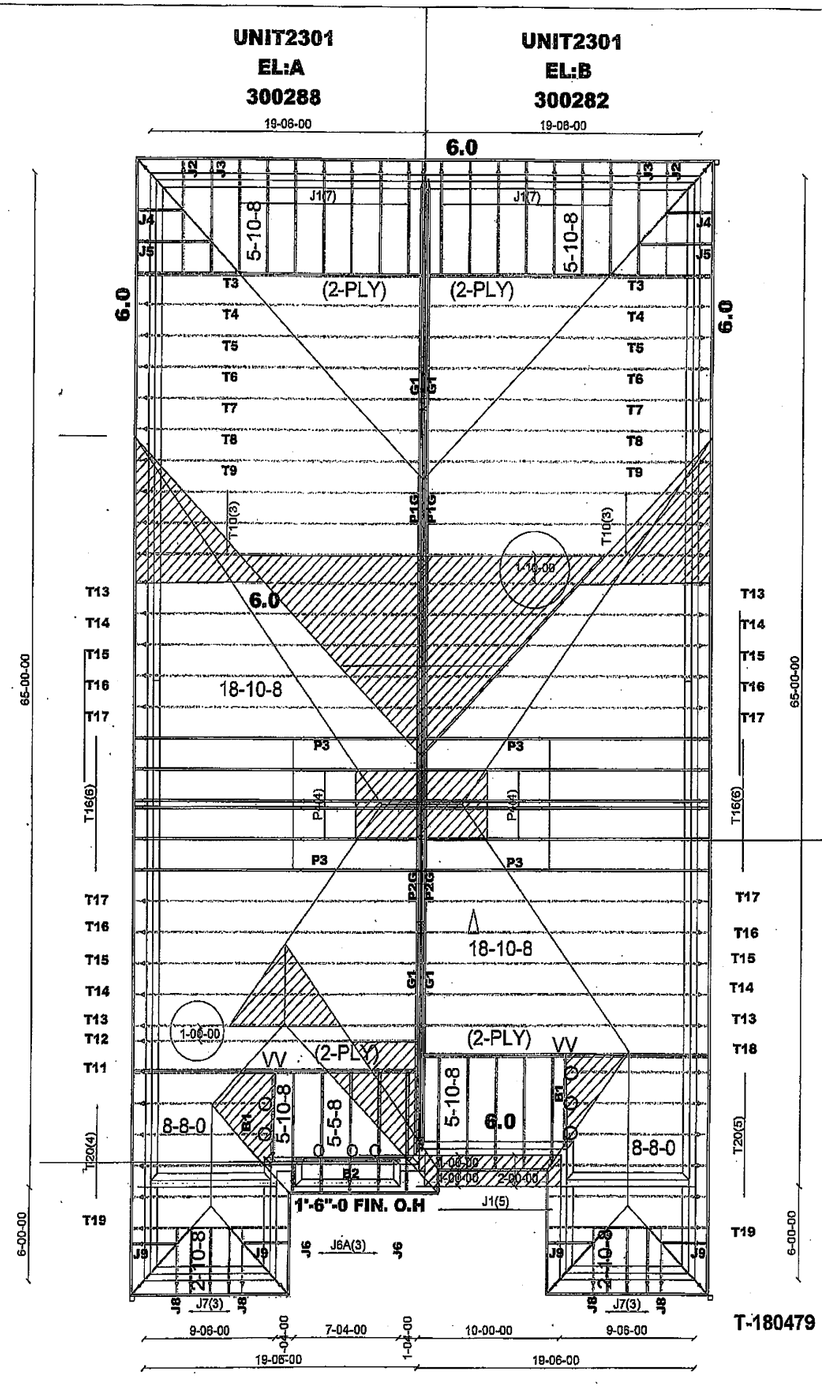


T-180479

8/12 PITCHES (TYP.)
UNLESS NOTED

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS26-2 - (VV)
LUS24 - (O)

ALL B-2-2X10
(FLUSH)

DENOTES:
CONVENTIONAL
FRAMING

DESIGN LOADS:

SNOW LOAD 23.3 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION. ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

JULY 24, 2018
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300288)
DWG #T1802100 THROUGH DWG #T1802119, INCLUSIVE, AND ALL DATED 7-20-18;
DWG #T1802121 AND DWG #T1802122 AND BOTH DATED 7-20-18;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

JULY 24, 2018
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300282)
DWG #T1802100 THROUGH DWG #T1802112, INCLUSIVE, AND ALL DATED 7-20-18;
DWG #T1802115 THROUGH DWG #T1802122, INCLUSIVE, AND ALL DATED 7-20-18;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

DATE: 7-24-18
BCIN:26064; FIRM:BCIN #29991
ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER
ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO TRUSS INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT.
ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY THE BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION.

LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH INDIVIDUAL LOT.

DWG #TAM 4 THROUGH DWG #TAM , 6 INCLUSIVE, DATED ; 8

STRUCTURAL COMPONENTS ONLY); THIS LAYOUT MUST BE READ TOGETHER WITH REFERENCED TRUSS COMPONENT DRAWINGS ANDSEALED HARDWARE DRAWINGS. HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY.
REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING REQUIREMENTS:
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2 X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS SPECIFIED ON REFERENCED SEALED TRUSS COMPONENT DRAWINGS USING 2 - 3-1/4" COMMON WIRE NAILS PER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF "X-BRACING" ALONG THESE SAME WEB MEMBERS AT LOCATIONS INDICATED (*) USING THE SAME NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
PROVIDE 2 X 4 #2 SPF T-BRACE ALONG FULL LENGTH OF LATERALLY BRACED WEB MEMBERS OF 1 PLY TRUSSES COMPONENTS REQUIRING "T-BRACE" USING 1 ROW OF 3-1/4" COMMON WIRE NAILS AT 6" O/C. SIMILARLY PROVIDE 2 X 6 BRACE WITH 2 ROWS OF NAILING FOR 2 PLY TRUSS COMPONENTS AND 2 X 8 BRACE WITH 3 ROWS OF NAILING FOR 3 PLY TRUSS COMPONENTS.

(1) 2 X 6 #2 SPF ON FLAT OVER PLYWOOD SHEATHING.
(2) 2 X 6 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS COMPONENTS AND AT LOCATIONS MARKED BY "X".
(3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O/C WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X 4 VERTICAL SUPPORTS TO TRUSS BASE BELOW WHERE NECESSARY.

ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE TRUSS COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH STUDS AT24" O/C.

ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE (CURRENT EDITION).

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION 3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES AND/OR CATEGORIES.
REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

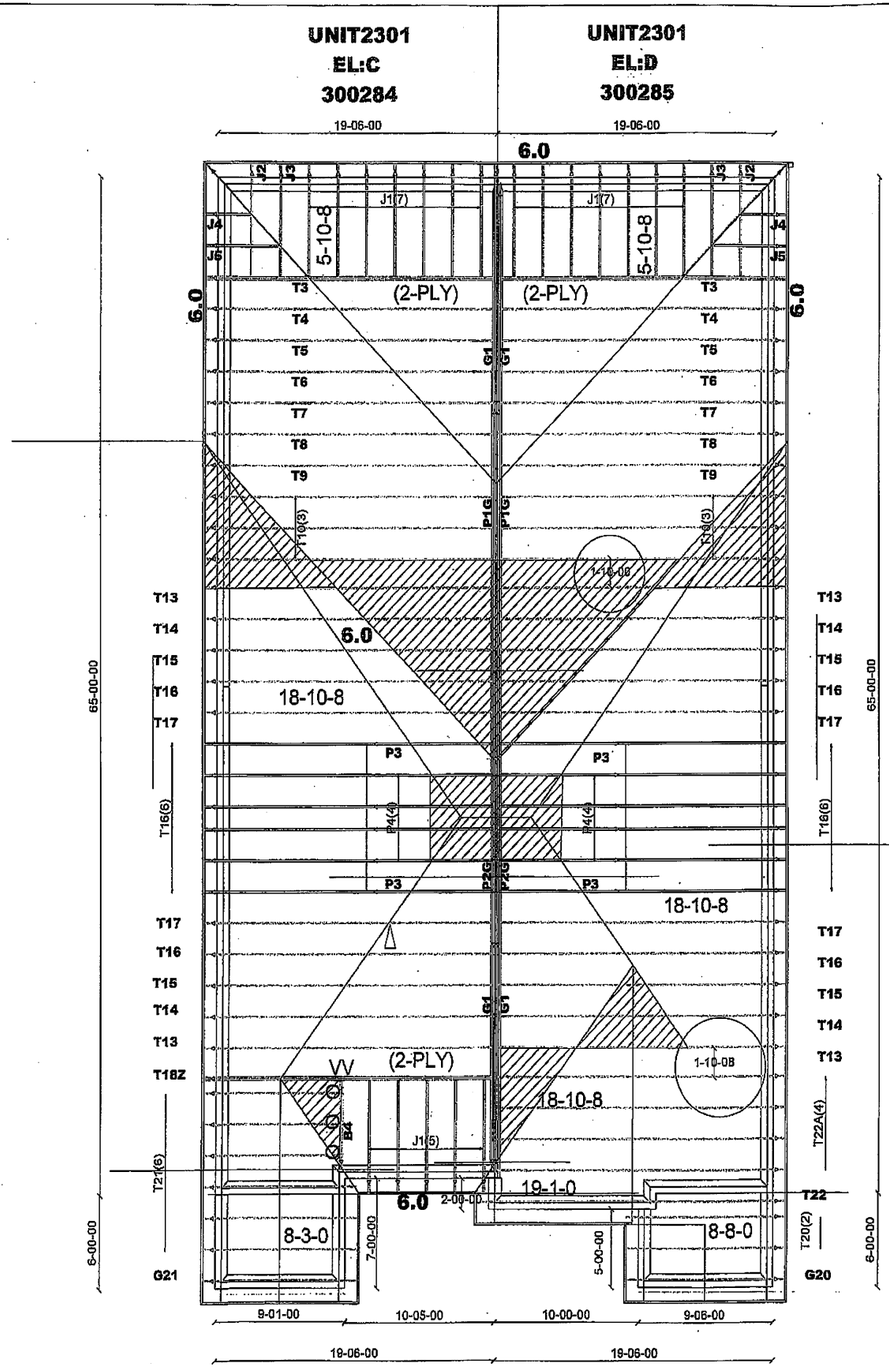
DWG #TAM 30220-18
BCIN: 26064
FIRM: 29991
SEALED STRUCTURAL COMPONENTS ONLY

M11,842

Quote Folder

TAMARACK	Job Track: 41239	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 2301 / A PAIRED WITH B
	Layout ID: 300280	Project: FORESTSIDE PH. 6	
	Plan Log: 98887	Date: 07/20/18 Designer: SUNNY / CH	

MiTek ver 7.5.0



8/12 PITCHES (TYP.) UNLESS NOTED

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS26-2 - (VV)
LUS24 - (O)

ALL B-2-2X10
(FLUSH)

DENOTES:
CONVENTIONAL
FRAMING

DESIGN LOADS:

SNOW LOAD 23.3 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PARTS OF THE OBC.LATEST EDITION. ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

JULY 24, 2018
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300284)
DWG #T1802100 THROUGH DWG #T1802112, INCLUSIVE, AND ALL DATED 7-20-18;
DWG #T1802115 THROUGH DWG #T1802119, INCLUSIVE, AND ALL DATED 7-20-18;
DWG #T1802124 THROUGH DWG #T1802126, INCLUSIVE, AND ALL DATED 7-20-18;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

JULY 24, 2018
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:41239/300285)
DWG #T1802100 THROUGH DWG #T1802112, INCLUSIVE, AND ALL DATED 7-20-18;
DWG #T1802115 THROUGH DWG #T1802119, INCLUSIVE, AND ALL DATED 7-20-18;
DWG #T1802122 AND DWG #T1802123 AND BOTH DATED 7-20-18;
DWG #T1802127 AND DWG #T1802128 AND BOTH DATED 7-20-18;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

DATE: 7-24 18
BCIN:26064; FIRM:BCIN #29991
ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER
ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO TRUSS INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT.
ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY THE BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION.

LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH INDIVIDUAL LOT.

DWG #TAM [illegible] THROUGH DWG #TAM , [illegible] INCLUSIVE, DATED ; [illegible]

STRUCTURAL COMPONENTS ONLY); THIS LAYOUT MUST BE READ TOGETHER WITH REFERENCED TRUSS COMPONENT DRAWINGS ANDSEALED HARDWARE DRAWINGS. HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY.
REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING REQUIREMENTS:
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2 X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS SPECIFIED ON REFERENCED SEALED TRUSS COMPONENT DRAWINGS USING 2 - 3-1/4" COMMON WIRE NAILS PER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF "X-BRACING" ALONG THESE SAME WEB MEMBERS AT LOCATIONS INDICATED (*) USING THE SAME NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
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(2) 2 X 6 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS COMPONENTS AND AT LOCATIONS MARKED BY "X".
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ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE (CURRENT EDITION).

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION 3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUAUFIED AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES AND/OR CATEGORIES.
REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

DWG #TAM 30221-18
BCIN: 26064
FIRM: 29991
SEALED STRUCTURAL COMPONENTS ONLY

M11,842

Quote Folder

TAMARACK TIMBER INC.	Job Track: 41239	Builder / Location: ROYAL PINE HOMES / BRAMPTON	Model / Elevation: UNIT 2301 / C PAIRED WITH D
	Layout ID: 300283	Project: FORESTSIDE PH. 6	
	Plan Log: 98887	Date: 07/20/18 Designer: SUNNY / CH	

MiTek ver 7.5.0

TAMARACK LUMBER INC
ALPA LUMBER GROUP

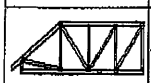
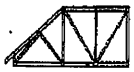
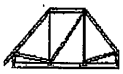
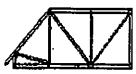
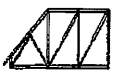
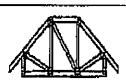
Delivery Shiplist

DATE	07/20/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300288 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2301 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	G1 GABLE	6.00 0.00	31-01-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	331.88 210.34		
	1 2 Ply	T3 HALF HIP	6.00 0.00	18-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	153.42 97.34		
	1	T4 HALF HIP	6.00 0.00	18-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	77.87 48.33		
	1	T5 HALF HIP	6.00 0.00	18-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 06-01-04	84.50 52.50		
	1	T6 HALF HIP	6.00 0.00	18-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-01-04	84.96 52.67		
	1	T7 HALF HIP	6.00 0.00	18-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	84.58 52.50		
	1	T8 HALF HIP	6.00 0.00	18-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-01-04	93.52 58.33		
	1	T9 HALF HIP	6.00 0.00	18-10-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-01-04	95.68 59.00		
	3	T10 MONOPITCH	6.00 0.00	18-10-08	10-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-07-04	258.84 159.99		
	1 2 Ply	T11 HIP GIRDER	8.00 0.00	18-10-08	05-07-13	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	175.48 107.34		
	1	T12 HIP	8.00 0.00	18-10-08	06-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	84.73 54.50		
	2	T13 HALF HIP	8.00 0.00	18-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	153.84 96.00		
	2	T14 HALF HIP	8.00 0.00	18-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	166.60 103.34		
	2	T15 HALF HIP	8.00 0.00	18-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	176.64 112.00		
	8	T16 HALF HIP	8.00 0.00	18-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	805.20 497.36		
	2	T17 HALF HIP	8.00 0.00	18-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	189.50 118.66		
	1	T19 HIP GIRDER	8.00 0.00	08-08-00	03-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	39.21 25.33		
	4	T20 COMMON	8.00 0.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	149.92 94.68		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

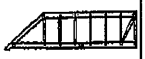
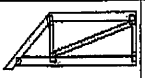
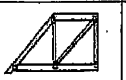
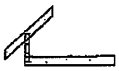
Delivery Shiplist

DATE	07/20/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300288 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2301 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	P1G GABLE	6.00 0.00	16-03-03	02-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-04-08	51.68 32.83		
	1	P2G GABLE	6.00 0.00	16-03-03	04-00-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-04-08	56.31 36.50		
	2	P3 PIGGYBACK	8.00 0.00	08-00-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	53.88 36.00		
	4	P4 PIGGYBACK	8.00 0.00	08-00-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	113.64 77.32		
	7	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	117.53 74.69		
	1	J2 JACK-OPEN	6.00 0.00	01-09-07	02-00-12	2 X 4	2 X 4	01-03-08 04-01-01	01-02-00 00-03-08	11.58 7.33		
	1	J3 JACK-OPEN	6.00 0.00	03-09-07	03-00-12	2 X 4	2 X 4	01-03-08 02-01-01	01-02-00 00-03-08	14.13 8.67		
	1	J4 JACK-OPEN	6.00 0.00	01-09-07	02-00-12	2 X 4	2 X 4	01-03-08 00-01-01	01-02-00 00-03-08	7.02 4.67		
	1	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	9.57 6.00		
	2	J6 JACK-OPEN	8.00 0.00	05-10-08	05-07-13	2 X 4	2 X 4	01-09-08 00-00-00	01-08-13 05-07-13	37.78 24.00		
	3	J6A JACK-OPEN	8.00 0.00	05-05-08	05-07-13	2 X 4	2 X 4	02-02-08 00-00-00	02-00-02 05-07-13	56.19 36.00		
	3	J7 JACK-OPEN	8.00 0.00	02-10-08	03-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-03-13	30.90 20.01		
	2	J8 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 01-01-01	01-04-13 00-03-08	17.62 10.66		
	2	J9 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 00-01-01	01-04-13 00-03-08	15.34 9.34		

TOTAL # TRUSS= **68.00** TOTAL BFT OF ALL TRUSSES= **2384.23** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3799.54** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **6.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

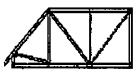
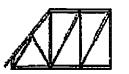
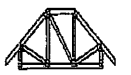
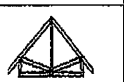
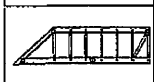
Delivery Shiplist

DATE	07/20/18
SALES REP	Mario

JOB TRACK:41239 LAYOUT ID: 300282 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2301 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	G1 GABLE	6.00 0.00	31-01-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	331.88 210.34		
	1 2 Ply	T3 HALF HIP	6.00 0.00	18-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	153.42 97.34		
	1	T4 HALF HIP	6.00 0.00	18-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	77.87 48.33		
	1	T5 HALF HIP	6.00 0.00	18-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 06-01-04	84.50 52.50		
	1	T6 HALF HIP	6.00 0.00	18-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-01-04	84.96 52.67		
	1	T7 HALF HIP	6.00 0.00	18-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	84.58 52.50		
	1	T8 HALF HIP	6.00 0.00	18-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-01-04	93.52 58.33		
	1	T9 HALF HIP	6.00 0.00	18-10-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-01-04	95.68 59.00		
	3	T10 MONOPITCH	6.00 0.00	18-10-08	10-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-07-04	258.84 159.99		
	2	T13 HALF HIP	8.00 0.00	18-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	153.84 96.00		
	2	T14 HALF HIP	8.00 0.00	18-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	166.60 103.34		
	2	T15 HALF HIP	8.00 0.00	18-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	176.64 112.00		
	8	T16 HALF HIP	8.00 0.00	18-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	805.20 497.36		
	2	T17 HALF HIP	8.00 0.00	18-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	189.50 118.66		
	1 2 Ply	T18 HALF HIP	8.00 0.00	18-10-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	181.94 111.34		
	1	T19 HIP GIRDER	8.00 0.00	08-08-00	03-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	39.21 25.33		
	5	T20 COMMON	8.00 0.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	187.40 118.35		
	1	P1G GABLE	6.00 0.00	16-03-03	02-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-04-08	51.68 32.83		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

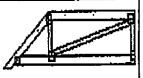
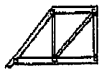
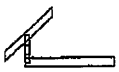
Delivery Shiplist

DATE	07/20/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300282 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2301 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	P2G GABLE	6.00 0.00	16-03-03	04-00-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-04-08	56.31 36.50		
	2	P3 PIGGYBACK	8.00 0.00	08-00-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	53.88 36.00		
	4	P4 PIGGYBACK	8.00 0.00	08-00-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	113.64 77.32		
	12	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	201.48 128.04		
	1	J2 JACK-OPEN	6.00 0.00	01-09-07	02-00-12	2 X 4	2 X 4	01-03-08 04-01-01	01-02-00 00-03-08	11.58 7.33		
	1	J3 JACK-OPEN	6.00 0.00	03-09-07	03-00-12	2 X 4	2 X 4	01-03-08 02-01-01	01-02-00 00-03-08	14.13 8.67		
	1	J4 JACK-OPEN	6.00 0.00	01-09-07	02-00-12	2 X 4	2 X 4	01-03-08 00-01-01	01-02-00 00-03-08	7.02 4.67		
	1	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	9.57 6.00		
	3	J7 JACK-OPEN	8.00 0.00	02-10-08	03-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-03-13	30.90 20.01		
	2	J8 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 01-01-01	01-04-13 00-03-08	17.62 10.66		
	2	J9 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 00-01-01	01-04-13 00-03-08	15.34 9.34		

TOTAL # TRUSS= **68.00** TOTAL BFT OF ALL TRUSSES= **2350.75** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3748.73** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

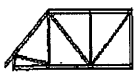
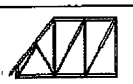
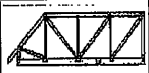
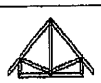
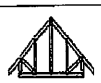
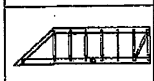
Delivery Shiplist

DATE	07/20/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300284 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2301 ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	G1 GABLE	6.00 0.00	31-01-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	331.88 210.34		
	1 2 Ply	T3 HALF HIP	6.00 0.00	18-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	153.42 97.34		
	1	T4 HALF HIP	6.00 0.00	18-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	77.87 48.33		
	1	T5 HALF HIP	6.00 0.00	18-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 06-01-04	84.50 52.50		
	1	T6 HALF HIP	6.00 0.00	18-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-01-04	84.96 52.67		
	1	T7 HALF HIP	6.00 0.00	18-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	84.58 52.50		
	1	T8 HALF HIP	6.00 0.00	18-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-01-04	93.52 58.33		
	1	T9 HALF HIP	6.00 0.00	18-10-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-01-04	95.68 59.00		
	3	T10 MONOPITCH	6.00 0.00	18-10-08	10-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-07-04	258.84 159.99		
	2	T13 HALF HIP	8.00 0.00	18-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	153.84 96.00		
	2	T14 HALF HIP	8.00 0.00	18-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	166.60 103.34		
	2	T15 HALF HIP	8.00 0.00	18-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	176.64 112.00		
	8	T16 HALF HIP	8.00 0.00	18-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	805.20 497.36		
	2	T17 HALF HIP	8.00 0.00	18-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	189.50 118.66		
	1 2 Ply	T18Z HALF HIP	8.00 0.00	18-10-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	181.94 111.34		
	6	T21 COMMON	8.00 0.00	08-03-00	04-01-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	216.00 136.02		
	1	G21 COMMON	8.00 0.00	08-03-00	04-01-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	36.60 23.67		
	1	P1G GABLE	6.00 0.00	16-03-03	02-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-04-08	51.68 32.83		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

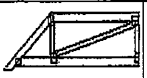
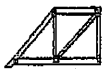
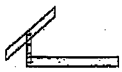
Delivery Shiplist

DATE	07/20/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300284 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2301 ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	P2G GABLE	6.00 0.00	16-03-03	04-00-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-04-08	56.31 36.50		
	2	P3 PIGGYBACK	8.00 0.00	08-00-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	53.88 36.00		
	4	P4 PIGGYBACK	8.00 0.00	08-00-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	113.64 77.32		
	12	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	201.48 128.04		
	1	J2 JACK-OPEN	6.00 0.00	01-09-07	02-00-12	2 X 4	2 X 4	01-03-08 04-01-01	01-02-00 00-03-08	11.58 7.33		
	1	J3 JACK-OPEN	6.00 0.00	03-09-07	03-00-12	2 X 4	2 X 4	01-03-08 02-01-01	01-02-00 00-03-08	14.13 8.67		
	1	J4 JACK-OPEN	6.00 0.00	01-09-07	02-00-12	2 X 4	2 X 4	01-03-08 00-01-01	01-02-00 00-03-08	7.02 4.67		
	1	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	9.57 6.00		

TOTAL # TRUSS= **62.00** TOTAL BFT OF ALL TRUSSES= **2326.75** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3710.86** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

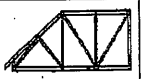
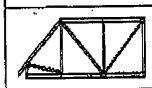
Delivery Shiplist

DATE	07/20/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300285 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2301 ELEVATION: D

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	G1 GABLE	6.00 0.00	31-01-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	331.88 210.34		
	1 2 Ply	T3 HALF HIP	6.00 0.00	18-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	153.42 97.34		
	1	T4 HALF HIP	6.00 0.00	18-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	77.87 48.33		
	1	T5 HALF HIP	6.00 0.00	18-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 06-01-04	84.50 52.50		
	1	T6 HALF HIP	6.00 0.00	18-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-01-04	84.96 52.67		
	1	T7 HALF HIP	6.00 0.00	18-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	84.58 52.50		
	1	T8 HALF HIP	6.00 0.00	18-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-01-04	93.52 58.33		
	1	T9 HALF HIP	6.00 0.00	18-10-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-01-04	95.68 59.00		
	3	T10 MONOPITCH	6.00 0.00	18-10-08	10-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-07-04	258.84 159.99		
	2	T13 HALF HIP	8.00 0.00	18-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	153.84 96.00		
	2	T14 HALF HIP	8.00 0.00	18-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	166.60 103.34		
	2	T15 HALF HIP	8.00 0.00	18-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	176.64 112.00		
	8	T16 HALF HIP	8.00 0.00	18-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	805.20 497.36		
	2	T17 HALF HIP	8.00 0.00	18-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	189.50 118.66		
	2	T20 COMMON	8.00 0.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	74.96 47.34		
	1	G20 COMMON	8.00 0.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	38.29 24.67		
	1	T22 COMMON	8.00 0.00	19-01-00	07-09-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	83.54 52.33		
	4	T22A COMMON	8.00 0.00	18-10-08	07-09-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	325.08 204.00		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

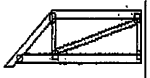
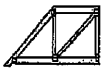
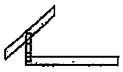
Delivery Shiplist

DATE	07/20/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300285 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2301 ELEVATION: D

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	P1G GABLE	6.00 0.00	16-03-03	02-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-04-08	51.68 32.83		
	1	P2G GABLE	6.00 0.00	16-03-03	04-00-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-04-08	56.31 36.50		
	2	P3 PIGGYBACK	8.00 0.00	08-00-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	53.88 36.00		
	4	P4 PIGGYBACK	8.00 0.00	08-00-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	113.64 77.32		
	7	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	117.53 74.69		
	1	J2 JACK-OPEN	6.00 0.00	01-09-07	02-00-12	2 X 4	2 X 4	01-03-08 04-01-01	01-02-00 00-03-08	11.58 7.33		
	1	J3 JACK-OPEN	6.00 0.00	03-09-07	03-00-12	2 X 4	2 X 4	01-03-08 02-01-01	01-02-00 00-03-08	14.13 8.67		
	1	J4 JACK-OPEN	6.00 0.00	01-09-07	02-00-12	2 X 4	2 X 4	01-03-08 00-01-01	01-02-00 00-03-08	7.02 4.67		
	1	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	9.57 6.00		

TOTAL # TRUSS= **56.00** TOTAL BFT OF ALL TRUSSES= **2330.71** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3714.24** LBS.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	G1	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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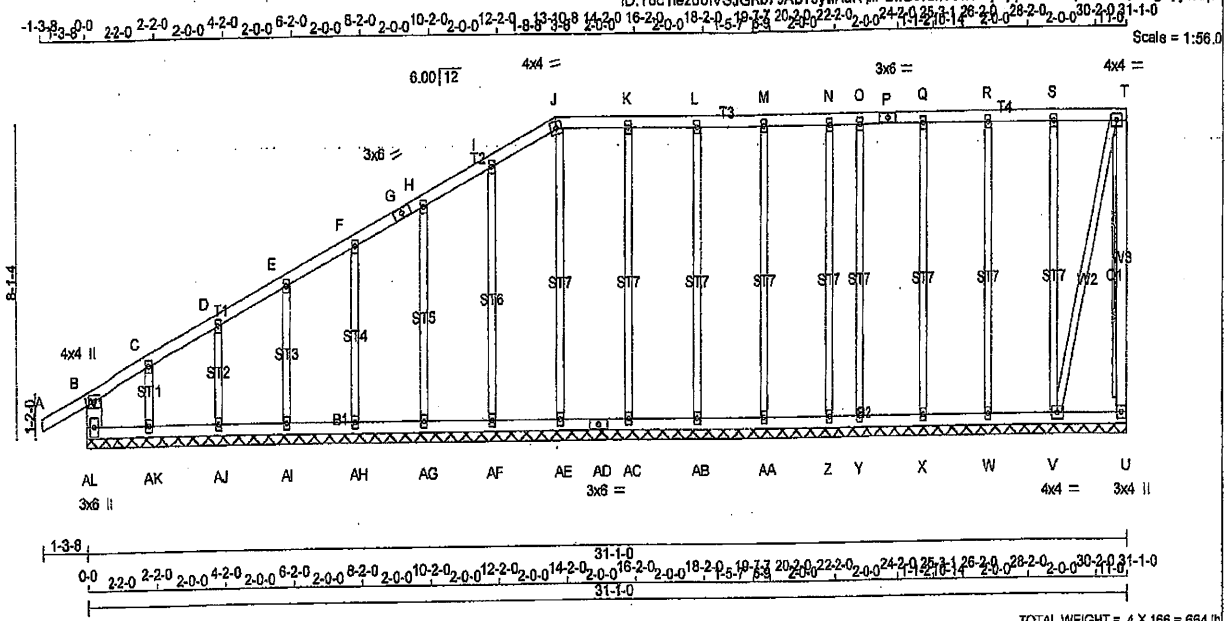


TOTAL WEIGHT = 4 X 166 = 664 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - P	2x4	DRY	No.2	SPF
P - T	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
AL - B	2x6	DRY	No.2	SPF
AL - AD	2x4	DRY	No.2	SPF
AD - U	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C, D, E, F, H, I, K, L, M, N, O, Q, R, S						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
P	TS-t	MT20	3.0	6.0		
T	TMVW-t	MT20	4.0	4.0		
U	BMV1+p	MT20	3.0	4.0		
V	BMWW1-t	MT20	4.0	4.0		
W, X, Y, Z, AA, AB, AC, AE, AF, AG, AH, AI, AJ, AK						
W	BMW1+w	MT20	2.0	4.0		
AD	BS-t	MT20	3.0	6.0		
AL	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT T-U

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

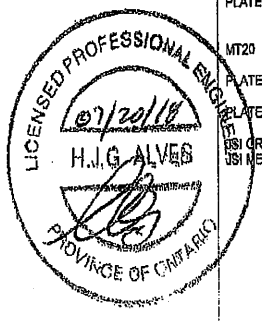
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	
B-C	-15 / 0	-84.9	-84.9	0.11 (1)	6.25	
C-D	0 / 9	-84.9	-84.9	0.04 (1)	10.00	
D-E	0 / 9	-84.9	-84.9	0.04 (1)	10.00	
E-F	0 / 13	-84.9	-84.9	0.04 (1)	10.00	
F-G	0 / 15	-84.9	-84.9	0.04 (1)	10.00	
G-H	0 / 15	-84.9	-84.9	0.04 (1)	10.00	
H-I	0 / 19	-84.9	-84.9	0.05 (1)	10.00	
I-J	0 / 12	-84.9	-84.9	0.05 (1)	10.00	
J-K	0 / 19	-84.9	-84.9	0.05 (1)	10.00	
K-L	0 / 19	-84.9	-84.9	0.05 (1)	10.00	
L-M	0 / 19	-84.9	-84.9	0.04 (1)	10.00	
M-N	0 / 19	-84.9	-84.9	0.04 (1)	10.00	
N-O	0 / 19	-84.9	-84.9	0.03 (1)	10.00	
O-P	0 / 19	-84.9	-84.9	0.04 (1)	10.00	
P-Q	0 / 19	-84.9	-84.9	0.04 (1)	10.00	
Q-R	0 / 19	-84.9	-84.9	0.04 (1)	10.00	
R-S	0 / 19	-84.9	-84.9	0.05 (1)	10.00	
S-T	0 / 19	-84.9	-84.9	0.05 (1)	10.00	
U-T	-6 / 0	0.0	0.0	0.00 (1)	7.81	
AL-B	-230 / 0	0.0	0.0	0.01 (1)	7.81	
AL-AK	0 / 0	-38.5	-38.5	0.03 (3)	10.00	
AK-AJ	-4 / 0	-38.5	-38.5	0.03 (3)	10.00	
AJ-AI	-8 / 0	-38.5	-38.5	0.02 (3)	10.00	
AI-AH	-11 / 0	-38.5	-38.5	0.02 (3)	6.25	
AH-AG	-14 / 0	-38.5	-38.5	0.02 (3)	6.25	
AG-AF	-15 / 0	-38.5	-38.5	0.02 (3)	6.25	
AF-AE	-17 / 0	-38.5	-38.5	0.02 (3)	6.25	
AE-AD	-19 / 0	-38.5	-38.5	0.02 (3)	6.25	
AD-AC	-19 / 0	-38.5	-38.5	0.02 (3)	6.25	
AC-AB	-19 / 0	-38.5	-38.5	0.02 (3)	6.25	
AB-AA	-19 / 0	-38.5	-38.5	0.03 (3)	6.25	
AA-Z	-19 / 0	-38.5	-38.5	0.03 (3)	6.25	
Z-Y	-19 / 0	-38.5	-38.5	0.02 (3)	6.25	
Y-X	-19 / 0	-38.5	-38.5	0.03 (3)	6.25	
X-W	-19 / 0	-38.5	-38.5	0.03 (3)	6.25	
W-V	-19 / 0	-38.5	-38.5	0.03 (3)	6.25	
V-U	0 / 0	-38.5	-38.5	0.03 (3)	10.00	

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X-Q	-174 / 0	0.22 (1)
Y-O	-122 / 0	0.15 (1)
Z-N	-122 / 0	0.15 (1)
AA-M	-174 / 0	0.22 (1)
AB-L	-185 / 0	0.21 (1)
AC-K	-198 / 0	0.25 (1)
AE-J	-147 / 0	0.19 (1)
AF-I	-187 / 0	0.17 (1)
AG-H	-165 / 0	0.10 (1)
AH-F	-170 / 0	0.07 (1)
AI-E	-186 / 0	0.04 (1)
AJ-D	-179 / 0	0.03 (1)
AK-C	-121 / 0	0.02 (1)
W-R	-167 / 0	0.21 (1)
V-S	-194 / 0	0.25 (1)
V-T	-73 / 0	0.10 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (V-W:3) , WB=0.25/1.00 (K-AC:1) , SSI=0.08/1.00 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.68 (V) (INPUT = 0.90)
SSI METAL= 0.08 (I) (INPUT = 1.00)

DWG NO. TAM T1802100
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T3	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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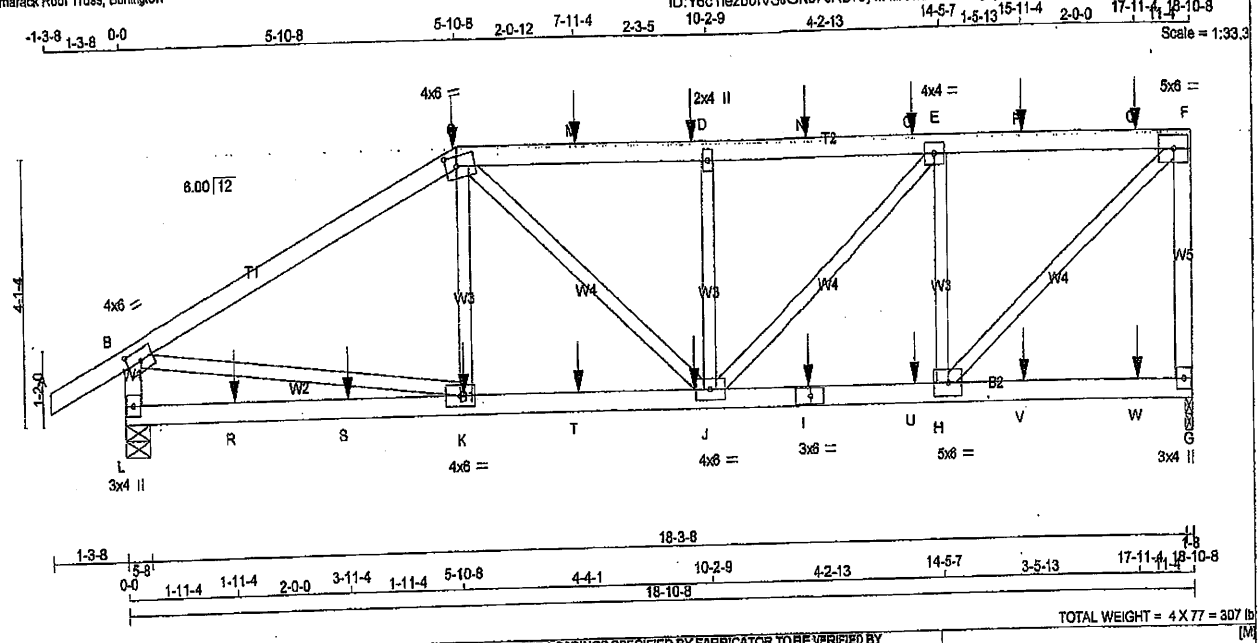

TOTAL WEIGHT = 4 X 77 = 307 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-G	1	12	TOP
L-B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L-I	1	12	SIDE(61.0)
I-G	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2263	0	2263	0	0	1-8	1-8
L	2141	0	2141	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1721	844 / 0	395 / 0	0 / 0	0 / 0	481 / 0	0 / 0
L	1626	803 / 0	369 / 0	0 / 0	0 / 0	454 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.06 (1)	10.00	
B-C	-2808 / 0	-84.9	-84.9	0.32 (1)	5.13	
C-M	-2870 / 0	-84.9	-84.9	0.22 (1)	5.18	
M-D	-2870 / 0	-84.9	-84.9	0.22 (1)	5.18	
D-N	-2870 / 0	-84.9	-84.9	0.26 (1)	5.13	
N-O	-2870 / 0	-84.9	-84.9	0.26 (1)	5.13	
O-E	-2870 / 0	-84.9	-84.9	0.26 (1)	5.13	
E-P	-2128 / 0	-84.9	-84.9	0.24 (1)	5.78	
P-Q	-2128 / 0	-84.9	-84.9	0.24 (1)	5.78	
Q-F	-2128 / 0	-84.9	-84.9	0.24 (1)	5.78	
G-F	-2117 / 0	0.0	0.0	0.29 (1)	7.62	
L-B	-1993 / 0	0.0	0.0	0.11 (1)	7.80	
L-R	0 / 0	-38.5	-38.5	0.21 (3)	10.00	
R-S	0 / 0	-38.5	-38.5	0.21 (3)	10.00	
S-K	0 / 0	-38.5	-38.5	0.21 (3)	10.00	
K-T	0 / 2508	-38.5	-38.5	0.40 (2)	10.00	
T-J	0 / 2508	-38.5	-38.5	0.40 (2)	10.00	
J-I	0 / 2128	-38.5	-38.5	0.32 (2)	10.00	
I-U	0 / 2128	-38.5	-38.5	0.32 (2)	10.00	
U-H	0 / 2128	-38.5	-38.5	0.32 (2)	10.00	
H-V	0 / 0	-38.5	-38.5	0.15 (3)	10.00	
V-W	0 / 0	-38.5	-38.5	0.15 (3)	10.00	
W-G	0 / 0	-38.5	-38.5	0.15 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	0 / 355	0.05 (3)
B-K	0 / 2531	0.31 (1)
H-F	0 / 2810	0.35 (1)
C-J	0 / 481	0.06 (1)
H-E	-1420 / 0	0.19 (1)
J-D	-703 / 0	0.09 (1)
J-E	0 / 998	0.13 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-275	-275	—	BACK	VERT	TOTAL
D	9-11-4	-100	-100	—	BACK	VERT	TOTAL
I	11-11-4	-73	-73	—	BACK	VERT	TOTAL
J	9-11-4	-73	-73	—	BACK	VERT	TOTAL
K	5-11-4	-73	-73	—	BACK	VERT	TOTAL
M	7-11-4	-100	-100	—	BACK	VERT	TOTAL
N	11-11-4	-100	-100	—	BACK	VERT	TOTAL
O	13-11-4	-100	-100	—	BACK	VERT	TOTAL
P	15-11-4	-100	-100	—	BACK	VERT	TOTAL
Q	17-11-4	-101	-101	—	BACK	VERT	TOTAL
R	1-11-4	-58	-58	—	BACK	VERT	TOTAL
S	3-11-4	-59	-59	—	BACK	VERT	TOTAL
T	7-11-4	-73	-73	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
07/20/18
R.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.40/1.00 (J-K:2) , WB=0.35/1.00 (F-H:1) , SSI=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (K) (INPUT = 0.90)
JSI METAL= 0.43 (I) (INPUT = 1.00)

DWG NO. TAM T180405
STRUCTURAL
COMPONENT ONLY

1/2

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 385.6 lbs FACTORED DOWN AT 5-10-8, 140.8 lbs FACTORED DOWN AT 7-11-4, 140.8 lbs FACTORED DOWN AT 9-11-4, 140.8 lbs FACTORED DOWN AT 11-11-4, 140.8 lbs FACTORED DOWN AT 13-11-4, AND 140.8 lbs FACTORED DOWN AT 15-11-4, AND 143.4 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 79.7 lbs FACTORED DOWN AT 1-11-4, 81.3 lbs FACTORED DOWN AT 3-11-4, 96.4 lbs FACTORED DOWN AT 5-11-4, 96.4 lbs FACTORED DOWN AT 7-11-4, 96.4 lbs FACTORED DOWN AT 9-11-4, 96.4 lbs FACTORED DOWN AT 11-11-4, 96.4 lbs FACTORED DOWN AT 13-11-4, AND 96.4 lbs FACTORED DOWN AT 15-11-4, AND 97.9 lbs FACTORED DOWN AT 17-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	13-11-4	-73	-73	—	BACK	VERT	TOTAL
V	15-11-4	-73	-73	—	BACK	VERT	TOTAL
W	17-11-4	-74	-74	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
07/20/18
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PROVINCE OF ONTARIO

DWG NO. TAM 71802105
STRUCTURAL
COMPONENT ONLY
3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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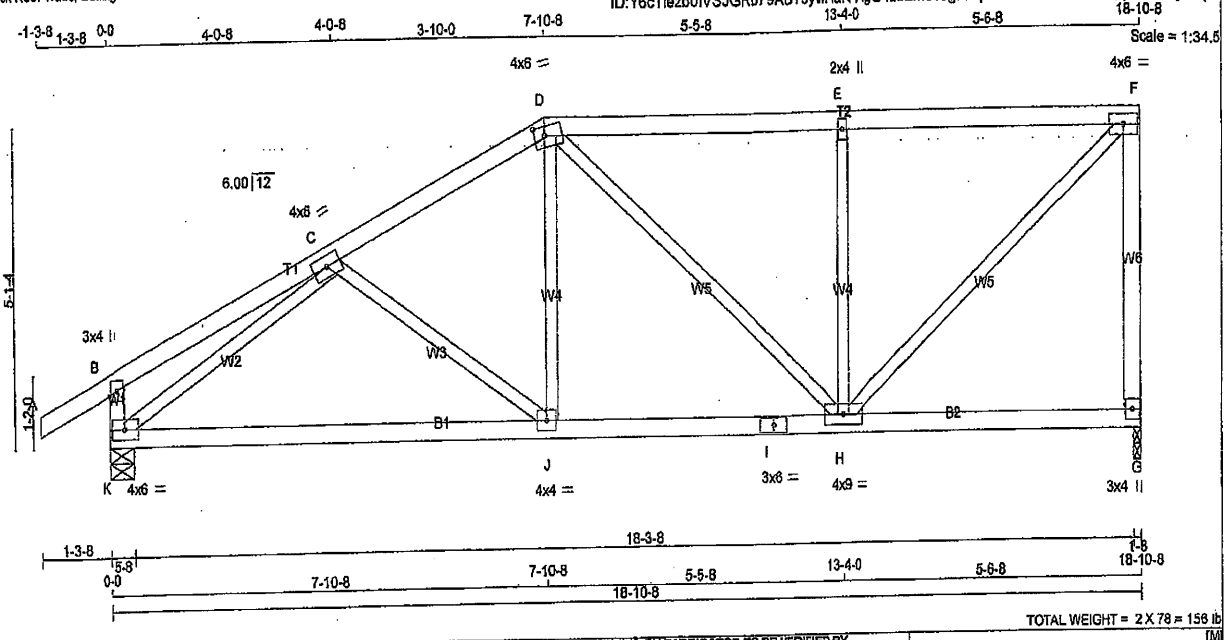

TOTAL WEIGHT = 2 X 78 = 156 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1160	0	1160	0	0	1-8	1-8
K	1284	0	1284	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	965	505 / 0	198 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX .CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 5	-84.9	-84.9	0.17 (1)	10.00
C- D	-1291 / 0	-84.9	-84.9	0.20 (1)	5.46
D- E	-1022 / 0	-84.9	-84.9	0.46 (1)	5.53
E- F	-1023 / 0	-84.9	-84.9	0.46 (1)	5.53
G- F	-1059 / 0	0.0	0.0	0.47 (1)	7.63
K- B	-272 / 0	0.0	0.0	0.03 (1)	7.61
K- J	0 / 1234	-38.5	-38.5	0.54 (2)	10.00
J- I	0 / 1149	-38.5	-38.5	0.54 (2)	10.00
I- H	0 / 1149	-38.5	-38.5	0.54 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.20 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-113 / 62	0.05 (1)
J- D	0 / 415	0.09 (2)
D- H	-182 / 0	0.18 (2)
H- E	-577 / 0	0.22 (1)
H- F	0 / 1354	0.30 (1)
K- C	-1518 / 0	0.58 (1)

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07/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 812 (0.28")

CSI: TC=0.47/1.00 (F-G:1) , BC=0.54/1.00 (J-K:2) , WB=0.58/1.00 (C-K:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM T300280

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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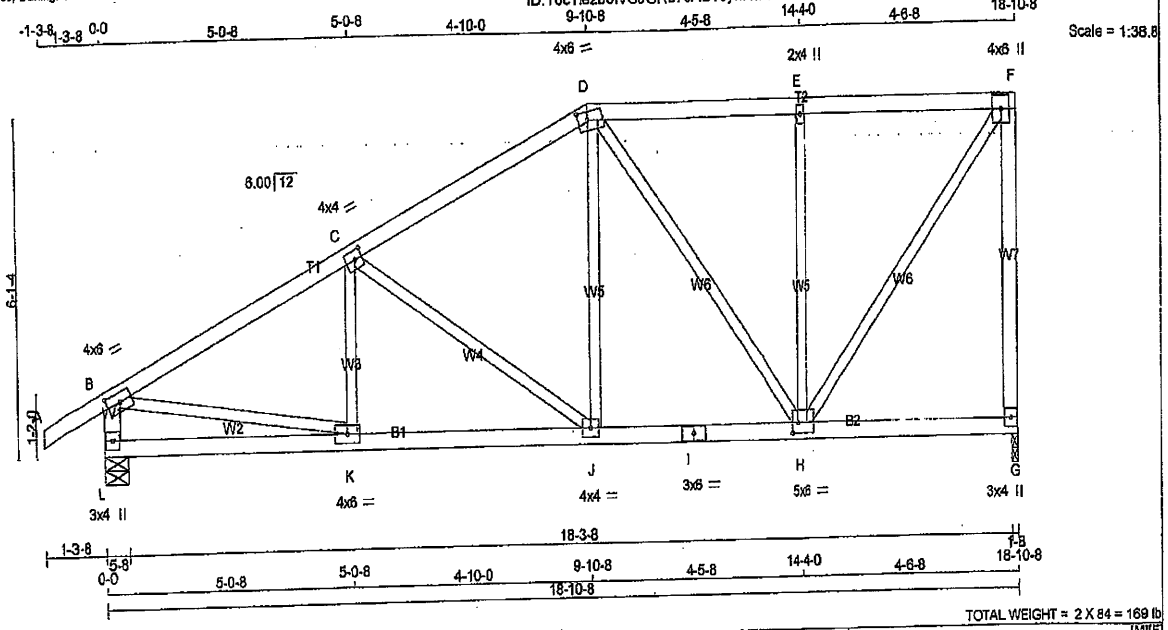

TOTAL WEIGHT = 2 X 84 = 169 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1160	0	1160	0	0	1-8	1-8
L	1284	0	1284	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
L	965	505 / 0	198 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-1493 / 0	-84.9	-84.9	0.27 (1)	5.10
C- D	-1074 / 0	-84.9	-84.9	0.25 (1)	5.80
D- E	-735 / 0	-84.9	-84.9	0.22 (1)	6.25
E- F	-735 / 0	-84.9	-84.9	0.22 (1)	6.25
G- F	-1088 / 0	0.0	0.0	0.79 (1)	7.57
L- B	-1203 / 0	0.0	0.0	0.12 (1)	7.29
L- K	0 / 0	-38.5	-38.5	0.18 (3)	10.00
K- J	0 / 1347	-38.5	-38.5	0.35 (2)	10.00
J- I	0 / 950	-38.5	-38.5	0.27 (2)	10.00
I- H	0 / 950	-38.5	-38.5	0.27 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.14 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-15 / 159	0.04 (3)
C- J	-491 / 0	0.31 (1)
J- D	0 / 462	0.10 (2)
D- H	-353 / 0	0.36 (1)
H- E	-470 / 0	0.28 (1)
H- F	0 / 1193	0.27 (1)
B- K	0 / 1366	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.79/1.00 (F-G:1) , BC=0.35/1.00 (J-K:2) , WB=0.36/1.00 (D-H:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE TYPE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T80207
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
300280	T6	2	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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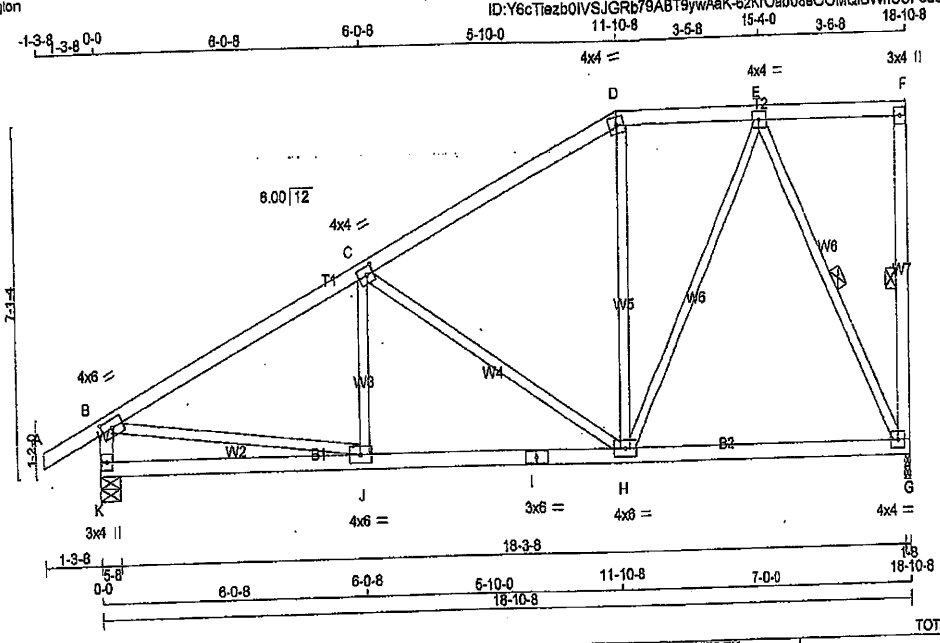


TOTAL WEIGHT = 2 X 85 = 170 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1160	0	1160	0	0	1-8	1-8
K	1264	0	1264	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	965	505 / 0	198 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

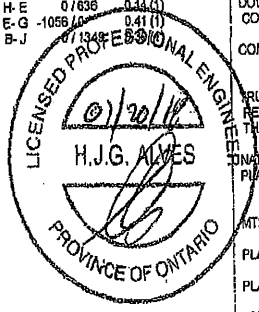
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B-C	-1474 / 0	-84.9	-84.9	0.40 (1)		4.95
C-D	-884 / 0	-84.9	-84.9	0.37 (1)		6.04
D-E	-768 / 0	-84.9	-84.9	0.14 (1)		6.25
E-F	0 / 0	-84.9	-84.9	0.17 (1)		10.00
G-F	-115 / 0	0.0	0.0	0.03 (1)		6.25
K-B	-1186 / 0	0.0	0.0	0.12 (1)		7.33
K-J	0 / 0	-38.5	-38.5	0.22 (3)		10.00
J-I	0 / 1336	-38.5	-38.5	0.49 (2)		10.00
I-H	0 / 1336	-38.5	-38.5	0.49 (2)		10.00
H-G	0 / 487	-38.5	-38.5	0.36 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	0 / 211	0.05 (3)
C-H	-680 / 0	0.66 (1)
H-D	0 / 154	0.04 (3)
H-E	0 / 636	0.44 (1)
E-G	-1056 / 0	0.41 (1)
B-J	0 / 1348	0.91 (1)



DWG NO. TAM 7180210B
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.49/1.00 (H-J:2) , WB=0.66/1.00 (C-H:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

TIONAL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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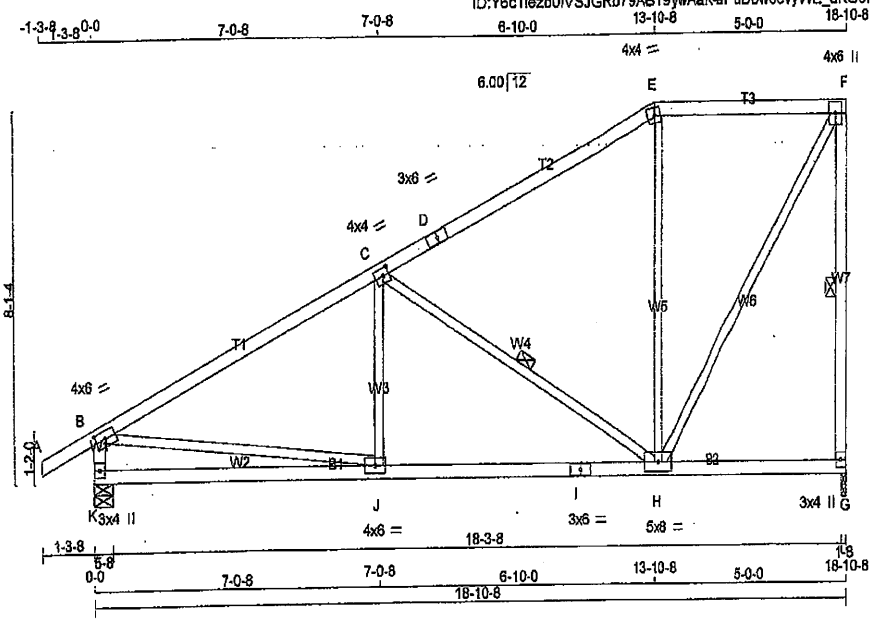

TOTAL WEIGHT = 2 X 85 = 169 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.76
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1160	0	1160	0	0	1-8	1-8
K	1284	0	1284	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	965	505 / 0	198 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	-1444 / 0	-84.9	-84.9	0.57 (1)	4.72
C-D	-650 / 0	-84.9	-84.9	0.52 (1)	6.25
D-E	-650 / 0	-84.9	-84.9	0.52 (1)	6.25
E-F	-551 / 0	-84.9	-84.9	0.27 (1)	6.25
G-F	-1090 / 0	0.0	0.0	0.32 (1)	8.05
K-B	-1175 / 0	0.0	0.0	0.12 (1)	7.35
K-J	0 / 0	-38.5	-38.5	0.36 (3)	10.00
J-I	0 / 1315	-38.5	-38.5	0.52 (2)	10.00
I-H	0 / 1315	-38.5	-38.5	0.52 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.22 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	0 / 309	0.07 (3)
C-H	-901 / 0	0.41 (1)
H-E	-156 / 72	0.20 (1)
H-F	0 / 1038	0.23 (1)
B-J	0 / 1324	0.30 (1)

LICENSED PROFESSIONAL ENGINEER
07/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1802104
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.52/1.00 (H-J:2) , WB=0.41/1.00 (C-H:1) , SSI=0.25/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (J) (INPUT = 0.80)
JSI METAL= 0.50 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T8	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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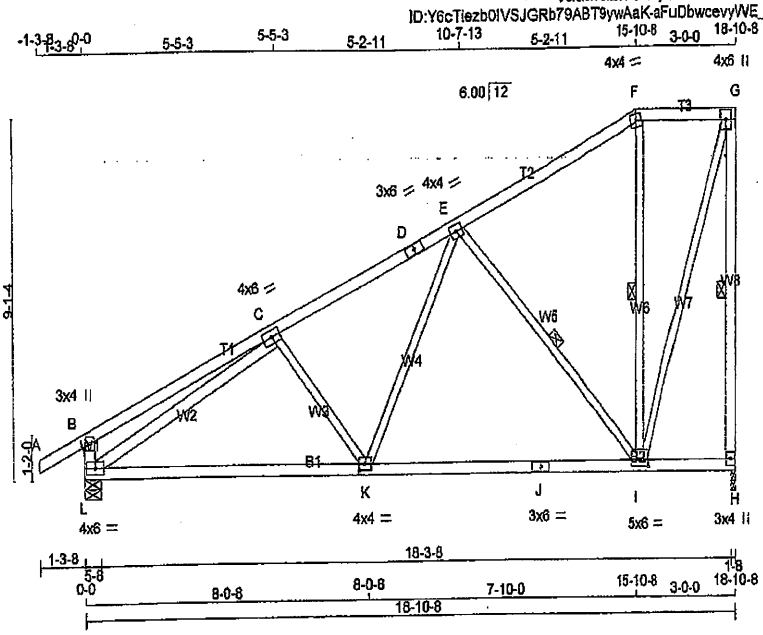

Scale = 1:54.5

TOTAL WEIGHT = 2 X 94 = 187 lb

(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	6.0	2.00	3.00
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1160	0	1160	0	0	1-8	1-8
L	1284	0	1284	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	860	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
L	965	505 / 0	198 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 10	-84.9	-84.9	0.27 (1)	10.00
C-D	-1267 / 0	-84.9	-84.9	0.28 (1)	5.41
D-E	-1267 / 0	-84.9	-84.9	0.28 (1)	5.41
E-F	-398 / 0	-84.9	-84.9	0.27 (1)	6.25
F-G	-336 / 0	-84.9	-84.9	0.10 (1)	6.25
H-G	-1148 / 0	0.0	0.0	0.44 (1)	5.94
L-B	-318 / 0	0.0	0.0	0.03 (1)	7.81
L-K	0 / 1277	-38.5	-38.5	0.64 (2)	10.00
K-J	0 / 909	-38.5	-38.5	0.59 (2)	10.00
J-I	0 / 909	-38.5	-38.5	0.59 (2)	10.00
I-H	0 / 0	-38.5	-38.5	0.24 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-K	-233 / 20	0.08 (1)
K-E	0 / 614	0.14 (2)
E-I	-882 / 0	0.39 (1)
I-F	-140 / 38	0.08 (1)
I-G	0 / 1075	0.24 (1)
L-C	-1531 / 0	0.78 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 953 (0.24")

CSI: TC=0.44/1.00 (G-H:1) , BC=0.64/1.00 (K-L:2) , WB=0.78/1.00 (C-L:1) , SSI=0.20/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

FINAL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.60 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T8002410
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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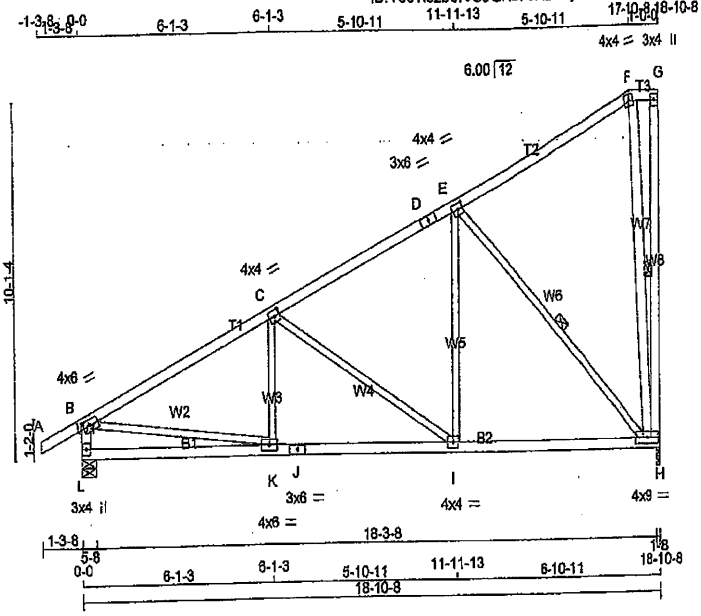

TOTAL WEIGHT = 2 X 96 = 191 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTW-m	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVWW1-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1160	0	1160	0	0	1-8	1-8
L	1284	0	1284	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
L	985	505 / 0	198 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, E-H, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	-1457 / 0	-84.9	-84.9	0.37 (1)	5.03
C-D	-900 / 0	-84.9	-84.9	0.33 (1)	6.07
D-E	-900 / 0	-84.9	-84.9	0.33 (1)	6.07
E-F	-49 / 0	-84.9	-84.9	0.34 (1)	6.25
F-G	0 / 0	-84.9	-84.9	0.01 (1)	10.00
H-G	-42 / 0	0.0	0.0	0.02 (1)	6.25
L-B	-1185 / 0	0.0	0.0	0.12 (1)	7.34
L-K	0 / 0	-38.5	-38.5	0.23 (3)	10.00
K-J	0 / 1316	-38.5	-38.5	0.48 (2)	10.00
J-I	0 / 1316	-38.5	-38.5	0.48 (2)	10.00
I-H	0 / 806	-38.5	-38.5	0.40 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	0 / 214	0.05 (3)
C-I	-611 / 0	0.58 (1)
I-E	0 / 856	0.15 (2)
E-H	-1104 / 0	0.68 (1)
F-H	-232 / 0	0.13 (1)
B-K	0 / 1329	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.48/1.00 (I-K:2) , WB=0.68/1.00 (E-H:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

FINAL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.49 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1802411
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T10	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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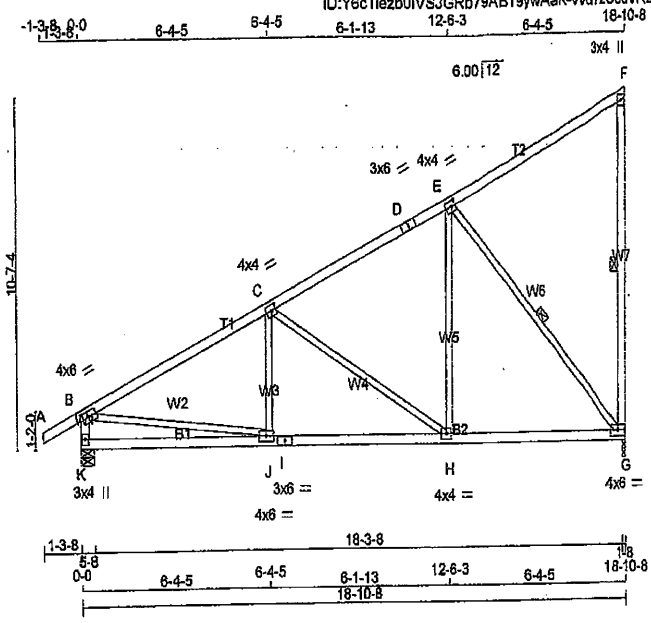

Scale = 1:65.2

TOTAL WEIGHT = 8 X 88 = 518 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1160	0	1160	0	0	1-8	1-8
K	1284	0	1284	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	965	505 / 0	198 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM TO	MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 26	-84.9 -84.9	0.11 (1)	10.00	J-C	0 / 242	0.08 (3)
B-C	-1449 / 0	-84.9 -84.9	0.41 (1)	4.99	C-H	-664 / 0	0.71 (1)
C-D	-841 / 0	-84.9 -84.9	0.37 (1)	6.15	H-E	0 / 666	0.15 (2)
D-E	-841 / 0	-84.9 -84.9	0.37 (1)	6.15	E-G	-1132 / 0	0.71 (1)
E-F	-24 / 0	-84.9 -84.9	0.40 (1)	8.25	B-J	0 / 1322	0.39 (1)
G-F	-217 / 0	0.0 0.0	0.12 (1)	8.25			
K-B	-1162 / 0	0.0 0.0	0.12 (1)	7.34			
K-J	0 / 0	-38.5 -38.5	0.26 (3)	10.00			
J-I	0 / 1310	-38.5 -38.5	0.45 (2)	10.00			
I-H	0 / 1310	-38.5 -38.5	0.45 (2)	10.00			
H-G	0 / 755	-38.5 -38.5	0.37 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.45/1.00 (H-J:2) , WB=0.71/1.00 (E-G:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.61 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 718021/12
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T11	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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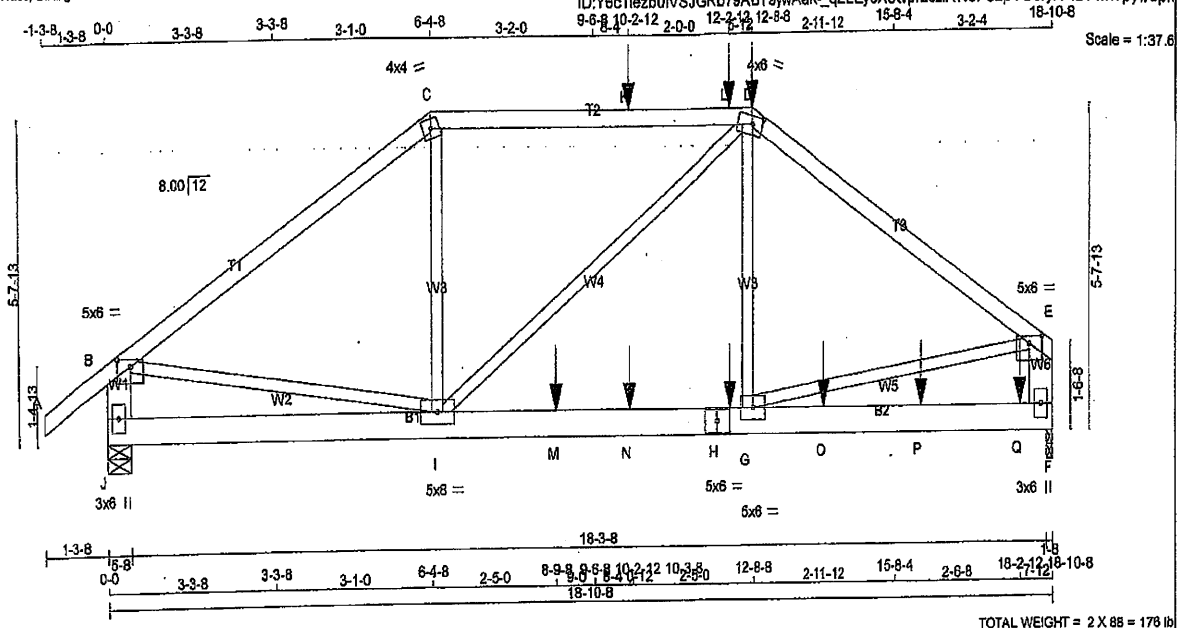

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-D	1	12	SIDE(61.0)
D-E	1	12	SIDE(61.0)
J-B	2	12	TOP
F-E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J-H	2	12	SIDE(0.0)
H-F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMVW-p	MT20	5.0	6.0	1.50	3.00
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	5.0	6.0		
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	2281	0	2281	0	0	5-8	5-8
F	2450	0	2450	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1704	928 / 0	322 / 0	0 / 0	0 / 0	454 / 0	0 / 0
F	1852	942 / 0	401 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00		B-I	0 / 2292	0.28 (1)
B-C	-2734 / 0	-84.9	-84.9	0.40 (1)	5.11		I-C	0 / 928	0.11 (1)
C-K	-2299 / 0	-84.9	-84.9	0.56 (1)	5.10		G-D	0 / 848	0.08 (2)
K-L	-2299 / 0	-84.9	-84.9	0.56 (1)	5.10		G-E	0 / 2440	0.30 (1)
L-D	-2299 / 0	-84.9	-84.9	0.56 (1)	5.10		I-D	-148 / 0	0.02 (1)
D-E	-2889 / 0	-84.9	-84.9	0.42 (1)	4.98				
J-B	-2260 / 0	0.0	0.0	0.08 (1)	7.81				
F-E	-2303 / 0	0.0	0.0	0.08 (1)	7.81				
J-I	0 / 0	-38.5	-38.5	0.25 (1)	10.00				
I-M	0 / 2412	-38.5	-38.5	0.64 (1)	10.00				
M-N	0 / 2412	-38.5	-38.5	0.64 (1)	10.00				
N-H	0 / 2412	-38.5	-38.5	0.64 (1)	10.00				
H-G	0 / 2412	-38.5	-38.5	0.64 (1)	10.00				
G-O	0 / 0	-38.5	-38.5	0.25 (2)	10.00				
O-P	0 / 0	-38.5	-38.5	0.25 (2)	10.00				
P-Q	0 / 0	-38.5	-38.5	0.25 (2)	10.00				
Q-F	0 / 0	-38.5	-38.5	0.25 (2)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	12-8-8	-59	-59	—	FRONT	VERT	DEAD
D	12-8-8	-229	-229	—	FRONT	VERT	SNOW
H	12-2-12	-57	-57	—	FRONT	VERT	TOTAL
K	10-2-12	-94	-94	—	FRONT	VERT	TOTAL
L	12-2-12	-92	-92	—	FRONT	VERT	TOTAL
M	6-9-8	-917	-917	—	FRONT	VERT	TOTAL
N	10-2-12	-70	-70	—	FRONT	VERT	TOTAL
O	14-2-12	-57	-57	—	FRONT	VERT	TOTAL
P	16-2-12	-57	-57	—	FRONT	VERT	TOTAL
Q	18-2-12	-76	-76	—	FRONT	VERT	TOTAL

DWG NO. TAM 11602/13
STRUCTURAL COMPONENT ONLY

1/2

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.56/1.00 (C-D:1) , BC=0.64/1.00 (G-I:1) , WB=0.30/1.00 (E-G:1) , SSI=0.31/1.00 (G-I:1)

BOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T11	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 417.3 lbs FACTORED DOWN AT 12-8-8, AND 133.6 lbs FACTORED DOWN AT 10-2-12, AND 130.0 lbs FACTORED DOWN AT 12-2-12 ON TOP CHORD, AND 1243.8 lbs FACTORED DOWN AT 8-9-8, 93.6 lbs FACTORED DOWN AT 10-2-12, 76.8 lbs FACTORED DOWN AT 12-2-12, 78.8 lbs FACTORED DOWN AT 14-2-12, AND 78.8 lbs FACTORED DOWN AT 16-2-12, AND 102.3 lbs FACTORED DOWN AT 18-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.33 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 71802413
STRUCTURAL COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T12	1	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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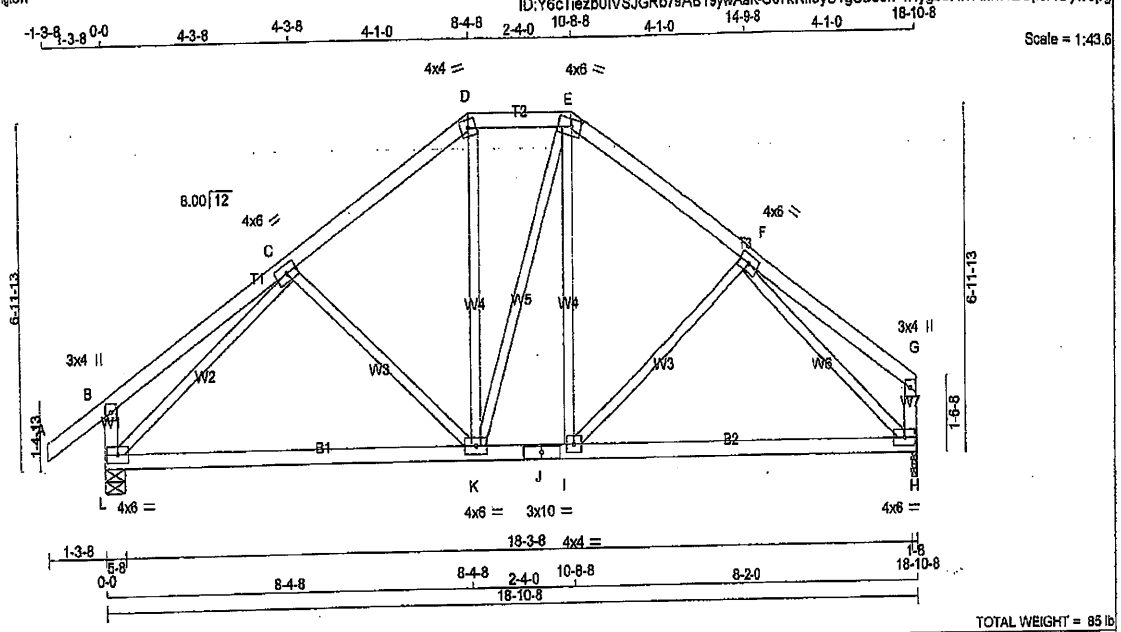

TOTAL WEIGHT = 85 lb

[M|F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	10.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1285	0	1285	0	0	5-8	5-8
H	1160	0	1160	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	966	506 / 0	198 / 0	0 / 0	0 / 0	282 / 0	0 / 0
H	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 9	-84.9	-84.9	0.21 (1)	10.00
C-D	-1030 / 0	-84.9	-84.9	0.18 (1)	5.99
D-E	-844 / 0	-84.9	-84.9	0.07 (1)	6.25
E-F	-1024 / 0	-84.9	-84.9	0.16 (1)	5.99
F-G	0 / 23	-84.9	-84.9	0.22 (1)	10.00
L-B	-277 / 0	0.0	0.0	0.03 (1)	7.81
H-G	-128 / 0	0.0	0.0	0.01 (1)	7.81
L-K	0 / 945	-38.5	-38.5	0.67 (2)	10.00
K-J	0 / 837	-38.5	-38.5	0.56 (2)	10.00
J-I	0 / 837	-38.5	-38.5	0.56 (2)	10.00
I-H	0 / 925	-38.5	-38.5	0.53 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
K-E	0 / 22	0.01 (2)
I-E	0 / 315	0.07 (2)
I-F	-124 / 48	0.07 (1)
L-C	-1288 / 0	0.69 (1)
F-H	-1291 / 0	0.65 (1)
K-D	0 / 338	0.08 (2)
K-C	-143 / 36	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 669 (0.33")

CSI: TC=0.22/1.00 (F-G:1) , BC=0.57/1.00 (K-L:2) , WB=0.69/1.00 (C-L:1) , SSI=0.20/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	769	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (E) (INPUT = 0.90)
JSI METAL= 0.86 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 1180 2/14
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T13	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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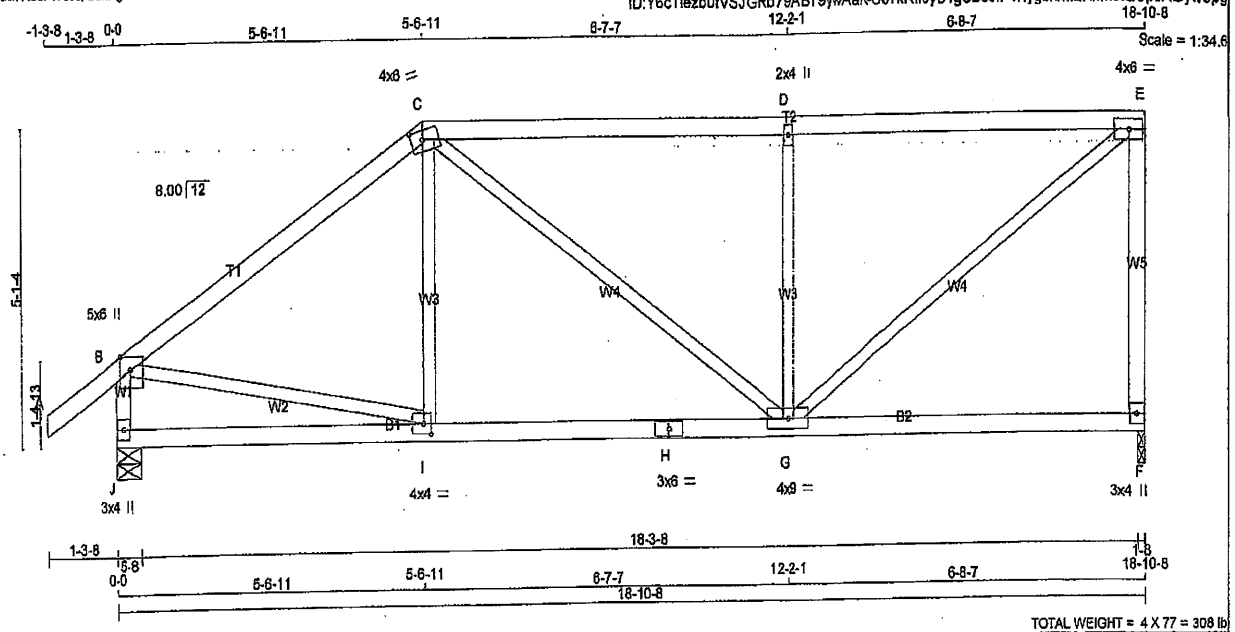


TOTAL WEIGHT = 4 X 77 = 308 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1160	0	1160	0	0	1-8	1-8
J	1285	0	1285	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	966	506 / 0	198 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

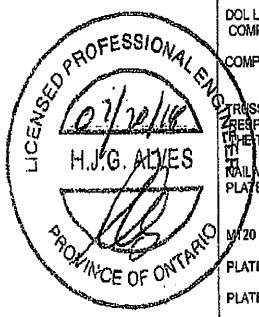
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	I-C	0 / 219	0.05 (3)
B-C	-1172 / 0	-84.9	-84.9	0.48 (1)	5.29	C-G	0 / 246	0.06 (1)
C-D	-1168 / 0	-84.9	-84.9	0.71 (1)	4.75	G-D	-699 / 0	0.27 (1)
D-E	-1168 / 0	-84.9	-84.9	0.71 (1)	4.75	G-E	0 / 1438	0.32 (1)
F-E	-1055 / 0	0.0	0.0	0.47 (1)	7.66	B-I	0 / 985	0.22 (1)
J-B	-1199 / 0	0.0	0.0	0.12 (1)	7.30			
J-I	0 / 0	-38.5	-38.5	0.24 (3)	10.00			
I-H	0 / 969	-38.5	-38.5	0.43 (2)	10.00			
H-G	0 / 969	-38.5	-38.5	0.43 (2)	10.00			
G-F	0 / 0	-38.5	-38.5	0.31 (3)	10.00			



DWG NO. TAM M802115
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.71/1.00 (D-E:1) , BC=0.43/1.00 (G-I:2) , WB=0.32/1.00 (E-G:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T14	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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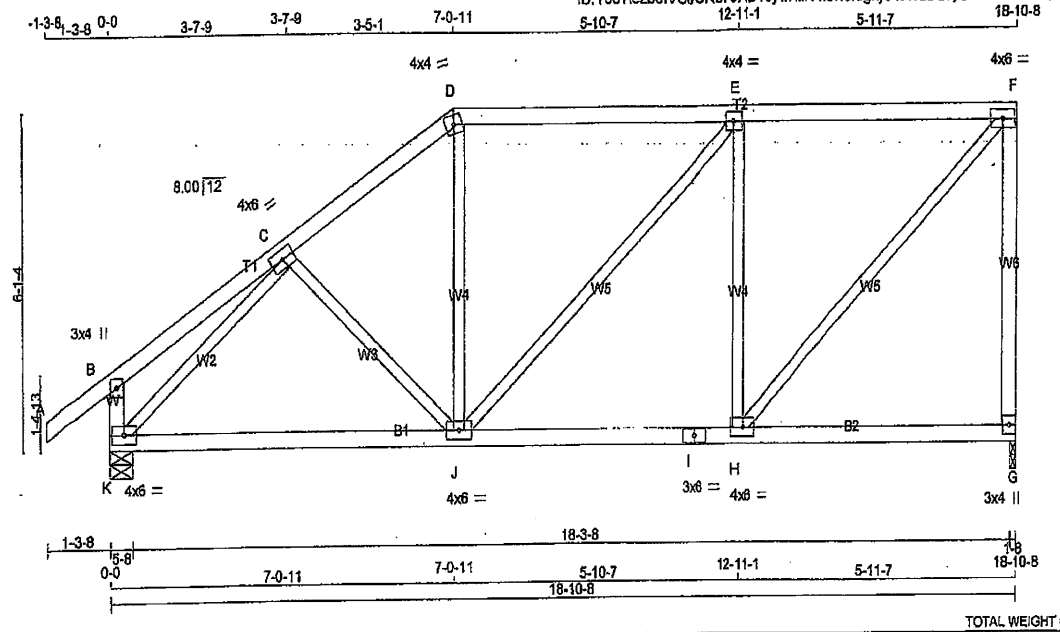

TOTAL WEIGHT = 4 X 83 = 333 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW-m	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	4.0	
F	TMVW-t	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMWW-t	MT20	4.0	6.0	
I	BS-t	MT20	3.0	6.0	
J	BMWWW-t	MT20	4.0	6.0	
K	BMVW1-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1160	0	1160	0	0	1-8	1-8
K	1285	0	1285	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	966	506 / 0	198 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C-J	-40 / 68	0.02 (1)
B-C	0 / 2	-84.9	-84.9	0.13 (1)	10.00	J-D	0 / 324	0.07 (2)
C-D	-1105 / 0	-84.9	-84.9	0.15 (1)	5.85	J-E	0 / 58	0.01 (3)
D-E	-910 / 0	-84.9	-84.9	0.53 (1)	5.63	H-E	-638 / 0	0.37 (1)
E-F	-893 / 0	-84.9	-84.9	0.53 (1)	5.66	H-F	0 / 1248	0.28 (1)
G-F	-1064 / 0	0.0	0.0	0.77 (1)	7.64	K-C	-1304 / 0	0.50 (1)
K-B	-262 / 0	0.0	0.0	0.03 (1)	7.81			
K-J	0 / 934	-38.5	-38.5	0.44 (2)	10.00			
J-I	0 / 893	-38.5	-38.5	0.43 (2)	10.00			
I-H	0 / 893	-38.5	-38.5	0.43 (2)	10.00			
H-G	0 / 0	-38.5	-38.5	0.21 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.77/1.00 (F-G:1) , BC=0.44/1.00 (J-K:2) , WB=0.50/1.00 (C-K:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
PROVINCE OF ONTARIO
07/20/18

DWG NO. TAM 1802116
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T15	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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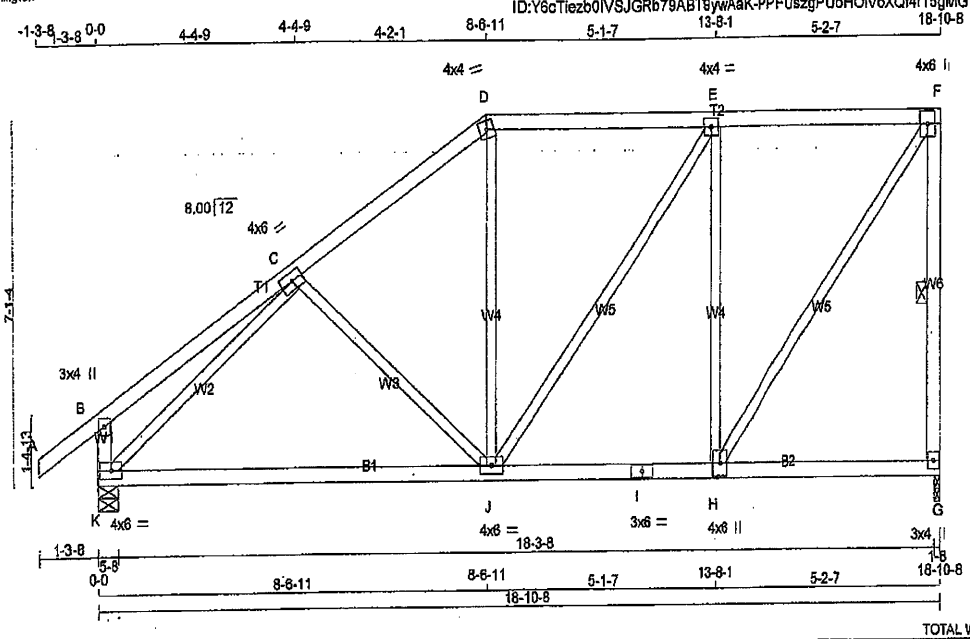


TOTAL WEIGHT = 4 X 88 = 353 lb [M]

LUMBER
N. L, G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1160	0	1160	0	0	1-8	1-8
K	1285	0	1285	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	966	506 / 0	198 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

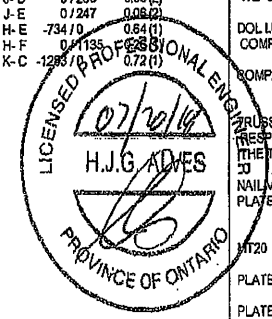
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 10	-84.9	-84.9	0.22 (1)	10.00
C-D	-1016 / 0	-84.9	-84.9	0.24 (1)	5.91
D-E	-831 / 0	-84.9	-84.9	0.39 (1)	6.09
E-F	-689 / 0	-84.9	-84.9	0.39 (1)	6.25
G-F	-1071 / 0	0.0	0.0	0.24 (1)	6.10
K-B	-279 / 0	0.0	0.0	0.03 (1)	7.81
K-J	0 / 944	-38.5	-38.5	0.57 (2)	10.00
J-I	0 / 689	-38.5	-38.5	0.54 (2)	10.00
I-H	0 / 689	-38.5	-38.5	0.54 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.19 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-157 / 32	0.09 (1)
J-D	0 / 280	0.06 (2)
J-E	0 / 247	0.06 (2)
H-E	-734 / 0	0.64 (1)
H-F	0 / 1135	0.98 (1)
K-C	-1293 / 0	0.72 (1)



DWG NO. TAM 1802117
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 615 (0.37")

CSI: TC=0.39/1.00 (D-E:1) , BC=0.57/1.00 (J-K:2) , WB=0.72/1.00 (C-K:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (K) (INPUT = 0.90)
JSI METAL= 0.31 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T16	16	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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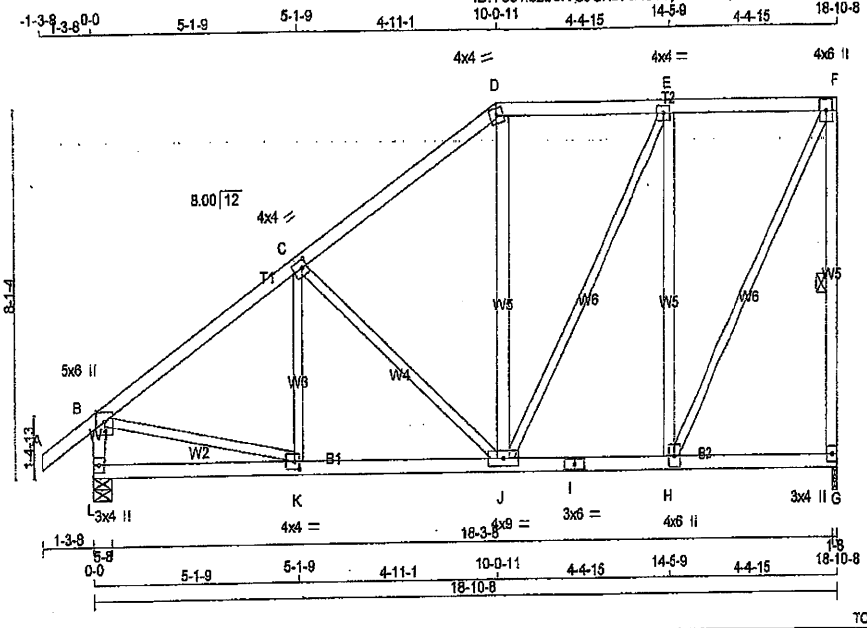


TOTAL WEIGHT = 16 X 101 = 1610 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1160	0	1160	0	0	1-8	1-8
L	1285	0	1285	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
L	986	508 / 0	198 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

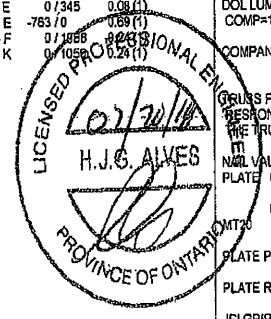
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1218 / 0	-84.9	-84.9	0.28 (1)	5.50
C- D	-886 / 0	-84.9	-84.9	0.26 (1)	6.25
D- E	-700 / 0	-84.9	-84.9	0.21 (1)	6.25
E- F	-535 / 0	-84.9	-84.9	0.21 (1)	6.25
G- F	-1090 / 0	0.0	0.0	0.32 (1)	8.05
L- B	-1205 / 0	0.0	0.0	0.12 (1)	7.29
L- K	0 / 0	-38.5	-38.5	0.19 (3)	10.00
K- J	0 / 1028	-38.5	-38.5	0.32 (2)	10.00
J- I	0 / 535	-38.5	-38.5	0.20 (2)	10.00
I- H	0 / 535	-38.5	-38.5	0.20 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.13 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 174	0.04 (3)
C- J	-442 / 0	0.37 (1)
J- D	0 / 197	0.03 (3)
J- E	0 / 345	0.08 (1)
H- E	-783 / 0	0.69 (1)
H- F	0 / 1068	0.24 (1)
B- K	0 / 1058	0.29 (1)



DWG NO. TAM 7180218
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT. CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.32/1.00 (F-G:1) , BC=0.32/1.00 (J-K:2) , WB=0.69/1.00 (E-H:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T17	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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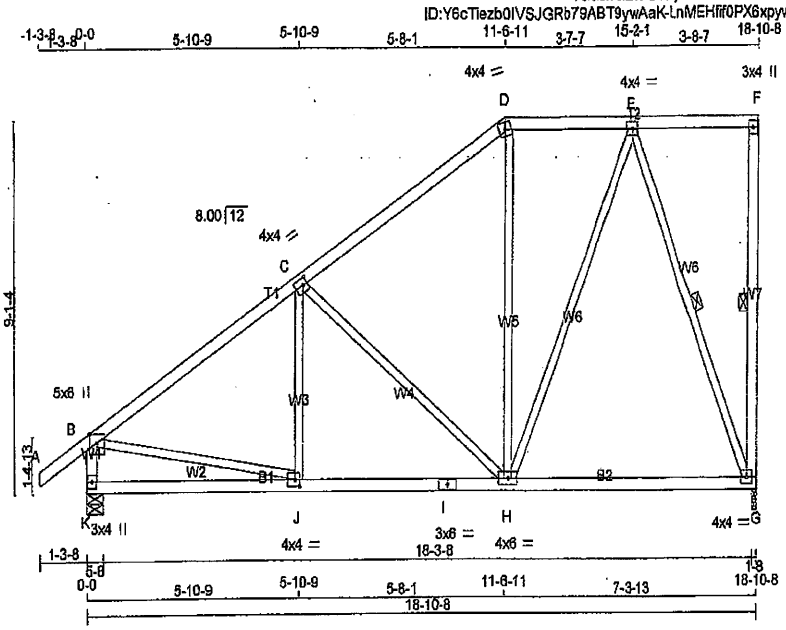


TOTAL WEIGHT = 4 X 95 = 379 lb

(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.00 1.50
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1160	0	1160	0	0	1-8	1-8
K	1285	0	1285	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	968	509 / 0	198 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

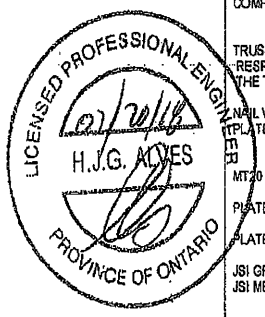
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1200 / 0	-84.9	-84.9	0.37 (1)	5.40
C-D	-759 / 0	-84.9	-84.9	0.35 (1)	6.25
D-E	-605 / 0	-84.9	-84.9	0.15 (1)	6.25
E-F	0 / 0	-84.9	-84.9	0.19 (1)	10.00
G-F	-120 / 0	0.0	0.0	0.05 (1)	6.25
K-B	-1190 / 0	0.0	0.0	0.12 (1)	7.32
K-J	0 / 0	-38.5	-38.5	0.21 (3)	10.00
J-I	0 / 1018	-38.5	-38.5	0.46 (2)	10.00
I-H	0 / 1018	-38.5	-38.5	0.46 (2)	10.00
H-G	0 / 389	-38.5	-38.5	0.37 (2)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	0 / 198	0.05 (3)
C-H	-547 / 0	0.64 (1)
H-D	0 / 165	0.04 (3)
H-E	0 / 585	0.13 (1)
E-G	-1004 / 0	0.64 (1)
B-J	0 / 1035	0.23 (1)



DWG NO. TAM 1180219
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.46/1.00 (H-J:2) , WB=0.64/1.00 (C-H:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (J) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T18	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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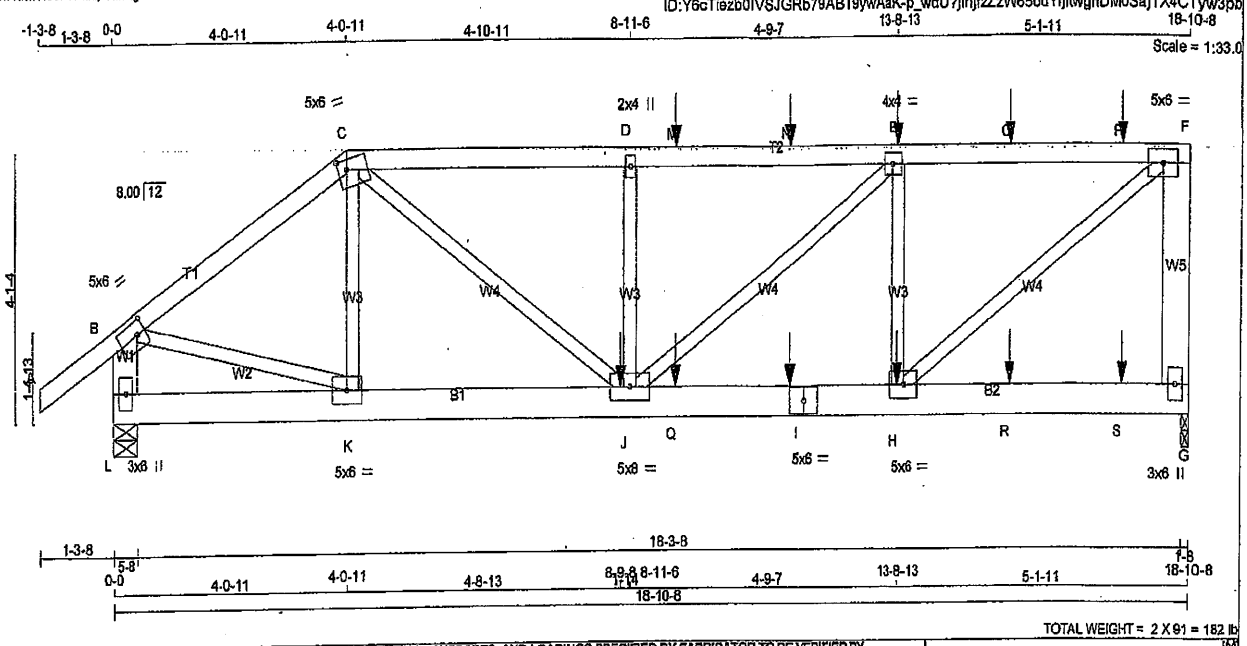


TOTAL WEIGHT = 2 X 91 = 182 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- G	2	12	TOP
L- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	2	12	SIDE(183.1)
I- G	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
J- D	1	6	SIDE(141.4)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	1.75
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2572	0	2572	0	0	1-8	1-8
L	2246	0	2246	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1942	993 / 0	416 / 0	0 / 0	0 / 0	533 / 0	0 / 0
L	1683	900 / 0	330 / 0	0 / 0	0 / 0	452 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00	K- C	-263 / 11	0.03 (1)
B- C	-2499 / 0	-84.9	-84.9	0.13 (1)	5.59	B- K	0 / 2125	0.26 (1)
C- D	-3797 / 0	-84.9	-84.9	0.23 (1)	4.84	H- F	0 / 3568	0.44 (1)
D- M	-3797 / 0	-84.9	-84.9	0.37 (1)	4.46	C- J	0 / 2182	0.27 (1)
M- N	-3797 / 0	-84.9	-84.9	0.37 (1)	4.46	H- E	-1602 / 0	0.20 (1)
N- E	-3797 / 0	-84.9	-84.9	0.37 (1)	4.46	J- D	-582 / 0	0.07 (1)
E- O	-2868 / 0	-84.9	-84.9	0.34 (1)	5.01	J- E	-1152 / 0	0.14 (1)
O- P	-2868 / 0	-84.9	-84.9	0.34 (1)	5.01			
P- F	-2868 / 0	-84.9	-84.9	0.34 (1)	5.01			
G- F	-2403 / 0	0.0	0.0	0.19 (1)	7.81			
L- B	-2189 / 0	0.0	0.0	0.08 (1)	7.81			
L- K	0 / 0	-38.5	-38.5	0.04 (2)	10.00			
K- J	0 / 2060	-38.5	-38.5	0.18 (1)	10.00			
J- Q	0 / 2888	-38.5	-38.5	0.26 (1)	10.00			
Q- I	0 / 2888	-38.5	-38.5	0.26 (1)	10.00			
I- H	0 / 2888	-38.5	-38.5	0.26 (1)	10.00			
H- R	0 / 0	-38.5	-38.5	0.08 (2)	10.00			
R- S	0 / 0	-38.5	-38.5	0.08 (2)	10.00			
S- G	0 / 0	-38.5	-38.5	0.08 (2)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	13-8-12	-100	-100	--	BACK	VERT	TOTAL
H	13-8-12	-73	-73	--	BACK	VERT	TOTAL
I	11-8-12	-73	-73	--	BACK	VERT	TOTAL
J	8-9-8	-917	-917	--	BACK	VERT	TOTAL
M	9-8-12	-100	-100	--	BACK	VERT	TOTAL
N	11-8-12	-100	-100	--	BACK	VERT	TOTAL
O	15-8-12	-100	-100	--	BACK	VERT	TOTAL
P	17-8-12	-100	-100	--	BACK	VERT	TOTAL
Q	9-8-12	-73	-73	--	BACK	VERT	TOTAL
R	15-8-12	-73	-73	--	BACK	VERT	TOTAL
S	17-8-12	-73	-73	--	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD] EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.37/1.00 (D-E:1) , BC=0.26/1.00 (H-J:1) , WB=0.44/1.00 (F-H:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL= 0.37 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 1800220
STRUCTURAL COMPONENT

1/2

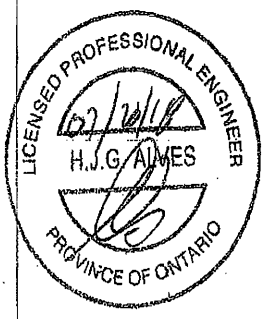
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T18	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 140.8 lbs FACTORED DOWN AT 9-8-12, 140.8 lbs FACTORED DOWN AT 11-8-12, 140.8 lbs FACTORED DOWN AT 13-8-12, AND 140.8 lbs FACTORED DOWN AT 15-8-12, AND 140.8 lbs FACTORED DOWN AT 17-8-12 ON TOP CHORD, AND 1243.8 lbs FACTORED DOWN AT 8-9-8, 98.4 lbs FACTORED DOWN AT 9-8-12, 98.4 lbs FACTORED DOWN AT 11-8-12, 98.4 lbs FACTORED DOWN AT 13-8-12, AND 98.4 lbs FACTORED DOWN AT 15-8-12, AND 98.4 lbs FACTORED DOWN AT 17-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 1180220
STRUCTURAL
COMPONENT ONLY

2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300283	T18Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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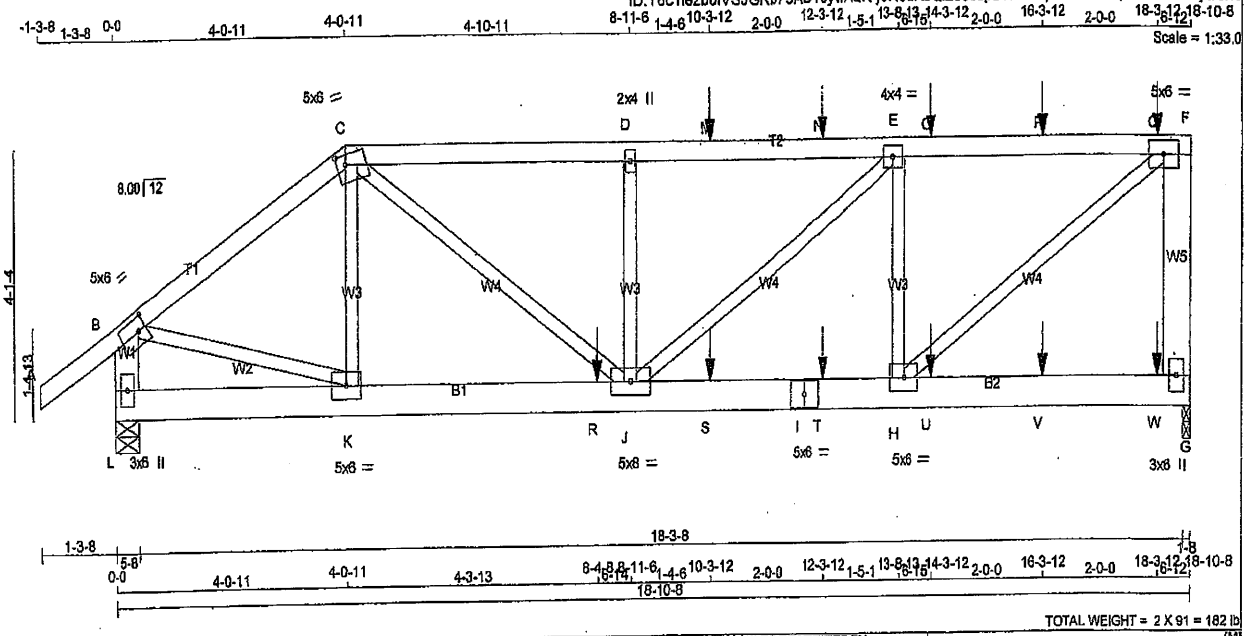

TOTAL WEIGHT = 2 X 91 = 182 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-F	1	12	SIDE(61.0)
F-G	2	12	TOP
L-B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L-I	2	12	SIDE(0.0)
I-G	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2599	0	2599	0	0	1-8	1-8
L	2244	0	2244	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1956	997 / 0	421 / 0	0 / 0	0 / 0	537 / 0	0 / 0
L	1680	802 / 0	327 / 0	0 / 0	0 / 0	451 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00	
B-C	-2529 / 0	-84.9	-84.9	0.13 (1)	5.57	
C-D	-3682 / 0	-84.9	-84.9	0.22 (1)	4.71	
D-M	-3682 / 0	-84.9	-84.9	0.39 (1)	4.48	
M-N	-3682 / 0	-84.9	-84.9	0.39 (1)	4.48	
N-E	-3682 / 0	-84.9	-84.9	0.39 (1)	4.48	
E-O	-2769 / 0	-84.9	-84.9	0.36 (1)	5.06	
O-P	-2769 / 0	-84.9	-84.9	0.36 (1)	5.06	
P-Q	-2769 / 0	-84.9	-84.9	0.36 (1)	5.06	
Q-F	-2769 / 0	-84.9	-84.9	0.36 (1)	5.06	
G-F	-2375 / 0	0.0	0.0	0.19 (1)	7.81	
L-B	-2211 / 0	0.0	0.0	0.08 (1)	7.81	
L-K	0 / 0	-38.5	-38.5	0.07 (1)	10.00	
K-R	0 / 2087	-38.5	-38.5	0.30 (1)	10.00	
R-J	0 / 2087	-38.5	-38.5	0.30 (1)	10.00	
J-S	0 / 2769	-38.5	-38.5	0.26 (1)	10.00	
S-I	0 / 2769	-38.5	-38.5	0.26 (1)	10.00	
I-T	0 / 2769	-38.5	-38.5	0.26 (1)	10.00	
T-H	0 / 2769	-38.5	-38.5	0.26 (1)	10.00	
H-U	0 / 0	-38.5	-38.5	0.10 (2)	10.00	
U-V	0 / 0	-38.5	-38.5	0.10 (2)	10.00	
V-W	0 / 0	-38.5	-38.5	0.10 (2)	10.00	
W-G	0 / 0	-38.5	-38.5	0.10 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
K-C	-138 / 63	0.02 (1)
B-K	0 / 2150	0.27 (1)
H-F	0 / 3421	0.42 (1)
C-J	0 / 2002	0.25 (1)
H-E	-1607 / 0	0.20 (1)
J-D	-541 / 0	0.07 (1)
J-E	0 / 1157	0.14 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	10-3-12	-100	-100	--	FRONT	VERT	TOTAL
N	12-3-12	-100	-100	--	FRONT	VERT	TOTAL
O	14-3-12	-100	-100	--	FRONT	VERT	TOTAL
P	16-3-12	-100	-100	--	FRONT	VERT	TOTAL
Q	18-3-12	-112	-112	--	FRONT	VERT	TOTAL
R	8-4-8	-908	-908	--	FRONT	VERT	TOTAL
S	10-3-12	-73	-73	--	FRONT	VERT	TOTAL
T	12-3-12	-73	-73	--	FRONT	VERT	TOTAL
U	14-3-12	-73	-73	--	FRONT	VERT	TOTAL
V	16-3-12	-73	-73	--	FRONT	VERT	TOTAL
W	18-3-12	-80	-80	--	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
07/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 699 (0.09")

CSI: TC=0.39/1.00 (D-E:1) , BC=0.30/1.00 (J-K:1) , WB=0.42/1.00 (F-H:1) , SSI=0.39/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.36 (F) (INPUT = 1.00)

DWG NO. TAM 1180425
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300283	T18Z	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 140.8 lbs FACTORED DOWN AT 10-3-12, 140.8 lbs FACTORED DOWN AT 12-3-12, 140.8 lbs FACTORED DOWN AT 14-3-12, AND 140.8 lbs FACTORED DOWN AT 16-3-12, AND 159.3 lbs FACTORED DOWN AT 18-3-12 ON TOP CHORD, AND 1232.1 lbs FACTORED DOWN AT 8-4-8, 96.4 lbs FACTORED DOWN AT 10-3-12, 96.4 lbs FACTORED DOWN AT 12-3-12, 96.4 lbs FACTORED DOWN AT 14-3-12, AND 96.4 lbs FACTORED DOWN AT 16-3-12, AND 107.1 lbs FACTORED DOWN AT 18-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
H.J.G. AYRES
PROVINCE OF ONTARIO

DWG NO. TAM T1802125
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T19	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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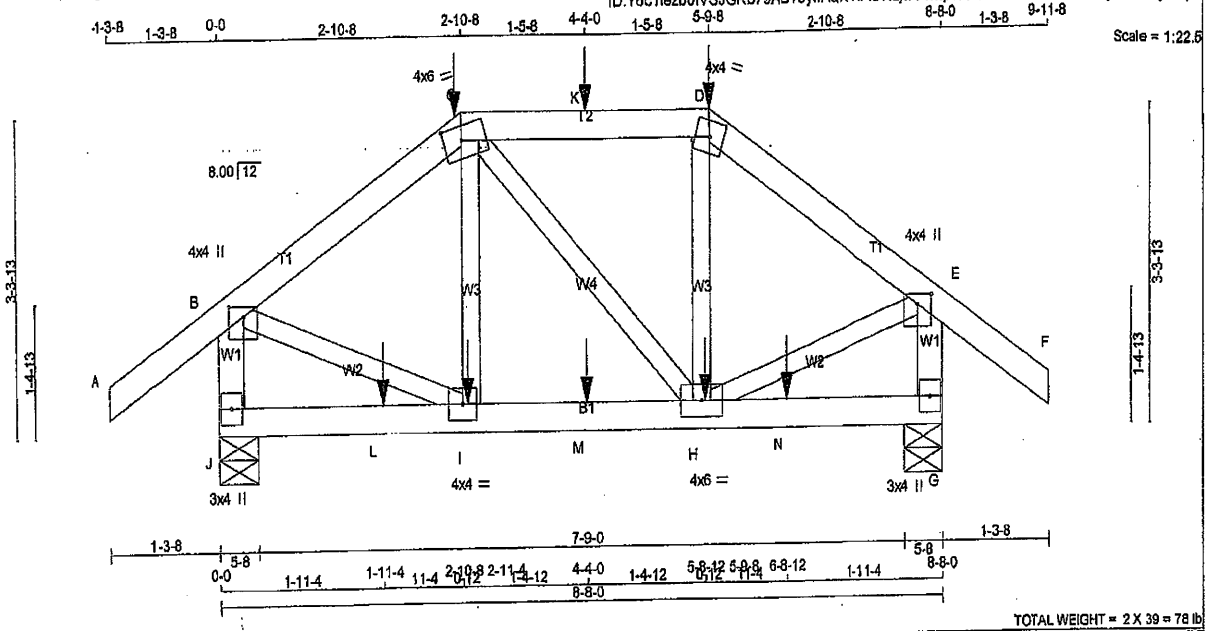

TOTAL WEIGHT = 2 X 39 = 78 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 23.8 lbs FACTORED DOWN AND 244.0 lbs FACTORED UP AT 2-10-8, AND 12.6 lbs FACTORED DOWN AND 61.1 lbs FACTORED UP AT 4-4-0, AND 23.8 lbs FACTORED DOWN AND 244.0 lbs FACTORED UP AT 5-9-8 ON TOP CHORD, AND 14.6 lbs FACTORED DOWN AT 1-11-4, 16.8 lbs FACTORED DOWN AT 2-11-4, 16.8 lbs FACTORED DOWN AT 4-4-0, AND 16.8 lbs FACTORED DOWN AT 5-8-12, AND 14.6 lbs FACTORED DOWN AT 6-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	706	0	706	0	0	5-8	5-8
G	706	0	706	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	534	269 / 0	115 / 0	0 / 0	0 / 0	150 / 0	0 / 0
G	534	269 / 0	115 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A-B	0 / 32	-84.9	-84.9	0.13 (1)	10.00
B-C	-471 / 0	-84.9	-84.9	0.12 (1)	6.25
C-K	-378 / 0	-84.9	-84.9	0.15 (1)	6.25
K-D	-378 / 0	-84.9	-84.9	0.15 (1)	6.25
D-E	-471 / 0	-84.9	-84.9	0.12 (1)	6.25
E-F	0 / 32	-84.9	-84.9	0.13 (1)	10.00
J-B	-657 / 0	0.0	0.0	0.07 (1)	7.81
G-E	-657 / 0	0.0	0.0	0.07 (1)	7.81
J-L	0 / 0	-38.5	-38.5	0.07 (3)	10.00
L-I	0 / 0	-38.5	-38.5	0.07 (3)	10.00
I-M	0 / 378	-38.5	-38.5	0.13 (2)	10.00
M-H	0 / 378	-38.5	-38.5	0.13 (2)	10.00
H-N	0 / 0	-38.5	-38.5	0.07 (3)	10.00
N-G	0 / 0	-38.5	-38.5	0.07 (3)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
I-C	0 / 116	0.03 (6)
C-H	0 / 0	0.00 (1)
H-D	0 / 107	0.08 (6)
B-I	0 / 402	0.10 (1)
H-E	0 / 402	0.10 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-10-8	-19	-19	161	FRONT	VERT	TOTAL
D	5-9-8	-19	-19	161	FRONT	VERT	TOTAL
H	5-8-12	-12	-12	—	FRONT	VERT	TOTAL
I	2-11-4	-12	-12	—	FRONT	VERT	TOTAL
K	4-4-0	-10	-10	54	FRONT	VERT	TOTAL
L	1-11-4	-6	-10	—	FRONT	VERT	TOTAL
M	4-4-0	-12	-12	—	FRONT	VERT	TOTAL
N	6-8-12	-6	-10	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.01")

CSI: TC=0.15/1.00 (C-D:1) , BC=0.13/1.00 (H-I:2) , WB=0.10/1.00 (E-H:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (H) (INPUT = 0.90)
JSI METAL= 0.14 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
07/20/18
PROVINCE OF ONTARIO

DWG NO. TAM 1807121
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	T20	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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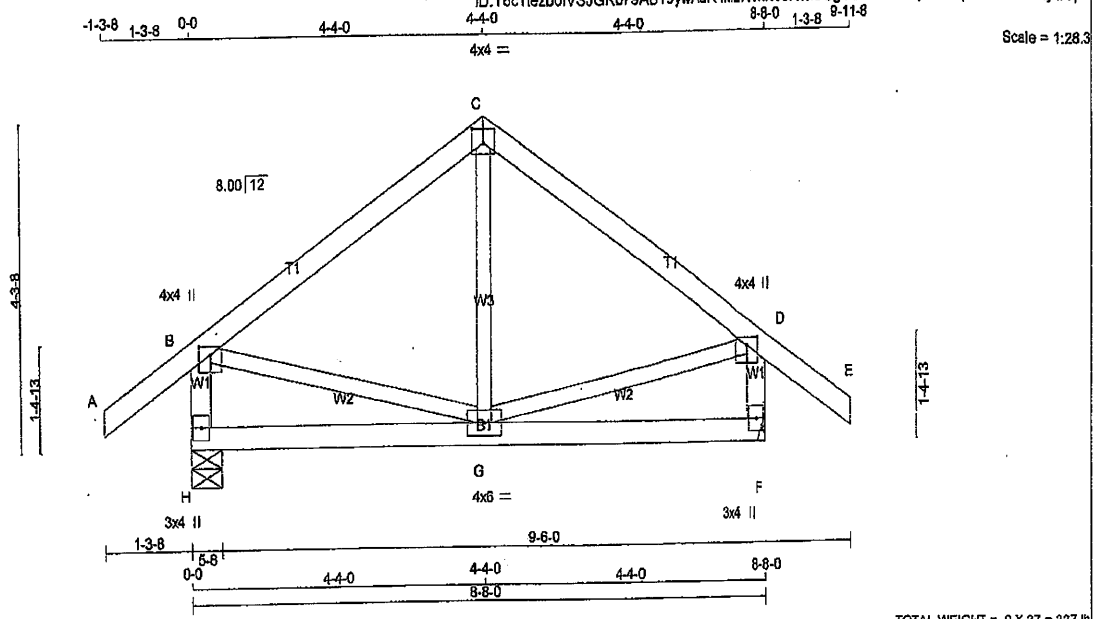


TOTAL WEIGHT = 9 X 37 = 337 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMWW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	651	0	651	0	0	5-8	5-8
F	651	0	651	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	486	266 / 0	91 / 0	0 / 0	0 / 0	129 / 0	0 / 0
F	486	266 / 0	91 / 0	0 / 0	0 / 0	129 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00	G-C	0 / 176	0.04 (3)
B-C	-362 / 0	-84.9	-84.9	1	0.21 (1)	6.25	B-G	0 / 302	0.07 (1)
C-D	-362 / 0	-84.9	-84.9	1	0.21 (1)	6.25	G-D	0 / 302	0.07 (1)
D-E	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00			
H-B	-587 / 0	0.0	0.0	1	0.06 (1)	7.81			
F-D	-587 / 0	0.0	0.0	1	0.06 (1)	7.81			
H-G	0 / 0	-38.5	-38.5	3	0.16 (3)	10.00			
G-F	0 / 0	-38.5	-38.5	3	0.16 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.16/1.00 (F-G:3) , WB=0.07/1.00 (B-G:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

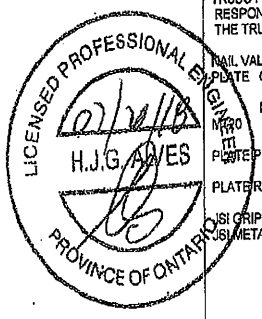
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.43 (G) (INPUT = 0.90)
SSI METAL= 0.12 (B) (INPUT = 1.00)



DWG NO. TAM 1150722
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300283	G20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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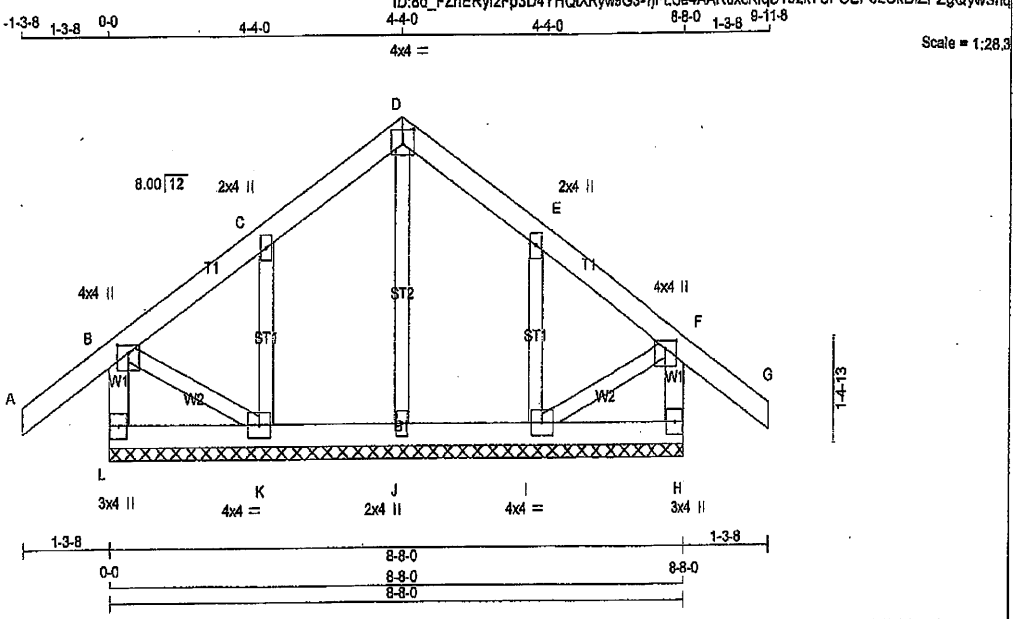

TOTAL WEIGHT = 38 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
L-B	-249 / 0	0.0	0.0	0.03	(1)	(1)	7.81
A-B	0 / 32	-84.9	-84.9	0.11	(1)	(1)	10.00
B-C	-28 / 0	-84.9	-84.9	0.11	(1)	(1)	6.25
C-D	-17 / 0	-84.9	-84.9	0.04	(1)	(1)	6.25
D-E	-17 / 0	-84.9	-84.9	0.04	(1)	(1)	6.25
E-F	-28 / 0	-84.9	-84.9	0.11	(1)	(1)	6.25
F-G	0 / 32	-84.9	-84.9	0.11	(1)	(1)	10.00
H-F	-249 / 0	0.0	0.0	0.03	(1)	(1)	7.81
L-K	0 / 0	-38.5	-38.5	0.04	(3)	(3)	10.00
K-J	0 / 7	-38.5	-38.5	0.04	(3)	(3)	10.00
J-I	0 / 7	-38.5	-38.5	0.04	(3)	(3)	10.00
I-H	0 / 0	-38.5	-38.5	0.04	(3)	(3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
J-D	-139 / 0	0.04	(1)
K-C	-172 / 0	0.03	(1)
I-E	-172 / 0	0.03	(1)
B-K	0 / 14	0.00	(1)
I-F	0 / 14	0.00	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (F-G:1) , BC=0.04/1.00 (I-J:3) , WB=0.04/1.00 (D-J:1) , SSI=0.08/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1957	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (F) (INPUT = 0.80)
JSI METAL= 0.09 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 1802123
STRUCTURAL COMPONENT [illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300283	T21	6	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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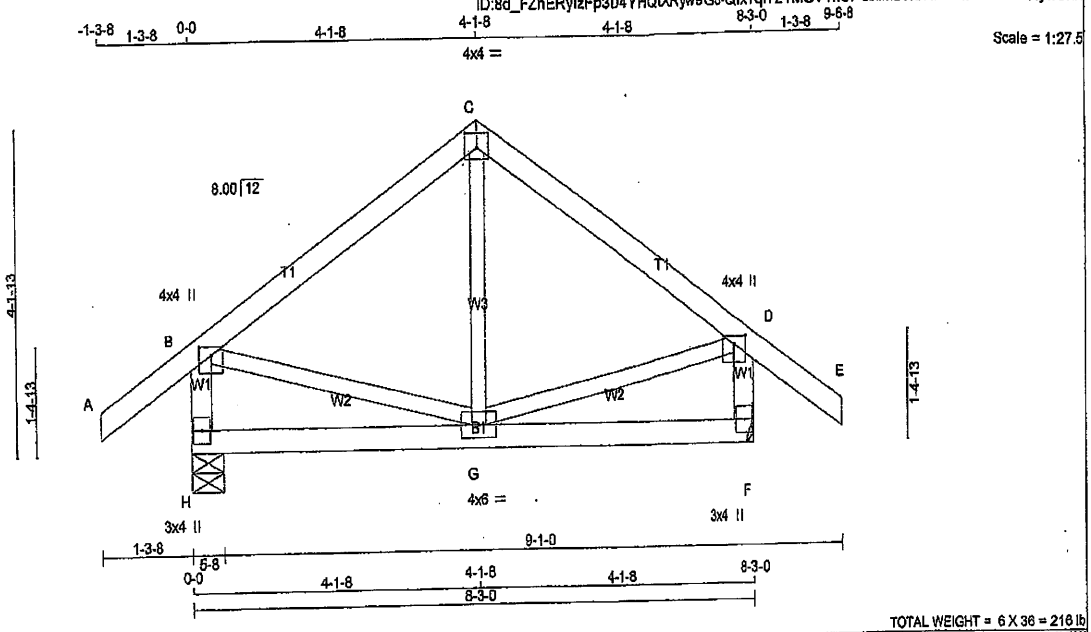


TOTAL WEIGHT = 6 X 36 = 216 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	625	0	625	0	0	5-8	5-8
F	625	0	625	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	466	256 / 0	87 / 0	0 / 0	0 / 0	124 / 0	0 / 0
F	466	256 / 0	87 / 0	0 / 0	0 / 0	124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	G-C	0 / 184	0.04 (3)
B-C	-339 / 0	-84.9	-84.9	0.19 (1)	6.25	B-G	0 / 282	0.06 (1)
C-D	-339 / 0	-84.9	-84.9	0.19 (1)	6.25	G-D	0 / 282	0.06 (1)
D-E	0 / 32	-84.9	-84.9	0.11 (1)	10.00			
H-B	-564 / 0	0.0	0.0	0.08 (1)	7.81			
F-D	-564 / 0	0.0	0.0	0.08 (1)	7.81			
H-G	0 / 0	-38.5	-38.5	0.15 (3)	10.00			
G-F	0 / 0	-38.5	-38.5	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.15/1.00 (G-H:3) , WB=0.08/1.00 (D-G:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

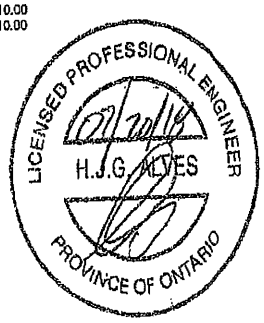
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (G) (INPUT = 0.80)
JSI METAL= 0.12 (D) (INPUT = 1.00)



DWG NO. TAM T1802126
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300283	G21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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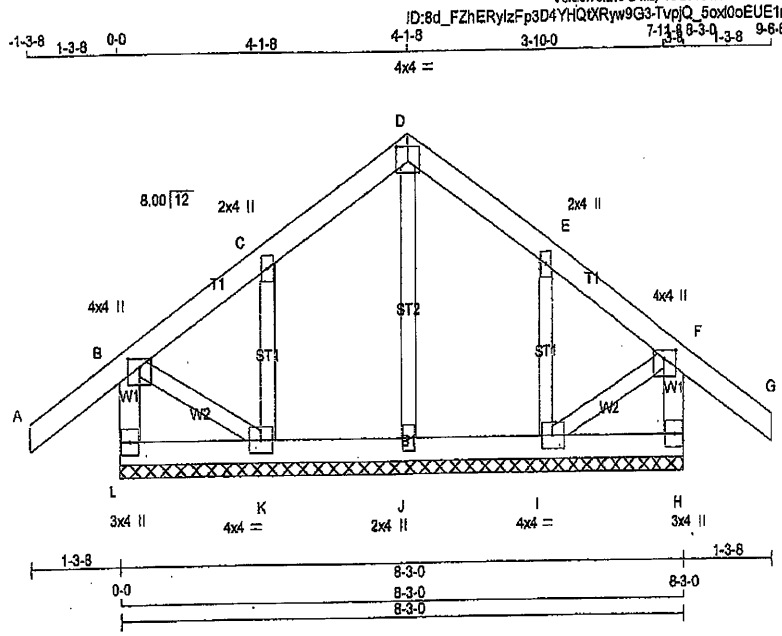


TOTAL WEIGHT = 37 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
L-B	-244 / 0	0.0	0.0	0.03 (1)		7.81
A-B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B-C	-30 / 0	-84.9	-84.9	0.11 (1)		6.25
C-D	-15 / 0	-84.9	-84.9	0.05 (1)		6.25
D-E	-15 / 0	-84.9	-84.9	0.05 (1)		6.25
E-F	-30 / 0	-84.9	-84.9	0.11 (1)		6.25
F-G	0 / 32	-84.9	-84.9	0.11 (1)		10.00
H-F	-244 / 0	0.0	0.0	0.03 (1)		7.81
L-K	0 / 0	-38.5	-38.5	0.03 (3)		10.00
K-J	0 / 7	-38.5	-38.5	0.03 (3)		10.00
J-I	0 / 7	-38.5	-38.5	0.03 (3)		10.00
I-H	0 / 0	-38.5	-38.5	0.03 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-D	-143 / 0	0.04 (1)
K-C	-157 / 0	0.03 (1)
I-E	-157 / 0	0.03 (1)
B-K	0 / 14	0.00 (1)
I-F	0 / 14	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (F-G:1) , BC=0.03/1.00 (J-K:3) , WB=0.04/1.00 (D-J:1) , SSI=0.08/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

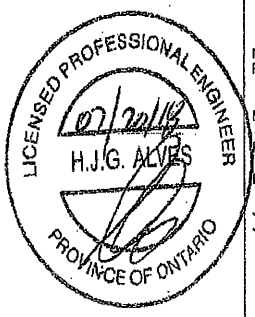
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)



DWG NO. TAM T1802124
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300283	T22	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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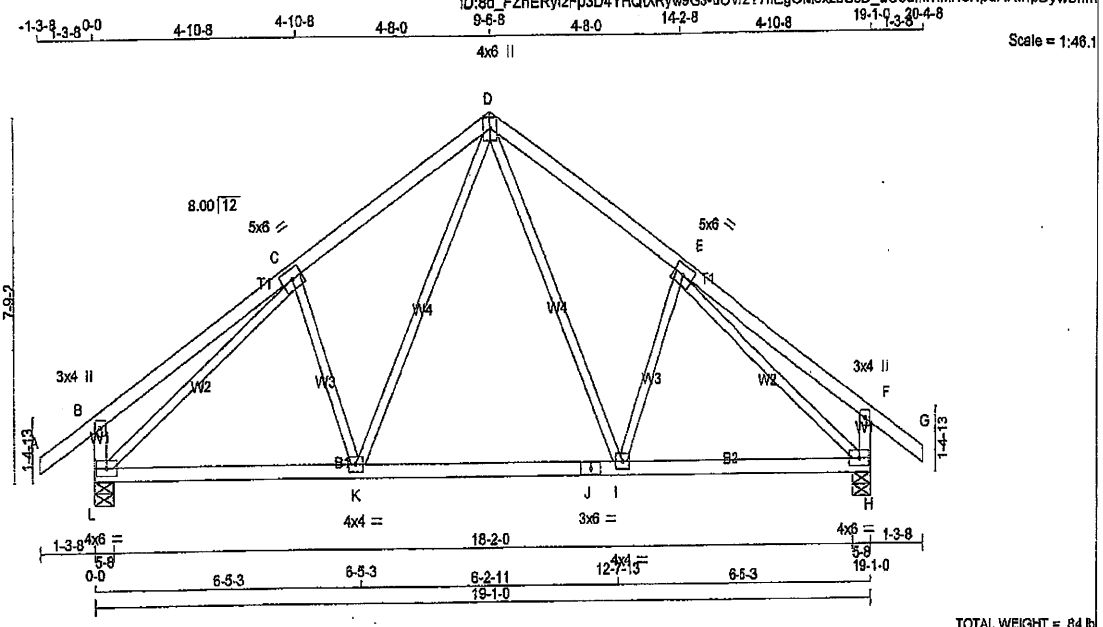


TOTAL WEIGHT = 84 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1294	0	1294	0	0	5-8	5-8
H	1294	0	1294	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	973	508 / 0	200 / 0	0 / 0	0 / 0	265 / 0	0 / 0
H	973	508 / 0	200 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

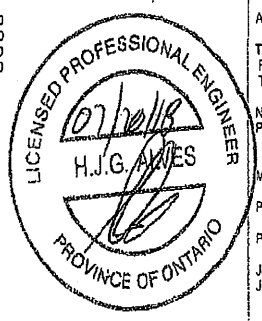
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32		-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 15		-84.9	-84.9	0.28 (1)	10.00
C-D	-1147 / 0		-84.9	-84.9	0.24 (1)	5.67
D-E	-1147 / 0		-84.9	-84.9	0.24 (1)	5.67
E-F	0 / 15		-84.9	-84.9	0.28 (1)	10.00
F-G	0 / 32		-84.9	-84.9	0.11 (1)	10.00
L-B	-292 / 0		0.0	0.0	0.03 (1)	7.81
H-F	-292 / 0		0.0	0.0	0.03 (1)	7.81
L-K	0 / 1009		-38.6	-38.5	0.40 (2)	10.00
K-J	0 / 739		-38.5	-38.5	0.37 (2)	10.00
J-I	0 / 738		-38.5	-38.5	0.37 (2)	10.00
I-H	0 / 1009		-38.6	-38.5	0.40 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
D-I	0 / 503		0.11 (1)
I-E	-208 / 29		0.07 (1)
K-D	0 / 503		0.11 (1)
C-K	-208 / 29		0.07 (1)
L-C	-1354 / 0		0.96 (1)
E-H	-1354 / 0		0.96 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.28/1.00 (E-F:1) , BC=0.40/1.00 (H-I:2) , WB=0.96/1.00 (E-H:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.42 (J) (INPUT = 1.00)

DWG NO. TAM 1160 2127
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300283	T22A	4	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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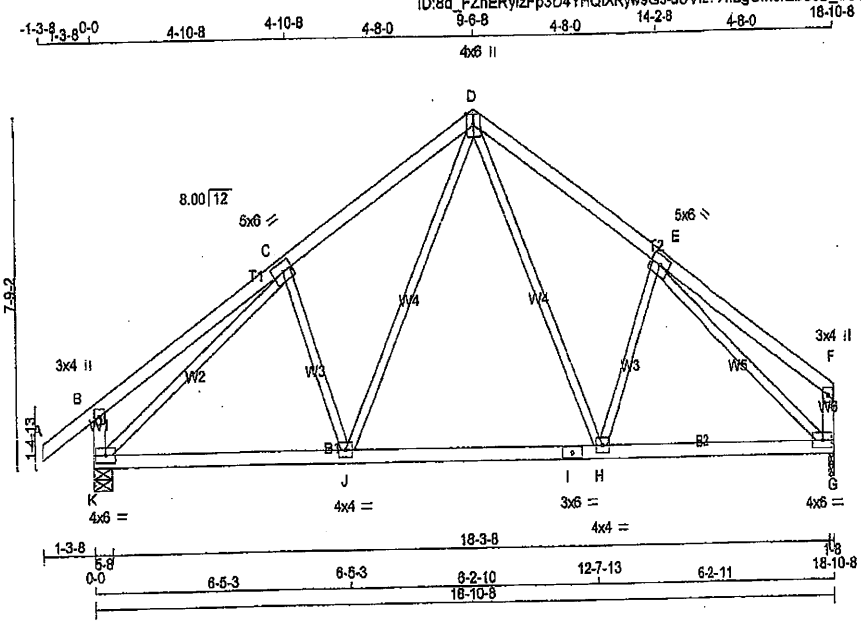


TOTAL WEIGHT = 4 X 81 = 325 lb

[M][F

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1285	0	1285	0	0	5-8	5-8
G	1160	0	1160	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	966	506 / 0	198 / 0	0 / 0	0 / 0	292 / 0	0 / 0
G	880	437 / 0	198 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

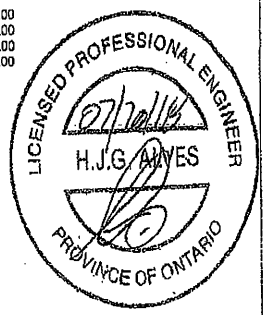
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B-C	0 / 15	-84.9	-84.9	0.28 (1)	10.00	
C-D	-1135 / 0	-84.9	-84.9	0.24 (1)	5.69	
D-E	-1123 / 0	-84.9	-84.9	0.25 (1)	5.71	
E-F	0 / 26	-84.9	-84.9	0.30 (1)	10.00	
K-B	-292 / 0	0.0	0.0	0.03 (1)	7.81	
G-F	-145 / 0	0.0	0.0	0.02 (1)	7.81	
K-J	0 / 1000	-38.5	-38.5	0.40 (2)	10.00	
J-I	0 / 726	-38.5	-38.5	0.37 (2)	10.00	
I-H	0 / 726	-38.5	-38.5	0.37 (2)	10.00	
H-G	0 / 982	-38.5	-38.5	0.39 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-210 / 28	0.07 (1)
J-D	0 / 506	0.11 (1)
D-H	0 / 479	0.11 (1)
H-E	-191 / 40	0.07 (1)
K-C	-1341 / 0	0.95 (1)
E-G	-1345 / 0	0.89 (1)



DWG NO. TAM H8021/28
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.30/1.00 (E-F:1) , BC=0.40/1.00 (J-K:2) , WB=0.95/1.00 (C-K:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (J) (INPUT = 0.80)
JSI METAL= 0.40 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	P1G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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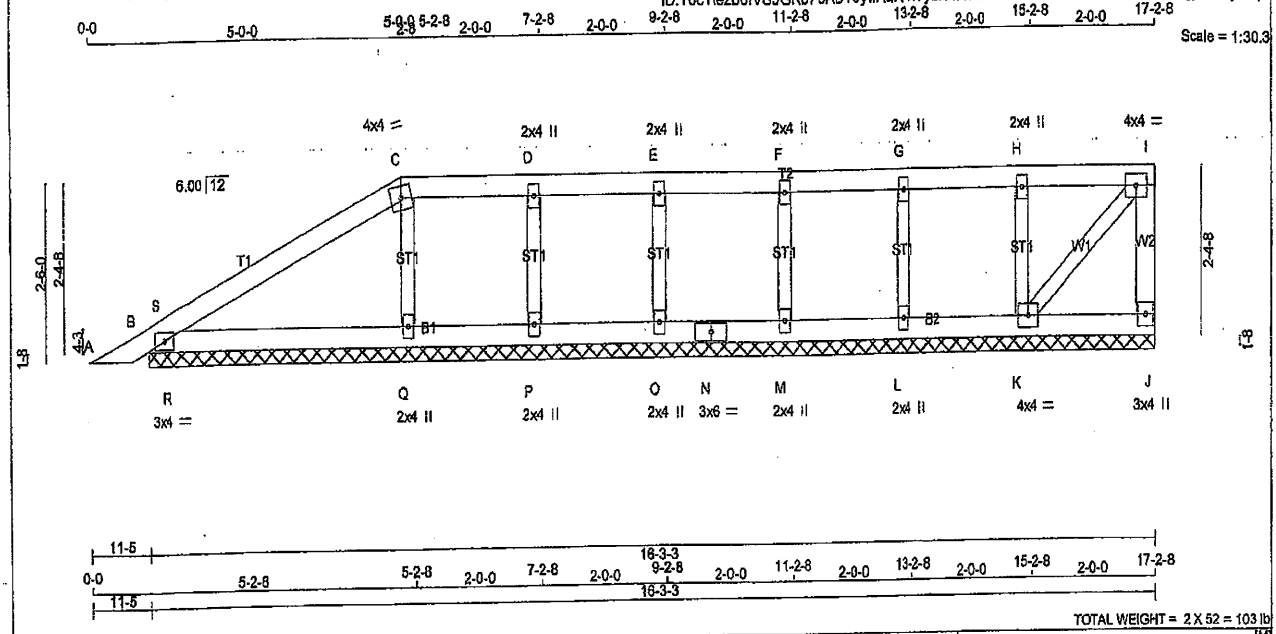

TOTAL WEIGHT = 2 X 52 = 103 lb

LUMBER
N. L. Q. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
B - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D, E, F, G, H					
D	TMW+w	MT20	2.0	4.0	
I	TMWW-t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	
K	BMWW1-t	MT20	4.0	4.0	
L, M, O, P, Q					
L	BMW1+w	MT20	2.0	4.0	
N	BS-t	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-84.9	-84.9	0.05 (1)	10.00
B-S	-54 / 0	-84.9	-84.9	0.04 (1)	6.25
S-C	-34 / 0	-84.9	-84.9	0.17 (1)	6.25
C-D	-17 / 0	-84.9	-84.9	0.05 (1)	6.25
D-E	-17 / 0	-84.9	-84.9	0.05 (1)	6.25
E-F	-17 / 0	-84.9	-84.9	0.04 (1)	6.25
F-G	-17 / 0	-84.9	-84.9	0.04 (1)	6.25
G-H	-17 / 0	-84.9	-84.9	0.05 (1)	6.25
H-I	-17 / 0	-84.9	-84.9	0.05 (1)	6.25
J-I	-89 / 0	0.0	0.0	0.01 (1)	7.81
B-R	0 / 27	-38.5	-38.5	0.12 (1)	10.00
R-Q	0 / 27	-38.5	-38.5	0.13 (1)	10.00
Q-P	0 / 17	-38.5	-38.5	0.13 (2)	10.00
P-O	0 / 17	-38.5	-38.5	0.03 (2)	10.00
O-N	0 / 17	-38.5	-38.5	0.03 (2)	10.00
N-M	0 / 17	-38.5	-38.5	0.03 (2)	10.00
M-L	0 / 17	-38.5	-38.5	0.02 (3)	10.00
L-K	0 / 17	-38.5	-38.5	0.03 (3)	10.00
K-J	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
K-H	-200 / 0	0.03 (1)
L-G	-161 / 0	0.02 (1)
M-F	-171 / 0	0.03 (1)
O-E	-161 / 0	0.02 (1)
P-D	-198 / 0	0.03 (1)
Q-C	-205 / 0	0.03 (1)
K-I	0 / 23	0.01 (1)
R-S	-169 / 59	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (C-S:1) , BC=0.13/1.00 (Q-R:1) , WB=0.03/1.00 (C-Q:1) , SSI=0.17/1.00 (B-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.60 (C) (INPUT = 0.90)
JSI METAL = 0.08 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07/20/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 1800101
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	P2G	2	1	TRUSS DESC.	

Temarack Roof Truss, Burlington

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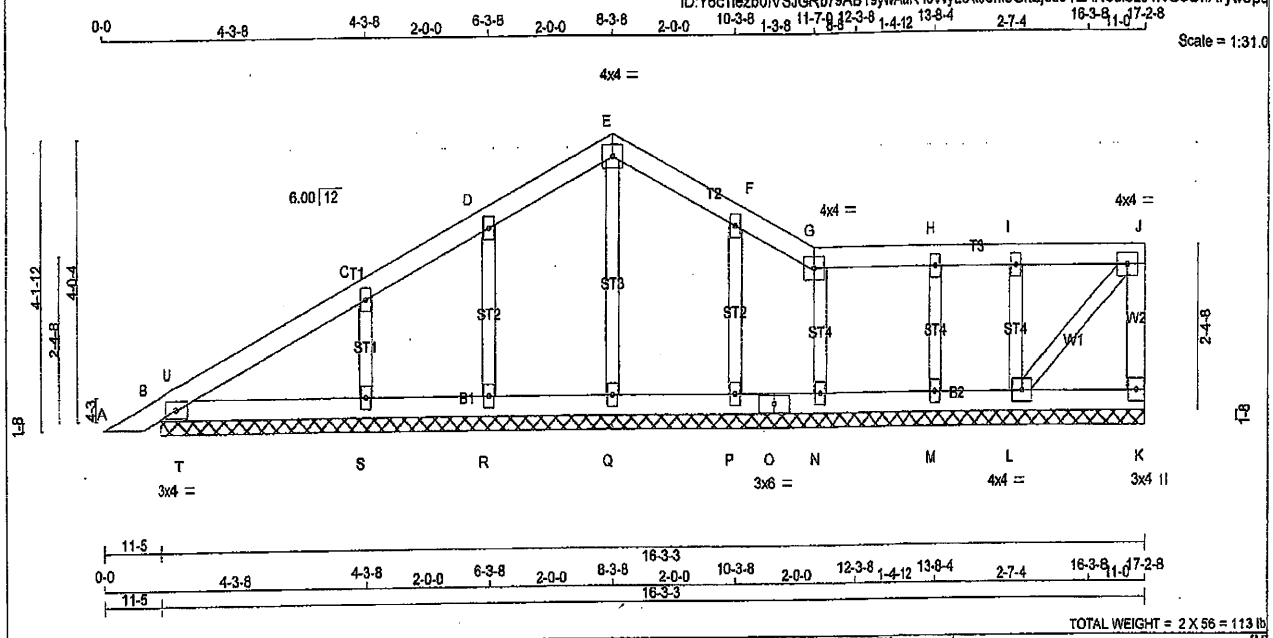


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
B - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, F, H, I						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0		
G	TTW-I	MT20	4.0	4.0		
J	TMVW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		
L	BMWW1-t	MT20	4.0	4.0		
M, N, P, Q, R, S						
M	BMW1+w	MT20	2.0	4.0		
O	BS-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

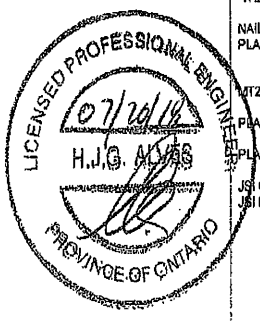
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A-B	0 / 15	-84.9	-84.9	0.05 (1)	10.00
B-U	-67 / 0	-84.9	-84.9	0.04 (1)	6.25
U-C	-18 / 0	-84.9	-84.9	0.09 (1)	6.25
C-D	-26 / 0	-84.9	-84.9	0.09 (1)	6.25
D-E	-19 / 0	-84.9	-84.9	0.04 (1)	6.25
E-F	-20 / 0	-84.9	-84.9	0.04 (1)	6.25
F-G	-6 / 0	-84.9	-84.9	0.04 (1)	10.00
G-H	-10 / 0	-84.9	-84.9	0.05 (1)	6.25
H-I	-10 / 0	-84.9	-84.9	0.04 (1)	6.25
I-J	-10 / 0	-84.9	-84.9	0.05 (1)	6.25
K-J	-87 / 0	0.0	0.0	0.01 (1)	7.81
B-T	0 / 25	-38.5	-38.5	0.05 (1)	10.00
T-S	0 / 25	-38.5	-38.5	0.07 (2)	10.00
S-R	0 / 15	-38.5	-38.5	0.07 (2)	10.00
R-Q	0 / 11	-38.5	-38.5	0.03 (2)	10.00
Q-P	0 / 11	-38.5	-38.5	0.03 (2)	10.00
P-O	0 / 16	-38.5	-38.5	0.02 (2)	10.00
O-N	0 / 16	-38.5	-38.5	0.02 (2)	10.00
N-M	0 / 10	-38.5	-38.5	0.02 (3)	10.00
M-L	0 / 10	-38.5	-38.5	0.03 (3)	10.00
L-K	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-E	-135 / 0	0.03 (1)
R-D	-165 / 0	0.03 (1)
S-C	-243 / 0	0.04 (1)
P-F	-180 / 0	0.03 (1)
N-G	-109 / 0	0.02 (1)
M-H	-161 / 0	0.02 (1)
L-I	-165 / 0	0.03 (1)
L-J	0 / 14	0.00 (1)
T-U	-41 / 61	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (C-U:1) , BC=0.07/1.00 (S-T:2) , WB=0.04/1.00 (C-S:1) , SSI=0.10/1.00 (C-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (G) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

DWG NO. TAM 1180202
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	P3	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Fri Jul 20 20:57:45 2018 Page 1
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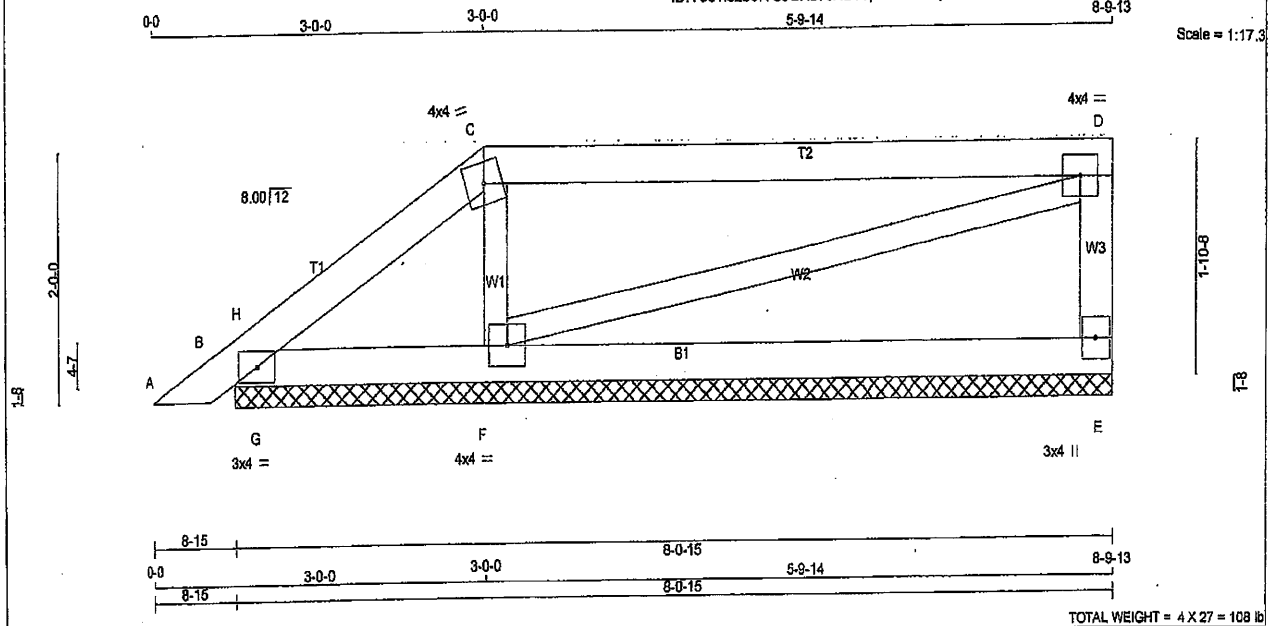


TOTAL WEIGHT = 4 X 27 = 108 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	337	0	337	0	0	8-0-15	8-0-15
B	173	0	173	0	0	8-0-15	8-0-15
F	542	0	542	0	0	8-0-15	8-0-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	252	137 / 0	48 / 0	0 / 0	0 / 0	67 / 0	0 / 0
B	117	100 / 0	0 / -5	0 / 0	0 / 0	22 / 0	0 / 0
F	425	170 / 0	127 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

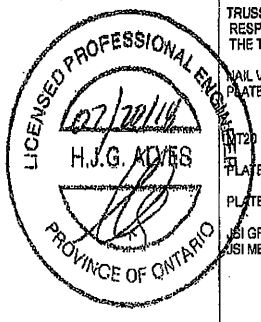
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- H	-5 / 46	-84.9	-84.9	0.05 (2)	10.00
H- C	-44 / 0	-84.9	-84.9	0.05 (2)	6.25
C- D	-9 / 3	-84.9	-84.9	0.49 (1)	10.00
E- D	-249 / 0	0.0	0.0	0.03 (1)	7.81
B- G	0 / 28	-38.5	-38.5	0.06 (1)	10.00
G- F	0 / 28	-38.5	-38.5	0.22 (3)	10.00
F- E	0 / 0	-38.5	-38.5	0.22 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-294 / 0	0.04 (1)
F- D	-3 / 9	0.00 (1)
G- H	-158 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.49/1.00 (C-D:1) , BC=0.22/1.00 (E-F:3) , WB=0.04/1.00 (C-F:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.20 (C) (INPUT = 0.90)
USI METAL= 0.05 (F) (INPUT = 1.00)

DWG NO. TAM 1802103
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300280	P4	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 16 2018 MiTek Industries, Inc. Fri Jul 20 20:57:46 2018 Page 1
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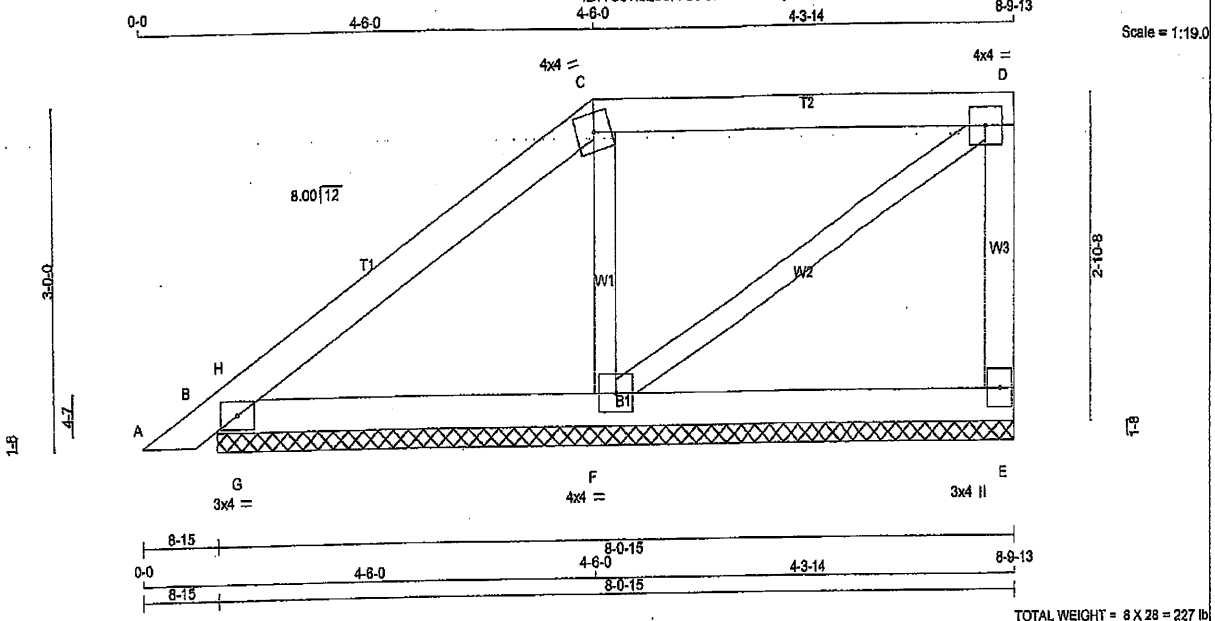


TOTAL WEIGHT = 8 X 28 = 227 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-I	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-I	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	285	0	285	0	0	8-0-15	8-0-15
B	305	0	305	0	0	8-0-15	8-0-15
F	482	0	482	0	0	8-0-15	8-0-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	198	109 / 0	37 / 0	0 / 0	0 / 0	52 / 0	0 / 0
B	224	135 / 0	32 / 0	0 / 0	0 / 0	56 / 0	0 / 0
F	373	163 / 0	101 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

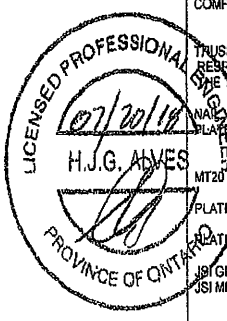
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	
B- H	-51 / 0	-84.9	-84.9	0.05 (1)	6.25	
H- C	-67 / 0	-84.9	-84.9	0.15 (1)	6.25	
C- D	-39 / 0	-84.9	-84.9	0.27 (1)	6.25	
E- D	-207 / 0	0.0	0.0	0.03 (1)	7.81	
B- G	0 / 50	-38.5	-38.5	0.11 (1)	10.00	
G- F	0 / 50	-38.5	-38.5	0.16 (2)	10.00	
F- E	0 / 0	-38.5	-38.5	0.15 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-285 / 0	0.05 (1)
F- D	0 / 48	0.01 (1)
G- H	-228 / 25	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (C-D:1) , BC=0.16/1.00 (F-G:2) , WB=0.05/1.00 (C-F:1) , SSI=0.20/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.80)
JSI METAL= 0.06 (C) (INPUT = 1.00)

DWG NO. TAM T1802104
STRUCTURAL
COMPONENT ONLY

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project information — Application number:

Building number, street name		Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description	

B. Individual who reviews and takes responsibility for design activities

Name		Firm	
SAM KATSOULAKOS		MICRO CITY ENGINEERING SERVICES INC.	
Street address R.R #1, PO BOX 61		Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750		Cell number

C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]

☐ House	☐ HVAC – House	☒ Building Structural
☐ Small Buildings	☐ Building Services	☐ Plumbing – House
☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing – All Buildings
☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems

Description of designer's work ROYAL PINE HOMES – FORESTSIDE PH 6 – UNIT 2301 – A PAIRED WITH B
(SCHEDULE IS NOT ISSUED AS LOT SPECIFIC)
REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM30220-18 DATED 7-24-18).
SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFED BY QUALIFIED BUILDING DESIGNER.

D. Declaration of Designer

I, SAM KATSOULAKOS declare that (choose one as appropriate):
(print name)

☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4.of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.

Individual BCIN: 26064

Firm BCIN: 29991

☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5.of Division C, of the Building Code.
Individual BCIN: _____

Basis for exemption from registration: _____

☐ The design work is exempt from the registration and qualification requirements of the Building Code.
Basis for exemption from registration and qualification: _____

I certify that:

1. The information contained in this schedule is true to the best of my knowledge.
2. I have submitted this application with the knowledge and consent of the firm.

Date 7-24-18 Signature of Designer

NOTE:

1. For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d).of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
2. Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG #TAM 30220-18S 7-24-18
DWG #TAM 30222-18S

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project information — Application number:

Building number, street name		Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description	

B. Individual who reviews and takes responsibility for design activities

Name		Firm	
SAM KATSOULAKOS		**MICRO CITY ENGINEERING SERVICES INC.**	
Street address **R.R #1, PO BOX 61**		Unit no.	Lot/con.
Municipality **GLENCOE**	Postal code **N0L 1M0**	Province **ONTARIO**	E-mail
Telephone number **(519) 287-2242 Business**	Fax number **(519) 287-5750**		Cell number

C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]

☐ House	☐ HVAC – House	☒ Building Structural
☐ Small Buildings	☐ Building Services	☐ Plumbing – House
☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing – All Buildings
☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems

Description of designer's work **ROYAL PINE HOMES – FORESTSIDE PH 6 – UNIT 2301 – C PAIRED WITH D**
(SCHEDULE IS NOT ISSUED AS LOT SPECIFIC)
REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM30221-18 DATED 7-24-18).
SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFED BY QUALIFIED BUILDING DESIGNER.

D. Declaration of Designer

I, SAM KATSOULAKOS declare that (choose one as appropriate):
(print name)

☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4.of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.

Individual BCIN: 26064

Firm BCIN: 29991

☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5.of Division C, of the Building Code.
Individual BCIN: _____

Basis for exemption from registration: _____

☐ The design work is exempt from the registration and qualification requirements of the Building Code.
Basis for exemption from registration and qualification: _____

I certify that:

1. The information contained in this schedule is true to the best of my knowledge.
2. I have submitted this application with the knowledge and consent of the firm.

Date 7-24-18 Signature of Designer

NOTE:

1. For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d).of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
2. Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG #TAM 30221 -18S 7-24-18
DWG #TAM 30223 -18S

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

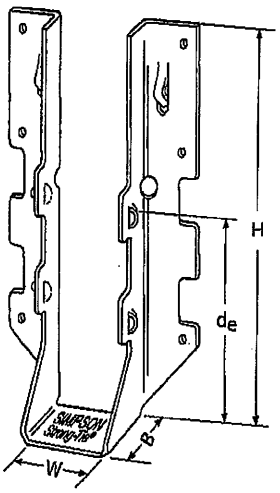
All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



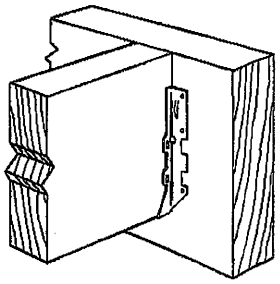
LUS28

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

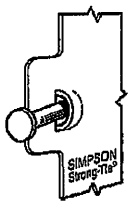
Options:

- These hangers cannot be modified


Typical LUS Installation

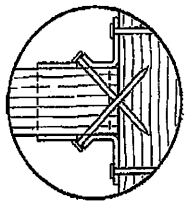
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.

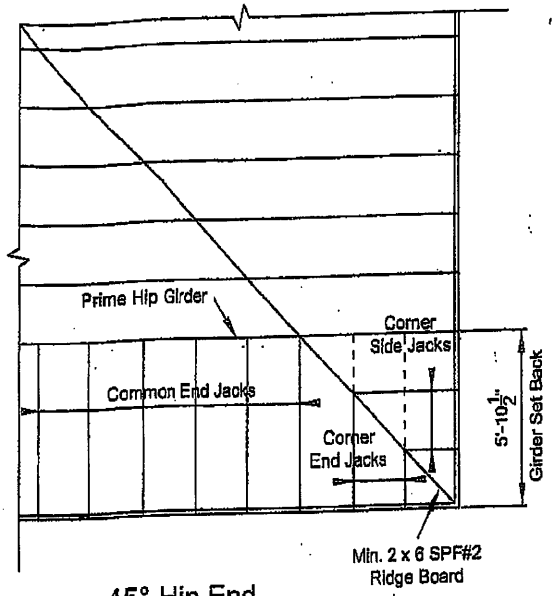
LICENSED PROFESSIONAL ENGINEER
4/5/2017
B. D. BUNTING
PROVINCE OF ONTARIO

LUMBER SPECIFICATION

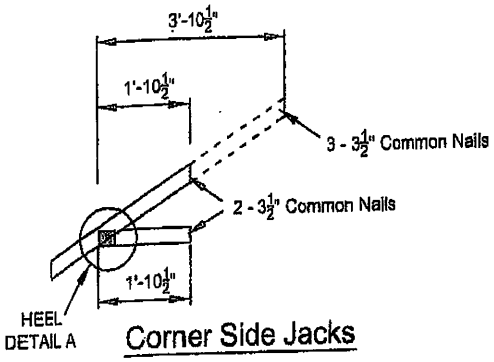
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

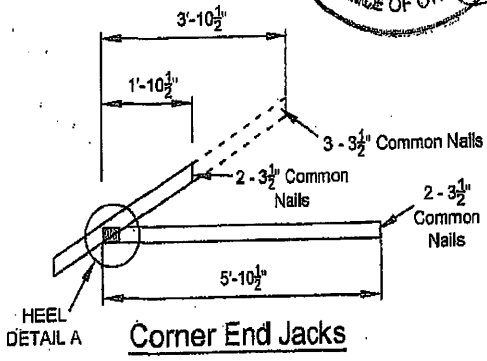
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



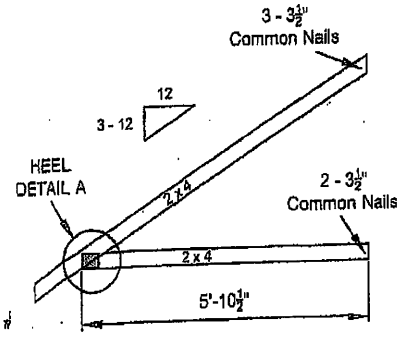
45° Hip End



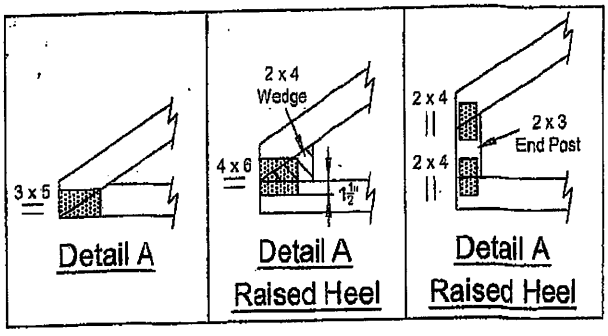
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

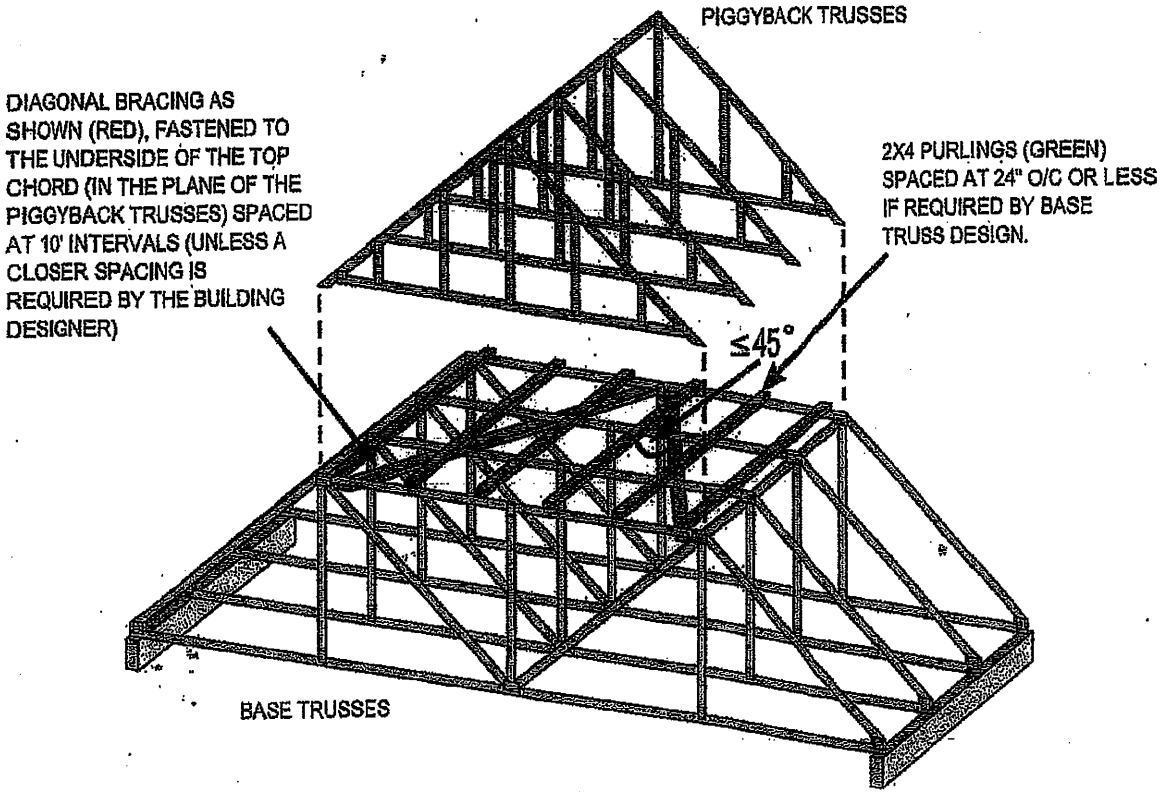
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

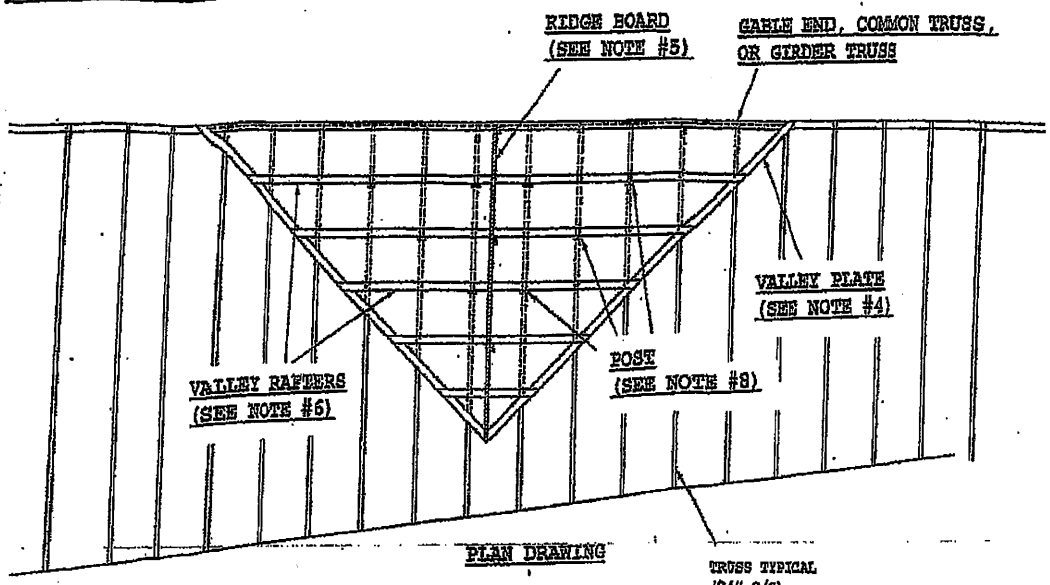
SKETCH FROM BCSI-CANADA 2013

MICRO CITY
ENGINEERING SERVICES INC.

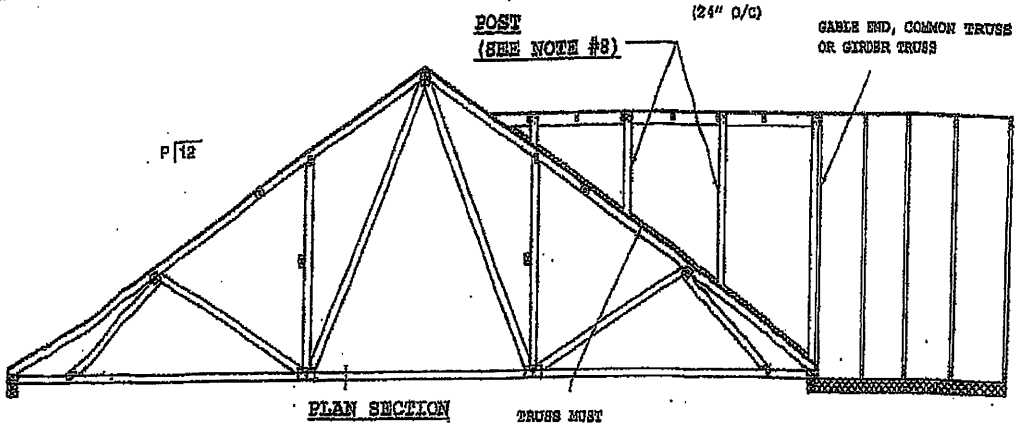
TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO BACK SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD, MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

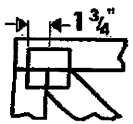
(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
2-5-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

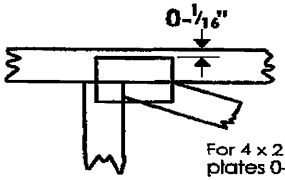
DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

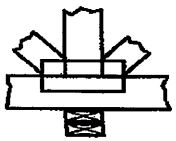
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

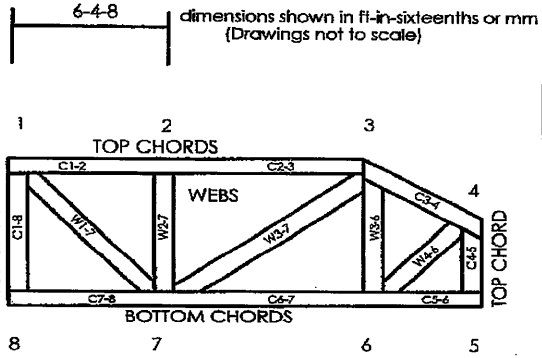
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

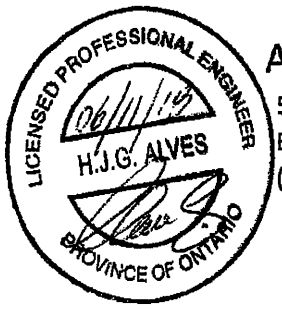
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018