / 0	0.70 (1)
J- C	-492 / 0	0.43 (1)
J- E	0 / 375	0.08 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.42/1.00 (A-B:1) , BC=0.32/1.00 (H-J:2) , WB=0.70/1.00 (E-H:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (F) (INPUT = 0.90)
JSI METAL= 0.47 (A) (INPUT = 1.00)

DWG NO. TAM T190411
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T69A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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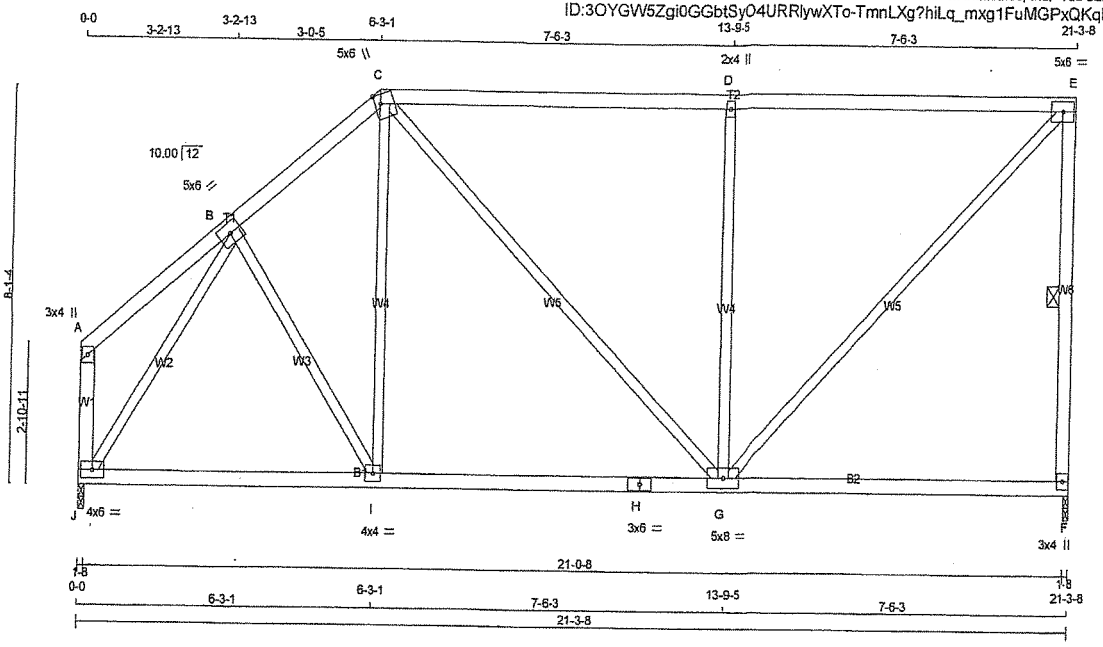


TOTAL WEIGHT = 101 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
F	1313	0	1313	0	0	1-8	1-8
J	1313	0	1313	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	996	496 / 0	224 / 0	0 / 0	0 / 0	277 / 0	0 / 0
J	996	496 / 0	224 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

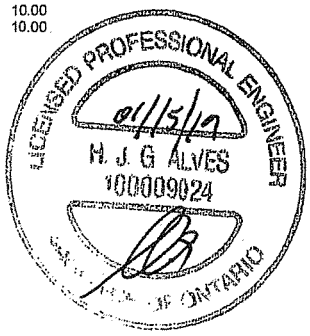
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-84.9	-84.9	0.12 (1)	10.00
B- C	-1019 / 0	-84.9	-84.9	0.14 (1)	6.04
C- D	-922 / 0	-84.9	-84.9	0.91 (1)	4.49
D- E	-923 / 0	-84.9	-84.9	0.91 (1)	4.49
F- E	-1196 / 0	0.0	0.0	0.35 (1)	5.85
J- A	-102 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 671	-38.5	-38.5	0.38 (2)	10.00
I- H	0 / 768	-38.5	-38.5	0.49 (2)	10.00
H- G	0 / 768	-38.5	-38.5	0.49 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.41 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- I	0 / 194	0.04 (2)
I- C	0 / 222	0.05 (3)
C- G	0 / 223	0.05 (1)
G- D	-792 / 0	1.00 (1)
G- E	0 / 1327	0.30 (1)
J- B	-1301 / 0	0.80 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.91/1.00 (D-E:1) , BC=0.49/1.00 (G-I:2) , WB=1.00/1.00 (D-G:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.30 (B) (INPUT = 1.00)

DWG NO. TAM T69011/2
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T70	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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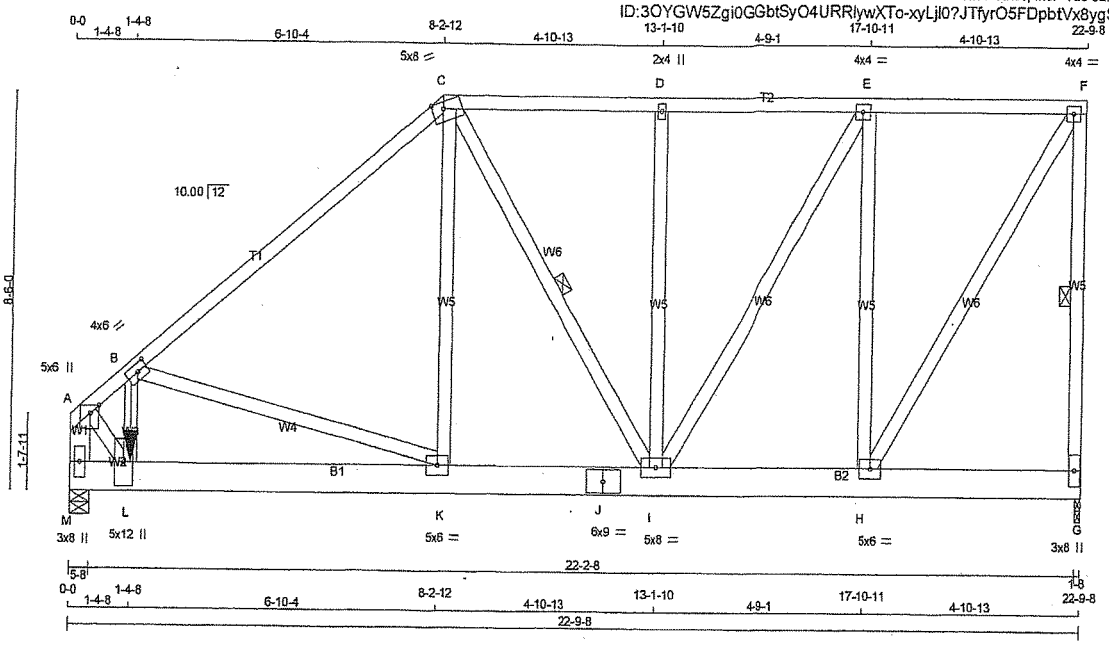

TOTAL WEIGHT = 3 X 161 = 484 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
M - J	2x8	DRY	No.2	SPF
J - G	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- G	1	12	TOP
M- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	TOP
J- G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	6.0	2.00	2.75
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	8.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	6.0	9.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	5.0	12.0		
M	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1841	0	1841	0	0	1-8	1-8
M	8190	0	8190	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SDIL
G	1394	702 / 0	307 / 0	0 / 0	0 / 0	385 / 0	0 / 0
M	6172	3192 / 0	1294 / 0	0 / 0	0 / 0	1886 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, C-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-5918 / 0	-84.9	-84.9	0.21 (1)		4.60
B- C	-2292 / 0	-84.9	-84.9	0.21 (1)		6.25
C- D	-1511 / 0	-84.9	-84.9	0.09 (1)		6.25
D- E	-1511 / 0	-84.9	-84.9	0.09 (1)		6.25
E- F	-969 / 0	-84.9	-84.9	0.09 (1)		6.25
G- F	-1760 / 0	0.0	0.0	0.18 (1)		6.25
M- A	-7597 / 0	0.0	0.0	0.17 (1)		6.52
M- L	0 / 0	-38.5	-38.5	0.12 (1)		10.00
L- K	0 / 4677	-38.5	-38.5	0.27 (1)		10.00
K- J	0 / 1758	-38.5	-38.5	0.11 (1)		10.00
J- I	0 / 1758	-38.5	-38.5	0.11 (1)		10.00
I- H	0 / 969	-38.5	-38.5	0.05 (1)		10.00
H- G	0 / 0	-38.5	-38.5	0.02 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	0 / 2745	0.15 (1)
B- K	-3100 / 0	0.77 (1)
K- C	0 / 1439	0.08 (1)
C- I	-473 / 0	0.07 (1)
I- O	-441 / 0	0.14 (1)
I- E	0 / 1066	0.06 (1)
H- E	-1367 / 0	0.42 (1)
H- F	0 / 1863	0.10 (1)
A- L	0 / 6095	0.33 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR	TYPE	TOTAL
L	1-4-8	-7219	-7219	—	TOP	VERT		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.27/1.00 (K-L:1) , WB=0.77/1.00 (B-K:1) , SSI=0.11/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.90)
JSI METAL= 0.56 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. GEELEN
100090924
PROVINCE OF ONTARIO
01/15/19

DWG NO. TAM T79011 13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T71	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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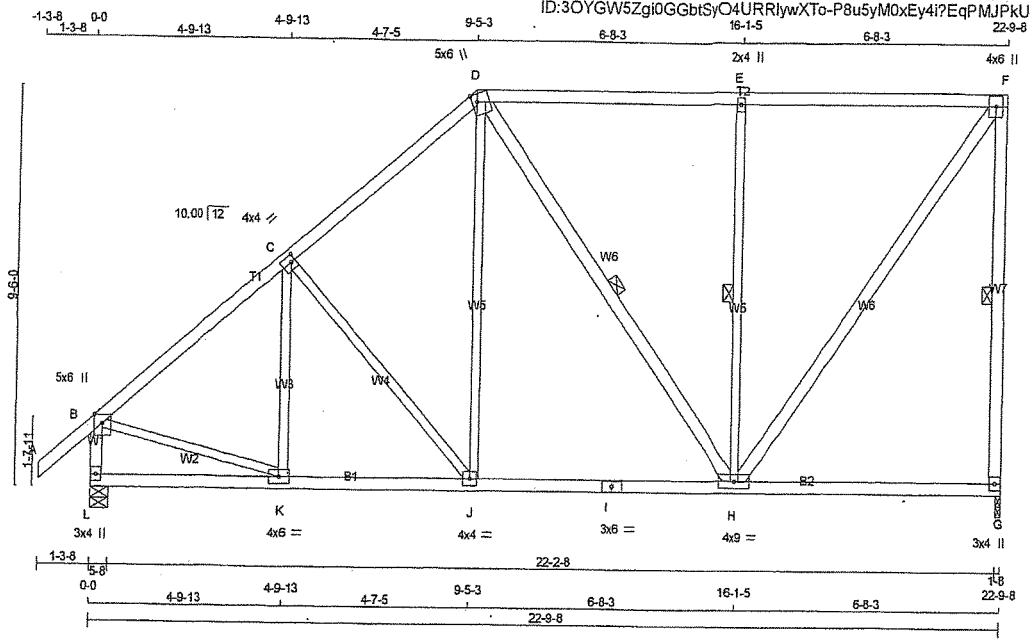


TOTAL WEIGHT = 122 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	8.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1406	0	1406	0	0	1-8	1-8
L	1523	0	1523	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
G	1066	531 / 0	239 / 0	0 / 0	0 / 0	296 / 0	0 / 0
L	1147	595 / 0	239 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

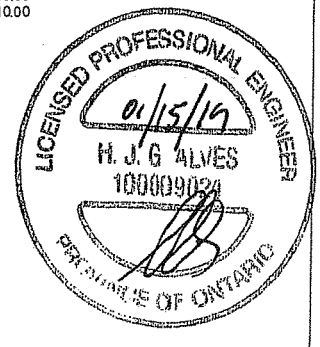
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	0 / 38	-84.9	-84.9	0.12 (1)	10.00	K-C	-91 / 105	0.04 (1)
B-C	-1350 / 0	-84.9	-84.9	0.37 (1)	5.15	C-J	-314 / 0	0.30 (1)
C-D	-1143 / 0	-84.9	-84.9	0.35 (1)	5.50	J-D	0 / 466	0.10 (2)
D-E	-793 / 0	-84.9	-84.9	0.69 (1)	5.53	D-H	-109 / 0	0.08 (1)
E-F	-794 / 0	-84.9	-84.9	0.69 (1)	5.53	H-E	-703 / 0	0.42 (1)
G-F	-1303 / 0	0.0	0.0	0.56 (1)	5.66	H-F	0 / 1346	0.22 (1)
L-B	-1446 / 0	0.0	0.0	0.15 (1)	6.80	B-K	0 / 1099	0.25 (1)
L-K	0 / 0	-38.5	-38.5	0.15 (3)	10.00			
K-J	0 / 1060	-38.5	-38.5	0.31 (2)	10.00			
J-I	0 / 857	-38.5	-38.5	0.44 (2)	10.00			
I-H	0 / 857	-38.5	-38.5	0.44 (2)	10.00			
H-G	0 / 0	-38.5	-38.5	0.34 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.69/1.00 (E-F:1) , BC=0.44/1.00 (H-J:2) , WB=0.42/1.00 (E-H:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

DWG NO. TAM T1901114
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T72	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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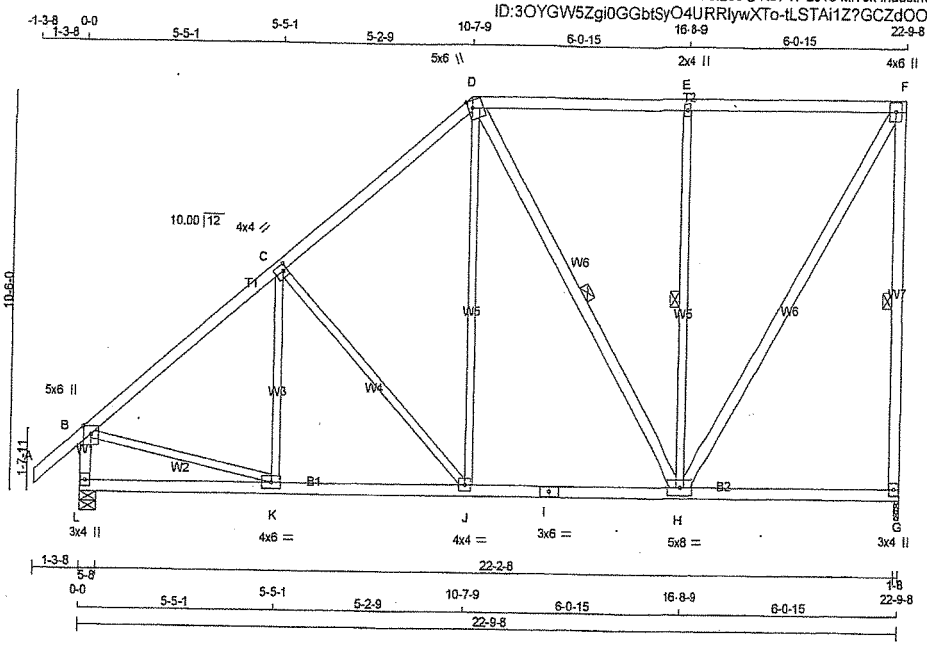


TOTAL WEIGHT = 128 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	ORY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
L - B	2x4	ORY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	ORY	No.2	SPF
H - F	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT2D	5.0	6.0	Edge	
C	TMWW-t	MT2D	4.D	4.0	2.00	1.25
D	TTWW+m	MT2D	5.0	6.0	2.25	1.50
E	TMW+w	MT2D	2.0	4.0		
F	TMVW+p	MT2D	4.0	6.0		
G	BMV1+p	MT2D	3.0	4.0		
H	BMWWW-t	MT2D	5.0	8.0		
I	BS-t	MT2D	3.0	6.0		
J	BMWW-t	MT2D	4.0	4.0		
K	BMWW-t	MT2D	4.0	6.0		
L	BMV1+p	MT2D	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
G	1406	0	1406	0	0	1-8	1-8
L	1523	0	1523	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1066	531 / 0	239 / 0	0 / 0	0 / 0	296 / 0	0 / 0
L	1147	595 / 0	239 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

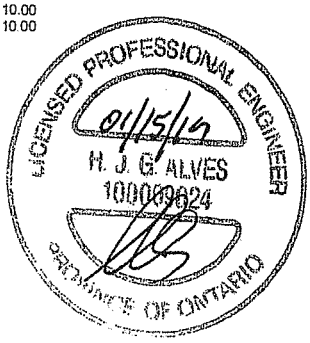
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 38	-84.9	-84.9	0.12 (1)		10.00
B-C	-1353 / 0	-84.9	-84.9	0.49 (1)		4.99
C-D	-1062 / 0	-84.9	-84.9	0.46 (1)		5.49
D-E	-668 / 0	-84.9	-84.9	0.55 (1)		6.24
E-F	-668 / 0	-84.9	-84.9	0.55 (1)		6.24
G-F	-1312 / 0	0.0	0.0	0.72 (1)		5.65
L-B	-1439 / 0	0.0	0.0	0.15 (1)		6.81
L-K	0 / 0	-38.5	-38.5	0.20 (3)		10.00
K-J	0 / 1065	-38.5	-38.5	0.34 (2)		10.00
J-I	0 / 792	-38.5	-38.5	0.37 (2)		10.00
I-H	0 / 792	-38.5	-38.5	0.37 (2)		10.00
H-G	0 / 0	-38.5	-38.5	0.28 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
K-C	-26 / 164	0.04 (3)
C-J	-413 / 0	0.53 (1)
J-D	0 / 525	0.12 (2)
D-H	-243 / 0	0.19 (1)
H-E	-639 / 0	0.49 (1)
H-F	0 / 1300	0.21 (1)
B-K	0 / 1096	0.25 (1)



DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.72/1.00 (F-G:1) , BC=0.37/1.00 (H-J:2) , WB=0.53/1.00 (C-J:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (K) (INPUT = D.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

DWG NO. TAM T72011B
STRUCTURAL
CONSIDERATIONS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T73	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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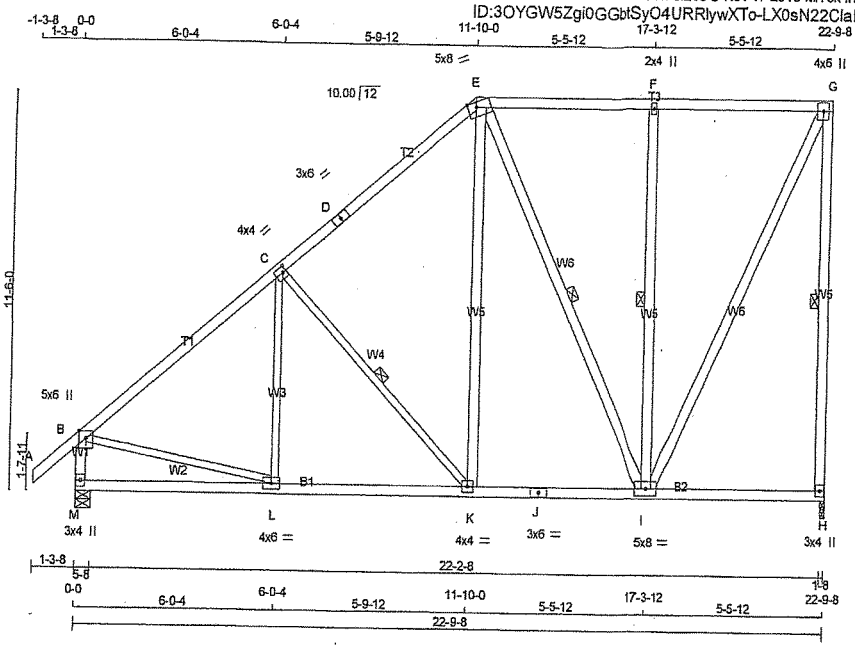


TOTAL WEIGHT = 141 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - E	2x4	ORY	No.2	SPF
E - G	2x4	ORY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
M - J	2x4	ORY	No.2	SPF
J - H	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x4	ORY	No.2	SPF
L - C	2x3	ORY	No.2	SPF
C - K	2x3	ORY	No.2	SPF
B - L	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMW+w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	8.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHDRD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1406	0	1406	0	0	1-8	1-8
M	1523	0	1523	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1066	531 / 0	239 / 0	0 / 0	0 / 0	296 / 0	0 / 0
M	1147	595 / 0	239 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-K, E-I, F-I.

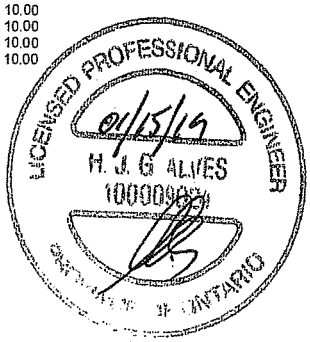
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.9	-84.9	0.12 (1)		10.00
B- C	-1347 / 0	-84.9	-84.9	0.45 (1)		5.07
C- D	-974 / 0	-84.9	-84.9	0.41 (1)		5.77
D- E	-974 / 0	-84.9	-84.9	0.41 (1)		5.77
E- F	-559 / 0	-84.9	-84.9	0.33 (1)		6.25
F- G	-560 / 0	-84.9	-84.9	0.33 (1)		6.25
H- G	-1321 / 0	0.0	0.0	0.93 (1)		5.63
M- B	-1431 / 0	0.0	0.0	0.15 (1)		6.82
M- L	0 / 0	-38.5	-38.5	0.27 (3)		10.00
L- K	0 / 1065	-38.5	-38.5	0.40 (2)		10.00
K- J	0 / 724	-38.5	-38.5	0.30 (2)		10.00
J- I	0 / 724	-38.5	-38.5	0.30 (2)		10.00
I- H	0 / 0	-38.5	-38.5	0.21 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
L- C	0 / 218	0.05 (3)
C- K	-508 / 0	0.26 (1)
K- E	0 / 585	0.09 (2)
E- I	-374 / 0	0.34 (1)
I- F	-575 / 0	0.40 (1)
I- G	0 / 1274	0.20 (1)
B- L	0 / 1090	0.25 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.93/1.00 (G-H:1) , BC=0.40/1.00 (K-L:2) , W8=0.40/1.00 (F-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM T1901116
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T74	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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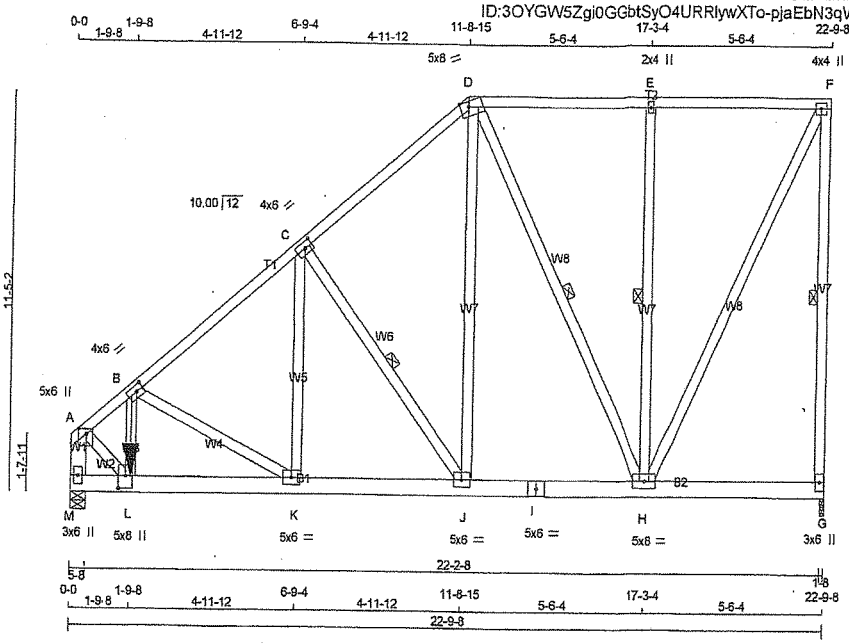


TOTAL WEIGHT = 2 X 164 = 328 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	1	12	TOP
D-F	1	12	TOP
F-G	1	12	TOP
M-A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-I	2	12	TOP-
I-G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.75
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	5.0	8.0	4.25	2.50
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
G	1818	0	1816	0	0	1-8	1-8
M	6214	0	6214	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1375	691 / 0	304 / 0	0 / 0	0 / 0	381 / 0	0 / 0
M	4690	2406 / 0	999 / 0	0 / 0	0 / 0	1286 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-J, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

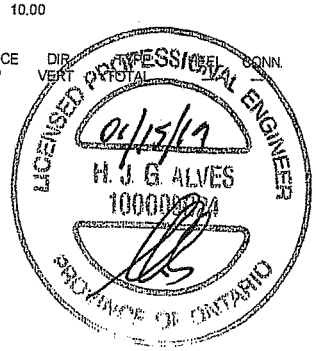
MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-5121 / 0	-84.9	-84.9	0.14 (1)		4.18
B-C	-2563 / 0	-84.9	-84.9	0.23 (1)		5.43
C-D	-1497 / 0	-84.9	-84.9	0.20 (1)		6.25
D-E	-775 / 0	-84.9	-84.9	0.24 (1)		6.25
E-F	-775 / 0	-84.9	-84.9	0.24 (1)		6.25
G-F	-1728 / 0	0.0	0.0	0.58 (1)		6.25
M-A	-6027 / 0	0.0	0.0	0.22 (1)		6.03
M-L	0 / 0	-38.5	-38.5	0.10 (1)		10.00
L-K	0 / 3954	-38.5	-38.5	0.36 (1)		10.00
K-J	0 / 1985	-38.5	-38.5	0.17 (1)		10.00
J-I	0 / 1142	-38.5	-38.5	0.12 (2)		10.00
I-H	0 / 1142	-38.5	-38.5	0.12 (2)		10.00
H-G	0 / 0	-38.5	-38.5	0.06 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-B	0 / 2420	0.21 (1)
B-K	-2242 / 0	0.43 (1)
K-C	0 / 1363	0.12 (1)
C-J	-1439 / 0	0.26 (1)
J-D	0 / 1330	0.12 (1)
D-H	-819 / 0	0.36 (1)
H-E	-579 / 0	0.19 (1)
H-F	0 / 1734	0.15 (1)
A-L	0 / 4785	0.42 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	CONN. TOTAL
L	1-9-8	-5218	-5218	---	TOP	VERT		



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , DBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.58/1.00 (F-G:1) , BC=0.36/1.00 (K-L:1) , WB=0.43/1.00 (B-K:1) , SSI=0.12/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.71 (L) (INPUT = 1.00)

DWG NO. TAM T1901117
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T75	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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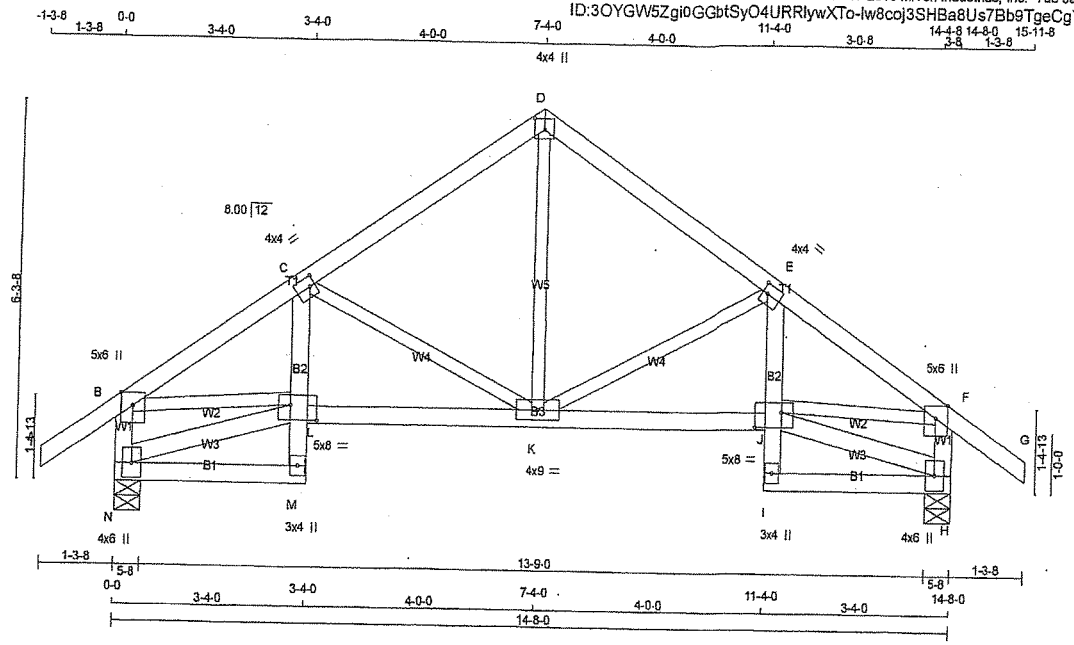


TOTAL WEIGHT = 2 X 72 = 144 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1+p	MT20	4.0	6.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	5.0	8.0	3.00	5.50
K	BMWWW-t	MT20	4.0	9.0		
L	BVMWW-I	MT20	5.0	8.0	3.00	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
N	1021	0	1021	0	0	5-8	5-8
H	1021	0	1021	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	767	405 / 0	154 / 0	0 / 0	0 / 0	207 / 0	0 / 0
H	767	405 / 0	154 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

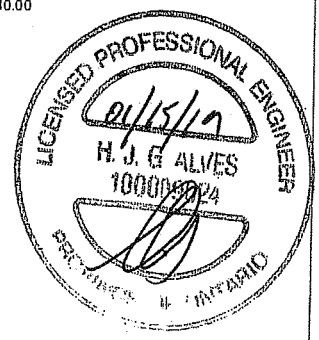
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1260 / 0	-84.9	-84.9	0.15 (1)	5.58
C-D	-838 / 0	-84.9	-84.9	0.17 (1)	6.25
D-E	-838 / 0	-84.9	-84.9	0.17 (1)	6.25
E-F	-1260 / 0	-84.9	-84.9	0.15 (1)	5.58
F-G	0 / 32	-84.9	-84.9	0.11 (1)	10.00
N-B	-949 / 0	0.0	0.0	0.10 (1)	7.81
H-F	-949 / 0	0.0	0.0	0.10 (1)	7.81
N-M	0 / 33	-38.5	-38.5	0.09 (3)	10.00
M-L	0 / 78	0.0	0.0	0.05 (1)	10.00
L-C	0 / 136	0.0	0.0	0.06 (2)	10.00
L-K	0 / 1082	-38.5	-38.5	0.29 (2)	10.00
K-J	0 / 1082	-38.5	-38.5	0.29 (2)	10.00
I-J	0 / 78	0.0	0.0	0.05 (1)	10.00
J-E	0 / 136	0.0	0.0	0.06 (2)	10.00
I-H	0 / 33	-38.5	-38.5	0.09 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
K-D	0 / 621	0.14 (1)
K-E	-451 / 0	0.16 (1)
C-K	-451 / 0	0.16 (1)
N-L	-34 / 0	0.01 (1)
B-L	0 / 1067	0.24 (1)
J-H	-34 / 0	0.01 (1)
J-F	0 / 1067	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS OESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = -L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.17/1.00 (C-D:1) , BC=0.29/1.00 (J-K:2) , WB=0.24/1.00 (F-J:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 1.00)

DWG NO. TAM T7901110
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T76X	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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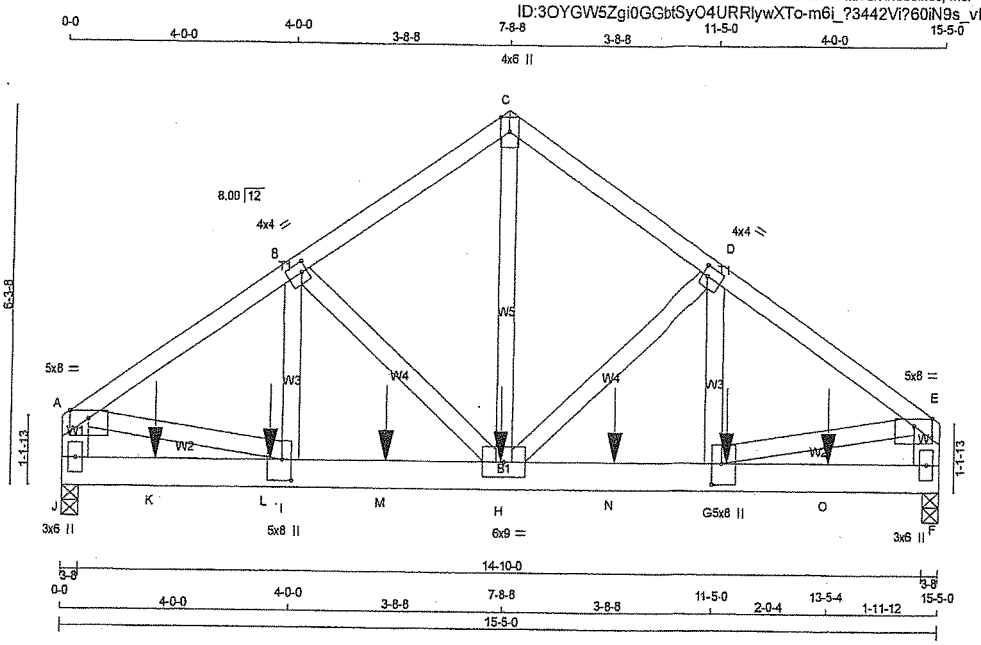


TOTAL WEIGHT = 2 X 83 = 167 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	ORY	No.2	SPF
ALL WEBS EXCEPT	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	TOP
J-A	2	12	TOP
F-E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J-F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
H-C	1	6	SIDE(142.8)
D-G	1	6	SIDE(142.8)
B-I	1	6	SIDE(142.8)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.00
E	TMVW-p	MT20	5.0	8.0	Edge	
F	BMV1+p	MT20	3.0	6.0		
G	BMWW+t	MT20	5.0	8.0	4.25	2.00
H	BMWWW-t	MT20	6.0	9.0		
I	BMWW+t	MT20	5.0	8.0	4.25	2.00
J	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
J	5389	0	5389	0	0	3-8	3-8
F	5257	0	5257	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	4067	2086 / 0	866 / 0	0 / 0	0 / 0	1115 / 0	0 / 0
F	3968	2035 / 0	845 / 0	0 / 0	0 / 0	1088 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-6325 / 0	-84.9	-84.9	0.27 (1)	3.72
B-C	-4740 / 0	-84.9	-84.9	0.17 (1)	4.30
C-D	-4740 / 0	-84.9	-84.9	0.17 (1)	4.30
D-E	-6382 / 0	-84.9	-84.9	0.27 (1)	3.70
J-A	-4647 / 0	0.0	0.0	0.16 (1)	6.70
F-E	-4688 / 0	0.0	0.0	0.17 (1)	6.68
J-K	0 / 0	-38.5	-38.5	0.43 (1)	10.00
K-L	0 / 0	-38.5	-38.5	0.43 (1)	10.00
L-I	0 / 0	-38.5	-38.5	0.43 (1)	10.00
I-M	0 / 5272	-38.5	-38.5	0.61 (1)	10.00
M-H	0 / 5272	-38.5	-38.5	0.61 (1)	10.00
H-N	0 / 5320	-38.5	-38.5	0.60 (1)	10.00
N-G	0 / 5320	-38.5	-38.5	0.60 (1)	10.00
G-O	0 / 0	-38.5	-38.5	0.39 (1)	10.00
O-F	0 / 0	-38.5	-38.5	0.39 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
H-C	0 / 4976	0.44 (1)
H-D	-1885 / 0	0.28 (1)
G-D	0 / 1829	0.16 (1)
B-H	-1820 / 0	0.27 (1)
I-B	0 / 1753	0.15 (1)
A-I	0 / 3866	0.47 (1)
G-E	0 / 5448	0.48 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	CONN
G	11-7-12	-1249	-1249	—	BACK	VERT	TOTAL	—	—
H	7-7-12	-1249	-1249	—	BACK	VERT	TOTAL	—	—
K	1-7-12	-1249	-1249	—	TOP	VERT	TOTAL	—	—
L	3-7-12	-1249	-1249	—	BACK	VERT	TOTAL	—	—
M	5-7-12	-1249	-1249	—	TOP	VERT	TOTAL	—	—
N	9-7-12	-1249	-1249	—	BACK	VERT	TOTAL	—	—
O	13-5-4	-1249	-1249	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.27/1.00 (D-E:1) , BC=0.61/1.00 (H-I:1) , WB=0.48/1.00 (E-G:1) , SSI=0.63/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

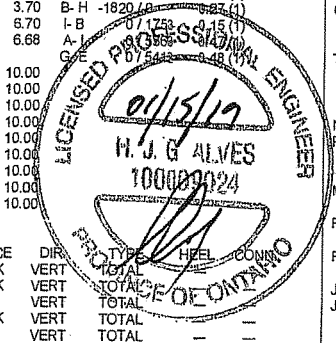
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.96 (G) (INPUT = 1.00)



DWG NO. TAM 1901119
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T77A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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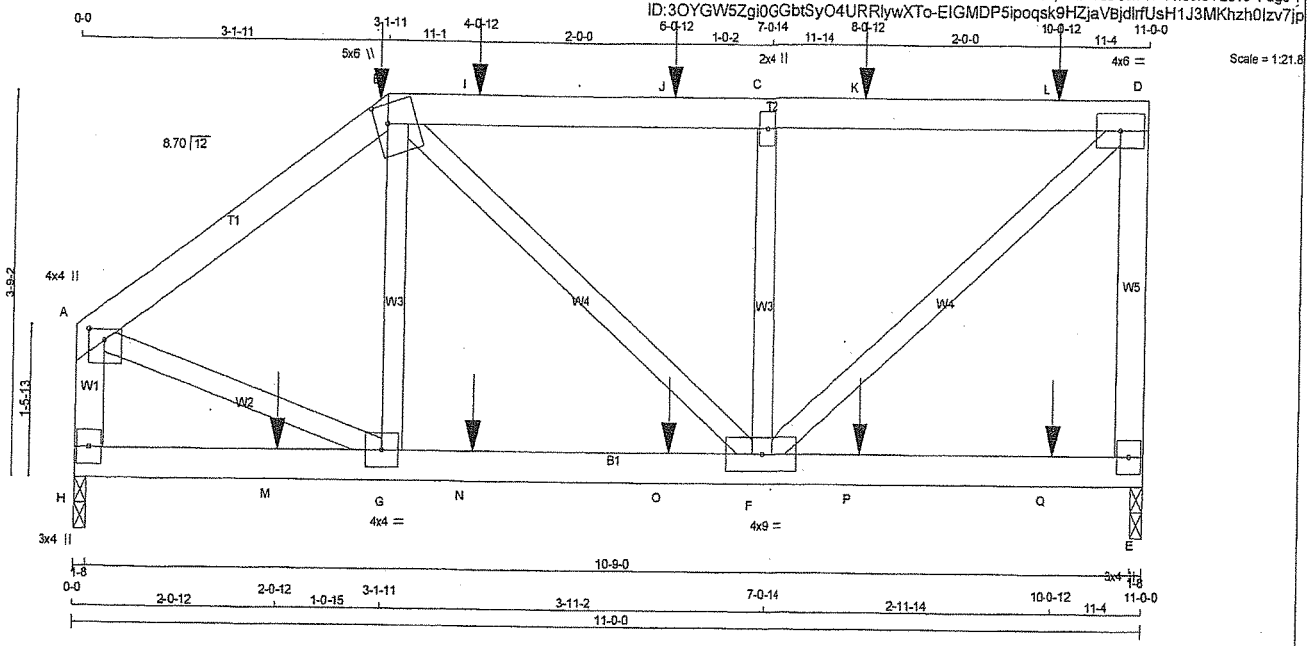

TOTAL WEIGHT = 46 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
E	965	0	965	0	0	1-8	1-8
H	922	0	922	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	731	368 / 0	161 / 0	0 / 0	0 / 0	202 / 0	0 / 0
H	697	353 / 0	152 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-835 / 0	-84.9	-84.9	0.17 (1)	6.25
B- I	-793 / 0	-84.9	-84.9	0.34 (1)	6.25
I- J	-793 / 0	-84.9	-84.9	0.34 (1)	6.25
J- C	-793 / 0	-84.9	-84.9	0.34 (1)	6.25
C- K	-794 / 0	-84.9	-84.9	0.34 (1)	6.25
K- L	-794 / 0	-84.9	-84.9	0.34 (1)	6.25
L- D	-794 / 0	-84.9	-84.9	0.34 (1)	6.25
E- D	-876 / 0	0.0	0.0	0.20 (1)	7.81
H- A	-866 / 0	0.0	0.0	0.10 (1)	7.81
H- M	0 / 0	-38.5	-38.5	0.12 (3)	10.00
M- G	0 / 0	-38.5	-38.5	0.12 (3)	10.00
G- N	0 / 673	-38.5	-38.5	0.26 (2)	10.00
N- O	0 / 873	-38.5	-38.5	0.26 (2)	10.00
O- F	0 / 673	-38.5	-38.5	0.26 (2)	10.00
F- P	0 / 0	-38.5	-38.5	0.17 (3)	10.00
P- Q	0 / 0	-38.5	-38.5	0.17 (3)	10.00
Q- E	0 / 0	-38.5	-38.5	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-42 / 127	0.03 (3)
B- F	0 / 162	0.04 (1)
F- C	-571 / 0	0.14 (1)
F- D	0 / 1054	0.26 (1)
A- G	0 / 1054	0.18 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-1-11	-20	-22	—	FRONT	VERT	DEAD	—	—
B	3-1-11	-93	-93	—	FRONT	VERT	SNOW	—	—
I	4-0-12	-69	-69	—	BACK	VERT	TOTAL	—	—
J	6-0-12	-65	-65	—	BACK	VERT	TOTAL	—	—
K	8-0-12	-65	-65	—	BACK	VERT	TOTAL	—	—
L	10-0-12	-68	-68	—	BACK	VERT	TOTAL	—	—
M	2-0-12	-30	-38	—	BACK	VERT	TOTAL	—	—
N	4-0-12	-30	-38	—	BACK	VERT	TOTAL	—	—
O	6-0-12	-30	-38	—	BACK	VERT	TOTAL	—	—
P	8-0-12	-30	-38	—	BACK	VERT	TOTAL	—	—
Q	10-0-12	-31	-39	—	BACK	VERT	TOTAL	—	—

DWG NO. TAM 7901120
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.34/1.00 (C-D:1) , BC=0.26/1.00 (F-G:2) , WB=0.26/1.00 (D-F:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.25 (G) (INPUT = 1.00)

PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO
01/15/19
N. J. G. ALVES
100000024

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T78A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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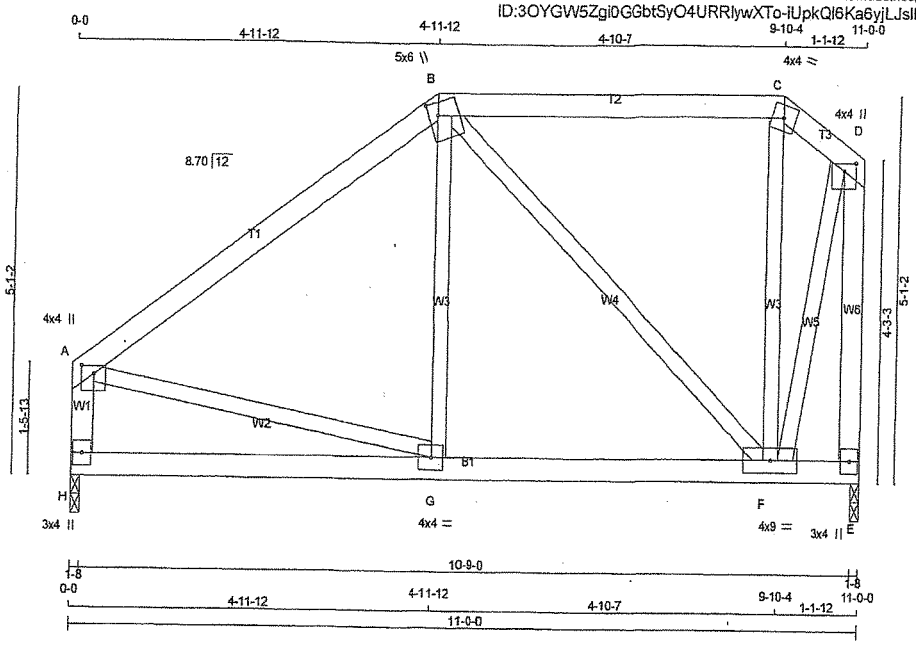


TOTAL WEIGHT = 52 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW+m	MT20	5.0	6.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	678	0	678	0	0	1-8	1-8
E	678	0	678	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	515	256 / 0	116 / 0	0 / 0	0 / 0	143 / 0	0 / 0
E	515	256 / 0	116 / 0	0 / 0	0 / 0	143 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-503 / 0	-84.9	-84.9	0.27 (1)	6.25	G- B	0 / 230	0.05 (3)
B- C	-160 / 0	-84.9	-84.9	0.26 (1)	6.25	B- F	-354 / 0	0.29 (1)
C- D	-202 / 0	-84.9	-84.9	0.01 (1)	6.25	F- C	-135 / 22	0.05 (1)
H- A	-600 / 0	0.0	0.0	0.06 (1)	7.81	A- G	0 / 417	0.09 (1)
E- D	-681 / 0	0.0	0.0	0.21 (1)	7.81	F- D	0 / 537	0.12 (1)
H- G	0 / 0	-38.5	-38.5	0.19 (3)	10.00			
G- F	0 / 409	-38.5	-38.5	0.23 (2)	10.00			
F- E	0 / 0	-38.5	-38.5	0.07 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.27/1.00 (A-B:1) , BC=0.23/1.00 (F-G:2) , WB=0.29/1.00 (B-F:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

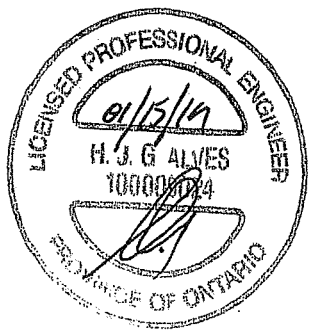
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (D) (INPUT = 0.90)
JSI METAL= 0.15 (G) (INPUT = 1.00)



DWG NO. TAM T7901121
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO
300400	T79A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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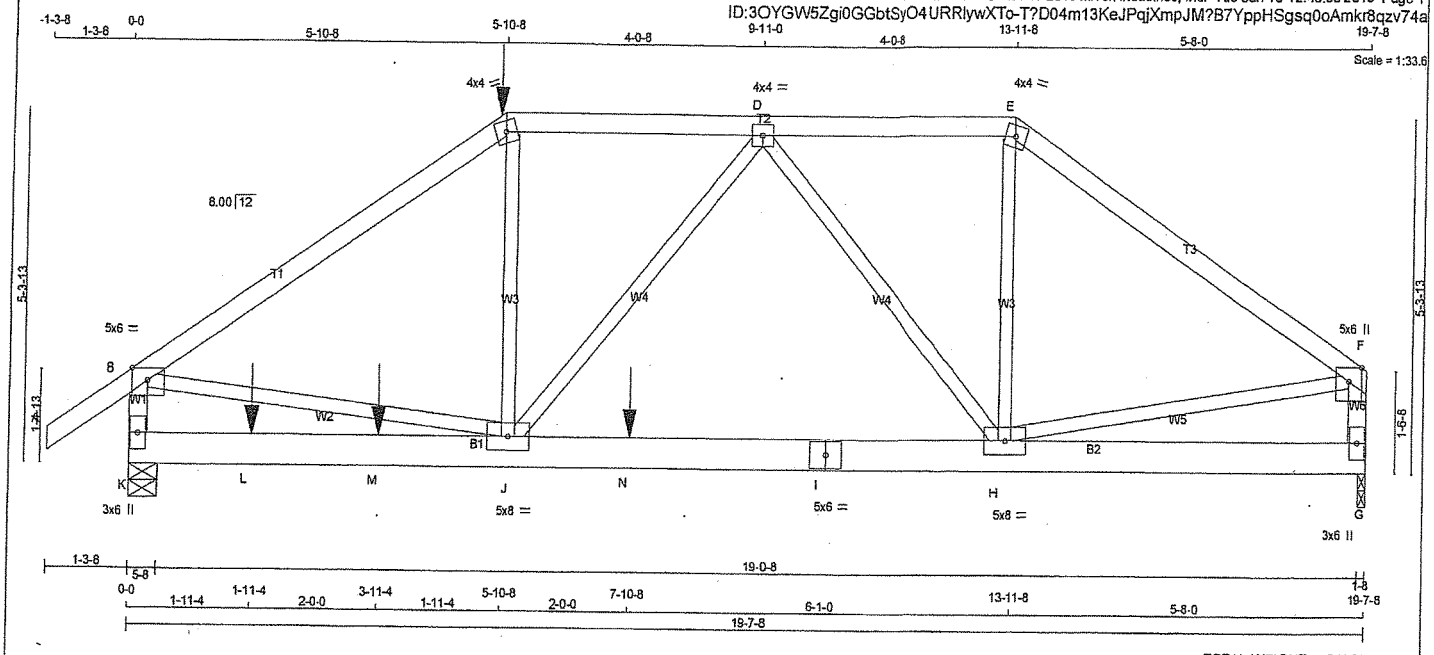


TOTAL WEIGHT = 2 X 91 = 181 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- F	1	12	TOP
K- B	1	12	TOP
G- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- I	2	12	SIDE(0.0)
I- G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
J- C	1	6	SIDE(19.6)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	2696	0	2696	0	0	5-8	5-8
G	1973	0	1973	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	0EAO	SOIL
K	2016	1090 / 0	385 / 0	0 / 0	0 / 0	541 / 0	0 / 0
G	1479	788 / 0	292 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

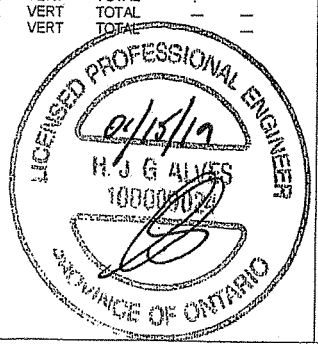
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00	J- C	0 / 919	0.11 (2)
B- C	-3330 / 0	-84.9	-84.9	0.40 (1)	4.74	J- D	0 / 372	0.05 (1)
C- D	-2799 / 0	-84.9	-84.9	0.15 (1)	5.33	D- H	-882 / 0	0.32 (1)
D- E	-2026 / 0	-84.9	-84.9	0.13 (1)	6.04	H- E	0 / 953	0.12 (1)
E- F	-2398 / 0	-84.9	-84.9	0.32 (1)	5.45	B- J	0 / 2805	0.35 (1)
K- B	-2674 / 0	0.0	0.0	0.15 (1)	8.97	H- F	0 / 2030	0.25 (1)
G- F	-1962 / 0	0.0	0.0	0.11 (1)	7.81			
K- L	0 / 0	-38.5	-38.5	0.38 (1)	10.00			
L- M	0 / 0	-38.5	-38.5	0.38 (1)	10.00			
M- J	0 / 0	-38.5	-38.5	0.38 (1)	10.00			
J- N	0 / 2570	-38.5	-38.5	0.64 (1)	10.00			
N- I	0 / 2570	-38.5	-38.5	0.64 (1)	10.00			
I- H	0 / 2570	-38.5	-38.5	0.64 (1)	10.00			
H- G	0 / 0	-38.5	-38.5	0.22 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-543	-543	—	BACK	VERT	TOTAL	—	—
L	1-11-4	-75	-95	—	BACK	VERT	TOTAL	—	—
M	3-11-4	-75	-95	—	BACK	VERT	TOTAL	—	—
N	7-10-8	-1440	-1440	—	TOP	VERT	TOTAL	—	—



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.64/1.00 (H-J:1) , WB=0.35/1.00 (B-J:1) , SSI=0.43/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.47 (F) (INPUT = 1.00)

DWG NO. TAM T79 01/29
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T80A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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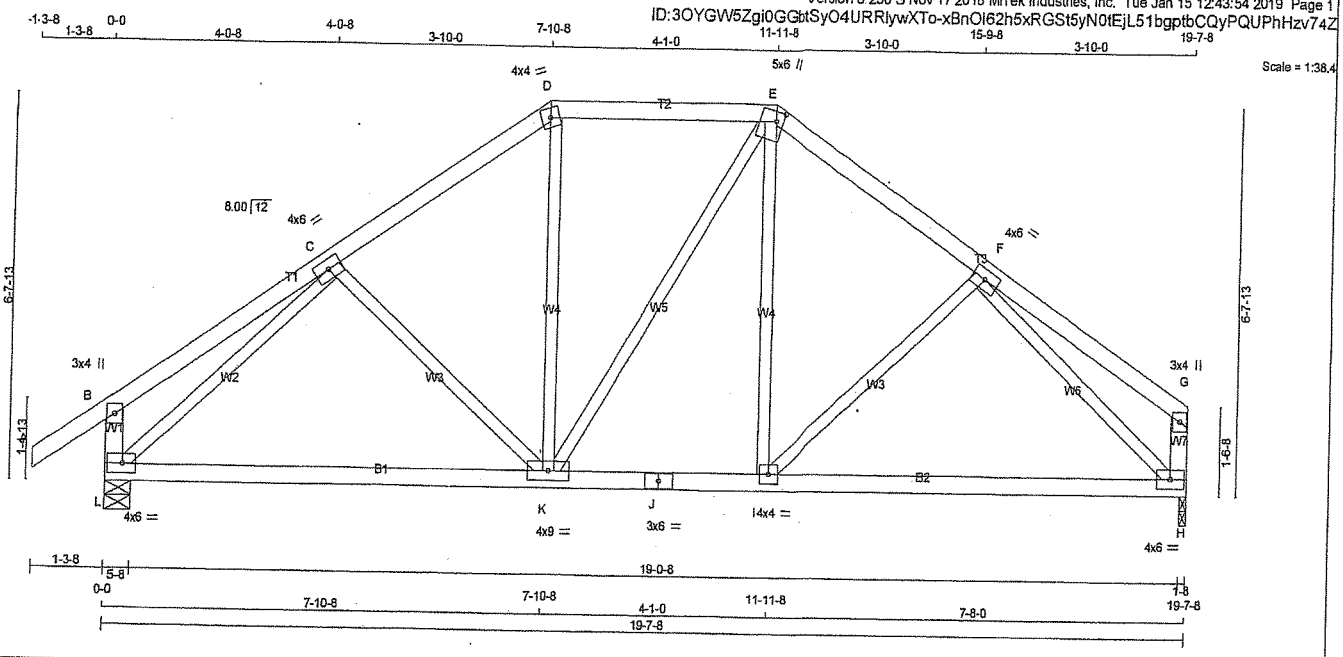


TOTAL WEIGHT = 85 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY. SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
L	1327	0	1327	0	0	5-8	5-8
H	1210	0	1210	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0
H	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

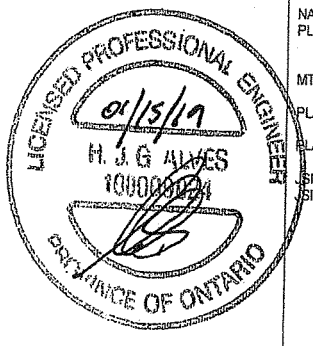
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 21	-84.9	-84.9	0.20 (1)	10.00
C-D	-1135 / 0	-84.9	-84.9	0.17 (1)	5.78
D-E	-931 / 0	-84.9	-84.9	0.19 (1)	6.19
E-F	-1122 / 0	-84.9	-84.9	0.16 (1)	5.82
F-G	0 / 21	-84.9	-84.9	0.19 (1)	10.00
L-B	-246 / 0	0.0	0.0	0.03 (1)	7.81
H-G	-120 / 0	0.0	0.0	0.01 (1)	7.81
L-K	0 / 1011	-38.5	-38.5	0.52 (2)	10.00
K-J	0 / 920	-38.5	-38.5	0.47 (2)	10.00
J-I	0 / 920	-38.5	-38.5	0.47 (2)	10.00
I-H	0 / 969	-38.5	-38.5	0.50 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-K	-115 / 49	0.06 (1)
K-D	0 / 343	0.08 (2)
K-E	0 / 20	0.00 (1)
I-E	0 / 318	0.07 (2)
I-F	-72 / 66	0.04 (1)
L-C	-1389 / 0	0.66 (1)
F-H	-1366 / 0	0.61 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 821 (0.29")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.52/1.00 (K-L:2) , WB=0.66/1.00 (C-L:1) , SSI=0.19/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (J) (INPUT = 0.90)
SI METAL= 0.62 (J) (INPUT = 1.00)

DWG NO. TAM 7190130
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T81	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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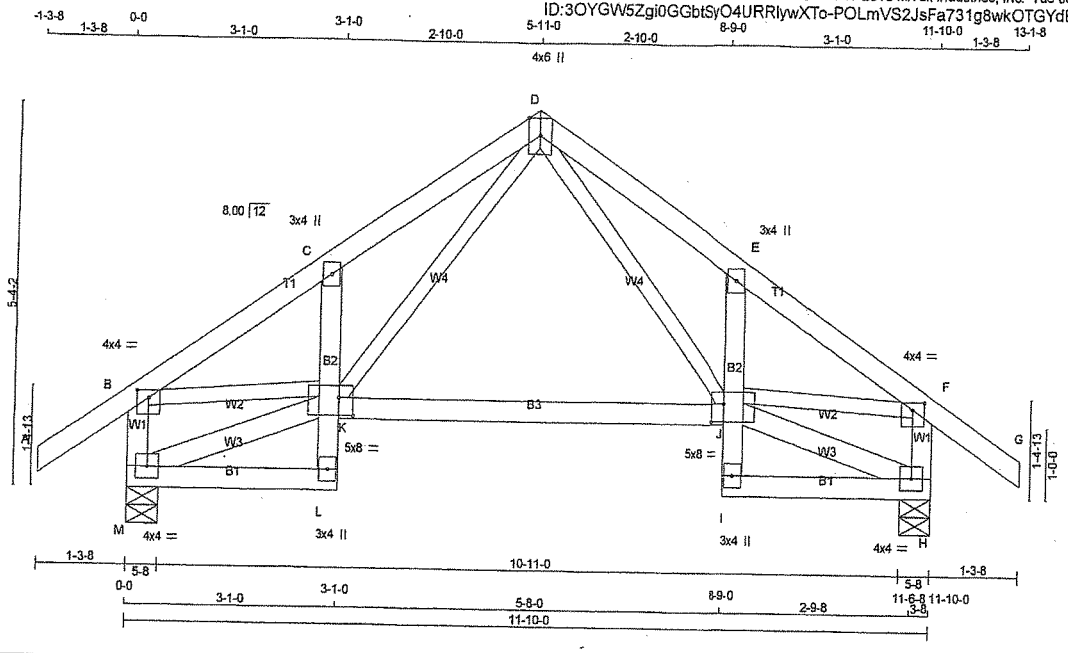


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	TMVW-p	MT20	4.0	4.0	1.25	2.00
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
K	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	846	0	846	0	0	5-8	5-8
H	846	0	846	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	634	339 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0
H	634	339 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

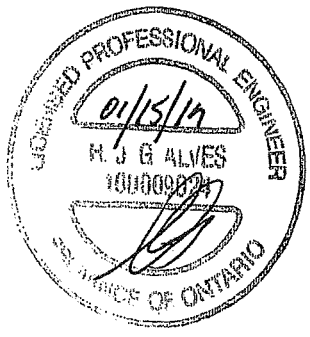
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32		-84.9	-84.9	0.11 (1)		10.00	D- J	0 / 543	0.12 (1)
B- C	-968 / 0		-84.9	-84.9	0.08 (1)		6.24	K- D	0 / 543	0.12 (1)
C- D	-994 / 0		-84.9	-84.9	0.10 (1)		6.16	M- K	-27 / 0	0.00 (1)
D- E	-994 / 0		-84.9	-84.9	0.10 (1)		6.16	J- H	-27 / 0	0.00 (1)
E- F	-968 / 0		-84.9	-84.9	0.08 (1)		6.24	B- K	0 / 817	0.18 (1)
F- G	0 / 32		-84.9	-84.9	0.11 (1)		10.00	J- F	0 / 817	0.18 (1)
M- B	-781 / 0		0.0	0.0	0.08 (1)		7.81			
H- F	-781 / 0		0.0	0.0	0.08 (1)		7.81			
M- L	0 / 25		-38.5	-38.5	0.08 (3)		10.00			
L- K	0 / 72		0.0	0.0	0.04 (1)		10.00			
K- C	-296 / 0		0.0	0.0	0.03 (1)		7.81			
K- J	0 / 494		-38.5	-38.5	0.36 (2)		10.00			
I- J	0 / 72		0.0	0.0	0.04 (1)		10.00			
J- E	-296 / 0		0.0	0.0	0.03 (1)		7.81			
I- H	0 / 25		-38.5	-38.5	0.08 (3)		10.00			



TOTAL WEIGHT = 2 X 60 = 120 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS.

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 894 (0.16")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.36/1.00 (J-K:2) , WB=0.18/1.00 (F-J:1) , SSI=0.12/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

DWG NO. TAM T1901131
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T82	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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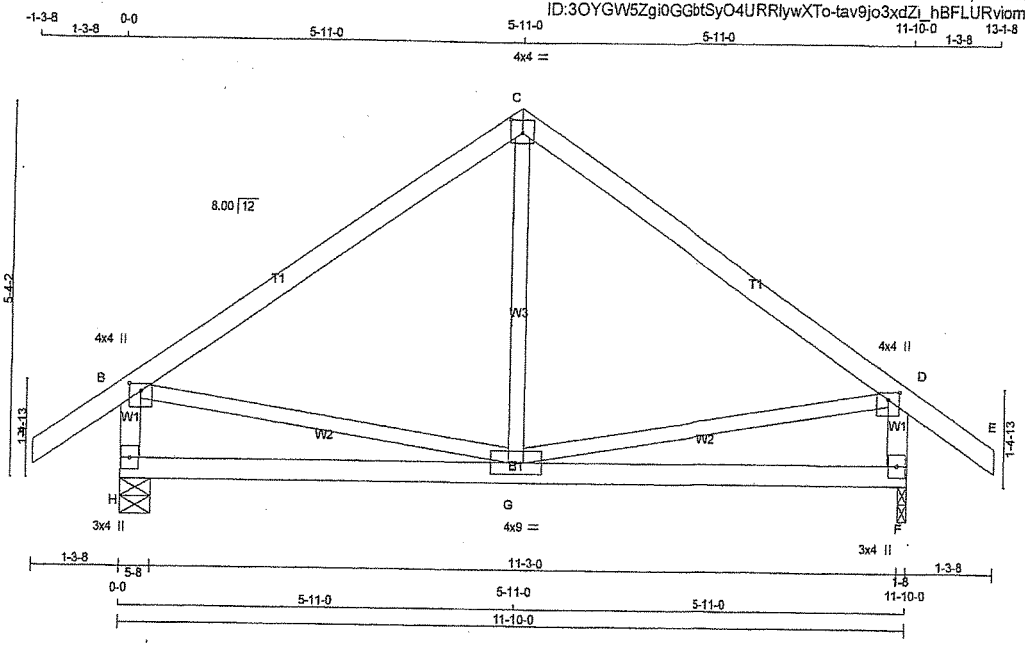


TOTAL WEIGHT = 2 X 49 = 98 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	846	0	846	0	0	5-8	5-8
F	846	0	846	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	634	339 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0
F	634	339 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

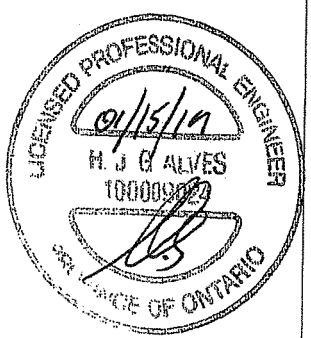
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11	(1)		10.00
B- C	-555 / 0	-84.9	-84.9	0.39	(1)		6.25
C- D	-555 / 0	-84.9	-84.9	0.39	(1)		6.25
D- E	0 / 32	-84.9	-84.9	0.11	(1)		10.00
H- B	-760 / 0	0.0	0.0	0.08	(1)		7.81
F- D	-780 / 0	0.0	0.0	0.08	(1)		7.81
H- G	0 / 0	-38.5	-38.5	0.31	(3)		10.00
G- F	0 / 0	-38.5	-38.5	0.31	(3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
G- C	0 / 264	0.06	(3)
B- G	0 / 469	0.11	(1)
G- D	0 / 469	0.11	(1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.39/1.00 (C-D:1) , BC=0.31/1.00 (G-H:3) , WB=0.11/1.00 (B-G:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

DWG NO. TAM 9190/132
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T82A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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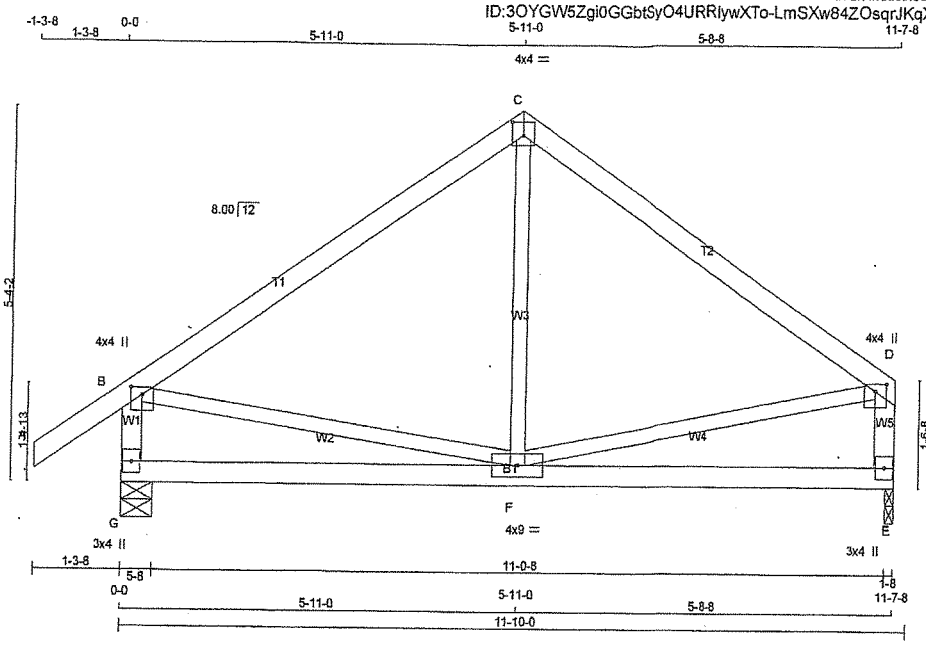

TOTAL WEIGHT = 47 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
O	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
G	834	0	834	0	0	5-8	5-8
E	717	0	717	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	624	335 / 0	122 / 0	0 / 0	0 / 0	168 / 0	0 / 0
E	544	271 / 0	122 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00	F- C	0 / 253	0.06 (3)
B- C	-536 / 0	-84.9	-84.9	0.39 (1)		6.25	B- F	0 / 453	0.10 (1)
C- D	-536 / 0	-84.9	-84.9	0.36 (1)		6.25	F- D	0 / 456	0.10 (1)
G- B	-746 / 0	0.0	0.0	0.08 (1)		7.81			
E- D	-634 / 0	0.0	0.0	0.07 (1)		7.81			
G- F	0 / 0	-38.5	-38.5	0.29 (3)		10.00			
F- E	0 / 0	-38.5	-38.5	0.29 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS.

TOP	CH.	LL =	23.3	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.29/1.00 (F-G:3) , WB=0.10/1.00 (D-F:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.59 (B) (INPUT = 0.90)
SI METAL= 0.17 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
M. J. G. ALVES
100
PROVINCE OF ONTARIO

DWG NO. TAM T1901133
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300401	T83	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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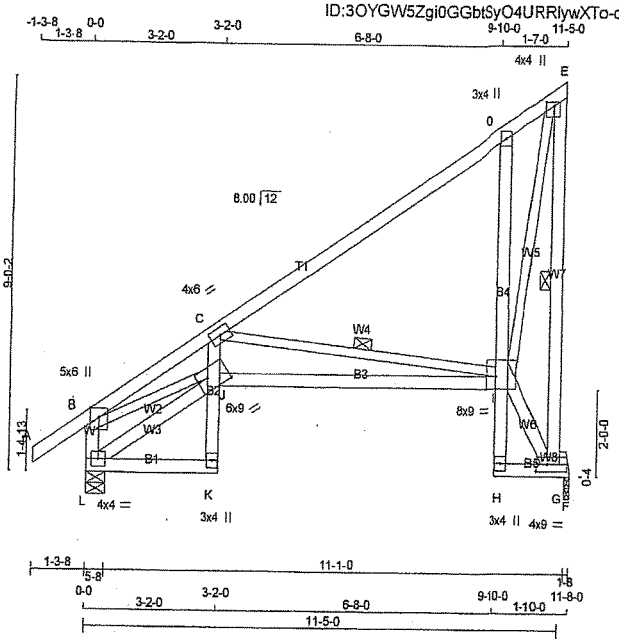


TOTAL WEIGHT = 3 X 73 = 219 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	ORY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - B	2x4	ORY	No.2	SPF
L - K	2x4	ORY	No.2	SPF
K - C	2x4	DRY	No.2	SPF
J - I	2x4	ORY	No.2	SPF
H - D	2x4	ORY	No.2	SPF
H - F	2x4	ORY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - J	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.25
G	BMVWW-t	MT20	4.0	9.0	2.00	3.25
H	BMV+p	MT20	3.0	4.0		
I	BVMWWW-I	MT20	8.0	9.0	Edge	3.25
J	BVMWW+w	MT20	6.0	9.0	6.00	3.50
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EOGE OF CHDRD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATDR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	699	0	699	0	0	1-8	1-8
L	836	0	836	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	531	260 / 0	123 / 0	0 / 0	0 / 0	149 / 0	0 / 0
L	626	335 / 0	123 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

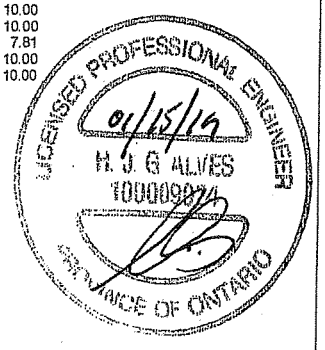
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-G, C-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C- I	-1436 / 0	0.48 (1)
B- C	-1844 / 0	-84.9	-84.9	0.33 (1)	4.57	I- G	-32 / 0	0.00 (1)
C- D	-279 / 0	-84.9	-84.9	0.44 (1)	6.25	I- E	0 / 1072	0.24 (1)
D- E	-385 / 0	-84.9	-84.9	0.43 (1)	6.25	L- J	-61 / 0	0.01 (1)
G- E	-756 / 0	0.0	0.0	0.28 (1)	6.25	B- J	0 / 1637	0.37 (1)
L- B	-744 / 0	0.0	0.0	0.08 (1)	7.81			
L- K	0 / 51	-38.5	-38.5	0.09 (3)	10.00			
K- J	0 / 74	0.0	0.0	0.15 (1)	10.00			
J- C	0 / 635	0.0	0.0	0.24 (1)	10.00			
J- I	0 / 1658	-38.5	-38.5	0.64 (2)	10.00			
H- I	0 / 148	0.0	0.0	0.07 (1)	10.00			
I- D	-569 / 0	0.0	0.0	0.25 (1)	7.81			
H- G	0 / 19	-38.5	-38.5	0.24 (1)	10.00			
G- F	0 / 0	-38.5	-38.5	0.10 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 747 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 443 (0.32")

CSI: TC=0.44/1.00 (C-D:1) , BC=0.64/1.00 (I-J:2) , WB=0.48/1.00 (C-I:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATIDN TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.73 (B) (INPUT = 1.00)

DWG NO. TAM 7790/134
STRUCTURAL
FOR CONSENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	RWG NO.
300402	G85	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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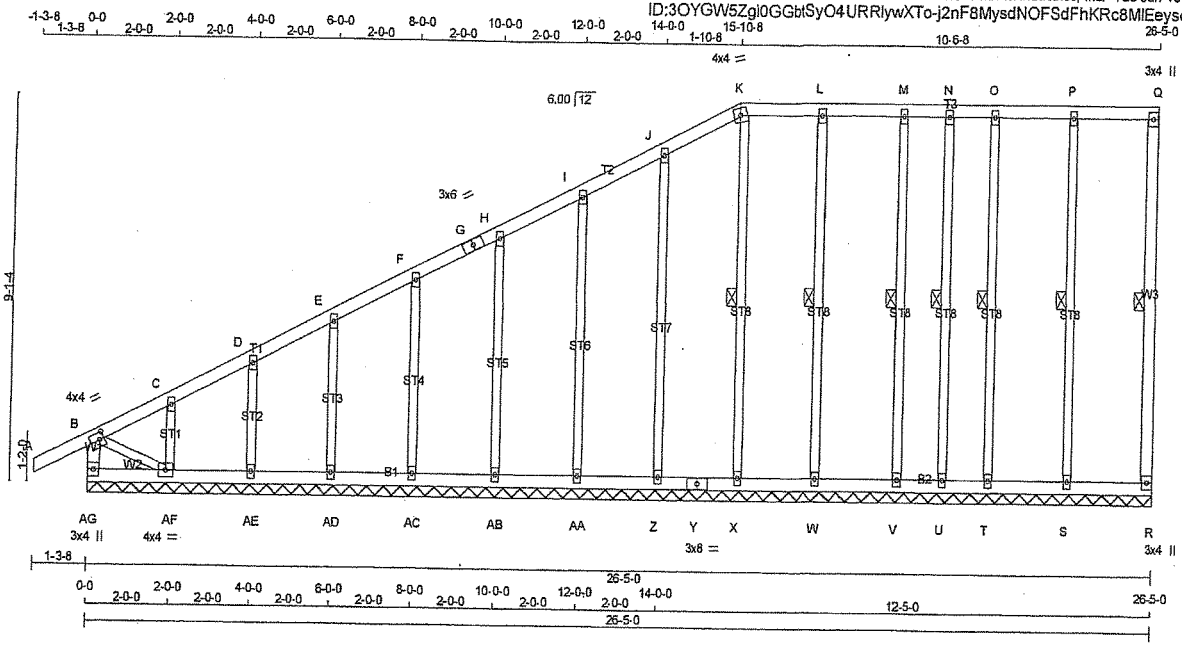


TOTAL WEIGHT = 2 X 144 = 288 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	ORY	No.2	SPF
G - K	2x4	ORY	No.2	SPF
K - Q	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
AG- B	2x4	DRY	No.2	SPF
AG- Y	2x4	ORY	No.2	SPF
Y - R	2x4	ORY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C, D, E, F, H, I, J, L, M, N, O, P						
C	TMV+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
K	TTW-m	MT20	4.0	4.0		
Q	TMV+p	MT20	3.0	4.0		
R	BMV1+p	MT20	3.0	4.0		
S, T, U, V, W, X, Z, AA, AB, AC, AD, AE						
S	BMW1+w	MT20	2.0	4.0		
Y	BS-t	MT20	3.0	6.0		
AF	BMWW1-t	MT20	4.0	4.0		
AG	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF Q-R, K-X, L-W, M-V, N-U, P-S, O-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

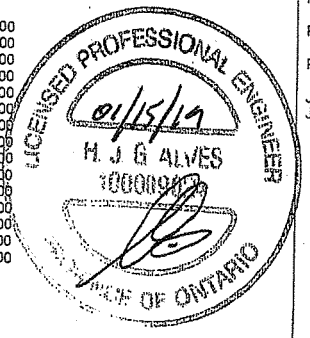
TOTAL LOAD CASES. (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-17 / 0	-84.9	-84.9	0.05 (1)	6.25
C- D	-19 / 0	-84.9	-84.9	0.05 (1)	6.25
D- E	-13 / 0	-84.9	-84.9	0.04 (1)	6.25
E- F	-10 / 0	-84.9	-84.9	0.04 (1)	6.25
F- G	-7 / 0	-84.9	-84.9	0.04 (1)	10.00
G- H	-7 / 0	-84.9	-84.9	0.04 (1)	10.00
H- I	-5 / 0	-84.9	-84.9	0.04 (1)	10.00
I- J	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
J- K	-9 / 0	-84.9	-84.9	0.04 (1)	10.00
K- L	0 / 0	-84.9	-84.9	0.05 (1)	10.00
L- M	0 / 0	-84.9	-84.9	0.05 (1)	10.00
M- N	0 / 0	-84.9	-84.9	0.03 (1)	10.00
N- O	0 / 0	-84.9	-84.9	0.03 (1)	10.00
O- P	0 / 0	-84.9	-84.9	0.05 (1)	10.00
P- Q	0 / 0	-84.9	-84.9	0.05 (1)	10.00
R- Q	-75 / 0	0.0	0.0	0.03 (1)	6.25
AG- B	-200 / 0	0.0	0.0	0.02 (1)	7.81
AG-AF	0 / 0	-38.5	-38.5	0.03 (3)	10.00
AF-AE	0 / 18	-38.5	-38.5	0.03 (3)	10.00
AE-AD	0 / 12	-38.5	-38.5	0.02 (3)	10.00
AD-AC	0 / 9	-38.5	-38.5	0.02 (3)	10.00
AC-AB	0 / 7	-38.5	-38.5	0.02 (3)	10.00
AB-AA	0 / 5	-38.5	-38.5	0.02 (3)	10.00
AA- Z	0 / 3	-38.5	-38.5	0.02 (3)	10.00
Z- Y	0 / 2	-38.5	-38.5	0.02 (3)	10.00
Y- X	0 / 2	-38.5	-38.5	0.02 (3)	10.00
X- W	0 / 0	-38.5	-38.5	0.03 (3)	10.00
W- V	0 / 0	-38.5	-38.5	0.03 (3)	10.00
V- U	0 / 0	-38.5	-38.5	0.02 (3)	10.00
U- T	0 / 0	-38.5	-38.5	0.02 (3)	10.00
T- S	0 / 0	-38.5	-38.5	0.03 (3)	10.00
S- R	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
AF- C	-185 / 0	0.03 (1)
AE- D	-164 / 0	0.03 (1)
AD- E	-189 / 0	0.04 (1)
AC- F	-168 / 0	0.07 (1)
AB- H	-169 / 0	0.10 (1)
AA- I	-166 / 0	0.15 (1)
Z- J	-185 / 0	0.24 (1)
X- K	-137 / 0	0.07 (1)
W- L	-201 / 0	0.11 (1)
V- M	-130 / 0	0.07 (1)
U- N	-87 / 0	0.05 (1)
S- P	-200 / 0	0.11 (1)
T- O	-129 / 0	0.07 (1)
B-AF	0 / 24	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (S-T:3) , WB=0.24/1.00 (J-Z:1) , SSI=0.08/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.08 (J) (INPUT = 1.00)

DWG NO. TAM T1901138
STRUCTURAL
CONFIDENTIALITY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	T86	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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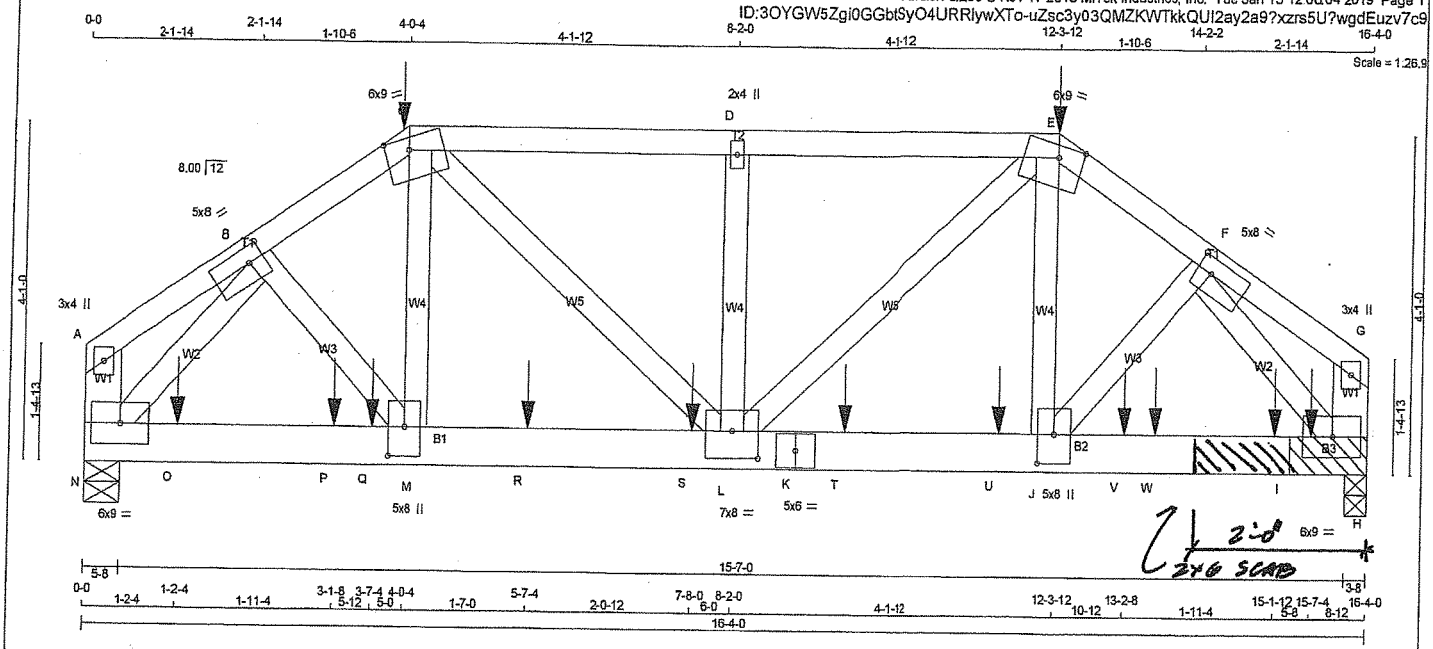

TOTAL WEIGHT = 2 X 90 = 180 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	ORY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	ORY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	ORY	No.2	SPF
K - H	2x6	ORY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT. SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-E	1	12	SIDE(61.0)
E-G	1	12	SIDE(61.0)
N-A	2	12	TOP
H-G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N-K	2	12	SIDE(19.9)
K-H	2	12	SIDE(19.9)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	8.0	2.25	2.25
C	TTWW-m	MT20	6.0	9.0	1.75	3.75
O	TMV+w	MT20	2.0	4.0		
E	TTWW-m	MT20	6.0	9.0	1.75	3.75
F	TMWW-t	MT20	5.0	8.0	2.25	2.25
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	6.0	9.0		
J	BMWW+t	MT20	5.0	8.0	4.25	2.50
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	7.0	8.0	4.00	4.00
M	BMWW+t	MT20	5.0	8.0	4.25	2.50
N	BMVW1-t	MT20	6.0	9.0		

DWG NO. TAM 1901122

STRUCTURAL COMPONENT ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	5339	0	5339	0	0	IN-SX	IN-SX
H	7237	0	7237	0	0	3-8	3-8 & BLDCK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	4019	2095 / 0	830 / 0	0 / 0	0 / 0	1093 / 0	0 / 0
H	5453	2824 / 0	1141 / 0	0 / 0	0 / 0	1488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

2x6 DRY SPF No.2 BEARING BLOCK 24" LONG AT JT. H ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 3" O.C. 21 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-14 / 0	-84.9	-84.9	0.05 (1)	6.25
B-C	-6563 / 0	-84.9	-84.9	0.19 (1)	3.70
C-D	-7817 / 0	-84.9	-84.9	0.33 (1)	3.30
D-E	-7817 / 0	-84.9	-84.9	0.33 (1)	3.30
E-F	-7595 / 0	-84.9	-84.9	0.25 (1)	3.39
F-G	-20 / 0	-84.9	-84.9	0.07 (1)	6.25
N-A	-110 / 0	0.0	0.0	0.00 (1)	7.81
H-G	-122 / 0	0.0	0.0	0.00 (1)	7.81
N-O	0 / 4336	-38.5	-38.5	0.56 (1)	10.00
O-P	0 / 4336	-38.5	-38.5	0.56 (1)	10.00
P-Q	0 / 4336	-78.3	-78.3	0.56 (1)	10.00
Q-M	0 / 4336	-78.3	-78.3	0.56 (1)	10.00
M-R	0 / 5500	-78.3	-78.3	0.63 (1)	10.00
R-S	0 / 5500	-78.3	-78.3	0.63 (1)	10.00
S-L	0 / 5500	-78.3	-78.3	0.63 (1)	10.00
L-K	0 / 6416	-78.3	-78.3	0.96 (1)	10.00
K-T	0 / 6416	-78.3	-78.3	0.96 (1)	10.00
T-U	0 / 6418	-78.3	-78.3	0.96 (1)	10.00
U-J	0 / 6416	-78.3	-78.3	0.96 (1)	10.00
J-V	0 / 5001	-78.3	-78.3	0.87 (1)	10.00
V-W	0 / 5001	-38.5	-38.5	0.87 (1)	10.00
W-I	0 / 5001	-38.5	-38.5	0.87 (1)	10.00
I-H	0 / 5001	-38.5	-38.5	0.87 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-M	0 / 1659	0.15 (1)
M-C	0 / 1179	0.10 (1)
C-L	0 / 3103	0.27 (1)
L-D	-349 / 0	0.03 (1)
L-E	0 / 1877	0.17 (1)
J-E	0 / 2564	0.23 (1)
J-F	0 / 1949	0.17 (1)
N-B	-8635 / 0	0.46 (1)
F-H	-7654 / 0	0.59 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-4	-26	-29	—	FRONT	VERT	DEAD	—	—
C	4-0-4	-122	-122	—	FRONT	VERT	SNOW	—	—
E	12-3-12	-26	-29	—	FRONT	VERT	DEAD	—	—
E	12-3-12	-122	-122	—	FRONT	VERT	SNOW	—	—
I	15-1-12	-30	-39	—	BACK	VERT	TOTAL	—	—
I	15-7-4	-1280	-1280	—	TOP	VERT	TOTAL	—	—
O	1-2-4	-30	-39	—	BACK	VERT	TOTAL	—	—
P	3-1-8	-608	-608	—	BACK	VERT	TOTAL	—	—
O	3-7-4	-927	-927	—	TOP	VERT	TOTAL	—	—
R	5-7-4	-640	-640	—	TOP	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 3-1-8
START SPAN CARRIED = 3-7-0
END DISTANCE = 13-2-8
END SPAN CARRIED = 3-7-0
END WALL WIDTH = 3-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.33/1.00 (C-D:1) , BC=0.96/1.00 (J-L:1) , WB=0.53/1.00 (F-H:1) , SSI=0.69/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100099924
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO
300399	T86	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:3OYGW5Zgi0GGbtSyO4URRlywXTo-uZsc3y03QMZKWTkkQUI2ay2a9?xzrs5U?wgdEuzv7c9

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	7-8-0	-1915	-1915	—	TOP	VERT	TOTAL	—	—
T	9-7-4	-1275	-1275	—	TOP	VERT	TOTAL	—	—
U	11-7-4	-1275	-1275	—	TOP	VERT	TOTAL	—	—
V	13-2-8	-608	-608	—	BACK	VERT	TOTAL	—	—
W	13-7-4	-1275	-1275	—	TOP	VERT	TOTAL	—	—

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.80 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H G ALVES
100008023
PROVINCE OF ONTARIO

DWG NO. TAM T7901122
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG ND.
300332	T90	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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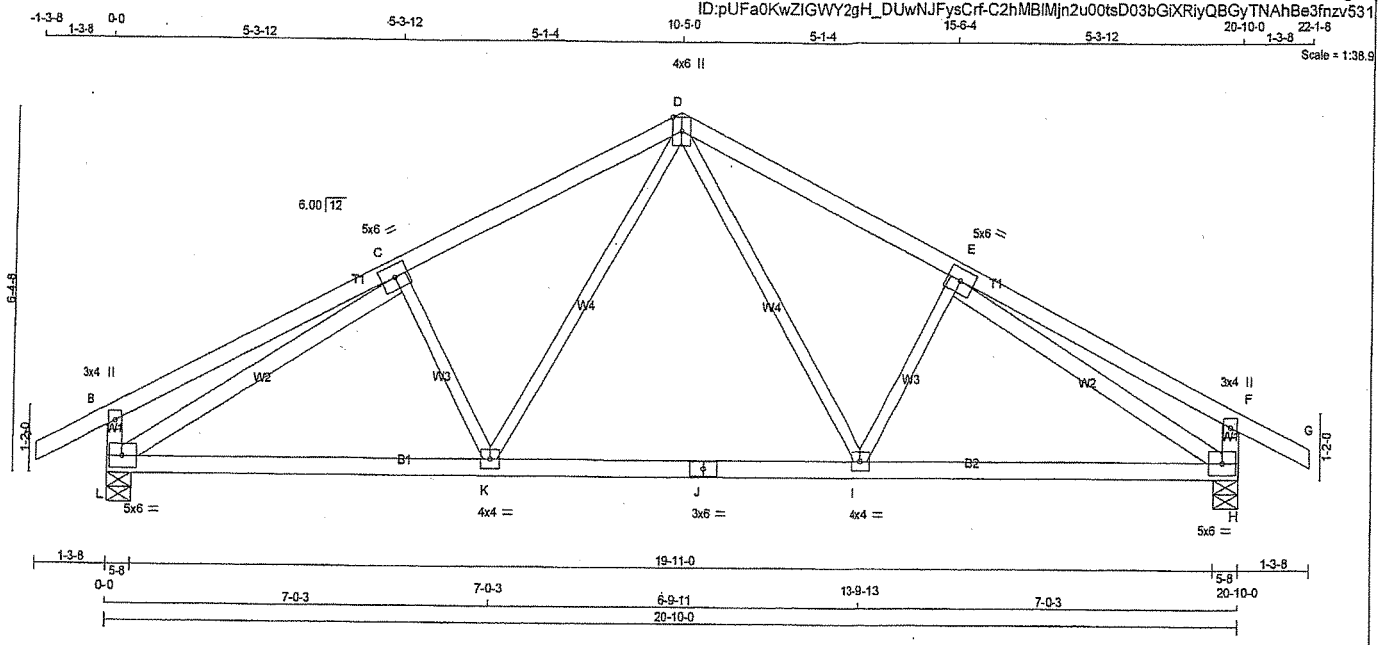


TOTAL WEIGHT = 86 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT2D	5.0	6.0		
D	TTWW+p	MT2D	4.0	6.0	Edge	
E	TMWW-t	MT2D	5.0	6.0		
F	TMV+p	MT2D	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT2D	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1400	0	1400	0	0	5-8	5-8
H	1400	0	1400	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1054	548 / 0	219 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	1054	548 / 0	219 / 0	0 / 0	0 / 0	287 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

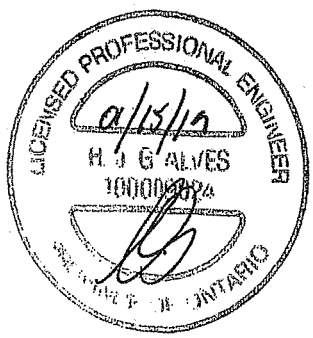
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 23	-84.9	-84.9	0.37 (1)	10.00
C- D	-1576 / 0	-84.9	-84.9	0.32 (1)	4.93
D- E	-1576 / 0	-84.9	-84.9	0.32 (1)	4.93
E- F	0 / 23	-84.9	-84.9	0.37 (1)	10.00
F- G	0 / 26	-84.9	-84.9	0.11 (1)	10.00
L- B	-284 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-284 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 1512	-38.5	-38.5	0.52 (2)	10.00
K- J	0 / 1095	-38.5	-38.5	0.48 (2)	10.00
J- I	0 / 1095	-38.5	-38.5	0.46 (2)	10.00
I- H	0 / 1512	-38.5	-38.5	0.52 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
D- I	0 / 601	0.14 (1)
I- E	-264 / 27	0.07 (1)
K- D	0 / 601	0.14 (1)
C- K	-264 / 27	0.07 (1)
L- C	-1816 / 0	0.87 (1)
E- H	-1816 / 0	0.87 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.37/1.00 (E-F:1) , BC=0.52/1.00 (H-I:2) , WB=0.87/1.00 (C-L:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.45 (C) (INPUT = 1.00)

DWG NO. TAM 1190491
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	G90	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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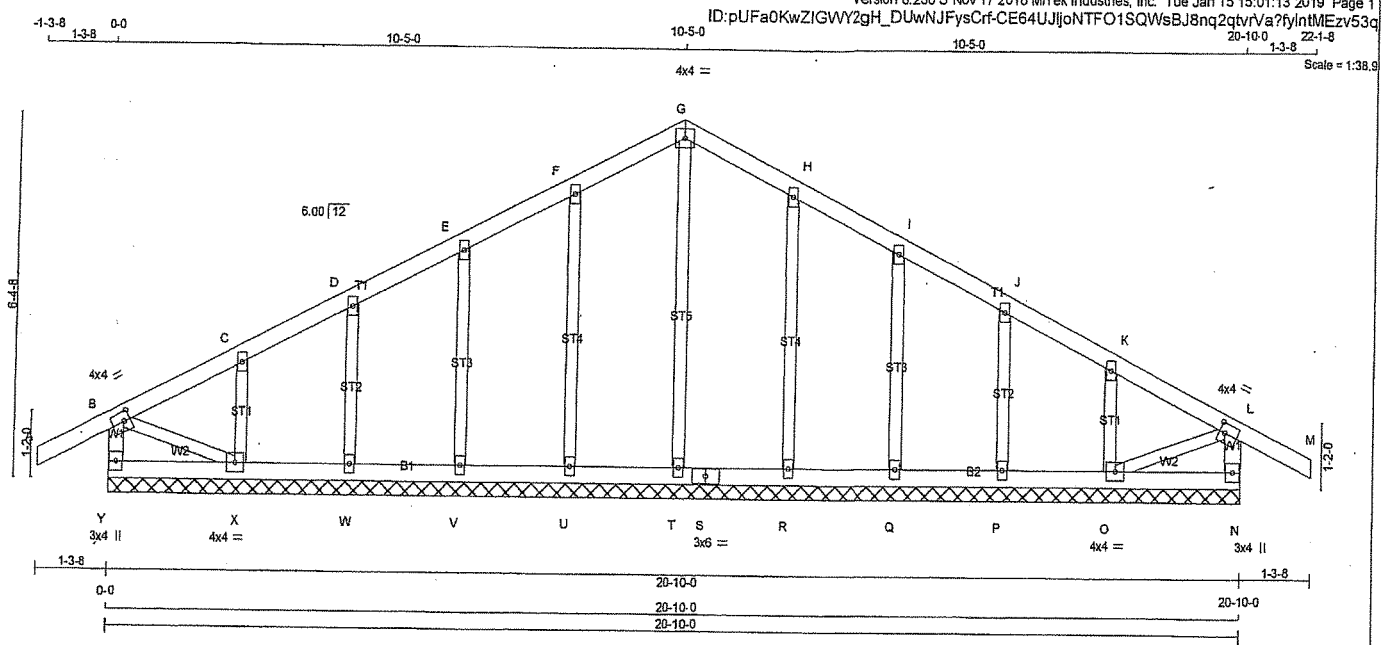


TOTAL WEIGHT = 85 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0		
L	TMVW-t	MT20	4.0	4.0	2.00	1.25
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, O, R, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

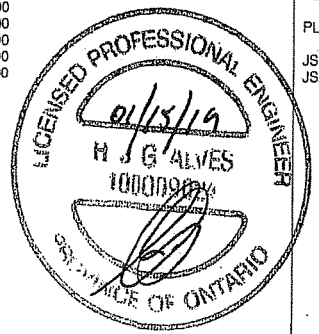
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-216 / 0	0.0	0.0	0.02 (1)		7.81
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B- C	-18 / 0	-84.9	-84.9	0.06 (1)		6.25
C- D	-23 / 0	-84.9	-84.9	0.06 (1)		6.25
D- E	-14 / 0	-84.9	-84.9	0.04 (1)		6.25
E- F	-10 / 0	-84.9	-84.9	0.05 (1)		6.25
F- G	-17 / 0	-84.9	-84.9	0.05 (1)		6.25
G- H	-17 / 0	-84.9	-84.9	0.05 (1)		6.25
H- I	-10 / 0	-84.9	-84.9	0.05 (1)		6.25
I- J	-14 / 0	-84.9	-84.9	0.04 (1)		6.25
J- K	-23 / 0	-84.9	-84.9	0.06 (1)		6.25
K- L	-18 / 0	-84.9	-84.9	0.06 (1)		6.25
L- M	0 / 26	-84.9	-84.9	0.11 (1)		10.00
N- L	-218 / 0	0.0	0.0	0.02 (1)		7.81
Y- X	0 / 0	-38.5	-38.5	0.04 (3)		10.00
X- W	0 / 17	-38.5	-38.5	0.04 (3)		10.00
W- V	0 / 13	-38.5	-38.5	0.03 (3)		10.00
V- U	0 / 11	-38.5	-38.5	0.03 (3)		10.00
U- T	0 / 8	-38.5	-38.5	0.02 (3)		10.00
T- S	0 / 8	-38.5	-38.5	0.02 (3)		10.00
S- R	0 / 8	-38.5	-38.5	0.02 (3)		10.00
R- O	0 / 11	-38.5	-38.5	0.03 (3)		10.00
Q- P	0 / 13	-38.5	-38.5	0.03 (3)		10.00
P- O	0 / 17	-38.5	-38.5	0.04 (3)		10.00
O- N	0 / 0	-38.5	-38.5	0.04 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-133 / 0	0.09 (1)
U- F	-192 / 0	0.08 (1)
V- E	-166 / 0	0.05 (1)
W- D	-157 / 0	0.03 (1)
X- C	-210 / 0	0.03 (1)
R- H	-192 / 0	0.08 (1)
O- I	-166 / 0	0.05 (1)
P- J	-157 / 0	0.03 (1)
O- K	-210 / 0	0.03 (1)
B- X	0 / 25	0.01 (1)
O- L	0 / 25	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.04/1.00 (O-P:3) , WB=0.09/1.00 (G-T:1) , SSI=0.08/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

DWG NO. TAM 1901172
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300401	T211	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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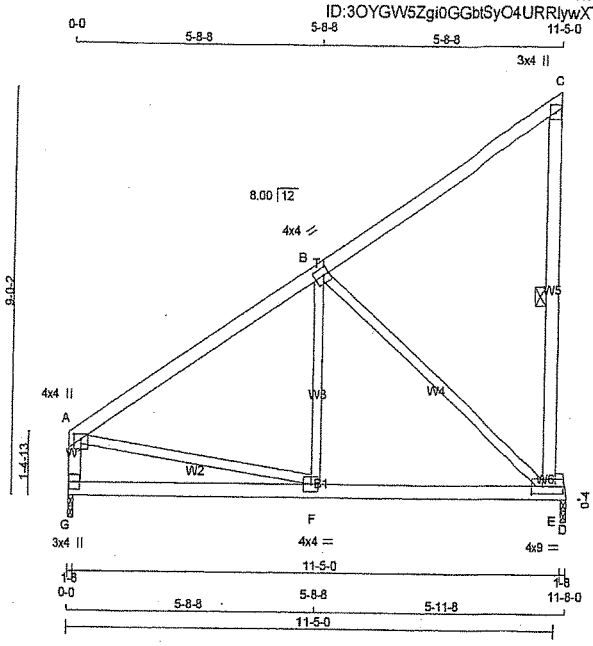


TOTAL WEIGHT = 3 X 54 = 162 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT2D	4.0	4.0	1.25	2.00
B	TMWW-t	MT2D	4.0	4.0	2.00	1.50
C	TMV+p	MT2D	3.0	4.0		
E	BMVWW-t	MT2D	4.0	9.0	2.00	3.25
F	BMWW-t	MT2D	4.0	4.0		
G	BMV1+p	MT2D	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	719	0	719	0	0	1-8	1-8
D	699	0	699	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	546	272 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0
D	531	260 / 0	123 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

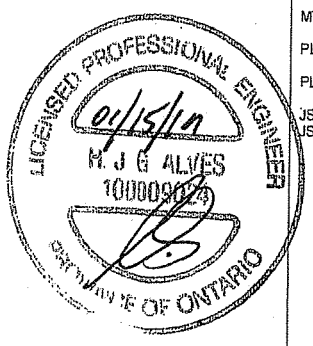
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-A	-643 / 0	0.0	0.0	0.07 (1)	7.81	A-F	0 / 547	0.12 (1)
A-B	-613 / 0	-84.9	-84.9	0.50 (1)	6.25	F-B	0 / 319	0.07 (3)
B-C	-33 / 0	-84.9	-84.9	0.46 (1)	6.25	B-E	-708 / 0	0.76 (1)
E-C	-183 / 0	0.0	0.0	0.07 (1)	6.25			
G-F	0 / 0	-38.5	-38.5	0.31 (2)	10.00			
F-E	0 / 538	-38.5	-38.5	0.39 (2)	10.00			
E-D	0 / 0	-38.5	-38.5	0.10 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.50/1.00 (A-B:1) , BC=0.39/1.00 (E-F:2) , WB=0.78/1.00 (B-E:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (A) (INPUT = 0.90)
JSI METAL= 0.20 (F) (INPUT = 1.00)

DWG NO. TAM T1901135
STRUCTURAL
CONCURRENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300401	T302	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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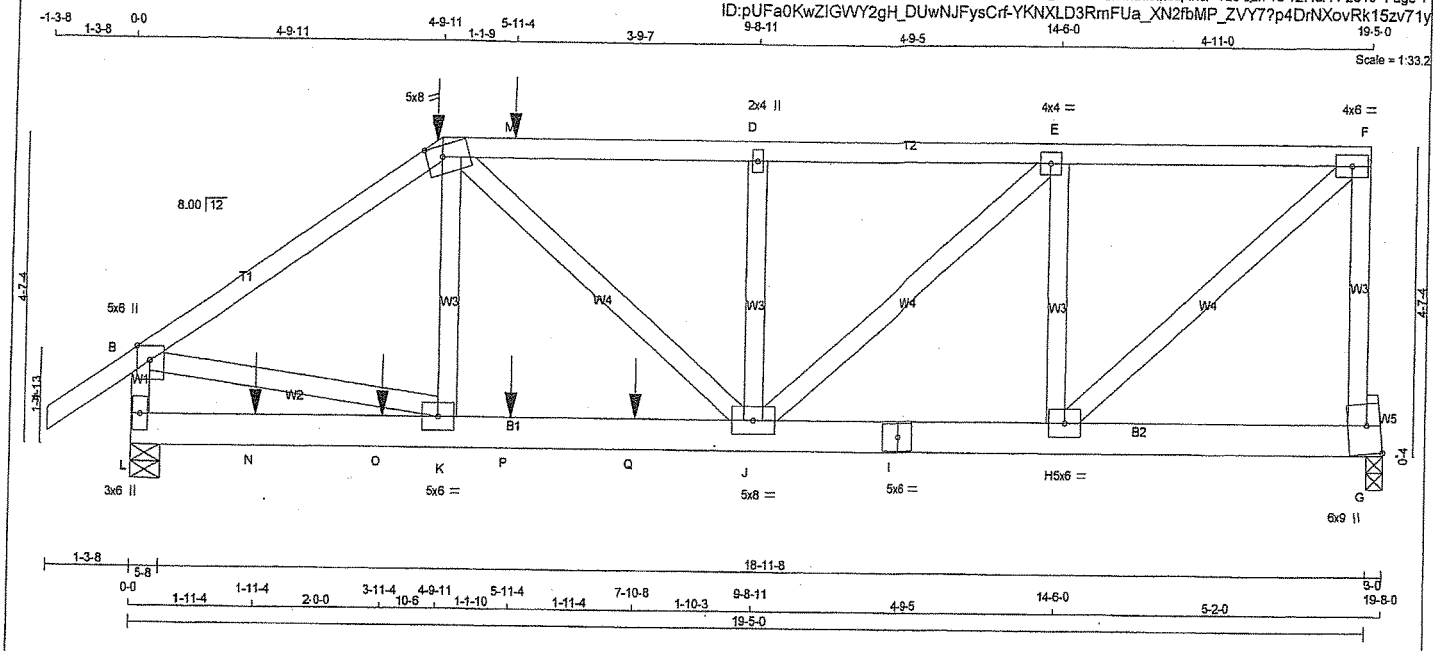


TOTAL WEIGHT = 2 X 105 = 210 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-G	1	12	TDP
L-B	1	12	TOP
BDTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L-I	2	12	SIDE(0.0)
I-G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TDP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TDP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TDP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMVW1m	MT20	6.0	9.0	2.50	5.00
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER DF PLATE TOUCHES EDGE OF CHDRD.

DIMENSIDNS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TD BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTDRED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
G	1899	0	1899	0	0	3-0	3-0
L	2636	0	2636	0	0	5-8	5-8

UNFACTORED REACTIDNS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1432	738 / 0	302 / 0	0 / 0	0 / 0	392 / 0	0 / 0
L	1982	1039 / 0	404 / 0	0 / 0	0 / 0	538 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTDRED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FDRCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00	K- C	0 / 491	0.04 (3)
B-C	-3124 / 0	-84.9	-84.9	0.25 (1)	5.00	C- J	0 / 709	0.06 (1)
C-M	-3148 / 0	-84.9	-84.9	0.25 (1)	4.97	J- D	-482 / 0	0.05 (1)
M-D	-3148 / 0	-84.9	-84.9	0.25 (1)	4.97	J- E	0 / 1685	0.13 (1)
D-E	-3148 / 0	-84.9	-84.9	0.21 (1)	5.02	H- E	-560 / 0	0.17 (1)
E-F	-1887 / 0	-84.9	-84.9	0.16 (1)	6.18	H-	0 / 2468	0.22 (1)
G-F	-1793 / 0	0.0	0.0	0.29 (1)	7.81	[illegible]	0 / 2648	0.29 (1)
L-B	-2581 / 0	0.0	0.0	0.14 (1)	7.06			
L-N	0 / 0	-38.5	-38.5	0.18 (2)	10.00			
N-O	0 / 0	-38.5	-38.5	0.18 (2)	10.00			
O-K	0 / 0	-38.5	-38.5	0.18 (2)	10.00			
K-P	0 / 2609	-38.5	-38.5	0.57 (1)	10.00			
P-O	0 / 2609	-38.5	-38.5	0.57 (1)	10.00			
O-J	0 / 2609	-38.5	-38.5	0.57 (1)	10.00			
J-I	0 / 1887	-38.5	-38.5	0.30 (1)	10.00			
I-H	0 / 1887	-38.5	-38.5	0.30 (1)	10.00			
H-G	0 / 0	-38.5	-38.5	0.06 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEIGHT	COMM.
C	4-9-11	-71	-80	—	FRONT	VERT	DEAD	—	—
C	4-9-11	-330	-330	—	FRONT	VERT	SNOW	—	—
M	5-11-4	-134	-134	—	FRONT	VERT	TOTAL	—	—
N	1-11-4	-70	-89	—	FRONT	VERT	TOTAL	—	—
O	3-11-4	-70	-89	—	FRONT	VERT	TOTAL	—	—
P	5-11-4	-70	-89	—	FRONT	VERT	TOTAL	—	—
O	7-10-8	-1278	-1278	—	TOP	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/DR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FRDM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TDP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BC8C 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.29/1.00 (F-G:1) , BC=0.57/1.00 (J-K:1) , WB=0.23/1.00 (B-K:1) , SSI=0.30/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LDAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NDT RESPONSIBLE FOR OUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

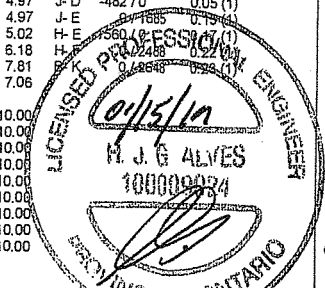
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TDL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 1.00)



DWG NO. TAM T2901136
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T303	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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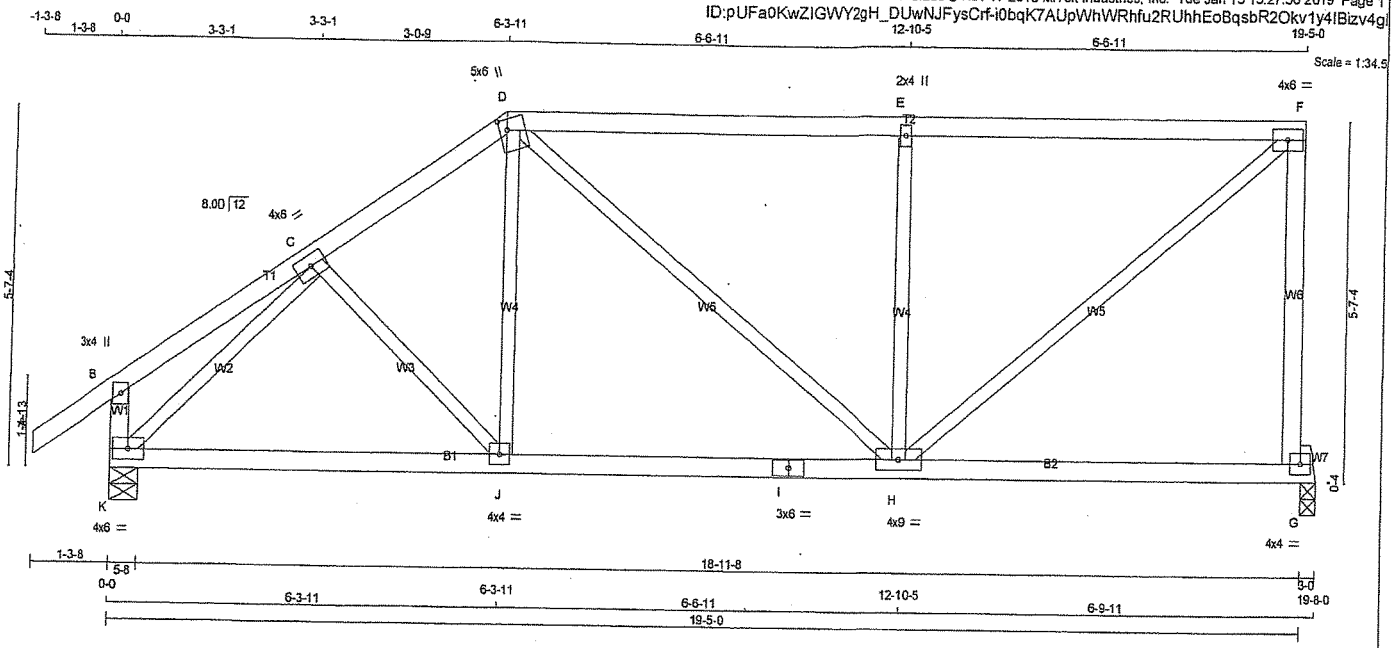


TOTAL WEIGHT = 83 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	ORY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1198	0	1198	0	0	3-0	3-0
K	1314	0	1314	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	908	452 / 0	204 / 0	0 / 0	0 / 0	252 / 0	0 / 0
K	989	516 / 0	204 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

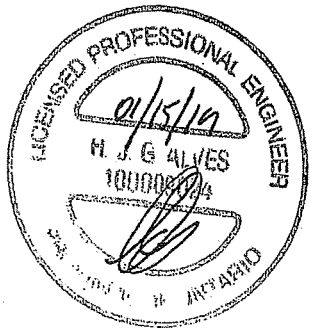
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C- J	0 / 90	0.02 (3)
B- C	0 / 15	-84.9	-84.9	0.12 (1)	10.00	J- D	0 / 308	0.07 (2)
C- D	-1205 / 0	-84.9	-84.9	0.14 (1)	5.68	D- H	0 / 130	0.03 (1)
D- E	-1094 / 0	-84.9	-84.9	0.68 (1)	4.94	H- E	-689 / 0	0.33 (1)
E- F	-1094 / 0	-84.9	-84.9	0.68 (1)	4.94	H- F	0 / 1405	0.32 (1)
G- F	-1094 / 0	0.0	0.0	0.62 (1)	7.56	K- C	-1395 / 0	0.44 (1)
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 981	-38.5	-38.5	0.40 (2)	10.00			
J- I	0 / 993	-38.5	-38.5	0.42 (2)	10.00			
I- H	0 / 993	-38.5	-38.5	0.42 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.29 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBD 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.42/1.00 (H-J:2) , WB=0.44/1.00 (C-K:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

DWG NO. TAM 1901175
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T304	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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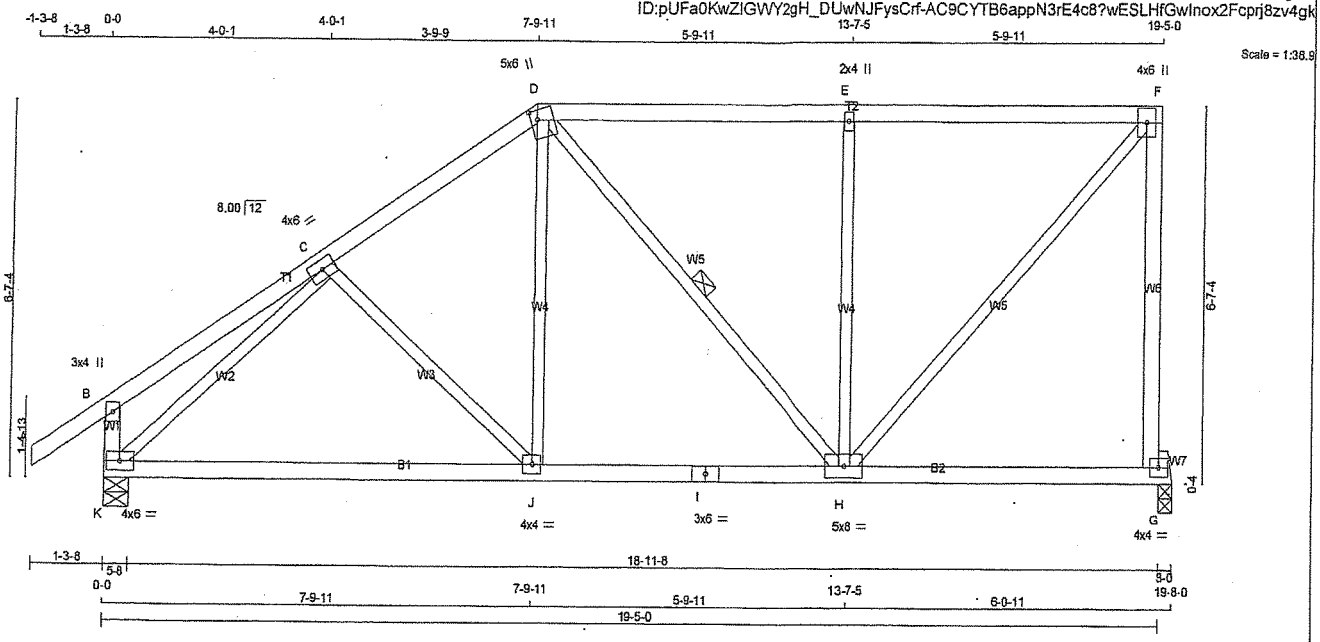


TOTAL WEIGHT = 88 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
G	1198	0	1198	0	0	3-0	3-0
K	1314	0	1314	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	908	452 / 0	204 / 0	0 / 0	0 / 0	252 / 0	0 / 0
K	989	516 / 0	204 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-H.

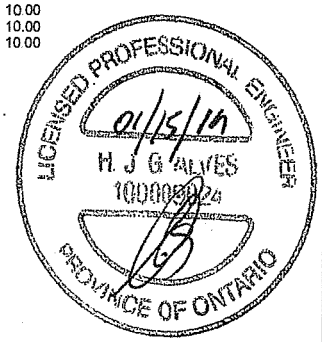
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	0 / 20	-84.9	-84.9	0.20 (1)		10.00
C- D	-1124 / 0	-84.9	-84.9	0.22 (1)		5.71
D- E	-845 / 0	-84.9	-84.9	0.51 (1)		5.83
E- F	-846 / 0	-84.9	-84.9	0.51 (1)		5.83
G- F	-1102 / 0	0.0	0.0	1.00 (1)		7.54
K- B	-245 / 0	0.0	0.0	0.03 (1)		7.81
K- J	0 / 1000	-38.5	-38.5	0.51 (2)		10.00
J- I	0 / 923	-38.5	-38.5	0.50 (2)		10.00
I- H	0 / 923	-38.5	-38.5	0.50 (2)		10.00
H- G	0 / 0	-38.5	-38.5	0.21 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-112 / 50	0.06 (1)
J- D	0 / 423	0.10 (2)
D- H	-128 / 0	0.06 (2)
H- E	-610 / 0	0.44 (1)
H- F	0 / 1244	0.28 (1)
K- C	-1375 / 0	0.64 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 883 (0.26")

CSI: TC=1.00/1.00 (F-G:1) , BC=0.51/1.00 (J-K:2) , WB=0.64/1.00 (C-K:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.38 (F) (INPUT = 1.00)

DWG NO. TAM T1901176
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T305	1	1	TRUSS OESC.	

Tamarack Roof Truss, Burlington

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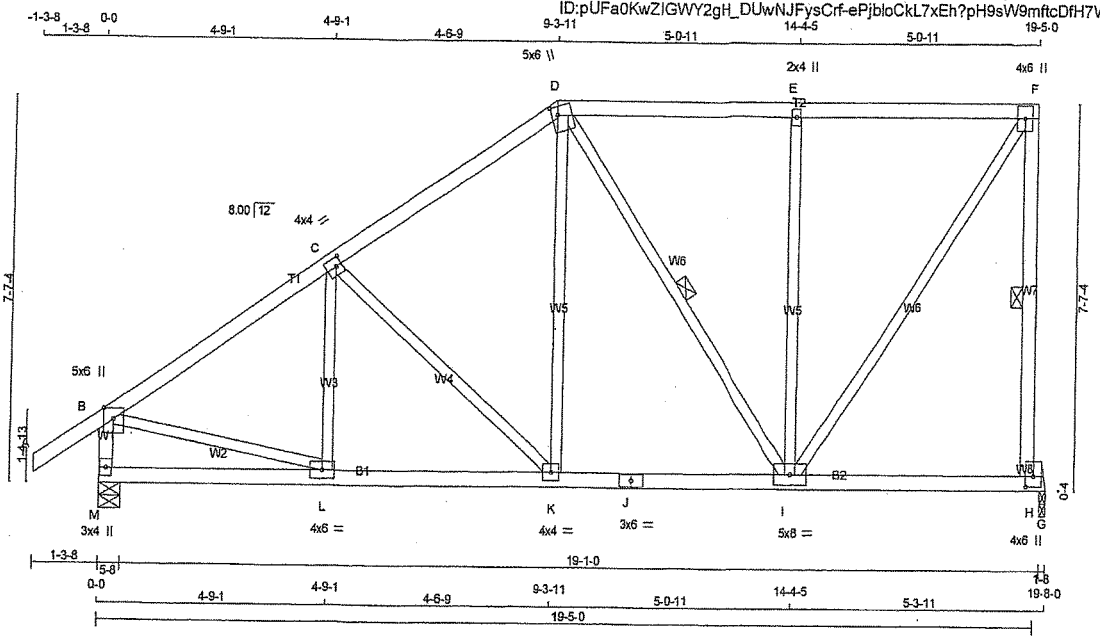


TOTAL WEIGHT = 95 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - F	2x4	ORY	No.2	SPF
H - F	2x4	ORY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1329	0	1329	0	0	5-8	5-8
G	1198	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, D-I.

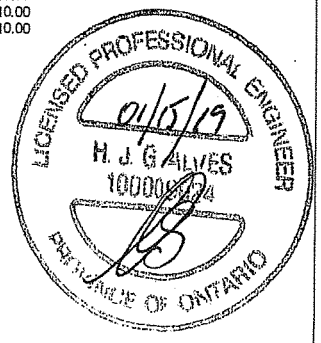
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1296 / 0	-84.9	-84.9	0.35 (1)	5.26
C-D	-1008 / 0	-84.9	-84.9	0.33 (1)	5.79
D-E	-705 / 0	-84.9	-84.9	0.37 (1)	6.25
E-F	-705 / 0	-84.9	-84.9	0.37 (1)	6.25
H-F	-1177 / 0	0.0	0.0	0.31 (1)	5.88
M-B	-1255 / 0	0.0	0.0	0.13 (1)	7.17
M-L	0 / 0	-38.5	-38.5	0.16 (3)	10.00
L-K	0 / 1099	-38.5	-38.5	0.31 (2)	10.00
K-J	0 / 821	-38.5	-38.5	0.39 (2)	10.00
J-I	0 / 821	-38.5	-38.5	0.39 (2)	10.00
I-H	0 / 0	-38.5	-38.5	0.40 (1)	10.00
H-G	0 / 0	-53.5	-53.5	0.17 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
L-C	-42 / 135	0.03 (3)
C-K	-385 / 0	0.27 (1)
K-D	0 / 424	0.10 (2)
D-I	-205 / 0	0.11 (1)
I-E	-529 / 0	0.56 (1)
I-F	0 / 1233	0.28 (1)
B-L	0 / 1127	0.25 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.37/1.00 (E-F:1) , BC=0.40/1.00 (H-I:1) , WB=0.56/1.00 (E-I:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM 1901/77
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T306	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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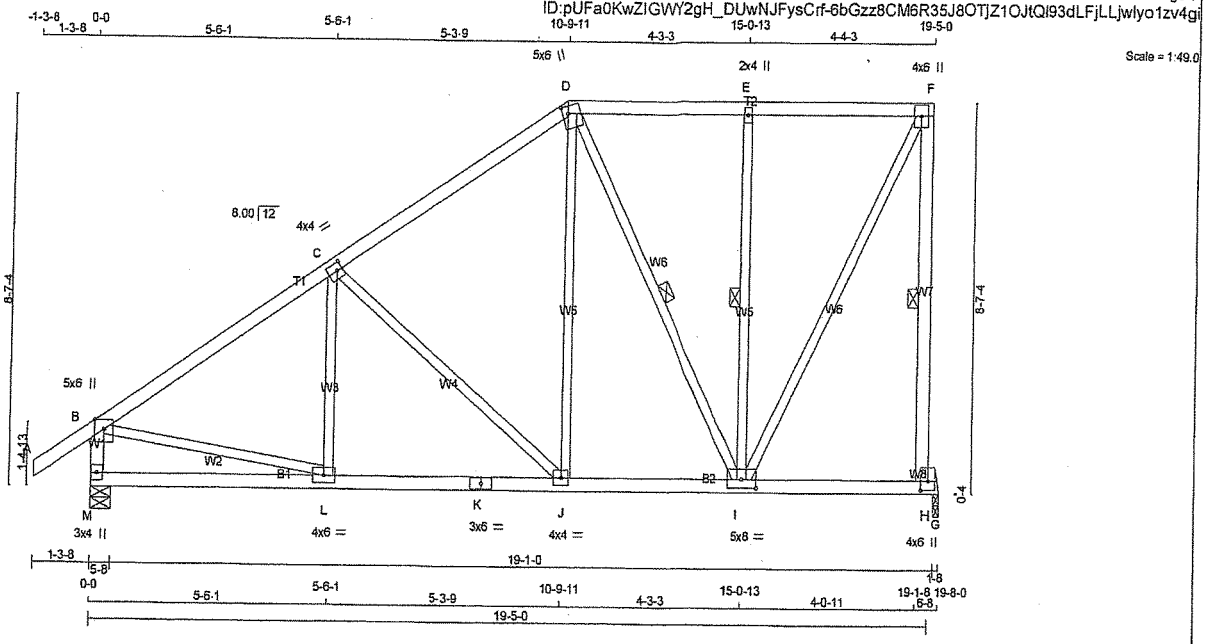


TOTAL WEIGHT = 101 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - G	2x4	ORY	No.2	SPF
H - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT2D	5.0	6.0	Edge	
C	TMWW-t	MT2D	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT2D	5.0	8.0	2.25	4.00
J	BMWW-t	MT2D	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, D-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

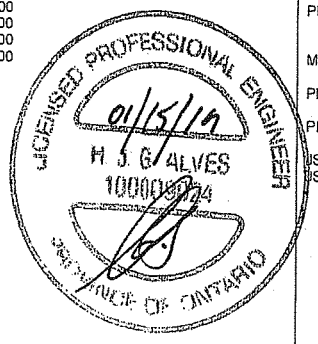
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	-1292 / 0	-84.9	-84.9	0.49 (1)		5.06
C- D	-887 / 0	-84.9	-84.9	0.45 (1)		5.87
D- E	-553 / 0	-84.9	-84.9	0.26 (1)		6.25
E- F	-553 / 0	-84.9	-84.9	0.26 (1)		6.25
H- F	-1200 / 0	0.0	0.0	0.41 (1)		5.84
M- B	-1245 / 0	0.0	0.0	0.13 (1)		7.20
M- L	0 / 0	-38.5	-38.5	0.22 (3)		10.00
L- K	0 / 1100	-38.5	-38.5	0.37 (2)		10.00
K- J	0 / 1100	-38.6	-38.5	0.37 (2)		10.00
J- I	0 / 718	-38.5	-38.5	0.31 (2)		10.00
I- H	0 / 0	-38.5	-38.5	0.40 (1)		10.00
H- G	0 / 0	-53.5	-53.5	0.17 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
L- C	0 / 200	0.05 (3)
C- J	-517 / 0	0.52 (1)
J- D	0 / 498	0.11 (2)
B- L	0 / 1121	0.25 (1)
I- F	0 / 1193	0.27 (1)
D- I	-360 / 0	0.22 (1)
I- E	-448 / 0	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.49/1.00 (B-C:1) , BC=0.40/1.00 (H-I:1) , WB=0.52/1.00 (C-J:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.88 (H) (INPUT = 0.90)
USI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM T1901178
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T307	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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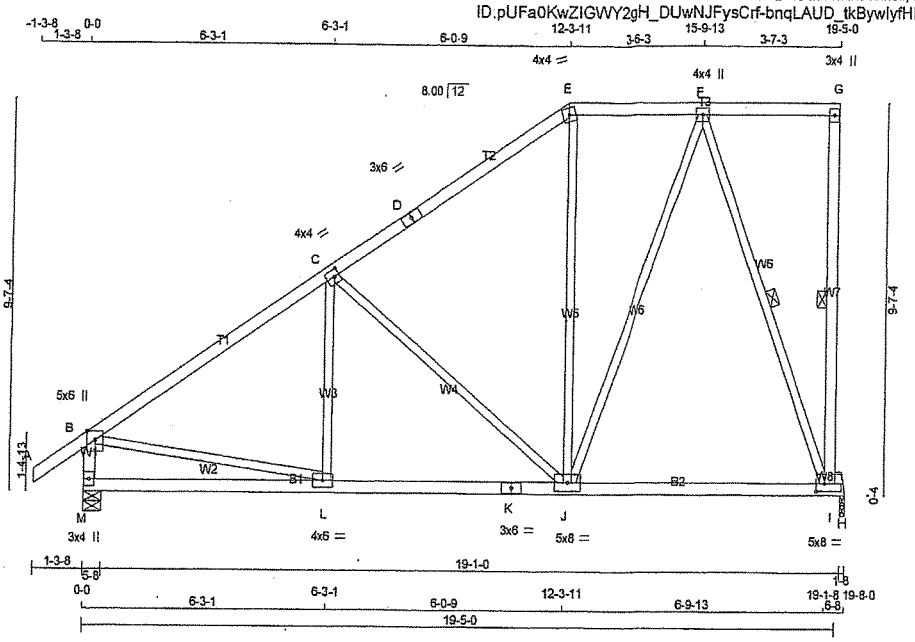

TOTAL WEIGHT = 99 lb

LUMBER

N. L. G. A. RULES

CHOROS	SIZE		LUMBER	DESCR.
A - O	2x4	ORY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	ORY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	ORY	No.2	SPF
K - H	2x4	ORY	No.2	SPF
I - H	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW+t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVWW-t	MT20	5.0	8.0	2.25	2.50
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1329	0	1329	0	0	5-8	5-8
H	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
H	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1284 / 0	-84.9	-84.9	0.66 (1)	4.82
C-D	-785 / 0	-84.9	-84.9	0.59 (1)	5.83
D-E	-785 / 0	-84.9	-84.9	0.59 (1)	5.83
E-F	-624 / 0	-84.9	-84.9	0.18 (1)	6.25
F-G	0 / 0	-84.9	-84.9	0.17 (1)	10.00
I-G	-117 / 0	0.0	0.0	0.05 (1)	6.25
M-B	-1226 / 0	0.0	0.0	0.13 (1)	7.24
M-L	0 / 0	-38.5	-38.5	0.24 (3)	10.00
L-K	0 / 1081	-38.5	-38.5	0.56 (2)	10.00
K-J	0 / 1081	-38.5	-38.5	0.56 (2)	10.00
J-I	0 / 390	-38.5	-38.5	0.53 (2)	10.00
I-H	0 / 0	-53.5	-53.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	0 / 217	0.05 (3)
C-J	-603 / 0	0.83 (1)
J-E	0 / 169	0.04 (3)
B-L	0 / 1097	0.25 (1)
J-F	0 / 680	0.15 (1)
F-I	-1082 / 0	0.77 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009924
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	OL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	OL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 792 (0.30")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.56/1.00 (J-L:2) , WB=0.83/1.00 (C-J:1) , SSI=0.47/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(ORY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.88 (I) (INPUT = 0.90)
SSI METAL= 0.55 (K) (INPUT = 1.00)

DWG NO. TAM T190179
STRUCTURAL
CLIENT COPY ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T308	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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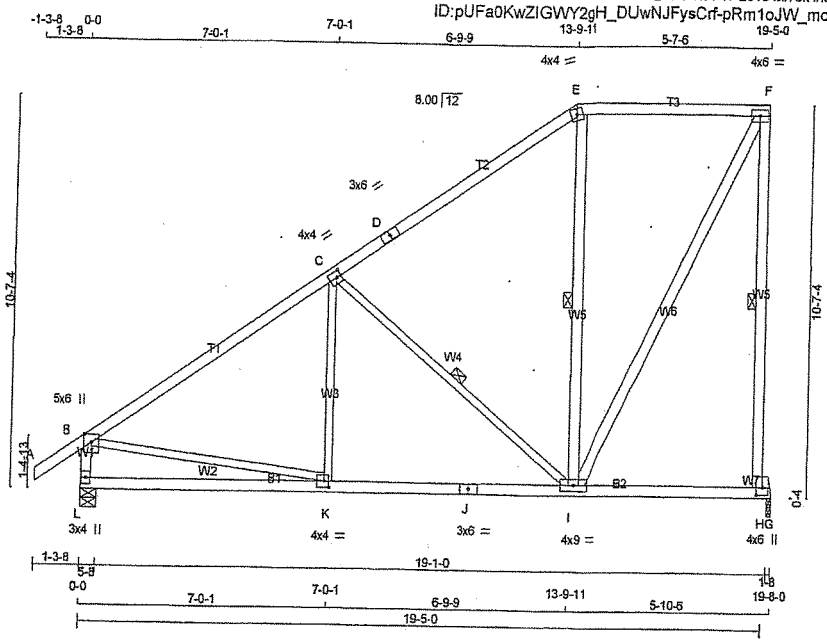


TOTAL WEIGHT = 104 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
H - F	2x4	ORY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	ORY	No.2	SPF
J - G	2x4	ORY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
O	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, C-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

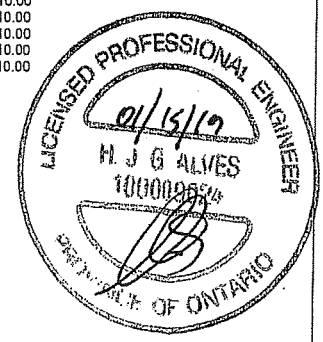
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	-1244 / 0	-84.9	-84.9	0.87 (1)		4.44
C- D	-637 / 0	-84.9	-84.9	0.75 (1)		5.85
D- E	-637 / 0	-84.9	-84.9	0.75 (1)		5.85
E- F	-494 / 0	-84.9	-84.9	0.47 (1)		6.25
H- F	-1169 / 0	0.0	0.0	0.66 (1)		5.90
L- B	-1219 / 0	0.0	0.0	0.13 (1)		7.26
L- K	0 / 0	-38.5	-38.5	0.34 (3)		10.00
K- J	0 / 1068	-38.5	-38.5	0.47 (2)		10.00
J- I	0 / 1068	-38.5	-38.5	0.47 (2)		10.00
I- H	0 / 0	-38.5	-38.5	0.41 (1)		10.00
H- G	0 / 0	-53.5	-53.5	0.17 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 295	0.07 (3)
C- I	-743 / 0	0.42 (1)
I- E	-123 / 97	0.07 (1)
I- F	0 / 1055	0.17 (1)
B- K	0 / 1080	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.87/1.00 (B-C:1) , BC=0.47/1.00 (I-K:2) , WB=0.42/1.00 (C-I:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM 179011/80
STRUCTURAL
ENGINEER ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T309	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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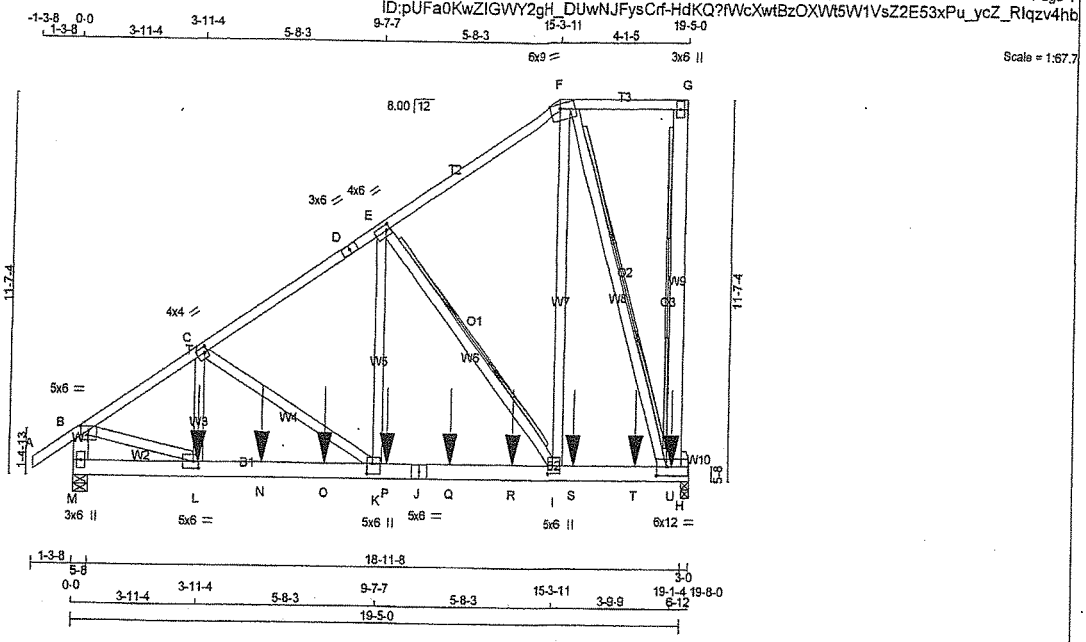

TOTAL WEIGHT = 4 X 143 = 572 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- G	1	12	TOP
G- H	2	12	SIDE(58.2)
M- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	TOP
J- H	2	5	SIDE(694.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED DN TDP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TDP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT2D	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.D	2.00	1.00
D	TS-t	MT2D	3.0	6.0		
E	TMWW-t	MT20	4.0	6.0	2.00	1.50
F	TTWW-m	MT20	6.D	9.D	1.75	3.75
G	TMV+p	MT20	3.0	6.0		
H	BMVWW1-t	MT20	6.0	12.0	3.25	4.00
I	BMWW+t	MT20	5.0	6.0	3.00	2.25
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	5.0	6.0	3.75	2.00
L	BMWW-t	MT2D	5.0	6.0	2.50	2.00
M	BMV1+p	MT20	3.D	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	11094	0	11094	0	0	3-0	3-0
M	8210	D	8210	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	8385	4265 / 0	1813 / 0	D / 0	0 / 0	2308 / 0	0 / 0
M	6197	3175 / 0	1323 / 0	0 / 0	0 / 0	1700 / 0	D / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT G-H, E-I, F-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.04 (1)		10.00
B- C	-10558 / 0	-84.9	-84.9	0.23 (1)		3.83
C- D	-8207 / 0	-84.9	-84.9	0.38 (1)		4.22
D- E	-8207 / 0	-84.9	-84.9	0.38 (1)		4.22
E- F	-3564 / 0	-84.9	-84.9	0.18 (1)		5.93
F- G	0 / 0	-84.9	-84.9	0.09 (1)		10.00
H- G	-174 / 0	0.0	0.0	0.01 (1)		7.81
M- B	-8348 / 0	0.0	0.0	0.15 (1)		6.74
M- L	0 / 0	-38.5	-38.5	0.18 (1)		10.00
L- N	0 / 8797	-38.5	-38.5	0.86 (1)		10.00
N- O	0 / 8797	-38.5	-38.5	0.86 (1)		10.00
O- K	0 / 8797	-38.5	-38.5	0.86 (1)		10.00
K- P	0 / 6842	-38.5	-38.5	0.79 (1)		10.00
P- J	0 / 6842	-38.5	-38.5	0.79 (1)		10.00
J- Q	0 / 6842	-38.5	-38.5	0.79 (1)		10.00
Q- R	0 / 6842	-38.5	-38.5	0.79 (1)		10.00
R- I	0 / 6842	-38.5	-38.5	0.79 (1)		10.00
I- S	0 / 3070	-38.5	-38.5	0.51 (1)		10.00
S- T	0 / 3070	-38.5	-38.5	0.51 (1)		10.00
T- U	0 / 3070	-38.5	-38.5	0.51 (1)		10.00
U- H	0 / 3070	-38.5	-38.5	0.51 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 2142	0.09 (1)
C- K	-2318 / 0	0.34 (1)
K- E	0 / 6996	0.28 (1)
E- I	-6310 / 0	0.34 (1)
I- F	0 / 9975	0.40 (1)
F- H	-8919 / 0	0.72 (1)
B- L	8974	0.55 (M)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	CONN.
L	3-11-4	-2633	-2633	—	TOP	VERT	TOTAL	—	—
N	5-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—
O	7-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—
P	9-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—
Q	11-11-4	-1768	-1768	—	BACK	VERT	TOTAL	—	—
R	13-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.38/1.00 (C-E:1) , BC=0.86/1.00 (K-L:1) , WB=0.72/1.00 (F-H:1) , SSI=0.58/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (B) (INPUT = 0.90)
USI METAL= 0.55 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100098824
PROVINCE OF ONTARIO

DWG NO. TAM 1790118 1
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG ND.
300333	T309	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	15-11-4	-1768	-1768	—	BACK	VERT	TOTAL	—	—
T	17-11-4	-1768	-1768	—	BACK	VERT	TOTAL	—	—
U	19-1-4	-1781	-1781	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/15/19
H J G ALVES
100099[illegible]
PROVINCE OF ONTARIO

DWG NO. TAM 7790118I
STRUCTURAL
COPY FOR APPROVAL ONLY

2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 41239	DRWG NO.
300582	T310Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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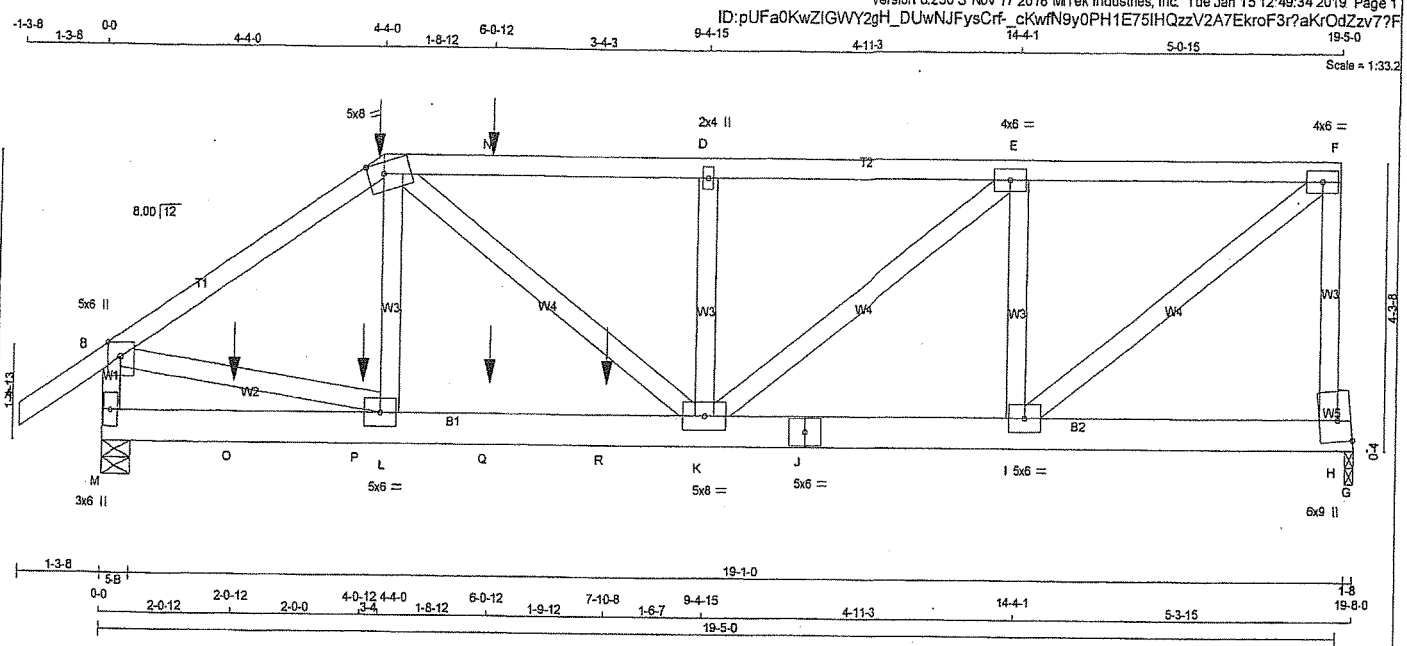

TOTAL WEIGHT = 2 X 102 = 205 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-H	1	12	TOP
M-B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-J	2	12	SIDE(0.0)
J-G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	6.0		
H	BMVWm	MT20	6.0	9.0	Edge	3.75
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	2766	0	2766	0	0	5-8	5-8
G	1959	0	1959	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2073	1118 / 0	418 / 0	0 / 0	0 / 0	538 / 0	0 / 0
G	1478	763 / 0	315 / 0	0 / 0	0 / 0	400 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A-B	0 / 32		-84.9	-84.9	0.06 (1)	10.00		L-C	0 / 367		0.03 (3)
B-C	-3315 / 0		-84.9	-84.9	0.21 (1)	4.93		C-K	0 / 1197		0.11 (1)
C-N	-3712 / 0		-84.9	-84.9	0.30 (1)	4.60		K-D	-504 / 0		0.05 (1)
N-D	-3712 / 0		-84.9	-84.9	0.30 (1)	4.60		K-E	0 / 1800		0.16 (1)
D-E	-3712 / 0		-84.9	-84.9	0.24 (1)	4.67		I-E	-1575 / 0		0.15 (1)
E-F	-2303 / 0		-84.9	-84.9	0.18 (1)	5.71		I-F	0 / 2909		0.26 (1)
H-F	-1955 / 0		0.0	0.0	0.27 (1)	7.81		B-L	0 / 2809		0.25 (1)
M-B	-2758 / 0		0.0	0.0	0.15 (1)	6.88					
M-O	0 / 0		-38.5	-38.5	0.18 (1)	10.00					
O-P	0 / 0		-38.5	-38.5	0.18 (1)	10.00					
P-L	0 / 0		-38.5	-38.5	0.18 (1)	10.00					
L-O	0 / 2763		-38.5	-38.5	0.62 (1)	10.00					
O-R	0 / 2763		-38.5	-38.5	0.62 (1)	10.00					
R-K	0 / 2763		-38.5	-38.5	0.62 (1)	10.00					
K-J	0 / 2303		-38.5	-38.5	0.34 (1)	10.00					
J-I	0 / 2303		-38.5	-38.5	0.34 (1)	10.00					
I-H	0 / 0		-38.5	-38.5	0.18 (1)	10.00					
H-G	0 / 0		-53.5	-53.5	0.07 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	CONN.
C	4-4-1	-58	-64	—	FRONT	VERT	DEAD	—	—
C	4-4-0	-120	-120	—	BACK	VERT	TOTAL	—	—
C	4-4-1	-266	-266	—	FRONT	VERT	SNOW	—	—
N	6-0-12	-114	-114	—	BACK	VERT	TOTAL	—	—
O	2-0-12	-46	-62	—	BACK	VERT	TOTAL	—	—
P	4-0-12	-51	-67	—	BACK	VERT	TOTAL	—	—
Q	6-0-12	-46	-62	—	BACK	VERT	TOTAL	—	—
R	7-10-8	-1502	-1502	—	TOP	VERT	TOTAL	—	—

DWG NO. TAM 8701137
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	OL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.62/1.00 (K-L:1) , WB=0.26/1.00 (F-I:1) , SSI=0.38/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) MAX	MIN	SHEAR (PSI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.64 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G ALVES
100000[illegible]4
PROVINCE OF ONTARIO
01/15/19

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T311	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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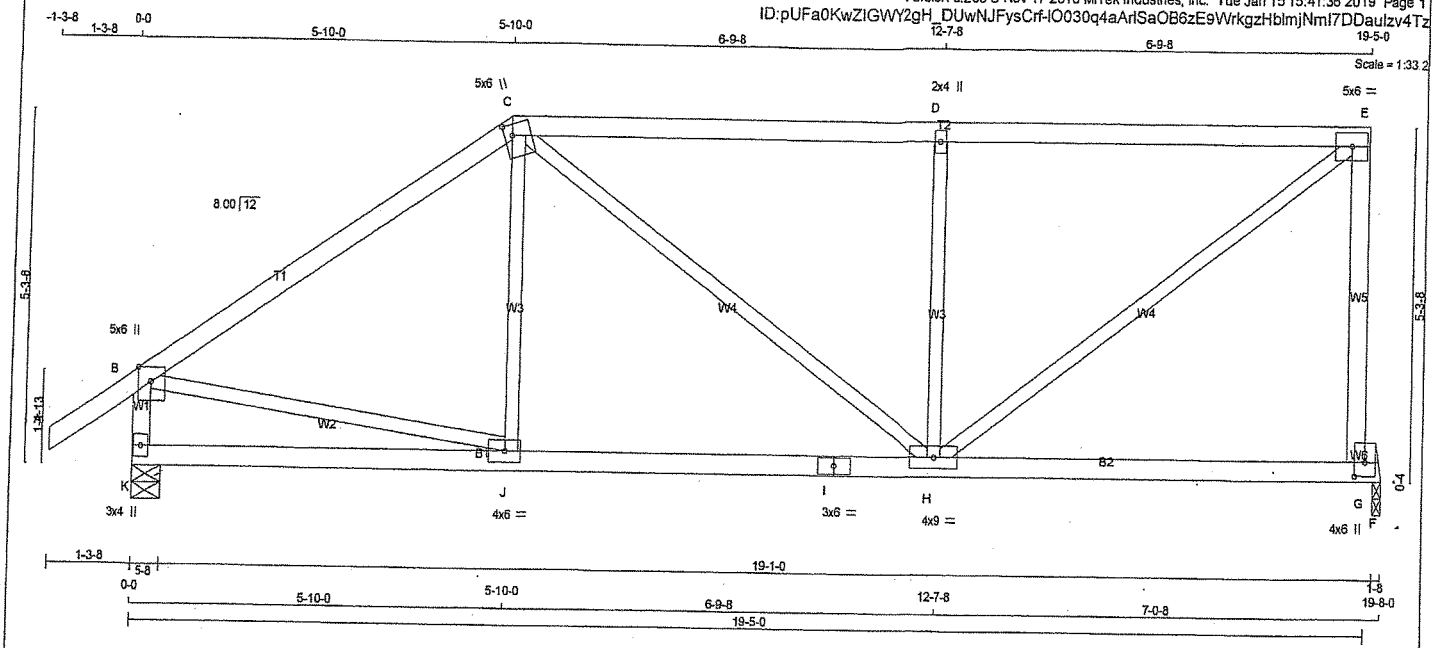


TOTAL WEIGHT = 80 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
O	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
G	BMVW+p	MT20	4.0	6.0	2.50	2.00
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
K	1329	0	1329	0	0	5-8	5-8
F	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
F	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

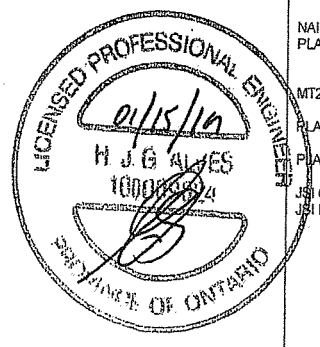
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	-1231 / 0	-84.9	-84.9	0.59 (1)		4.98
C- D	-1244 / 0	-84.9	-84.9	0.75 (1)		4.55
D- E	-1244 / 0	-84.9	-84.9	0.75 (1)		4.55
G- E	-1131 / 0	0.0	0.0	0.55 (1)		7.46
K- B	-1235 / 0	0.0	0.0	0.13 (1)		7.22
K- J	0 / 0	-38.5	-38.5	0.24 (3)		10.00
J- I	0 / 1025	-38.5	-38.5	0.55 (2)		10.00
I- H	0 / 1025	-38.5	-38.5	0.55 (2)		10.00
H- G	0 / 0	-38.5	-38.5	0.46 (2)		10.00
G- F	0 / 0	-53.5	-53.5	0.17 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 222	0.05 (3)
C- H	0 / 273	0.06 (1)
H- D	-713 / 0	0.30 (1)
H- E	0 / 1541	0.35 (1)
B- J	0 / 1040	0.23 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 835 (0.28")

CSI: TC=0.75/1.00 (D-E:1) , BC=0.55/1.00 (H-J:2) , WB=0.35/1.00 (E-H:1) , SSI=0.47/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.56 (G) (INPUT = 1.00)

DWG NO. TAM 1190183
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T312	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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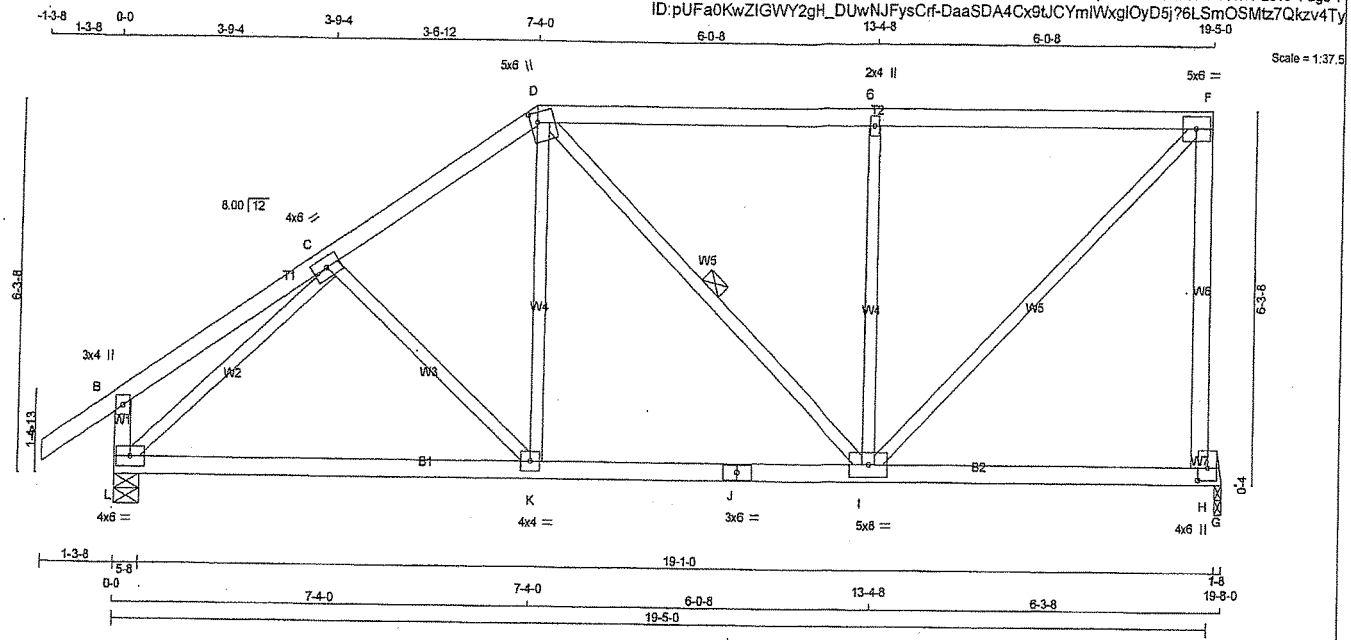


TOTAL WEIGHT = 86 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTOREO GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 19	-84.9	-84.9	0.17 (1)	10.00
C- D	-1171 / 0	-84.9	-84.9	0.19 (1)	5.66
D- E	-962 / 0	-84.9	-84.9	0.56 (1)	5.46
E- F	-963 / 0	-84.9	-84.9	0.56 (1)	5.46
H- F	-1146 / 0	0.0	0.0	0.90 (1)	7.43
L- B	-238 / 0	0.0	0.0	0.02 (1)	7.81
L- K	0 / 1010	-38.5	-38.5	0.46 (2)	10.00
K- J	0 / 962	-38.5	-38.5	0.45 (2)	10.00
J- I	0 / 962	-38.5	-38.5	0.45 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.43 (2)	10.00
H- G	0 / 0	-53.5	-53.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-72 / 63	0.03 (1)
K- D	0 / 368	0.08 (2)
D- I	-45 / 0	0.02 (3)
I- E	-634 / 0	0.40 (1)
I- F	0 / 1352	0.30 (1)
L- C	-1401 / 0	0.58 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.90/1.00 (F-H:1) , BC=0.46/1.00 (K-L:2) , WB=0.58/1.00 (C-L:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

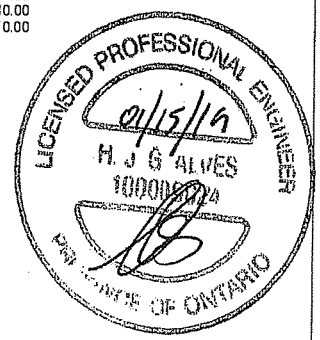
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.56 (H) (INPUT = 1.00)



DWG NO. TAM T1901184
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T313	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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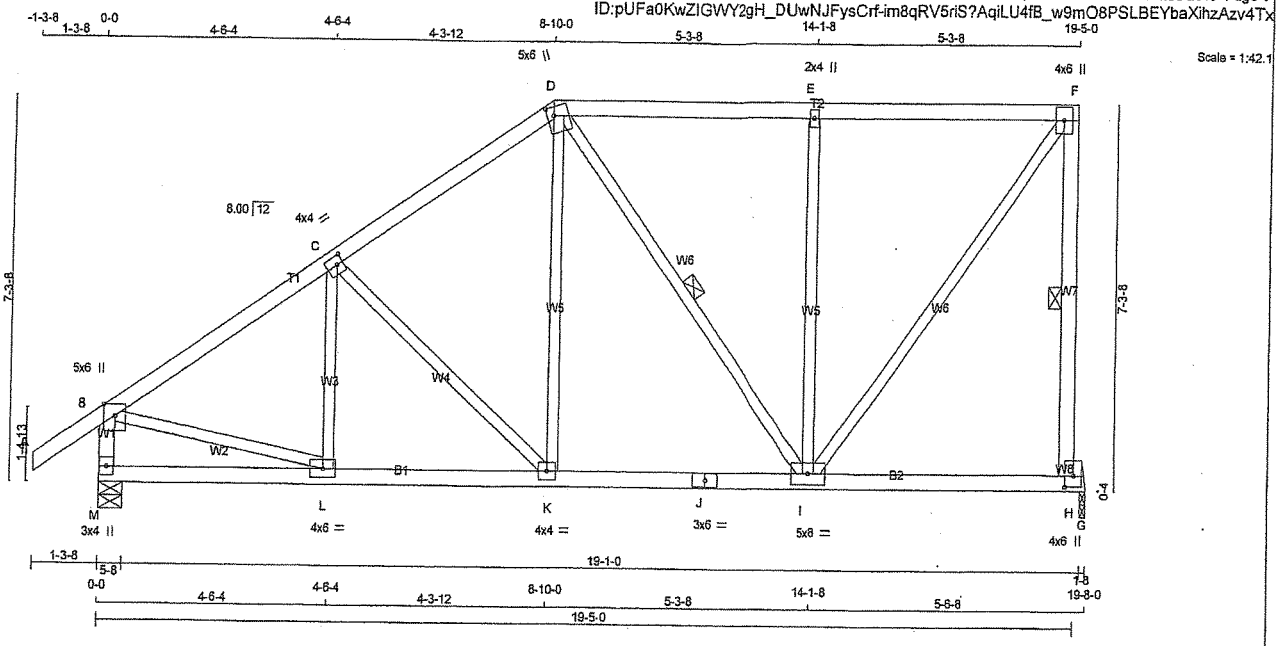


TOTAL WEIGHT = 93 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - O	2x4	ORY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
M - J	2x4	ORY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

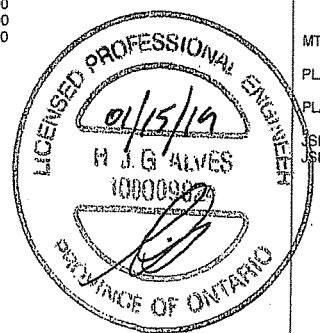
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B-C	-1294 / 0	-84.9	-84.9	0.31 (1)		5.31
C-D	-1043 / 0	-84.9	-84.9	0.29 (1)		5.77
D-E	-760 / 0	-84.9	-84.9	0.41 (1)		6.25
E-F	-760 / 0	-84.9	-84.9	0.41 (1)		6.25
H-F	-1171 / 0	0.0	0.0	0.26 (1)		5.90
M-B	-1258 / 0	0.0	0.0	0.13 (1)		7.17
M-L	0 / 0	-38.5	-38.5	0.14 (3)		10.00
L-K	0 / 1096	-38.5	-38.5	0.29 (2)		10.00
K-J	0 / B52	-38.5	-38.5	0.41 (2)		10.00
J-I	0 / 852	-38.5	-38.5	0.41 (2)		10.00
I-H	0 / 0	-38.5	-38.5	0.41 (1)		10.00
H-G	0 / 0	-53.5	-53.5	0.17 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-66 / 113	0.03 (3)
C-K	-342 / 0	0.22 (1)
K-D	0 / 400	0.09 (2)
D-I	-152 / 0	0.08 (1)
I-E	-554 / 0	0.52 (1)
I-F	0 / 1254	0.28 (1)
B-L	0 / 1127	0.25 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.41/1.00 (E-F:1) , BC=0.41/1.00 (I-K:2) , WB=0.52/1.00 (E-I:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (H) (INPUT = 0.90)
SI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM T1901185
STRUCTURAL
COPY OF ORIGINAL ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T314	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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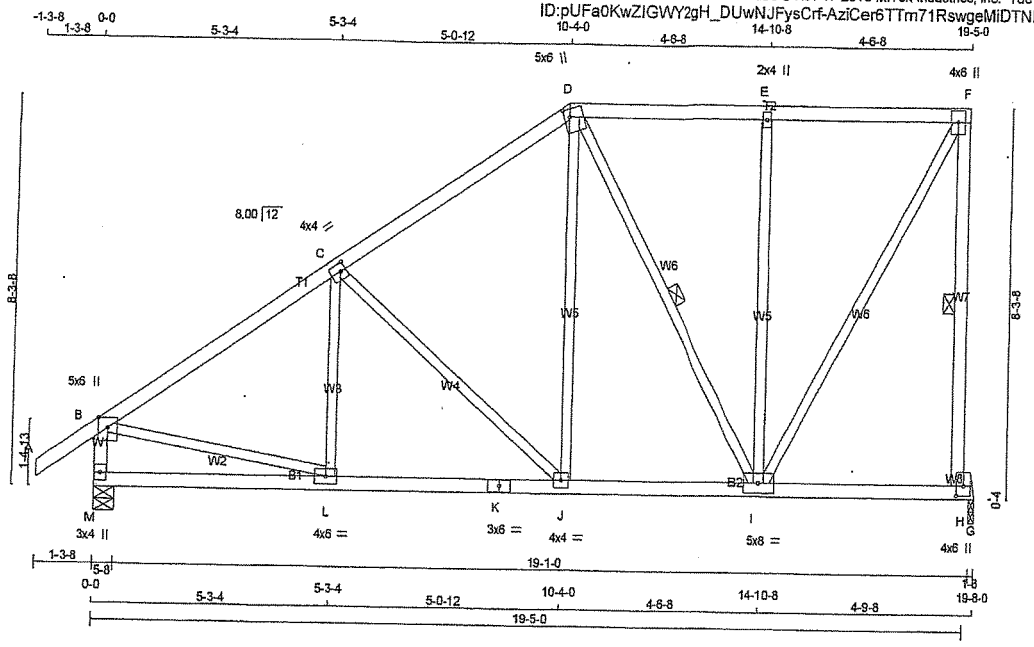


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
H	BMVW+p	MT20	4.0	6.0	2.50	2.00
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1329	0	1329	0	0	5-8	5-8
G	1196	0	1198	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

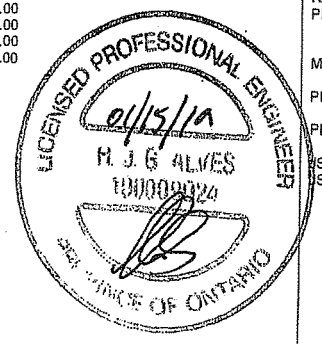
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1295 / 0	-84.9	-84.9	0.44 (1)	5.13
C-D	-926 / 0	-84.9	-84.9	0.41 (1)	5.85
D-E	-597 / 0	-84.9	-84.9	0.29 (1)	6.25
E-F	-597 / 0	-84.9	-84.9	0.29 (1)	6.25
H-F	-1192 / 0	0.0	0.0	0.37 (1)	5.85
M-B	-1248 / 0	0.0	0.0	0.13 (1)	7.19
M-L	0 / 0	-38.5	-38.5	0.20 (3)	10.00
L-K	0 / 1101	-38.5	-38.5	0.35 (2)	10.00
K-J	0 / 1101	-38.5	-38.5	0.35 (2)	10.00
J-I	0 / 751	-38.5	-38.5	0.34 (2)	10.00
I-H	0 / 0	-38.5	-38.5	0.40 (1)	10.00
H-G	0 / 0	-53.5	-53.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	0 / 181	0.04 (3)
C-J	-477 / 0	0.43 (1)
J-D	0 / 475	0.11 (2)
D-I	-313 / 0	0.18 (1)
I-E	-474 / 0	0.64 (1)
I-F	0 / 1201	0.27 (1)
B-L	0 / 1124	0.25 (1)



TOTAL WEIGHT = 99 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.44/1.00 (B-C:1) , BC=0.40/1.00 (H-I:1) , WB=0.64/1.00 (E-I:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (H) (INPUT = 0.90)
SI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM 1790186
STRUCTURAL
COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T315	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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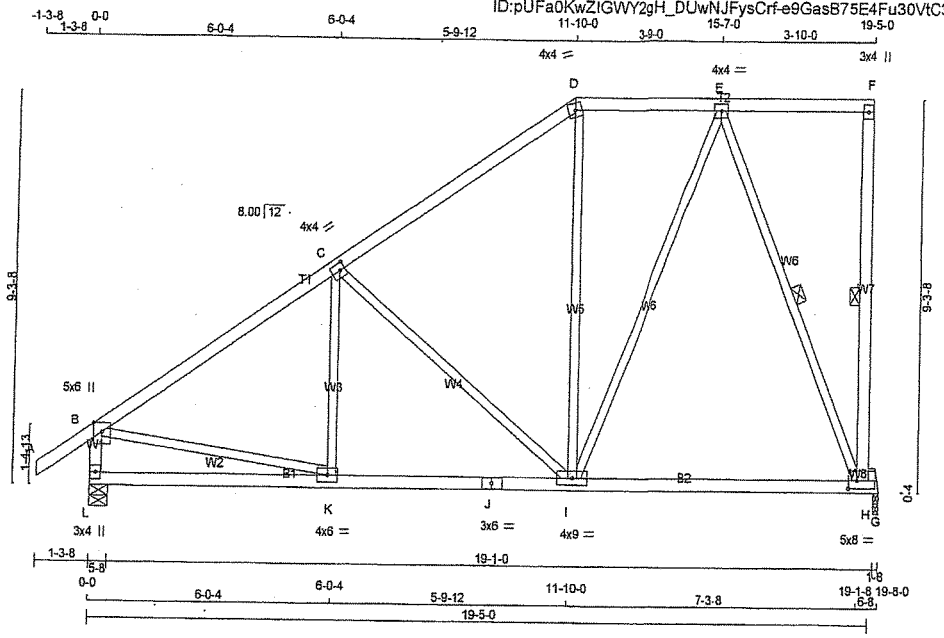


TOTAL WEIGHT = 98 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	5.0	8.0	2.25	2.50
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, E-H.

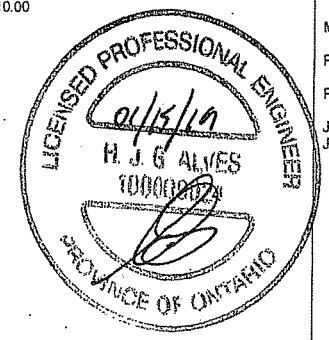
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B- C	-1268 / 0	-84.9	-84.9	0.60 (1)	4.92	
C- D	-831 / 0	-84.9	-84.9	0.55 (1)	5.82	
D- E	-663 / 0	-84.9	-84.9	0.21 (1)	6.25	
E- F	0 / 0	-84.9	-84.9	0.20 (1)	10.00	
H- F	-124 / 0	0.0	0.0	0.05 (1)	6.25	
L- B	-1228 / 0	0.0	0.0	0.13 (1)	7.23	
L- K	0 / 0	-38.5	-38.5	0.23 (3)	10.00	
K- J	0 / 1083	-38.5	-38.5	0.60 (2)	10.00	
J- I	0 / 1083	-38.5	-38.5	0.60 (2)	10.00	
I- H	0 / 422	-38.5	-38.5	0.57 (2)	10.00	
H- G	0 / 0	-53.5	-53.5	0.17 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 189	0.04 (3)
C- I	-556 / 0	0.70 (1)
I- D	0 / 184	0.04 (3)
B- K	0 / 1100	0.25 (1)
I- E	0 / 642	0.14 (1)
E- H	-1078 / 0	0.73 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 655 (0.36")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.60/1.00 (I-K:2) , WB=0.73/1.00 (E-H:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1904B7
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T316	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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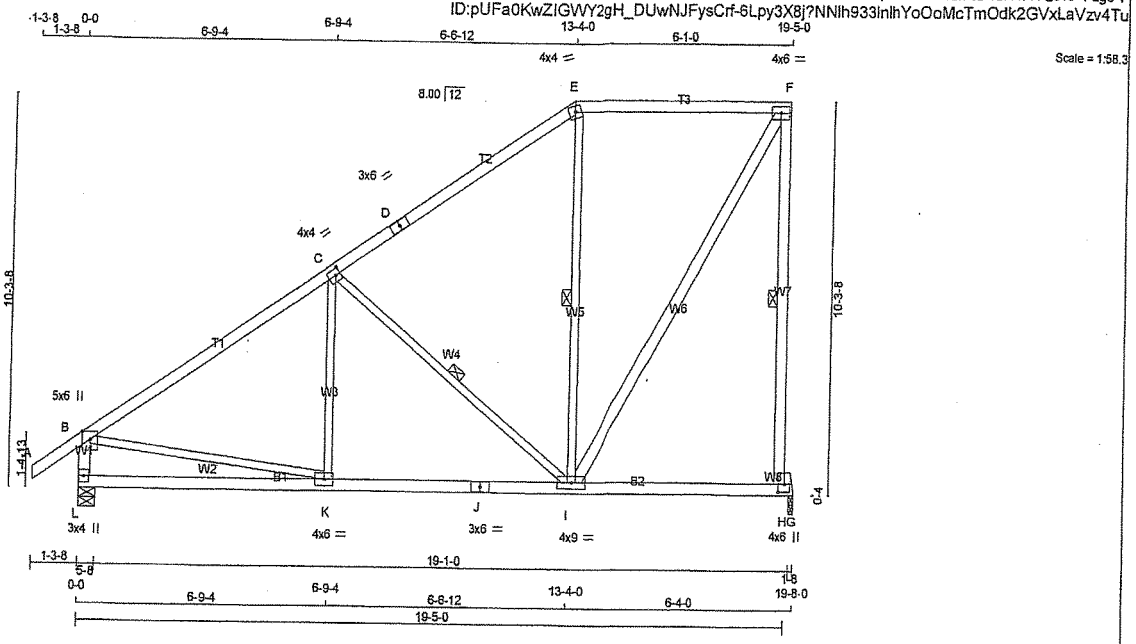


TOTAL WEIGHT = 99 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW+p	MT20	5.0	6.0	Edge		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50	
D	TS-t	MT20	3.0	6.0			
E	TTW-m	MT20	4.0	4.0			
F	TMVW-t	MT20	4.0	6.0			
H	BMVW+p	MT20	4.0	6.0	2.50	2.00	
I	BMWWW-t	MT20	4.0	9.0			
J	BS-t	MT20	3.0	6.0			
K	BMWW-t	MT20	4.0	6.0			
L	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
L	1329	0	1329	0	0	5-8	5-8
G	1196	0	1196	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
L	1000	522 / 0	207 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	909	446 / 0	207 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-H, C-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

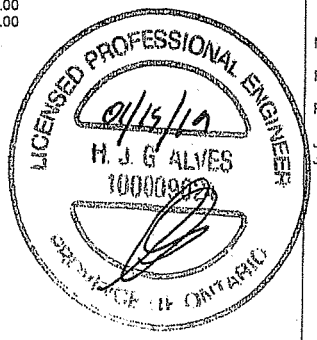
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11	(1)	10.00
B- C	-1251 / 0	-84.9	-84.9	0.80	(1)	4.58
C- D	-685 / 0	-84.9	-84.9	0.70	(1)	5.85
D- E	-685 / 0	-84.9	-84.9	0.70	(1)	5.85
E- F	-536 / 0	-84.9	-84.9	0.56	(1)	6.25
H- F	-1153 / 0	0.0	0.0	0.60	(1)	5.93
L- B	-1221 / 0	0.0	0.0	0.13	(1)	7.25
L- K	0 / 0	-38.5	-38.5	0.31	(3)	10.00
K- J	0 / 1073	-38.5	-38.5	0.50	(2)	10.00
J- I	0 / 1073	-38.5	-38.5	0.50	(2)	10.00
I- H	0 / 0	-38.5	-38.5	0.42	(1)	10.00
H- G	0 / 0	-53.5	-53.5	0.17	(1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 271	0.06 (3)
C- I	-699 / 0	0.36 (1)
I- E	-108 / 110	0.08 (1)
I- F	0 / 1045	0.17 (1)
B- K	0 / 1086	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.80/1.00 (B-C:1) , BC=0.50/1.00 (I-K:2) , WB=0.36/1.00 (C-I:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.56 (H) (INPUT = 1.00)

DWG NO. TAM T1901188
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T317	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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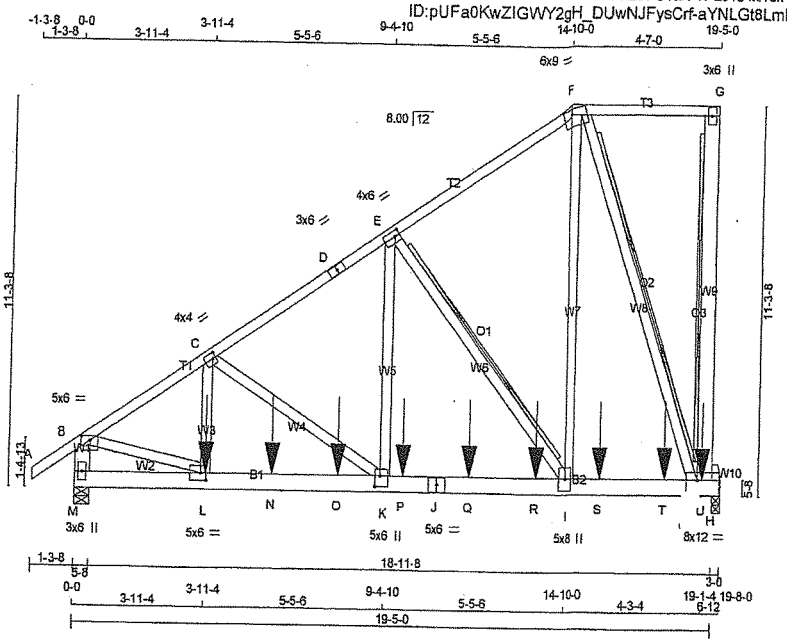


TOTAL WEIGHT = 4 X 141 = 565 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- G	1	12	TOP
G- H	2	12	SIDE(71.3)
M- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	TOP
J- H	2	5	SIDE(711.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0	2.00	1.75
F	TTWW-m	MT20	6.0	9.0	1.75	3.50
G	TMV+p	MT20	3.0	6.0		
H	BMVWW1-t	MT20	8.0	12.0	5.25	4.00
I	BMWW+t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	5.0	6.0	3.75	2.00
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
H	11302	0	11302	0	0	3-0	3-0
M	8331	0	8331	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
H	8543	4343 / 0	1848 / 0	0 / 0	0 / 0	2351 / 0	0 / 0
M	6289	3221 / 0	1343 / 0	0 / 0	0 / 0	1725 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT G-H, E-I, F-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.04 (1)	10.00	
B- C	-10687 / 0	-84.9	-84.9	0.24 (1)	3.81	
C- D	-8465 / 0	-84.9	-84.9	0.34 (1)	4.18	
D- E	-8465 / 0	-84.9	-84.9	0.34 (1)	4.18	
E- F	-4035 / 0	-84.9	-84.9	0.17 (1)	5.68	
F- G	0 / 0	-84.9	-84.9	0.11 (1)	10.00	
H- G	-194 / 0	0.0	0.0	0.01 (1)	7.81	
M- B	-8448 / 0	0.0	0.0	0.15 (1)	6.71	
M- L	0 / 0	-38.5	-38.5	0.16 (1)	10.00	
L- N	0 / 8903	-38.5	-38.5	0.83 (1)	10.00	
N- O	0 / 8903	-38.5	-38.5	0.83 (1)	10.00	
O- K	0 / 8903	-38.5	-38.5	0.83 (1)	10.00	
K- P	0 / 7055	-38.5	-38.5	0.77 (1)	10.00	
P- J	0 / 7055	-38.5	-38.5	0.77 (1)	10.00	
J- Q	0 / 7055	-38.5	-38.5	0.77 (1)	10.00	
Q- R	0 / 7055	-38.5	-38.5	0.77 (1)	10.00	
R- I	0 / 7055	-38.5	-38.5	0.77 (1)	10.00	
I- S	0 / 3468	-38.5	-38.5	0.59 (1)	10.00	
S- T	0 / 3468	-38.5	-38.5	0.59 (1)	10.00	
T- U	0 / 3468	-38.5	-38.5	0.59 (1)	10.00	
U- H	0 / 3468	-38.5	-38.5	0.59 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
L- C	0 / 2084	0.08	(1)
C- K	-2219 / 0	0.30	(1)
K- E	0 / 6767	0.27	(1)
E- I	-6094 / 0	0.31	(1)
I- F	0 / 1010	0.41	(1)
F- H	-8984 / 0	0.71	(1)
B- L	0 / 0	0.00	(1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	3-11-4	-2654	-2654	—	TOP	VERT	TOTAL	—	—
N	5-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—
O	7-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—
P	9-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.34/1.00 (C-E:1) , BC=0.83/1.00 (K-L:1) , WB=0.71/1.00 (F-H:1) , SSI=0.52/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.56 (L) (INPUT = 1.00)

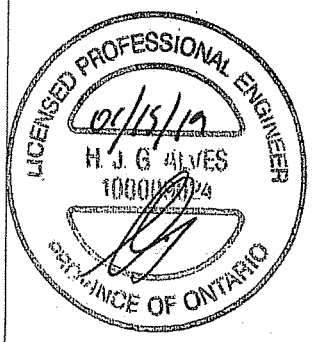
LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G ALVES
100009[illegible]
PROVINCE OF ONTARIO

DWG NO. T317 1901189
STRUCTURAL
CHECK FOR DEPTH ONLY
1/2

CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	11-11-4	-1807	-1807	—	FRONT	VERT	TOTAL	—	—
R	13-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—
S	15-11-4	-1807	-1807	—	FRONT	VERT	TOTAL	—	—
T	17-11-4	-1807	-1807	—	FRONT	VERT	TOTAL	—	—
U	19-1-4	-1820	-1820	—	FRONT	VERT	TOTAL	—	—



DWG NO. TAM T7901189
STRUCTURAL
CONSTRUCTION ONLY
3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T318	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington 7-10

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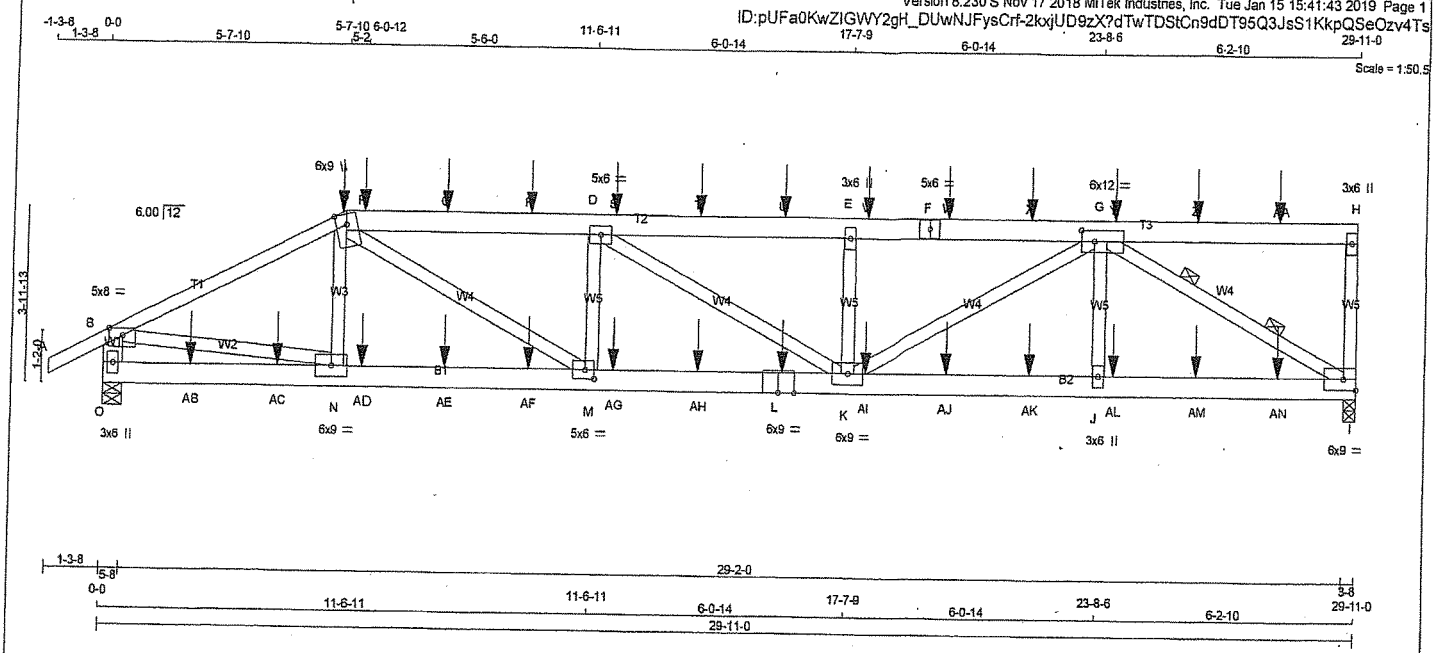

TOTAL WEIGHT = 160 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	ORY	1650F 1.5E	SPF
C - F	2x6	ORY	No.2	SPF
F - H	2x6	ORY	No.2	SPF
I - H	2x4	ORY	No.2	SPF
O - B	2x6	ORY	No.2	SPF
O - L	2x6	ORY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT2D	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWWW-t	MT20	6.0	12.0	3.00	3.75
H	TMV+p	MT20	3.0	6.0		
I	BMVW1-t	MT2D	6.0	9.0		Edge
J	BMW+w	MT20	3.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BS-t	MT2D	6.0	9.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BMWW-t	MT20	6.0	9.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2692	0	2692	0	0	3-8	3-8
O	2828	0	2828	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2040	1022 / 0	453 / 0	0 / 0	0 / 0	565 / 0	0 / 0
O	2133	1098 / 0	451 / 0	0 / 0	0 / 0	584 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS ANO PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.10 (1)	10.00
B-C	-4142 / 0	-84.9	-84.9	0.72 (1)	3.35
C-P	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
P-Q	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
Q-R	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
R-D	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
D-S	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
S-T	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
T-U	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
U-E	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
E-V	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
V-F	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
F-W	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
W-X	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
X-G	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
G-Y	0 / 0	-84.9	-84.9	0.31 (1)	10.00
Y-Z	0 / 0	-84.9	-84.9	0.31 (1)	10.00
Z-AA	0 / 0	-84.9	-84.9	0.31 (1)	10.00
AA-H	0 / 0	-84.9	-84.9	0.31 (1)	10.00
I-H	-286 / 0	0.0	0.0	0.06 (1)	7.81
O-B	-2713 / 0	0.0	0.0	0.19 (1)	6.29
O-AB	0 / 0	-38.5	-38.5	0.16 (3)	10.00
AB-AC	0 / 0	-38.5	-38.5	0.16 (3)	10.00
AC-N	0 / 0	-38.5	-38.5	0.16 (3)	10.00
N-AD	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
AD-AE	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
AE-AF	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
AF-M	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
M-AG	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
AG-AH	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
AH-L	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
L-K	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
K-AI	0 / 4000	-38.5	-38.5	0.63 (1)	10.00
AI-AJ	0 / 4000	-38.5	-38.5	0.63 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
N-C	-171 / 188	0.08 (3)
B-N	0 / 3750	0.66 (1)
G-I	-4598 / 0	0.70 (1)
C-M	0 / 2364	0.42 (1)
J-G	0 / 493	0.09 (3)
M-D	-877 / 0	0.14 (1)
K-G	0 / 2043	0.36 (1)
D-K	0 / 47	0.01 (3)
K-E	-706 / 0	0.12 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100099934
PROVINCE OF ONTARIO

ENGINEERING FOR THIS STRUCTURAL COMPONENT ONLY

T19011910

1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 870 (0.41")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.87/1.00 (K-M:1) , WB=0.70/1.00 (G-I:1) , SSI=0.32/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.90 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AJ-AK	0 / 4000	-38.5	-38.5	0.63 (1)	10.00			
AK- J	0 / 4000	-38.5	-38.5	0.63 (1)	10.00			
J-AL	0 / 4000	-38.5	-38.5	0.67 (1)	10.00			
AL-AM	0 / 4000	-38.5	-38.5	0.67 (1)	10.00			
AM-AN	0 / 4000	-38.5	-38.5	0.67 (1)	10.00			
AN- I	0 / 4000	-38.5	-38.5	0.67 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-7-10	-41	-46	—	FRONT	VERT	DEAD	—	—
C	5-7-10	-189	-189	—	FRONT	VERT	SNOW	—	—
L	16-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
P	6-0-12	-104	-104	—	BACK	VERT	TOTAL	—	—
Q	8-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
R	10-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
S	12-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
T	14-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
U	16-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
V	18-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
W	20-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
X	22-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
Y	24-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
Z	26-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
AA	28-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
AB	2-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AC	4-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AD	6-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AE	8-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AF	10-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AG	12-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AH	14-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
Ai	18-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AJ	20-0-12	-38	-46	—	BACK	VERT	TOTAL	—	—
AK	22-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AL	24-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AM	26-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AN	28-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. S. GRAVES
100099924
PROVINCE OF ONTARIO

DWG NO. TAM T1901190
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T318A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington 5-7-10

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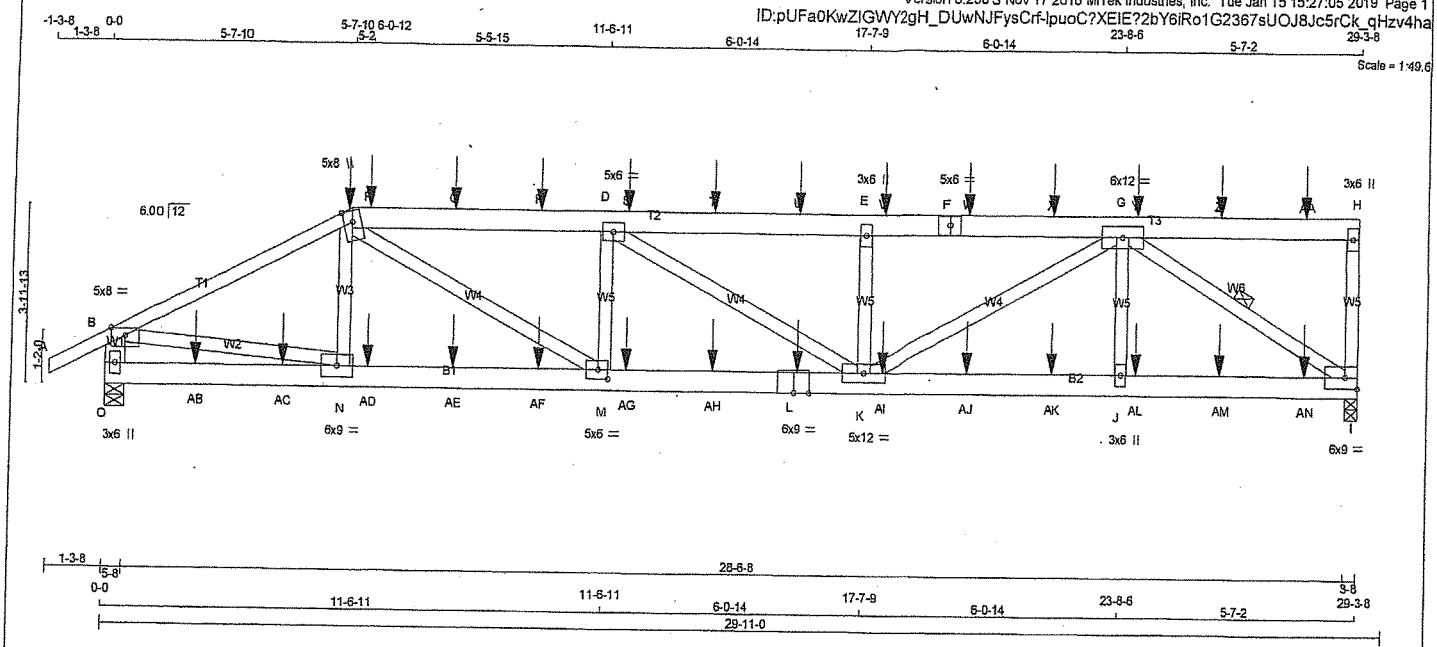


TOTAL WEIGHT = 157 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWWW-t	MT20	6.0	12.0		
H	TMV+p	MT20	3.0	8.0		
I	BMVW1-t	MT20	6.0	9.0		Edge
J	BMW+w	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	12.0		
L	BS-t	MT20	6.0	9.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWW-t	MT20	6.0	9.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2672	0	2672	0	0	3-8	3-8
O	2771	0	2771	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2025	1014 / 0	449 / 0	0 / 0	0 / 0	561 / 0	0 / 0
O	2090	1076 / 0	442 / 0	0 / 0	0 / 0	572 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS ANO PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-I.

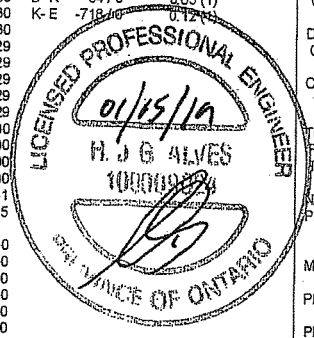
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.10 (1)	10.00
B- C	-4043 / 0	-84.9	-84.9	0.71 (1)	3.39
C- P	-5561 / 0	-84.9	-84.9	0.54 (1)	3.25
P- O	-5561 / 0	-84.9	-84.9	0.54 (1)	3.25
O- R	-5561 / 0	-84.9	-84.9	0.54 (1)	3.25
R- D	-5561 / 0	-84.9	-84.9	0.54 (1)	3.25
D- S	-5489 / 0	-84.9	-84.9	0.53 (1)	3.30
S- T	-5489 / 0	-84.9	-84.9	0.53 (1)	3.30
T- U	-5489 / 0	-84.9	-84.9	0.53 (1)	3.30
U- E	-5489 / 0	-84.9	-84.9	0.53 (1)	3.30
E- V	-5489 / 0	-84.9	-84.9	0.53 (1)	3.29
V- F	-5489 / 0	-84.9	-84.9	0.53 (1)	3.29
F- W	-5489 / 0	-84.9	-84.9	0.53 (1)	3.29
W- X	-5489 / 0	-84.9	-84.9	0.53 (1)	3.29
X- G	-5489 / 0	-84.9	-84.9	0.53 (1)	3.29
G- Y	0 / 0	-84.9	-84.9	0.28 (1)	10.00
Y- Z	0 / 0	-84.9	-84.9	0.28 (1)	10.00
Z-AA	0 / 0	-84.9	-84.9	0.28 (1)	10.00
AA- H	0 / 0	-84.9	-84.9	0.28 (1)	10.00
I- H	-270 / 0	0.0	0.0	0.06 (1)	7.81
O- B	-2657 / 0	0.0	0.0	0.19 (1)	6.35
O-AB	0 / 0	-38.5	-38.5	0.16 (3)	10.00
AB-AC	0 / 0	-38.5	-38.5	0.16 (3)	10.00
AC- N	0 / 0	-38.5	-38.5	0.16 (3)	10.00
N-AD	0 / 3631	-38.5	-38.5	0.58 (1)	10.00
AD-AE	0 / 3631	-38.5	-38.5	0.58 (1)	10.00
AE-AF	0 / 3631	-38.5	-38.5	0.58 (1)	10.00
AF- M	0 / 3631	-38.5	-38.5	0.58 (1)	10.00
M-AG	0 / 5561	-38.5	-38.5	0.85 (1)	10.00
AG-AH	0 / 5561	-38.5	-38.5	0.85 (1)	10.00
AH- L	0 / 5561	-38.5	-38.5	0.85 (1)	10.00
L- K	0 / 5561	-38.5	-38.5	0.85 (1)	10.00
K-AI	0 / 3603	-38.5	-38.5	0.58 (1)	10.00
AI-AJ	0 / 3803	-38.5	-38.5	0.58 (1)	10.00

WEBS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-158 / 195	0.04 (3)
B- N	0 / 3660	0.65 (1)
G- I	-4260 / 0	0.89 (1)
C- M	0 / 2252	0.40 (1)
J- G	0 / 465	0.09 (3)
M- D	-820 / 0	0.13 (1)
K- G	0 / 2181	0.39 (1)
D- K	-84 / 0	0.05 (1)
K- E	-718 / 0	0.12 (1)



DWG NO. T.M 7801/91
STRUCTURAL
ONLY 1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS.

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 924 (0.38")

CSI: TC=0.71/1.00 (B-C:1) , BC=0.65/1.00 (K-M:1) , WB=0.89/1.00 (G-I:1) , SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.87 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2

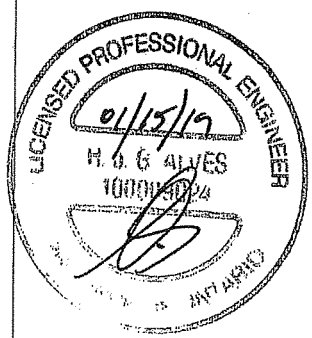
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AJ-AK	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AK- J	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
J-AL	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AL-AM	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AM-AN	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AN- I	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-7-10	-41	-46	--	FRONT	VERT	DEAD	--	--
C	5-7-10	-189	-189	--	FRONT	VERT	SNOW	--	--
L	16-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
P	6-0-12	-104	-104	--	FRONT	VERT	TOTAL	--	--
O	8-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
R	10-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
S	12-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
T	14-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
U	16-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
V	18-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
W	20-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
X	22-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
Y	24-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
Z	26-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
AA	28-0-12	-80	-80	--	FRONT	VERT	TOTAL	--	--
AB	2-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AC	4-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AD	6-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AE	8-0-12	-38	-46	--	FRONT	VERT	TOTAL	--	--
AF	10-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AG	12-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AH	14-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AI	18-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AJ	20-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AK	22-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AL	24-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AM	26-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--
AN	28-0-12	-36	-46	--	FRONT	VERT	TOTAL	--	--



DWG NO. T7901191

CONNECTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T319	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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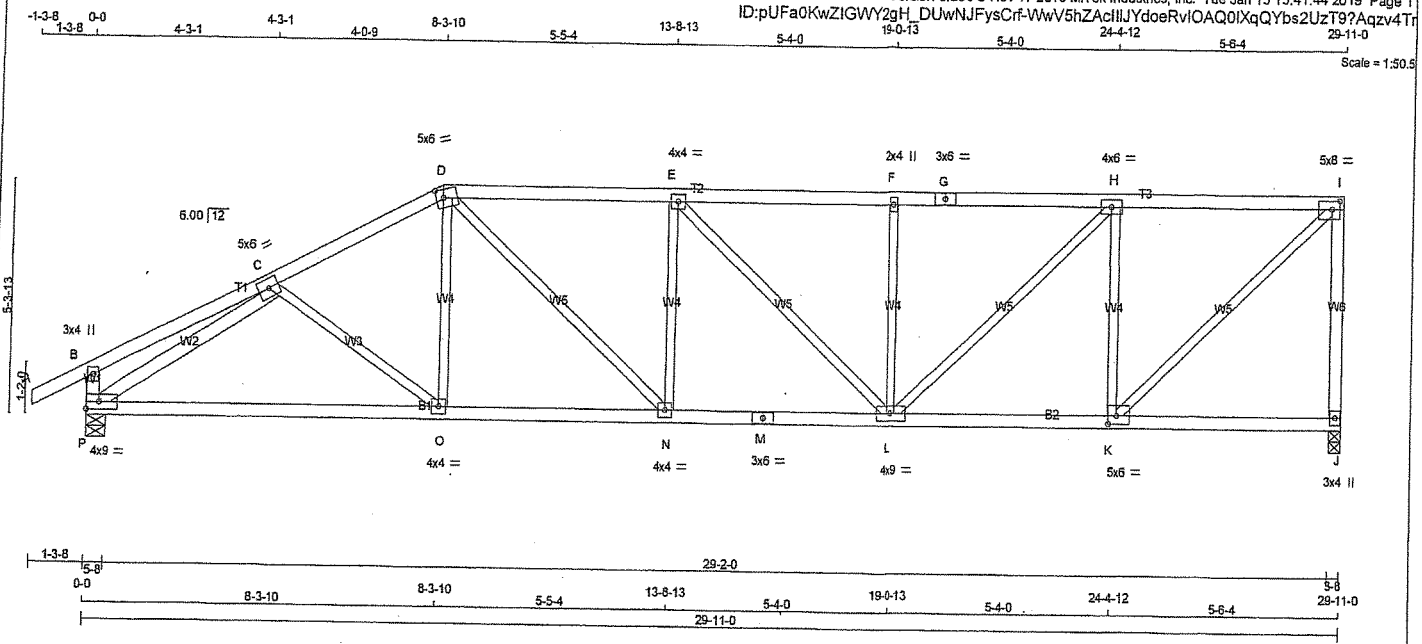


TOTAL WEIGHT = 125 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	ORY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - B	2x4	ORY	No.2	SPF
P - M	2x4	ORY	No.2	SPF
M - J	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - C	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.25	2.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.25	2.25
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1845	0	1845	0	0	3-8	3-8
P	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
P	1479	780 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, P

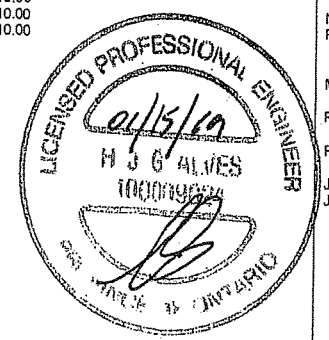
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	
B-C	0 / 16	-84.9	-84.9	0.21 (1)	10.00	
C-D	-2534 / 0	-84.9	-84.9	0.31 (1)	4.06	
D-E	-2786 / 0	-84.9	-84.9	0.53 (1)	3.67	
E-F	-2585 / 0	-84.9	-84.9	0.51 (1)	3.81	
F-G	-2585 / 0	-84.9	-84.9	0.52 (1)	3.80	
G-H	-2585 / 0	-84.9	-84.9	0.52 (1)	3.80	
H-I	-1725 / 0	-84.9	-84.9	0.44 (1)	4.58	
J-I	-1759 / 0	0.0	0.0	0.87 (1)	6.29	
P-B	-256 / 0	0.0	0.0	0.03 (1)	7.81	
P-O	0 / 2227	-38.5	-38.5	0.73 (2)	10.00	
O-N	0 / 2256	-38.5	-38.5	0.75 (2)	10.00	
N-M	0 / 2786	-38.5	-38.5	0.55 (1)	10.00	
M-L	0 / 2786	-38.5	-38.5	0.55 (1)	10.00	
L-K	0 / 1725	-38.5	-38.5	0.44 (2)	10.00	
K-J	0 / 0	-38.5	-38.5	0.21 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-O	0 / 151	0.04 (3)
O-O	0 / 367	0.08 (2)
P-C	-2725 / 0	0.84 (1)
K-I	0 / 2334	0.53 (1)
D-N	0 / 721	0.16 (1)
K-H	-1337 / 0	0.56 (1)
N-E	-327 / 0	0.14 (1)
L-H	0 / 1182	0.27 (1)
E-L	-276 / 0	0.29 (1)
L-F	-415 / 0	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	OL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 953 (0.38")

CSI: TC=0.87/1.00 (I-J:1) , BC=0.75/1.00 (N-O:2) , WB=0.84/1.00 (C-P:1) , SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.99 (M) (INPUT = 1.00)

ENG NO. TAM 11901/92
PRELIMINARY
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T319A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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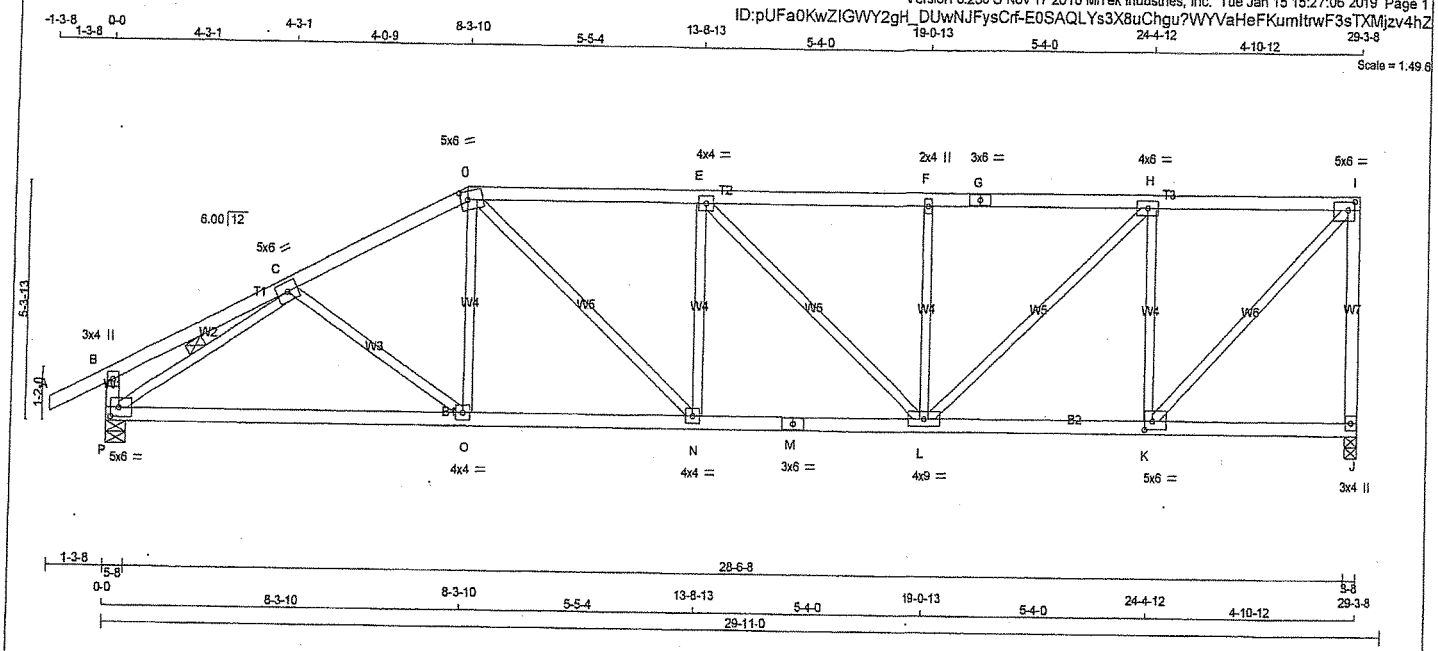


TOTAL WEIGHT = 122 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.25	2.25
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1807	0	1807	0	0	3-8	3-8
P	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
P	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

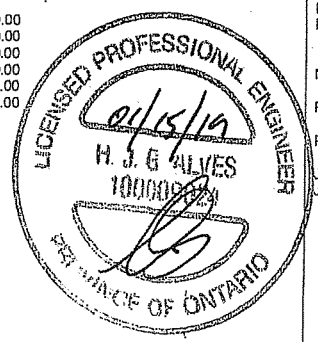
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 28	-84.9	-84.9	0.11 (1)	10.00	C-O	0 / 147	0.03 (3)
B-C	0 / 16	-84.9	-84.9	0.20 (1)	10.00	O-D	0 / 371	0.08 (2)
C-D	-2462 / 0	-84.9	-84.9	0.30 (1)	4.12	P-C	-2656 / 0	0.56 (1)
D-E	-2680 / 0	-84.9	-84.9	0.52 (1)	3.75	K-I	0 / 2190	0.49 (1)
E-F	-2442 / 0	-84.9	-84.9	0.49 (1)	3.93	D-N	0 / 664	0.15 (1)
F-G	-2442 / 0	-84.9	-84.9	0.44 (1)	3.99	K-H	-1351 / 0	0.57 (1)
G-H	-2442 / 0	-84.9	-84.9	0.44 (1)	3.99	N-E	-290 / 15	0.12 (1)
H-I	-1528 / 0	-84.9	-84.9	0.36 (1)	4.91	L-H	0 / 1257	0.28 (1)
J-I	-1732 / 0	0.0	0.0	0.85 (1)	6.33	E-L	-327 / 0	0.34 (1)
P-B	-257 / 0	0.0	0.0	0.03 (1)	7.81	L-F	-426 / 0	0.18 (1)
P-O	0 / 2170	-38.5	-38.5	0.72 (2)	10.00			
O-N	0 / 2192	-38.5	-38.5	0.74 (2)	10.00			
N-M	0 / 2680	-38.5	-38.5	0.54 (1)	10.00			
M-L	0 / 2680	-38.5	-38.5	0.54 (1)	10.00			
L-K	0 / 1528	-38.5	-38.5	0.38 (2)	10.00			
K-J	0 / 0	-38.5	-38.5	0.18 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 932 (0.36")

CSI: TC=0.85/1.00 (I-J:1) , BC=0.74/1.00 (N-O:2) , WB=0.57/1.00 (H-K:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (I) (INPUT = 0.90)
USI METAL= 0.95 (M) (INPUT = 1.00)

ENG NO. TAM T1901193
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T320	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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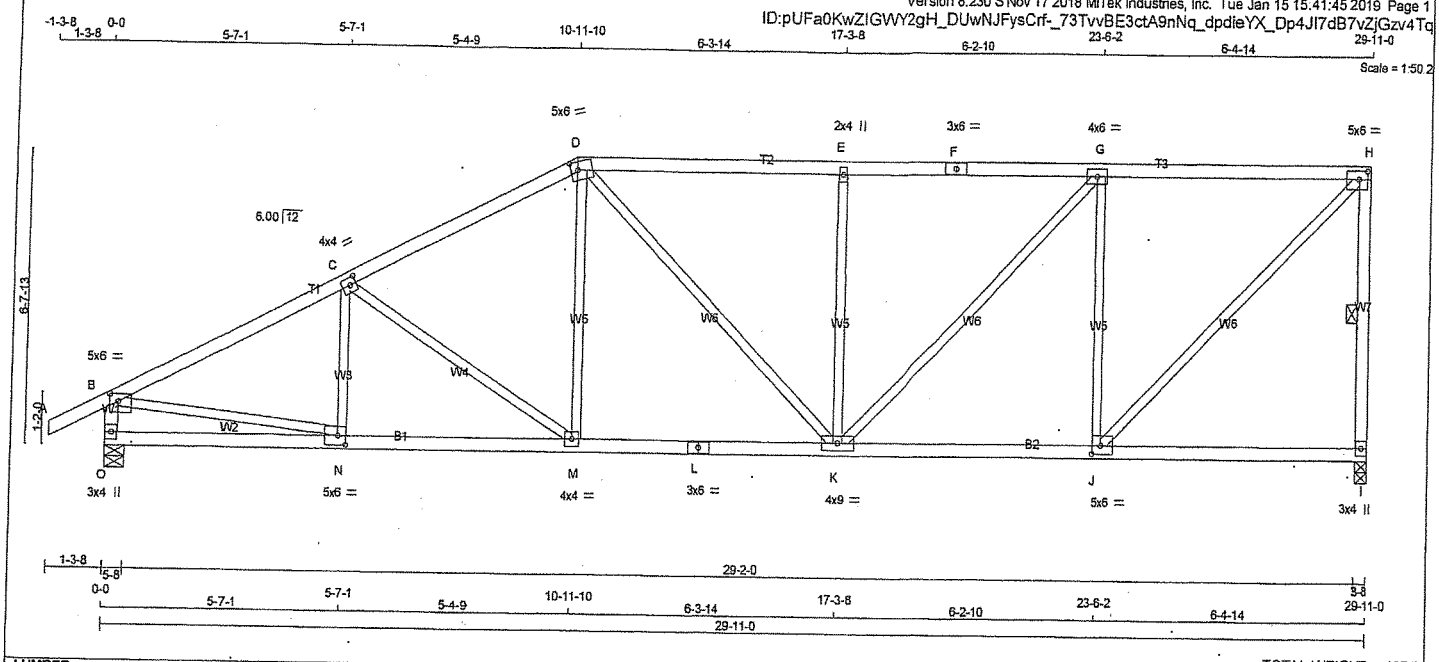


TOTAL WEIGHT = 127 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - F	2x4	ORY	No.2	SPF
F - H	2x4	ORY	No.2	SPF
I - H	2x4	ORY	No.2	SPF
O - B	2x4	ORY	No.2	SPF
O - L	2x4	ORY	No.2	SPF
L - I	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.25	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.25	2.50
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN,	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
I	1845	0	1845	0	0	3-8	3-8
O	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
O	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I.

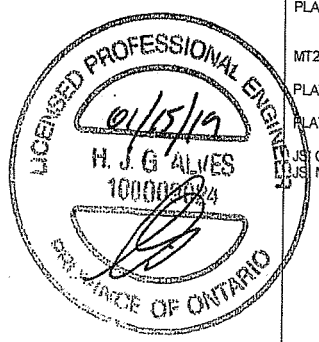
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B- C	-2661 / 0	-84.9	-84.9	0.62 (1)		3.67
C- D	-2295 / 0	-84.9	-84.9	0.56 (1)		3.95
D- E	-2184 / 0	-84.9	-84.9	0.61 (1)		3.97
E- F	-2184 / 0	-84.9	-84.9	0.63 (1)		3.94
F- G	-2184 / 0	-84.9	-84.9	0.63 (1)		3.94
G- H	-1537 / 0	-84.9	-84.9	0.56 (1)		4.62
I- H	-1746 / 0	0.0	0.0	0.35 (1)		5.04
O- B	-1869 / 0	0.0	0.0	0.19 (1)		6.13
O- N	0 / 0	-38.5	-38.5	0.20 (3)		10.00
N- M	0 / 2401	-38.5	-38.5	0.54 (1)		10.00
M- L	0 / 2038	-38.5	-38.5	0.48 (2)		10.00
L- K	0 / 2038	-38.5	-38.5	0.48 (2)		10.00
K- J	0 / 1537	-38.5	-38.5	0.49 (2)		10.00
J- I	0 / 0	-38.5	-38.5	0.29 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-143 / 106	0.03 (1)
C- M	-446 / 0	0.35 (1)
M- D	0 / 505	0.11 (2)
B- N	0 / 2429	0.55 (1)
J- H	0 / 2166	0.49 (1)
D- K	0 / 207	0.05 (1)
J- G	-1251 / 0	0.91 (1)
K- E	-578 / 0	0.42 (1)
K- G	0 / 925	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.63/1.00 (E-G:1) , BC=0.54/1.00 (M-N:1) , WB=0.91/1.00 (G-J:1) , SSI=0.25/1.00 (G-H:1)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LS GRIP= 0.89 (H) (INPUT = 0.90)
LS METAL= 0.78 (L) (INPUT = 1.00)

ENG PER TPIC 7901194

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T320A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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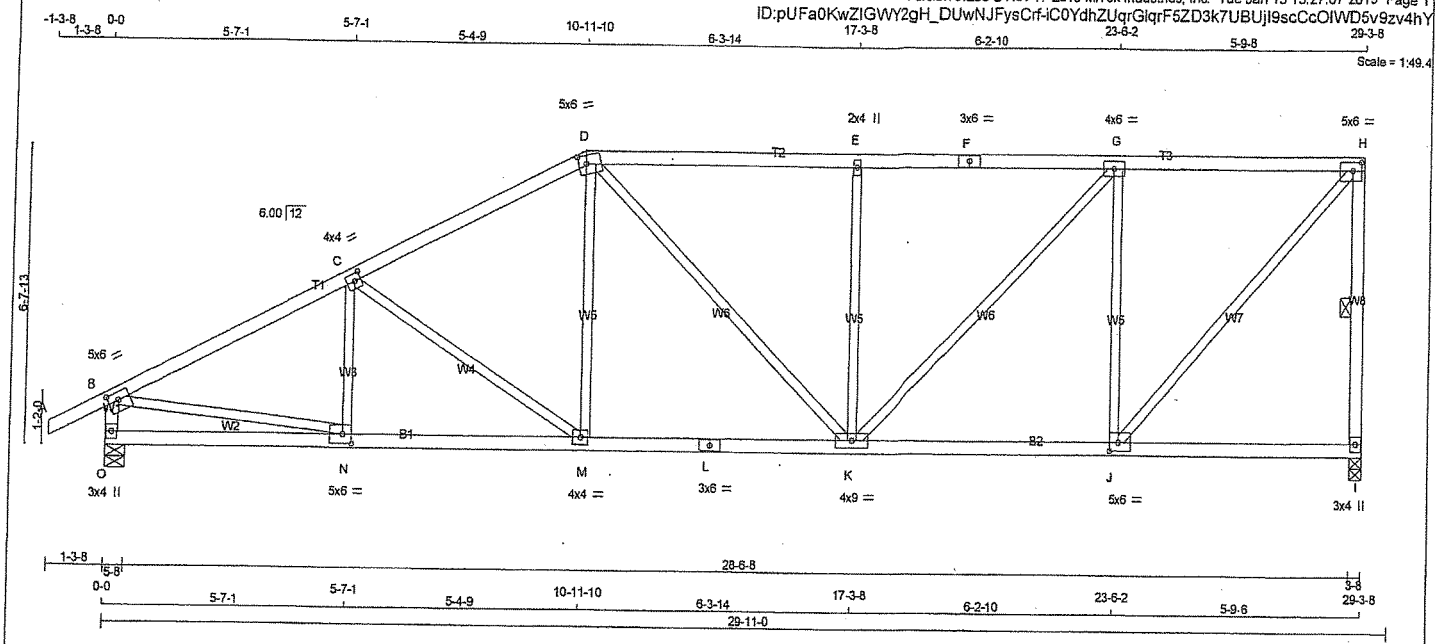


TOTAL WEIGHT = 125 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.25	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.25	2.50
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1807	0	1807	0	0	3-8	3-8
O	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
O	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I.

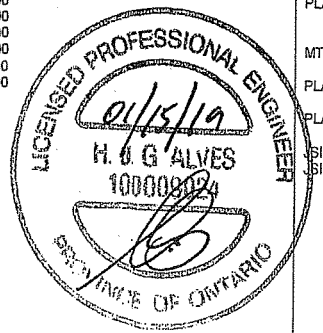
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	1	0.11 (1)	10.00
B- C	-2595 / 0	-84.9	-84.9	1	0.61 (1)	3.72
C- D	-2221 / 0	-84.9	-84.9	1	0.55 (1)	4.01
D- E	-2082 / 0	-84.9	-84.9	1	0.62 (1)	4.03
E- F	-2082 / 0	-84.9	-84.9	1	0.61 (1)	4.03
F- G	-2082 / 0	-84.9	-84.9	1	0.61 (1)	4.03
G- H	-1384 / 0	-84.9	-84.9	1	0.46 (1)	4.95
I- H	-1719 / 0	0.0	0.0	1	0.35 (1)	5.08
O- B	-1831 / 0	0.0	0.0	1	0.19 (1)	6.19
O- N	0 / 0	-38.5	-38.5	3	0.20 (3)	10.00
N- M	0 / 2342	-38.5	-38.5	1	0.52 (1)	10.00
M- L	0 / 1971	-38.5	-38.5	2	0.48 (2)	10.00
L- K	0 / 1971	-38.5	-38.5	2	0.48 (2)	10.00
K- J	0 / 1384	-38.5	-38.5	2	0.44 (2)	10.00
J- I	0 / 0	-38.5	-38.5	3	0.25 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-133 / 111	0.03 (1)
C- M	-456 / 0	0.36 (1)
M- D	0 / 506	0.11 (2)
B- N	0 / 2370	0.53 (1)
J- H	0 / 2058	0.46 (1)
D- K	0 / 157	0.04 (1)
J- G	-1265 / 0	0.92 (1)
K- E	-589 / 0	0.43 (1)
K- G	0 / 998	0.22 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS B.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.52/1.00 (M-N:1) , WB=0.92/1.00 (G-J:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	165B

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (J) (INPUT = 0.90)
SI METAL= 0.76 (L) (INPUT = 1.00)

DWG NO. T320A T1901195

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T321	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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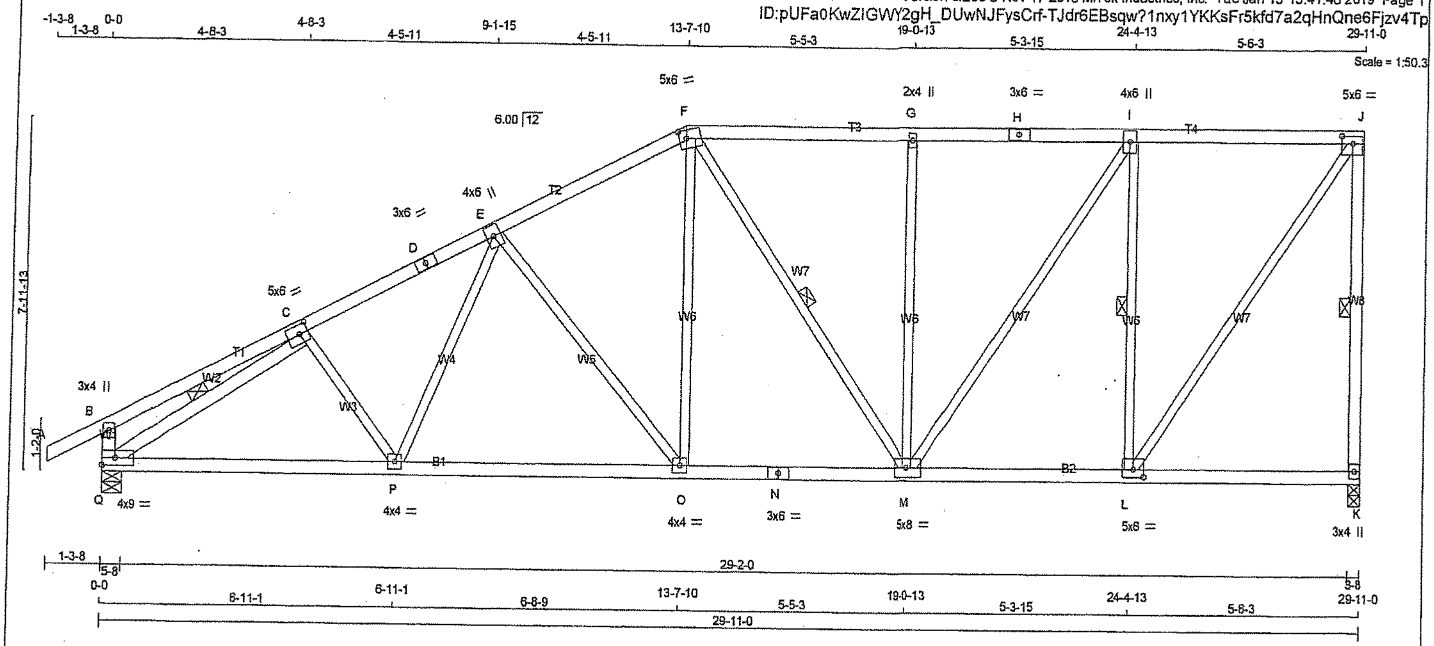

TOTAL WEIGHT = 140 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.00	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0	2.00	3.00
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1845	0	1845	0	0	3-8	3-8
O	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
K	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
O	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, C-Q, F-M, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 15	-84.9	-84.9	0.21 (1)	10.00
C-D	-2568 / 0	-84.9	-84.9	0.35 (1)	4.01
D-E	-2568 / 0	-84.9	-84.9	0.35 (1)	4.01
E-F	-2018 / 0	-84.9	-84.9	0.31 (1)	4.47
F-G	-1698 / 0	-84.9	-84.9	0.40 (1)	4.67
G-H	-1698 / 0	-84.9	-84.9	0.41 (1)	4.64
H-I	-1698 / 0	-84.9	-84.9	0.41 (1)	4.64
I-J	-1126 / 0	-84.9	-84.9	0.37 (1)	5.49
K-J	-1760 / 0	0.0	0.0	0.51 (1)	5.03
Q-B	-276 / 0	0.0	0.0	0.03 (1)	7.81
Q-P	0 / 2291	-38.5	-38.5	0.65 (2)	10.00
P-O	0 / 2181	-38.5	-38.5	0.63 (2)	10.00
O-N	0 / 1798	-38.5	-38.5	0.45 (2)	10.00
N-M	0 / 1798	-38.5	-38.5	0.45 (2)	10.00
M-L	0 / 1126	-38.5	-38.5	0.37 (2)	10.00
L-K	0 / 0	-38.5	-38.5	0.22 (3)	10.00

WEBS

MEMB FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-P	0 / 132	0.03 (3)
P-E	0 / 352	0.08 (2)
E-O	-608 / 0	0.59 (1)
O-F	0 / 696	0.16 (1)
Q-C	-2781 / 0	0.48 (1)
L-J	0 / 1932	0.43 (1)
F-M	-173 / 0	0.11 (1)
L-I	-1331 / 0	0.53 (1)
M-G	-495 / 0	0.60 (1)
M-I	0 / 1003	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.51/1.00 (J-K:1) , BC=0.65/1.00 (P-Q:2) , WB=0.60/1.00 (G-M:1) , SSI=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.69 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO TAM T1901/96
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T321A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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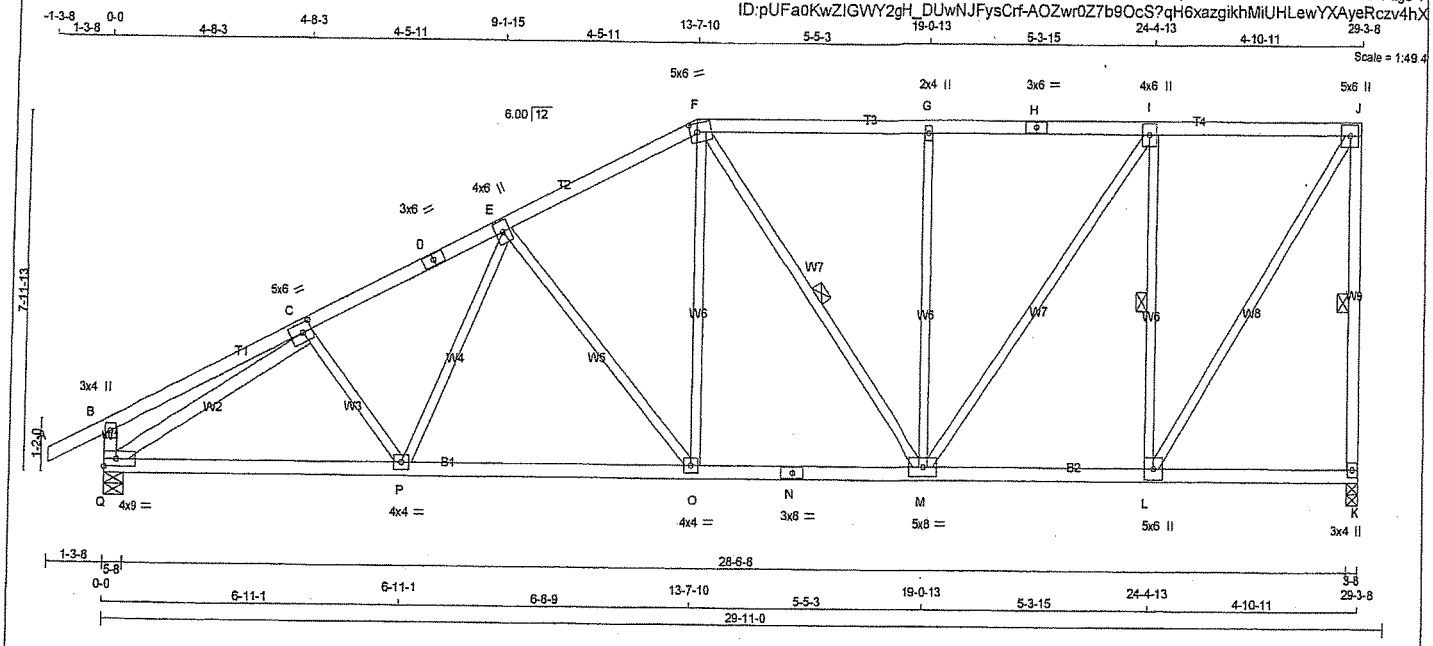


TOTAL WEIGHT = 138 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TMVW+p	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		
L	BMWW+t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
K	1807	0	1807	0	0	3-8	3-8
Q	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
Q	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-K, F-M, I-L.

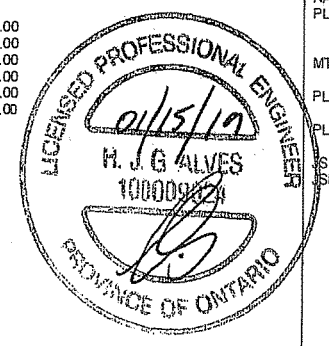
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B- C	0 / 15	-84.9	-84.9	0.21 (1)		10.00
C- D	-2499 / 0	-84.9	-84.9	0.35 (1)		4.06
D- E	-2499 / 0	-84.9	-84.9	0.35 (1)		4.06
E- F	-1941 / 0	-84.9	-84.9	0.30 (1)		4.55
F- G	-1604 / 0	-84.9	-84.9	0.40 (1)		4.75
G- H	-1604 / 0	-84.9	-84.9	0.40 (1)		4.75
H- I	-1604 / 0	-84.9	-84.9	0.40 (1)		4.75
I- J	-996 / 0	-84.9	-84.9	0.30 (1)		5.85
K- J	-1733 / 0	0.0	0.0	0.50 (1)		5.06
O- B	-276 / 0	0.0	0.0	0.03 (1)		7.81
O- P	0 / 2236	-38.5	-38.5	0.64 (2)		10.00
P- O	0 / 2116	-38.5	-38.5	0.62 (2)		10.00
O- N	0 / 1729	-38.5	-38.5	0.44 (2)		10.00
N- M	0 / 1729	-38.5	-38.5	0.44 (2)		10.00
M- L	0 / 996	-38.5	-38.5	0.32 (2)		10.00
L- K	0 / 0	-38.5	-38.5	0.19 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 127	0.03 (3)
P- E	0 / 360	0.08 (2)
E- O	-614 / 0	0.60 (1)
O- F	0 / 699	0.16 (1)
Q- C	-2713 / 0	0.98 (1)
L- J	0 / 1857	0.42 (1)
F- M	-217 / 0	0.13 (1)
L- I	-1345 / 0	0.54 (1)
M- G	-507 / 0	0.61 (1)
M- I	0 / 1067	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.50/1.00 (J-K:1) , BC=0.64/1.00 (P-Q:2) , WB=0.98/1.00 (C-Q:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (J) (INPUT = 0.90)
SI METAL= 0.67 (C) (INPUT = 1.00)

DRAWING NO. T1901197
PRELIMINARY
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T322	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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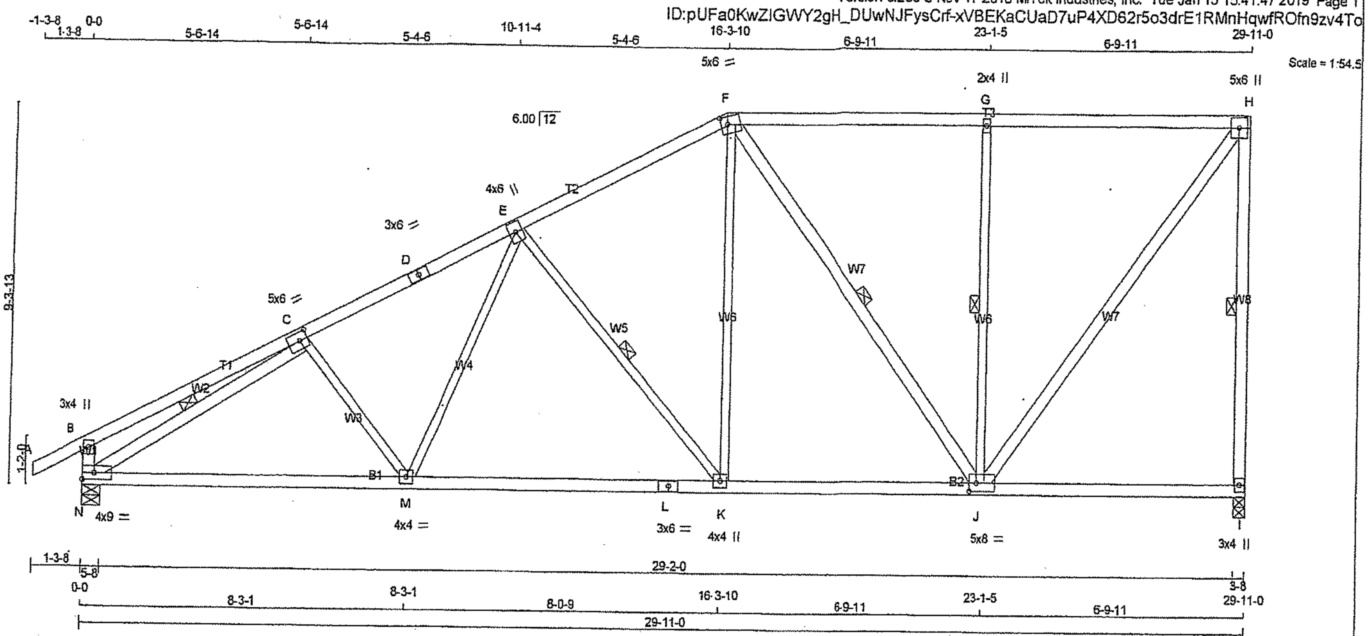

TOTAL WEIGHT = 4 X 144 = 578 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.50	2.25
K	BMWW+t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1845	0	1845	0	0	3-8	3-8
N	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
N	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 19	-84.9	-84.9	0.32 (1)	10.00
C- D	-2511 / 0	-84.9	-84.9	0.37 (1)	4.05
D- E	-2511 / 0	-84.9	-84.9	0.37 (1)	4.05
E- F	-1727 / 0	-84.9	-84.9	0.33 (1)	4.75
F- G	-1152 / 0	-84.9	-84.9	0.53 (1)	5.19
G- H	-1152 / 0	-84.9	-84.9	0.53 (1)	5.19
I- H	-1739 / 0	0.0	0.0	0.71 (1)	5.05
N- B	-305 / 0	0.0	0.0	0.03 (1)	7.81
N- M	0 / 2325	-38.5	-38.5	0.80 (2)	10.00
M- L	0 / 2055	-38.5	-38.5	0.77 (2)	10.00
L- K	0 / 2055	-38.5	-38.5	0.77 (2)	10.00
K- J	0 / 1536	-38.5	-38.5	0.53 (2)	10.00
J- I	0 / 0	-38.5	-38.5	0.33 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-127 / 87	0.04 (1)
M- E	0 / 533	0.12 (2)
E- K	-808 / 0	0.39 (1)
K- F	0 / 877	0.20 (1)
F- J	-640 / 0	0.44 (1)
J- G	-717 / 0	0.41 (1)
J- H	0 / 1906	0.31 (1)
N- C	-2782 / 0	0.58 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.71/1.00 (H-I:1) , BC=0.80/1.00 (M-N:2) , WB=0.58/1.00 (C-N:1) , SSI=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.68 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100008024
PROVINCE OF ONTARIO

BLDG NO. 7901/98
STRUCTURAL
CLEARANCE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T322A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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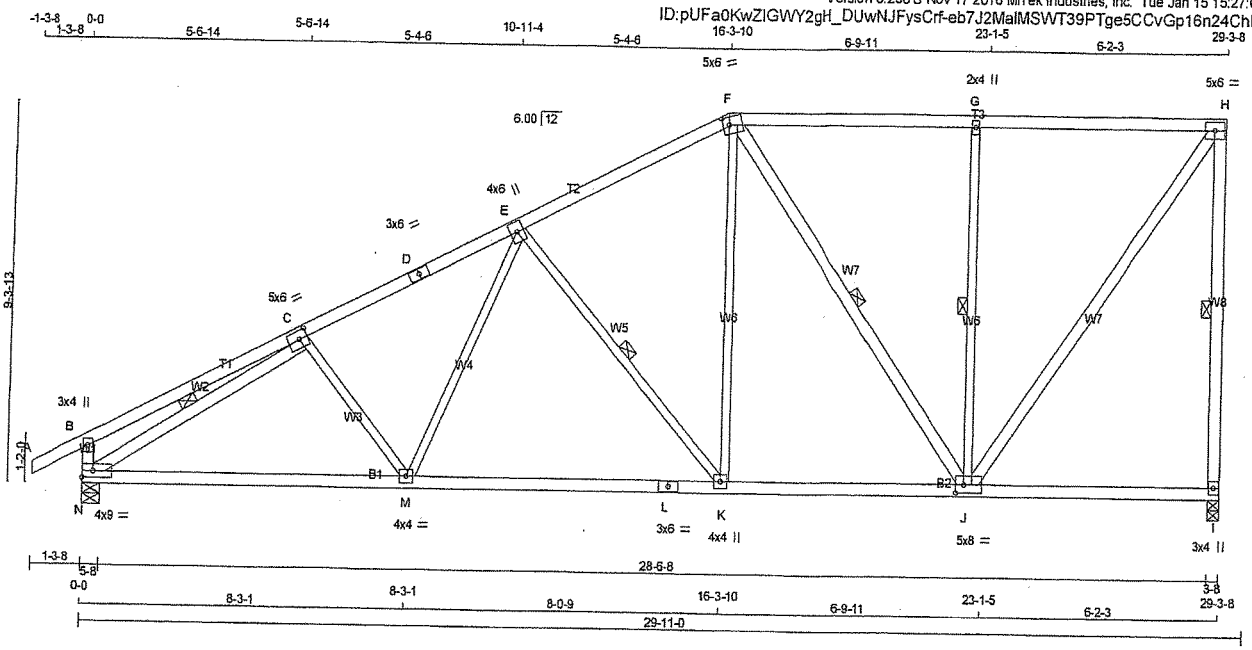


TOTAL WEIGHT = 4 X 143 = 570 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.50	2.50
K	BMWW+t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
I	1807	0	1807	0	0	3-8	3-8
N	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
N	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

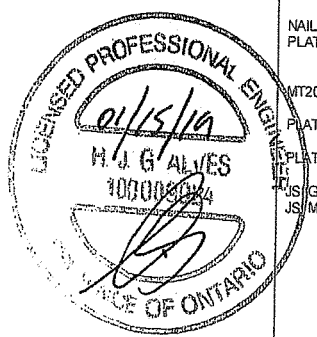
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	1	0.11 (1)	10.00
B- C	0 / 19	-84.9	-84.9	1	0.32 (1)	10.00
C- D	-2440 / 0	-84.9	-84.9	1	0.36 (1)	4.11
D- E	-2440 / 0	-84.9	-84.9	1	0.36 (1)	4.11
E- F	-1649 / 0	-84.9	-84.9	1	0.32 (1)	4.84
F- G	-1082 / 0	-84.9	-84.9	1	0.48 (1)	5.41
G- H	-1082 / 0	-84.9	-84.9	1	0.48 (1)	5.41
I- H	-1705 / 0	0.0	0.0	1	0.69 (1)	5.09
N- B	-305 / 0	0.0	0.0	1	0.03 (1)	7.81
N- M	0 / 2267	-38.5	-38.5	2	0.80 (2)	10.00
M- L	0 / 1988	-38.5	-38.5	2	0.76 (2)	10.00
L- K	0 / 1988	-38.5	-38.5	2	0.76 (2)	10.00
K- J	0 / 1466	-38.5	-38.5	2	0.49 (2)	10.00
J- I	0 / 0	-38.5	-38.5	3	0.29 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-135 / 83	0.05 (1)
M- E	0 / 541	0.12 (2)
E- K	-813 / 0	0.39 (1)
K- F	0 / 876	0.20 (1)
F- J	-659 / 0	0.44 (1)
J- G	-684 / 0	0.39 (1)
J- H	0 / 1845	0.30 (1)
N- C	-2713 / 0	0.57 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.69/1.00 (H-I:1) , BC=0.80/1.00 (M-N:2) , WB=0.57/1.00 (C-N:1) , SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS (GRIP= 0.89 (M) (INPUT = 0.90)
JS (METAL= 0.67 (C) (INPUT = 1.00)

DWG NO. T322A 7/201199
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T323	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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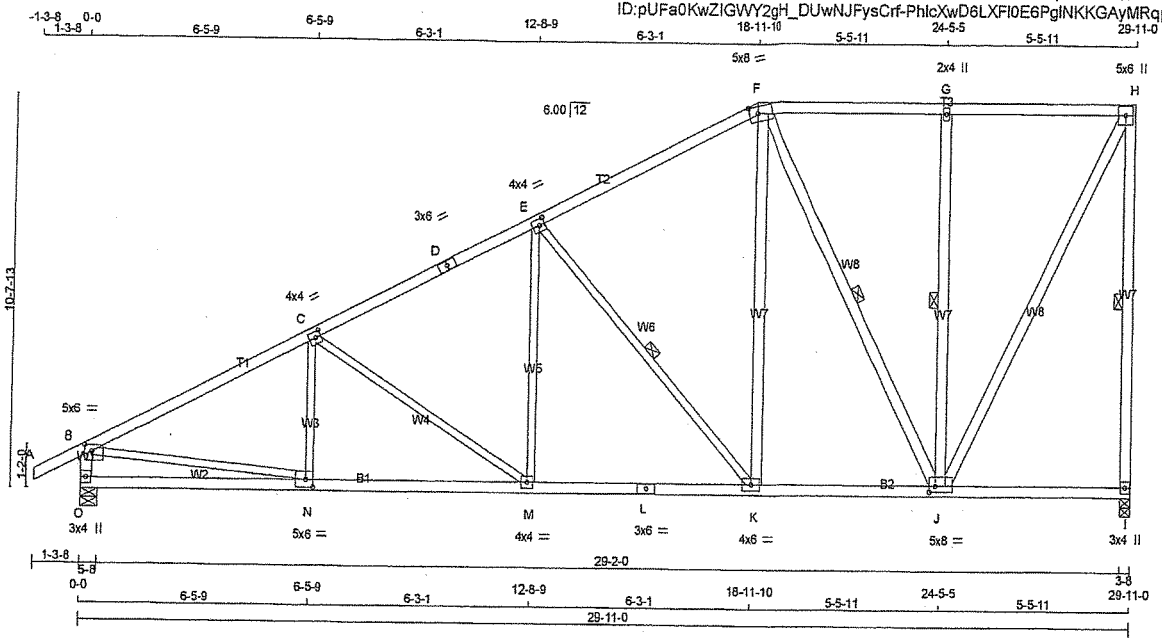


TOTAL WEIGHT = 161 lb

[M][F]

LUMBER

N. L. G A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES *(table is in inches)*

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTWW-m	MT20	5.0	8.0	2.25	3.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.00	2.00
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
I	1845	0	1845	0	0	3-8	3-8
O	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
O	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

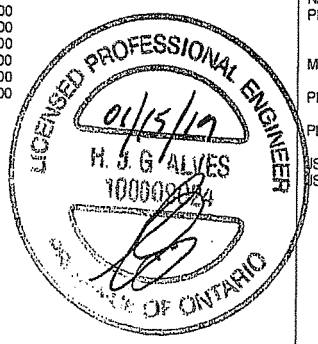
1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00		N- C	-49 / 183	0.04 (3)
B- C	-2660 / 0	-84.9	-84.9	0.59 (1)	3.72		C- M	-560 / 0	0.62 (1)
C- D	-2159 / 0	-84.9	-84.9	0.45 (1)	4.22		M- E	0 / 583	0.13 (2)
D- E	-2159 / 0	-84.9	-84.9	0.45 (1)	4.22		E- K	-1049 / 0	0.69 (1)
E- F	-1405 / 0	-84.9	-84.9	0.43 (1)	5.00		K- F	0 / 1009	0.16 (1)
F- G	-836 / 0	-84.9	-84.9	0.33 (1)	6.22		F- J	-883 / 0	0.67 (1)
G- H	-837 / 0	-84.9	-84.9	0.33 (1)	6.22		J- G	-576 / 0	0.33 (1)
I- H	-1759 / 0	0.0	0.0	1.00 (1)	5.03		J- H	0 / 1791	0.29 (1)
O- B	-1857 / 0	0.0	0.0	0.19 (1)	6.15		B- N	0 / 2421	0.54 (1)
O- N	0 / 0	-38.5	-38.5	0.28 (3)	10.00				
N- M	0 / 2400	-38.5	-38.5	0.60 (2)	10.00				
M- L	0 / 1931	-38.5	-38.5	0.49 (2)	10.00				
L- K	0 / 1931	-38.5	-38.5	0.49 (2)	10.00				
K- J	0 / 1249	-38.5	-38.5	0.37 (2)	10.00				
J- I	0 / 0	-38.5	-38.5	0.20 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=1.00/1.00 (H-I:1) , BC=0.60/1.00 (M-N:2) , WB=0.69/1.00 (E-K:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (B) (INPUT = 0.90)
USI METAL= 0.77 (L) (INPUT = 1.00)

DWG NO. TAM T1901200
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T323A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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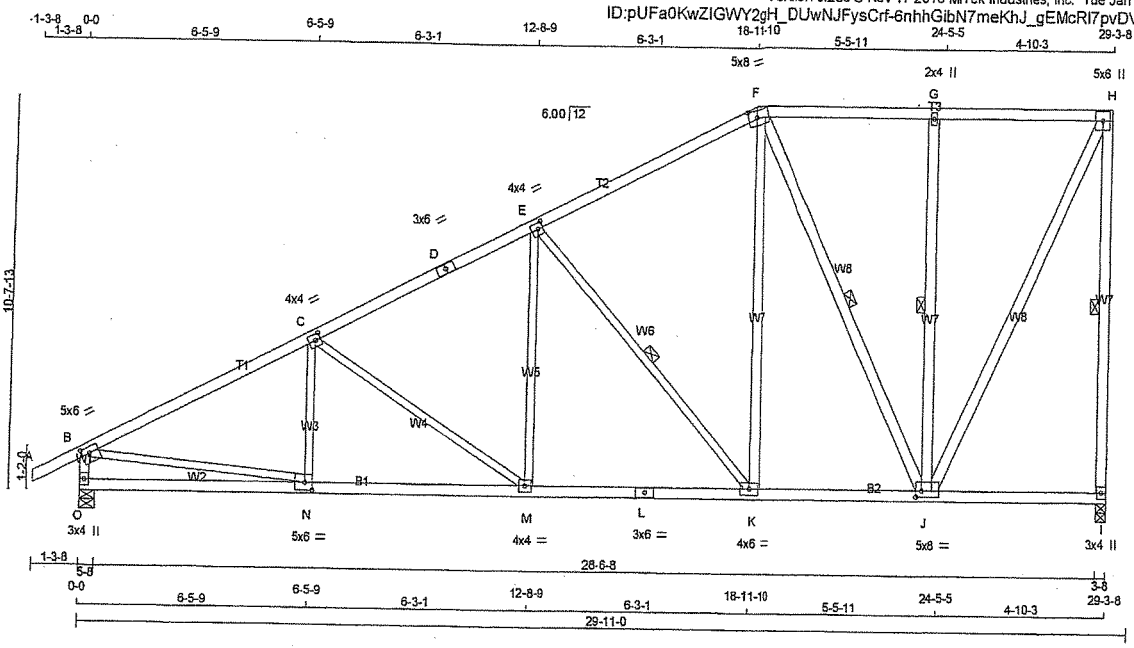


TOTAL WEIGHT = 159 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTWW-m	MT20	5.0	8.0	2.25	3.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.00	2.00
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1807	0	1807	0	0	3-8	3-8
O	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
O	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, E-K, F-J, G-J

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

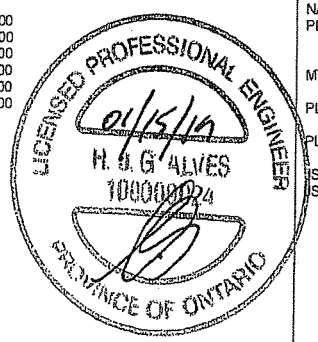
TOTAL LOAD CASES: (4)

CHORDS

MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	1	0.11 (1)	10.00
B- C	-2592 / 0	-84.9	-84.9	1	0.58 (1)	3.77
C- D	-2083 / 0	-84.9	-84.9	1	0.45 (1)	4.29
D- E	-2083 / 0	-84.9	-84.9	1	0.45 (1)	4.29
E- F	-1325 / 0	-84.9	-84.9	1	0.42 (1)	5.13
F- G	-776 / 0	-84.9	-84.9	1	0.29 (1)	8.25
G- H	-777 / 0	-84.9	-84.9	1	0.29 (1)	6.25
I- H	-1725 / 0	0.0	0.0	1	0.99 (1)	5.07
O- B	-1819 / 0	0.0	0.0	1	0.18 (1)	6.20
O- N	0 / 0	-38.5	-38.5	3	0.28 (3)	10.00
N- M	0 / 2339	-38.5	-38.5	2	0.59 (2)	10.00
M- L	0 / 1863	-38.5	-38.5	2	0.48 (2)	10.00
L- K	0 / 1863	-38.5	-38.5	2	0.48 (2)	10.00
K- J	0 / 1178	-38.5	-38.5	2	0.35 (2)	10.00
J- I	0 / 0	-38.5	-38.5	3	0.18 (3)	10.00

WEBS

MEMB FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-41 / 187	0.04 (3)
C- M	-568 / 0	0.63 (1)
M- E	0 / 588	0.13 (2)
E- K	-1054 / 0	0.69 (1)
K- F	0 / 1008	0.16 (1)
F- J	-900 / 0	0.66 (1)
J- G	-542 / 0	0.31 (1)
J- H	0 / 1741	0.28 (1)
B- N	0 / 2359	0.53 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.99/1.00 (H-I:1) , BC=0.59/1.00 (M-N:2) , WB=0.69/1.00 (E-K:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (B) (INPUT = 0.90)
SI METAL= 0.75 (L) (INPUT = 1.00)

DWG NO. TAM T1901201

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	P2	2	1	TRUSS DESC.	

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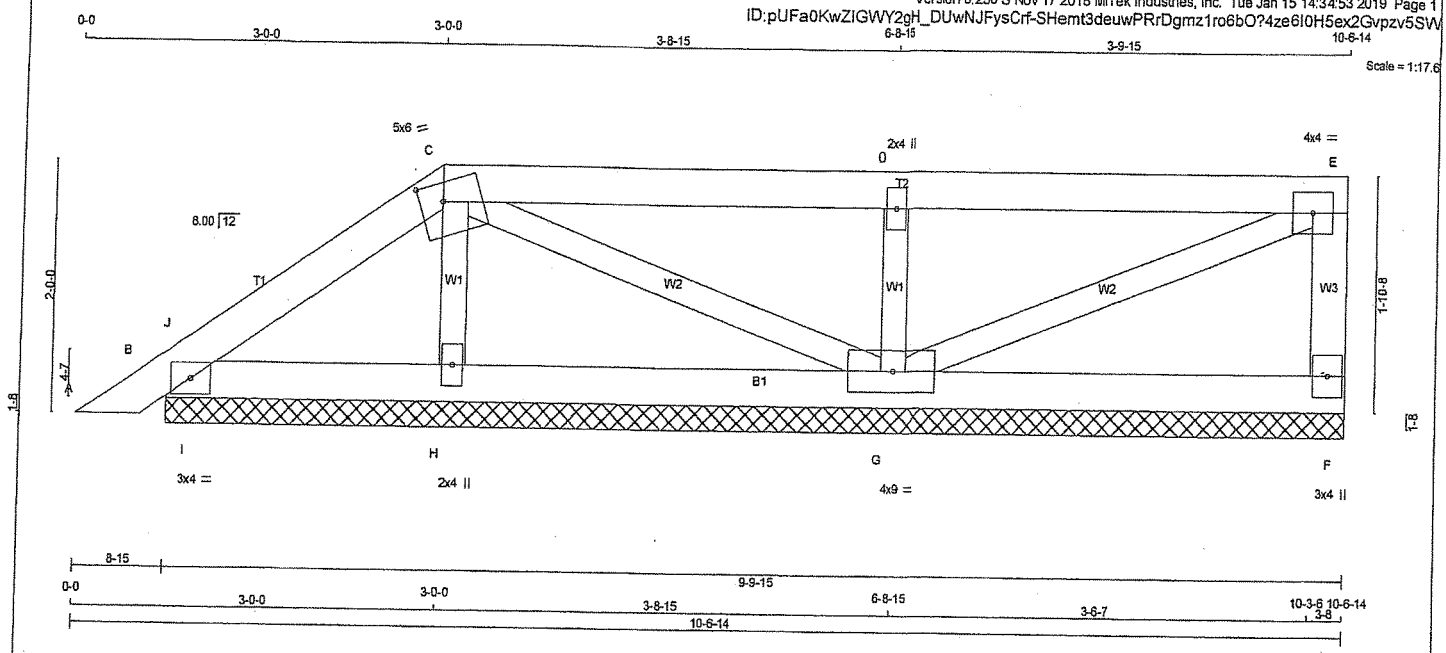


TOTAL WEIGHT = 2 X 33 = 67 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	1.75	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	181	0	181	0	0	9-9-15	9-9-15
B	209	0	209	0	0	9-9-15	9-9-15
H	300	0	300	0	0	9-9-15	9-9-15
G	579	0	579	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	137	67 / 0	32 / 0	0 / 0	0 / 0	39 / 0	0 / 0
B	152	96 / 0	19 / 0	0 / 0	0 / 0	37 / 0	0 / 0
H	233	100 / 0	64 / 0	0 / 0	0 / 0	69 / 0	0 / 0
G	436	225 / 0	92 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

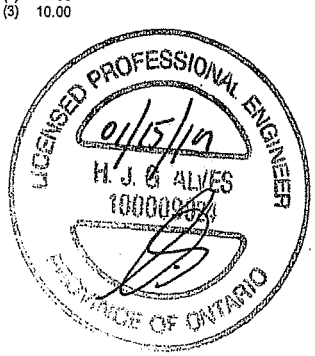
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	H- C	-161 / 0		0.02 (1)
B- J	-42 / 0	-84.9	-84.9	0.01 (3)	6.25	G- E	-2 / 0		0.00 (1)
J- C	-48 / 0	-84.9	-84.9	0.05 (1)	6.25	C- G	-32 / 0		0.01 (1)
C- D	0 / 2	-84.9	-84.9	0.21 (1)	10.00	G- D	-401 / 0		0.06 (1)
D- E	0 / 2	-84.9	-84.9	0.21 (1)	10.00	I- J	-105 / 6		0.00 (1)
F- E	-122 / 0	0.0	0.0	0.01 (1)	7.81				
B- I	0 / 39	-38.5	-38.5	0.05 (1)	10.00				
I- H	0 / 39	-38.5	-38.5	0.06 (2)	10.00				
H- G	0 / 28	-38.5	-38.5	0.11 (3)	10.00				
G- F	0 / 0	-38.5	-38.5	0.11 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-D:1) , BC=0.11/1.00 (G-H:3) , WB=0.06/1.00 (D-G:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

DWG No. T901212
STRUCTURAL
CONSIDERATION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	P3	2	1	TRUSS DESC.	

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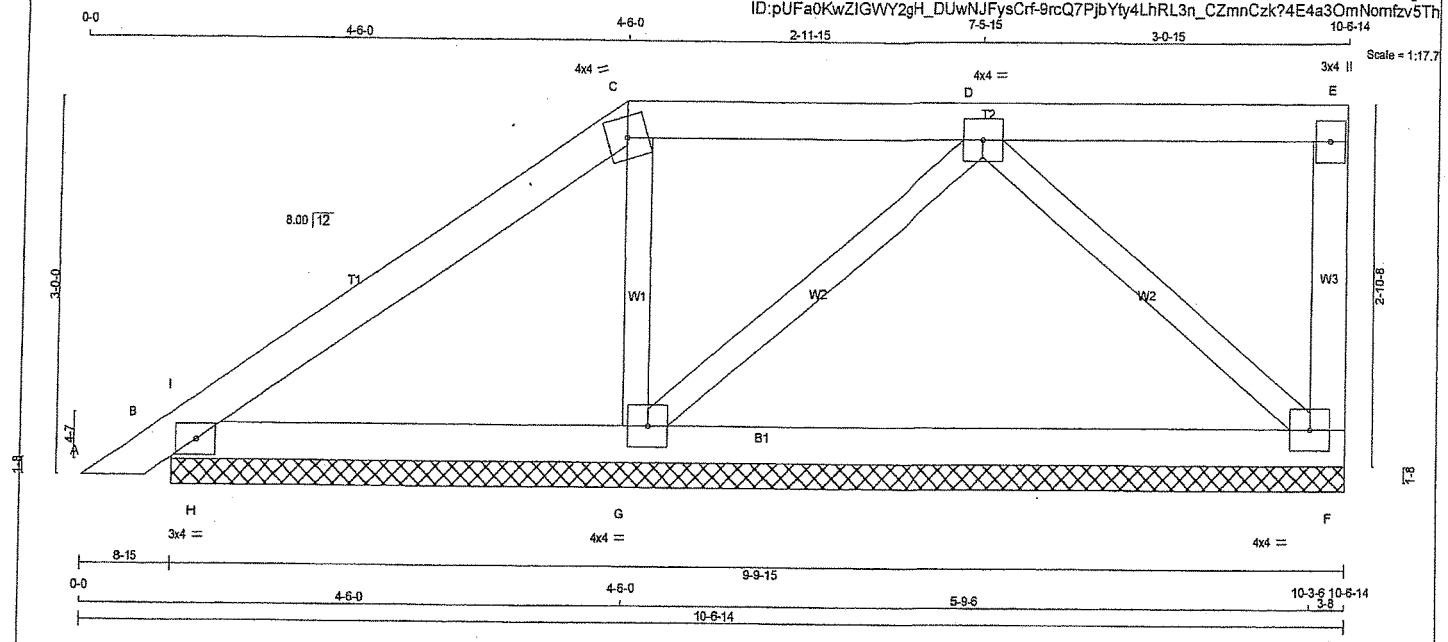

TOTAL WEIGHT = 2 X 35 = 69 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMWW-t	MT20	4.0	4.0	
E	TMV+p	MT20	3.0	4.0	
F	BMVW1-t	MT20	4.0	4.0	
G	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	355	0	355	0	0	9-9-15	9-9-15
B	274	0	274	0	0	9-9-15	9-9-15
G	639	0	639	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	266	144 / 0	51 / 0	0 / 0	0 / 0	71 / 0	0 / 0
B	198	128 / 0	22 / 0	0 / 0	0 / 0	48 / 0	0 / 0
G	494	217 / 0	133 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)		10.00
B- I	0 / 49	-84.9	-84.9	0.07 (1)		10.00
I- C	-47 / 0	-84.9	-84.9	0.14 (1)		6.25
C- D	-25 / 0	-84.9	-84.9	0.13 (1)		6.25
D- E	0 / 0	-84.9	-84.9	0.13 (1)		10.00
F- E	-100 / 0	0.0	0.0	0.01 (1)		7.81
B- H	0 / 33	-38.5	-38.5	0.14 (1)		10.00
H- G	0 / 33	-38.5	-38.5	0.25 (2)		10.00
G- F	0 / 197	-38.5	-38.5	0.27 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-208 / 0	0.04 (1)
G- D	-230 / 0	0.06 (1)
D- F	-257 / 0	0.07 (1)
H- I	-312 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED RDDF LIVE LOAD

CSI: TC=0.14/1.00 (C-I:1) , BC=0.27/1.00 (F-G:2) , WB=0.07/1.00 (D-F:1) , SSI=0.25/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (F) (INPUT = 0.90)
JSI METAL= 0.07 (F) (INPUT = 1.00)

DWG NO. T19012I3

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	P4	3	1	TRUSS DESC	

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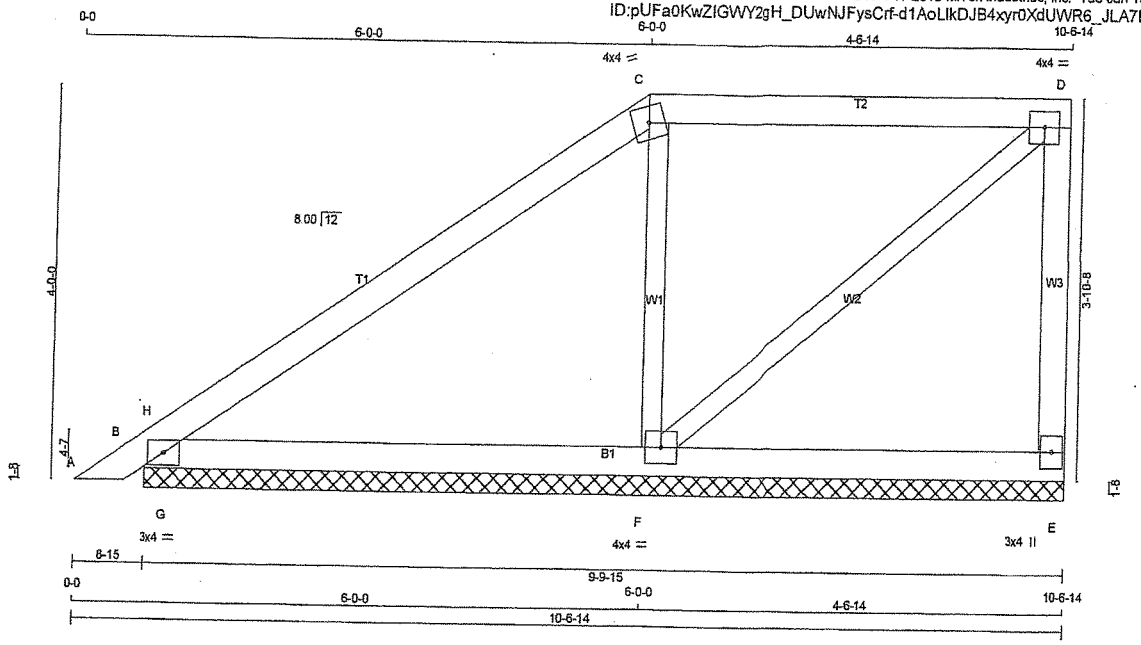


Scale = 1:22.6

TOTAL WEIGHT = 3 X 35 = 106 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	307	0	307	0	0	9-9-15	9-9-15
B	413	0	413	0	0	9-9-15	9-9-15
F	548	0	548	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	228	128 / 0	41 / 0	0 / 0	0 / 0	60 / 0	0 / 0
B	306	176 / 0	51 / 0	0 / 0	0 / 0	79 / 0	0 / 0
F	424	185 / 0	115 / 0	0 / 0	0 / 0	124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

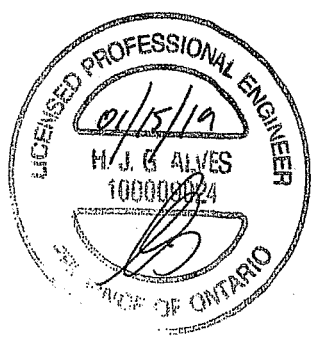
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	F- C	-330 / 0	0.06 (1)
B- H	-106 / 45	-84.9	-84.9	0.11 (1)	6.25	F- D	0 / 101	0.02 (1)
H- C	-115 / 0	-84.9	-84.9	0.31 (1)	6.25	G- H	-473 / 72	0.00 (1)
C- D	-79 / 0	-84.9	-84.9	0.30 (1)	6.25			
E- D	-257 / 0	0.0	0.0	0.06 (1)	7.81			
B- G	0 / 88	-38.5	-38.5	0.23 (1)	10.00			
G- F	0 / 88	-38.5	-38.5	0.25 (1)	10.00			
F- E	0 / 0	-38.5	-38.5	0.24 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-H:1) , BC=0.25/1.00 (F-G:1) , WB=0.08/1.00 (C-F:1) , SSI=0.41/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

DWG No. 190121 4
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	P5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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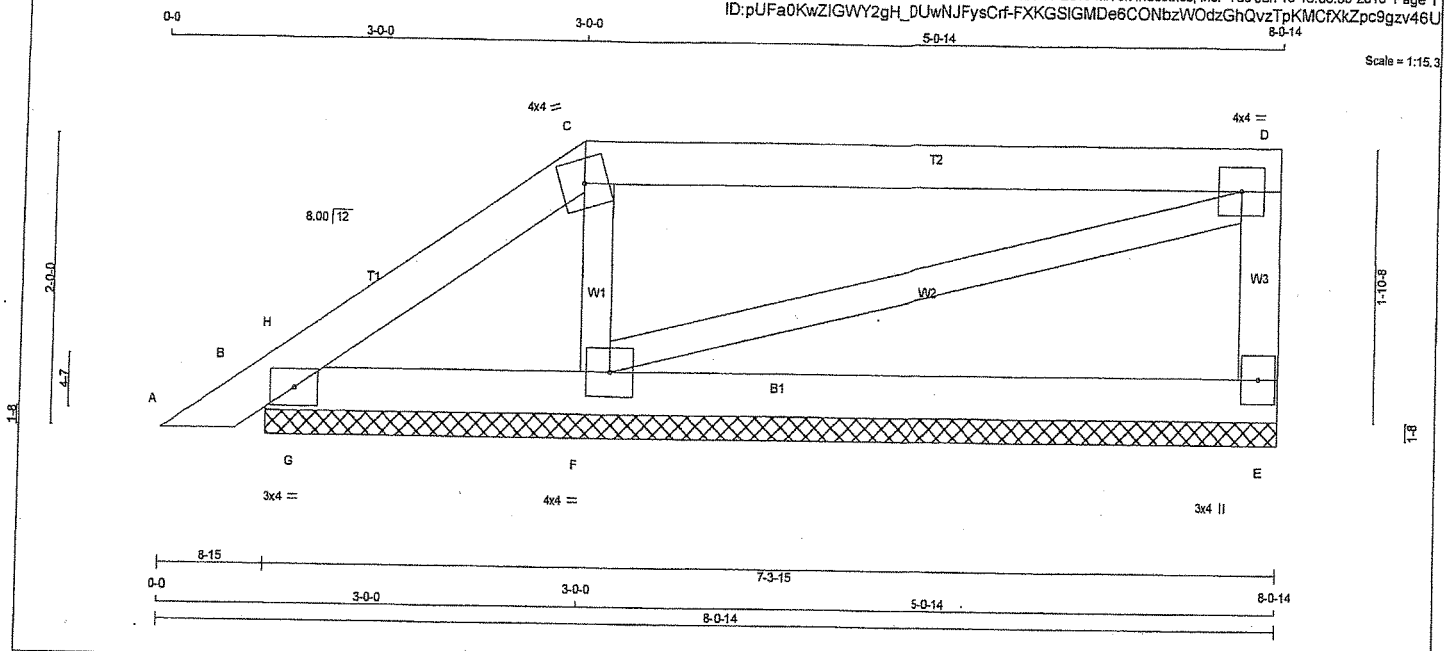

TOTAL WEIGHT = 2 X 25 = 49 lb

LUMBER

N. L. G. A. RULES

CHOROS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	295	0	295	0	0	7-3-15	7-3-15
B	179	0	179	0	0	7-3-15	7-3-15
F	486	0	486	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	221	120 / 0	42 / 0	0 / 0	0 / 0	59 / 0	0 / 0
B	125	95 / 0	3 / 0	0 / 0	0 / 0	26 / 0	0 / 0
F	379	157 / 0	109 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- H	0 / 26	-84.9	-84.9	0.03 (2)	10.00
H- C	-43 / 0	-84.9	-84.9	0.04 (1)	6.25
C- D	-13 / 0	-84.9	-84.9	0.37 (1)	6.25
E- D	-219 / 0	0.0	0.0	0.02 (1)	7.81
B- G	0 / 31	-38.5	-38.5	0.06 (1)	10.00
G- F	0 / 31	-38.5	-38.5	0.17 (3)	10.00
F- E	0 / 0	-38.5	-38.5	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-268 / 0	0.04 (1)
F- D	0 / 14	0.00 (1)
G- H	-164 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37/1.00 (C-D:1) , BC=0.17/1.00 (F-G:3) , WB=0.04/1.00 (C-F:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.04 (F) (INPUT = 1.00)

DWG NO. TAM 7190 1215
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	P6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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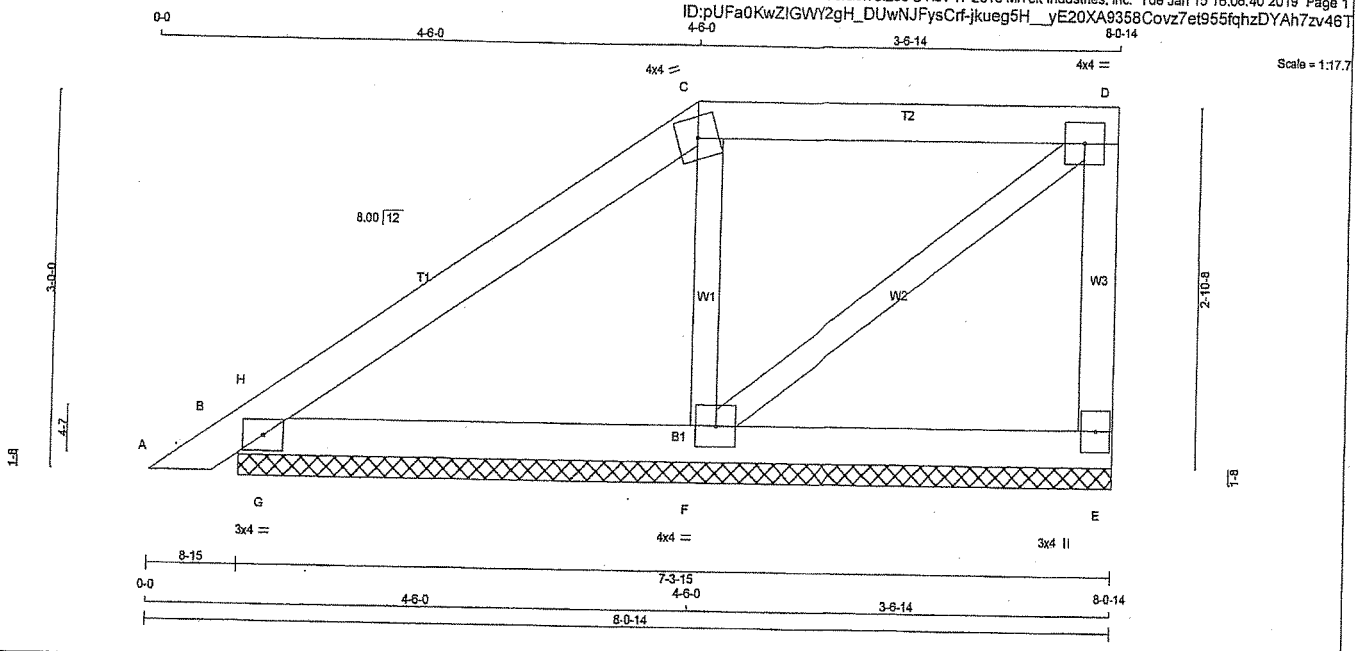

Scale = 1:17.7

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	225	0	225	0	0	7-3-15	7-3-15
B	304	0	304	0	0	7-3-15	7-3-15
F	431	0	431	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	167	93 / 0	30 / 0	0 / 0	0 / 0	44 / 0	0 / 0
B	224	132 / 0	35 / 0	0 / 0	0 / 0	57 / 0	0 / 0
F	333	147 / 0	89 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- H	-59 / 0	-84.9	-84.9	0.04 (1)	6.25
H- C	-67 / 0	-84.9	-84.9	0.15 (1)	6.25
C- D	-42 / 0	-84.9	-84.9	0.18 (1)	6.25
E- D	-182 / 0	0.0	0.0	0.03 (1)	7.81
B- G	0 / 52	-38.5	-38.5	0.12 (1)	10.00
G- F	0 / 52	-38.5	-38.5	0.14 (2)	10.00
F- E	0 / 0	-38.5	-38.5	0.13 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-255 / 0	0.04 (1)
F- D	0 / 52	0.01 (1)
G- H	-253 / 36	0.00 (1)

TOTAL WEIGHT = 2 X 26 = 52 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-D:1) , BC=0.14/1.00 (F-G:2) , WB=0.04/1.00 (C-F:1) , SSI=0.22/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1901216
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	P7	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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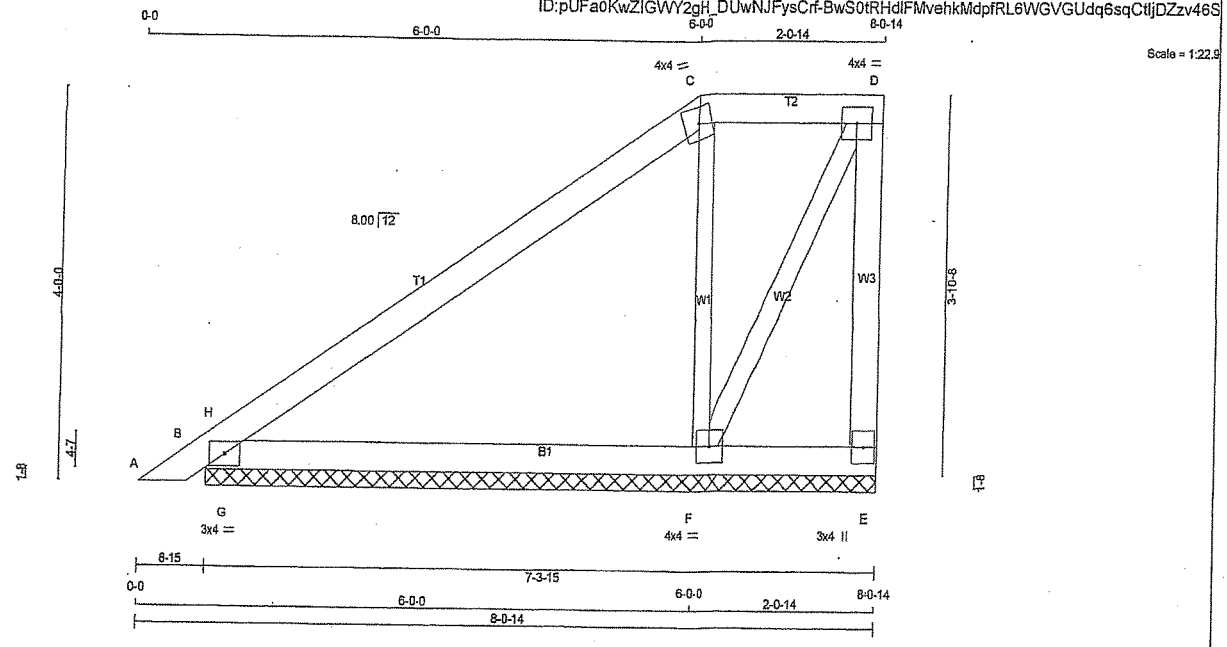


TOTAL WEIGHT = 4 X 28 = 113 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-t	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	129	0	129	0	0	7-3-15	7-3-15
B	390	0	390	0	0	7-3-15	7-3-15
F	442	0	442	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	92	63 / 0	7 / 0	0 / 0	0 / 0	21 / 0	0 / 0
B	290	162 / 0	51 / 0	0 / 0	0 / 0	76 / 0	0 / 0
F	343	147 / 0	95 / 0	0 / 0	0 / 0	101 / 0	0 / 0

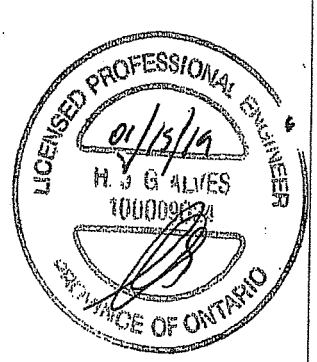
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	F- C	-247 / 0	0.06 (1)
B- H	-103 / 88	-84.9	-84.9	0.12 (1)	6.25	F- D	0 / 96	0.02 (1)
H- C	-73 / 0	-84.9	-84.9	0.31 (1)	6.25	G- H	-476 / 88	0.00 (1)
C- D	-46 / 0	-84.9	-84.9	0.06 (1)	6.25			
E- D	-172 / 0	0.0	0.0	0.04 (1)	7.81			
B- G	0 / 54	-38.5	-38.5	0.22 (1)	10.00			
G- F	0 / 54	-38.5	-38.5	0.24 (1)	10.00			
F- E	0 / 0	-38.5	-38.5	0.22 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-H:1) , BC=0.24/1.00 (F-G:1) , WB=0.06/1.00 (C-F:1) , SSI=0.41/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL.= 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

DWG NO. TAM 1901224
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	P8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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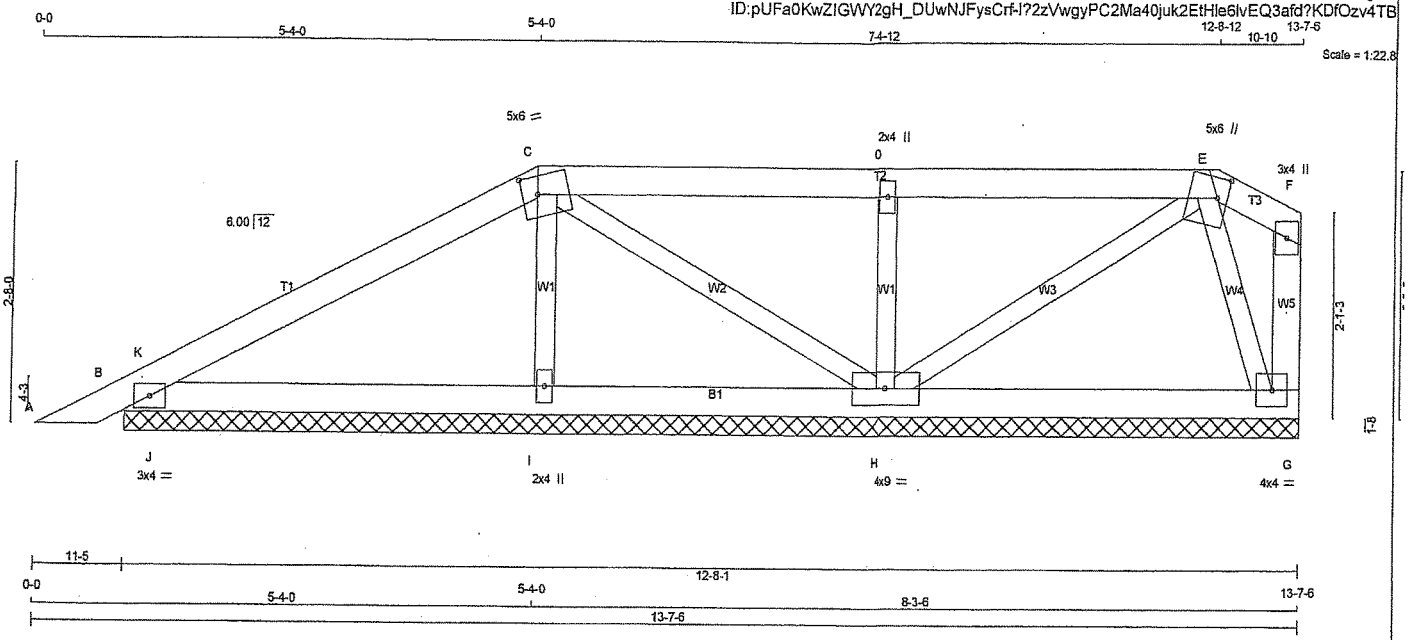


TOTAL WEIGHT = 44 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.25
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW1-t	MT20	4.0	9.0		
I	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	239	0	239	0	0	12-8-1	12-8-1
B	370	0	370	0	0	12-8-1	12-8-1
I	416	0	416	0	0	12-8-1	12-8-1
H	613	0	813	0	0	12-8-1	12-8-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	181	91 / 0	40 / 0	0 / 0	0 / 0	50 / 0	0 / 0
B	273	159 / 0	44 / 0	0 / 0	0 / 0	70 / 0	0 / 0
I	321	143 / 0	85 / 0	0 / 0	0 / 0	93 / 0	0 / 0
H	462	238 / 0	97 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)		10.00
B- K	-84 / 0	-84.9	-84.9	0.05 (3)		6.25
K- C	-98 / 0	-84.9	-84.9	0.21 (1)		6.25
C- D	-13 / 0	-84.9	-84.9	0.19 (1)		6.25
D- E	-13 / 0	-84.9	-84.9	0.19 (1)		6.25
E- F	0 / 0	-84.9	-84.9	0.01 (1)		10.00
G- F	-40 / 0	0.0	0.0	0.00 (1)		7.81
B- J	0 / 85	-38.5	-38.5	0.17 (1)		10.00
J- I	0 / 85	-38.5	-38.5	0.18 (1)		10.00
I- H	0 / 76	-38.5	-38.5	0.15 (2)		10.00
H- G	0 / 53	-38.5	-38.5	0.13 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
I- C	-207 / 0	0.03 (1)
C- H	-72 / 0	0.02 (1)
H- D	-389 / 0	0.06 (1)
E- G	-136 / 0	0.02 (1)
H- E	-46 / 0	0.01 (1)
J- K	-240 / 63	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-K:1) , BC=0.18/1.00 (I-J:1) , WB=0.06/1.00 (D-H:1) , SSI=0.22/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

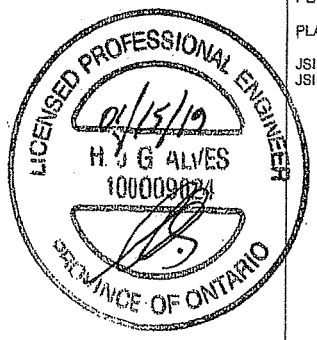
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)



AG A.O. TAM 1790/12/17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO
300333	P8A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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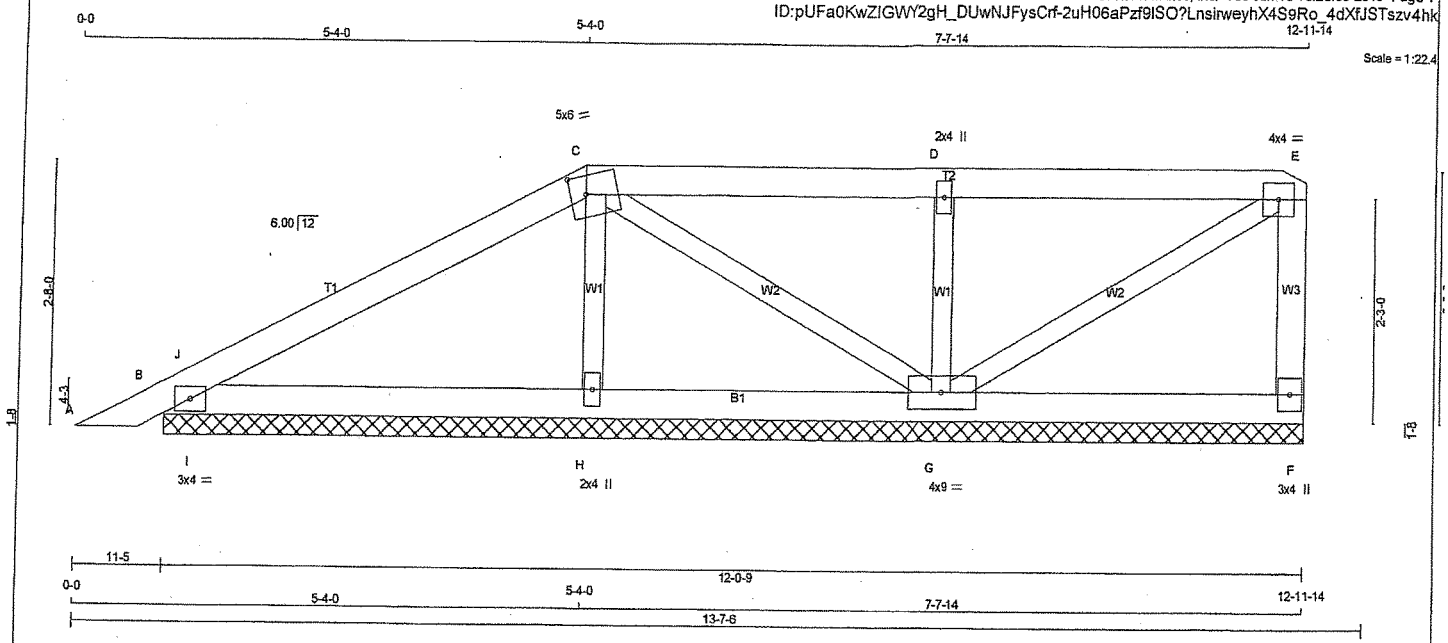

TOTAL WEIGHT = 41 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	189	0	189	0	0	12-0-9	12-0-9
B	367	0	367	0	0	12-0-9	12-0-9
H	419	0	419	0	0	12-0-9	12-0-9
G	586	0	586	0	0	12-0-9	12-0-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	144	69 / 0	34 / 0	0 / 0	0 / 0	41 / 0	0 / 0
B	270	158 / 0	43 / 0	0 / 0	0 / 0	69 / 0	0 / 0
H	325	141 / 0	88 / 0	0 / 0	0 / 0	95 / 0	0 / 0
G	440	234 / 0	87 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	H- C	-203 / 0	0.03 (1)
B- J	-77 / 0	-84.9	-84.9	0.05 (3)	6.25	C- G	-79 / 0	0.02 (1)
J- C	-92 / 0	-84.9	-84.9	0.21 (1)	6.25	G- O	-405 / 0	0.06 (1)
C- D	-5 / 0	-84.9	-84.9	0.21 (1)	10.00	G- E	0 / 6	0.00 (3)
D- E	-5 / 0	-84.9	-84.9	0.21 (1)	10.00	I- J	-243 / 60	0.00 (1)
F- E	-126 / 0	0.0	0.0	0.02 (1)	7.81			
B- I	0 / 80	-38.5	-38.5	0.17 (1)	10.00			
I- H	0 / 80	-38.5	-38.5	0.18 (1)	10.00			
H- G	0 / 71	-38.5	-38.5	0.15 (2)	10.00			
G- F	0 / 0	-38.5	-38.5	0.10 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-J:1) , BC=0.18/1.00 (H-I:1) , WB=0.06/1.00 (D-G:1) , SSI=0.22/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100089029
PROVINCE OF ONTARIO

TAM 01/15/19
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	P9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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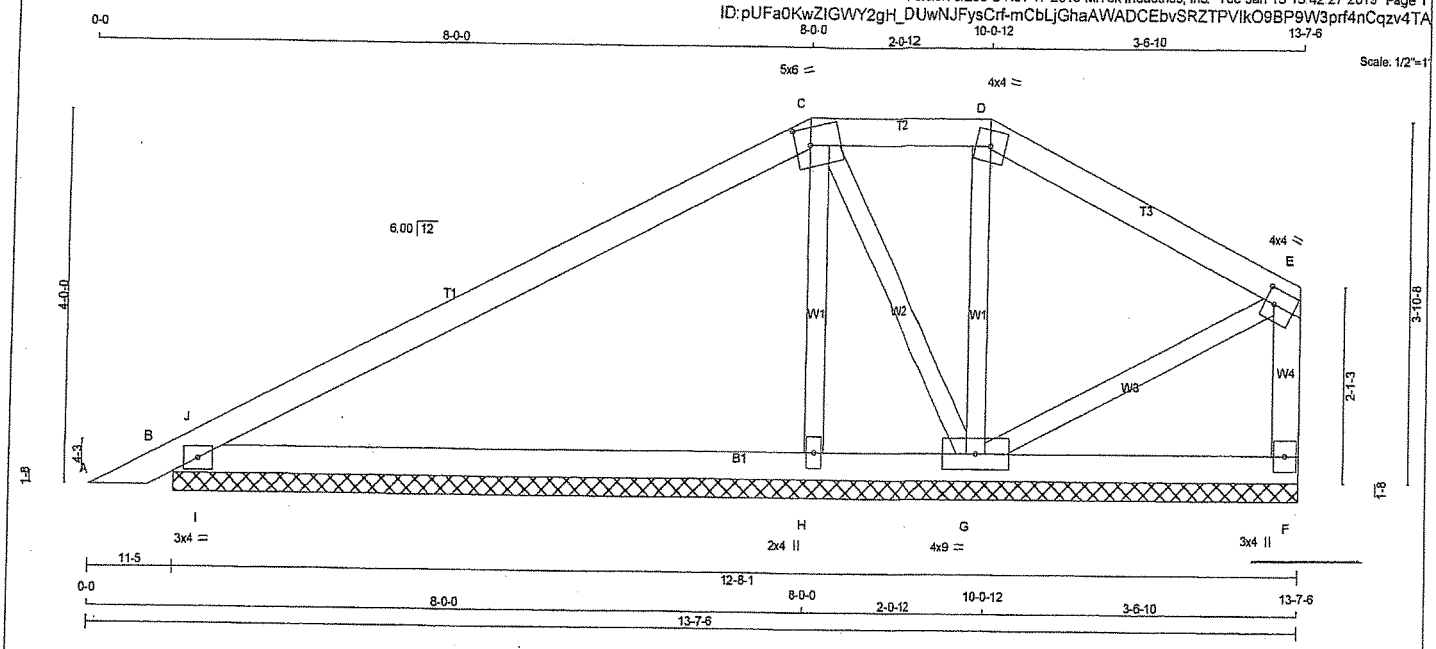


TOTAL WEIGHT = 45 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	309	0	309	0	0	12-8-1	12-8-1
B	556	0	556	0	0	12-8-1	12-8-1
H	525	0	525	0	0	12-8-1	12-8-1
G	247	0	247	0	0	12-8-1	12-8-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	232	121 / 0	48 / 0	0 / 0	0 / 0	63 / 0	0 / 0
B	413	233 / 0	72 / 0	0 / 0	0 / 0	108 / 0	0 / 0
H	415	156 / 0	132 / 0	0 / 0	0 / 0	128 / 0	0 / 0
G	177	121 / 0	15 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

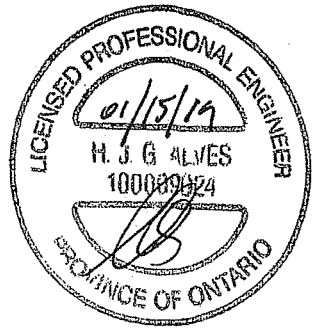
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	
B-J	-159 / 0	-84.9	-84.9	0.18 (1)	6.25	
J-C	-199 / 0	-84.9	-84.9	0.56 (1)	6.25	
C-D	-82 / 0	-84.9	-84.9	0.06 (1)	6.25	
D-E	-98 / 0	-84.9	-84.9	0.18 (1)	6.25	
F-E	-238 / 0	0.0	0.0	0.03 (1)	7.81	
B-I	0 / 170	-38.5	-38.5	0.40 (1)	10.00	
I-H	0 / 170	-38.5	-38.5	0.44 (2)	10.00	
H-G	0 / 166	-38.5	-38.5	0.44 (2)	10.00	
G-F	0 / 0	-38.5	-38.5	0.10 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-111 / 1	0.03 (1)
C-G	-175 / 0	0.05 (1)
G-D	-194 / 0	0.05 (1)
G-E	0 / 98	0.02 (1)
I-J	-626 / 127	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.58/1.00 (C-J:1) , BC=0.44/1.00 (H-I:2) , WB=0.05/1.00 (C-G:1) , SSI=0.54/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

C.TAM 01/15/2019
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	P9A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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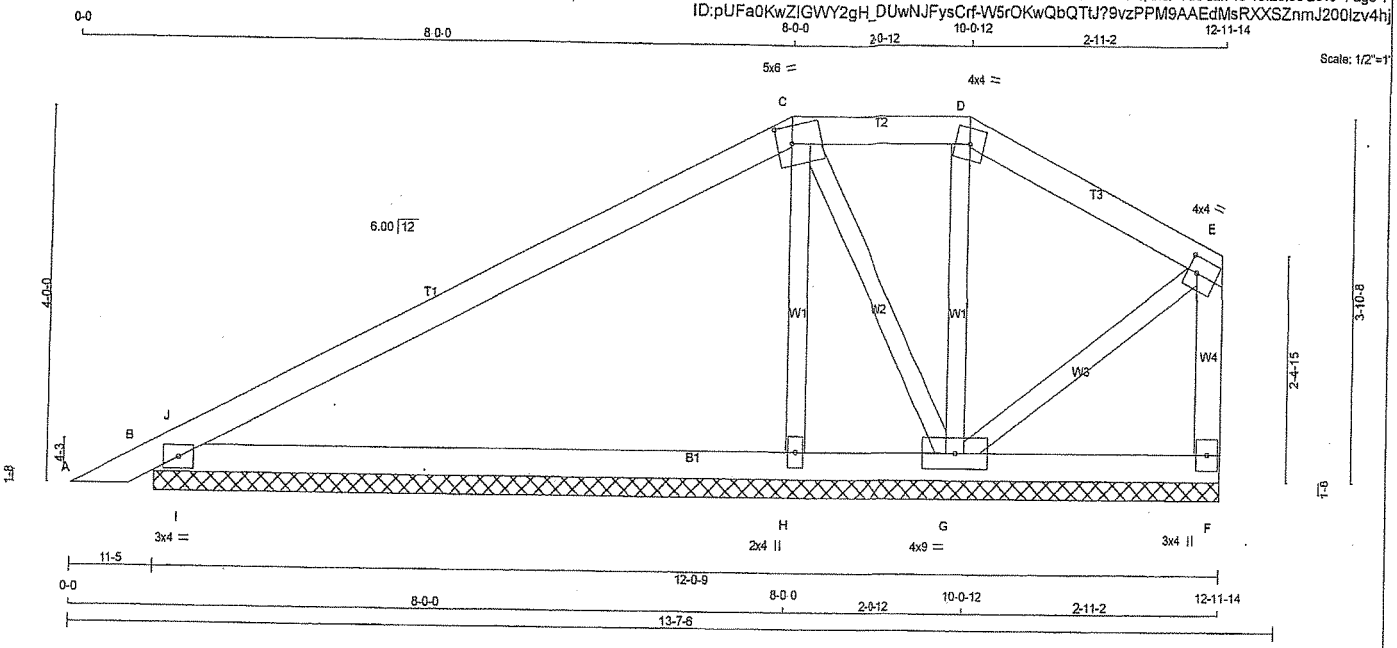

TOTAL WEIGHT = 43 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
F	287	0	287	0	0	12-0-9	12-0-9
B	554	0	554	0	0	12-0-9	12-0-9
H	537	0	537	0	0	12-0-9	12-0-9
G	183	0	183	0	0	12-0-9	12-0-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	217	111 / 0	46 / 0	0 / 0	0 / 0	59 / 0	0 / 0
B	411	232 / 0	71 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	426	156 / 0	138 / 0	0 / 0	0 / 0	132 / 0	0 / 0
G	125	103 / 0	0 / -2	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05	(1)		10.00
B- J	-157 / 0	-84.9	-84.9	0.18	(1)		6.25
J- C	-194 / 0	-84.9	-84.9	0.56	(1)		6.25
C- D	-76 / 0	-84.9	-84.9	0.06	(1)		6.25
D- E	-90 / 0	-84.9	-84.9	0.12	(1)		6.25
F- E	-221 / 0	0.0	0.0	0.03	(1)		7.81
B- I	0 / 165	-38.5	-38.5	0.40	(1)		10.00
I- H	0 / 165	-38.5	-38.5	0.44	(2)		10.00
H- G	0 / 162	-38.5	-38.5	0.44	(2)		10.00
G- F	0 / 0	-38.5	-38.5	0.09	(2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
H- C	-109 / 2	0.03	(1)
C- G	-180 / 0	0.05	(1)
G- D	-171 / 0	0.04	(1)
G- E	0 / 98	0.02	(1)
I- J	-628 / 125	0.00	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56/1.00 (C-J:1) , BC=0.44/1.00 (H-I:2) , WB=0.05/1.00 (C-G:1) , SSI=0.54/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

DWG NO. TAM T190 1220
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	P10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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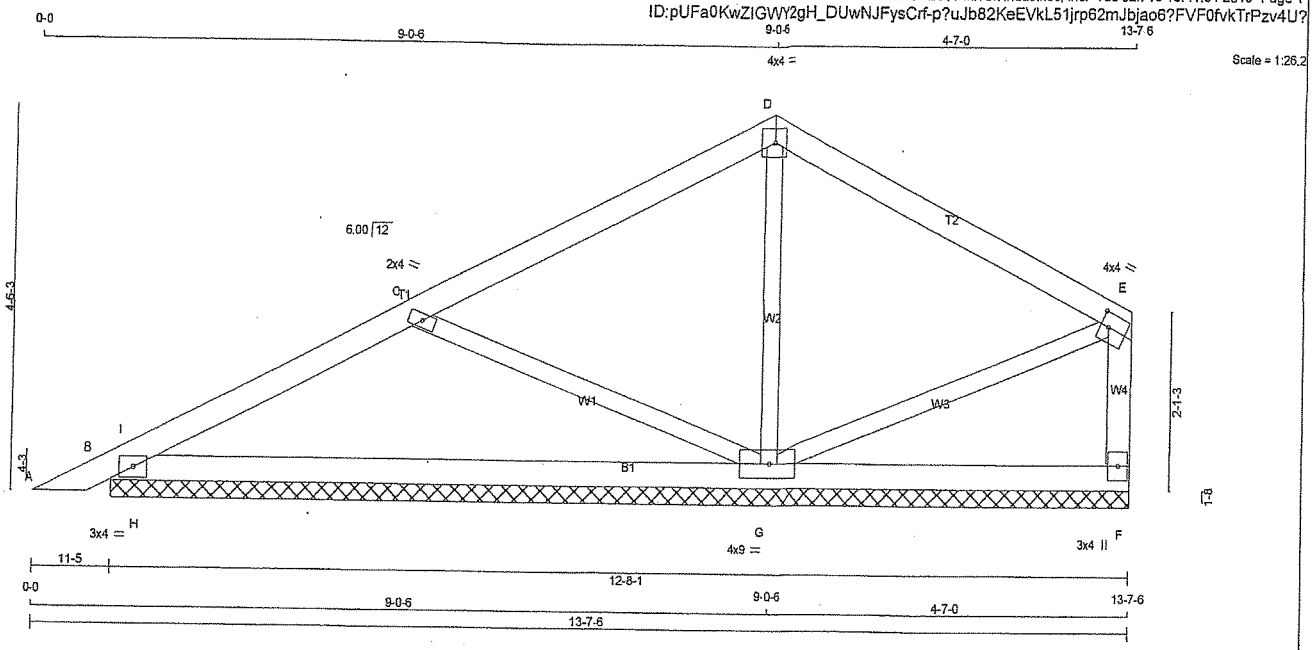

TOTAL WEIGHT = 44 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	154	0	154	0	0	12-8-1	12-8-1
B	500	0	500	0	0	12-8-1	12-8-1
G	983	0	983	0	0	12-8-1	12-8-1

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	116	59 / 0	25 / 0	0 / 0	0 / 0	32 / 0	0 / 0
B	374	202 / 0	72 / 0	0 / 0	0 / 0	100 / 0	0 / 0
G	746	370 / 0	169 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.9	-84.9		0.05 (1)	10.00	C- G	-556 / 0	0.22 (1)
B- I	-671 / 0	-84.9	-84.9		0.28 (3)	6.25	G- D	-432 / 0	0.12 (1)
I- C	-413 / 0	-84.9	-84.9		0.28 (3)	6.25	G- E	-105 / 0	0.04 (1)
C- D	0 / 89	-84.9	-84.9		0.28 (1)	10.00	H- I	0 / 542	0.00 (1)
D- E	0 / 109	-84.9	-84.9		0.32 (1)	10.00			
F- E	-107 / 0	0.0	0.0		0.01 (1)	7.81			
B- H	0 / 411	-38.5	-38.5		0.23 (3)	10.00			
H- G	0 / 411	-38.5	-38.5		0.38 (2)	10.00			
G- F	0 / 0	-38.5	-38.5		0.33 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (D-E:1) , BC=0.38/1.00 (G-H:2) , WB=0.22/1.00 (C-G:1) , SSI=0.34/1.00 (8-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (C) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009[illegible]
PROVINCE OF ONTARIO

PRELIM. TAM T1901221
STRUCTURAL
COMMENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	P10A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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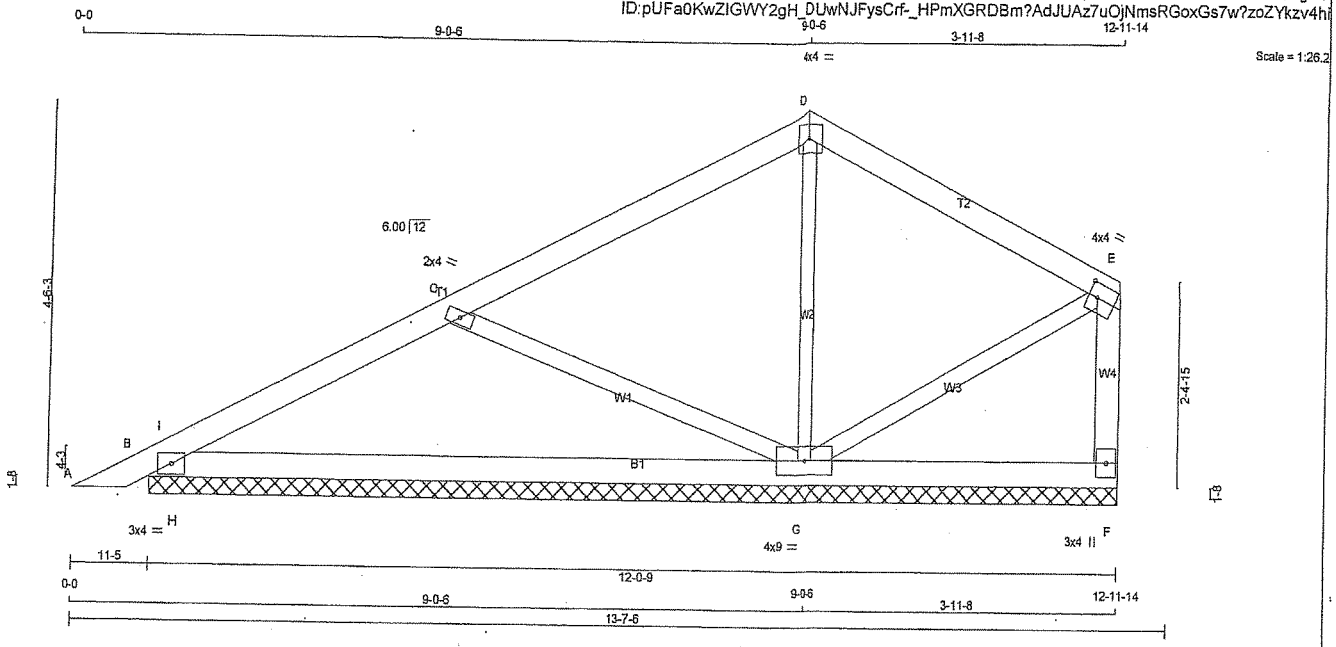


TOTAL WEIGHT = 43 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQR'D BRG IN-SX
F	91	0	91	0	0	12-0-9	12-0-9
B	498	0	498	0	0	12-0-9	12-0-9
G	971	0	971	0	0	12-0-9	12-0-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	69	34 / 0	16 / 0	0 / 0	0 / 0	19 / 0	0 / 0
B	373	201 / 0	72 / 0	0 / 0	0 / 0	100 / 0	0 / 0
G	736	367 / 0	165 / 0	0 / 0	0 / 0	205 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	
B- I	-668 / 0	-84.9	-84.9	0.28 (3)	6.25	
I- C	-408 / 0	-84.9	-84.9	0.28 (3)	6.25	
C- D	0 / 95	-84.9	-84.9	0.29 (1)	10.00	
D- E	0 / 115	-84.9	-84.9	0.24 (1)	10.00	
F- E	-62 / 0	0.0	0.0	0.01 (1)	7.81	
B- H	0 / 407	-38.5	-38.5	0.23 (3)	10.00	
H- G	0 / 407	-38.5	-38.5	0.37 (2)	10.00	
G- F	0 / 0	-38.5	-38.5	0.32 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	-557 / 0	0.22 (1)
G- D	-410 / 0	0.12 (1)
G- E	-116 / 0	0.04 (1)
H- I	0 / 548	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1) , BC=0.37/1.00 (G-H:2) , WB=0.22/1.00 (C-G:1) , SSI=0.34/1.00 (B-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

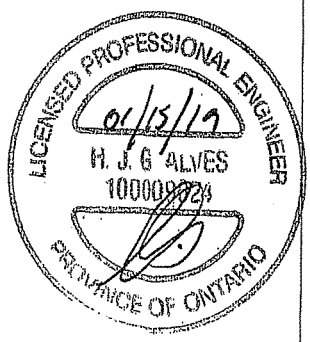
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (C) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)



...S TAM 1/90/222
STRUCTURAL
...MENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	P15G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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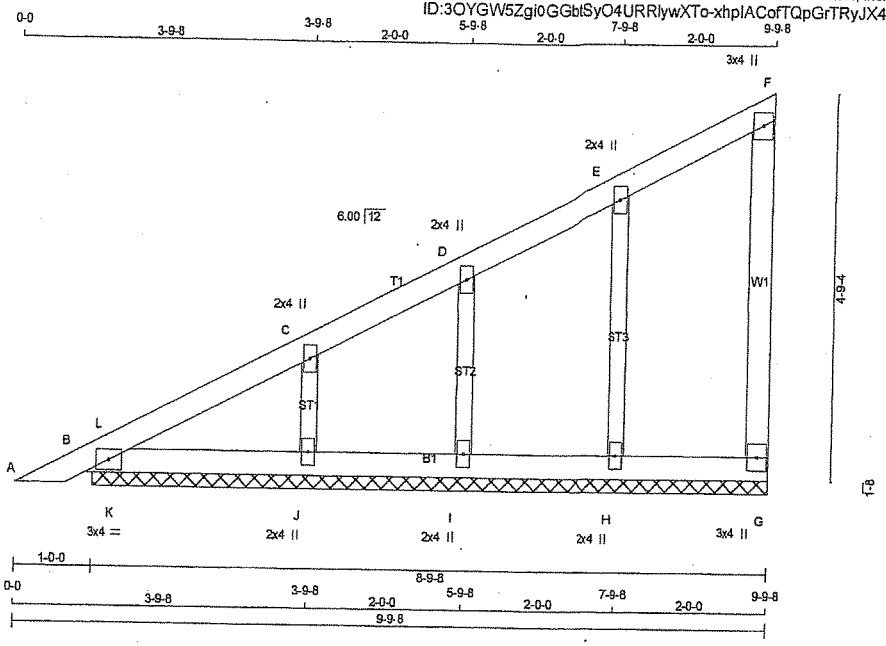

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C, D, E					
C	TMW+w	MT20	2.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	BMV1+p	MT20	3.0	4.0	
H, I, J					
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	H- E	-196 / 0	0.04 (1)
B- L	-36 / 0	-84.9	-84.9	0.03 (3)	6.25	I- D	-144 / 0	0.02 (1)
L- C	-8 / 2	-84.9	-84.9	0.07 (1)	10.00	J- C	-224 / 0	0.03 (1)
C- D	-16 / 0	-84.9	-84.9	0.07 (1)	6.25	K- L	-62 / 39	0.00 (1)
D- E	0 / 0	-84.9	-84.9	0.05 (1)	10.00			
E- F	-8 / 0	-84.9	-84.9	0.05 (1)	10.00			
G- F	-70 / 0	0.0	0.0	0.03 (1)	7.81			
B- K	0 / 18	-38.5	-38.5	0.05 (1)	10.00			
K- J	0 / 18	-38.5	-38.5	0.05 (1)	10.00			
J- I	0 / 8	-38.5	-38.5	0.06 (2)	10.00			
I- H	0 / 4	-38.5	-38.5	0.03 (3)	10.00			
H- G	0 / 0	-38.5	-38.5	0.03 (3)	10.00			

TOTAL WEIGHT = 2 X 33 = 65 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (C-D:1) , BC=0.06/1.00 (J-K:1) , WB=0.04/1.00 (E-H:1) , SSI=0.09/1.00 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100004924
PROVINCE OF ONTARIO

DWG NO. TAM 17901/23
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	P16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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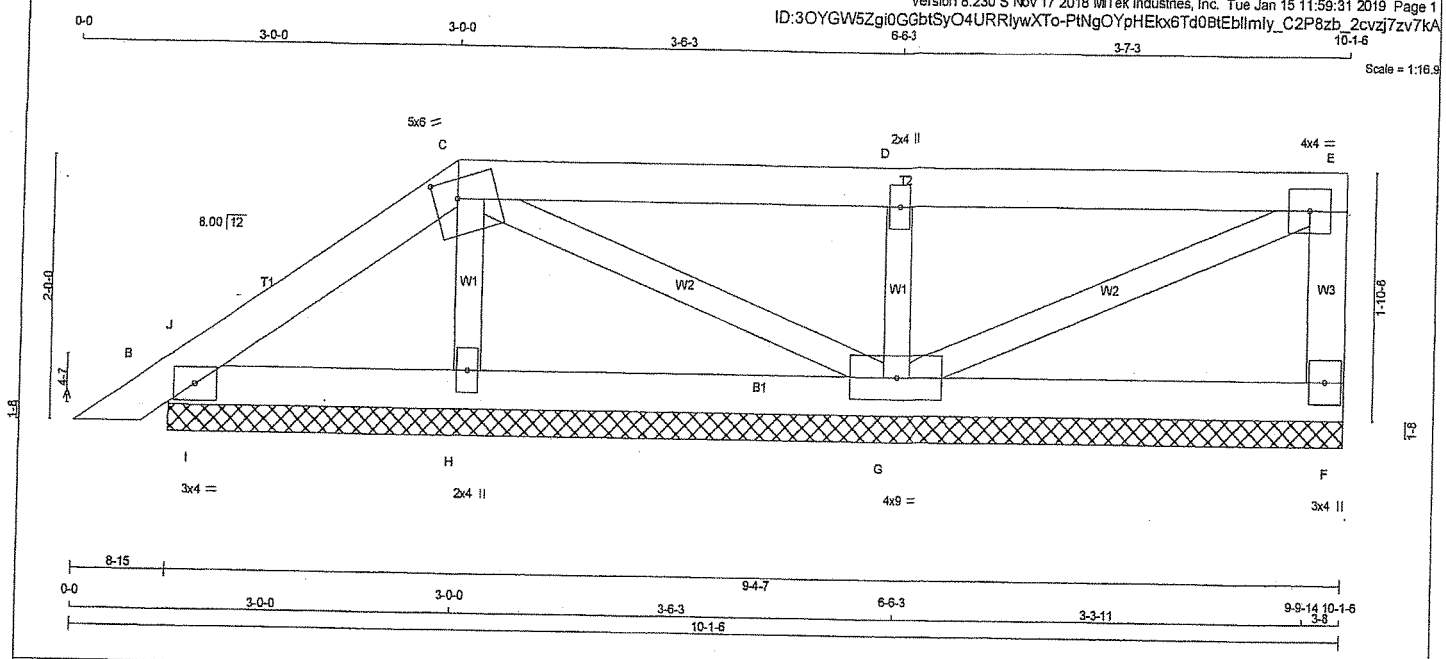


TOTAL WEIGHT = 32 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	ORY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	ORY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	1.75	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	170	0	170	0	0	9-4-7	9-4-7
B	210	0	210	0	0	9-4-7	9-4-7
H	287	0	287	0	0	9-4-7	9-4-7
G	545	0	545	0	0	9-4-7	9-4-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	129	63 / 0	30 / 0	0 / 0	0 / 0	36 / 0	0 / 0
B	153	96 / 0	19 / 0	0 / 0	0 / 0	38 / 0	0 / 0
H	223	96 / 0	61 / 0	0 / 0	0 / 0	66 / 0	0 / 0
G	410	212 / 0	86 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 DR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

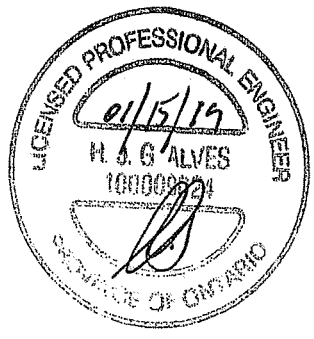
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- J	-43 / 0	-84.9	-84.9	0.01 (3)	6.25
J- C	-48 / 0	-84.9	-84.9	0.05 (1)	6.25
C- D	0 / 2	-84.9	-84.9	0.18 (1)	10.00
D- E	0 / 2	-84.9	-84.9	0.18 (1)	10.00
F- E	-115 / 0	0.0	0.0	0.01 (1)	7.81
B- I	0 / 39	-38.5	-38.5	0.05 (1)	10.00
I- H	0 / 39	-38.5	-38.5	0.06 (2)	10.00
H- G	0 / 29	-38.5	-38.5	0.10 (3)	10.00
G- F	0 / 0	-38.5	-38.5	0.10 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-153 / 0	0.02 (1)
G- E	-2 / 0	0.00 (1)
C- G	-33 / 0	0.01 (1)
G- D	-377 / 0	0.05 (1)
I- J	-102 / 10	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-D:1) , BC=0.10/1.00 (G-H:3) , WB=0.05/1.00 (D-G:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

ENG. NO. TAM 7490124
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	P17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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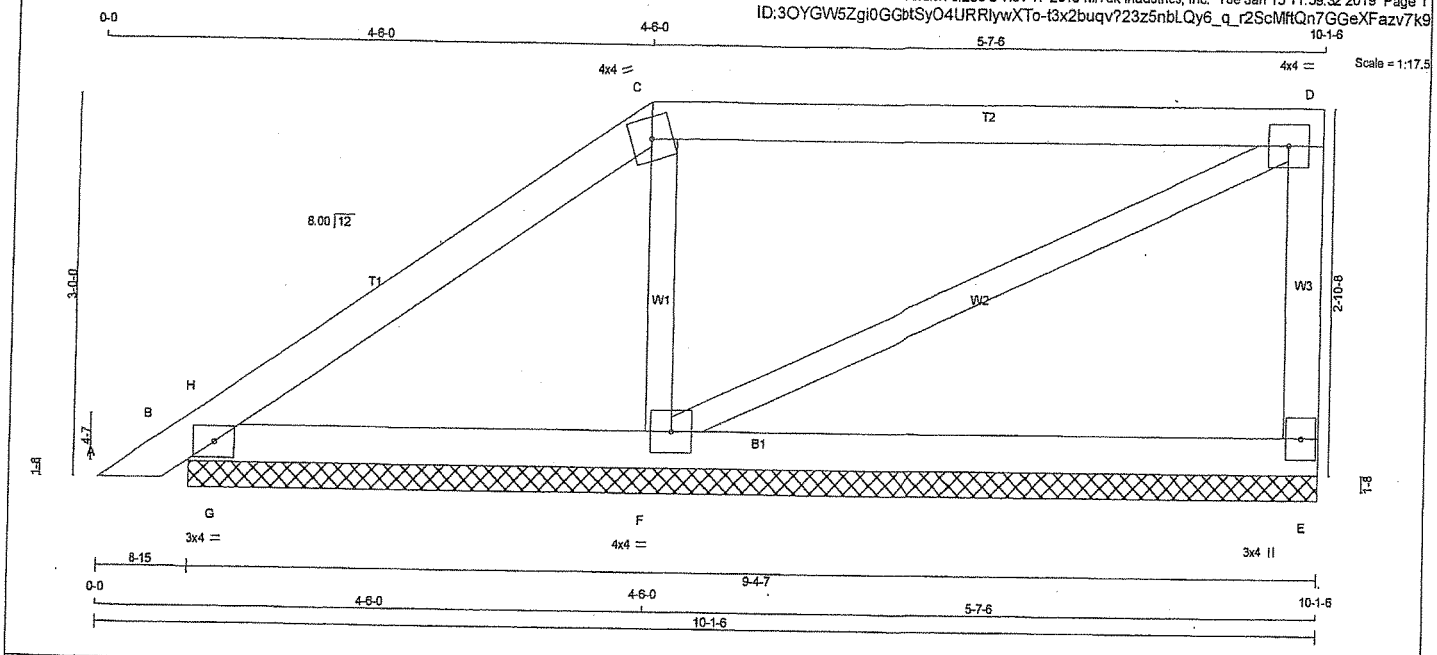


TOTAL WEIGHT = 32 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	339	0	339	0	0	9-4-7	9-4-7
B	293	0	293	0	0	9-4-7	9-4-7
F	580	0	580	0	0	9-4-7	9-4-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	253	136 / 0	47 / 0	0 / 0	0 / 0	67 / 0	0 / 0
B	212	135 / 0	25 / 0	0 / 0	0 / 0	52 / 0	0 / 0
F	450	193 / 0	124 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

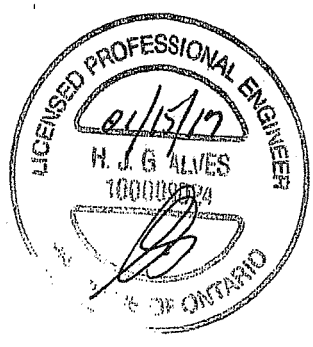
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	
B- H	-17 / 15	-84.9	-84.9	0.06 (1)	6.25	
H- C	-72 / 0	-84.9	-84.9	0.15 (1)	6.25	
C- D	-40 / 0	-84.9	-84.9	0.46 (1)	6.25	
E- D	-257 / 0	0.0	0.0	0.04 (1)	7.81	
B- G	0 / 54	-38.5	-38.5	0.14 (1)	10.00	
G- F	0 / 54	-38.5	-38.5	0.22 (2)	10.00	
F- E	0 / 0	-38.5	-38.5	0.22 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-337 / 0	0.06 (1)
F- D	0 / 45	0.01 (1)
G- H	-296 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BC8C 2018 , OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.46/1.00 (C-D:1) , BC=0.22/1.00 (F-G:2) , WB=0.06/1.00 (C-F:1) , SSI=0.24/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (8) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

DWG NO. TAM T290 1125
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300399	P18	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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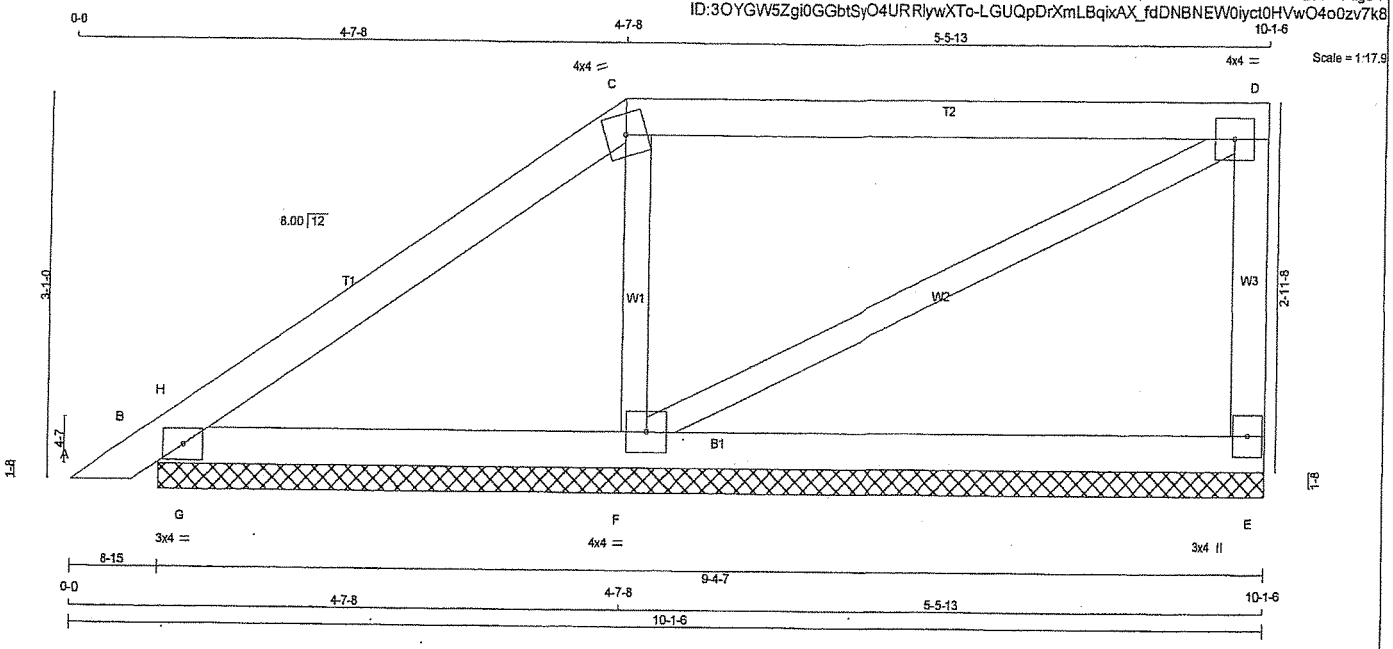


TOTAL WEIGHT = 2 X 32 = 65 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	333	0	333	0	0	9-4-7	9-4-7
B	304	0	304	0	0	9-4-7	9-4-7
F	575	0	575	0	0	9-4-7	9-4-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	249	136 / 0	46 / 0	0 / 0	0 / 0	66 / 0	0 / 0
B	221	139 / 0	28 / 0	0 / 0	0 / 0	54 / 0	0 / 0
F	446	192 / 0	122 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

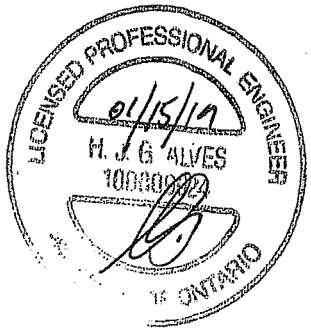
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- H	-26 / 15	-84.9	-84.9	0.06 (1)	6.25
H- C	-75 / 0	-84.9	-84.9	0.16 (1)	6.25
C- D	-44 / 0	-84.9	-84.9	0.44 (1)	6.25
E- D	-254 / 0	0.0	0.0	0.04 (1)	7.81
B- G	0 / 57	-38.5	-38.5	0.14 (1)	10.00
G- F	0 / 57	-38.5	-38.5	0.22 (2)	10.00
F- E	0 / 0	-38.5	-38.5	0.21 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-335 / 0	0.06 (1)
F- D	0 / 49	0.01 (1)
G- H	-306 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.44/1.00 (C-D:1) , BC=0.22/1.00 (F-G:2) , WB=0.06/1.00 (C-F:1) , SSI=0.25/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (8) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

DWG NO. TAM T190126
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300402	P19G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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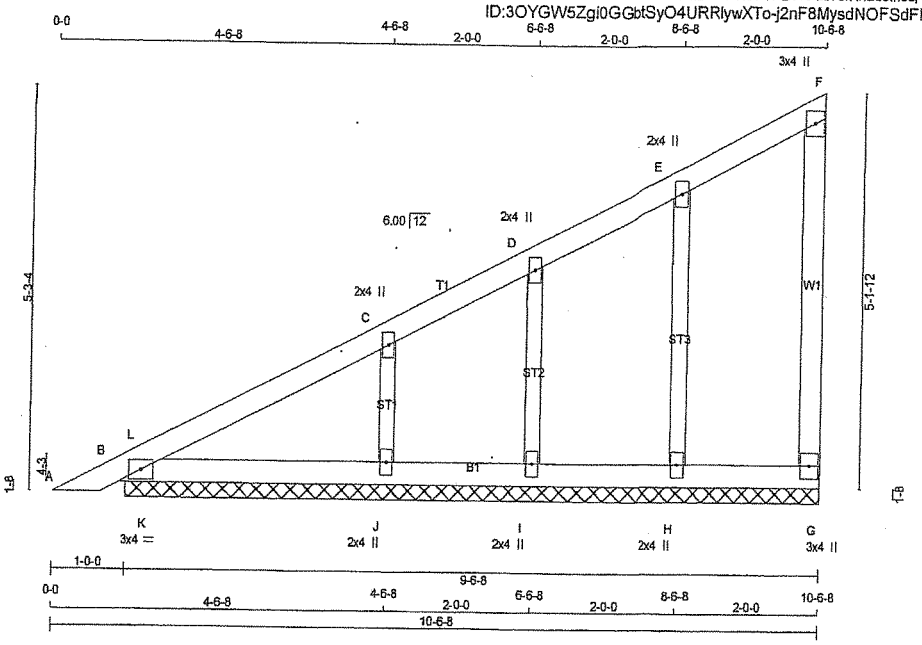


TOTAL WEIGHT = 2 X 36 = 72 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - G	2x4	ORY	No.2	SPF
ALL WEBS	2x3	ORY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C, D, E					
C	TMW+w	MT20	2.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	BMV1+p	MT20	3.0	4.0	
H, I, J					
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

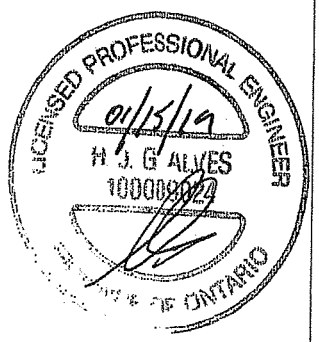
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	H-E	-200 / 0	0.05 (1)
B-L	-39 / 0	-84.9	-84.9	0.04 (3)	6.25	I-D	-122 / 0	0.02 (1)
L-C	-5 / 4	-84.9	-84.9	0.11 (1)	10.00	J-C	-277 / 0	0.04 (1)
C-D	-23 / 0	-84.9	-84.9	0.11 (1)	6.25	K-L	-98 / 80	0.00 (1)
D-E	0 / 1	-84.9	-84.9	0.05 (1)	10.00			
E-F	-8 / 0	-84.9	-84.9	0.05 (1)	10.00			
G-F	-70 / 0	0.0	0.0	0.03 (1)	7.81			
B-K	0 / 16	-38.5	-38.5	0.08 (1)	10.00			
K-J	0 / 16	-38.5	-38.5	0.09 (2)	10.00			
J-I	0 / 6	-38.5	-38.5	0.09 (2)	10.00			
I-H	0 / 3	-38.5	-38.5	0.03 (3)	10.00			
H-G	0 / 0	-38.5	-38.5	0.03 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (C-D:1) , BC=0.09/1.00 (J-K:2) , WB=0.05/1.00 (E-H:1) , SSI=0.11/1.00 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

DWG NO. TAM T1901139
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300402	P20	2	1	TRUSS DESC.	

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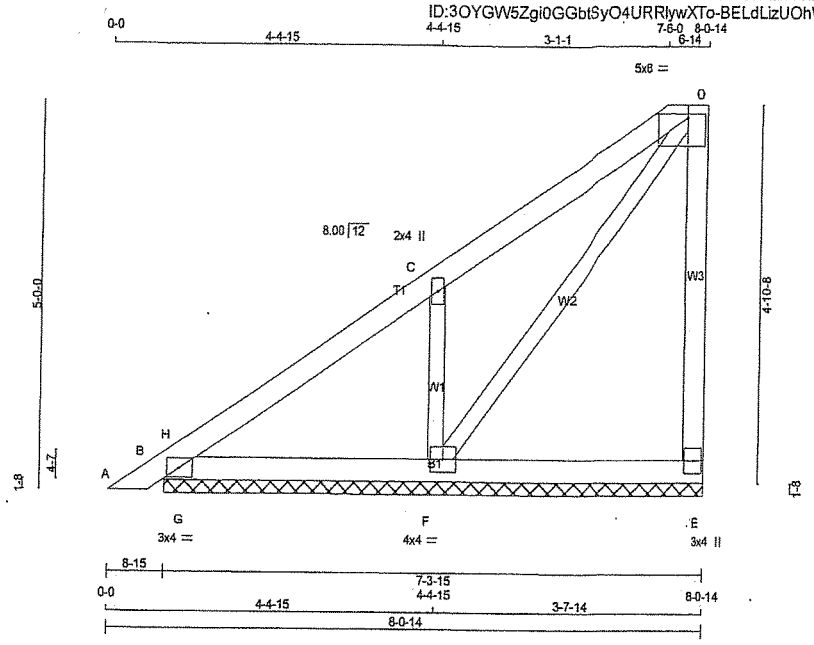

TOTAL WEIGHT = 2 X 30 = 81 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	ORY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMVMW*-p	MT20	5.0	8.0	0.50	5.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTOREO GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	172	0	172	0	0	7-3-15	7-3-15
B	229	0	229	0	0	7-3-15	7-3-15
F	558	0	558	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
E	131	64 / 0	30 / 0	0 / 0	0 / 0	37 / 0	0 / 0
B	170	96 / 0	30 / 0	0 / 0	0 / 0	45 / 0	0 / 0
F	423	212 / 0	94 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- H	-50 / 0	-84.9	-84.9	0.03 (3)	6.25
H- C	0 / 11	-84.9	-84.9	0.18 (1)	10.00
C- D	-18 / 0	-84.9	-84.9	0.18 (1)	6.25
E- D	-122 / 0	0.0	0.0	0.06 (1)	7.81
B- G	0 / 10	-38.5	-38.5	0.06 (1)	10.00
G- F	0 / 10	-38.5	-38.5	0.12 (2)	10.00
F- E	0 / 0	-38.5	-38.5	0.12 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-388 / 0	0.06 (1)
F- D	-3 / 6	0.00 (1)
G- H	-95 / 55	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-H:1) , BC=0.12/1.00 (F-G:2) , WB=0.06/1.00 (C-F:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (C) (INPUT = 0.90)
JSI METAL= 0.19 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM 7190140
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300403	P21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Jan 15 12:56:04 2019 Page 1
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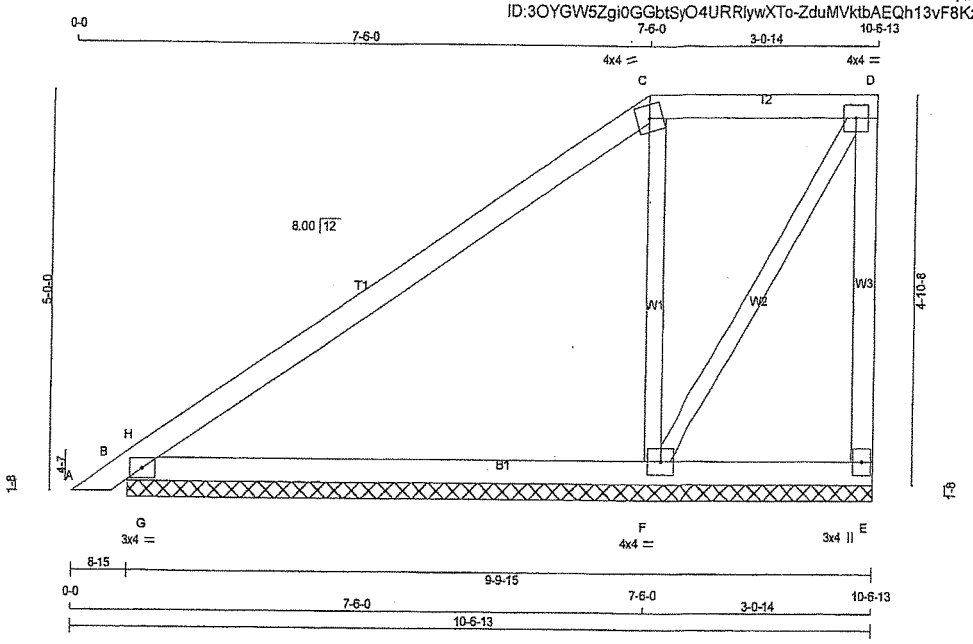


TOTAL WEIGHT = 2 X 37 = 75 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	OOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	241	0	241	0	0	9-9-15	9-9-15
B	505	0	505	0	0	9-9-15	9-9-15
F	522	0	522	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	176	109 / 0	23 / 0	0 / 0	0 / 0	44 / 0	0 / 0
B	376	208 / 0	68 / 0	0 / 0	0 / 0	99 / 0	0 / 0
F	407	171 / 0	115 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

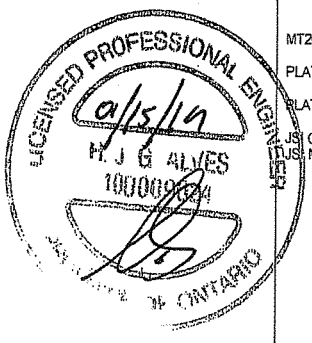
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- H	-167 / 174	-84.9	-84.9	0.22 (1)	6.25
H- C	-130 / 0	-84.9	-84.9	0.51 (1)	6.25
C- D	-90 / 0	-84.9	-84.9	0.14 (1)	6.25
E- D	-269 / 0	0.0	0.0	0.11 (1)	7.81
B- G	0 / 97	-38.5	-38.5	0.36 (1)	10.00
G- F	0 / 97	-38.5	-38.5	0.40 (1)	10.00
F- E	0 / 0	-38.5	-38.5	0.36 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-317 / 0	0.11 (1)
F- D	0 / 166	0.04 (1)
G- H	-764 / 142	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.51/1.00 (C-H:1) , BC=0.40/1.00 (F-G:1) , WB=0.11/1.00 (C-F:1) , SSI=0.64/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

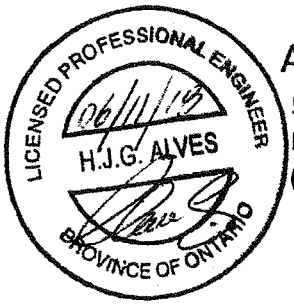
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1957	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS: GRIP= 0.38 (B) (INPUT = 0.90)
JS: METAL= 0.11 (C) (INPUT = 1.00)

DWG NO. TAM 1790442
STRUCTURAL
PERMIT ONLY



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213 Feb 09, 2018

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

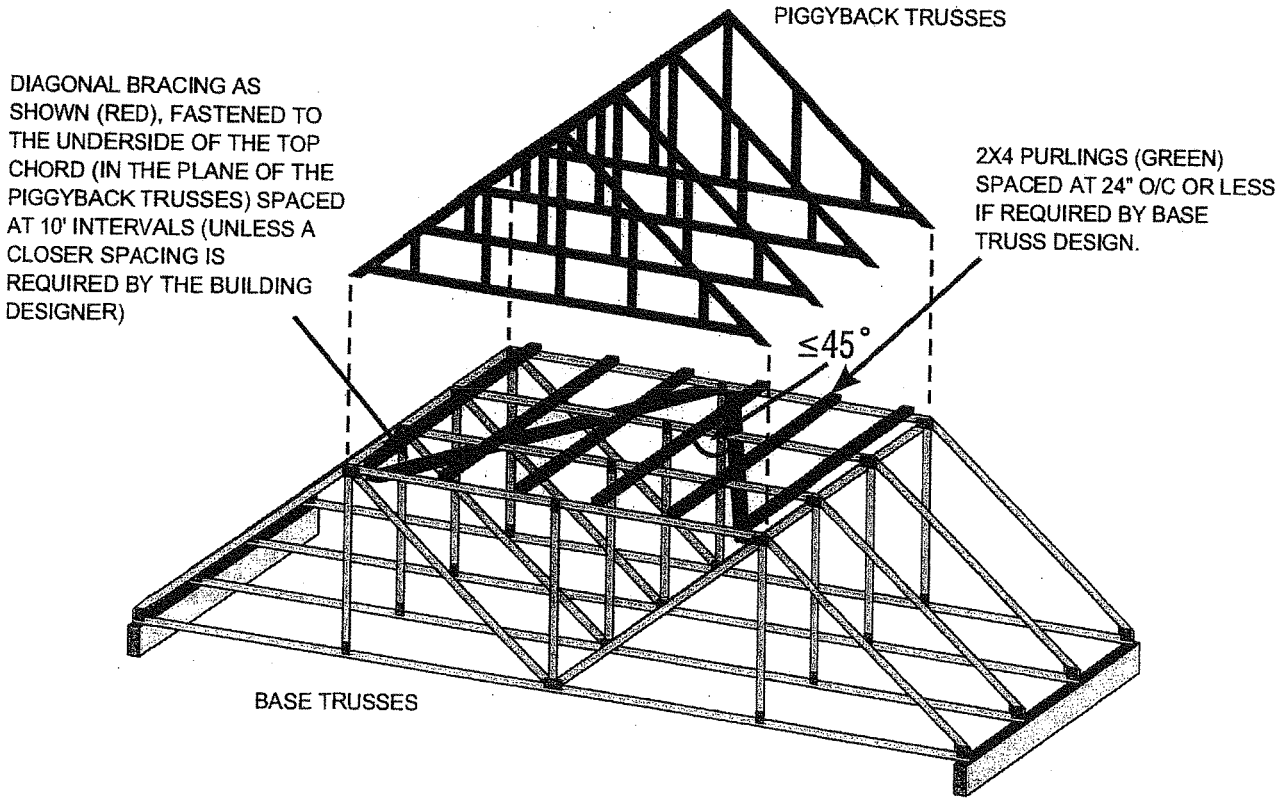
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

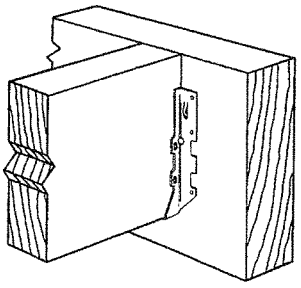
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

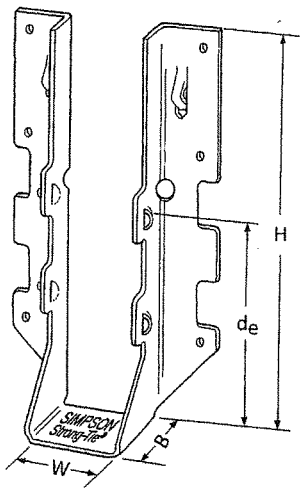
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



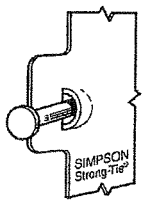
Typical LUS Installation



LUS28

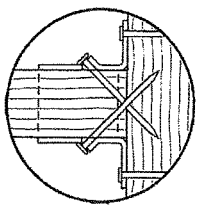
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty, or see strongtie.com.

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

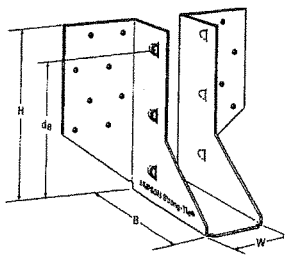
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

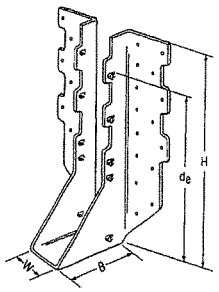
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

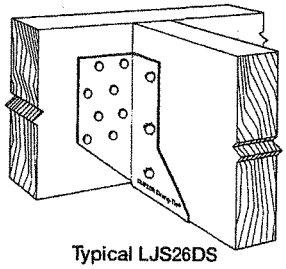
- See current catalogue for options



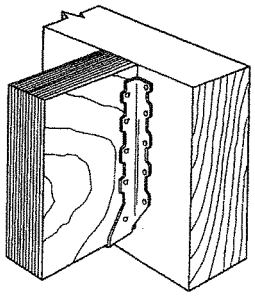
LJS26DS



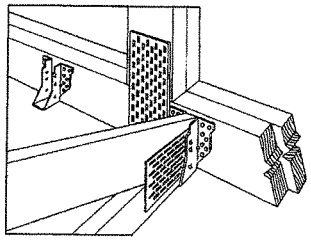
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



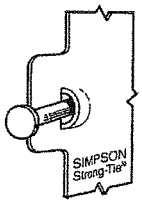
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

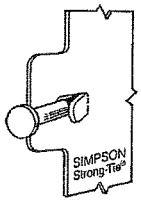
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

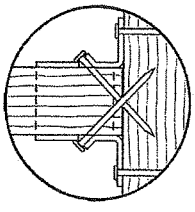


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

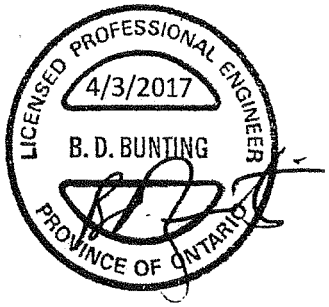
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

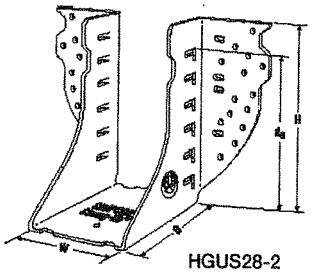
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

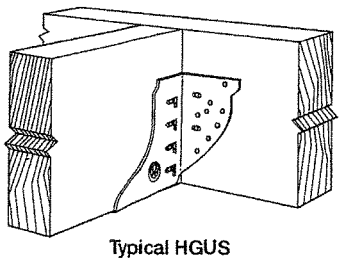
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

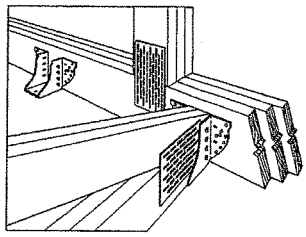
- See current catalogue for options



HGUS28-2



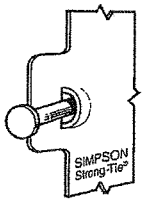
Typical HGUS Installation



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

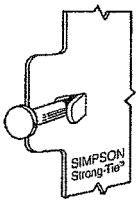
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

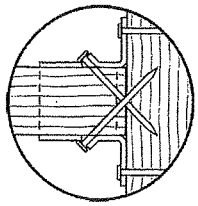


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHGUS17 3/17 exp. 6/19

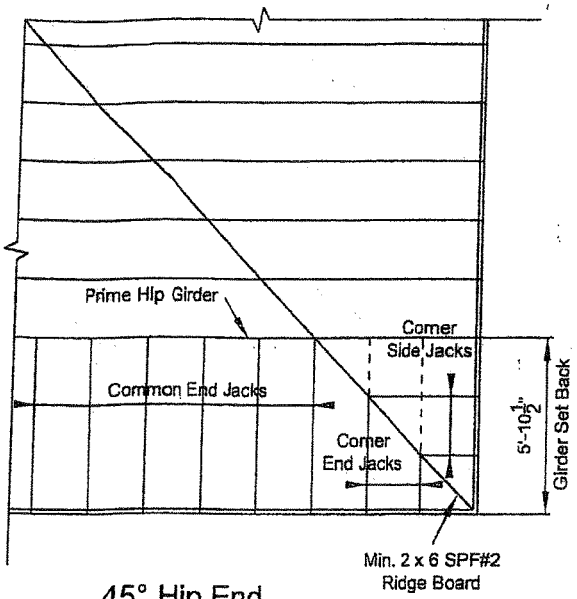
(800) 999-5099
strongtie.com

LUMBER SPECIFICATION

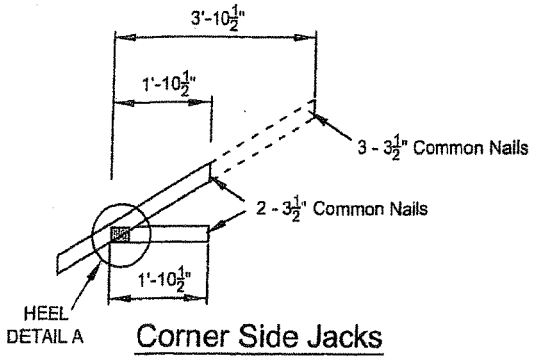
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

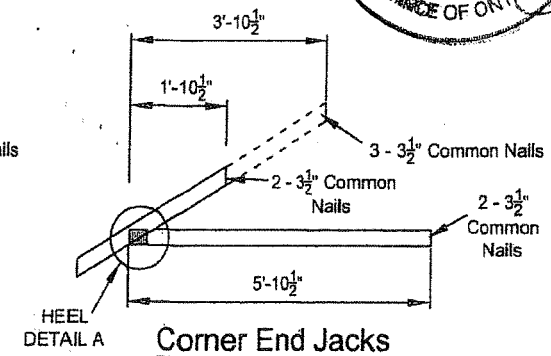
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



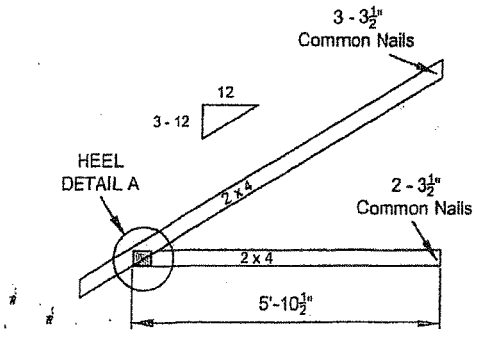
45° Hip End



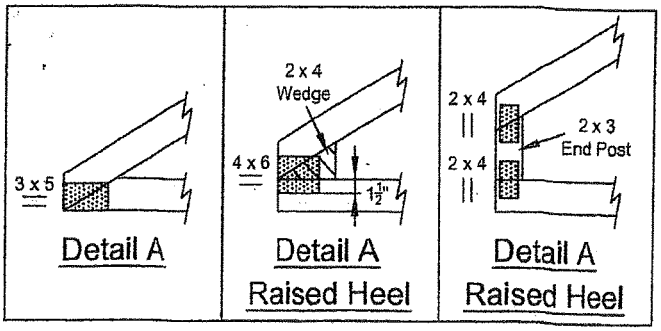
Corner Side Jacks



Corner End Jacks

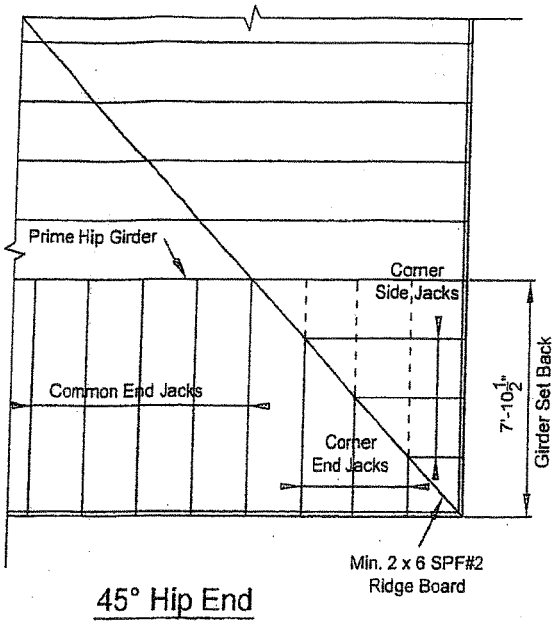


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

LUMBER SPECIFICATION

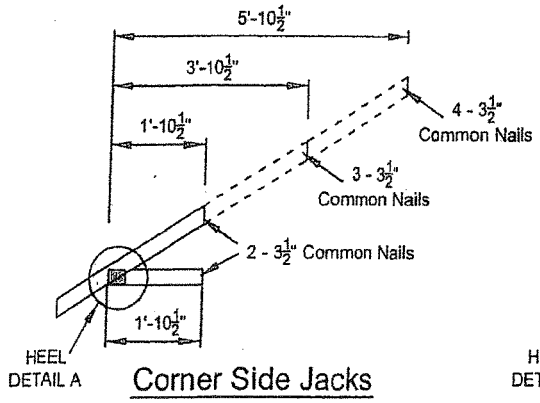
TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

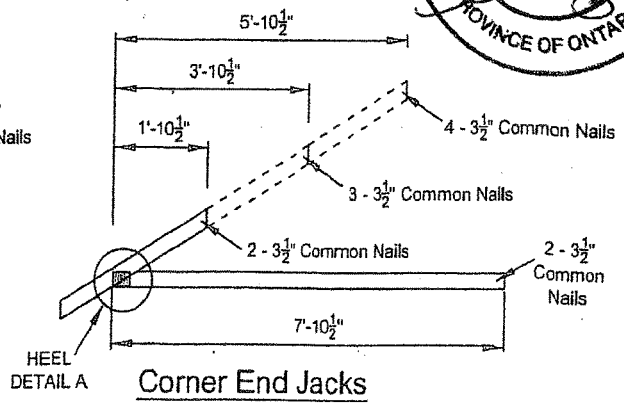
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

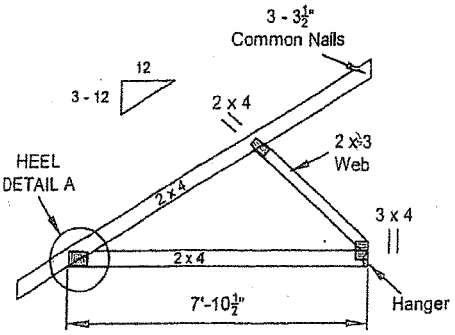
LICENSED PROFESSIONAL ENGINEER
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



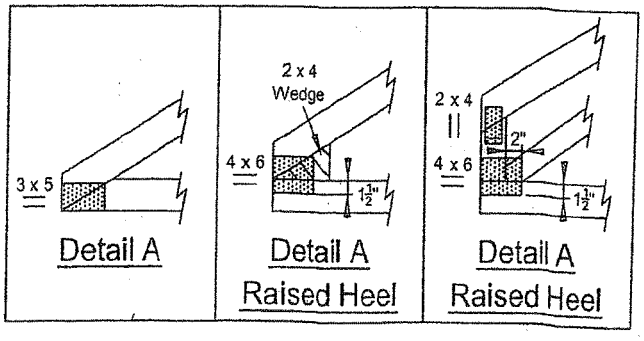
Corner Side Jacks



Corner End Jacks



Common End Jacks

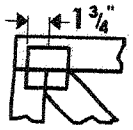


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

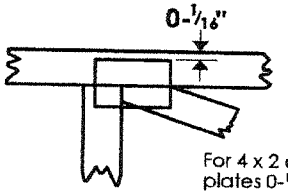
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

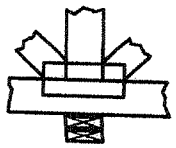
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

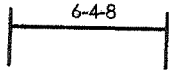
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

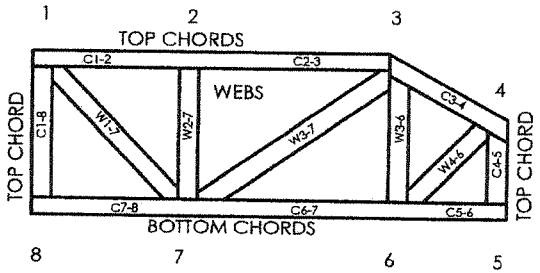
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensians shawn in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: Mii-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.