ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
HGUS28-2 -(##)
LUS26-2 -(VV)
LUS24 -(O)
LJS26DS -(V)

ALL B-2-2X10
(FLUSH)

DENOTE:

H- 12" RAISED CEILING

H3- 24" RAISED CEILING

8/12 PITCHES (TYP.)
UNLESS NOTED

T-180485

DENOTES:
CONVENTIONAL
FRAMING

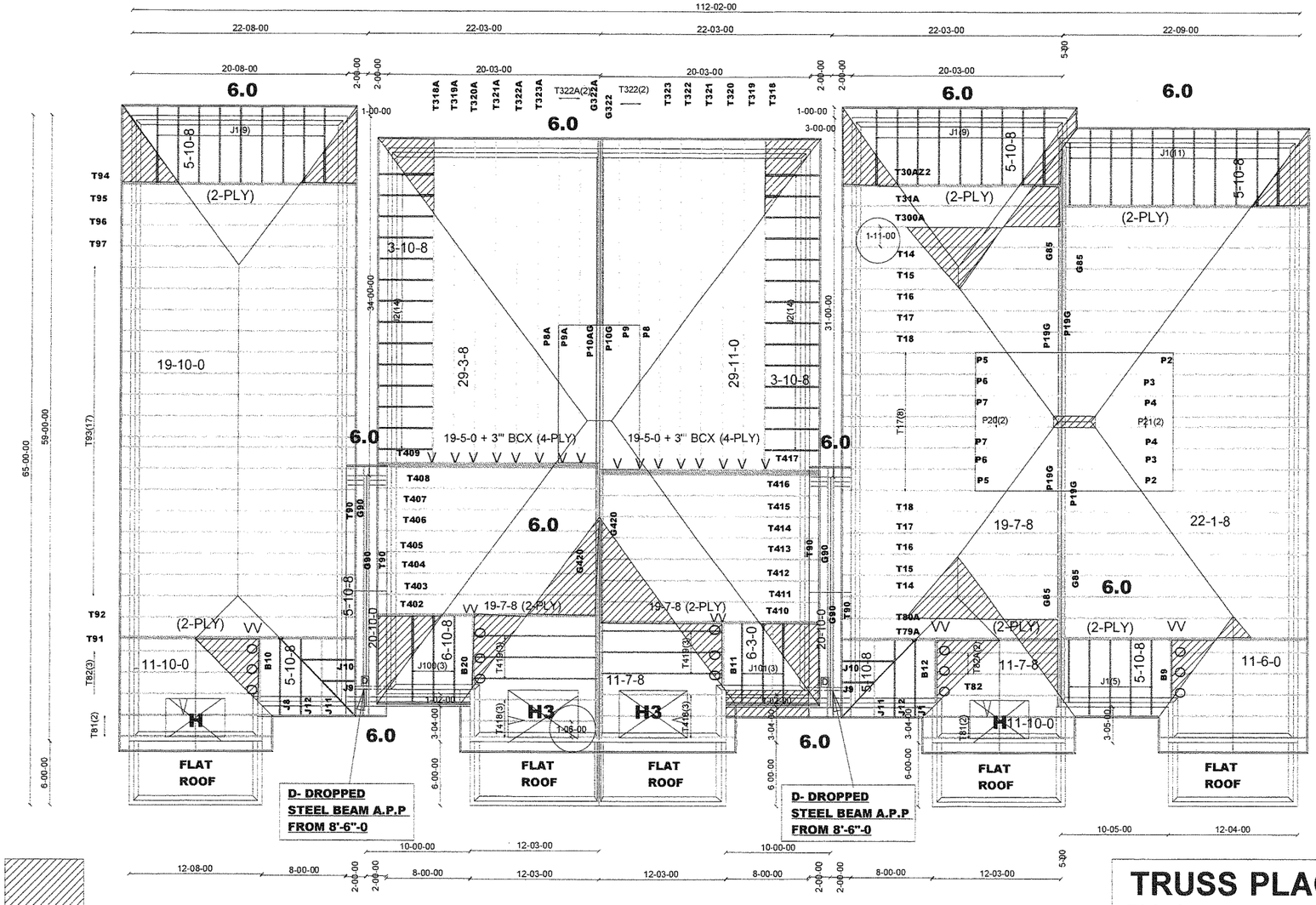
UNIT2202 END
B -LOT17
300438

UNIT2201
B(REV) -LOT16
300439

UNIT2201
B -LOT15
300583

UNIT2202
B(REV) -LOT14
300441

UNIT2203
B(REV) -LOT13
300403



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

TC LIVE 23.3 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

GROUND SNOW LOAD 27.2 PSF

Unit 14, DWG# T-1901129 to T-1901133, T-1901138 to T-1901140, T-1901157 to T-1901161, T-1901169 , T-1901171, T-1901172, T-1901215,T-1901216, T-1901224 , T-1901225, T-1901233, T-1800216, T1800218

Unit 13, DWG# T-1901138 to T-1901139, T-1901141 to T-1901150, T-1901154, T-1901212, T-1901213, T-1901214, T-1800216, T1800218

LICENSED PROFESSIONAL ENGINEER
04/15/19
H. J. G. ALVES
100089024
PROVINCE OF ONTARIO

DWG NO. TAM T19083 49
STRUCTURAL
COMPONENT ONLY

Unit 17, DWG# T-1901131 to T-1901132, T-1901171 to T-1901172 , T-1901226 to T-1901232 , T-1800216, T-1800218

Unit 16, DWG# T-1901171, T-1901172, T-1901191, T-1901193, T-1901195, T-1901197, T-1901199, T-1901201 , T-1901218, T-1901220, T-1901134 to T-1901241, T-1901250, T-1901251, T-1901253, T901254, T-1901256, T-1800216, T1800218

Unit 15, DWG# T-1901171 to T-1901172,T-1901190, T-1901192, T-1901194, T-1901196, T-1901198, T-1901200, T-1901217, T-1901219, T-1901242 to T-1901252, T-1901254, T-1901255, T-1800216, T1800218

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

M12128

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Job Track: 41239
Layout ID: 300434
Plan Log: 98887

Builder / Location: ROYAL PINE HOMES / BRAMPTON
Project: FORESTSIDE PH.6
Date: 4/10/2019 Designer: AC

Model / Elevation: BLOCK 5 - LOTS 13 TO 17
Mitek ver 8.2.3.229

TAMARACK LUMBER INC
ALPA LUMBER GROUP

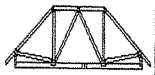
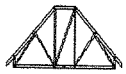
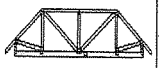
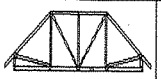
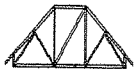
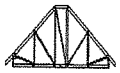
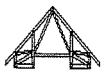
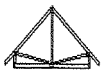
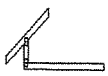
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300438 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2202 END ELEVATION: B LT 17

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T91 HIP GIRDER	8.00 0.00	19-10-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	206.78 126.00		
	1	T92 HIP	8.00 0.00	19-10-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	87.30 57.00		
	17	T93 COMMON	8.00 0.00	19-10-00	08-00-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	1469.65 923.61		
	1 2 Ply	T94 HIP GIRDER	8.00 0.00	19-10-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	201.18 124.68		
	1	T95 HIP	8.00 0.00	19-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	83.59 53.17		
	1	T96 HIP	8.00 0.00	19-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	84.88 53.33		
	1	T97 HIP	8.00 0.00	19-10-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	93.37 59.50		
	2	T81 ROOF	8.00 0.00	11-10-00	05-04-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	120.26 77.34		
	3	T82 COMMON	8.00 0.00	11-10-00	05-04-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	146.46 89.49		
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	85.23 54.00		
	9	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	151.11 96.03		
	1	J8 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	17.83 10.67		
	1	J9 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 00-01-01	01-04-13 00-03-08	7.67 4.67		
	1	J10 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	11.91 7.67		
	1	J11 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 04-01-01	01-04-13 00-03-08	12.23 7.33		
	1	J12 JACK-OPEN	8.00 0.00	03-09-07	03-11-02	2 X 4	2 X 4	01-03-08 02-01-01	01-04-13 00-03-08	14.97 9.33		

TOTAL # TRUSS= **46.00** TOTAL BFT OF ALL TRUSSES= **1804.82** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2876.28** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 **LAYOUT ID:** 300438 **LOCATION:**
BUILDER: ROYAL PINE-FORESTSIDE PH.6 **SUB-BUILDER:**
MODEL: UNIT2202 END **ELEVATION:** B LT 17

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

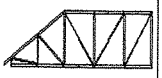
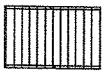
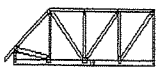
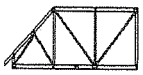
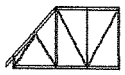
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300439 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2201 ELEVATION: B- LT 16

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T318A HALF HIP	6.00 0.00	29-03-08	03-11-13	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 03-11-13	157.44 95.00		
	1	T319A HALF HIP	6.00 0.00	29-03-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-03-13	121.58 75.00		
	1	T320A HALF HIP	6.00 0.00	29-03-08	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 06-07-13	125.30 79.83		
	1	T321A HALF HIP	6.00 0.00	29-03-08	07-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-11-13	136.79 85.00		
	3	T322A HALF HIP	6.00 0.00	29-03-08	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-03-13	421.80 258.99		
	1	G322A GABLE	6.00 0.00	29-03-08	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-03-13	162.55 103.17		
	1	T323A HALF HIP	6.00 0.00	29-03-08	10-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-07-13	158.91 99.33		
	1	G420 GABLE	0.00 0.00	24-09-00	09-01-13	2 X 4	2 X 4	00-00-00 00-00-00	09-01-13 09-01-13	152.75 94.83		
	1 2 Ply	T402 HALF HIP	8.00 0.00	19-07-08	04-07-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-07-04	209.48 126.00		
	1	T403 HALF HIP	8.00 0.00	19-07-08	05-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-07-04	83.11 54.00		
	1	T404 HALF HIP	8.00 0.00	19-07-08	06-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-07-04	87.89 56.17		
	1	T405 HALF HIP	8.00 0.00	19-07-08	07-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-07-04	95.05 59.83		
	1	T406 HALF HIP	8.00 0.00	19-07-08	08-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-07-04	94.22 59.50		
	1	T407 HALF HIP	8.00 0.00	19-07-08	09-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-07-04	99.25 61.83		
	1	T408 HALF HIP	8.00 0.00	19-07-08	10-07-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-07-04	103.74 64.33		
	1 4 Ply	T409 HALF HIP	8.00 0.00	19-07-08	11-07-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 00-05-08	572.08 354.68		
	3	T418 MONOPITCH	8.00 0.00	11-07-08	09-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-13	226.62 145.50		
	3	T419 JACK-CLOSED	8.00 0.00	11-07-08	09-01-13	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 09-01-13	163.05 102.51		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

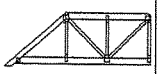
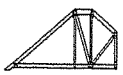
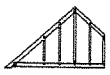
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300439 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2201 ELEVATION: B- LT.16

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T90 COMMON	6.00 / 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	81.86 / 51.00		
	1	G90 COMMON	6.00 / 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	85.23 / 54.00		
	1	P8A PIGGYBACK	6.00 / 0.00	12-00-09	02-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 02-03-00	41.19 / 25.33		
	1	P9A PIGGYBACK	6.00 / 0.00	12-00-09	03-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-03 / 02-04-15	43.42 / 28.67		
	1	P10AG GABLE	6.00 / 0.00	12-00-09	04-04-11	2 X 4	2 X 4	00-00-00 / 00-00-00	00-00-00 / 02-04-15	41.83 / 26.67		
	14	J2 JACK-OPEN	8.00 / 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-11-13	179.34 / 112.00		
	3	J100 JACK-OPEN	6.00 / 0.00	06-10-08	04-07-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 00-04-11	57.63 / 36.00		

TOTAL # TRUSS= **50.00** TOTAL BFT OF ALL TRUSSES= **2309.17** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3702.11** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

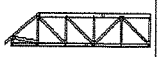
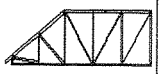
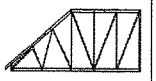
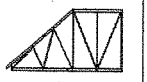
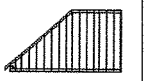
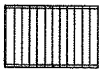
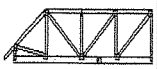
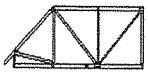
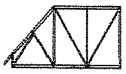
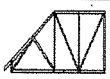
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300583 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2201 ELEVATION: B- LT.15

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T318 HALF HIP	6.00 0.00	29-11-00	03-11-13	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 03-11-13	160.30 97.67		
	1	T319 HALF HIP	6.00 0.00	29-11-00	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-03-13	123.36 75.00		
	1	T320 HALF HIP	6.00 0.00	29-11-00	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 06-07-13	127.07 79.83		
	1	T321 HALF HIP	6.00 0.00	29-11-00	07-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-11-13	138.49 85.67		
	3	T322 HALF HIP	6.00 0.00	29-11-00	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-03-13	427.32 261.00		
	1	T323 HALF HIP	6.00 0.00	29-11-00	10-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-07-13	160.65 99.33		
	1	G322 GABLE	6.00 0.00	29-11-00	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-03-13	163.98 103.83		
	1	G420 GABLE	0.00 0.00	24-09-00	09-01-13	2 X 4	2 X 4	00-00-00 00-00-00	09-01-13 09-01-13	152.75 94.83		
	1 2 Ply	T410 HALF HIP	8.00 0.00	19-07-08	04-03-08	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-03-08	204.88 124.00		
	1	T411 HALF HIP	8.00 0.00	19-07-08	05-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-08	79.93 49.67		
	1	T412 HALF HIP	8.00 0.00	19-07-08	06-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-03-08	86.36 54.00		
	1	T413 HALF HIP	8.00 0.00	19-07-08	07-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-03-08	91.36 56.67		
	1	T414 HALF HIP	8.00 0.00	19-07-08	08-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-03-08	92.70 57.50		
	1	T415 HALF HIP	8.00 0.00	19-07-08	09-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-03-08	97.66 60.00		
	1	T416 HALF HIP	8.00 0.00	19-07-08	10-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-03-08	99.21 61.33		
	1 4 Ply	T417 HALF HIP	8.00 0.00	19-07-08	11-03-08	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 00-05-08	564.52 350.68		
	3	T418 MONOPITCH	8.00 0.00	11-07-08	09-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-13	226.62 145.50		
	3	T419 JACK-CLOSED	8.00 0.00	11-07-08	09-01-13	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 09-01-13	163.05 102.51		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

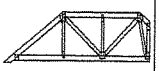
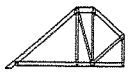
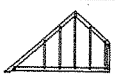
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300583 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2201 ELEVATION: B- LT 15

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	85.23 54.00		
	1	P8 PIGGYBACK	6.00 0.00	12-08-01	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-01-03	44.19 28.17		
	1	P9 PIGGYBACK	6.00 0.00	12-08-01	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 02-01-03	44.88 29.33		
	1	P10G GABLE	6.00 0.00	12-08-01	04-04-11	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-02-11	44.47 28.33		
	14	J2 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	179.34 112.00		
	3	J101 JACK-OPEN	6.00 0.00	06-03-00	04-03-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-03-08	53.10 33.99		

TOTAL # TRUSS= **50.00** TOTAL BFT OF ALL TRUSSES= **2295.84** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3693.28** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

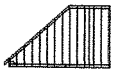
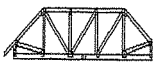
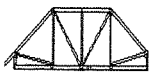
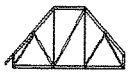
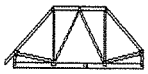
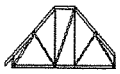
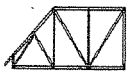
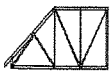
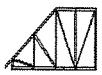
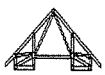
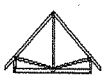
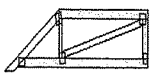
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300441 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2202 ELEVATION: B- LT 14

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	G85 GABLE	6.00 0.00	26-05-00	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-01-04	287.98 181.66		
	1 2 Ply	T30AZ2 HIP GIRDER	8.00 0.00	19-07-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	208.04 126.68		
	1	T31A HIP	8.00 0.00	19-07-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	81.31 52.50		
	1	T300A HIP	8.00 0.00	19-07-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	82.62 52.67		
	1 2 Ply	T79A HIP GIRDER	8.00 0.00	19-07-08	05-03-13	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	181.12 113.34		
	1	T80A HIP	8.00 0.00	19-07-08	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	85.03 55.83		
	2	T14 HALF HIP	8.00 0.00	19-07-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	170.92 108.00		
	2	T15 HALF HIP	8.00 0.00	19-07-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	180.80 114.66		
	2	T16 HALF HIP	8.00 0.00	19-07-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	195.66 121.66		
	10	T17 PIGGYBACK	8.00 0.00	19-07-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	967.10 606.70		
	2	T18 HALF HIP	8.00 0.00	19-07-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	196.86 122.66		
	2	T81 ROOF	8.00 0.00	11-10-00	05-04-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	120.26 77.34		
	1	T82 COMMON	8.00 0.00	11-10-00	05-04-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	48.82 29.83		
	2	T82A COMMON	8.00 0.00	11-07-08	05-04-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	93.10 59.66		
	1	T90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	81.86 51.00		
	1	G90 COMMON	6.00 0.00	20-10-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	85.23 54.00		
	2	P19G GABLE	6.00 0.00	09-07-03	05-01-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 05-01-12	71.70 45.00		
	2	P5 PIGGYBACK	8.00 0.00	07-03-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	49.28 33.66		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

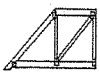
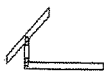
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 **LAYOUT ID:** 300441 **LOCATION:**
BUILDER: ROYAL PINE-FORESTSIDE PH.6 **SUB-BUILDER:**
MODEL: UNIT2202 **ELEVATION:** B- LT. 14

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	P6 PIGGYBACK	8.00 0.00	07-03-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	52.40 35.00		
	2	P7 PIGGYBACK	8.00 0.00	07-03-15	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 03-10-08	56.70 38.66		
	2	P20 PIGGYBACK	8.00 0.00	07-03-15	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 04-10-08	60.84 39.66		
	10	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	167.90 106.70		
	1	J9 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 00-01-01	01-04-13 00-03-08	7.67 4.67		
	1	J10 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	11.91 7.67		
	1	J11 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 04-01-01	01-04-13 00-03-08	12.23 7.33		
	1	J12 JACK-OPEN	8.00 0.00	03-09-07	03-11-02	2 X 4	2 X 4	01-03-08 02-01-01	01-04-13 00-03-08	14.97 9.33		

TOTAL # TRUSS= **58.00** TOTAL BFT OF ALL TRUSSES= **2255.87** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3572.31** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

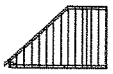
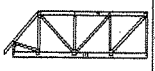
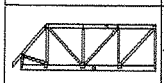
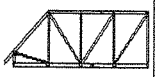
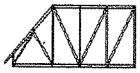
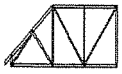
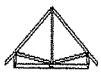
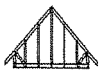
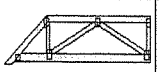
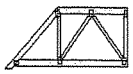
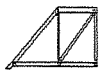
Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 LAYOUT ID: 300403 LOCATION:
BUILDER: ROYAL PINE-FORESTSIDE PH.6 SUB-BUILDER:
MODEL: UNIT2203 ELEVATION: B LT. 13

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	G85 GABLE	6.00 0.00	26-05-00	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-01-04	287.98 181.66		
	1 2 Ply	T1 HALF HIP	8.00 0.00	22-01-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	219.12 137.34		
	1 2 Ply	T1Z HALF HIP	8.00 0.00	22-01-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	219.12 137.34		
	2	T2 HALF HIP	8.00 0.00	22-01-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	184.50 115.34		
	2	T3 HALF HIP	8.00 0.00	22-01-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	200.40 128.00		
	2	T4 HALF HIP	8.00 0.00	22-01-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	194.90 123.34		
	2	T5 HALF HIP	8.00 0.00	22-01-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	209.34 131.66		
	10	T6 PIGGYBACK	8.00 0.00	22-01-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	1102.00 685.00		
	2	T7 HALF HIP	8.00 0.00	22-01-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	230.16 144.00		
	4	T11 COMMON	8.00 0.00	11-06-00	05-02-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	190.52 119.32		
	1	G11 GABLE	8.00 0.00	11-06-00	05-02-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	50.14 31.83		
	2	P19G GABLE	6.00 0.00	09-07-03	05-01-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 05-01-12	71.70 45.00		
	2	P2 PIGGYBACK	8.00 0.00	09-09-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	65.22 42.00		
	2	P3 PIGGYBACK	8.00 0.00	09-09-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	69.32 45.66		
	2	P4 PIGGYBACK	8.00 0.00	09-09-15	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 03-10-08	70.70 46.00		
	2	P21 PIGGYBACK	8.00 0.00	09-09-15	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 04-10-08	74.90 48.66		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		

TOTAL # TRUSS= **57.00** TOTAL BFT OF ALL TRUSSES= **2332.87** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3708.66** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/11/18
SALES REP	Mario

JOB TRACK: 41239 **LAYOUT ID:** 300403 **LOCATION:**
BUILDER: ROYAL PINE-FORESTSIDE PH.6 **SUB-BUILDER:**
MODEL: UNIT2203 **ELEVATION:** B LT. 13

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	
3	Hangers	LUS24	

TOTAL # ITEMS= **4.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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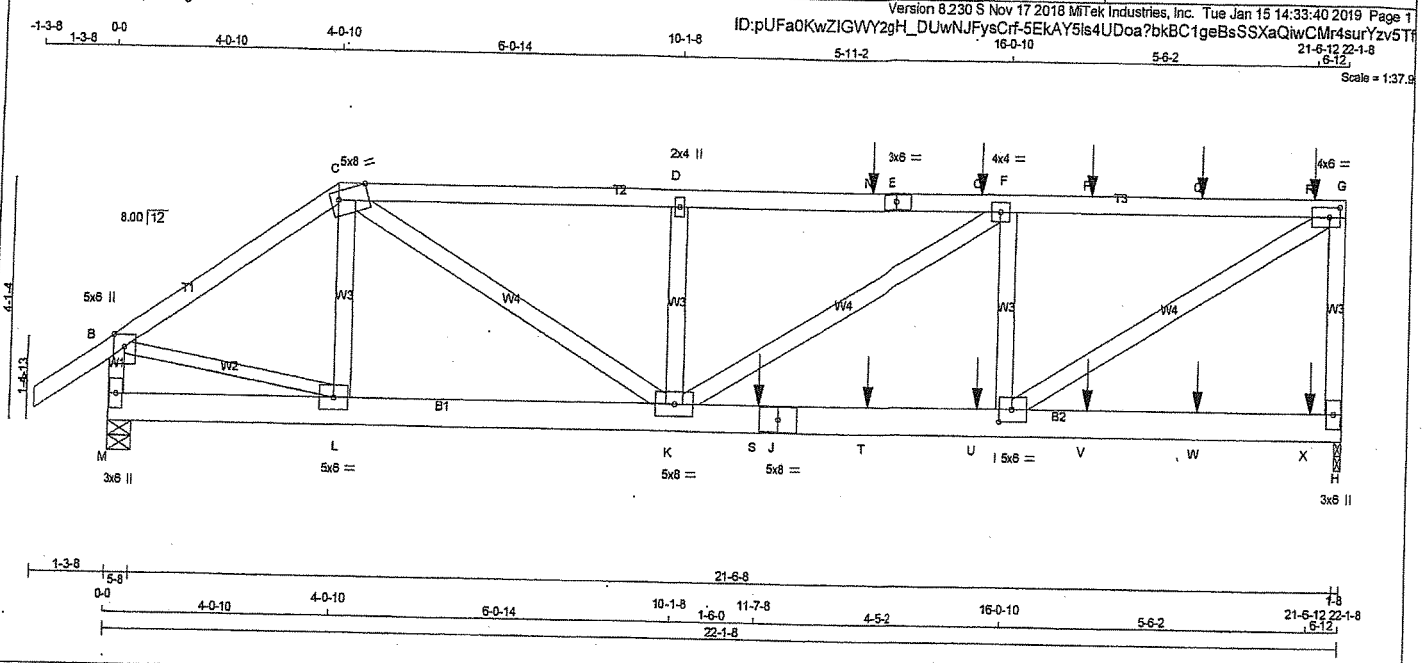
ID:pUFa0KwZIGWY2gH_DUwNJFysCrf-5EkAY5ls4UDoa?bkBC1geBsSSXaQiwCMr4surYzv5Tf



TOTAL WEIGHT = 2 X 110 = 219 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	OESCR.
A - C	2x4	ORY	No.2	SPF
C - E	2x4	ORY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	ORY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
M - J	2x6	ORY	No.2	SPF
J - H	2x6	ORY	No.2	SPF
ALL WEBS EXCEPT	2x4	ORY	No.2	SPF
B - L	2x3	ORY	No.2	SPF

DRY: SEASONEO LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	SIDE(0.0)
E- G	1	12	SIDE(0.0)
G- H	1	12	TOP
M- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	TOP
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENEO WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAO TO BE TRANSFERREO TO EACH PLY.

SIOE - PLF SHOWN IS THE EQUIVALENT UDL APPLIEO TO ONE SIOE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIEO ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	6.25
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0	2.00	2.25
H	BMV1+p	MT20	3.0	8.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	5.0	8.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INOICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTOREO GROSS REACTION VERT	HORZ	MAXIMUM FACTOREO GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORO BRG IN-SX
H	2749	0	2749	0	0	1-8	1-8
M	2307	0	2307	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WINO	DEAD	SOIL
H	2078	1057 / 0	449 / 0	0 / 0	0 / 0	572 / 0	0 / 0
M	1732	917 / 0	347 / 0	0 / 0	0 / 0	468 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORO TO BE SHEATHEO OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING OIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINEO.

LOADING

TOTAL LOAO CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	FACTOREO VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B- C	-2548 / 0	-84.9	-84.9	0.16 (1)	5.51
C- D	-4494 / 0	-84.9	-84.9	0.36 (1)	4.23
D- N	-4494 / 0	-84.9	-84.9	0.53 (1)	4.00
N- E	-4494 / 0	-84.9	-84.9	0.53 (1)	4.00
E- O	-4494 / 0	-84.9	-84.9	0.53 (1)	4.00
O- F	-4494 / 0	-84.9	-84.9	0.53 (1)	4.00
F- P	-3645 / 0	-84.9	-84.9	0.48 (1)	4.41
P- O	-3645 / 0	-84.9	-84.9	0.48 (1)	4.41
O- R	-3645 / 0	-84.9	-84.9	0.48 (1)	4.41
R- G	-3645 / 0	-84.9	-84.9	0.48 (1)	4.41
H- G	-2611 / 0	0.0	0.0	0.33 (1)	7.03
M- B	-2210 / 0	0.0	0.0	0.12 (1)	7.50
M- L	0 / 0	-38.5	-38.5	0.05 (1)	10.00
L- K	0 / 2100	-38.5	-38.5	0.36 (1)	10.00
K- S	0 / 3645	-38.5	-38.5	0.66 (1)	10.00
S- J	0 / 3645	-38.5	-38.5	0.66 (1)	10.00
J- T	0 / 3645	-38.5	-38.5	0.66 (1)	10.00
T- U	0 / 3645	-38.5	-38.5	0.66 (1)	10.00
U- I	0 / 3645	-38.5	-38.5	0.66 (1)	10.00
I- V	0 / 0	-38.5	-38.5	0.19 (2)	10.00
V- W	0 / 0	-38.5	-38.5	0.19 (2)	10.00
W- X	0 / 0	-38.5	-38.5	0.19 (2)	10.00
X- H	0 / 0	-38.5	-38.5	0.19 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-440 / 0	0.04 (1)
C- K	0 / 2807	0.25 (1)
K- D	-563 / 0	0.05 (1)
K- F	0 / 1004	0.09 (1)
I- F	-1440 / 0	0.13 (1)
I- G	0 / 4277	0.38 (1)
B- L	0 / 2476	0.27 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	13-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
O	15-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
P	17-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
Q	19-6-12	-102	-102	—	FRONT	VERT	TOTAL	—	—
R	21-6-12	-121	-121	—	FRONT	VERT	TOTAL	—	—
S	11-7-8	-1398	-1398	—	TOP	VERT	TOTAL	—	—
T	13-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
U	15-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
V	17-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
W	19-6-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
X	21-6-12	-63	-81	—	FRONT	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/15/19
R. J. B. ALVES
100111124
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIEO LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASEO ON A SLOPE OF 6.00/12 .

THIS TRUSS IS DESIGNED FOR RESIOENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAO) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. OEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
OALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.53/1.00 (D-F:1) , BC=0.86/1.00 (I-K:1) , WB=0.38/1.00 (G-I:1) , SSI=0.40/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Oeg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.82 (J) (INPUT = 1.00)

DWG NO. TAM T1901144
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300403	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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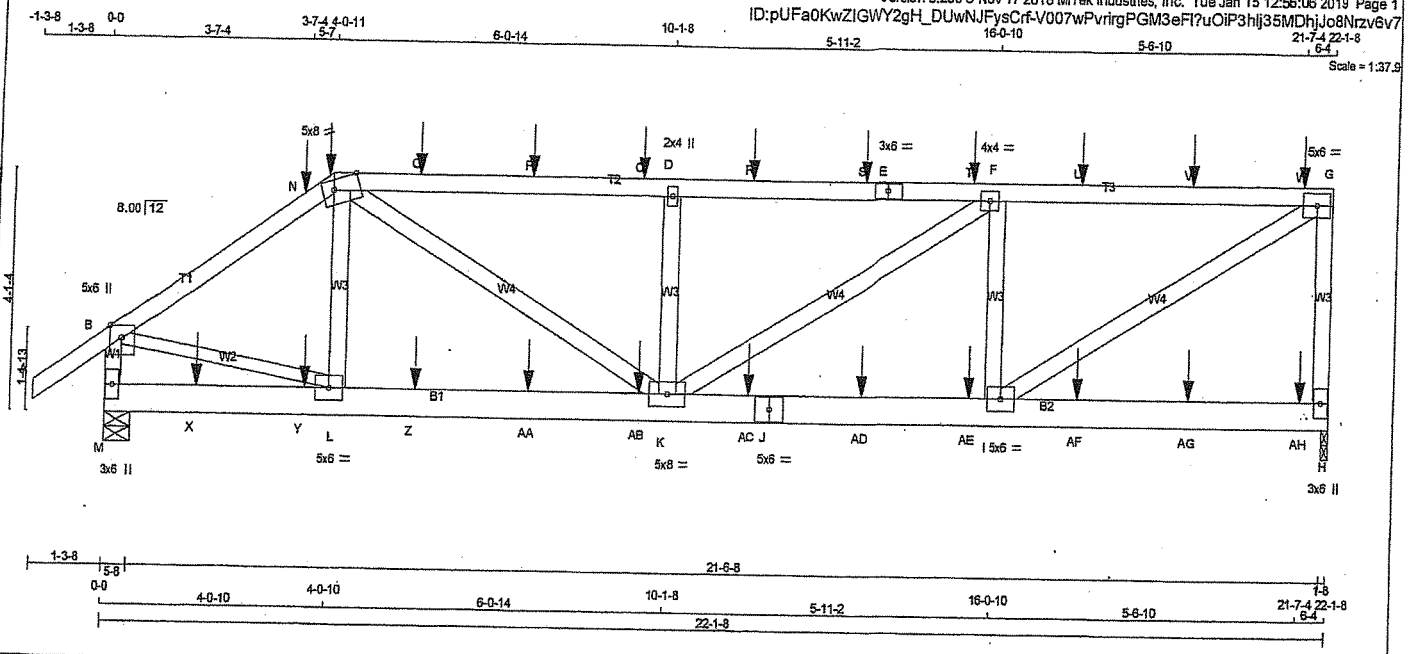


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF.
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- G	1	12	SIDE(81.0)
G- H	1	12	TOP
M- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- H	2	12	SIDE(122.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSLIMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED DN TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	5.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2348	0	2348	0	0	1-8	1-8
M	2458	0	2458	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1785	878 / 0	410 / 0	0 / 0	0 / 0	499 / 0	0 / 0
M	1853	957 / 0	389 / 0	0 / 0	0 / 0	506 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

OHORDS

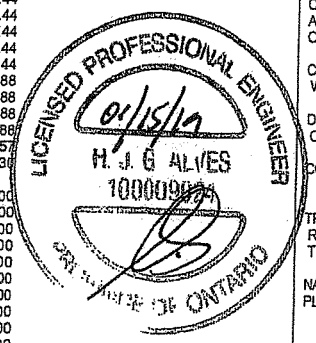
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B- N	-2718 / 0	-84.9	-84.9	0.19 (1)	5.34
N- C	-2716 / 0	-84.9	-84.9	0.19 (1)	5.34
C- O	-3592 / 0	-84.9	-84.9	0.47 (1)	4.46
O- P	-3592 / 0	-84.9	-84.9	0.47 (1)	4.46
P- Q	-3592 / 0	-84.9	-84.9	0.47 (1)	4.46
Q- D	-3592 / 0	-84.9	-84.9	0.47 (1)	4.46
D- R	-3592 / 0	-84.9	-84.9	0.47 (1)	4.44
R- S	-3592 / 0	-84.9	-84.9	0.47 (1)	4.44
S- E	-3592 / 0	-84.9	-84.9	0.47 (1)	4.44
E- T	-3592 / 0	-84.9	-84.9	0.47 (1)	4.44
T- F	-3592 / 0	-84.9	-84.9	0.47 (1)	4.44
F- U	-2897 / 0	-84.9	-84.9	0.44 (1)	4.88
U- V	-2897 / 0	-84.9	-84.9	0.44 (1)	4.88
V- W	-2897 / 0	-84.9	-84.9	0.44 (1)	4.88
W- G	-2897 / 0	-84.9	-84.9	0.44 (1)	4.88
H- G	-2156 / 0	0.0	0.0	0.27 (1)	7.57
M- B	-2371 / 0	0.0	0.0	0.13 (1)	7.36
M- X	0 / 0	-38.5	-38.5	0.08 (3)	10.00
X- Y	0 / 0	-38.5	-38.5	0.08 (3)	10.00
Y- L	0 / 0	-38.5	-38.5	0.08 (3)	10.00
L- Z	0 / 2272	-38.5	-38.5	0.21 (1)	10.00
Z-AA	0 / 2272	-38.5	-38.5	0.21 (1)	10.00
AA-AB	0 / 2272	-38.5	-38.5	0.21 (1)	10.00
AB- K	0 / 2272	-38.5	-38.5	0.21 (1)	10.00
K-AC	0 / 2897	-38.5	-38.5	0.26 (2)	10.00
AC- J	0 / 2897	-38.5	-38.5	0.26 (2)	10.00
J-AD	0 / 2897	-38.5	-38.5	0.26 (2)	10.00
AD-AE	0 / 2897	-38.5	-38.5	0.26 (2)	10.00
AE- I	0 / 2897	-38.5	-38.5	0.26 (2)	10.00
I-AF	0 / 0	-38.5	-38.5	0.11 (3)	10.00
AF-AG	0 / 0	-38.5	-38.5	0.11 (3)	10.00
AG-AH	0 / 0	-38.5	-38.5	0.11 (3)	10.00
AH- H	0 / 0	-38.5	-38.5	0.11 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-161 / 201	0.02 (3)
C- K	0 / 1547	0.14 (1)
K- D	-881 / 0	0.08 (1)
K- F	0 / 821	0.07 (1)
I- F	-1336 / 0	0.12 (1)
I- G	0 / 3400	0.30 (1)
B- L	0 / 2342	0.29 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-50	-56	—	FRONT	VERT	DEAD	—	—
C	4-0-11	-231	-231	—	FRONT	VERT	SNOW	—	—



DWG NO. TAM 1900143
STRUCTURAL COMPONENT ONLY

TOTAL WEIGHT = 2 X 110 = 219 lb [M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEPINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.47/1.00 (D-F:1) , BC=0.26/1.00 (I-K:2) , WB=0.30/1.00 (G-I:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1858

PLATE PLACEMENT TOL. = 0.250 inches

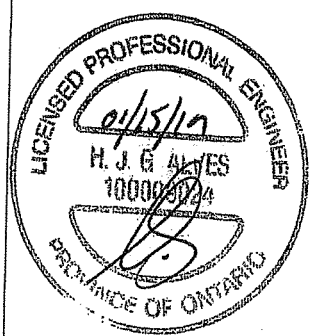
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	3-7-4	-125	-125	—	FRONT	VERT	TOTAL	—	—
O	5-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
P	7-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
Q	9-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
R	11-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
S	13-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
T	15-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
U	17-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
V	19-7-4	-102	-102	—	FRONT	VERT	TOTAL	—	—
W	21-7-4	-122	-122	—	FRONT	VERT	TOTAL	—	—
X	1-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Y	3-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Z	5-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AA	7-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AB	9-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AC	11-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AD	13-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AE	15-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AF	17-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AG	19-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AH	21-7-4	-64	-82	—	FRONT	VERT	TOTAL	—	—



DWG NO. TAM 1790193
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T2	2	1	TRUSS OESC.	

Tamerack Roof Truss, Burlington

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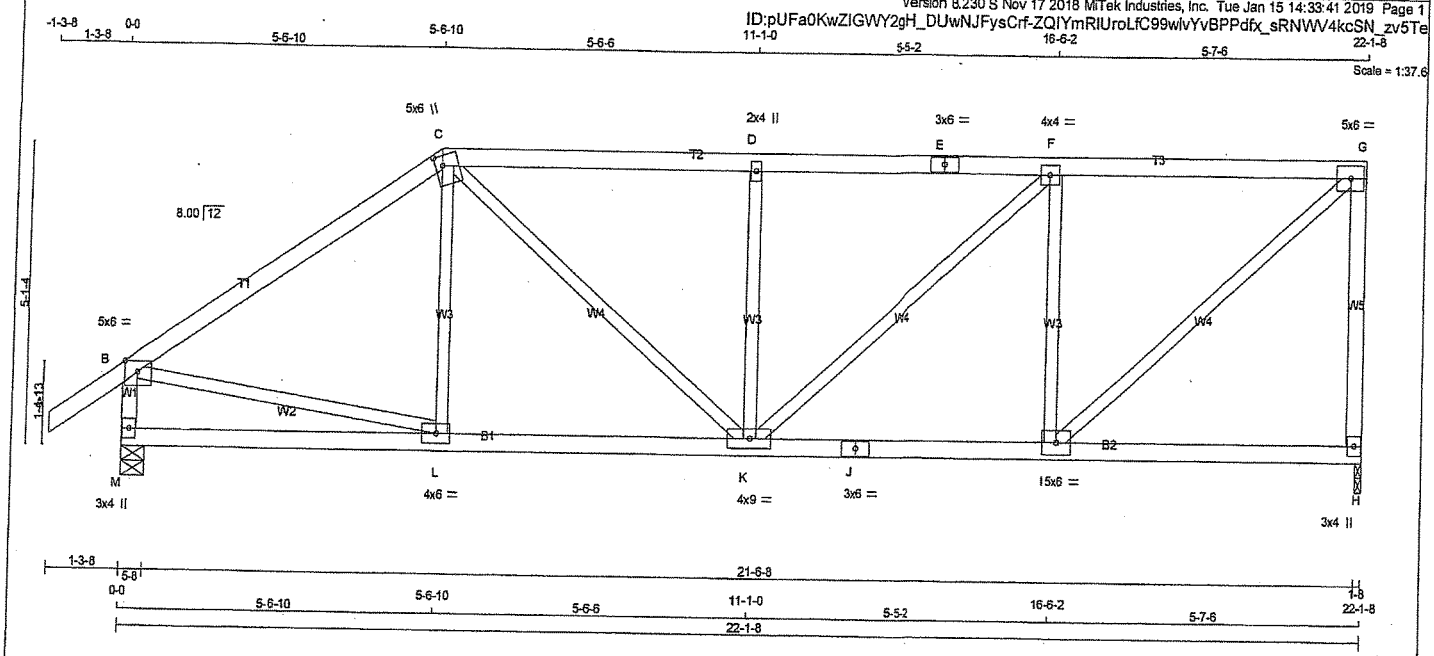

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1365	0	1365	0	0	1-8	1-8
M	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
M	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1457 / 0	-84.9	-84.9	0.55 (1)	4.74
C- D	-1632 / 0	-84.9	-84.9	0.40 (1)	4.73
D- E	-1632 / 0	-84.9	-84.9	0.42 (1)	4.71
E- F	-1632 / 0	-84.9	-84.9	0.42 (1)	4.71
F- G	-1265 / 0	-84.9	-84.9	0.40 (1)	5.22
H- G	-1278 / 0	0.0	0.0	0.56 (1)	7.13
M- B	-1394 / 0	0.0	0.0	0.14 (1)	6.90
M- L	0 / 0	-38.5	-38.5	0.23 (3)	10.00
L- K	0 / 1211	-38.5	-38.5	0.39 (2)	10.00
K- J	0 / 1265	-38.5	-38.5	0.39 (2)	10.00
J- I	0 / 1265	-38.5	-38.5	0.39 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.22 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 188	0.04 (3)
B- L	0 / 1233	0.28 (1)
I- G	0 / 1666	0.37 (1)
C- K	0 / 557	0.13 (1)
I- F	-841 / 0	0.32 (1)
K- D	-502 / 0	0.19 (1)
K- F	0 / 491	0.11 (1)

TOTAL WEIGHT = 2 X 92 = 184 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.56/1.00 (G-H:1) , BC=0.39/1.00 (I-K:2) , WB=0.37/1.00 (G-I:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.47 (J) (INPUT = 1.00)

DWG NO. TAM 7190 1145
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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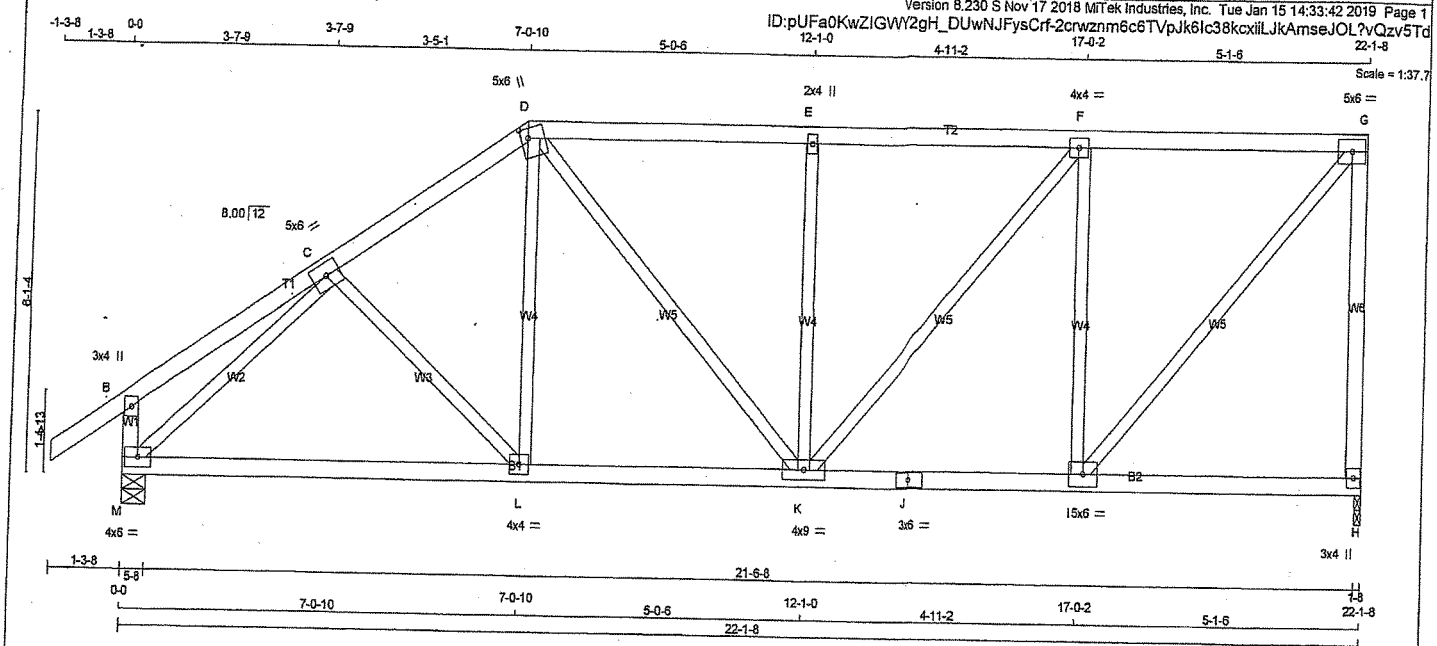

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1365	0	1385	0	0	1-8	1-8
M	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
M	1118	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LO1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 17	-84.9	-84.9	0.15 (1)	10.00
C- D	-1412 / 0	-84.9	-84.9	0.18 (1)	5.29
D- E	-1328 / 0	-84.9	-84.9	0.31 (1)	5.24
E- F	-1328 / 0	-84.9	-84.9	0.33 (1)	5.22
F- G	-977 / 0	-84.9	-84.9	0.31 (1)	5.88
H- G	-1287 / 0	0.0	0.0	0.93 (1)	7.11
M- B	-234 / 0	0.0	0.0	0.02 (1)	7.81
M- L	0 / 1164	-38.5	-38.5	0.47 (2)	10.00
L- K	0 / 1164	-38.5	-38.5	0.48 (2)	10.00
K- J	0 / 977	-38.5	-38.5	0.32 (2)	10.00
J- I	0 / 977	-38.5	-38.5	0.32 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.20 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-8 / 92	0.02 (3)
L- D	0 / 336	0.08 (2)
M- C	-1625 / 0	0.62 (1)
I- G	0 / 1479	0.33 (1)
D- K	0 / 250	0.06 (1)
I- F	-883 / 0	0.52 (1)
K- E	-456 / 0	0.27 (1)
K- F	0 / 543	0.12 (1)

TOTAL WEIGHT = 2 X 100 = 200 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.93/1.00 (G-H:1) , BC=0.48/1.00 (K-L:2) , WB=0.62/1.00 (C-M:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.41 (J) (INPUT = 1.00)

DWG NO. TAM T1901146
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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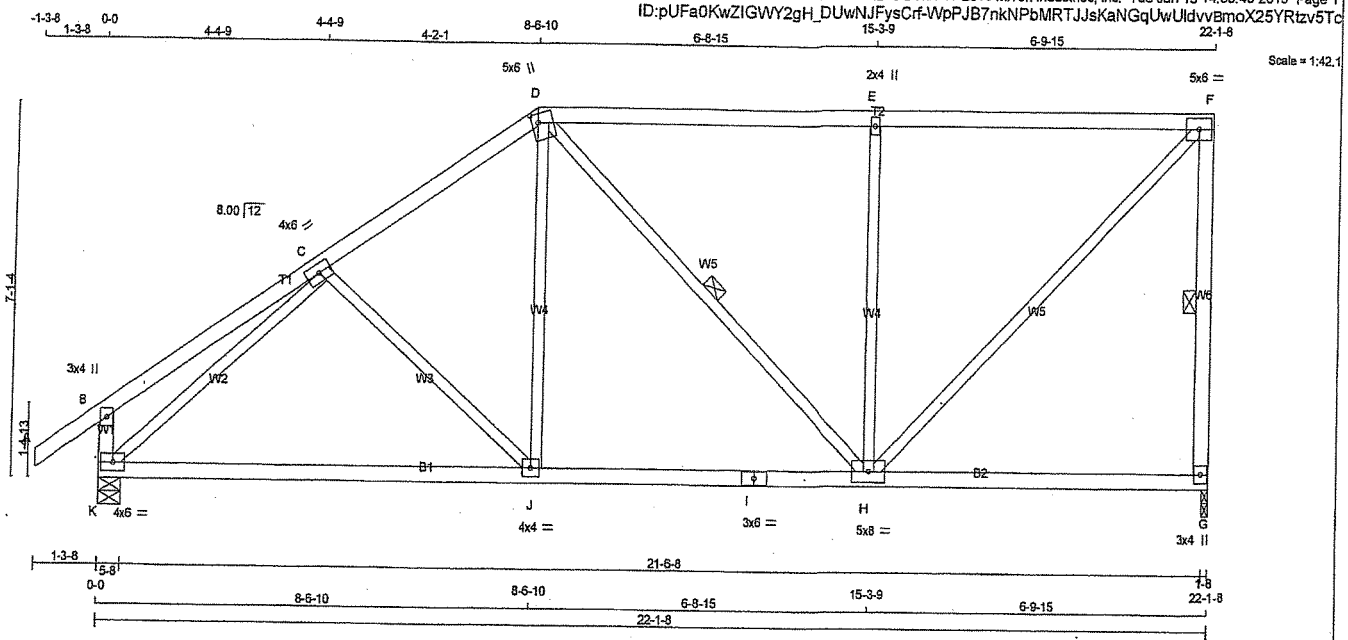


TOTAL WEIGHT = 2 X 97 = 195 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	OESCR.
A - D	2x4	ORY	No.2	SPF
D - F	2x4	ORY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	ORY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
O	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1365	0	1365	0	0	1-8	1-8
K	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
K	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

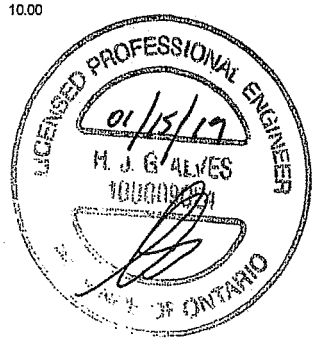
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C- J	-128 / 53	0.08 (1)
B- C	0 / 23	-84.9	-84.9	0.24 (1)	10.00	J- D	0 / 470	0.11 (2)
C- D	-1331 / 0	-84.9	-84.9	0.28 (1)	5.29	K- C	-1606 / 0	0.90 (1)
D- E	-1037 / 0	-84.9	-84.9	0.73 (1)	4.91	H- F	0 / 1465	0.33 (1)
E- F	-1037 / 0	-84.9	-84.9	0.73 (1)	4.91	D- H	-99 / 0	0.06 (2)
G- F	-1255 / 0	0.0	0.0	0.29 (1)	5.74	H- E	-714 / 0	0.62 (1)
K- B	-257 / 0	0.0	0.0	0.03 (1)	7.81			
K- J	0 / 1182	-38.5	-38.5	0.61 (2)	10.00			
J- I	0 / 1093	-38.5	-38.5	0.61 (2)	10.00			
I- H	0 / 1093	-38.5	-38.5	0.61 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/380 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 710 (0.37")

CSI: TC=0.73/1.00 (E-F:1) , BC=0.61/1.00 (J-K:2) , WB=0.90/1.00 (C-K:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

DWG NO. TAM T1901147
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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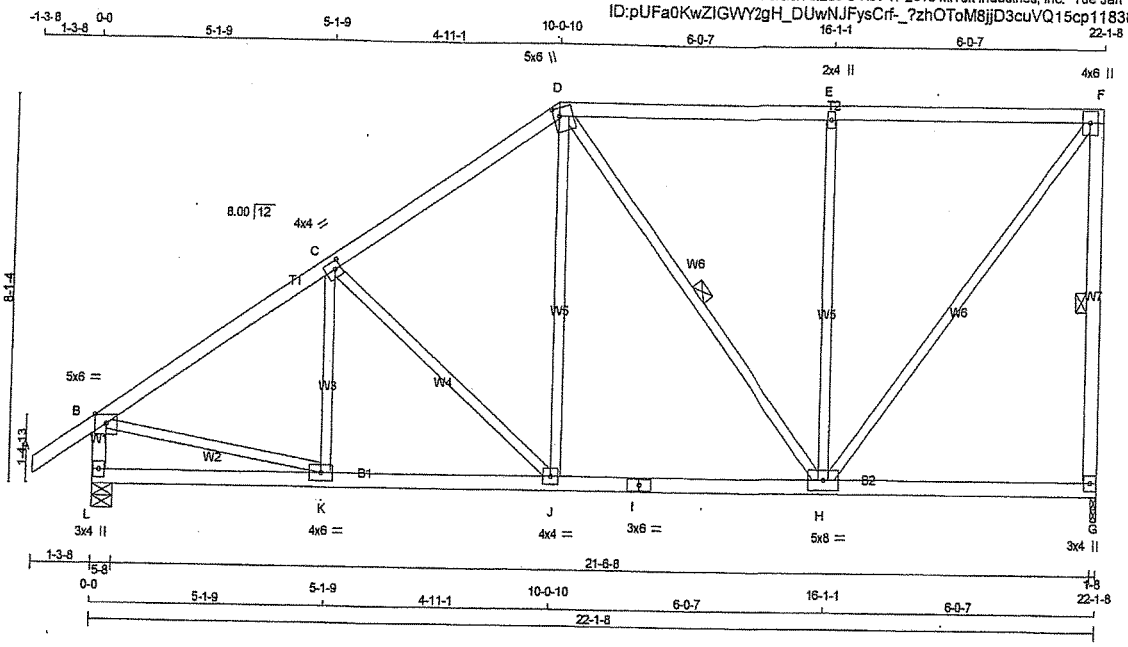


TOTAL WEIGHT = 2 X 105 = 209 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - O	2x4	ORY	No.2	SPF
O - F	2x4	ORY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
L - B	2x4	ORY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
O	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1385	0	1365	0	0	1-8	1-8
L	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
L	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

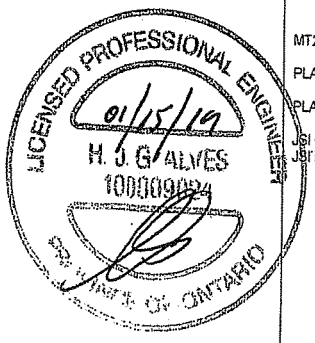
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A- B	0 / 32		-84.9	-84.9	0.11 (1)	10.00	K- C	-53 / 139		0.03 (3)
B- C	-1500 / 0		-84.9	-84.9	0.43 (1)	4.86	C- J	-410 / 0		0.35 (1)
C- D	-1191 / 0		-84.9	-84.9	0.40 (1)	5.34	J- O	0 / 491		0.11 (2)
D- E	-836 / 0		-84.9	-84.9	0.55 (1)	5.76	D- H	-222 / 0		0.15 (1)
E- F	-836 / 0		-84.9	-84.9	0.55 (1)	5.76	H- E	-634 / 0		0.60 (1)
G- F	-1271 / 0		0.0	0.0	0.38 (1)	5.71	H- F	0 / 1362		0.31 (1)
L- B	-1400 / 0		0.0	0.0	0.14 (1)	8.88	B- K	0 / 1299		0.29 (1)
L- K	0 / 0		-38.5	-38.5	0.18 (3)	10.00				
K- J	0 / 1271		-38.5	-38.5	0.34 (2)	10.00				
J- I	0 / 972		-38.5	-38.5	0.39 (2)	10.00				
I- H	0 / 972		-38.5	-38.5	0.39 (2)	10.00				
H- G	0 / 0		-38.5	-38.5	0.27 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.55/1.00 (E-F:1) , BC=0.39/1.00 (H-J:2) , WB=0.80/1.00 (E-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.39 (I) (INPUT = 1.00)

DWG NO. TAM T1901148
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T6	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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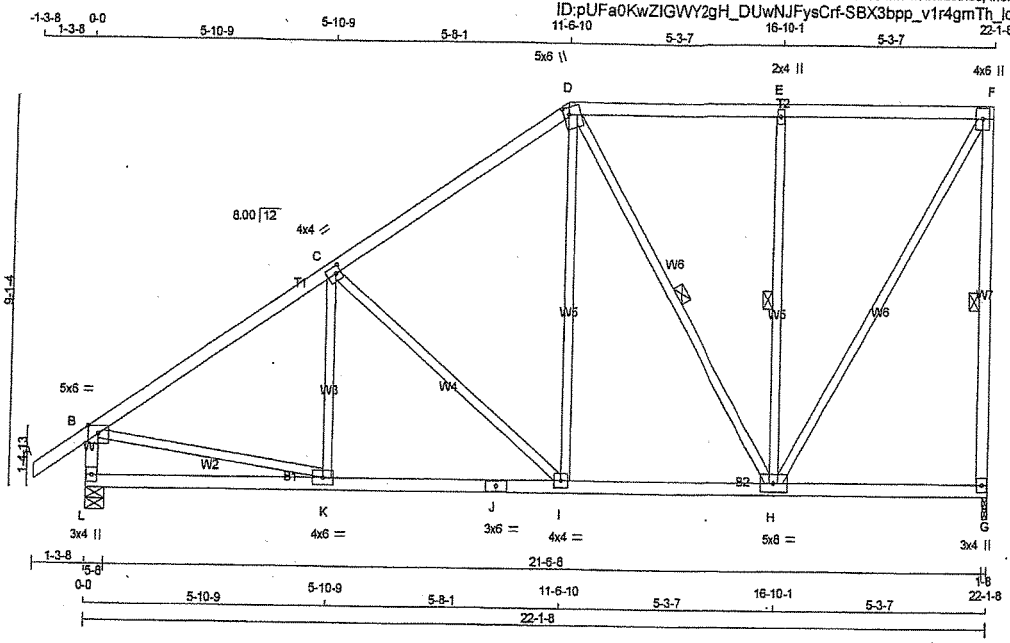


TOTAL WEIGHT = 9 X 110 = 992 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1365	0	1365	0	0	1-8	1-8
L	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
L	1116	579 / 0	232 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

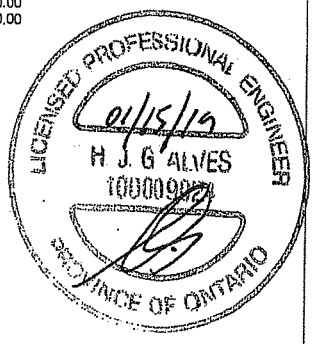
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B- C	-1497 / 0	-84.9	-84.9	0.42 (1)	4.91	
C- D	-1068 / 0	-84.9	-84.9	0.38 (1)	5.62	
D- E	-670 / 0	-84.9	-84.9	0.30 (1)	6.25	
E- F	-670 / 0	-84.9	-84.9	0.30 (1)	6.25	
G- F	-1282 / 0	0.0	0.0	0.49 (1)	5.69	
L- B	-1390 / 0	0.0	0.0	0.14 (1)	6.90	
L- K	0 / 0	-38.5	-38.5	0.25 (3)	10.00	
K- J	0 / 1272	-38.5	-38.5	0.41 (2)	10.00	
J- I	0 / 1272	-38.5	-38.5	0.41 (2)	10.00	
I- H	0 / 866	-38.5	-38.5	0.31 (2)	10.00	
H- G	0 / 0	-38.5	-38.5	0.20 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 207	0.05 (3)
C- I	-546 / 0	0.64 (1)
I- D	0 / 565	0.13 (2)
D- H	-383 / 0	0.29 (1)
H- E	-554 / 0	0.30 (1)
H- F	0 / 1296	0.29 (1)
B- K	0 / 1294	0.29 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.49/1.00 (F-G:1) , BC=0.41/1.00 (I-K:2) , WB=0.64/1.00 (C-I:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 CD MP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.46 (J) (INPUT = 1.00)

DWG NO. TAM T1901149
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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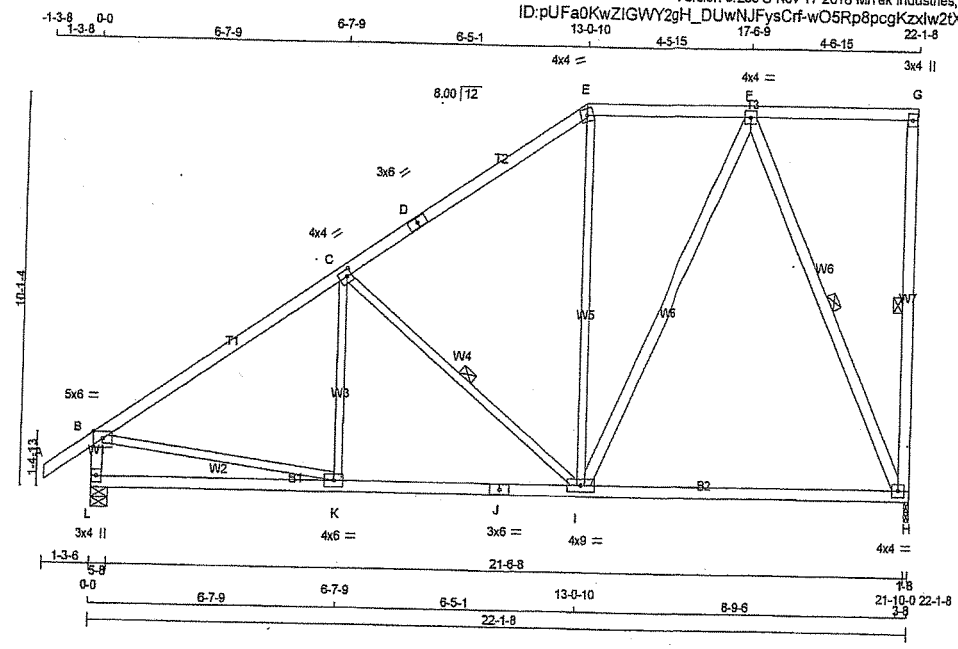


TOTAL WEIGHT = 2 X 115 = 230 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	ORY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
F - H	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT2D	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT2D	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT2D	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1365	0	1365	0	0	1-8	1-8
L	1481	0	1481	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1035	515 / 0	232 / 0	0 / 0	0 / 0	288 / 0	0 / 0
L	1116	579 / 0	232 / 0	0 / 0	D / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

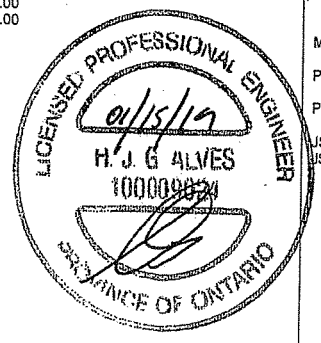
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B-C	-1472 / 0	-84.9	-84.9	0.55 (1)	4.74	
C-D	-962 / 0	-84.9	-84.9	0.49 (1)	5.64	
D-E	-962 / 0	-84.9	-84.9	0.49 (1)	5.64	
E-F	-770 / 0	-84.9	-84.9	0.23 (1)	6.25	
F-G	0 / 0	-84.9	-84.9	0.29 (1)	10.00	
H-G	-148 / 0	0.0	0.0	0.07 (1)	6.25	
L-B	-1372 / 0	0.0	0.0	0.14 (1)	6.94	
L-K	0 / 0	-38.5	-38.5	0.27 (3)	10.00	
K-J	0 / 1255	-38.5	-38.5	0.67 (2)	10.00	
J-I	0 / 1255	-38.5	-38.5	0.67 (2)	10.00	
I-H	0 / 502	-38.5	-38.5	0.56 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	0 / 215	0.05 (3)
C-I	-637 / 0	0.32 (1)
I-E	0 / 217	0.05 (3)
B-K	0 / 1272	0.29 (1)
I-F	0 / 656	0.11 (1)
F-H	-1187 / 0	0.74 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BC8C 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % DF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 642 (0.41")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.67/1.00 (I-K:2) , WB=0.74/1.00 (F-H:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.81 (B) (INPUT = 0.90)
USI METAL= 0.48 (J) (INPUT = 1.00)

DWG NO. TAM T7401/50
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	T11	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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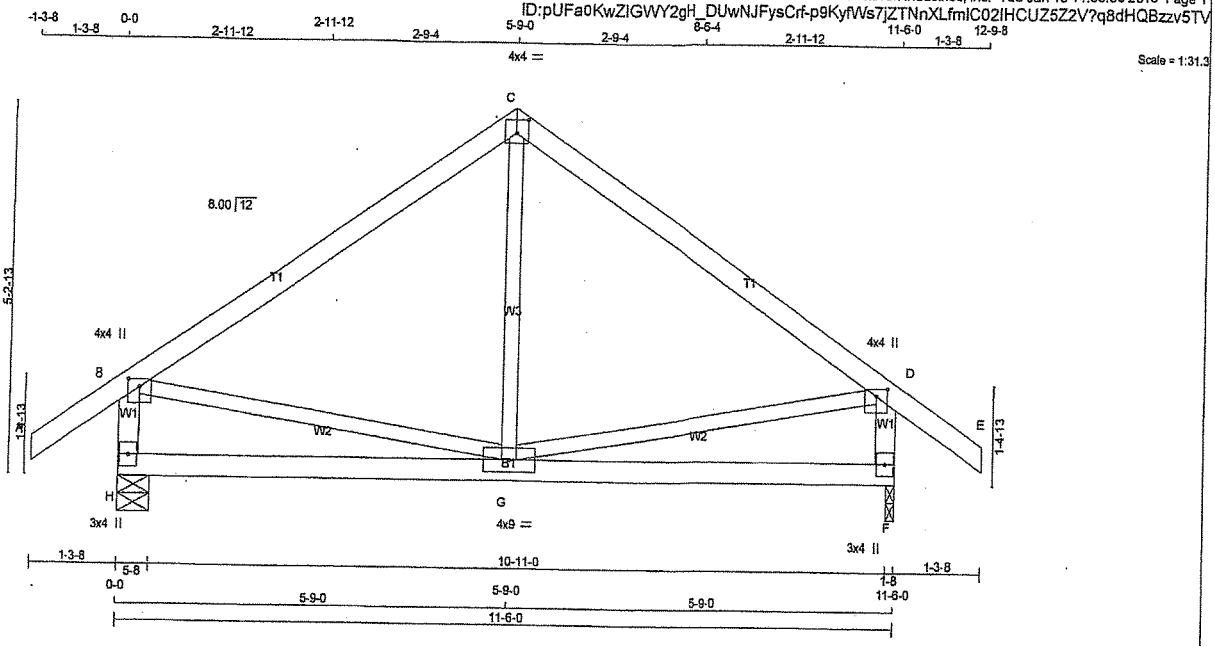

TOTAL WEIGHT = 3 X 48 = 143 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	826	0	826	0	0	5-8	5-8
F	826	0	826	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	618	332 / 0	121 / 0	0 / 0	0 / 0	166 / 0	0 / 0
F	618	332 / 0	121 / 0	0 / 0	0 / 0	186 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-536 / 0	-84.9	-84.9	0.36 (1)	6.25
C- D	-536 / 0	-84.9	-84.9	0.36 (1)	6.25
D- E	0 / 32	-84.9	-84.9	0.11 (1)	10.00
H- B	-742 / 0	0.0	0.0	0.08 (1)	7.81
F- D	-742 / 0	0.0	0.0	0.08 (1)	7.81
H- G	0 / 0	-38.5	-38.5	0.29 (3)	10.00
G- F	0 / 0	-38.5	-38.5	0.29 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 255	0.06 (3)
B- G	0 / 454	0.10 (1)
G- D	0 / 454	0.10 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009624
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.29/1.00 (G-H:3) , WB=0.10/1.00 (8-G:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

DWG NO. TAM T1901454
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300403	G11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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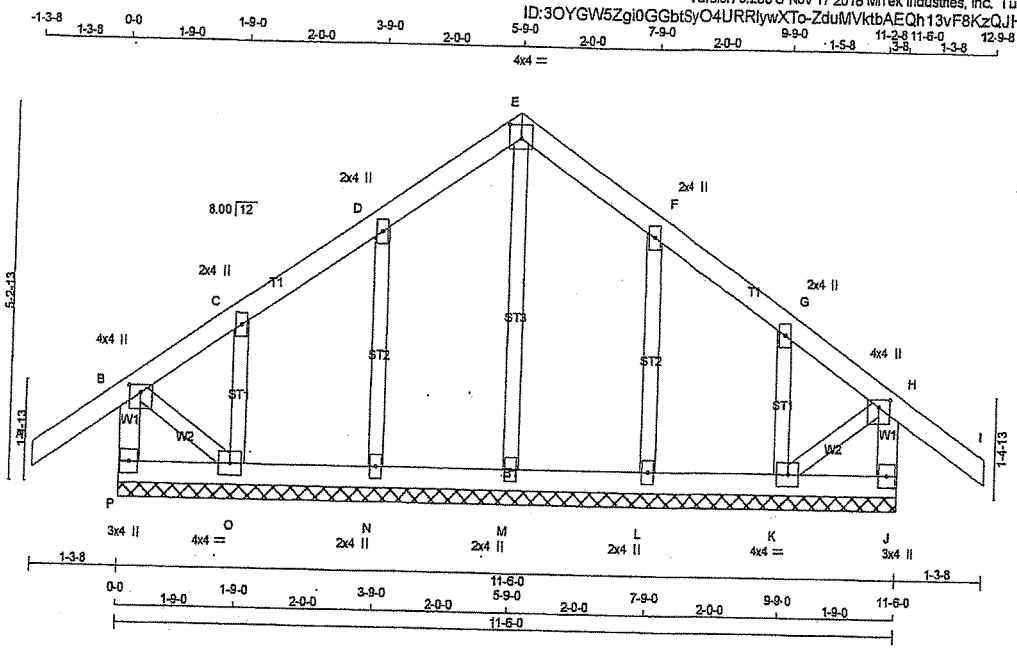

TOTAL WEIGHT = 50 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	ORY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	PACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-41 / 0	-84.9	-84.9	0.11 (1)	8.25
C-D	-4 / 0	-84.9	-84.9	0.05 (1)	10.00
D-E	-18 / 0	-84.9	-84.9	0.05 (1)	6.25
E-F	-18 / 0	-84.9	-84.9	0.05 (1)	6.25
F-G	-4 / 0	-84.9	-84.9	0.05 (1)	10.00
G-H	-41 / 0	-84.9	-84.9	0.11 (1)	8.25
H-I	0 / 32	-84.9	-84.9	0.11 (1)	10.00
P-B	-248 / 0	0.0	0.0	0.03 (1)	7.81
J-H	-248 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 0	-38.5	-38.5	0.03 (3)	10.00
O-N	0 / 11	-38.5	-38.5	0.03 (3)	10.00
N-M	0 / 6	-38.5	-38.5	0.02 (3)	10.00
M-L	0 / 6	-38.5	-38.5	0.02 (3)	10.00
L-K	0 / 11	-38.5	-38.5	0.03 (3)	10.00
K-J	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	-130 / 0	0.05 (1)
N-D	-205 / 0	0.05 (1)
O-C	-95 / 0	0.02 (1)
L-F	-205 / 0	0.05 (1)
K-G	-95 / 0	0.02 (1)
B-O	0 / 17	0.00 (1)
K-H	0 / 17	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (K-L:3) , WB=0.05/1.00 (E-M:1) , SSI=0.08/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.17 (B) (INPUT = 0.90)
SI METAL= 0.10 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G ALVES
100090942
PROVINCE OF ONTARIO

PERMIT NO. TAM 17901141
STRUCTURAL
FOR CONSENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T14	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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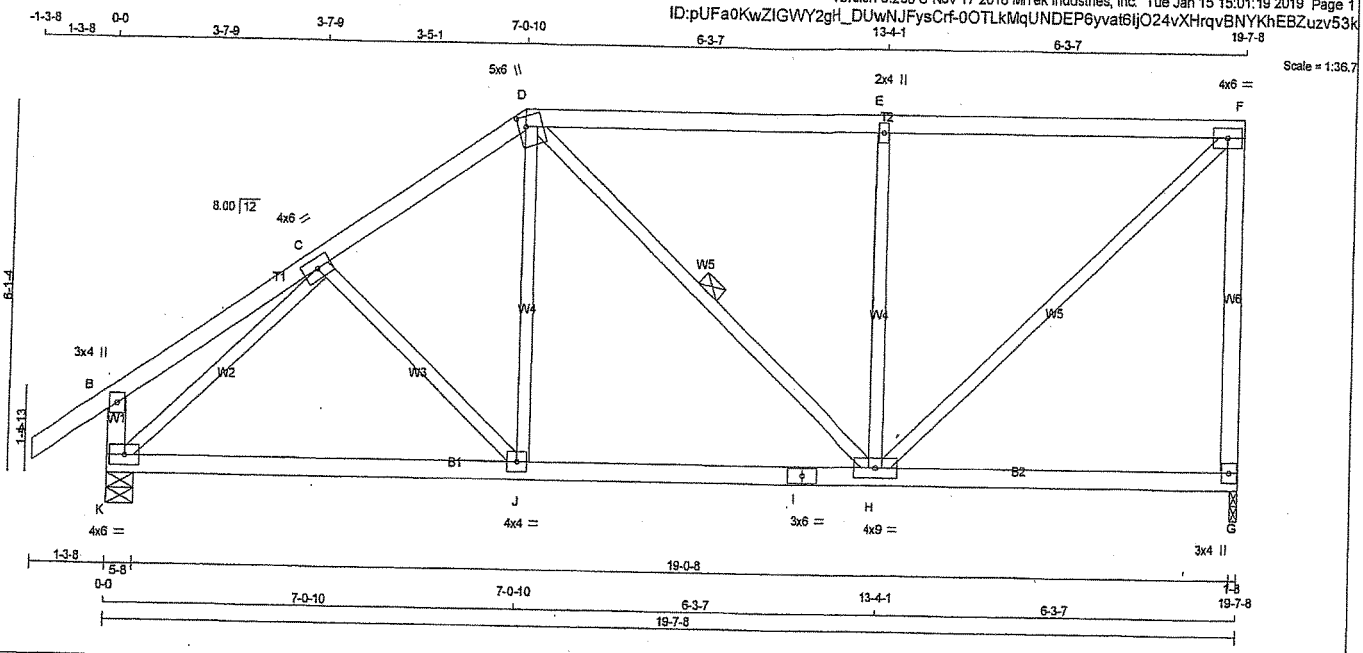


TOTAL WEIGHT = 2 X 85 = 171 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	ORY	No.2	SPF
K - I	2x4	ORY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-H.

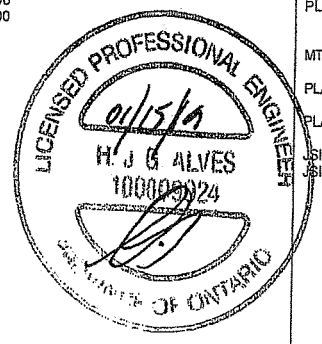
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 18	-84.9	-84.9	0.16 (1)	10.00
C- O	-1186 / 0	-84.9	-84.9	0.18 (1)	5.66
D- E	-985 / 0	-84.9	-84.9	0.61 (1)	5.29
E- F	-985 / 0	-84.9	-84.9	0.61 (1)	5.29
G- F	-1110 / 0	0.0	0.0	0.80 (1)	7.52
K- B	-234 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 1007	-38.5	-38.5	0.45 (2)	10.00
J- I	0 / 976	-38.5	-38.5	0.45 (2)	10.00
I- H	0 / 976	-38.5	-38.5	0.45 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.25 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-50 / 71	0.02 (1)
J- D	0 / 364	0.08 (2)
D- H	-36 / 13	0.02 (3)
H- E	-660 / 0	0.39 (1)
H- F	0 / 1338	0.30 (1)
K- C	-1405 / 0	0.54 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.80/1.00 (F-G:1) , BC=0.45/1.00 (H-J:2) , WB=0.54/1.00 (C-K:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (F) (INPUT = 0.90)
SSI METAL= 0.34 (C) (INPUT = 1.00)

DWG NO. TAM T1901157
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T15	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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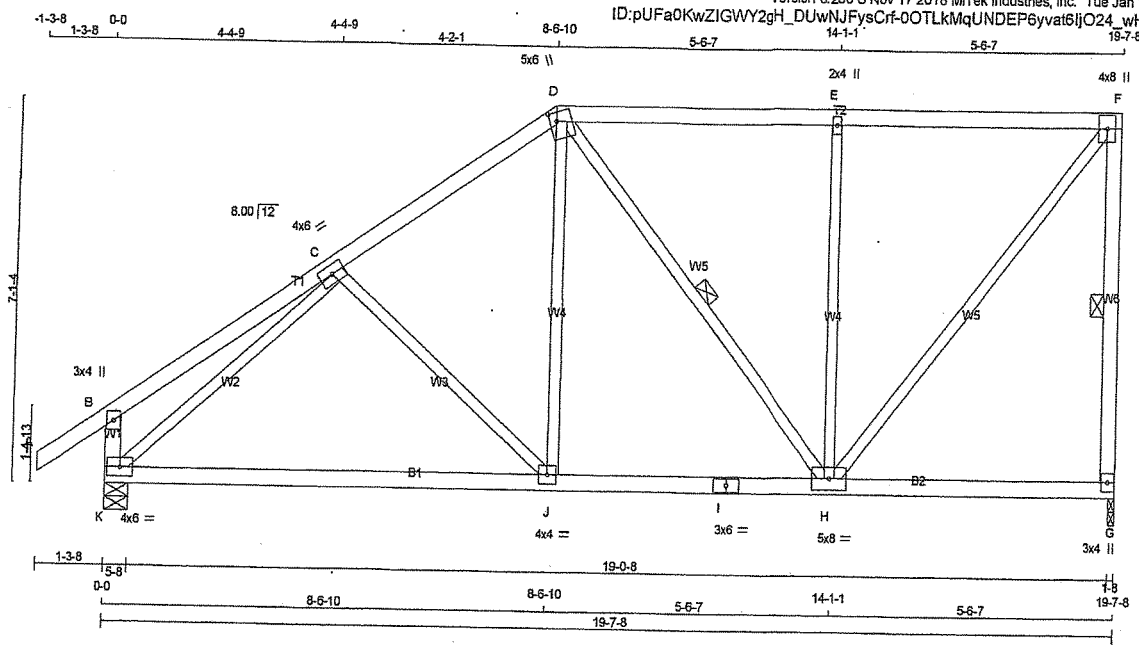


TOTAL WEIGHT = 2 X 90 = 181 lb [M]

LUMBER

N. L. G. A. RULES

CHOROS	SIZE		LUMBER	OESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	ORY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	ORY	No.2	SPF
K - I	2x4	ORY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is In inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 23	-84.9	-84.9	0.24 (1)	10.00
C- D	-1098 / 0	-84.9	-84.9	0.27 (1)	5.70
D- E	-766 / 0	-84.9	-84.9	0.45 (1)	6.15
E- F	-766 / 0	-84.9	-84.9	0.46 (1)	6.15
G- F	-1118 / 0	0.0	0.0	0.25 (1)	6.00
K- B	-256 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 1015	-38.5	-38.5	0.58 (2)	10.00
J- I	0 / 899	-38.5	-38.5	0.57 (2)	10.00
I- H	0 / 899	-38.5	-38.5	0.57 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
C- J	-165 / 34	0.10 (1)
J- D	0 / 482	0.11 (2)
D- H	-218 / 0	0.11 (2)
H- E	-581 / 0	0.51 (1)
H- F	0 / 1210	0.27 (1)
K- C	-1379 / 0	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 635 (0.37")

CSI: TC=0.46/1.00 (E-F:1) , BC=0.58/1.00 (J-K:2) , WB=0.77/1.00 (C-K:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

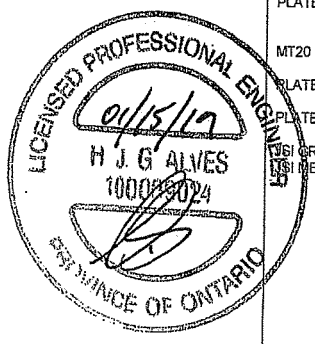
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LUMBER GRIP= 0.75 (F) (INPUT = 0.90)
PLATE METAL= 0.35 (F) (INPUT = 1.00)



DWG NO. TAM 190 1158
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T16	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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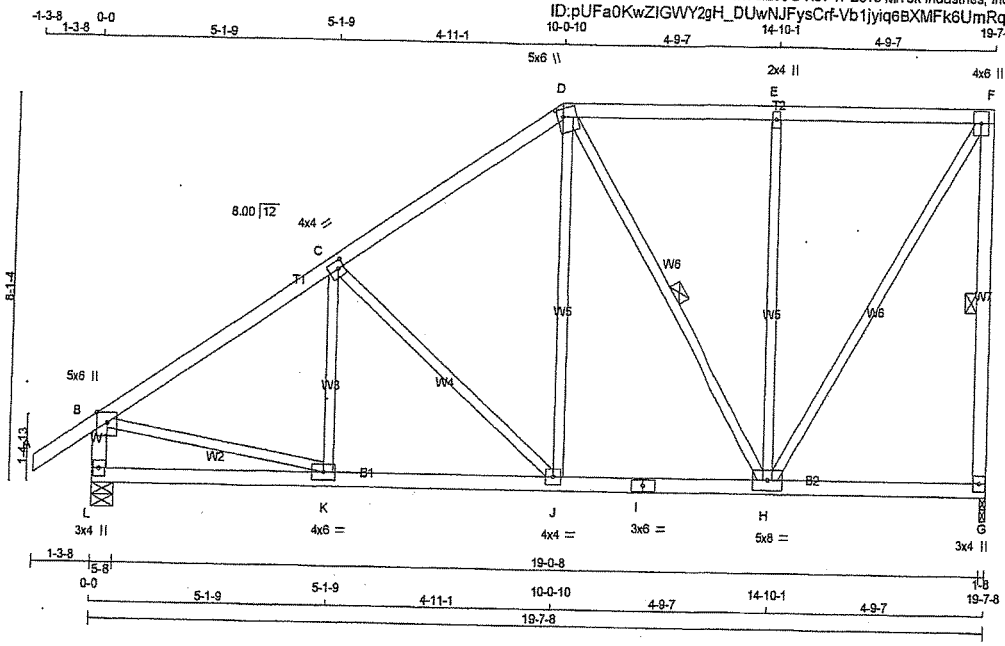

TOTAL WEIGHT = 2 X 98 = 196 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
G	1210	0	1210	0	0	1-8	1-8
L	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B- C	-1291 / 0	-84.9	-84.9	0.30 (1)		5.35
C- D	-948 / 0	-84.9	-84.9	0.28 (1)		6.02
D- E	-804 / 0	-84.9	-84.9	0.25 (1)		6.25
E- F	-604 / 0	-84.9	-84.9	0.25 (1)		6.25
G- F	-1135 / 0	0.0	0.0	0.34 (1)		5.96
L- B	-1247 / 0	0.0	0.0	0.13 (1)		7.19
L- K	0 / 0	-38.5	-38.5	0.19 (3)		10.00
K- J	0 / 1097	-38.5	-38.5	0.33 (2)		10.00
J- I	0 / 770	-38.5	-38.5	0.26 (2)		10.00
I- H	0 / 770	-38.5	-38.5	0.26 (2)		10.00
H- G	0 / 0	-38.5	-38.5	0.16 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-10 / 166	0.04 (3)
C- J	-447 / 0	0.38 (1)
J- D	0 / 484	0.11 (2)
D- H	-319 / 0	0.18 (1)
H- E	-501 / 0	0.63 (1)
H- F	0 / 1149	0.26 (1)
B- K	0 / 1121	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.34/1.00 (F-G:1) , BC=0.33/1.00 (J-K:2) , WB=0.63/1.00 (E-H:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100089124
PROVINCE OF ONTARIO

DWG NO. TAM 77901159
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T17	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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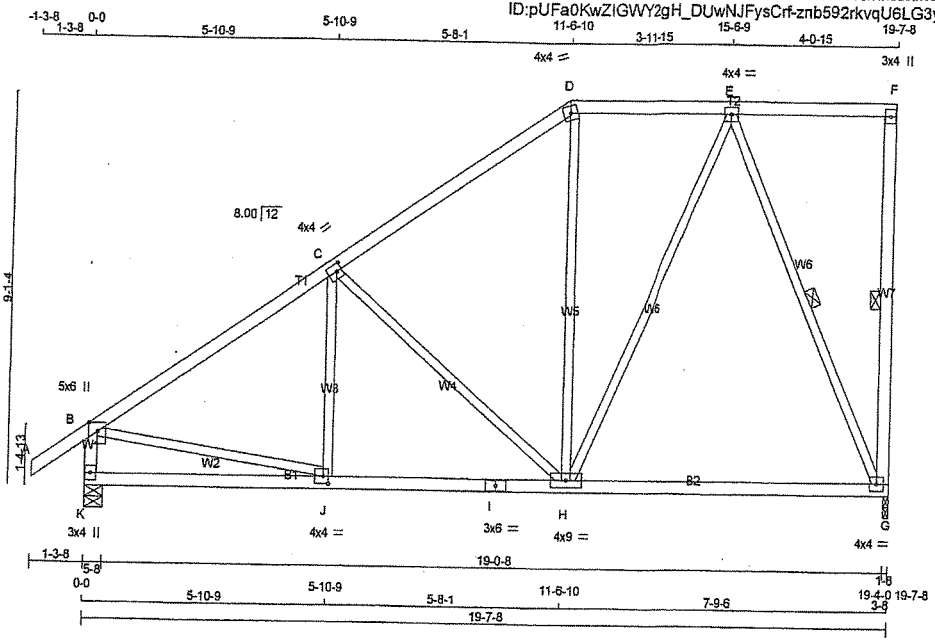


TOTAL WEIGHT = 9 X 97 = 870 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

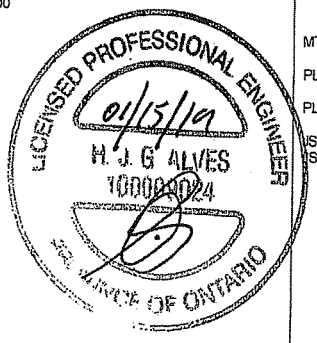
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B- C	-1272 / 0	-84.9	-84.9	0.40 (1)	5.24	
C- D	-843 / 0	-84.9	-84.9	0.38 (1)	6.14	
D- E	-674 / 0	-84.9	-84.9	0.18 (1)	6.25	
E- F	0 / 0	-84.9	-84.9	0.23 (1)	10.00	
G- F	-132 / 0	0.0	0.0	0.05 (1)	6.25	
K- B	-1230 / 0	0.0	0.0	0.13 (1)	7.23	
K- J	0 / 0	-38.5	-38.5	0.22 (3)	10.00	
J- I	0 / 1085	-38.5	-38.5	0.54 (2)	10.00	
I- H	0 / 1085	-38.5	-38.5	0.54 (2)	10.00	
H- G	0 / 441	-38.5	-38.5	0.44 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 180	0.04 (3)
C- H	-545 / 0	0.64 (1)
H- D	0 / 189	0.04 (3)
B- J	0 / 1103	0.25 (1)
H- E	0 / 579	0.13 (1)
E- G	-1050 / 0	0.70 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 893 (0.26")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.54/1.00 (H-J:2) , WB=0.70/1.00 (E-G:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.90 (J) (INPUT = 0.90)
SI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1901160
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T18	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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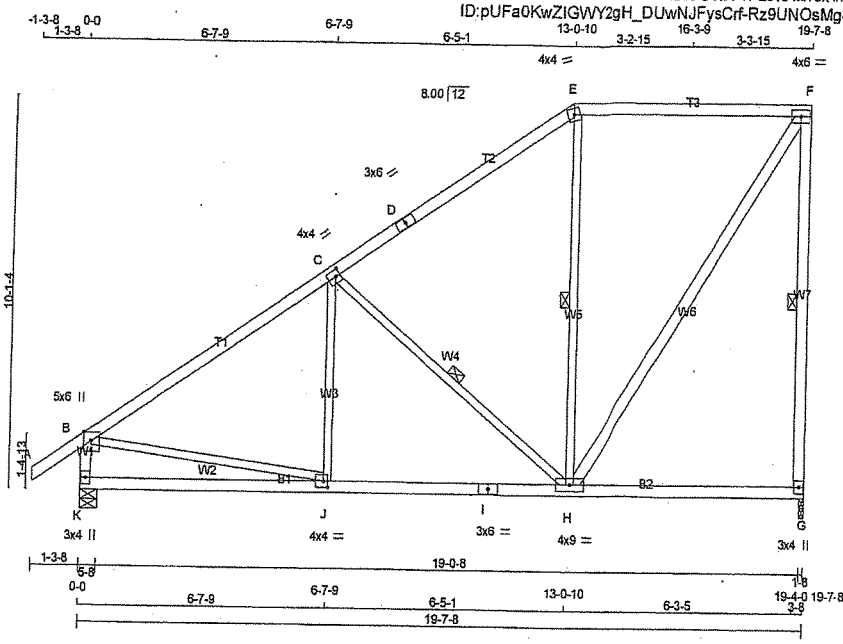


TOTAL WEIGHT = 2 X 98 = 197 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - E	2x4	ORY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
K - B	2x4	ORY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF
H - F	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

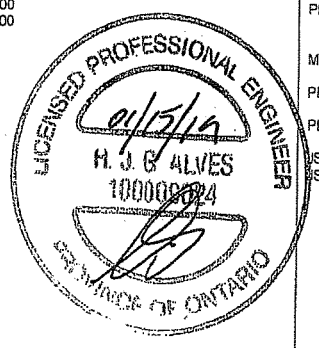
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00
B-C	-1255 / 0	-84.9	-84.9	1	0.53 (1)	5.06
C-D	-700 / 0	-84.9	-84.9	1	0.48 (1)	6.25
D-E	-700 / 0	-84.9	-84.9	1	0.48 (1)	6.25
E-F	-549 / 0	-84.9	-84.9	1	0.47 (1)	6.25
G-F	-1110 / 0	0.0	0.0	1	0.55 (1)	6.01
K-B	-1223 / 0	0.0	0.0	1	0.13 (1)	7.25
K-J	0 / 0	-38.5	-38.5	3	0.30 (3)	10.00
J-I	0 / 1075	-38.5	-38.5	2	0.43 (2)	10.00
I-H	0 / 1075	-38.5	-38.5	2	0.43 (2)	10.00
H-G	0 / 0	-38.5	-38.5	3	0.29 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	0 / 264	0.06 (3)
C-H	-687 / 0	0.34 (1)
H-E	-115 / 112	0.08 (1)
B-J	0 / 1090	0.25 (1)
H-F	0 / 997	0.16 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.55/1.00 (F-G:1) , BC=0.43/1.00 (H-J:2) , WB=0.34/1.00 (C-H:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.87 (J) (INPUT = 0.90)
USI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM 1901161
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300441	T30AZ2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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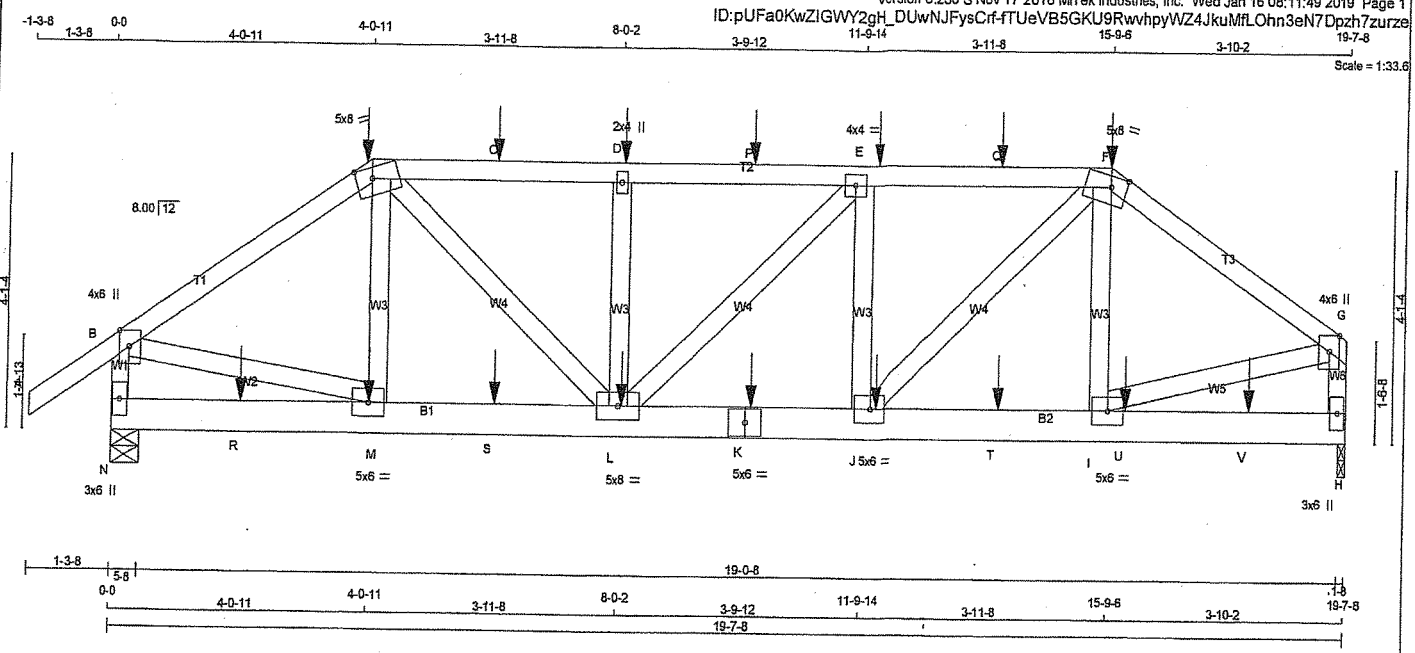

TOTAL WEIGHT = 2 X 104 = 208 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A-C 1	12	SIDE(61.0)
C-F 1	12	SIDE(61.0)
F-G 1	12	SIDE(61.0)
N-B 1	12	TOP
H-G 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
N-K 2	12	SIDE(183.1)
K-H 2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS		
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.00	3.00
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I, J, M						
I	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DWG NO. TAM 1190/225
STRUCTURAL
COMPONENT ONLY 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
N	2202	0	2202	0	0	5-8	5-8
H	2105	0	2105	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1656	865 / 0	338 / 0	0 / 0	0 / 0	454 / 0	0 / 0
H	1592	806 / 0	344 / 0	0 / 0	0 / 0	442 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B-C	-2415 / 0	-84.9	-84.9	0.16 (1)	5.63
C-O	-2821 / 0	-84.9	-84.9	0.17 (1)	5.29
O-D	-2821 / 0	-84.9	-84.9	0.17 (1)	5.29
D-P	-2821 / 0	-84.9	-84.9	0.16 (1)	5.29
P-E	-2821 / 0	-84.9	-84.9	0.16 (1)	5.29
E-O	-2803 / 0	-84.9	-84.9	0.17 (1)	5.29
O-F	-2803 / 0	-84.9	-84.9	0.17 (1)	5.29
F-G	-2324 / 0	-84.9	-84.9	0.14 (1)	5.73
N-B	-2109 / 0	0.0	0.0	0.12 (1)	7.63
H-G	-2006 / 0	0.0	0.0	0.11 (1)	7.78
N-R	0 / 0	-38.5	-38.5	0.05 (2)	10.00
R-M	0 / 0	-38.5	-38.5	0.05 (2)	10.00
M-S	0 / 1999	-38.5	-38.5	0.16 (1)	10.00
S-L	0 / 1999	-38.5	-38.5	0.16 (1)	10.00
L-K	0 / 2803	-38.5	-38.5	0.23 (1)	10.00
K-J	0 / 2803	-38.5	-38.5	0.23 (1)	10.00
J-T	0 / 1919	-38.5	-38.5	0.15 (1)	10.00
T-I	0 / 1919	-38.5	-38.5	0.16 (1)	10.00
I-U	0 / 0	-38.5	-38.5	0.05 (3)	10.00
U-V	0 / 0	-38.5	-38.5	0.05 (3)	10.00
V-H	0 / 0	-38.5	-38.5	0.05 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-176 / 139	0.02 (1)
C-L	0 / 1127	0.10 (1)
L-D	-560 / 0	0.05 (1)
L-E	0 / 25	0.00 (1)
J-E	-583 / 0	0.05 (1)
J-F	0 / 1212	0.11 (1)
I-F	-265 / 91	0.02 (1)
B-M	0 / 2062	0.18 (1)
I-G	0 / 2055	0.18 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HERE	CONN.
C	4-0-11	-50	-56	—	FRONT	VERT	DEAD	—	—
C	4-0-11	-110	-110	—	BACK	VERT	TOTAL	—	—
C	4-0-11	-231	-231	—	FRONT	VERT	SNOW	—	—
D	8-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
E	12-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
F	15-9-8	-48	-53	—	FRONT	VERT	DEAD	—	—
F	15-9-6	-108	-108	—	BACK	VERT	TOTAL	—	—
F	15-9-6	-219	-219	—	FRONT	VERT	SNOW	—	—
J	12-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
K	10-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
L	8-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
M	4-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
O	6-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
P	10-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	48.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EDUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.23/1.00 (J-L:1) , WB=0.18/1.00 (B-M:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100099774
PROVINCE OF ONTARIO

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300441	T30AZ2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	14-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
R	2-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
S	6-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
T	14-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
U	16-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
V	18-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/16/19
H J G ALVES
100089624
PROVINCE OF ONTARIO

DWG NO. TAM T1901225
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	T31A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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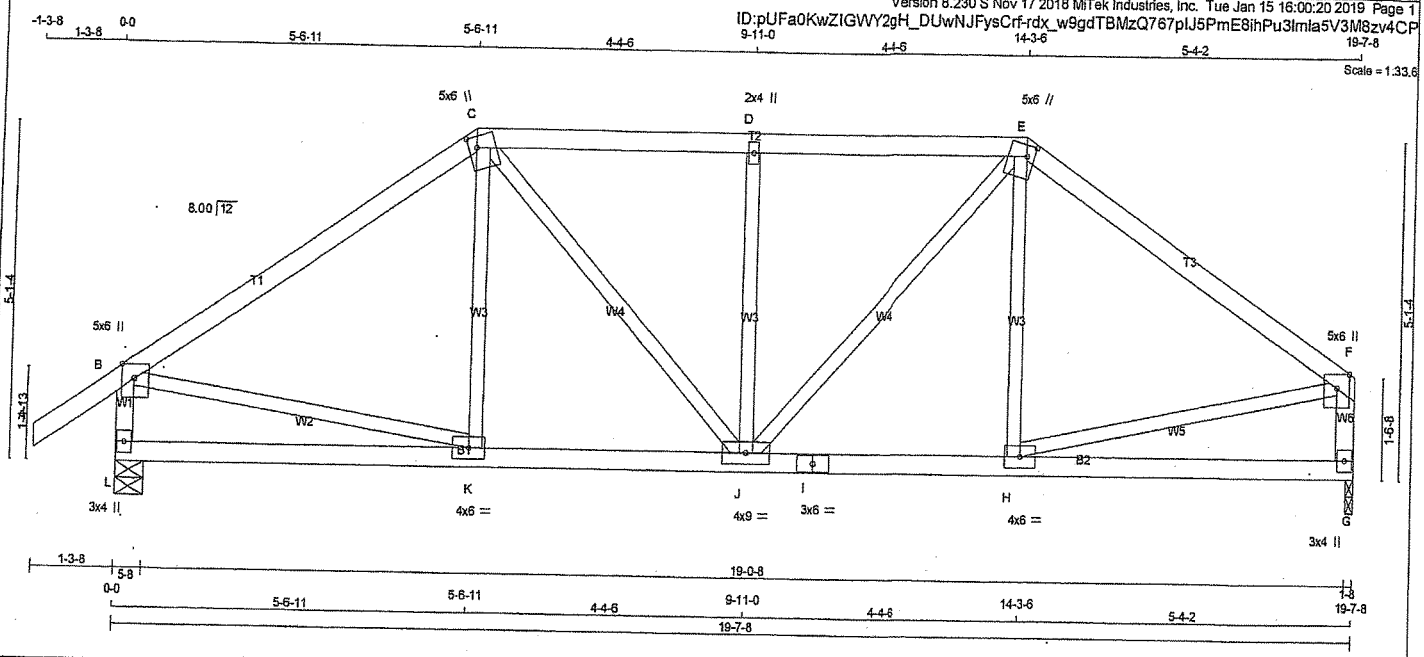


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	5.0	8.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1327	0	1327	0	0	5-8	5-8
G	1210	0	1210	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

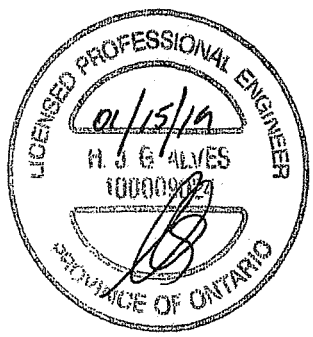
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	K-C	0 / 189	0.04 (3)
B-C	-1239 / 0	-84.9	-84.9	0.37 (1)	5.34	C-J	0 / 387	0.08 (1)
C-D	-1277 / 0	-84.9	-84.9	0.22 (1)	5.47	J-D	-448 / 0	0.17 (1)
D-E	-1277 / 0	-84.9	-84.9	0.22 (1)	5.47	J-E	0 / 404	0.09 (1)
E-F	-1210 / 0	-84.9	-84.9	0.34 (1)	5.43	H-E	0 / 167	0.04 (3)
L-B	-1238 / 0	0.0	0.0	0.13 (1)	7.22	B-K	0 / 1048	0.24 (1)
G-F	-1125 / 0	0.0	0.0	0.12 (1)	7.48	H-F	0 / 1031	0.23 (1)
L-K	0 / 0	-38.5	-38.5	0.21 (3)	10.00			
K-J	0 / 1030	-38.5	-38.5	0.35 (2)	10.00			
J-I	0 / 1006	-38.5	-38.5	0.33 (2)	10.00			
I-H	0 / 1006	-38.5	-38.5	0.33 (2)	10.00			
H-G	0 / 0	-38.5	-38.5	0.20 (3)	10.00			



TOTAL WEIGHT = 81 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT. CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.35/1.00 (J-K:2) , WB=0.24/1.00 (B-K:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM 7901169
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T79A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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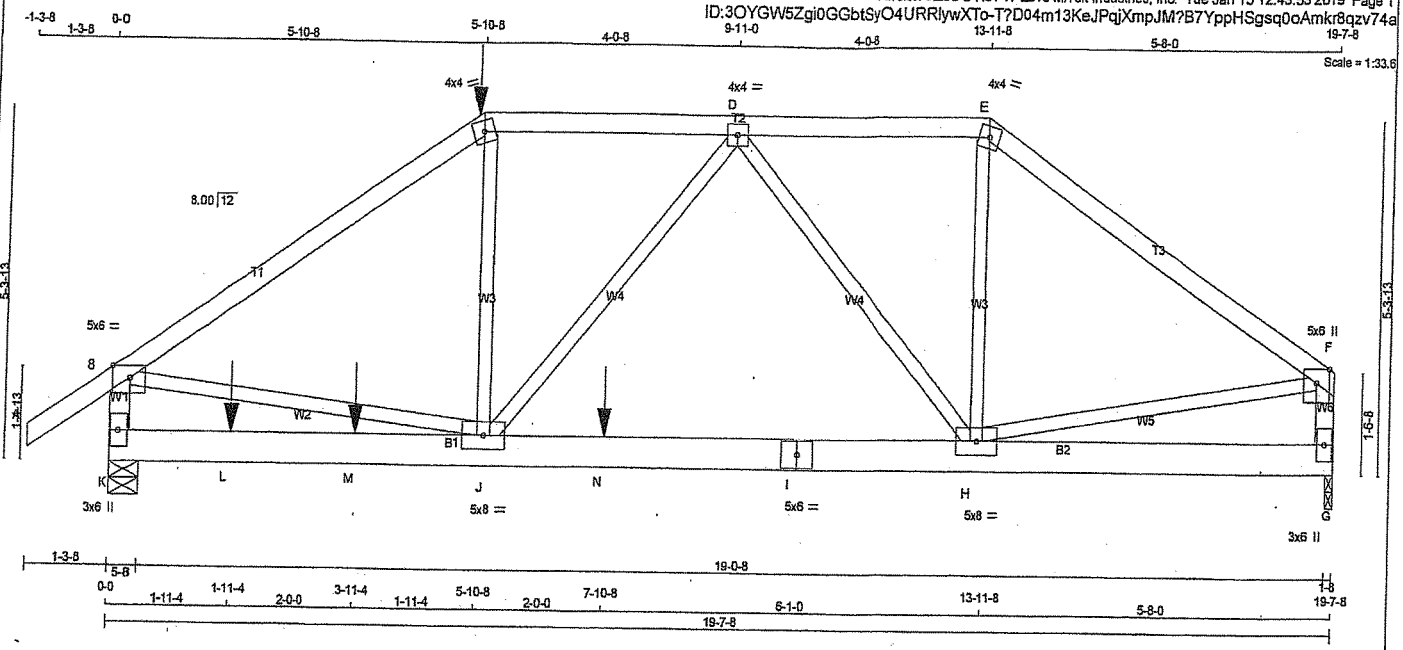


TOTAL WEIGHT = 2 X 91 = 181 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHOROS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-E	1	12	SIDE(61.0)
E-F	1	12	TOP
K-B	1	12	TOP
G-F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K-I	2	12	SIDE(0.0)
I-G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
J-C	1	6	SIDE(19.6)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
O	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	2698	0	2696	0	0	5-8	5-8
G	1973	0	1973	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	2018	1090 / 0	385 / 0	0 / 0	0 / 0	541 / 0	0 / 0
G	1479	788 / 0	292 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

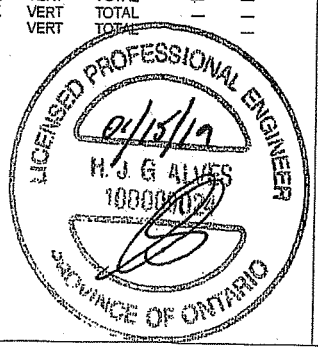
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B-C	-3330 / 0	-84.9	-84.9	0.40 (1)	4.74
C-D	-2799 / 0	-84.9	-84.9	0.15 (1)	5.33
D-E	-2026 / 0	-84.9	-84.9	0.13 (1)	6.04
E-F	-2398 / 0	-84.9	-84.9	0.32 (1)	5.45
K-B	-2674 / 0	0.0	0.0	0.15 (1)	6.97
G-F	-1962 / 0	0.0	0.0	0.11 (1)	7.81
K-L	0 / 0	-38.5	-38.5	0.38 (1)	10.00
L-M	0 / 0	-38.5	-38.5	0.38 (1)	10.00
M-J	0 / 0	-38.5	-38.5	0.38 (1)	10.00
J-N	0 / 2570	-38.5	-38.5	0.64 (1)	10.00
N-I	0 / 2570	-38.5	-38.5	0.64 (1)	10.00
I-H	0 / 2570	-38.5	-38.5	0.64 (1)	10.00
H-G	0 / 0	-38.5	-38.5	0.22 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	0 / 919	0.11 (2)
J-D	0 / 372	0.05 (1)
D-H	-882 / 0	0.32 (1)
H-E	0 / 953	0.12 (1)
B-J	0 / 2805	0.35 (1)
H-F	0 / 2030	0.25 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-543	-543	—	BACK	VERT	TOTAL	—	—
L	1-11-4	-75	-95	—	BACK	VERT	TOTAL	—	—
M	3-11-4	-75	-95	—	BACK	VERT	TOTAL	—	—
N	7-10-8	-1440	-1440	—	TOP	VERT	TOTAL	—	—



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.64/1.00 (H-J:1) , WB=0.35/1.00 (B-J:1) , SSI=0.43/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BENO=1.00 CDMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.47 (F) (INPUT = 1.00)

DWG NO. TAM T79A01/29
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T80A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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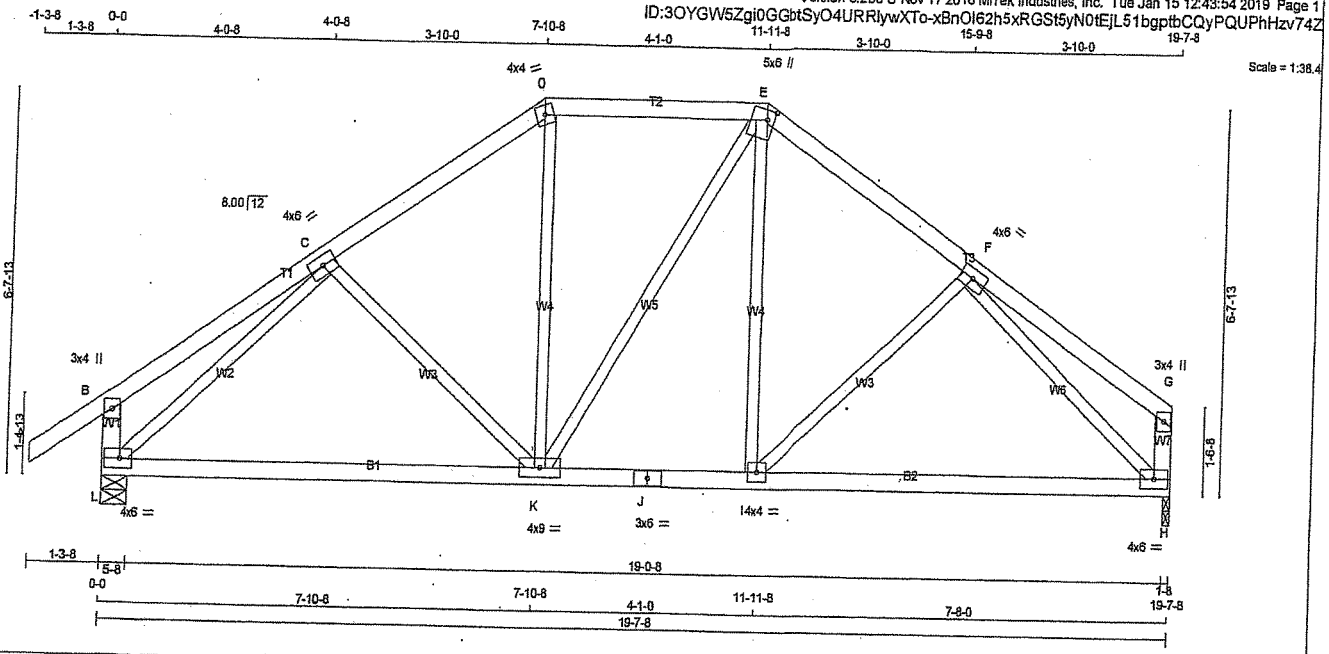


TOTAL WEIGHT = 85 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
L	1327	0	1327	0	0	5-8	5-8
H	1210	0	1210	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0
H	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

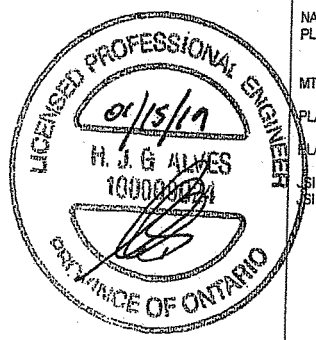
TOTAL LOAD CASES: (4)

OHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 21	-84.9	-84.9	0.20 (1)	10.00
C-D	-1135 / 0	-84.9	-84.9	0.17 (1)	5.78
D-E	-931 / 0	-84.9	-84.9	0.19 (1)	6.19
E-F	-1122 / 0	-84.9	-84.9	0.16 (1)	5.82
F-G	0 / 21	-84.9	-84.9	0.19 (1)	10.00
L-B	-246 / 0	0.0	0.0	0.03 (1)	7.81
H-G	-120 / 0	0.0	0.0	0.01 (1)	7.81
L-K	0 / 1011	-38.5	-38.5	0.52 (2)	10.00
K-J	0 / 920	-38.5	-38.5	0.47 (2)	10.00
J-I	0 / 920	-38.5	-38.5	0.47 (2)	10.00
I-H	0 / 969	-38.5	-38.5	0.50 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-K	-115 / 49	0.06 (1)
K-D	0 / 343	0.08 (2)
K-E	0 / 20	0.00 (1)
I-E	0 / 318	0.07 (2)
I-F	-72 / 66	0.04 (1)
L-C	-1389 / 0	0.66 (1)
F-H	-1366 / 0	0.61 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	48.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 821 (0.29")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.52/1.00 (K-L:2) , WB=0.66/1.00 (C-L:1) , SSI=0.19/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (J) (INPUT = 0.90)
SI METAL= 0.82 (J) (INPUT = 1.00)

DWG NO. TAM T1901130
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T81	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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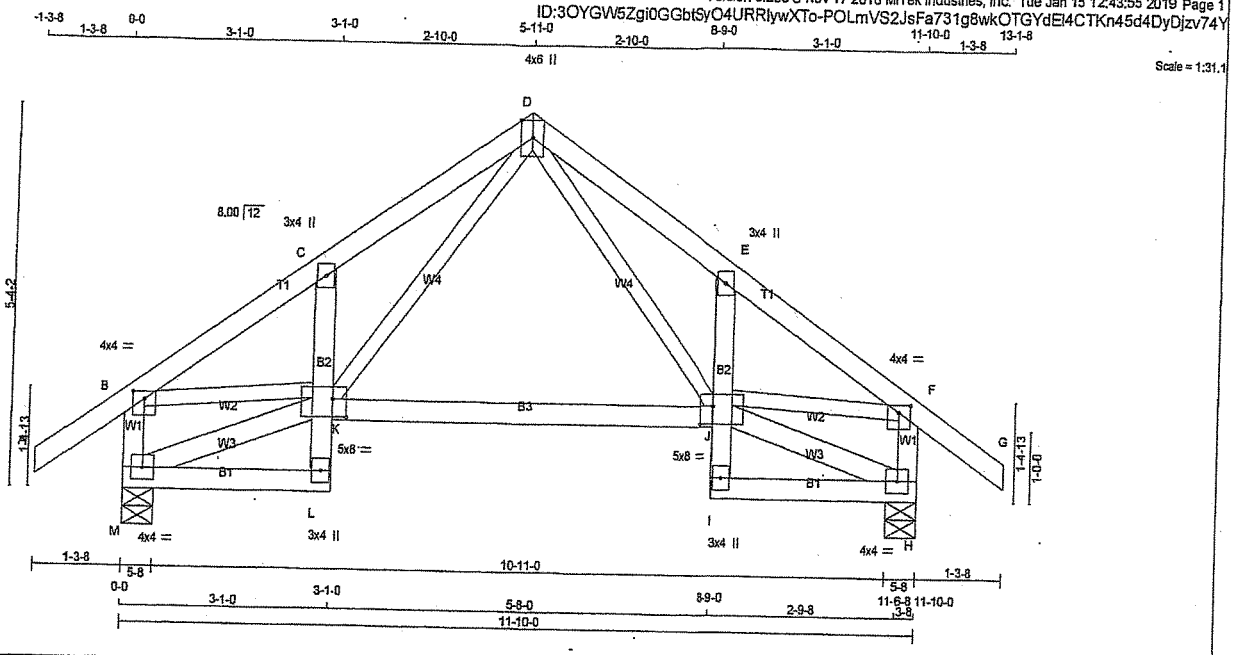


TOTAL WEIGHT = 2 X 60 = 120 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - G	2x4	ORY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
H - F	2x4	ORY	No.2	SPF
M - L	2x4	ORY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	ORY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	TMVW-p	MT20	4.0	4.0	1.25	2.00
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
K	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	846	0	846	0	0	IN-SX	IN-SX
H	846	0	846	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	634	339 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0
H	634	339 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

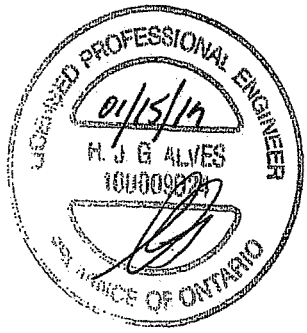
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A-B	0 / 32		-84.9	-84.9	0.11 (1)	10.00	D-J	0 / 543		0.12 (1)
B-C	-968 / 0		-84.9	-84.9	0.08 (1)	6.24	K-D	0 / 543		0.12 (1)
C-D	-994 / 0		-84.9	-84.9	0.10 (1)	6.16	M-K	-27 / 0		0.00 (1)
D-E	-994 / 0		-84.9	-84.9	0.10 (1)	6.16	J-H	-27 / 0		0.00 (1)
E-F	-968 / 0		-84.9	-84.9	0.08 (1)	6.24	B-K	0 / 817		0.18 (1)
F-G	0 / 32		-84.9	-84.9	0.11 (1)	10.00	J-F	0 / 817		0.18 (1)
M-B	-781 / 0		0.0	0.0	0.08 (1)	7.81				
H-F	-781 / 0		0.0	0.0	0.08 (1)	7.81				
M-L	0 / 25		-38.5	-38.5	0.08 (3)	10.00				
L-K	0 / 72		0.0	0.0	0.04 (1)	10.00				
K-C	-296 / 0		0.0	0.0	0.03 (1)	7.81				
K-J	0 / 494		-38.6	-38.5	0.36 (2)	10.00				
I-J	0 / 72		0.0	0.0	0.04 (1)	10.00				
J-E	-296 / 0		0.0	0.0	0.03 (1)	7.81				
I-H	0 / 25		-38.5	-38.5	0.08 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 894 (0.16")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.36/1.00 (J-K:2) , WB=0.18/1.00 (F-J:1) , SSI=0.12/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

DWG NO. TAM T1901131
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T82	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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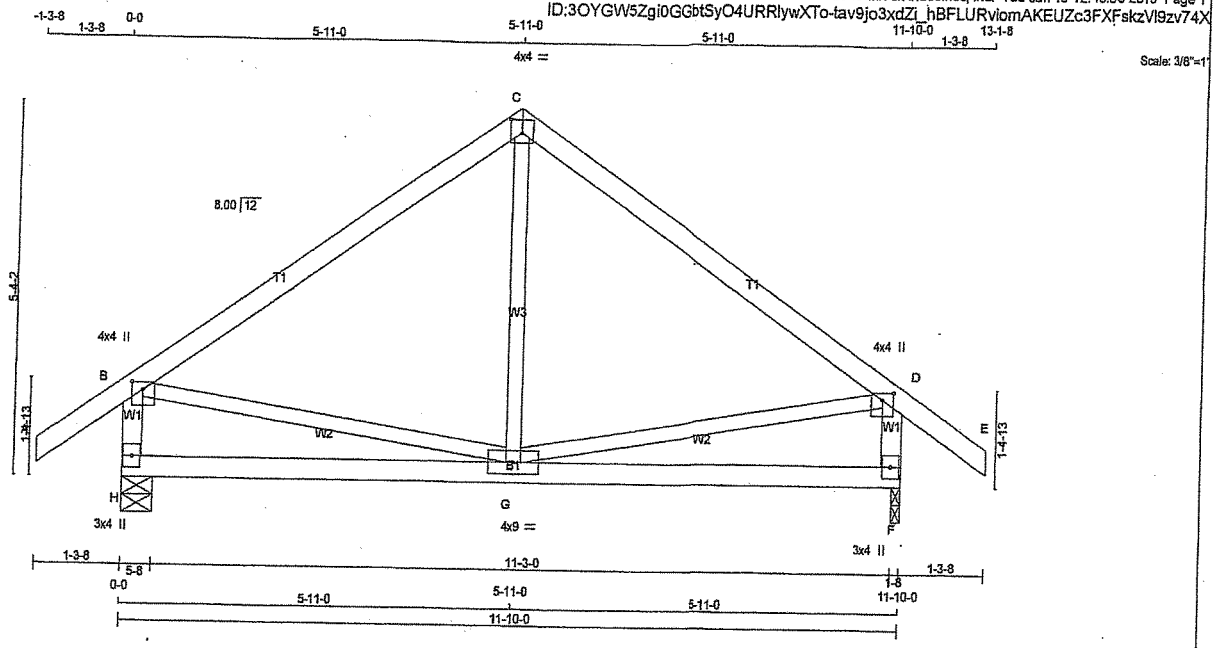


TOTAL WEIGHT = 2 X 49 = 98 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	846	0	846	0	0	5-8	5-8
F	846	0	846	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	634	339 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0
F	634	339 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	G- C	0 / 264	0.06 (3)
B- C	-555 / 0	-84.9	-84.9	0.39 (1)	6.25	B- G	0 / 469	0.11 (1)
C- D	-555 / 0	-84.9	-84.9	0.39 (1)	6.25	G- D	0 / 469	0.11 (1)
D- E	0 / 32	-84.9	-84.9	0.11 (1)	10.00			
H- B	-760 / 0	0.0	0.0	0.08 (1)	7.81			
F- D	-760 / 0	0.0	0.0	0.08 (1)	7.81			
H- G	0 / 0	-38.5	-38.5	0.31 (3)	10.00			
G- F	0 / 0	-38.5	-38.5	0.31 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	OL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.39/1.00 (C-D:1) , BC=0.31/1.00 (G-H:3) , WB=0.11/1.00 (B-G:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS DFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

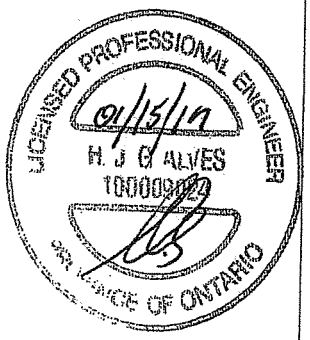
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)



DWG NO. TAM 9190132
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300400	T82A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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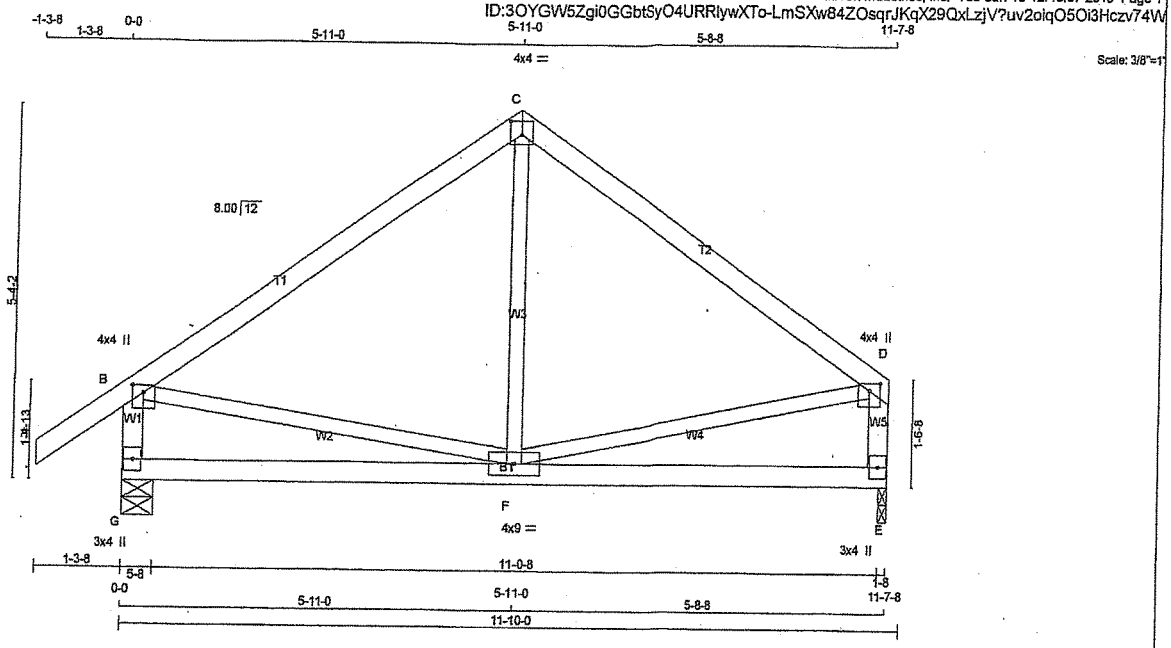

TOTAL WEIGHT = 47 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - O	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTOREO GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG
G	834	0	834	0	0	5-8	5-8
E	717	0	717	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	624	335 / 0	122 / 0	0 / 0	0 / 0	168 / 0	0 / 0
E	544	271 / 0	122 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LO)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00
B-C	-536 / 0	-84.9	-84.9	1	0.39 (1)	6.25
C-D	-536 / 0	-84.9	-84.9	1	0.36 (1)	6.25
G-B	-746 / 0	0.0	0.0	1	0.08 (1)	7.81
E-D	-634 / 0	0.0	0.0	1	0.07 (1)	7.81
G-F	0 / 0	-38.5	-38.5	1	0.29 (3)	10.00
F-E	0 / 0	-38.5	-38.5	1	0.29 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
F-C	0 / 253	0.06 (3)
B-F	0 / 453	0.10 (1)
F-D	0 / 456	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.29/1.00 (F-G:3) , WB=0.10/1.00 (D-F:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
R. J. G. ALVES
100[illegible]4
PROVINCE OF ONTARIO

DWG NO. TAM T1901133
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300402	G85	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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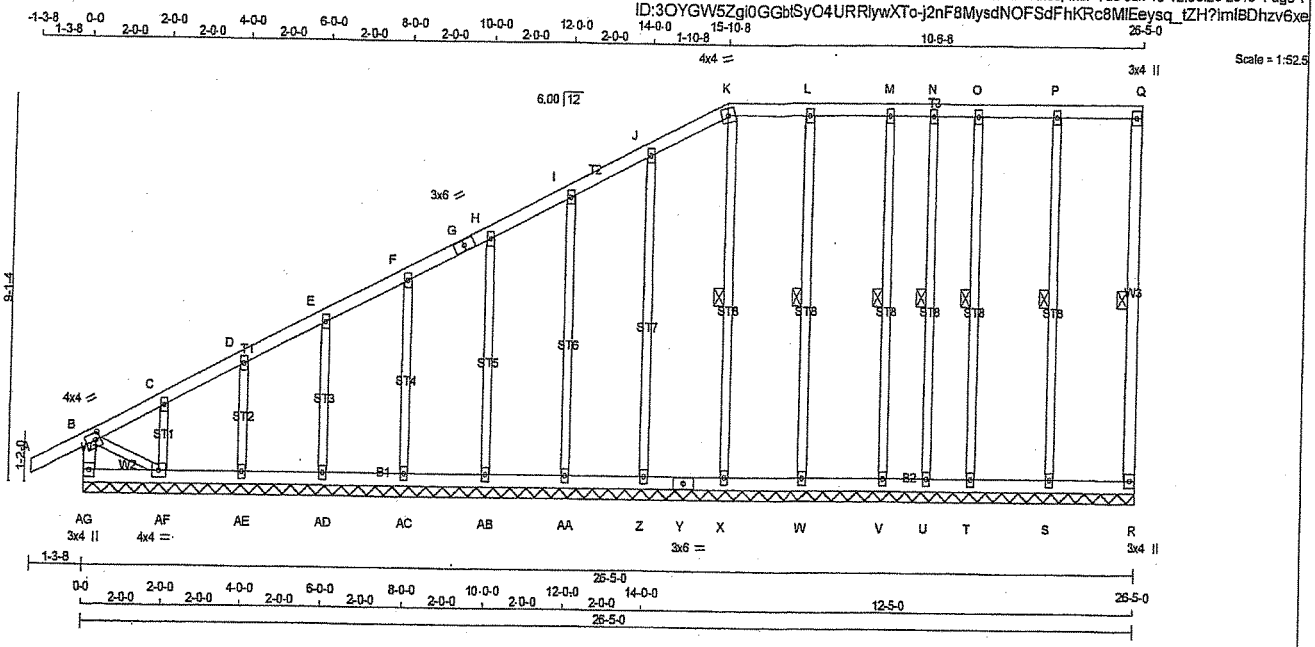


TOTAL WEIGHT = 2 X 144 = 288 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
K - Q	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
AG- B	2x4	DRY	No.2	SPF
AG- Y	2x4	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

ORY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C, D, E, F, H, I, J, L, M, N, O, P						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
K	TTW-m	MT20	4.0	4.0		
Q	TMV+p	MT20	3.0	4.0		
R	BMV1+p	MT20	3.0	4.0		
S, T, U, V, W, X, Z, AA, AB, AC, AD, AE						
S	BMW1+w	MT20	2.0	4.0		
Y	BS-t	MT20	3.0	6.0		
AF	BMWW1-t	MT20	4.0	4.0		
AG	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-R, K-X, L-W, M-V, N-U, P-S, O-T.

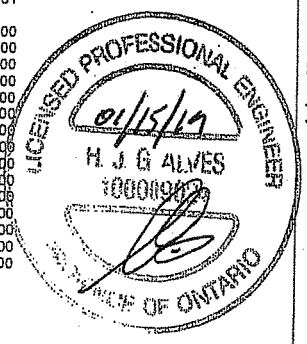
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C		-17 / 0	-84.9	-84.9	0.05 (1)	6.25
C- D		-19 / 0	-84.9	-84.9	0.05 (1)	6.25
D- E		-13 / 0	-84.9	-84.9	0.04 (1)	6.25
E- F		-10 / 0	-84.9	-84.9	0.04 (1)	6.25
F- G		-7 / 0	-84.9	-84.9	0.04 (1)	10.00
G- H		-7 / 0	-84.9	-84.9	0.04 (1)	10.00
H- I		-5 / 0	-84.9	-84.9	0.04 (1)	10.00
I- J		-3 / 0	-84.9	-84.9	0.04 (1)	10.00
J- K		-9 / 0	-84.9	-84.9	0.04 (1)	10.00
K- L		0 / 0	-84.9	-84.9	0.05 (1)	10.00
L- M		0 / 0	-84.9	-84.9	0.05 (1)	10.00
M- N		0 / 0	-84.9	-84.9	0.03 (1)	10.00
N- O		0 / 0	-84.9	-84.9	0.03 (1)	10.00
O- P		0 / 0	-84.9	-84.9	0.05 (1)	10.00
P- Q		0 / 0	-84.9	-84.9	0.05 (1)	10.00
R- O		-75 / 0	0.0	0.0	0.03 (1)	6.25
AG- B		-200 / 0	0.0	0.0	0.02 (1)	7.81
AG-AF		0 / 0	-38.5	-38.5	0.03 (3)	10.00
AF-AE		0 / 16	-38.5	-38.5	0.03 (3)	10.00
AE-AD		0 / 12	-38.5	-38.5	0.02 (3)	10.00
AD-AC		0 / 9	-38.5	-38.5	0.02 (3)	10.00
AC-AB		0 / 7	-38.5	-38.5	0.02 (3)	10.00
AB-AA		0 / 5	-38.5	-38.5	0.02 (3)	10.00
AA- Z		0 / 3	-38.5	-38.5	0.02 (3)	10.00
Z- Y		0 / 2	-38.5	-38.5	0.02 (3)	10.00
Y- X		0 / 2	-38.5	-38.5	0.02 (3)	10.00
X- W		0 / 0	-38.5	-38.5	0.03 (3)	10.00
W- V		0 / 0	-38.5	-38.5	0.03 (3)	10.00
V- U		0 / 0	-38.5	-38.5	0.02 (3)	10.00
U- T		0 / 0	-38.5	-38.5	0.02 (3)	10.00
T- S		0 / 0	-38.5	-38.5	0.03 (3)	10.00
S- R		0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AF- C		-185 / 0	0.03 (1)
AE- O		-164 / 0	0.03 (1)
AD- E		-169 / 0	0.04 (1)
AC- F		-168 / 0	0.07 (1)
AB- H		-189 / 0	0.10 (1)
AA- I		-166 / 0	0.15 (1)
Z- J		-185 / 0	0.24 (1)
X- K		-137 / 0	0.07 (1)
W- L		-201 / 0	0.11 (1)
V- M		-130 / 0	0.07 (1)
U- N		-87 / 0	0.05 (1)
S- P		-200 / 0	0.11 (1)
T- O		-129 / 0	0.07 (1)
B-AF		0 / 24	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (S-T:3) , WB=0.24/1.00 (J-Z:1) , SSI=0.08/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.08 (J) (INPUT = 1.00)

DWG NO. TAM T1401138
STRUCTURAL
CONFIDENTIAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	T90	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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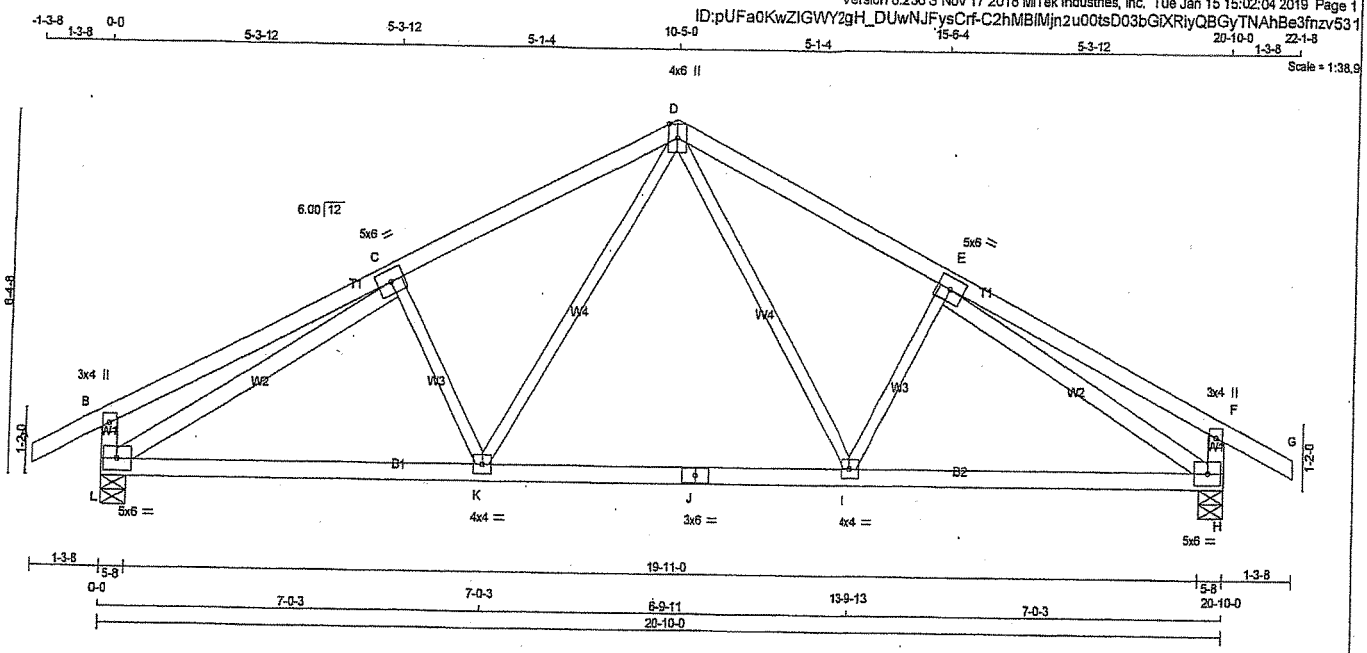


TOTAL WEIGHT = 86 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
L	1400	0	1400	0	0	5-8	5-8
H	1400	0	1400	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1054	548 / 0	219 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	1054	548 / 0	219 / 0	0 / 0	0 / 0	287 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

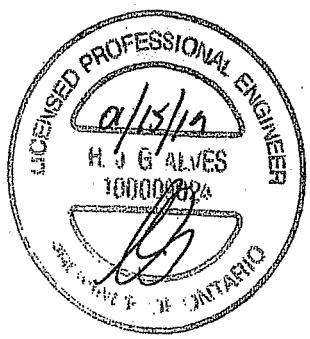
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LO1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 23	-84.9	-84.9	0.37 (1)	10.00
C- D	-1576 / 0	-84.9	-84.9	0.32 (1)	4.93
D- E	-1576 / 0	-84.9	-84.9	0.32 (1)	4.93
E- F	0 / 23	-84.9	-84.9	0.37 (1)	10.00
F- G	0 / 26	-84.9	-84.9	0.11 (1)	10.00
L- B	-284 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-284 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 1512	-38.5	-38.5	0.52 (2)	10.00
K- J	0 / 1095	-38.5	-38.5	0.46 (2)	10.00
J- I	0 / 1095	-38.5	-38.5	0.46 (2)	10.00
I- H	0 / 1512	-38.5	-38.5	0.52 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 601	0.14 (1)
I- E	-264 / 27	0.07 (1)
K- D	0 / 601	0.14 (1)
C- K	-264 / 27	0.07 (1)
L- O	-1816 / 0	0.87 (1)
E- H	-1818 / 0	0.87 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/380 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.37/1.00 (E-F:1) , BC=0.52/1.00 (H-I:2) , WB=0.87/1.00 (C-L:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.45 (C) (INPUT = 1.00)

DWG NO. TAM 1790491
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300332	G90	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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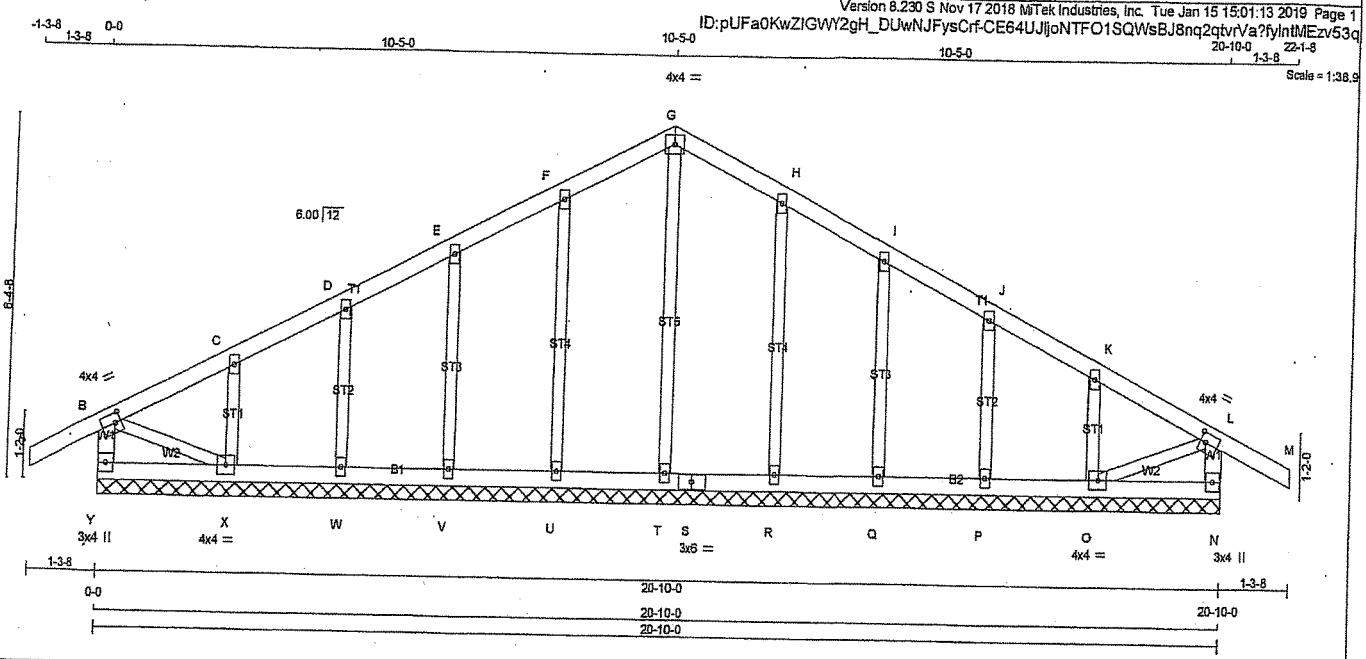


TOTAL WEIGHT = 85 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C, O, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0		
L	TMVW-t	MT20	4.0	4.0	2.00	1.25
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, O, R, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	8.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

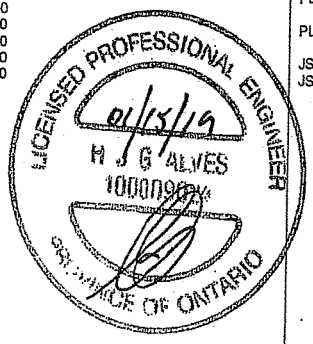
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
Y- B	-216 / 0	0.0	0.0	0.02 (1)		7.81
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B- C	-18 / 0	-84.9	-84.9	0.06 (1)		6.25
C- D	-23 / 0	-84.9	-84.9	0.06 (1)		6.25
D- E	-14 / 0	-84.9	-84.9	0.04 (1)		6.25
E- F	-10 / 0	-84.9	-84.9	0.05 (1)		6.25
F- G	-17 / 0	-84.9	-84.9	0.05 (1)		6.25
G- H	-17 / 0	-84.9	-84.9	0.05 (1)		6.25
H- I	-10 / 0	-84.9	-84.9	0.05 (1)		6.25
I- J	-14 / 0	-84.9	-84.9	0.04 (1)		6.25
J- K	-23 / 0	-84.9	-84.9	0.06 (1)		6.25
K- L	-18 / 0	-84.9	-84.9	0.06 (1)		6.25
L- M	0 / 26	-84.9	-84.9	0.11 (1)		10.00
N- L	-216 / 0	0.0	0.0	0.02 (1)		7.81
Y- X	0 / 0	-38.5	-38.5	0.04 (3)		10.00
X- W	0 / 17	-38.5	-38.5	0.04 (3)		10.00
W- V	0 / 13	-38.5	-38.5	0.03 (3)		10.00
V- U	0 / 11	-38.5	-38.5	0.03 (3)		10.00
U- T	0 / 8	-38.5	-38.5	0.02 (3)		10.00
T- S	0 / 8	-38.5	-38.5	0.02 (3)		10.00
S- R	0 / 8	-38.5	-38.5	0.02 (3)		10.00
R- O	0 / 11	-38.5	-38.5	0.03 (3)		10.00
Q- P	0 / 13	-38.5	-38.5	0.03 (3)		10.00
P- O	0 / 17	-38.5	-38.5	0.04 (3)		10.00
O- N	0 / 0	-38.5	-38.5	0.04 (3)		10.00

WEBS MEMB.	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
FR-TO		
T- G	-133 / 0	0.09 (1)
U- F	-192 / 0	0.08 (1)
V- E	-166 / 0	0.05 (1)
W- D	-157 / 0	0.03 (1)
X- C	-210 / 0	0.03 (1)
R- H	-192 / 0	0.08 (1)
O- I	-166 / 0	0.05 (1)
P- J	-157 / 0	0.03 (1)
O- K	-210 / 0	0.03 (1)
B- X	0 / 25	0.01 (1)
O- L	0 / 25	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.04/1.00 (O-P:3) , WB=0.09/1.00 (G-T:1) , SSI=0.08/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

DWG NO. TAM 1901172
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300438	T91	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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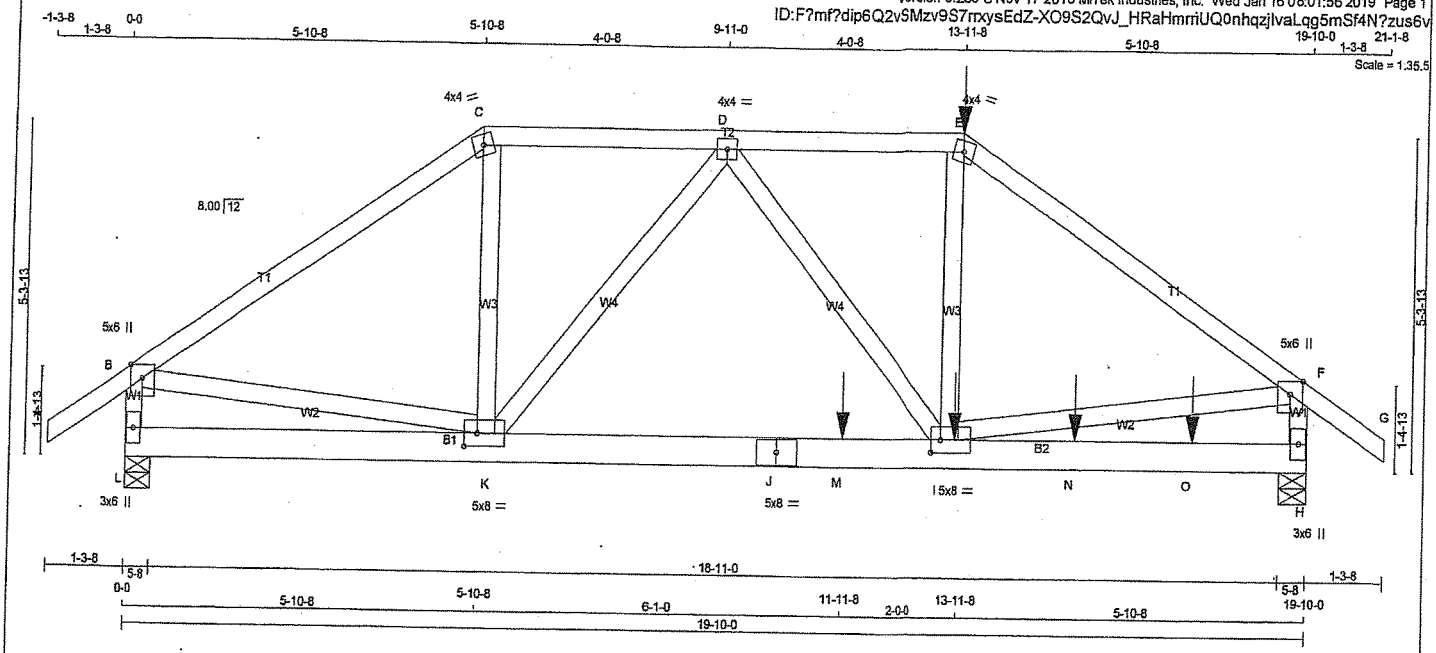

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	ORY	No.2	SPF
E - G	2x4	ORY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	ORY	No.2	SPF
L - J	2x6	ORY	No.2	SPF
J - H	2x6	ORY	No.2	SPF
ALL WEBS	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	SIDE(61.0)
E- G	1	12	SIDE(61.0)
L- B	1	12	TOP
H- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- L	2	12	TOP
J- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
I- E	1	6	SIDE(19.6)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0	2.50	2.00
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0	2.50	2.50
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 103 = 207 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION. DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2775	0	2775	0	0	5-8	5-8
L	2122	0	2122	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2083	1100 / 0	417 / 0	0 / 0	0 / 0	566 / 0	0 / 0
L	1588	855 / 0	306 / 0	0 / 0	0 / 0	426 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.06 (1)	10.00	K- C	0 / 1003	0.09 (1)
B- C	-2502 / 0	-84.9	-84.9	0.35 (1)	5.33	K- D	-893 / 0	0.22 (1)
C- D	-2128 / 0	-84.9	-84.9	0.14 (1)	5.93	D- I	0 / 888	0.63 (1)
D- E	-2907 / 0	-84.9	-84.9	0.15 (1)	5.24	I- E	0 / 989	0.09 (2)
E- F	-3441 / 0	-84.9	-84.9	0.40 (1)	4.67	B- K	0 / 2107	0.19 (2)
F- G	0 / 32	-84.9	-84.9	0.06 (1)	10.00	I- F	0 / 2899	0.26 (1)
L- B	-2096 / 0	0.0	0.0	0.12 (1)	7.64			
H- F	-2746 / 0	0.0	0.0	0.15 (1)	6.90			
L- K	0 / 0	-38.5	-38.5	0.21 (1)	10.00			
K- J	0 / 2673	-38.5	-38.5	0.63 (1)	10.00			
J- M	0 / 2673	-38.5	-38.5	0.63 (1)	10.00			
M- I	0 / 2673	-38.5	-38.5	0.63 (1)	10.00			
I- N	0 / 0	-38.5	-38.5	0.37 (1)	10.00			
N- O	0 / 0	-38.5	-38.5	0.37 (1)	10.00			
O- H	0 / 0	-38.5	-38.5	0.37 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	REEL	CONN.
E	13-11-8	-543	-543	—	FRONT	VERT	TOTAL	—	—
I	13-10-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
M	11-11-8	-1450	-1450	—	TOP	VERT	TOTAL	—	—
N	15-10-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
O	17-10-12	-75	-95	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.40/1.00 (E-F:1) , BC=0.63/1.00 (I-K:1) , WB=0.26/1.00 (F-I:1) , SSI=0.44/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.71 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100094024
PROVINCE OF ONTARIO

DWG NO. TAM T7901226
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300438	T92	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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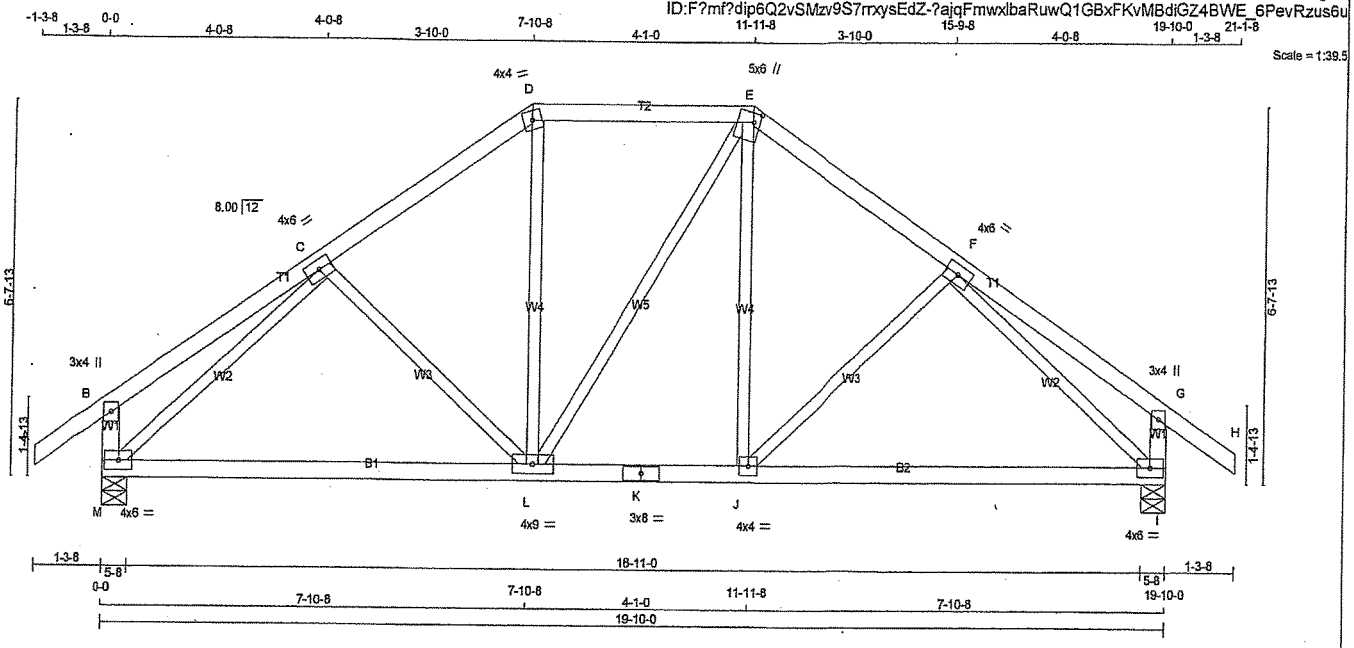


TOTAL WEIGHT = 87 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
O - E	2x4	ORY	No.2	SPF
E - H	2x4	ORY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
M - K	2x4	ORY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1340	0	1340	0	0	5-8	5-8
I	1340	0	1340	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1008	526 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0
I	1008	526 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

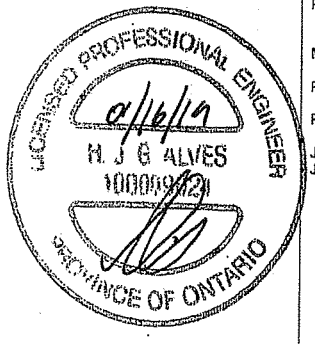
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 21	-84.9	-84.9	0.20 (1)	10.00
C-D	-1154 / 0	-84.9	-84.9	0.17 (1)	5.74
D-E	-947 / 0	-84.9	-84.9	0.19 (1)	6.15
E-F	-1154 / 0	-84.9	-84.9	0.17 (1)	5.74
F-G	0 / 21	-84.9	-84.9	0.20 (1)	10.00
G-H	0 / 32	-84.9	-84.9	0.11 (1)	10.00
M-B	-246 / 0	0.0	0.0	0.03 (1)	7.81
I-G	-246 / 0	0.0	0.0	0.03 (1)	7.81
M-L	0 / 1024	-38.5	-38.5	0.52 (2)	10.00
L-K	0 / 946	-38.5	-38.5	0.47 (2)	10.00
K-J	0 / 948	-38.5	-38.5	0.47 (2)	10.00
J-I	0 / 1024	-38.5	-38.5	0.52 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
C-L	-112 / 50	0.06 (1)
L-D	0 / 353	0.08 (2)
L-E	0 / 1	0.00 (2)
J-E	0 / 351	0.08 (2)
J-F	-112 / 50	0.06 (1)
M-C	-1407 / 0	0.67 (1)
F-I	-1406 / 0	0.67 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 813 (0.29")

CSI: TC=0.20/1.00 (F-G:1) , BC=0.52/1.00 (I-J:2) , WB=0.67/1.00 (C-M:1) , SSI=0.19/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (M) (INPUT = 0.90)
JSI METAL= 0.65 (K) (INPUT = 1.00)

DWG NO. TAM T790122-7
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300438	T93	17	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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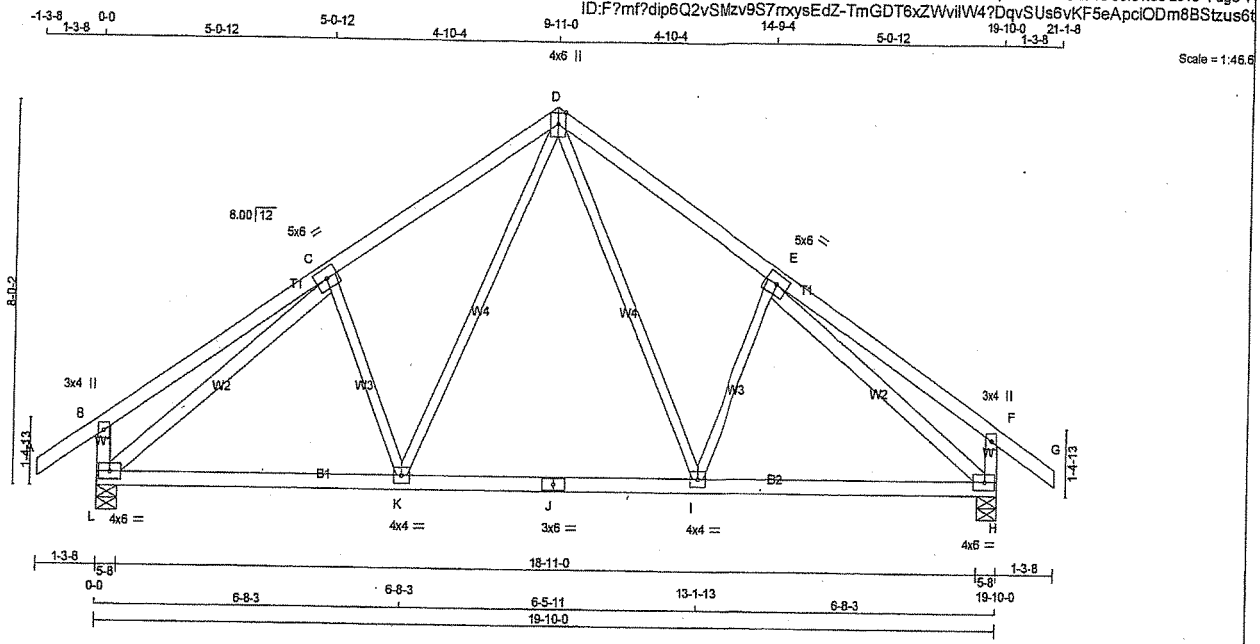

TOTAL WEIGHT = 17 X 90 = 1538 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
O - G	2x4	ORY	No.2	SPF
L - B	2x4	ORY	No.2	SPF
H - F	2x4	ORY	No.2	SPF
L - J	2x4	ORY	No.2	SPF
J - H	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	ORY	No.2	SPF
E - H	2x4	ORY	No.2	SPF

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1340	0	1340	0	0	5-8	5-8
H	1340	0	1340	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1008	526 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0
H	1008	526 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 28	-84.9	-84.9	0.34 (1)	10.00
C-D	-1221 / 0	-84.9	-84.9	0.28 (1)	5.49
D-E	-1221 / 0	-84.9	-84.9	0.28 (1)	5.49
E-F	0 / 28	-84.9	-84.9	0.34 (1)	10.00
F-G	0 / 32	-84.9	-84.9	0.11 (1)	10.00
L-B	-275 / 0	0.0	0.0	0.03 (1)	7.81
H-F	-275 / 0	0.0	0.0	0.03 (1)	7.81
L-K	0 / 1083	-38.5	-38.5	0.43 (2)	10.00
K-J	0 / 783	-38.5	-38.5	0.39 (2)	10.00
J-I	0 / 783	-38.5	-38.5	0.39 (2)	10.00
I-H	0 / 1083	-38.5	-38.5	0.43 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-I	0 / 541	0.12 (1)
I-E	-239 / 23	0.09 (1)
K-D	0 / 541	0.12 (1)
C-K	-239 / 23	0.09 (1)
L-C	-1447 / 0	0.80 (1)
E-H	-1447 / 0	0.80 (1)

LICENSED PROFESSIONAL ENGINEER
01/16/19
M J G ALVES
100099124
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.34/1.00 (B-C:1) , BC=0.43/1.00 (K-L:2) , WB=0.80/1.00 (C-L:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1887	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.37 (E) (INPUT = 1.00)

DRG NO. TAM T1901228
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300438	T94	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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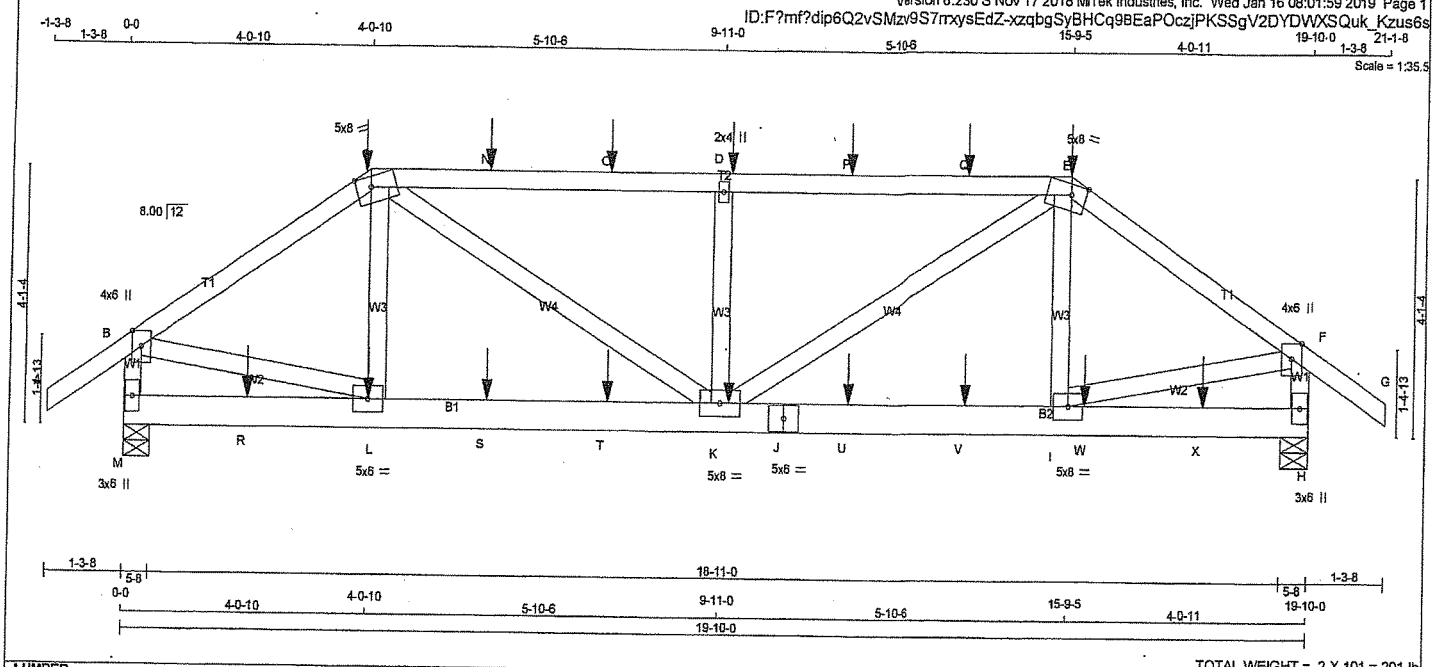


TOTAL WEIGHT = 2 X 101 = 201 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	ORY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	ORY	No.2	SPF
M - B	2x4	ORY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	ORY	No.2	SPF

DRY. SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-E	1	12	SIDE(61.0)
E-G	1	12	SIDE(61.0)
M-B	1	12	TOP
H-F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-J	2	12	SIDE(183.1)
J-H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.00	3.00
F	TMWW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	8.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DWG NO. TAM T1901229
STRUCTURAL
COMPONENT ONLY 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
M	2226	0	2226	0	0	5-8	5-8
H	2238	0	2238	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1674	874 / 0	341 / 0	0 / 0	0 / 0	459 / 0	0 / 0
H	1884	878 / 0	345 / 0	0 / 0	0 / 0	461 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

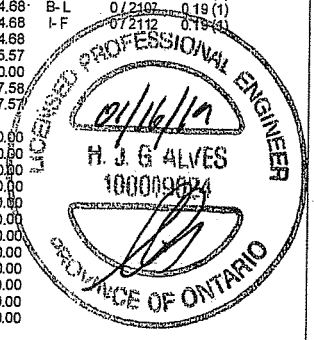
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B-C	-2467 / 0	-84.9	-84.9	0.16 (1)	5.58
C-N	-3092 / 0	-84.9	-84.9	0.49 (1)	4.68
N-O	-3092 / 0	-84.9	-84.9	0.49 (1)	4.68
O-D	-3092 / 0	-84.9	-84.9	0.49 (1)	4.68
D-P	-3092 / 0	-84.9	-84.9	0.49 (1)	4.68
P-O	-3092 / 0	-84.9	-84.9	0.49 (1)	4.68
Q-E	-3092 / 0	-84.9	-84.9	0.49 (1)	4.68
E-F	-2473 / 0	-84.9	-84.9	0.16 (1)	5.57
F-G	0 / 32	-84.9	-84.9	0.06 (1)	10.00
M-B	-2149 / 0	0.0	0.0	0.12 (1)	7.58
H-F	-2153 / 0	0.0	0.0	0.12 (1)	7.57
M-R	0 / 0	-38.5	-38.5	0.07 (3)	10.00
R-L	0 / 0	-38.5	-38.5	0.07 (3)	10.00
L-S	0 / 2045	-38.5	-38.5	0.19 (1)	10.00
S-T	0 / 2045	-38.5	-38.5	0.19 (1)	10.00
T-K	0 / 2045	-38.5	-38.5	0.19 (1)	10.00
K-J	0 / 2050	-38.5	-38.5	0.19 (1)	10.00
J-U	0 / 2050	-38.5	-38.5	0.19 (1)	10.00
U-V	0 / 2050	-38.5	-38.5	0.19 (1)	10.00
V-I	0 / 2050	-38.5	-38.5	0.19 (1)	10.00
I-W	0 / 0	-38.5	-38.5	0.07 (3)	10.00
W-X	0 / 0	-38.5	-38.5	0.07 (3)	10.00
X-H	0 / 0	-38.5	-38.5	0.07 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-118 / 214	0.02 (3)
C-K	0 / 1240	0.11 (1)
K-D	-965 / 0	0.09 (1)
K-E	0 / 1234	0.11 (1)
I-E	-120 / 213	0.02 (3)
B-L	0 / 2107	0.19 (1)
I-F	07 2112	0.19 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-50	-56	—	FRONT	VERT	DEAD	—	—
C	4-0-10	-110	-110	—	BACK	VERT	TOTAL	—	—
C	4-0-11	-231	-231	—	FRONT	VERT	SNOW	—	—
D	10-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
E	15-9-6	-50	-56	—	FRONT	VERT	DEAD	—	—
E	15-9-5	-108	-108	—	BACK	VERT	TOTAL	—	—
E	15-9-6	-231	-231	—	FRONT	VERT	SNOW	—	—
K	10-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
L	4-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
N	6-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
O	8-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
P	12-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBO 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23 3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.19/1.00 (K-L:1) , WB=0.19/1.00 (F-I:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.61 (F) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300438	T94	1	2	TRUSS OESC.	

Tamarack Roof Truss, Burlington

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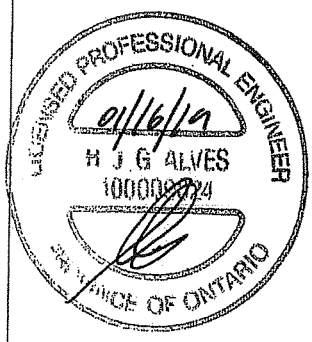
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	14-0-12	-102	-102	—	BACK	VERT	TOTAL	—	—
R	2-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
S	6-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
T	8-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
U	12-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
V	14-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
W	16-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—
X	18-0-12	-55	-70	—	BACK	VERT	TOTAL	—	—



DWG NO. TAM T1901229
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300438	T95	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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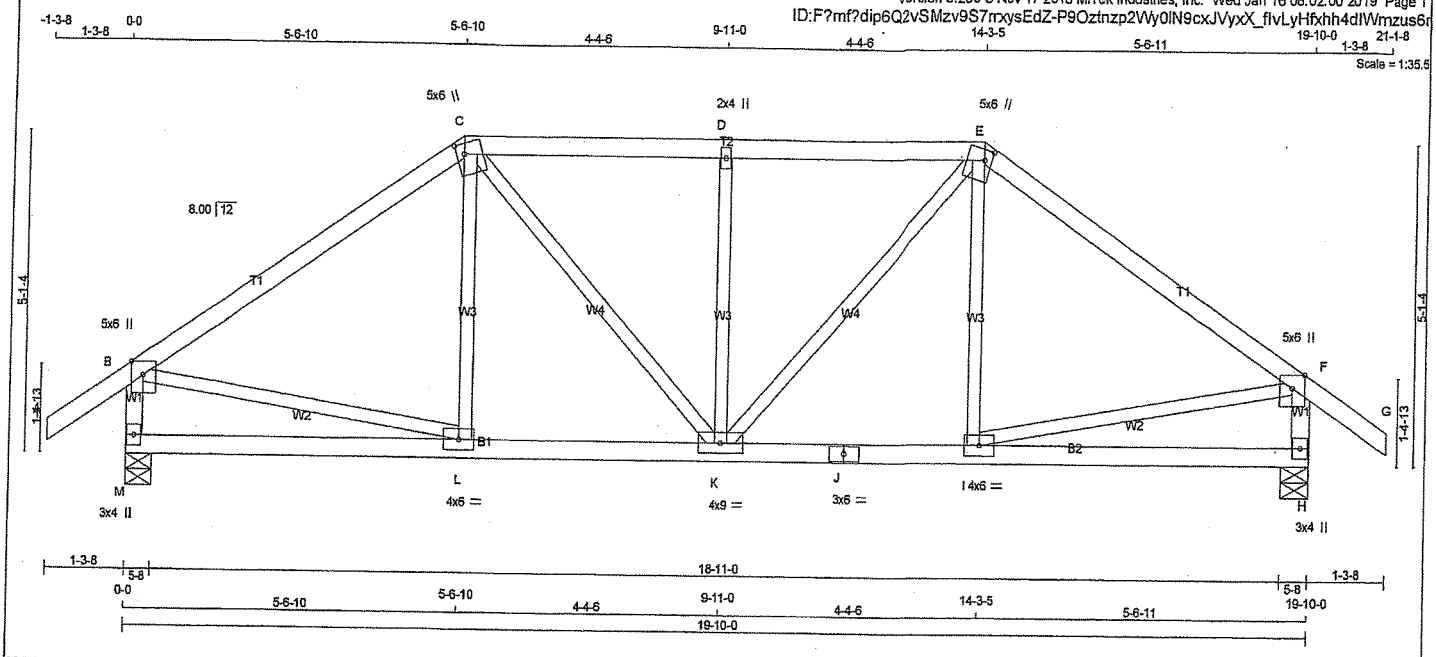


TOTAL WEIGHT = 84 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1340	0	1340	0	0	5-8	5-8
H	1340	0	1340	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1008	526 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0
H	1008	626 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING.

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

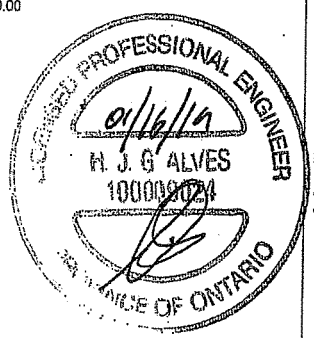
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-O	-1257 / 0	-84.9	-84.9	0.37 (1)	5.31
C-D	-1303 / 0	-84.9	-84.9	0.22 (1)	5.43
D-E	-1303 / 0	-84.9	-84.9	0.22 (1)	5.43
E-F	-1257 / 0	-84.9	-84.9	0.37 (1)	5.31
F-G	0 / 32	-84.9	-84.9	0.11 (1)	10.00
M-B	-1251 / 0	0.0	0.0	0.13 (1)	7.19
H-F	-1251 / 0	0.0	0.0	0.13 (1)	7.19
M-L	0 / 0	-38.5	-38.5	0.21 (3)	10.00
L-K	0 / 1045	-38.5	-38.5	0.35 (2)	10.00
K-J	0 / 1045	-38.5	-38.5	0.35 (2)	10.00
J-I	0 / 1045	-38.5	-38.5	0.35 (2)	10.00
I-H	0 / 0	-38.5	-38.5	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	0 / 189	0.05 (3)
C-K	0 / 384	0.09 (1)
K-D	-447 / 0	0.17 (1)
K-E	0 / 384	0.09 (1)
I-E	0 / 189	0.05 (3)
B-L	0 / 1064	0.24 (1)
I-F	0 / 1064	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % DF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.37/1.00 (E-F:1) , BC=0.35/1.00 (I-K:2) , WB=0.24/1.00 (F-I:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.53 (F) (INPUT = 1.00)

DWG NO. TAM T1901230
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300438	T96	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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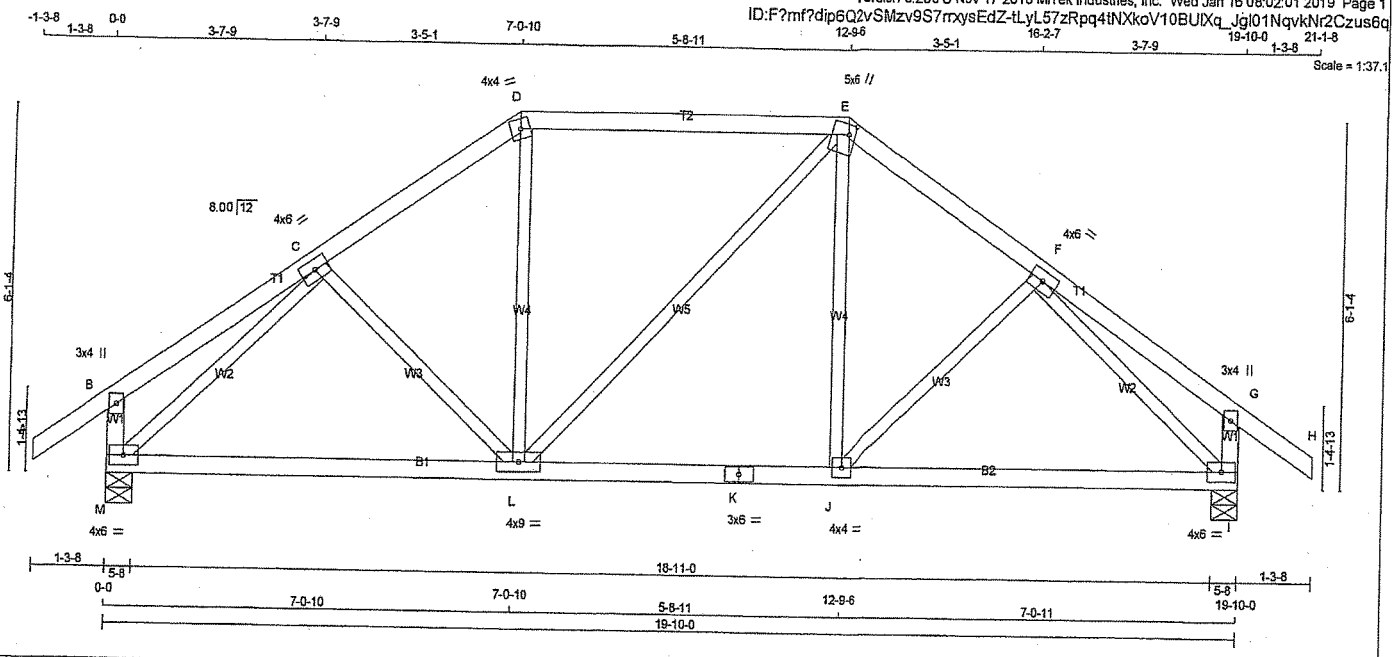

TOTAL WEIGHT = 85 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT2D	3.0	6.0		
L	BMWWW-t	MT2D	4.0	9.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1340	0	1340	0	0	5-8	5-8
I	1340	0	1340	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1008	526 / 0	208 / 0	0 / D	0 / 0	274 / 0	0 / 0
I	1008	526 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)		10.00
B-C	0 / 18	-84.9	-84.9	0.16 (1)		10.00
C-D	-1202 / 0	-84.9	-84.9	0.14 (1)		5.70
D-E	-989 / 0	-84.9	-84.9	0.37 (1)		5.79
E-F	-1201 / 0	-84.9	-84.9	0.14 (1)		5.70
F-G	0 / 18	-84.9	-84.9	0.16 (1)		10.00
G-H	0 / 32	-84.9	-84.9	0.11 (1)		10.00
M-B	-234 / 0	0.0	0.0	0.02 (1)		7.81
I-G	-234 / 0	0.0	0.0	0.02 (1)		7.81
M-L	0 / 1018	-38.5	-38.5	0.44 (2)		10.00
L-K	0 / 988	-38.5	-38.5	0.43 (2)		10.00
K-J	0 / 988	-38.5	-38.5	0.43 (2)		10.00
J-I	0 / 1017	-38.5	-38.5	0.44 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-L	-47 / 72	0.02 (1)
L-D	0 / 335	0.08 (2)
L-E	0 / 0	0.00 (1)
J-E	0 / 335	0.08 (2)
J-F	-47 / 72	0.02 (1)
M-C	-1420 / 0	0.55 (1)
F-I	-1420 / 0	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.37/1.00 (D-E:1) , BC=0.44/1.00 (I-J:2) , WB=0.55/1.00 (C-M:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY). (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.38 (K) (INPUT = 1.00)

DWG NO. TAM T1901231
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300438	T97	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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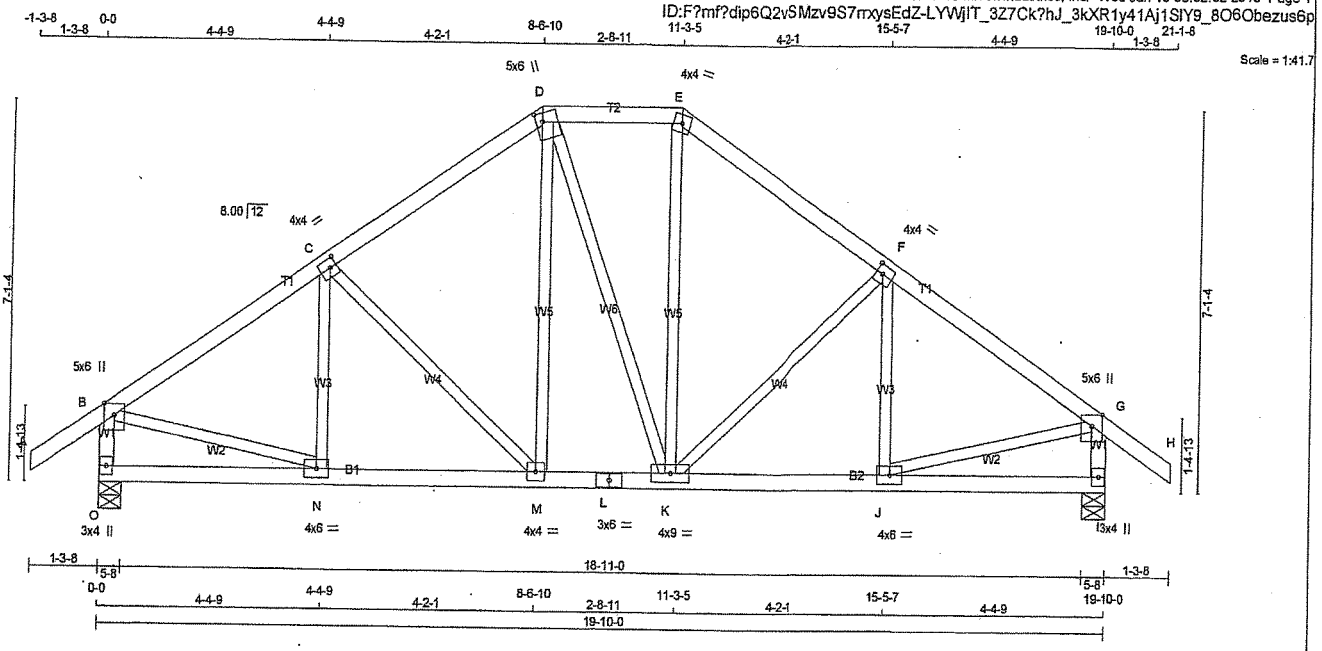


TOTAL WEIGHT = 93 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMWW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
O	1340	0	1340	0	0	5-8	5-8
I	1340	0	1340	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1008	526 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0
I	1008	526 / 0	208 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

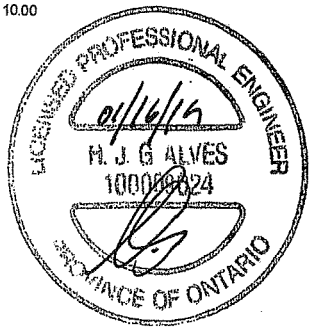
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1306 / 0	-84.9	-84.9	0.21 (1)	5.43
C-D	-1078 / 0	-84.9	-84.9	0.21 (1)	5.85
D-E	-882 / 0	-84.9	-84.9	0.09 (1)	6.25
E-F	-1080 / 0	-84.9	-84.9	0.21 (1)	5.84
F-G	-1305 / 0	-84.9	-84.9	0.21 (1)	5.43
G-H	0 / 32	-84.9	-84.9	0.11 (1)	10.00
O-B	-1271 / 0	0.0	0.0	0.13 (1)	7.14
I-G	-1270 / 0	0.0	0.0	0.13 (1)	7.14
O-N	0 / 0	-38.5	-38.5	0.14 (3)	10.00
N-M	0 / 1106	-38.5	-38.5	0.28 (2)	10.00
M-L	0 / 881	-38.5	-38.5	0.20 (1)	10.00
L-K	0 / 881	-38.5	-38.5	0.20 (1)	10.00
K-J	0 / 1105	-38.5	-38.5	0.28 (2)	10.00
J-I	0 / 0	-38.5	-38.5	0.14 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
N-C	-82 / 106	0.03 (3)
C-M	-315 / 0	0.19 (1)
M-D	0 / 341	0.08 (2)
D-K	0 / 5	0.00 (2)
K-E	0 / 347	0.08 (2)
K-F	-312 / 0	0.18 (1)
J-F	-85 / 103	0.02 (3)
B-N	0 / 1139	0.26 (1)
J-G	0 / 1138	0.26 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.28/1.00 (M-N:2) , WB=0.26/1.00 (B-N:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T7901232
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300441	T300A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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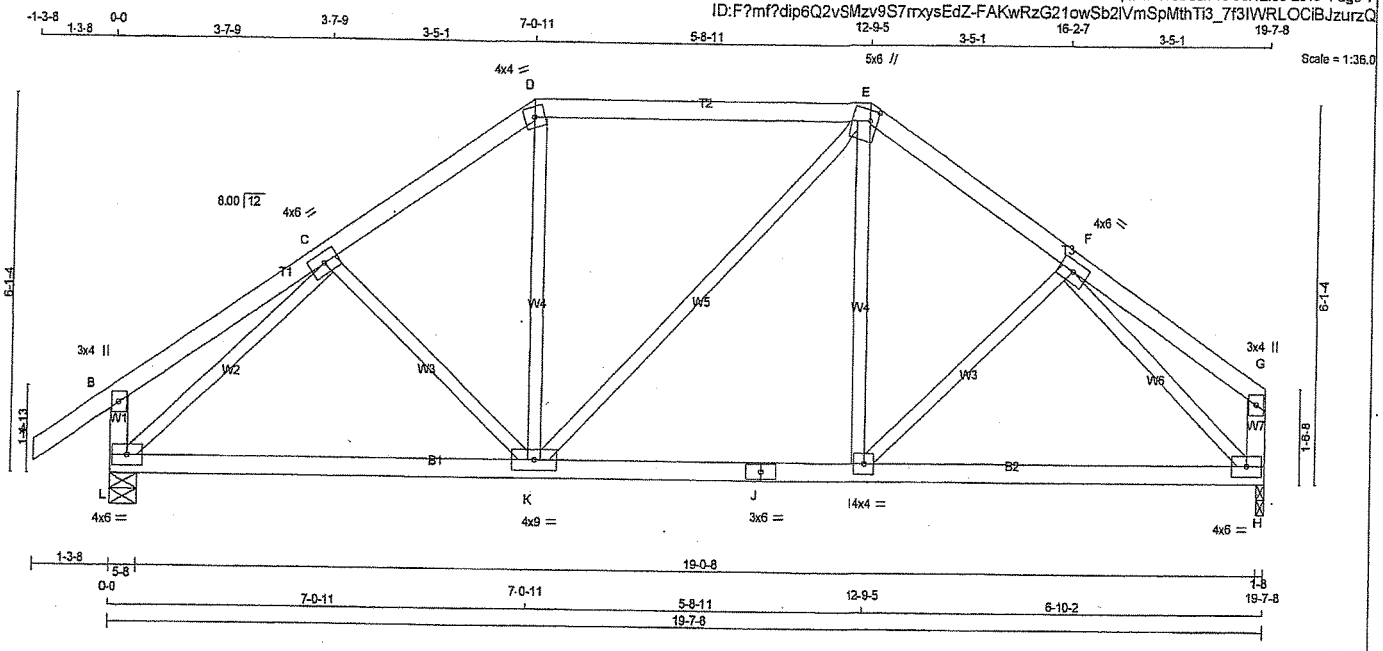

TOTAL WEIGHT = 83 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - E	2x4	ORY	No.2	SPF
E - G	2x4	ORY	No.2	SPF
L - B	2x4	ORY	No.2	SPF
H - G	2x4	ORY	No.2	SPF
L - J	2x4	ORY	No.2	SPF
J - H	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1327	0	1327	0	0	5-8	5-8
H	1210	0	1210	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0
H	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 18	-84.9	-84.9	0.16 (1)	10.00
C- D	-1183 / 0	-84.9	-84.9	0.14 (1)	5.73
D- E	-973 / 0	-84.9	-84.9	0.37 (1)	5.82
E- F	-1165 / 0	-84.9	-84.9	0.13 (1)	5.77
F- G	0 / 18	-84.9	-84.9	0.15 (1)	10.00
L- B	-234 / 0	0.0	0.0	0.02 (1)	7.81
H- G	-108 / 0	0.0	0.0	0.01 (1)	7.81
L- K	0 / 1005	-38.5	-38.5	0.44 (2)	10.00
K- J	0 / 958	-38.5	-38.5	0.42 (2)	10.00
J- I	0 / 958	-38.5	-38.5	0.42 (2)	10.00
I- H	0 / 956	-38.5	-38.5	0.42 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-50 / 70	0.02 (1)
K- D	0 / 326	0.07 (2)
K- E	0 / 21	0.00 (1)
I- E	0 / 301	0.07 (2)
I- F	-4 / 89	0.02 (3)
L- C	-1402 / 0	0.54 (1)
F- H	-1376 / 0	0.50 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.37/1.00 (D-E:1) , BC=0.44/1.00 (K-L:2) , WB=0.54/1.00 (C-L:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.35 (J) (INPUT = 1.00)

DWG NO. TAM T190 1233
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T318	1	1	TRUSS DESC.	

Tamarack Roof Truss, Budington 7-10

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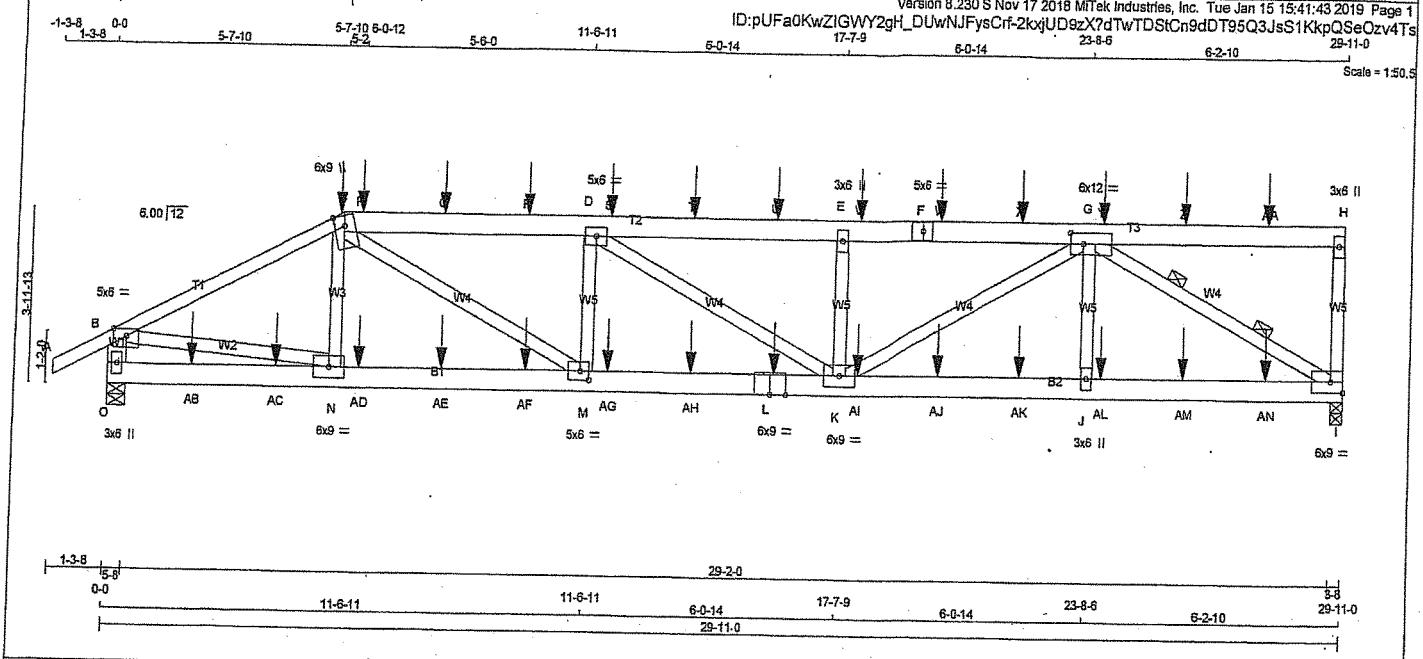

TOTAL WEIGHT = 160 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWWW-t	MT20	6.0	12.0	3.00	3.75
H	TMV+p	MT20	3.0	6.0		
I	BMVW1-t	MT20	6.0	9.0		Edge
J	BMW+w	MT20	3.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BS-t	MT20	6.0	9.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BMWW-t	MT20	6.0	9.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2692	0	2692	0	0	3-8	3-8
O	2828	0	2828	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2040	1022 / 0	453 / 0	0 / 0	0 / 0	565 / 0	0 / 0
O	2133	1098 / 0	451 / 0	0 / 0	0 / 0	584 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/3 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.10 (1)	10.00
B- C	-4142 / 0	-84.9	-84.9	0.72 (1)	3.35
C- P	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
P- O	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
Q- R	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
R- D	-5747 / 0	-84.9	-84.9	0.56 (1)	3.19
D- S	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
S- T	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
T- U	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
U- E	-5766 / 0	-84.9	-84.9	0.55 (1)	3.20
E- V	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
V- F	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
F- W	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
W- X	-5768 / 0	-84.9	-84.9	0.59 (1)	3.14
X- G	-5766 / 0	-84.9	-84.9	0.59 (1)	3.14
G- Y	0 / 0	-84.9	-84.9	0.31 (1)	10.00
Y- Z	0 / 0	-84.9	-84.9	0.31 (1)	10.00
Z-AA	0 / 0	-84.9	-84.9	0.31 (1)	10.00
AA- H	0 / 0	-84.9	-84.9	0.31 (1)	10.00
I- H	-286 / 0	0.0	0.0	0.06 (1)	7.81
O- B	-2713 / 0	0.0	0.0	0.19 (1)	6.29
O-AB	0 / 0	-38.5	-38.5	0.16 (3)	10.00
AB-AC	0 / 0	-38.5	-38.5	0.16 (3)	10.00
AC- N	0 / 0	-38.5	-38.5	0.16 (3)	10.00
N-AD	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
AD-AE	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
AE-AF	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
AF- M	0 / 3720	-38.5	-38.5	0.60 (1)	10.00
M-AG	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
AG-AH	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
AH- L	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
L- K	0 / 5747	-38.5	-38.5	0.87 (1)	10.00
K-AI	0 / 4000	-38.5	-38.5	0.63 (1)	10.00
AI-AJ	0 / 4000	-38.5	-38.5	0.63 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-171 / 188	0.03 (3)
B- N	0 / 3750	0.66 (1)
G- I	-4598 / 0	0.70 (1)
C- M	0 / 2364	0.42 (1)
J- G	0 / 493	0.09 (3)
M- D	-877 / 0	0.14 (1)
K- G	0 / 2043	0.36 (1)
D- K	0 / 47	0.01 (3)
K- E	-706 / 0	0.12 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

ENG to ONTARIO 11901890 STRUCTURAL CERTIFICATE ONLY 1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , O8C 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 870 (0.41")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.87/1.00 (K-M:1) , WB=0.70/1.00 (G-I:1) , SSI=0.32/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANIDN LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.90 (L) (INPUT = 1.00)

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AJ-AK	0 / 4000	-38.5	-38.5	0.63 (1)	10.00			
AK-J	0 / 4000	-38.5	-38.5	0.63 (1)	10.00			
J-AL	0 / 4000	-38.5	-38.5	0.67 (1)	10.00			
AL-AM	0 / 4000	-38.5	-38.5	0.67 (1)	10.00			
AM-AN	0 / 4000	-38.5	-38.5	0.67 (1)	10.00			
AN-I	0 / 4000	-38.5	-38.5	0.87 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-7-10	-41	-46	—	FRONT	VERT	DEAD	—	—
C	5-7-10	-189	-189	—	FRONT	VERT	SNOW	—	—
L	16-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
P	8-0-12	-104	-104	—	BACK	VERT	TOTAL	—	—
O	8-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
R	10-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
S	12-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
T	14-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
U	16-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
V	18-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
W	20-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
X	22-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
Y	24-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
Z	26-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
AA	28-0-12	-80	-80	—	BACK	VERT	TOTAL	—	—
AB	2-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AC	4-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AD	6-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AE	8-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AF	10-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AG	12-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AH	14-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AI	18-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AJ	20-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AK	22-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AL	24-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AM	26-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—
AN	28-0-12	-36	-46	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. S. GRAVES
100093774
PROVINCE OF ONTARIO

DWG NO. TAM 7901190
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T318A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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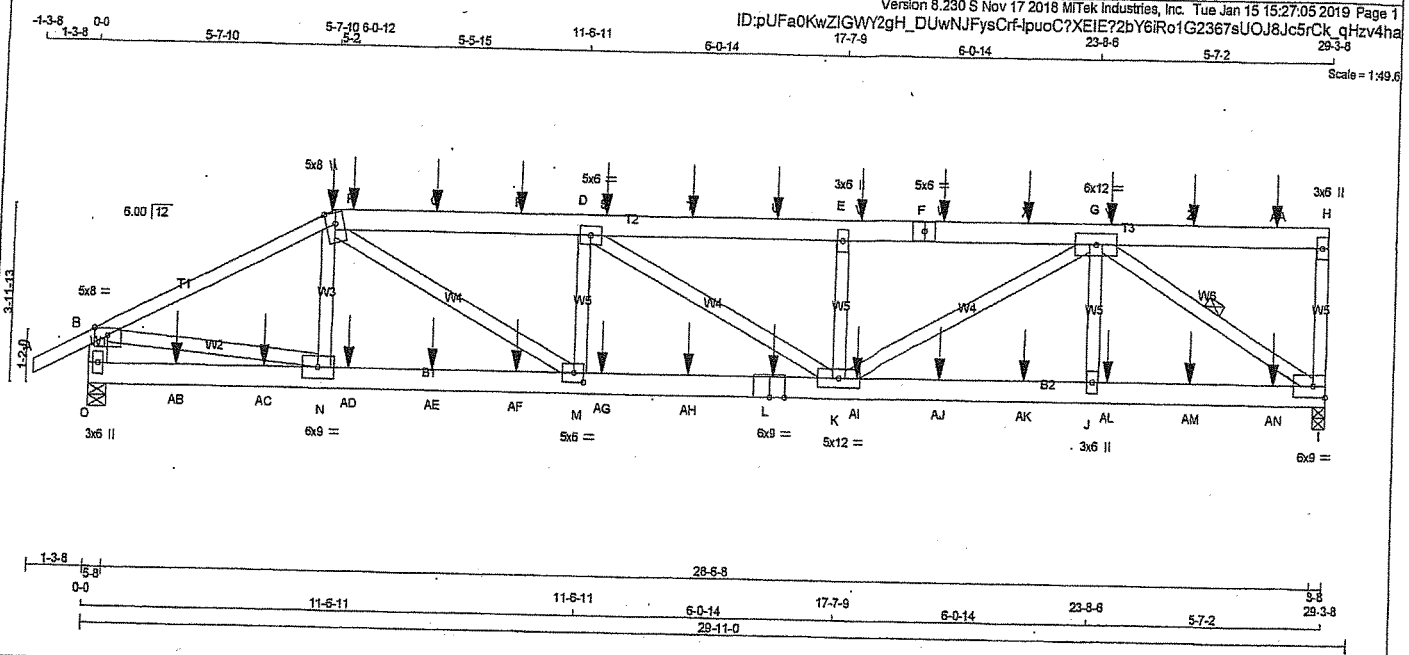


TOTAL WEIGHT = 157 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWWW-t	MT20	6.0	12.0		
H	TMV+p	MT20	3.0	6.0		
I	BMVW1-t	MT20	6.0	9.0	Edge	
J	BMW+w	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	12.0		
L	BS-t	MT20	6.0	9.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWW-t	MT20	6.0	9.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
I	2672	0	2672	0	0	3-8	3-8
O	2771	0	2771	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2025	1014 / 0	449 / 0	0 / 0	0 / 0	561 / 0	0 / 0
O	2090	1076 / 0	442 / 0	0 / 0	0 / 0	572 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

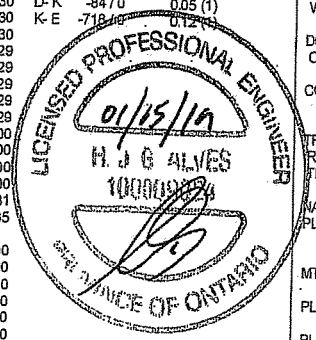
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.10 (1)	10.00
B-C	-4043 / 0	-84.9	-84.9	0.71 (1)	3.39
C-P	-5561 / 0	-84.9	-84.9	0.54 (1)	3.25
P-O	-5561 / 0	-84.9	-84.9	0.54 (1)	3.25
Q-R	-5561 / 0	-84.9	-84.9	0.54 (1)	3.25
R-D	-5561 / 0	-84.9	-84.9	0.54 (1)	3.25
D-S	-5489 / 0	-84.9	-84.9	0.53 (1)	3.30
S-T	-5489 / 0	-84.9	-84.9	0.53 (1)	3.30
T-U	-5489 / 0	-84.9	-84.9	0.53 (1)	3.30
U-E	-5489 / 0	-84.9	-84.9	0.53 (1)	3.30
E-V	-5489 / 0	-84.9	-84.9	0.53 (1)	3.29
V-F	-5489 / 0	-84.9	-84.9	0.53 (1)	3.29
F-W	-5489 / 0	-84.9	-84.9	0.53 (1)	3.29
W-X	-5489 / 0	-84.9	-84.9	0.53 (1)	3.29
X-G	-5489 / 0	-84.9	-84.9	0.53 (1)	3.29
G-Y	0 / 0	-84.9	-84.9	0.28 (1)	10.00
Y-Z	0 / 0	-84.9	-84.9	0.28 (1)	10.00
Z-AA	0 / 0	-84.9	-84.9	0.28 (1)	10.00
AA-H	0 / 0	-84.9	-84.9	0.28 (1)	10.00
I-H	-270 / 0	0.0	0.0	0.06 (1)	7.81
O-B	-2657 / 0	0.0	0.0	0.19 (1)	6.35
O-AB	0 / 0	-38.5	-38.5	0.16 (3)	10.00
AB-AC	0 / 0	-38.5	-38.5	0.18 (3)	10.00
AC-N	0 / 0	-38.5	-38.5	0.16 (3)	10.00
N-AD	0 / 3631	-38.5	-38.5	0.58 (1)	10.00
AD-AE	0 / 3631	-38.5	-38.5	0.58 (1)	10.00
AE-AF	0 / 3631	-38.5	-38.5	0.58 (1)	10.00
AF-M	0 / 3631	-38.5	-38.5	0.58 (1)	10.00
M-AG	0 / 5561	-38.5	-38.5	0.85 (1)	10.00
AG-AH	0 / 5561	-38.5	-38.5	0.85 (1)	10.00
AH-L	0 / 5561	-38.5	-38.5	0.85 (1)	10.00
L-K	0 / 5561	-38.5	-38.5	0.85 (1)	10.00
K-AI	0 / 3603	-38.5	-38.5	0.58 (1)	10.00
AI-AJ	0 / 3603	-38.5	-38.5	0.58 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-158 / 195	0.04 (3)
B-N	0 / 3660	0.65 (1)
G-I	-4260 / 0	0.89 (1)
C-M	0 / 2252	0.40 (1)
J-G	0 / 465	0.09 (3)
M-D	-820 / 0	0.13 (1)
K-G	0 / 2181	0.39 (1)
D-K	-84 / 0	0.05 (1)
K-E	-718 / 0	0.12 (1)



DWG NO. TM 7801/91
STRUCTURAL
COMPONENTS ONLY
1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIROER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 924 (0.38")

CSI: TC=0.71/1.00 (B-C:1) , BC=0.85/1.00 (K-M:1) , WB=0.89/1.00 (G-I:1) , SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.87 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T318A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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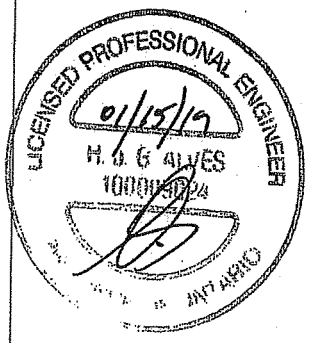
LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	LC1 TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AJ-AK	0 / 3803	-38.5	-38.5	0.58 (1)	10.00			
AK- J	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
J-AL	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AL-AM	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AM-AN	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			
AN- I	0 / 3603	-38.5	-38.5	0.58 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-7-10	-41	-46	—	FRONT	VERT	DEAD	—	—
C	5-7-10	-189	-189	—	FRONT	VERT	SNOW	—	—
L	16-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
P	6-0-12	-104	-104	—	FRONT	VERT	TOTAL	—	—
Q	8-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
R	10-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
S	12-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
T	14-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
U	16-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
V	18-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
W	20-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
X	22-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
Y	24-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
Z	26-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
AA	28-0-12	-80	-80	—	FRONT	VERT	TOTAL	—	—
AB	2-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AC	4-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AD	6-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AE	8-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AF	10-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AG	12-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AH	14-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AI	18-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AJ	20-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AK	22-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AL	24-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AM	26-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—
AN	28-0-12	-36	-46	—	FRONT	VERT	TOTAL	—	—



DWG NO. T4 T7901/91
STRUCTURAL
COMMENTS ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
300581	T319	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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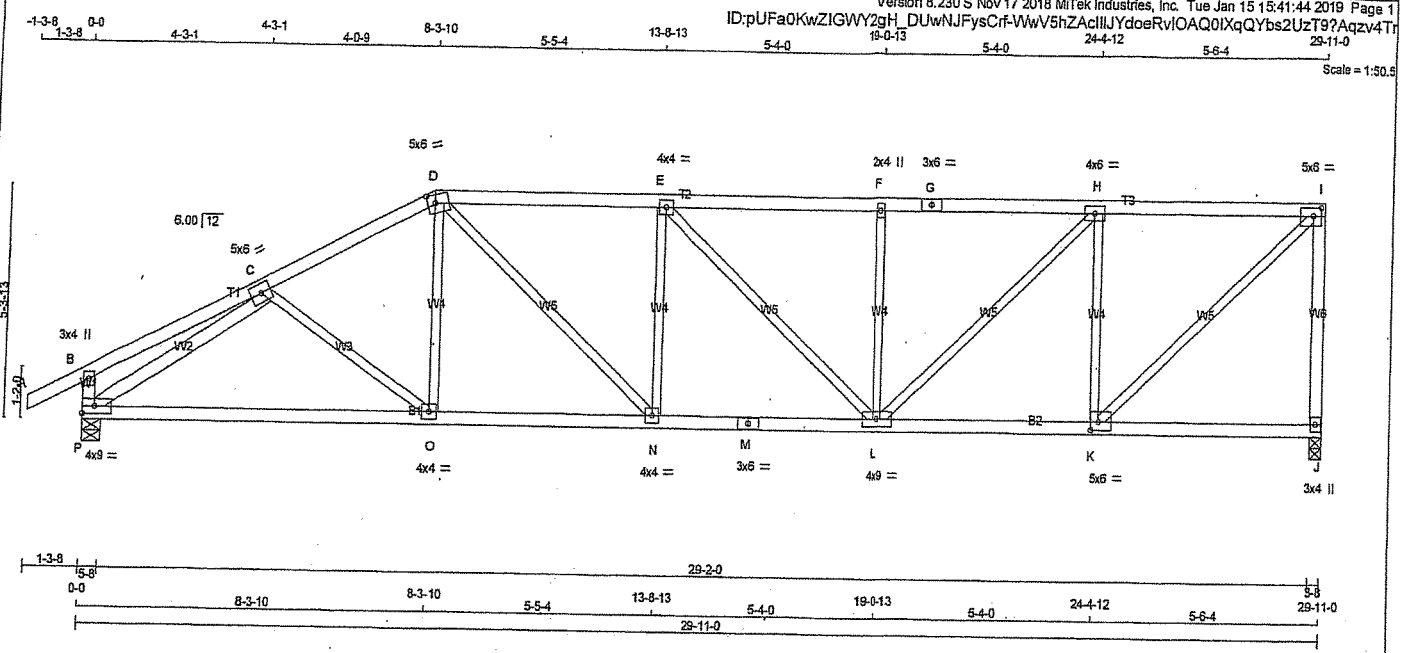


TOTAL WEIGHT = 125 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT2D	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.25	2.25
J	BMV1+p	MT2D	3.0	4.0		
K	BMWW-t	MT2D	5.0	6.0	2.25	2.25
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT2D	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTDRED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1845	0	1845	0	0	3-8	3-8
P	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
P	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

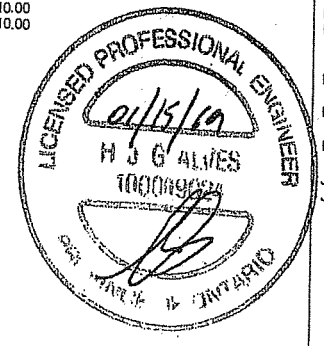
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM (PLF)	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 16	-84.9	-84.9	0.21 (1)	10.00
C-D	-2534 / 0	-84.9	-84.9	0.31 (1)	4.06
D-E	-2786 / 0	-84.9	-84.9	0.53 (1)	3.67
E-F	-2585 / 0	-84.9	-84.9	0.51 (1)	3.81
F-G	-2585 / 0	-84.9	-84.9	0.52 (1)	3.80
G-H	-2585 / 0	-84.9	-84.9	0.52 (1)	3.80
H-I	-1725 / 0	-84.9	-84.9	0.44 (1)	4.58
J-I	-1759 / 0	0.0	0.0	0.87 (1)	6.29
P-B	-256 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 2227	-38.5	-38.5	0.73 (2)	10.00
O-N	0 / 2256	-38.5	-38.5	0.75 (2)	10.00
N-M	0 / 2786	-38.5	-38.5	0.55 (1)	10.00
M-L	0 / 2786	-38.5	-38.5	0.55 (1)	10.00
L-K	0 / 1726	-38.5	-38.5	0.44 (2)	10.00
K-J	0 / 0	-38.5	-38.5	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-O	0 / 151	0.04 (3)
O-D	0 / 367	0.08 (2)
P-C	-2725 / 0	0.84 (1)
K-I	0 / 2334	0.53 (1)
D-N	0 / 721	0.18 (1)
K-H	-1337 / 0	0.56 (1)
N-E	-327 / 0	0.14 (1)
L-H	0 / 1182	0.27 (1)
E-L	-276 / 0	0.29 (1)
L-F	-415 / 0	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 953 (0.38")

CSI: TC=0.87/1.00 (I-J:1) , BC=0.75/1.00 (N-O:2) , WB=0.84/1.00 (C-P:1) , SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.99 (M) (INPUT = 1.00)

DWG NO. T&M 11901/92
CONVENTIONAL
GIRDER ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T319A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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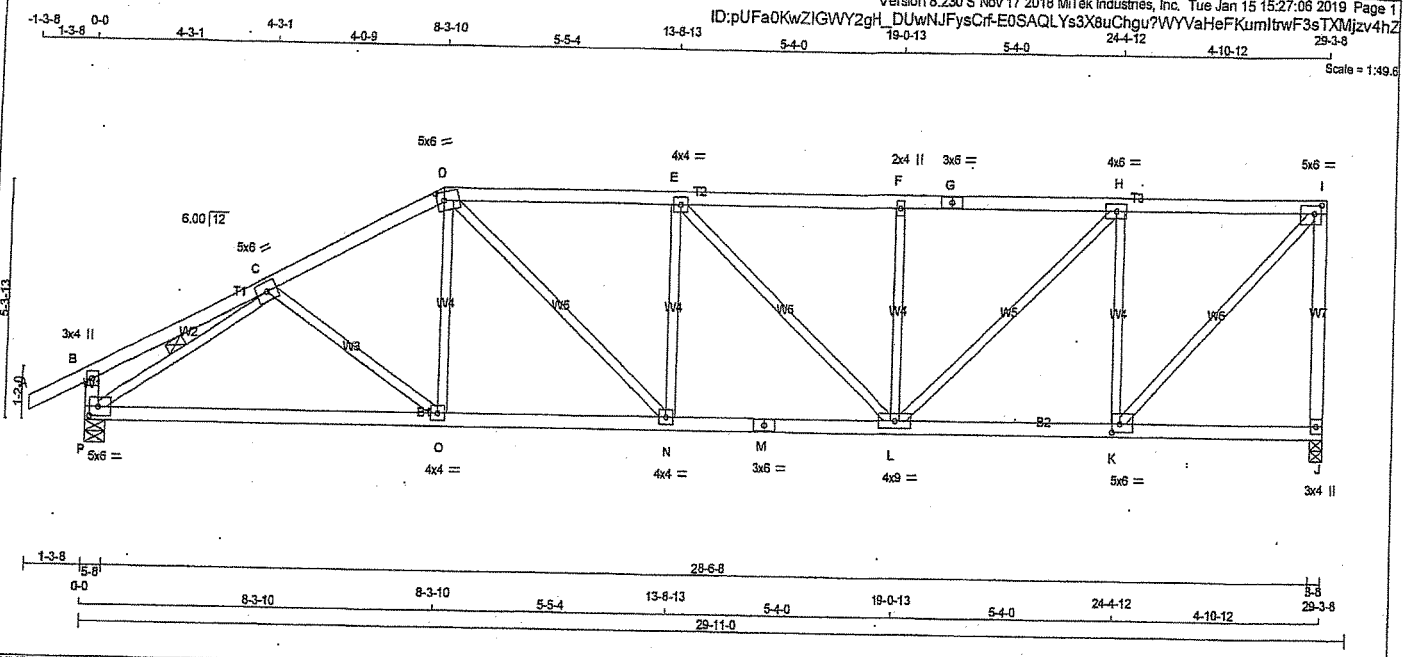


TOTAL WEIGHT = 122 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.25	2.25
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1807	0	1807	0	0	3-8	3-8
P	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
P	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

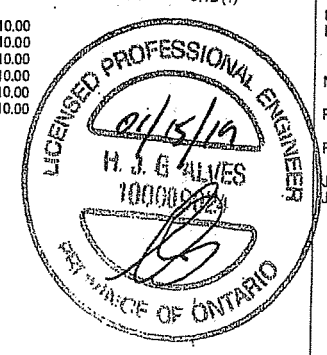
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 16	-84.9	-84.9	0.20 (1)	10.00
C-D	-2462 / 0	-84.9	-84.9	0.30 (1)	4.12
D-E	-2680 / 0	-84.9	-84.9	0.52 (1)	3.75
E-F	-2442 / 0	-84.9	-84.9	0.49 (1)	3.93
F-G	-2442 / 0	-84.9	-84.9	0.44 (1)	3.99
G-H	-2442 / 0	-84.9	-84.9	0.44 (1)	3.99
H-I	-1528 / 0	-84.9	-84.9	0.36 (1)	4.91
J-I	-1732 / 0	0.0	0.0	0.85 (1)	6.33
P-B	-257 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 2170	-38.5	-38.5	0.72 (2)	10.00
O-N	0 / 2192	-38.5	-38.5	0.74 (2)	10.00
N-M	0 / 2680	-38.5	-38.5	0.54 (1)	10.00
M-L	0 / 2880	-38.5	-38.5	0.54 (1)	10.00
L-K	0 / 1528	-38.5	-38.5	0.38 (2)	10.00
K-J	0 / 0	-38.5	-38.5	0.18 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LO)
C-O	0 / 147	0.03 (3)
O-D	0 / 371	0.08 (2)
P-C	-2656 / 0	0.56 (1)
K-I	0 / 2190	0.49 (1)
D-N	0 / 664	0.15 (1)
K-H	-1351 / 0	0.57 (1)
N-E	-290 / 15	0.12 (1)
L-H	0 / 1257	0.28 (1)
E-L	-327 / 0	0.34 (1)
L-F	-426 / 0	0.18 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 932 (0.38")

CSI: TC=0.85/1.00 (I-J:1) , BC=0.74/1.00 (N-O:2) , WB=0.57/1.00 (H-K:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (I) (INPUT = 0.90)
USI METAL= 0.95 (M) (INPUT = 1.00)

DWG NO. TAM T1901193
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
300581	T320	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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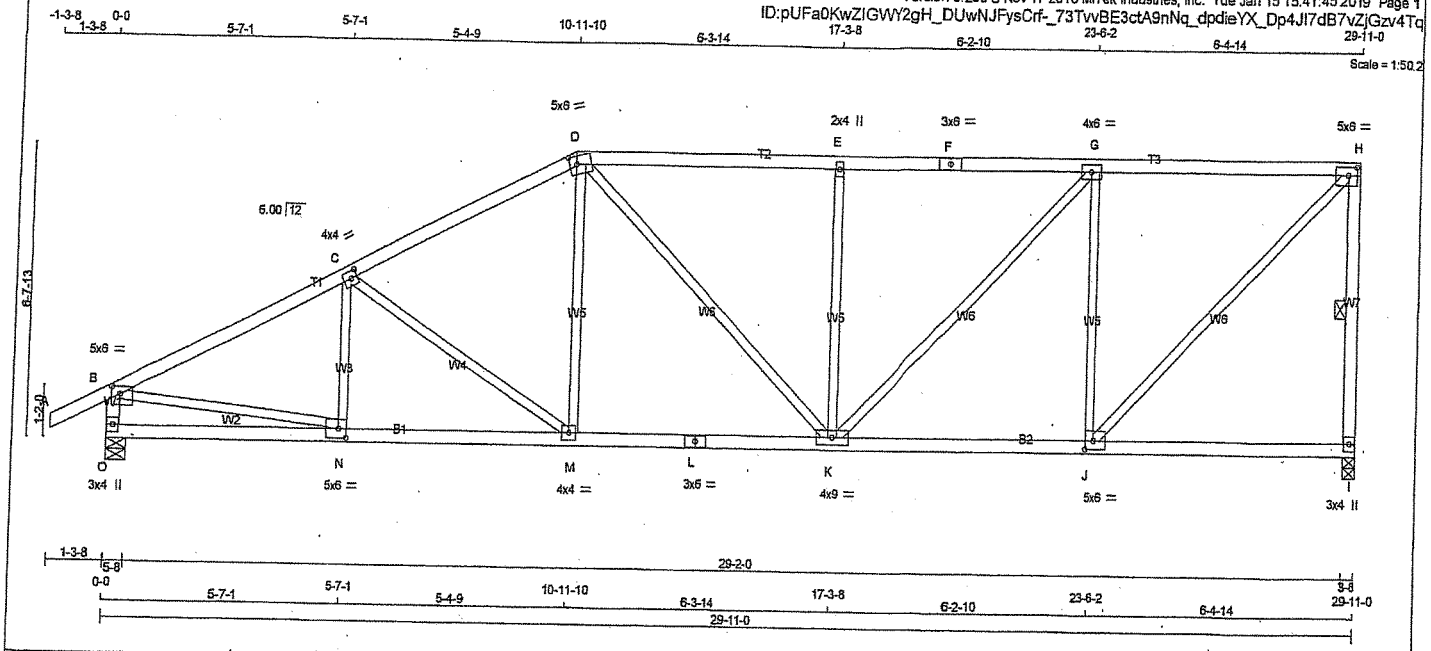

LUMBER

N. L. G. A. RULES

CHOROS	SIZE		LUMBER	OESCR.
A - O	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
O	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.25	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.25	2.50
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1845	0	1845	0	0	3-8	3-8
O	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	OEAD	SOIL
I	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
O	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHOROS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	N- C	-143 / 106	0.03 (1)
B- C	-2661 / 0	-84.9	-84.9	0.62 (1)	3.67	C- M	-446 / 0	0.35 (1)
C- O	-2295 / 0	-84.9	-84.9	0.56 (1)	3.95	M- O	0 / 505	0.11 (2)
D- E	-2184 / 0	-84.9	-84.9	0.61 (1)	3.97	B- N	0 / 2429	0.55 (1)
E- F	-2184 / 0	-84.9	-84.9	0.63 (1)	3.94	J- H	0 / 2166	0.49 (1)
F- G	-2184 / 0	-84.9	-84.9	0.63 (1)	3.94	D- K	0 / 207	0.05 (1)
G- H	-1537 / 0	-84.9	-84.9	0.56 (1)	4.62	J- G	-1251 / 0	0.91 (1)
I- H	-1746 / 0	0.0	0.0	0.35 (1)	5.04	K- E	-578 / 0	0.42 (1)
O- B	-1869 / 0	0.0	0.0	0.19 (1)	6.13	K- G	0 / 925	0.21 (1)
O- N	0 / 0	-38.5	-38.5	0.20 (3)	10.00			
N- M	0 / 2401	-38.5	-38.5	0.54 (1)	10.00			
M- L	0 / 2038	-38.5	-38.5	0.48 (2)	10.00			
L- K	0 / 2038	-38.5	-38.5	0.48 (2)	10.00			
K- J	0 / 1537	-38.5	-38.5	0.49 (2)	10.00			
J- I	0 / 0	-38.5	-38.5	0.29 (3)	10.00			

TOTAL WEIGHT = 127 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	OL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.83/1.00 (E-G:1) , BC=0.54/1.00 (M-N:1) , WB=0.91/1.00 (G-J:1) , SSI=0.25/1.00 (G-H:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 deg.

JS GRIP= 0.89 (H) (INPUT = 0.90)
JS METAL= 0.78 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
N. J. G. ALVES
100099824
PROVINCE OF ONTARIO

ENG NO. TAM 77901194
GENERAL
COMPANY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T320A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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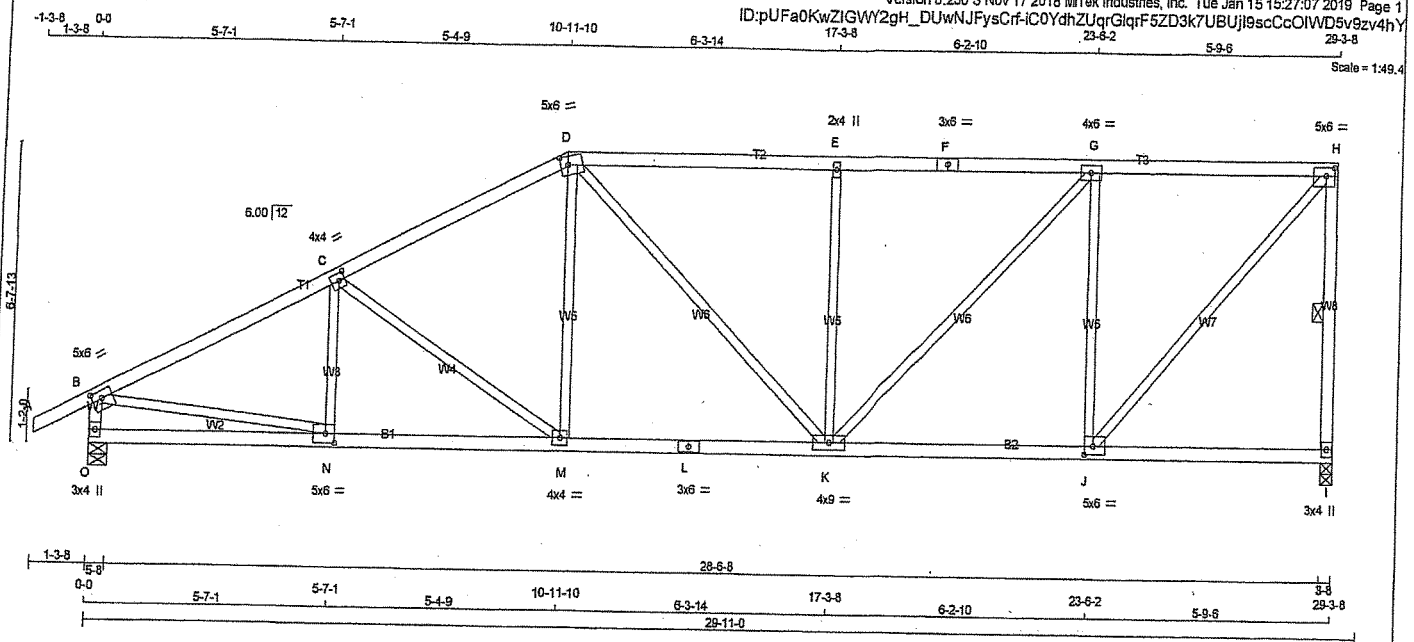


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
O	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	8.0	2.25	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.25	2.50
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	4.0		

TOTAL WEIGHT = 125 lb

[M]

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1807	0	1807	0	0	3-8	3-8
O	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
O	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I.

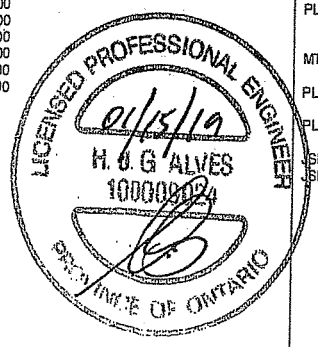
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2595 / 0	-84.9	-84.9	0.61 (1)	3.72
C- O	-2221 / 0	-84.9	-84.9	0.55 (1)	4.01
D- E	-2082 / 0	-84.9	-84.9	0.62 (1)	4.03
E- F	-2082 / 0	-84.9	-84.9	0.61 (1)	4.03
F- G	-2082 / 0	-84.9	-84.9	0.61 (1)	4.03
G- H	-1384 / 0	-84.9	-84.9	0.46 (1)	4.95
I- H	-1719 / 0	0.0	0.0	0.35 (1)	5.08
O- B	-1831 / 0	0.0	0.0	0.19 (1)	6.19
O- N	0 / 0	-38.5	-38.5	0.20 (3)	10.00
N- M	0 / 2342	-38.5	-38.5	0.52 (1)	10.00
M- L	0 / 1971	-38.5	-38.5	0.48 (2)	10.00
L- K	0 / 1971	-38.5	-38.5	0.48 (2)	10.00
K- J	0 / 1384	-38.5	-38.5	0.44 (2)	10.00
J- I	0 / 0	-38.5	-38.5	0.25 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-133 / 111	0.03 (1)
C- M	-456 / 0	0.36 (1)
M- D	0 / 508	0.11 (2)
B- N	0 / 2370	0.53 (1)
J- H	0 / 2058	0.46 (1)
D- K	0 / 157	0.04 (1)
J- G	-1265 / 0	0.92 (1)
K- E	-589 / 0	0.43 (1)
K- G	0 / 998	0.22 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.62/1.00 (D-E:1) , BC=0.52/1.00 (M-N:1) , WB=0.92/1.00 (G-J:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (J) (INPUT = 0.90)
SI METAL= 0.76 (L) (INPUT = 1.00)

ENGINEERING BY T1901195

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T321	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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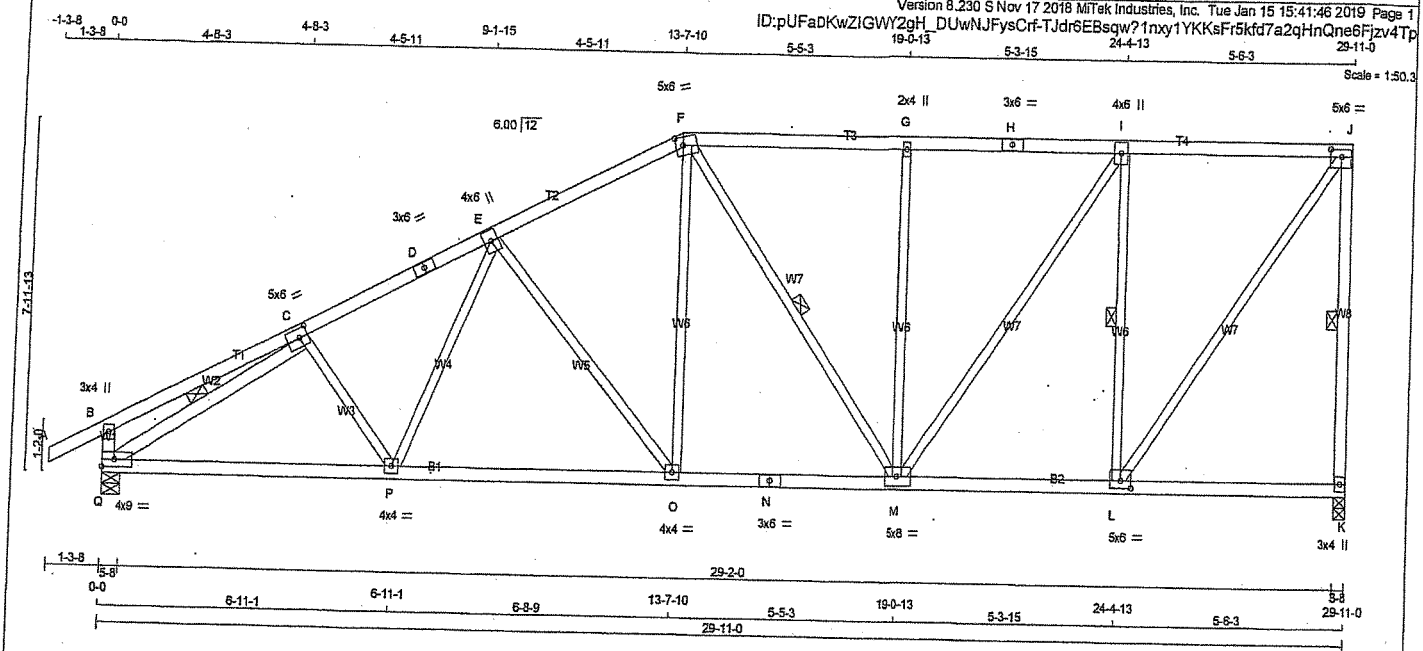

TOTAL WEIGHT = 140 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.00	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0	2.00	3.00
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	OOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1845	0	1845	0	0	3-8	3-8
Q	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
Q	1479	780 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-K, C-Q, F-M, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 15	-84.9	-84.9	0.21 (1)	10.00
C-D	-2588 / 0	-84.9	-84.9	0.35 (1)	4.01
D-E	-2568 / 0	-84.9	-84.9	0.35 (1)	4.01
E-F	-2018 / 0	-84.9	-84.9	0.31 (1)	4.47
F-G	-1698 / 0	-84.9	-84.9	0.40 (1)	4.67
G-H	-1698 / 0	-84.9	-84.9	0.41 (1)	4.64
H-I	-1698 / 0	-84.9	-84.9	0.41 (1)	4.64
I-J	-1126 / 0	-84.9	-84.9	0.37 (1)	5.49
K-J	-1760 / 0	0.0	0.0	0.51 (1)	5.03
O-B	-276 / 0	0.0	0.0	0.03 (1)	7.81
Q-P	0 / 2291	-38.5	-38.5	0.65 (2)	10.00
P-O	0 / 2181	-38.5	-38.5	0.63 (2)	10.00
O-N	0 / 1798	-38.5	-38.5	0.45 (2)	10.00
N-M	0 / 1798	-38.5	-38.5	0.45 (2)	10.00
M-L	0 / 1126	-38.5	-38.5	0.37 (2)	10.00
L-K	0 / 0	-38.5	-38.5	0.22 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-P	0 / 132	0.03 (3)
P-E	0 / 352	0.08 (2)
E-O	-608 / 0	0.59 (1)
O-F	0 / 696	0.16 (1)
Q-C	-2781 / 0	0.46 (1)
L-J	0 / 1932	0.43 (1)
F-M	-173 / 0	0.11 (1)
L-I	-1331 / 0	0.53 (1)
M-G	-495 / 0	0.60 (1)
M-I	0 / 1003	0.23 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100099824
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	48.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.51/1.00 (J-K:1) , BC=0.65/1.00 (P-Q:2) , WB=0.60/1.00 (G-M:1) , SSI=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (C) (INPUT = 0.90)
USI METAL= 0.69 (C) (INPUT = 1.00)

ENG NO TAM 71901/96
STRUCTURAL
COMPONENTS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T321A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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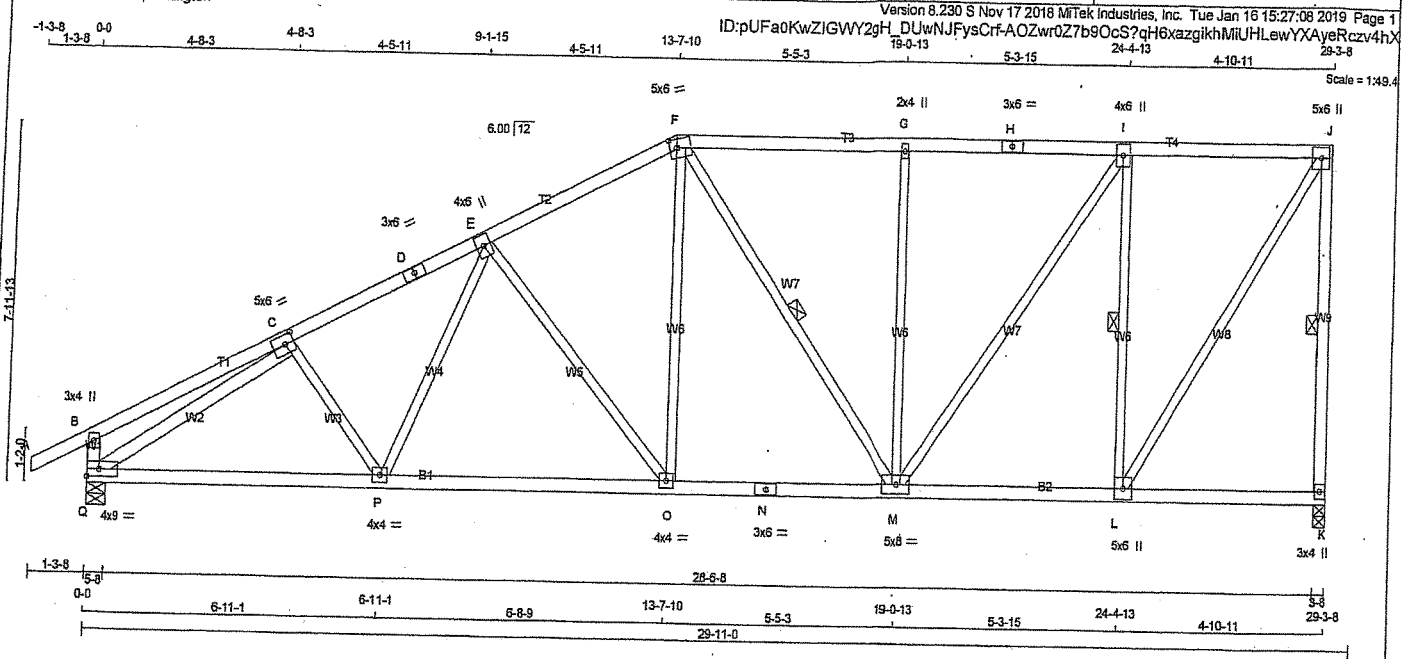


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TMVW+p	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		
L	BMWW+t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
K	1807	0	1807	0	0	3-8	3-8
Q	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
Q	1450	746 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-K, F-M, I-L.

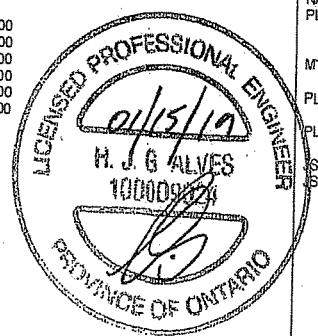
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B- C	0 / 15	-84.9	-84.9	0.21 (1)		10.00
C- D	-2499 / 0	-84.9	-84.9	0.35 (1)		4.06
D- E	-2499 / 0	-84.9	-84.9	0.35 (1)		4.06
E- F	-1941 / 0	-84.9	-84.9	0.30 (1)		4.55
F- G	-1604 / 0	-84.9	-84.9	0.40 (1)		4.75
G- H	-1604 / 0	-84.9	-84.9	0.40 (1)		4.75
H- I	-1604 / 0	-84.9	-84.9	0.40 (1)		4.75
I- J	-996 / 0	-84.9	-84.9	0.30 (1)		5.85
K- J	-1733 / 0	0.0	0.0	0.50 (1)		5.06
Q- B	-276 / 0	0.0	0.0	0.03 (1)		7.81
Q- P	0 / 2236	-38.5	-38.5	0.64 (2)		10.00
P- O	0 / 2116	-38.5	-38.5	0.62 (2)		10.00
O- N	0 / 1729	-38.5	-38.5	0.44 (2)		10.00
N- M	0 / 1729	-38.5	-38.5	0.44 (2)		10.00
M- L	0 / 996	-38.5	-38.5	0.32 (2)		10.00
L- K	0 / 0	-38.5	-38.5	0.19 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 127	0.03 (3)
P- E	0 / 360	0.08 (2)
E- O	-614 / 0	0.60 (1)
O- F	0 / 699	0.16 (1)
Q- C	-2713 / 0	0.98 (1)
L- J	0 / 1857	0.42 (1)
F- M	-217 / 0	0.13 (1)
L- I	-1345 / 0	0.54 (1)
M- G	-507 / 0	0.61 (1)
M- I	0 / 1067	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/380 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.50/1.00 (J-K:1) , BC=0.64/1.00 (P-Q:2) , WB=0.98/1.00 (C-Q:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (J) (INPUT = 0.90)
SI METAL= 0.67 (C) (INPUT = 1.00)

DWG NO. TAM T901197
STRUCTURAL
COPY OF ENGINEER

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T322	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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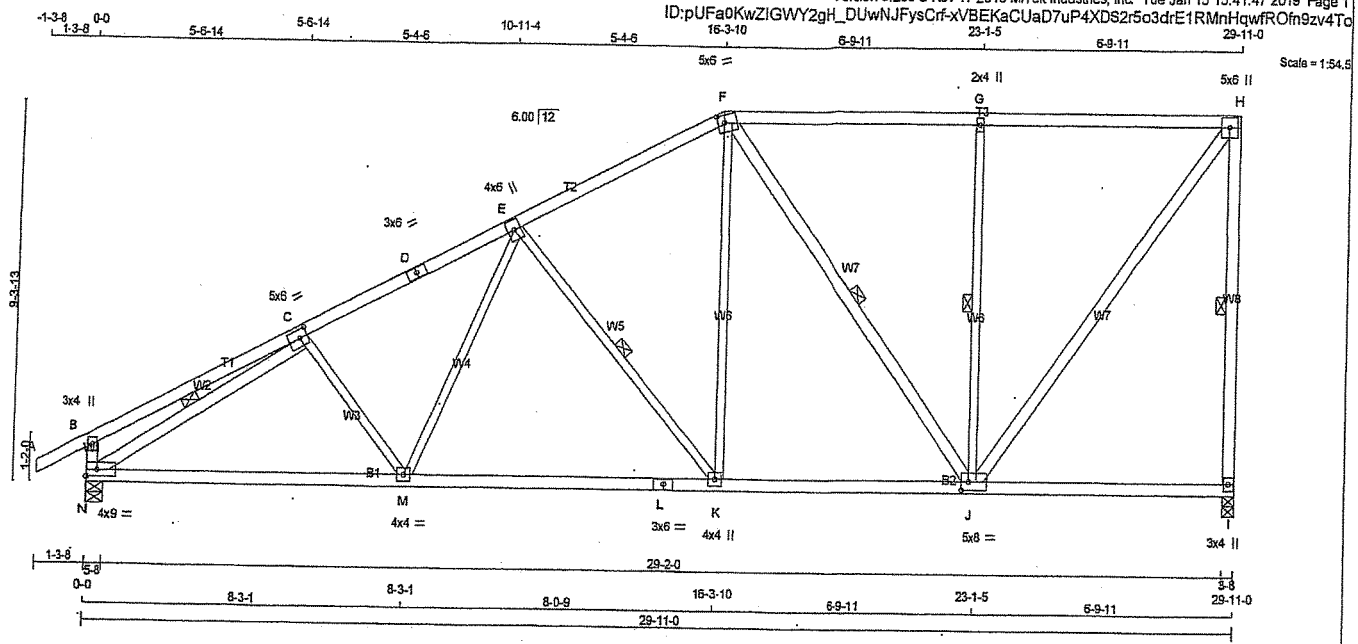


TOTAL WEIGHT = 4 X 144 = 578 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF

ORY: SEASONEO LUM8ER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
O	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.60	2.25
K	BMWW+t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INOICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHDRD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1845	0	1845	0	0	3-8	3-8
N	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
N	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR 8ETTER AT JOINT(S) I, N

BRACING

TOP CHORO TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACEO BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING 0IRECTLY APPLIED.

ALL PITCH BREAKS ANO PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

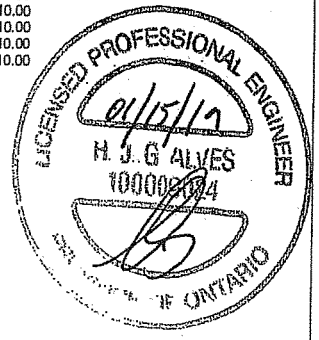
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B- C	0 / 19	-84.9	-84.9	0.32 (1)		10.00
C- D	-2511 / 0	-84.9	-84.9	0.37 (1)		4.05
D- E	-2511 / 0	-84.9	-84.9	0.37 (1)		4.05
E- F	-1727 / 0	-84.9	-84.9	0.33 (1)		4.75
F- G	-1152 / 0	-84.9	-84.9	0.53 (1)		5.19
G- H	-1152 / 0	-84.9	-84.9	0.53 (1)		5.19
I- H	-1739 / 0	0.0	0.0	0.71 (1)		5.05
N- B	-305 / 0	0.0	0.0	0.03 (1)		7.81
N- M	0 / 2325	-38.5	-38.5	0.80 (2)		10.00
M- L	0 / 2055	-38.5	-38.5	0.77 (2)		10.00
L- K	0 / 2055	-38.5	-38.5	0.77 (2)		10.00
K- J	0 / 1536	-38.5	-38.5	0.53 (2)		10.00
J- I	0 / 0	-38.5	-38.5	0.33 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
C- M	-127 / 87	0.04	(1)
M- E	0 / 533	0.12	(2)
E- K	-808 / 0	0.39	(1)
K- F	0 / 877	0.20	(1)
F- J	-640 / 0	0.44	(1)
J- G	-717 / 0	0.41	(1)
J- H	0 / 1906	0.31	(1)
N- C	-2782 / 0	0.58	(1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNEO FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, N8CC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF 8CBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.71/1.00 (H-I:1) , BC=0.80/1.00 (M-N:2) , WB=0.58/1.00 (C-N:1) , SSI=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.68 (C) (INPUT = 1.00)

DWG NO. T322 1901198
STRUCTURAL
CONNECT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T322A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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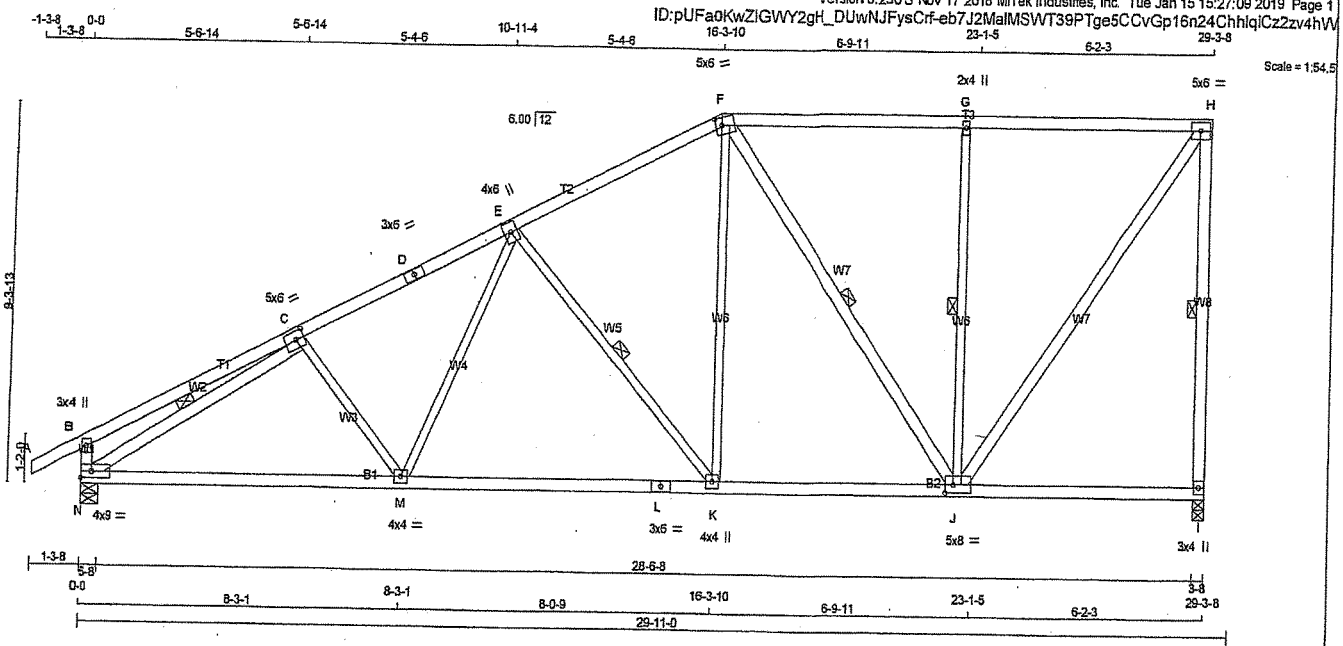

TOTAL WEIGHT = 4 X 143 = 570 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
P	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	6.0	2.50	2.50
K	BMWW+t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1807	0	1807	0	0	3-8	3-8
N	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS .SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1370	682 / 0	306 / 0	0 / 0	0 / 0	381 / 0	0 / 0
N	1450	745 / 0	306 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 19	-84.9	-84.9	0.32 (1)	10.00
C-D	-2440 / 0	-84.9	-84.9	0.36 (1)	4.11
D-E	-2440 / 0	-84.9	-84.9	0.36 (1)	4.11
E-F	-1649 / 0	-84.9	-84.9	0.32 (1)	4.84
F-G	-1082 / 0	-84.9	-84.9	0.48 (1)	5.41
G-H	-1082 / 0	-84.9	-84.9	0.48 (1)	5.41
I-H	-1705 / 0	0.0	0.0	0.69 (1)	5.09
N-B	-305 / 0	0.0	0.0	0.03 (1)	7.81
N-M	0 / 2267	-38.5	-38.5	0.80 (2)	10.00
M-L	0 / 1966	-38.5	-38.5	0.76 (2)	10.00
L-K	0 / 1966	-38.5	-38.5	0.76 (2)	10.00
K-J	0 / 1466	-38.5	-38.5	0.49 (2)	10.00
J-I	0 / 0	-38.5	-38.5	0.29 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-M	-135 / 83	0.05 (1)
M-E	0 / 541	0.12 (2)
E-K	-813 / 0	0.39 (1)
K-F	0 / 876	0.20 (1)
F-J	-659 / 0	0.44 (1)
J-G	-684 / 0	0.39 (1)
J-H	0 / 1845	0.30 (1)
N-C	-2713 / 0	0.57 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100080324
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED RDOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.69/1.00 (H-I:1) , BC=0.80/1.00 (M-N:2) , WB=0.57/1.00 (C-N:1) , SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.89 (M) (INPUT = 0.90)
JS METAL= 0.67 (C) (INPUT = 1.00)

DWG NO. TAM 1701197
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	G322	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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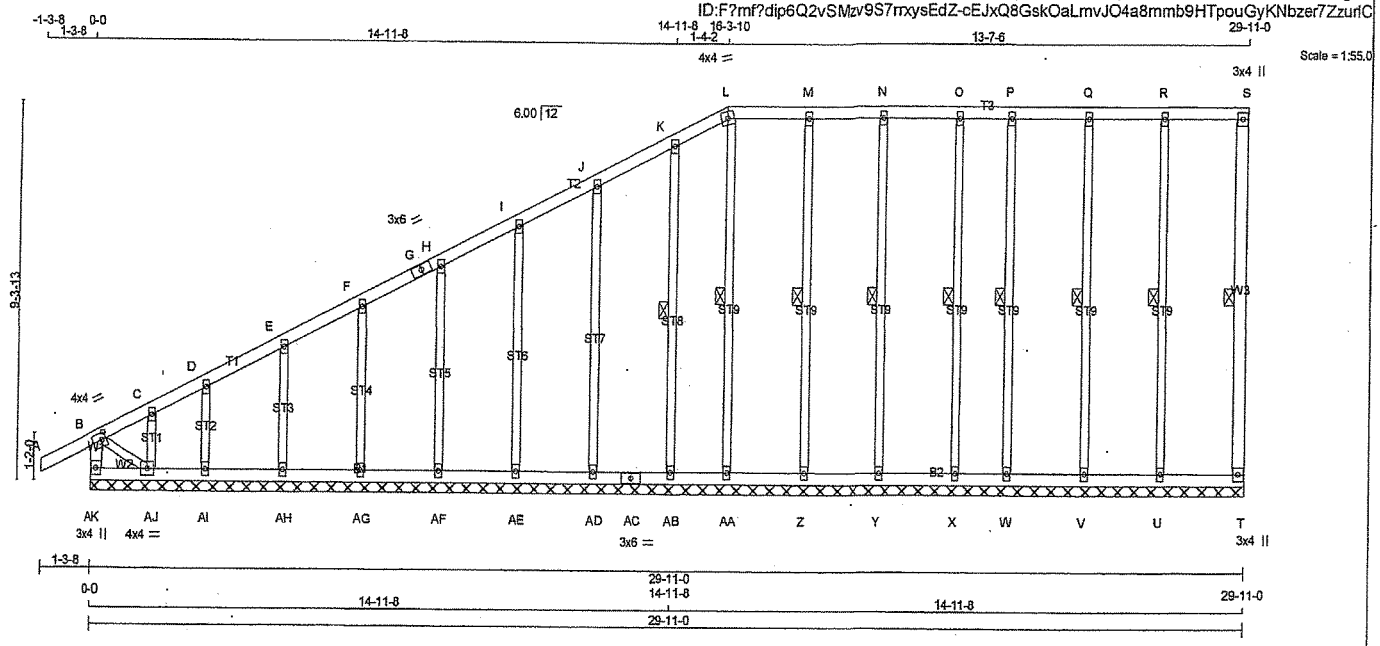


TOTAL WEIGHT = 164 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
L - S	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
AK- B	2x4	DRY	No.2	SPF
AK- AC	2x4	DRY	No.2	SPF
AC- T	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C, D, E, F, H, I, J, K, M, N, O, P, Q, R						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
L	TTW-m	MT20	4.0	4.0		
S	TMV+p	MT20	3.0	4.0		
T	BMV1+p	MT20	3.0	4.0		
U, V, W, X, Y, Z, AA, AB, AD, AE, AF, AG, AH, AI						
U	BMW1+w	MT20	2.0	4.0		
AC	BS-t	MT20	3.0	6.0		
AJ	BMWW1-t	MT20	4.0	4.0		
AK	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF S-T, K-AB, L-AA, M-Z, N-Y, O-X, P-W, O-V, R-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-46 / 0	-84.9	-84.9	0.11 (1)	6.25
C- D	-12 / 0	-84.9	-84.9	0.03 (1)	6.25
D- E	-14 / 0	-84.9	-84.9	0.04 (1)	6.25
E- F	-12 / 0	-84.9	-84.9	0.04 (1)	6.25
F- G	-9 / 0	-84.9	-84.9	0.04 (1)	10.00
G- H	-9 / 0	-84.9	-84.9	0.04 (1)	10.00
H- I	-7 / 0	-84.9	-84.9	0.04 (1)	10.00
I- J	-4 / 0	-84.9	-84.9	0.04 (1)	10.00
J- K	-4 / 0	-84.9	-84.9	0.04 (1)	10.00
K- L	-9 / 0	-84.9	-84.9	0.03 (1)	10.00
L- M	0 / 0	-84.9	-84.9	0.05 (1)	10.00
M- N	0 / 0	-84.9	-84.9	0.05 (1)	10.00
N- O	0 / 0	-84.9	-84.9	0.04 (1)	10.00
O- P	0 / 0	-84.9	-84.9	0.03 (1)	10.00
P- Q	0 / 0	-84.9	-84.9	0.04 (1)	10.00
Q- R	0 / 0	-84.9	-84.9	0.05 (1)	10.00
R- S	0 / 0	-84.9	-84.9	0.05 (1)	10.00
T- S	-75 / 0	0.0	0.0	0.03 (1)	6.25
AK- B	-253 / 0	0.0	0.0	0.03 (1)	7.81
AK-AJ	0 / 0	-38.5	-38.5	0.02 (3)	10.00
AJ-AI	0 / 18	-38.5	-38.5	0.02 (2)	10.00
AI-AH	0 / 14	-38.5	-38.5	0.03 (3)	10.00
AH-AG	0 / 10	-38.5	-38.5	0.03 (3)	10.00
AG-AF	0 / 8	-38.5	-38.5	0.02 (3)	10.00
AF-AE	0 / 6	-38.5	-38.5	0.02 (3)	10.00
AE-AD	0 / 4	-38.5	-38.5	0.03 (3)	10.00
AD-AC	0 / 3	-38.5	-38.5	0.03 (3)	10.00
AC-AB	0 / 3	-38.5	-38.5	0.03 (3)	10.00
AB-AA	0 / 1	-38.5	-38.5	0.02 (3)	10.00
AA- Z	0 / 0	-38.5	-38.5	0.02 (3)	10.00
Z- Y	0 / 0	-38.5	-38.5	0.02 (3)	10.00
Y- X	0 / 0	-38.5	-38.5	0.02 (3)	10.00
X- W	0 / 0	-38.5	-38.5	0.02 (3)	10.00
W- V	0 / 0	-38.5	-38.5	0.02 (3)	10.00
V- U	0 / 0	-38.5	-38.5	0.03 (3)	10.00
U- T	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-AJ	0 / 23	0.01 (1)
AB- K	-158 / 0	0.08 (1)
AD- J	-172 / 0	0.19 (1)
AE- I	-168 / 0	0.12 (1)
AF- H	-169 / 0	0.08 (1)
AG- F	-168 / 0	0.05 (1)
AH- E	-171 / 0	0.04 (1)
AI- D	-158 / 0	0.03 (1)
AJ- C	-50 / 0	0.01 (1)
AA- L	-115 / 0	0.07 (1)
Z- M	-194 / 0	0.11 (1)
Y- N	-170 / 0	0.10 (1)
X- O	-140 / 0	0.08 (1)
W- P	-139 / 0	0.08 (1)
V- Q	-170 / 0	0.10 (1)
U- R	-194 / 0	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (U-V:3) , WB=0.19/1.00 (J-AD:1) , SSI=0.08/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

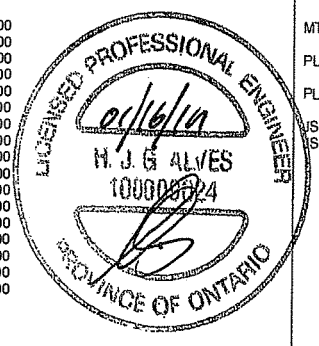
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.75 (B) (INPUT = 0.90)
(SI METAL= 0.07 (J) (INPUT = 1.00)



DWG NO. TAM T190125Z
STRUCTURAL COMPONENT ONLY

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	G322A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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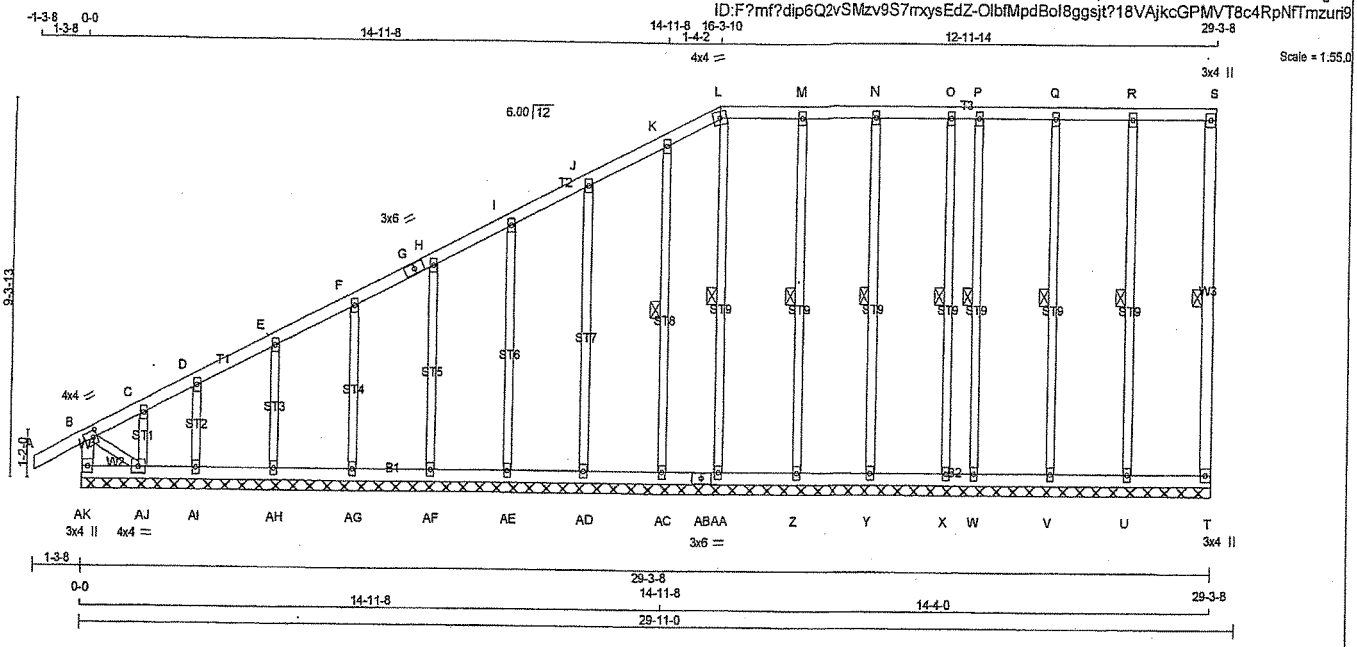


TOTAL WEIGHT = 163 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
L - S	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
AK- B	2x4	DRY	No.2	SPF
AK- AB	2x4	DRY	No.2	SPF
AB- T	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C, D, E, F, H, I, J, K, M, N, O, P, Q, R						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
L	TTW-m	MT20	4.0	4.0		
S	TMV+p	MT20	3.0	4.0		
T	BMV1+p	MT20	3.0	4.0		
U, V, W, X, Y, Z, AA, AC, AD, AE, AF, AG, AH, AI						
U	BMW1+w	MT20	2.0	4.0		
AB	BS-t	MT20	3.0	6.0		
AJ	BMWW1-t	MT20	4.0	4.0		
AK	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF S-T, K-AC, L-AA, M-Z, N-Y, O-X, P-W, O-V, R-U.

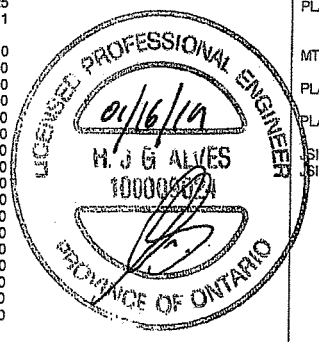
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)		10.00
B- C	-46 / 0	-84.9	-84.9	0.11 (1)		6.25
C- D	-12 / 0	-84.9	-84.9	0.03 (1)		6.25
D- E	-14 / 0	-84.9	-84.9	0.04 (1)		6.25
E- F	-12 / 0	-84.9	-84.9	0.04 (1)		6.25
F- G	-9 / 0	-84.9	-84.9	0.04 (1)		10.00
G- H	-9 / 0	-84.9	-84.9	0.04 (1)		10.00
H- I	-7 / 0	-84.9	-84.9	0.04 (1)		10.00
I- J	-4 / 0	-84.9	-84.9	0.04 (1)		10.00
J- K	-4 / 0	-84.9	-84.9	0.04 (1)		10.00
K- L	-9 / 0	-84.9	-84.9	0.03 (1)		10.00
L- M	0 / 0	-84.9	-84.9	0.05 (1)		10.00
M- N	0 / 0	-84.9	-84.9	0.05 (1)		10.00
N- O	0 / 0	-84.9	-84.9	0.04 (1)		10.00
O- P	0 / 0	-84.9	-84.9	0.03 (1)		10.00
P- Q	0 / 0	-84.9	-84.9	0.04 (1)		10.00
Q- R	0 / 0	-84.9	-84.9	0.05 (1)		10.00
R- S	0 / 0	-84.9	-84.9	0.05 (1)		10.00
T- S	-75 / 0	0.0	0.0	0.03 (1)		6.25
AK- B	-253 / 0	0.0	0.0	0.03 (1)		7.81
AK-AJ	0 / 0	-38.5	-38.5	0.02 (3)		10.00
AJ-AI	0 / 18	-38.5	-38.5	0.02 (2)		10.00
AI-AH	0 / 14	-38.5	-38.5	0.03 (3)		10.00
AH-AG	0 / 10	-38.5	-38.5	0.03 (3)		10.00
AG-AF	0 / 8	-38.5	-38.5	0.02 (3)		10.00
AF-AE	0 / 6	-38.5	-38.5	0.02 (3)		10.00
AE-AD	0 / 4	-38.5	-38.5	0.03 (3)		10.00
AD-AC	0 / 3	-38.5	-38.5	0.03 (3)		10.00
AC-AB	0 / 1	-38.5	-38.5	0.02 (3)		10.00
AB-AA	0 / 1	-38.5	-38.5	0.02 (3)		10.00
AA- Z	0 / 0	-38.5	-38.5	0.02 (3)		10.00
Z- Y	0 / 0	-38.5	-38.5	0.03 (3)		10.00
Y- X	0 / 0	-38.5	-38.5	0.03 (3)		10.00
X- W	0 / 0	-38.5	-38.5	0.02 (3)		10.00
W- V	0 / 0	-38.5	-38.5	0.02 (3)		10.00
V- U	0 / 0	-38.5	-38.5	0.03 (3)		10.00
U- T	0 / 0	-38.5	-38.5	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-AJ	0 / 23	0.01 (1)
AC- K	-158 / 0	0.08 (1)
AD- J	-172 / 0	0.19 (1)
AE- I	-168 / 0	0.12 (1)
AF- H	-169 / 0	0.08 (1)
AG- F	-168 / 0	0.05 (1)
AH- E	-171 / 0	0.04 (1)
AI- D	-158 / 0	0.03 (1)
AJ- C	-50 / 0	0.01 (1)
AA- L	-115 / 0	0.07 (1)
Z- M	-194 / 0	0.11 (1)
Y- N	-168 / 0	0.10 (1)
X- O	-114 / 0	0.06 (1)
W- P	-114 / 0	0.06 (1)
V- O	-169 / 0	0.10 (1)
U- R	-195 / 0	0.11 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (T-U:3) , WB=0.19/1.00 (J-AD:1) , SSI=0.08/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.75 (B) (INPUT = 0.90)
SI METAL= 0.07 (J) (INPUT = 1.00)

DWG NO. TAM T1901253
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	T323	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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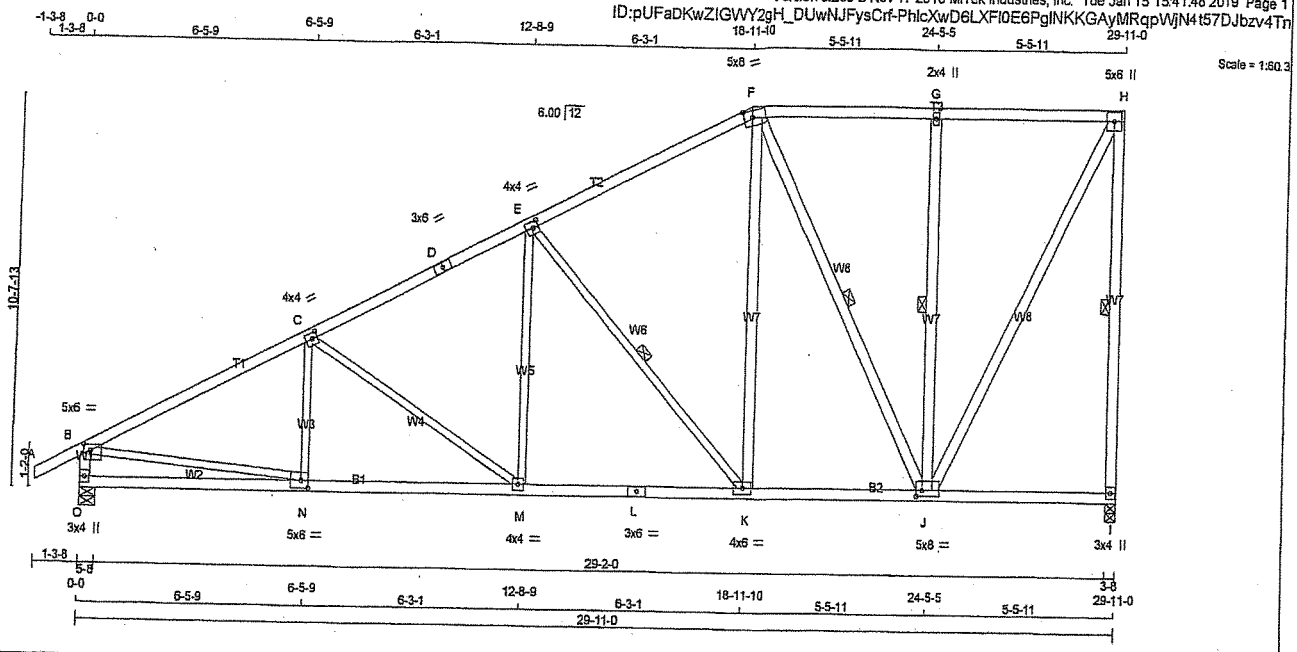


TOTAL WEIGHT = 161 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
O	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTWW-m	MT20	5.0	8.0	2.25	3.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.00	2.00
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HDRZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1845	0	1845	0	0	3-8	3-8
O	1960	0	1960	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNDW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1400	697 / 0	314 / 0	0 / 0	0 / 0	389 / 0	0 / 0
O	1479	760 / 0	314 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

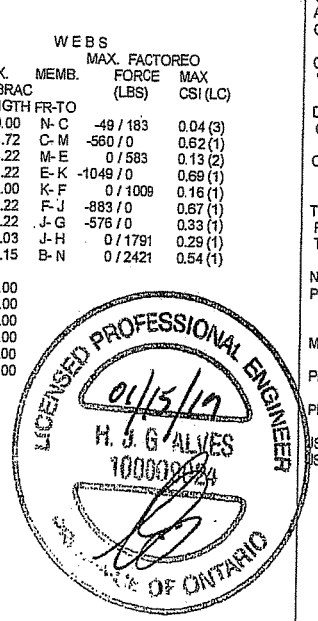
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.9	-84.9	0.11 (1)	10.00
B- C	-2660 / 0	-84.9	-84.9	0.59 (1)	3.72
C- D	-2159 / 0	-84.9	-84.9	0.45 (1)	4.22
D- E	-2159 / 0	-84.9	-84.9	0.45 (1)	4.22
E- F	-1405 / 0	-84.9	-84.9	0.43 (1)	5.00
F- G	-836 / 0	-84.9	-84.9	0.33 (1)	6.22
G- H	-837 / 0	-84.9	-84.9	0.33 (1)	6.22
I- H	-1759 / 0	0.0	0.0	1.00 (1)	5.03
O- B	-1857 / 0	0.0	0.0	0.19 (1)	6.15
O- N	0 / 0	-38.5	-38.5	0.28 (3)	10.00
N- M	0 / 2400	-38.5	-38.6	0.60 (2)	10.00
M- L	0 / 1931	-38.5	-38.5	0.49 (2)	10.00
L- K	0 / 1931	-38.5	-38.5	0.49 (2)	10.00
K- J	0 / 1249	-38.5	-38.5	0.37 (2)	10.00
J- I	0 / 0	-38.5	-38.5	0.20 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
N- C	-49 / 183	0.04 (3)
C- M	-560 / 0	0.62 (1)
M- E	0 / 583	0.13 (2)
E- K	-1049 / 0	0.69 (1)
K- F	0 / 1009	0.16 (1)
F- J	-883 / 0	0.67 (1)
J- G	-576 / 0	0.33 (1)
J- H	0 / 1791	0.29 (1)
B- N	0 / 2421	0.54 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=1.00/1.00 (H-I:1) , BC=0.60/1.00 (M-N:2) , WB=0.69/1.00 (E-K:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (B) (INPUT = 0.90)
USI METAL= 0.77 (L) (INPUT = 1.00)

ENG NO. TAM T1901200
STRUCTURAL
CODE COMPLIANCE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	T323A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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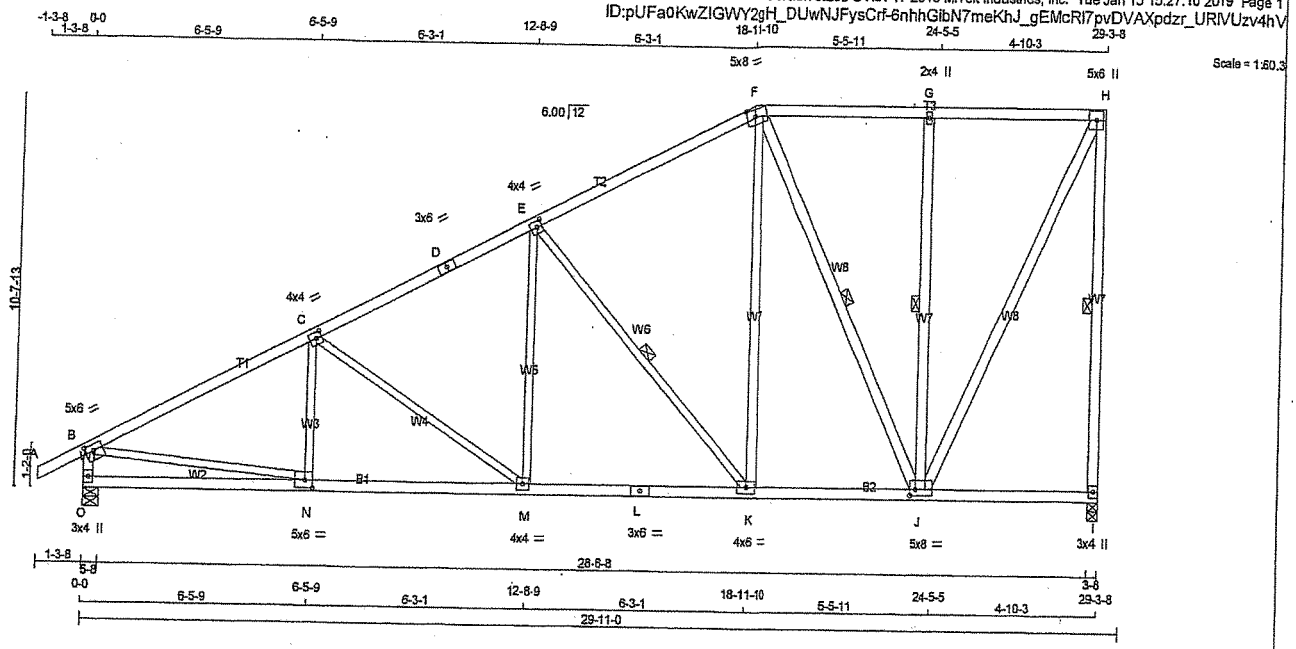


TOTAL WEIGHT = 159 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - O	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTWW-m	MT20	5.0	8.0	2.25	3.00
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0	2.00	2.00
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1807	0	1807	0	0	3-8	3-8
O	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1370	682 / 0	308 / 0	0 / 0	0 / 0	381 / 0	0 / 0
O	1450	745 / 0	308 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J.

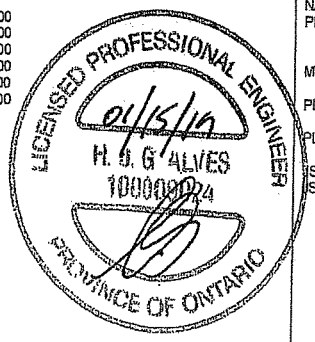
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.9	-84.9	0.11 (1)	10.00	
B-C	-2592 / 0	-84.9	-84.9	0.58 (1)	3.77	
C-D	-2083 / 0	-84.9	-84.9	0.45 (1)	4.29	
D-E	-2083 / 0	-84.9	-84.9	0.45 (1)	4.29	
E-F	-1325 / 0	-84.9	-84.9	0.42 (1)	5.13	
F-G	-776 / 0	-84.9	-84.9	0.29 (1)	6.25	
G-H	-777 / 0	-84.9	-84.9	0.29 (1)	6.25	
I-H	-1725 / 0	0.0	0.0	0.99 (1)	5.07	
O-B	-1819 / 0	0.0	0.0	0.18 (1)	6.20	
O-N	0 / 0	-38.5	-38.5	0.26 (3)	10.00	
N-M	0 / 2339	-38.5	-38.5	0.59 (2)	10.00	
M-L	0 / 1863	-38.5	-38.5	0.48 (2)	10.00	
L-K	0 / 1863	-38.5	-38.5	0.48 (2)	10.00	
K-J	0 / 1178	-38.5	-38.5	0.35 (2)	10.00	
J-I	0 / 0	-38.5	-38.5	0.18 (3)	10.00	

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-41 / 187	0.04 (3)
C-M	-568 / 0	0.63 (1)
M-E	0 / 588	0.13 (2)
E-K	-1054 / 0	0.69 (1)
K-F	0 / 1008	0.16 (1)
F-J	-900 / 0	0.66 (1)
J-G	-542 / 0	0.31 (1)
J-H	0 / 1741	0.28 (1)
B-N	0 / 2359	0.53 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BOBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.99/1:00 (H-I:1) , BC=0.59/1.00 (M-N:2) , WB=0.69/1.00 (E-K:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (B) (INPUT = 0.90)
SI METAL= 0.75 (L) (INPUT = 1.00)

DWG NO. TAM 1901201
STRUCTURAL
ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	T402	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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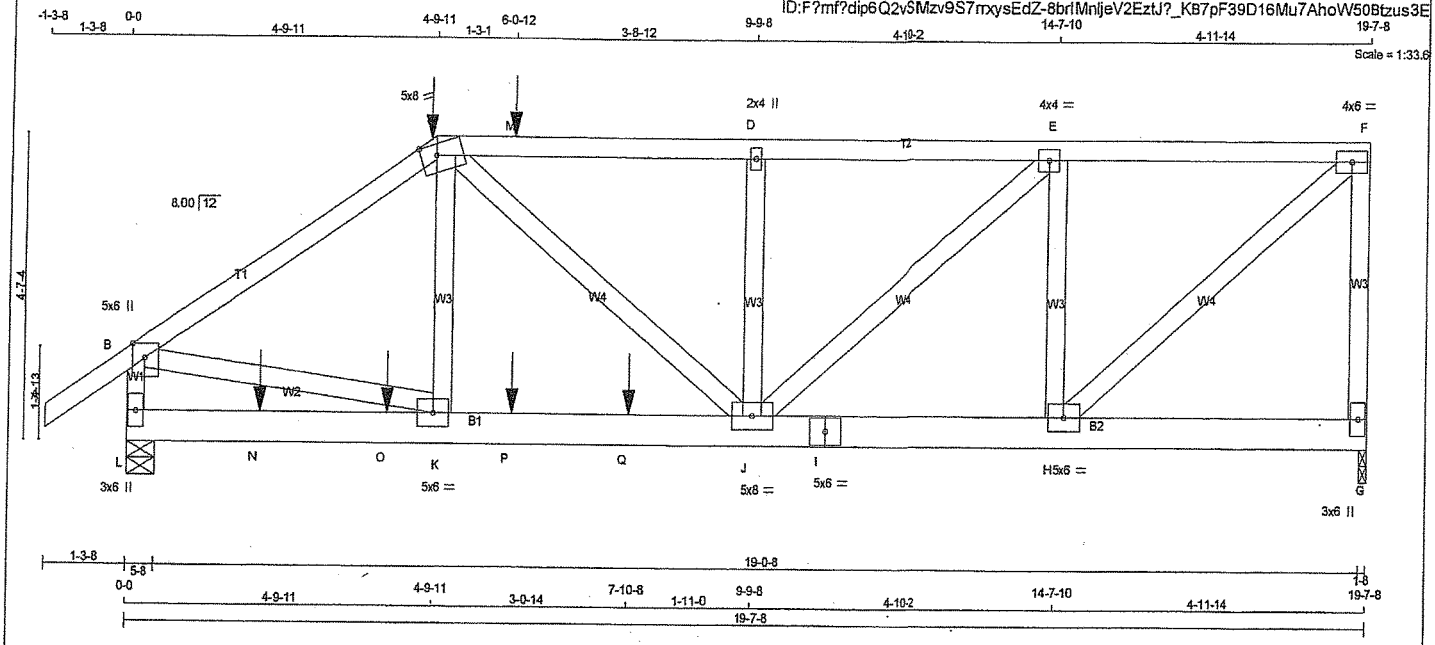


TOTAL WEIGHT = 2 X 105 = 209 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	ORY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x6	ORY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-G	1	12	TOP
L-B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L-I	2	12	SIDE(0.0)
I-G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTVW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REORD BRG
G	1905	0	1905	0	0	1-8	1-8
L	2652	0	2652	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SDIL
G	1437	740 / 0	303 / 0	0 / 0	0 / 0	393 / 0	0 / 0
L	1994	1046 / 0	407 / 0	0 / 0	0 / 0	541 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00
B-C	-3156 / 0	-84.9	-84.9	0.25 (1)	4.98
C-M	-3190 / 0	-84.9	-84.9	0.26 (1)	4.93
M-D	-3190 / 0	-84.9	-84.9	0.26 (1)	4.93
D-E	-3190 / 0	-84.9	-84.9	0.22 (1)	4.99
E-F	-1916 / 0	-84.9	-84.9	0.16 (1)	6.13
G-F	-1799 / 0	0.0	0.0	0.29 (1)	7.81
L-B	-2604 / 0	0.0	0.0	0.15 (1)	7.04
L-N	0 / 0	-38.5	-38.5	0.18 (2)	10.00
N-O	0 / 0	-38.5	-38.5	0.18 (2)	10.00
O-K	0 / 0	-38.6	-38.5	0.18 (2)	10.00
K-P	0 / 2636	-38.5	-38.5	0.58 (1)	10.00
P-Q	0 / 2636	-38.6	-38.5	0.58 (1)	10.00
Q-J	0 / 2636	-38.5	-38.5	0.58 (1)	10.00
J-I	0 / 1918	-38.5	-38.5	0.31 (1)	10.00
I-H	0 / 1916	-38.5	-38.5	0.31 (1)	10.00
H-G	0 / 0	-38.5	-38.5	0.06 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	0 / 503	0.04 (3)
C-J	0 / 725	0.06 (1)
J-D	-493 / 0	0.05 (1)
J-E	0 / 1690	0.15 (1)
H-E	-1569 / 0	0.17 (1)
H-F	0 / 2574	0.22 (1)
B-K	POV-2674	0.23 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	TOWN
C	4-9-11	-71	-80	—	FRONT	VERT	DEAD	—	—
C	4-9-11	-330	-330	—	FRONT	VERT	SNOW	—	—
M	6-0-12	-134	-134	—	FRONT	VERT	TOTAL	—	—
N	2-0-12	-70	-89	—	FRONT	VERT	TOTAL	—	—
O	4-0-12	-70	-89	—	FRONT	VERT	TOTAL	—	—
P	6-0-12	-70	-89	—	FRONT	VERT	TOTAL	—	—
Q	7-10-8	-1275	-1275	—	TOP	VERT	TOTAL	—	—

DWG NO. TAM1790234
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT ND FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.29/1.00 (F-G:1) , BC=0.58/1.00 (J-K:1) , WB=0.24/1.00 (B-K:1) , SSI=0.29/1.00 (J-K:1)

BOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

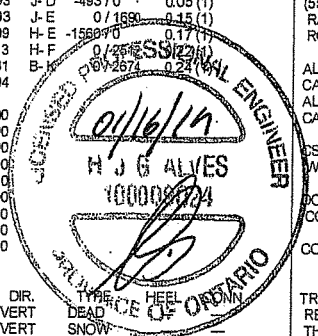
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	T403	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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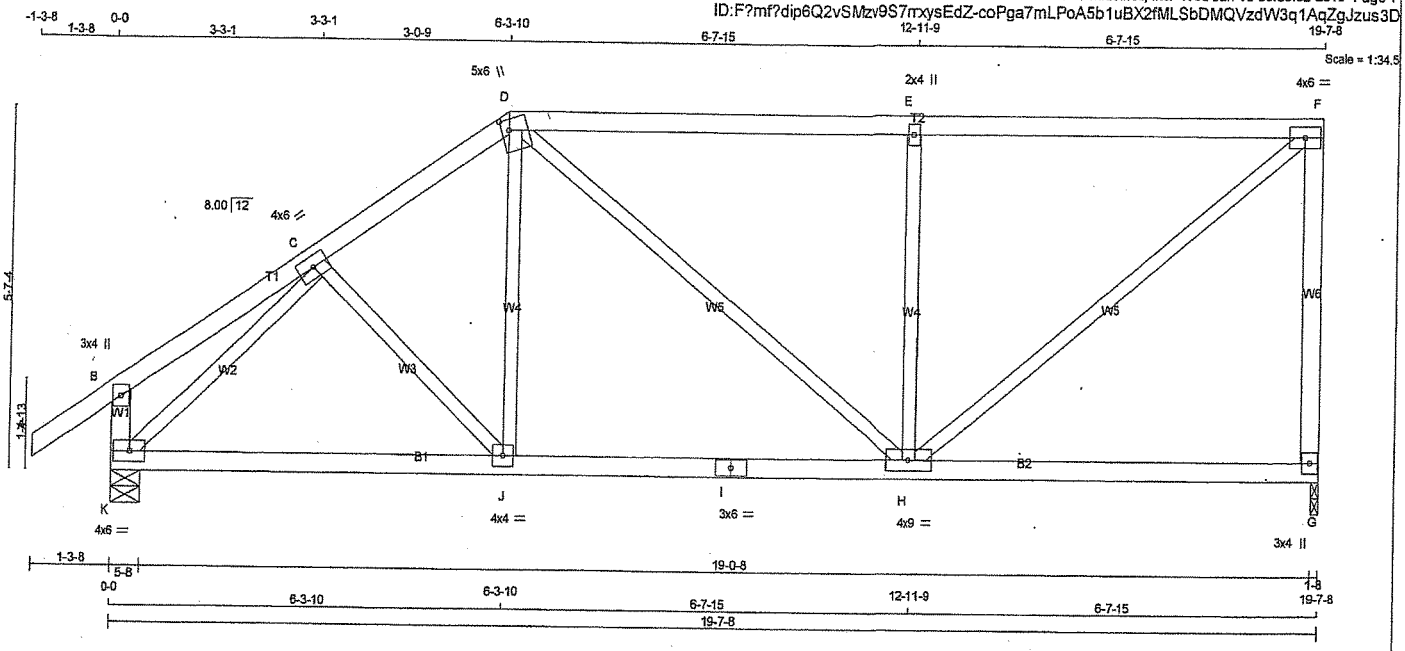


TOTAL WEIGHT = 83 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	ORY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	0 / 15	-84.9	-84.9	0.12 (1)	10.00
C- D	-1224 / 0	-84.9	-84.9	0.14 (1)	5.65
D- E	-1121 / 0	-84.9	-84.9	0.71 (1)	4.83
E- F	-1122 / 0	-84.9	-84.9	0.71 (1)	4.83
G- F	-1105 / 0	0.0	0.0	0.63 (1)	7.53
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 994	-38.5	-38.6	0.41 (2)	10.00
J- I	0 / 1009	-38.5	-38.5	0.43 (2)	10.00
I- H	0 / 1009	-38.5	-38.5	0.43 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.31 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	0 / 92	0.02 (3)
J- D	0 / 307	0.07 (2)
D- H	0 / 144	0.03 (1)
H- E	-700 / 0	0.33 (1)
H- F	0 / 1431	0.32 (1)
K- C	-1413 / 0	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LDAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED RODF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.71/1.00 (E-F:1) , BC=0.43/1.00 (H-J:2) , WB=0.45/1.00 (C-K:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

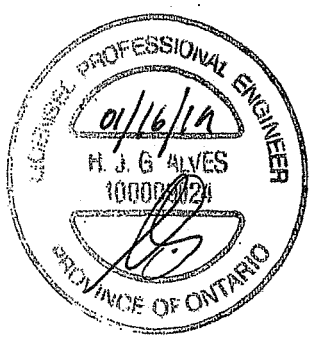
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.41 (I) (INPUT = 1.00)



DWG NO. TAM T1901235
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	T404	1	1	TRUSS OESC.	

Tamarack Roof Truss, Burlington

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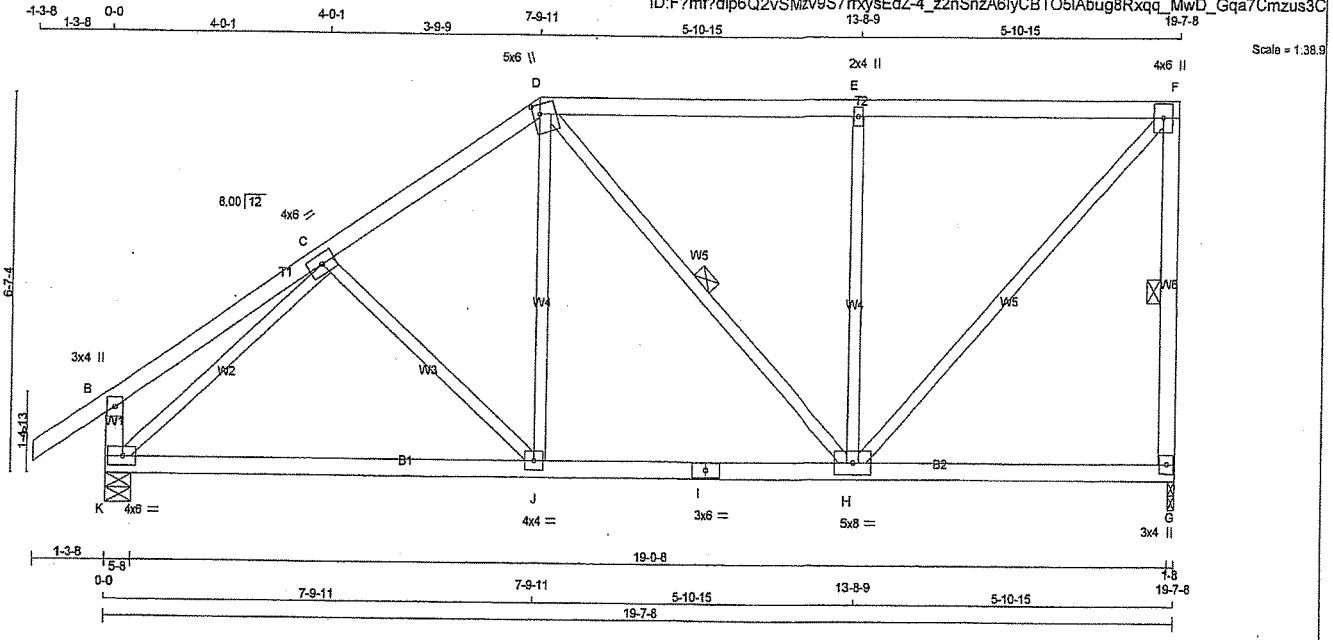


TOTAL WEIGHT = 86 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - O	2x4	ORY	No.2	SPF
D - F	2x4	ORY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	ORY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

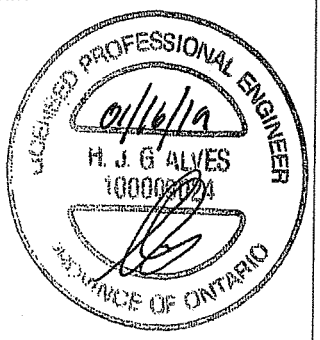
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	C- J	-109 / 51	0.05 (1)
B- C	0 / 20	-84.9	-84.9	0.20 (1)	10.00	J- D	0 / 422	0.09 (2)
C- D	-1143 / 0	-84.9	-84.9	0.22 (1)	5.68	D- H	-118 / 0	0.06 (2)
D- E	-868 / 0	-84.9	-84.9	0.53 (1)	5.73	H- E	-621 / 0	0.45 (1)
E- F	-868 / 0	-84.9	-84.9	0.53 (1)	5.73	H- F	0 / 1266	0.28 (1)
G- F	-1113 / 0	0.0	0.0	0.22 (1)	6.01	K- C	-1393 / 0	0.65 (1)
K- B	-245 / 0	0.0	0.0	0.03 (1)	7.81			
K- J	0 / 1013	-38.5	-38.5	0.51 (2)	10.00			
J- I	0 / 939	-38.5	-38.5	0.50 (2)	10.00			
I- H	0 / 939	-38.5	-38.5	0.50 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.22 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 892 (0.26")

CSI. TC=0.53/1.00 (E-F:1) , BC=0.51/1.00 (J-K:2) , WB=0.65/1.00 (C-K:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BENO=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.39 (F) (INPUT = 1.00)

DWG NO. TAM 19901236
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	T405	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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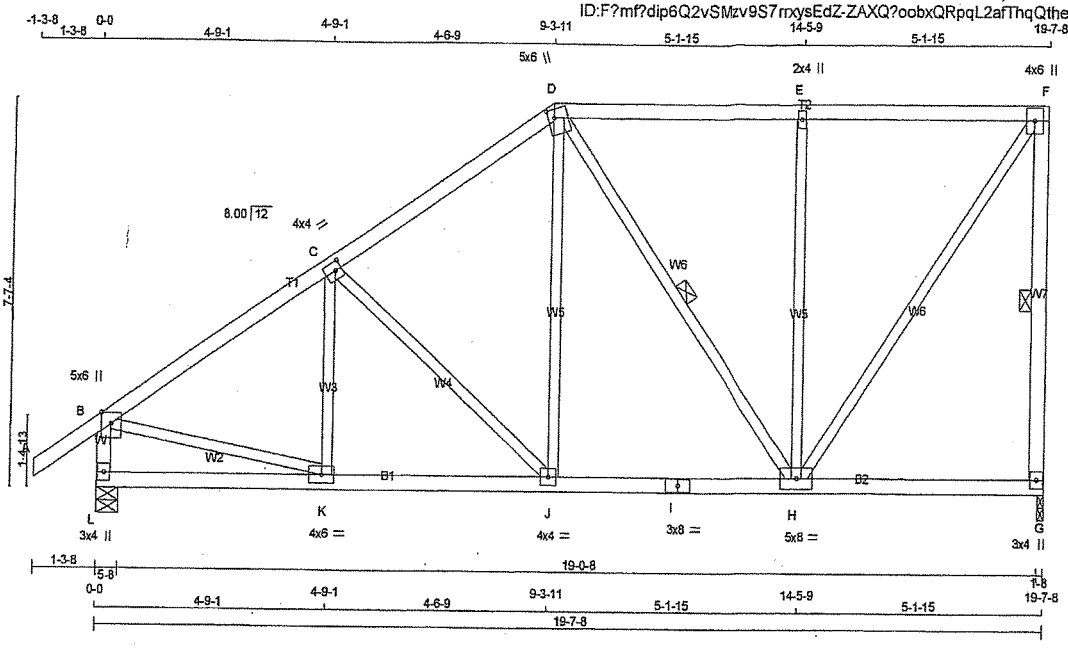


TOTAL WEIGHT = 95 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT2D	4.0	6.0		
G	BMV1+p	MT2D	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT2D	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1210	0	1210	0	0	1-8	1-8
L	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
L	999	521 / 0	208 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

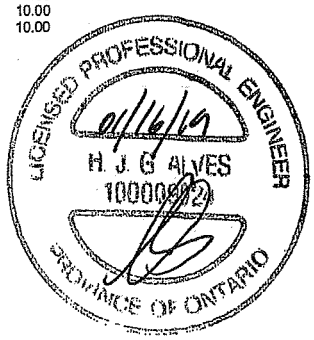
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	
B- C	-1291 / 0	-84.9	-84.9	0.35 (1)	5.26	
C- D	-1008 / 0	-84.9	-84.9	0.33 (1)	5.79	
D- E	-684 / 0	-84.9	-84.9	0.39 (1)	8.25	
E- F	-684 / 0	-84.9	-84.9	0.39 (1)	6.25	
G- F	-1130 / 0	0.0	0.0	0.29 (1)	5.98	
L- B	-1252 / 0	0.0	0.0	0.13 (1)	7.19	
L- K	0 / 0	-38.5	-38.5	0.15 (3)	10.00	
K- J	0 / 1095	-38.5	-38.5	0.30 (2)	10.00	
J- I	0 / 821	-38.5	-38.5	0.30 (2)	10.00	
I- H	0 / 821	-38.5	-38.5	0.30 (2)	10.00	
H- G	0 / 0	-38.5	-38.5	0.20 (3)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-48 / 131	0.03 (3)
C- J	-379 / 0	0.27 (1)
J- D	0 / 446	0.10 (2)
D- H	-240 / 0	0.13 (1)
H- E	-541 / 0	0.57 (1)
H- F	0 / 1179	0.27 (1)
B- K	0 / 1123	0.25 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.39/1.00 (E-F:1) , BC=0.30/1.00 (H-J:2) , WB=0.57/1.00 (E-H:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM 7790/237
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	T406	1	1	TRUSS OESC.	

Tamarack Roof Truss, Burlington

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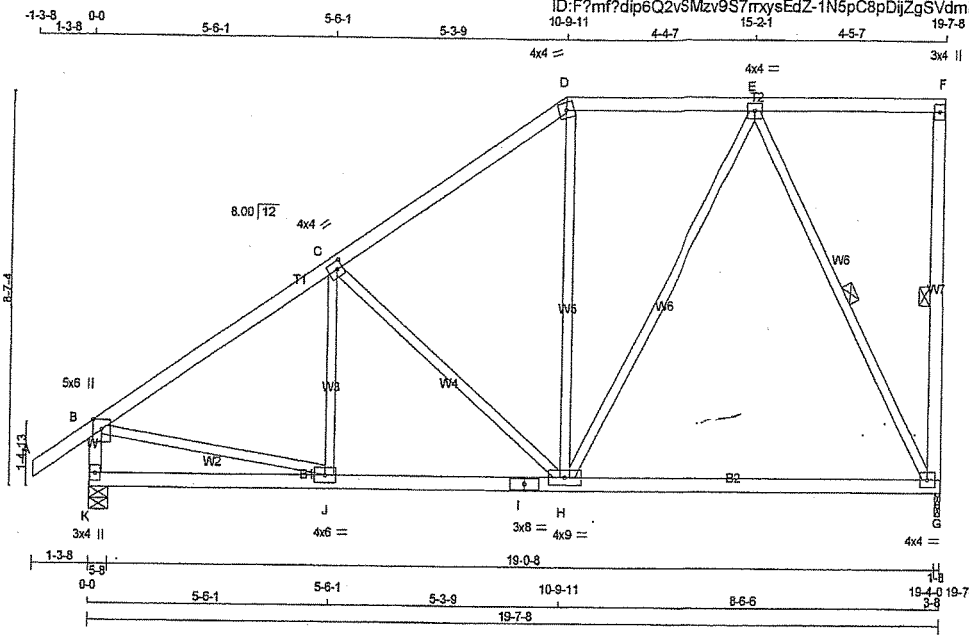


TOTAL WEIGHT = 94 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
G - F	2x4	ORY	No.2	SPF
K - B	2x4	ORY	No.2	SPF
K - I	2x4	ORY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
O	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

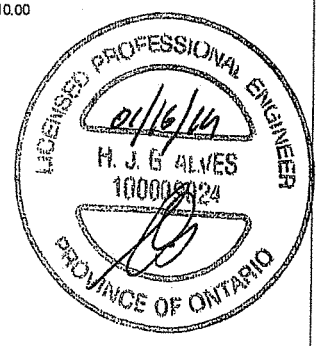
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1275 / 0	-84.9	-84.9	0.35 (1)	5.31
C- D	-915 / 0	-84.9	-84.9	0.33 (1)	6.03
D- E	-736 / 0	-84.9	-84.9	0.22 (1)	6.25
E- F	0 / 0	-84.9	-84.9	0.27 (1)	10.00
G- F	-144 / 0	0.0	0.0	0.05 (1)	6.25
K- B	-1233 / 0	0.0	0.0	0.13 (1)	7.23
K- J	0 / 0	-38.5	-38.5	0.20 (3)	10.00
J- I	0 / 1086	-38.5	-38.5	0.61 (2)	10.00
I- H	0 / 1086	-38.5	-38.5	0.61 (2)	10.00
H- G	0 / 497	-38.5	-38.5	0.52 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-27 / 131	0.03 (3)
C- H	-468 / 0	0.47 (1)
H- D	0 / 222	0.05 (3)
B- J	0 / 1107	0.25 (1)
H- E	0 / 529	0.12 (2)
E- G	-1054 / 0	0.65 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	OL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 634 (0.37")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.61/1.00 (H-J:2) , WB=0.65/1.00 (E-G:1) , SSI=0.21/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.78 (I) (INPUT = 1.00)

DWG NO. TAM T1901238
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	T407	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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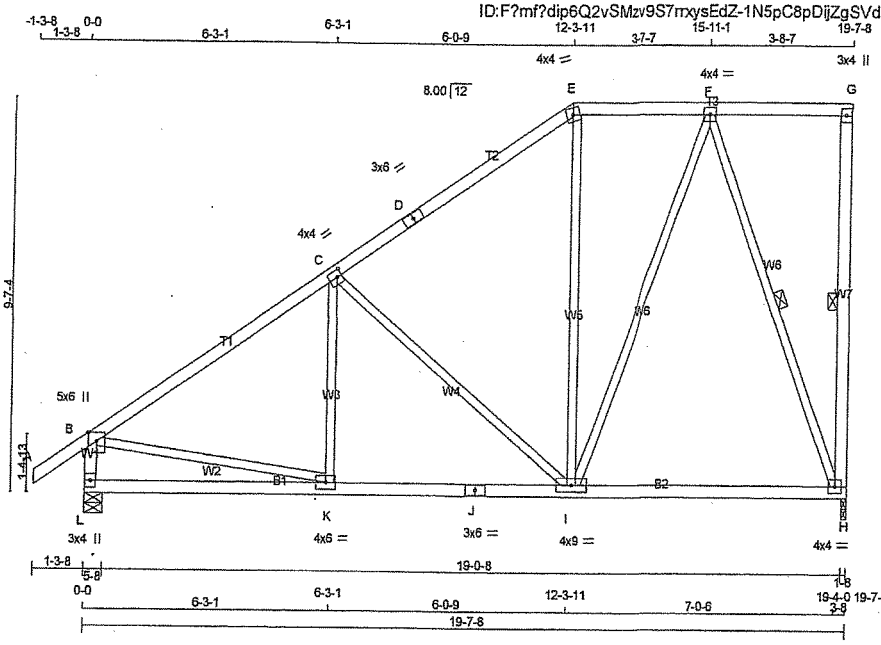


TOTAL WEIGHT = 99 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1210	0	1210	0	0	1-8	1-8
L	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
L	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, F-H.

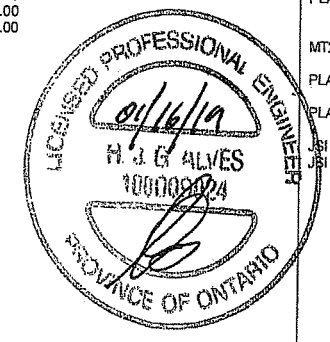
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAO LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1265 / 0	-84.9	-84.9	0.46 (1)	5.15
C- D	-772 / 0	-84.9	-84.9	0.43 (1)	6.24
D- E	-772 / 0	-84.9	-84.9	0.43 (1)	6.24
E- F	-612 / 0	-84.9	-84.9	0.15 (1)	6.25
F- G	0 / 0	-84.9	-84.9	0.19 (1)	10.00
H- G	-120 / 0	0.0	0.0	0.05 (1)	6.25
L- B	-1227 / 0	0.0	0.0	0.13 (1)	7.24
L- K	0 / 0	-38.5	-38.5	0.24 (3)	10.00
K- J	0 / 1082	-38.5	-38.5	0.48 (2)	10.00
J- I	0 / 1082	-38.5	-38.5	0.48 (2)	10.00
I- H	0 / 388	-38.5	-38.5	0.37 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 225	0.05 (3)
C- I	-618 / 0	0.85 (1)
I- E	0 / 165	0.04 (3)
B- K	0 / 1098	0.25 (1)
I- F	0 / 635	0.14 (1)
F- H	-1051 / 0	0.76 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.48/1.00 (I-K:2) , WB=0.85/1.00 (C-I:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.85 (I) (INPUT = 0.90)
SSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1901239
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	T408	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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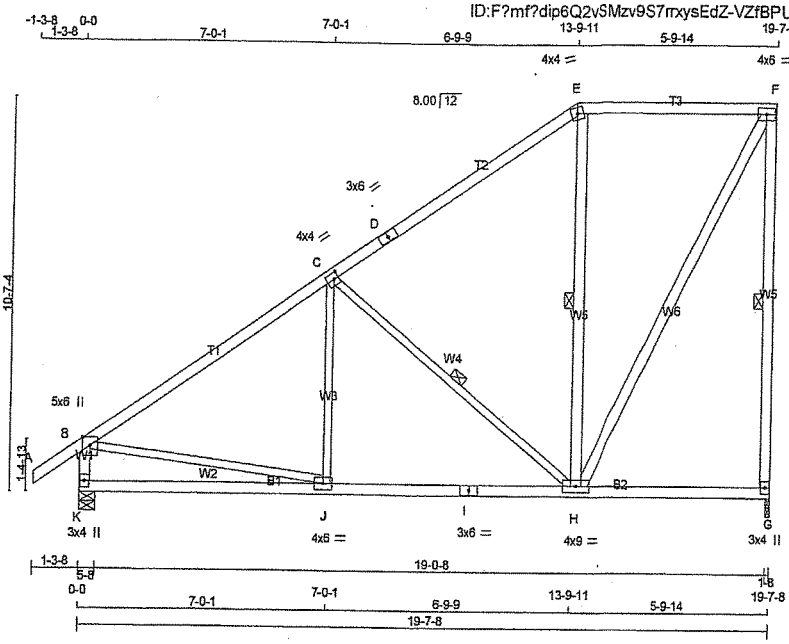


TOTAL WEIGHT = 104 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	91B	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

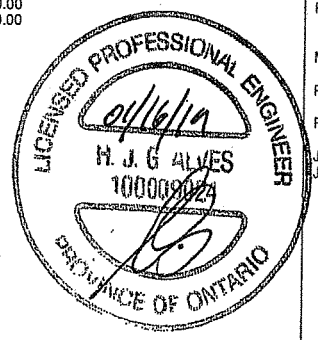
TOTAL LOAD CASES. (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1243 / 0	-84.9	-84.9	0.60 (1)	4.96
C-D	-625 / 0	-84.9	-84.9	0.54 (1)	6.25
D-E	-625 / 0	-84.9	-84.9	0.54 (1)	6.25
E-F	-484 / 0	-84.9	-84.9	0.37 (1)	6.25
G-F	-1126 / 0	0.0	0.0	0.64 (1)	5.98
K-B	-1219 / 0	0.0	0.0	0.13 (1)	7.26
K-J	0 / 0	-38.5	-38.5	0.35 (3)	10.00
J-I	0 / 1067	-38.5	-38.5	0.48 (2)	10.00
I-H	0 / 1067	-38.5	-38.5	0.48 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.26 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	0 / 302	0.07 (3)
C-H	-755 / 0	0.42 (1)
H-E	-138 / 92	0.08 (1)
H-F	0 / 1005	0.16 (1)
B-J	0 / 1080	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIO 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.64/1.00 (F-G:1) , BC=0.48/1.00 (H-J:2) , WB=0.42/1.00 (C-H:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES.

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	61B	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.82 (B) (INPUT = 0.90)
USI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1901240
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	T409	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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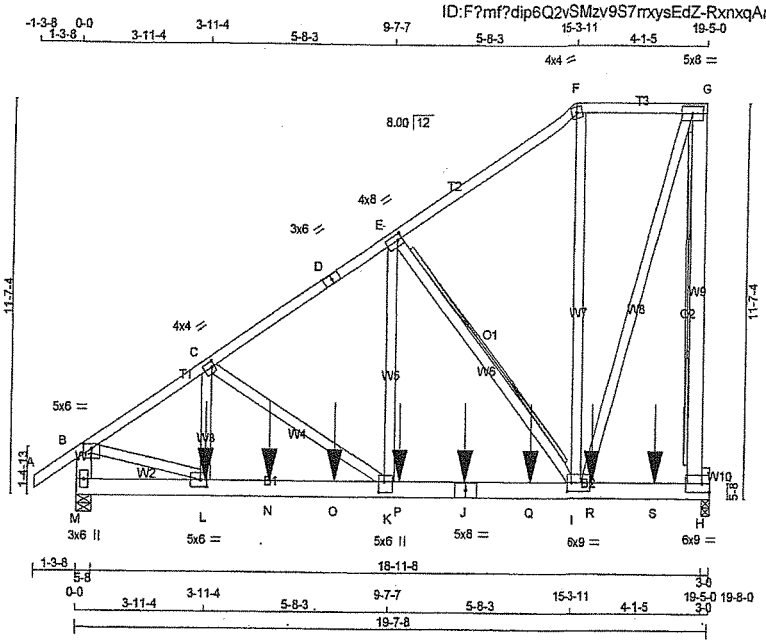

TOTAL WEIGHT = 4 X 143 = 572 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- G	1	12	TOP
G- H	2	12	TOP
M- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	5	SIDE(307.5)
J- H	2	5	SIDE(615.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	6.0	2.00	1.75
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	8.0		
H	BMVW1-t	MT20	6.0	9.0		Edge
I	BMWWW-t	MT20	6.0	9.0	3.00	2.00
J	BS-t	MT20	5.0	8.0		
K	BMWW+t	MT20	5.0	6.0	3.75	2.00
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	9345	0	9345	0	0	3-0	3-0
M	8177	0	8177	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	7062	3595 / 0	1525 / 0	0 / 0	0 / 0	1943 / 0	0 / 0
M	8173	3162 / 0	1317 / 0	0 / 0	0 / 0	1693 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT G-H, E-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.9	-84.9	0.04 (1)	10.00	L- C	0 / 2156	0.09 (1)
B- C	-10516 / 0	-84.9	-84.9	0.23 (1)	3.84	C- K	-2323 / 0	0.34 (1)
C- D	-8156 / 0	-84.9	-84.9	0.39 (1)	4.22	K- E	0 / 6970	0.28 (1)
D- E	-8156 / 0	-84.9	-84.9	0.39 (1)	4.22	E- I	-6297 / 0	0.34 (1)
E- F	-3528 / 0	-84.9	-84.9	0.20 (1)	5.94	I- F	0 / 1571	0.06 (1)
F- G	-2929 / 0	-84.9	-84.9	0.10 (1)	6.25	B- L	0 / 9897	0.36 (1)
H- G	-8455 / 0	0.0	0.0	0.54 (1)	7.81	I- G	0 / 8793	0.35 (1)
M- B	-8317 / 0	0.0	0.0	0.15 (1)	6.74			
M- L	0 / 0	-38.5	-38.5	0.18 (1)	10.00			
L- N	0 / 8761	-38.5	-38.5	0.85 (1)	10.00			
N- O	0 / 8761	-38.5	-38.5	0.85 (1)	10.00			
O- K	0 / 8761	-38.5	-38.5	0.85 (1)	10.00			
K- P	0 / 6802	-38.5	-38.5	0.79 (1)	10.00			
P- J	0 / 6802	-38.5	-38.5	0.79 (1)	10.00			
J- Q	0 / 6802	-38.5	-38.5	0.79 (1)	10.00			
O- I	0 / 6802	-38.5	-38.5	0.79 (1)	10.00			
I- R	0 / 0	-38.5	-38.5	0.42 (1)	10.00			
R- S	0 / 0	-38.5	-38.5	0.42 (1)	10.00			
S- H	0 / 0	-38.5	-38.5	0.42 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	11-11-4	-1768	-1768	—	BACK	VERT	TOTAL		
L	3-11-4	-2633	-2633	—	TOP	VERT	TOTAL	—	—
N	5-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—
O	7-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—
P	9-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—
O	13-11-4	-1768	-1768	—	TOP	VERT	TOTAL	—	—
R	15-11-4	-1768	-1768	—	BACK	VERT	TOTAL	—	—
S	17-11-4	-1768	-1768	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.54/1.00 (G-H:1) , BC=0.85/1.00 (K-L:1) , WB=0.38/1.00 (B-L:1) , SSI=0.57/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.67 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100090924
PROVINCE OF ONTARIO

DWG NO. TAM T190124I
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	T410	1	2	TRUSS OESC.	

Tamarack Roof Truss, Burlington

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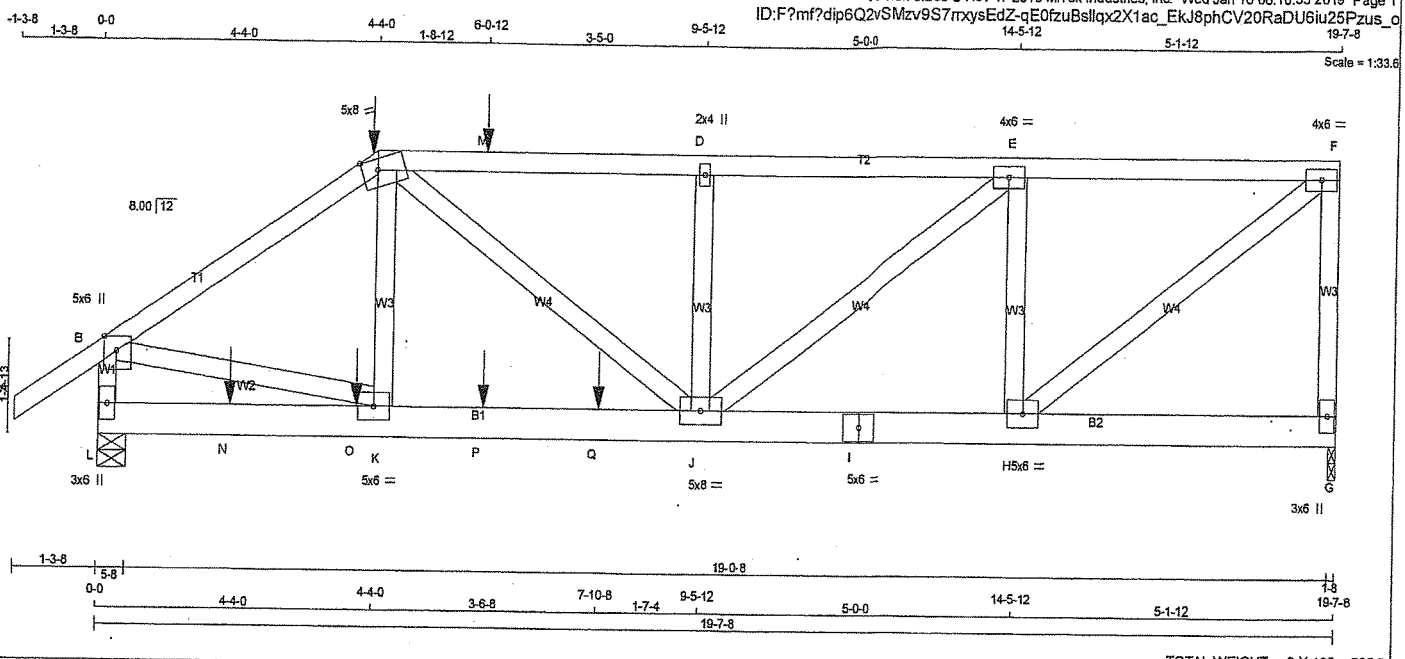


TOTAL WEIGHT = 2 X 102 = 205 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	2	12	SIDE(0.0)
I- G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	8.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EOGE OF CHORO.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1988	0	1988	0	0	1-8	1-8
L	2800	0	2800	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1499	773 / 0	315 / 0	0 / 0	0 / 0	410 / 0	0 / 0
L	2100	1115 / 0	417 / 0	0 / 0	0 / 0	589 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID-CEILING DIRECTLY APPLIED.

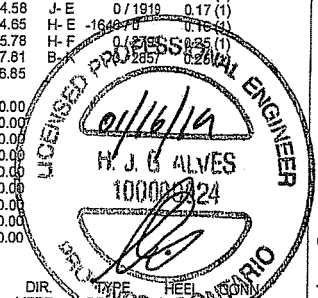
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.06 (1)	10.00	K- C	0 / 401	0.04 (3)
B- C	-3357 / 0	-84.9	-84.9	0.21 (1)	4.90	C- J	0 / 1177	0.10 (1)
C- M	-3737 / 0	-84.9	-84.9	0.31 (1)	4.58	J- D	-510 / 0	0.05 (1)
M- D	-3737 / 0	-84.9	-84.9	0.31 (1)	4.58	J- E	0 / 1919	0.17 (1)
D- E	-3737 / 0	-84.9	-84.9	0.25 (1)	4.65	H- E	-1640 / 0	0.16 (1)
E- F	-2227 / 0	-84.9	-84.9	0.18 (1)	5.78	H- F	0 / 2795	0.25 (1)
G- F	-1875 / 0	0.0	0.0	0.26 (1)	7.81	B- I	0 / 2857	0.26 (1)
L- B	-2789 / 0	0.0	0.0	0.16 (1)	6.85			
L- N	0 / 0	-38.5	-38.5	0.19 (1)	10.00			
N- O	0 / 0	-38.5	-38.5	0.19 (1)	10.00			
O- K	0 / 0	-38.5	-38.5	0.19 (1)	10.00			
K- P	0 / 2799	-38.5	-38.5	0.63 (1)	10.00			
P- Q	0 / 2799	-38.5	-38.5	0.63 (1)	10.00			
Q- J	0 / 2799	-38.5	-38.5	0.63 (1)	10.00			
J- I	0 / 2227	-38.5	-38.5	0.36 (1)	10.00			
I- H	0 / 2227	-38.5	-38.5	0.36 (1)	10.00			
H- G	0 / 0	-38.5	-38.5	0.06 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN
C	4-4-1	-58	-64	—	FRONT	VERT	DEAD	—	—
C	4-4-0	-120	-120	—	BACK	VERT	TOTAL	—	—
C	4-4-1	-266	-286	—	FRONT	VERT	SNOW	—	—
M	6-0-12	-114	-114	—	BACK	VERT	TOTAL	—	—
N	2-0-12	-61	-77	—	BACK	VERT	TOTAL	—	—
O	4-0-12	-61	-77	—	BACK	VERT	TOTAL	—	—
P	6-0-12	-61	-77	—	BACK	VERT	TOTAL	—	—
Q	7-10-8	-1513	-1513	—	TOP	VERT	TOTAL	—	—



DWG NO. TAM T1901242
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NDN STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED RD OF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE OEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.31/1.00 (C-D:1) , BC=0.63/1.00 (J-K:1) , WB=0.25/1.00 (B-K:1) , SSI=0.38/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTDR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Oeg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.65 (B) (INPUT = 1.00)

JDB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	T411	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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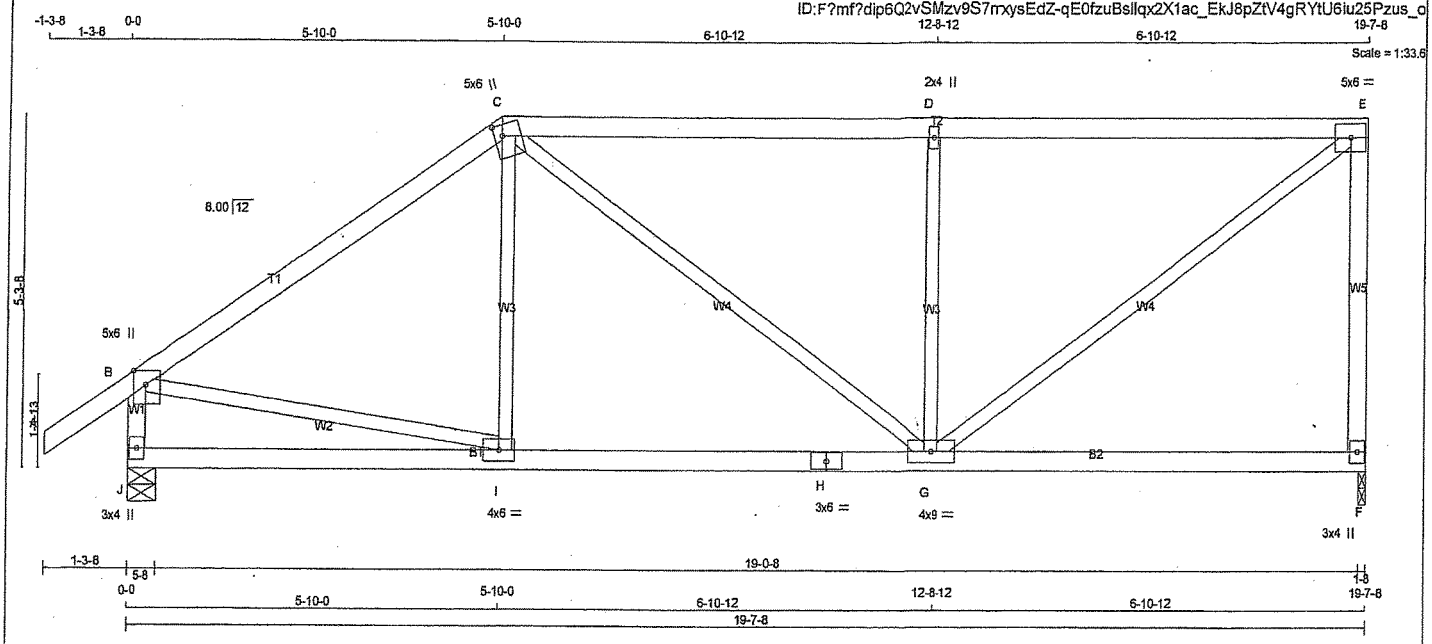


TOTAL WEIGHT = 80 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1210	0	1210	0	0	1-8	1-8
J	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
J	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

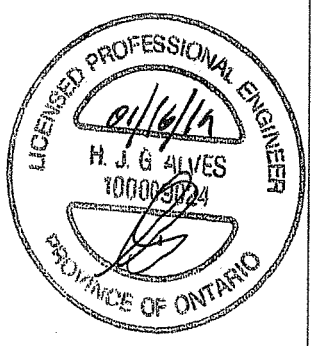
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00	I-C	0 / 233	0.06 (3)
B-C	-1233 / 0	-84.9	-84.9	0.59 (1)	4.97	C-G	0 / 238	0.05 (1)
C-D	-1219 / 0	-84.9	-84.9	0.78 (1)	4.53	G-D	-724 / 0	0.30 (1)
D-E	-1219 / 0	-84.9	-84.9	0.78 (1)	4.53	G-E	0 / 1502	0.34 (1)
F-E	-1102 / 0	0.0	0.0	0.54 (1)	7.54	B-I	0 / 1042	0.23 (1)
J-B	-1236 / 0	0.0	0.0	0.13 (1)	7.22			
J-I	0 / 0	-38.5	-38.5	0.26 (3)	10.00			
I-H	0 / 1027	-38.5	-38.5	0.46 (2)	10.00			
H-G	0 / 1027	-38.5	-38.5	0.46 (2)	10.00			
G-F	0 / 0	-38.5	-38.5	0.34 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.78/1.00 (D-E:1) , BC=0.46/1.00 (G-I:2) , WB=0.34/1.00 (E-G:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM T190243
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	T412	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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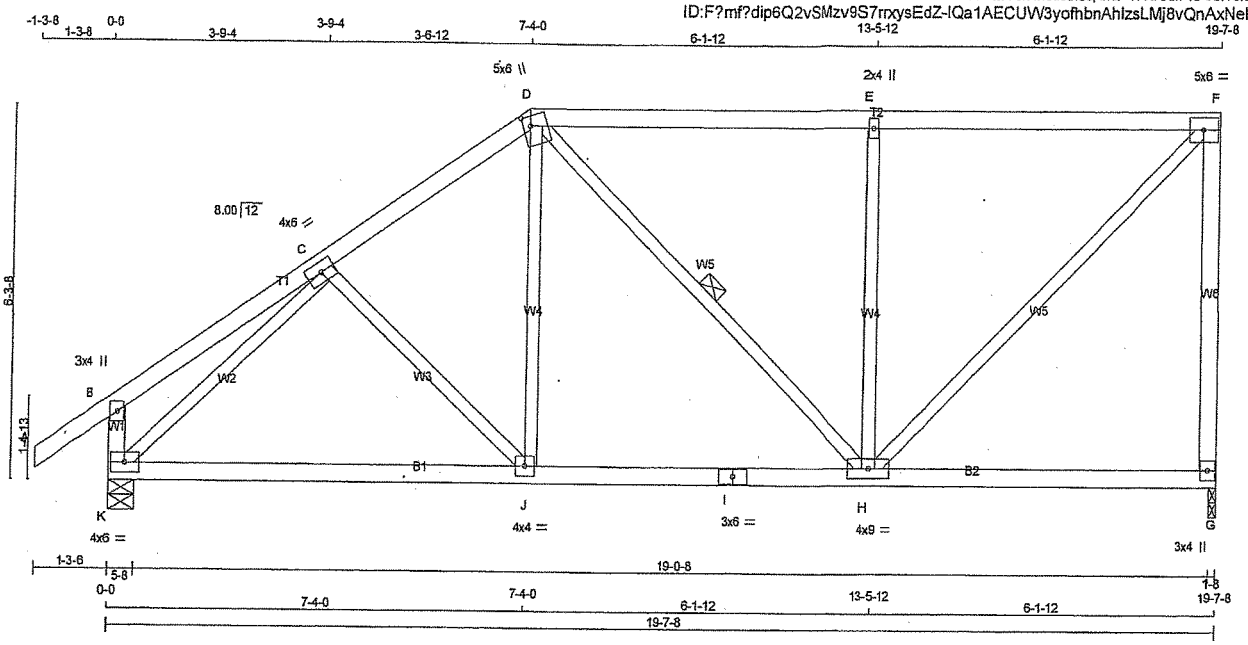


TOTAL WEIGHT = 86 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT2D	3.0	4.0		
C	TMVVV-t	MT20	4.0	6.0		
D	TTVVV+m	MT20	5.0	6.0	2.00	1.50
E	TMV+w	MT20	2.0	4.0		
F	TMVV-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVVVV-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVVV-t	MT20	4.0	4.0		
K	BMVV1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

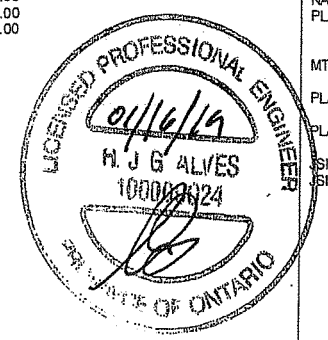
1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	1	0.11 (1)	10.00
B-C	0 / 19	-84.9	-84.9	1	0.17 (1)	10.00
C-D	-1171 / 0	-84.9	-84.9	1	0.19 (1)	5.66
D-E	-939 / 0	-84.9	-84.9	1	0.58 (1)	5.46
E-F	-940 / 0	-84.9	-84.9	1	0.58 (1)	5.46
G-F	-1111 / 0	0.0	0.0	1	0.87 (1)	7.51
K-B	-238 / 0	0.0	0.0	1	0.02 (1)	7.81
K-J	0 / 1010	-38.5	-38.5	2	0.47 (2)	10.00
J-I	0 / 963	-38.5	-38.5	2	0.47 (2)	10.00
I-H	0 / 963	-38.5	-38.5	2	0.47 (2)	10.00
H-G	0 / 0	-38.5	-38.5	3	0.23 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-72 / 63	0.03 (1)
J-D	0 / 386	0.09 (2)
D-H	-62 / 0	0.03 (3)
H-E	-645 / 0	0.41 (1)
H-F	0 / 1309	0.29 (1)
K-C	-1401 / 0	0.58 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.87/1.00 (F-G:1) , BC=0.47/1.00 (J-K:2) , WB=0.58/1.00 (C-K:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (H) (INPUT = 0.90)
SI METAL= 0.34 (C) (INPUT = 1.00)

DWG NO. TAM T1901244
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	T413	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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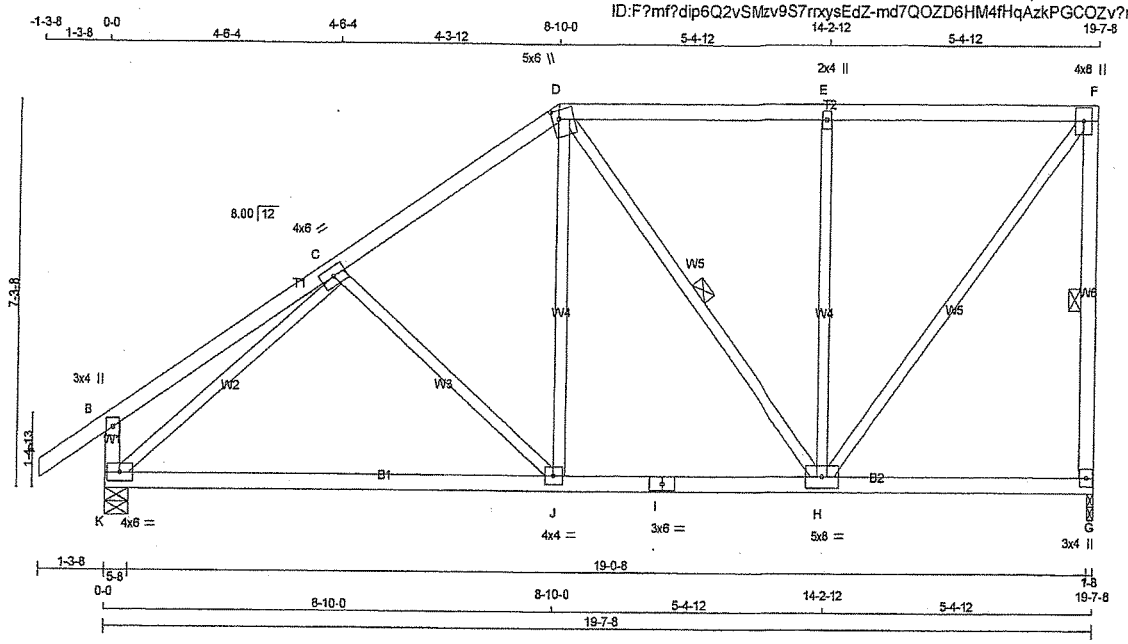


TOTAL WEIGHT = 91 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

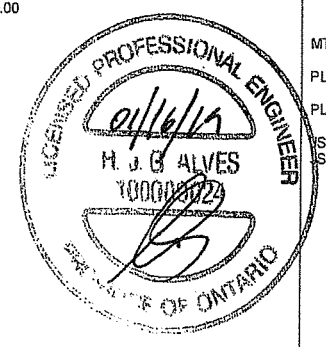
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	0 / 24	-84.9	-84.9	0.26 (1)	10.00
C-D	-1078 / 0	-84.9	-84.9	0.29 (1)	5.71
D-E	-731 / 0	-84.9	-84.9	0.43 (1)	6.25
E-F	-731 / 0	-84.9	-84.9	0.43 (1)	6.25
G-F	-1117 / 0	0.0	0.0	0.27 (1)	6.00
K-B	-260 / 0	0.0	0.0	0.03 (1)	7.81
K-J	0 / 1014	-38.5	-38.5	0.61 (2)	10.00
J-I	0 / 883	-38.5	-38.5	0.60 (2)	10.00
I-H	0 / 883	-38.5	-38.5	0.60 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.20 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-185 / 28	0.12 (1)
J-D	0 / 506	0.11 (2)
D-H	-255 / 0	0.13 (2)
H-E	-566 / 0	0.53 (1)
H-F	0 / 1192	0.27 (1)
K-C	-1373 / 0	0.83 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 943 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 564 (0.42")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.61/1.00 (J-K:2) , WB=0.83/1.00 (C-K:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.74 (K) (INPUT = 0.90)
SI METAL= 0.45 (I) (INPUT = 1.00)

DWG NO. TAM T1901245
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	T414	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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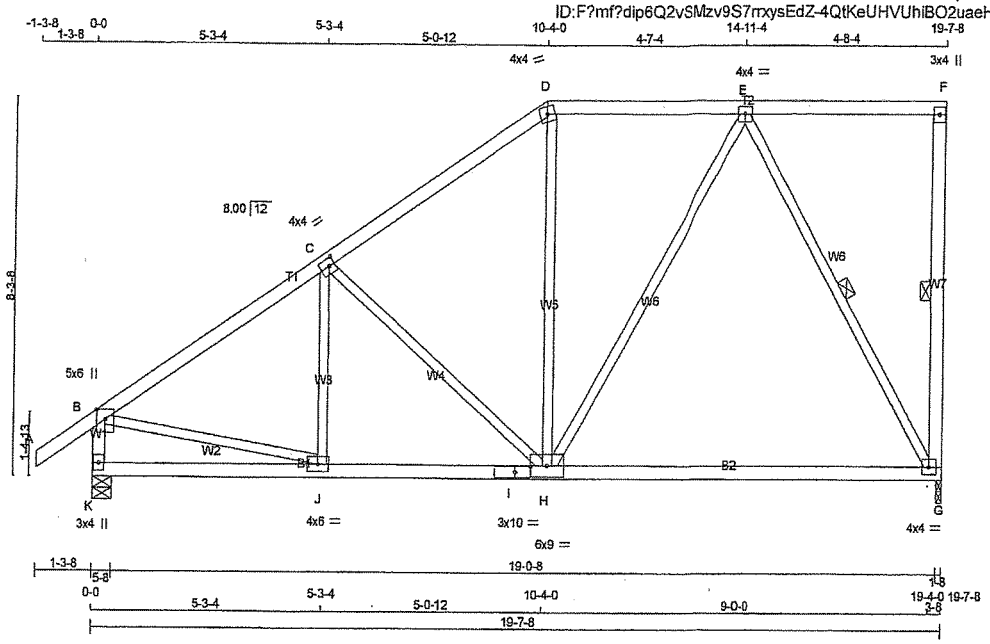


TOTAL WEIGHT = 93 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	6.0	9.0		
I	BS-t	MT20	3.0	10.0	1.50	4.25
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION				INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT		IN-SX	IN-SX
G	1210	0	1210	0	0		1-8	1-8
K	1327	0	1327	0	0		5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

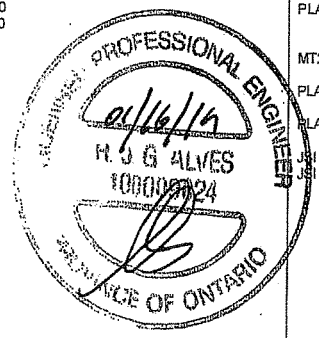
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1275 / 0	-84.9	-84.9	0.32 (1)	5.35
C- D	-960 / 0	-84.9	-84.9	0.30 (1)	5.97
D- E	-775 / 0	-84.9	-84.9	0.24 (1)	8.25
E- F	0 / 0	-84.9	-84.9	0.30 (1)	10.00
G- F	-151 / 0	0.0	0.0	0.05 (1)	6.25
K- B	-1234 / 0	0.0	0.0	0.13 (1)	7.23
K- J	0 / 0	-38.5	-38.5	0.20 (3)	10.00
J- I	0 / 1085	-38.5	-38.5	0.66 (2)	10.00
I- H	0 / 1085	-38.5	-38.5	0.66 (2)	10.00
H- G	0 / 538	-38.5	-38.5	0.58 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-60 / 96	0.02 (3)
C- H	-418 / 0	0.38 (1)
H- D	0 / 246	0.06 (2)
B- J	0 / 1108	0.25 (1)
H- E	0 / 502	0.11 (2)
E- G	-1059 / 0	0.63 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 870 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 521 (0.45")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.66/1.00 (H-J:2) , WB=0.63/1.00 (E-G:1) , SSI=0.22/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.94 (I) (INPUT = 1.00)

DWG NO. TAM T90 1246
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	T415	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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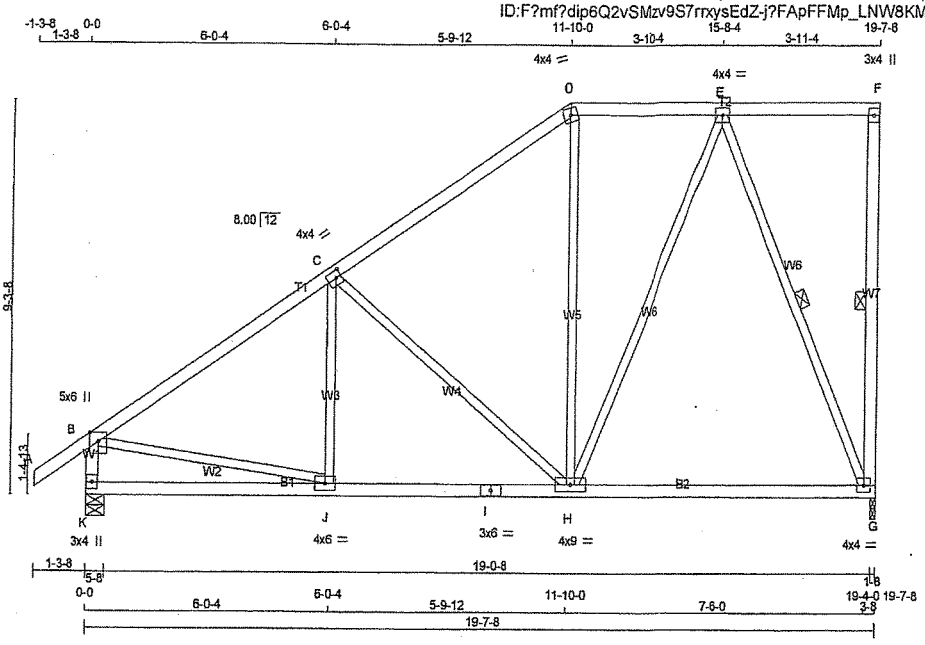


TOTAL WEIGHT = 98 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT2D	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT2D	3.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

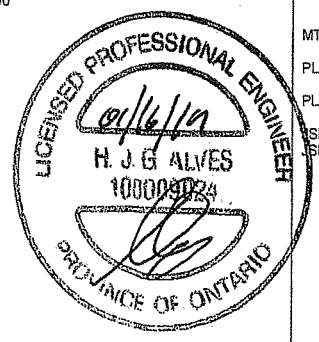
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1269 / 0	-84.9	-84.9	0.42 (1)	5.21
C-D	-816 / 0	-84.9	-84.9	0.39 (1)	6.18
D-E	-651 / 0	-84.9	-84.9	0.17 (1)	6.25
E-F	0 / 0	-84.9	-84.9	0.21 (1)	10.00
G-F	-128 / 0	0.0	0.0	0.05 (1)	6.25
K-B	-1229 / 0	0.0	0.0	0.13 (1)	7.23
K-J	0 / 0	-38.5	-38.5	0.22 (3)	10.00
J-I	0 / 1084	-38.5	-38.5	0.51 (2)	10.00
I-H	0 / 1084	-38.5	-38.5	0.51 (2)	10.00
H-G	0 / 421	-38.5	-38.5	0.42 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	0 / 198	0.05 (3)
C-H	-573 / 0	0.72 (1)
H-D	0 / 180	0.04 (3)
B-J	0 / 1101	0.25 (1)
H-E	0 / 600	0.14 (1)
E-G	-1050 / 0	0.72 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.51/1.00 (H-J:2) , W8=0.72/1.00 (E-G:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.82 (B) (INPUT = 0.90)
SSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T190 1247
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	T416	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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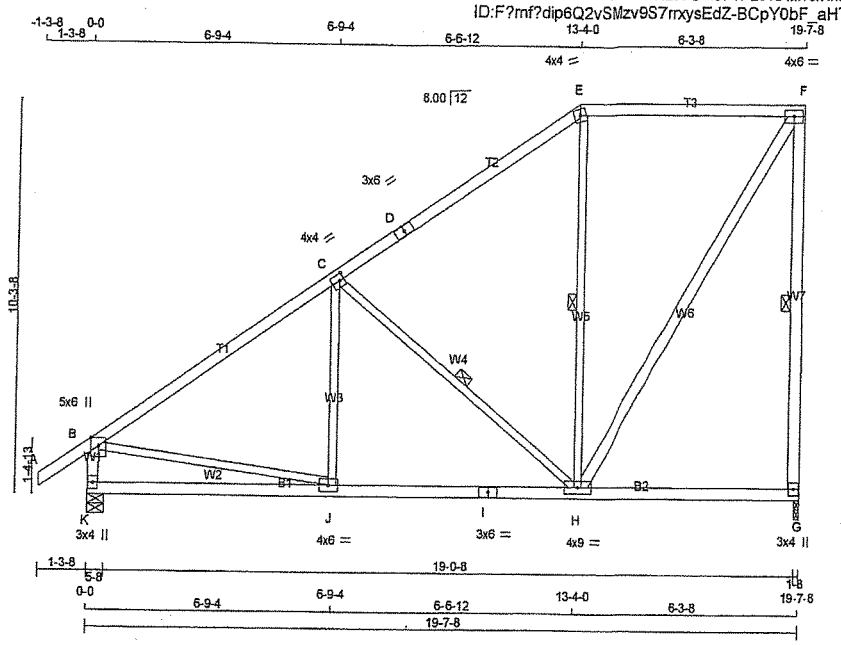


TOTAL WEIGHT = 99 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	ORY	No.2	SPF
E - F	2x4	ORY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	ORY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
ALL WEBS EXCEPT	2x3	ORY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
O	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1210	0	1210	0	0	1-8	1-8
K	1327	0	1327	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	918	457 / 0	206 / 0	0 / 0	0 / 0	255 / 0	0 / 0
K	999	521 / 0	206 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

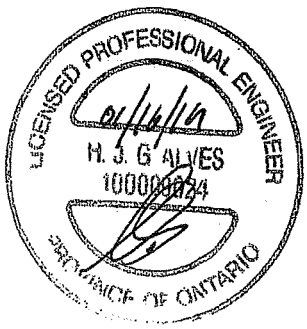
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B- C	-1251 / 0	-84.9	-84.9	0.55 (1)	5.02
C- D	-673 / 0	-84.9	-84.9	0.50 (1)	6.25
D- E	-673 / 0	-84.9	-84.9	0.50 (1)	6.25
E- F	-526 / 0	-84.9	-84.9	0.43 (1)	6.25
G- F	-1116 / 0	0.0	0.0	0.58 (1)	6.01
K- B	-1221 / 0	0.0	0.0	0.13 (1)	7.25
K- J	0 / 0	-38.5	-38.5	0.32 (3)	10.00
J- I	0 / 1073	-38.5	-38.5	0.45 (2)	10.00
I- H	0 / 1073	-38.5	-38.5	0.45 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.28 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
J- C	0 / 278	0.07 (3)
C- H	-712 / 0	0.37 (1)
H- E	-123 / 105	0.09 (1)
H- F	0 / 1000	0.16 (1)
B- J	0 / 1086	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.58/1.00 (F-G:1) , BC=0.45/1.00 (H-J:2) , WB=0.37/1.00 (C-H:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1901248
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	T417	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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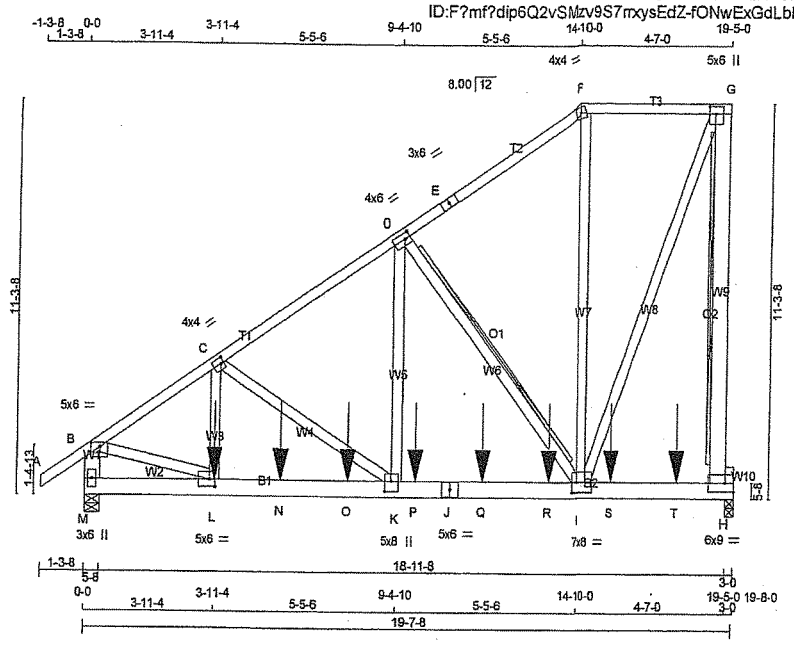


TOTAL WEIGHT = 4 X 141 = 565 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN).	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-E	1	12	TOP
E-F	1	12	TOP
F-G	1	12	TOP
G-H	2	12	TOP
M-B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-J	2	12	TOP
J-H	2	5	SIDE(828.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TMWW-t	MT20	4.0	6.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW+p	MT20	5.0	6.0	2.50	2.25
H	BMVW1-t	MT20	6.0	9.0		Edge
I	BMWWW-t	MT20	7.0	8.0	3.50	2.00
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	5.0	6.0	3.50	2.50
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	9513	0	9513	0	0	3-0	3-0
M	8309	0	8309	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	7189	3659 / 0	1553 / 0	0 / 0	0 / 0	1978 / 0	0 / 0
M	6273	3214 / 0	1339 / 0	0 / 0	0 / 0	1720 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT G-H, D-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAO LENGTH
A-B	0 / 32	-84.9	-84.9	0.04 (1)	10.00
B-C	-10657 / 0	-84.9	-84.9	0.24 (1)	3.81
C-D	-8423 / 0	-84.9	-84.9	0.37 (1)	4.17
D-E	-3993 / 0	-84.9	-84.9	0.19 (1)	5.68
E-F	-3993 / 0	-84.9	-84.9	0.19 (1)	5.68
F-G	-3320 / 0	-84.9	-84.9	0.13 (1)	6.12
H-G	-8356 / 0	0.0	0.0	0.50 (1)	7.81
M-B	-8427 / 0	0.0	0.0	0.15 (1)	6.71
M-L	0 / 0	-38.5	-38.5	0.16 (1)	10.00
L-N	0 / 8877	-38.5	-38.5	0.83 (1)	10.00
N-O	0 / 8877	-38.5	-38.5	0.83 (1)	10.00
O-K	0 / 8877	-38.5	-38.5	0.83 (1)	10.00
K-P	0 / 7024	-38.5	-38.5	0.77 (1)	10.00
P-J	0 / 7024	-38.5	-38.5	0.77 (1)	10.00
J-Q	0 / 7024	-38.5	-38.5	0.77 (1)	10.00
Q-R	0 / 7024	-38.5	-38.5	0.77 (1)	10.00
R-I	0 / 7024	-38.5	-38.5	0.77 (1)	10.00
I-S	0 / 0	-38.5	-38.5	0.48 (1)	10.00
S-T	0 / 0	-38.5	-38.5	0.48 (1)	10.00
T-H	0 / 0	-38.5	-38.5	0.48 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	0 / 2081	0.08 (1)
C-K	-2225 / 0	0.30 (1)
K-D	0 / 8765	0.27 (1)
D-I	-6104 / 0	0.31 (1)
I-F	0 / 1819	0.07 (1)
I-G	0 / 8821	0.38 (1)
B-L	0 / 8877	0.37 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	3-11-4	-2663	-2663	—	TOP	VERT	TOTAL	—	—
N	5-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—
O	7-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—
P	9-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—
Q	11-11-4	-1807	-1807	—	FRONT	VERT	TOTAL	—	—
R	13-11-4	-1807	-1807	—	TOP	VERT	TOTAL	—	—
S	15-11-4	-1807	-1807	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TDTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.50/1.00 (G-H:1) , BC=0.83/1.00 (K-L:1) , WB=0.37/1.00 (B-L:1) , SSI=0.52/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTDR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

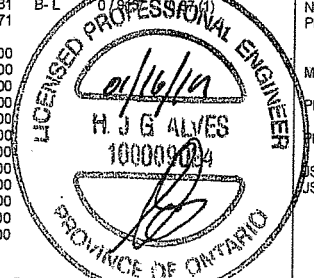
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.89 (I) (INPUT = 0.90)
USI METAL= 0.55 (L) (INPUT = 1.00)



DWG NO. TAM T1901249
STRUCTURAL COMPONENT ONLY

1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	T417	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	17-11-4	-1807	-1807	—	FRONT	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100092024
PROVINCE OF ONTARIO

DWG NO. TAM T190129
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	T418	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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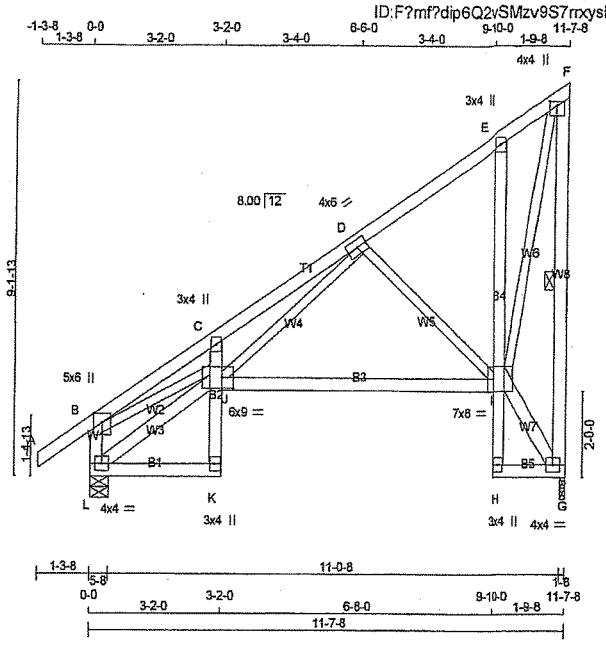


TOTAL WEIGHT = 3 X 76 = 227 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	ORY	No.2	SPF
L - J	2x4	ORY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMVW1-t	MT20	4.0	4.0		
H	BMV+p	MT20	3.0	4.0		
I	BVMWWW-I	MT20	7.0	8.0	Edge	2.00
J	BVMWWW-I	MT20	6.0	9.0	3.00	3.25
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	717	0	717	0	0	1-8	1-8
L	834	0	834	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	544	271 / 0	122 / 0	0 / 0	0 / 0	151 / 0	0 / 0
L	624	335 / 0	122 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

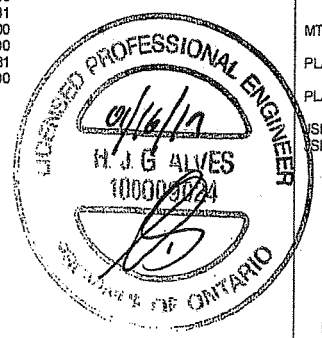
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.9	-84.9	0.11 (1)	10.00
B-C	-1688 / 0	-84.9	-84.9	0.15 (1)	5.01
C-D	-1720 / 0	-84.9	-84.9	0.17 (1)	4.93
D-E	-236 / 0	-84.9	-84.9	0.13 (1)	6.25
E-F	-225 / 0	-84.9	-84.9	0.04 (1)	8.25
G-F	-670 / 0	0.0	0.0	0.26 (1)	6.25
L-B	-746 / 0	0.0	0.0	0.08 (1)	7.81
L-K	0 / 44	-38.5	-38.5	0.08 (3)	10.00
K-J	0 / 74	0.0	0.0	0.13 (1)	10.00
J-C	-265 / 0	0.0	0.0	0.09 (1)	7.81
J-I	0 / 586	-38.5	-38.5	0.48 (2)	10.00
H-I	0 / 40	0.0	0.0	0.04 (1)	10.00
I-E	-213 / 0	0.0	0.0	0.06 (1)	7.81
H-G	0 / 13	-38.5	-38.5	0.03 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-G	-20 / 0	0.00 (1)
I-F	0 / 757	0.17 (1)
D-I	-548 / 0	0.22 (1)
J-D	0 / 1216	0.27 (1)
L-J	-53 / 0	0.01 (1)
B-J	0 / 1449	0.33 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BC8C 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 780 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 463 (0.30")

CSI: TC=0.26/1.00 (F-G:1) , BC=0.48/1.00 (I-J:2) , WB=0.33/1.00 (B-J:1) , SSI=0.14/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.87 (F) (INPUT = 0.90)
SI METAL= 0.66 (B) (INPUT = 1.00)

DWG NO. TAM T1901250
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	T419	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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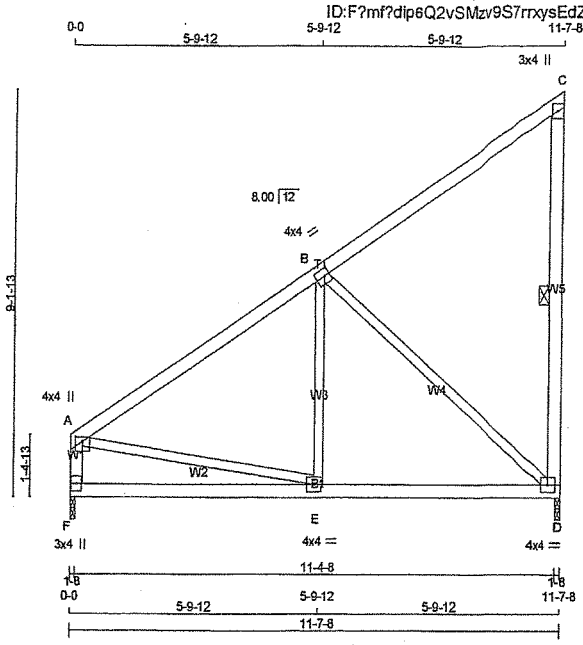


Scale = 1:50.4

TOTAL WEIGHT = 3 X 54 = 163 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
F	717	0	717	0	0	1-8	1-8
D	717	0	717	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	544	271 / 0	122 / 0	0 / 0	0 / 0	151 / 0	0 / 0
D	544	271 / 0	122 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-632 / 0	0.0	0.0	0.07 (1)	7.81	A- E	0 / 532	0.12 (1)
A- B	-595 / 0	-84.9	-84.9	0.38 (1)	6.25	E- B	0 / 304	0.07 (3)
B- C	-34 / 0	-84.9	-84.9	0.37 (1)	6.25	B- D	-687 / 0	0.77 (1)
D- C	-186 / 0	0.0	0.0	0.07 (1)	6.25			
F- E	0 / 0	-38.5	-38.5	0.29 (3)	10.00			
E- D	0 / 523	-38.5	-38.5	0.35 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.38/1.00 (A-B:1) , BC=0.35/1.00 (D-E:2) , WB=0.77/1.00 (B-D:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

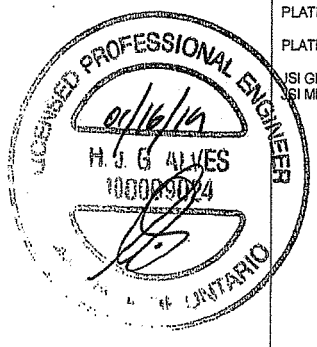
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.61 (A) (INPUT = 0.90)
SI METAL= 0.20 (E) (INPUT = 1.00)



DWG NO. TAM T7901257
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	G420	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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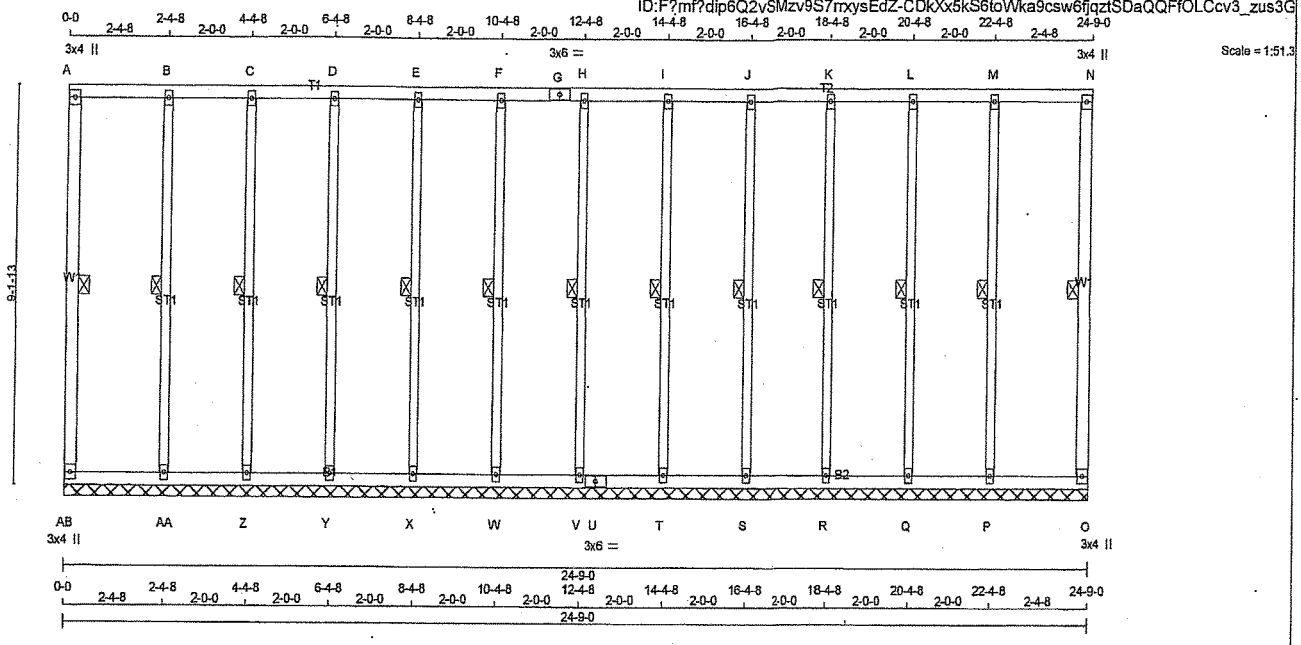

Scale = 1:51.3

TOTAL WEIGHT = 153 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AB- A	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - N	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
AB- U	2x4	DRY	No.2	SPF
U - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMV+p	MT20	3.0	4.0	
B, C, D, E, F, H, I, J, K, L, M					
B	TMW+w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	6.0	
N	TMV+p	MT20	3.0	4.0	
O	BMV1+p	MT20	3.0	4.0	
P, Q, R, S, T, V, W, X, Y, Z, AA					
P	BMW1+w	MT20	2.0	4.0	
U	BS-t	MT20	3.0	6.0	
AB	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-AB, N-O, H-V, F-W, E-X, D-Y, C-Z, B-AA, I-T, J-S, K-R, L-Q, M-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
AB- A	-91 / 0	0.0	0.0	0.02 (1)	6.25
A- B	-3 / 0	-84.9	-84.9	0.05 (1)	10.00
B- C	-3 / 0	-84.9	-84.9	0.05 (1)	10.00
C- D	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
D- E	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
E- F	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
F- G	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
G- H	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
H- I	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
I- J	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
J- K	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
K- L	-3 / 0	-84.9	-84.9	0.04 (1)	10.00
L- M	-3 / 0	-84.9	-84.9	0.05 (1)	10.00
M- N	-3 / 0	-84.9	-84.9	0.05 (1)	10.00
O- N	-91 / 0	0.0	0.0	0.02 (1)	6.25
AB-AA	0 / 3	-38.5	-38.5	0.04 (3)	10.00
AA- Z	0 / 3	-38.5	-38.5	0.04 (3)	10.00
Z- Y	0 / 3	-38.5	-38.5	0.02 (3)	10.00
Y- X	0 / 3	-38.5	-38.5	0.02 (3)	10.00
X- W	0 / 3	-38.5	-38.5	0.02 (3)	10.00
W- V	0 / 3	-38.5	-38.5	0.02 (3)	10.00
V- U	0 / 3	-38.5	-38.5	0.02 (3)	10.00
U- T	0 / 3	-38.5	-38.5	0.02 (3)	10.00
T- S	0 / 3	-38.5	-38.5	0.02 (3)	10.00
S- R	0 / 3	-38.5	-38.5	0.02 (3)	10.00
R- Q	0 / 3	-38.5	-38.5	0.02 (3)	10.00
Q- P	0 / 3	-38.5	-38.5	0.04 (3)	10.00
P- O	0 / 3	-38.5	-38.5	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- H	-170 / 0	0.09 (1)
W- F	-170 / 0	0.09 (1)
X- E	-170 / 0	0.09 (1)
Y- D	-170 / 0	0.09 (1)
Z- C	-164 / 0	0.09 (1)
AA- B	-201 / 0	0.11 (1)
T- I	-170 / 0	0.09 (1)
S- J	-170 / 0	0.09 (1)
R- K	-170 / 0	0.09 (1)
Q- L	-164 / 0	0.09 (1)
P- M	-201 / 0	0.11 (1)

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100088024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = **24.0** IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (A-B:1) , BC=0.04/1.00 (AA-AB:3) , WB=0.11/1.00 (B-AA:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (H) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)

DWG NO. TAM T1901254
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	P2	2	1	TRUSS OESC.	

Tamarack Roof Truss, Burlington

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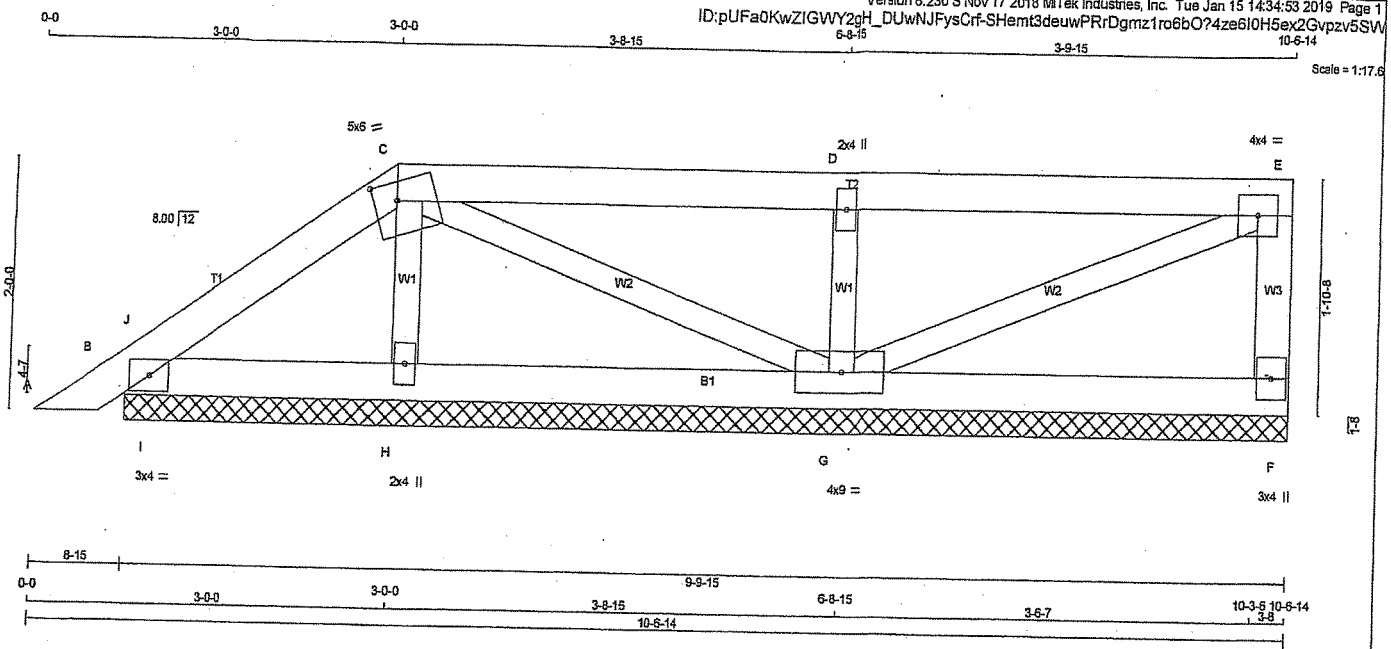


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	1.75	2.25
O	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTOREO GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
F	181	0	181	0	0	9-9-15	9-9-15
B	209	0	209	0	0	9-9-15	9-9-15
H	300	0	300	0	0	9-9-15	9-9-15
G	579	0	579	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WINO	DEAD	SOIL
F	137	67 / 0	32 / 0	0 / 0	0 / 0	39 / 0	0 / 0
B	152	96 / 0	19 / 0	0 / 0	0 / 0	37 / 0	0 / 0
H	233	100 / 0	64 / 0	0 / 0	0 / 0	69 / 0	0 / 0
G	436	225 / 0	92 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORO TO BE SHEATHEO OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORO LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

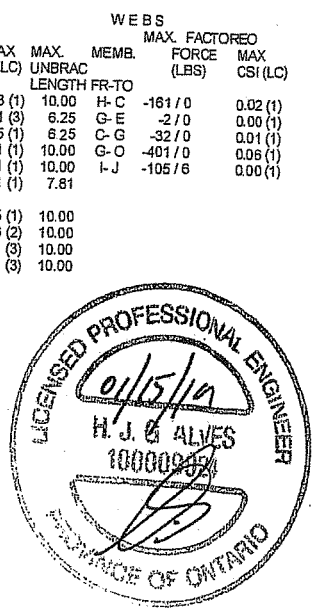
TOTAL LOAD CASES: (4)

CHORDS

MEMB FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	
B- J	-42 / 0	-84.9	-84.9	0.01 (3)	6.25	
J- C	-48 / 0	-84.9	-84.9	0.05 (1)	6.25	
C- D	0 / 2	-84.9	-84.9	0.21 (1)	10.00	
D- E	0 / 2	-84.9	-84.9	0.21 (1)	10.00	
F- E	-122 / 0	0.0	0.0	0.01 (1)	7.81	
B- I	0 / 39	-38.5	-38.5	0.05 (1)	10.00	
I- H	0 / 39	-38.5	-38.5	0.06 (2)	10.00	
H- G	0 / 28	-38.5	-38.5	0.11 (3)	10.00	
G- F	0 / 0	-38.5	-38.5	0.11 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-161 / 0	0.02 (1)
G- E	-2 / 0	0.00 (1)
C- G	-32 / 0	0.01 (1)
G- O	-401 / 0	0.06 (1)
I- J	-105 / 6	0.00 (1)



TOTAL WEIGHT = 2 X 33 = 67 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	OL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	OL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-D:1) , BC=0.11/1.00 (G-H:3) , WB=0.06/1.00 (D-G:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

BCIN Acc. Form 01901212
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	P3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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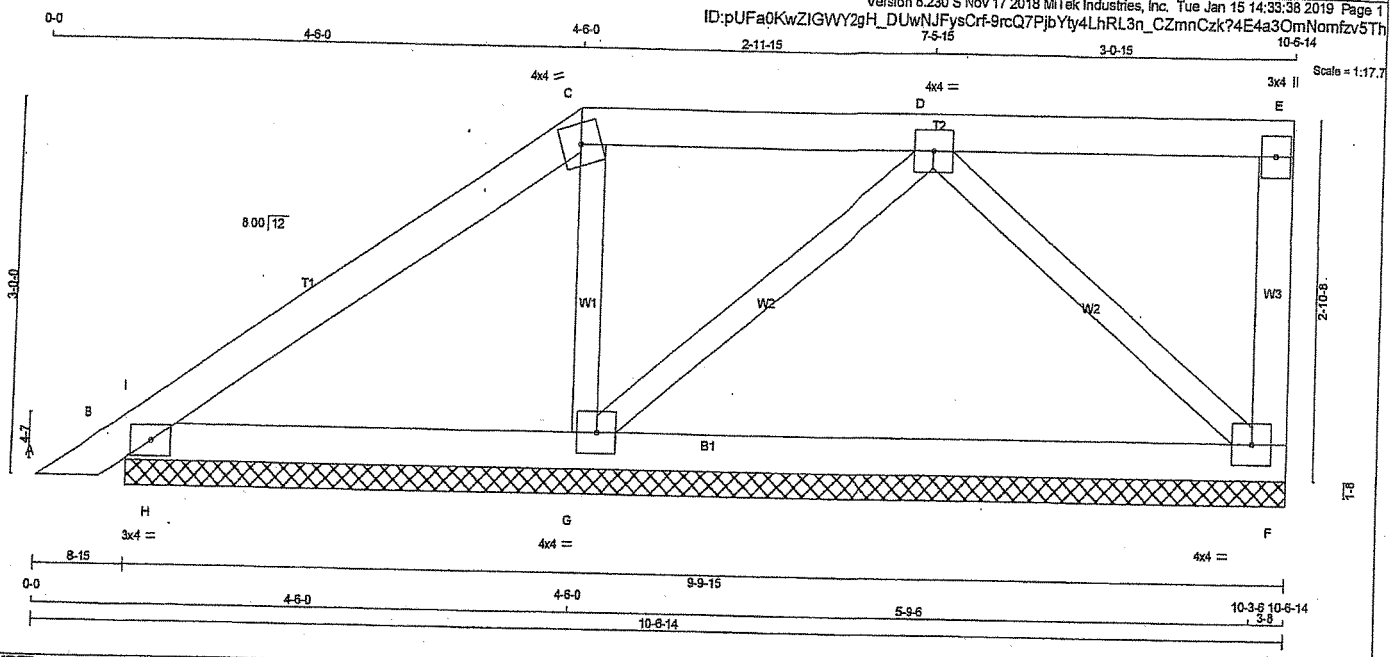

TOTAL WEIGHT = 2 X 35 = 69 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMWW-t	MT20	4.0	4.0	
E	TMV+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	
G	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	355	0	355	0	0	9-9-15	9-9-15
B	274	0	274	0	0	9-9-15	9-9-15
G	639	0	639	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	266	144 / 0	51 / 0	0 / 0	0 / 0	71 / 0	0 / 0
B	198	128 / 0	22 / 0	0 / 0	0 / 0	48 / 0	0 / 0
G	494	217 / 0	133 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	
B- I	0 / 49	-84.9	-84.9	0.07 (1)	10.00	
I- C	-47 / 0	-84.9	-84.9	0.14 (1)	6.25	
C- D	-25 / 0	-84.9	-84.9	0.13 (1)	6.25	
D- E	0 / 0	-84.9	-84.9	0.13 (1)	10.00	
F- E	-100 / 0	0.0	0.0	0.01 (1)	7.81	
B- H	0 / 33	-38.5	-38.5	0.14 (1)	10.00	
H- G	0 / 33	-38.5	-38.5	0.25 (2)	10.00	
G- F	0 / 197	-38.5	-38.5	0.27 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
G- C	-208 / 0	0.04 (1)
G- O	-230 / 0	0.06 (1)
D- F	-257 / 0	0.07 (1)
H- I	-312 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. 4LVES
100009924
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (C-I:1) , BC=0.27/1.00 (F-G:2) , WB=0.07/1.00 (D-F:1) , SSI=0.25/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION UVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (F) (INPUT = 0.90)
JSI METAL= 0.07 (F) (INPUT = 1.00)

DESIGN BY TAM 19 01 21 3
STRUCTURAL
CONSULTANTS LTD.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300330	P4	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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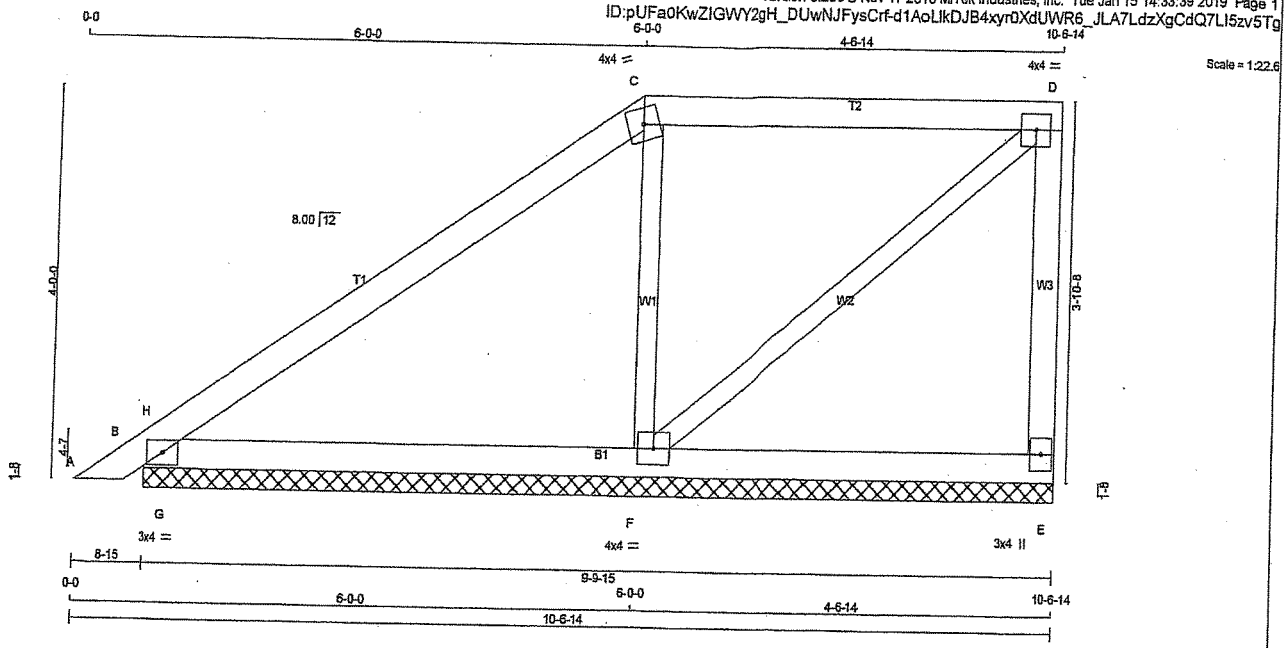


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	307	0	307	0	0	9-9-15	9-9-15
B	413	0	413	0	0	9-9-15	9-9-15
F	548	0	548	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	228	128 / 0	41 / 0	0 / 0	0 / 0	60 / 0	0 / 0
B	306	176 / 0	51 / 0	0 / 0	0 / 0	79 / 0	0 / 0
F	424	185 / 0	115 / 0	0 / 0	0 / 0	124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

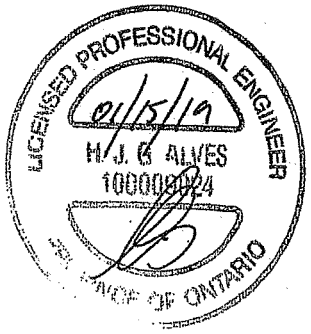
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- H	-106 / 45	-84.9	-84.9	0.11 (1)	6.25
H- C	-115 / 0	-84.9	-84.9	0.31 (1)	6.25
C- D	-79 / 0	-84.9	-84.9	0.30 (1)	6.25
E- D	-257 / 0	0.0	0.0	0.06 (1)	7.81
B- G	0 / 88	-38.5	-38.5	0.23 (1)	10.00
G- F	0 / 88	-38.5	-38.5	0.25 (1)	10.00
F- E	0 / 0	-38.5	-38.5	0.24 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
F- C	-330 / 0	0.08 (1)
F- D	0 / 101	0.02 (1)
G- H	-473 / 72	0.00 (1)



TOTAL WEIGHT = 3 X 35 = 106 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-H:1) , BC=0.25/1.00 (F-G:1) , WB=0.08/1.00 (C-F:1) , SSI=0.41/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

DWG No. ... T90 1214
STRUCTURAL
COMPONENTS

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	P5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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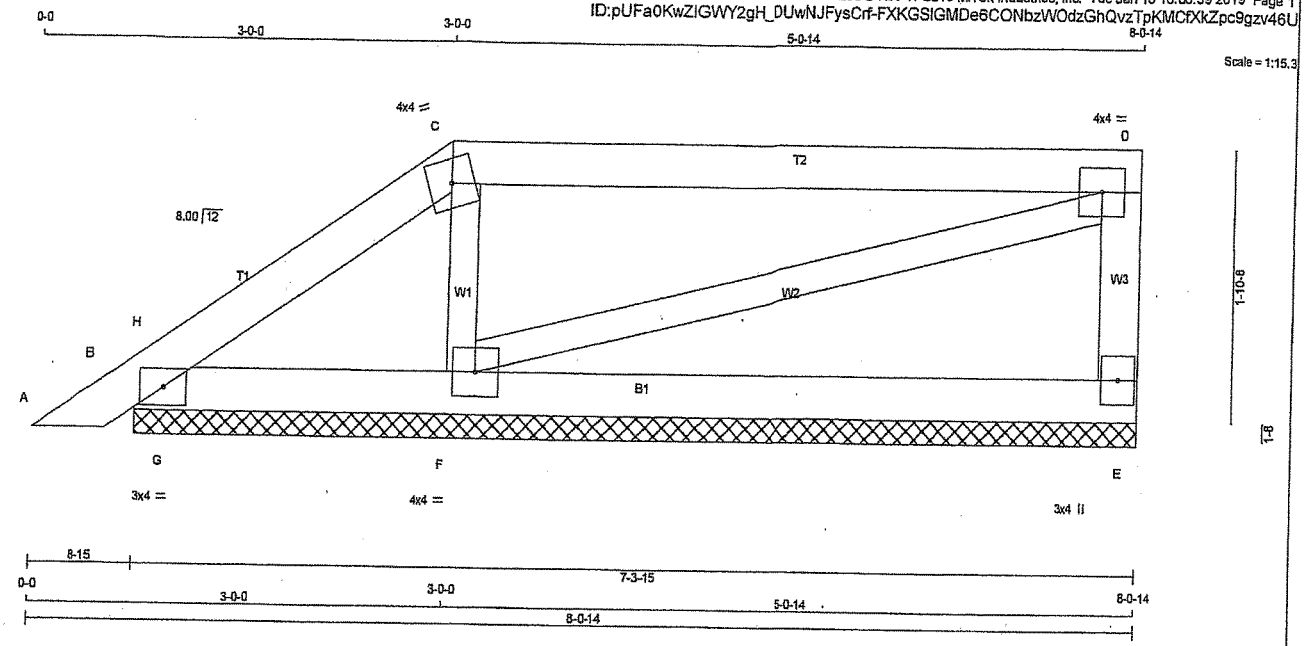

TOTAL WEIGHT = 2 X 25 = 49 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	OESCR.
A - C	2x4	ORY	No.2	SPF
C - D	2x4	ORY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	295	0	295	0	0	7-3-15	7-3-15
B	179	0	179	0	0	7-3-15	7-3-15
F	486	0	486	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	221	120 / 0	42 / 0	0 / 0	0 / 0	59 / 0	0 / 0
B	125	95 / 0	3 / 0	0 / 0	0 / 0	26 / 0	0 / 0
F	379	157 / 0	109 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 14	-84.9	-84.9	0.03 (1)		10.00
B-H	0 / 26	-84.9	-84.9	0.03 (2)		10.00
H-C	-43 / 0	-84.9	-84.9	0.04 (1)		6.25
C-D	-13 / 0	-84.9	-84.9	0.37 (1)		6.25
E-D	-219 / 0	0.0	0.0	0.02 (1)		7.81
B-G	0 / 31	-38.5	-38.5	0.06 (1)		10.00
G-F	0 / 31	-38.5	-38.5	0.17 (3)		10.00
F-E	0 / 0	-38.5	-38.5	0.17 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
F-C	-268 / 0	0.04 (1)
F-D	0 / 14	0.00 (1)
G-H	-164 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37/1.00 (C-D:1) , BC=0.17/1.00 (F-G:3) , WB=0.04/1.00 (C-F:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.04 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 7190 1215
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	P6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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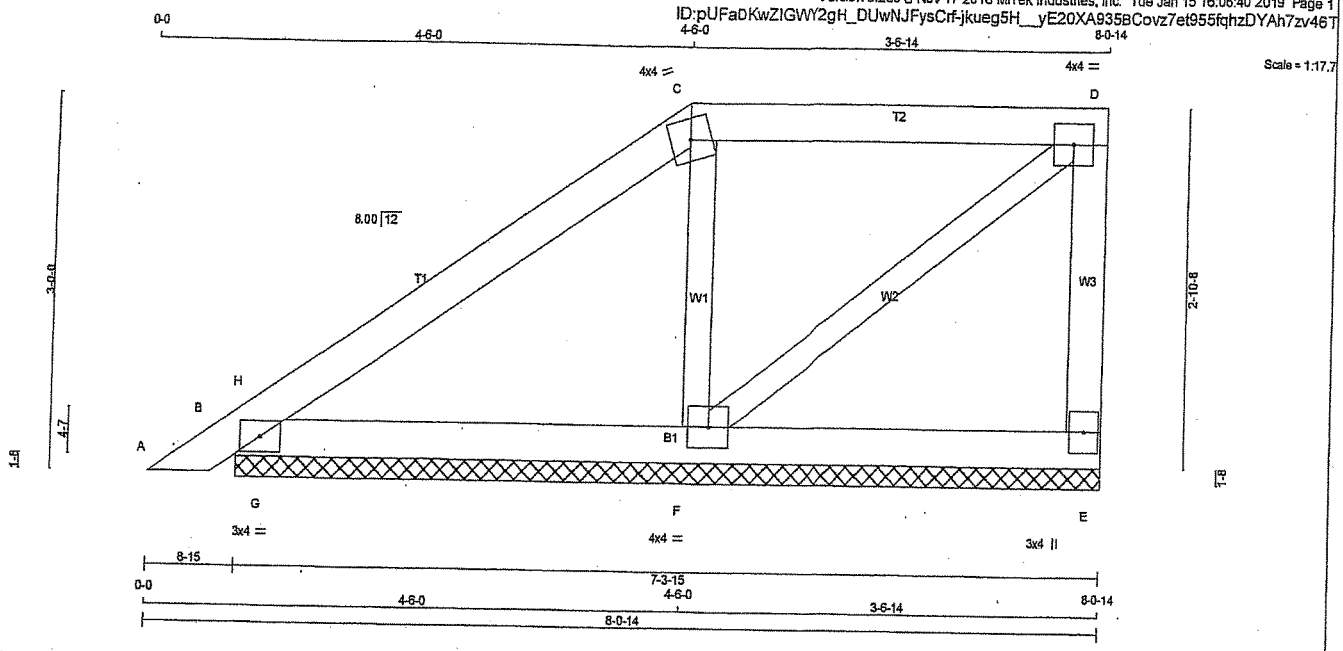

TOTAL WEIGHT = 2 X 26 = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	225	0	225	0	0	7-3-15	7-3-15
B	304	0	304	0	0	7-3-15	7-3-15
F	431	0	431	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	167	93 / 0	30 / 0	0 / 0	0 / 0	44 / 0	0 / 0
B	224	132 / 0	35 / 0	0 / 0	0 / 0	57 / 0	0 / 0
F	333	147 / 0	89 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- H	-59 / 0	-84.9	-84.9	0.04 (1)	6.25
H- C	-67 / 0	-84.9	-84.9	0.15 (1)	6.25
C- D	-42 / 0	-84.9	-84.9	0.18 (1)	6.25
E- D	-182 / 0	0.0	0.0	0.03 (1)	7.81
B- G	0 / 52	-38.5	-38.5	0.12 (1)	10.00
G- F	0 / 52	-38.5	-38.5	0.14 (2)	10.00
F- E	0 / 0	-38.5	-38.5	0.13 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-255 / 0	0.04 (1)
F- D	0 / 52	0.01 (1)
G- H	-253 / 36	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-D:1) , BC=0.14/1.00 (F-G:2) , WB=0.04/1.00 (C-F:1) , SSI=0.22/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SEOTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009924
PROVINCE OF ONTARIO

DWG NO. TAM 1190121G
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300334	P7	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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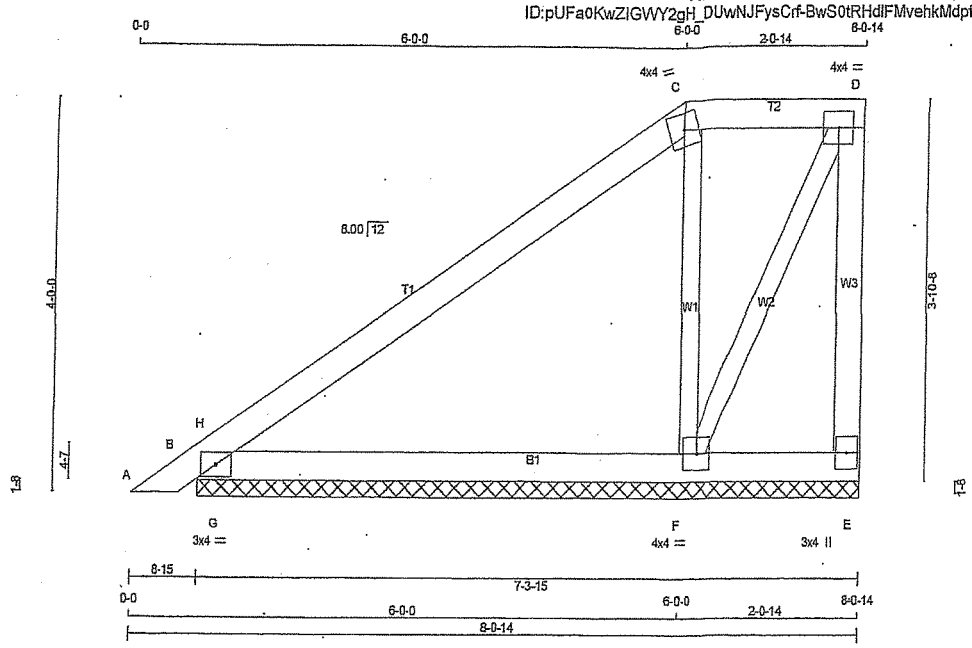

TOTAL WEIGHT = 4 X 28 = 113 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	ORY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	129	0	129	0	0	7-3-15	7-3-15
B	390	0	390	0	0	7-3-15	7-3-15
F	442	0	442	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	92	63 / 0	7 / 0	0 / 0	0 / 0	21 / 0	0 / 0
B	290	162 / 0	51 / 0	0 / 0	0 / 0	76 / 0	0 / 0
F	343	147 / 0	95 / 0	0 / 0	0 / 0	101 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.9	-84.9	0.03 (1)	10.00
B- H	-103 / 88	-84.9	-84.9	0.12 (1)	6.25
H- C	-73 / 0	-84.9	-84.9	0.31 (1)	6.25
C- D	-46 / 0	-84.9	-84.9	0.06 (1)	6.25
E- D	-172 / 0	0.0	0.0	0.04 (1)	7.81
B- G	0 / 54	-38.5	-38.5	0.22 (1)	10.00
G- F	0 / 54	-38.5	-38.5	0.24 (1)	10.00
F- E	0 / 0	-38.5	-38.5	0.22 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-247 / 0	0.06 (1)
F- D	0 / 96	0.02 (1)
G- H	-476 / 88	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-H:1) , BC=0.24/1.00 (F-G:1) , WB=0.06/1.00 (C-F:1) , SSI=0.41/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL.= 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100099[illegible]
PROVINCE OF ONTARIO

DWG NO. TAM 1790 1224
STRUCTURAL
[illegible] ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	P8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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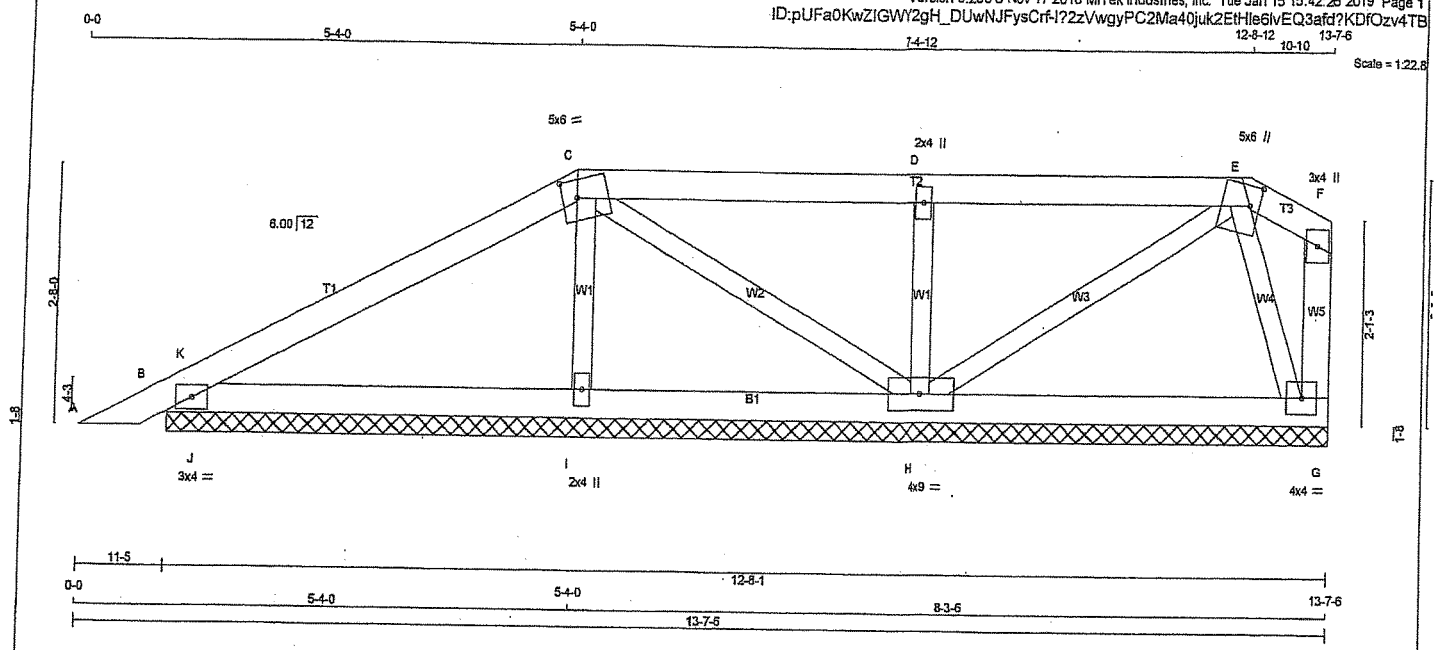

TOTAL WEIGHT = 44 lb [M]

LUMBER
N. L. G. A. RULES

CHOROS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
O	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.25
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW1-t	MT20	4.0	9.0		
I	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	239	0	239	0	0	12-8-1	12-8-1
B	370	0	370	0	0	12-8-1	12-8-1
I	416	0	416	0	0	12-8-1	12-8-1
H	613	0	613	0	0	12-8-1	12-8-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	UVE	PERM.LIVE	WIND	DEAD	SOIL
G	181	91 / 0	40 / 0	0 / 0	0 / 0	50 / 0	0 / 0
B	273	159 / 0	44 / 0	0 / 0	0 / 0	70 / 0	0 / 0
I	321	143 / 0	85 / 0	0 / 0	0 / 0	93 / 0	0 / 0
H	462	238 / 0	97 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-84.9	-84.9	0.05 (1)	10.00
B-K	-84 / 0	-84.9	-84.9	0.05 (3)	8.25
K-C	-98 / 0	-84.9	-84.9	0.21 (1)	6.25
C-D	-13 / 0	-84.9	-84.9	0.19 (1)	6.25
D-E	-13 / 0	-84.9	-84.9	0.19 (1)	6.25
E-F	0 / 0	-84.9	-84.9	0.01 (1)	10.00
G-F	-40 / 0	0.0	0.0	0.00 (1)	7.81
B-J	0 / 85	-38.5	-38.5	0.17 (1)	10.00
J-I	0 / 85	-38.5	-38.5	0.18 (1)	10.00
I-H	0 / 76	-38.5	-38.5	0.15 (2)	10.00
H-G	0 / 53	-38.5	-38.5	0.13 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
I-C	-207 / 0	0.03 (1)
C-H	-72 / 0	0.02 (1)
H-D	-389 / 0	0.06 (1)
E-G	-136 / 0	0.02 (1)
H-E	-46 / 0	0.01 (1)
J-K	-240 / 63	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6:00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-K:1) , BC=0.18/1.00 (I-J:1) , WB=0.06/1.00 (D-H:1) , SSI=0.22/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

ENG NO. TAM 1790 12/17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	P8A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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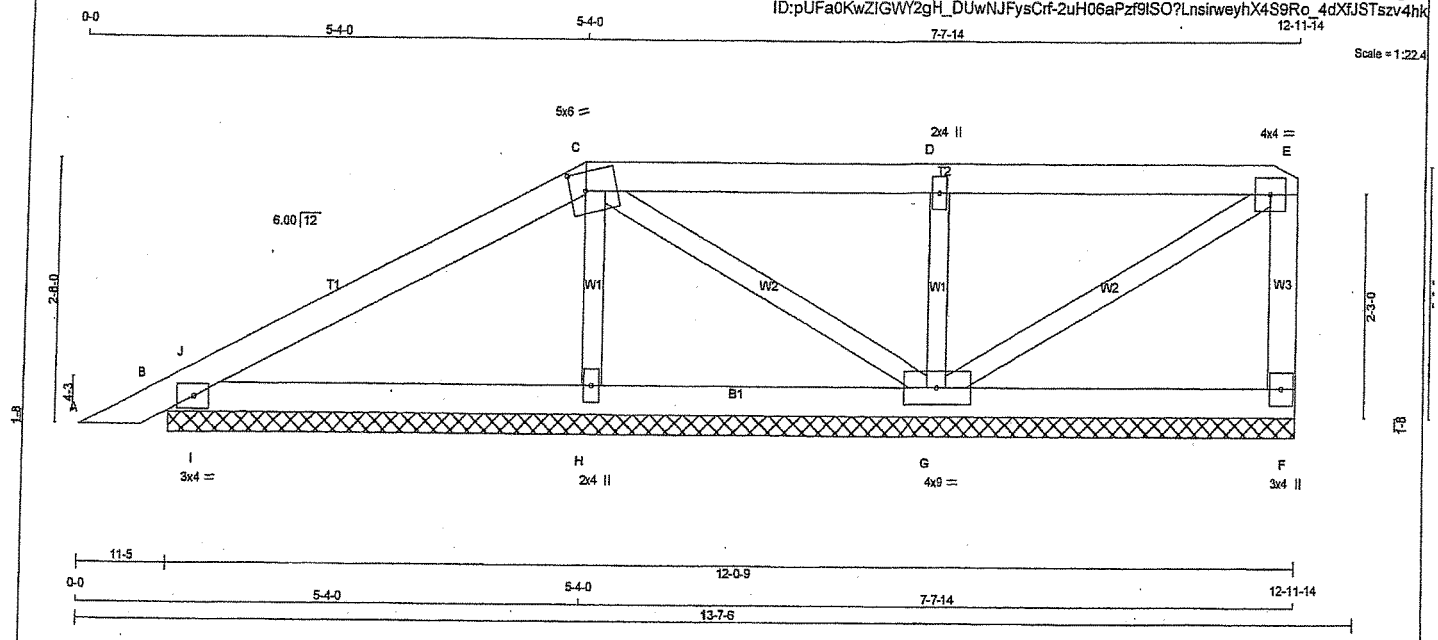


TOTAL WEIGHT = 41 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	189	0	189	0	0	12-0-9	12-0-9
B	367	0	367	0	0	12-0-9	12-0-9
H	419	0	419	0	0	12-0-9	12-0-9
G	586	0	586	0	0	12-0-9	12-0-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	144	69 / 0	34 / 0	0 / 0	0 / 0	41 / 0	0 / 0
B	270	158 / 0	43 / 0	0 / 0	0 / 0	69 / 0	0 / 0
H	325	141 / 0	88 / 0	0 / 0	0 / 0	95 / 0	0 / 0
G	440	234 / 0	87 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

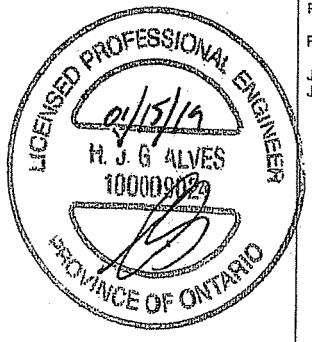
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00
B- J	-77 / 0	-84.9	-84.9	0.05 (3)	6.25
J- C	-92 / 0	-84.9	-84.9	0.21 (1)	6.25
C- D	-5 / 0	-84.9	-84.9	0.21 (1)	10.00
D- E	-5 / 0	-84.9	-84.9	0.21 (1)	10.00
F- E	-126 / 0	0.0	0.0	0.02 (1)	7.81
B- I	0 / 80	-38.5	-38.5	0.17 (1)	10.00
I- H	0 / 80	-38.5	-38.5	0.18 (1)	10.00
H- G	0 / 71	-38.5	-38.5	0.15 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.10 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-203 / 0	0.03 (1)
C- G	-79 / 0	0.02 (1)
G- D	-405 / 0	0.06 (1)
G- E	0 / 6	0.00 (3)
I- J	-243 / 60	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-J:1) , BC=0.18/1.00 (H-I:1) , WB=0.06/1.00 (D-G:1) , SSI=0.22/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

...TAM 7790/218
...CTURAL
...NT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300581	P9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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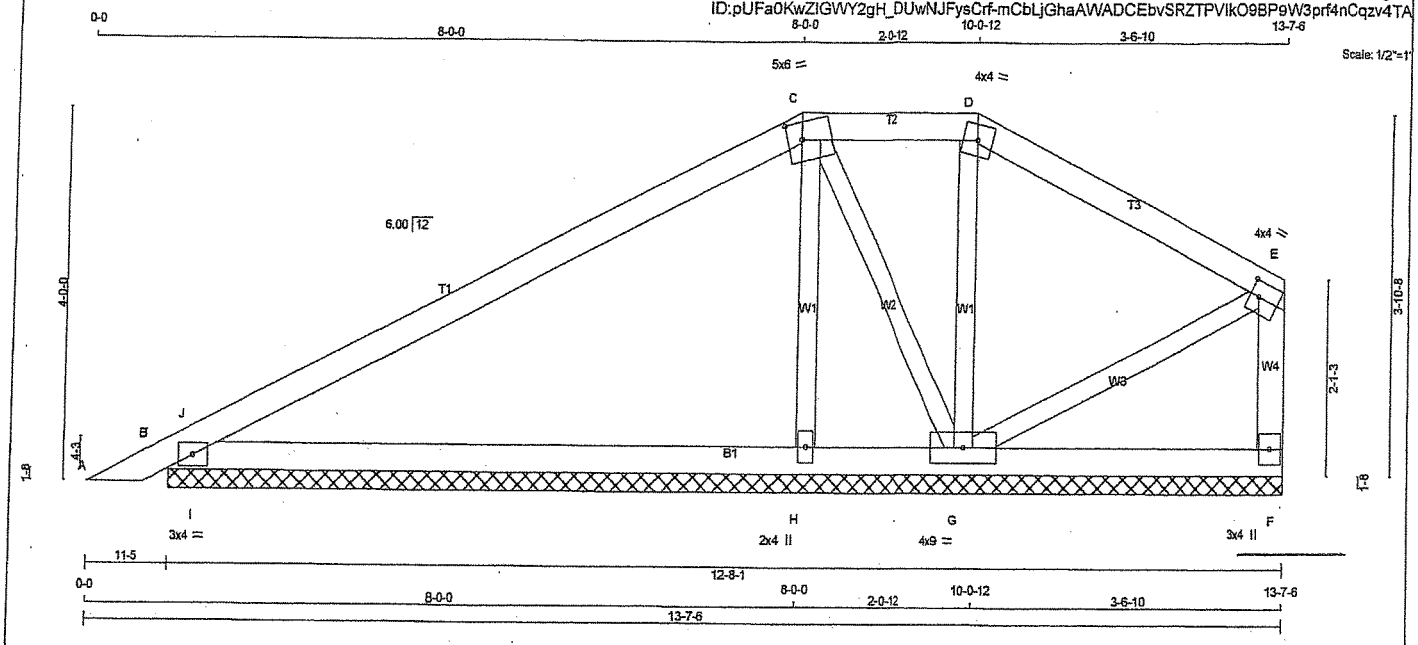


TOTAL WEIGHT = 45 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	309	0	309	0	0	12-8-1	12-8-1
B	556	0	556	0	0	12-8-1	12-8-1
H	525	0	525	0	0	12-8-1	12-8-1
G	247	0	247	0	0	12-8-1	12-8-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	232	121 / 0	48 / 0	0 / 0	0 / 0	63 / 0	0 / 0
B	413	233 / 0	72 / 0	0 / 0	0 / 0	108 / 0	0 / 0
H	415	156 / 0	132 / 0	0 / 0	0 / 0	128 / 0	0 / 0
G	177	121 / 0	15 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

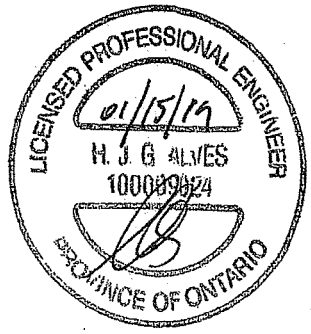
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)		10.00
B- J	-159 / 0	-84.9	-84.9	0.18 (1)		6.25
J- C	-199 / 0	-84.9	-84.9	0.56 (1)		6.25
C- D	-82 / 0	-84.9	-84.9	0.06 (1)		6.25
D- E	-98 / 0	-84.9	-84.9	0.18 (1)		6.25
F- E	-238 / 0	0.0	0.0	0.03 (1)		7.81
B- I	0 / 170	-38.5	-38.5	0.40 (1)		10.00
I- H	0 / 170	-38.5	-38.5	0.44 (2)		10.00
H- G	0 / 166	-38.5	-38.5	0.44 (2)		10.00
G- F	0 / 0	-38.5	-38.5	0.10 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-111 / 1	0.03 (1)
C- G	-175 / 0	0.05 (1)
G- D	-194 / 0	0.05 (1)
G- E	0 / 98	0.02 (1)
I- J	-626 / 127	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56/1.00 (C-J:1) , BC=0.44/1.00 (H-I:2) , WB=0.05/1.00 (C-G:1) , SSI=0.54/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

... TAM 1/90/2019
... TURAL
... NT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300333	P9A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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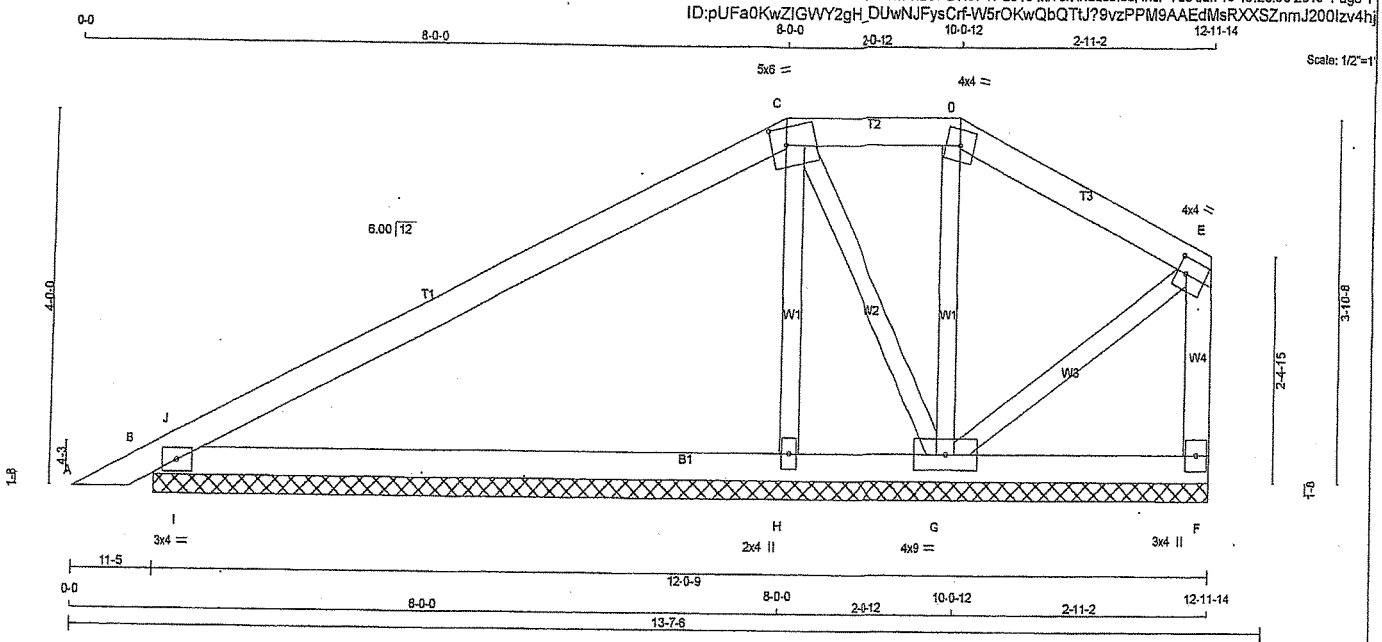

TOTAL WEIGHT = 43 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
O	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
F	287	0	287	0	0	12-0-9	12-0-9
B	654	0	554	0	0	12-0-9	12-0-9
H	537	0	537	0	0	12-0-9	12-0-9
G	183	0	183	0	0	12-0-9	12-0-9

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	217	111 / 0	46 / 0	0 / 0	0 / 0	59 / 0	0 / 0
B	411	232 / 0	71 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	426	156 / 0	138 / 0	0 / 0	0 / 0	132 / 0	0 / 0
G	125	103 / 0	0 / -2	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00
B- J	-157 / 0	-84.9	-84.9	0.18 (1)	6.25
J- C	-194 / 0	-84.9	-84.9	0.56 (1)	6.25
C- D	-76 / 0	-84.9	-84.9	0.06 (1)	8.25
D- E	-90 / 0	-84.9	-84.9	0.12 (1)	6.25
F- E	-221 / 0	0.0	0.0	0.03 (1)	7.81
B- I	0 / 165	-38.5	-38.5	0.40 (1)	10.00
I- H	0 / 165	-38.5	-38.5	0.44 (2)	10.00
H- G	0 / 162	-38.5	-38.5	0.44 (2)	10.00
G- F	0 / 0	-38.5	-38.5	0.09 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-109 / 2	0.03 (1)
C- G	-180 / 0	0.05 (1)
G- D	-171 / 0	0.04 (1)
G- E	0 / 98	0.02 (1)
I- J	-628 / 125	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
01/15/19
H. J. G. ALVES
100099
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.6	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56/1.00 (C-J:1) , BC=0.44/1.00 (H-I:2) , WB=0.05/1.00 (C-G:1) , SSI=0.54/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

DWG NO. TAM 7790 1220
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300439	P10AG	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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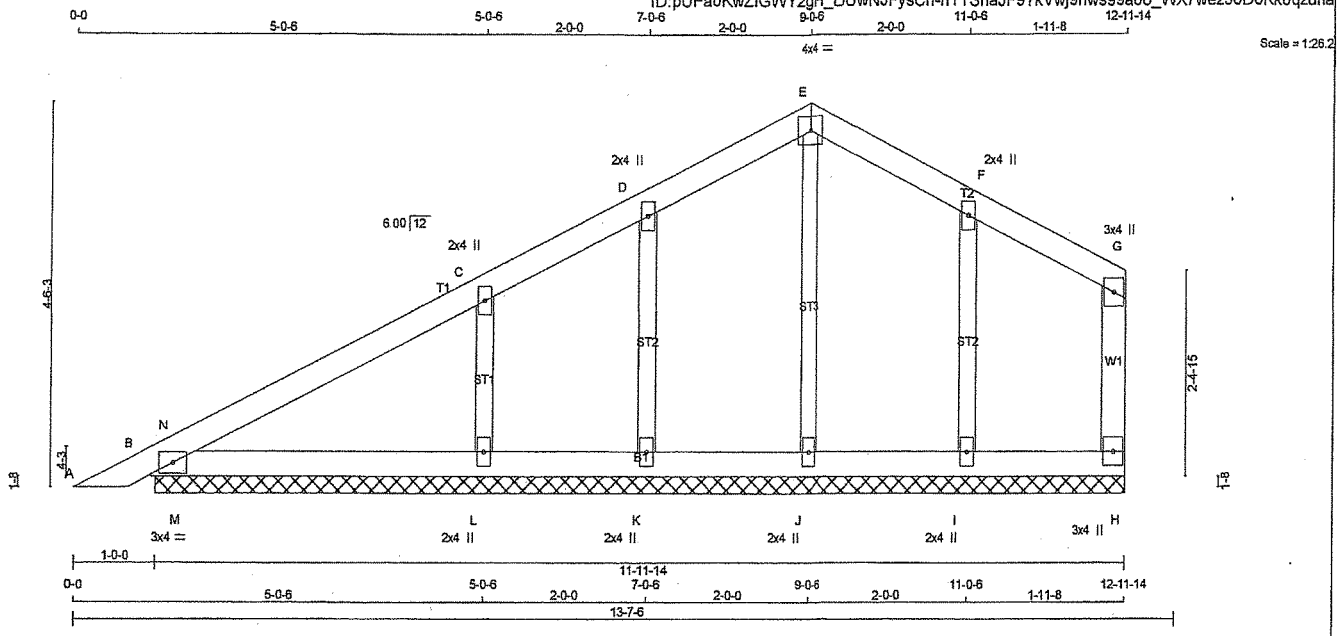

TOTAL WEIGHT = 42 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, F						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	J- E	-151 / 0	0.04 (1)
B- N	-46 / 0	-84.9	-84.9	0.05 (3)	6.25	K- D	-136 / 0	0.03 (1)
N- C	0 / 6	-84.9	-84.9	0.15 (1)	10.00	L- C	-309 / 0	0.05 (1)
C- D	-18 / 0	-84.9	-84.9	0.15 (1)	6.25	I- F	-206 / 0	0.04 (1)
D- E	0 / 0	-84.9	-84.9	0.05 (1)	10.00	M- N	-126 / 75	0.00 (1)
E- F	-4 / 0	-84.9	-84.9	0.05 (1)	10.00			
F- G	0 / 9	-84.9	-84.9	0.06 (1)	10.00			
H- G	-59 / 0	0.0	0.0	0.01 (1)	7.81			
B- M	0 / 8	-38.5	-38.5	0.09 (1)	10.00			
M- L	0 / 8	-38.5	-38.5	0.11 (2)	10.00			
L- K	-1 / 0	-38.5	-38.5	0.11 (2)	10.00			
K- J	-4 / 0	-38.5	-38.5	0.03 (2)	10.00			
J- I	-4 / 0	-38.5	-38.5	0.03 (3)	10.00			
I- H	0 / 0	-38.5	-38.5	0.03 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-N:1) , BC=0.11/1.00 (L-M:2) , WB=0.05/1.00 (C-L:1) , SSI=0.13/1.00 (B-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (C) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

DWG NO. TAM T1901256
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300583	P10G	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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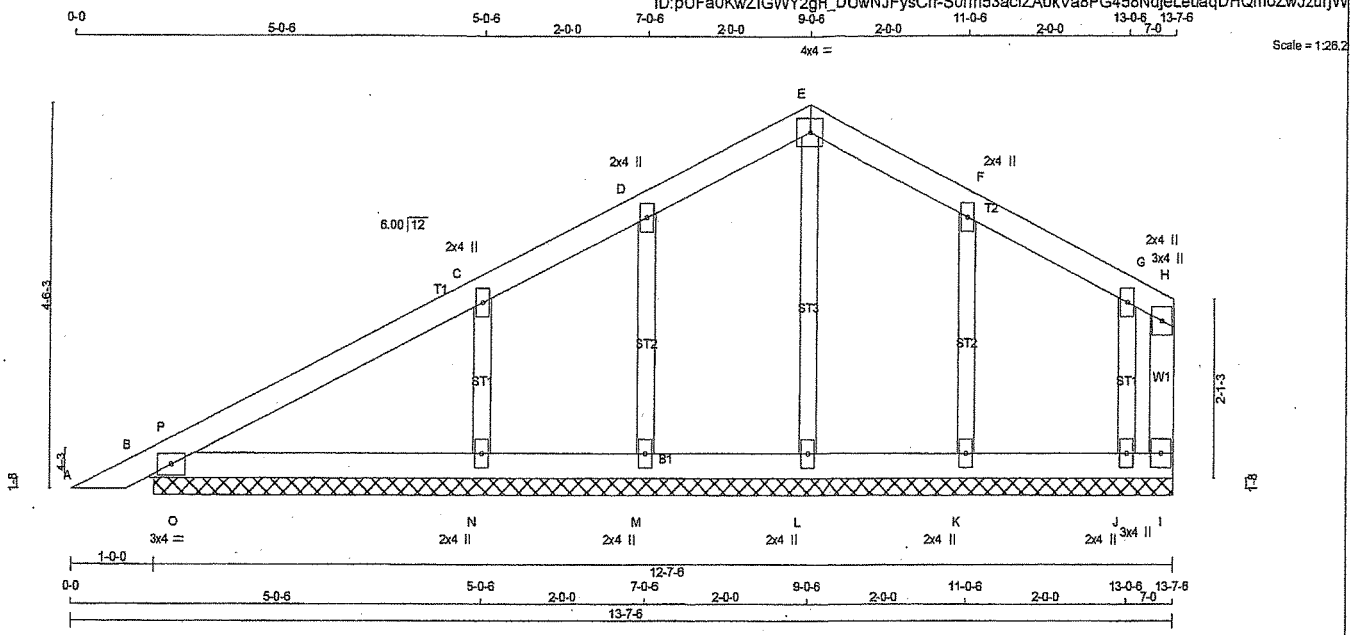


TOTAL WEIGHT = 44 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT2D	3.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT2D	4.0	4.0		
H	TMV+p	MT2D	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		
J, K, L, M, N						
J	BMW1+w	MT2D	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT I FOR 150 LBS FACTORED UPLIFT

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) OK w/ toe NAILING

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

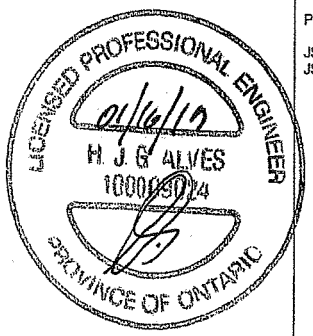
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.9	-84.9	0.05 (1)	10.00
B- P	-46 / 0	-84.9	-84.9	0.05 (3)	6.25
P- C	0 / 9	-84.9	-84.9	0.15 (1)	10.00
C- D	-14 / 0	-84.9	-84.9	0.15 (1)	6.25
D- E	0 / 5	-84.9	-84.9	0.05 (1)	10.00
E- F	0 / 0	-84.9	-84.9	0.05 (1)	10.00
F- G	0 / 10	-84.9	-84.9	0.05 (1)	10.00
G- H	0 / 13	-84.9	-84.9	0.02 (1)	10.00
I- H	0 / 6	0.0	0.0	0.00 (1)	10.00
B- O	0 / 5	-38.5	-38.5	0.09 (1)	10.00
O- N	0 / 5	-38.5	-38.5	0.11 (2)	10.00
N- M	-5 / 0	-38.5	-38.5	0.11 (2)	10.00
M- L	-8 / 0	-38.5	-38.5	0.03 (2)	10.00
L- K	-8 / 0	-38.5	-38.5	0.03 (2)	10.00
K- J	-4 / 0	-38.5	-38.5	0.02 (3)	10.00
J- I	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- E	-155 / 0	0.04 (1)
M- D	-136 / 0	0.03 (1)
N- C	-309 / 0	0.05 (1)
K- F	-201 / 0	0.04 (1)
J- G	-121 / 0	0.02 (1)
O- P	-126 / 75	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-P:1) , BC=0.11/1.00 (N-O:2) , WB=0.05/1.00 (C-N:1) , SSI=0.14/1.00 (B-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (C) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

DWG NO. TAM T7901255
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300402	P19G	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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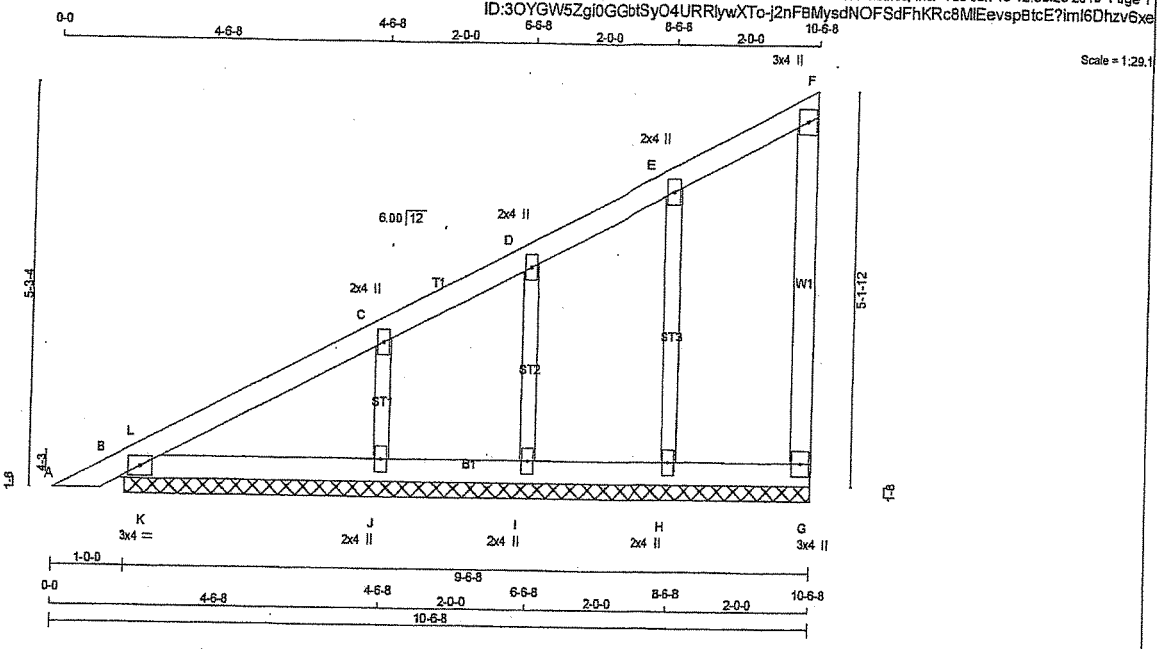


TOTAL WEIGHT = 2 X 36 = 72 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, E						
C	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 15	-84.9	-84.9	0.05 (1)	10.00	
B-L	-39 / 0	-84.9	-84.9	0.04 (3)	6.25	
L-C	-5 / 4	-84.9	-84.9	0.11 (1)	10.00	
C-D	-23 / 0	-84.9	-84.9	0.11 (1)	6.25	
D-E	0 / 1	-84.9	-84.9	0.05 (1)	10.00	
E-F	-8 / 0	-84.9	-84.9	0.05 (1)	10.00	
G-F	-70 / 0	0.0	0.0	0.03 (1)	7.81	
B-K	0 / 16	-38.5	-38.5	0.08 (1)	10.00	
K-J	0 / 16	-38.5	-38.5	0.09 (2)	10.00	
J-I	0 / 6	-38.5	-38.5	0.09 (2)	10.00	
I-H	0 / 3	-38.5	-38.5	0.03 (3)	10.00	
H-G	0 / 0	-38.5	-38.5	0.03 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	MAX CSI (LC)
H-E	-200 / 0	0.05 (1)
I-D	-122 / 0	0.02 (1)
J-C	-277 / 0	0.04 (1)
K-L	-98 / 80	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (C-D:1) , BC=0.09/1.00 (J-K:2) , WB=0.05/1.00 (E-H:1) , SSI=0.11/1.00 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

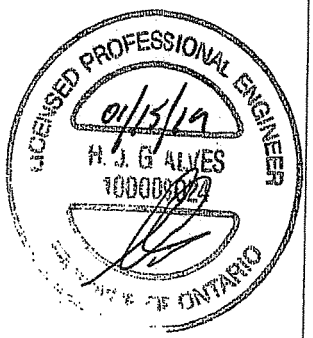
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TDL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)



DWG NO. TAM T1901137
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300402	P20	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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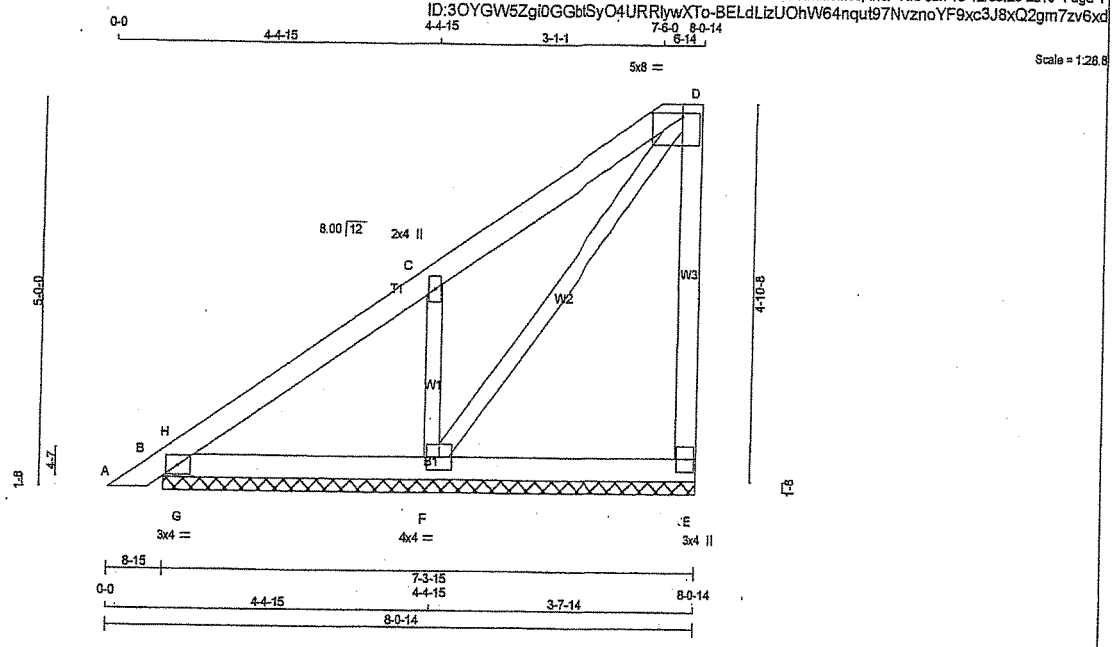


TOTAL WEIGHT = 2 X 30 = 61 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DES CR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMWMW*-p	MT20	5.0	8.0	0.50	5.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	172	0	172	0	0	7-3-15	7-3-15
B	229	0	229	0	0	7-3-15	7-3-15
F	558	0	558	0	0	7-3-15	7-3-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	131	64 / 0	30 / 0	0 / 0	0 / 0	37 / 0	0 / 0
B	170	96 / 0	30 / 0	0 / 0	0 / 0	45 / 0	0 / 0
F	423	212 / 0	94 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

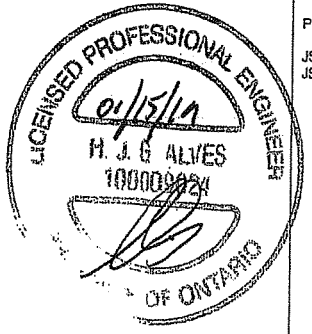
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	
B-H	-50 / 0	-84.9	-84.9	0.03 (3)	6.25	
H-C	0 / 11	-84.9	-84.9	0.16 (1)	10.00	
C-D	-18 / 0	-84.9	-84.9	0.18 (1)	6.25	
E-D	-122 / 0	0.0	0.0	0.06 (1)	7.81	
B-G	0 / 10	-38.5	-38.5	0.06 (1)	10.00	
G-F	0 / 10	-38.5	-38.5	0.12 (2)	10.00	
F-E	0 / 0	-38.5	-38.5	0.12 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-368 / 0	0.06 (1)
F-D	-3 / 6	0.00 (1)
G-H	-95 / 55	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-H:1) , BC=0.12/1.00 (F-G:2) , WB=0.06/1.00 (C-F:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

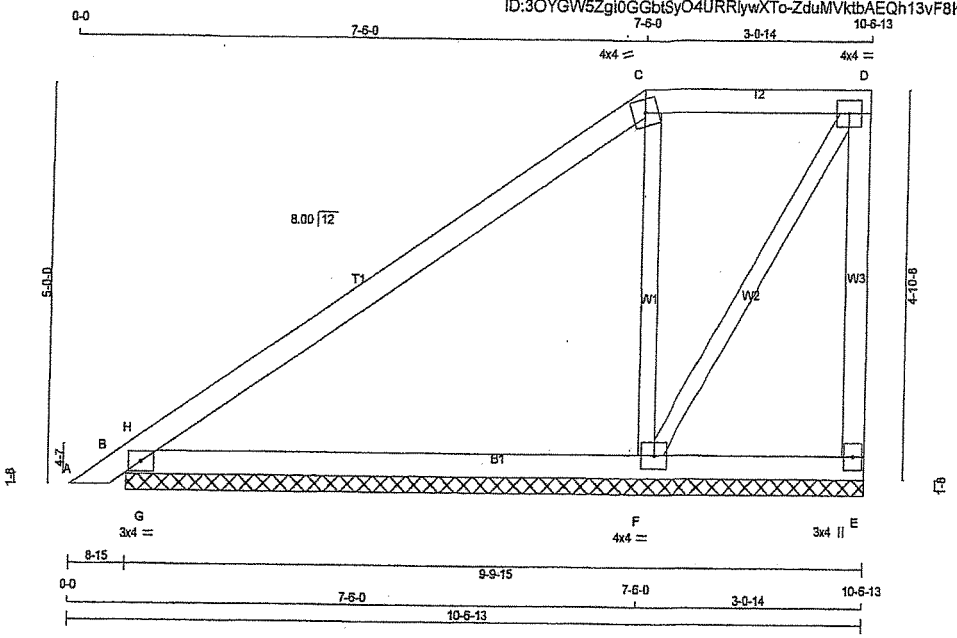
JSI GRIP= 0.25 (C) (INPUT = 0.90)
JSI METAL= 0.19 (C) (INPUT = 1.00)

DWG NO. TAM 7190140
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
300403	P21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Jan 15 12:56:04 2019 Page 1
ID:3OYGW5Zgi0GGbtSyO4URRlywXTo-ZduMVktbAEQh13vF8KzQJHKlWy?UdVhPG?J1Jzzv6v9



TOTAL WEIGHT = 2 X 37 = 75 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	241	0	241	0	0	9-9-15	9-9-15
B	505	0	505	0	0	9-9-15	9-9-15
F	522	0	622	0	0	9-9-15	9-9-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	176	109 / 0	23 / 0	0 / 0	0 / 0	44 / 0	0 / 0
B	376	208 / 0	68 / 0	0 / 0	0 / 0	99 / 0	0 / 0
F	407	171 / 0	115 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 14	-84.9	-84.9	0.03 (1)	10.00	
B-H	-167 / 174	-84.9	-84.9	0.22 (1)	6.25	
H-C	-130 / 0	-84.9	-84.9	0.51 (1)	6.25	
C-D	-90 / 0	-84.9	-84.9	0.14 (1)	6.25	
E-D	-289 / 0	0.0	0.0	0.11 (1)	7.81	
B-G	0 / 97	-38.5	-38.5	0.36 (1)	10.00	
G-F	0 / 97	-38.5	-38.5	0.40 (1)	10.00	
F-E	0 / 0	-38.5	-38.5	0.36 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-317 / 0	0.11 (1)
F-D	0 / 166	0.04 (1)
G-H	-764 / 142	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.51/1.00 (C-H:1) , BC=0.40/1.00 (F-G:1) , WB=0.11/1.00 (C-F:1) , SSI=0.64/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

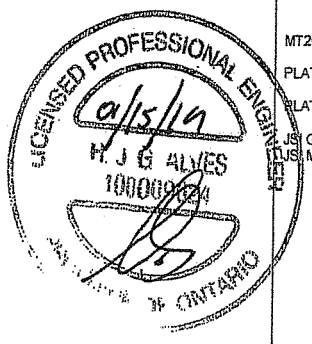
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

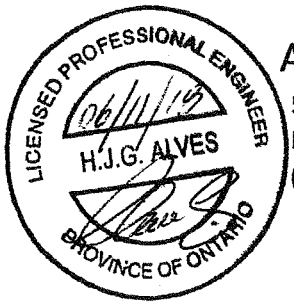
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.38 (B) (INPUT = 0.90)
JS METAL= 0.11 (C) (INPUT = 1.00)



DWG NO. TAM T190442
STRUCTURAL



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

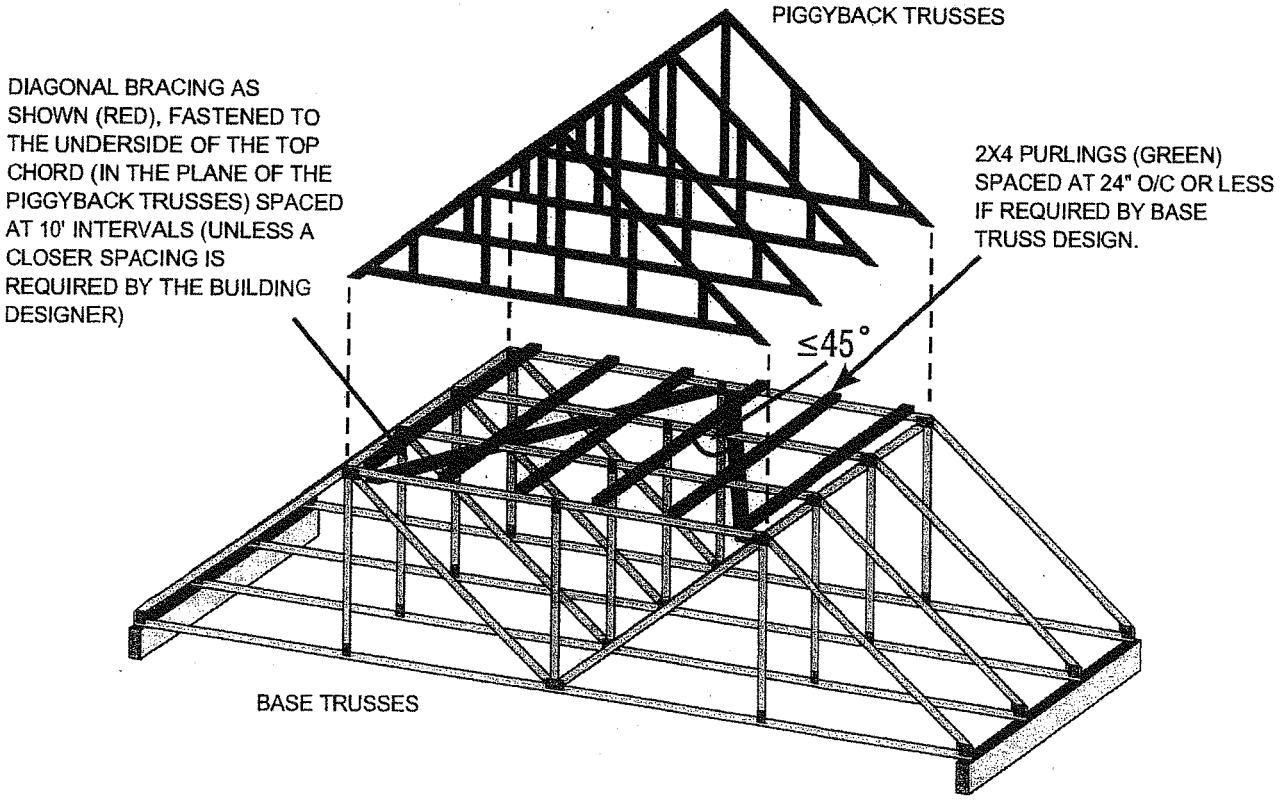
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

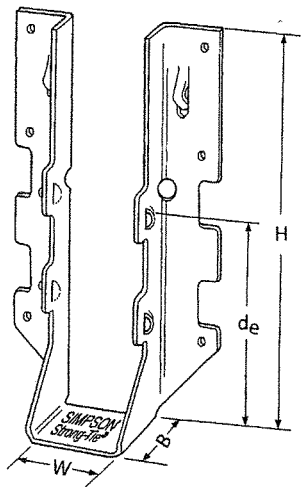
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

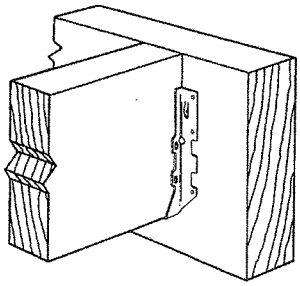
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

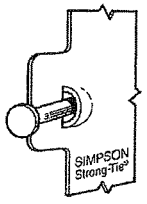


LUS28


Typical LUS Installation

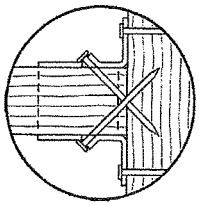
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

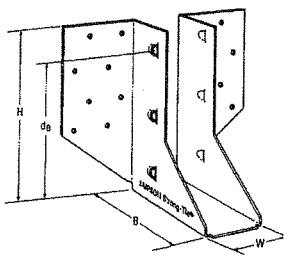
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

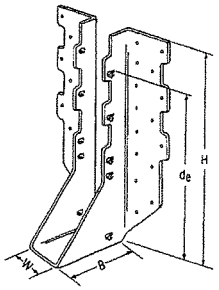
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

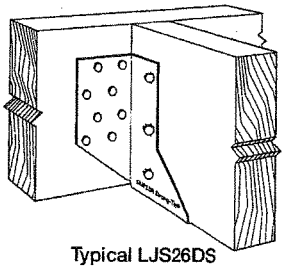
- See current catalogue for options



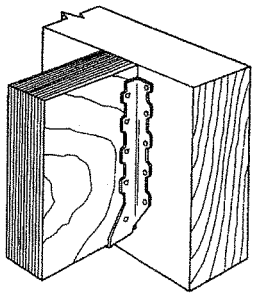
LJS26DS



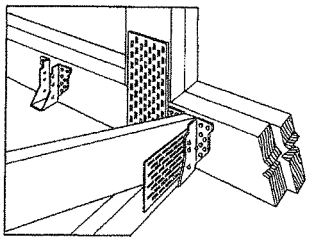
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



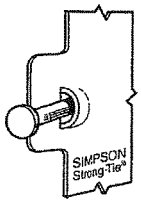
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

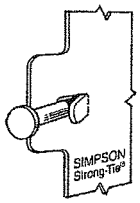
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅝	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

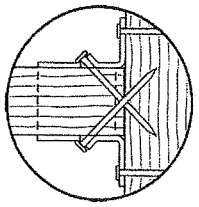


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

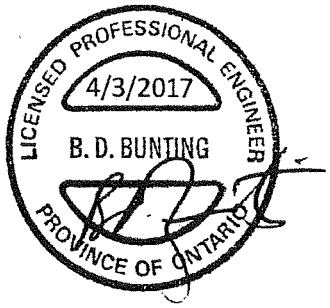
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019 and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty, or see strongtie.com.

T-SPECHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

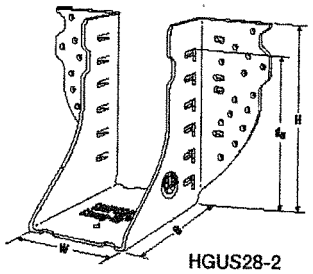
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

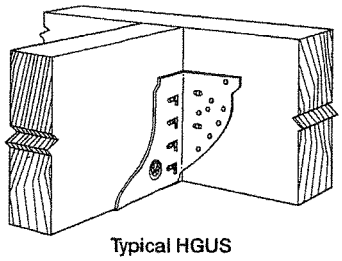
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

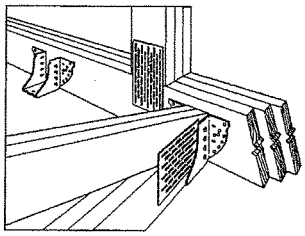
Options:

- See current catalogue for options



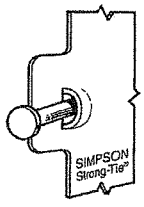
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

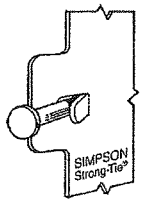
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

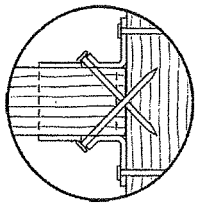


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LIMIT STATES DESIGN

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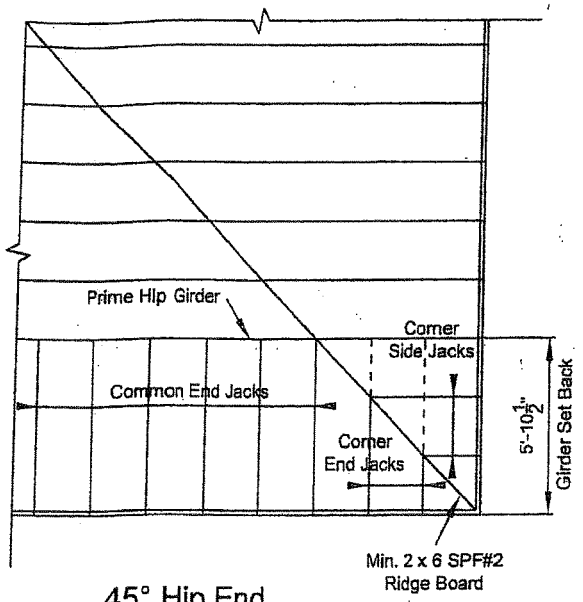


45° Hip End

LUMBER SPECIFICATION

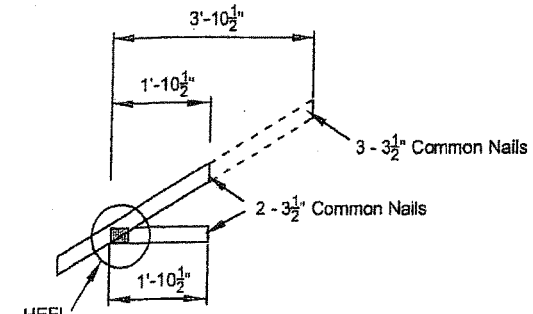
TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

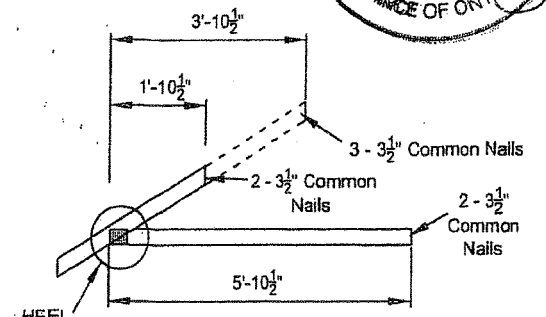
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

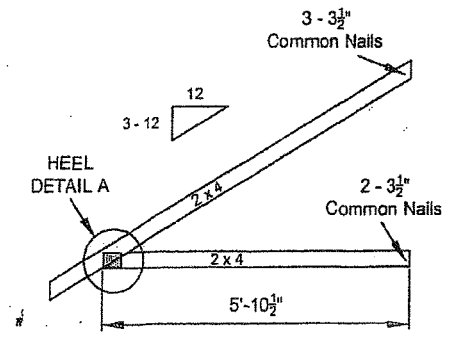
LICENSED PROFESSIONAL ENGINEER
06/11/16
H.J.G. ALVES
PROVINCE OF ONTARIO



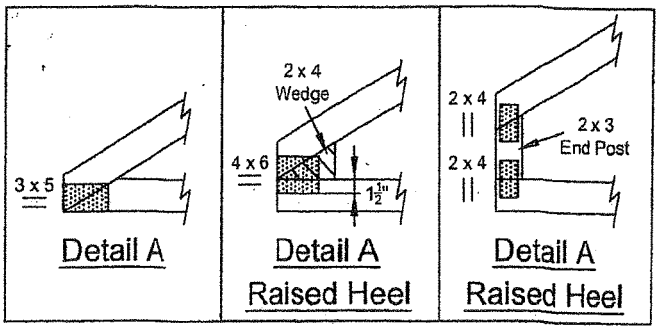
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

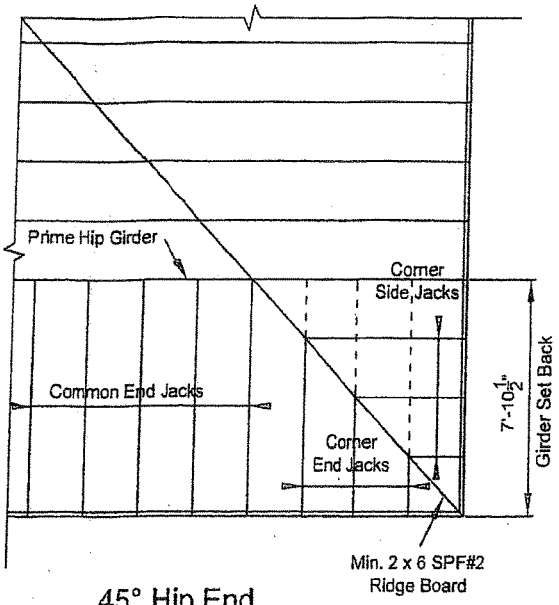
T-1800216

LUMBER SPECIFICATION

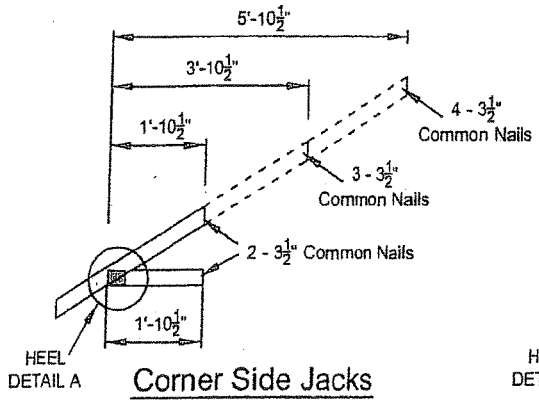
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

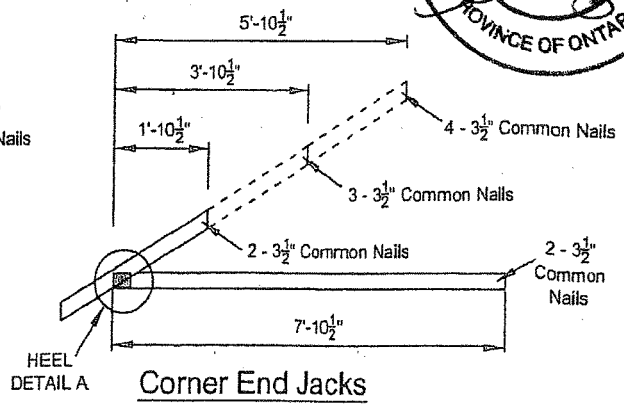
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F



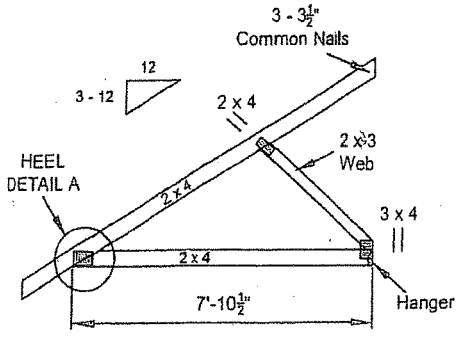
45° Hip End



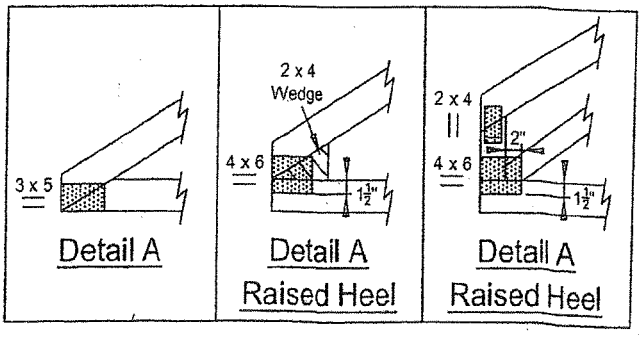
Corner Side Jacks



Corner End Jacks



Common End Jacks



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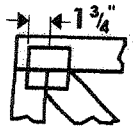
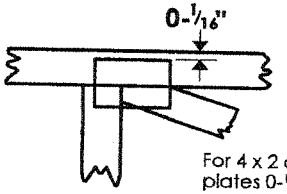
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

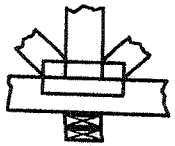
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

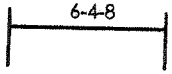


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

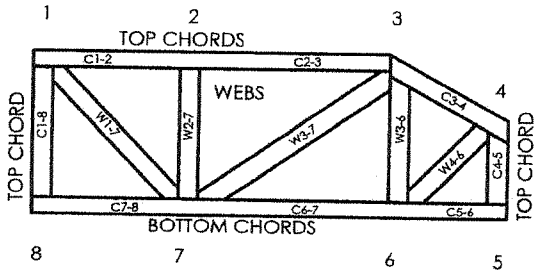
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: Mii-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.