Layout Name

Design Method

Description

May 29, 2018

Sales Rep

Designer

Shipping

Canada

L4A 7X4

Job Path

905-642-4400

Ground Floor

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Deck

Deflection Girder

Floor

Loads

Live

Dead

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

Kott Lumber Company

D:\Users\rochavillo\WORK FROM

\GRANELLI HOME CORP\MODELS

LSD

2012

40

15

480

360

480

360

360

240

480

360

OSB

3/4"

Nailed & Glued

LIANA 3/LIANA 3 ELEV 1/FLOOR

Building Code NBCC 2010 / OBC

HOME\GREEN YORK HOMES

Project

RCO

LSD

Created

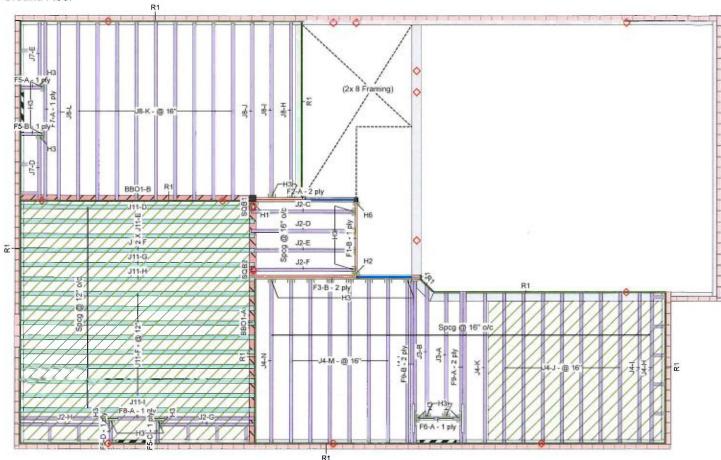
LIANA 3 (ELEV.1 & 2)

GRANELLI HOME CORP.

GREEN YORK HOMES

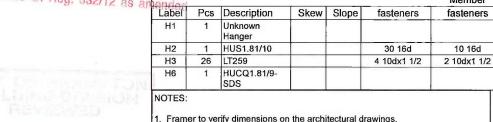
BRAMPTON, ONT.





Engineered floor joiets shall be installed in accordance with the supplier's layout and specifications forming part of the permit drawings.

All work shall conform to the Building Code O. Reg. 332/12 as a



LVL/LSL (Flush) Label Description

Forex

Forex

Label Description

I Joist (Flush)

J11 NJ60U

F8 NJH

F7 NJH

F9 NJH

F6 NJH

F5 NJH

J8 NJH

J4 NJH

J3 NJH

J2 NJH

J7 NJH

Label Description

Label Description

Norbord Rimboard

Plus 1.125 X 9.5

Rim Board

R1

Blocking

langer

ntario

BLK1 NJH

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

F3

F2

F1

Width Depth

Width | Depth |

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

9.5

Width Depth Qty Plies

9.5 LinFt

1.75

1.75

1.75

3.5

2.5

2.5

2.5

2.5

2.5

2.5

2.5

2.5

2.5

2.5

1.125

2.5

Width Depth

Qty

Qty

Qty Plies

Beam/Girder

Plies

2

2

Plies

Pcs Length

Pcs Length

15 18-0-0

12-0-0

8-0-0

6-0-0

18-0-0

14-0-0

12-0-0

2-0-0

8-0-0

12

14-0-0

18 12-0-0

2 10-0-0

2 6-0-0

Pcs Length

Pcs Length

Varies 15-0-0

Supported

Member

4-0-0 Builder

2

2

1

4

13

6

14

1

Framer to verify dimensions on the architectural drawings.

2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.

1. Install single-ply flush window header along inside face of

rimboard/rimjoist.

5. Refer to Nascor specifier guide for installation works.

6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.

Load transfer blocks to be installed under all point loads.

8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

ARCHITECTURAL DRAWINGS

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Liana 3

M-2057

THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

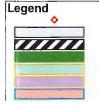
ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.





1. OBC 2012 O.Reg 332/12 as amended

Load from Above

Norbord Rimboard Plus 1.125 X 9.5

Forex 2.0E-3000Fb LVL 1.75 X 9.5

Wall Opening

NJ60U 9.5

NJH 9.5

2. Nascor CCMC - 13535-R

3. LVL CCMC -14056-R

4. CAN/CSA-O86-09

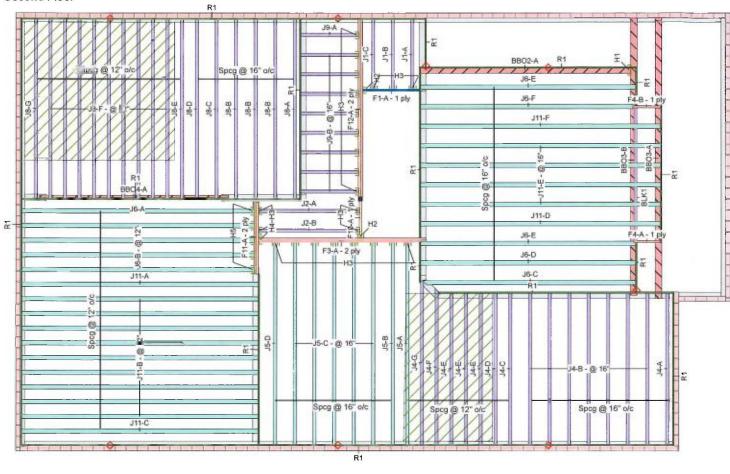
5. CCMC -12787-R APA PR-L310(C)

EWP Studio Simpson Strong-Tie® Component Solutions™

19-447156.000.00RR



Second Floor



THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Liana 3

NJ60H 9.5 NJ60U 9.5 NJH 9.5

> (Dropped) Forex 2.0E-3000Fb LVL 1.75 X 9.5

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

Legend



Load from Above

Norbord Rimboard Plus 1,125 X 9,5

Forex 2.0E-3000Fb LVL 1.75 X 9.5

fastened as per the hanger manufacturer's standards.

bolting requirements Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than

rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

of 5 PSF

and structural drawings. Project Engineer to review and approve the deviation prior

								FAGE 20 OF 30	
Second	Floor								
LVL/LS	L (Flush)							ALACCO!	
	Description	Width	Depth	Qty	Plies	Pcs	Length	NASCO	
F12	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	14-0-0		
F3	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	12-0-0	Layout Name LIANA 3 (ELEV.2)	
F11	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	6-0-0	Design Method	
F1	Forex 2.0E-3000Fb LVL	1.75	9.5			1	6-0-0	Description	
F10	Forex 2.0E-3000Fb LVL	1.75	9.5			1	4-0-0	GRANELLI HOME CORP. BRAMPTON, ONT.	
F4	Forex 2.0E-3000Fb LVL	1.75	9.5			2	2-0-0	Created May 29, 2018	
LVL/LS	L (Dropped)								
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	Builder	
BBO4	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	14-0-0	GREEN YORK HOMES Sales Rep	
I Joist (Flush)							RM	
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	Designer	
J5	NJ60H	2.5	9.5			8	14-0-0	RCO	
J11	NJ60U	3.5	9.5			17	18-0-0	11111	
J6	NJ60U	3.5	9.5			10	16-0-0	Shipping	
J8	NJH	2.5	9.5			17	14-0-0	Project	
J4	NJH	2.5	9.5			15	12-0-0	Builder's Project	
J2	NJH	2.5	9.5			2	8-0-0	Kott Lumber Company	
J1	NJH	2.5	9.5			3	6-0-0	14 Anderson Blvd	
J9	NJH	2.5	9.5			9	4-0-0	, , , , , , , , , , , , , , , , , , ,	
Rim Bo	1							Stouffville, Ontario	
Label		Width	Depth	Qty	Plies	Pcs	Length	Canada	
R1	Norbord Rimboard Plus 1.125 X 9.5	1.125	9.5			19	12	L4A 7X4 905-642-4400	
Blockin	g							Job Path	
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	D:\Users\rochavillo\WORK FROM	
DUZA	N1 31 1	0.5	0.5	12.54		14-3	0.00	1	

Label | Description Width | Depth | Qty | Plies | Pcs | Length BLK1 NJH 2.5 9.5 LinFt Varies 8-0-0 Hanger

					Beam/Girder	Supported Member	1
Label	Pcs	Description	Skew	Slope	fasteners	fasteners	1
H1	1	Unknown Hanger					
H2	2	HUS1.81/10			30 16d	10 16d	þ
НЗ	24	LT259			4 10dx1 1/2	2 10dx1 1/2	lı
H4	1	HGUS410			46 16d	16 16d	1
H5	5	1T350			4 10d	2 10dv1 1/2	1.

NOTES:

- 1. Framer to verify dimensions on the architectural drawings. 2. Double joist only require filler/backer ply when supporting
- another member using a face-mounted hanger. 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- 4. Install single-ply flush window header along inside face of
- . Refer to Nascor specifier guide for installation works.
- . Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- Load transfer blocks to be installed under all point loads. . It shall be the framer's responsibility that floor joists and beams are
- Refer to Multiple Member Connection Detail to ply to ply nailing or

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may deviate from the architectural

HOME\GREEN YORK HOMES GRANELLI HOME CORP\MODELS \LIANA 3\LIANA 3 ELEV 2\FLOOR

Second Floor Design Method LSD Building Code NBCC 2010 / OBC 2012 Floor Loads 40 Live 15 Dead **Deflection Joist** LL Span L/ 480 TL Span L/ 360 LL Cant 2L/ 480 TL Cant 2L/ 360 Deflection Girder LL Span L/ 360 TL Span L/ 240

480

360

OSB

5/8"

Nailed & Glued

Gypsum 1/2"

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

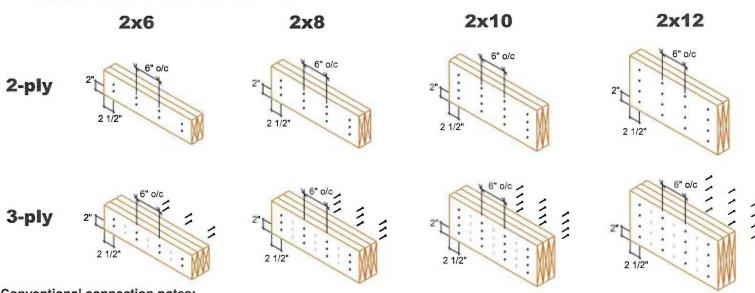
Ceiling:

Deck



MULTIPLE MEMBER CONNECTIONS

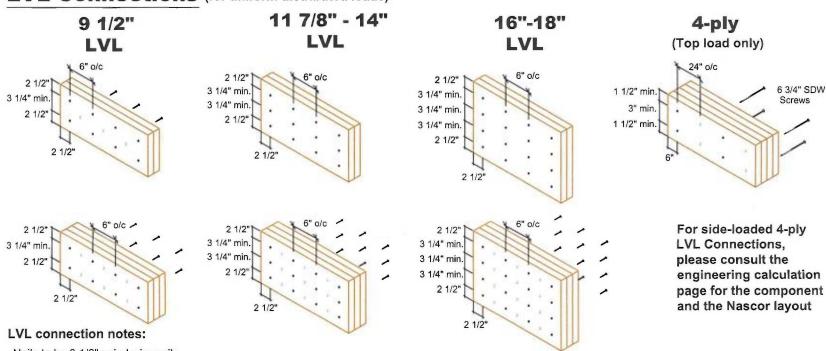
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

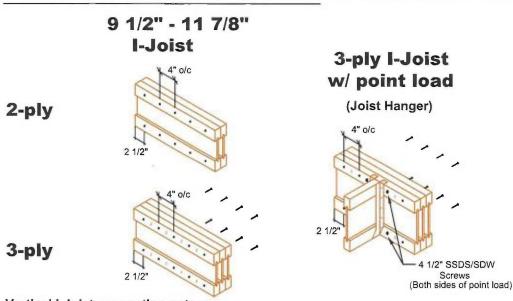
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails. -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI-PLY CONNECTION **DETAILS**

Scale: NTS

KOTT 3228 Moodie Drive Ottawa, ON **K2H 7V1** Ph: 613-838-2775

Fx: 613-838-4751

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project: Address: **GREEN YORK HOMES**

5/31/2018

Date: **RCO**

Designer:

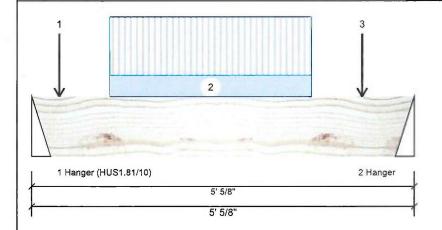
Job Name: LIANA 3 (ELEV.1)

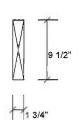
Project #:

Forex 2.0E-3000Fb LVL F1-B

1.750" X 9.500" - PASSED

Level: Ground Floor





Wind

0

0

1.25D+1.5L

1.25D+1.5L

0

n

677 L

629 L

Member Information

MEHIDEL HILOHII	lation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactor	red Reactions	UNPATTER	NED lb (Uplift)
Brg	Live	Dead	Snow

136

127

338

314

3.000"

3.000"

1

2

Hanger

Hanger

2 -

Bearing	s and Fac	tored	Reactions				
Bearing	Length	Can	React D/L lb	Total	1d Case	Ld Comb	

170 / 507

159 / 471

Analysis Results

ı	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	716 ft-lb	2'6 7/16"	11362 ft-lb	0.063 (6%)	1.25D+1.5L	L
l	Unbraced	716 ft-lb	2'6 7/16"	7908 ft-lb	0.091 (9%)	1.25D+1.5L	L
l	Shear	672 lb	11 3/4"	4638 lb	0.145 (14%)	1.25D+1.5L	L
	Perm Defl in.	0.003 (L/17255)	2'6 7/16"	0.156 (L/360)	0.020 (2%)	D	Uniform
l	LL Defl inch	0.008 (L/6957)	2'6 7/16"	0.156 (L/360)	0.050 (5%)	L	L
ı	TI Deflinch	0.011 (L/4958)	2'6 7/16"	0.234 (L/240)	0.050 (5%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA

17%

16%

USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-4-6		Far Face	48 lb	128 lb	0 lb	0 lb	J2
2	Part. Uniform	1-0-6 to 3-8-6		Far Face	52 PLF	139 PLF	0 PLF	0 PLF	
3	Point	4-4-6		Far Face	57 lb	153 lb	0 lb	0 lb	J2
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply Refer to manufacturers product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals. Damaged Beams must not be used. Design assumes top edge is laterally restrained. Provide lateral support at bearing points to avoid lateral displacement and rotation.

For flat roofs provide proper drainage to prevent ponding









EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018 Designer: RCO

Job Name: LIANA 3 (ELEV.1)

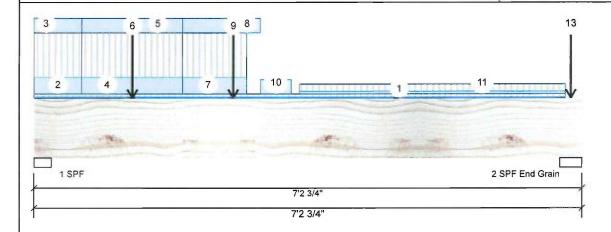
Project #:

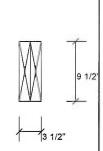
Forex 2.0E-3000Fb LVL F2-A

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





M	em	ber	Inf	orn	nati	on

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application:

Design Method: LSD Building Code: NBCC 2010 / OBC 2012

Load Sharing: No Deck: Not Checked Vibration: Not Checked Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1088	645	0	0
2	1613	756	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3003 ft-lb	2'7 9/16"	22724 ft-lb	0.132 (13%)	1.25D+1.5L	L.
Unbraced	3003 ft-lb	2'7 9/16"	21802 ft-lb	0.138 (14%)	1.25D+1.5L	L
Shear	1838 lb	11 1/2"	9277 lb	0.198 (20%)	1.25D+1.5L	L
Perm Defl in.	0.014 (L/5673)	3'1 13/16"	0.228 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.024 (L/3355)	3'1 9/16"	0.228 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.039 (L/2108)	3'1 5/8"	0.342 (L/240)	0.110 (11%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.750"	41%	807 / 1632	2438	L	1.25D+1.5L
2 - SPF End	3.500"	37%	945 / 2419	3364	L	1.25D+1.5L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



и	T Editing district fact based on fall based in fall					1				
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 7-0-2	(Span)0-10-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Part. Uniform	0-0-0 to 0-7-9		Тор	92 PLF	246 PLF	0 PLF	0 PLF	J8
	3	Part. Uniform	0-0-0 to 0-7-9		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
	4	Part. Uniform	0-7-9 to 1-11-9		Тор	92 PLF	246 PLF	0 PLF	0 PLF	J8
ı	5	Part. Uniform	0-7-9 to 1-11-9		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply

regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318





Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

GREEN YORK HOMES

5/31/2018 Date:

Designer: RCO

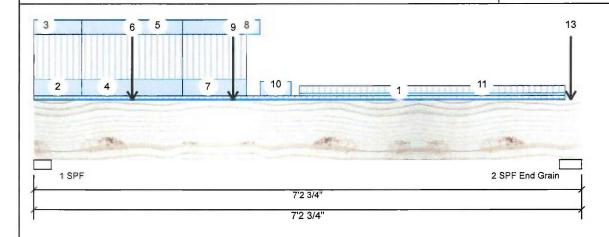
Job Name: LIANA 3 (ELEV.1)

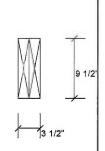
Project #:

F2-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





Continued fro	m page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	1-3-9		Far Face	121 lb	322 lb	0 lb	0 lb	J8
7	Part. Uniform	1-11-9 to 2-9-11		Тор	92 PLF	246 PLF	0 PLF	0 PLF	J8
8	Part. Uniform	1-11-9 to 2-11-14		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
9	Point	2-7-9		Far Face	93 lb	248 lb	0 lb	0 lb	J8
10	Part. Uniform	2-11-14 to 3-4-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
11	Part. Uniform	3-6-2 to 7-0-2		Тор	15 PLF	40 PLF	0 PLF	0 PLF	
12	Point	7-1-0		Near Face	127 lb	314 lb	0 lb	dl 0	F1
13	Point	7-1-0		Тор	379 lb	869 lb	0 lb	dl 0	C3
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. LVL beams must not be out or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

1. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

5/31/2018 Date: Designer: RCO

Job Name: LIANA 3 (ELEV.1)

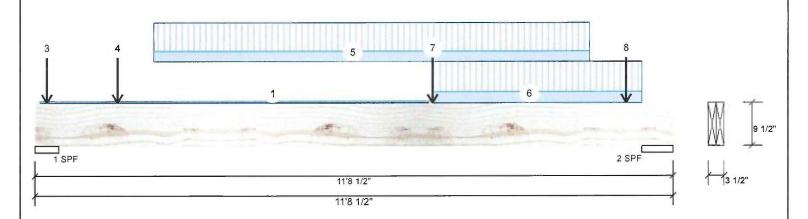
Project #:

Forex 2.0E-3000Fb LVL F3-B

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



Mem	her	Inform	nation

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential)

Design Method: NBCC 2010 / OBC 2012 **Building Code:**

Load Sharing: Deck: Not Checked Not Checked Vibration:

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	1523	642	0	0
2	2170	865	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	5.250"	27%	803 / 2285	3088	L	1.25D+1.5L	
2 - SPF	6.875"	29%	1081 / 3255	4335	L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10162 ft-lb	6'11 3/4"	22724 ft-lb	0.447 (45%)	1.25D+1.5L	L
Unbraced	10162 ft-lb	6'11 3/4"	20411 ft-lb	0.498 (50%)	1.25D+1.5L	L
Shear	3978 lb	10'4 7/8"	9277 lb	0.429 (43%)	1.25D+1.5L	L
Perm Defl in.	0.091 (L/1425)	6'	0.361 (L/360)	0.250 (25%)	D	Uniform
LL Defl inch	0.227 (L/571)	6' 1/8"	0.361 (L/360)	0.630 (63%)	L.	L
TL Defl inch	0.319 (L/408)	6' 1/8"	0.541 (L/240)	0.590 (59%)	D+L	L

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

1	/ Lateral Sieriue	illess lado pased oil i	uli section width.					_		
ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
ı	1	Tie-In	0-1-2 to 7-2-12	(Span)0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	2	Point	0-2-10		Тор	37 lb	99 lb	0 lb	0 lb	J11
	3	Point	0-2-10		Тор	24 lb	0 lb	0 lb	0 lb	Wall Self Weight
ı	4	Point	1-6-3		Near Face	103 lb	276 lb	0 lb	0 lb	J4
ı	5	Part. Uniform	2-2-3 to 10-2-3		Near Face	84 PLF	224 PLF	0 PLF	0 PLF	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

page 3 per substance of the state of the sta

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







Continued from page 1

Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Address:

Project:

GREEN YORK HOMES

5/31/2018 Date:

RCO Designer:

Job Name: LIANA 3 (ELEV.1)

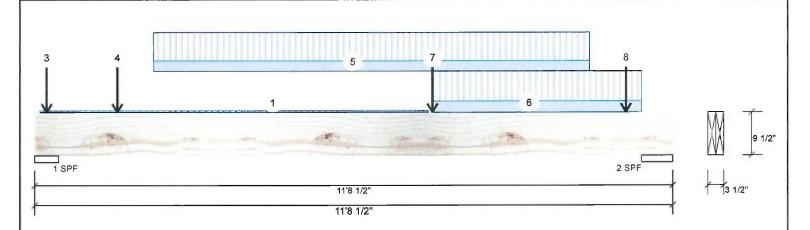
Project #:

Forex 2.0E-3000Fb LVL F3-B

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



I	Continued from	i page i									
ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
ı	6	Part, Uniform	7-3-10 to 11-1-10		Тор	90 PLF	240 PLF	0 PLF	0 PLF		
ı	7	Point	7-3-10		Far Face	136 lb	338 lb	0 lb	0 lb	F1	
ı	8	Point	10-10-3		Near Face	73 lb	195 lb	0 lb	0 lb	J4	
I		Self Weight				8 PLF					

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL, not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andling & installation:
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation 1.

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project: Address: GREEN YORK HOMES

5/31/2018 Date:

Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Project #:

1

2

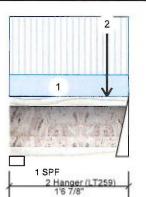
2 -

Hanger

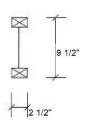
1 - SPF 2.375"

2.000"

9.500" - PASSED F5-A NJH



Level: Ground Floor



Wind

0

0

1.25D+1.5L

1.25D+1.5L

0

0

122 L

226 L

Member Information

1'6 7/8'

Ì	Type:	Girder	Application:	Floor (Residential)
ı	Plies:	1	Design Method:	LSD
ı	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
	Deflection LL:	360	Load Sharing:	No
ı	Deflection TL:	240	Deck:	Not Checked
ı	Importance:	Normal	Vibration:	Not Checked
ı	General Load			
ı	Floor Live:	40 PSF		
ı	Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Snow Dead

115

Bearings and	Factored Re	eactions	

29 / 93

53 / 173

23

43

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	43 ft-lb	11 9/16"	3830 ft-lb	0.011 (1%)	1.25D+1.5L	L
Unbraced	43 ft-lb	11 9/16"	3779 ft-lb	0.011 (1%)	1.25D+1.5L	L
Shear	213 lb	1'5 5/8"	1580 lb	0.135 (13%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/31491)	11 3/8"	0.044 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/22944)	11 3/8"	0.067 (L/240)	0.010 (1%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT** CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

8%

14%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-3-7		Far Face	28 lb	76 lb	0 lb	0 lb	J7

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the interapplication, and to verify the dimensions and loads. Lumber

- Dry service conditions, unless noted otherwise
 Unist not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- andling & Installation

 Loist flanges must not be cut or drilled

 Refer to latest copy of the Jusist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-py fastening details and handling/erection details

 Damaged blosts must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3,5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

GREEN YORK HOMES

Project: Address:

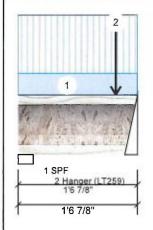
Date: 5/31/2018

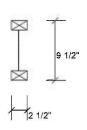
Designer: RCO

Job Name: LIANA 3 (ELEV.1)

9.500" - PASSED NJH F5-B

Level: Ground Floor





Wind

0

0

Mana	har li	nforma	つキェヘト
IVICIII	vei ii		auvi

	Туре:	Girder	Application:	Floor (Residential)
	Plies:	1	Design Method:	LSD
	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
	Deflection LL:	360	Load Sharing:	No
	Deflection TL:	240	Deck:	Not Checked
	Importance:	Normal	Vibration:	Not Checked
l	General Load			
ı	Floor Live:	40 PSF		
l	Dead:	15 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead

23

43

Live

62

113

1

2

Hanger

Bearings	Bearings and Factored Reactions										
Bearing	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.					
1 - SPF	2.375"	8%	29 / 92	121	L	1.25D+1.5L					
2-	2 000"	14%	53 / 170	223	1	1 25D+1 5I					

Snow

0

0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	43 ft-lb	11 1/2"	3830 ft-lb	0.011 (1%)	1.25D+1.5L	L	
Unbraced	43 ft-lb	11 1/2"	3779 ft-lb	0.011 (1%)	1.25D+1.5L	L	
Shear	210 lb	1'5 5/8"	1580 lb	0.133 (13%)	1.25D+1.5L	L	
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
LL Defl inch	0.001 (L/31811)	11 3/8"	0.044 (L/360)	0.010 (1%)	L	L	
TL Defl inch	0.001 (L/23114)	11 3/8"	0.067 (L/240)	0.010 (1%)	D+L	L	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-3-7		Near Face	28 lb	74 lb	0 lb	0 lb	J1

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Joist not to be treated with fire retardant or corrosive

chemicals

- Handling & Installation
- I Noist flanges must not be out or drilled

 Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole chart, bridging details, stiffener tables, web hole chart, bridging details, multi-byl festening details and handling/erection details

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3,5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott









Client:

Address:

Project:

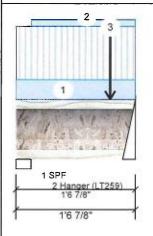
GREEN YORK HOMES

5/31/2018 Date: Designer: **RCO**

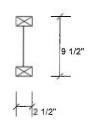
Job Name: LIANA 3 (ELEV.1)

Project #:

9.500" - PASSED F5-C NJH



Level: Ground Floor



Member	Informatio	r

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method:

Building Code:

LSD NBCC 2010 / OBC 2012

Load Sharing: No Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	70	33	0	0
2	141	69	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	9%	42 / 106	147	L	1.25D+1.5L
2 - Hanger	2.000"	19%	87 / 211	298	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	63 ft-lb	1'1 1/8"	3830 ft-lb	0.016 (2%)	1.25D+1.5L	L
Unbraced	63 ft-lb	1'1 1/8"	3779 ft-lb	0.017 (2%)	1.25D+1.5L	L
Shear	283 lb	1'5 5/8"	1580 lb	0.179 (18%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/47832)	1' 5/8"	0.044 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/23647)	1' 5/8"	0.044 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.001 (L/15824)	1' 5/8"	0.067 (L/240)	0.020 (2%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-2-14		Near Face	54 lb	110 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the austomer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- 1. Lipist flanges must not be cut or drilled
 2. Refer to latest copy of the Lipist product information details for framing details, stiffener tables, web hole chart, bradging details, multi-ply fasterining details and handling/erection details
 3. Damaged Lipists must not be used
 4. Dosign assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent poorline.

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018

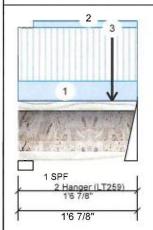
Designer: RCO

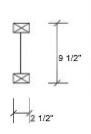
Job Name: LIANA 3 (ELEV.1)

Project #:

9.500" - PASSED F₅-D NJH

Level: Ground Floor





Wind

0

0

0

0

Member Inform	nation
Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012 Load Sharing:

No Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Snow Brg Live Dead

33

67

70

136

1

2

Hanger

l	Bearing	s and Fac	tored l	Reactions				
1	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1	1 - SPF	2.375"	9%	41 / 104	145	L	1.25D+1.5L	
4	2 -	2.000"	18%	84 / 205	288	L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	61 ft-lb	1'1"	3830 ft-lb	0.016 (2%)	1.25D+1.5L	L
Unbraced	61 ft-lb	1'1"	3779 ft-lb	0.016 (2%)	1.25D+1.5L	L
Shear	274 lb	1'5 5/8"	1580 lb	0.173 (17%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/49408)	1' 7/16"	0.044 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/24289)	1' 9/16"	0.044 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/16284)	1' 1/2"	0.067 (L/240)	0.010 (1%)	D+L	L

Deck: Vibration:

> READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-2-14		Far Face	51 lb	105 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upost not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Librist flanges must not be out or drilled

 2. Refer to latest copy of the Librist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged Librists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengthers 3,5 inches
 For flat roofs provide proper drainage to prevent ponding.

Nascor by Kott



Manufacturer Info





EWP Studio Simpson Strong-Tie® Component Solutions™

Client: **GREEN YORK HOMES**

Project:

Address:

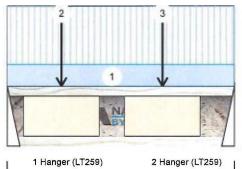
Date:

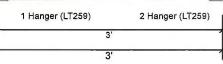
5/31/2018 Designer: **RCO**

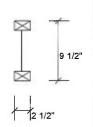
Job Name: LIANA 3 (ELEV.1)

9.500" - PASSED F6-A NJH

Level: Ground Floor







Wind

0

Member	Information
Туре:	Girder

Type.	Girdei
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: Not Checked Deck: Vibration: Not Checked

260 2 233 88

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

Snow

0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	360 ft-lb	2' 9/16"	3830 ft-lb	0.094 (9%)	1.25D+1.5L	L
Unbraced	360 ft-lb	2' 9/16"	3411 ft-lb	0.105 (11%)	1.25D+1.5L	L
Shear	505 lb	1 1/4"	1580 lb	0.319 (32%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/16963)	1'8 1/4"	0.093 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.005 (L/6359)	1'8 1/4"	0.093 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.007 (L/4625)	1'8 1/4"	0.140 (L/240)	0.050 (5%)	D+L	L

Bearings and Factored Reactions

Live

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	32%	122 / 390	512	L	1.25D+1.5L	
2 - Hanger	2.000"	29%	109 / 350	459	L	1.25D+1.5L	

D

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

LL Defl inch	0.005 (L/6359)	1'8 1/4"	0.093 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.007 (L/4625)	1'8 1/4"	0.140 (L/240)	0.050 (5%)	D+L	L
esign Not	es					
1 Fill all hang	er nailing holes.					

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL OINT LOADS OVER BEARINGS



	4 Dottom hange	bideed at bearings.					LOUISI FOVE	2 OAFU BEN	KINGS.	
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
1	2	Point	0-8-9		Far Face	70 lb	188 lb	0 lb	dl 0	J3
,	3	Point	2-0-9		Far Face	76 lb	202 lb	0 lb	dl 0	J3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Usist flanges must not be cut or drilled
 2. Refer to latest copy of the Libist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-py flastening details and handling/drection details
 2. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™

Client: **GREEN YORK HOMES**

> Project: Address:

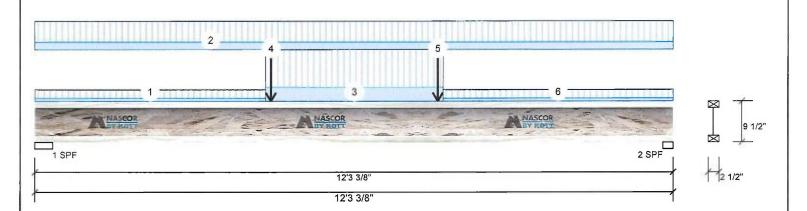
5/31/2018 Date:

Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Project #:

F7-A NJH 9.500" - PASSED Level: Ground Floor



Member Info	rmation			Unfactor	red Reac	tions U	NPATTERN	ED lb ((Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	1	Design Method:	LSD	1	327		123		0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	320		120		0	0
Deflection LL:	360	Load Sharing:	No	1.77						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored I	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	4.125"	41%	154 / 490	644	L	1.25D+1.5L
h:. D				2 - SPF	2.375"	40%	150 / 480	630	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2352 ft-lb	6'2 11/16"	3830 ft-lb	0.614 (61%)	1.25D+1.5L	L
Unbraced	2352 ft-lb	6'2 11/16"	2368 ft-lb	0.993 (99%)	1.25D+1.5L	L
Shear	629 lb	3 3/8"	1580 lb	0.398 (40%)	1.25D+1.5L	L
Perm Defl in.	0.071 (L/2010)	6'2 1/2"	0.395 (L/360)	0.180 (18%)	D	Uniform
LL Defi inch	0.188 (L/756)	6'2 9/16"	0.395 (L/360)	0.480 (48%)	L	L
TL Defl inch	0.259 (L/549)	6'2 9/16"	0.593 (L/240)	0.440 (44%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 4'9" o.c.

3 5	Bottom flange praced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-5-5	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-3-6	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	4-5-5 to 7-10-5	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	4-6-9		Far Face	43 lb	113 lb	0 lb	0 lb	F5
5	Point	7-9-1		Far Face	43 lb	115 lb	0 lb	0 lb	F5
6	Tie-In	7-10-5 to 12-3-6	(Span)0-4-11	Top	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive

Handling & Installation

- Livist Banges must not be cut or drilled
 Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multiply festening details and handling/erection details
 Damaged IJoists must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott





Client: Project:

GREEN YORK HOMES

Address:

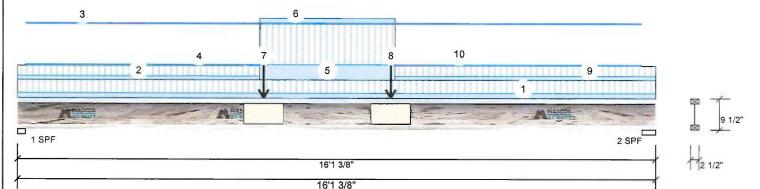
Date: 5/31/2018 Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Project #:

9.500" - PASSED F8-A NJH

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift)

Girder Type: Plies: 1 Moisture Condition: Dry Deflection II: 360 Deflection TL: 240 Importance: Normal General Load Floor Live: 40 PSF Dead: 15 PSF

Application: Floor (Residential) Design Method: LSD NBCC 2010 / OBC 2012 Building Code: Load Sharing: No Deck: Not Checked Vibration: Not Checked

Brg Live Dead Snow Wind 345 167 0 0 1 2 342 0 0 164

Bearings and Factored Reactions Bearing Length Cap. React D/L lb

Total Ld. Case Ld. Comb. 1 - SPF 2.375" 46% 208 / 518 726 L 1.25D+1.5L 2 - SPF 4.125" 45% 206 / 513 718 L 1.25D+1.5L

Analysis Results

Analysis Location Allowed Comb. Capacity Case Moment 3768 ft-lb 8' 3/16" 3830 ft-lb 0.984 (98%) 1.25D+1.5L L 3768 ft-lb Unbraced 8' 3/16" 3771 ft-lb 0.999 1.25D+1.5L L (100%)721 lb 0.456 (46%) 1.25D+1.5L L Shear 1 5/8" 1580 lb Perm Defl in. 0.221 (L/852) 7'11 5/8" 0.523 (L/360) 0.420 (42%) D Uniform LL Defl inch 0.456 (L/413) 7'11 5/8" 0.523 (L/360) 0.870 (87%) L L TL Defl inch 0.677 (L/278) 7'11 5/8" 0.785 (L/240) 0.860 (86%) D+L L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 1'5" o.c.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-1-6	(Span)0-6-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 6-1-6	(Span)0-5-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part, Uniform	0-2-6 to 15-9-1		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 6-1-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
5	Tie-In	6-1-6 to 9-6-6	(Span)1-8-11 to 1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Part. Uniform	6-1-6 to 9-6-6		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
7	Point	6-2-10		Near Face	67 lb	136 lb	0 lb	0 lb	F5

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 Uses not to be treated with fire retardant or correspond

Handling & Installation

- andling & Installation

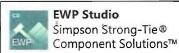
 Loist flanges must not be cut or drilled
 Refer to latest copy of the I-Joist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-pyl fastening details and
 handling/erection details
 Damaged blosts must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roots provide proper drainage to prevent ponding

Manufacturer Info Nascor by Kott



Page 2 of 2



Client:

Project: Address: GREEN YORK HOMES

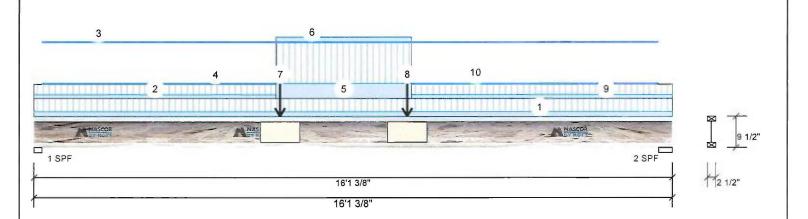
5/31/2018 Date: Designer: RCO

Job Name: LIANA 3 (ELEV.1)

Project #:

9.500" - PASSED F8-A NJH

Level: Ground Floor



ŀ	Continued from p	page 1								
l	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
١	8	Point	9-5-2		Near Face	69 lb	141 lb	0 lb	0 lb	F5
l	9	Tie-In	9-6-6 to 16-1-6	(Span)0-5-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	10	Part. Uniform	9-6-6 to 15-9-1		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Doist not to be treated with fire retardant or corrosive

- Handling & Installation

 1. Moist flanges must not be cut or drilled

 2. Refer to latest copy of the Moist steffener stables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged Moists must not be used

 4. Design assumes to flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Manufacturer Info

Nascor by Kott









Client:

Project: Address: GREEN YORK HOMES

Date: 5/31/2018

Designer: RCO

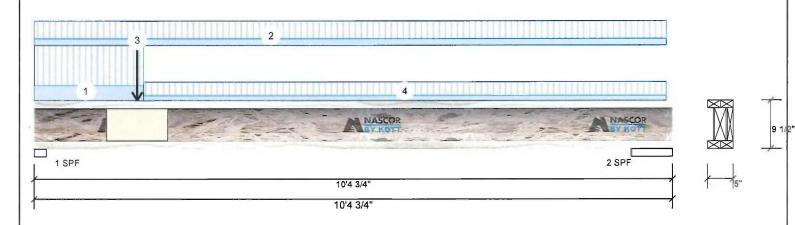
Job Name: LIANA 3 (ELEV.1)

Project #:

F9-A NJH 9.500"

2-Ply - PASSED

Level: Ground Floor



Unfactored Reactions UNPATTERNED Ib (Uplift) Member Information Live Dead Snow Type: Girder Application: Floor (Residential) Plies: 2 Design Method: LSD 535 201 0 1 NBCC 2010 / OBC 2012 Moisture Condition: Dry **Building Code:** 328 123 0 2 Deflection LL: Load Sharing: Deflection TL: 240 Deck: Not Checked Not Checked Importance: Normal Vibration: General Load Floor Live: 40 PSF Bearings and Factored Reactions Dead: 15 PSF Cap. React D/L lb Total Ld. Case Bearing Length 1 - SPF 2.375" 33% 251 / 802 1053 L 154 / 492 646 L 2 - SPF 8.000" 20%

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	1673 ft-lb	4'1 3/4"	7660 ft-lb	0.218 (22%)	1.25D+1.5L	L	
Unbraced	1673 ft-lb	4'1 3/4"	1685 ft-lb	0.993 (99%)	1.25D+1.5L	L	
Shear	1027 lb	1 5/8"	3160 lb	0.325 (32%)	1.25D+1.5L	L	
Perm Defl in.	0.018 (L/6275)	4'8 7/16"	0.322 (L/360)	0.060 (6%)	D	Uniform	
LL Defl inch	0.049 (L/2357)	4'8 7/16"	0.322 (L/360)	0.150 (15%)	L	L	
TL Defl inch	0.068 (L/1713)	4'8 7/16"	0.483 (L/240)	0.140 (14%)	D+L	L	

Design Notes

1 0	Girders are designed to be supported	on the bottom ed	dge only.			CONNECTIO			
2 N	fultiple plies must be fastened togeth	er as per manufa	acturer's details	S.		NAILING OR	BOLTING RE	QUIREME	NTS.
3 T	op loads must be supported equally t	oy all plies.				PASS THRU	FRAMING SO	QUASH	
4 T	op flange must be laterally braced at	a maximum of 8	'7" o.c.			BLOCK IS RI			
5 B	ottom flange braced at bearings.					POINT LOAD	S OVER BEA	RINGS.	
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Com



Wind

0

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 10-3-9	(Span)1-6-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-8-2		Far Face	88 lb	233 lb	0 lb	0 lb	F6
4	Tie-In	1-9-6 to 10-3-9	(Span)1-1-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Lioust flanges must not be cut or drilled Refer to latest copy of the Lioust product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details Damaged blosts must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- Toward attent appoint a dealing points to show lateral displacement and rotation.

 We stiffeners for point load as shown Minimum point load bearing length>= 3,5 inches.

 For flat roofs provide proper drainage to prevent
- ponding

Manufacturer Info

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER

Nascor by Kott







EWP Studio Simpson'Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

Designer: RCO

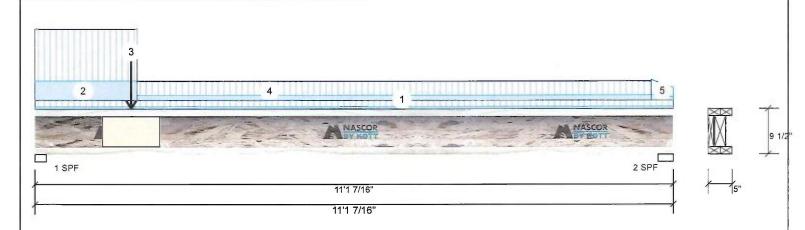
Job Name: LIANA 3 (ELEV.1)

Project #:

Date:

F9-B NJH 9.500" 2-Ply - PASSED Level: Ground Floor

5/31/2018



Member Info	rmation			Unfactore	d Reacti	ons UNPATTERN	IED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	453	169	0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	192	72	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Fact	ored Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Cas	e Ld. Comb.
				1 - SPF 2.	.375"	28% 212 / 679	891 L	1.25D+1.5L
				2 - SPE 3	188"	12% 90 / 288	377	1.25D+1.5I

Analysis Results

_	-							
Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
	Moment	1293 ft-lb	3'10 13/16"	7660 ft-lb	0.169 (17%)	1.25D+1.5L	L	
	Unbraced	1293 ft-lb	3'10 13/16"	1300 ft-lb	0.994 (99%)	1.25D+1.5L	L	
	Shear	871 lb	1 5/8"	3160 lb	0.275 (28%)	1.25D+1.5L	L	
	Perm Defl in.	0.017 (L/7653)	5'1 5/16"	0.359 (L/360)	0.050 (5%)	D	Uniform	
l	LL Defl inch	0.045 (L/2865)	5'1 1/4"	0.359 (L/360)	0.130 (13%)	L	L	
ı	TL Defl inch	0.062 (L/2084)	5'1 1/4"	0.539 (L/240)	0.120 (12%)	D+L	L	

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 9'6" o.c.

5 Bottom flange	braced at bearings.					FOINT LOAD	3 OVER BEA	IKINGS.	
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-1-7	(Span)0-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-8-2		Near Face	97 lb	260 lb	0 lb	0 lb	F6
4	Tie-In	1-9-6 to 10-8-14	(Span)0-11-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	10-8-14 to 11-1-7	(Span) 0-11-14 to	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Ualst not to be treated with fire retardant or corrosive

Handling & Installation

- Loost flanges must not be cut or drilled

 Refer to latest copy of the Libist product information details for framing details, stiffener tables, web hole chart, bridging details, multiply fastening details and handling/erection details

 Damaged Loists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengthers 3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

GREEN YORK HOMES

5/31/2018 Date:

Designer: RCO

Job Name: LIANA 3 (ELEV.1)

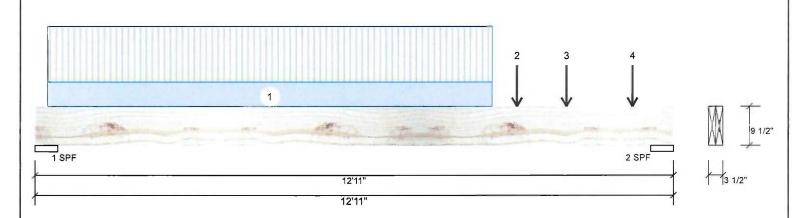
Project #:

Forex 2.0E-3000Fb LVL BBO4-A

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Girder 2 Ory	Application: Design Method:	Floor (Residential) LSD	Brg	Live		Dead	Canus	347 - 1
	Design Method:	ISD				Dead	Snow	Wind
Ory		LOD	1	1515		707	0	0
	Building Code:	NBCC 2010 / OBC 2012	2	1536		676	0	0
360	Load Sharing:	No						
240	Deck:	Not Checked						
Normal	Vibration:	Not Checked						
10 PSF			Bearings	and Fact	tored R	leactions		
15 PSF			Bearing	Length	Cap.	React D/L lb	Total Ld. Case	Ld. Comb.
			1 - SPF	5.500"	27%	884 / 2273	3156 L	1.25D+1.5L
			2 - SPF	5.500"	27%	845 / 2304	3148 L	1.25D+1.5L
24	40 ormal) PSF	Deck: Vibration:	Deck: Not Checked Vibration: Not Checked	Load Sharing: No Deck: Not Checked	Load Sharing: No Deck: Not Checked Vibration: Not Checked DPSF Bearings and Fact	Load Sharing: No Deck: Not Checked	Load Sharing: No Deck: Not Checked Vibration: Not Checked	Load Sharing: No Deck: Not Checked Vibration: Not Checked

Analysis Results

L								
ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
l	Moment	9326 ft-lb	6'5 3/8"	22724 ft-lb	0.410 (41%)	1.25D+1.5L	L	
l	Unbraced	9326 ft-lb	6'5 3/8"	19820 ft-lb	0.471 (47%)	1.25D+1.5L	L	
١	Shear	2846 lb	11'8 3/4"	9277 lb	0.307 (31%)	1.25D+1.5L	L	
l	Perm Defl in.	0.116 (L/1250)	6'5 5/16"	0.404 (L/360)	0.290 (29%)	D	Uniform	
l	LL Defl inch	0.253 (L/574)	6'5 1/2"	0.404 (L/360)	0.630 (63%)	L	L	
	TL Defl inch	0.370 (L/393)	6'5 1/2"	0.606 (L/240)	0.610 (61%)	D+L	L	

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



/ Lateral Si	enderness ratio based o	n full section wiath.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-3-1 to 9-3-1		Тор	107 PLF	244 PLF	0 PLF	0 PLF	
2	Point	9-9-1		Тор	92 lb	244 lb	0 lb	0 lb	J8
3	Point	10-9 -1		Тор	107 lb	285 lb	0 lb	dl 0	J8
4	Point	12-1-1		Тор	122 lb	326 lb	0 lb	0 lb	J8
	Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation
- andling & Installation.

 LVL beams must not be cut or drilled.
 Refer to manufacturer's product information regarding installation requirements, multip-fly fastening details, beam strength values, and code approvals.
 Damaged Beams must not be used.
 Design assumes top edge is laterally restrained.
 Provide lateral support at bearing points to avoid lateral displacement and rotation.

- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318









Client: **GREEN YORK HOMES**

Project: Address:

5/31/2018

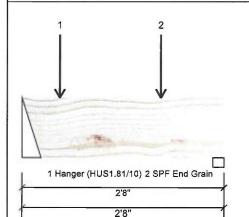
Designer: RCO

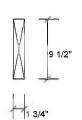
Job Name: LIANA 3 (ELEV.1)

Level: Second Floor

Date:

F10-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED





Wind

0

0

1.25D+1.5L

0

0

280 L

Member	Information
Туре:	Girder

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No Deck: Not Checked

Not Checked

1

2

Hanger

End Grain

2 - SPF 1.750"

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Dead Snow Live

174

139

Bearing	s and Fa	ctored Re	actions				•
Bearing	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.	
1-	3.000"	9%	88 / 261	349	L	1.25D+1.5L	

71 / 209

70

57

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	208 ft-lb	1'10"	11362 ft-lb	0.018 (2%)	1.25D+1.5L	L
Unbraced	208 ft-lb	1'10"	10456 ft-lb	0.020 (2%)	1.25D+1.5L	L
Shear	344 lb	11 3/4"	4638 lb	0.074 (7%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/68055)	1'9 1/8"	0.080 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/27103)	1'9 7/8"	0.080 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/19389)	1'9 11/16"	0.120 (L/240)	0.010 (1%)	D+L	L

Vibration:

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

12%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-6-0		Far Face	51 lb	136 lb	0 lb	0 lb	J2
2	Point	1-10-0		Far Face	66 lb	177 lb	0 lb	0 lb	J2
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at beaming points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

Date: 5/31/2018 RCO

Job Name: LIANA 3 (ELEV.1)

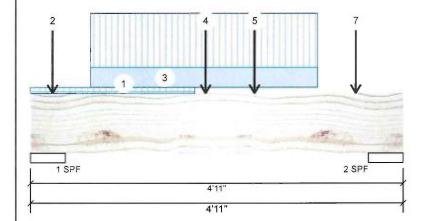
Project #:

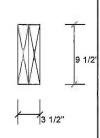
Designer:

F11-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor





Member Information

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:**

Load Sharing: Deck: Not Checked Not Checked Vibration:

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1661	666	0	0
2	1662	664	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	5.500"	28%	833 / 2492	3325	L	1.25D+1.5L	
2-SPF	5.500"	28%	830 / 2493	3324	L	1.25D+1.5L	

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
l	Moment	4701 ft-lb	2'3 3/4"	22724 ft-lb	0.207 (21%)	1.25D+1.5L	L	
l	Unbraced	4701 ft-lb	2'3 3/4"	22724 ft-lb	0.207 (21%)	1.25D+1.5L	L	
l	Shear	3520 lb	3'8 3/4"	9277 lb	0.379 (38%)	1.25D+1.5L	L	
l	Perm Defl in.	0.008 (L/5989)	2'3 3/4"	0.138 (L/360)	0.060 (6%)	D	Uniform	
l	LL Defl inch	0.021 (L/2406)	2'3 3/4"	0.138 (L/360)	0.150 (15%)	L	L	
l	TL Defl inch	0.029 (L/1716)	2'3 3/4"	0.206 (L/240)	0.140 (14%)	D+L	L	
Н								-

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-2-0	(Span)1-3-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-3-8		Far Face	79 lb	211 lb	0 lb	0 lb	J6
3	Part. Uniform	0-9-8 to 3-9-8		Far Face	119 PLF	316 PLF	0 PLF	0 PLF	
4	Point	2-3-12		Near Face	616 lb	1523 lb	0 lb	0 lb	F3
5	Point	2-11-8		Near Face	51 lb	136 lb	0 lb	0 lb	J2
6	Point	4-3-8		Far Face	108 lb	287 lb	0 lb	0 lb	J6
	1 2 3 4 5	1 Tie-In 2 Point 3 Part. Uniform 4 Point 5 Point	1 Tie-In 0-0-0 to 2-2-0 2 Point 0-3-8 3 Part. Uniform 0-9-8 to 3-9-8 4 Point 2-3-12 5 Point 2-11-8	1 Tie-In 0-0-0 to 2-2-0 (Span)1-3-9 2 Point 0-3-8 3 Part. Uniform 0-9-8 to 3-9-8 4 Point 2-3-12 5 Point 2-11-8	1 Tie-In 0-0-0 to 2-2-0 (Span)1-3-9 Top 2 Point 0-3-8 Far Face 3 Part. Uniform 0-9-8 to 3-9-8 Far Face 4 Point 2-3-12 Near Face 5 Point 2-11-8 Near Face	1 Tie-In 0-0-0 to 2-2-0 (Span)1-3-9 Top 15 PSF 2 Point 0-3-8 Far Face 79 lb 3 Part. Uniform 0-9-8 to 3-9-8 Far Face 119 PLF 4 Point 2-3-12 Near Face 616 lb 5 Point 2-11-8 Near Face 51 lb	1 Tie-In 0-0-0 to 2-2-0 (Span)1-3-9 Top 15 PSF 40 PSF 2 Point 0-3-8 Far Face 79 lb 211 lb 3 Part. Uniform 0-9-8 to 3-9-8 Far Face 119 PLF 316 PLF 4 Point 2-3-12 Near Face 616 lb 1523 lb 5 Point 2-11-8 Near Face 51 lb 136 lb	1 Tie-In 0-0-0 to 2-2-0 (Span)1-3-9 Top 15 PSF 40 PSF 0 PSF 2 Point 0-3-8 Far Face 79 lb 211 lb 0 lb 3 Part. Uniform 0-9-8 to 3-9-8 Far Face 119 PLF 316 PLF 0 PLF 4 Point 2-3-12 Near Face 616 lb 1523 lb 0 lb 5 Point 2-11-8 Near Face 51 lb 136 lb 0 lb	1 Tie-In 0-0-0 to 2-2-0 (Span)1-3-9 Top 15 PSF 40 PSF 0 PSF 0 PSF 2 Point 0-3-8 Far Face 79 lb 211 lb 0 lb 0 lb 3 Part. Uniform 0-9-8 to 3-9-8 Far Face 119 PLF 316 PLF 0 PLF 0 PLF 4 Point 2-3-12 Near Face 616 lb 1523 lb 0 lb 0 lb 5 Point 2-11-8 Near Face 51 lb 136 lb 0 lb 0 lb

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top adge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer info

Forex APA: PR-L318





Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

Project: Address:

GREEN YORK HOMES

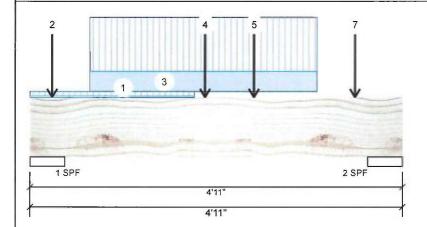
5/31/2018 Date: Designer: RCO

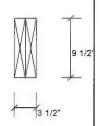
Job Name: LIANA 3 (ELEV.1)

F11-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor





..Continued from page 1

Load Type 7 Point

Self Weight

4-3-8

Location Trib Width

Side Near Face

Dead 61 lb

Live 162 lb

Snow

0 lb

Wind Comments 0 lb J2

8 PLF

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design onteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top adge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address:

GREEN YORK HOMES

Date: 5/31/2018

Designer: RCO

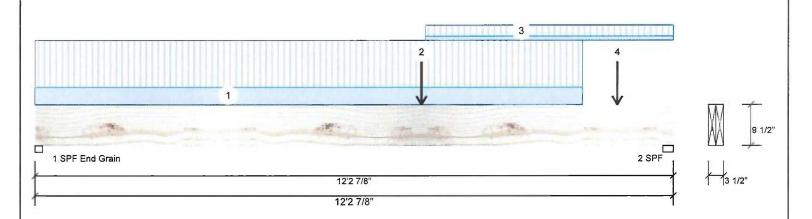
Job Name: LIANA 3 (ELEV.1)

Project #:

F12-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



2

Туре:	Girder	Application:	Floor (Residential)	
Plies:	2	Design Method:	LSD	
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	l
Deflection LL:	360	Load Sharing:	No	
Deflection TL:	240	Deck:	Not Checked	
Importance:	Normal	Vibration:	Not Checked	
General Load				ŀ
Floor Live:	40 PSF			
Dead:	15 PSF			ſ

Unfactored Reactions UNPATTERNED lb (Uplift)						
Brg	Live	Dead	Snow	Wind		
1	730	322	0	0		

386

L											
	Bearing:	s and Fac	tored	Reactions							
ſ	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.				
1	1 - SPF End Grain	1.750"	33%	403 / 1095	1497	L	1.25D+1.5L				
1	2 - SPF	2.375"	36%	482 / 1340	1822	L	1.25D+1.5L				

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6660 ft-lb	7'4 15/16"	22724 ft-lb	0.293 (29%)	1.25D+1.5L	L
Unbraced	6660 ft-lb	7'4 15/16"	19870 ft-lb	0.335 (34%)	1.25D+1.5L	L
Shear	1898 lb	11'3 3/4"	9277 lb	0.205 (20%)	1.25D+1.5L	L
Perm Defl in.	0.070 (L/2071)	6'4 7/16"	0.401 (L/360)	0.170 (17%)	D	Uniform
LL Defl inch	0.163 (L/883)	6'4 3/4"	0.401 (L/360)	0.410 (41%)	L	L
TL Defl inch	0.233 (L/619)	6'4 5/8"	0.601 (L/240)	0.390 (39%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

894



0

0

0

_	o Lateral Sieriae	mood radio bacca o								
Г	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Part. Uniform	0-0-0 to 10-6-0		Far Face	28 PLF	75 PLF	0 PLF	0 PLF	
ı	2	Point	7-4-15		Near Face	255 lb	661 lb	0 lb	0 lb	F1
1	3	Tie-In	7-5-13 to 12-2-14	(Span)0-10-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	4	Point	11-2-0		Far Face	34 lb	91 lb	0 lb	0 lb	J9
ı		Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

chemicals

- andling & installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Address:

Project:

GREEN YORK HOMES

5/31/2018 Date:

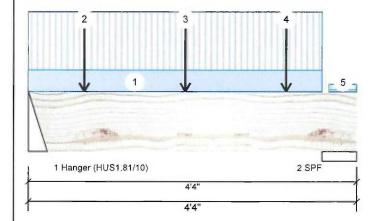
RCO Designer:

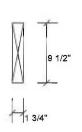
Job Name: LIANA 3 (ELEV.1)

Project #:

F1-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED

Level: Second Floor





Wind

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Member Informati	lon
------------------	-----

I	Type:	Girder	Application:	Floor (Residential)
l	Plies:	1	Design Method:	LSD
l	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
l	Deflection LL:	360	Load Sharing:	No
l	Deflection TL:	240	Deck:	Not Checked
l	Importance:	Normal	Vibration:	Not Checked
l	General Load			
l	Floor Live:	40 PSF		
١	Dead:	15 PSF		
ı				

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

Cap. React D/L lb

34%

20%

255

Live

661

2	604	235	0	0
<u></u>				
Bearing	s and Factored	Reactions		

319 / 992

294 / 907

Snow

0

Total Ld. Case

1311 L

1200 L

Analysis Results

Design Notes

1 Fill all hanger nailing holes.

4 Bottom braced at bearings.

3 Top braced at bearings.

2 Girders are designed to be supported on the bottom edge only.

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1179 ft-lb	2' 13/16"	11362 ft-lb	0.104 (10%)	1.25D+1.5L	L
Unbraced	1179 ft-lb	2' 13/16"	9138 ft-lb	0.129 (13%)	1.25D+1.5L	L
Shear	844 lb	11 3/4"	4638 lb	0.182 (18%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/11669)	2' 7/8"	0.125 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.010 (L/4503)	2' 7/8"	0.125 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.014 (L/3249)	2' 7/8"	0.188 (L/240)	0.070 (7%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL**

Bearing Length

2 - SPF 5.500"

Hanger

3.000"

POINT LOADS OVER BEARINGS

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-10-8		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
2	Point	0-8-13		Far Face	39 lb	105 lb	0 lb	0 lb	J7
3	Point	2-0-13		Far Face	47 lb	126 lb	0 lb	0 lb	J7
4	Point	3-4-13		Far Face	36 lb	96 lb	0 lb	0 lb	J7
5	Tie-In	3-11-10 to 4-4-0	(Span)1-1-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				4 PLF				



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address:

GREEN YORK HOMES

5/31/2018 Date: **RCO** Designer:

Job Name: LIANA 3 (ELEV.1)

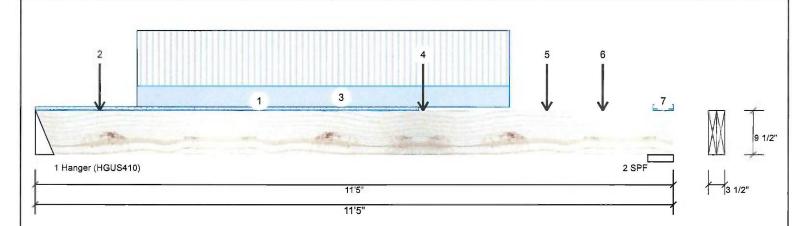
Project #:

Forex 2.0E-3000Fb LVL F3-A

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Bra

Member Information

Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

3					
1	1523	616	0	0	
2	1514	622	0	0	
W 6000					

Snow

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8863 ft-lb	5'10"	22724 ft-lb	0.390 (39%)	1.25D+1.5L	L
Unbraced	8863 ft-lb	5'10"	20442 ft-lb	0.434 (43%)	1.25D+1.5L	L
Shear	3021 lb	10'2 3/4"	9277 lb	0.326 (33%)	1.25D+1.5L	L
Perm Defl in.	0.080 (L/1615)	5'8 3/16"	0.358 (L/360)	0.220 (22%)	D	Uniform
LL Defl inch	0.198 (L/650)	5'8 1/8"	0.358 (L/360)	0.550 (55%)	L	L
TL Defl inch	0.278 (L/464)	5'8 1/8"	0.538 (L/240)	0.520 (52%)	D+L	L

Bearings and Factored Reactions Can React D/Llh

Live

	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
ľ	1-	4.000"	29%	770 / 2284	3055	L	1.25D+1.5L	
1	Hanger 2 - SPF	5.500"	26%	777 / 2271	3048	L	1.25D+1.5L	

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Wind

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 6-10-6	(Span)0-7-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-1-13		Near Face	129 lb	344 lb	0 lb	0 lb	J5
3	Part. Uniform	1-9-13 to 8-5-13		Near Face	103 PLF	275 PLF	0 PLF	0 PLF	
4	Point	6-11-4		Far Face	70 lb	174 lb	0 lb	0 lb	F10
5	Point	9-1-13		Near Face	120 lb	321 lb	0 lb	0 lb	J5
6	Point	10-1-13		Near Face	109 lb	268 lb	0 lb	0 lb	J5

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVI beams must not be cut or drilled

 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
- approvers
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318







Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

Project: Address: **GREEN YORK HOMES**

5/31/2018

Date: RCO Designer:

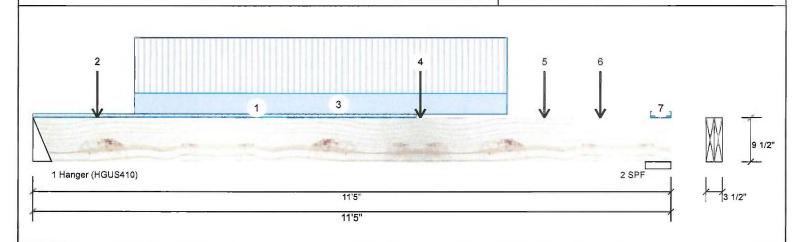
Job Name: LIANA 3 (ELEV.1)

Project #:

F3-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



.Continued from page 1

ID 7

Load Type Tie-In

Self Weight

Location Trib Width 11-0-10 to 11-5-0 (Span)1-1-3

Side Top

Dead 15 PSF 8 PLF

Live 40 PSF

Snow 0 PSF Wind Comments

0 PSF

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. LVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project: Address: **GREEN YORK HOMES**

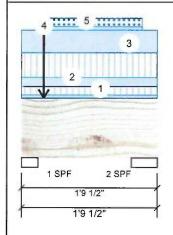
Date: Designer:

5/31/2018 RCO

Job Name: LIANA 3 (ELEV.2)

Project #:

1.750" X 9.500" - PASSED Level: Second Floor Forex 2.0E-3000Fb LVL F4-A



Member Information					Unfactored Reactions UNPATTERNED Ib (Uplift)					
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind	
Plies:	1	Design Method:	LSD	1	78		255	323	0	
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	90		124	53	0	
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total Ld. Case	Ld. Comb.	
				1 - SPF	2.625"	33%	319 / 524	843 L	1.25D+1.5S +0.5L	
analysis Result	s			2 - SPF	4.125"	8%	156 / 135	290 L	1.25D+1.5L	
Analysis Act	ual Loca	ation Allowed Capaci	ty Comb. Case							

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	114 ft-lb	6"	11362 ft-lb	0.010 (1%)	1.25D+1.5S +0.5L	L
Unbraced	114 ft-lb	6"	11362 ft-lb	0.010 (1%)	1.25D+1.5S +0.5L	L
Shear	54 lb	8 5/8"	4638 lb	0.012 (1%)	1.25D+1.5S	L
Perm Defl in.	0.000 (L/49777)	8 7/16"	0.045 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.000 (L/43590)	6 3/16"	0.045 (L/360)	0.010 (1%)	S+0.5L	L
TL Defl inch	0.001 (L/23529)	7 1/2"	0.068 (L/240)	0.010 (1%)	D+S+0.5L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

1 Dottom b	racea at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-8	(Span)1-2-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-8	(Span)3-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-0-0 to 1-9-8		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Point	0-3-8		Тор	182 lb	0 lb	345 lb	0 lb	F14 F14
5	Part. Uniform	0-3-10 to 1-7-2		Тор	10 PLF	0 PLF	24 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled.
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.
 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318







EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

GREEN YORK HOMES

Project: Address:

Date: 5/31/2018

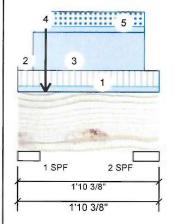
Designer: RCO

Job Name: LIANA 3 (ELEV.2)

Project #:

F4-B Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED

Level: Second Floor



Ld. Comb.

1.25D+1.5S +0.5L 1.25D+1.5L

Membei	Information
-	0: 1

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method: LSD Building Code: NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked

Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Live Brg Dead Snow Wind 24 231 323 0 2 25 87 53 0

Cap. React D/L lb

289 / 497

109 / 38

Bearings and Factored Reactions Bearing Length

А	nalysis	Actual	Location	Allowed	Capacity	Comb.	Case
N	floment flowers	104 ft-lb	5 1/4"	11362 ft-lb	0.009 (1%)	1.25D+1.5S +0.5L	L
L	Inbraced	104 ft-lb	5 1/4"	11362 ft-lb	0.009 (1%)	1.25D+1.5S +0.5L	L
S	hear	57 lb	9 1/2"	4638 lb	0.012 (1%)	1.25D+1.5S	L
Р	erm Defl in.	0.000 (L/57884)	9"	0.045 (L/360)	0.010 (1%)	D	Uniform
L	L Defl inch	0.000 (L/49057)	5 1/16"	0.045 (L/360)	0.010 (1%)	S+0.5L	L
Т	L Defl inch	0.001 (L/27348)	7 3/8"	0.068 (L/240)	0.010 (1%)	D+S+0.5L	L

Vibration:

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

25%

5%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

1 - SPF 3.500"

2 - SPF 4.125"



Total Ld. Case

786 L

147 L

Design Notes

Analysis Results

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 Bottom	braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-10-6	(Span)1-3-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 0-2-6		Тор	21 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Part. Uniform	0-2-6 to 1-8-0		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Point	0-4-6		Тор	182 lb	0 lb	345 lb	0 lb	F14 F14
5	Part. Uniform	0-4-8 to 1-8-0		Тор	10 PLF	0 PLF	24 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the custamer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer info

APA: PR-L318



