Ground Floor

THIS CERTIFICATION IS TO CONFIRM THAT:

F11-B - 1 ply

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

F11-A - 1 ply

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

> N.A. EL-MASRI Jun 04, 2018 NE0618-017

Engineered floor joists shall be installed in accordance with the supplier's layout and specifications forming part of the permit drawings.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

2"x4" block (1/16" longer than rim depth) @ 16" o/c.

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may be deviate from the architectural drawings. Project Engineer to review and approve the deviation prior to construction

Point Load Support

Pcs Length 2 18-0-0 Layout Name 16-0-0 2 AMELIA 2 EL- 1 & 2 14-0-0 Design Method LSD 2 6-0-0 Description **GREEN YORK HOMES** 4-0-0 GRANELLI HOMES PROJECT BRAMPTON,ON Created Pcs Length May 28, 2018 10 18-0-0 8 18-0-0 Builder 4 14-0-0 Sales Rep 3 4-0-0 16-0-0 Designer 30 14-0-0 13 12-0-0 Shipping 3 4-0-0 Project Builder's Project Pcs Length 13 12 Kott Lumber Company 14 Anderson Blvd Stouffville, Ontario Canada Pcs Length L4A 7X4 Varies 37-0-0 905-642-4400 Supported Ground Floor Member Design Method fasteners Building Code NBCC 2010 / OBC 6 10d

1 #8x1 1/4WS Floor 16 16d .oads 10 16d ive

> Dead Deflection Joist LL Span L/

LSD

2012

40

15

480

360

480

360

360

240

480

360

OSB

3/4"

Nailed & Glued

LL Cant 2L/ TL Cant 2L/

Deflection Girder LL Span L/

TL Span L/

TL Span L/ LL Cant 2L/

TL Cant 2L/

Decking Deck

Thickness Fastener

Vibration

Architectural Drawing Info

Legend

Ground Floor .VL/LSL

F10

F9

F6

F7

F5

Joist

Label Description

Forex

Forex

Forex

Forex

Label Description

J8 NJ60H

F13 NJH

F12 NJH

F11 NJH

J5 NJH

J4 NJH

J3 NJH

J2 NJH

Label Description

11.875

Label Description

Norbord Rimboa

Plus 1,125 X

Label Pcs Description

3 HU310-2

34 LF2511

1 HGUS410

1 HUS1.81/10

member using a face-mounted hanger.

Rim parallel to joists: 1-1/8" rimboard with

Rim Board

R1

Blocking

Hanger

H2

H3

H4

NOTES:

H5

requirements.

BLK1 NJH

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

Width Depth

11.875

11.875

11 875

11.875

11.875

1.75

1.75

1.75

1.75

1.75

Width Depth

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

11.875

2.5 11.875 LinFt

Skew Slope

1.125

Framer to verify dimensions on the architectural drawings

Refer to Nascor specifier guide for installation details.

Load transfer blocks to be installed under all point loads.

are fastened as per the hanger manufacturer's standards.

It shall be the framer's responsibility that floor joists and beams

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting

Double joist only require filler/backer ply when supporting another

Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls.

Install single-ply flush window header along inside face of rimboard/rimjoist

Squash blocks recommended to be installed at end bearing on all first level

joists which support loading from above exceeding two levels floor or roof.

Width Depth Qty Plies

Width Depth Qty Plies

Qty

Plies

2

Qty Plies

2

Beam/Girder

fasteners

14 16d

12 10d

46 16d

30 16d

2

Load from Above Norbord Rimboard Plus 1.125 X 11.875 NJ60H 11.875 NJH 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

5.25 X 10.25 (Dropped)

JARDIN DESIGN GROUP 64 JARDIN DR, SUITE 3A VAUGHAN, ON L4K 3P3 Project # 17-55 Model: AMELIA 2

Date: MAY 22,2018 REV: 2

EWP Studio Simpson Strong-Tie® Component Solutions™

1. OBC 2012 O.Reg 332/12 as amended

5. CCMC -12787-R APA PR-L310(C)

2. Nascor CCMC - 13535-R

JOISTS SPACING 16"O/C

NOTED OTHERWISE

3. LVL CCMC -12904-R

4. CAN/CSA-O86-09

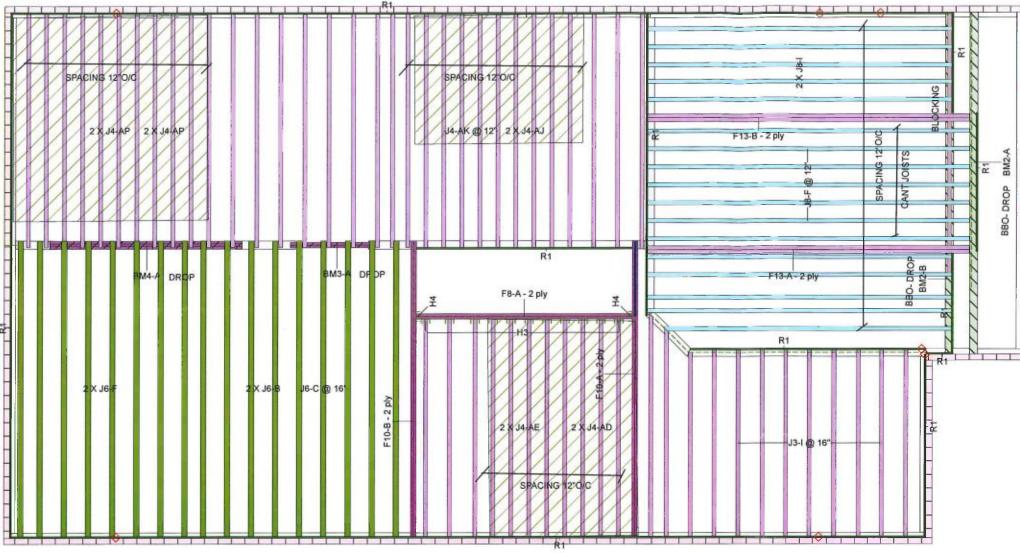
UNLESS

F11-C - 1 ply

All work shall conform to the Ontario Building Code O. Reg. 332/12 as amended

19-444463-000.00-ER.

Second Floor



Architectural Drawing Info

JARDIN DESIGN GROUP 64 JARDIN DR. SUITE 3A VALIGHAN ON 14K 3P3 Project # 17-55 Model: AMELIA 2 Date: MAY 22.2018

JOISTS SPACING 16"O/C UNLESS NOTED OTHERWISE

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

EWP Studio Simpson Strong-Tie® Component Solutions™

THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

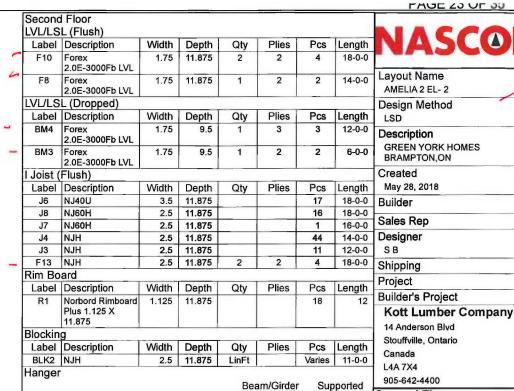
ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Skew Slope

fasteners

12 10d

46 16d

НЗ H4 NOTES:

Label Pcs Description

11 LF2511

2 HGUS410

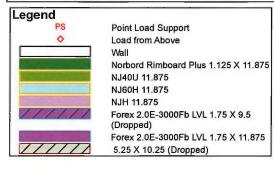
- Framer to verify dimensions on the architectural drawings.
- Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls. Install single-ply flush window header along inside face of rimboard/rimjoist
- Refer to Nascor specifier guide for installation details.
- Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- Load transfer blocks to be installed under all point loads. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists; 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth) @ 16" o/c. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

Hatch area represents ceramic tiled floor with an additional dead load of 5 PSF.

The framing shown on this layout may be deviate from the architectural drawings. Project Engineer to review and approve the deviation prior to construction



M-2057

Supported

Member

fasteners

1 #8x1 1/4WS

16 16d

Second Floor

Building Code NBCC 2010 / OBC

LSD

2012

40

15

480

360

480

360

360

240

480

360

OSB

5/8"

Nailed & Glued

Gypsum 1/2"

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Ceiling:

Deck

Deflection Girder

Floor

Loads

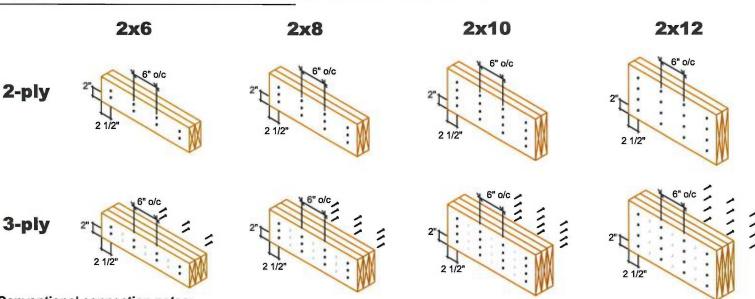
Live

Dead

TIPLE MEMBER CONNECTIONS

ON-AMELIA 2 ELE-1-2

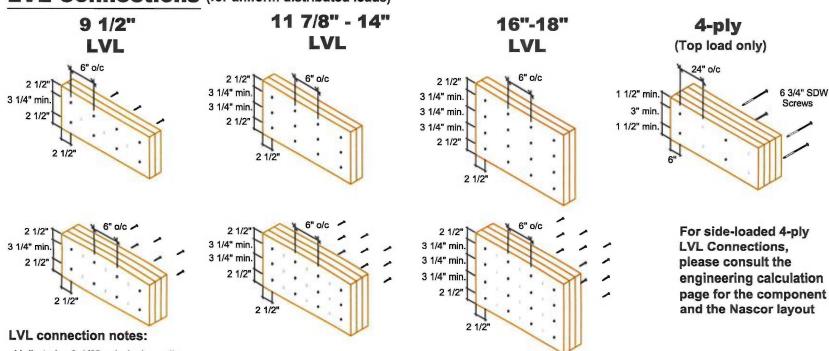
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

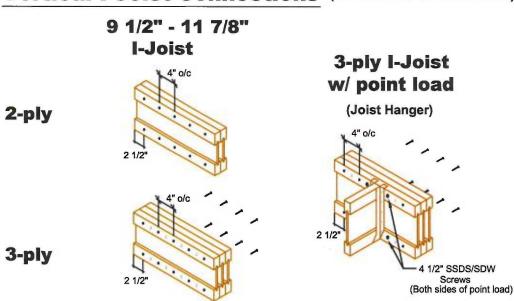
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails.
- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI-PLY CONNECTION

Scale: NTS

KOTT 3228 Moodie Drive Ottawa, ON **K2H 7V1** Ph: 613-838-2775 Fx: 613-838-4751

DETAILS Date: November 30, 2016

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

GREEN YORK HOMES-BRAMPTON-ON-AMELIA 2 ELE-1-2

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

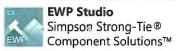
COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.





Client: Project: Address: Date: 5/30/2018

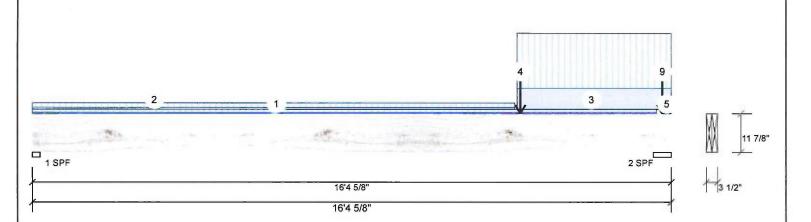
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED Level: Ground Floor



Member Inforn	lember Information					Unfactored Reactions UNPATTERNED Ib (Uplift)							
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind			
Plies:	2	Design Method:	LSD	1	970		457		0	0			
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	3301		1385		0	0			
Deflection LL:	360	Load Sharing:	No										
Deflection TL:	240	Deck:	Not Checked										
Importance:	Normal	Vibration:	Not Checked										
General Load													
Floor Live:	40 PSF	1		Bearing	s and Fac	tored	Reactions						
Dead:	15 PSF	1		Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.			
				1 - SPF	2.375"	40%	571 / 1454	2025	L	1.25D+1.5L			
				2-SPF	5.500"	56%	1731 / 4951	6682	L	1.25D+1.5L			

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	19001 ft-lb	12'6 3/16"	34261 ft-lb	0.555 (55%)	1.25D+1.5L	L	
Unbraced	19001 ft-lb	12'6 3/16"	22688 ft-lb	0.837 (84%)	1.25D+1.5L	L	
Shear	5852 lb	15'	11596 lb	0.505 (50%)	1.25D+1.5L	L	
Perm Defl in.	0.158 (L/1204)	8'11 7/8"	0.528 (L/360)	0.300 (30%)	D	Uniform	
LL Defi inch	0.363 (L/523)	9'1"	0.528 (L/360)	0.690 (69%)	L	L	
TL Defl inch	0.522 (L/365)	9' 5/8"	0.793 (L/240)	0.660 (66%)	D+L	L	

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3), Assumed point load size; beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-0-2	(Span)0-8-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-4-7	(Span) 0-11-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	12-5-3 to 16-4-10		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
4	Point	12-6-3		Near Face	1124 lb	2798 lb	0 lb	0 lb	F9
5	Part. Uniform	16-1-14 to 16-4-10		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318



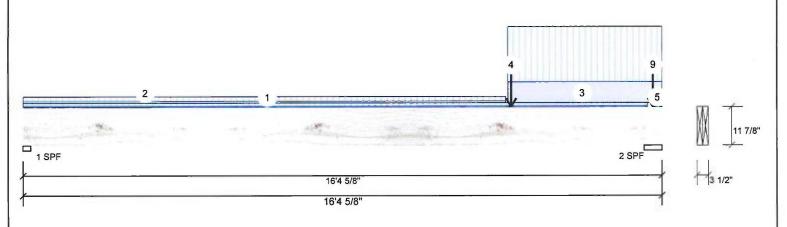


Client: Project: Address: Date: 5/30/2018

SB Designer: AMELIA 2 EL- 1 Job Name:

Project #:

Forex 2.0E-3000Fb LVL 1.750" X 11.875" 2-Ply - PASSED Level: Ground Floor F10-C



	Continued fro	om page 1							
	ID	Load Type	Location Trib Width	Side	Dead	Live	Snow	Wind	Comments
	6	Point	16-1-14	Тор	10 lb	26 lb	0 lb	0 lb	J1
ı	7	Point	16-1-14	Тор	2 lb	5 lb	0 lb	0 lb	J1
	8	Point	16-1-14	Тор	11 lb	30 lb	0 lb	0 lb	J4
	9	Point	16-1-14	Тор	9 lb	0 lb	0 lb	0 lb	Wall Self Weight
		Self Weight			10 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318

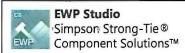


Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Page 2 of 2



Client: Project: Address:

5/30/2018 Date:

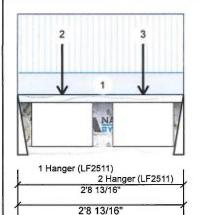
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #

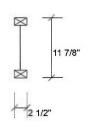
11.875" - PASSED F11-A NJH

Level: Ground Floor



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Hanger



Member Information Girder Application: Floor (Residential) Type: Plies: Design Method: LSD Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Not Checked Deflection TL: 240 Deck: Importance: Normal Vibration: Not Checked General Load Floor Live: 40 PSF 15 PSF Dead:

Brg	Live	Dead	Snow	Wind	
1	277	104	0	0	
2	287	107	0	0	

Unfactored Reactions UNPATTERNED Ib (Uplift)

Bearings and Factored Reactions Ld. Comb. Bearing Length Cap. React D/L lb Total Ld. Case 1.25D+1.5L 2.000" 34% 130 / 416 546 L Hanger 35% 134 / 430 1.25D+1.5L 2 -2.000" 565 L

Analysis Results Analysis Actual Location Allowed Capacity Comb. 333 ft-lb 1' 5/16" 5390 ft-lb 0.062 (6%) 1.25D+1.5L L Moment 333 ft-lb 0.067 (7%) 1.25D+1.5L L Unbraced 1' 5/16" 4941 ft-lb 559 lb 2'7 9/16" 1810 lb 0.309 (31%) 1.25D+1.5L L Shear Perm Defl in. 0.001 1'3 1/16" 0.084 (L/360) 0.020 (2%) D Uniform (L/22796) LL Defl inch 0.004 (L/8538) 1'3 1/16" 0.084 (L/360) 0.040 (4%) L L TL Defl inch 0.005 (L/6212) 1'3 1/16" 0.126 (L/240) 0.040 (4%) D+L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-8-13	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-8-13		Far Face	94 lb	251 lb	0 lb	0 lb	J3
3	Point	2-0-13		Far Face	91 lb	243 lb	0 lb	0 lb	J3



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design enteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- IJoist flanges must not be cut or drilled Refer to latest copy of the IJoist product Information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/er
- :anumigreecon details

 Damaged Loists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott









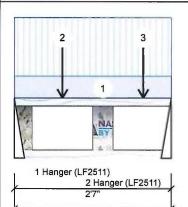
Client: Project: Address: Date: 5/30/2018

Designer: Job Name: AMELIA 2 EL- 1

Level: Ground Floor

Project #:

11.875" - PASSED F11-B NJH



2'7"

11 7/8"

MACIN	DOF	Intor	matio

Ī	Туре:	Girder
	Plies:	1
	Moisture Condition:	Dry
	Deflection LL:	360
	Deflection TL:	240
	Importance:	Normal
	General Load	
	Floor Live:	40 PSF
	Dead:	15 PSF

Application: Floor (Residential)

Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No

Not Checked Deck: Not Checked Vibration:

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	
1	320	120	0	0	
2	382	143	0	0	

Bearings and Factored Reactions

Bearing	Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	2.000"	39%	150 / 480	630	L	1.25D+1.5L
2 - Hanger	2.000"	46%	179 / 572	751	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	417 ft-lb	9 1/2"	5390 ft-lb	0.077 (8%)	1.25D+1.5L	L
Unbraced	417 ft-lb	9 1/2"	5011 ft-lb	0.083 (8%)	1.25D+1.5L	L
Shear	746 lb	2'5 3/4"	1810 lb	0.412 (41%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/18292)	9 1/2"	0.079 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.004 (L/6850)	9 1/2"	0.079 (L/360)	0.050 (5%)	L	L
TL Defl inch	0.006 (L/4984)	9 1/2"	0.119 (L/240)	0.050 (5%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings

+ Dottom na	+ bottom nange braced at bearings.											
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments			
1	Tie-In	0-0-0 to 2-7-0	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF				
2	Point	0-9-8		Far Face	128 lb	342 lb	0 lb	0 lb	J5			
3	Point	2-1-8		Far Face	110 lb	293 lb	0 lb	0 lb	J5			

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Lidist flanges must not be cut or drilled

 Refer to latest copy of the Lidist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erecton details
 Damaged Loists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott

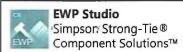


Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

Jun 04, 2018







Client: Project: Address:

5/30/2018 Date:

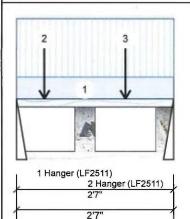
Designer: SB

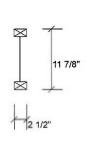
Job Name: AMELIA 2 EL- 1

Level: Ground Floor

Project #

11.875" - PASSED F11-C NJH





Wind

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Member Information

Type.	Girdei
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method:

Building Code: NBCC 2010 / OBC 2012

Load Sharing: No

Not Checked Deck: Not Checked Vibration:

Unfactored Reactions UNPATTERNED Ib (Uplift)

3	-			
1	384	144	0	0
2	322	121	0	0
ı				

Cap. React D/L lb

180 / 576

151 / 482

Snow

Total Ld. Case

756 L

633 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	420 ft-lb	1'9 1/2"	5390 ft-lb	0.078 (8%)	1.25D+1.5L	L
Unbraced	420 ft-lb	1'9 1/2"	5011 ft-lb	0.084 (8%)	1.25D+1.5L	L
Shear	751 lb	1 1/4"	1810 lb	0.415 (41%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/18157)	1'9 1/2"	0.079 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.004 (L/6812)	1'9 1/2"	0.079 (L/360)	0.050 (5%)	L	L
TL Defl inch	0.006 (L/4954)	1'9 1/2"	0.119 (L/240)	0.050 (5%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

47%

39%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Bearings and Factored Reactions

Bearing Length 2.000"

2.000"

1 -Hanger

2 -

Hanger

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

									٥	
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 2-7-0	(Span)1-3-7 to 1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Point	0-5-8		Far Face	111 lb	295 lb	0 lb	0 lb	J5	
3	Point	1-9-8		Far Face	129 lb	344 lb	0 lb	0 lb	J5	



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive
- Handling & Installation
- Libert angues must not be out or drilled Refer to latest copy of the Libert product information details for framing details, stiftener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details Damaged Liberts must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario Canada







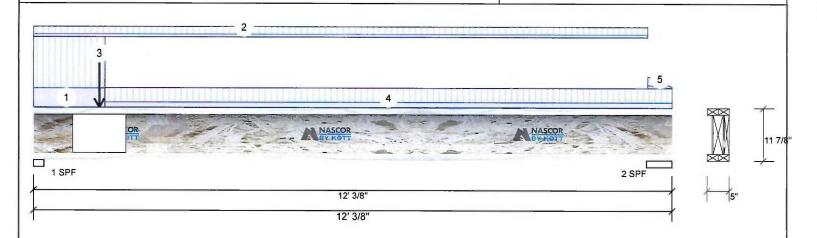
Project: Address:

Date: 5/30/2018 Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

F12-A NJH 11.875" 2-Ply - PASSED Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Girder Type: Application: Floor (Residential) Dead Snow Wind Plies: 2 Design Method: 474 177 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 193 72 0 0 Deflection LL: 360 Load Sharing: Deflection TL: Not Checked Importance: Normal Vibration: Not Checked General Load Bearings and Factored Reactions Floor Live: 40 PSF Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 28% 222 / 711 933 L 1.25D+1.5L 2 - SPF 5.754" 10% 90 / 290 380 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1243 ft-lb	4'8 13/16"	10780 ft-lb	0.115 (12%)	1.25D+1.5L	L
Unbraced	1243 ft-lb	4'8 13/16"	1248 ft-lb	0.996 (100%)	1.25D+1.5L	L
Shear	914 lb	1 5/8"	3620 lb	0.252 (25%)	1.25D+1.5L	L
Perm Defl in.	0.011 (L/12159)	5'6 3/16"	0.383 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.030 (L/4552)	5'6 3/16"	0.383 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.042 (L/3312)	5'6 3/16"	0.574 (L/240)	0.070 (7%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 11'2" o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-4-2	(Span)3-1-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 11-6-14	(Span)0-5-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Far Face	107 lb	287 lb	0 lb	0 lb	F11
4	Tie-In	1-4-2 to 12-0-6	(Span)0-10-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	11-6-14 to 12-0-6	(Span)0-6-5 to 0-0-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation

 Loist flanges must not be cut or drilled

 Refer to latest copy of the Lioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged Loists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 5. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

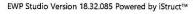
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







Wind

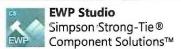
0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Page 1 of 1



Project: Address: 5/30/2018

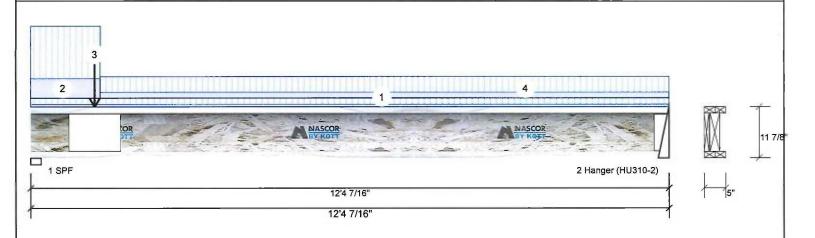
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

2-Ply - PASSED F12-B 11.875" NJH

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Girder Type: Application: Floor (Residential) Live Dead Snow Plies: 2 Design Method: 473 178 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 0 2 193 72 Deflection LL: Load Sharing: Not Checked Deflection TL: 240 Deck: Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions 40 PSF Floor Live: 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case 1 - SPF 2.375" 28% 222 / 710 932 L

Analysis Kes	uits					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1321 ft-lb	5'1 9/16"	10780 ft-lb	0.123 (12%)	1.25D+1.5L	L
Unbraced	1321 ft-lb	5'1 9/16"	1322 ft-lb	0.999 (100%)	1.25D+1.5L	L
Shear	913 lb	1 5/8"	3620 lb	0.252 (25%)	1.25D+1.5L	L
Perm Defl in.	0.013 (L/11026)	5'10 1/8"	0.403 (L/360)	0.030 (3%)	D	Uniform

LL Defl inch 0.035 (L/4136) 5'10 1/8" 0.403 (L/360) 0.090 (9%) L L TL Defl inch 0.048 (L/3008) 5'10 1/8" 0.604 (L/240) 0.080 (8%) D+L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

10%

91 / 290

380 L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

2.500"

Hanger

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange must be laterally braced at a maximum of 10'10" o.c.
- 6 Bottom flange braced at bearings.

7 Web st	iffeners required at Beari	ng 2.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-4-7	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-4-2	(Span)3-1-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Near Face	104 lb	277 lb	0 lb	0 lb	F11
4	Tie-In	1-4-2 to 12-4-7	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	,

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

chemicals

- Handling & Installation

- andling & Installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the Lioist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged Lioist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid

- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



Jun 04, 2018



Client: Project: Address:

5/30/2018 Date:

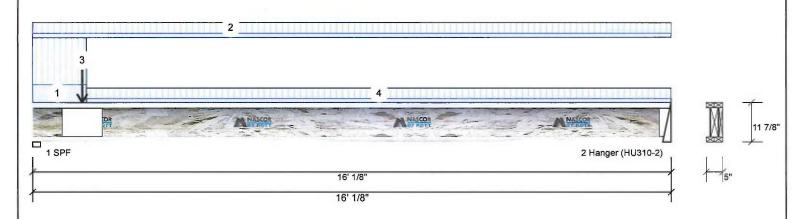
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" 2-Ply - PASSED F13-C NJH

Level: Ground Floor



Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib	(Uplift)
-------------------------------------	----------

Brg	Live	Dead	Snow	Wind
1	629	236	0	0
2	243	91	0	0
1				

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	37%	295 / 944	1238	L	1.25D+1.5L
2 - Hanger	2.500"	13%	114 / 364	478	L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2103 ft-lb	6'10 15/16"	10780 ft-lb	0.195 (20%)	1.25D+1.5L	L
Unbraced	2103 ft-lb	6'10 15/16"	2107 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	1219 lb	1 5/8"	3620 lb	0.337 (34%)	1.25D+1.5L	L
Perm Defl in.	0.033 (L/5696)	7'8"	0.524 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.088 (L/2135)	7'8"	0.524 (L/360)	0.170 (17%)	L	L
TL Defl inch	0.122 (L/1553)	7'8"	0.786 (L/240)	0.150 (15%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange must be laterally braced at a maximum of 9' o.c.
- 6 Bottom flange braced at bearings.
- 7 Web stiffeners required at Bearing 2.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

	indamon at pagini2 -									_
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 16-0-2	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	1-2-14		Far Face	143 lb	382 lb	0 lb	0 lb	F11	
4	Tie-In	1-4-2 to 16-0-2	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		

FL-MASRI Jun 04, 2018

Page 1 of 1

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 Use to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- LIGHTING OF ITSCHIPTION

 Lioist flanges must not be cut or drilled

 Refer to latest copy of the IJoist product information details for framing details, stifficer tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged IJoist must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
- Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







Wind

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™

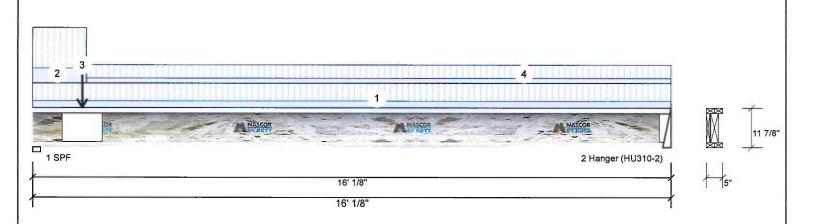
Client: Project: Address: Date: 5/30/2018

Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

F13-D NJH 11.875" 2-Ply - PASSED Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED lb (Uplift) Girden Type: Application: Floor (Residential) Live Dead Snow Plies: 2 Design Method: LSD 1 723 271 NBCC 2010 / OBC 2012 Moisture Condition: Dry Building Code: 2 398 149 0 Deflection LL: 360 Load Sharing: Deflection TL: Not Checked 240 Deck: Importance: Normal Vibration: Not Checked General Load 40 PSF **Bearings and Factored Reactions** Floor Live: Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case

Analysis Kes	Suits					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3230 ft-lb	7'5 3/4"	10780 ft-lb	0.300 (30%)	1.25D+1.5L	L
Unbraced	3230 ft-lb	7'5 3/4"	3243 ft-lb	0.996 (100%)	1.25D+1.5L	L
Shear	1400 lb	1 5/8"	3620 lb	0.387 (39%)	1.25D+1.5L	L
Perm Defl in.	0.051 (L/3734)	7'9 13/16"	0.524 (L/360)	0.100 (10%)	D	Uniform

LL Defl inch 0.135 (L/1400) 7'9 13/16" 0.524 (L/360) 0.260 (26%) L TL Defl inch 0.185 (L/1018) 7'9 13/16" 0.786 (L/240) 0.240 (24%) D+L L

Design Notes

Analysis Dosylfs

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange must be laterally braced at a maximum of 7'5" o.c.
- 6 Bottom flange braced at bearings.
- 7 Web stiffeners required at Bearing 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

43%

22%

339 / 1084

187 / 597

1423 L

784 L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

1 - SPF 2.375"

2 -

Hanger

2.500

L	/ vveb stiffeners	required at Bearing 2.								
Γ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 16-0-2	(Span)1-4-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Point	1-2-14		Near Face	120 lb	320 lb	0 lb	0 lb	F11
	4	Tie-In	1-4-2 to 16-0-2	(Span)1-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- andling & installation

 Loist flanges must not be out or drilled

 Refer to latest copy of the Lloist product information details for framing details, stiffener tables, web hole chart, bridging details, must-by fastening details and handling/erection details

 Demaged Lloiss must not be used

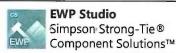
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Client: Project: Address:

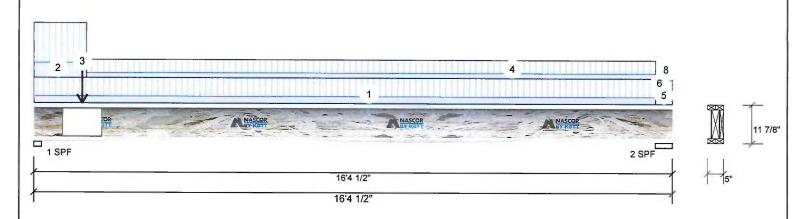
5/30/2018 Date:

Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" F13-E NJH 2-Ply - PASSED Level: Ground Floor



Member Infor	mation			Unfacto	red Reac	tions U	INPATTERN	ED Ib ((Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
Plies:	2	Design Method:	LSD	1	741		278		0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	416		157		0	0
Deflection LL:	360	Load Sharing:	No	1 -						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing	s and Fac	tored I	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	2.375"	44%	348 / 1112	1460	L	1.25D+1.5L
				2-SPF	5.250"	23%	196 / 624	821	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3385 ft-lb	7'6 13/16"	10780 ft-lb	0.314 (31%)	1.25D+1.5L	L
Unbraced	3385 ft-lb	7'6 13/16"	3390 ft-lb	0.999 (100%)	1.25D+1.5L	L
Shear	1436 lb	1 5/8"	3620 lb	0.397 (40%)	1.25D+1.5L	L
Perm Defl in.	0.054 (L/3540)	7'10 3/4"	0.529 (L/360)	0.100 (10%)	D	Uniform
LL Defi inch	0.143 (L/1328)	7'10 3/4"	0.529 (L/360)	0.270 (27%)	L	L
TL Defl inch	0.197 (L/966)	7'10 3/4"	0.793 (L/240)	0.250 (25%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'3" o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 15-11-4	(Span)1-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0 to 3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Far Face	121 lb	322 lb	0 lb	0 lb	F11
4	Tie-In	1-4-2 to 15-11-4	(Span)1-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	15-11-4 to 16-4-8	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Tie-In	15-11-4 to 16-4-8	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Continued or	n page 2								

Jun 04 2018

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inhended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Idoist not to be treated with fire retardant or corrosive

chemicals

- LINIATING & INSCALIATION

 I Joist flanges must not be cut or drilled

 Refer to latest copy of the Judist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-loy fastening details and handling/erection details

 Damaged Loists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

 Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

5/30/2018 Date:

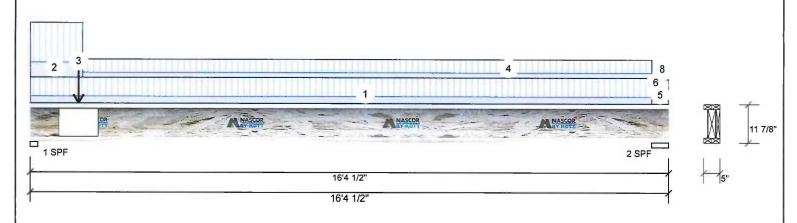
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" F13-E 2-Ply - PASSED NJH

Level: Ground Floor



..Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Part. Uniform	16-1-14 to 16-4-8		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
8	Part. Uniform	16-1-14 to 16-4-8		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.**

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- 1. Lijoist flagges must not be cut or drilled
 2. Refer to latest copy of the Lijoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
 3. Damaged Lijoists must not be used
 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent populing.

Manufacturer Info







EWP Studio Simpson Strong-Tie® Component Solutions™

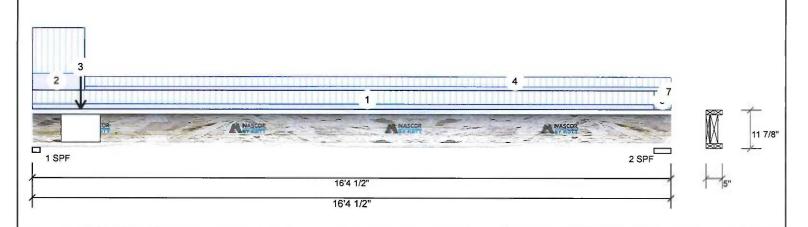
Client: Project: Address:

5/30/2018 Date:

Designer: SB Job Name: AMELIA 2 EL- 1

11.875" 2-Ply - PASSED F13-F NJH

Level: Ground Floor



Member Inforr	nation			Unfacto	red Reac	tions U	INPATTERN	ED lb (Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N
Plies:	2	Design Method:	LSD	1	673		252		0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	287		108		0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fac	tored I	Reactions		
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case
				1 - SPF	2.375"	40%	316 / 1010	1326	L

Bearings and Factored Reactions											
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.					
1 - SPF	2.375"	40%	316 / 1010	1326	L	1.25D+1.5L					
2 SDE	5 250"	16%	135 / 430	566	1	1 25D+1 5I					

Wind

0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2438 ft-lb	7'1 7/8"	10780 ft-lb	0.226 (23%)	1.25D+1.5L	L
Unbraced	2438 ft-lb	7'1 7/8"	2454 ft-lb	0.994 (99%)	1.25D+1.5L	L
Shear	1305 lb	1 5/8"	3620 lb	0.360 (36%)	1.25D+1.5L	L
Perm Defl in.	0.039 (L/4884)	7'9 3/8"	0.529 (L/360)	0.070 (7%)	D	Uniform
LL Defl inch	0.104 (L/1832)	7'9 3/8"	0.529 (L/360)	0.200 (20%)	L	L
TL Defl inch	0.143 (L/1332)	7'9 3/8"	0.793 (L/240)	0.180 (18%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'5" o.c.

5 Bottom flange braced at bearings.

L.	5 Bollom hange	braceu at bearings.								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 15-11-4	(Span)0-11-0 to 0-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0 to 3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	F11
	3	Point	1-2-14		Near Face	144 lb	384 lb	0 lb	0 lb	F11
	4	Tie-In	1-4-2 to 16-4-8	(Span)0-8-0 to 0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	N.A. EL-M
	5	Tie-In	15-11-4 to 16-4-8	(Ѕрап)0-4-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	CXChoat E
	6	Part. Uniform	16-1-14 to 16-4-8		Тор	2 PLF	0 PLF	0 PLF	0 PLF	Tun Ozor 3
	7	Part. Uniform	16-1-14 to 16-4-8		Тор	1 PLF	0 PLF	0 PLF	0 PLF	3417
_										

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 Use I Joist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Lioist flanges must not be cut or drilled Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, must-hoy fastening details and handling/erection details Damaged IJoists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roots provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address:

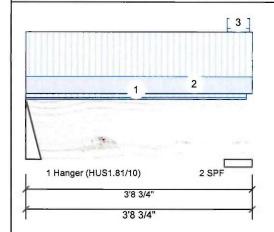
5/30/2018 Date: Designer: SB

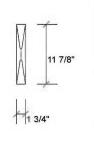
Job Name: AMELIA 2 EL- 1

Project #:

1.750" X 11.875" - PASSED F5-A Forex 2.0E-3000Fb LVL

Level: Ground Floor





Ld. Comb.

1.25D+1.5L

Member Intorn	nation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load		W 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind	-
1	462	182	0	0	
2	534	210	0	0	

Unfactored Reactions UNPATTERNED Ib (Uplift)

1	Analysis Res	nalysis Results									
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case				
	Moment	646 ft-lb	1'9 1/8"	17130 ft-lb	0.038 (4%)	1.25D+1.5L	L				
	Unbraced	646 ft-lb	1'9 1/8"	13452 ft-lb	0.048 (5%)	1.25D+1.5L	L				
	Shear	304 lb	1'2 1/8"	5798 lb	0.052 (5%)	1.25D+1.5L	L				
	Perm Defl in.	0.001 (L/32215)	1'9 1/8"	0.105 (L/360)	0.010 (1%)	D	Uniform				
	LL Defi inch	0.003 (L/12665)	1'9 1/8"	0.105 (L/360)	0.030 (3%)	L	L				
	TL Defl inch	0.004 (L/9091)	1'9 1/8"	0.157 (L/240)	0.030 (3%)	D+L	L				

Hanger 2 - SPF 5.500" 18% 262 / 801 1063 L 1.25D+1.5L

227 / 693

Total Ld. Case

920 L

Cap. React D/L lb

24%

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY **Design Notes** NAILING OR BOLTING REQUIREMENTS.

> PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Bearings and Factored Reactions

Bearing Length

3.000"

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-7-9	(Span)1-1-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-8-12		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
3	Tie-In	3-3-12 to 3-8-3	(Span)2-7-10 to 2-7-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF				



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design cnterna and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318







Total Ld. Case

Wind

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address: Date:

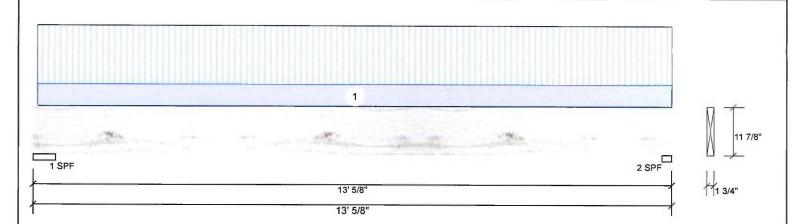
5/30/2018 Designer:

Job Name: AMELIA 2 EL- 1

Project #:

F6-A Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

Level: Ground Floor



Member Inform	nation			Unfacto	red Reac	tions U	INPATTERN	ED lb ((Uplift)
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w
Plies:	1	Design Method:	LSD	1	120		77		0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	117		74		0
Deflection LL:	360	Load Sharing:	No	12					7
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	s and Fac	tored l	Reactions		
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case
				1 - SPF	5.500"	5%	96 / 180	276	L
				2 - SPF	2.375"	11%	93 / 176	269	1

Ana	lysis	Resu	its
-----	-------	------	-----

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	824 ft-lb	6'7 7/8"	17130 ft-lb	0.048 (5%)	1.25D+1.5L	L
	Unbraced	824 ft-lb	6'7 7/8"	3591 ft-lb	0.229 (23%)	1.25D+1.5L	L
	Shear	222 lb	1'4 5/8"	5798 lb	0.038 (4%)	1.25D+1.5L	L
	Perm Defl in.	0.014 (L/10405)	6'7 7/8"	0.417 (L/360)	0.030 (3%)	D	Uniform
	LL Defl inch	0.023 (L/6605)	6'7 7/8"	0.417 (L/360)	0.050 (5%)	L	L
	TL Defl inch	0.037 (L/4040)	6'7 7/8"	0.626 (L/240)	0.060 (6%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-1-2 to 13-0-10	(Span)0-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF				

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Project: Address:

5/30/2018 Date:

Designer: SB

Job Name: AMELIA 2 EL- 1

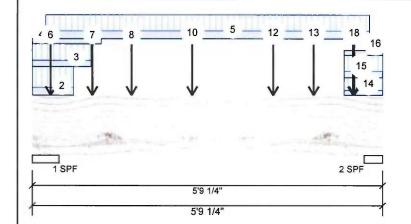
Project #:

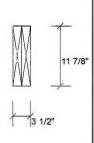
Forex 2.0E-3000Fb LVL F7-A

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor





Member Information

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Design Method:

Building Code:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

Load Sharing:

Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	3319	1453	0	0
2	3053	1319	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.250"	60%	1817 / 4978	6795	L	1.25D+1.5L
2-SPF	3.625"	80%	1648 / 4579	6227	L	1.25D+1.5L

Analysis Results

L	,						
Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	5096 ft-lb	2'7 5/8"	34261 ft-lb	0.149 (15%)	1.25D+1.5L	L
l	Unbraced	5096 ft-lb	2'7 5/8"	33024 ft-lb	0.154 (15%)	1.25D+1.5L	L
l	Shear	4068 lb	1'4 3/8"	11596 lb	0.351 (35%)	1.25D+1.5L	L
l	Perm Defl in.	0.008 (L/7346)	2'9 3/4"	0.172 (L/360)	0.050 (5%)	D	Uniform
l	LL Defl inch	0.019 (L/3189)	2'9 1/2"	0.172 (L/360)	0.110 (11%)	L	L
L	TL Defl inch	0.028 (L/2224)	2'9 9/16"	0.258 (L/240)	0.110 (11%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



/ Lateral	sienderness ratio based	on tull section wiath.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
2	Part. Uniform	0-0-0 to 0-8-2		Тор	122 PLF	326 PLF	0 PLF	0 PLF	J1
3	Part. Uniform	0-0-0 to 0-11-10		Тор	96 PLF	255 PLF	0 PLF	0 PLF	J4
4	Part. Uniform	0-0-0 to 1-1-10		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
5	Part. Uniform	0-2-10 to 5-6-10		Far Face	123 PLF	253 PLF	0 PLF	0 PLF	
6	Point	0-3-10		Near Face	111 lb	296 lb	0 lb	0 lb	J8

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design ontenia and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

and ling & installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ple
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at beering points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: Date:

5/30/2018 Designer: SB

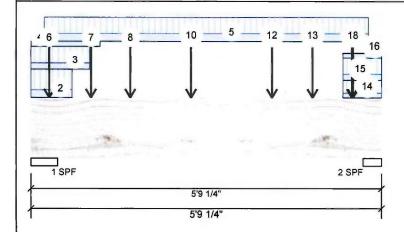
Job Name: AMELIA 2 EL- 1

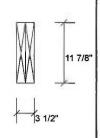
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED

Level: Ground Floor





Continued	from page 1							
ID	Load Type	Location Trib	b Width Side	Dead	Live	Snow	Wind	Comments
7	Point	0-11-14	Тор	520 lb	1340 lb	0 lb	0 lb	вмз вмз
8	Point	1-7-10	Near Face	149 lb	398 lb	0 lb	0 lb	F13
10	Point	2-7-10	Near Face	128 lb	342 lb	0 lb	0 lb	J5
12	Point	3-11-10	Near Face	110 lb	293 lb	0 lb	0 lb	J5
13	Point	4-7-10	Near Face	91 lb	243 lb	0 lb	0 lb	F13
14	Part. Uniform	5-1-10 to 5-9-4	Тор	75 PLF	199 PLF	0 PLF	0 PLF	J1
15	Part. Uniform	5-1-10 to 5-9-4	Тор	96 PLF	255 PLF	0 PLF	0 PLF	J4
16	Part. Uniform	5-1-10 to 5-9-4	Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
17	Point	5-3-7	Тор	445 lb	1139 lb	0 lb	0 lb	вмз вмз
18	Point	5-3-10	Near Face	81 lb	215 lb	0 lb	0 lb	J8
	Self Weight			10 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

 1. LVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Wind

0

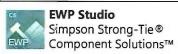
0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Page 1 of 2



Client: Project: Address: Date: 5/30/2018

Designer: SB Job Name: AMELIA 2 EL- 1

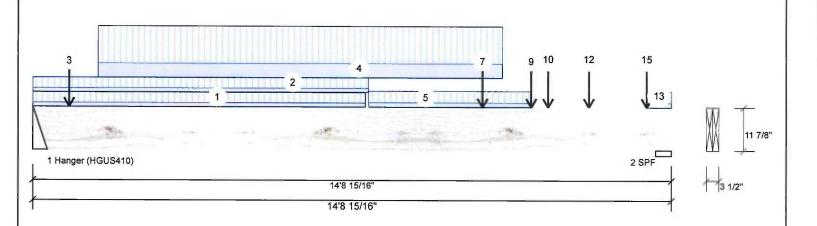
Project #:

Forex 2.0E-3000Fb LVL F9-A

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor



Unfactored Reactions UNPATTERNED Ib (Uplift) Туре: Girder Application: Floor (Residential) Live Dead Bra Snow Plies: Design Method: LSD 2798 1124 0 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2830 2 1139 0 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load Bearings and Factored Reactions Floor Live: 40 PSF Bearing Length Dead: **15 PSF** Cap. React D/L lb Total Ld. Case 1405 / 4197 4.000" 54% 5602 L Hanger

_							
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	20001 ft-lb	7'5 13/16"	34261 ft-lb	0.584 (58%)	1.25D+1.5L	L
l	Unbraced	20001 ft-lb	7'5 13/16"	24916 ft-lb	0.803 (80%)	1.25D+1.5L	L
l	Shear	5567 lb	13'5 7/16"	11596 lb	0.480 (48%)	1.25D+1.5L	L
l	Perm Defl in.	0.160 (L/1060)	7'4 7/8"	0.472 (L/360)	0.340 (34%)	D	Uniform
l	LL Defl inch	0.400 (L/426)	7'4 7/8"	0.472 (L/360)	0.850 (85%)	L	L
l	TL Defl inch	0.560 (L/304)	7'4 7/8"	0.709 (L/240)	0.790 (79%)	D+L	L

Design Notes

Analysis Results

Member Information

1 Fill all hanger nailing holes.

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

60%

1424 / 4245

5668 1

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

2 - SPF 4.376"

/ Laterar	ololidolilloss latto basca	on full scotlon width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 7-8-1	(Span)3-6-1 to 3-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 7-8-15	(Span)3-5-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-10-1		Near Face	107 lb	286 lb	0 lb	0 lb	J4
4	Part. Uniform	1-6-1 to 10-10-1		Near Face	93 PLF	247 PLF	0 PLF	0 PLF	
5	Tie-In	7-9-0 to 11-6-1	(Span)3-9-15 to 3-9-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	0								

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design critena and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and to Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-pty fastening details, beam strength values, and code

approves

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid

lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada



Jun 04% 2018



EL-MASRI

Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: Date:

5/30/2018 Designer:

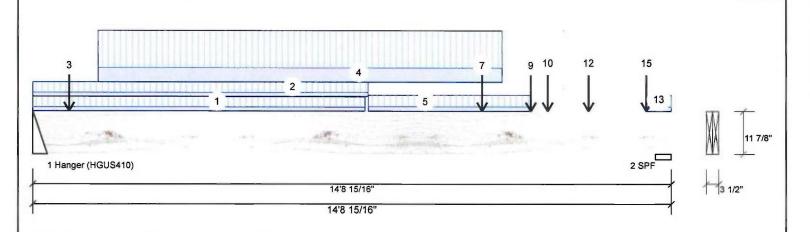
Job Name: AMELIA 2 EL- 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED

Level: Ground Floor



.Continued	from page 1						750		
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	10-4-9		Тор	13 lb	35 lb	0 lb	0 lb	
7	Point	10-4-9		Far Face	182 lb	462 lb	0 lb	0 lb	F5
8	Point	11-6-1		Far Face	32 lb	84 lb	0 lb	0 lb	J2
9	Point	11-6-1		Near Face	80 lb	213 lb	0 lb	0 lb	J4
10	Point	11-10-12		Near Face	72 lb	193 lb	0 lb	0 lb	F12
11	Point	12-10-1		Far Face	34 lb	92 lb	0 lb	0 lb	J2
12	Point	12-10-1		Near Face	94 lb	251 lb	0 lb	0 lb	J3
13	Tie-In	14-2-1 to 14-8-15	(Span)3-9-14 to 3-9-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
14	Point	14-2-1		Far Face	25 lb	67 lb	0 lb	0 lb	J2
15	Point	14-2-1		Near Face	91 lb	242 lb	0 lb	0 lb	J3
	Self Weight				10 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design cnterie and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, must-ply fastening details, beam strength values, and code approvals
 - approvais

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid

 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA; PR-L318









Client: Project: Address: Date:

5/30/2018 SB

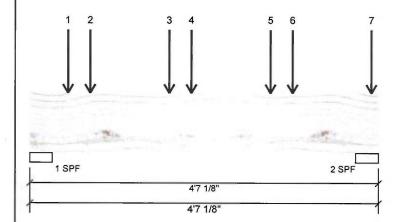
Designer; Job Name: AMELIA 2 EL- 1

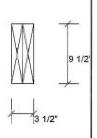
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED Level: Second Floor





Member Information Girder Type:

Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load 40 PSF Floor Live: Dead: 15 PSF Application: Design Method:

Building Code:

Deck:

Vibration:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

Load Sharing:

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1340	520	0	0
2	1139	445	0	0

Bearings and Factored Reactions

Bearing L	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1-SPF 3	3.516"	35%	650 / 2010	2660	L	1.25D+1.5L
2-SPF 3	3.625"	29%	556 / 1709	2265	L	1,25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2504 ft-lb	2'1 1/2"	22724 ft-lb	0.110 (11%)	1.25D+1.5L	L
Unbraced	2504 ft-lb	2'1 1/2"	22724 ft-lb	0.110 (11%)	1.25D+1.5L	L
Shear	1923 ib	3'6 3/4"	9277 lb	0.207 (21%)	1.25D+1.5L	L
Perm Defl in.	0.005 (L/10742)	2'1 9/16"	0.137 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.012 (L/4171)	2'1 9/16"	0.137 (L/360)	0.090 (9%)	L	L
TL Defi inch	0.016 (L/3005)	2'1 9/16"	0.206 (L/240)	0.080 (8%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size; beam
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS**



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	_
1	Point	0-6-0		Тор	162 lb	432 lb	0 lb	0 lb	J1	
2	Point	0-9-8		Тор	127 lb	338 lb	0 lb	0 lb	J4	
3	Point	1-10-0		Тор	162 lb	432 lb	0 lb	0 lb	J1	
4	Point	2-1-8		Тор	127 lb	338 lb	0 lb	0 lb	J4	
5	Point	3-2-0		Тор	162 lb	432 lb	0 lb	0 lb	J1	
Continued o	n page 2									

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

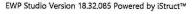
6. For flat roofs provide proper drainage to prevent

Manufacturer Info

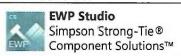
Forex APA: PR-L318







Page 2 of 2



Client: Project: Address:

5/30/2018 Date: Designer: S B

Job Name: AMELIA 2 EL- 1

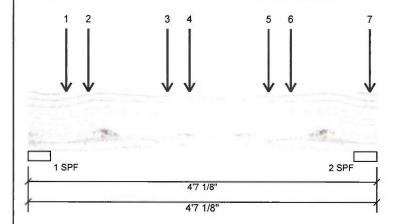
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



.. Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	3-5-8		Тор	127 lb	338 lb	0 lb	0 lb	J4
7	Point	4-6-0		Тор	63 lb	169 lb	0 lb	0 lb	J1
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- reasoning becaus, beam strength values, and code approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







Client: Project: Address: Date: 5/30/2018

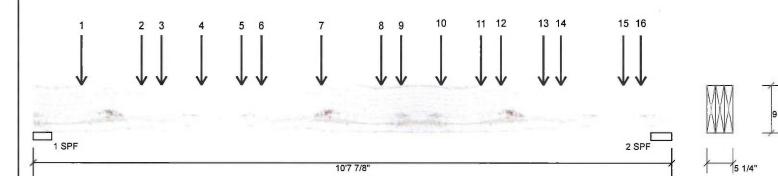
Designer: SB Job Name: AMELIA 2 EL- 1

Project #:

BM4-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

3-Ply - PASSED

Level: Second Floor



10'7 7/8"

Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Type: Girder Application: Floor (Residential) Live Dead Snow Wind Plies: Design Method: 2952 1269 0 0 1 NBCC 2010 / OBC 2012 Moisture Condition: Dry **Building Code:** 2 3096 1297 0 0 Deflection LL: 360 Load Sharing: Yes Deflection TL: Not Checked 240 Deck: Importance: Vibration: Not Checked Normal General Load Bearings and Factored Reactions Floor Live: 40 PSF 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.688" 50% 1586 / 4428 6014 L 1.25D+1.5L 2 - SPF 4.188" 46% 1621 / 4644 1.25D+1.5L 6265 L

Analysis Results

I	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
I	Moment	15176 ft-lb	4'9 11/16"	35449 ft-lb	0.428 (43%)	1.25D+1.5L	L
I	Unbraced	15176 ft-lb	4'9 11/16"	35449 ft-lb	0.428 (43%)	1.25D+1.5L	L
I	Shear	5599 lb	1' 7/16"	13915 lb	0.402 (40%)	1.25D+1.5L	L
I	Perm Defl in.	0.086 (L/1413)	5'3 9/16"	0.337 (L/360)	0.250 (25%)	D	Uniform
I	LL Defl inch	0.201 (L/605)	5'3 5/8"	0.337 (L/360)	0.600 (60%)	L	L
ı	TL Defl inch	0.287 (L/424)	5'3 5/8"	0.506 (L/240)	0.570 (57%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size; beam
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS**



Luterars	Edicial Sichachicss ratio based on rail section within									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Point	0-9-11		Тор	278 lb	686 lb	0 lb	0 lb	J1 J4	
2	Point	1-9-11		Тор	117 lb	257 lb	0 lb	0 lb	J4	
3	Point	2-1-11		Тор	162 lb	431 lb	0 lb	0 lb	J1	
4	Point	2-9-11		Тор	117 lb	257 lb	0 lb	0 lb	J4	
5	Point	3-5-11		Тор	162 lb	431 lb	0 lb	0 lb	J1	
continued on	page 2									

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forey





Page 2 of 2



Project: Address:

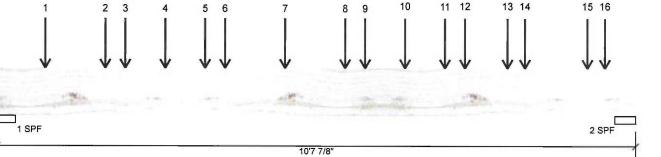
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5/30/2018 Date: Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

1.750" X 9.500" 3-Ply - PASSED Level: Second Floor



10'7 7/8"

	1
MM	9 1/2"
5 1/4	

Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	3-9-11		Тор	117 lb	257 lb	0 lb	0 lb	J4
7	Point	4-9-11		Тор	278 lb	686 lb	0 lb	0 lb	J1 J4
8	Point	5-9-11		Тор	117 lb	257 lb	0 lb	0 lb	J4
9	Point	6-1-11		Тор	162 lb	431 lb	0 lb	0 lb	J1
10	Point	6-9-11		Тор	117 lb	257 lb	0 lb	0 lb	J4
11	Point	7-5-11		Тор	144 lb	385 lb	0 lb	0 lb	J1
12	Point	7-9-11		Тор	116 lb	255 lb	0 lb	0 lb	J4
13	Point	8-6-3		Тор	144 lb	385 lb	0 lb	0 lb	J1
14	Point	8-9-11		Тор	122 lb	299 lb	0 lb	0 lb	J4
15	Point	9-10-3		Тор	163 lb	434 lb	0 lb	0 lb	J1
16	Point	10-1-11		Тор	128 lb	340 lb	0 lb	d10	J4
	Self Weight				11 PLF				
1									

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to venify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Andling & Installation

LVL beams must not be out or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top adge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318









Client: Project: Address:

5/30/2018 Date:

SB Designer:

Job Name: AMELIA 2 EL- 1

Project #:

F10-A Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor

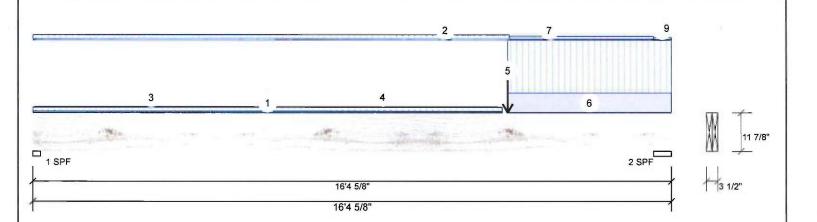
Unfactored Reactions UNPATTERNED Ib (Uplift)

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH

BLOCK IS REQUIRED AT ALL

POINT LOADS OVER BEARING



Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	818		446	0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	2494		1160	0	0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions		
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	2.375"	35%	557 / 1228	1785 L	1.25D+1.5L
				2-SPF	5.500"	44%	1450 / 3741	5191 L	1.25D+1.5L

Analysis Results

Member Information

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	15265 ft-lb	12'2 1/4"	34261 ft-lb	0.446 (45%)	1.25D+1.5L	L
l	Unbraced	15265 ft-lb	12'2 1/4"	22688 ft-lb	0.673 (67%)	1.25D+1.5L	L
l	Shear	4477 lb	15'	11596 lb	0.386 (39%)	1.25D+1.5L	L
l	Perm Defl in.	0.145 (L/1312)	8'10 3/4"	0.528 (L/360)	0.270 (27%)	D	Uniform
	LL Defl inch	0.290 (L/656)	9'	0.528 (L/360)	0.550 (55%)	L	L
1	TL Defl inch	0.435 (L/437)	8'11 9/16"	0.793 (L/240)	0.550 (55%)	D+L	L

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

6 Lateral slenderness ratio based on full section width. ID Load Type Location Trib Width Side Dead Live Snow

Wind Comment 0 PSF Tie-In 15 PSF 40 PSF 0 PSF 1 0-0-0 to 12-0-8 (Span) Top 0-10-11 40 PSF 0 PSF Tie-In 0-0-0 to 12-2-10 (Span)0-10-5 Top 15 PSF 0 PSF 2 3 Part. Uniform 0-0-9 to 11-11-1 2 PLF 0 PLF 0 PLF 0 PLF Top Tapered Start 0-0-10 Top 1 PLF 0 PLF 0 PLF 0 PLF 0 PLF End 11-11-1 0 PLF 0 PLF 0 PLF Point 12-2-4 Far Face 862 lb 1824 lb 0 lb 0 lb F8

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4

ED PROFESSIONAL

EL-MASRL





Continued from page 1

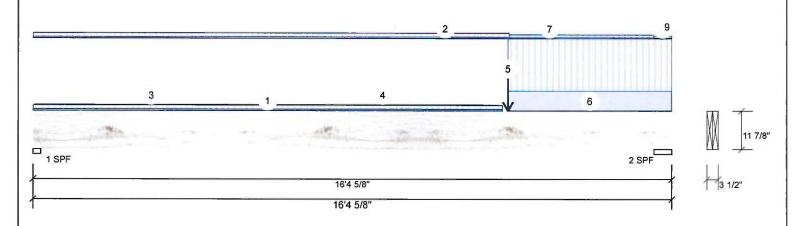
Client: Project: Address:

5/30/2018 Date: Designer: S B

Job Name: AMELIA 2 EL- 1

Project #:

Forex 2.0E-3000Fb LVL 2-Ply - PASSED Level: Second Floor F10-A 1.750" X 11.875"



ı	continued from p	age i								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	6	Part. Uniform	12-2-4 to 16-4-10		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
	7	Tie-In	12-2-10 to 15-11-2	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	8	Tie-In	15-11-2 to 16-4-10	(Span)0-4-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	9	Tie-In	16-0-4 to 16-4-10	(Span)0-11-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
		Self Weight				10 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design orteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

 LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-pily fastening details, beam strength values, and code resterning details, beam strength values, and code approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Wind 0 0

Ld. Comb. 1.25D+1.5L

1.25D+1.5L

SOPROFESSIONAL

EL-MASRI

Page 1 of 1



Client: Project: Address: Date: 5/30/2018

Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor

2 - SPF 5.500"

29%

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH

POINT LOADS OVER BEARINGS

0 PSF

0 PSF

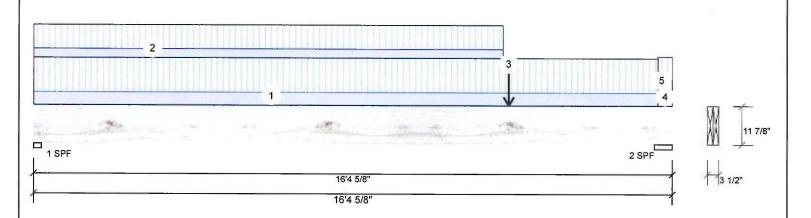
0 PSF

0 lb

BLOCK IS REQUIRED AT ALL

977 / 2443

3420 L



Member Inform	nation			Unfacto	red Reac	tions U	NPATTERN	ED lb (Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N
Plies:	2	Design Method:	LSD	1	700		368		0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	1629		782		0
Deflection LL:	360	Load Sharing:	No	-					•
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load						_			
Floor Live:	40 PSF			Bearings	and Fac	tored I	Reactions		
Dead:	15 PSF			Bearing			React D/L lb	Total	Ld. Case
				1 - SPF	2.375"	30%	460 / 1050	1511	L

Analysis Results

l	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	12574 ft-lb	12'2 1/4"	34261 ft-lb	0.367 (37%)	1.25D+1.5L	L
l	Unbraced	12574 ft-lb	12'2 1/4"	22688 ft-lb	0.554 (55%)	1.25D+1.5L	L
l	Shear	3351 lb	15'	11596 lb	0.289 (29%)	1.25D+1.5L	L
l	Perm Defl in.	0.118 (L/1617)	8'10 3/8"	0.528 (L/360)	0.220 (22%)	D	Uniform
	LL Defl inch	0.241 (L/788)	8'11 7/16"	0.528 (L/360)	0.460 (46%)	L	L
	TL Defl inch	0.359 (L/530)	8'11 1/16"	0.793 (L/240)	0.450 (45%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.

Tie-In

- 4 Top braced at bearings.

	braced at bearings. slenderness ratio based	on full section width.							(
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comment
1	Tie-In	0-0-0 to 16-0-4	(Span)0-11-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

(Span)0-8-0 Top

Point 3 12-2-4 Near Face 816 lb Tie-In 16-0-4 to 16-4-10 (Span)0-4-0 15 PSF Top 5 Tie-In 16-0-4 to 16-4-10 (Span)0-8-0 Top 15 PSF Self Weight 10 PLF

0-0-0 to 12-0-8

For flat roofs provide proper drainage to prevent ponding

15 PSF

40 PSF

1854 lb

40 PSF

40 PSF

Manufacturer Info Forex



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4

2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation

chemicals

- and ling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation



0 PSF

0 PSF

0 PSF

0 lb F8

Client: Project: Address:

5/30/2018 Date:

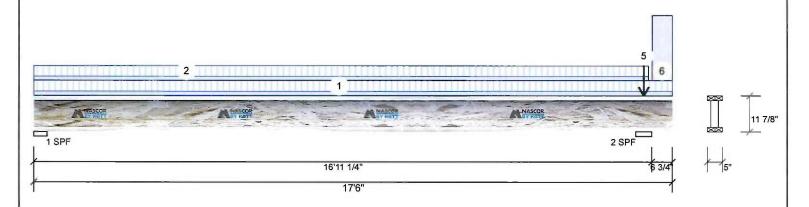
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" 2-Ply - PASSED F13-A NJH

Level: Second Floor



Member Information Type: Girder Application: Floor (Residential) Plies: 2 Design Method: Moisture Condition: Dry NBCC 2010 / OBC 2012 **Building Code:** Deflection LL: 360 Load Sharing: No Deflection TL: Not Checked Deck: 240 Importance: Not Checked Normal Vibration: General Load Floor Live: 40 PSF Dead: 15 PSF

Unfactored Reactions	UNPATTERNED Ib	(Uplift)
-----------------------------	----------------	----------

Brg	Live	Dead	Snow	Wind
1	224	82	0 (-2)	0
2	261	300	217	0
1				

Bearings and Factored Reactions

Posting Longth

bearing	Lengui	Cap.	React D/L ID	Iolai	Lu. Case	La. Comb.
1 - SPF	4.375"	12%	102 / 337	440	L_	1.25D+1.5L
2 - SPF	5.250"	24%	375 / 500	875	LL	1.25D+1.5L +0.5S

Can Boart D/Lih

Analysis Posults

•	tilalysis ites	uits					
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Neg Moment	-103 ft-lb	16'11 1/4"	9702 ft-lb	0.011 (1%)	1.25D+1.5S +0.5L	_L
	Unbraced	-103 ft-lb	16'11 1/4"	9651 ft-lb	0.011 (1%)	1.25D+1.5S +0.5L	_L
	Pos Moment	1711 ft-lb	8'4 1/2"	10780 ft-lb	0.159 (16%)	1.25D+1.5L	L_
	Unbraced	1711 ft-lb	8'4 1/2"	1717 ft-lb	0.996 (100%)	1.25D+1.5L	L_
	Shear	589 lb	16'11 1/4"	3186 lb	0.185 (18%)	1.25D+1.5S	_L
	Perm Defl in.	0.027 (L/7357)	8'4 1/4"	0.542 (L/360)	0.050 (5%)	D	Uniform
	LL Defl inch	0.076 (L/2581)	8'5 1/4"	0.542 (L/360)	0.140 (14%)	L	L_
	TL Defl inch	0.102 (L/1911)	8'4 15/16"	0.813 (L/240)	0.130 (13%)	D+L	L_
	LL Cant	-0.008 (2L/1791)	Rt Cant	0.200 (2L/480)	0.038 (4%)	L	L_
	TL Cant	-0.010 (2L/1395)	Rt Cant	0.300 (2L/360)	0.032 (3%)	D+L	<u>L_</u>

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS**



Total Id Coss Id Comb

PAGE 31 UF 30

Page 1 of 2

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 9'9" o.c.
- 5 Bottom flange must be laterally braced at a maximum of 10'5" o.c.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Libriting on Installation.

 Librit flanges must not be cut or drilled
 Refer to latest copy of the Librit product information
 details for framing details, sufferier tables, web hole
 chart, bridging details, multi-byl fastering details and
 handling/erection details.

 Damaged Librits must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

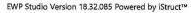
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







Client: Project: Address:

5/30/2018 Date:

Designer: SB

Job Name: AMELIA 2 EL- 1

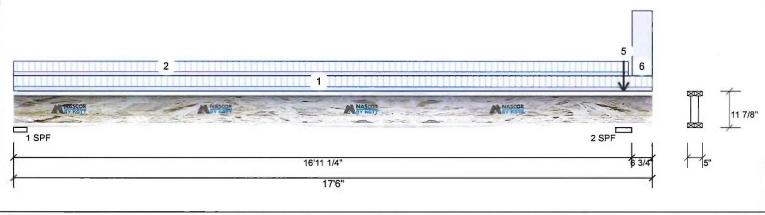
Project #:

F13-A 11.875" 2-Ply - PASSED NJH

Level: Second Floor

TAGE 32 OF 30

Page 2 of 2



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 17-6-0	(Span)0-8-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 16-10-2	(Span)0-7-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	16-8-10		Тор	35 lb	0 lb	0 lb	0 lb	Wall Self Weight
4	Point	16-8-10		Тор	126 lb	27 lb	215 lb	0 lb	F1 F1
5	Point	16-8-10		Тор	5 lb	0 lb	0 lb	0 lb	Wall Self Weight
6	Part. Uniform	16-11-6 to 17-6-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design ortheria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component sultability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

- | 5. Provide lateral support at bearing points to avoid lateral displacement and rotation |
 | 1. Uoist flanges must not be cut or drilled |
 | 2. Refer to latest copy of the Uoist product Information details for framing details, stiffener tables, web hole chart, bridging details, must-ply fastening details and handling/erection details |
 | 2. Demaged Uoists must not be used |
 | 3. Demaged Uoists must not be used |
 | 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
 | 5. Provide lateral support at bearing points to avoid lateral displacement and rotation |
 | 6. The provide proper drainage to prevent pointing to a provide proper drainage to prevent ponding |
 | 7. For flat rods provide proper drainage to prevent ponding |
 | 8. Installation |
 | 9. Provide lateral support at bearing points to avoid lateral displacement and rotation |
 | 9. Provide lateral support at bearing points to avoid lateral displacement and rotation |
 | 9. Provide lateral support at bearing points to avoid lateral displacement and rotation |
 | 9. Provide lateral support at bearing points to avoid lateral displacement and rotation |
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 | 9. Provide lateral support at bearing points to avoid |
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 | 9. Provide lateral support at bearing rotation |
 | 9. Provide lateral support at bearing rotation |
 | 9. Provide lateral support at lateral support |
 | 9. Provide lateral support at lateral support |
 | 9. Provide lateral support |
 | 9

Manufacturer Info

Nascor by Kott







Client: Project: Address:

5/30/2018 Date:

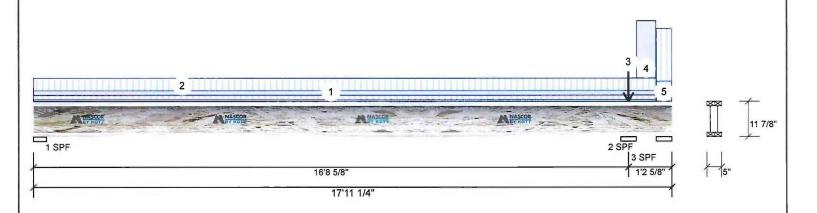
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" 2-Ply - PASSED F13-B NJH

Level: Second Floor



3

Member Inform	nation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		
		1	

Unfacto	red Reaction	S UNPATTER	NED lb (Uplif	t)	
Brg	Live	Dead	Snow	Wind	_
1	171	64	0	0	
2	764	335	0	0	

0

0

(-160)

Bearings and Factored Reactions Cap. React D/L lb Bearing Length Total Ld. Case Ld. Comb. 1 - SPF 4.375" 9% 80 / 257 337 L_ 1.25D+1.5L 23% 2 - SPF 5.250" 419 / 1146 1565 LL 1.25D+1.5L -961 L_ 1.25D+1.5L 3 - SPF 5.250" 27% -224 / -737 (-961)

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-889 ft-lb	16'8 5/8"	10780 ft-lb	0.082 (8%)	1.25D+1.5L	LL
Unbraced	-889 ft-lb	16'8 5/8"	10039 ft-lb	0.089 (9%)	1.25D+1.5L	LL
Pos Moment	1135 ft-lb	7'4"	10780 ft-lb	0.105 (11%)	1.25D+1.5L	L_
Unbraced	1135 ft-lb	7'4"	1139 ft-lb	0.997 (100%)	1.25D+1.5L	L_
Shear	1095 lb	16'8 5/8"	3620 lb	0.302 (30%)	1.25D+1.5L	LL
Perm Defl in.	0.018 (L/11068)	8' 5/16"	0.547 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.047 (L/4149)	8' 5/16"	0.547 (L/360)	0.090 (9%)	L	L_
TL Defl inch	0.065 (L/3018)	8' 5/16"	0.821 (L/240)	0.080 (8%)	D+L	L_

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

0 (-494)

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Tie-down connection required at bearing 3 for uplift 961 lb (Combination 1.25D+1.5L, Load Case L).
- 5 Top flange must be laterally braced at a maximum of 11'6" o.c.
- 6. Bottom flange must be laterally braced at a maximum of 10'5" o.c.

				-,						
Γ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 17-6-0	(Span)0-3-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Tie-In	0-0-0 to 17-6-0	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ľ	3	Point	16-8-10		Тор	31 lb	0 lb	0 lb	0 lb	Wall Self Weight

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- Lioist flanges must not be out or drilled
 Refer to latest copy of the Lioist product information details for framing details, suffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum
- point load bearing length>= 3.5 inches For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



EOPROFESSIONA,



Client: Project: Address:

5/30/2018 Date: Designer: SB

Job Name: AMELIA 2 EL- 1

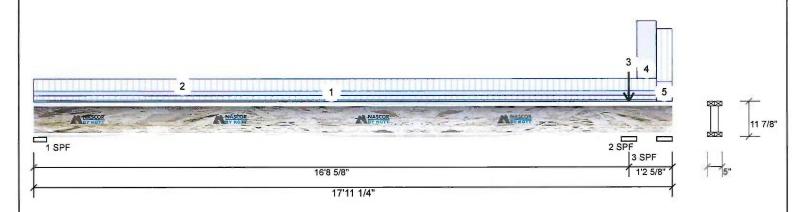
Project #:

2-Ply - PASSED 11.875" F13-B NJH

Level: Second Floor

FAGE 34 UF 30

Page 2 of 2



..Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
4	Part. Uniform	16-11-6 to 17-5-14		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
5	Tie-In	17-6-0 to 17-11-4	(Span)3-8-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Lidist flanges must not be cut or drilled
 Refer to latest copy of the Lidist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged Lidisk must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length ≥ 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott





Client: Project: Address:

5/30/2018 Date:

Designer: SB

Job Name: AMELIA 2 EL- 1

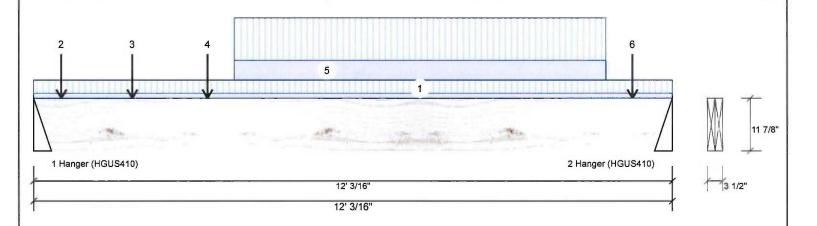
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor



Member Information Type: Girder Application: Floor (Residential) Plies: 2 Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: Load Sharing: 360 No Not Checked Deflection TL: 240 Deck: Importance: Normal Vibration: Not Checked General Load Floor Live: 40 PSF Dead: 15 PSF

Unfacto	Unfactored Reactions UI		NED lb (Uplif	t)
Brg	Live	Dead	Snow	Wind
1	1854	816	0	0

862

1824

Bearing Length

4.000"

4.000"

1 -

2 -

Hanger

Hanger

Bearings and Factored Reactions	

Cap. React D/L lb

37% 1078 / 2736

1021 / 2780

0

Total Ld. Case

3801 L

3814 L

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10792 ft-lb	6' 7/16"	34261 ft-lb	0.315 (31%)	1.25D+1.5L	L
Unbraced	10792 ft-lb	6' 7/16"	28134 ft-lb	0.384 (38%)	1.25D+1.5L	L
Shear	3604 lb	1'3 1/8"	11596 lb	0.311 (31%)	1.25D+1.5L	L
Perm Defl in.	0.065 (L/2121)	6' 7/16"	0.383 (L/360)	0.170 (17%)	D	Uniform
LL Defl inch	0.140 (L/983)	6' 1/8"	0.383 (L/360)	0.370 (37%)	L	L
TL Defl inch	0.205 (L/671)	6' 1/4"	0.574 (L/240)	0.360 (36%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

37%

REFER TO MULTIPLE MEMBER TO MEMBE CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

PROFESSIONA

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Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

/ Lateral	siendemess ratio based o	on rull section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-0-3	(Span)3-9-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-6-4		Near Face	90 lb	240 lb	0 lb	0 lb	J4
3	Point	1-10-4		Near Face	124 lb	330 lb	0 lb	0 lb	J4
4	Point	3-3-4		Near Face	109 lb	290 lb	0 lb	0 lb	J4
5	Part. Uniform	3-9-4 to 10-9-4		Near Face	113 PLF	240 PLF	0 PLF	0 PLF	
6	Point	11-3-4		Near Face	109 lb	227 lb	0 lb	0 lb	J4
	Self Weight				10 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply

regarding installation requirements, multi-ply fastening details, beam strength values, and code Damaged Beams must not be used

Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding



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