

THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Engineered floor joists shall be installed in accordance with the supplier's layout and specifications forming part of the permit drawings.

abelia conform to the Ontario be any Code D. Rep. 332/12 as amended

LVL/LSL Qty Plies Label Description Width | Depth | F12 Forex 1.75 11.875 2 2.0E-3000Fb LVL F9 1.75 11.875 2.0E-3000Fb LVL F8 Forex 11.875 1.75 2.0E-3000Fb LVL F7 Forex 1.75 11.875 2.0E-3000Fb LVL Joist Label Description Width Depth Qty Plies 3.5 11.875 J8 NJ60U J7 NJ60U 3.5 11.875 F14 NJH 2.5 11.875 F13 NJH 2.5 11.875 J11 NJH 2.5 11.875 J5 NJH 2.5 11.875 J4 NJH 2.5 11.875 J3 NJH 2.5 11.875 J2 NJH 2.5 11.875 J1 NJH 2.5 11.875 Rim Board Label Description Width | Depth Qty Plies 1.125 11.875 Norbord Rimboard Plus 1.125 X Blocking Width Depth Label Description Qty Plies BLK1 NJH 2.5 11.875 LinFt Hanger Beam/Girder Pcs Description Skew Slope fasteners Label 2 HUS1.81/10 19 LF2511 H4 12 10d 2 LF3511 12 10dx1 1/2 Н6 1 LF2511 2 HUS1.81/10 30 10dx1 1/2 H8 Framer to verify dimensions on the architectural drawings. member using a face-mounted hanger. Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls. Refer to Nascor specifier guide for installation details. joists which support loading from above exceeding two levels floor or roof. Load transfer blocks to be installed under all point loads.

- 2. Double joist only require filler/backer ply when supporting another
- Install single-ply flush window header along inside face of rimboard/rimjoist
- Squash blocks recommended to be installed at end bearing on all first level
- 3. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth) @ 16" o/c. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

Hatch area represents ceramic tiled floor with an additional dead load of 5 PSF.

The framing shown on this layout may be deviate from the architectural drawings. Project Engineer to review and approve the deviation prior to construction.

Deflection Joist LL Span L/ 480 TL Span L/ 360 480 LL Cant 2L/ 360 TL Cant 2L/ **Deflection Girder** LL Span L/ 360 240 TL Span L/ 480 LL Cant 2L/ 360 TL Cant 2L/ Decking Deck **OSB** Thickness 3/4" Nailed & Glued Fastener Vibration Ceiling: Gypsum 1/2" M-2057

Legend PS Point Load Support Load from Above Norbord Rimboard Plus 1.125 X 11.875 NJ60U 11.875 NJH 11.875

/////

Architectural Drawing Info

JARDIN DESIGN GROUP 64 JARDIN DR, SUITE 3A VAUGHAN, ON L4K 3P3 Project # 17-55 Model: AMELIA 1

Pcs Length

12-0-0

8-0-0

6-0-0

4-0-0

Length

20-0-0

18-0-0

4-0-0

16-0-0

14-0-0

12-0-0

10-0-0

8-0-0

6-0-0

20-0-0

Layout Name

Design Method

Description

May 25, 2018

Created

Builder

Sales Rep

Designer

Shipping

Canada L4A 7X4

Hoor

oads

Dead

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

905-642-4400

Ground Floor esign Method

Kott Lumber Company

Building Code NBCC 2010 / OBC

LSD

2012

40

15

Project

LSD

AMELIA 1 EL-1 & 2

GREEN YORK HOMES

2

3

1

Pcs

25

2

4

5

8

9

4

6

Pcs Length

Pcs Length

Varies 46-0-0

Supported

Member

fasteners

1 #8x1 1/4WS

2 #8x1 1/4WS

Date: MAY 22,2018 REV: 2 Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R

JOISTS SPACING 16"O/C

NOTED OTHERWISE

- 3. LVL CCMC -12904-R
- 4. CAN/CSA-O86-09

UNLESS

5. CCMC -12787-R APA PR-L310(C)



EWP Studio

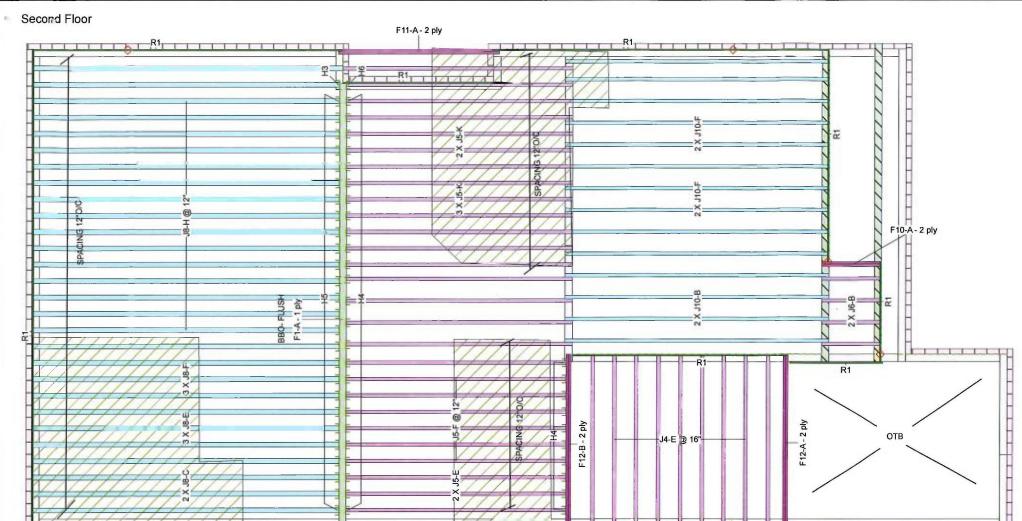
Simpson Strong-Tie® Component Solutions™

EWP Studio Version 18.32.085 Powered by iStruct™

19-444365.000.00 RR.

5.25 X 10.25 (Dropped)







CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

BLOCK IS REQUIRED AT ALL **POINT LOADS OVER BEARINGS**

REFER TO MULTIPLE MEMBER TO MEMBER

PASS THRU FRAMING SQUASH

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

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| | | | -7.5 | | | | 0 | | | FAGE 10 UF | ۷۱ |
|--------------------|-------------------|---------------------------------|------------|---------|-----------|----------------|---------------|-------------|----------|--|------|
| Secono | d Floor | | | | | | _ | | | | |
| .VL/LS | _ | | | | | | | | | ALACCI | |
| | Descr | iption | Widt | _ | epth | Qty | Plies | Pcs | Length | NASC | |
| F12 | Forex 2.0E-3 | 000Fb LVL | 1.7 | 5 1 | 1.875 | 2 | 2 | 4 | 12-0-0 | | |
| F11 | Forex 2.0E-3 | 000Fb LVL | 1.7 | 5 1 | 1.875 | 1 | 2 | 2 | 10-0-0 | Layout Name AMELIA 1 EL-1 & 2 | |
| F10 | Forex 2.0E-3 | 000Fb LVL | 1.7 | 5 1 | 1.875 | 1 | 2 | 2 | 4-0-0 | Design Method | |
| Joist | | | | | | | v v | | | | |
| Label | Descr | iption | Widt | h [| epth | Qty | Plies | Pcs | Length | Description | |
| J10 | NJ60H | | 2. | 5 1 | 1.875 | | | 14 | 18-0-0 | GREEN YORK HOMES | |
| J8 | NJ60U | | 3. | 5 1 | 1.875 | | | 28 | 20-0-0 | Created | |
| J11 | NJH | | 2. | 5 1 | 1.875 | | | 1 | 16-0-0 | May 25, 2018 | |
| J5 | NJH | | 2. | | 1.875 | | | 26 | 14-0-0 | Builder | |
| J4 | NJH | | 2. | - | 1.875 | | | 9 | 12-0-0 | Calaa Ban | |
| J6_ | NJH | | 2. | 5 1 | 1.875 | | | 4 | 4-0-0 | Sales Rep | |
| Rim Bo | ~ | | | | | | 1 1 | | | Designer | |
| | Descr | | Widt | | epth | Qty | Plies | Pcs | Length | SB | |
| R1 | Norbor Plus 1. | d Rimboard | 1.12 | 5 1 | 1.875 | | | 14 | 12 | Shipping | |
| | 11.875 | | | | | | | | | Project | |
| langer | | | | | | | | | | Builder's Project | |
| | | | | | | В | eam/Girder | Sur | ported | Kott Lumber Comp | 2001 |
| | | | | | | | | | ember | The state of the s | ally |
| Label | Pcs | Descriptio | n | Skev | v Slo | pe | fasteners | fas | teners | 14 Anderson Blvd | |
| НЗ | 1 | HUC410 (M | 1in) | 17.0 | | | 14 16d | 6 | 3 10d | Stouffville, Ontario | |
| H4 | 35 | LF2511 | | | | | 12 10d | 1 #8> | <1 1/4WS | Canada | |
| H5 | 26 | LF3511 | | | | | 12 10d | 2 #8> | (1 1/4WS | L4A 7X4 | |
| H6 | 1 | LF2511 | | | | | | | | 905-642-4400 | |
| OTES: | | | | | | | | | | Second Floor | |
| _ | | | | | | and the second | | | | Design Method | LSD |
| | | rify dimensionally require t | | | | | | r | 7 | Building Code NBCC 20 | |
| | | g a face-mou | | | | i suppo | rung anounc | • | | | 2012 |
| | | ocking @ 24 | | | | | | | S 1 | Floor | |
| | | -ply flush win cor specifier | | | | | | rd/rimjoi: | st | Loads | 40 |
| | | ks recomme | | | | | | I first lev | el | Live | 40 |
| joists | which s | upport loadir | ng from | abov | e excee | ding tw | o levels floo | | | Dead | 15 |
| | | r blocks to be | | | | | | | | Deflection Joist | 400 |
| | | e framer's res | | | | | | | | LL Span L/ | 480 |
| alt la | sieneu i | as per the ha | inger n | ianuia | cluier | stanue | iius. | | | TL Span L/ | 360 |
| Refer to | Multiple | Member Co | nnectio | n Det | ail to pl | to ply | nailing or bo | lting | - 1 | LL Cant 2L/ | 480 |
| equirem | ents. | | | | | | | | | TL Cant 2L/ | 360 |
| im nam | llel to io | ists: 1-1/8" r | imboar | d with | | | | | | Deflection Girder | |
| | | " longer than | | | | /c. | | | | LL Span L/ | 360 |
| | | ents and str | | | | | | | - 1 | TL Span L/ | 240 |
| | | such as bea | | | | | | | - | LL Cant 2L/ | 480 |
| | | and footings bracing for I | | | | | | | | TL Cant 2L/ | 360 |
| ompone esponsil | | | alc/di S | الالمام | y are li | C | | | | Decking | |
| 4 | | | | | | | | | | Deck | OSB |
| latala co | | | in Aller d | n | | 1.1545 | . 1 . 4 1 1 | | | | |

the Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may be deviate from the architectural drawings. Project Engineer to review and approve the deviation prior

Legend Architectural Drawing Info Point Load Support JARDIN DESIGN GROUP Load from Above 64 JARDIN DR, SUITE 3A Wall VAUGHAN, ON L4K 3P3 Norbord Rimboard Plus 1.125 X 11.875 Project # 17-55 NJ60H 11.875 Model: AMELIA 1 Date: MAY 22,2018 REV : 2 NJ60U 11.875 NJH 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875 5 X 10.25 5.25 X 10.25 (Dropped)

> M-2057 LOT 3

Thickness

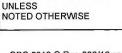
Fastener

Vibration

Ceiling:







JOISTS SPACING 16"O/C

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)



EWP Studio

Simpson Strong-Tie® Component Solutions™ 5/8"

Nailed & Glued

Gypsum 1/2"

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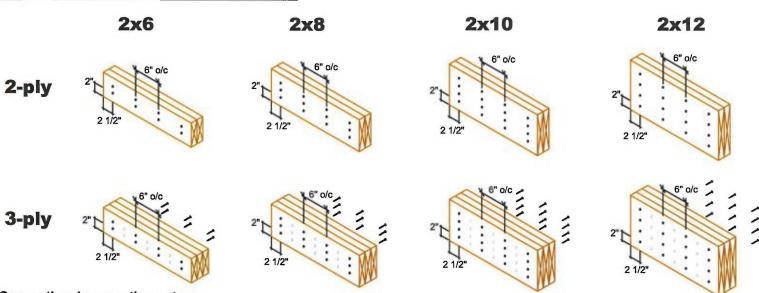
PAGE 2 OF 21

5

MULTIPLE MEMBER CONNECTIONS

GREEN YORK HOMES-BRAMPTON-ON-AMELIA 1 ELE-1-2

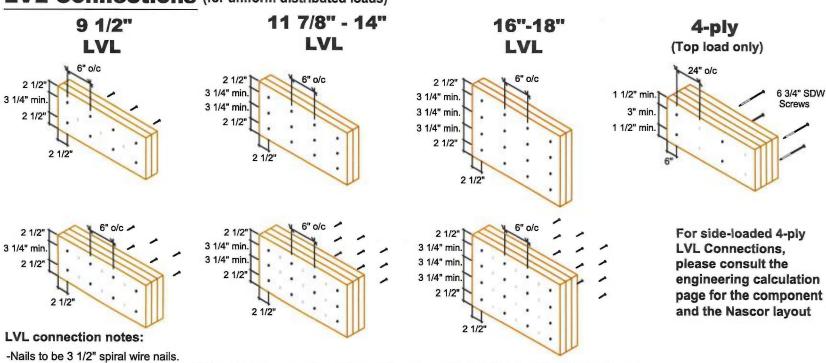
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

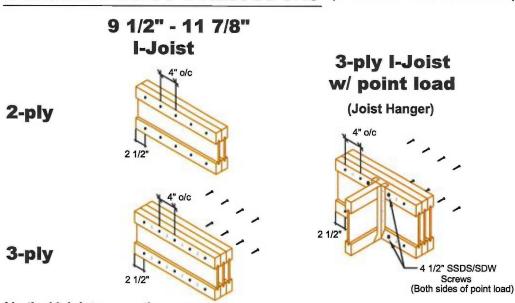
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI -PLY CONNECTION DETAILS

Date: November 30, 2016 Scale: NTS KOTT 3228 Moodie Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775 Fx: 613-838-4751

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

GREEN YORK HOMES-BRAMPTON-ON-AMELIA 1 ELE-1-2

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



| , Lateral | Sicride mess ratio based | Off full Section with. | | | | | | | |
|-----------|--------------------------|------------------------|--------------|----------|---------|---------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 10-3-12 | (Span)1-2-12 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Part. Uniform | 0-1-3 to 10-1-3 | | Тор | 3 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 3 | Point | 1-1-13 | | Far Face | 99 lb | 265 lb | 0 lb | 0 lb | J5 |
| 4 | Point | 1-10-13 | | Far Face | 106 lb | 283 lb | 0 lb | 0 lb | J5 |
| 5 | Part. Uniform | 2-6-13 to 10-4-14 | | Far Face | 104 PLF | 277 PLF | 0 PLF | 0 PLF | |
| | | | | | | | | | |

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer info

Forex

APA: PR-L318





Part. Uniform 9-11-6 to 10-4-14 Top 80 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight 8 Part. Uniform 10-1-3 to 10-2-4 Top 3 PLF 0 PLF 0 PLF 0 PLF 9 Point 10-2-2 Top 738 lb 1580 lb 0 lb 0 lb F12 F12 10 Part. Uniform 10-4-5 to 10-4-14 Top 106 PLF 246 PLF 0 PLF 0 PLF Self Weight 10 PLF

> REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

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Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
 - LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex









Client: Project: Address: Date: 5/30/2018

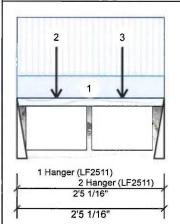
SB Designer:

Job Name: AMELIA 1 EL-1 & 2

Project #:

F13-A 11.875" - PASSED NJH

Level: Ground Floor



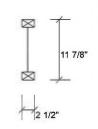
Mombar Information



Unfactored Reactions UNPATTERNED Ib (Uplift)

142

139



Wind

0

0

0

Page 1 of 1

| Туре: | Girder | Application: | Floor (Residential) |
|--------------------|---------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition | on: Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |

| Bearings | and | Factored | Reactions |
|----------|-----|----------|-----------|

Live

380

372

1

2

| bearing: | earings and Factored Reactions | | | | | | | | | | | |
|---------------|--------------------------------|------|--------------|-------|----------|------------|--|--|--|--|--|--|
| Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. | | | | | | |
| 1 - Hanger | 2.000" | 46% | 178 / 570 | 748 | L | 1.25D+1.5L | | | | | | |
| 2 - Hanger | 2.000" | 45% | 174 / 558 | 732 | L | 1.25D+1.5L | | | | | | |

Analysis Actual Location Allowed Capacity Comb. Case Moment 415 ft-lb 1'5 7/16" 5390 ft-lb 0.077 (8%) 1.25D+1.5L L Unbraced 415 ft-lb 1'5 7/16" 5079 ft-lb 0.082 (8%) 1.25D+1.5L L Shear 742 lb 1 1/4" 1810 lb 0.410 (41%) 1.25D+1.5L L Perm Defl in. 0.002 1'3 1/2" 0.074 (L/360) 0.020 (2%) D Uniform (L/16731) LL Defi inch 0.004 (L/6272) 1'3 7/16" 0.074 (L/360) 0.060 (6%) L L

1'3 7/16" 0.111 (L/240) 0.050 (5%) D+L

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Design Notes

Analysis Results

1 Fill all hanger nailing holes.

TL Defl inch 0.006 (L/4562)

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

| l | ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments | |
|---|----|-----------|----------------|-------------|-----------|--------|--------|-------|-------|----------|--|
| l | 1 | Tie-In | 0-0-0 to 2-5-1 | (Span)1-7-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | |
| | 2 | Point | 0-7-13 | | Near Face | 125 lb | 334 lb | 0 lb | 0 lb | J7 | |
| | 3 | Point | 1-8-13 | | Near Face | 128 lb | 341 lb | 0 lb | 0 lb | J7 | |
| | | | | | | | | | | | |

L

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

- Handling & Installation
 Installation
 Indist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastering details and handling/erection details
 Damaged Lioists must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info Nascor by Kott





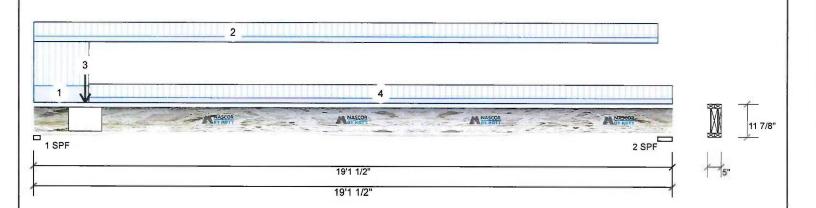
Project: Address: Date: 5/30/2018

Designer: SB

Job Name: AMELIA 1 EL-1 & 2

Project #:

11.875" F14-A NJH 2-Ply - PASSED Level: Ground Floor



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Girder Type: Application: Floor (Residential) Dead Plies: 2 Design Method: 745 279 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 362 136 Deflection LL: Load Sharing: Deflection TL: Deck: Not Checked Importance: Normal Vibration: Not Checked General Load Floor Live: 40 PSF Bearings and Factored Reactions 15 PSF Dead:

| Bearing Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. | |
|----------------|------|--------------|-------|----------|------------|--|
| 1 - SPF 2.375" | 44% | 348 / 1117 | 1465 | L | 1.25D+1.5L | |
| 2 - SPE 5 250" | 20% | 170 / 543 | 713 | L | 1.25D+1.5l | |

Analysis Results

| Γ | Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---|---------------|----------------|------------|---------------|-------------|------------|---------|
| l | Moment | 3582 ft-lb | 8'6 5/8" | 10780 ft-lb | 0.332 (33%) | 1.25D+1.5L | L |
| l | Unbraced | 3582 ft-lb | 8'6 5/8" | 3606 ft-lb | 0.993 (99%) | 1.25D+1.5L | L |
| l | Shear | 1445 lb | 1 5/8" | 3620 lb | 0.399 (40%) | 1.25D+1.5L | L |
| l | Perm Defl in. | 0.076 (L/2921) | 9'1 15/16" | 0.620 (L/360) | 0.120 (12%) | D | Uniform |
| l | LL Defl inch | 0.204 (L/1095) | 9'1 15/16" | 0.620 (L/360) | 0.330 (33%) | L | L |
| l | TL Defl inch | 0.280 (L/796) | 9'1 15/16" | 0.931 (L/240) | 0.300 (30%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'1" o.c.

| 5 Bottom f | lange braced at bearing | IS. | | | | | | | |
|------------|-------------------------|------------------|-------------------|----------|--------|--------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 1-7-12 | (Span)2-10-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 18-8-4 | (Span) 0-10-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 1-6-8 | | Far Face | 142 lb | 380 lb | 0 lb | 0 lb | F13 |
| 4 | Tie-In | 1-7-12 to 19-1-8 | (Span)0-10-5 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |



PAGE / UF ZI

Wind

0

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Page 1 of 1

Calculated Structured Designs is responsible only of the Calculated Structure Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation

 Loist flanges must not be cut or drilled
 Refer to latest copy of the Joist product information
 details for framing details, stiffener tables, web hote
 chart, bridging details, multi-hy fastening details and
 handling/erection details
 Demaged Joists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





Client: Project: Address: Date: 5/30/2018

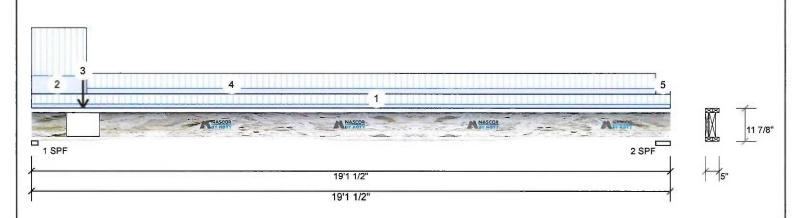
Designer:

Job Name: AMELIA 1 EL-1 & 2

Project #:

11.875" 2-Ply - PASSED F14-B NJH

Level: Ground Floor



| Member Inform | nation | | | Unfacto | red React | ions U | NPATTERN | ED Ib (| Uplift) |
|---------------------|--------|----------------|----------------------|----------|-----------|---------|--------------|---------|-----------------|
| Туре: | Girder | Application: | Floor (Residential) | Brg | Live | | Dead | Snov | W |
| Plies: | 2 | Design Method: | LSD | 1 | 689 | | 258 | | 0 |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 317 | | 119 | | 0 |
| Deflection LL: | 360 | Load Sharing: | No | N-2- | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | |
| General Load | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings | and Fact | tored I | Reactions | | |
| Dead: | 15 PSF | | | Bearing | Length | Сар. | React D/L lb | Total | Ld. Case |
| | | | | 1 - SPF | 2.375" | 41% | 322 / 1033 | 1355 | L |
| | | | | 2 - SPF | 5.250" | 17% | 148 / 475 | 623 | L |

Analysis Results

| Analysis Actual Location Allowed Capacity Comb. Case Moment 3147 ft-lb 8'5 1/8" 10780 ft-lb 0.292 (29%) 1.25D+1.5L L Unbraced 3147 ft-lb 8'5 1/8" 3159 ft-lb 0.996 (100%) 1.25D+1.5L L Shear 1337 lb 1 5/8" 3620 lb 0.369 (37%) 1.25D+1.5L L Perm Defl in. 0.067 (L/3321) 9'1 5/8" 0.620 (L/360) 0.110 (11%) D Uniform LL Defl inch 0.179 (L/1244) 9'1 5/8" 0.620 (L/360) 0.290 (29%) L L TL Defl inch 0.247 (L/905) 9'1 5/8" 0.931 (L/240) 0.270 (27%) D+L L | | | | | | | | |
|---|---|---------------|----------------|----------|---------------|-------------|------------|---------|
| Unbraced 3147 ft-lb 8'5 1/8" 3159 ft-lb 0.996 (1.25D+1.5L (100%)) L 25D+1.5L L (100%) Shear 1337 lb 1 5/8" 3620 lb 0.369 (37%) 1.25D+1.5L L L Perm Defl in. 0.067 (L/3321) 9'1 5/8" 0.620 (L/360) 0.110 (11%) D Uniform LL Defl inch 0.179 (L/1244) 9'1 5/8" 0.620 (L/360) 0.290 (29%) L L | Γ | Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
| (100%) Shear 1337 lb 1 5/8" 3620 lb 0.369 (37%) 1.25D+1.5L L Perm Defl in. 0.067 (L/3321) 9'1 5/8" 0.620 (L/360) 0.110 (11%) D Uniform LL Defl inch 0.179 (L/1244) 9'1 5/8" 0.620 (L/360) 0.290 (29%) L L | l | Moment | 3147 ft-lb | 8'5 1/8" | 10780 ft-lb | 0.292 (29%) | 1.25D+1.5L | L |
| Perm Defl in. 0.067 (L/3321) 9'1 5/8" 0.620 (L/360) 0.110 (11%) D Uniform LL Defl inch 0.179 (L/1244) 9'1 5/8" 0.620 (L/360) 0.290 (29%) L L | | Unbraced | 3147 ft-lb | 8'5 1/8" | 3159 ft-lb | | 1.25D+1.5L | L |
| LL Defl inch 0.179 (L/1244) 9'1 5/8" 0.620 (L/360) 0.290 (29%) L L | l | Shear | 1337 lb | 1 5/8" | 3620 lb | 0.369 (37%) | 1.25D+1.5L | L |
| | l | Perm Defl in. | 0.067 (L/3321) | 9'1 5/8" | 0.620 (L/360) | 0.110 (11%) | D | Uniform |
| TL Defl inch 0.247 (L/905) 9'1 5/8" 0.931 (L/240) 0.270 (27%) D+L L | l | LL Defl inch | 0.179 (L/1244) | 9'1 5/8" | 0.620 (L/360) | 0.290 (29%) | L | L |
| | l | TL Defl inch | 0.247 (L/905) | 9'1 5/8" | 0.931 (L/240) | 0.270 (27%) | D+L | L |

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'6" o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

| ID | Load Tuna | Location | Trib Width | Side | Dood | Live | Cnow | Wind | Commonto |
|-----|-----------|------------------|-------------------|-----------|--------|--------|-------|--------|----------|
| יוו | Load Type | Location | THE WIGHT | Side | Dead | Live | Snow | vviria | Comments |
| 1 | Tie-In | 0-0-0 to 19-1-8 | (Span)0-7-4 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 1-7-12 | (Span)2-10-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 1-6-8 | | Near Face | 139 lb | 372 lb | 0 lb | 0 lb | F13 |
| 4 | Tie-In | 1-7-12 to 18-8-4 | (Span) 0-10-12 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 5 | Tie-In | 18-8-4 to 19-1-8 | (Ѕрап)0-4-12 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 1 | | | | | | | | | |



Wind

Case

0

Ld. Comb. 1.25D+1.5L 1.25D+1.5L

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- arruding & Installation

 Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole chart, bridging details, stiffener tables, web hole chart, bridging details, multiply fastening details and handling/erecton details

 Damaged Joists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length ≥ 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







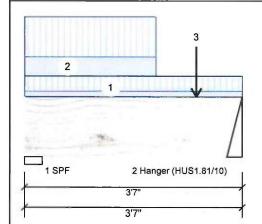
Client: Project: Address: 5/30/2018

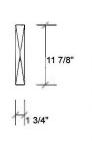
Designer: SB Job Name: AMELIA 1 EL-1 & 2

Project #:

F7-A Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

Level: Ground Floor





Wind

0

0

Page 1 of 1

| Туре: | Girder | Application: | Floor (Residential) | E |
|---------------------|--------|----------------|----------------------|---|
| Plies: | 1 | Design Method: | LSD | ı |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 | ı |
| Deflection LL: | 360 | Load Sharing: | No | l |
| Deflection TL: | 240 | Deck: | Not Checked | l |
| Importance: | Normal | Vibration: | Not Checked | ı |
| General Load | | | | F |
| Floor Live: | 40 PSF | | | E |
| Dead: | 15 PSF | | | r |
| | 200 | | | ı |

Location Allowed

2'4 7/8" 5798 lb

1'9" 17130 ft-lb

1'9" 13404 ft-lb

1'9 5/16" 0.105 (L/360) 0.010 (1%) D

1'9 5/16" 0.105 (L/360) 0.010 (1%) L

1'9 5/16" 0.158 (L/240) 0.010 (1%) D+L

Bearings and Factored Reactions

Live

254

207

| Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. | |
|---------|--------|------|--------------|-------|----------|------------|--|
| 1 - SPF | 3.500" | 14% | 155 / 381 | 535 | L | 1.25D+1.5L | |
| 2 - | 3.000" | 11% | 127 / 311 | 438 | L | 1.25D+1.5L | |

1

2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Unfactored Reactions UNPATTERNED Ib (Uplift)

124

102

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Analysis Results

Actual

354 ft-lb

354 ft-lb

(L/50089)

557 lb

Analysis

Moment

Shear

Unbraced

(L/24524) TL Defl inch 0.002 (L/16463)

Perm Defl in. 0.001

LL Defl inch 0.002

Member Information

Design Notes

1 Fill all hanger nailing holes.

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

| l | ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|---|----|---------------|-----------------|------------|----------|--------|---------|-------|-------|----------|
| l | 1 | Part. Uniform | 0-0-0 to 3-7-0 | | Тор | 15 PLF | 40 PLF | 0 PLF | 0 PLF | |
| l | 2 | Part. Uniform | 0-0-0 to 2-1-14 | | Far Face | 51 PLF | 105 PLF | 0 PLF | 0 PLF | |
| l | 3 | Point | 2-9-14 | | Far Face | 45 lb | 91 lb | 0 lb | 0 lb | J1 |
| l | | Self Weight | | | | 5 PLF | | | | |

Comb.

0.021 (2%) 1.25D+1.5L L

0.026 (3%) 1.25D+1.5L L

0.096 (10%) 1.25D+1.5L L

Capacity

Case

Uniform



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- fastisming approvals approvals approvals approvals Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318







FAGE IU UF ZI NEU0 10-U 10

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: Date: 5/30/2018

SB Designer:

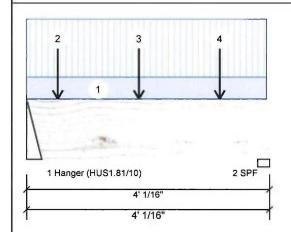
Job Name: AMELIA 1 EL-1 & 2

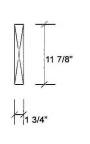
Project #:

Forex 2.0E-3000Fb LVL F8-A

1.750" X 11.875" - PASSED

Level: Ground Floor





Wind

0

0

1.25D+1.5L

0

0

Page 1 of 1

| Type: | Girder | Application: | Floor (Residential) |
|---------------------|--------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |

Bearings and Factored Reactions

Live

700

647

1 2

| Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. |
|---------------|--------|------|--------------|-------|----------|------------|
| 1 - Hanger | 3.000" | 36% | 369 / 1050 | 1418 | L | 1.25D+1.5L |

341 / 971

Analysis Results

15 PSF

Dead:

Member Information

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|------------|---------------|-------------|------------|---------|
| Moment | 1230 ft-lb | 1'10 3/16" | 17130 ft-lb | 0.072 (7%) | 1.25D+1.5L | L |
| Unbraced | 1230 ft-lb | 1'10 3/16" | 12089 ft-lb | 0.102 (10%) | 1.25D+1.5L | L |
| Shear | 855 lb | 1'2 1/8" | 5798 lb | 0.148 (15%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.003 (L/16552) | 1'11 1/16" | 0.123 (L/360) | 0.020 (2%) | D | Uniform |
| LL Defl inch | 0.006 (L/7014) | 1'11 1/4" | 0.123 (L/360) | 0.050 (5%) | L | L |
| TL Defl inch | 0.009 (L/4926) | 1'11 3/16" | 0.184 (L/240) | 0.050 (5%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

51%

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

295

273

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Rottom braced at hearings

| 4 BOLLOTT | braced at bearings. | | | | | | | | |
|-----------|---------------------|-----------------|------------|----------|--------|---------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Part. Uniform | 0-0-0 to 3-11-7 | | Тор | 90 PLF | 240 PLF | 0 PLF | 0 PLF | |
| 2 | Point | 0-6-3 | | Far Face | 54 lb | 113 lb | 0 lb | 0 lb | J1 |
| 3 | Point | 1-10-3 | | Far Face | 76 lb | 157 lb | 0 lb | 0 lb | J1 |
| 4 | Point | 3-2-3 | | Far Face | 63 lb | 129 lb | 0 lb | 0 lb | J1 |
| | Self Weight | | | | 5 PLF | | | | |



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client: Project: Address: Date: 5/30/2018

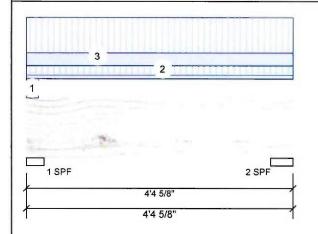
Designer: SB

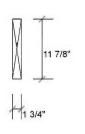
Job Name: AMELIA 1 EL-1 & 2

Project #:

F8-B Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

Level: Ground Floor





0

0

Member Information Girder Type:

| Plies: | 1 |
|---------------------|--------|
| Moisture Condition: | Dry |
| Deflection LL: | 360 |
| Deflection TL: | 240 |
| Importance: | Normal |
| General Load | |
| Floor Live: | 40 PSF |
| Dead: | 15 PSF |
| | |

Application: Floor (Residential) Design Method:

Vibration:

Building Code: NBCC 2010 / OBC 2012 Load Sharing:

Not Checked

Deck: Not Checked 2

Unfactored Reactions UNPATTERNED Ib (Uplift) Live Dead Snow Wind 113 53 0 0

53

| l | Bearings and Factored Reactions | | | | | | | | | | |
|---|---------------------------------|--------|------|--------------|-------|----------|------------|--|--|--|--|
| ľ | Bearing | Length | Сар. | React D/L lb | Total | Ld. Case | Ld. Comb. | | | | |
| l | 1 - SPF | 3.500" | 6% | 66 / 169 | 235 | L | 1.25D+1.5L | | | | |
| ł | 2 - SPF | 4.375" | 5% | 66 / 170 | 237 | L | 1.25D+1.5L | | | | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case | |
|---------------|--------------------|------------|---------------|------------|------------|------|--|
| Moment | 197 ft-lb | 2'1 7/8" | 17130 ft-lb | 0.012 (1%) | 1.25D+1.5L | L | |
| Unbraced | 197 ft-lb | 2'1 7/8" | 11602 ft-lb | 0.017 (2%) | 1.25D+1.5L | L | |
| Shear | 100 lb | 1'2 5/8" | 5798 lb | 0.017 (2%) | 1.25D+1.5L | L | |
| Perm Defl in. | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | | |
| LL Defl inch | 0.001 (L/44414) | 2'1 15/16" | 0.128 (L/360) | 0.010 (1%) | L | L | |
| TL Defl inch | 0.002 (L/30243) | 2'1 15/16" | 0.193 (L/240) | 0.010 (1%) | D+L | L | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

113

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

| D | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|---|---------------|-----------------|-------------|------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 0-2-6 | (Span)0-9-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 4-4-10 | (Span)0-6-9 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Part. Uniform | 0-0-0 to 4-4-10 | | Тор | 15 PLF | 40 PLF | 0 PLF | 0 PLF | |
| | Self Weight | | | | 5 PLF | | | | |



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







Project: Address:

5/30/2018 Date:

SB Designer:

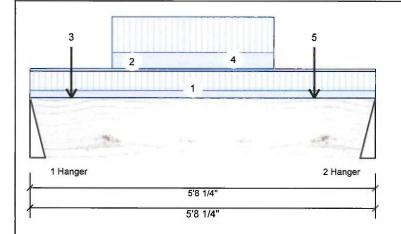
Job Name: AMELIA 1 EL-1 & 2

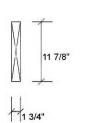
Project #:

Forex 2.0E-3000Fb LVL F8-C

1.750" X 11.875" - PASSED

Level: Ground Floor





Wind

Page 1 of 1

| Member Info | rmation | | | Unfactored Reactions UNPATTERNED Ib (Uplift) | | | | | |
|--------------------|---------|----------------|----------------------|--|----------|-------|--------------|----------|------|
| Туре: | Girder | Application: | Floor (Residential) | Brg | Live | | Dead | Snow | |
| Plies: | 1 | Design Method: | LSD | 1 | 527 | | 274 | 0 | |
| Moisture Condition | on: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 507 | | 265 | 0 | |
| Deflection LL: | 360 | Load Sharing: | No | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | |
| General Load | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings | and Fact | tored | Reactions | | |
| Dead: | 15 PSF | | | Bearing | Length | Can | React D/L lb | Total Ld | Case |

| | | | - | - | |
|---|-----|-------|---|---|--|
| 2 | 507 | 265 | 0 | 0 | |
| | | | | | |
| | | | | | |
| | | 2 507 | • | · | |

Analysis Results Actual Location Allowed Analysis Capacity Comb. Case Moment 1470 ft-lb 2'10 1/4" 17130 ft-lb 0.086 (9%) 1.25D+1.5L L Unbraced 1470 ft-lb 2'10 1/4" 8461 ft-lb 0.174 (17%) 1.25D+1.5L L 947 lb 0.163 (16%) 1.25D+1.5L L Shear 1'2 1/8" 5798 lb Perm Defl in. 0.006 2'10 3/16" 0.177 (L/360) 0.030 (3%) D Uniform (L/11263) LL Defl inch 0.011 (L/5840) 2'10 3/16" 0.177 (L/360) 0.060 (6%) L

2'10 3/16" 0.266 (L/240) 0.060 (6%) D+L

| Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. | |
|---------------|--------|------|--------------|-------|----------|------------|--|
| 1 - Hanger | 3.000" | 29% | 343 / 790 | 1133 | L | 1.25D+1.5L | |
| 2 - | 3.000" | 28% | 331 / 761 | 1092 | L | 1.25D+1.5L | |

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Design Notes

1 Fill all hanger nailing holes.

TL Defl inch 0.017 (L/3846)

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

| | minden at bearings. | | | | | | | | |
|----|---------------------|----------------|--------------|----------|--------|---------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 5-8-4 | (Span)3-6-14 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Part. Uniform | 0-0-0 to 5-8-4 | | Тор | 9 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 3 | Point | 0-8-3 | | Far Face | 65 lb | 132 lb | 0 lb | 0 lb | J2 |
| 4 | Part. Uniform | 1-4-3 to 4-0-3 | | Far Face | 62 PLF | 127 PLF | 0 PLF | 0 PLF | |
| 5 | Point | 4-8-3 | | Far Face | 78 lb | 157 lb | 0 lb | 0 lb | J2 |
| | Self Weight | | | | 5 PLF | | | | |
| | | | | | | | | | ' (|

1



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- chemicals
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals

- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address: Date: 5/30/2018

Designer: SB

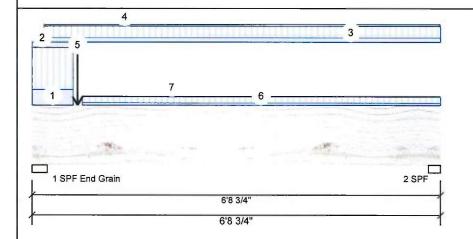
Job Name: AMELIA 1 EL-1 & 2

Project #:

F9-A Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



| tormation |
|-----------|
| |

| Туре: | Girder | Application: | Floor (Residential) |
|---------------------|--------|-----------------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind | |
|-----|------|------|------|------|--|
| 1 | 790 | 361 | 0 | 0 | |
| 2 | 174 | 97 | 0 | 0 | |
| | | | | | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case | |
|---------------|--------------------|------------|---------------|-------------|------------|---------|--|
| Moment | 905 ft-lb | 1'8 13/16" | 17130 ft-lb | 0.053 (5%) | 1.25D+1.5L | L | |
| Unbraced | 905 ft-lb | 1'8 13/16" | 7020 ft-lb | 0.129 (13%) | 1.25D+1.5L | L | |
| Shear | 1461 lb | 1'2 1/8" | 5798 lb | 0.252 (25%) | 1.25D+1.5L | L | |
| Perm Defl in. | 0.004 (L/17866) | 3' 1/4" | 0.214 (L/360) | 0.020 (2%) | D | Uniform | |
| LL Defl inch | 0.008 (L/9140) | 2'11 1/8" | 0.214 (L/360) | 0.040 (4%) | L | L | |
| TL Defl inch | 0.013 (L/6048) | 2'11 9/16" | 0.320 (L/240) | 0.040 (4%) | D+L | L | |

Bearings and Factored Reactions

| Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. | |
|-------------------------|--------|------|--------------|-------|----------|------------|--|
| 1 - SPF End Grain | 3.000" | 42% | 451 / 1185 | 1636 | L | 1.25D+1.5L | |
| 2 - SPF | 2 375" | 15% | 122 / 261 | 383 | 1 | 1.25D+1.5I | |

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING

| | ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|---|----|---------------|------------------|--------------|-----------|--------|--------|-------|-------|----------|
| | 1 | Tie-In | 0-0-0 to 0-8-2 | (Span)3-11-5 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| | 2 | Part. Uniform | 0-0-0 to 0-8-2 | | Тор | 10 PLF | 0 PLF | 0 PLF | 0 PLF | |
| | 3 | Tie-In | 0-2-6 to 6-8-12 | (Span)1-0-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| | 4 | Part. Uniform | 0-2-6 to 6-8-11 | | Тор | 3 PLF | 0 PLF | 0 PLF | 0 PLF | |
| | 5 | Point | 0-9-0 | | Near Face | 295 lb | 700 lb | 0 lb | 0 lb | F8 |
| | 6 | Tie-In | 0-9-14 to 6-8-12 | (Span)0-7-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| | 7 | Part. Uniform | 0-9-14 to 6-8-11 | | Тор | 1 PLF | 0 PLF | 0 PLF | 0 PLF | |
| | | Self Weight | | | | 5 PLF | | | | |
| ı | | | | | | | | | | |



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

applicates
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client: Project: Address: Date: 5/30/2018

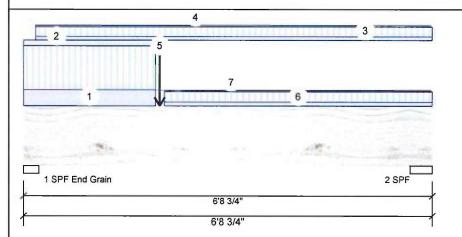
Designer: SB

Job Name: AMELIA 1 EL-1 & 2

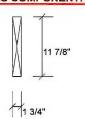
Project #:

F9-B Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

Level: Ground Floor



READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



1.25D+1.5L

PAGE 14 UF ZI

Page 1 of 1

| lember Infor | mation | | | Unfactored Reactions UNPATTERNED Ib (Uplift) | | | | | | |
|--------------------|--------|----------------|----------------------|--|-----------|-------|--------------|-------|----------|------------|
| Туре: | Girder | Application: | Floor (Residential) | Brg | Live | | Dead | Snov | N | Wind |
| Plies: | 1 | Design Method: | LSD | 1 | 335 | | 183 | 1 | 0 | 0 |
| Moisture Condition | n: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 192 | | 112 | i | 0 | 0 |
| Deflection LL: | 360 | Load Sharing: | No | | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | 9 | | | | | | |
| General Load | | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings | s and Fac | tored | Reactions | | | |
| Dead: | 15 PSF | | | Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. |
| | | | | 1 - SPF End | 3.000" | 19% | 229 / 502 | 731 | L | 1.25D+1.5L |
| Analysis Resul | ts | | | Grain | | | | | | |

Analysis Results Analysis Actual Location Allowed Capacity Comb. Case Moment 1057 ft-lb 2'2 7/8" 17130 ft-lb 0.062 (6%) 1.25D+1.5L L 1057 ft-lb Unbraced 2'2 7/8" 7207 ft-lb 0.147 (15%) 1.25D+1.5L L 521 lb 0.090 (9%) 1.25D+1.5L L Shear 1'2 1/8" 5798 lb Perm Defl in, 0.005 2'11" 0.208 (L/360) 0.020 (2%) D (L/15834)

Uniform

L

L

LL Defl inch 0.009 (L/8720) 2'10 3/8" 0.208 (L/360) 0.040 (4%) L TL Defl inch 0.013 (L/5623) 2'10 9/16" 0.312 (L/240) 0.040 (4%) D+L REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

140 / 288

428 L

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

2 - SPF 4.375"

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|------------------|--------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 2-2-0 | (Span)3-4-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Part. Uniform | 0-0-0 to 2-2-0 | | Тор | 9 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 3 | Tie-In | 0-2-6 to 6-8-12 | (Span)0-9-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 4 | Part. Uniform | 0-2-6 to 6-8-8 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 5 | Point | 2-2-14 | | Far Face | 102 lb | 207 lb | 0 lb | 0 lb | F7 |
| 6 | Tie-In | 2-3-12 to 6-8-12 | (Span)0-9-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 7 | Part. Uniform | 2-3-12 to 6-8-7 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |
| | Self Weight | | | | 5 PLF | | | | |
| | | | | | | | | | |



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the lineade application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

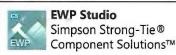
Manufacturer Info







Page 1 of 2



Client: Project: Address:

5/30/2018 Date:

Designer: SB

Job Name: AMELIA 1 EL-1 & 2

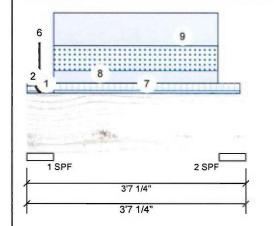
Project #:

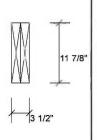
F10-A Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED

Brg

Level: Second Floor





Wind

| Member Information | | | | | | |
|---------------------|--------|--|--|--|--|--|
| Туре: | Girder | | | | | |
| Plies: | 2 | | | | | |
| Moisture Condition: | Dry | | | | | |
| Deflection LL: | 360 | | | | | |
| Deflection TL: | 240 | | | | | |
| Importance: | Normal | | | | | |
| General Load | | | | | | |
| Floor Live: | 40 PSF | | | | | |
| Dead: | 15 DSE | | | | | |

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No Deck:

Not Checked Not Checked

| Unfactored Reactions UNPATTERNED lb (I | plift) |
|--|--------|
|--|--------|

Dead

| 1 | 1/4 | 515 | 491 | U |
|---|-----|-----|-----|---|
| 2 | 31 | 177 | 82 | 0 |
| | | | | |
| | | | | |

Snow

Bearings and Factored Reactions

Live

| _ | | | | | | | |
|---------|--------|------|--------------|-------|----------|---------------------|--|
| Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. | |
| 1 - SPF | 5.250" | 13% | 643 / 824 | 1467 | L | 1.25D+1.5S +0.5L | |
| 2 - SPF | 5.250" | 3% | 247 / 0 | 247 | Uniform | 1.4D | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|----------|---------------|------------|----------|---------|
| Moment | 180 ft-lb | 1'9 5/8" | 22269 ft-lb | 0.008 (1%) | 1.4D | Uniform |
| Unbraced | 180 ft-lb | 1'9 5/8" | 22269 ft-lb | 0.008 (1%) | 1.4D | Uniform |
| Shear | 77 lb | 2'2 7/8" | 7537 lb | 0.010 (1%) | 1.4D | Uniform |
| Perm Defl in. | 0.001 (L/62496) | 1'9 5/8" | 0.095 (L/360) | 0.010 (1%) | D | Uniform |
| LL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| TL Defl inch | 0.001 (L/40207) | 1'9 5/8" | 0.143 (L/240) | 0.010 (1%) | D+S+0.5L | L |

Vibration:

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size; beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

| / Lateral | sienderness ratio based of | on full section wiath. | | | | | | | |
|-----------|----------------------------|------------------------|--------------|------|--------|--------|--------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 0-5-4 | (Span)0-7-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 0-5-4 | (Span)0-8-5 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 0-2-8 | | Тор | 285 lb | 138 lb | 397 lb | 0 lb | F3 F3 |
| 4 | Point | 0-2-8 | | Тор | 5 lb | 0 lb | 11 lb | 0 lb | |

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding





Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

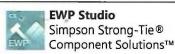
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EL-MASRL





Page 2 of 2



Client: Project: Address: Date: 5/30/2018

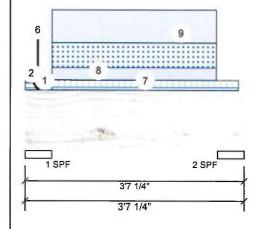
Designer: SB

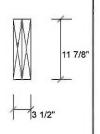
Job Name: AMELIA 1 EL-1 & 2

Project #:

F10-A Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





| Continued from p | age 1 | | | | | | | | |
|------------------|---------------|-----------------|-------------------|------|--------|--------|--------|-------|------------------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 5 | Point | 0-2-8 | | Тор | 7 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |
| 6 | Point | 0-2-8 | | Тор | 37 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |
| 7 | Tie-In | 0-5-4 to 3-6-2 | (Span) 0-10-13 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 8 | Part. Uniform | 0-5-4 to 3-1-12 | | Тор | 30 PLF | 0 PLF | 61 PLF | 0 PLF | |
| 9 | Part. Uniform | 0-5-4 to 3-1-12 | | Тор | 80 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight |
| | Self Weight | | | | 10 PLF | | | | |

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

LVI beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







0 PSF 0-0-0 to 9-9-12 15 PSF 40 PSF 1 Tie-In (Span)1-0-5 Top 0 PSF 0-0-0 to 0-2-6 2 Part, Uniform Top 106 PLF 0 PLF 212 PLF 0 PLF Part. Uniform 3 0-0-0 to 0-2-6 40 PLF 0 PLF 0 PLF 0 PLF Top Wall Self Weight Part. Uniform 0-2-6 to 9-5-8 211 PLF 0 PLF Top 423 PLF 0 PLF

Top

Continued on page 2...

5

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design cnteries and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Part. Uniform

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation

0-2-6 to 9-5-8

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply regarding installation requirements, make p., fastening details, beam strength values, and code
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

80 PLF

For flat roofs provide proper drainage to prevent ponding

0 PLF

Manufacturer Info Forex

APA: PR-L318

0 PLF



0 PLF

Wall Self Weight

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 14A7X4



REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

Manufacturer Info

APA: PR-L318







Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

Date:

5/30/2018 Designer:

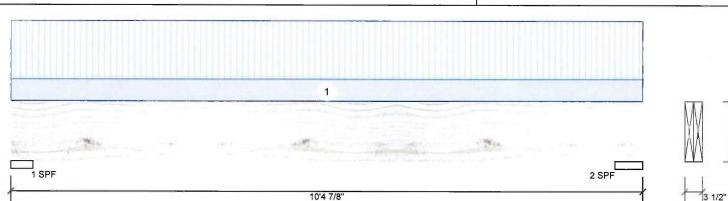
Job Name: AMELIA 1 EL-1 & 2

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor



10'4 7/8"

Floor (Residential)

11 7/8"

Member Information

Girder Type: Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240

Importance: Normal General Load

40 PSF Floor Live: Dead: 15 PSF Application:

Design Method: LSD

NBCC 2010 / OBC 2012 **Building Code:**

Load Sharing: Deck:

Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 110 | 91 | 0 | 0 |
| 2 | 112 | 92 | 0 | 0 |

Bearings and Factored Reactions

| Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. |
|---------|--------|------|--------------|-------|----------|------------|
| 1 - SPF | 4.375" | 3% | 113 / 166 | 279 | L | 1.25D+1.5L |
| 2-SPF | 5.500" | 2% | 115 / 169 | 284 | L | 1.25D+1.5L |

Analysis Results

| ſ | Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---|---------------|--------------------|----------|---------------|------------|------------|---------|
| l | Moment | 637 ft-lb | 5'1 7/8" | 34261 ft-lb | 0.019 (2%) | 1.25D+1.5L | L |
| l | Unbraced | 637 ft-lb | 5'1 7/8" | 29876 ft-lb | 0.021 (2%) | 1.25D+1.5L | L |
| ĺ | Shear | 209 lb | 1'3 1/2" | 11596 lb | 0.018 (2%) | 1.25D+1.5L | L |
| l | Perm Defl in. | 0.004 (L/27966) | 5'1 7/8" | 0.324 (L/360) | 0.010 (1%) | D | Uniform |
| ١ | LL Defl inch | 0.005 (L/22923) | 5'1 7/8" | 0.324 (L/360) | 0.020 (2%) | L | L |
| ١ | TL Defl inch | 0.009 (L/12597) | 5'1 7/8" | 0.485 (L/240) | 0.020 (2%) | D+L | L |

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

0 PSF

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

Tie-In

6 Lateral slenderness ratio based on full section width. ID Load Type Location Trib Width Side Dead Live Snow Wind

> 0-0-0 to 10-4-14 (Span)1-0-14 Top Self Weight

15 PSF 10 PLF 40 PSF

Comments

0 PSF

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



EL-MASRI

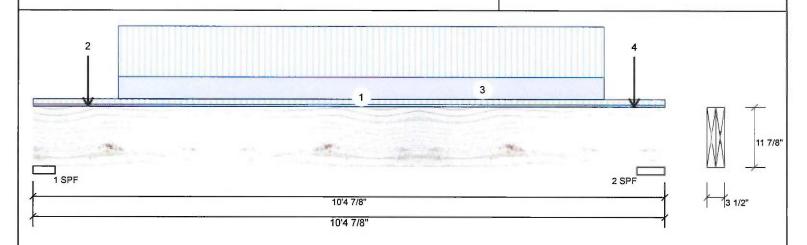
Simpson Strong-Tie® Component Solutions™

Project: Address:

Job Name: AMELIA 1 EL-1 & 2

Project #:

Forex 2.0E-3000Fb LVL 2-Ply - PASSED Level: Second Floor 1.750" X 11.875"



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Girder Wind Type: Application: Floor (Residential) Live Dead Plies: 2 Design Method: 1419 664 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 1580 738 0 0 Deflection LL: 360 Load Sharing: Deflection TL: Not Checked Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 4.375" 31% 830 / 2129 2958 L 1.25D+1.5L

Analysis Results

| _ | | | | | | | | |
|---|---------------|----------------|------------|---------------|-------------|------------|---------|--|
| Γ | Analysis | Actual | Location | Allowed | Capacity | Comb. | Case | |
| l | Moment | 7276 ft-lb | 5'1 13/16" | 34261 ft-lb | 0.212 (21%) | 1.25D+1.5L | L | |
| l | Unbraced | 7276 ft-lb | 5'1 13/16" | 29876 ft-lb | 0.244 (24%) | 1.25D+1.5L | L | |
| l | Shear | 3077 lb | 1'3 1/2" | 11596 lb | 0.265 (27%) | 1.25D+1.5L | L | |
| l | Perm Defl in. | 0.033 (L/3555) | 5'1 7/8" | 0.324 (L/360) | 0.100 (10%) | D | Uniform | |
| l | LL Defl inch | 0.070 (L/1656) | 5'1 7/8" | 0.324 (L/360) | 0.220 (22%) | L | L | |
| l | TL Defl inch | 0.103 (L/1130) | 5'1 7/8" | 0.485 (L/240) | 0.210 (21%) | D+L | L | |

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

28%

922 / 2370

3292 1

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

2 - SPF 5.500"



1.25D+1.5L

| o Lateral o | Eater at civil deli 1000 ratio basea off rail section within | | | | | | | | | | |
|-------------|--|------------------|--------------|----------|---------|---------|-------|-------|----------|--|--|
| מו | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments | | |
| 1 | Tie-In | 0-0-0 to 10-4-14 | (Span)1-5-12 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | | |
| 2 | Point | 0-10-13 | | Far Face | 115 lb | 262 lb | 0 lb | 0 lb | J5 | | |
| 3 | Part. Uniform | 1-4-13 to 9-4-13 | | Far Face | 118 PLF | 268 PLF | 0 PLF | 0 PLF | | | |
| 4 | Point | 9-10-13 | | Far Face | 128 lb | 285 lb | 0 lb | 0 lb | J5 | | |
| | Self Weight | | | | 10 PLF | | | | | | |
| | | | | | | | | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

chemicals

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-L318



