

NE0618-008
GREEN YORK HOMES-BELLE 1 EL 2

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

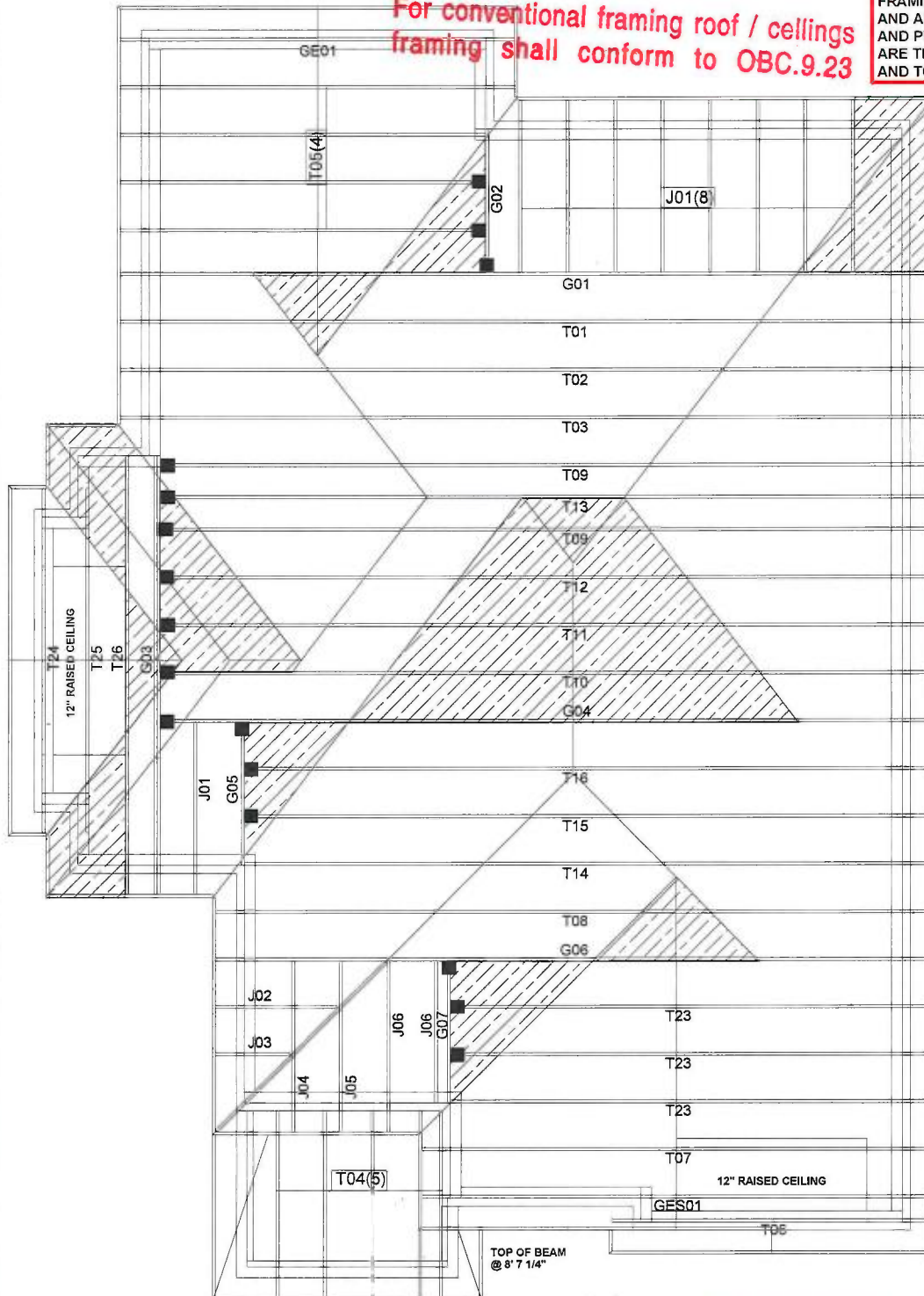
- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



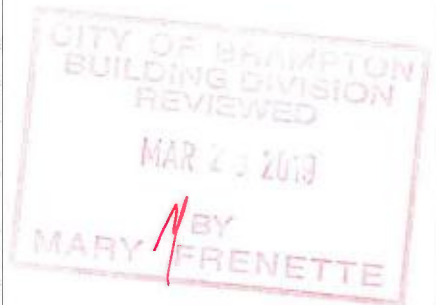
All work shall conform to the Ontario Building Code O. Reg. 332/12 as amended

THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.

For conventional framing roof / ceilings framing shall conform to OBC.9.23



ENG.JOB:NE0618-008



The components shown on this layout conform to our recommendations and are adequate to support the design loads required by the Ontario Building Code O.Reg 332/12 as amended and as shown on the engineering calculation page of each component.
The framing direction and girder locations shown on this layout conform to that shown on the architectural drawing.
Deflection: LL = L/360
1.33LL+DL=L/360

Architectural Drawing Info:
Date: MAY 22, 2018
Project number: 17-55
Model: BELLE 1

19-447194 RR



CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C.
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 S.P.F. @ 24" O/C WITH A 2x4 VERTICAL POST TO THE TRUSS UNDERNEATH EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

HANGER LEGEND:

▼ LUS24 ■ LJS26DS
● HGUS26 ✕ HGUS26-2

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3 PSF
BOT CH. LL = 0 PSF
DL = 7 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24" O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA S16-09
- TPIC 2011

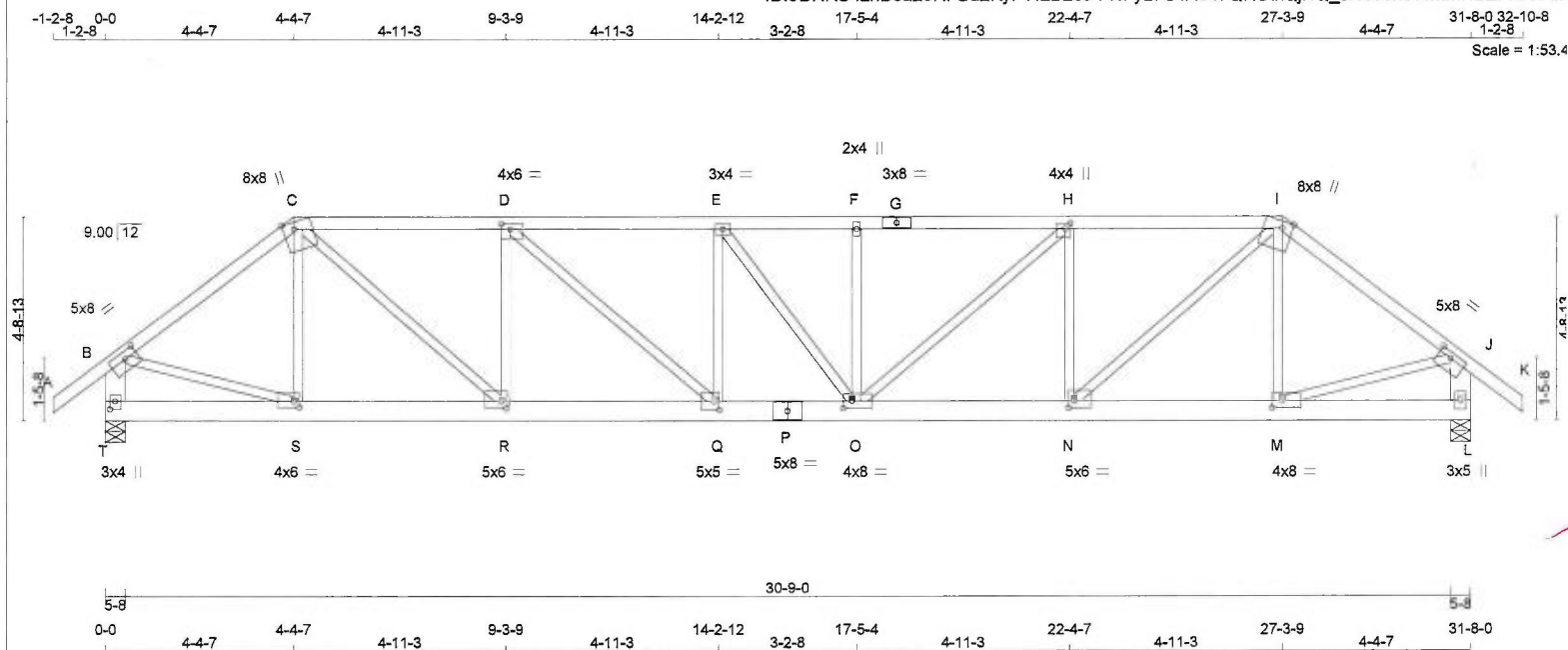
Model: BELLE 1 EL 2

Customer: GREEN YORK HOMES

Project: GRANELLI HOMES

Location: BRAMPTON

Date: 5/31/2018 Drawn by: BB



TOTAL WEIGHT = 153 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	2100F 1.8E	SPF
G - I	2x4	DRY	2100F 1.8E	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
--------------------	-----	-----	------	-----

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
J	TYPE	PLATES	W	LEN	Y X
B	TMWV-t	MT20	5.0	8.0	2.00 3.50
C	TTWV+m	MT20	8.0	8.0	Edge 3.00
D	TMWV-t	MT20	4.0	6.0	1.50 2.50
E	TMWV-t	MT20	3.0	4.0	
F	TMWV+m	MT20	2.0	4.0	
G	TS-t	MT20	3.0	8.0	
H	TMWV-t	MT20	4.0	4.0	1.50 1.50
I	TTWV+m	MT20	8.0	8.0	Edge 3.00
J	TMWV-t	MT20	5.0	8.0	1.50 3.50
L	BMV1+p	MT20	3.0	5.0	
M	BMWV-t	MT20	4.0	8.0	2.00 2.75
N	BMWV-t	MT20	5.0	6.0	2.00 1.50
O	BMWVW-t	MT20	4.0	8.0	2.00 2.50
P	BS-t	MT20	5.0	8.0	
Q	BMWV-t	MT20	5.0	5.0	2.50 1.50
R	BMWV-t	MT20	5.0	6.0	2.00 1.50
S	BMWV-t	MT20	4.0	6.0	1.75 1.50
T	BMV1+p	MT20	3.0	4.0	2.25 1.50

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BOILING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
T	2635	0	2635	0	0	5-8
L	3106	0	3106	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1847	1306 / 0	0 / 0	0 / 0	0 / 0	540 / 0	0 / 0
L	2179	1529 / 0	0 / 0	0 / 0	0 / 0	649 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T. L.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

LOADING
TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO			LENGTH	FR-TO			
A-B	0 / 30	-77.4	-77.4	0.10 (1)	10.00	S-C	-456 / 0	0.15 (1)	
B-C	-2959 / 0	-77.4	-77.4	0.49 (1)	3.60	C-R	0 / 2874	0.71 (1)	
C-D	-4509 / 0	-77.4	-77.4	0.34 (1)	3.85	R-D	-1798 / 0	0.58 (1)	
D-E	-8064 / 0	-77.4	-77.4	0.45 (1)	3.29	D-Q	0 / 2076	0.51 (1)	
E-F	-5938 / 0	-145.9	-145.9	0.36 (1)	3.34	Q-E	-204 / 11	0.07 (1)	
F-G	-5938 / 0	-145.9	-145.9	0.65 (1)	3.06	E-O	-213 / 0	0.10 (1)	
G-H	-5938 / 0	-145.9	-145.9	0.65 (1)	3.06	O-F	-583 / 0	0.19 (1)	
H-I	-4923 / 0	-145.9	-145.9	0.57 (1)	3.42	H-F	0 / 1355	0.34 (1)	
I-J	-3519 / 0	-77.4	-77.4	0.57 (1)	3.25	N-H	-1711 / 0	0.55 (1)	
J-K	0 / 30	-77.4	-77.4	0.10 (1)	10.00	N-I	0 / 2831	0.70 (1)	
T-B	-2604 / 0	0.0	0.0	0.19 (1)	6.39	M-I	-479 / 0	0.15 (1)	
L-J	-3046 / 0	0.0	0.0	0.22 (1)	6.00	B-S	0 / 2427	0.60 (1)	
						M-J	0 / 2885	0.71 (1)	
T-S	0 / 0	-17.5	-17.5	0.05 (4)	10.00				
S-R	0 / 2352	-17.5	-17.5	0.33 (1)	10.00				
R-Q	0 / 4509	-17.5	-17.5	0.67 (1)	10.00				
Q-P	0 / 8064	-33.0	-33.0	0.88 (1)	10.00				
P-O	0 / 8064	-33.0	-33.0	0.88 (1)	10.00				
O-N	0 / 4923	-33.0	-33.0	0.73 (1)	10.00				
N-M	0 / 2798	-33.0	-33.0	0.41 (1)	10.00				
M-L	0 / 0	-33.0	-33.0	0.08 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	27-3-9	-254	-254	—	FRONT	VERT	TOTAL
Q	14-2-12	-1117	-1117	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-4-7
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 17-5-4 OF SPAN
MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 833 (0.46")

CSI: TC=0.65/1.00 (F-H:1), BC=0.88/1.00 (O-Q:1),
WR=0.71/1.00 (J-M:1), SS=0.37/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-BELLE 1 EL 2	DRWG NO.
NE0618-008	G01	1	1	TRUSS DESC.		PAGE 4 OF 46

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:24:46 2018 Page 2

ID:9DRKC4znB5a23RFBu2KyPWzB2c8-FNFyL7S4A347QRCTkQJNQ_JrW56h6WmmHBZFKEzAll'

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 253.5 lbs FACTORED DOWN AT 27-3-9 ON TOP CHORD, AND 1117.1 lbs FACTORED DOWN AT 14-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1656

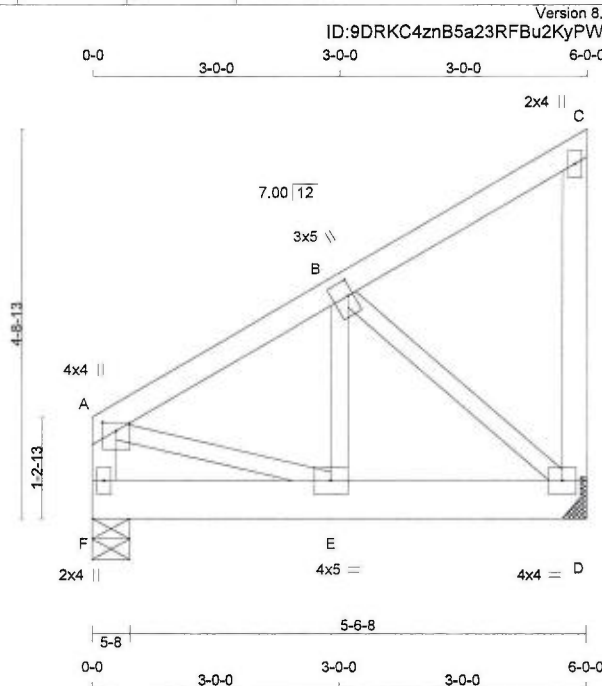
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 1.00 (P) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1/28.0

TOTAL WEIGHT = 31 lb [M]

LUMBER				DESCR.
CHORDS	SIZE	LUMBER		
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY				SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TMVW+t	MT20	3.0	5.0	2.25 0.75
C	TMV+p	MT20	2.0	4.0	
D	BMVW+t	MT20	4.0	4.0	
E	BMVW+t	MT20	4.0	5.0	
F	BMV+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	UP	BRG	BRG	BRG
F	1118	0	1118	0	0	5-8	5-8		
D	1118	0	1118	0	0	MECHANICAL			

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
F	784	549 / 0	0 / 0	0 / 0	0 / 0	236 / 0	0 / 0		
D	784	549 / 0	0 / 0	0 / 0	0 / 0	236 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. MEMB. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)
FR-TO		FROM TO			FR-TO		
F-A	-749 / 0	0.0	0.0 0.08 (1)	7.81	A-E	0 / 790	0.20 (1)
A-B	-870 / 0	-77.4	-77.4 0.12 (1)	6.25	E-B	0 / 821	0.20 (1)
B-C	-12 / 0	-77.4	-77.4 0.11 (1)	6.25	B-D	-1005 / 0	0.26 (1)
D-C	-92 / 0	0.0	0.0 0.03 (1)	7.81			
F-E	0 / 0	-295.2	-295.2 0.17 (1)	10.00			
E-D	0 / 761	-295.2	-295.2 0.27 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 14-2-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 14-2-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.27/1.00 (D-E:1),
WB=0.26/1.00 (B-D:1), SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	8.0	2.25	3.00
B	TTVW-m	MT20	6.0	8.0	Edge	3.25
C	TMVW-w	MT20	2.0	4.0		
D	TMVW-t	MT20	3.0	4.0		
E	TTVW+m	MT20	8.0	8.0	Edge	3.00
F	TMVW-t	MT20	6.0	8.0	1.75	4.00
H	BMV1+p	MT20	3.0	4.0	2.25	1.50
I	BMVW-t	MT20	6.0	6.0	3.00	1.50
J	BMVW+t	MT20	4.0	10.0	5.00	1.75
K	BS-t	MT20	5.0	8.0		
L	BMVWV-t	MT20	5.0	8.0		
M	BMVW-t	MT20	8.0	8.0	4.00	3.25
N	BMV1+t	MT20	6.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3602.1 lbs FACTORED DOWN AT 11-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

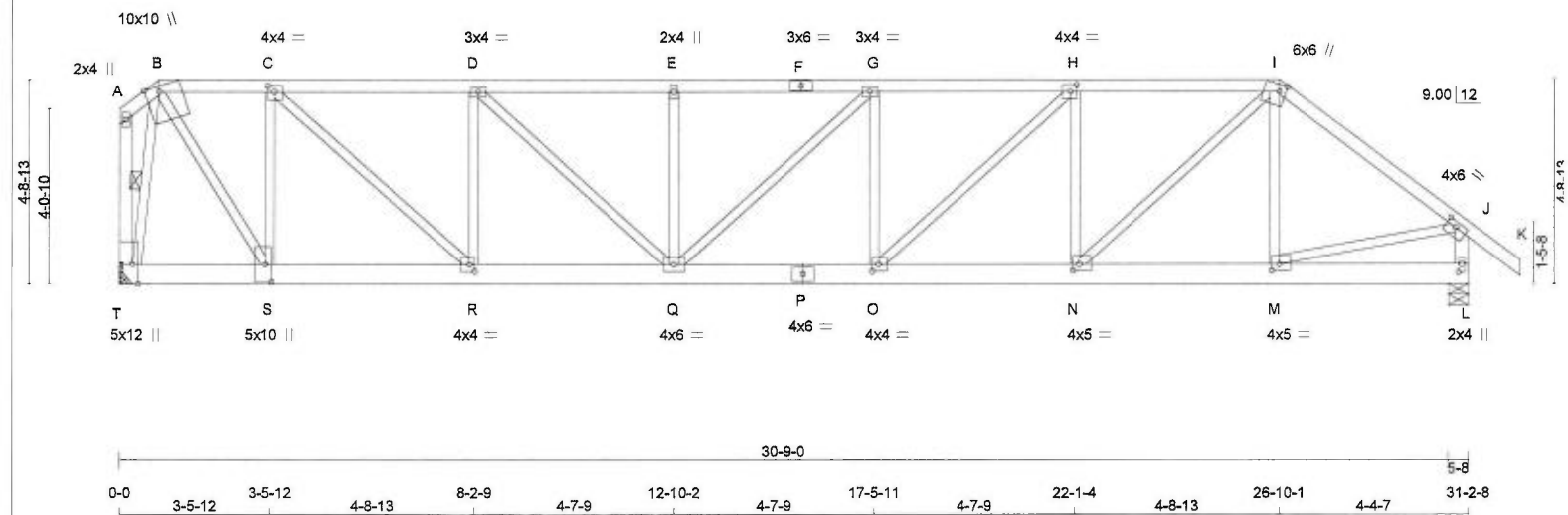
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 1.00 (K) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

T - A

2x4

DRY

No.2

SPF

B - F

2x4

DRY

No.2

SPF

F - I

2x4

DRY

No.2

SPF

I - K

2x4

DRY

No.2

SPF

L - J

2x4

DRY

No.2

SPF

A - B

2x4

DRY

No.2

SPF

T - P

2x6

DRY

No.2

SPF

P - L

2x6

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES

(table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

A

TMV+p

MT20

2.0

4.0

B

TTWW+m

MT20

10.0

10.0

Edge

4.00

C

TMWW-t

MT20

4.0

4.0

1.75

1.75

D

TMWW-t

MT20

3.0

4.0

E

TMW+w

MT20

2.0

4.0

F

TS-t

MT20

3.0

6.0

G

TMWW-t

MT20

3.0

4.0

H

TMWW-t

MT20

4.0

4.0

2.00

1.75

I

TTWW+m

MT20

6.0

6.0

2.00

1.75

J

TMVW-t

MT20

4.0

6.0

1.50

2.75

L

BMV1+p

MT20

2.0

4.0

2.25

1.00

M

BMWW-t

MT20

4.0

5.0

1.75

2.00

N

BMWW-t

MT20

4.0

5.0

1.75

1.75

O

BMWW-t

MT20

4.0

4.0

2.00

1.75

P

BS-t

MT20

4.0

6.0

Q

BMWWW-t

MT20

4.0

6.0

R

BMWW-t

MT20

4.0

4.0

2.00

1.50

S

BMWW+t

MT20

5.0

10.0

5.00

1.75

T

BMVW1+t

MT20

5.0

12.0

Edge

1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 0-0

END SETBACK = 6-0-0

END WALL WIDTH = 5-8

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDTL LOADS BASED ON 55 % OF GSL.

LOADS APPLIED TO FIRST 3-5-12 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")

ALLOWABLE DEFL.(TL)= L/360 (1.04")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.50/1.00 (G-H-1) , BC=0.50/1.00 (O-Q-1) ,

WB=0.92/1.00 (B-S-1) , SSI=0.19/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

INPUT

REQRD

GROSS REACTION

GROSS REACTION

BRG

BRG

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

T

3602

0

3602

0

0

MECHANICAL

MECHANICAL

L

1828

0

1828

0

0

5-8

5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT T TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN.

COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

T

2528

1769 / 0

0 / 0

0 / 0

0 / 0

759 / 0

0 / 0

L

1281

909 / 0

0 / 0

0 / 0

0 / 0

372 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF B-T. DBS = 8-0-0 . CBF = 176 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FACTORED

WEBS

MAX. FACTORED

MEMB.

FORCE

VERT. LOAD

MEMB.

FORCE

MAX

(LBS)

(PLF)

(LBS)

CSI (LC)

FR-TO

FROM

TO

LENGTH

FR-TO

T-A

-86 / 0

0.0

0.0

0.02 (1)

7.81

B-S

0 / 3736

0.92 (1)

B-C

-2626 / 0

-145.9

-145.9

0.25 (1)

4.02

S-C

-1089 / 0

0.35 (1)

C-D

-3396 / 0

-77.4

-77.4

0.45 (1)

3.43

C-R

0 / 1047

0.26 (1)

D-E

-3627 / 0

-77.4

-77.4

0.48 (1)

3.30

R-D

-592 / 0

0.19 (1)

E-F

-3627 / 0

-77.4

-77.4

0.44 (1)

3.34

D-Q

0 / 316

0.08 (1)

F-G

-3627 / 0

-77.4

-77.4

0.44 (1)

3.34

Q-E

-354 / 0

0.11 (1)

G-H

-3416 / 0

-77.4

-77.4

0.50 (1)

3.36

Q-G

0 / 289

0.07 (1)

H-I

-2742 / 0

-77.4

-77.4

0.42 (1)

3.79

O-G

-546 / 0

0.18 (1)

I-J

-1936 / 0

-77.4

-77.4

0.38 (1)

4.43

O-H

0 / 925

0.23 (1)

J-K

0 / 30

-77.4

-77.4

0.10 (1)

10.00

N-H

-1031 / 0

0.33 (1)

L-J

-1797 / 0

0.0

0.0

0.20 (1)

6.19

N-I

0 / 1632

0.40 (1)

A-B

0 / 0

-145.9

-145.9

0.02 (1)

10.00

M-I

-265 / 0

0.09 (1)

M-J

0 / 1588

0.39 (1)

T-S

0 / 724

-33.0

-33.0

0.17 (1)

10.00

T-B

-3526 / 0

0.68 (1)

S-R

0 / 2626

-17.5

-17.5

0.42 (1)

10.00

R-Q

0 / 3396

-17.5

-17.5

0.49 (1)

10.00

Q-P

0 / 3416

-17.5

-17.5

0.50 (1)

10.00

P-O

0 / 3416

-17.5

-17.5

0.50 (1)

10.00

O-N

0 / 2743

-17.5

-17.5

0.40 (1)

10.00

N-M

0 / 1539

-17.5

-17.5

0.23 (1)

10.00

M-L

0 / 0

-17.5

-17.5

0.04 (4)

10.00

FACTORED CONCENTRATED LOADS (LBS)

JT

LOC.

LC1

MAX-

MAX+

FACE

DIR.

TYPE

S

3-5-12

-2077

-2077

-

FRONT

VERT

TOTAL

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

LICENSED PROFESSIONAL ENGINEER

N.A. EL-MASRI

PROVINCE OF ONTARIO

Jun 02, 2018

CONTINUED ON PAGE 2

KOTT

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2077.5 lbs FACTORED DOWN AT 3-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

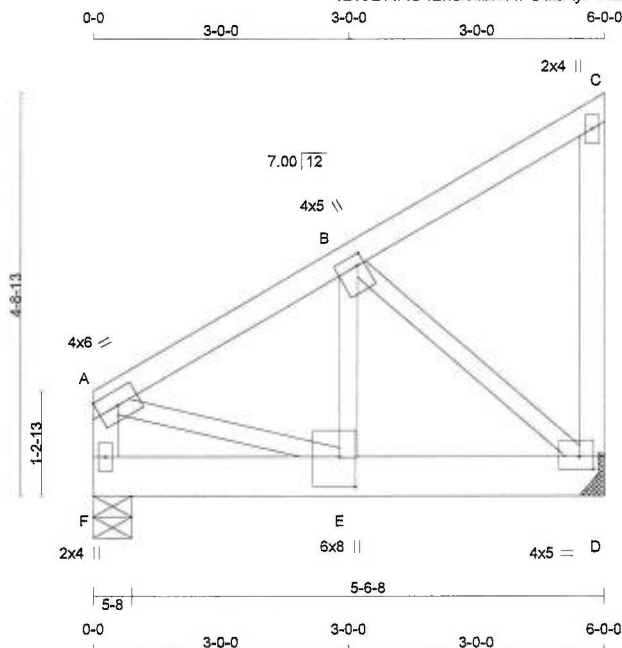
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.77 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 31 lb
[M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+t	MT20	4.0	6.0		Edge
B	TMVW+t	MT20	4.0	5.0	1.50	1.00
C	TMV+p	MT20	2.0	4.0		
D	BMVW1+t	MT20	4.0	5.0	1.75	2.00
E	BMVW+t	MT20	6.0	8.0	4.25	2.25
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
F	2078 0	2078 0	5-8	5-8
D	2078 0	2078 0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	1458	1020 / 0	0 / 0	0 / 0	0 / 0	0 / 0	438 / 0	0 / 0
D	1458	1020 / 0	0 / 0	0 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
F-A	-1315 / 0	0.0	0.0 0.15 (1)	A-E	0 / 1467	0.36 (1)	
A-B	-1627 / 0	-77.4	-77.4 0.13 (1)	E-B	0 / 1772	0.44 (1)	
B-C	-10 / 0	-77.4	-77.4 0.09 (1)	B-D	-1866 / 0	0.49 (1)	
D-C	-97 / 0	0.0	0.0 0.03 (1)				
F-E	0 / 0	-615.3	-615.3 0.35 (1)				
E-D	0 / 1413	-615.3	-615.3 0.54 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 3.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder

START DISTANCE = 0-0

START SPAN CARRIED = 27-8-0

END DISTANCE = 6-0-0

END SPAN CARRIED = 27-8-0

END WALL WIDTH = 5-8

APPLIED TO FRONT SIDE OF BOTTOM CHORD.

- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.15/1.00 (A-F:1), BC=0.54/1.00 (D-E:1), WB=0.49/1.00 (B-D:1), SSI=0.72/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PSI)	(PL)	(PL)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



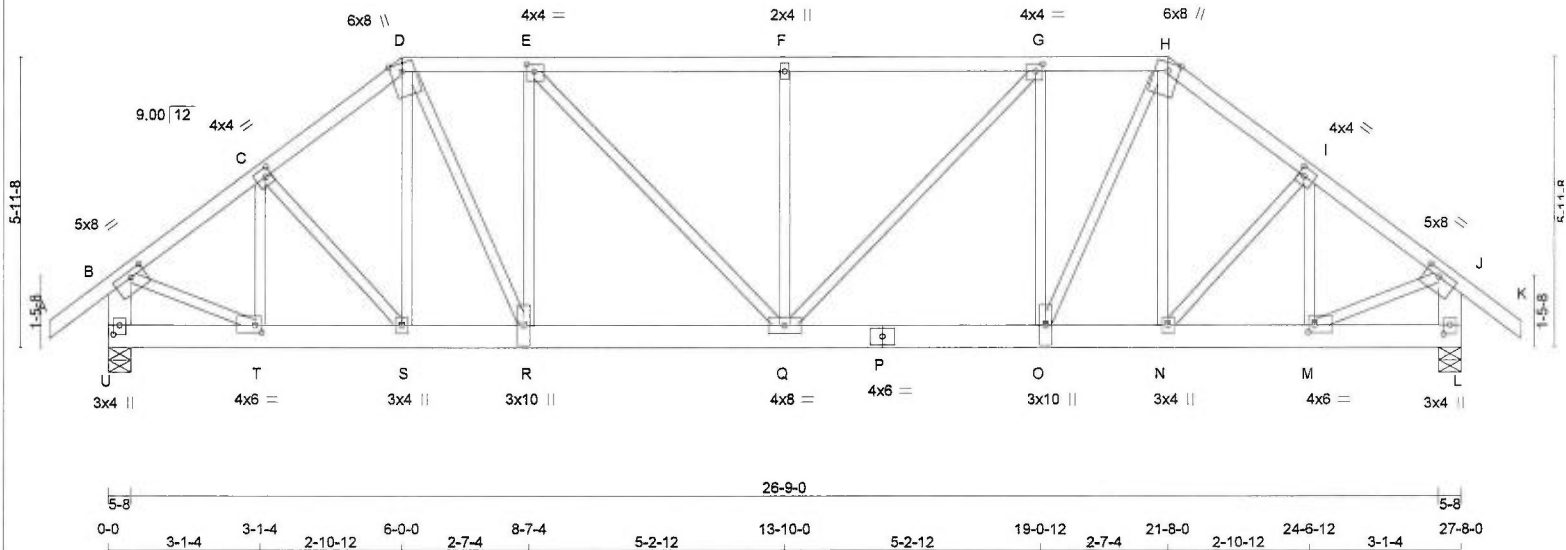
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				*** SPECIAL LOADS ANALYSIS ***			
A - D	2x4	DRY	No.2	FACTORED				GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.			
D - H	2x4	DRY	No.2	GROSS REACTION				LOADS WERE DERIVED FROM USER INPUT			
H - K	2x4	DRY	No.2	JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	NO FURTHER MODIFICATIONS WERE MADE			
U - B	2x6	DRY	No.2	U	2936	0	2936 0 0 5-8	SPECIFIED LOADS:			
L - J	2x6	DRY	No.2	L	2010	0	2010 0 0 5-8	TOP CH. LL = 23.3 PSF			
U - P	2x6	DRY	No.2	UNFACTORED REACTIONS				DL = 3.0 PSF			
P - L	2x6	DRY	No.2	JT	COMBINED	SNOW	LIVE	BOT CH. LL = 0.0 PSF			
				U	2061	1442 / 0	0 / 0	DL = 7.0 PSF			
				L	1408	1002 / 0	0 / 0	TOTAL LOAD = 33.3 PSF			
ALL WEBS 2x3 DRY No.2				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L				SPACING = 24.0 IN. C/C			
EXCEPT				BRACING				LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
DRY: SEASONED LUMBER.				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.				GIRDER TYPE: CPrimeHip			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				SIDE SETBACK = 6-0-0			
				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				END SETBACK = 6-0-0			
				LOADING				END WALL WIDTH = 5-8			
				TOTAL LOAD CASES: (4)				CORNER FRAMING TYPE: CONVENTIONAL			
				CHORDS				END JACK TYPE: CONVENTIONAL			
				MEMB. MAX. FACTORED FORCE (LBS)				APPLIED TO FRONT SIDE			
				VERT. LOAD LC1 MAX (PLF)				- ADD'L LOADS BASED ON 55 % OF GSL			
				CSI (LC)				LOADS APPLIED TO FIRST 8-7-4 OF SPAN			
				UNBRACED LENGTH FR-TO				MEASURED FROM THE LEFT.			
				MEMB. MAX. FACTORED FORCE (LBS)				*** NON STANDARD GIRDER ***			
				CSI (LC)				ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.			
				FR-TO				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
				A-B				THIS DESIGN COMPLIES WITH:			
				B-C				- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014			
				C-D				- CSA 086-09			
				D-E				- TPIC 2011			
				E-F				(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD			
				F-G				ALLOWABLE DEFL.(LL)= L/360 (0.92")			
				G-H				CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")			
				H-I				ALLOWABLE DEFL.(TL)= L/360 (0.92")			
				I-J				CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")			
				J-K				CSI: TC=0.50/1.00 (E-F:1) , BC=0.53/1.00 (Q-R:1) ,			
				K-L				WB=0.64/1.00 (B-T:1) , SSI=0.20/1.00 (D-E:1)			
				U-B				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00			
				L-J				COMP=1.00 SHEAR=1.00 TENS= 1.00			
				U-T				COMPANION LIVE LOAD FACTOR = 0.50			
				T-S				CONTINUED ON PAGE 2			
				S-R				KOTT			
				R-Q							
				Q-P							
				P-O							
				O-N							
				N-M							
				M-L							



READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 6-0-0 ON TOP CHORD, AND 1461.9 lbs FACTORED DOWN AT 8-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

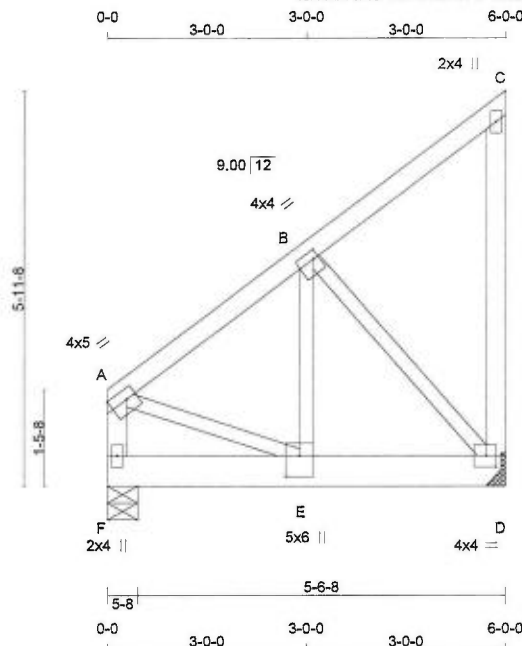
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.75 (P) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 34 lb
[M]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS 2x3 DRY No.2				SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	5.0	1.75	Edge
B	TMVW-t	MT20	4.0	4.0	1.50	1.00
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	4.0	2.00	1.75
E	BMVW+t	MT20	5.0	6.0	3.75	2.50
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
F	1461	0	0	1461	0	0	5-8	5-8	
D	1461	0	0	1461	0	0	MECHANICAL		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
F	1026	718 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0	
D	1026	718 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO		FR-TO		
F-A	-956 / 0	0.0	0.0 0.11 (1)	7.81	A-E	0 / 833	0.21 (1)
A-B	-969 / 0	-77.4	-77.4 0.13 (1)	6.15	E-B	0 / 1173	0.29 (1)
B-C	-15 / 0	-77.4	-77.4 0.11 (1)	8.25	B-D	-1169 / 0	0.37 (1)
D-C	-92 / 0	0.0	0.0 0.06 (1)	7.81			
F-E	0 / 0	-409.8	-409.8 0.24 (1)	10.00			
E-D	0 / 787	-409.8	-409.8 0.34 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 19-0-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 19-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.13/1.00 (A-B:1), BC=0.34/1.00 (D-E:1), WB=0.37/1.00 (B-D:1), SSI=0.48/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



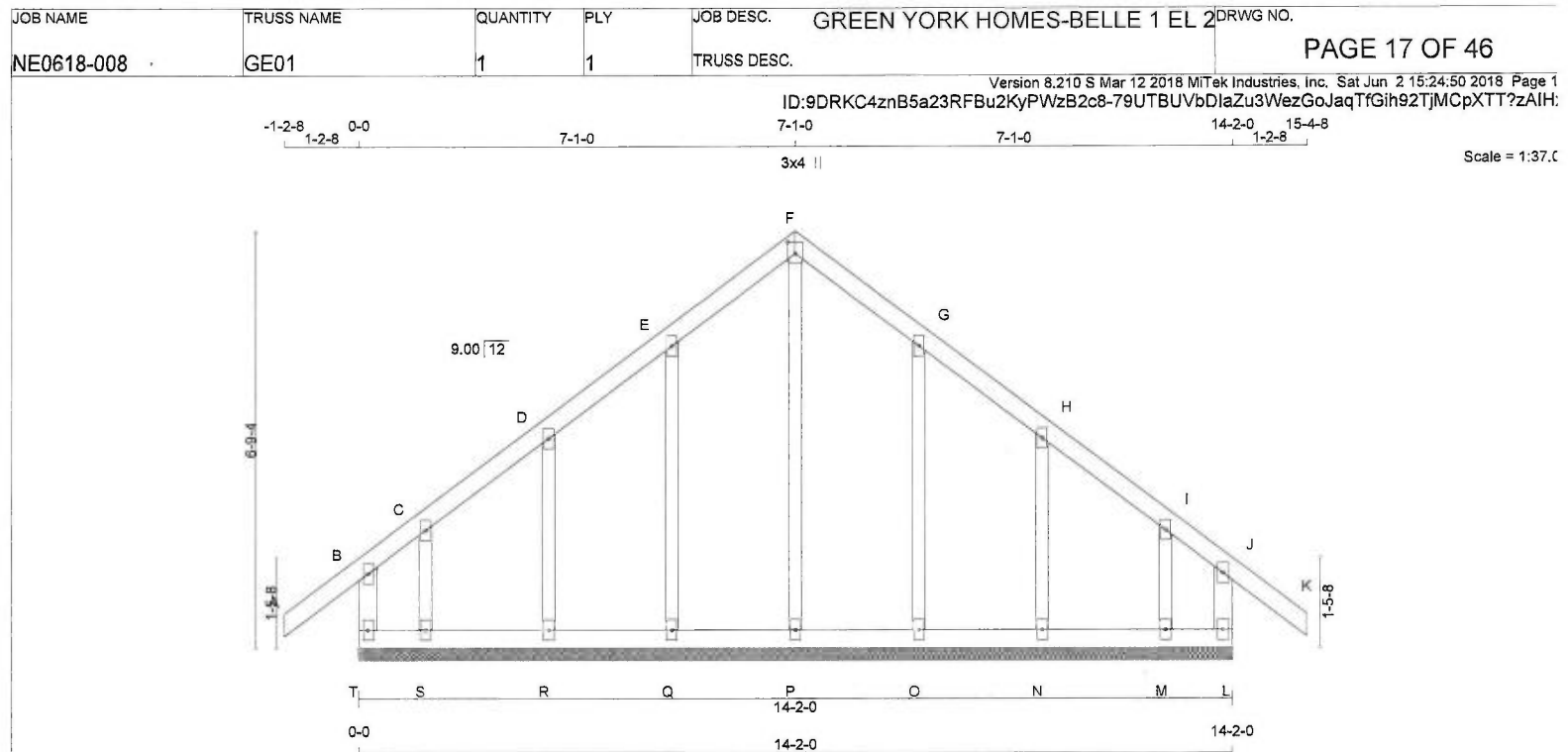
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.31 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 63 lb [M]

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER	
BEARINGS	
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.	
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.	
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)	
BRACING	
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.	
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.	
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C, D, E, G, H, I					
C	TMW+w	MT20	2.0	4.0	
F	TTW+p	MT20	3.0	4.0	2.25 1.50
J	TMV+p	MT20	2.0	4.0	
L	BMV1+p	MT20	2.0	4.0	
M, N, O, P, Q, R, S					
M	BMW1+w	MT20	2.0	4.0	
T	BMV1+p	MT20	2.0	4.0	

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM TO		FR-TO			
T-B	-165 / 0	0.0 0.0	0.03 (1)	P-F	-189 / 0	0.14 (1)	
A-B	0 / 30	-77.4 -77.4	0.09 (1)	Q-E	-157 / 0	0.06 (1)	
B-C	-8 / 0	-77.4 -77.4	0.06 (1)	R-D	-159 / 0	0.04 (1)	
C-D	0 / 20	-77.4 -77.4	0.04 (1)	S-C	-73 / 0	0.01 (1)	
D-E	0 / 21	-77.4 -77.4	0.04 (1)	O-G	-157 / 0	0.06 (1)	
E-F	0 / 24	-77.4 -77.4	0.04 (1)	N-H	-159 / 0	0.04 (1)	
F-G	0 / 24	-77.4 -77.4	0.04 (1)	M-I	-73 / 0	0.01 (1)	
G-H	0 / 21	-77.4 -77.4	0.04 (1)				
H-I	0 / 20	-77.4 -77.4	0.04 (1)				
I-J	-8 / 0	-77.4 -77.4	0.06 (1)				
J-K	0 / 30	-77.4 -77.4	0.09 (1)				
L-J	-165 / 0	0.0 0.0	0.03 (1)				
T-S	-9 / 0	-17.5 -17.5	0.01 (1)				
S-R	-13 / 0	-17.5 -17.5	0.01 (4)				
R-Q	-17 / 0	-17.5 -17.5	0.01 (4)				
Q-P	-20 / 0	-17.5 -17.5	0.01 (4)				
P-O	-20 / 0	-17.5 -17.5	0.01 (4)				
O-N	-17 / 0	-17.5 -17.5	0.01 (4)				
N-M	-13 / 0	-17.5 -17.5	0.01 (4)				
M-L	-9 / 0	-17.5 -17.5	0.01 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	33.3	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (J-K:1), BC=0.01/1.00 (Q-R:4), WB=0.14/1.00 (F-P:1), SSI=0.06/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

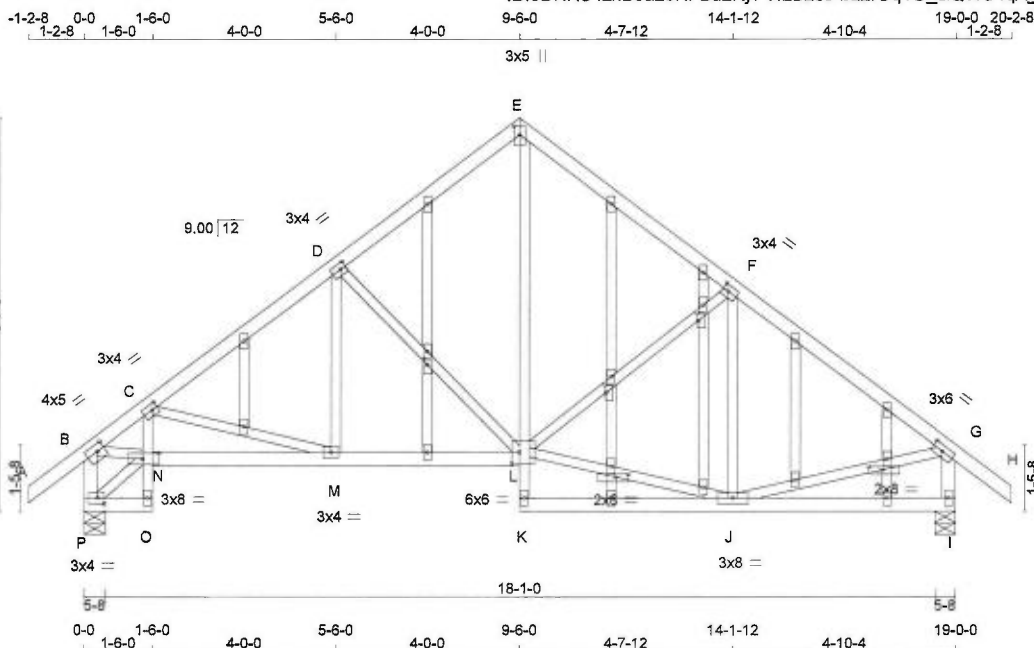
JSI GRIP= 0.14 (F) (INPUT = 0.90)
JSI METAL= 0.06 (F) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:50.2

TOTAL WEIGHT = 111 lb [M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
P - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
P - O	2x4	DRY	No.2
O - C	2x3	DRY	No.2
N - L	2x4	DRY	No.2
K - E	2x3	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
I	998	0	0	998	0	0	0	5-8	5-8
P	1005	0	0	1005	0	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	698	501 / 0	0 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
P	703	505 / 0	0 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED (LBS)	MAX (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	C-M	-209 / 0	0.06 (1)
B-C	-1312 / 0	-77.4	-77.4 0.09 (1)	5.57	M-D	0 / 118	0.04 (4)
C-D	-1055 / 0	-77.4	-77.4 0.16 (1)	5.95	D-L	-397 / 0	0.24 (1)
D-E	-747 / 0	-77.4	-77.4 0.15 (1)	6.25	L-J	0 / 743	0.17 (1)
E-F	-763 / 0	-77.4	-77.4 0.23 (1)	6.25	L-F	-185 / 0	0.11 (1)
F-G	-888 / 0	-77.4	-77.4 0.23 (1)	6.24	J-F	-231 / 16	0.09 (1)
G-H	0 / 30	-77.4	-77.4 0.09 (1)	10.00	B-N	0 / 1040	0.23 (1)
P-B	-979 / 0	0.0	0.0 0.10 (1)	7.81	J-G	0 / 751	0.17 (1)
I-G	-964 / 0	0.0	0.0 0.10 (1)	7.81	P-N	-22 / 0	0.00 (1)
P-O	0 / 17	-17.5	-17.5 0.01 (4)	10.00			
O-N	0 / 14	0.0	0.0 0.05 (1)	10.00			
N-C	0 / 41	0.0	0.0 0.06 (1)	10.00			
N-M	0 / 1057	-17.5	-17.5 0.21 (1)	10.00			
M-L	0 / 856	-17.5	-17.5 0.18 (1)	10.00			
K-L	0 / 33	0.0	0.0 0.02 (1)	10.00			
L-E	0 / 623	0.0	0.0 0.16 (1)	10.00			
K-J	0 / 6	-17.5	-17.5 0.11 (4)	10.00			
J-I	0 / 0	-17.5	-17.5 0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.23/1.00 (F-G:1), BC=0.21/1.00 (M-N:1), WB=0.24/1.00 (D-L:1), SSI=0.14/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

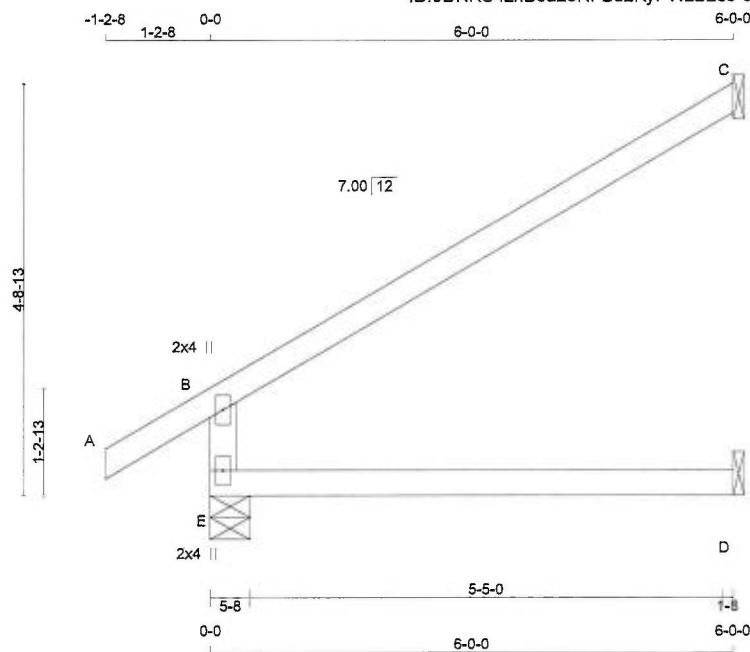
JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:26.5

TOTAL WEIGHT = 9 X 17 = 157 lb
[MIF]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
E	451	0	0	451	0	5-8	5-8		
C	174	0	0	174	0	1-8	1-8		
D	44	0	0	49	0	1-8	1-8		

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
E	314	234 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0		
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0		
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
E-B	-389 / 0	0.0	0.0 0.11 (4)				7.81
A-B	0 / 25	-77.4	-77.4 0.09 (1)				10.00
B-C	-29 / 0	-77.4	-77.4 0.36 (1)				6.25
E-D	0 / 0	-17.5	-17.5 0.13 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY)		SHEAR (PLI)		SECTION (PLI)	
MAX	MIN	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656	

PLATE PLACEMENT TOL. = 0.250 inches

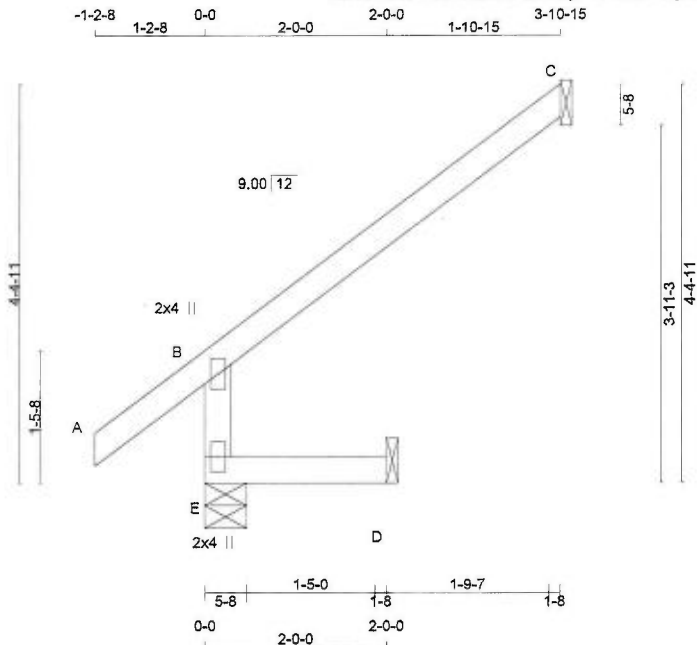
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:25.4

TOTAL WEIGHT = 11 lb

[M

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	308	0	308	0	5-8	5-8
C	114	0	114	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	212	174 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	FR-TO
FR-TO		FROM	TO	FR-TO			
E-B	-289 / 0	0.0	0.0 0.01 (4)			7.81	
A-B	0 / 30	-77.4	-77.4 0.09 (1)			10.00	
B-C	-23 / 0	-77.4	-77.4 0.20 (1)			6.25	
E-D	0 / 0	-17.5	-17.5 0.02 (4)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD				= 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

A circular professional engineer seal for N.A. El-Masri, a Licensed Professional Engineer in the Province of Ontario. The seal includes the text "LICENSED PROFESSIONAL ENGINEER", "N.A. EL-MASRI", and "PROVINCE OF ONTARIO". A signature is written across the seal, and the date "Jun 02, 2018" is stamped at the bottom.

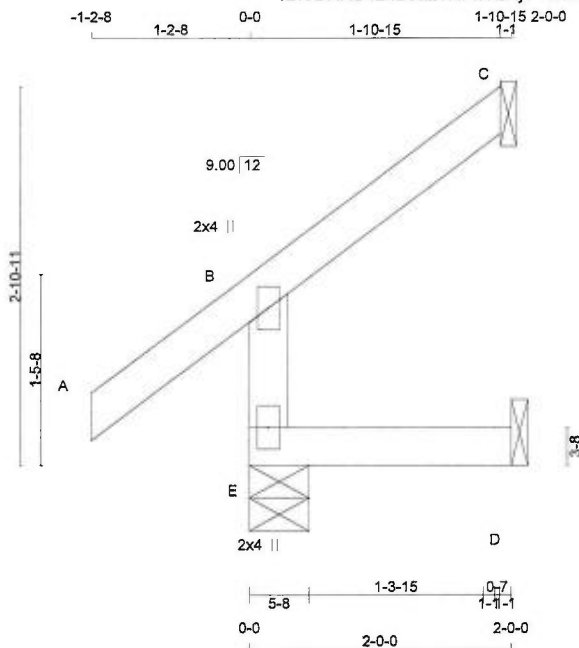
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

The logo for KOTT, featuring a stylized green and blue 'K' followed by the word "KOTT" in bold blue capital letters.



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 8 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS				FACTORED GROSS REACTION				MAXIMUM FACTORED GROSS REACTION				INPUT BRG				REQRD BRG			
JT	VERT	HORZ	DOWN	JT	VERT	HORZ	DOWN	JT	VERT	HORZ	DOWN	JT	VERT	HORZ	DOWN	JT	VERT	HORZ	DOWN
E	211	0	211	0	0	0	0	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8
C	56	0	56	0	0	0	0	1-8	1-8	1-8	1-8	1-8	1-8	1-8	1-8	1-8	1-8	1-8	1-8
D	16	0	16	0	0	0	0	1-8	1-8	1-8	1-8	1-8	1-8	1-8	1-8	1-8	1-8	1-8	1-8

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	146	116 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)
FR-TO				FR-TO			
E-B	-192 / 0	0.0	0.0 (4)	7.61			
A-B	0 / 30	-77.4	-77.4 (1)	10.00			
B-C	-11 / 0	-77.4	-77.4 (1)	6.25			
E-D	0 / 0	-17.5	-17.5 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

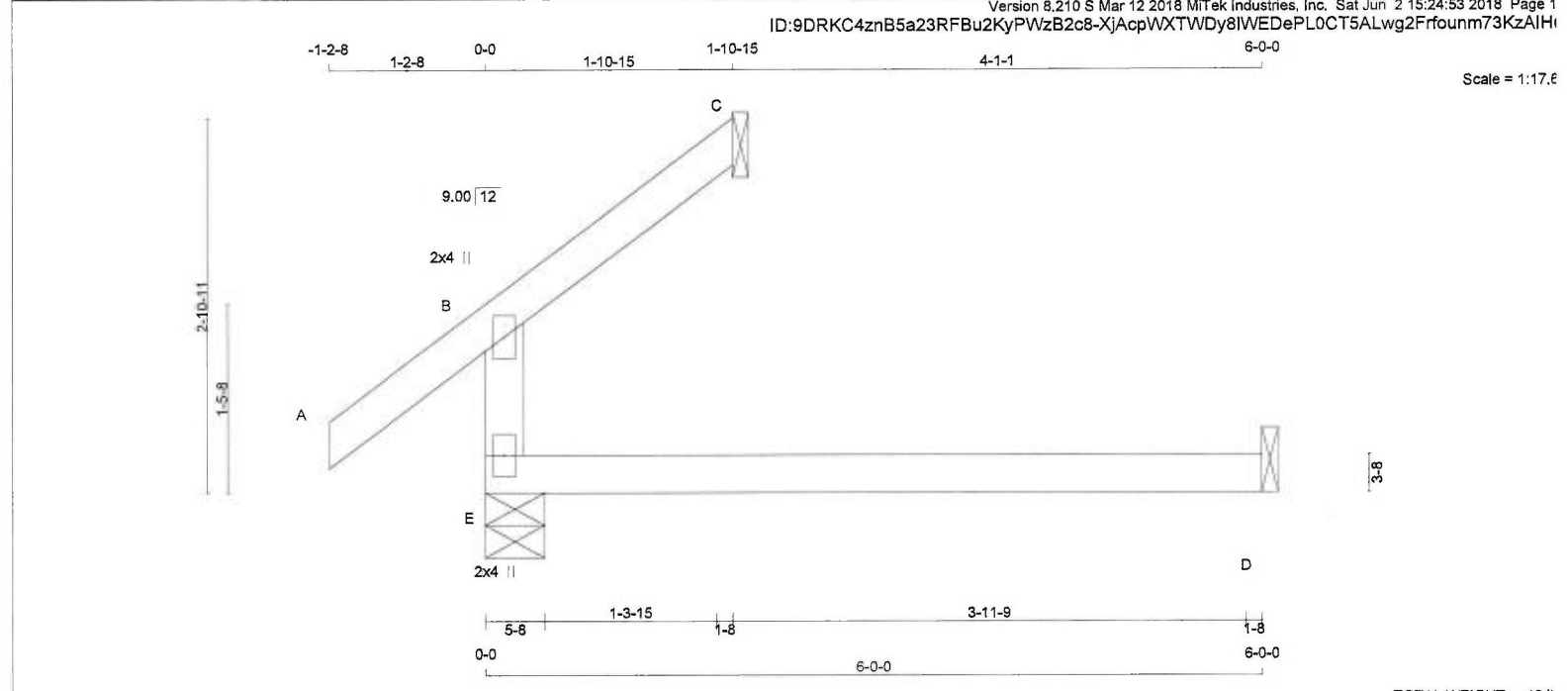
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



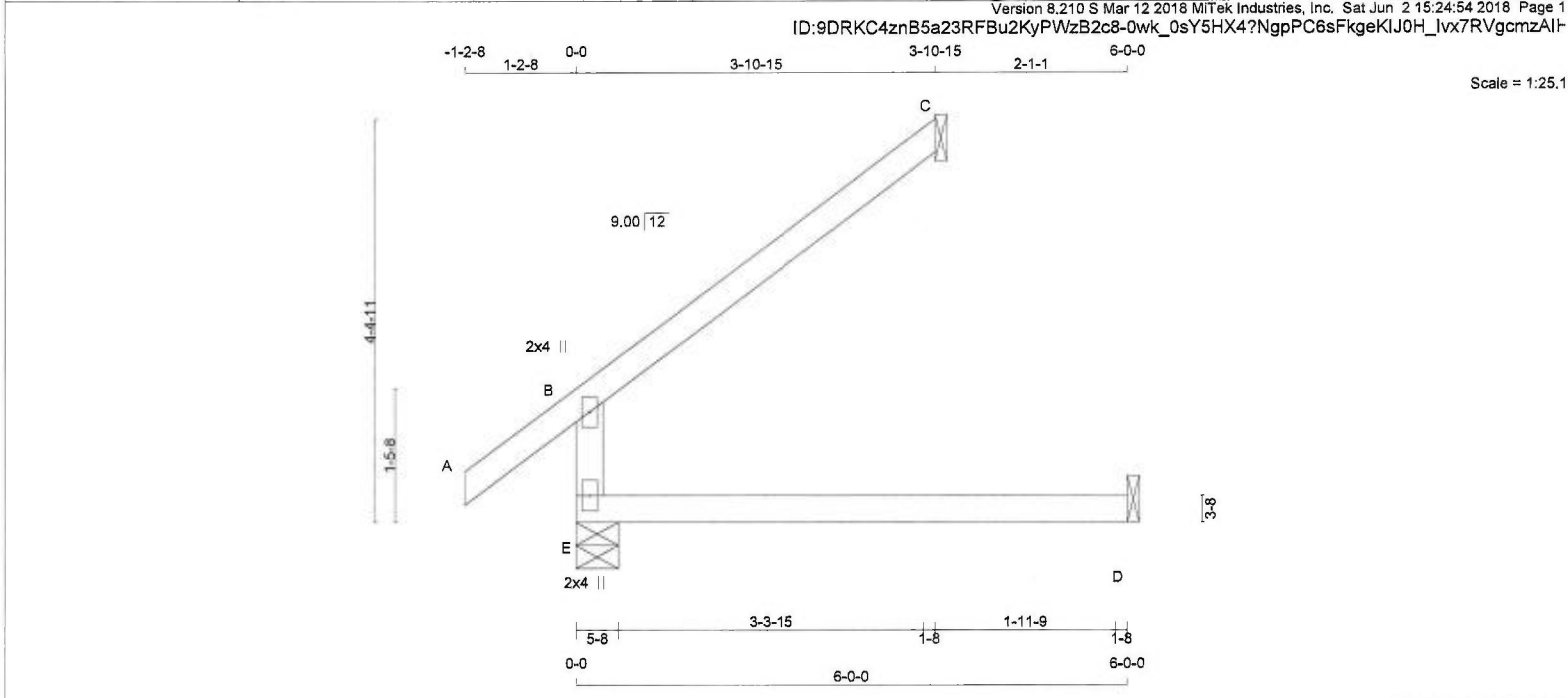


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF E - B 2x4 DRY No.2 SPF A - C 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.									
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMV+p MT20 2.0 4.0 E BMV1+p MT20 2.0 4.0									
DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.0 PSF TOTAL LOAD = 33.3 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, CBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03") CSI: TC=0.10/1.00 (B-E:4) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (D-E:4) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (Psi) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.13 (B) (INPUT = 0.90) JSI METAL= 0.05 (B) (INPUT = 1.00)									
BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT BRG REQRD BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX E 253 0 253 0 0 5-8 5-8 C 56 0 56 0 0 1-8 1-8 D 44 0 49 0 0 1-8 1-8 SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL E 179 116 / 0 0 / 0 0 / 0 0 / 0 64 / 0 0 / 0 C 38 34 / 0 0 / 0 0 / 0 0 / 0 4 / 0 0 / 0 D 35 0 / 0 0 / 0 0 / 0 0 / 0 35 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MEMB. FORCE FACTORED VERT. LOAD LC1 MAX. MEMB. FORCE FACTORED (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) FR-TO FROM TO E-B -192 / 0 0.0 0.0 0.10 (4) 7.81 A-B 0 / 30 -77.4 -77.4 0.09 (1) 10.00 B-C -11 / 0 -77.4 -77.4 0.04 (1) 6.25 E-D 0 / 0 -17.5 -17.5 0.13 (4) 10.00									



READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.



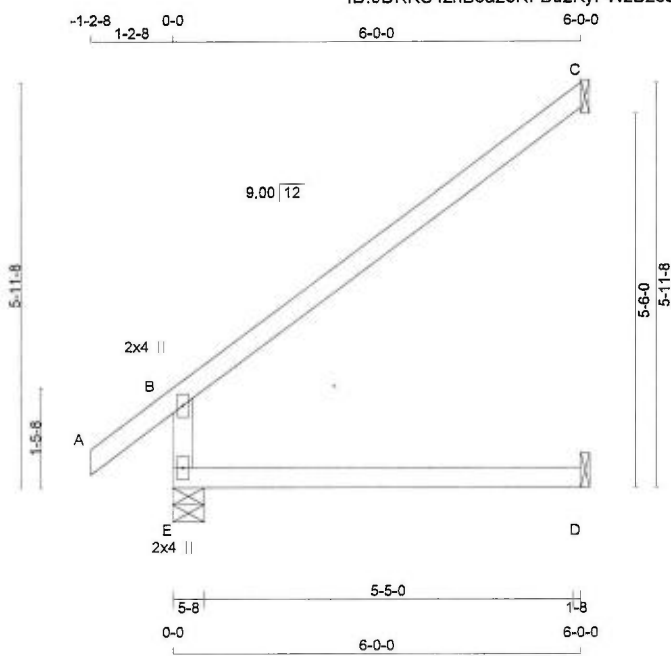


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER E - B 2x4 DRY No.2 A - C 2x4 DRY No.2 E - D 2x4 DRY No.2 DRY: SEASONED LUMBER.					DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.0 PSF TOTAL LOAD = 33.3 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, CBC 2012, ABC 2014 - CSA 088-09 - TPIC 2011 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03") CSI: TC=0.15/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (Psi) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.20 (B) (INPUT = 0.90) JSI METAL= 0.07 (B) (INPUT = 1.00)					UNFACTORED REACTIONS 1ST LOASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL E 245 174 / 0 0 / 0 0 / 0 0 / 0 71 / 0 0 / 0 C 77 68 / 0 0 / 0 0 / 0 0 / 0 9 / 0 0 / 0 D 35 0 / 0 0 / 0 0 / 0 0 / 0 35 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) C H O R D S W E B S MAX. FACTORED MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX MEMB. MAX. FACTORED (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) FR-TO FROM TO E-B -289 / 0 0.0 0.0 0.10 (4) 7.81 A-B 0 / 30 -77.4 -77.4 0.09 (1) 10.00 B-C -23 / 0 -77.4 -77.4 0.15 (1) 6.25 E-D 0 / 0 -17.5 -17.5 0.13 (4) 10.00				
---	--	--	--	--	--	--	--	--	--	--	--	--	--	--



READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.





Scale = 1:33.5

TOTAL WEIGHT = 2 X 19 = 37 lb [M]F

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	JT	VERT	JT	VERT	JT	VERT
E	451	E	451	E	5-8	E	5-8
C	174	C	174	C	1-8	C	1-8
D	44	D	49	D	1-8	D	1-8

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	314	235 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (PLF)
FR-TO		FROM	TO	FR-TO		FROM	TO
E-B	-390 / 0	0.0	0.0	0.10 (4)	7.81		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00		
B-C	-35 / 0	-77.4	-77.4	0.37 (1)	6.25		
E-D	0 / 0	-17.5	-17.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.37/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
			2284
			1656

PLATE PLACEMENT TOL. = 0.250 inches

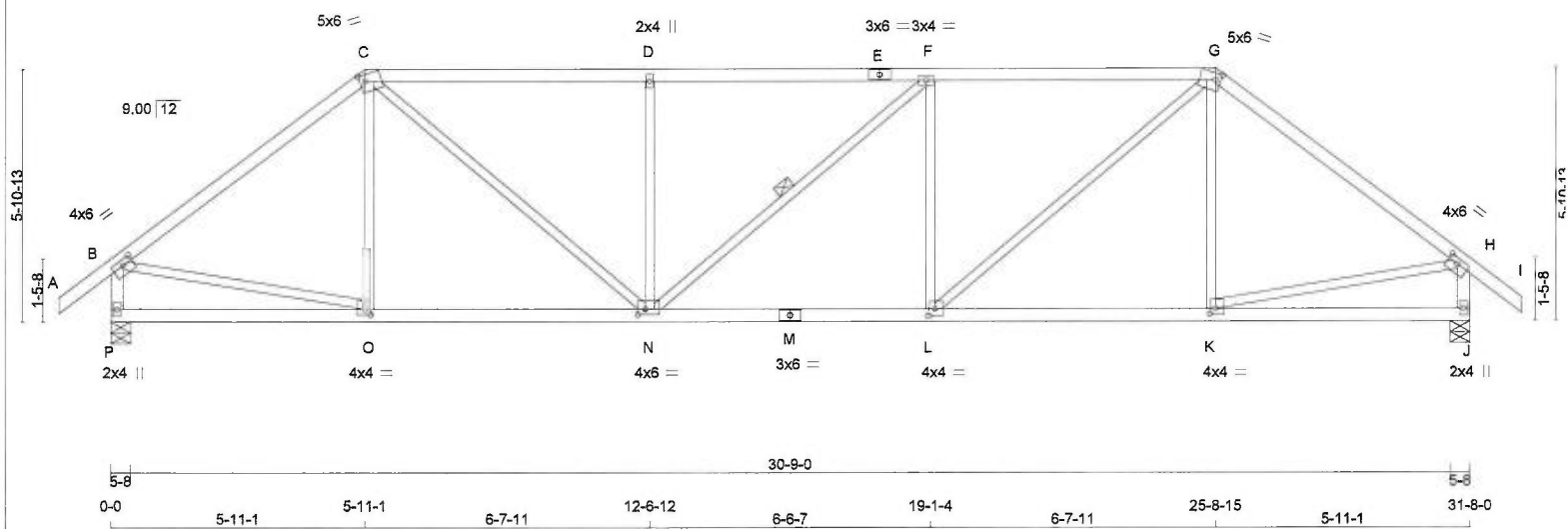
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
 JSI METAL= 0.10 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



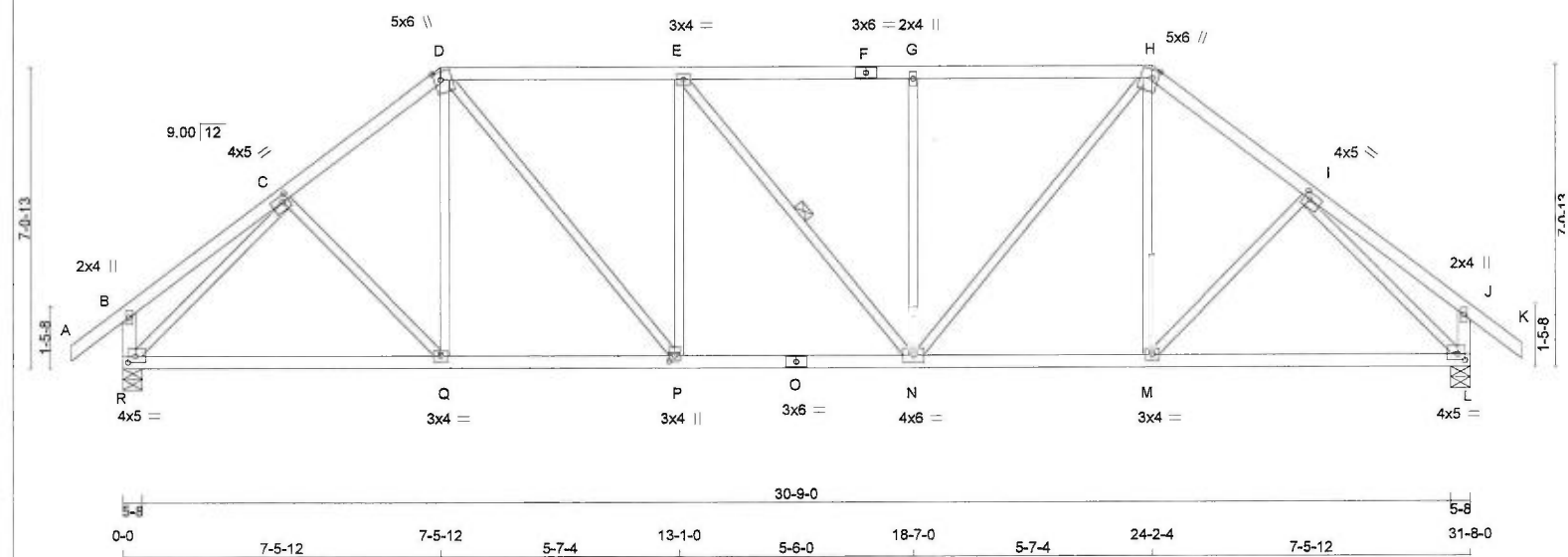


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS				DESIGN CRITERIA			
A - C	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:		
C - E	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL =	23.3	PSF
E - G	2x4	DRY	No.2	SPF	JT VERT	HORZ	DOWN	HORZ	DL =	3.0	PSF
G - I	2x4	DRY	No.2	SPF	P 1602	0	1602	0	BOT CH. LL =	0.0	PSF
P - B	2x4	DRY	No.2	SPF	J 1602	0	1602	0	DL =	7.0	PSF
J - H	2x4	DRY	No.2	SPF					TOTAL LOAD =	33.3	PSF
P - M	2x4	DRY	No.2	SPF							
M - J	2x4	DRY	No.2	SPF							
ALL WEBS	2x3	DRY	No.2	SPF	UNFACTORED REACTIONS				SPACING = 24.0 IN. C/C		
EXCEPT					1ST LCASE	MAX./MIN. COMPONENT REACTIONS			LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM		
					JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
					P 1122	798 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0
					J 1122	798 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0
					BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010		
					BRACING				THIS DESIGN COMPLIES WITH:		
					TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014		
					ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				- CSA 086-09		
					1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N. DBS = 20-0-0, CBF = 0 LBS.				- TPIC 2011		
					DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS; 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.				(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD		
					END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				ALLOWABLE DEFL.(LL)= L/360 (1.06")		
					LOADING				CALCULATED VERT. DEFL.(LL) = L/999 (0.11")		
					TOTAL LOAD CASES: (4)				ALLOWABLE DEFL.(TL)= L/360 (1.06")		
									CALCULATED VERT. DEFL.(TL) = L/999 (0.20")		
									CSI: TC=0.63/1.00 (F-G:1), BC=0.41/1.00 (L-N:1),		
									WB=0.30/1.00 (B-O:1), SSI=0.24/1.00 (F-G:1)		
									DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10		
									COMP=1.10 SHEAR=1.10 TENS= 1.10		
									COMPANION LIVE LOAD FACTOR = 0.50		
									TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.		
									NAIL VALUES		
									PLATE GRIP(DRY) SHEAR SECTION		
									(PSI) (PLI) (PLI)		
									MAX MIN MAX MIN MAX MIN		
									MT20 618 354 1667 822 2284 1656		
									PLATE PLACEMENT TOL. = 0.250 inches		
									PLATE ROTATION TOL. = 5.0 Deg.		
									JSI GRIP= 0.89 (G) (INPUT = 0.90)		
									JSI METAL= 0.62 (M) (INPUT = 1.00)		



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



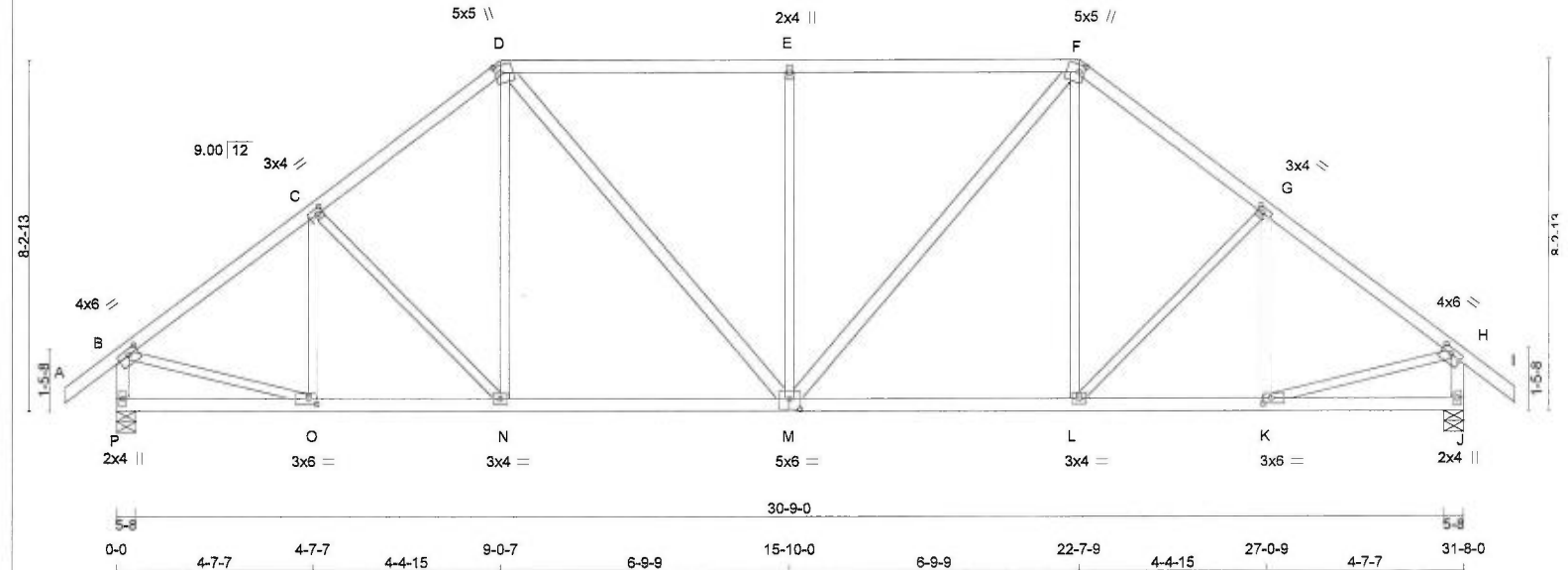


LUMBER N. L. G. A. RULES CHORDS SIZE A - D 2x4 DRY No.2 D - F 2x4 DRY No.2 F - H 2x4 DRY No.2 H - K 2x4 DRY No.2 R - B 2x4 DRY No.2 L - J 2x4 DRY No.2 R - O 2x4 DRY No.2 O - L 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 EXCEPT DRY: SEASONED LUMBER.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.0 PSF TOTAL LOAD = 33.3 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.06") CALCULATED VERT. DEFL.(LL)= L/999 (0.08") ALLOWABLE DEFL.(TL)= L/360 (1.06") CALCULATED VERT. DEFL.(TL)= L/999 (0.16") CSI: TC=0.40/1.00 (D-E:1), BC=0.33/1.00 (N-P:1), WB=0.87/1.00 (L-L:1), SSI=0.20/1.00 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.90 (N) (INPUT = 0.90) JSI METAL= 0.54 (I) (INPUT = 1.00)																																																																																																																																																																											
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMV+p MT20 2.0 4.0 C TMVW+t MT20 4.0 5.0 1.75 1.75 D TTWW+m MT20 5.0 6.0 2.25 1.75 E TMVW+t MT20 3.0 4.0 F TS-t MT20 3.0 6.0 G TMVW+w MT20 2.0 4.0 H TTWW+m MT20 5.0 6.0 2.25 1.75 I TMVW+t MT20 4.0 5.0 1.75 1.75 J TMV+p MT20 2.0 4.0 L BMVW1-t MT20 4.0 5.0 1.75 2.00 M BMVW-t MT20 3.0 4.0 N BMVWW-t MT20 4.0 6.0 O BS-t MT20 3.0 6.0 P BMVW+t MT20 3.0 4.0 1.50 1.50 Q BMVW-t MT20 3.0 4.0 R BMVW1-t MT20 4.0 5.0 1.75 2.00				UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL R 1122 798 / 0 0 / 0 0 / 0 324 / 0 0 / 0 L 1122 798 / 0 0 / 0 0 / 0 324 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.62 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. 1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N. DBS = 20-0-0. CBF = 0 LBS. DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) <table> <tr> <th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr> <tr> <th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. FACTORED VERT. LOAD (LC)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. FACTORED VERT. LOAD (LC)</th><th></th></tr> <tr> <td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td><td></td></tr> <tr> <td>A-B</td><td>0 / 30</td><td>-77.4 -77.4</td><td>0.09 (1)</td><td>C-Q</td><td>0 / 55</td><td>0.02 (4)</td><td></td></tr> <tr> <td>B-C</td><td>0 / 19</td><td>-77.4 -77.4</td><td>0.16 (1)</td><td>Q-D</td><td>0 / 120</td><td>0.04 (4)</td><td></td></tr> <tr> <td>C-D</td><td>-1624 / 0</td><td>-77.4 -77.4</td><td>0.20 (1)</td><td>D-P</td><td>0 / 716</td><td>0.16 (1)</td><td></td></tr> <tr> <td>D-E</td><td>-1741 / 0</td><td>-77.4 -77.4</td><td>0.40 (1)</td><td>P-E</td><td>-465 / 0</td><td>0.40 (1)</td><td></td></tr> <tr> <td>E-F</td><td>-1739 / 0</td><td>-77.4 -77.4</td><td>0.40 (1)</td><td>E-N</td><td>-2 / 0</td><td>0.00 (1)</td><td></td></tr> <tr> <td>F-G</td><td>-1739 / 0</td><td>-77.4 -77.4</td><td>0.40 (1)</td><td>N-G</td><td>-464 / 0</td><td>0.40 (1)</td><td></td></tr> <tr> <td>G-H</td><td>-1739 / 0</td><td>-77.4 -77.4</td><td>0.40 (1)</td><td>H-N</td><td>0 / 714</td><td>0.16 (1)</td><td></td></tr> <tr> <td>H-I</td><td>-1624 / 0</td><td>-77.4 -77.4</td><td>0.20 (1)</td><td>M-H</td><td>0 / 121</td><td>0.04 (4)</td><td></td></tr> <tr> <td>I-J</td><td>0 / 19</td><td>-77.4 -77.4</td><td>0.16 (1)</td><td>M-I</td><td>0 / 55</td><td>0.02 (4)</td><td></td></tr> <tr> <td>J-K</td><td>0 / 30</td><td>-77.4 -77.4</td><td>0.09 (1)</td><td>R-C</td><td>-1839 / 0</td><td>0.87 (1)</td><td></td></tr> <tr> <td>R-B</td><td>-213 / 0</td><td>0.0 0.0</td><td>0.02 (1)</td><td>L-L</td><td>-1839 / 0</td><td>0.87 (1)</td><td></td></tr> <tr> <td>L-J</td><td>-213 / 0</td><td>0.0 0.0</td><td>0.02 (1)</td><td></td><td></td><td></td><td></td></tr> <tr> <td>R-Q</td><td>0 / 1266</td><td>-17.5 -17.5</td><td>0.32 (1)</td><td></td><td></td><td></td><td></td></tr> <tr> <td>Q-P</td><td>0 / 1283</td><td>-17.5 -17.5</td><td>0.32 (1)</td><td></td><td></td><td></td><td></td></tr> <tr> <td>P-O</td><td>0 / 1741</td><td>-17.5 -17.5</td><td>0.33 (1)</td><td></td><td></td><td></td><td></td></tr> <tr> <td>O-N</td><td>0 / 1741</td><td>-17.5 -17.5</td><td>0.33 (1)</td><td></td><td></td><td></td><td></td></tr> <tr> <td>N-M</td><td>0 / 1284</td><td>-17.5 -17.5</td><td>0.33 (1)</td><td></td><td></td><td></td><td></td></tr> <tr> <td>M-L</td><td>0 / 1266</td><td>-17.5 -17.5</td><td>0.32 (1)</td><td></td><td></td><td></td><td></td></tr> </table>				CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)		FR-TO		FROM TO		FR-TO				A-B	0 / 30	-77.4 -77.4	0.09 (1)	C-Q	0 / 55	0.02 (4)		B-C	0 / 19	-77.4 -77.4	0.16 (1)	Q-D	0 / 120	0.04 (4)		C-D	-1624 / 0	-77.4 -77.4	0.20 (1)	D-P	0 / 716	0.16 (1)		D-E	-1741 / 0	-77.4 -77.4	0.40 (1)	P-E	-465 / 0	0.40 (1)		E-F	-1739 / 0	-77.4 -77.4	0.40 (1)	E-N	-2 / 0	0.00 (1)		F-G	-1739 / 0	-77.4 -77.4	0.40 (1)	N-G	-464 / 0	0.40 (1)		G-H	-1739 / 0	-77.4 -77.4	0.40 (1)	H-N	0 / 714	0.16 (1)		H-I	-1624 / 0	-77.4 -77.4	0.20 (1)	M-H	0 / 121	0.04 (4)		I-J	0 / 19	-77.4 -77.4	0.16 (1)	M-I	0 / 55	0.02 (4)		J-K	0 / 30	-77.4 -77.4	0.09 (1)	R-C	-1839 / 0	0.87 (1)		R-B	-213 / 0	0.0 0.0	0.02 (1)	L-L	-1839 / 0	0.87 (1)		L-J	-213 / 0	0.0 0.0	0.02 (1)					R-Q	0 / 1266	-17.5 -17.5	0.32 (1)					Q-P	0 / 1283	-17.5 -17.5	0.32 (1)					P-O	0 / 1741	-17.5 -17.5	0.33 (1)					O-N	0 / 1741	-17.5 -17.5	0.33 (1)					N-M	0 / 1284	-17.5 -17.5	0.33 (1)					M-L	0 / 1266	-17.5 -17.5	0.32 (1)				
CHORDS				WEBS																																																																																																																																																																											
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)																																																																																																																																																																									
FR-TO		FROM TO		FR-TO																																																																																																																																																																											
A-B	0 / 30	-77.4 -77.4	0.09 (1)	C-Q	0 / 55	0.02 (4)																																																																																																																																																																									
B-C	0 / 19	-77.4 -77.4	0.16 (1)	Q-D	0 / 120	0.04 (4)																																																																																																																																																																									
C-D	-1624 / 0	-77.4 -77.4	0.20 (1)	D-P	0 / 716	0.16 (1)																																																																																																																																																																									
D-E	-1741 / 0	-77.4 -77.4	0.40 (1)	P-E	-465 / 0	0.40 (1)																																																																																																																																																																									
E-F	-1739 / 0	-77.4 -77.4	0.40 (1)	E-N	-2 / 0	0.00 (1)																																																																																																																																																																									
F-G	-1739 / 0	-77.4 -77.4	0.40 (1)	N-G	-464 / 0	0.40 (1)																																																																																																																																																																									
G-H	-1739 / 0	-77.4 -77.4	0.40 (1)	H-N	0 / 714	0.16 (1)																																																																																																																																																																									
H-I	-1624 / 0	-77.4 -77.4	0.20 (1)	M-H	0 / 121	0.04 (4)																																																																																																																																																																									
I-J	0 / 19	-77.4 -77.4	0.16 (1)	M-I	0 / 55	0.02 (4)																																																																																																																																																																									
J-K	0 / 30	-77.4 -77.4	0.09 (1)	R-C	-1839 / 0	0.87 (1)																																																																																																																																																																									
R-B	-213 / 0	0.0 0.0	0.02 (1)	L-L	-1839 / 0	0.87 (1)																																																																																																																																																																									
L-J	-213 / 0	0.0 0.0	0.02 (1)																																																																																																																																																																												
R-Q	0 / 1266	-17.5 -17.5	0.32 (1)																																																																																																																																																																												
Q-P	0 / 1283	-17.5 -17.5	0.32 (1)																																																																																																																																																																												
P-O	0 / 1741	-17.5 -17.5	0.33 (1)																																																																																																																																																																												
O-N	0 / 1741	-17.5 -17.5	0.33 (1)																																																																																																																																																																												
N-M	0 / 1284	-17.5 -17.5	0.33 (1)																																																																																																																																																																												
M-L	0 / 1266	-17.5 -17.5	0.32 (1)																																																																																																																																																																												



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 149 lb [M]F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - I	2x4	DRY	No.2 SPF
P - B	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
P - M	2x4	DRY	No.2 SPF
M - J	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF
D - M	2x4	DRY	No.2 SPF
M - F	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	5.0	5.0	2.00	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	5.0	2.00	1.50
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	3.0	6.0	1.50	2.25
L	BMVW-t	MT20	3.0	4.0		
M	BSVW-t	MT20	5.0	6.0	3.00	3.00
N	BMVW-t	MT20	3.0	4.0		
O	BMVW-t	MT20	3.0	6.0	1.50	2.25
P	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
P	1602	0	1602	0	5-8	5-8
J	1602	0	1602	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
P	1122	798 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0
J	1122	798 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		
A-B	0 / 30	-77.4 -77.4 0.09 (1)	10.00	O-C -249 / 0 0.09 (1)
B-C	-1644 / 0	-77.4 -77.4 0.24 (1)	4.95	C-N -152 / 0 0.11 (1)
C-D	-1563 / 0	-77.4 -77.4 0.23 (1)	5.05	N-D 0 / 210 0.05 (1)
D-E	-1560 / 0	-77.4 -77.4 0.51 (1)	4.67	D-M 0 / 506 0.08 (1)
E-F	-1560 / 0	-77.4 -77.4 0.51 (1)	4.67	M-E -646 / 0 0.86 (1)
F-G	-1563 / 0	-77.4 -77.4 0.23 (1)	5.05	M-F 0 / 506 0.08 (1)
G-H	-1644 / 0	-77.4 -77.4 0.24 (1)	4.95	L-F 0 / 210 0.05 (1)
H-I	0 / 30	-77.4 -77.4 0.09 (1)	10.00	L-G -152 / 0 0.11 (1)
P-B	-1566 / 0	0.0 0.0 0.16 (1)	6.59	K-G -249 / 0 0.09 (1)
J-H	-1566 / 0	0.0 0.0 0.16 (1)	6.59	B-O 0 / 1374 0.31 (1)
				K-H 0 / 1374 0.31 (1)
P-O	0 / 0	-17.5 -17.5 0.08 (4)	10.00	
O-N	0 / 1334	-17.5 -17.5 0.28 (1)	10.00	
N-M	0 / 1231	-17.5 -17.5 0.29 (1)	10.00	
M-L	0 / 1231	-17.5 -17.5 0.29 (1)	10.00	
L-K	0 / 1334	-17.5 -17.5 0.28 (1)	10.00	
K-J	0 / 0	-17.5 -17.5 0.08 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.51/1.00 (D-E:1), BC=0.29/1.00 (L-M:1), WB=0.86/1.00 (E-M:1), SS=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

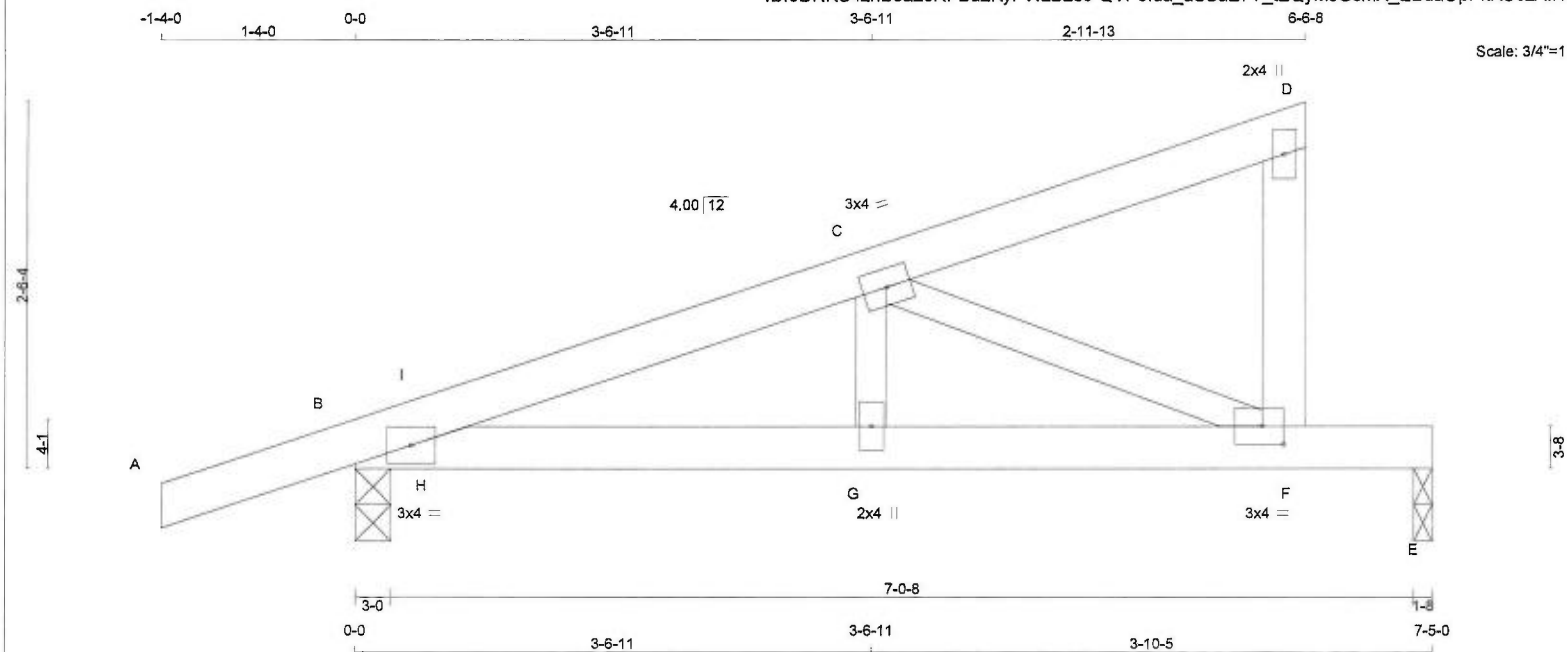
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.45 (H) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
F - D	2x4	DRY	No.2 SPF
B - E	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMBVW-I	MT20	3.0	4.0		
D	TMBV+P	MT20	2.0	4.0		
F	TMBVW-I	MT20	3.0	4.0	1.50	1.75
G	TMBV+W	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	454	0	454	0	3-0	3-0
E	288	0	288	0	1-8	1-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	317	235 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
E	204	134 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 16	-77.4 -77.4	0.10 (1)	10.00	G-C	0 / 197	0.04 (1)
B-I	-667 / 0	-77.4 -77.4	0.01 (1)	6.25	C-F	-676 / 0	0.13 (1)
I-C	-654 / 0	-77.4 -77.4	0.07 (1)	6.25	H-I	-95 / 8	0.00 (1)
C-D	-6 / 0	-77.4 -77.4	0.07 (1)	10.00			
F-D	-85 / 0	0.0 0.0	0.01 (1)	7.81			
B-H	0 / 625	-17.5 -17.5	0.17 (1)	10.00			
H-G	0 / 625	-17.5 -17.5	0.19 (1)	10.00			
G-F	0 / 625	-17.5 -17.5	0.43 (1)	10.00			
F-E	0 / 0	-17.5 -17.5	0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.10/1.00 (A-B:1), BC=0.43/1.00 (F-G:1), WB=0.13/1.00 (C-F:1), SS=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

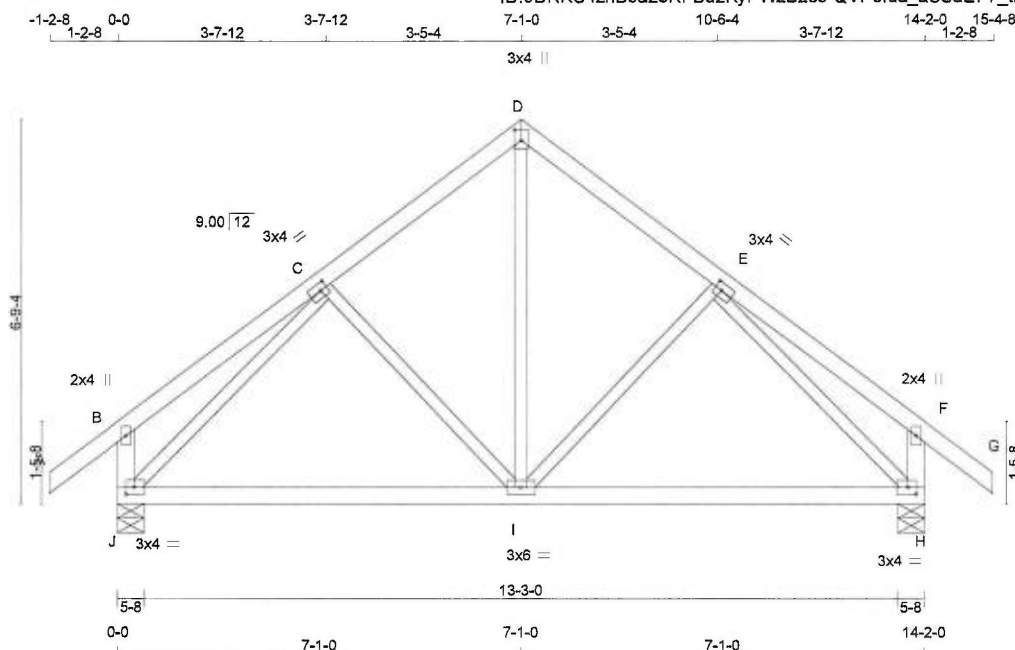
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.23 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:40.5

TOTAL WEIGHT = $4 \times 63 = 253$ lb

LUMBER

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4 DRY	No.2
D - G	2x4 DRY	No.2
J - B	2x4 DRY	No.2
H - F	2x4 DRY	No.2
J - H	2x4 DRY	No.2

DESCR.
SPF
SPF
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
		GROSS REACTION		GROSS REACTION		BRG	BRG
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
J		772	0	772	0	0	5-8
H		772	0	772	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	540	390 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
H	540	390 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J. H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

LOADING
TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	FORCE	MAX.	
	(LBS)	(PLF)	CSI (LC)	UNBRAC			(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH		FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	I-D	0 / 365	0.08 (1)	
B-C	0 / 19	-77.4	-77.4	0.15 (1)	10.00	I-E	-145 / 4	0.06 (1)	
C-D	-500 / 0	-77.4	-77.4	0.12 (1)	6.25	C-I	-145 / 4	0.06 (1)	
D-E	-500 / 0	-77.4	-77.4	0.12 (1)	6.25	J-C	-711 / 0	0.30 (1)	
E-F	0 / 19	-77.4	-77.4	0.15 (1)	10.00	E-H	-711 / 0	0.30 (1)	
F-G	0 / 30	-77.4	-77.4	0.09 (1)	10.00				
J-B	-205 / 0	0.0	0.0	0.02 (1)	7.81				
H-F	-205 / 0	0.0	0.0	0.02 (1)	7.81				
J-I	0 / 485	-17.5	-17.5	0.28 (4)	10.00				
I-H	0 / 485	-17.5	-17.5	0.28 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.15/1.00 (E-F:1) , BC=0.28/1.00 (H-I:4) ,
WB=0.30/1.00 (E-H:1) , SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	NAIL VALUES		SHEAR		SECTION
	PLATE	GRIP(DRY) (PSI)	(PLI)		(PLI)
		MAX MIN	MAX MIN		MAX MIN
MT20		618 354	1667 822		2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

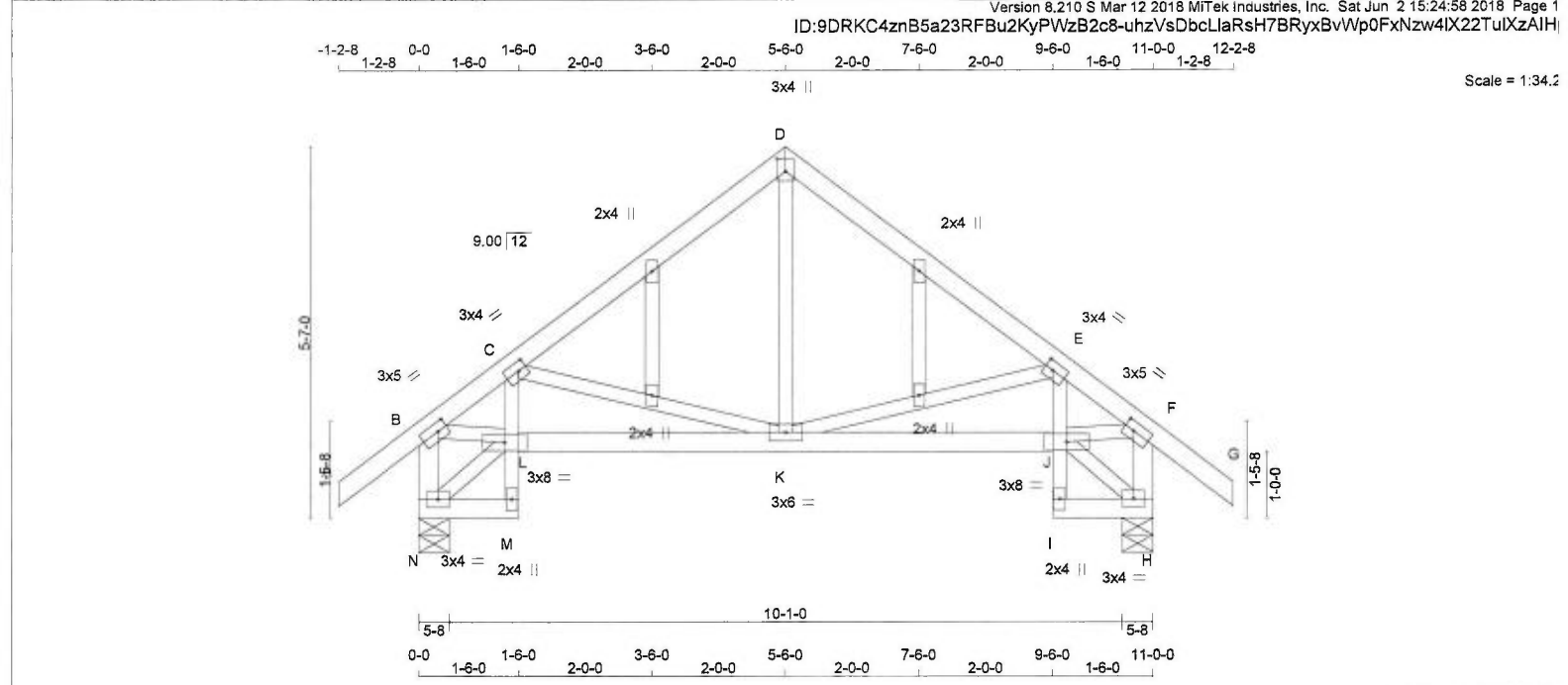
PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				SPECIFIED LOADS:			
A - D	2x4	DRY	No.2	FACTORED				TOP CH. LL = 23.3 PSF			
D - G	2x4	DRY	No.2	GROSS REACTION				DL = 3.0 PSF			
N - B	2x4	DRY	No.2	JT	VERT	HORZ	DOWN	BOT CH. LL = 0.0 PSF			
H - F	2x4	DRY	No.2	N	622	0	622	DL = 7.0 PSF			
N - M	2x4	DRY	No.2	H	622	0	622	TOTAL LOAD = 33.3 PSF			
M - C	2x3	DRY	No.2	UNFACTORED REACTIONS				SPACING = 24.0 IN. C/C			
L - J	2x4	DRY	No.2	1ST LCASE	MAX./MIN. COMPONENT REACTIONS			THIS TRUSS IS DESIGNED FOR RESIDENTIAL			
I - E	2x3	DRY	No.2	JT	COMBINED	SNOW	LIVE	OR SMALL BUILDING REQUIREMENTS OF			
I - H	2x4	DRY	No.2	H	434	317 / 0	0 / 0	PART 9, NBCC 2010			
ALL WEBS EXCEPT	2x3	DRY	No.2	H	434	317 / 0	0 / 0	THIS DESIGN COMPLIES WITH:			
ALL GABLE WEBS	2x3	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
DRY: SEASONED LUMBER.				BRACING				- CSA 086-09			
GABLE STUDS SPACED AT 2-0-0 OC.				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				- TPIC 2011			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				DESIGN ASSUMPTIONS			
				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				- OVERHANG NOT TO BE ALTERED OR CUT OFF.			

PLATES (table is in inches)							LOADING							WEBS						
JT	TYPE	PLATES	W	LEN	Y	X	CHORDS				MEMB.			MEMB.			MAX. FACTORED			
							MAX. FACTORED				FORCE			FORCE			MAX. FACTORED			
							VERT. LOAD				LC1 MAX			MAX. UNBRAC			MAX. FACTORED			
							(PLF)				CSI (LC)			LENGTH			MAX. FACTORED			
							FR-TO				FROM TO			FR-TO			MAX. FACTORED			
B	TMVW-t	MT20	3.0	5.0	1.50	1.75	A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	K-D	0 / 225	0.05 (1)					
C	TMVW-t	MT20	3.0	4.0	1.50	1.50	B-C	-747 / 0	-77.4	-77.4	0.11 (1)	6.25	K-E	-301 / 0	0.09 (1)					
D	TTW+p	MT20	3.0	4.0	2.25	1.50	C-D	-427 / 0	-77.4	-77.4	0.16 (1)	6.25	C-K	-301 / 0	0.09 (1)					
E	TMVW-t	MT20	3.0	4.0	1.50	1.50	D-E	-427 / 0	-77.4	-77.4	0.16 (1)	6.25	B-L	0 / 609	0.14 (1)					
F	TMVW-t	MT20	3.0	5.0	1.50	1.75	E-F	-747 / 0	-77.4	-77.4	0.11 (1)	6.25	J-F	0 / 609	0.14 (1)					
H	BMVW1-t	MT20	3.0	4.0			F-G	0 / 30	-77.4	-77.4	0.09 (1)	10.00	N-L	-13 / 0	0.00 (1)					
I	BMV+p	MT20	2.0	4.0			N-B	-602 / 0	0.0	0.0	0.06 (1)	7.81	J-H	-13 / 0	0.00 (1)					
J	BMVWW-I	MT20	3.0	8.0			H-F	-602 / 0	0.0	0.0	0.06 (1)	7.81								
K	BMVWW-t	MT20	3.0	6.0																
L	BMVWW-I	MT20	3.0	8.0																
M	BMV+p	MT20	2.0	4.0																
N	BMVW1-t	MT20	3.0	4.0																
O, P, Q, R																				
O	NP+w	MT20	2.0	4.0																

TOTAL LOAD CASES: (4)													
CHORDS													
MEMB. FORCE (LBS)													
VERT. LOAD LC1 MAX (PLF)													
CSI (LC)													
MAX. UNBRAC LENGTH													
MEMB. FORCE (LBS)													
MAX. FACTORED CSI (LC)													
ALLOWABLE DEFL.(LL)= L/360 (0.37")													
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")													
ALLOWABLE DEFL.(TL)= L/360 (0.37")													
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.02")													
CSI: TC=0.16/1.00 (C-D:1) , BC=0.15/1.00 (J-K:1) ,													
WB=0.14/1.00 (F-J:1) , SSI=0.11/1.00 (C-D:1)													
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10													
COMP=1.10 SHEAR=1.10 TENS= 1.10													
COMPANION LIVE LOAD FACTOR = 0.50													
AUTOSOLVE HEELS OFF													
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .													
NAIL VALUES													

ALL TRUSS BREAKS AND PERIMETER CORNER JOINTS MUST BE EXTERNALLY RESTRAINED.

55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.16/1.00 (C-D:1), BC=0.15/1.00 (J-K:1),
WB=0.14/1.00 (F-J:1), SS=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

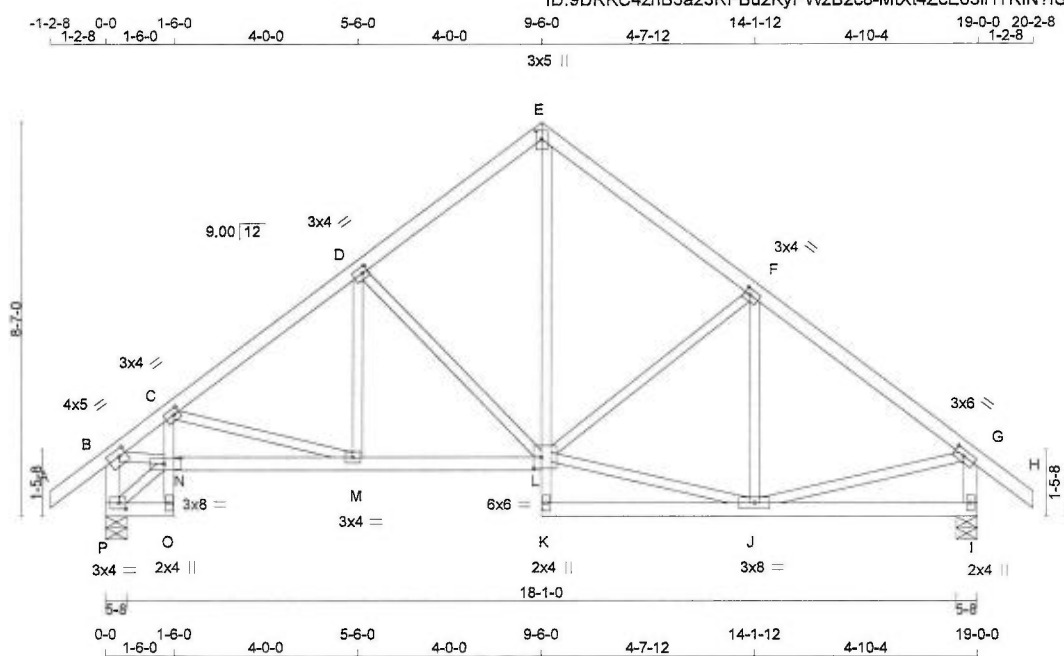
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
P - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
P - O	2x4	DRY	No.2
O - C	2x3	DRY	No.2
N - L	2x4	DRY	No.2
K - E	2x3	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	5.0	1.75 2.00
C	TMVW-t	MT20	3.0	4.0	1.50 1.50
D	TMVW-t	MT20	3.0	4.0	1.50 1.50
E	TTV+p	MT20	3.0	5.0	2.25 1.50
F	TMVW-t	MT20	3.0	4.0	1.50 1.50
G	TMVW-t	MT20	3.0	6.0	1.50 2.75
I	BMV1+p	MT20	2.0	4.0	
J	BMVWW-t	MT20	3.0	8.0	
K	BMV+p	MT20	2.0	4.0	
L	BMVWW-t	MT20	6.0	6.0	3.00 2.00
M	BMVW-t	MT20	3.0	4.0	
N	BMVW-t	MT20	3.0	8.0	1.50 4.25
O	BMV+p	MT20	2.0	4.0	
P	BMVW1-t	MT20	3.0	4.0	1.50 1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
I		998	0	998	0	5-8	5-8	5-8	5-8
P		1005	0	1005	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
I		698	501 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0	
P		703	505 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.		FACTORED		MEMB.		FACTORED	
		FORCE (LBS)	VERT. LOAD (PLF)			FORCE (LBS)	MAX
			CSI (LC)				CSI (LC)
FR-TO		FROM TO		LENGTH FR-TO			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	C-M	-209 / 0	0.06 (1)
B-C	-1312 / 0	-77.4	-77.4 0.09 (1)	5.57	M-D	0 / 118	0.04 (4)
C-D	-1055 / 0	-77.4	-77.4 0.16 (1)	5.95	D-L	-397 / 0	0.24 (1)
D-E	-747 / 0	-77.4	-77.4 0.15 (1)	6.25	L-J	0 / 743	0.17 (1)
E-F	-753 / 0	-77.4	-77.4 0.23 (1)	6.25	L-F	-185 / 0	0.11 (1)
F-G	-888 / 0	-77.4	-77.4 0.23 (1)	6.24	J-F	-231 / 16	0.09 (1)
G-H	0 / 30	-77.4	-77.4 0.09 (1)	10.00	B-N	0 / 1040	0.23 (1)
P-B	-979 / 0	0.0	0.0 0.10 (1)	7.81	J-G	0 / 751	0.17 (1)
I-G	-964 / 0	0.0	0.0 0.10 (1)	7.81	P-N	-22 / 0	0.00 (1)
P-O	0 / 17	-17.5	-17.5 0.01 (4)	10.00			
O-N	0 / 14	0.0	0.0 0.05 (1)	10.00			
N-C	0 / 41	0.0	0.0 0.06 (1)	10.00			
N-M	0 / 1057	-17.5	-17.5 0.21 (1)	10.00			
M-L	0 / 856	-17.5	-17.5 0.18 (1)	10.00			
K-L	0 / 33	0.0	0.0 0.02 (1)	10.00			
L-E	0 / 623	0.0	0.0 0.16 (1)	10.00			
K-J	0 / 6	-17.5	-17.5 0.11 (4)	10.00			
J-I	0 / 0	-17.5	-17.5 0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.23/1.00 (F-G:1), BC=0.21/1.00 (M-N:1), WB=0.24/1.00 (D-L:1), SI=0.14/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

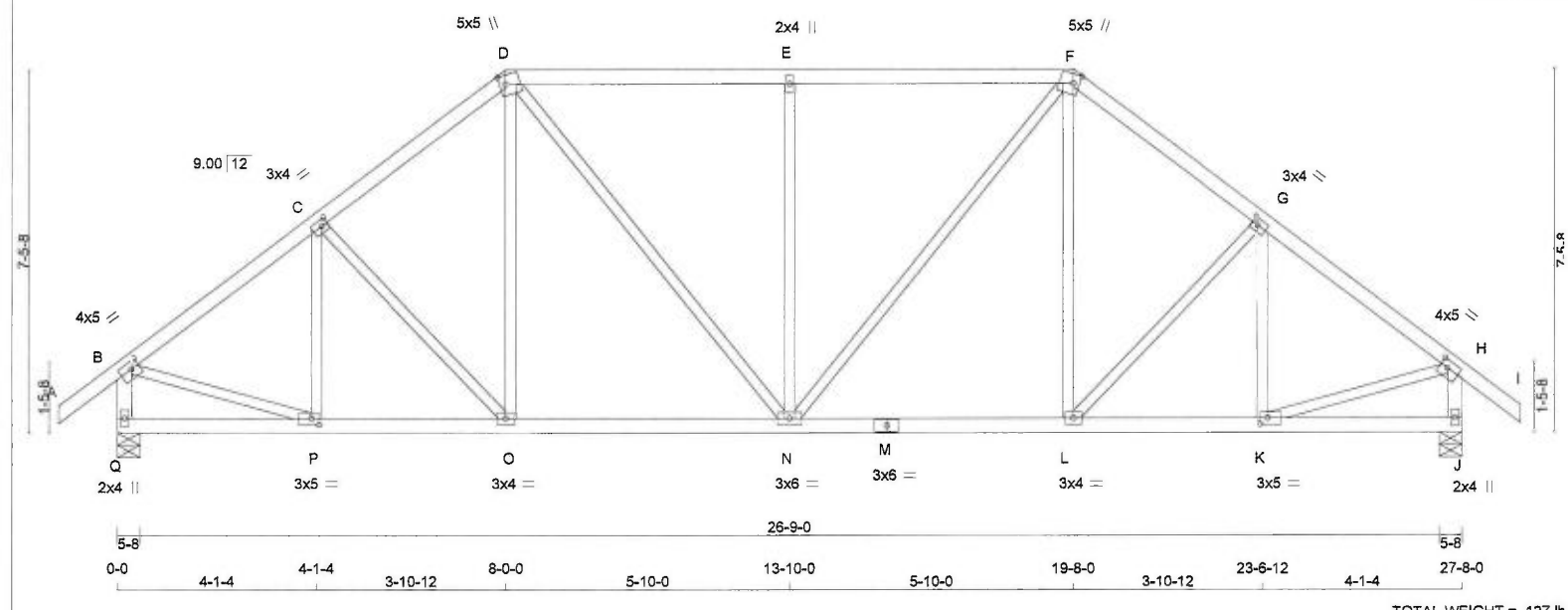
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED				TOP CH. LL = 23.3 PSF			
A - D	2x4	DRY	No.2	GROSS REACTION				DL = 3.0 PSF			
D - F	2x4	DRY	No.2	MAXIMUM FACTORED				BOT CH. LL = 0.0 PSF			
F - I	2x4	DRY	No.2	DOWN HORZ UPLIFT				DL = 7.0 PSF			
Q - B	2x4	DRY	No.2	INPUT				TOTAL LOAD = 33.3 PSF			
J - H	2x4	DRY	No.2	BRG							
M - J	2x4	DRY	No.2	IN-SX							
Q - M	2x4	DRY	No.2	REQRD							
M - J	2x4	DRY	No.2	BRG							
ALL WEBS	2x3	DRY	No.2	UNFACTORED REACTIONS				SPACING = 24.0 IN. C/C			
EXCEPT				1ST LCASE				LOADING IN FLAT SECTION BASED ON A			
				MAX./MIN. COMPONENT REACTIONS				SLOPE OF 2.00/12 MINIMUM			
				JT COMBINED				THIS TRUSS IS DESIGNED FOR RESIDENTIAL			
				Q 989				OR SMALL BUILDING REQUIREMENTS OF			
				J 989				PART 9, NBCC 2010			
								THIS DESIGN COMPLIES WITH:			
								- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
								- CSA 086-09			
								- TPIC 2011			
								(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.			
								RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED			
								ROOF LIVE LOAD			
								ALLOWABLE DEFL.(LL)= L/360 (0.92")			
								CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")			
								ALLOWABLE DEFL.(TL)= L/360 (0.92")			
								CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")			
								CSI: TC=0.36/1.00 (D-E:1), BC=0.23/1.00 (O-P:1)			
								WB=0.55/1.00 (E-N:1), SSI=0.22/1.00 (D-E:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
								COMP=1.10 SHEAR=1.10 TENS= 1.10			
								COMPANION LIVE LOAD FACTOR = 0.50			
								TRUSS PLATE MANUFACTURER IS NOT			
								RESPONSIBLE FOR QUALITY CONTROL IN			
								THE TRUSS MANUFACTURING PLANT.			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION			
								(PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 618 354 1667 822 2284 1656			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.90 (P) (INPUT = 0.90)			
								JSI METAL= 0.46 (H) (INPUT = 1.00)			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.75	2.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	5.0	5.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	5.0	2.25	1.50
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	5.0	1.75	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	3.0	5.0	1.50	1.75
L	BMVW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-w	MT20	3.0	6.0		
O	BMVW-t	MT20	3.0	4.0		
P	BMVW-t	MT20	3.0	5.0	1.50	1.75
Q	BMV1+p	MT20	2.0	4.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MEMB. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX. MEMB. UNBRAC LENGTH FR-TO
A-B	0 / 30	-77.4 -77.4 0.09 (1)	10.00	P-C	-242 / 0
B-C	-1388 / 0	-77.4 -77.4 0.18 (1)	5.35	C-O	-116 / 0
C-D	-1334 / 0	-77.4 -77.4 0.17 (1)	5.44	O-D	0 / 174
D-E	-1317 / 0	-77.4 -77.4 0.36 (1)	5.21	D-N	0 / 423
E-F	-1317 / 0	-77.4 -77.4 0.36 (1)	5.21	N-E	-552 / 0
F-G	-1334 / 0	-77.4 -77.4 0.17 (1)	5.44	N-F	0 / 423
G-H	-1388 / 0	-77.4 -77.4 0.18 (1)	5.35	L-F	0 / 174
H-I	0 / 30	-77.4 -77.4 0.09 (1)	10.00	L-G	-116 / 0
Q-B	-1380 / 0	0.0 0.0 0.14 (1)	6.92	K-G	-242 / 0
J-H	-1380 / 0	0.0 0.0 0.14 (1)	6.92	B-P	0 / 1169
				K-H	0 / 1169
Q-P	0 / 0	-17.5 -17.5 0.06 (4)	10.00		
P-O	0 / 1127	-17.5 -17.5 0.23 (1)	10.00		
O-N	0 / 1050	-17.5 -17.5 0.23 (1)	10.00		
N-M	0 / 1050	-17.5 -17.5 0.23 (1)	10.00		
M-L	0 / 1050	-17.5 -17.5 0.23 (1)	10.00		
L-K	0 / 1127	-17.5 -17.5 0.23 (1)	10.00		
K-J	0 / 0	-17.5 -17.5 0.06 (4)	10.00		

LICENSED PROFESSIONAL ENGINEER

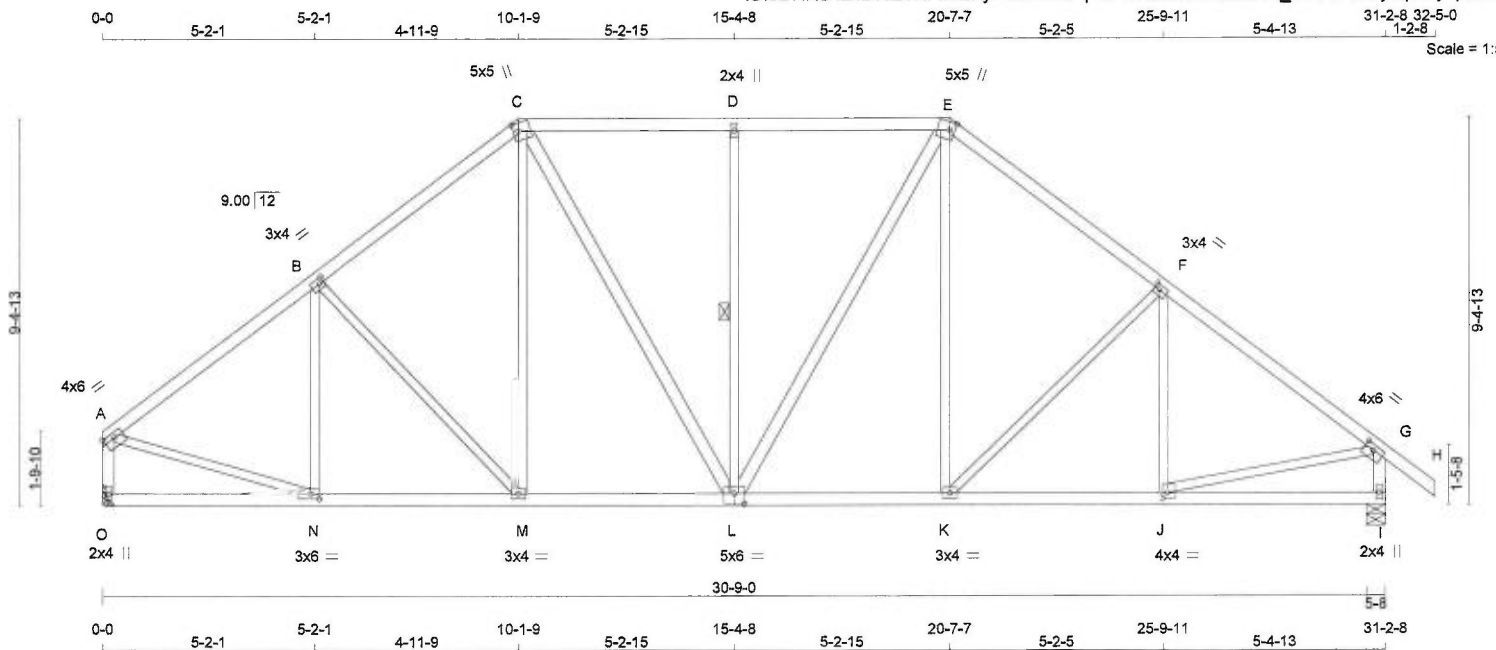
N.A. EL-MASRI

PROVINCIAL OF ONTARIO

Jun 02, 2018

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



TOTAL WEIGHT = 2 X 154 = 309 lb
[MIF]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - L	2x4	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-t	MT20	4.0	6.0 1.50 Edge
B	TMVW-t	MT20	3.0	4.0 1.50 1.50
C	TTWW+m	MT20	5.0	5.0 2.25 1.50
D	TMVW+w	MT20	2.0	4.0
E	TTWW+m	MT20	5.0	5.0 2.25 1.50
F	TMVW-t	MT20	3.0	4.0 1.50 1.50
G	TMVW-t	MT20	4.0	6.0 1.50 2.75
I	BMV1+p	MT20	2.0	4.0
J	BMVW-t	MT20	4.0	4.0 1.50 1.50
K	BMVW-t	MT20	3.0	4.0
L	BSVW-w	MT20	5.0	6.0 3.00 3.00
M	BMVW-t	MT20	3.0	4.0
N	BMVW-t	MT20	3.0	6.0 1.50 2.50
O	BMV1+p	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
O	1480	0	1480	0	0	MECHANICAL
I	1580	0	1580	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT O TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE			
O	1039	727 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
I	1107	787 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-L. DBS = 20-0-0. CBF = 62 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. UNBRACED LENGTH FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)		MAX. UNBRACED LENGTH FR-TO
	FROM	TO	FROM	TO			FROM	TO	
A-B	-1528 / 0	-77.4	-77.4	0.30 (1)	5.03	N-B	-258 / 8	0.13 (1)	0.20 (1)
B-C	-1426 / 0	-77.4	-77.4	0.29 (1)	5.17	B-M	-187 / 0	0.05 (1)	0.06 (1)
C-D	-1299 / 0	-77.4	-77.4	0.29 (1)	5.34	M-C	0 / 224	0.06 (1)	0.29 (1)
D-E	-1299 / 0	-77.4	-77.4	0.29 (1)	5.34	C-L	0 / 362	0.05 (1)	0.06 (1)
E-F	-1458 / 0	-77.4	-77.4	0.31 (1)	5.10	L-D	-494 / 0	0.05 (1)	0.06 (1)
F-G	-1639 / 0	-77.4	-77.4	0.33 (1)	4.85	L-E	0 / 312	0.06 (1)	0.06 (1)
G-H	0 / 30	-77.4	-77.4	0.09 (1)	10.00	K-E	0 / 280	0.06 (1)	0.06 (1)
O-A	-1442 / 0	0.0	0.0	0.16 (1)	6.81	K-F	-271 / 0	0.08 (1)	0.08 (1)
I-G	-1541 / 0	0.0	0.0	0.16 (1)	6.63	J-F	-177 / 36	0.29 (1)	0.31 (1)
O-N	0 / 0	-17.5	-17.5	0.11 (4)	10.00	A-N	0 / 1294	0.31 (1)	
N-M	0 / 1244	-17.5	-17.5	0.25 (1)	10.00	J-G	0 / 1363		
M-L	0 / 1118	-17.5	-17.5	0.23 (1)	10.00				
L-K	0 / 1144	-17.5	-17.5	0.23 (1)	10.00				
K-J	0 / 1334	-17.5	-17.5	0.27 (1)	10.00				
J-I	0 / 0	-17.5	-17.5	0.12 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.33/1.00 (F-G:1), BC=0.27/1.00 (J-K:1),
WB=0.31/1.00 (G-J:1), SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

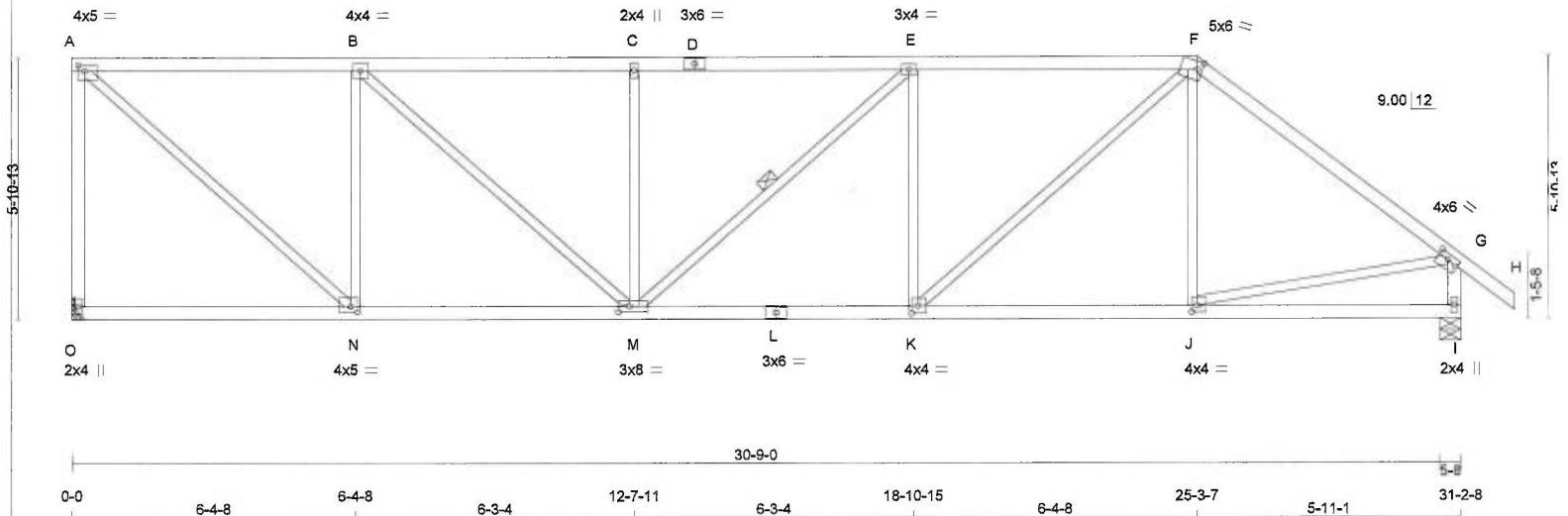
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.50 (J) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
O - A	2x4	DRY	No.2 SPF
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - H	2x4	DRY	No.2 SPF
I - G	2x4	DRY	No.2 SPF
O - L	2x4	DRY	No.2 SPF
L - I	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	5.0	1.50	1.75
B	TMVW-t	MT20	4.0	4.0		
C	TMVW-w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMVW-t	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TMVW-t	MT20	4.0	6.0	1.50	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	4.0	1.75	1.50
K	BMVW-t	MT20	4.0	4.0	2.00	1.75
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	8.0	1.50	3.00
N	BMVW-t	MT20	4.0	5.0	1.50	1.75
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ UPLIFT		
O	1480	0	1480	0	0	MECHANICAL
I	1580	0	1580	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT O TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE	PERM. LIVE				
O	1039	727 / 0	0 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
I	1107	787 / 0	0 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M. DBS = 20-0-0. CBF = 1 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE), FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	
		FROM	TO			FROM	TO
FR-TO							
O-A	-1435 / 0	0.0	0.0	0.94 (1)	6.82	A-N	0 / 1876
A-B	-1408 / 0	-77.4	-77.4	0.54 (1)	4.82	N-B	-1114 / 0
B-C	-2029 / 0	-77.4	-77.4	0.60 (1)	4.10	B-M	0 / 833
C-D	-2029 / 0	-77.4	-77.4	0.60 (1)	4.10	M-C	-447 / 0
D-E	-2029 / 0	-77.4	-77.4	0.60 (1)	4.10	M-E	-5 / 4
E-F	-2032 / 0	-77.4	-77.4	0.61 (1)	4.10	K-E	-549 / 0
F-G	-1609 / 0	-77.4	-77.4	0.62 (1)	4.50	K-F	0 / 997
G-H	0 / 30	-77.4	-77.4	0.09 (1)	10.00	J-F	-120 / 63
I-G	-1539 / 0	0.0	0.0	0.16 (1)	6.84	J-G	0 / 1308
O-N	0 / 0	-17.5	-17.5	0.17 (4)	10.00		
N-M	0 / 1408	-17.5	-17.5	0.32 (1)	10.00		
M-L	0 / 2032	-17.5	-17.5	0.39 (1)	10.00		
L-K	0 / 2032	-17.5	-17.5	0.39 (1)	10.00		
K-J	0 / 1283	-17.5	-17.5	0.29 (1)	10.00		
J-I	0 / 0	-17.5	-17.5	0.16 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.94/1.00 (A-O:1), BC=0.39/1.00 (K-M:1),
WB=0.60/1.00 (B-N:1), SSI=0.23/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

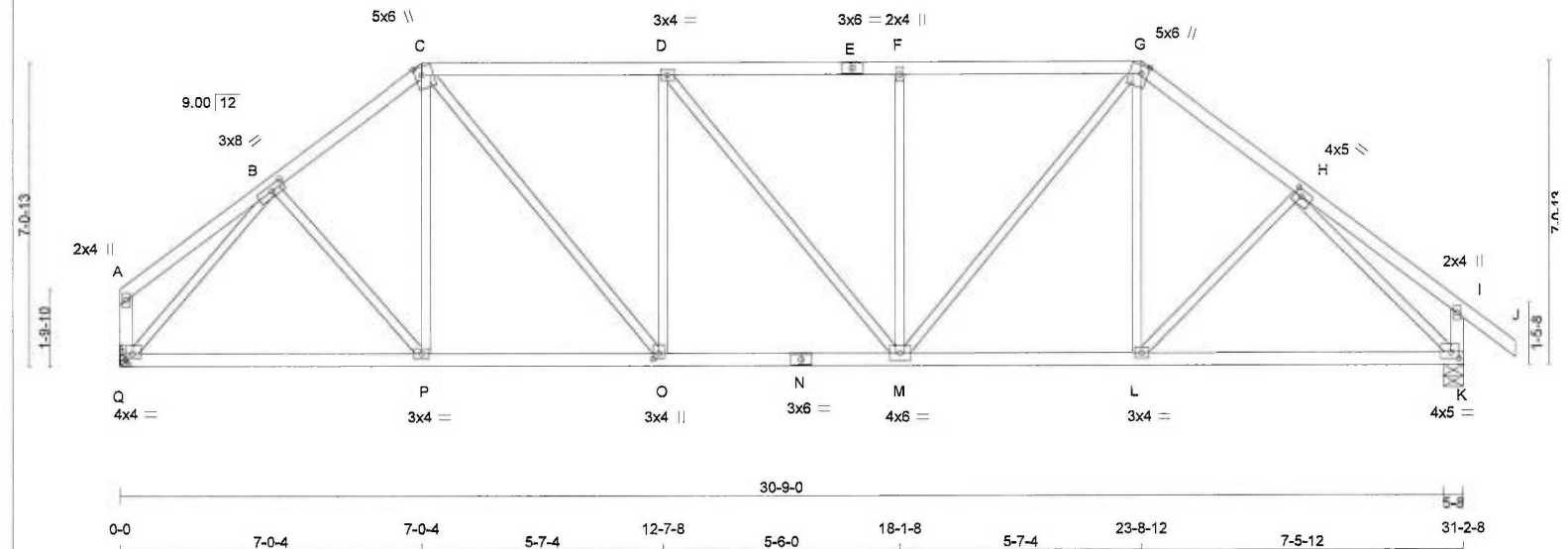
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.60 (L) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - A	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	2.0	4.0	
B	TMVW-t	MT20	3.0	8.0	1.50 3.75
C	TTVW+m	MT20	5.0	6.0	2.25 1.75
D	TMVW-t	MT20	3.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMVW+w	MT20	2.0	4.0	
G	TTVW+m	MT20	5.0	6.0	2.25 1.75
H	TMVW-t	MT20	4.0	5.0	1.75 2.00
I	TMV+p	MT20	2.0	4.0	
K	BMVW1-t	MT20	4.0	5.0	1.75 2.25
L	BMVW-t	MT20	3.0	4.0	
M	BMVW-t	MT20	4.0	6.0	
N	BS-t	MT20	3.0	6.0	
O	BMVW+t	MT20	3.0	4.0	1.50 1.50
P	BMVW-t	MT20	3.0	4.0	
Q	BMVW1-t	MT20	4.0	4.0	1.50 1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED		MAXIMUM FACTORED	
GROSS REACTION		GROSS REACTION	
JT	VERT	HORZ	UPLIFT
Q	1480	0	1480
K	1580	0	1580

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT Q TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1039	727 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
K	1107	787 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.		MAX. FACTORED		MEMB.		MAX. FACTORED	
		FORCE (LBS)	VERT. LOAD (PLF)			FORCE (LBS)	VERT. LOAD (PLF)
			CS1 (LC)				CS1 (LC)
FR-TO				FR-TO			
A-B	0 / 17	-77.4	-77.4 0.14 (1)	B-P	0 / 111	0.03 (4)	
B-C	-1526 / 0	-77.4	-77.4 0.17 (1)	P-C	0 / 88	0.03 (4)	
C-D	-1681 / 0	-77.4	-77.4 0.40 (1)	C-O	0 / 747	0.17 (1)	
D-E	-1697 / 0	-77.4	-77.4 0.40 (1)	O-D	-486 / 0	0.42 (1)	
E-F	-1697 / 0	-77.4	-77.4 0.40 (1)	D-M	0 / 25	0.01 (1)	
F-G	-1697 / 0	-77.4	-77.4 0.39 (1)	M-F	-464 / 0	0.40 (1)	
G-H	-1594 / 0	-77.4	-77.4 0.20 (1)	M-G	0 / 686	0.15 (1)	
H-I	0 / 19	-77.4	-77.4 0.16 (1)	L-G	0 / 125	0.04 (4)	
I-J	0 / 30	-77.4	-77.4 0.09 (1)	L-H	0 / 54	0.02 (4)	
Q-A	-108 / 0	0.0	0.0 0.01 (1)	Q-B	-1739 / 0	0.81 (1)	
K-I	-213 / 0	0.0	0.0 0.02 (1)	H-K	-1809 / 0	0.86 (1)	
Q-P	0 / 1130	-17.5	-17.5 0.29 (1)				
P-O	0 / 1204	-17.5	-17.5 0.30 (1)				
O-N	0 / 1681	-17.5	-17.5 0.32 (1)				
N-M	0 / 1681	-17.5	-17.5 0.32 (1)				
M-L	0 / 1260	-17.5	-17.5 0.32 (1)				
L-K	0 / 1246	-17.5	-17.5 0.32 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.40/1.00 (C-D:1), BC=0.32/1.00 (L-M:1), WB=0.86/1.00 (H-K:1), SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

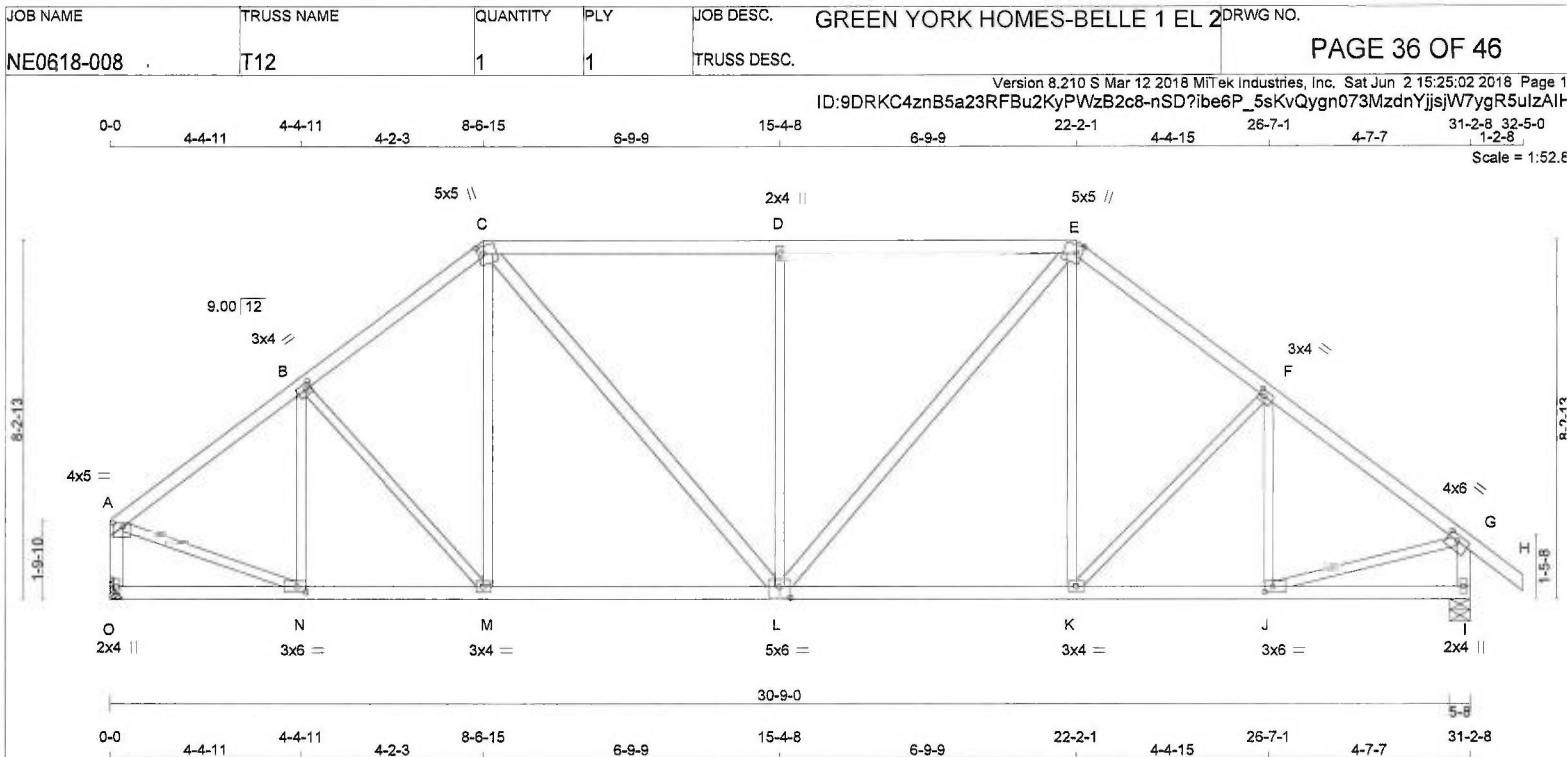
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.54 (H) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

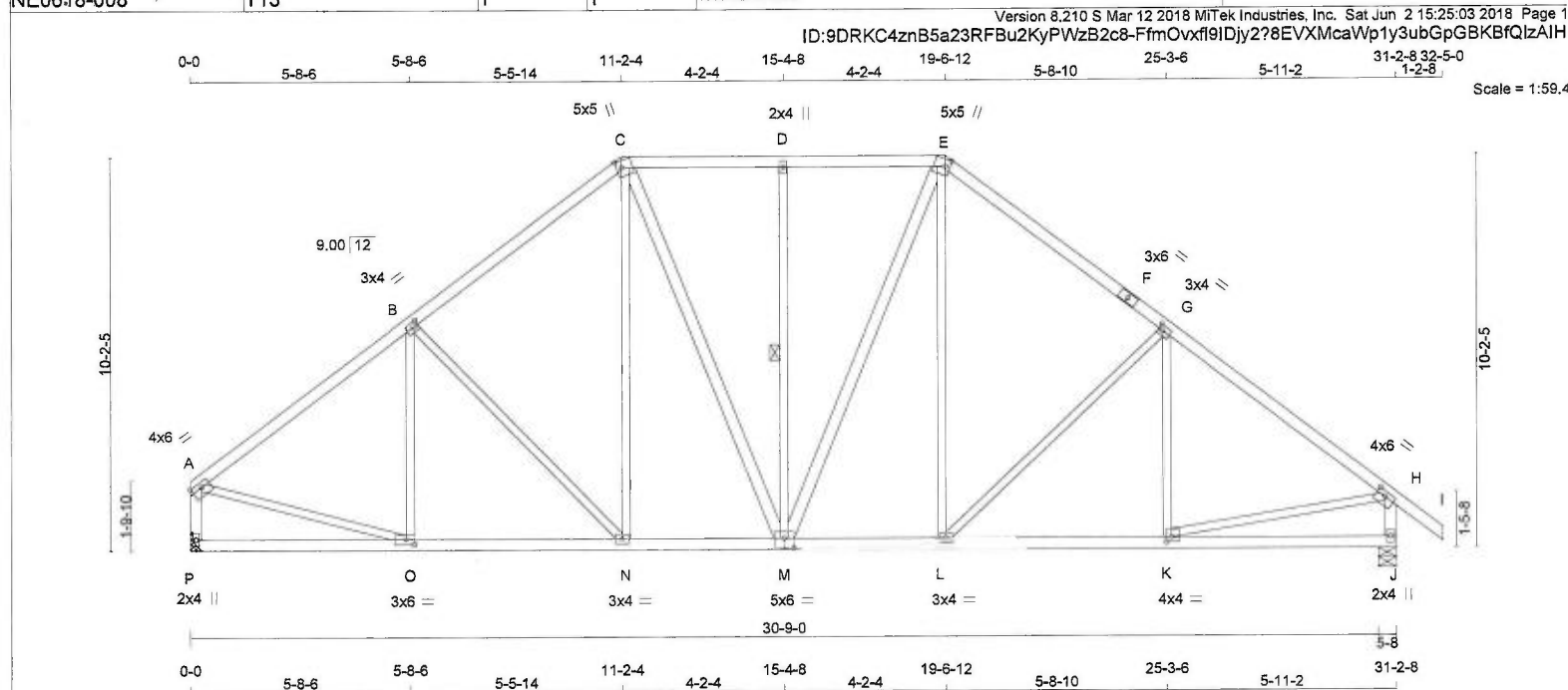




LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 147 lb				
N. L. G. A. RULES				BEARINGS				[M]F				
CHORDS		SIZE	LUMBER	DESCR.						DESIGN CRITERIA		
A - C	2x4	DRY	No.2	SPF					SPECIFIED LOADS:			
C - E	2x4	DRY	No.2	SPF					TOP CH. LL = 23.3 PSF			
E - H	2x4	DRY	No.2	SPF					DL = 3.0 PSF			
O - A	2x4	DRY	No.2	SPF					BOT CH. LL = 0.0 PSF			
I - G	2x4	DRY	No.2	SPF					DL = 7.0 PSF			
O - L	2x4	DRY	No.2	SPF					TOTAL LOAD = 33.3 PSF			
L - I	2x4	DRY	No.2	SPF					SPACING = 24.0 IN. C/C			
ALL WEBS		2x3	DRY	No.2	SPF					LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM		
EXCEPT										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010		
C - L	2x4	DRY	No.2	SPF							THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011	
L - E	2x4	DRY	No.2	SPF							(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD	
DRY: SEASONED LUMBER.				UNFACTORED REACTIONS				ALLOWABLE DEFL.(LL)= L/360 (1.04") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (1.04") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")				
				1ST LCASE MAX./MIN. COMPONENT REACTIONS				CSI: TC=0.51/1.00 (C-D:1), BC=0.29/1.00 (K-L:1), WB=0.86/1.00 (D-L:1), SS=0.26/1.00 (C-D:1)				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10				
O	1039	727 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0	COMPANION LIVE LOAD FACTOR = 0.50				
I	1107	787 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0	TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .				
				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I				NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656				
				BRACING				PLATE PLACEMENT TOL. = 0.250 inches				
				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				PLATE ROTATION TOL. = 5.0 Deg.				
				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.				JSI GRIP= 0.88 (A) (INPUT = 0.90) JSI METAL= 0.45 (G) (INPUT = 1.00)				
				LOADING				KOTT				
				TOTAL LOAD CASES: (4)								
				CHORDS								
				MAX. FACTORED								
				MEMB. FORCE (LBS)								
				VERT. LOAD (PLF)								
				CSI (LC)								
				MAX. UNBRACED LENGTH								
				MEMB. FORCE (LBS)								
				MAX. FACTORED								
				CSI (LC)								
				FR-TO								
				A- B -1490 / 0								
				-77.4 -77.4 0.21 (1)								
				5.18								
				N- B -333 / 0								
				0.13 (1)								
				B- C -1485 / 0								
				-77.4 -77.4 0.21 (1)								
				5.18								
				B- M -65 / 0								
				0.05 (1)								
				C- D -1517 / 0								
				-77.4 -77.4 0.51 (1)								
				4.72								
				M- C 0 / 149								
				0.04 (4)								
				D- E -1517 / 0								
				-77.4 -77.4 0.51 (1)								
				4.72								
				C- L 0 / 535								
				0.09 (1)								
				E- F -1532 / 0								
				-77.4 -77.4 0.23 (1)								
				5.10								
				L- D -646 / 0								
				0.86 (1)								
				F- G -1617 / 0								
				-77.4 -77.4 0.24 (1)								
				4.99								
				L- E 0 / 478								
				0.08 (1)								
				G- H 0 / 30								
				-77.4 -77.4 0.09 (1)								
				10.00								
				K- E 0 / 213								
				0.05 (1)								
				O- A -1446 / 0								
				0.0 0.0 0.16 (1)								
				6.80								
				K- F -156 / 0								
				0.12 (1)								
				I- G -1544 / 0								
				0.0 0.0 0.16 (1)								
				6.62								
				J- F -243 / 0								
				0.09 (1)								
				A- N 0 / 1277								
				0.29 (1)								
				J- G 0 / 1351								
				0.30 (1)								
				O- N 0 / 0								
				-17.5 -17.5 0.07 (4)								
				10.00								
				N- M 0 / 1210								
				-17.5 -17.5 0.26 (1)								
				10.00								
				M- L 0 / 1169								
				-17.5 -17.5 0.28 (1)								
				10.00								
				L- K 0 / 1206								
				-17.5 -17.5 0.29 (1)								
				10.00								
				K- J 0 / 1312								
				-17.5 -17.5 0.28 (1)								
				10.00								
				J- I 0 / 0								
				-17.5 -17.5 0.08 (4)								
				10.00								

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCE OF ONTARIO
Jun 02, 2018



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - A	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
C - M	2x4	DRY	No.2
M - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	Edge
B	TMVW-t	MT20	3.0	4.0	1.50	1.50
C	TTWW+m	MT20	5.0	5.0	2.25	1.50
D	TMVW-t	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	5.0	2.25	1.50
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	4.0	1.50	1.50
L	BMVW-t	MT20	3.0	4.0		
M	BSVWW-I	MT20	5.0	6.0	3.00	3.00
N	BMVW-t	MT20	3.0	4.0		
O	BMVW-t	MT20	3.0	6.0	1.50	2.50
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
P	1480	0	1480	0	MECHANICAL	
J	1580	0	1580	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT P TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1039	727 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
J	1107	787 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M. DBS = 20-0-0. CBF = 49 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER MEMB). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH		
FR-TO		FROM	TO		FR-TO				
A-B	-1543 / 0	-77.4	-77.4	0.37 (1)	4.92	O-B	-215 / 31	0.12 (1)	
B-C	-1379 / 0	-77.4	-77.4	0.35 (1)	5.16	B-N	-263 / 0	0.35 (1)	
C-D	-1184 / 0	-77.4	-77.4	0.18 (1)	5.68	N-C	0 / 273	0.06 (1)	
D-E	-1184 / 0	-77.4	-77.4	0.18 (1)	5.68	C-M	0 / 270	0.04 (1)	
E-F	-1403 / 0	-77.4	-77.4	0.38 (1)	5.08	M-D	-389 / 0	0.28 (1)	
F-G	-1403 / 0	-77.4	-77.4	0.38 (1)	5.08	M-E	0 / 222	0.04 (1)	
G-H	-1645 / 0	-77.4	-77.4	0.41 (1)	4.75	E-L	0 / 324	0.07 (1)	
H-I	0 / 30	-77.4	-77.4	0.09 (1)	10.00	L-G	-343 / 0	0.47 (1)	
P-A	-1439 / 0	0.0	0.0	0.16 (1)	6.81	K-G	-141 / 57	0.08 (1)	
J-H	-1538 / 0	0.0	0.0	0.16 (1)	6.64	A-O	0 / 1301	0.29 (1)	
						K-H	0 / 1366	0.31 (1)	
P-O	0 / 0	-17.5	-17.5	0.14 (4)	10.00				
O-N	0 / 1259	-17.5	-17.5	0.27 (1)	10.00				
N-M	0 / 1079	-17.5	-17.5	0.22 (1)	10.00				
M-L	0 / 1097	-17.5	-17.5	0.22 (1)	10.00				
L-K	0 / 1342	-17.5	-17.5	0.29 (1)	10.00				
K-J	0 / 0	-17.5	-17.5	0.15 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.41/1.00 (G-H:1), BC=0.29/1.00 (K-L:1), WB=0.47/1.00 (G-L:1), SSI=0.18/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

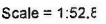
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.50 (K) (INPUT = 1.00)



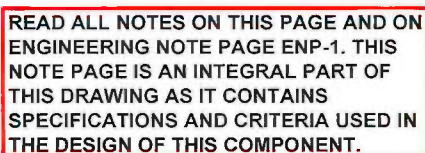
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

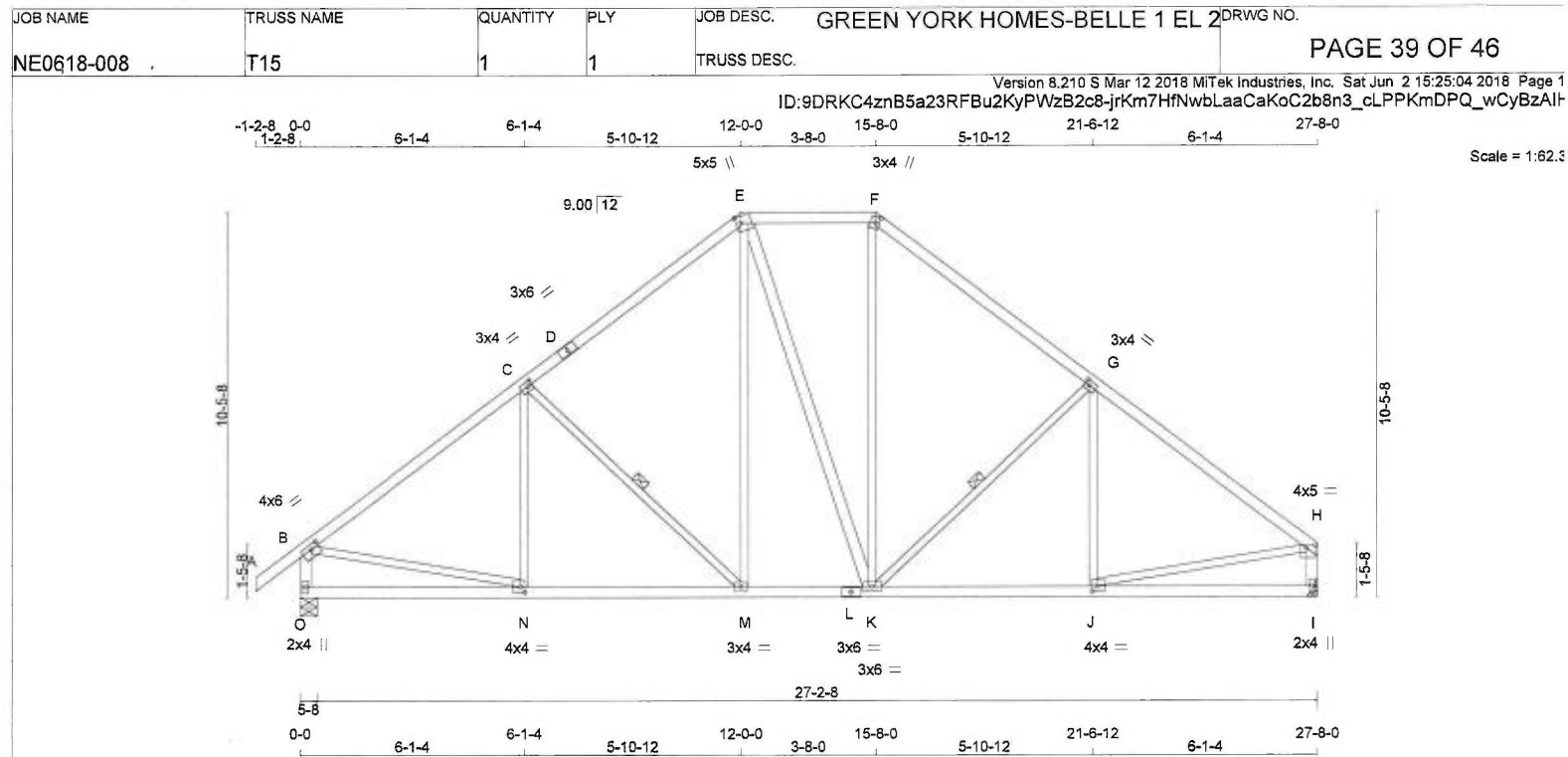
ID:9DRKC4znB5a23RFBu2KyPWzB2c8-jrKm7HfNwbLaaCaKoC2b8n30oLQzKmDPQ_wCyBzAlt



TOTAL WEIGHT = 135 lb
[M][F]

JST METAL= 0.38 (B) (INPUT = 1.00)





LUMBER
N. L. G. A. RULES
CHORDS

CHORDS	SIZE	DRY	No.2	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT E - K	2x3	DRY	No.2	SPF
E - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	5.0	2.25	1.50
F	TTWW+m	MT20	3.0	4.0	2.00	1.25
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-p	MT20	4.0	5.0	1.00	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	4.0	1.75	1.50
K	BMVW-t	MT20	3.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	4.0	1.75	1.50
O	BMV1+p	MT20	2.0	4.0		



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
O	1412	0	1412	0	5-8	5-8
I	1312	0	1312	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LC CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE			
O	989	705 / 0	0 / 0	0 / 0	0 / 0	284 / 0	0 / 0
I	921	644 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, G-K. DBS = 20-0-0, CBF = 49 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	N-C	-94 / 75	0.05 (1)	
B-C	-1422 / 0	-77.4 -77.4	0.42 (1)	C-M	-395 / 0	0.18 (1)	
C-D	-1134 / 0	-77.4 -77.4	0.39 (1)	M-E	0 / 347	0.08 (1)	
D-E	-1134 / 0	-77.4 -77.4	0.39 (1)	E-K	0 / 3	0.00 (1)	
E-F	-883 / 0	-77.4 -77.4	0.14 (1)	K-F	0 / 349	0.08 (1)	
F-G	-1135 / 0	-77.4 -77.4	0.39 (1)	G-K	-393 / 0	0.18 (1)	
G-H	-1421 / 0	-77.4 -77.4	0.42 (1)	J-G	-96 / 74	0.05 (1)	
O-B	-1369 / 0	0.0 0.0	0.14 (1)	B-N	0 / 1184	0.27 (1)	
I-H	-1269 / 0	0.0 0.0	0.13 (1)	J-H	0 / 1183	0.27 (1)	
O-N	0 / 0	-17.5 -17.5	0.16 (4)				
N-M	0 / 1164	-17.5 -17.5	0.27 (1)				
M-L	0 / 882	-17.5 -17.5	0.19 (1)				
L-K	0 / 882	-17.5 -17.5	0.19 (1)				
K-J	0 / 1163	-17.5 -17.5	0.27 (1)				
J-I	0 / 0	-17.5 -17.5	0.16 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.42/1.00 (B-C:1), BC=0.27/1.00 (M-N:1), WB=0.27/1.00 (B-N:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

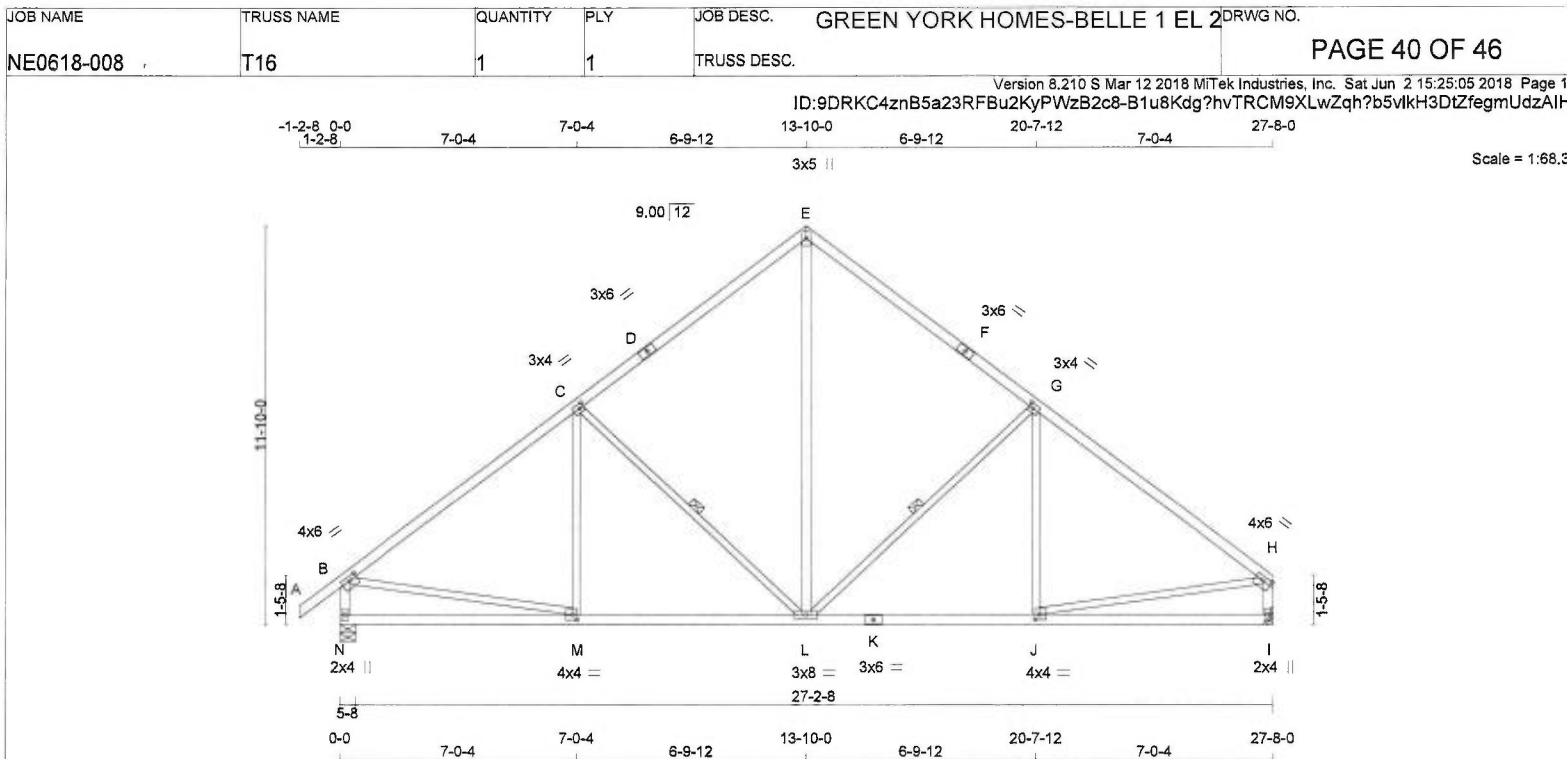
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.44 (N) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
L - E 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	3.0	5.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	1.50	Edge
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	4.0	1.75	1.50
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	3.0	8.0		
M	BMVW-t	MT20	4.0	4.0	1.75	1.50
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP
N	1412	0	1412	0
I	1312	0	1312	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	989	705 / 0	0 / 0	0 / 0	0 / 0	284 / 0	0 / 0
I	921	644 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF G-L, C-L. DBS = 20-0-0. CBF = 62 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FR-TO)	MAX. LC1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	L-E	0 / 797	0.13 (1)
B-C	-1409 / 0	-77.4	-77.4	0.58 (1)	L-G	-500 / 0	0.30 (1)
C-D	-1032 / 0	-77.4	-77.4	0.53 (1)	J-G	-51 / 100	0.04 (1)
D-E	-1032 / 0	-77.4	-77.4	0.53 (1)	C-L	-489 / 0	0.30 (1)
E-F	-1032 / 0	-77.4	-77.4	0.53 (1)	M-C	-51 / 100	0.04 (1)
F-G	-1032 / 0	-77.4	-77.4	0.53 (1)	B-M	0 / 1173	0.26 (1)
G-H	-1409 / 0	-77.4	-77.4	0.58 (1)	J-H	0 / 1173	0.26 (1)
N-B	-1352 / 0	0.0	0.0	0.14 (1)			
I-H	-1262 / 0	0.0	0.0	0.13 (1)			
N-M	0 / 0	-17.5	-17.5	0.20 (4)			
M-L	0 / 1158	-17.5	-17.5	0.29 (1)			
L-K	0 / 1158	-17.5	-17.5	0.29 (1)			
K-J	0 / 1158	-17.5	-17.5	0.29 (1)			
J-I	0 / 0	-17.5	-17.5	0.20 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.58/1.00 (G-H:1), BC=0.29/1.00 (J-L:1),
WB=0.30/1.00 (G-L:1), SSI=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

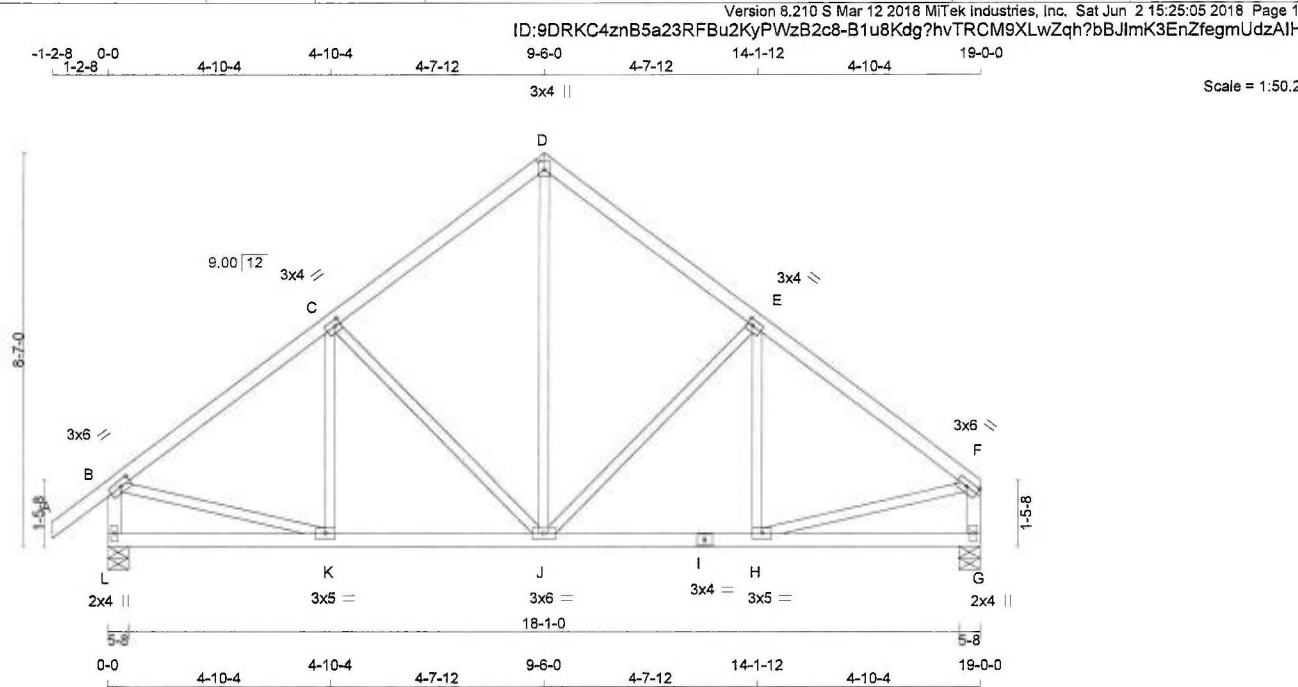
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (L) (INPUT = 0.90)
JSI METAL= 0.43 (M) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 85 = 255 lb [M]F

LUMBER			
N, L, G, A. RULES	CHORDS	SIZE	DRY
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	6.0	1.50	2.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW-t	MT20	3.0	6.0	1.50	Edge
G	BMV1+p	MT20	2.0	4.0		
H	BMVW-t	MT20	3.0	5.0		
I	BS-t	MT20	3.0	4.0		
J	BMVW-t	MT20	3.0	6.0		
K	BMVW-t	MT20	3.0	5.0		
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	BRG	BRG	IN-SX
L	1001	0	0	1001	0	0	5-8	5-8	5-8
G	901	0	0	901	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	701	503 / 0	0 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
G	632	442 / 0	0 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	J-D	0 / 513	0.12 (1)	
B-C	-891 / 0	-77.4 -77.4	0.23 (1)	J-E	-304 / 0	0.25 (1)	
C-D	-679 / 0	-77.4 -77.4	0.23 (1)	H-E	-79 / 56	0.03 (1)	
D-E	-679 / 0	-77.4 -77.4	0.23 (1)	C-J	-303 / 0	0.25 (1)	
E-F	-891 / 0	-77.4 -77.4	0.23 (1)	K-C	-79 / 56	0.03 (1)	
L-B	-866 / 0	0.0 0.0	0.10 (1)	B-K	0 / 753	0.17 (1)	
G-F	-866 / 0	0.0 0.0	0.09 (1)	H-F	0 / 753	0.17 (1)	
L-K	0 / 0	-17.5 -17.5	0.09 (4)				
K-J	0 / 734	-17.5 -17.5	0.16 (1)				
J-I	0 / 734	-17.5 -17.5	0.16 (1)				
I-H	0 / 734	-17.5 -17.5	0.16 (1)				
H-G	0 / 0	-17.5 -17.5	0.09 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.23/1.00 (E-F:1), BC=0.16/1.00 (H-J:1), WB=0.25/1.00 (E-J:1), SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

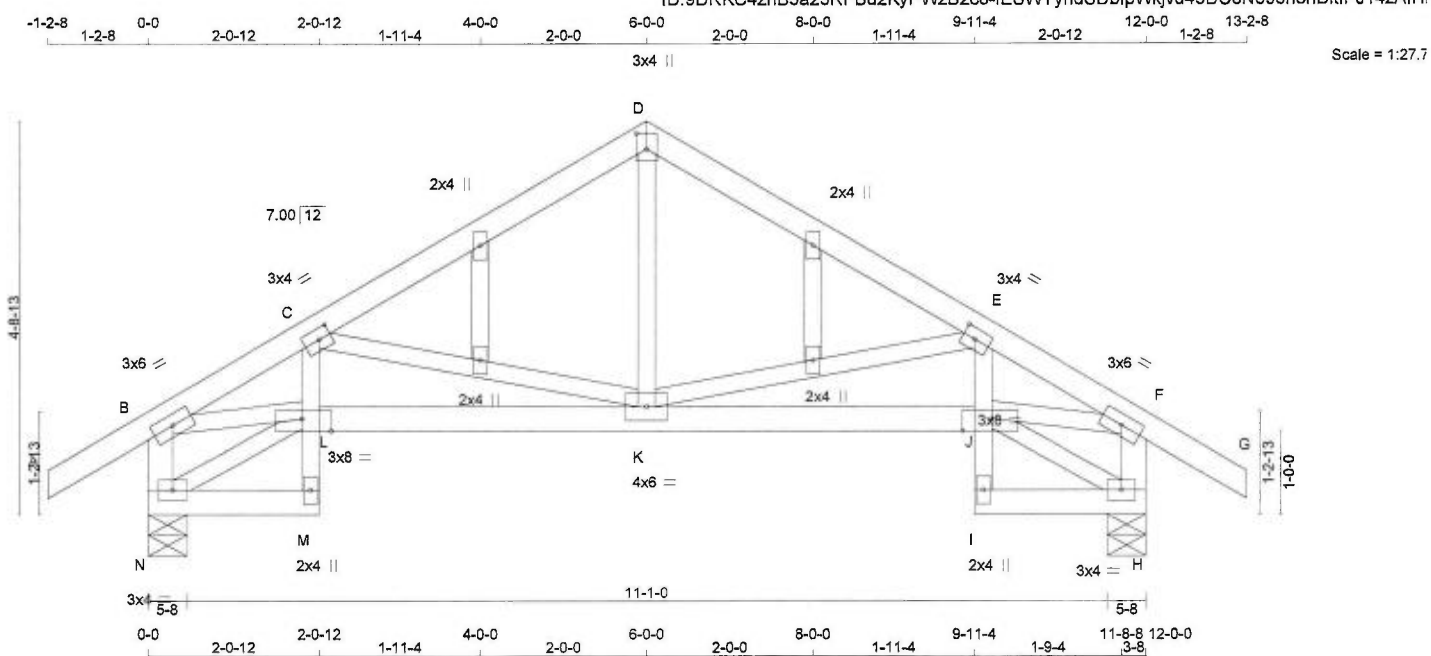
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

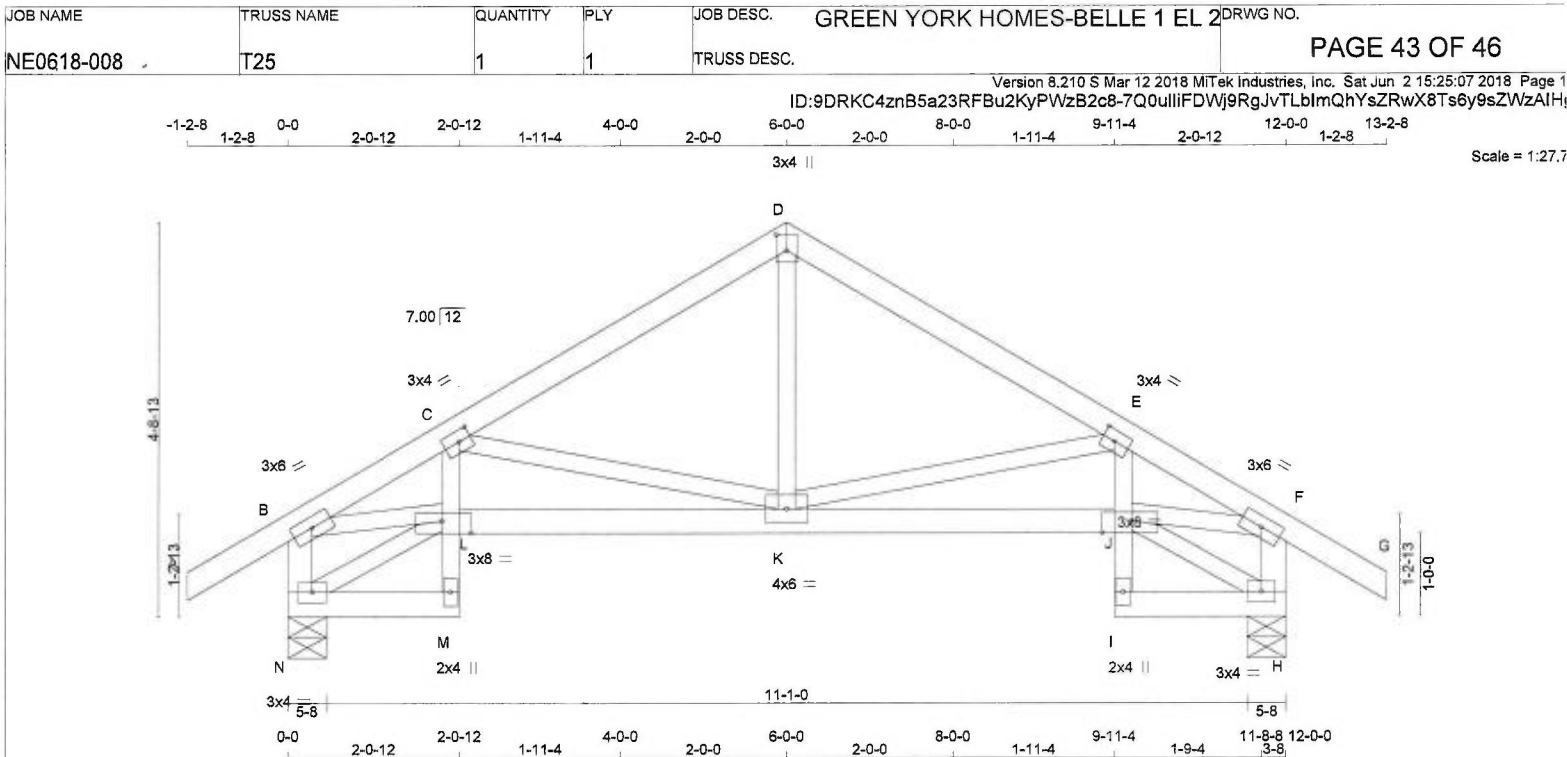




LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS										DESIGN CRITERIA																				
A - D	2x4	DRY	No.2	SPF	FACTORED		MAXIMUM FACTORED		INPUT		REQRD		SPECIFIED LOADS:																					
D - G	2x4	DRY	No.2	SPF	GROSS REACTION		GROSS REACTION		BRG		BRG		TOP CH. LL = 23.3 PSF																					
N - B	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	DL = 3.0 PSF																					
H - F	2x4	DRY	No.2	SPF	N	668	0	668	0	0	5-8	5-8	BOT CH. LL = 0.0 PSF																					
N - M	2x4	DRY	No.2	SPF	H	668	0	668	0	0	5-8	5-8	DL = 7.0 PSF																					
M - C	2x3	DRY	No.2	SPF	UNFACTORED REACTIONS										TOTAL LOAD = 33.3 PSF																			
L - J	2x4	DRY	No.2	SPF											SPACING = 24.0 IN./C/C																			
I - E	2x3	DRY	No.2	SPF																					THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010									
I - H	2x4	DRY	No.2	SPF																														
ALL WEBS EXCEPT				SPF	ALLOWABLE DEFL.(LL)= L/360 (0.40") CALCULATED VERT. DEFL.(LL) = L/999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.40") CALCULATED VERT. DEFL.(TL) = L/999 (0.05") CSI: TC=0.16/1.00 (C-D:1), BC=0.22/1.00 (K-L:1), WB=0.23/1.00 (B-L:1), SSI=0.12/1.00 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656																													
ALL GABLE WEBS EXCEPT				SPF											BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS EXCEPT				SPF																														
ALL GABLE WEBS EXCEPT				SPF	2x3 DRY No.2																													
ALL GABLE WEBS EXCEPT				SPF											2x3 DRY No.2																			
ALL GABLE WEBS EXCEPT				SPF																					2x3 DRY No.2									
ALL GABLE WEBS																																		



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 52 lb
(M)F

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x3	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x3	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	6.0		
C	TMVW-t	MT20	3.0	4.0	1.50	1.75
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.75
F	TMVW-t	MT20	3.0	6.0		
H	BMVW1-t	MT20	3.0	4.0		
I	BMV+p	MT20	2.0	4.0		
J	BMVWW-t	MT20	3.0	8.0	Edge 4.25	
K	BMVWW-t	MT20	4.0	6.0		
L	BMVWW-t	MT20	3.0	8.0	Edge 4.25	
M	BMV+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
N	668	0	668	0	0	5-8	5-8	5-8	
H	668	0	668	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS

		1ST LOASE		MAX./MIN. COMPONENT REACTIONS					
JT		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
N	467	339 / 0	0 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0	
H	467	339 / 0	0 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE	MEMB.	FORCE
(LBS)	(PLF)	(LBS)	(PLF)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 25	K-D	0 / 318
B-C	-1183 / 0	K-E	-574 / 0
C-D	-599 / 0	C-K	-574 / 0
D-E	-599 / 0	B-L	0 / 1044
E-F	-1183 / 0	J-F	0 / 1044
F-G	0 / 25	N-L	-26 / 0
N-B	-638 / 0	J-H	-26 / 0
H-F	-638 / 0		
N-M	0 / 22		
M-L	0 / 19		
L-C	0 / 187		
L-K	0 / 1065		
K-J	0 / 1065		
I-J	0 / 19		
J-E	0 / 167		
I-H	0 / 22		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.16/1.00 (C-D:1), BC=0.22/1.00 (K-L:1), WB=0.23/1.00 (B-L:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

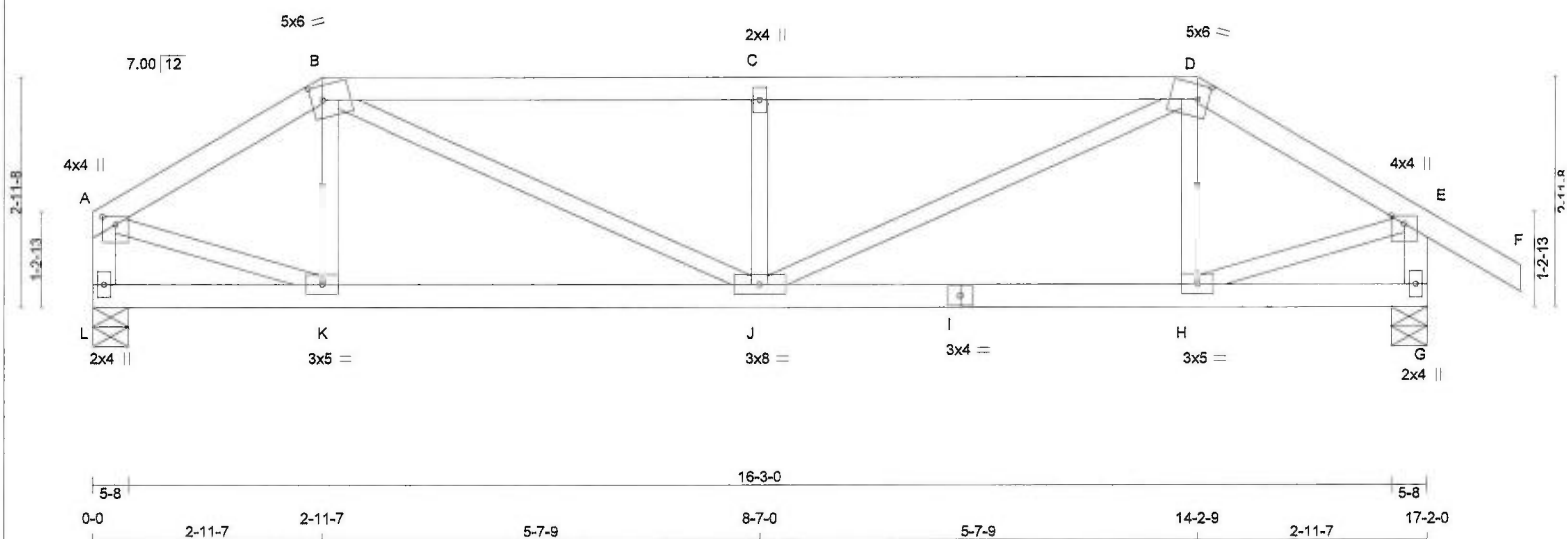
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.32 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

SPF

ALL WEBS

EXCEPT

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
L	814	0	814	0
G	913	0	913	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN.	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
L	571	400 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0	
G	639	459 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT.	LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	MAX. FACTORED	FORCE	MAX	CS1 (LC)
FR-TO									LENGTH	FR-TO				
A-B		-895 / 0		-77.4	-77.4	0.13	(1)	6.25	K-B		-143 / 24		0.02	(1)
B-C		-1426 / 0		-77.4	-77.4	0.45	(1)	4.90	B-J		0 / 730		0.16	(1)
C-D		-1426 / 0		-77.4	-77.4	0.45	(1)	4.90	J-C		-530 / 0		0.09	(1)
D-E		-895 / 0		-77.4	-77.4	0.13	(1)	6.25	J-D		0 / 730		0.16	(1)
E-F		0 / 25		-77.4	-77.4	0.09	(1)	10.00	H-D		-143 / 24		0.02	(1)
L-A		-800 / 0		0.0	0.0	0.08	(1)	7.81	A-K		0 / 806		0.18	(1)
G-E		-899 / 0		0.0	0.0	0.09	(1)	7.81	H-E		0 / 806		0.18	(1)
L-K		0 / 0		-17.5	-17.5	0.08	(4)	10.00						
K-J		0 / 767		-17.5	-17.5	0.17	(1)	10.00						
J-I		0 / 767		-17.5	-17.5	0.17	(1)	10.00						
I-H		0 / 767		-17.5	-17.5	0.17	(1)	10.00						
H-G		0 / 0		-17.5	-17.5	0.08	(4)	10.00						

TOTAL WEIGHT = 64 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")

CALCULATED VERT. DEFL.(LL) = L/999 (0.04")

ALLOWABLE DEFL.(TL)= L/360 (0.57")

CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.45/1.00 (B-C:1), BC=0.17/1.00 (J-K:1), WB=0.18/1.00 (A-K:1), SS=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (A) (INPUT = 0.90)

JSI METAL= 0.27 (I) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW-m	MT20	5.0	6.0	2.25	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMVW-t	MT20	3.0	5.0		
I	BS-t	MT20	3.0	4.0		
J	BMVW-t	MT20	3.0	8.0		
K	BMVW-t	MT20	3.0	5.0		
L	BMV1+p	MT20	2.0	4.0		



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



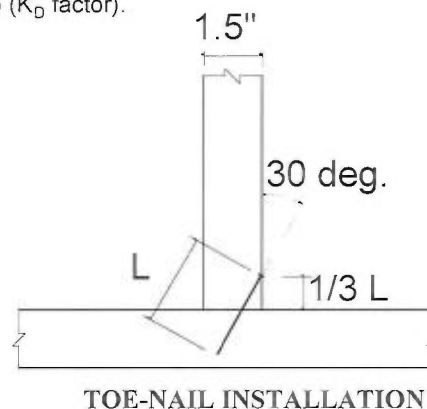
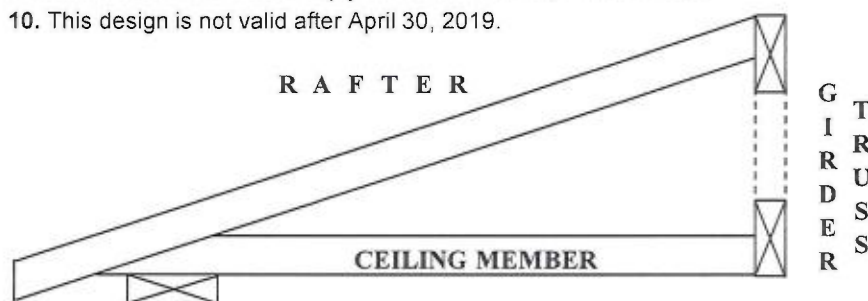
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

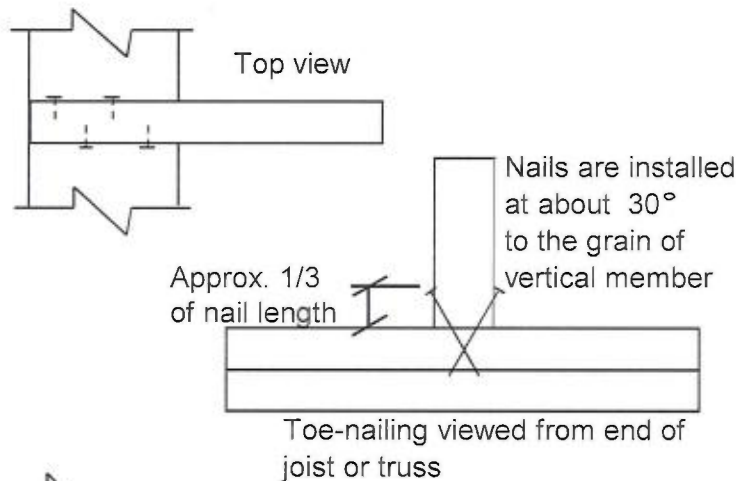
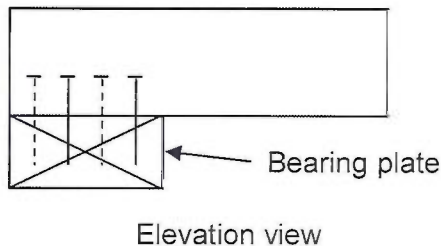
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

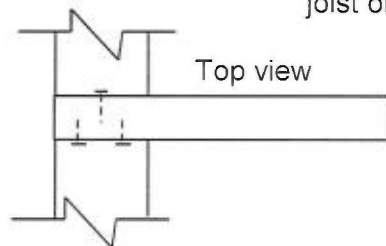
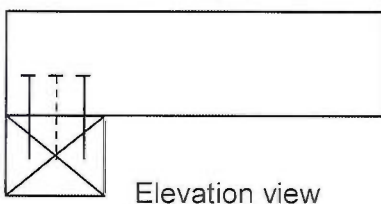
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



PEO
Certificate No. 10889485



April 26, 2017