



ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0618-034

GREEN YORK HOMES - GRANELLI HOME CORP -
INDIGO 1 ELEV 1**RESPONSIBILITIES**

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

*For conventional framing roof / ceilings
framing shall conform to OBC.9.23*

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



For conventional framing roof / ceilings
framing shall conform to OBC.9.23

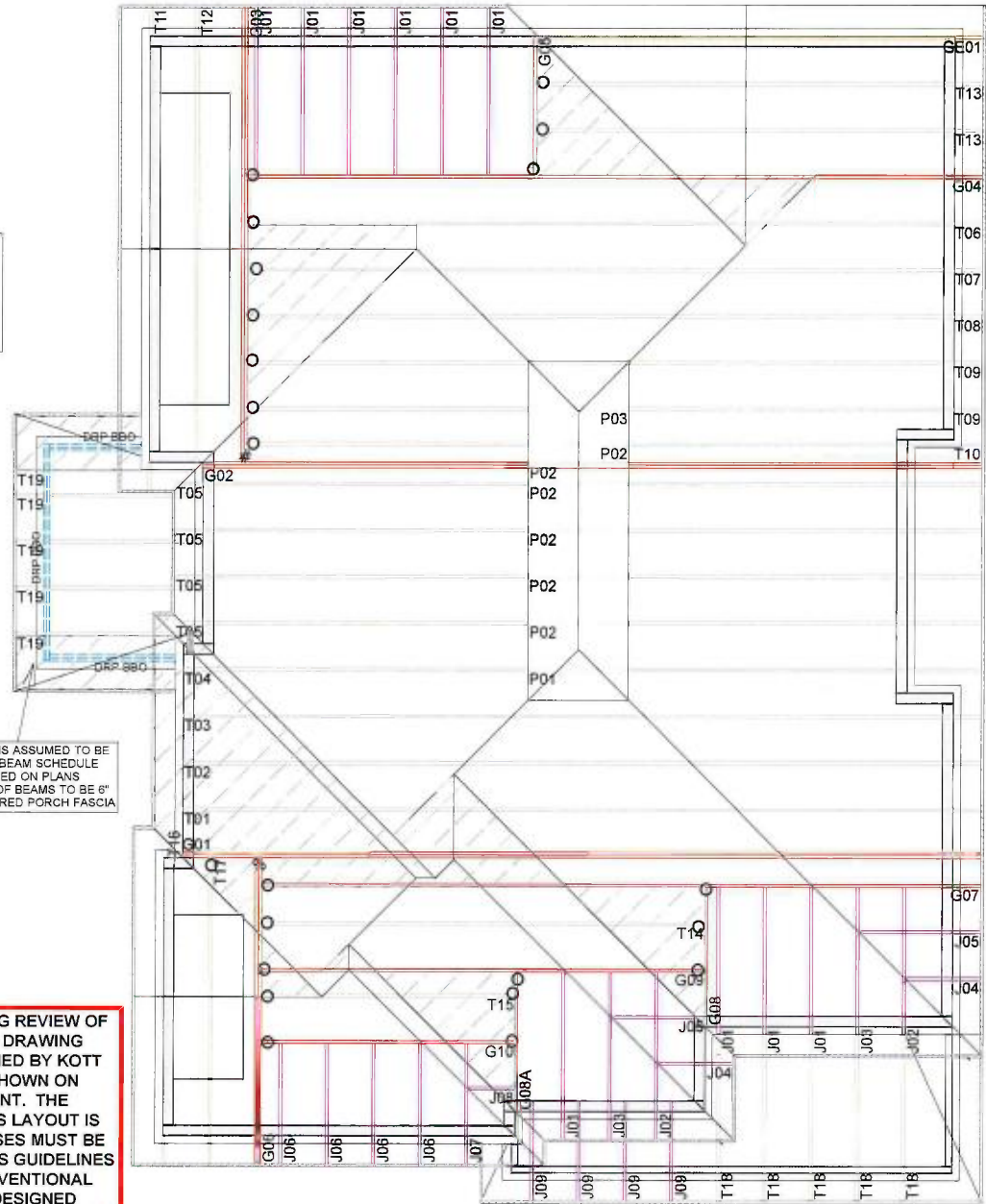
Layout based on plans provided by
Jardin Design Group Inc.
Revision #2 dated May 22, 2018



PORCH BEAMS ASSUMED TO BE
2x2x10, NO BEAM SCHEDULE
PROVIDED ON PLANS
UNDERSIDE OF BEAMS TO BE 6"
BELOW COVERED PORCH FASCIA

CITY OF BRAMPTON
BUILDING DIVISION
REVIEWED
MAR 14 2019
BY
MARY FRENETTE

THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF
STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING
AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT
LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON
THE CALCULATION PAGE OF EACH COMPONENT. THE
PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS
SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE
INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES
AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL
FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED
AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY
AND PERMANENT BRACINGS OF THE ROOF AND BUILDING
ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER
AND TO CONFORM TO BCSI GUIDELINES.



GARAGE TOP OF PLATE
TO MATCH UNDERSIDE
OF SOFFIT

19-447257.00.00
RR.

ROOF TRUSS LAYOUT



CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24
- HGUS28-2
O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS.
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

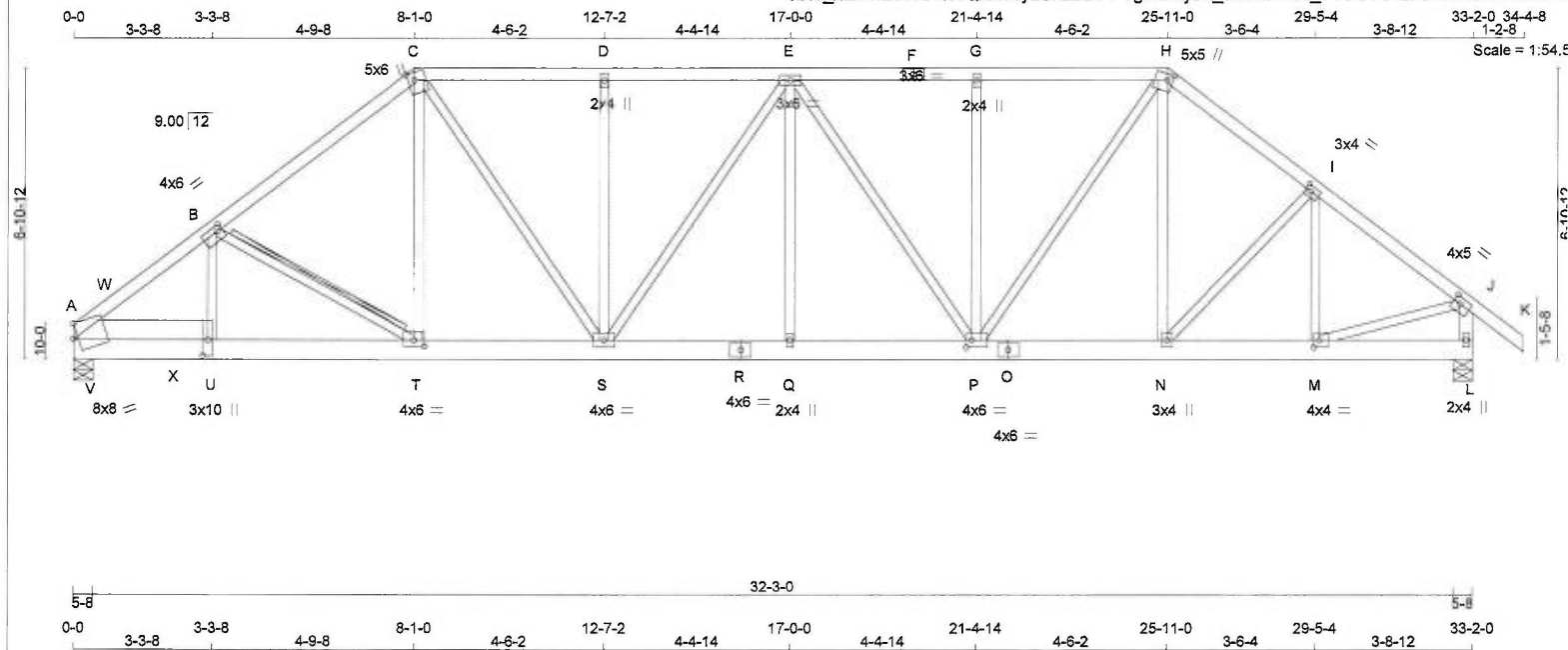
THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011

ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	INDIGO 1 ELE 1
Customer:	GREEN YORK HOMES
Project:	GRANELLI HOME
Location:	BRAMPTON, ON
Drawn By:	DM
Date:	5/31/2018

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS
MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C RAFTER THAT
CROSS MEET OVER TRUSSES TO BE 2X4 SPF @ 24" O/C WITH 2X4 VERT POST TO THE
TRUSS BELOW EA CROSS PT. VERT POS. THAN 6" TO HAVE A LATERAL BRACING SO THAT
THE DISTANCE BETEN PT & VERT ROWS OF BRACING DOES NOT EXCEED 6'



TOTAL WEIGHT = $2 \times 176 = 352 \text{ lb}$

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
A - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- H	1	12	TOP
H- K	1	12	TOP
L- J	1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
A- R	2	12	SIDE(114.0)
R- O	2	12	
O- L	2	12	
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			MAXIMUM FACTORED			INPUT	REQRD
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	6414	0	6414	0	0	5-8	5-8
L	2179	0	2179	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	
A	4506	3124 / 0	0 / 0	0 / 0	0 / 0	1382 / 0	0 / 0
L	1529	1073 / 0	0 / 0	0 / 0	0 / 0	456 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT B-T

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-W	-7362 / 0	-77.4	-77.4 0.18 (1)	3.51	U-B	0 / 3977	0.49 (1)		
V-W	-8254 / 0	-77.4	-77.4 0.31 (1)	3.22	B-T	-3767 / 0	0.60 (1)		
B-C	-4244 / 0	-77.4	-77.4 0.25 (1)	4.45	T-C	0 / 2133	0.26 (1)		
C-D	-3479 / 0	-77.4	-77.4 0.17 (1)	4.88	C-S	0 / 114	0.01 (1)		
D-E	-3479 / 0	-77.4	-77.4 0.17 (1)	4.88	S-D	-373 / 0	0.14 (1)		
E-F	-2758 / 0	-77.4	-77.4 0.15 (1)	5.36	S-E	0 / 398	0.05 (1)		
F-G	-2756 / 0	-77.4	-77.4 0.15 (1)	5.36	Q-E	0 / 89	0.02 (4)		
G-H	-2755 / 0	-77.4	-77.4 0.15 (1)	5.36	E-P	-894 / 0	0.56 (1)		
H-I	-2452 / 0	-77.4	-77.4 0.11 (1)	5.66	P-G	-377 / 0	0.14 (1)		
I-J	-2305 / 0	-77.4	-77.4 0.11 (1)	5.80	P-H	0 / 1428	0.18 (1)		
J-K	0 / 30	-77.4	-77.4 0.05 (1)	10.00	N-H	0 / 64	0.01 (4)		
L-J	-2130 / 0	0.0	0.0 0.12 (1)	7.60	N-I	0 / 121	0.01 (1)		
					M-I	-502 / 0	0.07 (1)		
A-V	0 / 3718	-246.3	-246.3 0.24 (1)	10.00	M-J	0 / 1930	0.24 (1)		
V-X	0 / 3718	-246.3	-246.3 0.40 (1)	10.00	V-W	0 / 528	0.00 (1)		
X-U	0 / 3718	-18.2	-18.2 0.40 (1)	10.00	W-U	0 / 3007	0.20 (1)		
U-T	0 / 6624	-18.2	-18.2 0.58 (1)	10.00					
T-S	0 / 3414	-18.2	-18.2 0.27 (1)	10.00					
S-R	0 / 3256	-18.2	-18.2 0.23 (1)	10.00					
R-Q	0 / 3256	-18.2	-18.2 0.23 (1)	10.00					
Q-P	0 / 3256	-18.2	-18.2 0.23 (1)	10.00					
P-O	0 / 1942	-18.2	-18.2 0.15 (1)	10.00					
O-N	0 / 1942	-18.2	-18.2 0.15 (1)	10.00					
N-M	0 / 1858	-18.2	-18.2 0.14 (1)	10.00					
M-L	0 / 0	-18.2	-18.2 0.02 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	3-3-8	-4771	-4771	=	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder

START DISTANCE = 0-0
START SPAN CARRIED = 12-0-0
END DISTANCE = 2-5-0
END SPAN CARRIED = 12-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL

*** NON STANDARD GIRDER ***

ADD'TL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.11")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
 ALLOWABLE DEFL.(TL)= $L/360$ (1.11")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.31/1.00 (B-W:1), BC=0.58/1.00 (T-U:1),
WB=0.60/1.00 (B-T:1), SSI=0.15/1.00 (U-V:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



PLATES (table is in inches) <table> <tr> <th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr> <tr><td>A</td><td>TMBMW1-m</td><td>MT20</td><td>8.0</td><td>8.0</td><td>4.25</td><td>Edge</td></tr> <tr><td>B</td><td>TMWW-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td>1.50</td><td>1.50</td></tr> <tr><td>C</td><td>TTWW+m</td><td>MT20</td><td>5.0</td><td>6.0</td><td>2.25</td><td>1.75</td></tr> <tr><td>D</td><td>TMW+w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr> <tr><td>E</td><td>TMWWW-t</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr> <tr><td>F</td><td>TS-t</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr> <tr><td>G</td><td>TMW+w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr> <tr><td>H</td><td>TTWW+m</td><td>MT20</td><td>5.0</td><td>5.0</td><td>1.75</td><td>1.50</td></tr> <tr><td>I</td><td>TMWW-t</td><td>MT20</td><td>3.0</td><td>4.0</td><td>1.50</td><td>1.50</td></tr> <tr><td>J</td><td>TMVW-t</td><td>MT20</td><td>4.0</td><td>5.0</td><td>1.75</td><td>2.00</td></tr> <tr><td>L</td><td>BMV1+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr> <tr><td>M</td><td>BMWW-t</td><td>MT20</td><td>4.0</td><td>4.0</td><td>2.00</td><td>1.50</td></tr> <tr><td>N</td><td>BMWW+t</td><td>MT20</td><td>3.0</td><td>4.0</td><td></td><td></td></tr> <tr><td>O</td><td>BS-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr> <tr><td>P</td><td>BMWWW-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td>2.00</td><td>1.50</td></tr> <tr><td>Q</td><td>BMW+w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr> <tr><td>R</td><td>BS-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr> <tr><td>S</td><td>BMWWW-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr> <tr><td>T</td><td>BMWW-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td>1.75</td><td>3.00</td></tr> <tr><td>U</td><td>BMWW+t</td><td>MT20</td><td>3.0</td><td>10.0</td><td>4.50</td><td>1.50</td></tr> </table> Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4771.0 lbs FACTORED DOWN AT 3-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.	JT	TYPE	PLATES	W	LEN	Y	X	A	TMBMW1-m	MT20	8.0	8.0	4.25	Edge	B	TMWW-t	MT20	4.0	6.0	1.50	1.50	C	TTWW+m	MT20	5.0	6.0	2.25	1.75	D	TMW+w	MT20	2.0	4.0			E	TMWWW-t	MT20	3.0	6.0			F	TS-t	MT20	3.0	6.0			G	TMW+w	MT20	2.0	4.0			H	TTWW+m	MT20	5.0	5.0	1.75	1.50	I	TMWW-t	MT20	3.0	4.0	1.50	1.50	J	TMVW-t	MT20	4.0	5.0	1.75	2.00	L	BMV1+p	MT20	2.0	4.0			M	BMWW-t	MT20	4.0	4.0	2.00	1.50	N	BMWW+t	MT20	3.0	4.0			O	BS-t	MT20	4.0	6.0			P	BMWWW-t	MT20	4.0	6.0	2.00	1.50	Q	BMW+w	MT20	2.0	4.0			R	BS-t	MT20	4.0	6.0			S	BMWWW-t	MT20	4.0	6.0			T	BMWW-t	MT20	4.0	6.0	1.75	3.00	U	BMWW+t	MT20	3.0	10.0	4.50	1.50						
JT	TYPE	PLATES	W	LEN	Y	X																																																																																																																																																			
A	TMBMW1-m	MT20	8.0	8.0	4.25	Edge																																																																																																																																																			
B	TMWW-t	MT20	4.0	6.0	1.50	1.50																																																																																																																																																			
C	TTWW+m	MT20	5.0	6.0	2.25	1.75																																																																																																																																																			
D	TMW+w	MT20	2.0	4.0																																																																																																																																																					
E	TMWWW-t	MT20	3.0	6.0																																																																																																																																																					
F	TS-t	MT20	3.0	6.0																																																																																																																																																					
G	TMW+w	MT20	2.0	4.0																																																																																																																																																					
H	TTWW+m	MT20	5.0	5.0	1.75	1.50																																																																																																																																																			
I	TMWW-t	MT20	3.0	4.0	1.50	1.50																																																																																																																																																			
J	TMVW-t	MT20	4.0	5.0	1.75	2.00																																																																																																																																																			
L	BMV1+p	MT20	2.0	4.0																																																																																																																																																					
M	BMWW-t	MT20	4.0	4.0	2.00	1.50																																																																																																																																																			
N	BMWW+t	MT20	3.0	4.0																																																																																																																																																					
O	BS-t	MT20	4.0	6.0																																																																																																																																																					
P	BMWWW-t	MT20	4.0	6.0	2.00	1.50																																																																																																																																																			
Q	BMW+w	MT20	2.0	4.0																																																																																																																																																					
R	BS-t	MT20	4.0	6.0																																																																																																																																																					
S	BMWWW-t	MT20	4.0	6.0																																																																																																																																																					
T	BMWW-t	MT20	4.0	6.0	1.75	3.00																																																																																																																																																			
U	BMWW+t	MT20	3.0	10.0	4.50	1.50																																																																																																																																																			

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284
	1656				

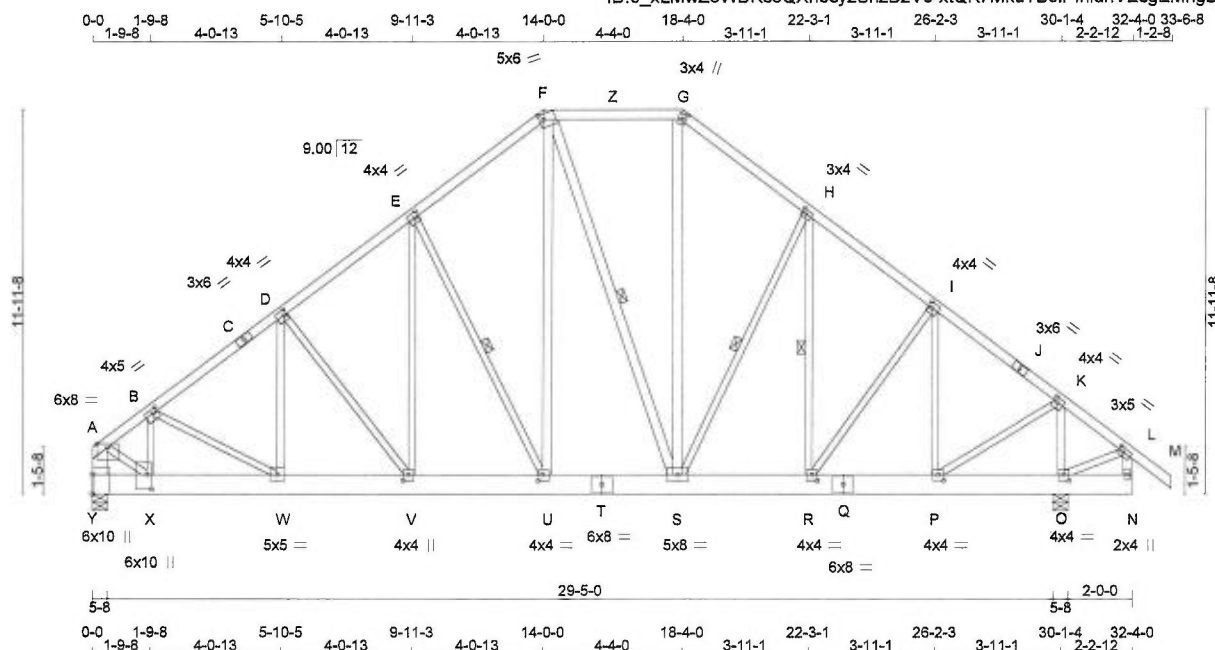
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.63 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:71.6

TOTAL WEIGHT = $2 \times 224 = 448$ lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - G	2x4	DRY No.2	SPF
G - J	2x4	DRY No.2	SPF
J - M	2x4	DRY No.2	SPF
Y - A	2x6	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
Y - T	2x8	DRY No.2	SPF
T - Q	2x8	DRY No.2	SPF
Q - N	2x8	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
U - F	2x4	DRY No.2	SPF
F - S	2x4	DRY No.2	SPF
S - G	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A-C	1	12	TOP
C-F	1	12	TOP
F-G	1	12	TOP
G-J	1	12	TOP
J-M	1	12	TOP
N-L	1	12	TOP
Y-A	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
Y-T	2	12	SIDE(0.0)
T-Q	2	12	TOP
Q-N	2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAIL S.

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Y	8153	0	8153	0	0	5-8	5-8
Q	2210	0	2210	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Y	5730	3964 / 0	0 / 0	0 / 0	0 / 0	1767 / 0	0 / 0
O	1552	1081 / 0	0 / 0	0 / 0	0 / 0	472 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y. O

BRACING

FOR SECTION F-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-U, F-S, H-S, H-R. DBS = 20-0-0 . CBF = 177 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	-7089 / 0	-77.4	-77.4	0.19 (1)	3.58	X-B	0 / 3020	0.37 (1)	
B-C	-4025 / 0	-77.4	-77.4	0.17 (1)	4.62	B-W	-2803 / 0	0.51 (1)	
C-D	-4025 / 0	-77.4	-77.4	0.17 (1)	4.62	W-D	0 / 1898	0.23 (1)	
D-E	-2688 / 0	-77.4	-77.4	0.13 (1)	5.45	D-V	-1792 / 0	0.73 (1)	
E-F	-1921 / 0	-77.4	-77.4	0.11 (1)	6.20	V-E	0 / 1385	0.17 (1)	
F-Z	-1343 / 0	-84.9	-84.9	0.16 (1)	2.00	E-U	-1414 / 0	0.44 (1)	
Z-G	-1343 / 0	-84.9	-84.9	0.16 (1)	2.00	U-F	0 / 1364	0.12 (1)	
G-H	-1690 / 0	-77.4	-77.4	0.10 (1)	6.25	F-S	-564 / 0	0.24 (1)	
H-I	-1689 / 0	-77.4	-77.4	0.10 (1)	6.25	S-G	0 / 701	0.06 (1)	
I-J	-1400 / 0	-77.4	-77.4	0.08 (1)	6.25	S-H	-46 / 0	0.01 (1)	
J-K	-1400 / 0	-77.4	-77.4	0.08 (1)	6.25	H-R	-289 / 0	0.07 (1)	
K-L	-5 / 57	-77.4	-77.4	0.08 (1)	10.00	R-I	0 / 408	0.05 (1)	
L-M	0 / 30	-77.4	-77.4	0.05 (1)	10.00	P-I	-806 / 0	0.21 (1)	
Y-A	-7407 / 0	0.0	0.0	0.26 (1)	5.51	P-K	0 / 1382	0.17 (1)	
N-L	-114 / 0	0.0	0.0	0.01 (1)	7.81	O-K	-1912 / 0	0.18 (1)	
						A-X	0 / 6453	0.80 (1)	
						O-L	-28 / 7	0.00 (1)	
Y-X	0 / 0	-18.2	-18.2	0.32 (1)	10.00				
X-W	0 / 5670	-18.2	-18.2	0.63 (1)	10.00				
W-V	0 / 3234	-18.2	-18.2	0.21 (1)	10.00				
V-U	0 / 2152	-18.2	-18.2	0.15 (1)	10.00				
U-T	0 / 1536	-18.2	-18.2	0.11 (1)	10.00				
T-S	0 / 1536	-18.2	-18.2	0.11 (1)	10.00				
S-R	0 / 1354	-18.2	-18.2	0.10 (1)	10.00				
R-Q	0 / 1120	-18.2	-18.2	0.08 (1)	10.00				
Q-P	0 / 1120	-18.2	-18.2	0.08 (1)	10.00				
P-O	-26 / 7	-18.2	-18.2	0.07 (1)	6.25				
O-N	0 / 0	-18.2	-18.2	0.07 (1)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= $L/120$ (0.22")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL)= $L/120$ (0.22")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.26/1.00 (A-Y:1), BC=0.63/1.00 (W-X:1),
WB=0.80/1.00 (A-X:1), SSI=0.22/1.00 (X-Y:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

CONTINUED ON PAGE 2



PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	8.0	Edge	
B	TMVW-t	MT20	4.0	5.0	1.50	1.50
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	TTWV-m	MT20	5.0	6.0	2.00	1.50
G	TTW+m	MT20	3.0	4.0	2.00	1.25
H	TMVW-t	MT20	3.0	4.0	1.50	1.50
I	TMVW-t	MT20	4.0	4.0	2.00	1.50
J	TS-t	MT20	3.0	6.0		
K	TMVW-t	MT20	4.0	4.0	1.75	1.50
L	TMVW-t	MT20	3.0	5.0	1.50	1.75
N	BMV+p	MT20	2.0	4.0		
O	BMVW1-t	MT20	4.0	4.0	2.25	2.00
P	BMVW-t	MT20	4.0	4.0	2.25	2.00
Q	BS-t	MT20	6.0	8.0		
R	BMVW-t	MT20	4.0	4.0	2.25	2.00
S	BMVWV-t	MT20	5.0	8.0		
T	BS-t	MT20	6.0	8.0		
U	BMVW-t	MT20	4.0	4.0	2.25	1.75
V	BMVW-t	MT20	4.0	4.0	2.25	2.00
W	BMVW-t	MT20	5.0	5.0		
X	BMVW-t	MT20	6.0	10.0	5.50	1.75
Y	BMV1+t	MT20	6.0	10.0	7.25	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 7139.4 lbs FACTORED DOWN AT 1-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	1-9-8	-7139	-7139	—	BACK	VERT	TOTAL

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

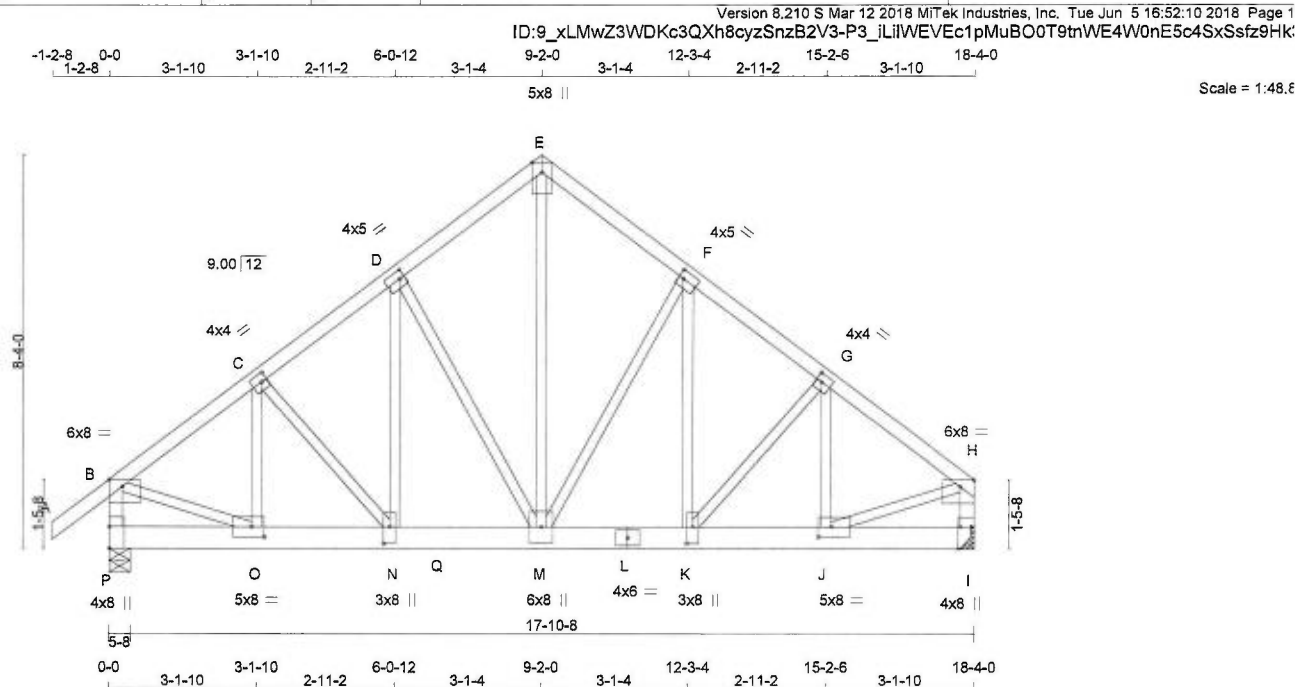
JSI GRIP= 0.90 (W) (INPUT = 0.90)

JSI METAL= 0.62 (X) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
P - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - L	2x6	DRY	No.2
L - I	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - E	12	TOP
E - H	12	TOP
P - B	12	TOP
I - H	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P - L	8	SIDE(334.1)
L - I	8	SIDE(334.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX
P	5403	0	5403	0	0	5-8	5-8		
I	7140	0	7140	0	0	MECHANICAL			

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
JT	COMBINED	1ST LCASE	SNOW	MAX./MIN. LIVE	PERM. LIVE	WIND	DEAD
P	3794	2843	0	0/0	0/0	0/0	1150/0
I	5016	3478	0	0/0	0/0	0/0	1538/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A - B	0 / 30	-77.4 -77.4	0.05 (1)	M - E	0 / 6221	0.77 (1)	
B - C	-5910 / 0	-77.4 -77.4	0.19 (1)	M - F	-1970 / 0	0.72 (1)	
C - D	-6744 / 0	-77.4 -77.4	0.22 (1)	K - F	0 / 2209	0.27 (1)	
D - E	-5385 / 0	-77.4 -77.4	0.18 (1)	K - G	-509 / 0	0.09 (1)	
E - F	-5387 / 0	-77.4 -77.4	0.18 (1)	J - G	0 / 394	0.05 (1)	
F - G	-6584 / 0	-77.4 -77.4	0.21 (1)	D - M	-2223 / 0	0.81 (1)	
G - H	-6996 / 0	-77.4 -77.4	0.25 (1)	N - D	0 / 2537	0.31 (1)	
P - B	-5342 / 0	0.0 0.0	0.30 (1)	C - N	0 / 1006	0.12 (1)	
I - H	-6186 / 0	0.0 0.0	0.35 (1)	O - C	-1511 / 0	0.17 (1)	
				B - O	0 / 4989	0.62 (1)	
				J - H	0 / 5902	0.73 (1)	
P - O	0 / 0	-18.2 -18.2	0.04 (1)				
O - N	0 / 4732	-18.2 -18.2	0.39 (1)				
N - Q	0 / 5389	-18.2 -18.2	0.64 (1)				
Q - M	0 / 5389	-686.5 -686.5	0.64 (1)				
M - L	0 / 5266	-686.5 -686.5	0.63 (1)				
L - K	0 / 5266	-686.5 -686.5	0.63 (1)				
K - J	0 / 5598	-686.5 -686.5	0.53 (1)				
J - I	0 / 0	-686.5 -686.5	0.24 (1)				

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX-	MAX+	FACE
N	6-0-12	-3116	-3116		BACK

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CstdGirder
START DISTANCE = 7-0-0
START SPAN CARRIED = 30-5-0
END DISTANCE = 18-4-0
END SPAN CARRIED = 30-5-0
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.35/1.00 (H-I:1) , BC=0.64/1.00 (M-N:1) , WB=0.81/1.00 (D-M:1) , SS=0.43/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



CONTINUED ON PAGE 2



PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMWW-t	MT20	4.0	5.0	2.00	1.25
E	TTW+p	MT20	5.0	8.0	Edge	
F	TMWW-t	MT20	4.0	5.0	2.00	1.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	6.0	8.0	Edge	3.50
I	BMV1+t	MT20	4.0	8.0	Edge	0.50
J	BMWW-t	MT20	5.0	8.0	2.50	3.25
K	BMWW+t	MT20	3.0	8.0	4.25	1.50
L	BS-t	MT20	4.0	6.0		
M	BMWWW+t	MT20	6.0	8.0		
N	BMWW+t	MT20	3.0	8.0	4.25	1.50
O	BMWW-t	MT20	5.0	8.0	2.50	3.25
P	BMV1+t	MT20	4.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3115.7 lbs FACTORED DOWN AT 6-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

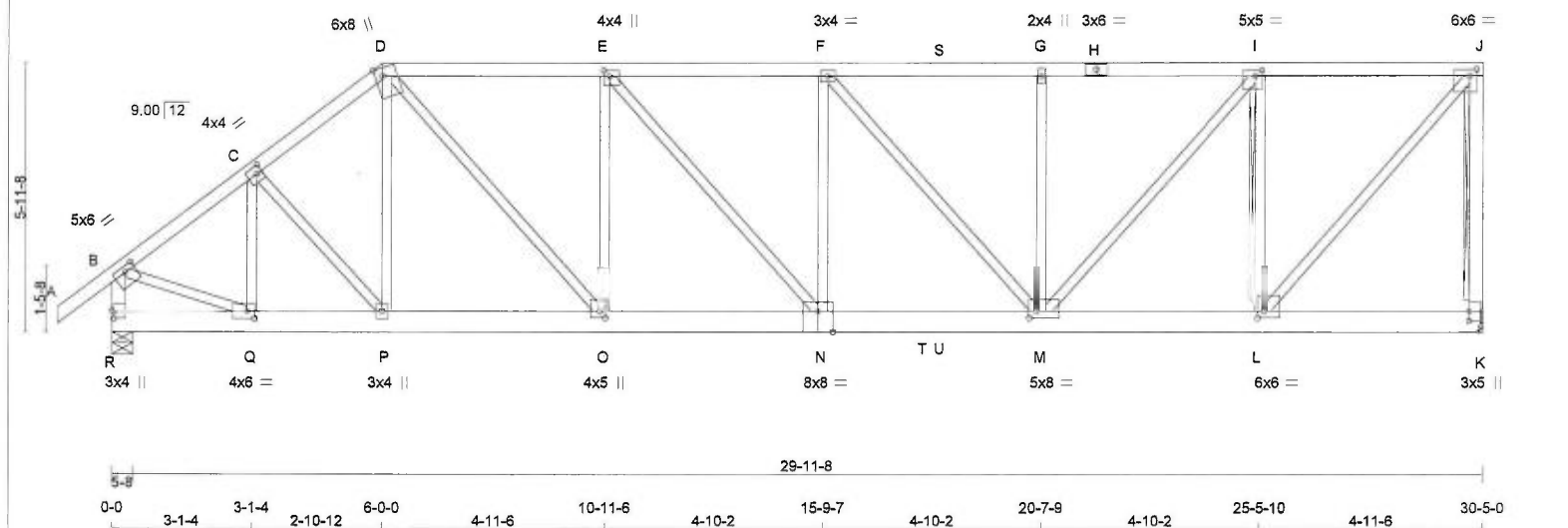
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.79 (L) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
N - K	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	1.50	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	6.0	8.0	Edge	2.00
E	TMVW-t	MT20	4.0	4.0	1.50	1.50
F	TMVW-t	MT20	3.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	5.0	5.0	1.50	1.75
J	TMVW-t	MT20	6.0	6.0	1.75	1.75
K	BMV1+p	MT20	3.0	5.0	2.50	
L	BMVW-t	MT20	6.0	6.0	1.75	1.75
M	BMVW-t	MT20	5.0	8.0	1.75	2.00
N	BSW-t	MT20	8.0	8.0	Edge	4.00
O	BMVW-t	MT20	4.0	5.0	1.75	1.50
P	BMVW-t	MT20	3.0	4.0		
Q	BMVW-t	MT20	4.0	6.0	2.00	2.00
R	BMV1+p	MT20	3.0	4.0	2.00	0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
K	3116	0	3116	0
R	2333	0	2333	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
K	2189	1518 / 0
R	1637	1148 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.42 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT J-K, I-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	WEBS
	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE	FORCE
	(LBS)	(LBS)
FR-TO	FROM	TO
A-B	0 / 30	-77.4 -77.4 0.10 (1)
B-C	-2373 / 0	-77.4 -77.4 0.19 (1)
C-D	-2674 / 0	-77.4 -77.4 0.21 (1)
D-E	-3359 / 0	-77.4 -77.4 0.53 (1)
E-F	-4296 / 0	-77.4 -77.4 0.67 (1)
F-S	-4029 / 0	-77.4 -77.4 0.69 (1)
S-G	-4029 / 0	-145.9 -145.9 0.69 (1)
G-H	-4029 / 0	-145.9 -145.9 0.95 (1)
H-I	-4029 / 0	-145.9 -145.9 0.95 (1)
I-J	-2419 / 0	-145.9 -145.9 0.72 (1)
K-J	-3018 / 0	0.0 0.0 0.60 (1)
R-B	-2274 / 0	0.0 0.0 0.25 (1)
R-Q	0 / 0	-18.2 -18.2 0.04 (1)
Q-P	0 / 1909	-18.2 -18.2 0.15 (1)
P-O	0 / 2121	-18.2 -18.2 0.17 (1)
O-N	0 / 3359	-18.2 -18.2 0.38 (1)
N-T	0 / 4296	-18.2 -18.2 0.76 (1)
T-U	0 / 4296	-18.2 -18.2 0.76 (1)
U-M	0 / 4296	-34.4 -34.4 0.76 (1)
M-L	0 / 2419	-34.4 -34.4 0.29 (1)
L-K	0 / 0	-34.4 -34.4 0.06 (4)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	18-0-12	-1425	-1425	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

LEFT SETBACK = 6-0-0

RIGHT SETBACK = 0-0

END SETBACK = 6-0-0

END WALL WIDTH = 5-8

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDTL LOADS BASED ON 55 % OF GSL.

LOADS APPLIED TO FIRST 12-0-0 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")

CALCULATED VERT. DEFL.(LL)= L/999 (0.23")

ALLOWABLE DEFL.(TL)= L/360 (1.01")

CALCULATED VERT. DEFL.(TL)= L/908 (0.40")

CSI: TC=0.95/1.00 (G-I:1), BC=0.76/1.00 (M-N:1), WB=0.90/1.00 (J-L:1), SSI=0.55/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1424.8 lbs FACTORED DOWN AT 18-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

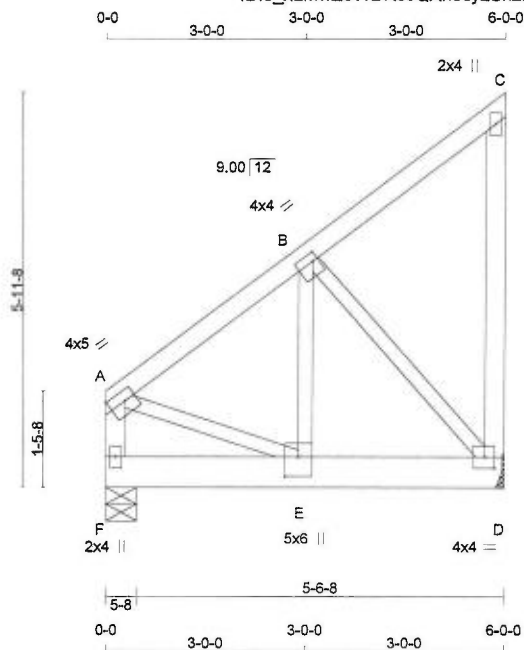
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.69 (N) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-MS6SmOmm6UKG7WHP3xElswBuHHFFvYmQZxYz9Hk



Scale = 1:34.7

TOTAL WEIGHT = 34 lb
[M]

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	4.0	5.0	1.75 Edge
B	TMVW-t	MT20	4.0	4.0	1.50 1.00
C	TMV+p	MT20	2.0	4.0	
D	BMVW1-t	MT20	4.0	4.0	2.00 1.75
E	BMVW+t	MT20	5.0	6.0	3.50 2.50
F	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
D	1425	0	1425	0	0	MECHANICAL
F	1425	0	1425	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
D	1001	694 / 0	0 / 0	0 / 0	0 / 0	307 / 0	0 / 0
F	1001	694 / 0	0 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FACTORED MAX CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	-944 / 0	-77.4	-77.4 0.13 (1)	E-B	0 / 1137	6.21	0.28 (1)
B-C	-15 / 0	-77.4	-77.4 0.11 (1)	B-D	-1140 / 0	6.25	0.36 (1)
D-C	-92 / 0	0.0	0.0 0.06 (1)	A-E	0 / 813	7.81	0.20 (1)
F-A	-935 / 0	0.0	0.0 0.10 (1)				
F-E	0 / 0	-397.7	-397.7 0.23 (1)			10.00	
E-D	0 / 767	-397.7	-397.7 0.33 (1)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder

START DISTANCE = 0-0
START SPAN CARRIED = 18-0-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 18-0-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.13/1.00 (A-B:1), BC=0.33/1.00 (D-E:1), WB=0.36/1.00 (B-D:1), SSI=0.47/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PL)	(PL)	(PL)		
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



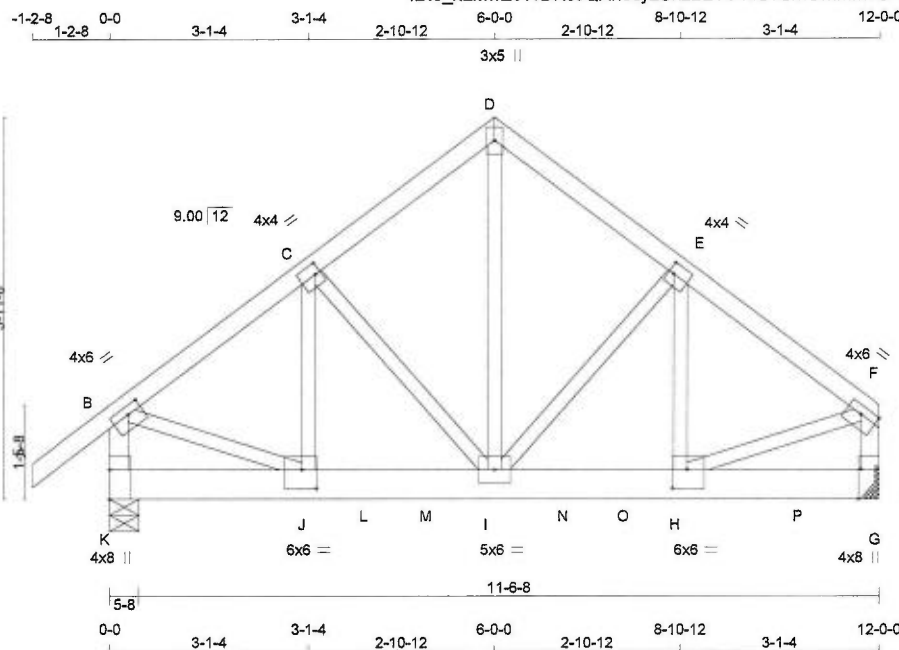
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NEQ618-034	G05	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 1	PAGE 12 OF 61
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MITek Industries, Inc. Tue Jun 5 16:52:12 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-MS6SmOmmm6UKG7WHlp3xEIswBuHHFFfYmQZxYz9Hk

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.30 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:35.5

TOTAL WEIGHT = 2 X 63 = 126 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - G	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A- D	12	TOP
D- F	12	TOP
K- B	12	TOP
G- F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K- G	12	SIDE(204.2)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
K	2362 0	2362 0	0 0	5-8	5-8
G	4771 0	4771 0	0 0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
K	COMBINED	SNOW LIVE PERM.LIVE	0/0	495/0	0/0
G	3352	2324/0 0/0	0/0	1028/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A- B	0/30	-77.4 -77.4	0.05 (1)	I- D	0/2692	0.33 (1)	
B- C	-2449/0	-77.4 -77.4	0.08 (1)	I- E	-1610/0	0.27 (1)	
C- D	-2419/0	-77.4 -77.4	0.07 (1)	H- E	0/1759	0.22 (1)	
D- E	-2415/0	-77.4 -77.4	0.07 (1)	C- I	-59/0	0.01 (1)	
E- F	-3715/0	-77.4 -77.4	0.11 (1)	J- C	-189/0	0.02 (1)	
K- B	-2343/0	0.0 0.0	0.13 (1)	B- J	0/2075	0.26 (1)	
G- F	-3348/0	0.0 0.0	0.19 (1)	H- F	0/3141	0.39 (1)	
K- J	0/0	-18.2 -18.2	0.01 (4)				
J- L	0/1966	-18.2 -18.2	0.27 (1)				
L- M	0/1966	-18.2 -18.2	0.27 (1)				
M- I	0/1966	-231.4 -231.4	0.27 (1)				
I- N	0/2976	-18.2 -18.2	0.49 (1)				
N- O	0/2976	-18.2 -18.2	0.49 (1)				
O- H	0/2976	-426.6 -426.6	0.49 (1)				
H- P	0/0	-426.6 -426.6	0.82 (1)				
P- G	0/0	-18.2 -18.2	0.62 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	4-0-12	-747	-747	—	FRONT	VERT	TOTAL
N	7-1-12	-1519	-1519	—	FRONT	VERT	TOTAL
P	10-9-12	-2292	-2292	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 5-0-0
START SPAN CARRIED = 10-11-0
END DISTANCE = 6-2-8
END SPAN CARRIED = 10-11-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 8-1-0
START SPAN CARRIED = 19-1-0
END DISTANCE = 10-10-8
END SPAN CARRIED = 19-1-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.05")

CSI: TC=0.19/1.00 (F-G:1), BC=0.62/1.00 (G-H:1),
WB=0.39/1.00 (F-H:1), SSI=0.57/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

CONTINUED ON PAGE 2



PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	3.0	5.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW-t	MT20	4.0	6.0	1.50	Edge
G	BMV1+t	MT20	4.0	8.0	Edge	0.50
H	BMWW-t	MT20	6.0	6.0	3.50	2.75
I	BMWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	6.0	6.0	3.50	2.75
K	BMV1+t	MT20	4.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 747.3 lbs FACTORED DOWN AT 4-0-12, AND 1518.7 lbs FACTORED DOWN AT 7-1-12, AND 2291.6 lbs FACTORED DOWN AT 10-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

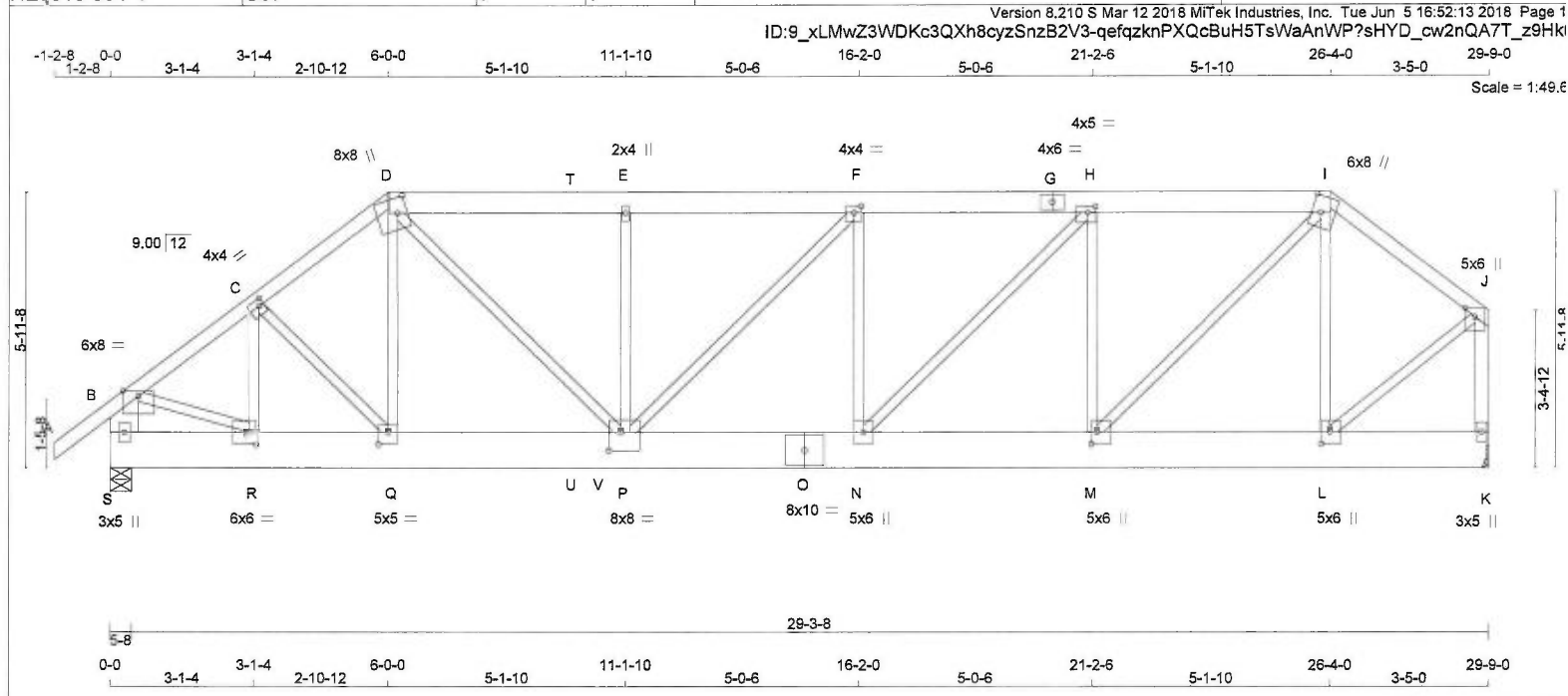
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
S - B	2x8	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - O	2x10	DRY	No.2	SPF
O - K	2x10	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	1.75	1.25
D	TTWW+m	MT20	8.0	8.0	4.00	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0	1.75	1.75
G	TS-t	MT20	4.0	6.0		
H	TMWW-t	MT20	4.0	5.0	1.50	2.00
I	TTWW+m	MT20	6.0	8.0	4.25	2.25
J	TMVW+p	MT20	5.0	6.0	2.25	2.50
K	BMV1+p	MT20	3.0	5.0		
L	BMWW+t	MT20	5.0	6.0	3.00	2.25
M	BMWW+t	MT20	5.0	6.0	3.00	1.50
N	BMWW+t	MT20	5.0	6.0		
O	BS-t	MT20	8.0	10.0		
P	BMWW-t	MT20	8.0	8.0	4.75	3.00
Q	BMWW-t	MT20	5.0	5.0	3.00	2.50
R	BMWW-t	MT20	6.0	6.0	3.00	1.75
S	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
S	3397	0	3397	0
K	2292	0	2292	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2387	1656 / 0	0 / 0	0 / 0	0 / 0	0 / 0	731 / 0	0 / 0
K	1610	1120 / 0	0 / 0	0 / 0	0 / 0	0 / 0	490 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)
FR-TO				FR-TO			
A-B	0 / 30	-77.4	-77.4 0.10 (1)	R-C	-1176 / 0	0.24	(1)
B-C	-3596 / 0	-77.4	-77.4 0.29 (1)	C-Q	0 / 750	0.19	(1)
C-D	-4243 / 0	-77.4	-77.4 0.34 (1)	Q-D	0 / 225	0.08	(4)
D-T	-4884 / 0	-145.9	-145.9 0.45 (1)	D-P	0 / 2144	0.53	(1)
T-E	-4884 / 0	-77.4	-77.4 0.45 (1)	P-E	-576 / 0	0.25	(1)
E-F	-4884 / 0	-77.4	-77.4 0.30 (1)	F-P	0 / 1039	0.26	(1)
F-G	-4172 / 0	-77.4	-77.4 0.25 (1)	N-F	-1112 / 0	0.48	(1)
G-H	-4172 / 0	-77.4	-77.4 0.25 (1)	N-H	0 / 1671	0.41	(1)
H-I	-3027 / 0	-77.4	-77.4 0.20 (1)	M-H	-1674 / 0	0.73	(1)
I-J	-1757 / 0	-77.4	-77.4 0.22 (1)	M-I	0 / 2364	0.59	(1)
S-B	-3194 / 0	0.0	0.0 0.17 (1)	L-I	-942 / 0	0.41	(1)
K-J	-2313 / 0	0.0	0.0 0.38 (1)	B-R	0 / 3003	0.74	(1)
				L-J	0 / 1809	0.45	(1)
S-R	0 / 0	-34.4	-34.4 0.15 (1)				
R-Q	0 / 2886	-34.4	-34.4 0.42 (1)				
Q-U	0 / 3402	-34.4	-34.4 0.67 (1)				
U-V	0 / 3402	-18.2	-18.2 0.87 (1)				
V-P	0 / 3402	-18.2	-18.2 0.87 (1)				
P-O	0 / 4172	-18.2	-18.2 0.45 (1)				
O-N	0 / 4172	-18.2	-18.2 0.45 (1)				
N-M	0 / 3027	-18.2	-18.2 0.33 (1)				
M-L	0 / 1392	-18.2	-18.2 0.19 (1)				
L-K	0 / 0	-18.2	-18.2 0.06 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	6-0-0	-348	-348	---	BACK	VERT	TOTAL
V	10-7-4	-1961	-1961	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-0-0
RIGHT SETBACK = 3-5-0
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO BACK SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 10-0-0 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL)= L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL)= L/999 (0.25")

CSI: TC=0.45/1.00 (D-E:1), BC=0.67/1.00 (P-Q:1), WB=0.74/1.00 (B-R:1), SSI=0.85/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 1	DRWG NO.
NE0618-034	G07	1	1	TRUSS DESC.		PAGE 16 OF 61

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 16:52:13 2018 Page 2
ID:9_xLMwZ3WdKc3QXh8cyzSszB2V3-qefqzknPXQcBuH5TsVvAnWP?sHYD_cw2nQA7T_z9Hk

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 348.1 lbs FACTORED DOWN AT 6-0-0
ON TOP CHORD, AND 1961.4 lbs FACTORED
DOWN AT 10-7-4 ON BOTTOM CHORD. DESIGN
FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

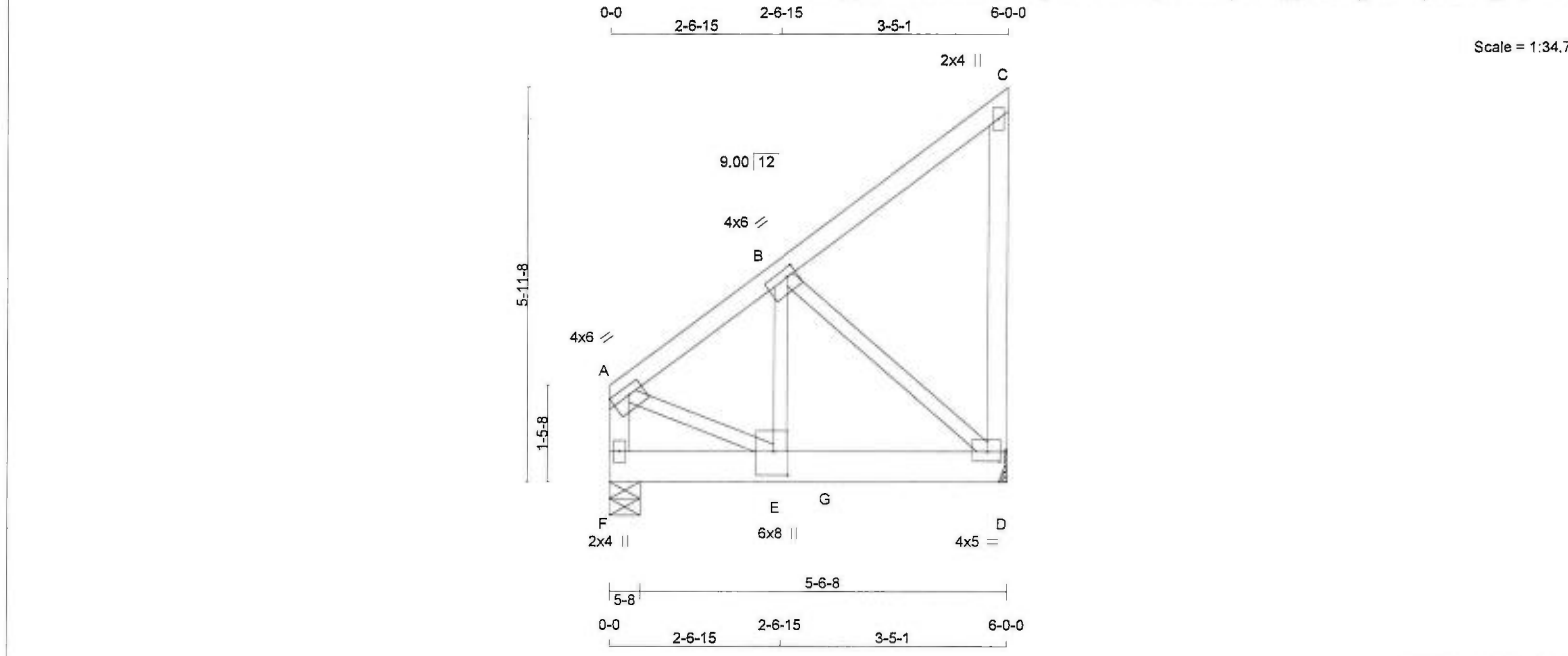
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.72 (R) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	1961	0	1961	0	MECHANICAL	5-8
F	1629	0	1629	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder

START DISTANCE = 3-4-0

START SPAN CARRIED = 19-1-0

END DISTANCE = 6-0-0

END SPAN CARRIED = 19-1-0

END WALL WIDTH = 0-0

APPLIED TO BACK SIDE OF BOTTOM CHORD.

- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")

ALLOWABLE DEFL.(TL)= L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.18/1.00 (A-F:1) , BC=0.56/1.00 (D-E:1) , WB=0.59/1.00 (B-D:1) , SSI=0.44/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+t	MT20	4.0	6.0	1.50	Edge
B	TMVW+t	MT20	4.0	6.0	1.50	1.75
C	TMV+p	MT20	2.0	4.0		
D	BMVW1+t	MT20	4.0	5.0	1.75	2.25
E	BMVW+t	MT20	6.0	8.0	4.25	2.75
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1927.2 lbs FACTORED DOWN AT 2-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1378	955 / 0	0 / 0	0 / 0	0 / 0	423 / 0	0 / 0
F	1144	793 / 0	0 / 0	0 / 0	0 / 0	351 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (LC)	
FR-TO		FROM TO		FR-TO			
A- B	-1681 / 0	-77.4 -77.4	0.15 (1)	4.96	E- B	0 / 1921	
B- C	-12 / 0	-77.4 -77.4	0.12 (1)	6.25	B- D	-1800 / 0	
D- C	-112 / 0	0.0 0.0	0.07 (1)	7.81	A- E	0 / 1466	
F- A	-1643 / 0	0.0 0.0	0.18 (1)	6.42			
F- E	0 / 0	-18.2 -18.2	0.07 (1)	10.00			
E- G	0 / 1358	-18.2 -18.2	0.56 (1)	10.00			
G- D	0 / 1358	-426.6 -426.6	0.56 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	2-4-12	-1927	-1927	—	BACK	VERT	TOTAL

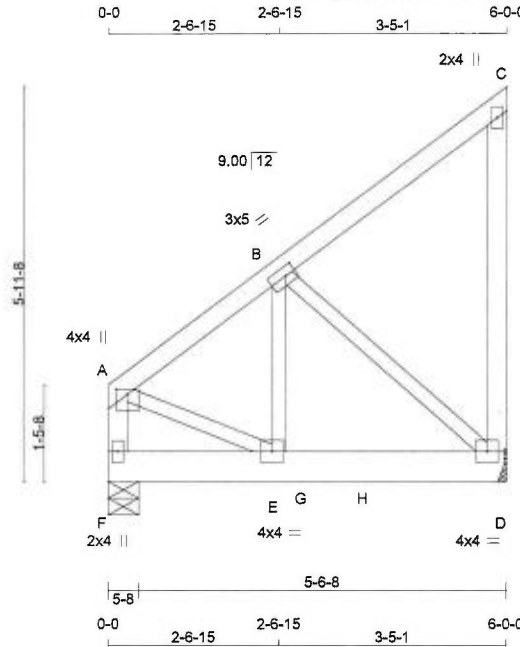
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 1	DRWG NO.
NE0618-034	G08	1	1	TRUSS DESC.		PAGE 18 OF 61

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 16:52:14 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-lqDDb3o1ljk2VRggQE5PJyFlhwCj5dC?4vg?Qz9Hk

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.89 (D) (INPUT = 0.90)
 JSI METAL= 0.43 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.





Scale = 1:34.7

TOTAL WEIGHT = 34 lb
[M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TMWW-t	MT20	3.0	5.0	1.50 1.75
C	TMV+p	MT20	2.0	4.0	
D	BMVW1-t	MT20	4.0	4.0	
E	BMWW-t	MT20	4.0	4.0	
F	BMV1+p	MT20	2.0	4.0	

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 707.4 lbs. FACTORED DOWN AT 2-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
D	1005 0	1005 0	0 0	IN-SX	IN-SX
F	720 0	720 0	0 0	MECHANICAL	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	706	490 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0
F	506	351 / 0	0 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	-734 / 0	-77.4	-77.4 0.13 (1)	E-B	0 / 688	6.25	0.17 (1)
B-C	-14 / 0	-77.4	-77.4 0.12 (1)	B-D	-799 / 0	6.25	0.26 (1)
D-C	-108 / 0	0.0	0.0 0.07 (1)	A-E	0 / 650	7.81	0.16 (1)
F-A	-765 / 0	0.0	0.0 0.09 (1)				
F-E	0 / 0	-18.2	-18.2 0.13 (1)			10.00	
E-G	0 / 603	-18.2	-18.2 0.30 (1)			10.00	
G-H	0 / 603	-18.2	-18.2 0.30 (1)			10.00	
H-D	0 / 603	-231.4	-231.4 0.30 (1)			10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	2-11-12	-707	-707	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 3-11-0
START SPAN CARRIED = 10-11-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 10-11-0
END WALL WIDTH = 0-0
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.13/1.00 (A-B:1), BC=0.30/1.00 (D-E:1), WB=0.26/1.00 (B-D:1), SSI=0.56/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



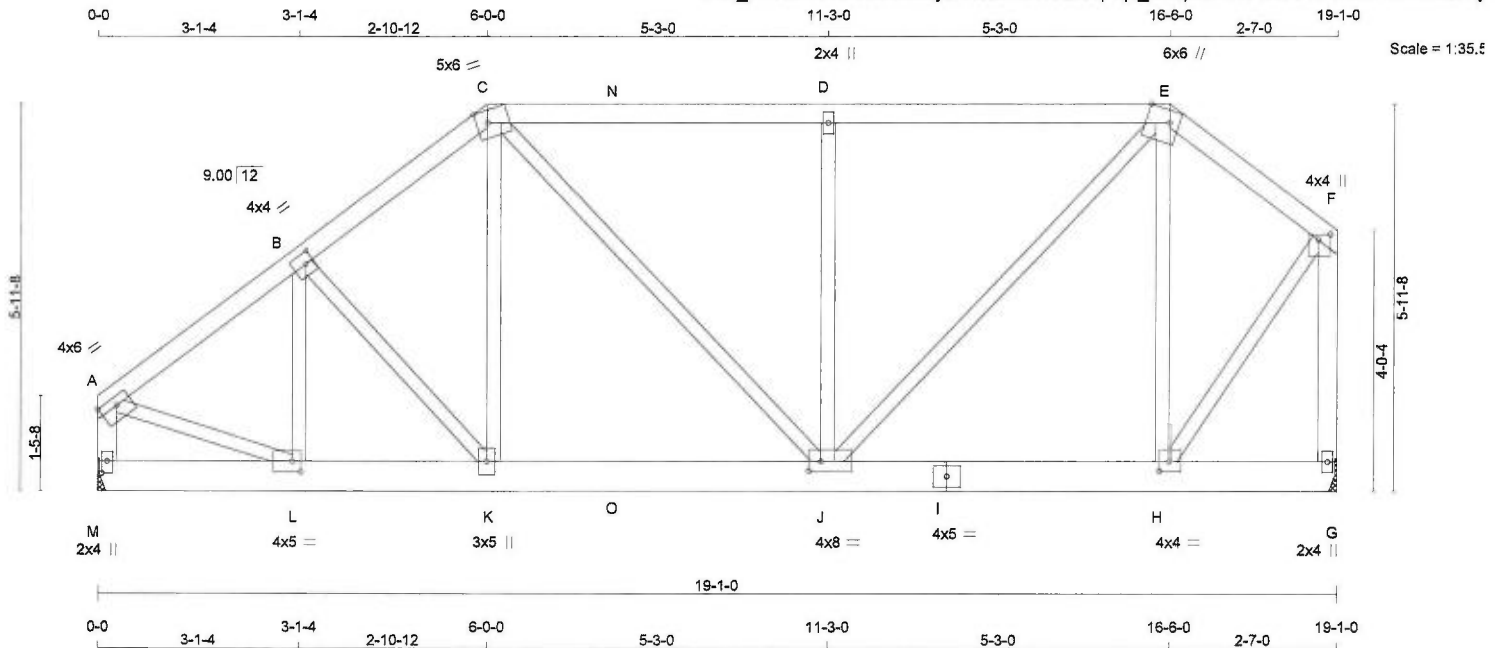
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 1	DRWG NO.
NE0618-034	G08A	1	1	TRUSS DESC.		PAGE 20 OF 61

Version 8.210 S Mar 12 2018 MITek Industries, Inc. Tue Jun 5 16:52:15 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSszB2V3-m1nbOPpf31sv7aFs_xcesxUQJ5JXSdyLEkfDXsz9Hk

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.82 (B) (INPUT = 0.90)
 JSI METAL= 0.23 (E) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 100 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
M - A	2x4	DRY	No.2
G - F	2x4	DRY	No.2
M - I	2x6	DRY	No.2
I - G	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	4.0	6.0	1.50 Edge
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C	TTWW-m	MT20	5.0	6.0	Edge 2.00
D	TMVW-w	MT20	2.0	4.0	
E	TTWW+m	MT20	6.0	6.0	Edge 4.25
F	TMVW+p	MT20	4.0	4.0	1.00 2.25
G	BMV1+p	MT20	2.0	4.0	
H	BMVW-t	MT20	4.0	4.0	1.75 1.75
I	BS-t	MT20	4.0	5.0	
J	BMVWVW-t	MT20	4.0	8.0	1.75 2.25
K	BMVW+t	MT20	3.0	5.0	
L	BMVW-t	MT20	4.0	5.0	2.00 1.50
M	BMV1+p	MT20	2.0	4.0	2.25 1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 6-0-0 ON TOP CHORD, AND 1004.9 lbs FACTORED DOWN AT 8-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
M	1923 0	1923 0	0	MECHANICAL	IN-SX
G	1520 0	1520 0	0	MECHANICAL	IN-SX

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M, G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1354	925 / 0	0 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0	0 / 0
G	1067	746 / 0	0 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	-1969 / 0	-77.4	-77.4 0.16 (1)	L-B	-709 / 0	0.16 (1)	
B-C	-2283 / 0	-77.4	-77.4 0.17 (1)	B-K	0 / 339	0.08 (1)	
C-N	-1710 / 0	-145.9	-145.9 0.51 (1)	K-C	0 / 787	0.19 (1)	
N-D	-1710 / 0	-77.4	-77.4 0.51 (1)	C-J	-167 / 0	0.19 (1)	
D-E	-1710 / 0	-77.4	-77.4 0.51 (1)	J-D	-527 / 0	0.28 (1)	
E-F	-826 / 0	-77.4	-77.4 0.11 (1)	J-E	0 / 1556	0.39 (1)	
M-A	-1823 / 0	0.0	0.0 0.20 (1)	H-E	-889 / 0	0.47 (1)	
G-F	-1484 / 0	0.0	0.0 0.38 (1)	A-L	0 / 1673	0.41 (1)	
				H-F	0 / 1103	0.27 (1)	
M-L	0 / 0	-34.4	-34.4 0.11 (1)				
L-K	0 / 1585	-34.4	-34.4 0.61 (1)				
K-O	0 / 1825	-34.4	-34.4 0.84 (1)				
O-J	0 / 1825	-18.2	-18.2 0.84 (1)				
J-I	0 / 642	-18.2	-18.2 0.30 (1)				
I-H	0 / 642	-18.2	-18.2 0.30 (1)				
H-G	0 / 0	-18.2	-18.2 0.03 (1)				

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	FACE	DIR.	TYPE
C	6-0-0	-348	-348	BACK	VERT	TOTAL
O	8-0-8	-1005	-1005	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-0-0
RIGHT SETBACK = 2-7-0
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO BACK SIDE
- ADDTL LOADS BASED ON 55 % OF GSL
LOADS APPLIED TO FIRST 8-0-0 OF SPAN
MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.51/1.00 (C-D:1), BC=0.84/1.00 (J-K:1),
WB=0.47/1.00 (E-H:1), SSI=0.50/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 1	DRWG NO.
NE0618-034	G09	1	1	TRUSS DESC.		PAGE 22 OF 61

Version 8.210 S Mar 12 2018 MITek Industries, Inc. Tue Jun 5 16:52:16 2018 Page 2
ID:9_xLMwZ3WdKc3QXh8cyzSnzB2V3-EDLzdgHqL_mlkq2Xf7tO81W9VWFB1xUTOn3Jz9Hj

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
		822	2284
			1656

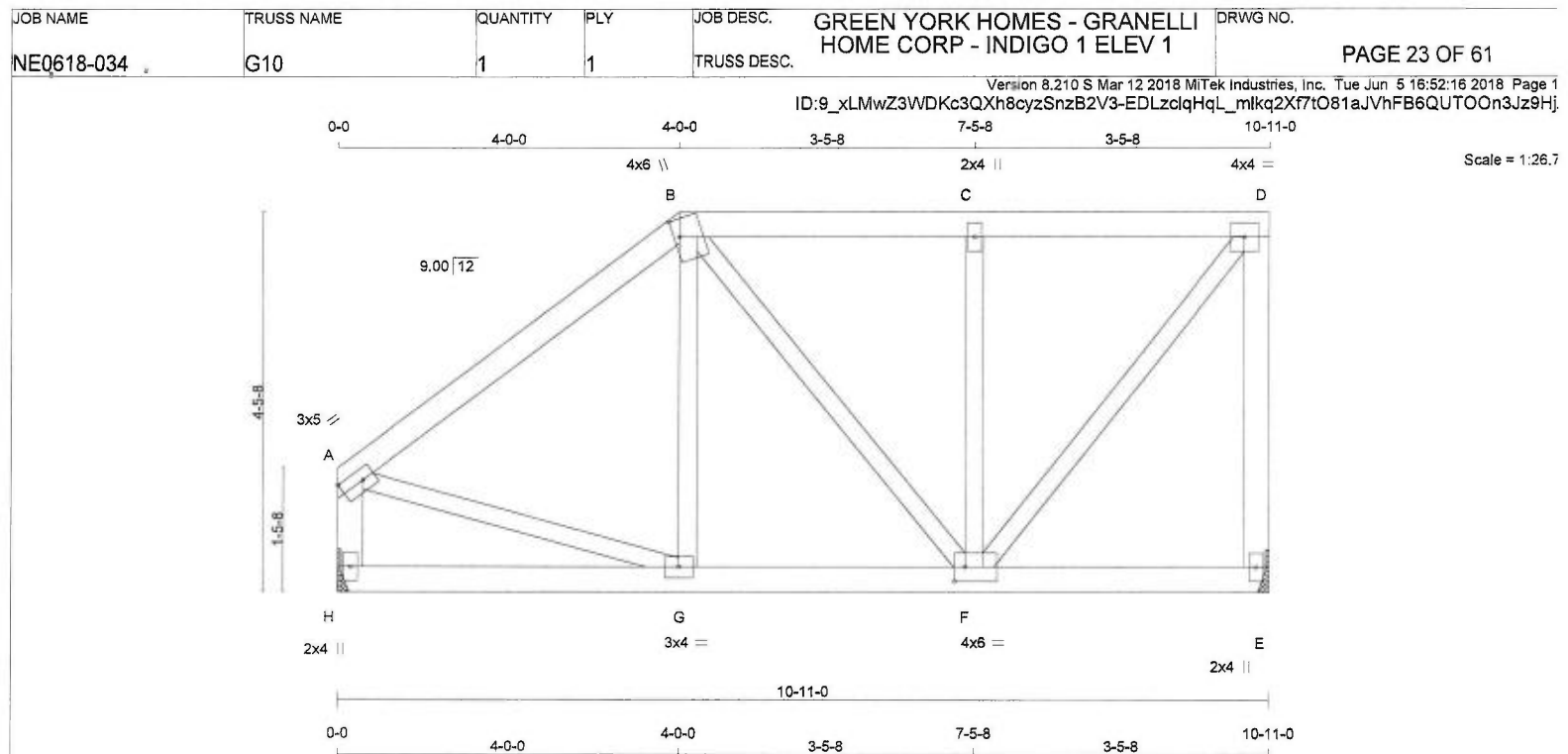
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.51 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					DESIGN CRITERIA				
A - B	2x4	DRY	No.2	SPF	BEARINGS					SPECIFIED LOADS:				
B - D	2x4	DRY	No.2	SPF	FACTORED					TOP CH. LL = 23.3 PSF				
E - D	2x4	DRY	No.2	SPF	GROSS REACTION					DL = 3.0 PSF				
H - A	2x4	DRY	No.2	SPF	DOWN					BOT CH. LL = 0.0 PSF				
H - E	2x4	DRY	No.2	SPF	UP					DL = 7.3 PSF				
ALL WEBS EXCEPT					MECHANICAL					TOTAL LOAD = 33.6 PSF				
DRY: SEASONED LUMBER.					UNFACTORED REACTIONS					SPACING = 24.0 IN. C/C				
					1ST CASE					LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM				
					MAX./MIN. COMPONENT REACTIONS					GIRDER TYPE: CPrimeHip				
					FACTORED					LEFT SETBACK = 4'-0"				
					DOWN					RIGHT SETBACK = 0'-0"				
					UP					END SETBACK = 4'-0"				
					MECHANICAL					END WALL WIDTH = 5'-8"				
					MECHANICAL					CORNER FRAMING TYPE: CONVENTIONAL				
					MECHANICAL					END JACK TYPE: CONVENTIONAL				
					MECHANICAL					APPLIED TO BACK SIDE				
					MECHANICAL					- ADDTL LOADS BASED ON 55 % OF GSL				
					MECHANICAL					THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010				
					MECHANICAL					THIS DESIGN COMPLIES WITH:				
					MECHANICAL					- PART 9 OF OBC 2012, BCBC 2012, ABC 2014				
					MECHANICAL					- CSA 086-09				
					MECHANICAL					- TPIC 2011				
					MECHANICAL					(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD				
					MECHANICAL					ALLOWABLE DEFL.(LL)= L/360 (0.36")				
					MECHANICAL					CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")				
					MECHANICAL					ALLOWABLE DEFL.(TL)= L/360 (0.36")				
					MECHANICAL					CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")				
					MECHANICAL					CSI: TC=0.24/1.00 (A-B:1), BC=0.14/1.00 (F-G:1), WB=0.18/1.00 (D-F:1), SSI=0.20/1.00 (C-D:1)				
					MECHANICAL					DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00				
					MECHANICAL					COMPANION LIVE LOAD FACTOR = 0.50				
					MECHANICAL					TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.				
					MECHANICAL					NAIL VALUES				
					MECHANICAL					PLATE GRIP(DRY) SHEAR SECTION				
					MECHANICAL					(PSI) (PLI) (PLI)				
					MECHANICAL					MAX MIN MAX MIN MAX MIN				
					MECHANICAL					MT20 618 354 1667 822 2284 1656				
					MECHANICAL					CONTINUED ON PAGE 2				

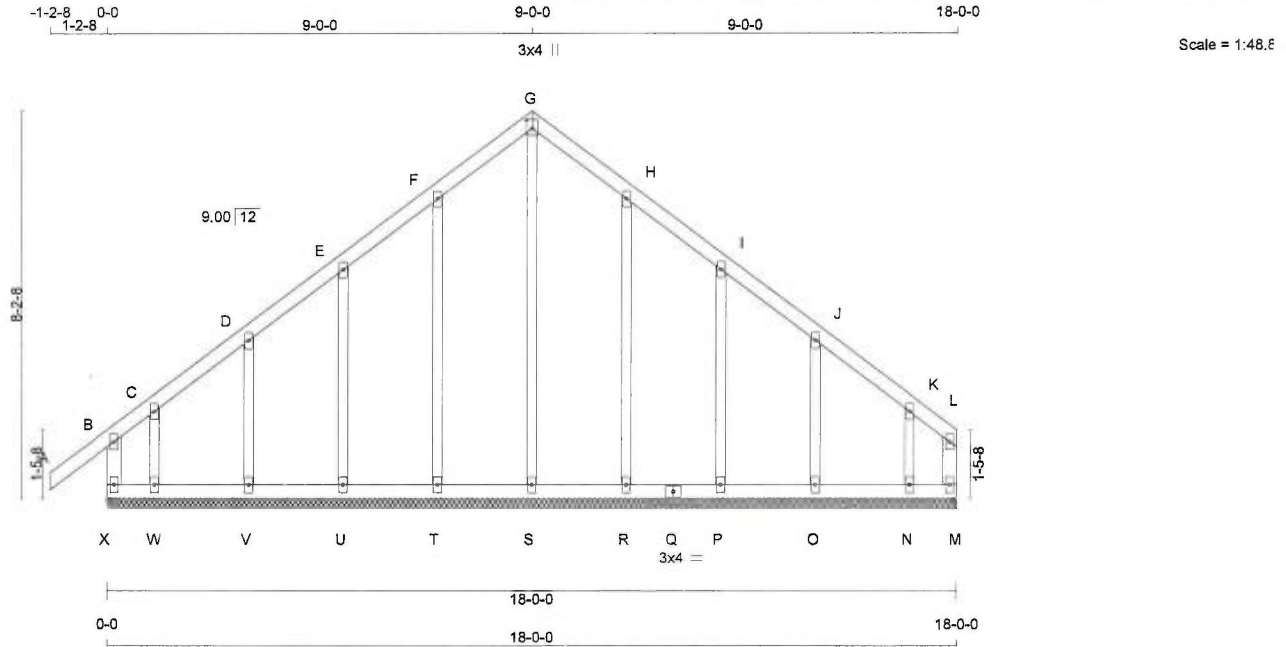
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 1	DRWG NO.
NE0618-034	G10	1	1	TRUSS DESC.		PAGE 24 OF 61

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 16:52:16 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-EDLzclqHqL_mlkq2Xf7tO81aJvhFB6QUTOOn3Jz9Hj

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.21 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 82 lb
[M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
X - B	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - L	2x4	DRY	No.2
M - L	2x4	DRY	No.2
X - Q	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C, D, E, F, H, I, J, K					
C	TMW+w	MT20	2.0	4.0	
G	TTW+p	MT20	3.0	4.0	2.25 1.50
L	TMV+p	MT20	2.0	4.0	
M	BMV1+p	MT20	2.0	4.0	
N, O, P, R, S, T, U, V, W					
N	BMW1+w	MT20	2.0	4.0	
Q	BS-t	MT20	3.0	4.0	
X	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING					
TOTAL LOAD CASES: (4)					
CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO	
X-B	-189 / 0	0.0 0.0 0.02 (1)	7.81	S-G	-167 / 0
A-B	0 / 30	-77.4 -77.4 0.09 (1)	10.00	T-F	-158 / 0
B-C	-38 / 0	-77.4 -77.4 0.08 (1)	6.25	U-E	-149 / 0
C-D	0 / 0	-77.4 -77.4 0.04 (1)	10.00	V-D	-162 / 0
D-E	-2 / 0	-77.4 -77.4 0.04 (1)	10.00	W-C	-54 / 0
E-F	0 / 4	-77.4 -77.4 0.04 (1)	10.00	R-H	-158 / 0
F-G	0 / 5	-77.4 -77.4 0.04 (1)	10.00	P-I	-150 / 0
G-H	0 / 5	-77.4 -77.4 0.04 (1)	10.00	Q-J	-158 / 0
H-I	0 / 4	-77.4 -77.4 0.04 (1)	10.00	N-K	-117 / 0
I-J	-1 / 0	-77.4 -77.4 0.04 (1)	10.00		
J-K	-3 / 0	-77.4 -77.4 0.04 (1)	10.00		
K-L	-5 / 0	-77.4 -77.4 0.02 (1)	10.00		
M-L	-30 / 0	0.0 0.0 0.01 (1)	7.81		
X-W	0 / 7	-18.2 -18.2 0.03 (1)	10.00		
W-V	0 / 5	-18.2 -18.2 0.02 (4)	10.00		
V-U	0 / 0	-18.2 -18.2 0.02 (4)	10.00		
U-T	-2 / 0	-18.2 -18.2 0.01 (4)	10.00		
T-S	-4 / 0	-18.2 -18.2 0.01 (4)	10.00		
S-R	-4 / 0	-18.2 -18.2 0.01 (4)	10.00		
R-Q	-2 / 0	-18.2 -18.2 0.01 (4)	10.00		
Q-P	-2 / 0	-18.2 -18.2 0.01 (4)	10.00		
P-O	0 / 0	-18.2 -18.2 0.02 (4)	10.00		
O-N	0 / 5	-18.2 -18.2 0.02 (4)	10.00		
N-M	0 / 10	-18.2 -18.2 0.01 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (A-B:1), BC=0.03/1.00 (W-X:1), WB=0.21/1.00 (G-S:1), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

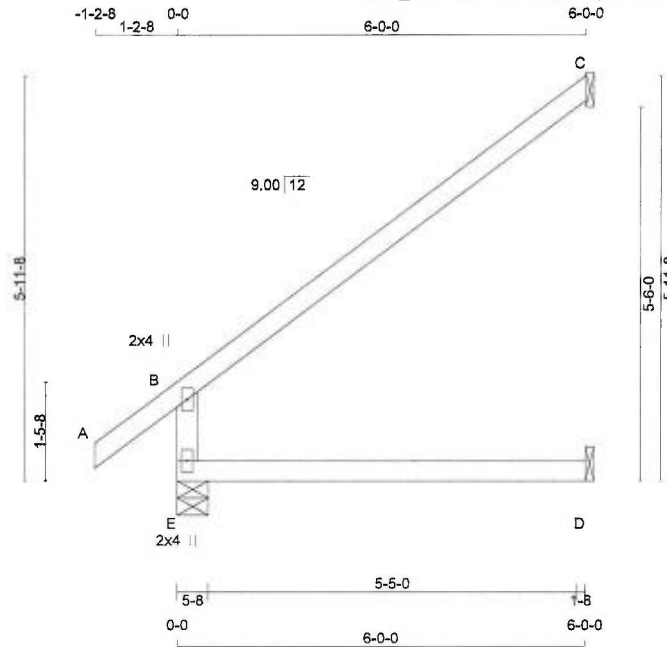
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (G) (INPUT = 0.90)
JSI METAL= 0.06 (G) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:33.5

TOTAL WEIGHT = 10 X 19 = 185 lb
[M][F]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
E	454	0	0	454	0	0	0	5-8	5-8
C	174	0	0	174	0	0	0	1-8	1-8
D	46	0	0	52	0	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	316	235 / 0	0 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC) (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH FR-TO	MAX. FACTORED (LC)
FR-TO				FR-TO			
E-B	-390 / 0	0.0	0.0 0.11 (4)	7.81			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00			
B-C	-35 / 0	-77.4	-77.4 0.37 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.3 PSF	
TOTAL LOAD = 33.6 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.37/1.00 (B-C:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

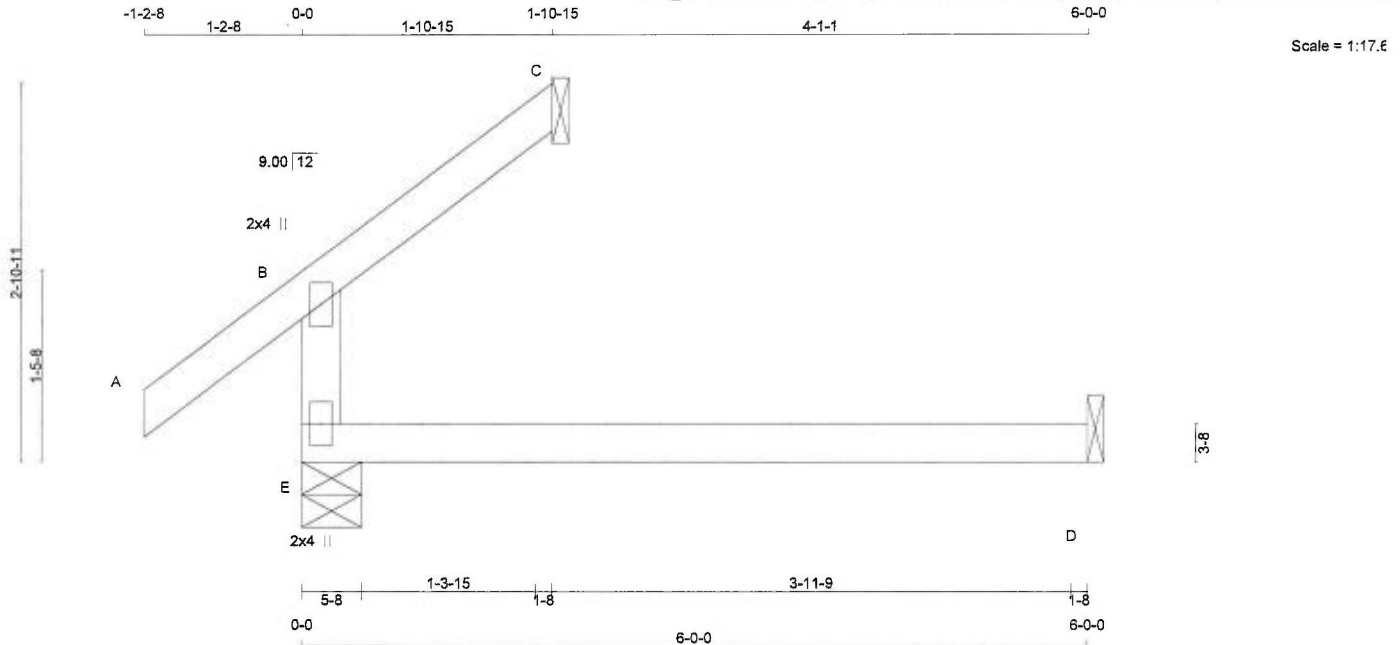
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



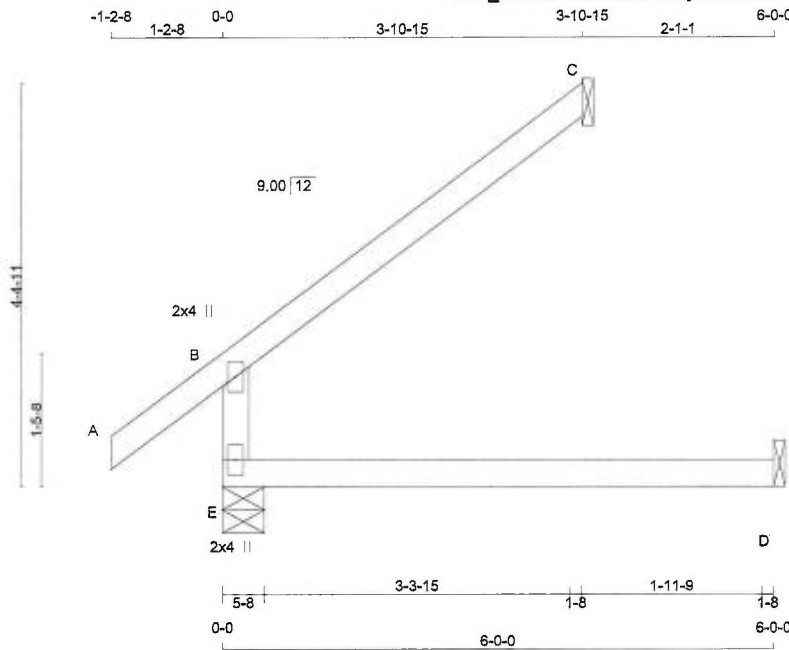


LUMBER N. L. G. A. RULES CHORDS SIZE E - B 2x4 DRY LUMBER No.2 A - C 2x4 DRY LUMBER No.2 E - D 2x4 DRY LUMBER No.2 DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><td></td><td>FACTORED</td><td>MAXIMUM FACTORED</td><td>INPUT</td><td>REQRD</td></tr><tr><td></td><td>GROSS REACTION</td><td>GROSS REACTION</td><td>BRG</td><td>BRG</td></tr><tr><td>JT</td><td>VERT HORZ</td><td>DOWN HORZ</td><td>UPLIFT IN-SX</td><td>IN-SX</td></tr><tr><td>E</td><td>256 0</td><td>256 0</td><td>5-8</td><td>5-8</td></tr><tr><td>C</td><td>56 0</td><td>56 0</td><td>1-8</td><td>1-8</td></tr><tr><td>D</td><td>46 0</td><td>52 0</td><td>1-8</td><td>1-8</td></tr></table>					FACTORED	MAXIMUM FACTORED	INPUT	REQRD		GROSS REACTION	GROSS REACTION	BRG	BRG	JT	VERT HORZ	DOWN HORZ	UPLIFT IN-SX	IN-SX	E	256 0	256 0	5-8	5-8	C	56 0	56 0	1-8	1-8	D	46 0	52 0	1-8	1-8	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 33.6 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04") CSI: TC=0.11/1.00 (B-E:4) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (D-E:4) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.13 (B) (INPUT = 0.90) JSI METAL= 0.05 (B) (INPUT = 1.00)																																																																																						
	FACTORED	MAXIMUM FACTORED	INPUT	REQRD																																																																																																																								
	GROSS REACTION	GROSS REACTION	BRG	BRG																																																																																																																								
JT	VERT HORZ	DOWN HORZ	UPLIFT IN-SX	IN-SX																																																																																																																								
E	256 0	256 0	5-8	5-8																																																																																																																								
C	56 0	56 0	1-8	1-8																																																																																																																								
D	46 0	52 0	1-8	1-8																																																																																																																								
PLATES (table is in inches) <table><tr><td>JT</td><td>TYPE</td><td>PLATES</td><td>W</td><td>LEN</td><td>Y</td><td>X</td></tr><tr><td>B</td><td>TMV+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr><tr><td>E</td><td>BMV1+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr></table>				JT	TYPE	PLATES	W	LEN	Y	X	B	TMV+p	MT20	2.0	4.0			E	BMV1+p	MT20	2.0	4.0			SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D UNFACTORED REACTIONS <table><tr><td></td><td>1ST LCASE</td><td colspan="6">MAX./MIN. COMPONENT REACTIONS</td></tr><tr><td>JT</td><td>COMBINED</td><td>SNOW</td><td>LIVE</td><td>PERM.LIVE</td><td>WIND</td><td>DEAD</td><td>SOIL</td></tr><tr><td>E</td><td>181</td><td>116 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>66 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>38</td><td>34 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>4 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>37</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>37 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><td colspan="4">CHORDS</td><td colspan="4">WEBS</td></tr><tr><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td><td>FACTORED VERT. LOAD (PLF)</td><td>MAX LC1 MAX CSI (LC)</td><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td><td>MAX UNBRAC LENGTH FR-TO</td><td>MAX FACTORED CSI (LC)</td></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>E- B</td><td>-192 / 0</td><td>0.0</td><td>0.0 0.11 (4)</td><td>E- B</td><td>-192 / 0</td><td>7.81</td><td></td></tr><tr><td>A- B</td><td>0 / 30</td><td>-77.4</td><td>-77.4 0.09 (1)</td><td>A- B</td><td>0 / 30</td><td>10.00</td><td></td></tr><tr><td>B- C</td><td>-11 / 0</td><td>-77.4</td><td>-77.4 0.04 (1)</td><td>B- C</td><td>-11 / 0</td><td>6.25</td><td></td></tr><tr><td>E- D</td><td>0 / 0</td><td>-18.2</td><td>-18.2 0.14 (4)</td><td>E- D</td><td>0 / 0</td><td>10.00</td><td></td></tr></table>					1ST LCASE	MAX./MIN. COMPONENT REACTIONS						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	E	181	116 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0	C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0	D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH FR-TO	MAX FACTORED CSI (LC)	FR-TO		FROM TO		FR-TO				E- B	-192 / 0	0.0	0.0 0.11 (4)	E- B	-192 / 0	7.81		A- B	0 / 30	-77.4	-77.4 0.09 (1)	A- B	0 / 30	10.00		B- C	-11 / 0	-77.4	-77.4 0.04 (1)	B- C	-11 / 0	6.25		E- D	0 / 0	-18.2	-18.2 0.14 (4)	E- D	0 / 0	10.00	
JT	TYPE	PLATES	W	LEN	Y	X																																																																																																																						
B	TMV+p	MT20	2.0	4.0																																																																																																																								
E	BMV1+p	MT20	2.0	4.0																																																																																																																								
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS																																																																																																																										
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL																																																																																																																					
E	181	116 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0																																																																																																																					
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0																																																																																																																					
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0																																																																																																																					
CHORDS				WEBS																																																																																																																								
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH FR-TO	MAX FACTORED CSI (LC)																																																																																																																					
FR-TO		FROM TO		FR-TO																																																																																																																								
E- B	-192 / 0	0.0	0.0 0.11 (4)	E- B	-192 / 0	7.81																																																																																																																						
A- B	0 / 30	-77.4	-77.4 0.09 (1)	A- B	0 / 30	10.00																																																																																																																						
B- C	-11 / 0	-77.4	-77.4 0.04 (1)	B- C	-11 / 0	6.25																																																																																																																						
E- D	0 / 0	-18.2	-18.2 0.14 (4)	E- D	0 / 0	10.00																																																																																																																						
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.																																																																																																																												

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 16:52:18 2018 Page 1
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-AcTj0RrXLYET_2_Rf4ALUZ6wDIInf2mnwitu7Bz9Hj



Scale = 1:25.1

TOTAL WEIGHT = 2 X 16 = 31 lb
[M]F

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DESCR. SPF
SPF SPF
SPF SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX	
	VERT	HORZ	DOWN	HORZ			
E	353	0	353	0	5-8	5-8	
C	114	0	114	0	1-8	1-8	
D	46	0	52	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	247	174 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING					
TOTAL LOAD CASES: (4)					
MEMB.	CHORDS		WEBS		MAX. MEMB. FORCE (LBS)
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	
FR-TO		FROM	TO		
E-B	-289 / 0	0.0	0.0	0.11 (4)	7.81
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00
B-C	-23 / 0	-77.4	-77.4	0.15 (1)	6.25
E-D	0 / 0	-18.2	-18.2	0.14 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.15/1.00 (B-C:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284

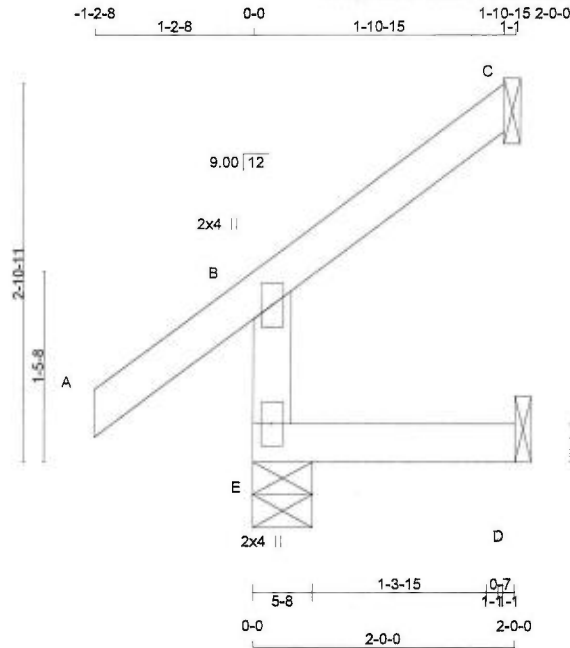
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:17.6

TOTAL WEIGHT = 2 X 8 = 16 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DESCR. SPF

SPF

SPF

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	212	0	212	0	5-8	5-8
C	56	0	56	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	DOWN				
E	147	116 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MAX. MEMB. UNBRACED LENGTH	WEBS		MAX. MEMB. UNBRACED LENGTH
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX		MAX. FACTORED FORCE (LBS)	MAX	
FR-TO			FROM	TO				
E-B	-192 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00			
B-C	-11 / 0	-77.4	-77.4	0.05 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

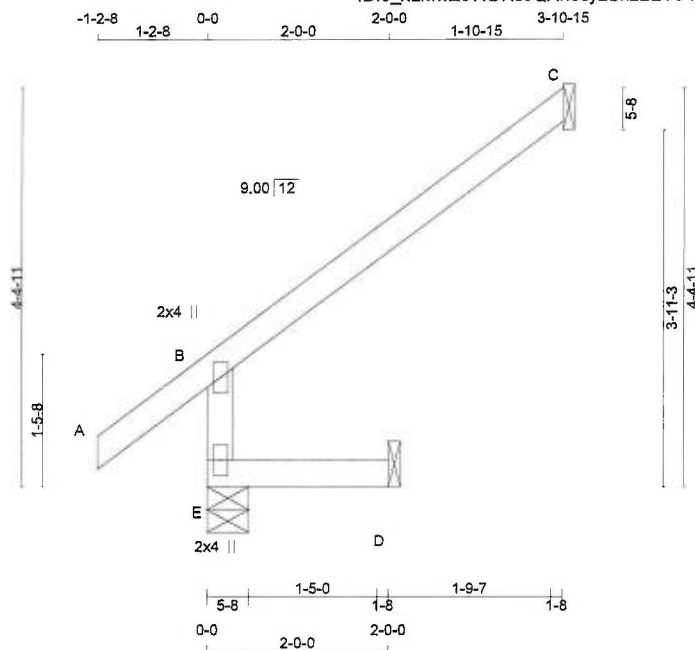
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:25.4

TOTAL WEIGHT = 2 X 11 = 22 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DESCR. SPF SPF SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	UP	BRG	BRG	IN-SX
E	309	0	0	309	0	0	5-8	5-8	5-8
C	114	0	0	114	0	0	1-8	1-8	1-8
D	17	0	0	19	0	0	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	213	174 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FACTORED FORCE (LBS)	LC1 MAX (LC)	UNBRACED LENGTH FR-TO
FR-TO		FROM	TO	FR-TO			
E-B	-289 / 0	0.0	0.0 0.01 (4)	7.81			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00			
B-C	-23 / 0	-77.4	-77.4 0.20 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.20/1.00 (B-C:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	618	354
	1667	822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

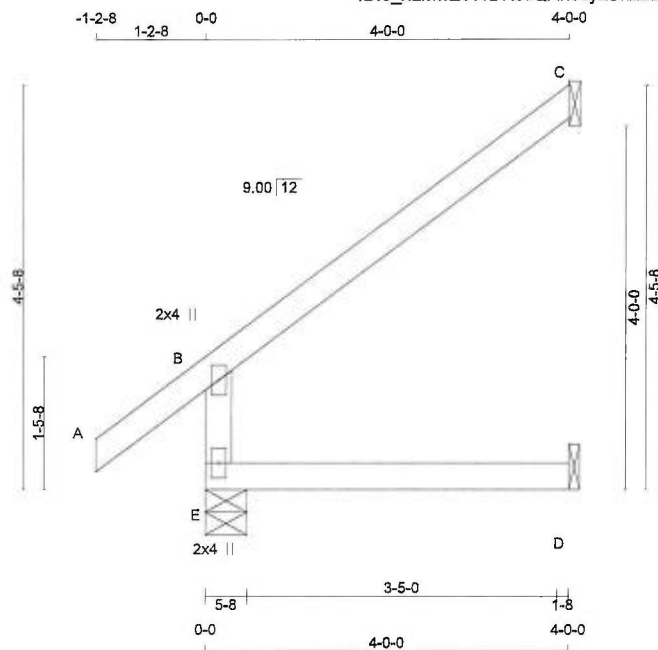
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)

JSI METAL= 0.07 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 4 X 13 = 54 lb
[M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	335	0	335	0	5-8	5-8
C	116	0	116	0	1-8	1-8
D	32	0	35	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	233	177 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
C	79	70 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED (LC)
FR-TO		FROM	TO			
E-B	-294 / 0	0.0	0.0	0.05 (4)	7.81	
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	
B-C	-23 / 0	-77.4	-77.4	0.21 (1)	6.25	
E-D	0 / 0	-18.2	-18.2	0.06 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (B-C:1), BC=0.06/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

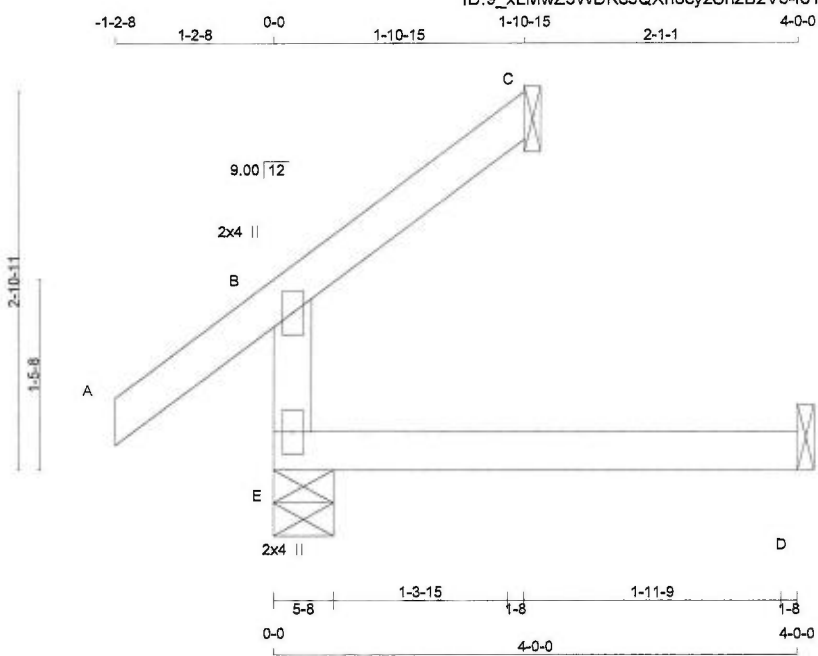
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Version 8.210 S Mar 12 2018 MITek Industries, Inc. Tue Jun 5 16:52:19 2018 Page 1
ID:9_xLMwZ3WdKc3QXh8cyzSnzB2V3-fo16Ens96GMKcCYdDnha0nf6vii7OV0x9MdRfez9Hj



Scale = 1:17.6

TOTAL WEIGHT = 10 lb
[M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DESCR. SPF SPF SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		REQRD BRG IN-SX
	VERT	HORZ	DOWN	UPLIFT	
E	234	0	234	0	5-8
C	56	0	56	0	1-8
D	32	0	35	0	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	164	116 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		FROM	TO	
E-B	-192 / 0	0.0	0.0	0.05 (4)	7.81				
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00				
B-C	-11 / 0	-77.4	-77.4	0.05 (1)	6.25				
E-D	0 / 0	-18.2	-18.2	0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.09/1.00 (A-B:1), BC=0.06/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
	(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX	MIN
MT20	618	354	1667	822 2284 1656

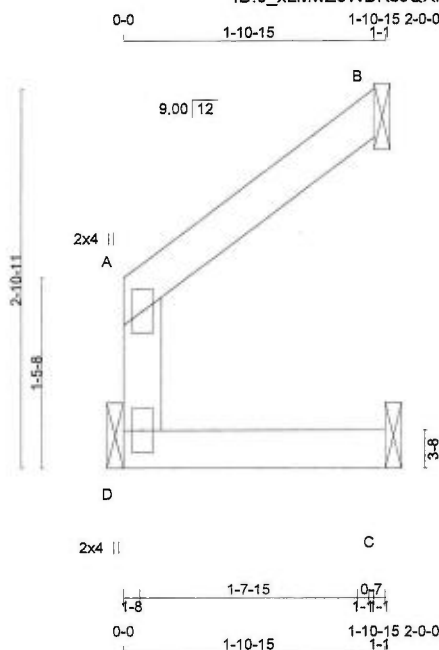
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:17.6

TOTAL WEIGHT = 6 lb
(M)F

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS				SPF	
D - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	2.0	4.0	
D	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	92	0	92	0	1-8	1-8
B	70	0	70	0	1-8	1-8
C	22	0	22	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D, B, C

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND			
D	65	45/0	0/0	0/0	0/0	20/0	0/0	
B	47	41/0	0/0	0/0	0/0	6/0	0/0	
C	17	3/0	0/0	0/0	0/0	14/0	0/0	

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. MEMB. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)
FR-TO		FROM	TO			
D-A	-78/0	0.0	0.0	0.01 (1)	7.81	
A-B	-3/0	-77.4	-77.4	0.04 (1)	10.00	
D-C	0/0	-18.2	-18.2	0.02 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.04/1.00 (A-B:1), BC=0.02/1.00 (C-D:4),
WB=0.00/1.00 (n/a:0), SS=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

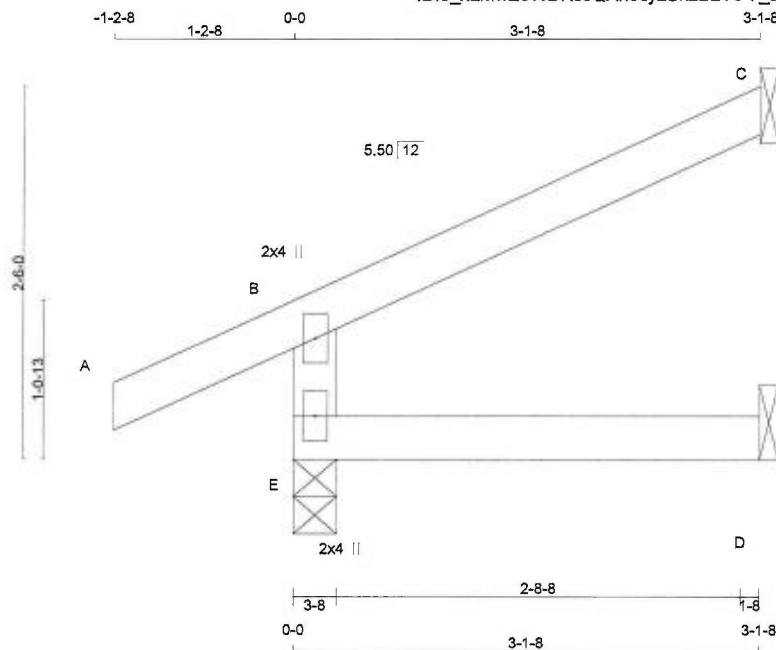
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:15.5

TOTAL WEIGHT = 4 X 10 = 39 lb
[M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DESCR. SPF SPF SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
E	282	0	0	282	0	0	0	3-8	3-8
C	91	0	0	91	0	0	0	1-8	1-8
D	24	0	0	27	0	0	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	195	150/0	0/0	0/0	0/0	45/0	0/0
C	62	55/0	0/0	0/0	0/0	7/0	0/0
D	20	0/0	0/0	0/0	0/0	20/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	LC1 MAX (LC)
FR-TO		FROM	TO				
E-B	-249/0	0.0	0.0 0.03 (4)	E-B	7.81		
A-B	0/20	-77.4	-77.4 0.09 (1)	A-B	10.00		
B-C	-13/0	-77.4	-77.4 0.13 (1)	B-C	6.25		
E-D	0/0	-18.2	-18.2 0.04 (4)	E-D	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.3 PSF	
TOTAL LOAD = 33.6 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (B-C:1), BC=0.04/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822
		2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

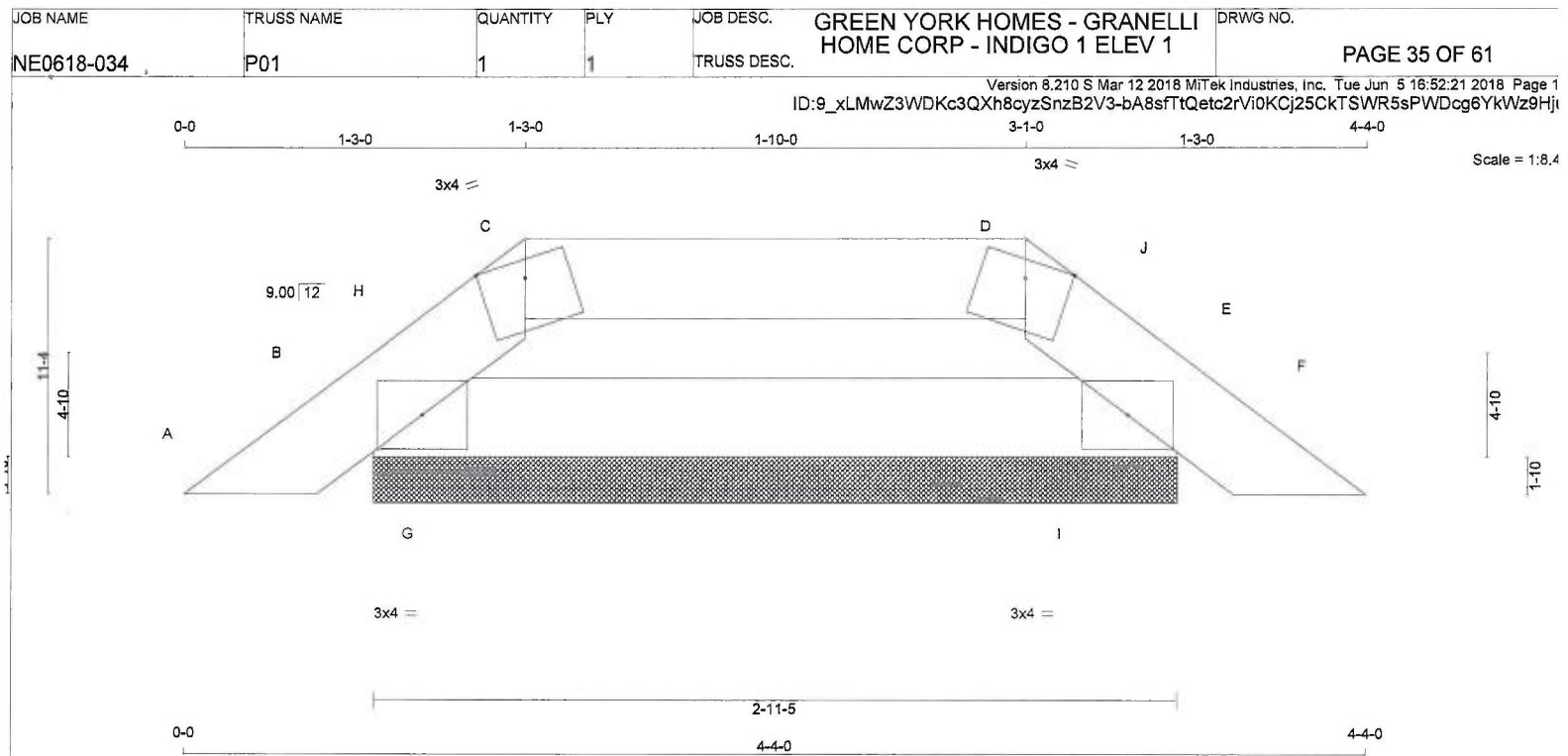
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)

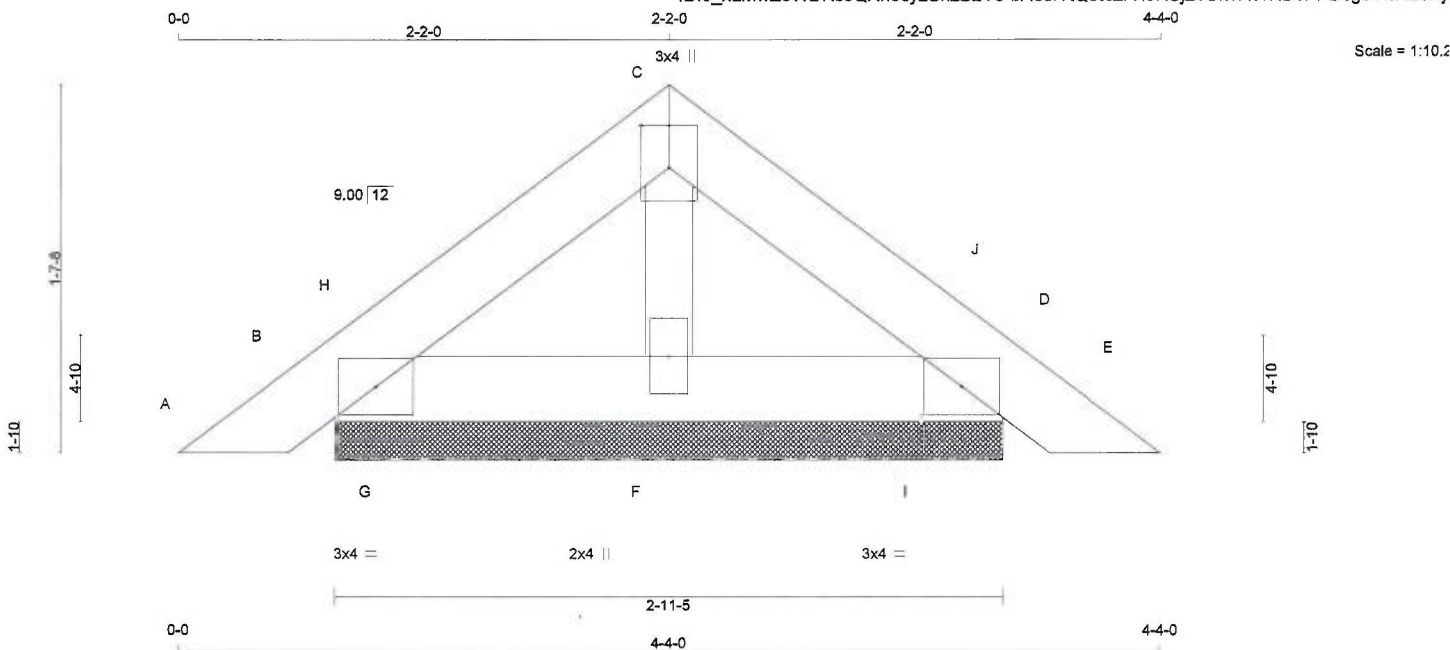
JSI METAL= 0.05 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 9 lb									
[M]									
LUMBER									
N. L. G. A. RULES									
CHORDS SIZE LUMBER DESCR. SPF									
A - C 2x4 DRY No.2 SPF									
C - D 2x4 DRY No.2 SPF									
D - F 2x4 DRY No.2 SPF									
B - E 2x4 DRY No.2 SPF									
DRY: SEASONED LUMBER.									
PLATES (table is in inches)									
JT TYPE PLATES W LEN Y X									
B TMB1-I MT20 3.0 4.0									
C TT-m MT20 3.0 4.0 Edge									
D TT-m MT20 3.0 4.0 Edge									
E TMB1-I MT20 3.0 4.0									
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.									
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED MAXIMUM FACTORED INPUT REQRD									
GROSS REACTION GROSS REACTION BRG BRG									
JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX									
B 188 0 188 0 0 2-11-5 2-11-5									
E 188 0 188 0 0 2-11-5 2-11-5									
UNFACTORED REACTIONS									
1ST LCASE MAX./MIN. COMPONENT REACTIONS									
JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL									
B 131 97 / 0 0 / 0 0 / 0 0 / 0 34 / 0 0 / 0									
E 131 97 / 0 0 / 0 0 / 0 0 / 0 34 / 0 0 / 0									
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS WEBS									
MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) LC1 MAX (LC) UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX (LC)									
FR-TO FROM TO									
A- B 0 / 13 -77.4 -77.4 0.02 (1) 10.00 G- H -39 / 22 0.00 (1)									
B- H -68 / 0 -77.4 -77.4 0.01 (4) 6.25 I- J -39 / 22 0.00 (1)									
H- C -150 / 0 -77.4 -77.4 0.02 (1) 6.25									
C- D -127 / 0 -77.4 -77.4 0.03 (1) 6.25									
D- J -150 / 0 -77.4 -77.4 0.02 (1) 6.25									
J- E -68 / 0 -77.4 -77.4 0.01 (4) 6.25									
E- F 0 / 13 -77.4 -77.4 0.02 (1) 10.00									
B- G 0 / 32 -18.2 -18.2 0.02 (1) 10.00									
G- I 0 / 127 -18.2 -18.2 0.03 (1) 10.00									
I- E 0 / 32 -18.2 -18.2 0.02 (1) 10.00									
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.									
DESIGN CRITERIA									
SPECIFIED LOADS:									
TOP CH. LL = 23.3 PSF									
DL = 3.0 PSF									
BOT CH. LL = 0.0 PSF									
DL = 7.3 PSF									
TOTAL LOAD = 33.6 PSF									
SPACING = 24.0 IN. C/C									
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014									
- CSA 086-09									
- TPIC 2011									
DESIGN ASSUMPTIONS									
-OVERHANG NOT TO BE ALTERED OR CUT OFF.									
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD									
CSI: TC=0.03/1.00 (C-D:1), BC=0.03/1.00 (G-I:1), WB=0.00/1.00 (G-H:1), SSI=0.06/1.00 (C-D:1)									
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10									
COMPANION LIVE LOAD FACTOR = 0.50									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
NAIL VALUES									
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)									
MAX MIN MAX MIN MAX MIN									
MT20 618 354 1667 822 2284 1656									
PLATE PLACEMENT TOL. = 0.250 inches									
PLATE ROTATION TOL. = 5.0 Deg.									
JSI GRIP= 0.13 (E) (INPUT = 0.90)									
JSI METAL= 0.05 (B) (INPUT = 1.00)									
LICENSED PROFESSIONAL ENGINEER									
N.A. EL-MASRI									
APPROVING OF ONTARIO									
Jun 05, 2018									
KOTT									



TOTAL WEIGHT = 6 X 10 = 61 lb
[M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMB1-I	MT20	3.0	4.0		
F	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
B	127	0	127	0	0	2-11-5	2-11-5	2-11-5	2-11-5
D	127	0	127	0	0	2-11-5	2-11-5	2-11-5	2-11-5
F	121	0	121	0	0	2-11-5	2-11-5	2-11-5	2-11-5

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
B	88	70 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	
D	88	70 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	
F	86	55 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 13	-77.4	-77.4 0.02 (1)	10.00	F-C	-54 / 0	0.01 (1)
B-H	-27 / 0	-77.4	-77.4 0.00 (1)	6.25	G-H	-62 / 0	0.00 (1)
H-C	-31 / 0	-77.4	-77.4 0.02 (1)	6.25	I-J	-62 / 0	0.00 (1)
C-J	-31 / 0	-77.4	-77.4 0.02 (1)	6.25			
J-D	-27 / 0	-77.4	-77.4 0.00 (1)	6.25			
D-E	0 / 13	-77.4	-77.4 0.02 (1)	10.00			
B-G	0 / 24	-18.2	-18.2 0.02 (1)	10.00			
G-F	0 / 24	-18.2	-18.2 0.02 (1)	10.00			
F-I	0 / 24	-18.2	-18.2 0.02 (1)	10.00			
I-D	0 / 24	-18.2	-18.2 0.02 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.02/1.00 (D-E:1), BC=0.02/1.00 (B-G:1), WB=0.01/1.00 (C-F:1), SSI=0.04/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

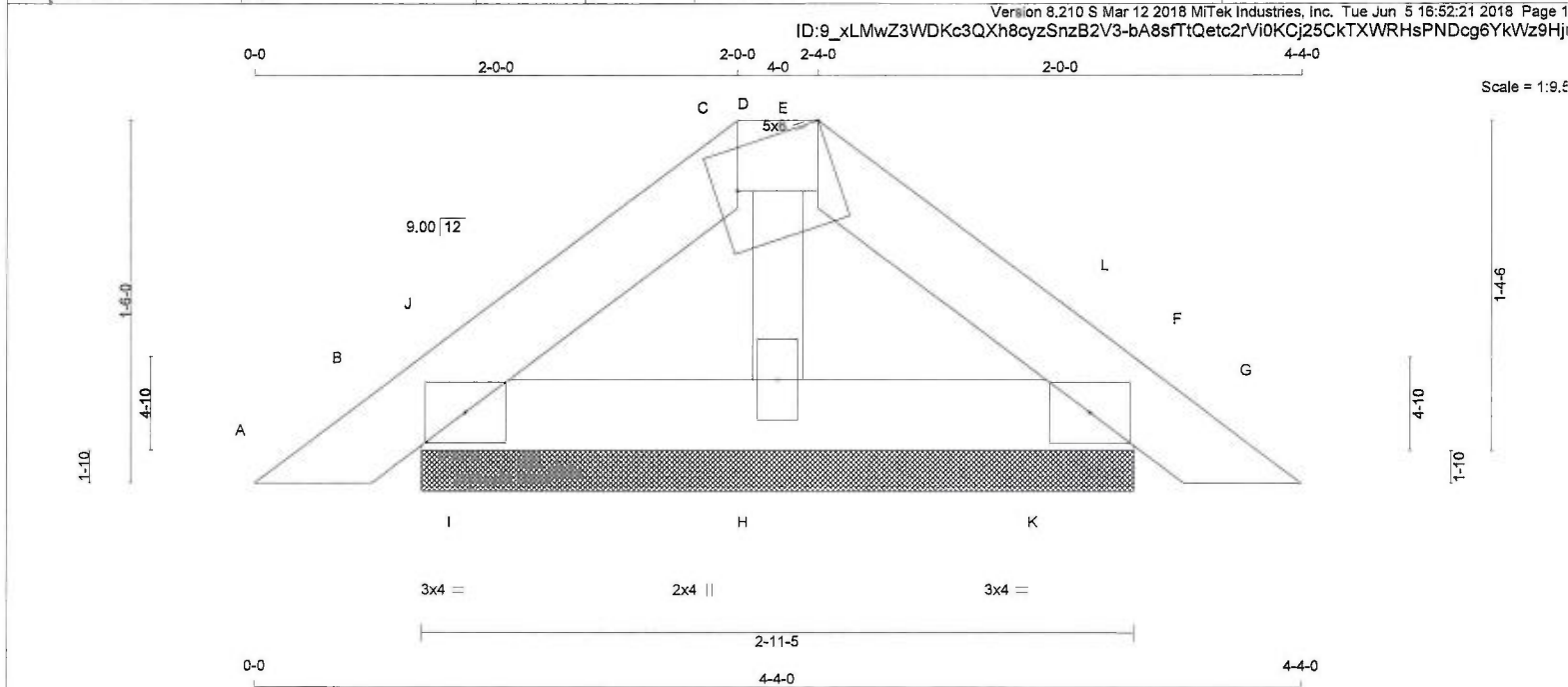
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (D) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

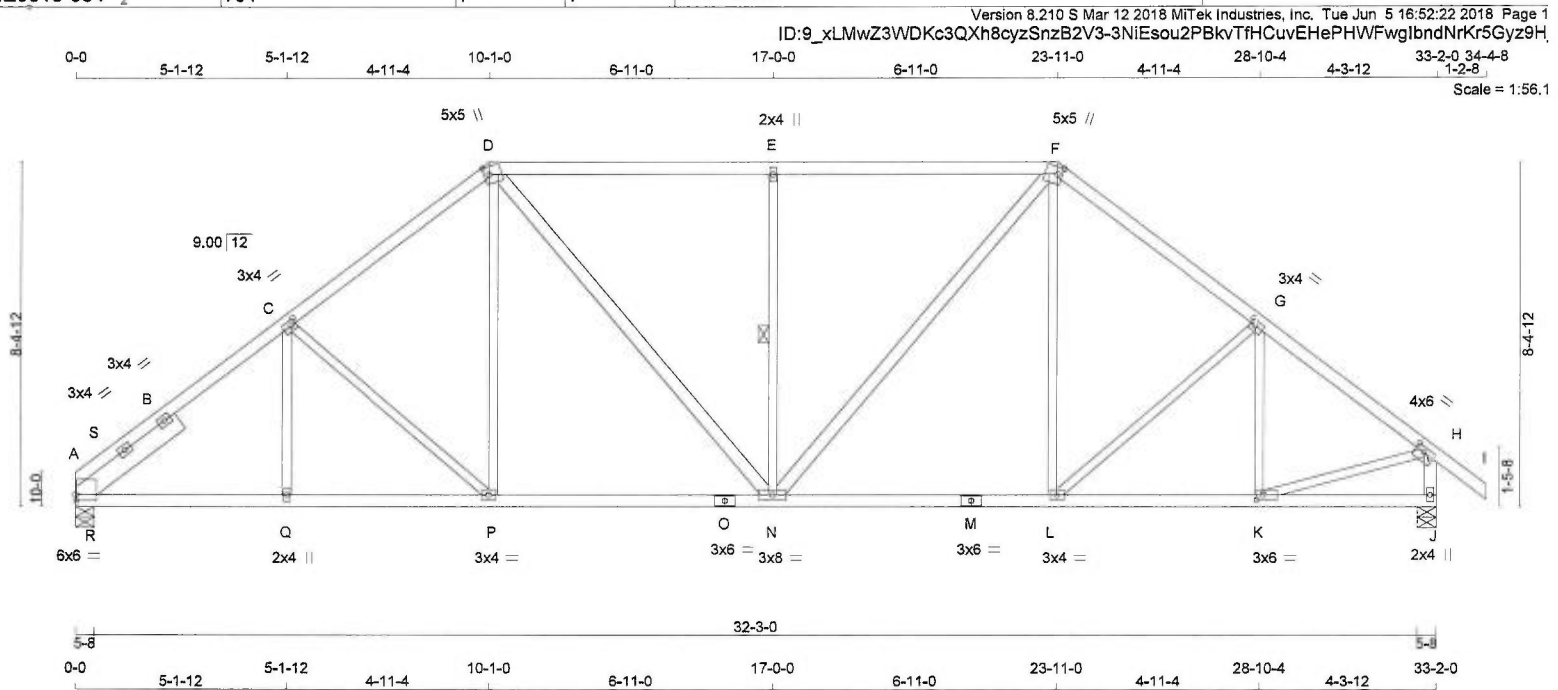




TOTAL WEIGHT = 10 LB									
[M]									
LUMBER									
N. L. G. A. RULES									
CHORDS SIZE DRY LUMBER DESCR.									
A - C 2x4 DRY No.2 SPF									
C - E 2x4 DRY No.2 SPF									
E - G 2x4 DRY No.2 SPF									
B - F 2x4 DRY No.2 SPF									
ALL WEBS 2x3 DRY No.2 SPF									
DRY: SEASONED LUMBER.									
PLATES (table is in inches)									
JT TYPE PLATES W LEN Y X									
B TMB1-I MT20 3.0 4.0									
C, D, E									
C									
C TTTW*-m MT20 5.0 6.0 Edge 5.00									
F TMB1-I MT20 3.0 4.0									
H BMW1+w MT20 2.0 4.0									
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.									
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED MAXIMUM FACTORED INPUT REQD									
GROSS REACTION GROSS REACTION BRG BRG									
JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX									
B 125 0 125 0 0 2-11-5 2-11-5									
F 125 0 125 0 0 2-11-5 2-11-5									
H 126 0 126 0 0 2-11-5 2-11-5									
UNFACTORED REACTIONS									
1ST LCASE MAX./MIN. COMPONENT REACTIONS									
JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL									
B 86 68 / 0 0 / 0 0 / 0 0 / 0 18 / 0 0 / 0									
F 86 68 / 0 0 / 0 0 / 0 0 / 0 18 / 0 0 / 0									
H 89 57 / 0 0 / 0 0 / 0 0 / 0 32 / 0 0 / 0									
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS WEBS									
MAX. FACTORED FACTORED MAX. FACTORED									
MEMB. FORCE VERT. LOAD LC1 MAX. MEMB. FORCE MAX									
(LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC)									
FR-TO FROM TO									
A-B 0 / 13 -77.4 -77.4 0.02 (1) 10.00 H-D -67 / 0 0.01 (1)									
B-J -34 / 0 -77.4 -77.4 0.00 (4) 6.25 I-J -47 / 2 0.00 (1)									
J-C -32 / 0 -77.4 -77.4 0.01 (1) 6.25 K-L -47 / 2 0.00 (1)									
C-D -22 / 0 -77.4 -77.4 0.01 (1) 6.25									
D-E -22 / 0 -77.4 -77.4 0.01 (1) 6.25									
E-L -32 / 0 -77.4 -77.4 0.01 (1) 6.25									
L-F -34 / 0 -77.4 -77.4 0.00 (4) 6.25									
F-G 0 / 13 -77.4 -77.4 0.02 (1) 10.00									
B-I 0 / 25 -18.2 -18.2 0.02 (1) 10.00									
I-H 0 / 25 -18.2 -18.2 0.02 (1) 10.00									
H-K 0 / 25 -18.2 -18.2 0.02 (1) 10.00									
K-F 0 / 25 -18.2 -18.2 0.02 (1) 10.00									
DESIGN CRITERIA									
SPECIFIED LOADS:									
TOP CH. LL = 23.3 PSF									
DL = 3.0 PSF									
BOT CH. LL = 0.0 PSF									
DL = 7.3 PSF									
TOTAL LOAD = 33.6 PSF									
SPACING = 24.0 IN. C/C									
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014									
- CSA 086-09									
- TPIC 2011									
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD									
CSI: TC=0.02/1.00 (F-G:1), BC=0.02/1.00 (H-K:1), WB=0.01/1.00 (D-H:1), SSI=0.03/1.00 (F-K:1)									
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10									
COMPANION LIVE LOAD FACTOR = 0.50									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
NAIL VALUES									
PLATE GRIP(DRY) SHEAR SECTION									
(PSI) (PLI) (PLI)									
MAX MIN MAX MIN MAX MIN									
MT20 618 354 1667 822 2284 1656									
PLATE PLACEMENT TOL. = 0.250 inches									
PLATE ROTATION TOL. = 5.0 Deg.									
JSI GRIP= 0.09 (F) (INPUT = 0.90)									
JSI METAL= 0.02 (F) (INPUT = 1.00)									
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.									

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 153 lb
[M][F]

LUMBER		N. L. G. A. RULES		LUMBER		DESCR	
CHORDS	SIZE						
A - D	2x4	DRY	No.2			SPF	
D - F	2x4	DRY	No.2			SPF	
F - I	2x4	DRY	No.2			SPF	
J - H	2x4	DRY	No.2			SPF	
A - O	2x4	DRY	No.2			SPF	
O - M	2x4	DRY	No.2			SPF	
M - J	2x4	DRY	No.2			SPF	

REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	6.0	6.0	1.75	0.25
A	RT-I	MT20	3.0	4.0		
A	RT-t	MT20	3.0	4.0		
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTVVW+m	MT20	5.0	5.0	2.25	1.50
E	TMW-w	MT20	2.0	4.0		
F	TTVVW+m	MT20	5.0	5.0	2.00	1.50
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TMVV-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	3.0	6.0	1.50	2.00
L	BMWW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWWW-t	MT20	3.0	8.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	3.0	4.0		
Q	BMW-w	MT20	2.0	4.0		

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	1586	0	1586	0	0	5-8	5-8
J	1686	0	1686	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1114	772 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0
J	1182	833 / 0	0 / 0	0 / 0	0 / 0	349 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N. DBS = 20-0-0 . CBF = 82 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	FORCE	MAX.	
	(LBS)	(PLF)	CSI	(LC)	UNBRAC		(LBS)	CSI	(LC)
FR-TO		FROM			LENGTH	FR-TO			
A-S	-2059 / 0	-77.4	-77.4	0.08 (1)	4.67	Q-C	0 / 82	0.03 (4)	
S-B	-1503 / 0	-77.4	-77.4	0.28 (1)	5.07	C-P	-286 / 0	0.23 (1)	
B-C	-1503 / 0	-77.4	-77.4	0.28 (1)	5.07	P-D	0 / 311	0.07 (1)	
C-D	-1741 / 0	-77.4	-77.4	0.31 (1)	4.75	D-N	0 / 462	0.07 (1)	
D-E	-1685 / 0	-77.4	-77.4	0.54 (1)	4.49	N-E	-657 / 0	0.29 (1)	
E-F	-1685 / 0	-77.4	-77.4	0.54 (1)	4.49	N-F	0 / 569	0.09 (1)	
F-G	-1668 / 0	-77.4	-77.4	0.27 (1)	4.90	L-F	0 / 200	0.05 (4)	
G-H	-1738 / 0	-77.4	-77.4	0.25 (1)	4.83	L-G	-131 / 0	0.11 (1)	
H-I	0 / 30	-77.4	-77.4	0.09 (1)	10.00	K-G	-288 / 0	0.09 (1)	
J-H	-1651 / 0	0.0	0.0	0.17 (1)	6.45	K-H	0 / 1460	0.33 (1)	
						R-S	0 / 1241	0.00 (1)	
A-R	0 / 872	-18.2	-18.2	0.30 (1)	10.00	R-B	-1130 / 0	0.10 (1)	
R-Q	0 / 1597	-18.2	-18.2	0.42 (1)	10.00				
Q-P	0 / 1597	-18.2	-18.2	0.35 (1)	10.00				
P-O	0 / 1385	-18.2	-18.2	0.32 (1)	10.00				
O-N	0 / 1385	-18.2	-18.2	0.32 (1)	10.00				
N-M	0 / 1316	-18.2	-18.2	0.31 (1)	10.00				
M-L	0 / 1316	-18.2	-18.2	0.31 (1)	10.00				
L-K	0 / 1413	-18.2	-18.2	0.31 (1)	10.00				
K-J	0 / 0	-18.2	-18.2	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL LOAD				=	33.6 PSF

SPACING = 24.0 IN. C/C

f.
LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.11")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.11")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.54/1.00 (D-E:1), BC=0.42/1.00 (Q-R:1),
WB=0.33/1.00 (H-K:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

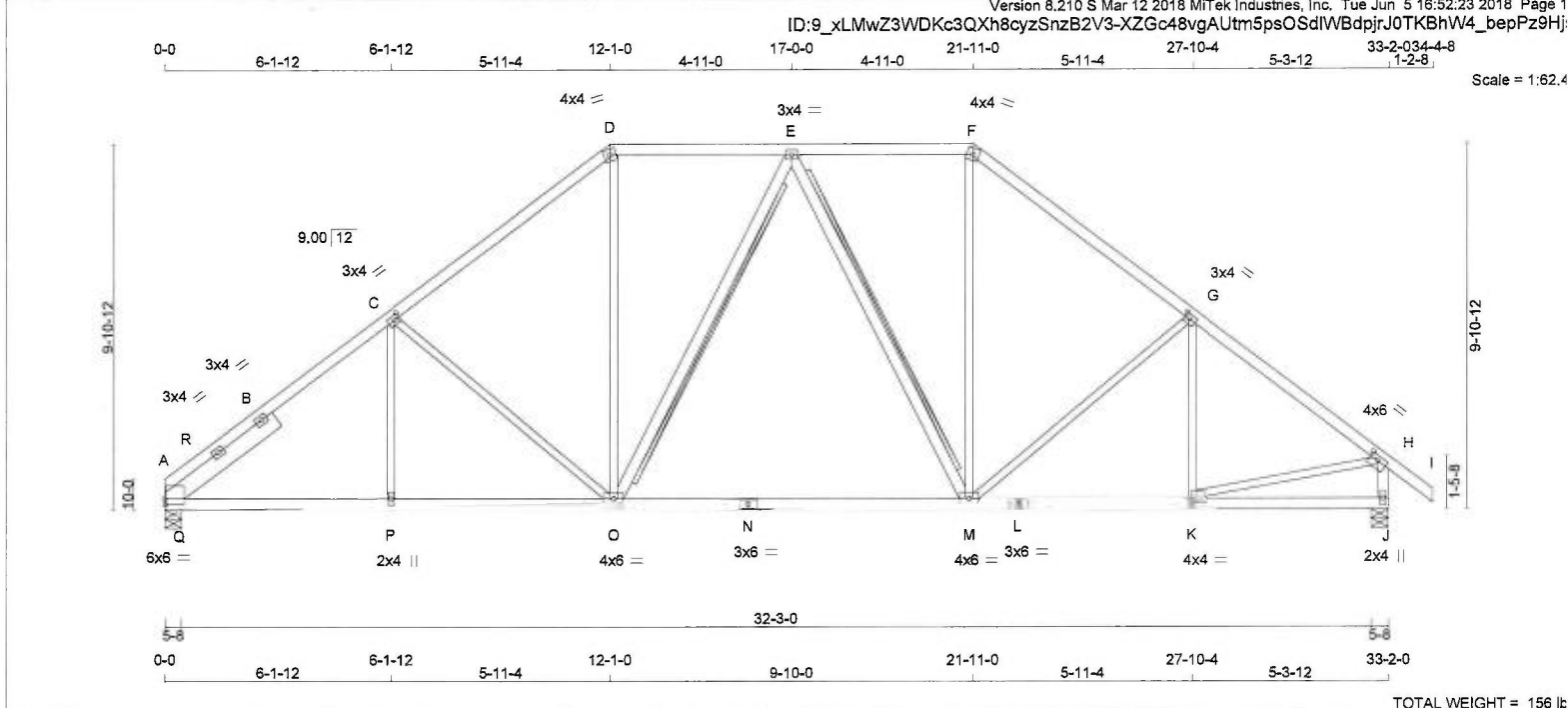
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)

JST METAL= 0.47 (H) (INPUT = 1.00)





LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

A - D

2x4

DRY

No.2

SPF

D - F

2x4

DRY

No.2

SPF

F - I

2x4

DRY

No.2

SPF

J - H

2x4

DRY

No.2

SPF

A - N

2x4

DRY

No.2

SPF

N - L

2x4

DRY

No.2

SPF

L - J

2x4

DRY

No.2

SPF

REINFORCING MEMBERS

HW1

2x6

DRY

No.2

SPF

ALL WEBS

EXCEPT

O - E

2x4

DRY

No.2

SPF

E - M

2x4

DRY

No.2

SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

INPUT

REQRD

GROSS REACTION

GROSS REACTION

BRG

BRG

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

A

1586

0

1586

0

0

5-8

5-8

J

1686

0

1686

0

0

5-8

5-8

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

A

1114

772 / 0

0 / 0

0 / 0

0 / 0

342 / 0

0 / 0

J

1182

833 / 0

0 / 0

0 / 0

0 / 0

349 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-O, E-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.11")

CALCULATED VERT. DEFL.(LL) = L/999 (0.07")

ALLOWABLE DEFL.(TL)= L/360 (1.11")

CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.42/1.00 (C-D:1), BC=0.42/1.00 (O-P:1), WB=0.53/1.00 (C-O:1), SSI=0.23/1.00 (A-Q:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)

JSI METAL= 0.54 (K) (INPUT = 1.00)

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

A

TMBMR1-I

MT20

6.0

6.0

1.75

0.25

A

RT-t

MT20

3.0

4.0

A

RT-t

MT20

3.0

4.0

C

TMWW-t

MT20

3.0

4.0

1.50

1.50

D

TTW-m

MT20

4.0

4.0

E

TMWW-t

MT20

3.0

4.0

F

TTW-m

MT20

4.0

4.0

G

TMWW-t

MT20

3.0

4.0

1.50

1.50

H

TMVV-t

MT20

4.0

6.0

1.50

2.75

J

BMV1+p

MT20

2.0

4.0

K

BMWW-t

MT20

4.0

4.0

1.50

1.50

L

BS-t

MT20

3.0

6.0

M

BMWWW-t

MT20

4.0

6.0

N

BS-t

MT20

3.0

6.0

O

BMWWW-t

MT20

4.0

6.0

P

BMW+w

MT20

2.0

4.0

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.



CHORDS

MEMB.

MAX. FACTORED

FORCE

VERT. LOAD

LC1

MAX

MAX.

MEMB.

MAX. FACTORED

FORCE

MAX

FR-TO

A-R

-2016 / 0

-77.4

-77.4

0.05 (1)

4.75

P-C

0 / 95

0.03 (4)

R-B

-1510 / 0

-77.4

-77.4

0.31 (1)

5.04

C-O

-410 / 0

0.53 (1)

B-C

-1510 / 0

-77.4

-77.4

0.31 (1)

5.04

O-D

0 / 617

0.14 (1)

C-D

-1622 / 0

-77.4

-77.4

0.42 (1)

4.77

O-E

-212 / 0

0.18 (1)

D-E

-1286 / 0

-77.4

-77.4

0.25 (1)

5.41

E-M

-303 / 0

0.25 (1)

E-F

-1245 / 0

-77.4

-77.4

0.25 (1)

5.48

M-F

0 / 603

0.14 (1)

F-G

-1581 / 0

-77.4

-77.4

0.39 (1)

4.85

M-G

-272 / 0

0.35 (1)

G-H

-1775 / 0

-77.4

-77.4

0.37 (1)

4.64

K-G

-213 / 18

0.09 (1)

H-I

0 / 30

-77.4

-77.4

0.09 (1)

10.00

K-H

0 / 1479

0.33 (1)

J-H

-1643 / 0

0.0

0.0

0.17 (1)

6.46

Q-R

0 / 1099

0.00 (1)

A-Q

0 / 882

-18.2

-18.2

0.27 (1)

10.00

Q-B

-1055 / 0

0.12 (1)

Q-P

0 / 1592

-18.2

-18.2

0.39 (1)

10.00

P-O

0 / 1592

-18.2

-18.2

0.42 (1)

10.00

O-N

0 / 1381

-18.2

-18.2

0.39 (1)

10.00

N-M

0 / 1381

-18.2

-18.2

0.39 (1)

10.00

M-L

0 / 1447

-18.2

-18.2

0.39 (1)

10.00

L-K

0 / 1447

-18.2

-18.2

0.39 (1)

10.00

K-J

0 / 0

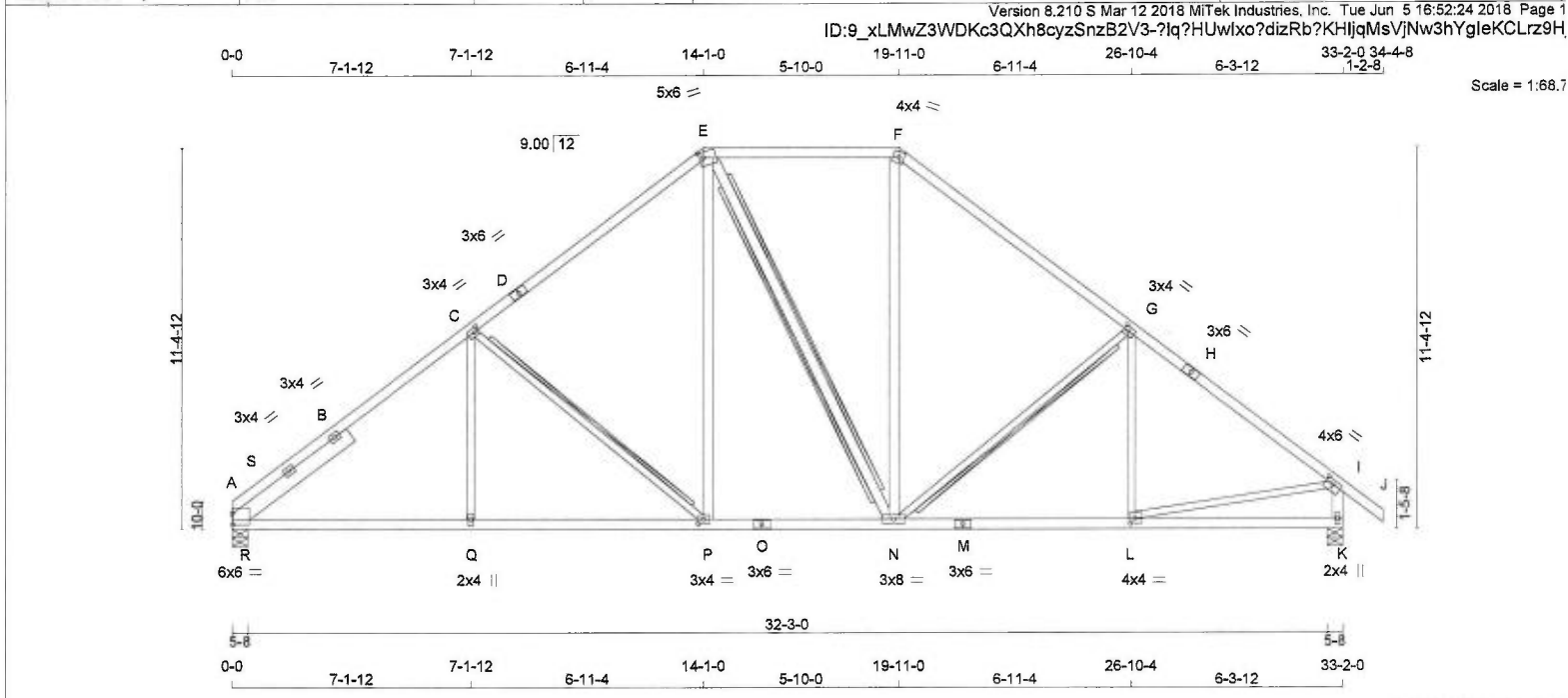
-18.2

-18.2

0.11 (4)

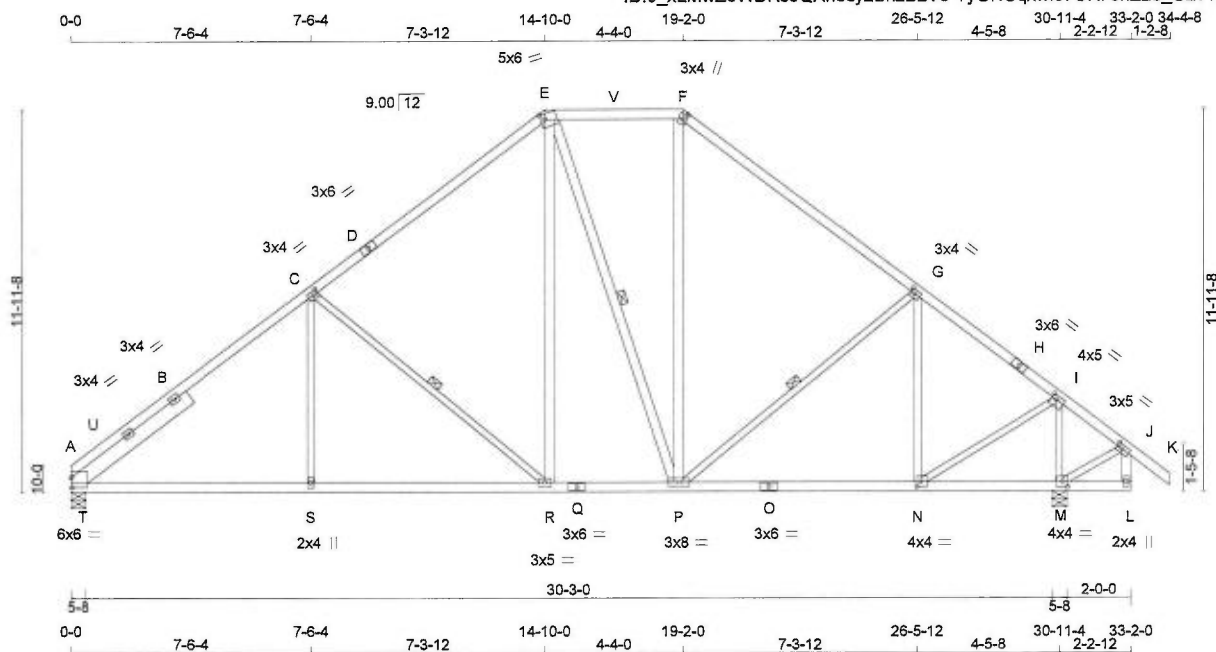
10.00





LUMBER			N. L. G. A. RULES			CHORDS			SIZE			LUMBER			DESCR.		
A - D			2x4			DRY			No.2			SPF			SPF		
D - E			2x4			DRY			No.2			SPF			SPF		
E - F			2x4			DRY			No.2			SPF			SPF		
F - H			2x4			DRY			No.2			SPF			SPF		
H - J			2x4			DRY			No.2			SPF			SPF		
K - I			2x4			DRY			No.2			SPF			SPF		
A - O			2x4			DRY			No.2			SPF			SPF		
O - M			2x4			DRY			No.2			SPF			SPF		
M - K			2x4			DRY			No.2			SPF			SPF		
REINFORCING MEMBERS			HW1			2x6			DRY			No.2			SPF		
ALL WEBS			2x3			DRY			No.2			SPF			SPF		
EXCEPT			P - E			2x4			DRY			No.2			SPF		
E - N			2x4			DRY			No.2			SPF			SPF		
N - F			2x4			DRY			No.2			SPF			SPF		
DRY: SEASONED LUMBER.																	
PLATES (table is in inches)																	
JT		TYPE		PLATES		W		LEN		Y		X					
A		TMBMR1-I		MT20		6.0		6.0		4.00		2.5					
A		RT-t		MT20		3.0		4.0									
A		RT-t		MT20		3.0		4.0									
C		TMWW-t		MT20		3.0		4.0		1.50		1.50					
D		TS-t		MT20		3.0		6.0									
E		TTWW-m		MT20		5.0		6.0		2.00		1.50					
F		TTW-m		MT20		4.0		4.0									
G		TMWW-t		MT20		3.0		4.0		1.50		1.50					
H		TS-t		MT20		3.0		6.0									
I		TMVW-t		MT20		4.0		6.0		1.50		2.75					
K		BMV1+p		MT20		2.0		4.0									
L		BMWW-t		MT20		4.0		4.0		1.50		1.50					
M		BS-t		MT20		3.0		6.0									
N		BMWWW-t		MT20		3.0		8.0									
O		BS-t		MT20		3.0		6.0									
P		BMWW-t		MT20		3.0		4.0		1.50		1.75					
Q		BMW+w		MT20		2.0		4.0									
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.																	
LICENSED PROFESSIONAL ENGINEER																	
N.A. EL-MASRI																	
Jun 05, 2018																	
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER																	
BEARINGS																	
FACTORED			MAXIMUM FACTORED			INPUT			REQD								
GROSS REACTION			GROSS REACTION			BRG			BRG								
JT			VERT			HORZ			UPLIFT			IN-SX					
A			1586			0			1586			0					
K			1686			0			1686			0					
UNFACTORED REACTIONS																	
1ST LCASE			MAX./MIN. COMPONENT REACTIONS														
JT			COMBINED			SNOW			LIVE			PERM.LIVE					
A			1114			772 / 0			0 / 0			0 / 0					
K			1182			833 / 0			0 / 0			0 / 0					
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K																	
BRACING																	
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.																	
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.																	
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.																	
2x4 DRY SPF No.2 T-BRACE AT C-P, G-N																	
2x4 DRY SPF No.2 I-BRACE AT E-N																	
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.																	
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW																	
LOADING																	
TOTAL LOAD CASES: (4)																	
CHORDS			WEBS														
MAX. FACTORED			FACTORED			MAX. FACTORED			MAX. FACTORED			MAX. FACTORED					
MEMB.			FORCE			VERT. LOAD			MEMB.			FORCE					
			(LBS)			(PLF)						(LBS)					
						CSI (LC)						CSI (LC)					
FR-TO			FROM			TO			FR-TO			LENGTH					
A-S			-1947 / 0			-77.4 -77.4			0.07 (1)			4.80					
S-B			-1509 / 0			-77.4 -77.4			0.45 (1)			4.89					
B-C			-1509 / 0			-77.4 -77.4			0.45 (1)			4.89					
C-D			-1478 / 0			-77.4 -77.4			0.54 (1)			4.77					
D-E			-1478 / 0			-77.4 -77.4			0.54 (1)			4.77					
E-F			-1145 / 0			-77.4 -77.4			0.36 (1)			5.51					
F-G			-1462 / 0			-77.4 -77.4			0.55 (1)			4.78					
G-H			-1792 / 0			-77.4 -77.4			0.53 (1)			4.43					
H-I			-1792 / 0			-77.4 -77.4			0.53 (1)			4.43					
I-J			0 / 30			-77.4 -77.4			0.09 (1)			10.00					
K-I			-1640 / 0			0.0 0.0			0.17 (1)			6.47					
A-R			0 / 891			-18.2 -18.2			0.24 (4)			10.00					
R-Q			0 / 1582			-18.2 -18.2			0.34 (1)			10.00					
Q-P			0 / 1582			-18.2 -18.2			0.34 (1)			10.00					
P-O			0 / 1165			-18.2 -18.2			0.27 (1)			10.00					
O-N			0 / 1165			-18.2 -18.2			0.27 (1)			10.00					
N-M			0 / 1465			-18.2 -18.2			0.34 (1)			10.00					
M-L			0 / 1465			-18.2 -18.2			0.34 (1)			10.00					
L-K			0 / 0			-18.2 -18.2			0.20 (4)			10.00					
DESIGN CRITERIA																	
SPECIFIED LOADS:																	
TOP CH.			LL			=			23.3			PSF					
			DL			=			3.0			PSF					
BOT CH.			LL			=			0.0			PSF					
			DL			=			7.3			PSF					
TOTAL LOAD						=			33.6			PSF					
SPACING = 24.0 IN./C																	
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM																	
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010																	
THIS DESIGN COMPLIES WITH:																	
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014																	
- CSA 086-09																	
- TPIC 2011																	
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD																	
ALLOWABLE DEFL.(LL)= L/360 (1.11")																	
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")																	
ALLOWABLE DEFL.(TL)= L/360 (1.11")																	
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")																	
CSI: TC=0.55/1.00 (F-G:1) , BC=0.34/1.00 (Q-R:1) , WB=0.37/1.00 (C-P:1) , SSI=0.24/1.00 (A-R:4)																	
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10																	
COMPANION LIVE LOAD FACTOR = 0.50																	
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .																	
NAIL VALUES																	
PLATE GRIP(DRY)			SHEAR			SECTION											
			(PSI)			(PLI)			(PLI)								
MAX MIN			MAX MIN			MAX MIN			MAX MIN								
MT20			618 354			1667 822			2284 1656								
PLATE PLACEMENT TOL. = 0.250 inches																	
PLATE ROTATION TOL. = 5.0 Deg.																	
JSI GRIP= 0.88 (P) (INPUT = 0.90)																	
JSI METAL= 0.55 (L) (INPUT = 1.00)																	
CONTINUED ON PAGE 2																	
KOTT																	





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
L - J	2x4	DRY	No.2
A - Q	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
REINFORCING MEMBERS			
HW1	2x6	DRY	No.2
ALL WEBS EXCEPT			
R - E	2x4	DRY	No.2
E - P	2x4	DRY	No.2
P - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A TMBMR1-I	MT20	6.0	6.0	4.00	0.25
A RT-t	MT20	3.0	4.0		
A RT-t	MT20	3.0	4.0		
C TMWW-t	MT20	3.0	4.0	1.50	1.50
D TS-t	MT20	3.0	6.0		
E TTWW-m	MT20	5.0	6.0	2.00	1.50
F TTW-m	MT20	3.0	4.0	2.00	1.25
G TMWW-t	MT20	3.0	4.0	1.50	1.50
H TS-t	MT20	3.0	6.0		
I TMWW-t	MT20	4.0	5.0	1.50	1.75
J TMVV-t	MT20	3.0	5.0	1.50	1.75
L BMV-p	MT20	2.0	4.0		
M BMWW1-t	MT20	4.0	4.0	2.00	1.75
N BMWW-t	MT20	4.0	4.0	1.75	1.50
O BS-t	MT20	3.0	6.0		
P BMWWW-t	MT20	3.0	8.0		
Q BS-t	MT20	3.0	6.0		
R BMWW-t	MT20	3.0	5.0		
S BMW-w	MT20	2.0	4.0		

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT		GROSS REACTION		GROSS REACTION		BRG		BRG	
A	1479	0	1479	0	0	5-8		5-8	
M	1825	0	1825	0	0	5-8		5-8	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
A	1041	712 / 0	0 / 0	0 / 0	0 / 0	328 / 0	0 / 0	0 / 0
M	1282	893 / 0	0 / 0	0 / 0	0 / 0	389 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, M

BRACING

FOR SECTION E-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, E-P, G-P. DBS = 20-0-0. CBF = 76 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-U	-1771 / 0	-77.4	-77.4 0.09 (1)	4.96	S-C	0 / 157	0.05 (4)
U-B	-1393 / 0	-77.4	-77.4 0.49 (1)	4.98	C-R	-606 / 0	0.40 (1)
B-C	-1393 / 0	-77.4	-77.4 0.49 (1)	4.98	R-E	0 / 481	0.08 (1)
C-D	-1256 / 0	-77.4	-77.4 0.57 (1)	5.02	E-P	-169 / 0	0.15 (1)
D-E	-1256 / 0	-77.4	-77.4 0.57 (1)	5.02	P-F	0 / 281	0.05 (1)
E-V	-928 / 0	-84.9	-84.9 0.28 (1)	2.00	P-G	-19 / 3	0.01 (1)
V-F	-928 / 0	-84.9	-84.9 0.28 (1)	2.00	N-G	-566 / 0	0.39 (1)
F-G	-1188 / 0	-77.4	-77.4 0.56 (1)	5.14	N-I	0 / 1276	0.29 (1)
G-H	-1126 / 0	-77.4	-77.4 0.42 (1)	5.43	M-I	-1680 / 0	0.31 (1)
H-I	-1126 / 0	-77.4	-77.4 0.42 (1)	5.43	M-J	-158 / 0	0.03 (1)
I-J	0 / 184	-77.4	-77.4 0.08 (1)	10.00	T-U	0 / 780	0.00 (1)
J-K	0 / 30	-77.4	-77.4 0.09 (1)	10.00	T-B	-866 / 0	0.14 (1)
L-J	0 / 14	0.0	0.0 0.00 (4)	10.00			
A-T	0 / 840	-18.2	-18.2 0.24 (4)	10.00			
T-S	0 / 1446	-18.2	-18.2 0.35 (1)	10.00			
S-R	0 / 1446	-18.2	-18.2 0.35 (1)	10.00			
R-Q	0 / 985	-18.2	-18.2 0.25 (1)	10.00			
Q-P	0 / 985	-18.2	-18.2 0.25 (1)	10.00			
P-O	0 / 939	-18.2	-18.2 0.24 (1)	10.00			
O-N	0 / 939	-18.2	-18.2 0.24 (1)	10.00			
N-M	-141 / 0	-18.2	-18.2 0.13 (4)	6.25			
M-L	0 / 0	-18.2	-18.2 0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.57/1.00 (C-E-1), BC=0.35/1.00 (S-T-1), WB=0.40/1.00 (C-R-1), SSI=0.24/1.00 (A-T-4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

CONTINUED ON PAGE 2



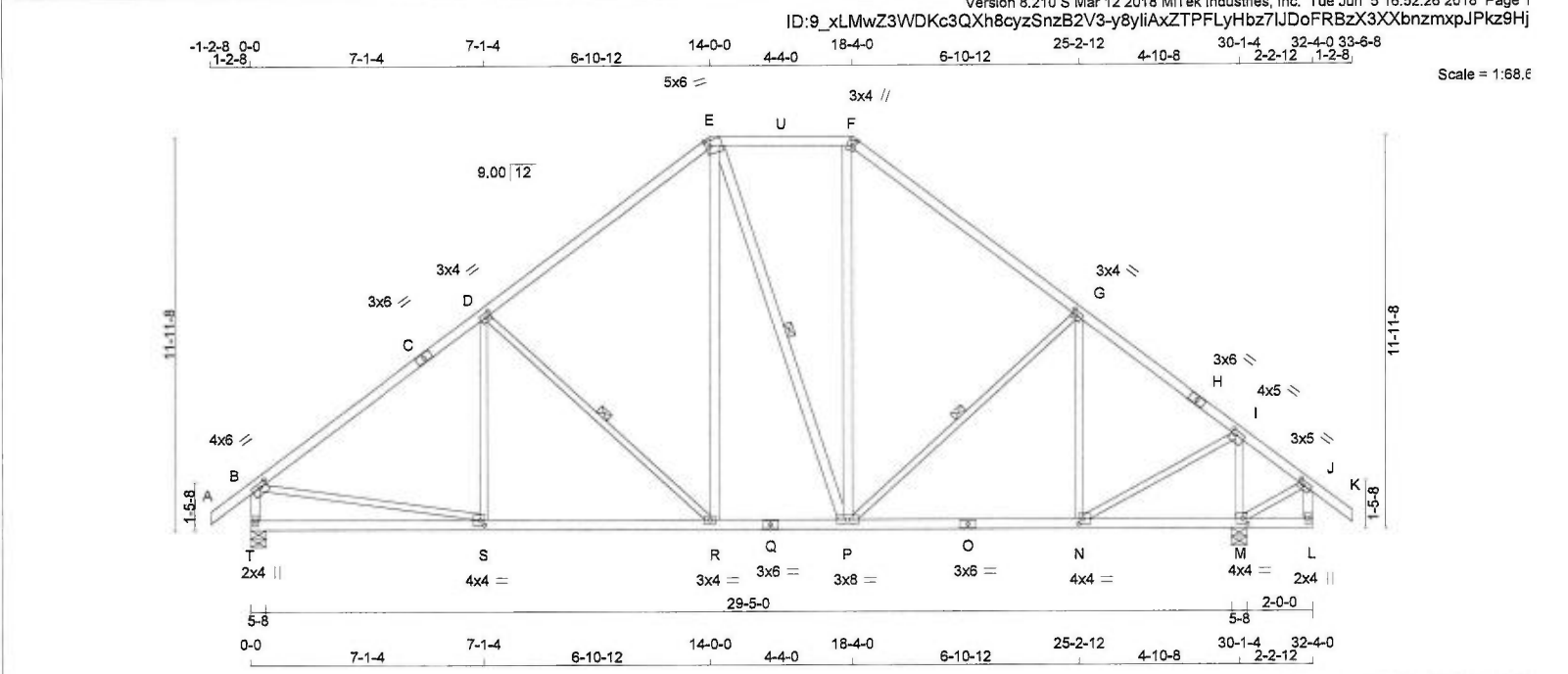
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 1	DRWG NO.
NE0618-034	T04	1	1	TRUSS DESC.		PAGE 42 OF 61

Version 8.210 \$ Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 16:52:25 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-TyONUqxwi67UK70nZ2o_G2v1v7j0o7EpXI4ItHz9Hj

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER N. L. G. A. RULES				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS				BEARINGS				SPECIFIED LOADS:			
A - C	2x4	DRY	No.2	UNFACTORED REACTIONS				LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.			
C - E	2x4	DRY	No.2	1ST LCASE				SPACING = 24.0 IN. C/C			
E - F	2x4	DRY	No.2	COMBINED				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010			
F - H	2x4	DRY	No.2	SNOW				THIS DESIGN COMPLIES WITH:			
H - K	2x4	DRY	No.2	LIVE				- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014			
T - B	2x4	DRY	No.2	PERM.LIVE				- CSA 086-09			
L - J	2x4	DRY	No.2	WIND				- TPIC 2011			
T - Q	2x4	DRY	No.2	DEAD				(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD			
Q - O	2x4	DRY	No.2	SOIL				ALLOWABLE DEFL.(LL)= L/360 (1.00")			
O - L	2x4	DRY	No.2	ALL WEBS EXCEPT				CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")			
				R - E				ALLOWABLE DEFL.(TL)= L/360 (1.00")			
				E - P				CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")			
				P - F				CANTILEVER DEFLECTION:			
				F - G				ALLOWABLE DEFL.(LL)= L/120 (0.22")			
				G - H				CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")			
				H - I				ALLOWABLE DEFL.(TL)= L/120 (0.22")			
				I - J				CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")			
				J - K				CS: TC=0.62/1.00 (B-D:1) , BC=0.33/1.00 (R-S:4) , WB=0.38/1.00 (G-N:1) , SSI=0.21/1.00 (B-D:1)			
				K - L				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
				L - M				COMP=1.10 SHEAR=1.10 TENS= 1.10			
				M - N				COMPANION LIVE LOAD FACTOR = 0.50			
				N - O				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .			
				O - P				NAIL VALUES			
				P - Q				PLATE GRIP(DRY) SHEAR SECTION			
				Q - R				(PSI) (PLI) (PLI)			
				R - S				MAX MIN MAX MIN MAX MIN			
				S - R				MT20 618 354 1667 822 2284 1656			
				R - Q				CONTINUED ON PAGE 2			
				Q - P							
				P - O							
				O - N							
				N - M							
				M - L							

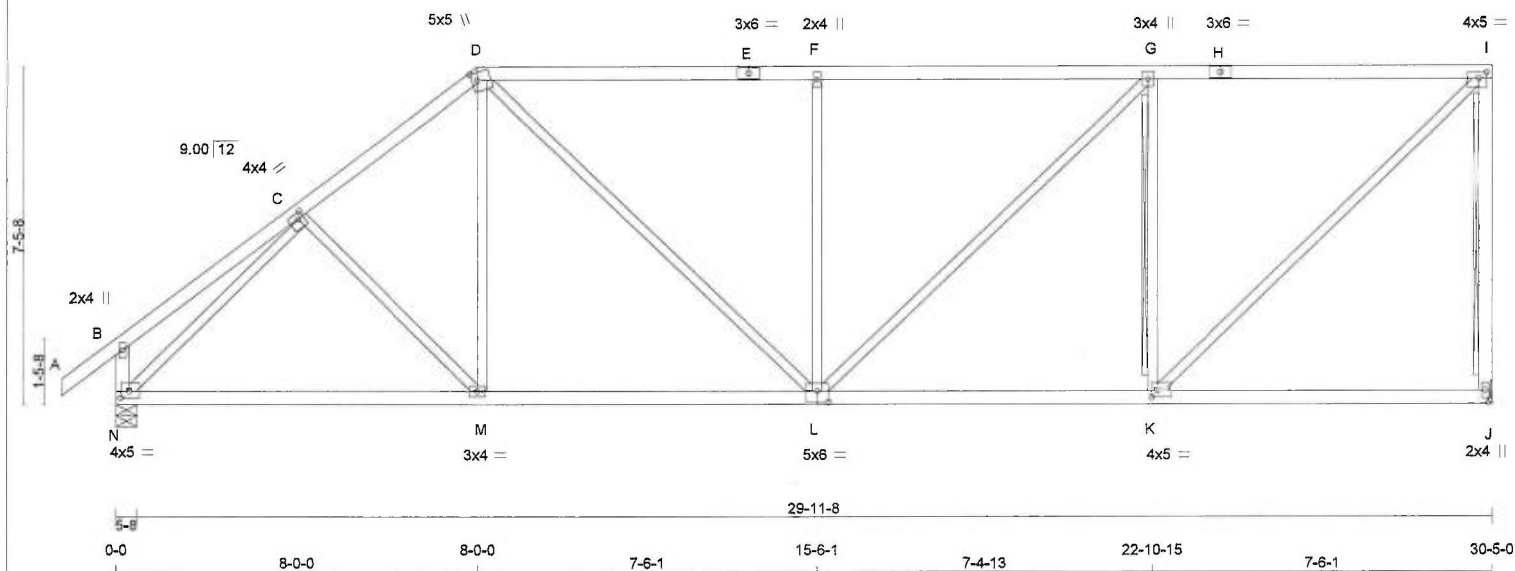
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 1	DRWG NO.
NE0618-034	T05	4	1	TRUSS DESC.		PAGE 44 OF 61

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 16:52:26 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-y8yfiAxZTPFLyHbz7IJDoFRBzX3XXbnzmxpJPkz9Hj

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.49 (S) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 133 lb
[M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	DOWN	UP
J	1454	0	1454	0	0
N	1554	0	1554	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	1022	708 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
N	1090	769 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT I-J, G-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	C-M	-26 / 46	0.02 (4)
B-C	0 / 20	-77.4	-77.4	0.19 (1)	M-D	0 / 182	0.05 (4)
C-D	-1541 / 0	-77.4	-77.4	0.23 (1)	D-L	0 / 541	0.12 (1)
D-E	-1609 / 0	-77.4	-77.4	0.76 (1)	L-F	-628 / 0	0.63 (1)
E-F	-1609 / 0	-77.4	-77.4	0.76 (1)	L-G	0 / 540	0.12 (1)
F-G	-1609 / 0	-77.4	-77.4	0.76 (1)	G-H	-1010 / 0	0.39 (1)
G-H	-1221 / 0	-77.4	-77.4	0.70 (1)	H-I	0 / 1688	0.38 (1)
H-I	-1221 / 0	-77.4	-77.4	0.70 (1)	I-J	-1773 / 0	0.96 (1)
I-J	-1399 / 0	0.0	0.0	0.43 (1)			
J-K	-220 / 0	0.0	0.0	0.02 (1)			
N-B							
N-M	0 / 1233	-18.2	-18.2	0.37 (4)			
M-L	0 / 1217	-18.2	-18.2	0.38 (4)			
L-K	0 / 1221	-18.2	-18.2	0.34 (4)			
K-J	0 / 0	-18.2	-18.2	0.25 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.76/1.00 (F-G:1), BC=0.38/1.00 (L-M:4), WB=0.96/1.00 (C-N:1), SSI=0.27/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

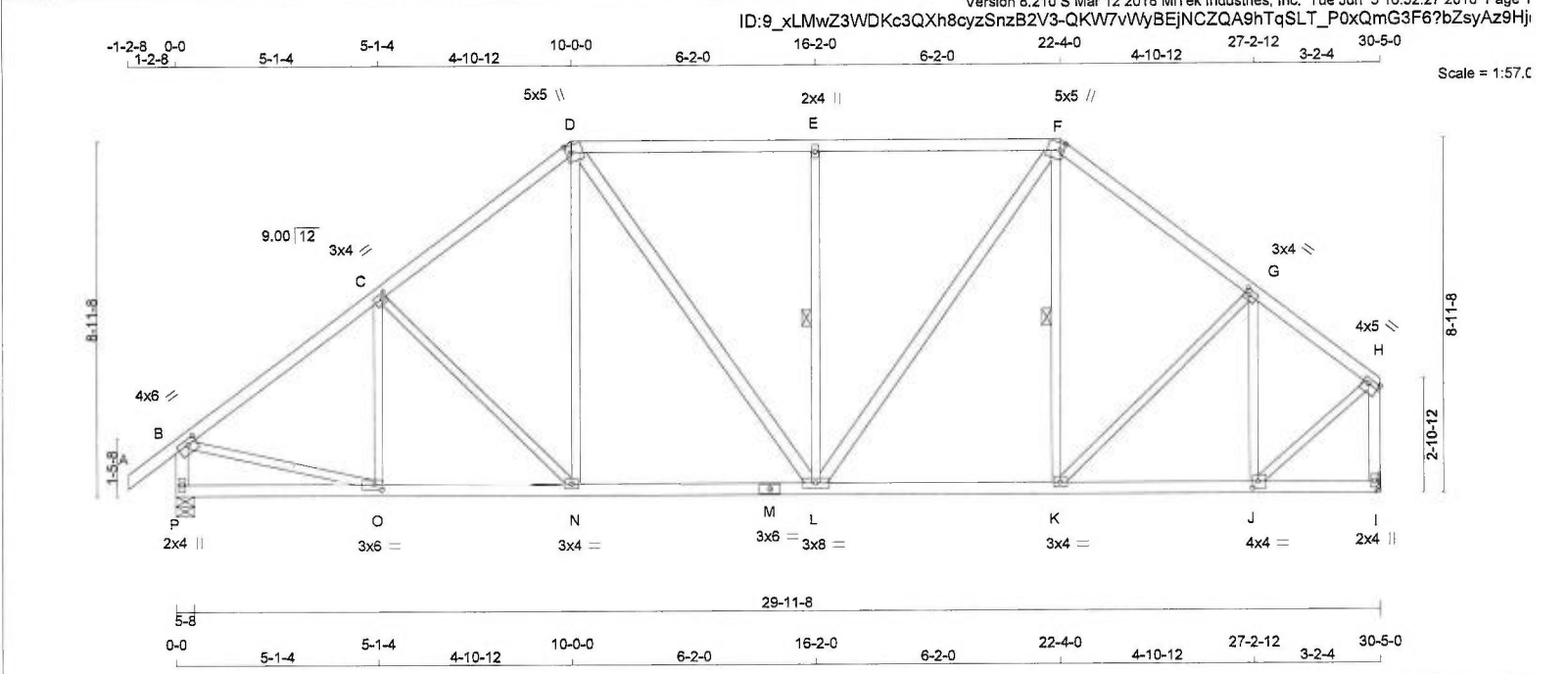
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.65 (C) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

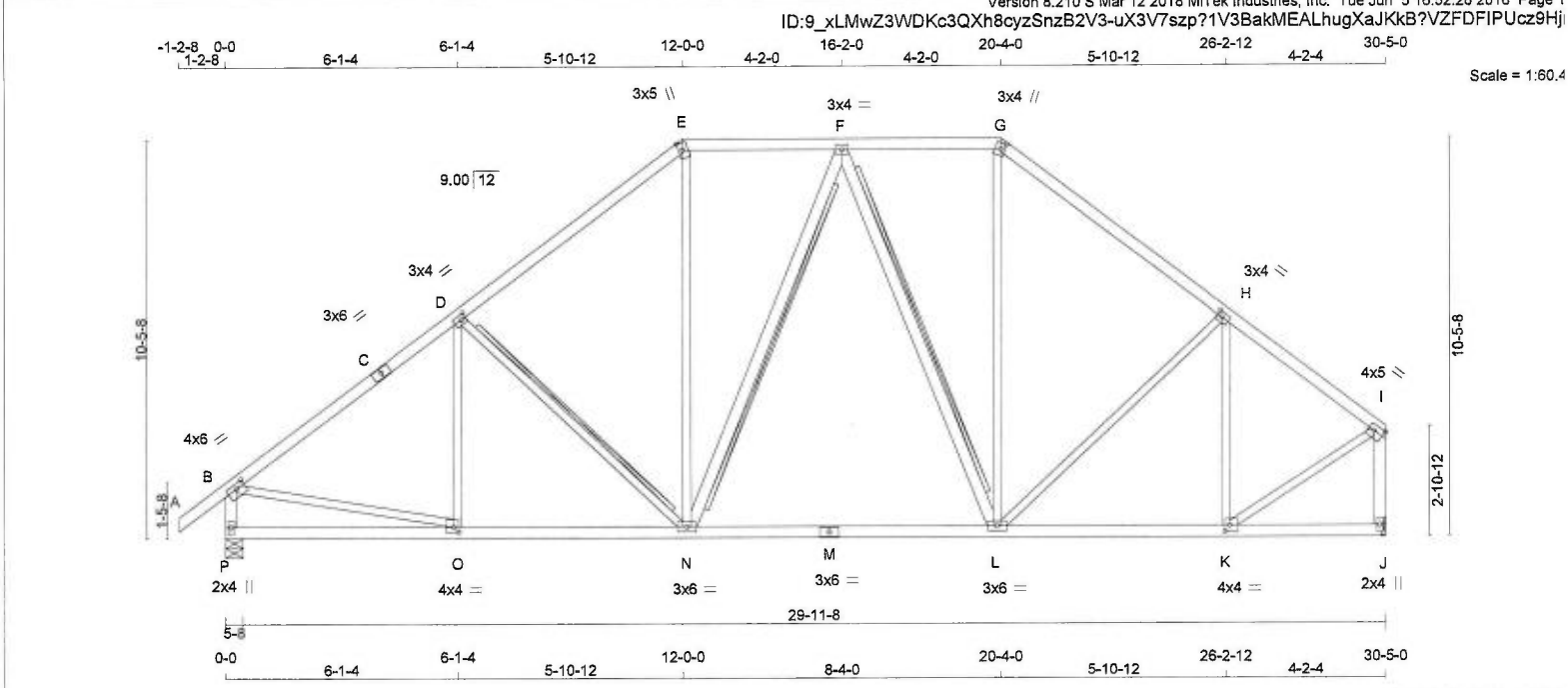




LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x4 DRY No.2 SPF F - H 2x4 DRY No.2 SPF P - B 2x4 DRY No.2 SPF I - H 2x4 DRY No.2 SPF P - M 2x4 DRY No.2 SPF M - I 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT D - L 2x4 DRY No.2 SPF L - F 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 33.6 PSF SPACING = 24.0 IN./C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.01") CALCULATED VERT. DEFL.(LL) = L/999 (0.05") ALLOWABLE DEFL.(TL)= L/360 (1.01") CALCULATED VERT. DEFL.(TL) = L/999 (0.10") CSI: TC=0.41/1.00 (D-E:1), BC=0.27/1.00 (N-O:1), WB=0.30/1.00 (E-L:1), SSI=0.23/1.00 (E-F:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (G) (INPUT = 0.90) JSI METAL= 0.44 (B) (INPUT = 1.00)			
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-t MT20 4.0 6.0 1.50 2.75 C TMVW-t MT20 3.0 4.0 1.50 1.50 D TTWW-t MT20 5.0 5.0 2.25 1.50 E TMVW-t MT20 2.0 4.0 F TTWW-t MT20 5.0 5.0 2.25 1.25 G TMVW-t MT20 3.0 4.0 1.50 1.50 H TMVW-t MT20 4.0 5.0 1.50 Edge I BMV1-p MT20 2.0 4.0 J BMVW-t MT20 4.0 4.0 2.00 1.50 K BMVW-t MT20 3.0 4.0 L BMVW-t MT20 3.0 6.0 M BS-t MT20 3.0 6.0 N BMVW-t MT20 3.0 4.0 O BMVW-t MT20 3.0 6.0 1.50 2.25 P BMV1-p MT20 2.0 4.0 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX P 1554 0 1554 0 0 5-8 5-8 I 1454 0 1454 0 0 MECHANICAL A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS. UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL P 1090 769 / 0 0 / 0 0 / 0 321 / 0 I 1022 708 / 0 0 / 0 0 / 0 313 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. 1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, F-K. DBS = 20-0-0 . CBF = 73 LBS. DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MEMB. MAX. FACTORED MAX. FACTORED MEMB. MAX. FACTORED FORCE VERT. LOAD LC1 MAX UNBRAC FORCE MAX (LBS) (PLF) CSI (LC) LENGTH FR-TO (LBS) CSI (LC) FR-TO FROM TO A-B 0 / 30 -77.4 -77.4 0.09 (1) 10.00 O-C -191 / 26 0.08 (1) B-C -1596 / 0 -77.4 -77.4 0.29 (1) 4.95 C-N -236 / 0 0.22 (1) C-D -1445 / 0 -77.4 -77.4 0.28 (1) 5.16 N-D 0 / 268 0.06 (1) D-E -1318 / 0 -77.4 -77.4 0.41 (1) 5.15 D-L 0 / 317 0.05 (1) E-F -1318 / 0 -77.4 -77.4 0.41 (1) 5.15 L-E -585 / 0 0.30 (1) F-G -1279 / 0 -77.4 -77.4 0.24 (1) 5.45 L-F 0 / 541 0.09 (1) G-H -1062 / 0 -77.4 -77.4 0.19 (1) 5.90 K-F -22 / 71 0.02 (4) P-B -1514 / 0 0.0 0.0 0.16 (1) 6.68 K-G 0 / 181 0.04 (1) I-H -1432 / 0 0.0 0.0 0.22 (1) 6.82 J-G -629 / 0 0.26 (1) B-O 0 / 1330 0.30 (1) J-H 0 / 1125 0.25 (1) P-O 0 / 0 -18.2 -18.2 0.10 (4) 10.00 O-N -18.2 -18.2 0.27 (1) 10.00 N-M -18.2 -18.2 0.26 (1) 10.00 M-L -18.2 -18.2 0.26 (1) 10.00 L-K -18.2 -18.2 0.24 (1) 10.00 K-J -18.2 -18.2 0.21 (1) 10.00 J-I -18.2 -18.2 0.06 (4) 10.00			

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





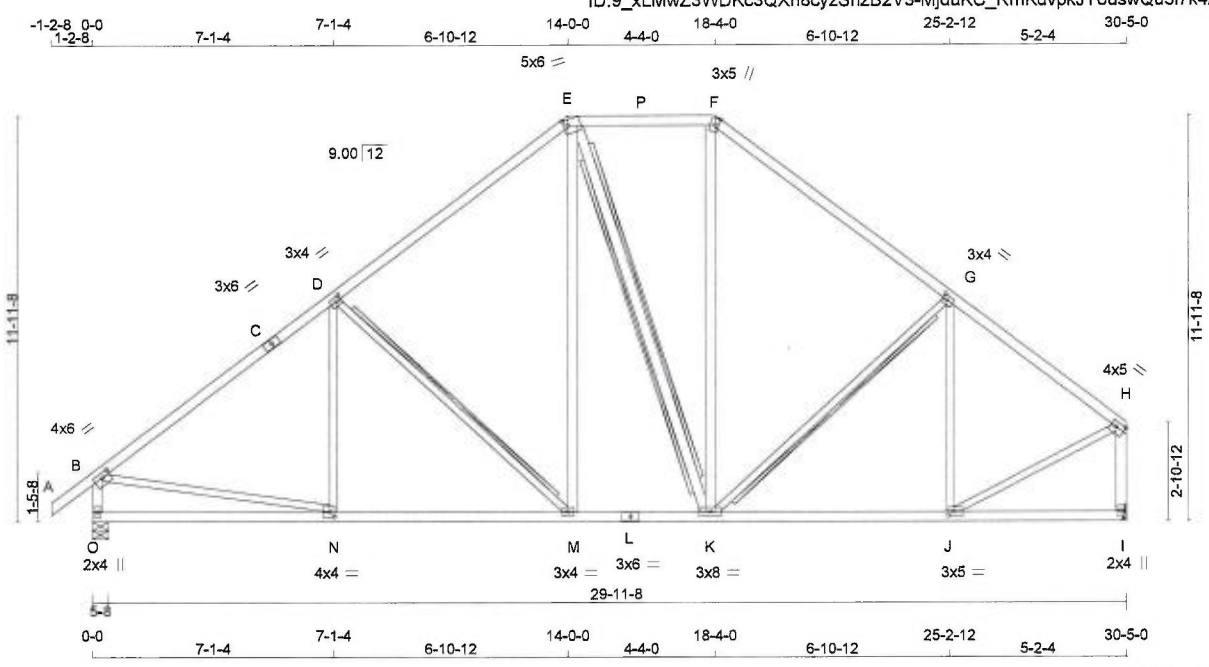
										TOTAL WEIGHT = 152 lb														
										[M][F]														
LUMBER N. L. G. A. RULES				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER																DESIGN CRITERIA				
CHORDS		SIZE	LUMBER	DESCR.	BEARINGS																SPECIFIED LOADS:			
A - C		2x4	DRY	No.2	SPF	FACTORED		MAXIMUM FACTORED		INPUT		REQRD		TOP CH. LL = 23.3 PSF										
C - E		2x4	DRY	No.2	SPF	GROSS REACTION		GROSS REACTION		BRG		BRG		DL = 3.0 PSF										
E - G		2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BOT CH. LL = 0.0 PSF										
G - I		2x4	DRY	No.2	SPF	P	1554	0	1554	0	0	5-8	5-8	DL = 7.3 PSF										
P - B		2x4	DRY	No.2	SPF	J	1454	0	1454	0	0	MECHANICAL		TOTAL LOAD = 33.6 PSF										
J - I		2x4	DRY	No.2	SPF	A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J TO RESIST THE MAX FACTORED REACTIONS.																		
P - M		2x4	DRY	No.2	SPF																			
M - J		2x4	DRY	No.2	SPF																			
ALL WEBS EXCEPT		2x3	DRY	No.2	SPF																			
N - F		2x4	DRY	No.2	SPF																			
F - L		2x4	DRY	No.2	SPF																			
DRY: SEASONED LUMBER.																								
										UNFACTORED REACTIONS														
										1ST LCASE MAX/MIN. COMPONENT REACTIONS														
JT		COMBINED		SNOW		LIVE		PERM.LIVE		WIND		DEAD		SOIL										
P		1090		769 / 0		0 / 0		0 / 0		0 / 0		321 / 0		0 / 0										
J		1022		708 / 0		0 / 0		0 / 0		0 / 0		313 / 0		0 / 0										
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010																								

PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-t MT20 4.0 6.0 1.50 2.75 C TS-t MT20 3.0 6.0 D TMVW-t MT20 3.0 4.0 1.50 1.50 E TTW+m MT20 3.0 5.0 Edge 1.25 F TMVW-t MT20 3.0 4.0 G TTW+m MT20 3.0 4.0 2.00 1.25 H TMVW-t MT20 3.0 4.0 1.50 1.50 I TMVW-t MT20 4.0 5.0 1.75 Edge J BMV1+p MT20 2.0 4.0 K BMVW-t MT20 4.0 4.0 2.00 1.50 L BMVW-t MT20 3.0 6.0 M BS-t MT20 3.0 6.0 N BMVW-t MT20 3.0 6.0 O BMVW-t MT20 4.0 4.0 1.50 1.50 P BMV1+p MT20 2.0 4.0 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 2x4 DRY SPF No.2 T-BRACE AT D-N, F-N, F-L FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4)			
--	--	--	--	--	--	--	--

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0/30	-77.4 -77.4	0.09 (1)	10.00	O-D	-129/58	0.07 (1)
B-C	-1605/0	-77.4 -77.4	0.43 (1)	4.77	D-N	-366/0	0.19 (1)
C-D	-1605/0	-77.4 -77.4	0.43 (1)	4.77	N-E	0/488	0.11 (1)
D-E	-1344/0	-77.4 -77.4	0.40 (1)	5.14	N-F	-111/8	0.10 (1)
E-F	-1051/0	-77.4 -77.4	0.18 (1)	5.94	F-L	-307/0	0.27 (1)
F-G	-978/0	-77.4 -77.4	0.17 (1)	6.10	L-G	0/421	0.09 (1)
G-H	-1245/0	-77.4 -77.4	0.35 (1)	5.36	L-H	0/16	0.01 (4)
H-I	-1176/0	-77.4 -77.4	0.29 (1)	5.55	K-H	-504/0	0.29 (1)
P-B	-1507/0	0.0 0.0	0.16 (1)	6.68	B-O	0/1333	0.30 (1)
J-I	-1423/0	0.0 0.0	0.22 (1)	6.84	K-I	0/1138	0.26 (1)
P-O	0/0	-18.2 -18.2	0.14 (4)	10.00			
O-N	0/1311	-18.2 -18.2	0.32 (1)	10.00			
N-M	0/1092	-18.2 -18.2	0.29 (1)	10.00			
M-L	0/1092	-18.2 -18.2	0.29 (1)	10.00			
L-K	0/971	-18.2 -18.2	0.28 (4)	10.00			
K-J	0/0	-18.2 -18.2	0.09 (4)	10.00			

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.49 (G) (INPUT = 1.00)



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
M - E	2x4	DRY	No.2
E - K	2x4	DRY	No.2
K - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	4.0	6.0 1.50 2.75
C	TS-t	MT20	3.0	6.0
D	TMVW-t	MT20	3.0	4.0 1.50 1.50
E	TTWW-m	MT20	5.0	6.0 2.00 1.50
F	TTW-m	MT20	3.0	5.0 Edge 1.25
G	TMVW-t	MT20	3.0	4.0 1.50 1.50
H	TMVW-t	MT20	4.0	5.0 1.75 Edge
I	BMV1+p	MT20	2.0	4.0
J	BMVW-t	MT20	3.0	5.0 1.50 1.75
K	BMVW-t	MT20	3.0	8.0
L	BS-t	MT20	3.0	6.0
M	BMVW-t	MT20	3.0	4.0
N	BMVW-t	MT20	4.0	4.0 1.75 1.50
O	BMV1+p	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION		REQRD BRG
	VERT	HORZ	
O	1570	0	5-8
I	1471	0	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS						
JT	COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
O	1102	769 / 0	0 / 0	0 / 0	0 / 0	333 / 0
I	1035	708 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
FOR SECTION E-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-M, G-K
2x4 DRY SPF No.2 I-BRACE AT E-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	N-D	-62 / 104	0.05 (1)
B-C	-1622 / 0	-77.4 -77.4	0.62 (1)	4.49	D-M	-500 / 0	0.37 (1)
C-D	-1622 / 0	-77.4 -77.4	0.62 (1)	4.49	M-E	0 / 428	0.07 (1)
D-E	-1242 / 0	-77.4 -77.4	0.56 (1)	5.04	E-K	-94 / 0	0.09 (1)
E-P	-934 / 0	-84.9 -84.9	0.28 (1)	2.00	K-F	0 / 302	0.05 (1)
P-F	-934 / 0	-84.9 -84.9	0.28 (1)	2.00	K-G	-163 / 0	0.12 (1)
F-G	-1196 / 0	-77.4 -77.4	0.50 (1)	5.24	J-G	-394 / 0	0.30 (1)
G-H	-1270 / 0	-77.4 -77.4	0.42 (1)	5.21	B-N	0 / 1346	0.30 (1)
O-B	-1518 / 0	0.0 0.0	0.16 (1)	6.67	J-H	0 / 1171	0.26 (1)
I-H	-1437 / 0	0.0 0.0	0.22 (1)	6.81			
O-N	0 / 0	-18.2 -18.2	0.23 (4)	10.00			
N-M	0 / 1329	-18.2 -18.2	0.34 (1)	10.00			
M-L	0 / 965	-18.2 -18.2	0.23 (1)	10.00			
L-K	0 / 965	-18.2 -18.2	0.23 (1)	10.00			
K-J	0 / 1050	-18.2 -18.2	0.26 (1)	10.00			
J-I	0 / 0	-18.2 -18.2	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.62/1.00 (B-D:1), BC=0.34/1.00 (M-N:1), WB=0.37/1.00 (D-M:1), SSI=0.21/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	822 2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

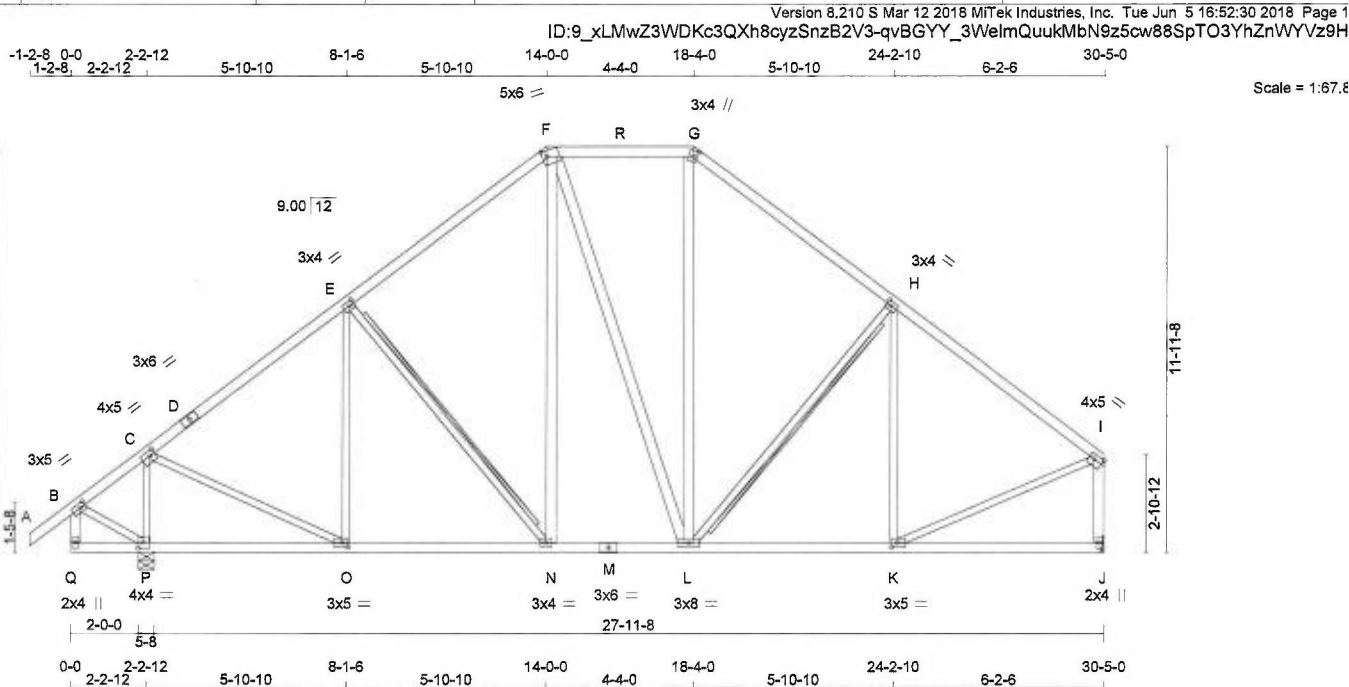
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)

JSI METAL= 0.50 (N) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - I	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
N - F	2x4	DRY	No.2
F - L	2x4	DRY	No.2
L - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TMWW-t	MT20	4.0	5.0	1.75	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	3.0	4.0	1.50	1.50
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TTW-m	MT20	3.0	4.0	2.00	1.25
H	TMWW-t	MT20	3.0	4.0	1.50	1.50
I	TMVW-t	MT20	4.0	5.0	1.75	Edge
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	3.0	5.0	1.50	2.00
L	BMWWW-t	MT20	3.0	8.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	3.0	4.0		
O	BMWW-t	MT20	3.0	5.0	1.50	2.00
P	BMWW1-t	MT20	4.0	4.0	2.00	1.75
Q	BMV+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
P	1694	0	1694	0	5-8	5-8
J	1347	0	1347	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1189	829 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
J	948	648 / 0	0 / 0	0 / 0	0 / 0	301 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
FOR SECTION F-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N, H-L

FASTEN T AND L-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	P-C	-1546 / 0	0.28 (1)	
B-C	0 / 151	-77.4	-77.4	0.24 (1)	10.00	C-O	0 / 1070	0.24 (1)	
C-D	-1086 / 0	-77.4	-77.4	0.33 (1)	5.67	O-E	-351 / 0	0.36 (1)	
D-E	-1086 / 0	-77.4	-77.4	0.33 (1)	5.67	E-N	-142 / 0	0.10 (1)	
E-F	-1016 / 0	-77.4	-77.4	0.35 (1)	5.79	N-F	0 / 201	0.03 (1)	
F-R	-801 / 0	-84.9	-84.9	0.28 (1)	2.00	F-L	0 / 32	0.01 (1)	
R-G	-801 / 0	-84.9	-84.9	0.28 (1)	2.00	L-G	0 / 249	0.04 (1)	
G-H	-1035 / 0	-77.4	-77.4	0.39 (1)	5.68	H-H	-253 / 0	0.18 (1)	
H-I	-1167 / 0	-77.4	-77.4	0.41 (1)	5.40	K-H	-269 / 21	0.28 (1)	
Q-B	-7 / 0	0.0	0.0	0.00 (1)	7.81	B-P	-97 / 0	0.02 (1)	
J-I	-1302 / 0	0.0	0.0	0.20 (1)	7.08	K-I	0 / 1039	0.23 (1)	
Q-P	0 / 0	-18.2	-18.2	0.10 (4)	10.00				
P-O	-87 / 0	-18.2	-18.2	0.10 (4)	6.25				
O-N	0 / 879	-18.2	-18.2	0.21 (1)	10.00				
N-M	0 / 790	-18.2	-18.2	0.18 (1)	10.00				
M-L	0 / 790	-18.2	-18.2	0.18 (1)	10.00				
L-K	0 / 960	-18.2	-18.2	0.25 (4)	10.00				
K-J	0 / 0	-18.2	-18.2	0.17 (4)	10.00				

TOTAL WEIGHT = 163 lb [M]F

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
	23.3	PSF
	3.0	PSF
BOT CH.	LL	0.0
	DL	7.3
	PSF	
TOTAL LOAD		33.6

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.41/1.00 (H-I:1), BC=0.25/1.00 (K-L:4), WB=0.36/1.00 (E-O:1), SSI=0.18/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



CONTINUED ON PAGE 2



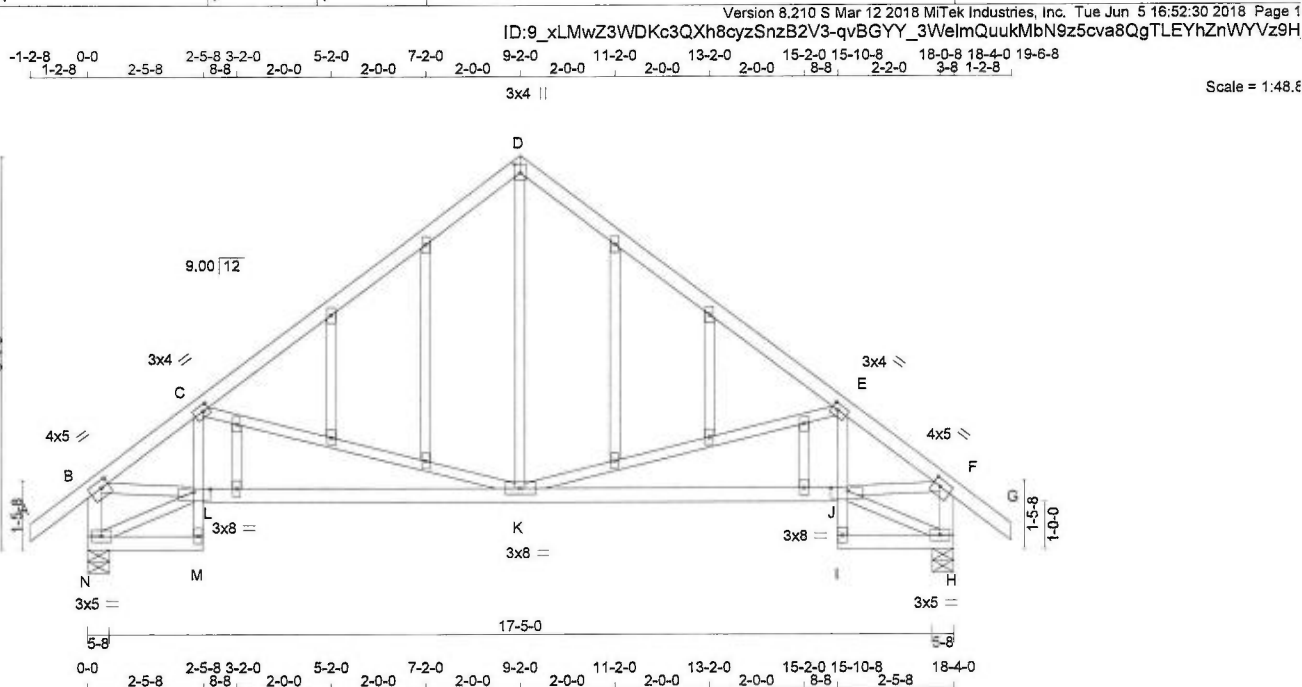
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 1	DRWG NO.
NE0618-034	T10	1	1	TRUSS DESC.		PAGE 50 OF 61

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 16:52:30 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSszB2V3-qvBGYY_3WelmQuukMbN9z5cw88SpTO3YhZnWYVz9H

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 95 lb [M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x3	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x3	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2-0-0	OC.	

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-t	MT20	4.0	5.0	1.75 2.00
C TMVW-t	MT20	3.0	4.0	1.50 1.50
D TTVW-p	MT20	3.0	4.0	2.25 1.50
E TMVW-t	MT20	3.0	4.0	1.50 1.50
F TMVW-t	MT20	4.0	5.0	1.75 2.00
H BMVW1-t	MT20	3.0	5.0	
I BMVW-p	MT20	2.0	4.0	
J BVWVW-t	MT20	3.0	8.0	0.75 4.25
K BVWVW-t	MT20	3.0	8.0	
L BVWVW-t	MT20	3.0	8.0	0.75 4.25
M BMVW-p	MT20	2.0	4.0	
N BMVW1-t	MT20	3.0	5.0	
O P, Q, R, S, T, U, V, W, X, Y, Z				
O NP+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	BRG	BRG	IN-SX
N	977	0	0	977	0	0	5-8	5-8	5-8
H	977	0	0	977	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	684	487 / 0	0 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0
H	684	487 / 0	0 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	MEMB.
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	K-D	0 / 451	0.10 (1)
B-C	-1317 / 0	-77.4 -77.4	0.36 (1)	5.20	K-E	-559 / 0	0.54 (1)
C-D	-764 / 0	-77.4 -77.4	0.45 (1)	6.24	C-K	-559 / 0	0.54 (1)
D-E	-764 / 0	-77.4 -77.4	0.45 (1)	6.24	N-L	-14 / 0	0.00 (1)
E-F	-1317 / 0	-77.4 -77.4	0.36 (1)	5.20	B-L	0 / 1121	0.25 (1)
F-G	0 / 30	-77.4 -77.4	0.09 (1)	10.00	J-H	-14 / 0	0.00 (1)
N-B	-950 / 0	0.0 0.0	0.10 (1)	7.81	J-F	0 / 1121	0.25 (1)
H-F	-950 / 0	0.0 0.0	0.10 (1)	7.81			
N-M	0 / 13	-18.2 -18.2	0.03 (4)	10.00			
M-L	0 / 24	0.0 0.0	0.04 (1)	10.00			
L-C	0 / 60	0.0 0.0	0.04 (1)	10.00			
L-K	0 / 1127	-18.3 -18.3	0.32 (1)	10.00			
K-J	0 / 1127	-18.3 -18.3	0.32 (1)	10.00			
I-J	0 / 24	0.0 0.0	0.04 (1)	10.00			
J-E	0 / 60	0.0 0.0	0.04 (1)	10.00			
I-H	0 / 13	-18.2 -18.2	0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.45/1.00 (C-D:1), BC=0.32/1.00 (J-K:1), WB=0.54/1.00 (E-K:1), SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

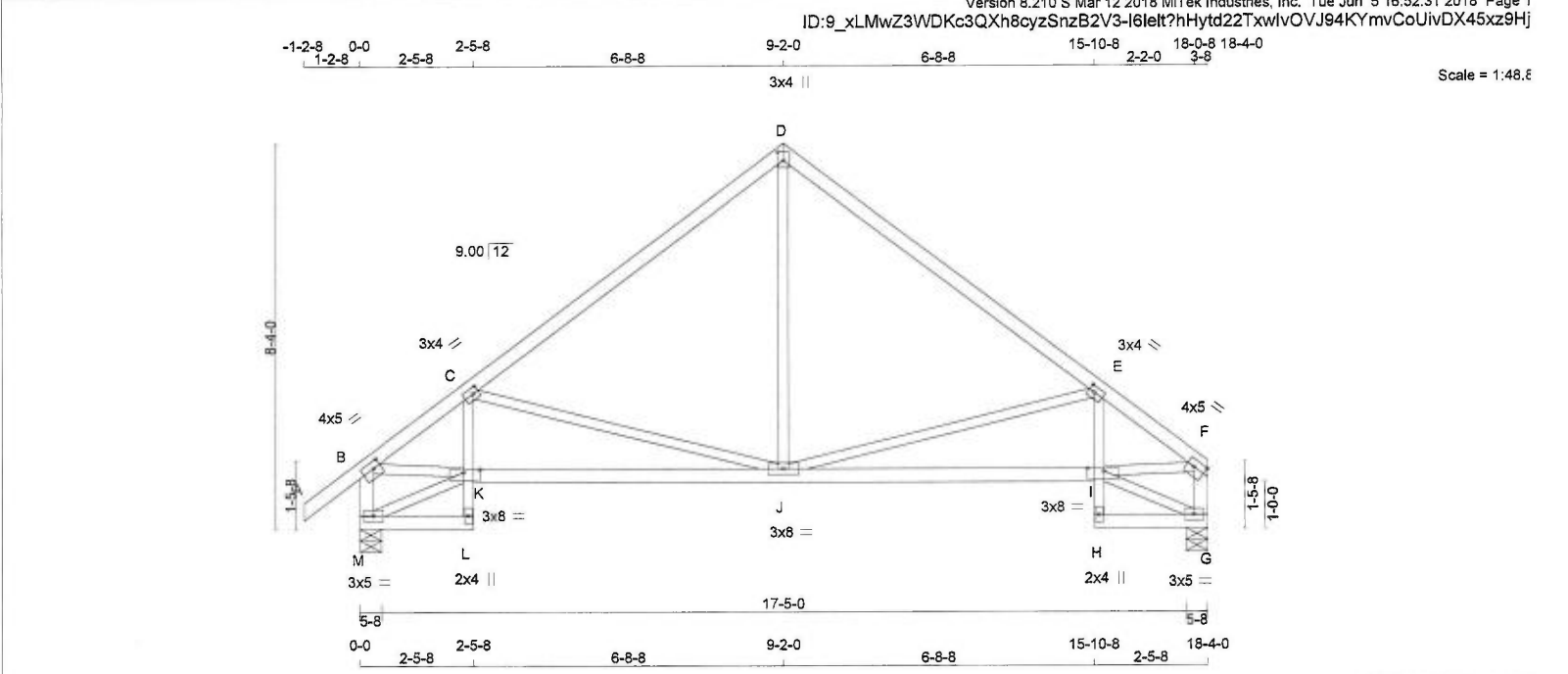
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
H - E	2x3	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
M	977	0	977	0	5-8	5-8
G	876	0	876	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	684	487 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0
G	616	427 / 0	0 / 0	0 / 0	0 / 0	189 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD (PLF)

LC1

MAX. CSI (LC)

UNBRAC LENGTH

FR-TO

MEMB.

MAX. FACTORED FORCE (LBS)

MAX. CSI (LC)

A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	J-D	0 / 451	0.10 (1)
B-C	-1317 / 0	-77.4	-77.4	0.36 (1)	5.20	J-E	-559 / 0	0.54 (1)
C-D	-784 / 0	-77.4	-77.4	0.45 (1)	6.24	C-J	-559 / 0	0.54 (1)
D-E	-784 / 0	-77.4	-77.4	0.45 (1)	6.24	M-K	-14 / 0	0.00 (1)
E-F	-1317 / 0	-77.4	-77.4	0.36 (1)	5.20	B-K	0 / 1121	0.25 (1)
M-B	-950 / 0	0.0	0.0	0.10 (1)	7.81	I-G	-14 / 0	0.00 (1)
G-F	-849 / 0	0.0	0.0	0.09 (1)	7.81	I-F	0 / 1121	0.25 (1)
M-L	0 / 13	-18.2	-18.2	0.03 (4)	10.00			
L-K	0 / 24	0.0	0.0	0.04 (1)	10.00			
K-C	0 / 60	0.0	0.0	0.04 (1)	10.00			
K-J	0 / 1127	-18.3	-18.3	0.32 (1)	10.00			
J-I	0 / 1127	-18.3	-18.3	0.32 (1)	10.00			
H-I	0 / 24	0.0	0.0	0.04 (1)	10.00			
I-E	0 / 60	0.0	0.0	0.04 (1)	10.00			
H-G	0 / 13	-18.2	-18.2	0.03 (4)	10.00			

WEBS

MEMB.

MAX. FACTORED FORCE (LBS)

MAX. CSI (LC)

J-D	0 / 451	0.10 (1)
J-E	-559 / 0	0.54 (1)
C-J	-559 / 0	0.54 (1)
M-K	-14 / 0	0.00 (1)
B-K	0 / 1121	0.25 (1)
I-G	-14 / 0	0.00 (1)
I-F	0 / 1121	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 088-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")

CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03")

ALLOWABLE DEFL.(TL)= L/360 (0.61")

CALCULATED VERT. DEFL.(TL)= L/ 999 (0.09")

CSI: TC=0.45/1.00 (D-E:1), BC=0.32/1.00 (J-K:1), WB=0.54/1.00 (C-J:1), SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

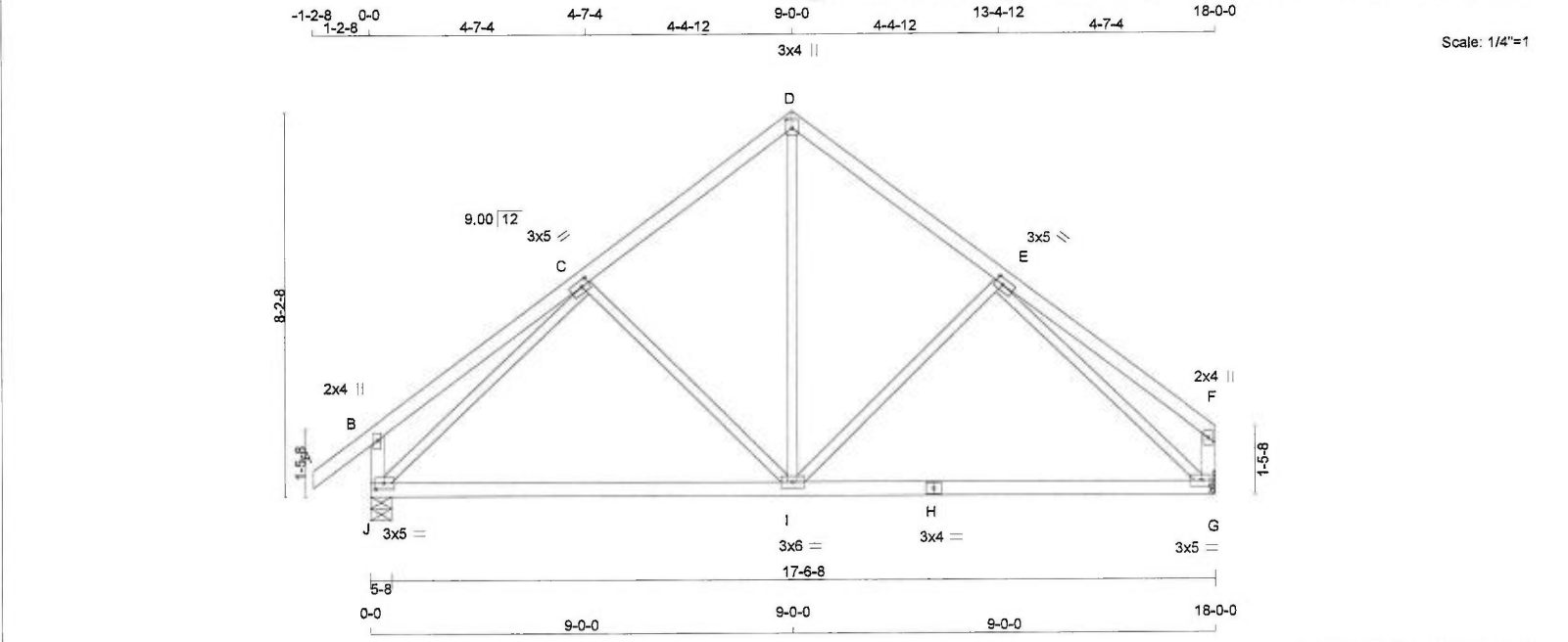
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)

JSI METAL= 0.43 (F) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

A - D

2x4

DRY

No.2

SPF

D - F

2x4

DRY

No.2

SPF

J - B

2x4

DRY

No.2

SPF

G - H

2x4

DRY

No.2

SPF

J - H

2x4

DRY

No.2

SPF

H - G

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
J	961	0	961	0
G	861	0	861	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")

CALCULATED VERT. DEFL.(LL) = L/999 (0.02")

ALLOWABLE DEFL.(TL)= L/360 (0.60")

CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.25/1.00 (B-C:1), BC=0.47/1.00 (I-J:4), WB=0.66/1.00 (C-J:1), SSI=0.14/1.00 (I-J:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) (PSI)

GRIP (PLI)

SECTION (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)

JSI METAL= 0.27 (H) (INPUT = 1.00)

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

J

673

480 / 0

0 / 0

0 / 0

0 / 0

193 / 0

0 / 0

G

605

419 / 0

0 / 0

0 / 0

0 / 0

185 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

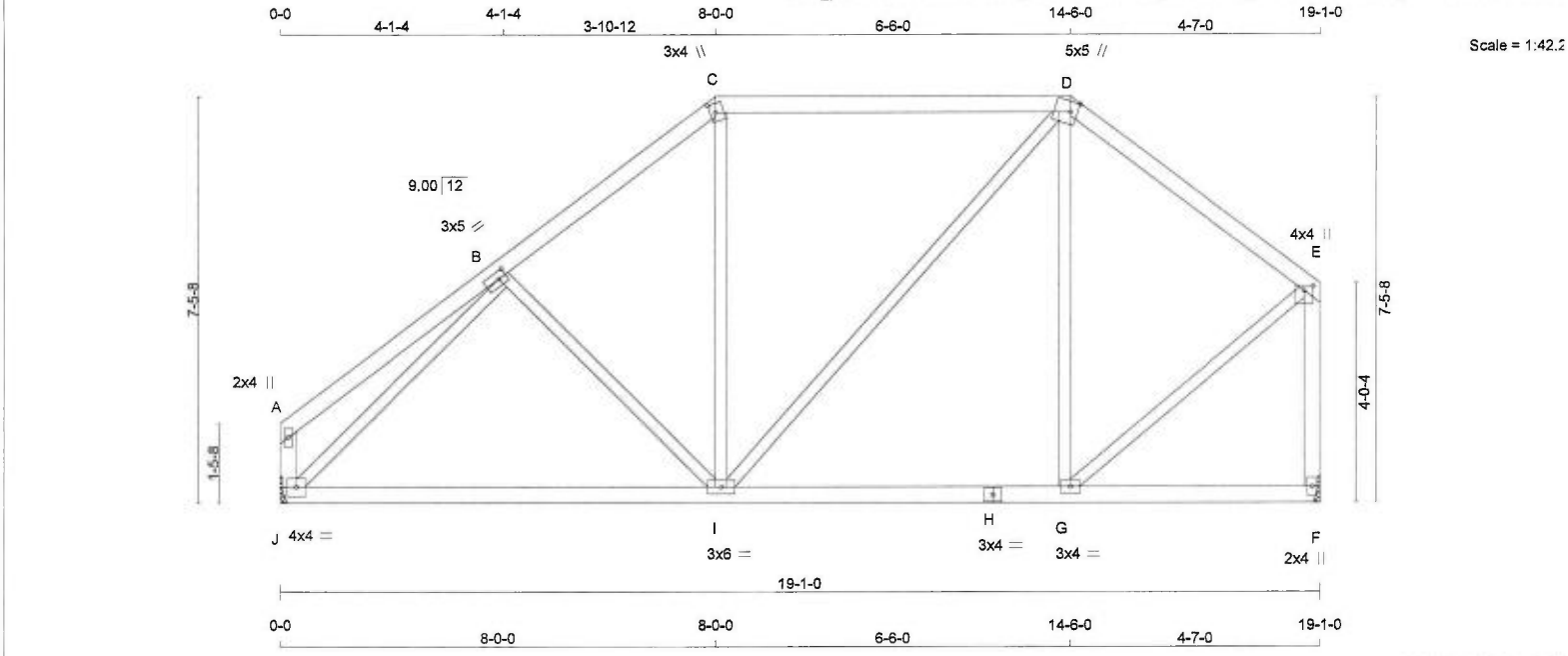
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. LC1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	I-D	0 / 503	10.00	0.11 (1)
B-C	0 / 25	-77.4	-77.4 0.25 (1)	I-E	-212 / 0	10.00	0.15 (1)
C-D	-667 / 0	-77.4	-77.4 0.20 (1)	C-I	-212 / 0	6.25	0.15 (1)
D-E	-667 / 0	-77.4	-77.4 0.20 (1)	J-C	-941 / 0	6.25	0.66 (1)
E-F	0 / 25	-77.4	-77.4 0.25 (1)	E-G	-941 / 0	10.00	0.66 (1)
J-B	-233 / 0	0.0	0.0 0.02 (1)			7.81	
G-F	-132 / 0	0.0	0.0 0.01 (1)			7.81	
J-I	0 / 664	-18.2	-18.2 0.47 (4)			10.00	
I-H	0 / 664	-18.2	-18.2 0.47 (4)			10.00	
H-G	0 / 664	-18.2	-18.2 0.47 (4)			10.00	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

DRY: SEASONED LUMBER.

ALL WEBS EXCEPT

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS EXCEPT

2x3

DRY

No.2

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
J	912	0	912	0	0	MECHANICAL			
F	912	0	912	0	0	MECHANICAL			
A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F TO RESIST THE MAX FACTORED REACTIONS.									

DESIGNED CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.3 PSF
TOTAL LOAD		=	33.6 PSF
SPACING = 24.0 IN. C/C			

UNFACTORED REACTIONS									
JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL		
	SNOW	LIVE	PERM. LIVE						
J	641	444 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0		
F	641	444 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0		

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MEMB.	MAX. FACTORED FORCE (LBS)		MAX. UNBRACED LENGTH (LC)	
	FR-TO	FROM TO	LC1	MAX		FR-TO	FROM TO	LC1	MAX
A-B	0 / 22	-77.4	-77.4	0.20 (1)	10.00	B-I	-147 / 10	0.09 (1)	
B-C	-780 / 0	-77.4	-77.4	0.16 (1)	6.25	I-C	0 / 107	0.04 (4)	
C-D	-807 / 0	-77.4	-77.4	0.42 (1)	6.25	I-D	0 / 234	0.05 (1)	
D-E	-569 / 0	-77.4	-77.4	0.21 (1)	6.25	G-D	-260 / 0	0.26 (1)	
J-A	-118 / 0	0.0	0.0	0.01 (1)	7.81	J-B	-1021 / 0	0.55 (1)	
F-E	-878 / 0	0.0	0.0	0.24 (1)	7.81	G-E	0 / 579	0.13 (1)	
J-I	0 / 710	-18.2	-18.2	0.31 (4)	10.00				
I-H	0 / 451	-18.2	-18.2	0.30 (4)	10.00				
H-G	0 / 451	-18.2	-18.2	0.30 (4)	10.00				
G-F	0 / 0	-18.2	-18.2	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")

CALCULATED VERT. DEFL.(LL) = L/999 (0.02")

ALLOWABLE DEFL.(TL)= L/360 (0.64")

CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.42/1.00 (C-D:1), BC=0.31/1.00 (I-J:4), WB=0.55/1.00 (B-J:1), SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

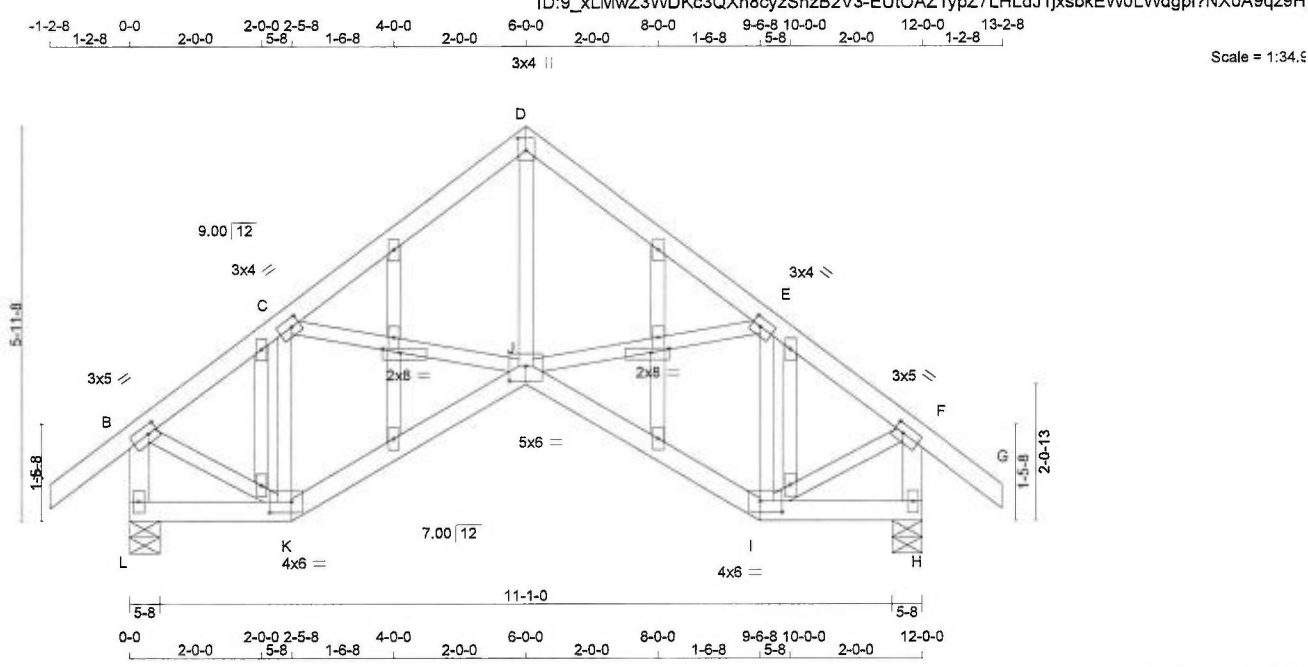
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)

JSI METAL= 0.30 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

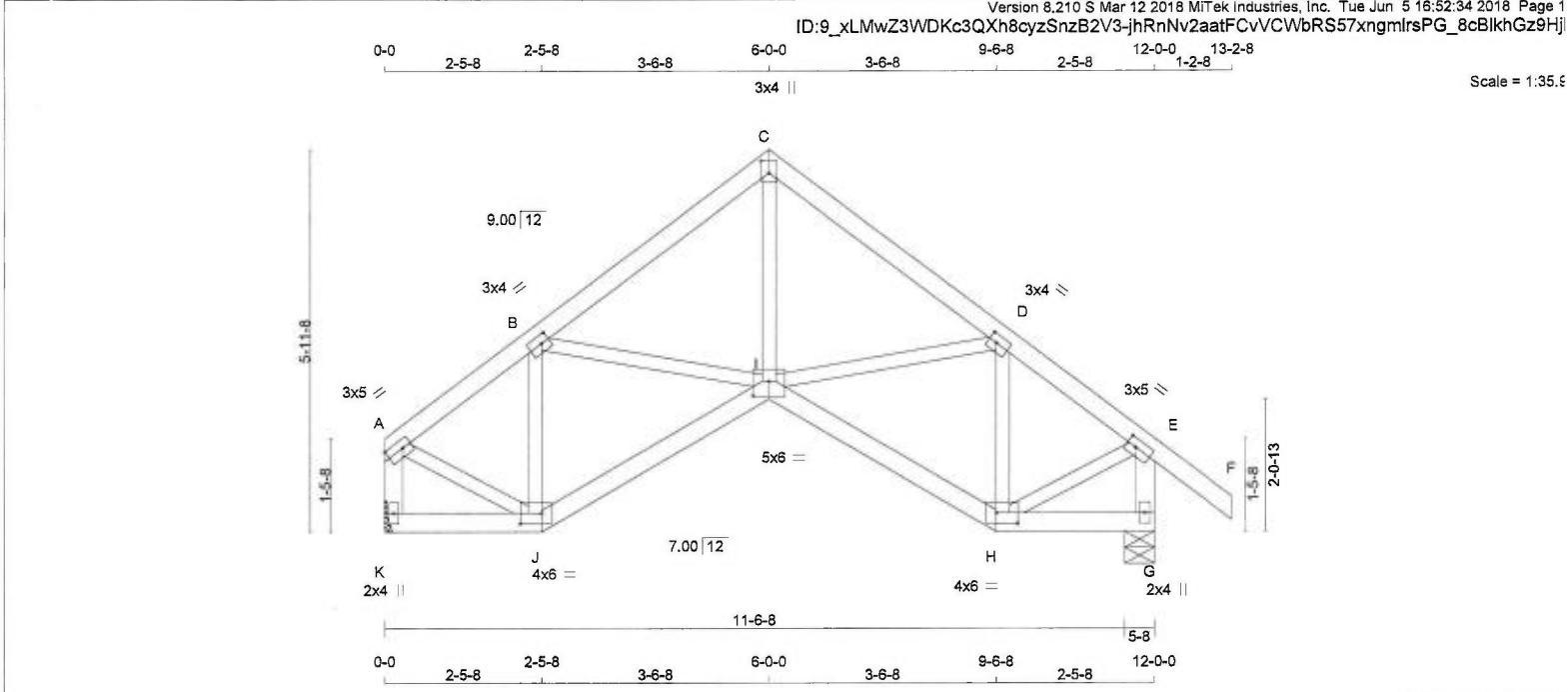




LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - G 2x4 DRY No.2 SPF L - B 2x4 DRY No.2 SPF H - F 2x4 DRY No.2 SPF L - K 2x4 DRY No.2 SPF J - I 2x4 DRY No.2 SPF I - H 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT ALL GABLE WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2-0-0 OC.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX L 674 0 674 0 0 5-8 5-8 H 674 0 674 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL L 471 340 / 0 0 / 0 0 / 0 0 / 0 131 / 0 0 / 0 H 471 340 / 0 0 / 0 0 / 0 0 / 0 131 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MAX. FACTORED MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX MEMB. MAX. FACTORED (LBS) (PLF) CSI (LC) UNBRAC MEMB. FORCE MAX LENGTH FR-TO FR-TO CSI (LC) FR-TO A-B 0 / 30 -77.4 -77.4 0.09 (1) 10.00 J-D 0 / 496 0.11 (1) B-C -494 / 0 -77.4 -77.4 0.10 (1) 6.25 J-E 0 / 88 0.02 (1) C-D -622 / 0 -77.4 -77.4 0.12 (1) 6.25 I-E -369 / 0 0.07 (1) D-E -622 / 0 -77.4 -77.4 0.12 (1) 6.25 C-J 0 / 88 0.02 (1) E-F -494 / 0 -77.4 -77.4 0.10 (1) 6.25 K-C -369 / 0 0.07 (1) F-G 0 / 30 -77.4 -77.4 0.09 (1) 10.00 B-K 0 / 454 0.10 (1) L-B -651 / 0 0.0 0.0 0.07 (1) 7.81 I-F 0 / 454 0.10 (1) H-F -651 / 0 0.0 0.0 0.07 (1) 7.81 L-K 0 / 0 -18.2 -18.2 0.03 (4) 10.00 K-J 0 / 464 -18.3 -18.3 0.11 (1) 10.00 J-I 0 / 464 -18.3 -18.3 0.11 (1) 10.00 I-H 0 / 0 -18.2 -18.2 0.03 (4) 10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 33.6 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.40") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.40") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03") CSI: TC=0.12/1.00 (D-E:1) , BC=0.11/1.00 (I-J:1) , WB=0.11/1.00 (D-J:1) , SSI=0.10/1.00 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.82 (I) (INPUT = 0.90) JSI METAL= 0.18 (B) (INPUT = 1.00)			
---	--	--	--	--	--	--	--	---	--	--	--

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



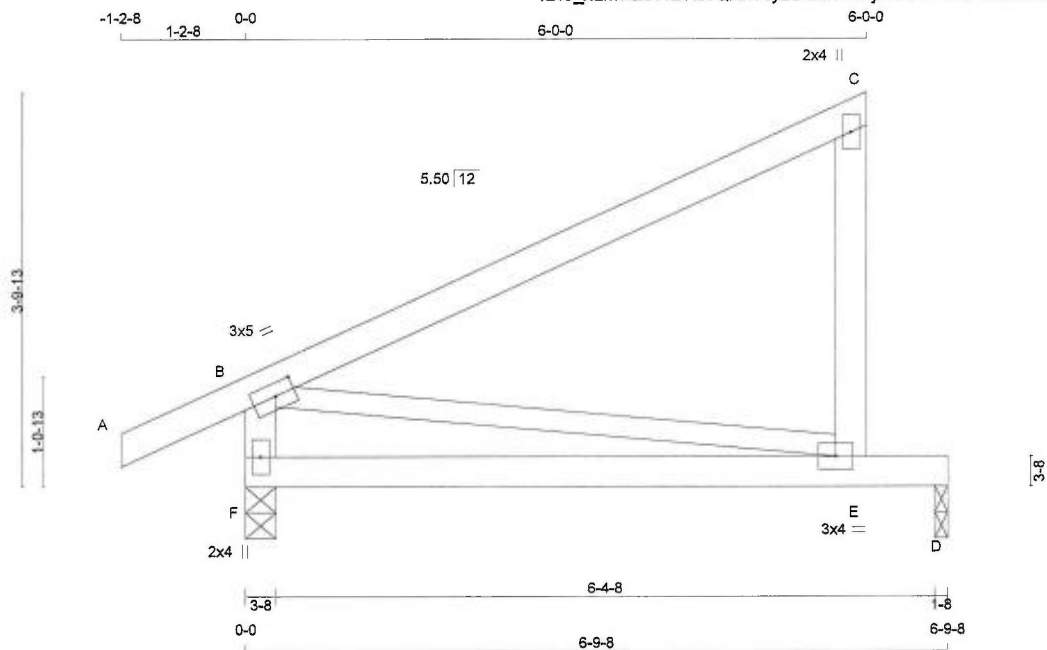


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - F 2x4 DRY No.2 SPF K - A 2x4 DRY No.2 SPF G - E 2x4 DRY No.2 SPF K - J 2x4 DRY No.2 SPF J - I 2x4 DRY No.2 SPF I - H 2x4 DRY No.2 SPF H - G 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER.					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th></th><th>FACTORED</th><th>MAXIMUM FACTORED</th><th>INPUT</th><th>REQRD</th></tr><tr><th></th><th>GROSS REACTION</th><th>GROSS REACTION</th><th>BRG</th><th>BRG</th></tr><tr><th>JT</th><th>VERT</th><th>HORZ</th><th>DOWN</th><th>HORZ</th></tr><tr><td>K</td><td>574</td><td>0</td><td>574</td><td>0</td></tr><tr><td>G</td><td>674</td><td>0</td><td>674</td><td>0</td></tr></table> A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K TO RESIST THE MAX FACTORED REACTIONS. UNFACTORED REACTIONS <table><tr><th>JT</th><th>1ST LCASE</th><th>MAX./MIN. COMPONENT REACTIONS</th><th></th><th></th><th></th><th></th><th></th></tr><tr><th></th><th>COMBINED</th><th>SNOW</th><th>LIVE</th><th>PERM.LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr><tr><td>K</td><td>403</td><td>279 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>124 / 0</td><td>0 / 0</td></tr><tr><td>G</td><td>471</td><td>340 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>131 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4)											FACTORED	MAXIMUM FACTORED	INPUT	REQRD		GROSS REACTION	GROSS REACTION	BRG	BRG	JT	VERT	HORZ	DOWN	HORZ	K	574	0	574	0	G	674	0	674	0	JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS							COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	K	403	279 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0	G	471	340 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 33.6 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 27.2 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.40") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.40") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03") CSI: TC=0.12/1.00 (C-D:1), BC=0.11/1.00 (I-J:1), WB=0.11/1.00 (C-I:1), SSI=0.10/1.00 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2264 1656				
	FACTORED	MAXIMUM FACTORED	INPUT	REQRD																																																																								
	GROSS REACTION	GROSS REACTION	BRG	BRG																																																																								
JT	VERT	HORZ	DOWN	HORZ																																																																								
K	574	0	574	0																																																																								
G	674	0	674	0																																																																								
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS																																																																										
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL																																																																					
K	403	279 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0																																																																					
G	471	340 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0																																																																					

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
Jun 05, 2018

KOTT



TOTAL WEIGHT = 5 X 26 = 129 lb
[M]F

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
E - C	2x4	DRY	No.2
F - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.25
C	TMV+p	MT20	2.0	4.0		
E	BMVW-t	MT20	3.0	4.0		
F	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX
F	419	0	419	0	3-8	3-8	
D	267	0	267	0	1-8	1-8	

UNFACTORED REACTIONS							
JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE			
F	292	215 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
D	189	123 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LOAD LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 20	-77.4	-77.4 0.09 (1)	B-E	0 / 0	0.00 (1)	
B-C	0 / 0	-77.4	-77.4 0.47 (1)				
E-C	-232 / 0	0.0	0.0 0.05 (1)				
F-B	-330 / 0	0.0	0.0 0.03 (1)				
F-E	0 / 0	-18.2	-18.2 0.30 (1)				
E-D	0 / 0	-18.2	-18.2 0.28 (1)				

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/401 (0.20")

CSI: TC=0.47/1.00 (B-C:1) , BC=0.30/1.00 (E-F:1) , WB=0.00/1.00 (B-E:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284

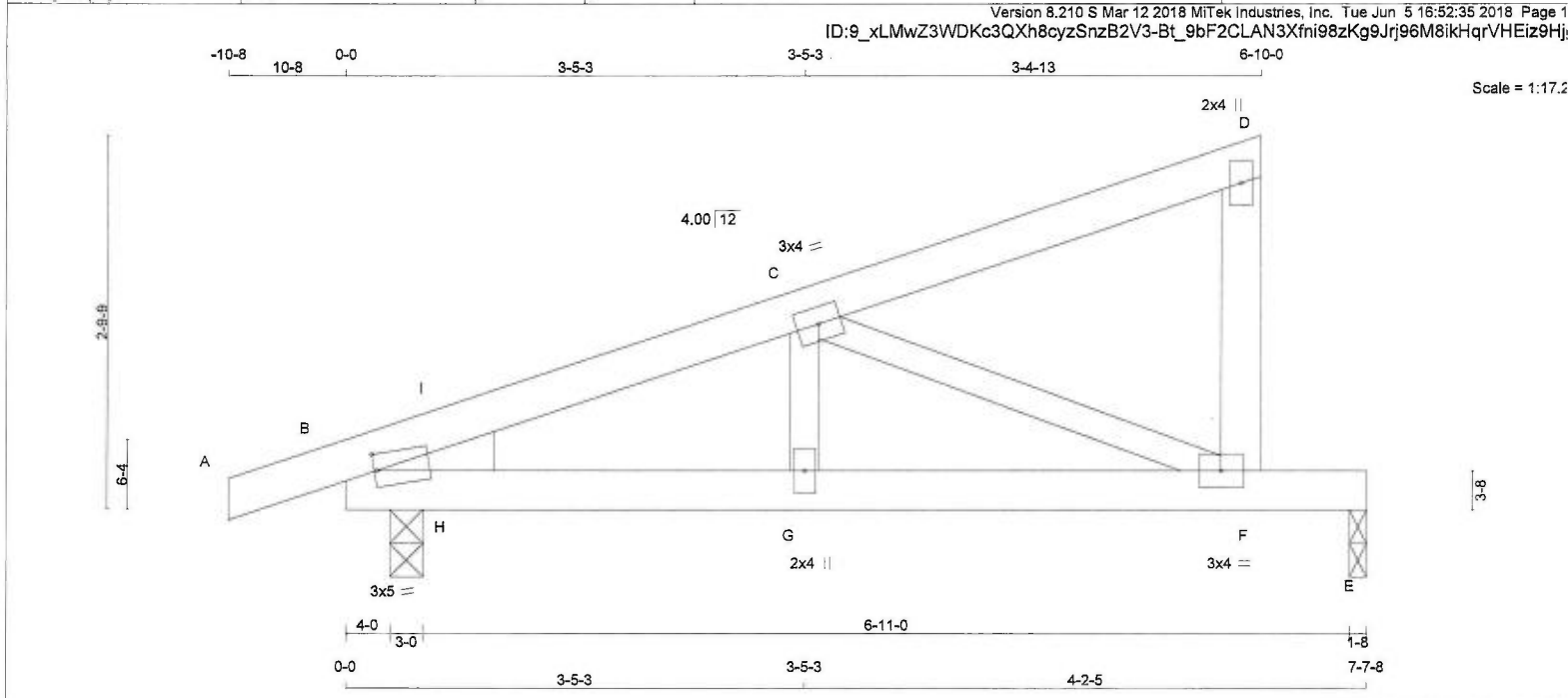
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.06 (F) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS

SIZE

DRY

LUMBER

DESCR.

SPF

SPF

SPF

SPF

SPF

ALL WEBS

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	5.0	1.50	0.25
C	TMWW-t	MT20	3.0	4.0		
D	TMV-p	MT20	2.0	4.0		
F	BMVW-t	MT20	3.0	4.0		
G	BMW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT
E	306	0	0
B	433	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
E	217
B	302

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	FORCE	MAX	CS1 (LC)
FR-TO							LENGTH	FR-TO			
A-B	0/3	-77.4	-77.4	0.05	(1)	10.00	G-C	0/170	0.04	(1)	
B-I	-864/0	-77.4	-77.4	0.04	(1)	6.25	C-F	-632/0	0.14	(1)	
I-C	-615/0	-77.4	-77.4	0.07	(1)	6.25	H-I	-4/23	0.00	(1)	
C-D	-6/0	-77.4	-77.4	0.10	(1)	10.00					
F-D	-114/0	0.0	0.0	0.02	(1)	7.81					
B-H	0/586	-18.2	-18.2	0.10	(1)	10.00					
H-G	0/586	-18.2	-18.2	0.18	(1)	10.00					
G-F	0/586	-18.2	-18.2	0.42	(1)	10.00					
F-E	0/0	-18.2	-18.2	0.32	(1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")

CALCULATED VERT. DEFL.(LL) = L/999 (0.04")

ALLOWABLE DEFL.(TL)= L/360 (0.25")

CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.10/1.00 (C-D:1), BC=0.42/1.00 (F-G:1), WB=0.14/1.00 (C-F:1), SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)

JSI METAL= 0.22 (F) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

LICENSED PROFESSIONAL ENGINEER

N.A. EL-MASRI

APPROVING OF ONTARIO

Jun 05, 2018



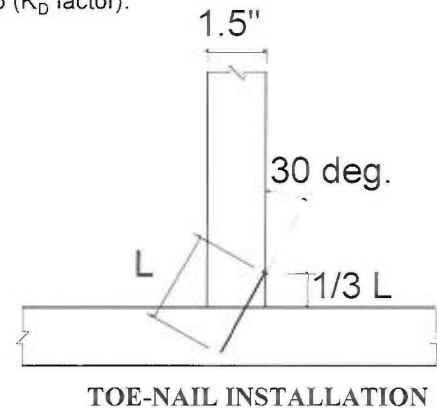
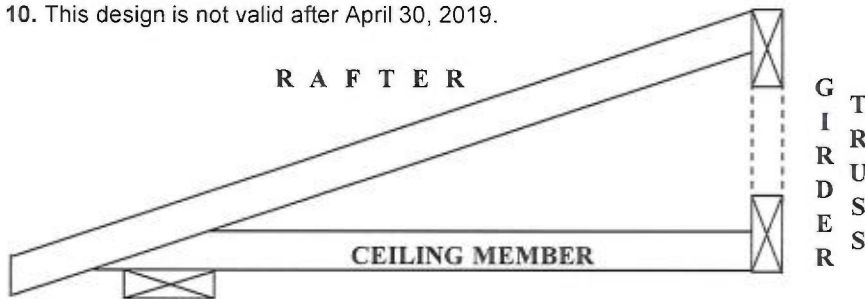
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

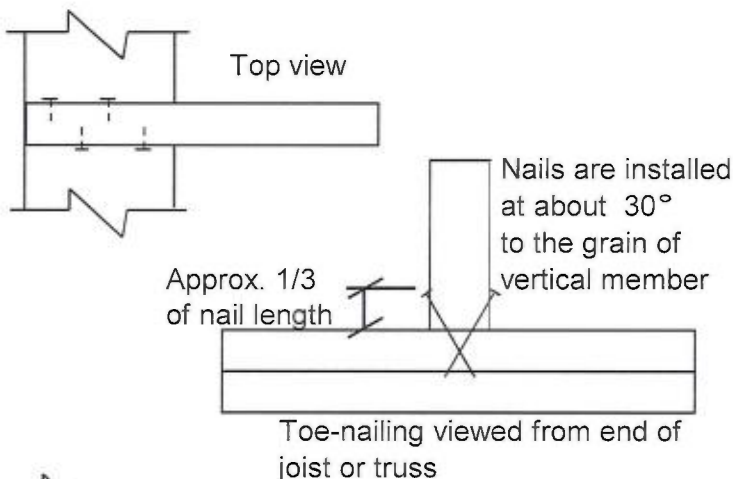
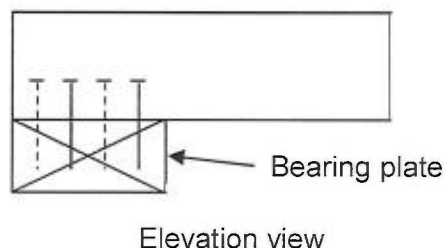
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

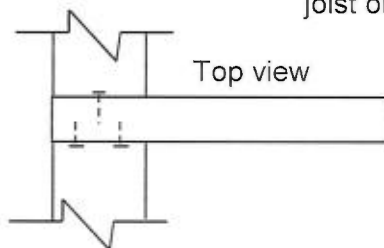
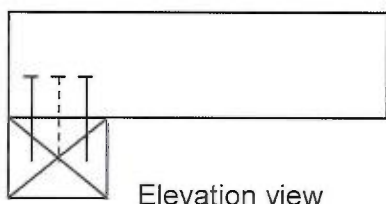
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek Canada Inc
100 Industrial Rd
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017