Ground Floor d la lilia F15-D - 2 ply (2x 8 Framing) F11-D - 1 ply J1-G F14-B - 2 ply J3-L / /57-P/ J3-M //J7-Q/ J7-U .17-V 12-0/ ₩ J7-W J7-Z /J3-Q J7-Y J3-S J7-AA J7-AB F8-A - 1 ply

THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Indigo 1

Legend



Load from Above Wall Opening Norbord Rimboard Plus 1.125 X 11.875 NJ60U 11.875 NJH 11 875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

Engineered floor joists shall be installed in accordance with the supplier's layout and specifications forming part of the permit drawings.

F12 Forex 1.75 11.875 2 2.0E-3000Fb LVL Forex 1.75 11.875 2 4-0-0 2.0E-3000Fb LVL Joist (Flush) Label Description Width Depth Qty Plies Pcs Length J7 NJ60U 19 20-0-0 3.5 11.875 J6 NJ60U 3.5 11.875 13 18-0-0 F15 NJH 2.5 11.875 18-0-0 8 F8 NJH 2.5 11.875 12-0-0 F7 NJH 2.5 11.875 10-0-0 F6 NJH 2.5 11.875 2 4-0-0 F5 NJH 2.5 11.875 2-0-0 4 J8 NJH 2.5 11.875 10 16-0-0 J3 NJH 13 2.5 11.875 12-0-0 J1 NJH 2.5 11.875 4 4-0-0 Rim Board Label Description Width Qty Plies Pcs | Length Depth 1.125 11.875 Norbord Rimboard 12 Plus 1.125 X

Width Depth

11.875

1.75

Qty Plies

2

2

Label	Description	Width	Depth	Qty	Plies	Pcs	Length
BLK1	NJH	2.5	11.875	LinFt		Varies	21-0-0

					Beam/Girder	Supported Member	l
Label	Pcs	Description	Skew	Slope	fasteners	fasteners	1
H1	1	HUS1.81/10			30 16d	10 16d	1
H2	3	LS90					1
НЗ	20	LT251188			4 10dx1 1/2	2 10dx1 1/2	1

Ground Floor LVL/LSL (Flush)

F14 Forex

Label Description

2.0E-3000Fb LVL

- Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- 4. Install single-ply flush window header along inside face of rimboard/rimjoist
- . Refer to Nascor specifier guide for installation works.
- Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof
- Load transfer blocks to be installed under all point loads
- B. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or boltina requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and oundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior

All work shall conform to the Ontario

Bullding Code O. Reg. 332/12 as amended

Pcs Length 12-0-0 Layout Name 6-0-0 INDIGO 1 (ELEV.2) Design Method Description GRANELLI HOME CORP. BRAMPTON, ONT. Created May 30, 2018 Builder **GREEN YORK HOMES** Sales Rep Designer Shipping Project **Builder's Project Kott Lumber Company** 14 Anderson Blvd Stouffville, Ontario Canada L4A 7X4 905-642-4400 Job Path D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES **IGRANELLI HOME CORPIMODELS** VINDIGO 1\INDIGO 1 ELEV.2\FLOOF **NNDIGO 1 (ELEV.2).isl Ground Floor** Design Method LSD Building Code NBCC 2010 / OBC 2012 Loads Live Dead 15 **Deflection Joist** 480

LL Span L/ TL Span L/ LL Cant 2L/ TL Cant 2L/

360

480

360

360

240

480

360

OSB

3/4"

Deflection Girder LL Span L/ TL Span L/

LL Cant 2L/ TL Cant 2L/

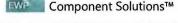
Decking Deck

Thickness Nailed & Glued Fastener

Vibration

BUILDING DIVISION

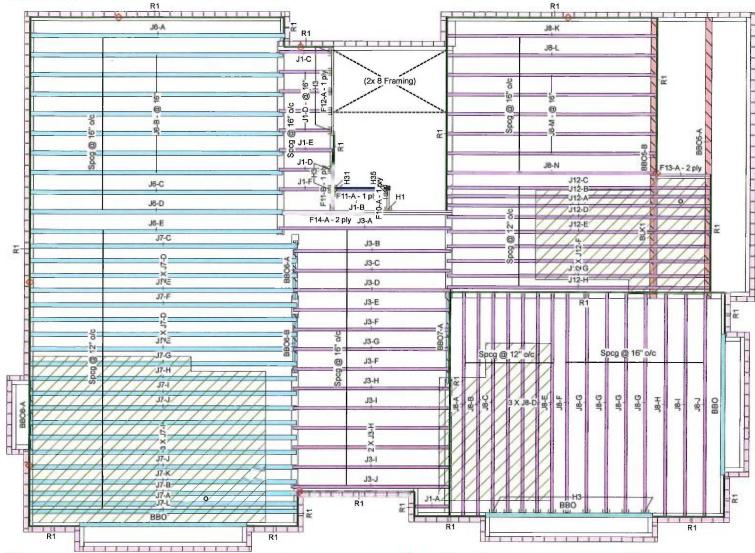




EWP Studio

Simpson Strong-Tie®

Second Floor



THIS CERTIFICATION IS TO CONFIRM THAT:

THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Indigo 1

Legend



Load from Above Norbord Rimboard Plus 1.125 X 11.875 NJ60U 11.875 NJH 11.875

Forex 2.0E-3000Fb LVL 1.75 X 11.875

- OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

J6 NJ60U J12 NJH J8 NJH J3 NJH J1 NJH Rim Board Label Description Width Depth Norbord Rimboard 1.125 11.875 Plus 1.125 X 11.875 Blocking Label Description BLK1 NJH Hanger Label Pcs Description 2 HUS1.81/10 H1 НЗ 19 LT251188 H5 HUCQ1.81/9-SDS

Second Floor LVL/LSL (Flush)

F13

F12

F11

F10

Joist (Flush)

J7 NJ60U

Label Description

Forex

Forex

Forex

Label Description

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

Width Depth

11.875

11.875

11.875

11.875

Width Depth

2.5

3.5 11.875

3.5 11.875

2.5 11.875

2.5 11.875

2.5 11 875

1.75

1.75

1.75

1.75

Qty

Qty

Qty Plies

Plies

2

2

Plies

2

2

11

23

15

Width Depth Qty Plies Pcs Length 2.5 11.875 LinFt Varies 6-0-0 Beam/Girder Supported Member Skew Slope fasteners fasteners 30 16d 10 16d 4 10dx1 1/2 2 10dx1 1/2

NOTES:

- Framer to verify dimensions on the architectural drawings.
- Double joist only require filler/backer ply when supporting another member using a face-mounted hanger
- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of
- rimboard/rimioist.
- . Refer to Nascor specifier guide for installation works. 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- 8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior

Pcs Length Layout Name 6-0-0 INDIGO 1 (ELEV.2) 6-0-0 Design Method LSD 4-0-0 Description GRANELLI HOME CORP. 2-0-0 BRAMPTON, ONT. Created Pcs Length May 30, 2018 20 20-0-0 Builder 18-0-0 **GREEN YORK HOMES** 18-0-0 Sales Rep 16-0-0 RM 14 12-0-0 10 4-0-0 Designer **RCO** Pcs Length Shipping 12 Project Builder's Project **Kott Lumber Company** 14 Anderson Blvd

> \GRANELLI HOME CORP\MODELS VINDIGO 1\INDIGO 1 ELEV.2\FLOOR VNDIGO 1 (ELEV.2).isl

Stouffville, Ontario

Canada

L4A 7X4

Job Path

905-642-4400

Second Floor LSD Design Method

D:\Users\rochavillo\WORK FROM

HOME\GREEN YORK HOMES

Building Code NBCC 2010 / OBC Floor Loads Live 40 Dead 15 Deflection Joist LL Span L/ 480 360 TL Span L/ LL Cant 2L/ 480 TL Cant 2L/ 360

Deflection Girder LL Span L/ TL Span L/ LL Cant 2L/ TL Cant 2L/ Decking

OSB Deck **Thickness** Fastener Nailed & Glued

360

240

480

360

Vibration Ceiling: Gypsum 1/2"

M-2057

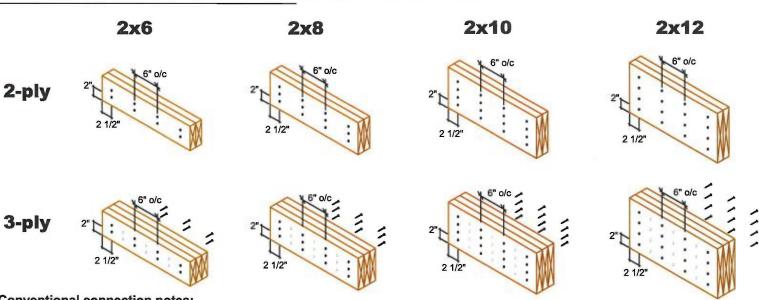




ULTIPLE MEMBER CONNECTIONS

GRANELLI HOME CORP-INDIGO 1 (ELEV.1&2

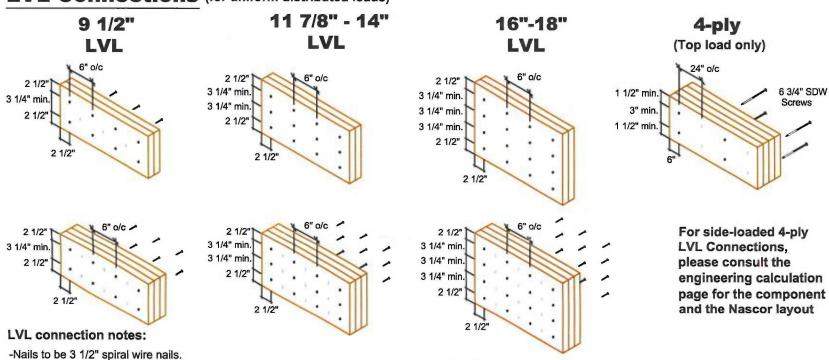
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

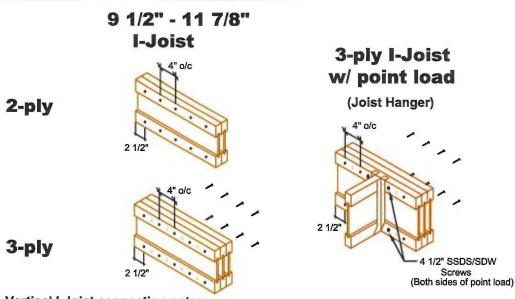
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI-PLY CONNECTION **DETAILS**

Scale: NTS

Date: November 30, 2016

KOTT 3228 Moodie Drive Ottawa, ON **K2H 7V1** Ph: 613-838-2775 Fx: 613-838-4751

Engineering Note Page (ENP-2) M-2057

REVISION 2009-10-09

LOT 21

Please read all notes prior to installation of the component

GREEN YORK HOMES-GRANELLI HOME CORP-INDIGO 1 (ELEV. 4&2

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is only limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

Date: 6/1/2018

Designer: **RCO**

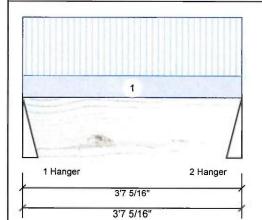
Job Name: INDIGO 1 (ELEV.1)

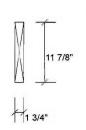
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	

40 PSF

15 PSF

Actual

83 ft-lb

83 ft-lb

Application: Design Method:

Building Code:

Load Sharing:

LSD

NBCC 2010 / OBC 2012

No

Deck: Vibration:

Capacity

Not Checked Not Checked

Floor (Residential)

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	53	28	0	0
2	53	28	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	3.000"	3%	35 / 79	114	L	1.25D+1.5L
2 - Hanger	3.000"	3%	35 / 79	114	L	1.25D+1.5L

Comb.

1.25D+1.5L L

0.005 (0%) 1.25D+1.5L L 0.006 (1%) 1.25D+1.5L L

Shear 40 lb 1'2 1/8" 5798 lb 0.007 (1%) Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%)

TL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%)

Location Allowed

1'9 11/16" 17130 ft-lb

1'9 11/16" 13233 ft-lb

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

ASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

Floor Live:

Analysis

Moment

Unbraced

Analysis Results

Dead:

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-7-5	(Span)1-5-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA; PR-L318







EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

Brg

1

2

6/1/2018

Page 1 of 2

Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

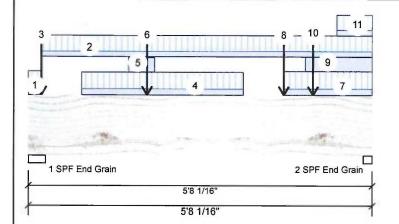
Project #

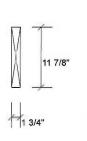
Date:

Forex 2.0E-3000Fb LVL F12-B

1.750" X 11.875" - PASSED

Level: Ground Floor





Wind

0

viember intorn	iation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Bearings	and	Factored	Reactions

Live

798

933

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

349

449

Snow

0

0

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.375"	37%	436 / 1197	1633	L	1.25D+1.5L
2 - SPF End Grain	1.750"	86%	562 / 1399	1961	L	1.25D+1.5L

Analysis Results Analysis Actual

Mambar Information

Location Allowed Capacity Comb. Moment 2299 ft-lb 2'4 7/16" 17130 ft-lb 0.134 (13%) 1.25D+1.5L L Unbraced 2299 ft-lb 2'4 7/16" 8377 ft-lb 0.274 (27%) 1.25D+1.5L L Shear 1466 lb 4'7 3/16" 5798 lb 0.253 (25%) 1.25D+1.5L L Perm Defl in. 0.008 (L/7964) 2'9 5/8" 0.179 (L/360) 0.050 (5%) D Uniform LL Defl inch 0.018 (L/3518) 2'9 5/8" 0.179 (L/360) 0.100 (10%) L L

TL Defl inch 0.026 (L/2440) 2'9 5/8" 0.268 (L/240) 0.100 (10%) D+L Design Notes

1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.

2 Girders are designed to be supported on the bottom edge only.

3 Top braced at bearings.

4 Bottom braced at bearings.

CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER

READ ALL NOTES ON THIS PAGE AND ON

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS

CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH

BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



4 Bottom Braces at Bearings.										
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 0-2-9	(Span)3-9-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-2-8 to 5-8-1	(Span)3-4-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	0-2-9		Near Face	23 lb	61 lb	0 lb	0 lb	J1	
4	Part. Uniform	0-10-9 to 3-6-9		Near Face	27 PLF	72 PLF	0 PLF	0 PLF		
5	Part. Uniform	1-7-13 to 2-0-15		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
6	Point	1-11-7		Тор	188 lb	468 lb	0 lb	0 lb	F11 F11	
7	Tie-In	4-2-9 to 5-8-1	(Span)3-9-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
Continued on	n page 2									

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

. LVL beams must not be cut or drilled
. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

fasterning vocation, approvals approvals approvals approvals Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

Address:

GREEN YORK HOMES

Project:

6/1/2018

Designer: RCO

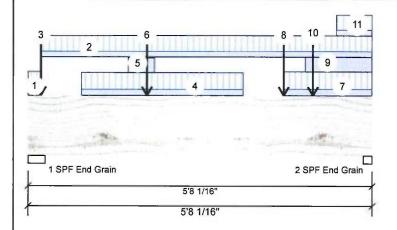
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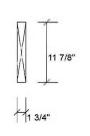
Project #:

Date:

F12-B Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

Level: Ground Floor





ı	Continued from	n page 1				·				
	ID	Load Type	Location 7	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	8	Point	4-2-9		Near Face	38 lb	101 lb	0 lb	0 lb	J1
	9	Part. Uniform	4-6-13 to 5-8-1		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
	10	Point	4-8-5		Тор	154 lb	381 lb	0 lb	0 lb	F11 F11
	11	Part. Uniform	5-1-1 to 5-8-1		Тор	25 PLF	65 PLF	0 PLF	0 PLF	J1
ı		Self Weight				5 PLF				

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design ortheria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

- payprovals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







Client:

GREEN YORK HOMES

Project: Address: Date:

6/1/2018

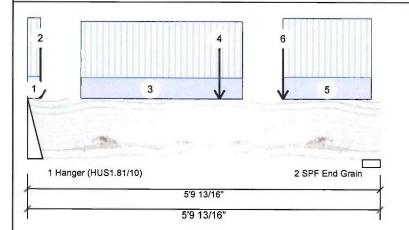
Designer: RCO

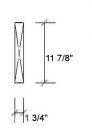
Job Name: INDIGO 1 (ELEV.1)

Project #:

1.750" X 11.875" - PASSED F12-C Forex 2.0E-3000Fb LVL

Level: Ground Floor





Wind

0

0

Member Inform	nation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		
		l l	

Bearing	s and Fac	tored F	Reactions			
Bearing			React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	3.000"	28%	276 / 805	1081	L	1.25D+1.5L
2 - SPF	3.500"	27%	314 / 920	1234	L	1.25D+1.5L

Snow

0

0

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

221

251

Live

537

613

1

2

End

Grain

Analysis Results Analysis Actual Location Allowed Capacity Comb. Case Moment 2367 ft-lb 3'1 15/16" 17130 ft-lb 0.138 (14%) 1.25D+1.5L L Unbraced 2367 ft-lb 3'1 15/16" 8328 ft-lb 0.284 (28%) 1.25D+1.5L L Shear 1110 lb 4'7 3/16" 5798 lb 0.191 (19%) 1.25D+1.5L L Perm Defl in. 0.007 (L/9287) 3'1 15/16" 0.180 (L/360) 0.040 (4%) D Uniform LL Defl inch 0.017 (L/3759) 3'1 15/16" 0.180 (L/360) 0.100 (10%) L L TL Defl inch 0.024 (L/2676) 3'1 15/16" 0.270 (L/240) 0.090 (9%) D+L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

OPROFESSIONAL OF	
N.A. EL-MASRI	
X hat than	1
Jun 05, 2018	

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ı	ID	Load Type	Location	Trib \A fieldh	C:4-	Dood	I OHT LOAD			0
ı	טו	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
ı	1	Tie-In	0-0-0 to 0-2-9	(Span)3-9-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	2	Point	0-2-9		Far Face	23 lb	61 lb	0 lb	0 lb	J1
ı	3	Part. Uniform	0-10-9 to 3-6-9		Far Face	27 PLF	72 PLF	0 PLF	0 PLF	
I	4	Point	3-1-15		Тор	264 lb	671 lb	0 lb	0 lb	C4
ı	5	Tie-In	4-2-9 to 5-8-1	(Span)3-9-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
I	6	Point	4-2-9		Far Face	38 lb	101 lb	0 lb	0 lb	J1
I		Self Weight				5 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid

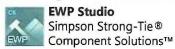
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







Client: Project: Address:

GREEN YORK HOMES

Date: 6/1/2018

RCO Designer:

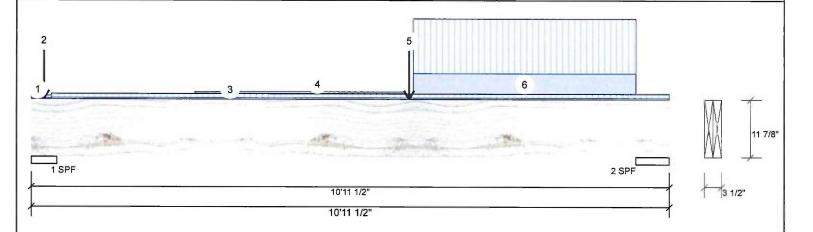
Job Name: INDIGO 1 (ELEV.1)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



Member Into	rmation			Unfactor	ed React	ions U	NPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	٧	Wind
Plies:	2	Design Method:	LSD	1	532		295	(0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1275		542	(0	0
Deflection LL:	360	Load Sharing:	No	1000						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load					_					
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF	5.250"	10%	369 / 798	1167	L	1.25D+1.5L
				2 - SPF	6.875"	17%	678 / 1913	2590	L	1.25D+1.5L
nalveie Rocu	ltc			_						

Analysis Results

Ī	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	5728 ft-lb	6'5 7/8"	34261 ft-lb	0.167 (17%)	1.25D+1.5L	L
l	Unbraced	5728 ft-lb	6'5 7/8"	29540 ft-lb	0.194 (19%)	1.25D+1.5L	L
l	Shear	2022 lb	9'5 1/2"	11596 lb	0.174 (17%)	1.25D+1.5L	L
ı	Perm Defl in.	0.023 (L/5246)	5'11 1/2"	0.336 (L/360)	0.070 (7%)	D	Uniform
ı	LL Defl inch	0.053 (L/2301)	6' 3/8"	0.336 (L/360)	0.160 (16%)	L	L
	TL Defl inch	0.076 (L/1600)	6' 1/16"	0.504 (L/240)	0.150 (15%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

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PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



/ Lateral	sienderness ratio based	on full section wiath.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-2	(Span)0-7-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-2-10		Тор	37 lb	0 lb	0 lb	0 lb	Wall Self Weight
3	Tie-In	0-4-2 to 10-11-8	(Span)0-11-11 to 0-11-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Tie-In	2-9-11 to 6-5-0	(Span)0-4-5 to 0-4-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Continued on page 2...

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Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

6/1/2018

Designer: RCO

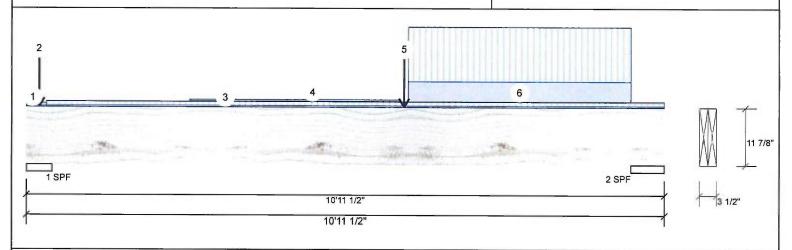
Job Name: INDIGO 1 (ELEV.1)

Project #:

Date:

F14-B Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



.. Continued from page 1

ID	Load Type	Location Trib	Width Side	Dead	Live	Snow	Wind	Comments
5	Point	6-5-14	Far Face	221 lb	537 lb	0 lb	0 lb	F12
6	Part. Uniform	6-6-12 to 10-4-10	Тор	101 PLF	270 PLF	0 PLF	0 PLF	
	Self Weight			10 PLF				

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Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be out or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid lateral displacement and rotation

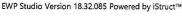
For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex







EWP Studio Simpson Strong-Tie® Component Solutions™

GREEN YORK HOMES

Project: Address: Date: 6/1/2018

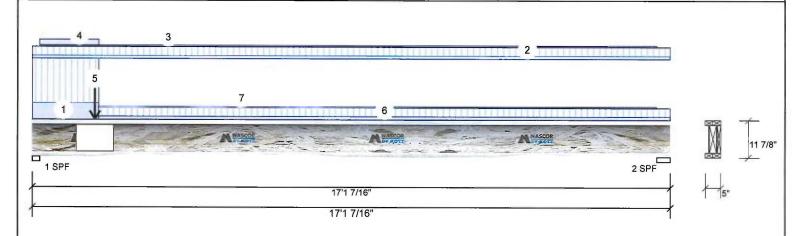
Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #:

2-Ply - PASSED F15-A 11.875" NJH

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Girden Type: Application: Floor (Residential) Live Dead Snow Wind Plies: 2 Design Method: 755 381 0 0 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 279 144 0 0 Deflection LL: 360 Load Sharing: Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load 40 PSF Bearings and Factored Reactions Floor Live: Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 49% 477 / 1133 1609 L 1.25D+1.5L 2 - SPF 4.319" 17% 180 / 419 599 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2960 ft-lb	6'8 1/8"	10780 ft-lb	0.275 (27%)	1.25D+1.5L	L
Unbraced	2960 ft-lb	6'8 1/8"	2970 ft-lb	0.997 (100%)	1.25D+1.5L	L
Shear	1588 lb	1 5/8"	3620 lb	0.439 (44%)	1.25D+1.5L	L
Perm Defl in.	0.065 (L/3060)	8' 1/16"	0.556 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.127 (L/1581)	7'11 15/16"	0.556 (L/360)	0.230 (23%)	L	L
TL Defl inch	0.192 (L/1042)	8'	0.834 (L/240)	0.230 (23%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'9" o.c.

5 Bottom flange braced at bearings.

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PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



3 BUILDIN I	liange braced at bearings								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 17-1-7	(Span)0-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-5 to 16-9-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-5 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	240 lb	480 lb	0 lb	0 lb	F6
6	Tie-In	1-9-6 to 17-1-7	(Span)0-8-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 16-9-1		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

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Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Andling & Installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the Lioist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handing/erection details
 Damaged Lioists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





Client:

GREEN YORK HOMES

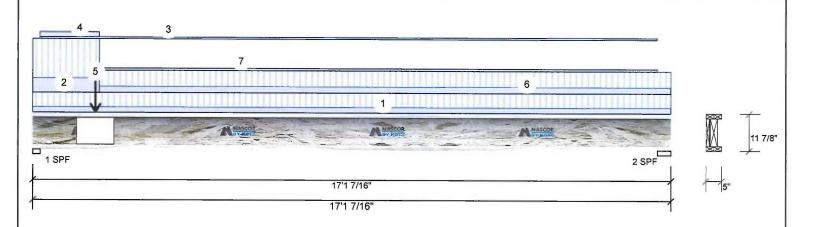
Project: Address: Date: 6/1/2018

Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #:

F15-B NJH 11.875" 2-Ply - PASSED Level: Ground Floor



Member Inform	nation			Unfacto	red Reac	tions L	JNPATTERN	ED lb (Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N
Plies:	2	Design Method:	LSD	1	832		417		0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	496		248		0
Deflection LL:	360	Load Sharing:	No	_					•
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions		
Dead:	15 PSF			Bearing		_	React D/L lb	Total	Ld. Case
				1 - SPF	2.375"	53%	521 / 1248	1769	L
				2 SDE	4 310"	200/	210 / 744	1054	

		_									
Bearings and Factored Reactions											
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.					
1 - SPF	2.375"	53%	521 / 1248	1769	L	1.25D+1.5L					
2 - SPF	4.319"	29%	310 / 744	1054	L	1.25D+1.5L					

Analysis Results Analysis Actual Location Allowed Capacity Comb. Case 4604 ft-lb Moment 7'9 15/16" 10780 ft-lb 0.427 (43%) 1.25D+1.5L L Unbraced 4604 ft-lb 7'9 15/16" 4611 ft-lb 0.998 1.25D+1.5L L (100%)1744 lb Shear 1 5/8" 3620 lb 0.482 (48%) 1.25D+1.5L L Perm Defl in. 0.100 (L/2004) 8'3 3/16" 0.556 (L/360) 0.180 (18%) D Uniform LL Defl inch 0.198 (L/1013) 8'3 3/16" 0.556 (L/360) 0.360 (36%) L L TL Defl inch 0.298 (L/673) 8'3 3/16" 0.834 (L/240) 0.360 (36%) D+L L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 6'2" o.c.

5 Bottom flange braced at bearings.

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Wind

0

0

2 DOMOTTI	ange braced at bearings).							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 17-1-7	(Span)1-3-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 16-9-1		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Near Face	172 lb	343 lb	0 lb	0 lb	F6
6	Tie-In	1-9-6 to 17-1-7	(Span)1-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 16-9-1		Тор	4 PLF	0 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

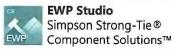
- andling & Installation
 | Joint flanges must not be cut or drilled
 | Refer to latest copy of the IJoist product information
 | details for framing details, sufficient tables, web hole
 | total the flange details, multi-ple fastening details and
 | handling/erection details
 | Damaged IJoists must not be used
 | Design assumes top flange to be laterally restrained
 | by attached sheathing or as specified in engineering
 | notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
- point load bearing length>= 3,5 inches For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





Client:

GREEN YORK HOMES

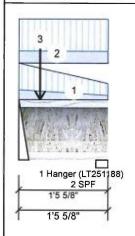
Project: Address: Date: 6/1/2018

Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

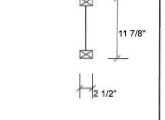
Project #:

F5-A NJH 11.875" - PASSED



. .

Level: Ground Floor



Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF	1	

earing	s and Fac	tored I	Reactions			
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld.
1 -	2.000"	13%	49 / 156	204	L	1.25

Dead

39

30

Unfactored Reactions UNPATTERNED Ib (Uplift)

Live

104

79

Hanger 2 - SPF 2.375"

1 2

В

13% 49 / 156 9% 37 / 119 204 L

156 L

Snow

0

0

1.25D+1.5L

1.25D+1.5L

Comb.

Wind

0

0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	47 ft-lb	7 3/4"	5390 ft-lb	0.009 (1%)	1.25D+1.5L	L
Unbraced	47 ft-lb	7 3/4"	5334 ft-lb	0.009 (1%)	1.25D+1.5L	L
Shear	180 lb	1 1/4"	1810 lb	0.100 (10%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/34775)	7 13/16"	0.041 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/25291)	7 13/16"	0.061 (L/240)	0.010 (1%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY

NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-10	(Span)2-8-1 to 1-2-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-4-3		Far Face	12 lb	32 lb	0 lb	0 lb	J1

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- natruing & installation

 1. Just flanges must not be cut or drilled

 2. Refer to latest copy of the Jusic product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/eroction details

 3. Damaged Jusists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths—3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address:

GREEN YORK HOMES

Date:

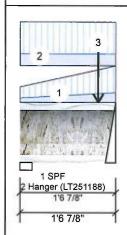
6/1/2018 RCO

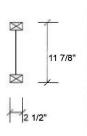
Designer: Job Name: INDIGO 1 (ELEV.1)

Project #:

11.875" - PASSED F5-B NJH

Level: Ground Floor





Wind

Member Inform	nation	
Туре:	Girder	
Plies:	1	1
Moisture Condition:	Dry	
Deflection LL:	360	
Deflection TL:	240	
Importance:	Normal	
General Load		
Floor Live:	40 PSF	
Dead:	15 PSF	

Application: Floor (Residential) Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: Deck:

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead

1	83	31	0	0
1 2	114	43	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	10%	39 / 124	163	L	1.25D+1.5L
2 - Hanger	2.000"	14%	53 / 172	225	L	1.25D+1.5L

Analysis Results

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	52 ft-lb	10 3/8"	5390 ft-lb	0.010 (1%)	1.25D+1.5L	L
ĺ	Unbraced	52 ft-lb	10 3/8"	5319 ft-lb	0.010 (1%)	1.25D+1.5L	L
	Shear	201 lb	1'5 5/8"	1810 lb	0.111 (11%)	1.25D+1.5L	L
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
	LL Defl inch	0.000 (L/33591)	10 5/16"	0.044 (L/360)	0.010 (1%)	L	L
	TL Defl inch	0.001 (L/24438)	10 5/16"	0.067 (L/240)	0.010 (1%)	D+L	L

Vibration:

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT** CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings

L	T Doctor hang	c braceu at bearings.								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 1-6-14	(Span)1-1-15 to 2-8-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Point	1-3-7		Far Face	13 lb	35 lb	0 lb	0 lb	.11

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

andling & Installation

Liost flanges must not be cut or drilled
Refer to latest copy of the Librist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

Damaged Liosts must not be used

Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: **GREEN YORK HOMES**

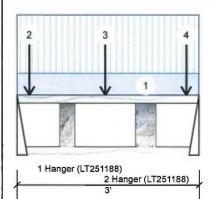
Date: 6/1/2018

Designer:

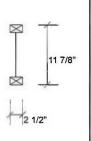
Job Name: INDIGO 1 (ELEV.1)

Project #:

F6-A NJH 11.875" - PASSED Level: Ground Floor



3'



Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
mportance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfacto	Live	Dood	Coon	•	
Brg	rive	Dead	Snow	Wind	
1	441	166	0	0	
2	439	165	0	0	

Analysis Results Analysis Actual Location Allowed Comb. Case Capacity 571 ft-lb 1'5 7/16" 5390 ft-lb Moment 0.106 (11%) 1.25D+1.5L L Unbraced 571 ft-lb 1'5 7/16" 4800 ft-lb 0.119 (12%) 1.25D+1.5L L 862 lb 1 1/4" 1810 lb Shear 0.476 (48%) 1.25D+1.5L L Perm Defl in. 0.002 1'5 7/16" 0.093 (L/360) 0.020 (2%) D Uniform (L/14698) LL Defl inch 0.006 (L/5525) 1'5 7/16" 0.093 (L/360) 0.070 (7%) L TL Defl inch 0.008 (L/4016) 1'5 7/16" 0.140 (L/240) 0.060 (6%) D+L L

Bearings and Factored Reactions

_							
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	54%	207 / 662	869	L	1.25D+1.5L	
2 - Hanger	2.000"	53%	206 / 659	865	L	1.25D+1.5L	

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- om flange braced at bearings

4 000	tom hange braced at bearings								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-2-7		Far Face	81 lb	215 lb	0 lb	0 lb	J4
3	Point	1-5-7		Far Face	126 lb	335 lb	0 lb	0 lb	J4
4	Point	2-9-7		Far Face	85 lb	227 lb	0 lb	0 lb	J4

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation

 Loist flanges must not be cut or drilled

 Refer to latest copy of the IJoist product information

 details for framing details, stiffener tables, web hole

 chart, bridging details, multiply fastening details and

 handling/erection details

 Damaged IJoists must not be used

 Design assumes top flange to be laterally restrained

 by attached sheathing or as specified in engineering

 notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches.
 For flat roofs provide proper drainage to prevent ponding.

Manufacturer Info

Nascor by Kott







Client:

GREEN YORK HOMES

Project: Address: Date: 6/1/2018

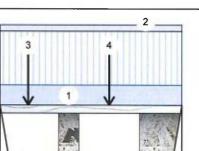
RCO Designer:

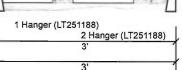
Job Name: INDIGO 1 (ELEV.1)

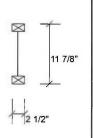
Level: Ground Floor

Project #:

F6-B NJH 11.875" - PASSED







Wind

1.25D+1.5L

PROFESSIONAL CHARLES

Member Information							
Туре:	Girder	Application:	Floor (Residential)				
Plies:	1	Design Method:	LSD				
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012				
Deflection LL:	360	Load Sharing:	No				
Deflection TL:	240	Deck:	Not Checked				
Importance:	Normal	Vibration:	Not Checked				
General Load							
Floor Live:	40 PSF						
Dead:	15 PSF						

/ OBC 2012	2	343		172		0	0	
d								
d								
	Bearings	s and Fac	tored	Reactions				
	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	_
	1 - Hanger	2.000"	63%	300 / 721	1021	L	1.25D+1.5L	

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

240

215 / 515

Snow

729 L

Live

480

1

Analysis Results Location Allowed Analysis Actual Capacity Comb. Case 749 ft-lb Moment 1'9 9/16" 5390 ft-lb 0.139 (14%) 1.25D+1.5L L 749 ft-lb 1'9 9/16" 4800 ft-lb 0.156 (16%) 1.25D+1.5L L Unbraced Shear 1013 lb 1 1/4" 1810 lb 0.560 (56%) 1.25D+1.5L L Perm Defl in. 0.004 (L/9092) 1'9 9/16" 0.093 (L/360) 0.040 (4%) D Uniform LL Defl inch 0.007 (L/4555) 1'9 9/16" 0.093 (L/360) 0.080 (8%) L L TL Defl inch 0.011 (L/3035) 1'9 9/16" 0.140 (L/240) 0.080 (8%) D+L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

45%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL**

2.000"

Hanger

4 Bottom flange braced at bearings.

2 Girders are designed to be supported on the bottom edge only.

1 Fill all hanger nailing holes.

3 Top flange unbraced.

Design Notes

$\overline{}$							HPOINT LOADS	IVER HEA	DINGS	
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
0	2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
	3	Point	0-5-9		Near Face	152 lb	304 lb	dl 0	0 lb	J8
	4	Point	1-9-9		Near Face	209 lb	416 lb	0 lb	0 lb	J8

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Iandling & Installation

 Joist flanges must not be cut or drilled

 Refer to latest copy of the Liest product information details for faming details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged Lioists must not be used besign assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Manufacturer Info

Nascor by Kott







Client:

Address:

GREEN YORK HOMES

Project:

6/1/2018 Date:

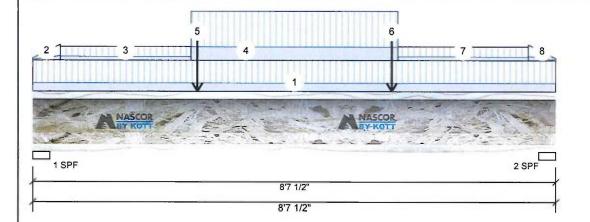
Designer: RCO

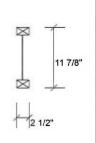
Job Name: INDIGO 1 (ELEV.1)

Project #:

11.875" - PASSED NJH F7-A

Level: Ground Floor





Mind

Member Information

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

15 PSF

Application: Design Method:

Building Code:

Floor (Residential)

NBCC 2010 / OBC 2012

Load Sharing: No

Not Checked Deck: Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dig	LIVE	Dead	SHOW	VVIIIG
1	272	102	0	0
2	272	102	0	0

Bearings and Factored Reactions

	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - SPF	3.375"	31%	127 / 408	535	L	1.25D+1.5L	
l	2-SPF	3.375"	31%	127 / 408	535	L	1.25D+1.5L	

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1259 ft-lb	4'3 3/4"	5390 ft-lb	0.234 (23%)	1.25D+1.5L	L
Unbraced	1259 ft-lb	4'3 3/4"	1315 ft-lb	0.958 (96%)	1.25D+1.5L	L
Shear	525 lb	8'4 7/8"	1810 lb	0.290 (29%)	1.25D+1.5L	L
Perm Defl in.	0.014 (L/7253)	4'3 13/16"	0.273 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.036 (L/2720)	4'3 13/16"	0.273 (L/360)	0.130 (13%)	L	L
TL Defl inch	0.050 (L/1978)	4'3 13/16"	0.409 (L/240)	0.120 (12%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange unbraced.
- 3 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 8-7-8	(Span)1-0-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-5-7	(Span)0-0-13 to 0-6-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	0-5-7 to 2-7-4	(Span)0-5-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Tie-In	2-7-4 to 6-0-4	(Span)1-7-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Point	2-8-8		Near Face	39 lb	104 lb	0 lb	0 lb	F5
6	Point	5-11-0		Near Face	39 lb	104 lb	0 lb	0 lb	F5
7	Tie-In	6-0-4 to 8-2-1	(Span)0-5-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
8	Tie-In	8-2-1 to 8-7-8	(Span)0-6-4 to 0-0-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

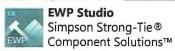
LIJGIST Ranges must not be cut or drilled Refer to latest copy of the LiJoist product information details for framing details, stiffener bables, web hole chart, bridging details, multi-ply fastering details and handling/erection details. Damaged LiJoist must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





Client:

GREEN YORK HOMES

Project: Address:

6/1/2018

Designer: RCO

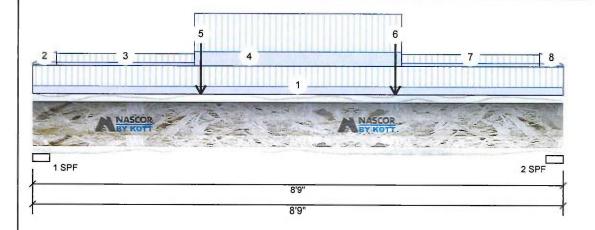
Job Name: INDIGO 1 (ELEV.1)

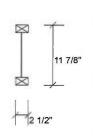
Project #:

Date:

11.875" - PASSED F7-B NJH

Level: Ground Floor





Member Inform	nation
Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	

40 PSF

15 PSF

Application: Design Method: **Building Code:** Load Sharing: Deck: Vibration:

Floor (Residential) NBCC 2010 / OBC 2012 No

Not Checked Not Checked

Unfactored Reactions UNPATTERNED ID (Uplift)	tored Reactions UNPATTERNED Ib (U)	plift)
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Brg	Live	Dead	Snow	Wind	
1	275	103	0	0	
2	275	103	0	0	

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.375"	31%	129 / 413	542	L	1.25D+1.5L
2 - SPF	3 375"	31%	129 / 413	542	1	1 250+1 51

Analysis Results

Floor Live:

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1323 ft-lb	4'4 1/2"	5390 ft-lb	0.246 (25%)	1.25D+1.5L	L
Unbraced	1323 ft-lb	4'4 1/2"	1326 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	533 lb	2 5/8"	1810 lb	0.294 (29%)	1.25D+1.5L	L
Perm Defl in.	0.015 (L/6852)	4'4 9/16"	0.277 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.039 (L/2576)	4'4 9/16"	0.277 (L/360)	0.140 (14%)	L	L
TL Defl inch	0.053 (L/1872)	4'4 9/16"	0.416 (1/240)	0.130 (13%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 8'2" o.c.

3 Bottom flange braced at bearings

3 BULLUIII	nange braced at bearing	S							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 8-9-0	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-4-11	(Span)0-0-13 to 0-5-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	0-4-11 to 2-8-0	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Tie-In	2-8-0 to 6-1-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Point	2-9-4		Near Face	43 lb	114 lb	0 lb	0 lb	F5
6	Point	5-11-12		Near Face	43 lb	114 lb	di 0	0 lb	F5
7	Tie-In	6-1-0 to 8-4-5	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
8	Tie-In	8-4-5 to 8-9-0	(Span)0-5-8 to 0-0-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Idoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Lioist flanges must not be cut or drilled Refer to latest copy of the iJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-py fastering details and handling/erection details Damaged Lioists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeness for point load as shown Minimum point load bearing lengths—3.5 Inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address: Date: 6/1/2018

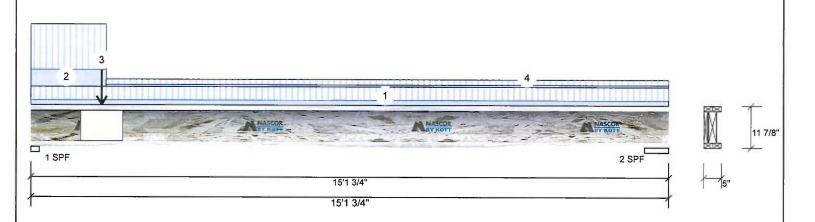
Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #:

F8-A 11.875" 2-Ply - PASSED NJH

Level: Ground Floor



Member Inform	nation			Unfactor	Unfactored Reactions UNPATTERNED Ib (Uplift)					
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	2	Design Method:	LSD	1	705		265		0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	272		102		0	0
Deflection LL:	360	Load Sharing:	No	-	-		1177			
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	2.375"	42%	331 / 1057	1388	L	1.25D+1.5L
nalysis Deculé	<u> </u>			2-SPF	6.875"	15%	128 / 409	536	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2312 ft-lb	5'6 3/8"	10780 ft-lb	0.214 (21%)	1.25D+1.5L	L
Unbraced	2312 ft-lb	5'6 3/8"	2331 ft-lb	0.992 (99%)	1.25D+1.5L	L
Shear	1365 lb	1 5/8"	3620 lb	0.377 (38%)	1.25D+1.5L	L
Perm Defl in.	0.031 (L/5557)	6'10 3/4"	0.483 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.083 (L/2086)	6'10 3/4"	0.483 (L/360)	0.170 (17%)	L	L
TL Defl inch	0.115 (L/1517)	6'10 3/4"	0.725 (L/240)	0.160 (16%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'8" o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 15-1-12	(Span)1-0-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	1-8-2		Near Face	166 lb	441 lb	0 lb	0 lb	F6	
4	Tie-In	1-9-6 to 15-1-12	(Span)0-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF		

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- andling & installation

 Loist flanges must not be cut or drilled
 Refer to latest copy of the Lloist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged Lloists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

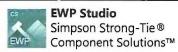
Manufacturer Info

Nascor by Kott









Client:

Address:

GREEN YORK HOMES

Project:

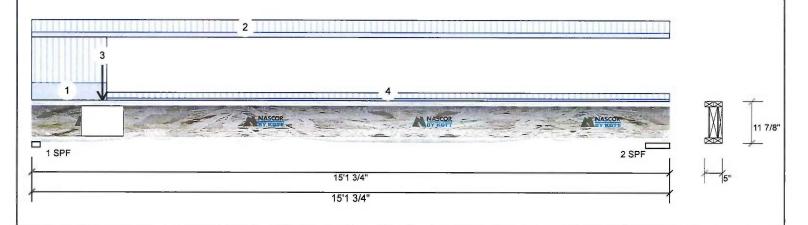
Date: 6/1/2018

Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #:

11.875" F8-B NJH 2-Ply - PASSED Level: Ground Floor



Member Info	rmation			Unfacto	red Reac	tions U	INPATTERN	ED lb ((Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	W	Wind
Plies:	2	Design Method:	LSD	1	690		259		0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	259		97		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF	2.375"	41%	324 / 1036	1360	L	1.25D+1.5L
				2-SPF	6.875"	14%	122 / 389	510	L	1.25D+1.5L

Analysis Results

ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	2227 ft-lb	5'5 1/16"	10780 ft-lb	0.207 (21%)	1.25D+1.5L	L
	Unbraced	2227 ft-lb	5'5 1/16"	2234 ft-lb	0.997 (100%)	1.25D+1.5L	L
l	Shear	1336 lb	1 5/8"	3620 lb	0.369 (37%)	1.25D+1.5L	L
l	Perm Defl in.	0.030 (L/5780)	6'10 9/16"	0.483 (L/360)	0.060 (6%)	D	Uniform
ı	LL Defl inch	0.080 (L/2169)	6'10 9/16"	0.483 (L/360)	0.170 (17%)	L	L
l	TL Defl inch	0.110 (L/1577)	6'10 9/16"	0.725 (L/240)	0.150 (15%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'9" o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
١	1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 15-1-12	(Span) 0-10-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Point	1-8-2		Far Face	165 lb	439 lb	0 lb	0 lb	F6
l	4	Tie-In	1-9-6 to 15-1-12	(Span)0-5-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orderia and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation

 Loist flanges must not be out or drilled

 Refer to latest copy of the Lloist product information details for framing details, sufficient rables, web hole chart, bridging details, multi-ply fastening details and handing/erection details

 Damaged Lloists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3,5 inches

 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project:

Address:

GREEN YORK HOMES

6/1/2018

Designer: RCO

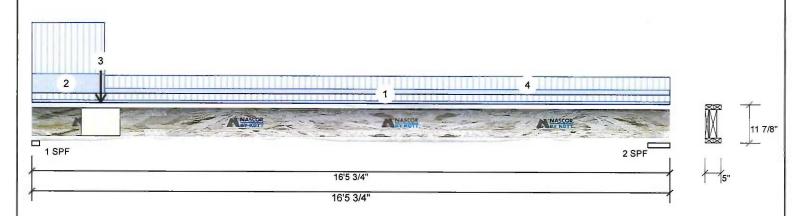
Job Name: INDIGO 1 (ELEV.2)

Project #:

Date:

2-Ply - PASSED F15-A 11.875" NJH

Level: Ground Floor



Member Inform	nation			Unfacto	red React	tions U	NPATTERN	ED lb ((Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	W	Wind
Plies:	2	Design Method:	LSD	1	684		256		0	0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	273		102		0	0
Deflection LL:	360	Load Sharing:	No	1 -					-	0.70
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored R	eactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	2.375"	41%	320 / 1026	1346	L	1.25D+1.5L
				2 - SPF	6.875"	15%	128 / 410	538	L	1.25D+1.5L

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	2487 ft-lb	6'2 13/16"	10780 ft-lb	0.231 (23%)	1.25D+1.5L	L
	Unbraced	2487 ft-lb	6'2 13/16"	2497 ft-lb	0.996 (100%)	1.25D+1.5L	L
l	Shear	1325 lb	1 5/8"	3620 lb	0.366 (37%)	1.25D+1.5L	L
l	Perm Defl in.	0.039 (L/4815)	7'6 7/8"	0.528 (L/360)	0.070 (7%)	D	Uniform
l	LL Defl inch	0.105 (L/1805)	7'6 7/8"	0.528 (L/360)	0.200 (20%)	L	L
L	TL Defl inch	0.145 (L/1313)	7'6 7/8"	0.792 (L/240)	0.180 (18%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'4" o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS**



Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
Tie-In	0-0-0 to 16-5-12	(Span)0-5-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Tie-In	0-0-0 to 1-10-8	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Point	1-9-4		Near Face	158 lb	422 lb	0 lb	0 lb	F6
Tie-In	1-10-8 to 16-5-12	(Span)0-10-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Tie-In Tie-In Point	Tie-In 0-0-0 to 16-5-12 Tie-In 0-0-0 to 1-10-8 Point 1-9-4	Tie-In 0-0-0 to 16-5-12 (Span)0-5-9 Tie-In 0-0-0 to 1-10-8 (Span)3-5-0 Point 1-9-4	Tie-In 0-0-0 to 16-5-12 (Span)0-5-9 Top Tie-In 0-0-0 to 1-10-8 (Span)3-5-0 Top Point 1-9-4 Near Face	Tie-In 0-0-0 to 16-5-12 (Span)0-5-9 Top 15 PSF Tie-In 0-0-0 to 1-10-8 (Span)3-5-0 Top 15 PSF Point 1-9-4 Near Face 158 lb	Tie-In 0-0-0 to 16-5-12 (Span)0-5-9 Top 15 PSF 40 PSF Tie-In 0-0-0 to 1-10-8 (Span)3-5-0 Top 15 PSF 40 PSF Point 1-9-4 Near Face 158 lb 422 lb	Tie-In 0-0-0 to 16-5-12 (Span)0-5-9 Top 15 PSF 40 PSF 0 PSF Tie-In 0-0-0 to 1-10-8 (Span)3-5-0 Top 15 PSF 40 PSF 0 PSF Point 1-9-4 Near Face 158 lb 422 lb 0 lb	Tie-In 0-0-0 to 16-5-12 (Span)0-5-9 Top 15 PSF 40 PSF 0 PSF 0 PSF Tie-In 0-0-0 to 1-10-8 (Span)3-5-0 Top 15 PSF 40 PSF 0 PSF 0 PSF Point 1-9-4 Near Face 158 lb 422 lb 0 lb 0 lb

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation

 Joist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handlingferedion details

 Damaged IJoists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths—3.5 inches
 For flat roofs provide proper drainage to prevent
- ponding

Manufacturer Info

Nascor by Kott







Client:

GREEN YORK HOMES

Project: Address: Date:

6/1/2018

RCO

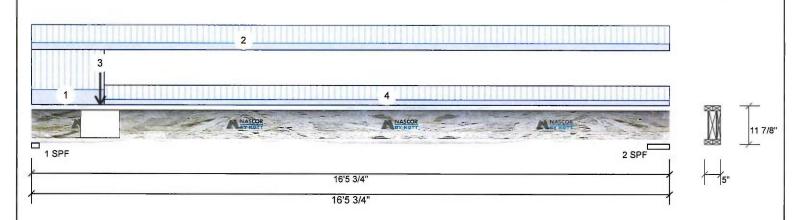
Designer:

Project #:

Job Name: INDIGO 1 (ELEV.2)

2-Ply - PASSED F15-B 11.875" NJH

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Type: Girder Brg Dead Wind Application: Floor (Residential) Live Snow Plies: 2 Design Method: LSD 846 317 0 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 505 189 0 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: **15 PSF** Cap. React D/L lb Bearing Length Total Ld. Case Ld. Comb. 50% 1666 L 1 - SPF 2.375" 397 / 1269 1.25D+1.5L 2 - SPF 6.875" 27% 237 / 758 1.25D+1.5L 994 L

Ana	lysis	Results	

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4073 ft-lb	7'3 9/16"	10780 ft-lb	0.378 (38%)	1.25D+1.5L	L
Unbraced	4073 ft-lb	7'3 9/16"	4080 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	1639 lb	1 5/8"	3620 lb	0.453 (45%)	1.25D+1.5L	L
Perm Defl in.	0.065 (L/2937)	7'9 11/16"	0.528 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.172 (L/1102)	7'9 11/16"	0.528 (L/360)	0.330 (33%)	L	L
TL Defl inch	0.237 (L/801)	7'9 11/16"	0.792 (L/240)	0.300 (30%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 6'8" o.c.

5 Bottom flange braced at bearings.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



0

0

ID Load Type Location Trib Width Side Dead Live Snow Wind Comments
ID Load Type Location Trib Width Side Dead Live Snow Wind Comments
1 Tie-In 0-0-0 to 1-10-8 (Span)3-5-0 Top 15 PSF 40 PSF 0 PSF 0 PSF
2 Tie-In 0-0-0 to 16-5-12 (Span)1-6-7 Top 15 PSF 40 PSF 0 PSF 0 PSF
Point 1-9-4 Far Face 136 lb 362 lb 0 lb 6
4 Tie-In 1-10-8 to 16-5-12 (Span)1-2-9 Top 15 PSF 40 PSF 0 PSF 0 PSF

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Uses not to be treated with fire retardant or corrosive

Handling & Installation

- Library & Installation
 Library amust not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, suffiend rables, web hole
 chart, bridging details, multi-ply fastening details and
 handing/erection details
 Damaged IJoists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent populars.

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™ Client: GREEN YORK HOMES

Address:

Project:

Date: 6/1/2018

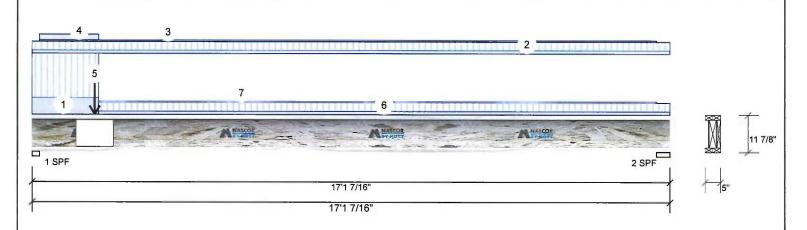
RCO Job Name: INDIGO 1 (ELEV.2)

Project #:

Designer:

11.875" 2-Ply - PASSED F15-C NJH

Level: Ground Floor



Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	VVind
1	755	381	0	0
2	279	144	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	2.375"	49%	477 / 1133	1609	L	1.25D+1.5L	
2 - SPF	4.319"	17%	180 / 419	599	L	1.25D+1.5L	

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2960 ft-lb	6'8 1/8"	10780 ft-lb	0.275 (27%)	1.25D+1.5L	L
Unbraced	2960 ft-lb	6'8 1/8"	2970 ft-lb	0.997 (100%)	1.25D+1.5L	L
Shear	1588 lb	1 5/8"	3620 lb	0.439 (44%)	1.25D+1.5L	L
Perm Defl in.	0.065 (L/3060)	8' 1/16"	0.556 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.127 (L/1581)	7'11 15/16"	0.556 (L/360)	0.230 (23%)	L	L
TL Defl inch	0.192 (L/1042)	8'	0.834 (L/240)	0.230 (23%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'9" o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



PAGE 24 OF 38

Page 1 of 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 17-1-7	(Span)0-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-5 to 16-9-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-5 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	240 lb	480 lb	0 lb	0 lb	F6
6	Tie-In	1-9-6 to 17-1-7	(Span)0-8-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 16-9-1		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

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Dry service conditions, unless noted otherwise
 Use to be treated with fire retardant or corrosive

Handling & Installation

- Library & Installation
 Library amust not be cut or drilled
 Refer to latest copy of the Library trables, web note
 chart, bridging details, striflent rables, web note
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged Library must be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent populing

Manufacturer Info





Client: Project:

Address:

GREEN YORK HOMES

Date: 6/1/2018

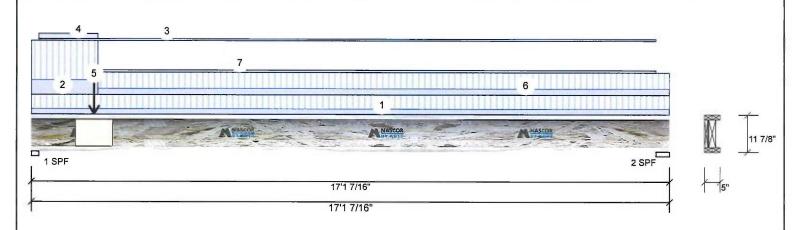
Designer: **RCO**

Job Name: INDIGO 1 (ELEV.2)

Project #

F15-D 11.875" 2-Ply - PASSED NJH

Level: Ground Floor



Member Information							
Туре:	Girder						
Plies:	2						
Moisture Condition:	Dry						
Deflection LL:	360						
Deflection TL:	240						
Importance:	Normal						
General Load							
Floor Live:	40 PSF						

15 PSF

Application: Floor (Residential) Design Method: LSD

Building Code: NBCC 2010 / OBC 2012

Load Sharing: No Deck:

Not Checked Not Checked Vibration:

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	832	417	0	0
2	496	248	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	2.375"	53%	521 / 1248	1769	L	1.25D+1.5L	
2-SPF	4.319"	29%	310 / 744	1054	L	1.25D+1.5L	

Analysis Results

Dead:

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	4604 ft-lb	7'9 15/16"	10780 ft-lb	0.427 (43%)	1.25D+1.5L	L
	Unbraced	4604 ft-lb	7'9 15/16"	4611 ft-lb	0.998 (100%)	1.25D+1.5L	L
	Shear	1744 lb	1 5/8"	3620 lb	0.482 (48%)	1.25D+1.5L	L
	Perm Defl in.	0.100 (L/2004)	8'3 3/16"	0.556 (L/360)	0.180 (18%)	D	Uniform
	LL Defl inch	0.198 (L/1013)	8'3 3/16"	0.556 (L/360)	0.360 (36%)	L	L
	TL Defl inch	0.298 (L/673)	8'3 3/16"	0.834 (L/240)	0.360 (36%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 6'2" o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



5 Bottom i	nange braced at bearings								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 17-1-7	(Span)1-3-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 16-9-1		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
4	Part, Uniform	0-2-6 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Near Face	172 lb	343 lb	0 lb	0 lb	F6
6	Tie-In	1-9-6 to 17-1-7	(Span)1-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 16-9-1		Тор	4 PLF	0 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component sultability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Use to be treated with fire retardant or corrosive

Handling & Installation

- Librist flarges must not be out or drilled Refer to latest copy of the Librist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details Damaged Librist must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent appoint.

Manufacturer Info







EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address: Date: Designer:

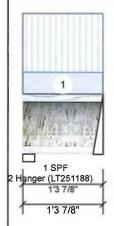
6/1/2018 **RCO**

Job Name: INDIGO 1 (ELEV.2)

Project #:

11.875" - PASSED NJH F5-A

Level: Ground Floor



11 7/8"

	mation

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application:

Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012

Floor (Residential)

Load Sharing: No Not Checked Deck:

Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	43	16	0	0
2	41	16	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2,375"	5%	20 / 65	86	L	1.25D+1.5L
2 - Hanger	2.000"	5%	19 / 62	82	L	1.25D+1.5L

Analysis Results

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	_
	Moment	19 ft-lb	8 1/8"	5390 ft-lb	0.003 (0%)	1.25D+1.5L	L	
	Unbraced	19 ft-lb	8 1/8"	5352 ft-lb	0.003 (0%)	1.25D+1.5L	L	
	Shear	68 lb	1 5/8"	1810 lb	0.038 (4%)	1.25D+1.5L	L	
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
l	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

ASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-3-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

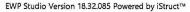
- Librist flarges must not be cut or drilled.
 Refer to latest copy of the Librist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details.
 Damaged Librists must not be used.
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address: Date: 6/1/2018

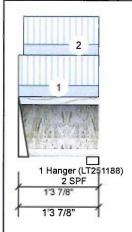
Designer: RCO

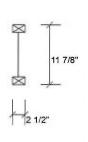
Job Name: INDIGO 1 (ELEV.2)

Project #:

F5-B 11.875" - PASSED NJH

Level: Ground Floor





Member Inform	nation
Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Design Method:

Building Code:

Load Sharing:

Deck: Vibration: Floor (Residential)

NBCC 2010 / OBC 2012

No Not Checked

Not Checked

2

Brg

1 H

2 - SPF 2.375"

Live 74 83

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead Snow 28 31

0 0 0 0

Wind

Ld. Comb. 1.25D+1.5L

1.25D+1.5L

15 PSF

Bearings and Factored Reactions

earing	Length	Cap. Re	eact D/L lb	Total	Ld. Case	
-	2.000"	9%	35 / 112	147	L	
langer						

10%

39 / 125

164 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	35 ft-lb	7 3/4"	5390 ft-lb	0.007 (1%)	1.25D+1.5L	L
Unbraced	35 ft-lb	7 3/4"	5352 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	131 lb	1 1/4"	1810 lb	0.072 (7%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/40749)	7 3/4"	0.036 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.000 (L/29636)	7 3/4"	0.054 (L/240)	0.010 (1%)	D+L	L

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY

NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

1 Fill all hanger nailing holes.

2 Girders are designed to be supported on the bottom edge only.

3 Top flange unbraced.

4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-3-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-0 to 1-3-14	(Span)2-11-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andImg & Installation

Loist flanges must not be cut or drilled

Refer to latest copy of the IJoist product information

details for framing details, stiffener tables, web hole

chart, bridging details, multi-ply fastening details and

handing/erection details

Damaged IJoists must not be used

Design assumes top flange to be laterally restrained

by attached sheathing or as specified in engineering

notes.

5. Provide lateral support at bearing points to avoid

lateral displacement and rotation

8. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

7. For flat roofs provide proper drainage to prevent

7.

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario Canada 14A7X4



EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: **GREEN YORK HOMES**

Address:

Date: 6/1/2018

Designer: RCO

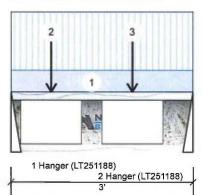
Job Name: INDIGO 1 (ELEV.2)

Level: Ground Floor

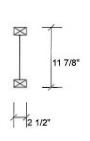
Project #:

11.875" - PASSED NJH F6-A





3'



FAGE ZO OF 39

Page 1 of 1

Member	Information
MEHIDEL	IIIIOIIIIauoii

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Design Method:

Building Code: Load Sharing:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

No

Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	422	158	0	0
2	362	136	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	607 ft-lb	1'11 15/16"	5390 ft-lb	0.113 (11%)	1.25D+1.5L	L
Unbraced	607 ft-lb	1'11 15/16"	4800 ft-lb	0.126 (13%)	1.25D+1.5L	L
Shear	823 lb	1 1/4"	1810 lb	0.455 (45%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/13854)	1'11 1/16"	0.093 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.006 (L/5198)	1'11 1/16"	0.093 (L/360)	0.070 (7%)	L	L
TL Defl inch	0.009 (L/3780)	1'11 1/16"	0.140 (L/240)	0.060 (6%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	51%	197 / 633	830	L	1.25D+1.5L	
2 - Hanger	2.000"	44%	170 / 543	713	L	1.25D+1.5L	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.

L	4 Bottom lange braced at bearings.						E CHILL COLLEGE COLLEGE			
Г	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Point	0-7-15		Far Face	117 lb	313 lb	0 lb	0 lb	J8
	3	Point	1-11-15		Far Face	136 lb	362 lb	0 lb	0 lb	.18

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation

 Loist flanges must not be cut or drilled
 Refer to latest copy of the Lioist product information
 details for framing details, sufferer tables, web hole
 chart, bridging details, multi-ply featening details and
 handling/erector details
- handling/erection details

 Damaged Lloists must not be used

 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering

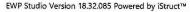
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Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

GREEN YORK HOMES

Project: Address:

Date: 6/1/2018

Designer: RÇO

Job Name: INDIGO 1 (ELEV.2)

Level: Ground Floor

Unfactored Reactions UNPATTERNED Ib (Uplift)

Live

480

343

Bearings and Factored Reactions

Dead

240

172

Cap. React D/L lb

300 / 721

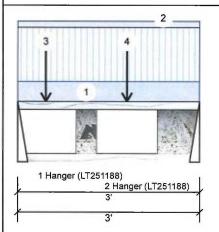
215 / 515

63%

45%

Project #:

11.875" - PASSED F6-B NJH



11 7/8"

Snow

0

Total Ld. Case

1021 L

729 L

FAGE 29 OF 39

Page 1 of 1

Member Information

Type: Girder Plies: 1 Moisture Condition: Dry Deflection II: 360 Deflection TL: 240 Importance: Normal General Load Floor Live: 40 PSF Dead: 15 PSF Application: Design Method:

Building Code:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

1

2

Bearing Length

Hanger

Hanger

2 -

2.000"

2.000"

Load Sharing: No

Deck: Not Checked Vibration: Not Checked

Analysis Results

ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	749 ft-lb	1'9 9/16"	5390 ft-lb	0.139 (14%)	1.25D+1.5L	L
l	Unbraced	749 ft-lb	1'9 9/16"	4800 ft-lb	0.156 (16%)	1.25D+1.5L	L
l	Shear	1013 lb	1 1/4"	1810 lb	0.560 (56%)	1.25D+1.5L	L
l	Perm Defl in.	0.004 (L/9092)	1'9 9/16"	0.093 (L/360)	0.040 (4%)	D	Uniform
l	LL Defl inch	0.007 (L/4555)	1'9 9/16"	0.093 (L/360)	0.080 (8%)	L	L
l	TI Deflinch	0.011 (1/3035)	1'9 9/16"	0 140 (1/240)	0.080 (8%)	D+I	ī

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS

CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL**

SPROMA	
A TONK	
N.A. EL-MASRI	
Chart Emagn	1
TO WAS THE REAL PROPERTY.	
Jun 05, 2018	

Wind

0

O

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

_							-IPOINT LOADS (AVED BEAD	INGS	
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
2	1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
	3	Point	0-5-9		Near Face	152 lb	304 lb	0 lb	0 lb	J8
	4	Point	1-9-9		Near Face	209 lb	416 lb	0 lb	0 lb	J8

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Anoling & Installation
 Librating are must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web note
 chart, bridging details, multi-up fastening details and
 handling/erection details
 Damaged IJoists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott





EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address: Date:

6/1/2018

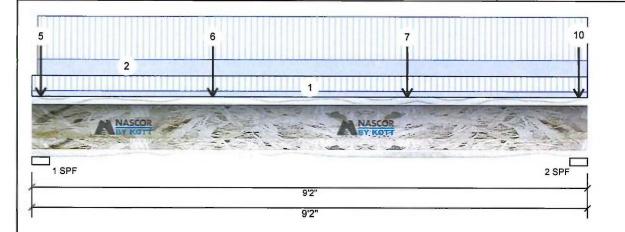
Designer: RCO

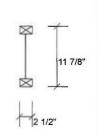
Job Name: INDIGO 1 (ELEV.2)

Project #:

11.875" - PASSED F7-A NJH

Level: Ground Floor





Wind

0

0

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF	i l	

Bearings	and	Factored	Reactions
Dessina	Longil	h Con	Denet D/L I

Live

326

328

1

2

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

394

395

Snow

69

69

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	3,500"	59%	492 / 523	1015	L	1.25D+1.5L +0.5S	
2 - SPF	3.500"	59%	493 / 527	1020	L	1.25D+1.5L +0.5S	

Analysis Results

ı	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	1148 ft-lb	4'7"	5174 ft-lb	0.222 (22%)	1.25D+1.5L	L
	Unbraced	1148 ft-lb	4'7"	1155 ft-lb	0.994 (99%)	1.25D+1.5L	L
ı	Shear	489 lb	8'11 1/4"	1738 lb	0.281 (28%)	1.25D+1.5L	L
ı	Perm Defl in.	0.014 (L/7693)	4'7"	0.290 (L/360)	0.050 (5%)	D	Uniform
ı	LL Defl inch	0.036 (L/2894)	4'7"	0.290 (L/360)	0.120 (12%)	L+0.5S	L
	TL Defl inch	0.050 (L/2103)	4'7"	0.435 (L/240)	0.110 (11%)	D+L+0.5\$	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL**



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 8'8" o.c.

3 Bottom	flange braced at bearings					POINT LOAD	S OVER BEAL	VER BEARINGS.	
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 9-2-0	(Span)0-6-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-2 to 9-2-0	(Span)1-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-1-12		Тор	18 lb	0 lb	0 lb	0 lb	Wall Self Weight
4	Point	0-1-12		Тор	261 lb	71 lb	69 lb	0 lb	F3 F3
5	Point	0-1-12		Тор	19 lb	0 lb	0 lb	0 lb	Wall Self Weight
6	Point	2-11-12		Near Face	28 lb	74 lb	0 lb	0 lb	F5
7	Point	6-2-4		Near Face	28 lb	74 lb	0 lb	0 lb	F5
8	Point	9-0-4		Тор	261 lb	71 lb	69 lb	0 lb	F3 F3
9	Point	9-0-4		Тор	19 lb	0 lb	0 lb	0 lb	Wall Self Weight
10	Point	9-0-4		Тор	18 lb	0 lb	0 lb	0 lb	Wall Self Weight

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- 1. Lioist flanges must not be cut or drilled
 2. Refer to latest copy of the Lioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
 3. Damaged Lioists must not be used
 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott



Member Information

Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address: **GREEN YORK HOMES**

6/1/2018

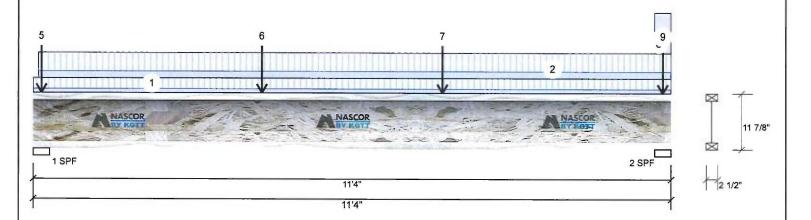
Date: Designer: RCO

Job Name: INDIGO 1 (ELEV.2)

Project #:

11.875" - PASSED F8-A NJH

Level: Ground Floor



Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored	Reactions	UNPATTERNED	lb	(Uplift)
------------	-----------	-------------	----	----------

Brg	Live	Dead	Snow	Wind	
1	405	660	136	0	
2	407	644	136	0	

Bearings and Factored Reactions

Bearing	Length	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	3.500"	92%	825 / 675	1500	L	1.25D+1.5L +0.5S	
2 - SPF	3,500"	91%	805 / 679	1484	L	1.25D+1.5L +0.5S	

Analysis Results Analysis Actual Location Allowed Capacity Comb. Moment 1671 ft-lb 5'8" 4851 ft-lb 0.344 (34%) 1.25D+1.5L L 1671 ft-lb Unbraced 0.994 (99%) 1.25D+1.5L L 5'8" 1680 ft-lb 1629 lb 586 lb 11'1 1/4" 0.360 (36%) 1.25D+1.5L L Perm Defl in. 0.028 (L/4732) 5'8" 0.362 (L/360) 0.080 (8%) D Uniform LL Defl inch 0.073 (L/1788) 5'8" 0.362 (L/360) 0.200 (20%) L+0.5S TL Defl inch 0.101 (L/1298) 5'8" 0.544 (L/240) 0.180 (18%) D+L+0.5S

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 7'2" o.c.

3 Bottom	flange braced at bearings	5.				POINT LOAD	S OVER BEAF	RINGS.	July 3, 2010	,
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 11-4-0	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-1-2 to 11-4-0	(Span)1-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	0-1-12		Тор	21 lb	0 lb	0 lb	0 lb	Wall Self Weight	
4	Point	0-1-12		Тор	512 lb	102 lb	136 lb	0 lb	F4 F4	
5	Point	0-1-12		Тор	13 lb	0 lb	0 lb	0 lb	Wall Self Weight	
6	Point	4-0-12		Near Face	16 lb	41 lb	0 lb	0 lb	F5	
7	Point	7-3-4		Near Face	16 lb	41 lb	0 lb	0 lb	F5	
8	Part. Uniform	11-0-8 to 11-4-0		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
9	Point	11-2-4		Тор	510 lb	102 lb	136 lb	0 lb	F4 F4	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Libist flanges must not be cut or drilled Refer to latest copy of the Lloist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply festering details and handling/erection details Damaged Lloists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent populars.
- 7.

Manufacturer Info

Nascor by Kott



EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address:

6/1/2018

Page 1 of 1

Designer: RCO

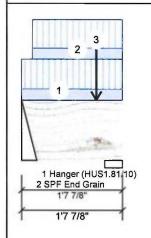
Job Name: INDIGO 1 (ELEV.1)

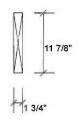
Project #:

Date:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Second Floor





Wind

0

0

Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Live

121

182

1

2

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

49

73

Snow

0

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	6%	62 / 182	244	L	1.25D+1.5L	
2 - SPF End Grain	3.500"	8%	91 / 273	363	L	1.25D+1.5L	

Analysis Results

l	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
ı	Moment	72 ft-lb	10 7/16"	17130 ft-lb	0.004 (0%)	1.25D+1.5L	L	
l	Unbraced	72 ft-lb	10 7/16"	16556 ft-lb	0.004 (0%)	1.25D+1.5L	L	
١	Shear	95 lb	1'2 1/8"	5798 lb	0.016 (2%)	1.25D+1.5L	L	
l	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	LL Defl inch	0.000 (L/67338)	10 3/8"	0.041 (L/360)	0.010 (1%)	L	L	
ı	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

,									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-7-14	(Span)3-11-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-2-2 to 1-7-14	(Span)3-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-12		Far Face	23 lb	60 lb	0 lb	0 lb	J1
	Self Weight				5 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Inlended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product informatior regarding installation requirements, multi-ply fastening details, beam strength values, and code
- particular details, usern sterigin values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info



EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Address:

GREEN YORK HOMES

Project:

Date: 6/1/2018

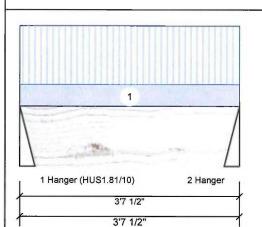
Designer: RCO

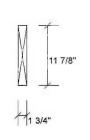
Job Name: INDIGO 1 (ELEV.1)

Level: Second Floor

Project #:

1.750" X 11.875" - PASSED F11-A Forex 2.0E-3000Fb LVL





Member Information Type: Girder Application: Floor (Residential) Plies: LSD Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load 40 PSF Floor Live: Dead: 15 PSF

	Live	Dead	Snow	Wind
1	489	192	0	0
2	489	192	0	0

240 / 734

240 / 734

Cap. React D/L lb

Unfactored Reactions UNPATTERNED Ib (Uplift)

Analysis Results Analysis Actual Location Allowed Capacity Comb. Case 709 ft-lb 1.25D+1.5L L Moment 1'9 3/4" 17130 ft-lb 0.041 (4%) Unbraced 709 ft-lb 1'9 3/4" 13201 ft-lb 0.054 (5%) 1.25D+1.5L L Shear 1'2 1/8" 5798 lb 0.059 (6%) 1.25D+1.5L L Perm Defl in. 0.001 1'9 3/4" 0.108 (L/360) 0.010 (1%) D Uniform (L/29614) LL Defl inch 0.003 1'9 3/4" 0.108 (L/360) 0.030 (3%) L L (L/11600) TL Defl inch 0.005 (L/8335) 1'9 3/4" 0.162 (L/240) 0.030 (3%) D+L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

25%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY **NAILING OR BOLTING REQUIREMENTS**

ASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Bearing Length

Hanger

Hanger

2 -

3.000"

3.000"



Total Ld. Case Ld. Comb.

1.25D+1.5L

1.25D+1.5L

973 L

973 L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

T DOLLOTTI	bracea at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-7-8		Тор	101 PLF	270 PLF	0 PLF	0 PLF	
	Self Weight				5 PLF				

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Forex APA: PR-L318

Manufacturer Info



EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address:

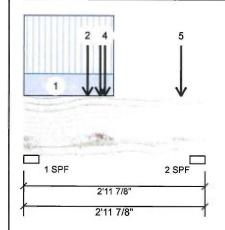
6/1/2018 Designer: RCO

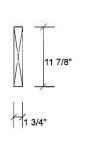
Job Name: INDIGO 1 (ELEV.1)

Project #:

1.750" X 11.875" - PASSED F11-B Forex 2.0E-3000Fb LVL

_evel: Second Floor





Type:	Girder	Application:
Plies:	1	Design Method
Moisture Condition:	Dry	Building Code:
Deflection LL:	360	Load Sharing:
Deflection TL:	240	Deck:
mportance:	Normal	Vibration:
General Load		
Floor Live:	40 PSF	
Dead:	15 PSF	

Floor (Residential) Brg NBCC 2010 / OBC 2012

Unfactored Reactions UNPATTERNED Ib (Uplift) Live Dead Snow Wind 468 188 0 0 2 381 0 154 0

Bearings and Factored Reactions Cap. React D/L lb Bearing Length Total Ld. Case Ld. Comb. 1 - SPF 3.000" 29% 235 / 701 936 L 1.25D+1.5L 2 - SPF 3.000" 24% 193 / 571 764 1 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	889 ft-lb	1'4"	17130 ft-lb	0.052 (5%)	1.25D+1.5L	L
Unbraced	889 ft-lb	1'4"	14588 ft-lb	0.061 (6%)	1.25D+1.5L	L
Shear	757 lb	1'9 3/4"	5798 lb	0.131 (13%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/23259)	1'4"	0.087 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.003 (L/9252)	1'4"	0.087 (L/360)	0.040 (4%)	L	L
TL Defl inch	0.005 (L/6619)	1'4"	0.131 (L/240)	0.040 (4%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-12	(Span)3-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-0-9		Near Face	23 lb	60 lb	0 lb	0 lb	J1
3	Point	1-3-1		Far Face	37 lb	98 lb	0 lb	0 lb	J1
4	Point	1-4-0		Near Face	192 lb	489 lb	0 lb	0 lb	F11
5	Point	2-7-1		Far Face	34 lb	90 lb	0 lb	0 lb	J1
	Self Weight				5 PLF				

LSD

No Not Checked Not Checked

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design onteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply featening details, beam strength values, and code approvals

- approvais
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex



EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

Project:

GREEN YORK HOMES

Address:

6/1/2018

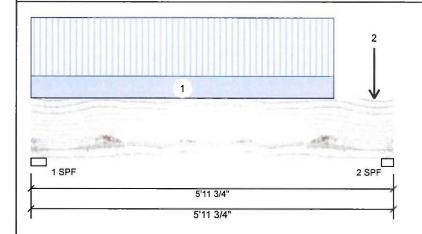
Designer: RCO

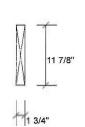
Job Name: INDIGO 1 (ELEV.1)

Project #:

1.750" X 11.875" - PASSED F12-A Forex 2.0E-3000Fb LVL

Level: Second Floor





Member	Intormation

Type:	Giraer
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

15 PSF

Application:

LSD Design Method: NBCC 2010 / OBC 2012 **Building Code:** No

Floor (Residential)

Load Sharing:

Deck: Not Checked Vibration: Not Checked

Unfactored	Reactions	UNPATTERNED	lb	(Uplift)
------------	-----------	--------------------	----	----------

Brg	Live	Dead	Snow	Wind	
1	214	95	0	0	
2	202	90	0	0	
1					

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	3.000"	14%	118 / 322	440	L	1.25D+1.5L	
2 - SPF	2.375"	16%	112 / 303	415	L	1.25D+1.5L	

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	575 ft-lb	2'11 3/4"	17130 ft-lb	0.034 (3%)	1.25D+1.5L	L
Unbraced	575 ft-lb	2'11 3/4"	7949 ft-lb	0.072 (7%)	1.25D+1.5L	L
Shear	529 lb	4'10 1/4"	5798 lb	0.091 (9%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/31643)	3'	0.189 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.005 (L/14020)	2'11 15/16"	0.189 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.007 (L/9715)	2'11 15/16"	0.283 (L/240)	0.020 (2%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 4-11-15		Far Face	27 PLF	72 PLF	0 PLF	0 PLF	
2	Point	5-7-15		Far Face	21 lb	57 lb	0 lb	0 lb	J1
	Self Weight				5 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation
 LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Camaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address:

Date: 6/1/2018 Designer: RCO

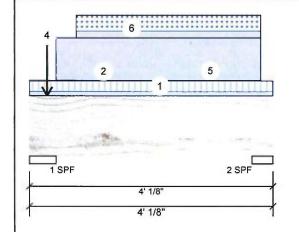
Job Name: INDIGO 1 (ELEV.1)

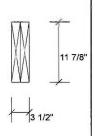
Project #:

Forex 2.0E-3000Fb LVL F13-A

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Member Inform	ation
Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

15 PSF

Application: Design Method:

Building Code:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

Load Sharing: No

Not Checked Vibration: Not Checked

Unfactored	Reactions	UNPAT	TERNED	lb	(Uplift)

Big	Live	Dead	Show	VVIIIQ
1	33	158	31	0
2	32	162	42	0
1				

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.250"	3%	198 / 50	248	L	1.25D+1.5L
2 - SPF	4.125"	4%	203 / 63	265	L	1.25D+1.5S

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	208 ft-lb	2' 3/4"	23297 ft-lb	0.009 (1%)	1.25D+1.5S	L
Unbraced	208 ft-lb	2' 3/4"	23297 ft-lb	0.009 (1%)	1.25D+1.5S	L
Shear	104 lb	1'4 3/8"	7885 lb	0.013 (1%)	1.25D+1.5S	L
Perm Defl in.	0.001 (L/65472)	2' 11/16"	0.112 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- al slenderness ratio based on full section width

7 Lateral sienderness ratio based on full section width.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-0-2	(Span)0-9-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-8-2		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
4	Point	0-3-8		Тор	8 lb	0 lb	0 lb	0 lb	Wall Self Weight
5	Part. Uniform	0-5-4 to 3-9-12		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
6	Part. Uniform	0-9-4 to 3-9-12		Тор	10 PLF	0 PLF	24 PLF	0 PLF	
	Self Weight				10 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex





EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project:

Address:

GREEN YORK HOMES

Date: 6/1/2018

Designer: Job Name: INDIGO 1 (ELEV.1)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor

	4	2	
1	3	5	+
1 ADDE			11 7
1 SPF	11'7 3/4"	2 SPF 3 1/2	2"

Nember Inform	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	2	Design Method:	LSD	1	313		173		0	0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	268		159		0	0
Deflection LL:	360	Load Sharing:	No	1 -					7	•
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing	s and Fac	tored I	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	2.750"	12%	216 / 470	686	L	1.25D+1.5L
				2-SPF	5.500"	5%	199 / 402	601	L	1.25D+1.5L

Analysis	Results
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Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1948 ft-lb	6'1 1/2"	34261 ft-lb	0.057 (6%)	1.25D+1.5L	L
Unbraced	1948 ft-lb	6'1 1/2"	28546 ft-lb	0.068 (7%)	1.25D+1.5L	L
Shear	551 lb	1'1 7/8"	11596 lb	0.048 (5%)	1.25D+1.5L	L
Perm Defl in.	0.012 (L/10848)	5'9 1/8"	0.369 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.022 (L/5964)	5'9 1/4"	0.369 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.035 (L/3848)	5'9 1/4"	0.554 (L/240)	0.060 (6%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



6 Lateral slend	erness ratio based o	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-5-2	(Span)1-7-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 11-3-8	(Span)1-1-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	3-6-14 to 7-2-6	(Span)1-4-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	7-3-4		Far Face	49 lb	121 lb	0 lb	0 lb	F10
5	Tie-In	11-3-6 to 11-7-12	(Span)0-8-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Tie-In	11-3-8 to 11-7-12	(Span)0-3-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				10 PLF				

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Lumber

Dry service conditions, unless noted otherwise
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Handling & Installation

1. LVL beams must not be out or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-pity fastering details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info





