NE0618-037 PAGE 1 OF 31

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

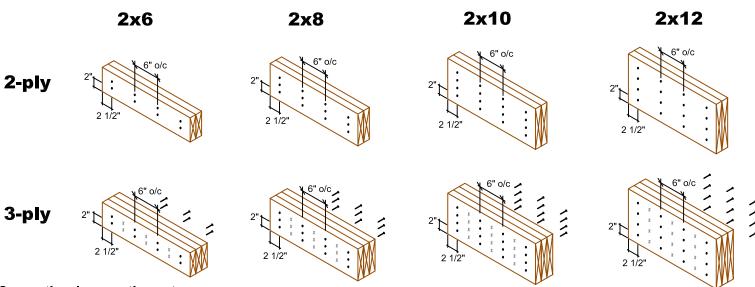
Do not drill any hole, cut or notch a certified building component without a written preauthorization.



NE0618-037 **PAGE 2 OF 31**

MULTIPLE MEMBER CONNECTIONS

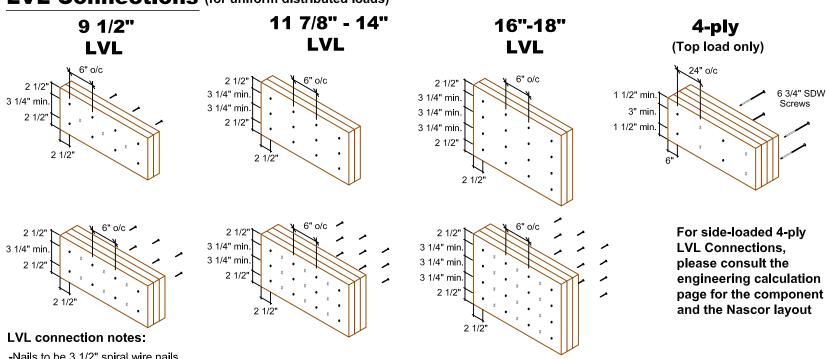
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

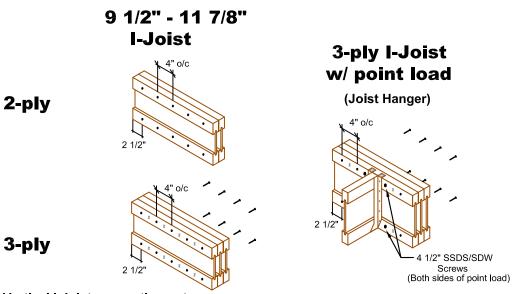
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails.
- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

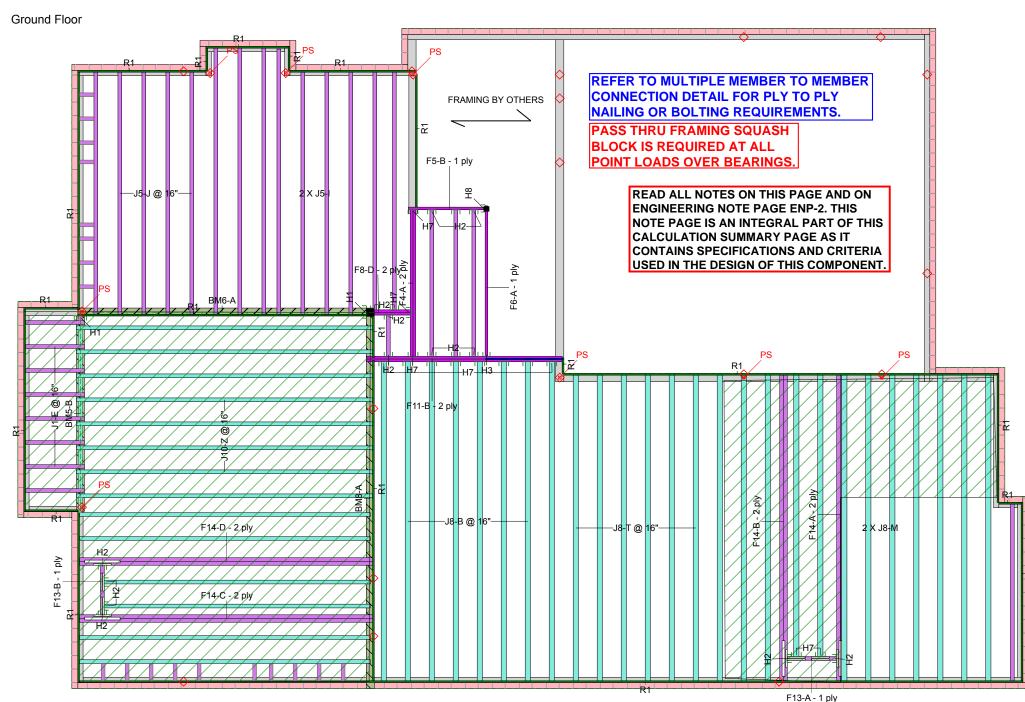


MULTI-PLY CONNECTION **DETAILS**

KOTT 3228 Moodie Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775 Fx: 613-838-4751

Scale: NTS

NE0618-037 **PAGE 3 OF 31**



Architectural Drawing Info

JARDIN DESIGN GROUP 64 JARDIN DR. SUITE 3A VAUGHAN,ON L4K 3P3 Proiect # 17-55 Model: CELESTIAL Date: MAY 22,2018

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)



EWP Studio

EWP Studio Version 18.32.085 Powered by iStruct™

Simpson Strong-Tie® Component Solutions™



JOISTS SPACING 16"O/C

NOTED OTHERWISE

THIS CERTIFICATION IS TO CONFIRM THAT:

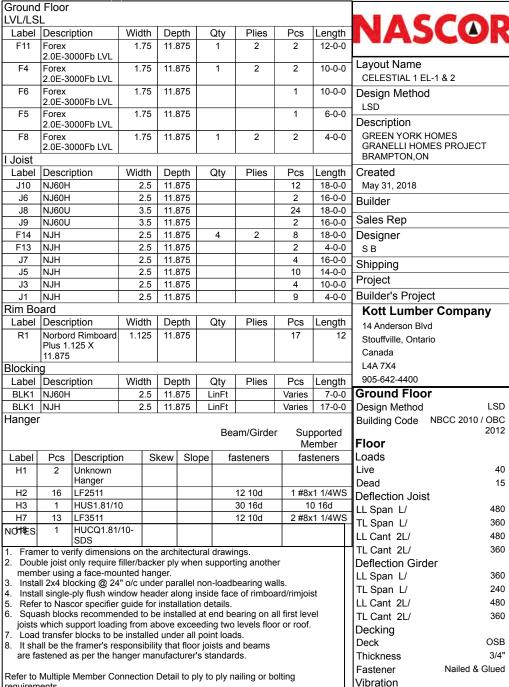
1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY **SHOWN ON THIS LAYOUT.**

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE

THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR** LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.





Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth) @ 16" o/c.

responsibility of others

to construction

Legend

All other components and structural elements supporting

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may be deviate from the architectural

drawings. Project Engineer to review and approve the deviation prior

Point Load Support

Norbord Rimboard Plus 1.125 X 11.875

Forex 2.0E-3000Fb LVL 1.75 X 11.875

Load from Above

NJ60U 11.875

5.25 X 10.25 (Dropped)

NJH 11.875

the floor system such as beams, walls, columns and

foundation walls and footings including anchorage of components and bracing for lateral stability are the

FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO MEMBER CONNECTION DETAIL.

KOTT

NE0618-037 **PAGE 4 OF 31**



Client: Project: Address:

6/1/2018 Designer: SB

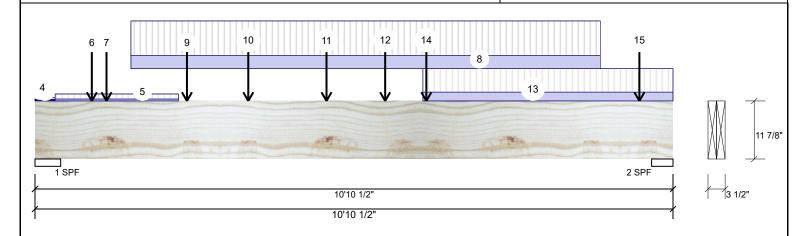
Job Name: CELESTIAL 1 EL-1

Project #

Forex 2.0E-3000Fb LVL F11-B

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



| Member Infor | mation | | | Unfactored Reactions UNPATTERNED Ib (Uplift) | | | | |
|--------------------|--------|----------------|----------------------|--|----------|-------------------|----------------|------------|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind |
| Plies: | 2 | Design Method: | LSD | 1 | 2997 | 1419 | 0 | 0 |
| Moisture Condition | n: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 3283 | 1366 | 0 | 0 |
| Deflection LL: | 360 | Load Sharing: | No | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | |
| General Load | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings a | nd Facto | red Reactions | | |
| Dead: | 15 PSF | | | Bearing Le | ength | Cap. React D/L lb | Total Ld. Case | Ld. Comb. |
| | | | | 1 - SPF 5. | 250" | 55% 1774 / 4495 | 6270 L | 1.25D+1.5L |
| \makesis Dosul | | | | 2 - SPF 4. | 375" | 70% 1707 / 4925 | 6632 L | 1.25D+1.5L |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|------------|---------------|-------------|------------|---------|
| Moment | 16043 ft-lb | 5'4 13/16" | 34261 ft-lb | 0.468 (47%) | 1.25D+1.5L | L |
| Unbraced | 16043 ft-lb | 5'4 13/16" | 29422 ft-lb | 0.545 (55%) | 1.25D+1.5L | L |
| Shear | 6167 lb | 1'4 3/8" | 11596 lb | 0.532 (53%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.077 (L/1592) | 5'4 11/16" | 0.340 (L/360) | 0.230 (23%) | D | Uniform |
| LL Defl inch | 0.173 (L/707) | 5'5 5/8" | 0.340 (L/360) | 0.510 (51%) | L | L |
| TL Defl inch | 0.250 (L/489) | 5'5 5/16" | 0.510 (L/240) | 0.490 (49%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|----------------|--------------|-----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 0-4-2 | (Span)0-4-13 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 0-4-2 | (Span)0-11-3 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Part. Uniform | 0-0-0 to 0-2-9 | | Тор | 1 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 4 | Part. Uniform | 0-0-0 to 0-2-8 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 5 | Tie-In | 0-4-2 to 2-5-5 | (Span)2-7-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 6 | Point | 0-11-10 | | Near Face | 135 lb | 360 lb | 0 lb | 0 lb | J8 |

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





NE0618-037 **PAGE 5 OF 31**



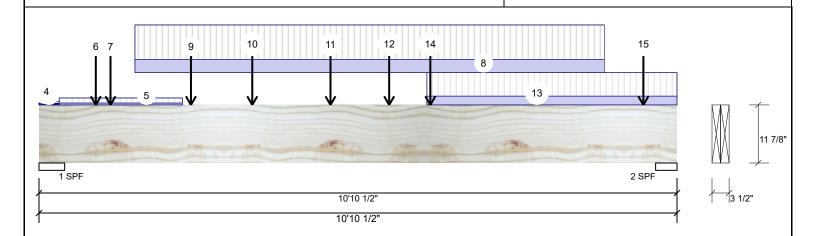
Client: Project: Address:

6/1/2018 Designer: SB

Job Name: CELESTIAL 1 EL-1

Project #:

2-Ply - PASSED Level: Ground Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL F11-B



| Continued from page 1 | | | | | | | | | |
|-----------------------|---------------|------------------|------------|-----------|---------|---------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 7 | Point | 1-2-10 | | Far Face | 19 lb | 50 lb | 0 lb | 0 lb | J1 |
| 8 | Part. Uniform | 1-7-10 to 9-7-10 | | Near Face | 133 PLF | 354 PLF | 0 PLF | 0 PLF | |
| 9 | Point | 2-7-1 | | Far Face | 564 lb | 718 lb | 0 lb | 0 lb | F4 |
| 10 | Point | 3-7-10 | | Far Face | 73 lb | 194 lb | 0 lb | 0 lb | J3 |
| 11 | Point | 4-11-10 | | Far Face | 71 lb | 190 lb | 0 lb | 0 lb | J3 |
| 12 | Point | 5-11-10 | | Far Face | 52 lb | 139 lb | 0 lb | 0 lb | J3 |
| 13 | Part. Uniform | 6-7-5 to 10-10-8 | | Тор | 90 PLF | 240 PLF | 0 PLF | 0 PLF | |
| 14 | Point | 6-8-1 | | Far Face | 103 lb | 198 lb | 0 lb | 0 lb | F6 |
| 15 | Point | 10-3-10 | | Near Face | 172 lb | 458 lb | 0 lb | 0 lb | J8 |
| | Self Weight | | | | 10 PLF | | | | |

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Page 2 of 2

NE0618-037 **PAGE 6 OF 31**



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

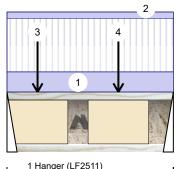
6/1/2018 Designer: SB

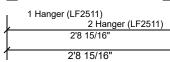
Job Name: CELESTIAL 1 EL-1

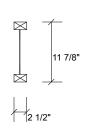
Level: Ground Floor

Project #:

F13-A 11.875" - PASSED NJH







Page 1 of 1

Member Information

| Type: | Girder | Application: | Floor (Residential) |
|---------------------|--------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 421 | 209 | 0 | 0 |
| 2 | 344 | 171 | 0 | 0 |
| I | | | | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|------------|---------------|-------------|------------|---------|
| Moment | 562 ft-lb | 1'10 1/16" | 5390 ft-lb | 0.104 (10%) | 1.25D+1.5L | L |
| Unbraced | 562 ft-lb | 1'10 1/16" | 4936 ft-lb | 0.114 (11%) | 1.25D+1.5L | L |
| Shear | 887 lb | 1 1/4" | 1810 lb | 0.490 (49%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.003 (L/11493) | 1'10 1/16" | 0.085 (L/360) | 0.030 (3%) | D | Uniform |
| LL Defl inch | 0.005 (L/5711) | 1'10 1/16" | 0.085 (L/360) | 0.060 (6%) | L | L |
| TL Defl inch | 0.008 (L/3815) | 1'10 1/16" | 0.127 (L/240) | 0.060 (6%) | D+L | L |

Bearings and Factored Reactions

| Bearing | Length | Cap. Re | eact D/L lb | Total | Ld. Case | Ld. Comb. |
|---------------|--------|---------|-------------|-------|----------|------------|
| 1 - Hanger | 2.000" | 55% | 261 / 632 | 893 | L | 1.25D+1.5L |
| 2 - Hanger | 2.000" | 45% | 213 / 515 | 729 | L | 1.25D+1.5L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

Load Type

Part. Uniform

Tie-In

Point

Point

| Live | Snow | Wind | Comments | |
|--------|--------|--------------|--------------------|--------------------|
| 40 PSF | 0 PSF | 0 PSF | | |
| 0 PLF | 0 PLF | 0 PLF | | |
| | 40 PSF | 40 PSF 0 PSF | 40 PSF 0 PSF 0 PSF | 40 PSF 0 PSF 0 PSF |

0 lb J9

0 lb J9

0 lb

0 lb

ID

2

3

4

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

Location

0-6-1

1-10-1

0-0-0 to 2-8-15

0-0-0 to 2-8-15

Trib Width

(Span)1-3-7

Side

Top

Top

Far Face

Far Face

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, stifferer tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid

157 lb

188 lb

316 lb

378 lb

- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







NE0618-037 **PAGE 7 OF 31**



EWP Studio Simpson Strong-Tie®

Client: Project: Address:

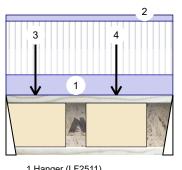
6/1/2018 Designer: SB

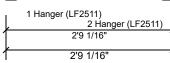
Job Name: CELESTIAL 1 EL-1

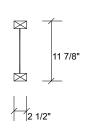
Level: Ground Floor

Project #:

F13-B 11.875" - PASSED NJH







Member Information Type: Plies: 1 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance:

Normal General Load 40 PSF 15 PSF

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Not Checked Deck: Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 409 | 203 | 0 | 0 |
| 2 | 327 | 162 | 0 | 0 |

Bearings and Factored Reactions

Hanger

| Bearing | Length | Cap. Re | act D/L lb | Total | Ld. Case | Ld. Comb. |
|---------------|--------|---------|------------|-------|----------|------------|
| 1 - Hanger | 2.000" | 54% | 254 / 614 | 868 | L | 1.25D+1.5L |
| 2 - | 2 000" | 43% | 203 / 491 | 693 | 1 | 1 25D+1 5I |

Analysis Results

Floor Live:

Dead:

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|------------|---------------|-------------|------------|---------|
| Moment | 551 ft-lb | 1'9 13/16" | 5390 ft-lb | 0.102 (10%) | 1.25D+1.5L | L |
| Unbraced | 551 ft-lb | 1'9 13/16" | 4931 ft-lb | 0.112 (11%) | 1.25D+1.5L | L |
| Shear | 862 lb | 1 1/4" | 1810 lb | 0.476 (48%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.003 (L/11772) | 1'9 13/16" | 0.085 (L/360) | 0.030 (3%) | D | Uniform |
| LL Defl inch | 0.005 (L/5836) | 1'9 13/16" | 0.085 (L/360) | 0.060 (6%) | L | L |
| TL Defl inch | 0.008 (L/3902) | 1'9 13/16" | 0.127 (L/240) | 0.060 (6%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

| <u> </u> | J | | | | | | | | |
|----------|---------------|----------------|--------------|-----------|--------|--------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 2-9-1 | (Span)1-3-13 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Part. Uniform | 0-0-0 to 2-9-1 | | Тор | 3 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 3 | Point | 0-5-13 | | Near Face | 149 lb | 299 lb | 0 lb | 0 lb | J6 |
| 4 | Point | 1-9-13 | | Near Face | 181 lb | 365 lb | 0 lb | 0 lb | J6 |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid

alteral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







NE0618-037 **PAGE 8 OF 31**



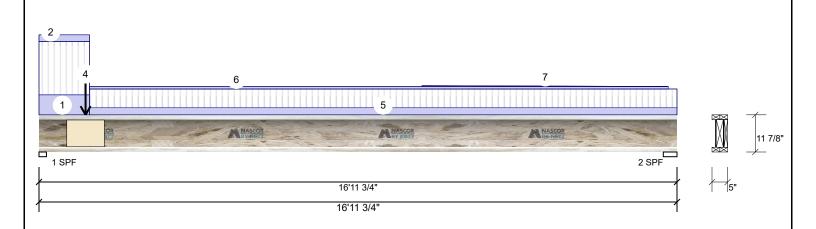
Client: Project: Address: Date: 6/1/2018 Designer: SB

Job Name: CELESTIAL 1 EL-1

Project #:

2-Ply - PASSED F14-A NJH 11.875"

Level: Ground Floor



| nation | | | Unfactore | d Reactio | ons UNPATTERN | IED lb (Uplift) | |
|--------|-----------------------------|---|--|--|---|--|---|
| Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind |
| 2 | Design Method: | LSD | 1 | 562 | 282 | 0 | 0 |
| Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 216 | 111 | 0 | 0 |
| 360 | Load Sharing: | No | | | | | |
| 240 | Deck: | Not Checked | | | | | |
| Normal | Vibration: | Not Checked | | | | | |
| | | | | | | | |
| 40 PSF | | | Bearings a | nd Facto | red Reactions | | |
| 15 PSF | | | Bearing Le | ength | Cap. React D/L lb | Total Ld. Case | Ld. Comb. |
| | | | 1 - SPF 2. | .375" | 36% 353 / 842 | 1195 L | 1.25D+1.5L |
| | | | 2 - SPF 4. | .375" | 13% 139 / 324 | 463 L | 1.25D+1.5L |
| | Girder 2 Dry 360 240 Normal | Girder 2 Design Method: Dry Building Code: 10 10 10 10 10 10 10 10 10 1 | Girder Application: Floor (Residential) 2 Design Method: LSD Dry Building Code: NBCC 2010 / OBC 2012 360 Load Sharing: No 240 Deck: Not Checked Normal Vibration: Not Checked | Application: Floor (Residential) Brg 1 | Application: Floor (Residential) Brg Live | Application: Floor (Residential) Brg Live Dead | Application: Floor (Residential) Brg Live Dead Snow |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|------------|---------------|-----------------|------------|---------|
| Moment | 2101 ft-lb | 7'3 15/16" | 10780 ft-lb | 0.195 (19%) | 1.25D+1.5L | L |
| Unbraced | 2101 ft-lb | 7'3 15/16" | 2107 ft-lb | 0.997 (100%) | 1.25D+1.5L | L |
| Shear | 1177 lb | 1 5/8" | 3620 lb | 0.325 (33%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.046 (L/4345) | 8'1 1/8" | 0.551 (L/360) | 0.080 (8%) | D | Uniform |
| LL Defl inch | 0.089 (L/2232) | 8' 13/16" | 0.551 (L/360) | 0.160 (16%) | L | L |
| TL Defl inch | 0.135 (L/1475) | 8' 15/16" | 0.827 (L/240) | 0.160 (16%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 9' o.c.

5 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-------------------|--------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 1-4-2 | (Span)3-1-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Part. Uniform | 0-0-0 to 1-4-2 | | Тор | 8 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 4 | Point | 1-2-14 | | Far Face | 171 lb | 344 lb | 0 lb | 0 lb | F13 |
| 5 | Tie-In | 1-4-2 to 16-11-12 | (Span)1-1-6 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 6 | Part. Uniform | 1-4-2 to 16-8-15 | | Тор | 3 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 7 | Tapered Start | 10-2-5 | | Тор | 1 PLF | 0 PLF | 0 PLF | 0 PLF | |
| | End | 16-9-0 | | | 0 PLF | 0 PLF | 0 PLF | 0 PLF | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott

KOTT





NE0618-037 **PAGE 9 OF 31**



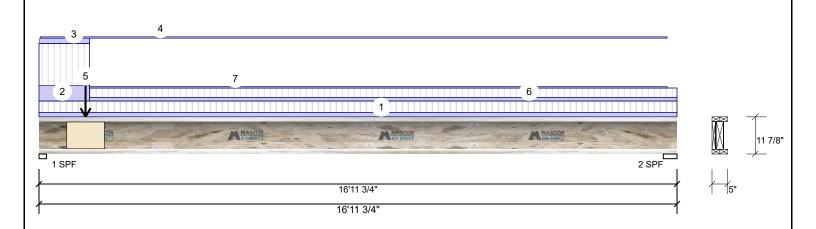
Client: Project: Address: Date: 6/1/2018 Designer: SB

Job Name: CELESTIAL 1 EL-1

Project #:

2-Ply - PASSED F14-B NJH 11.875"

Level: Ground Floor



| Member Inform | mation | | | Unfactor | ed Reacti | ons UNPATTERNI | D lb (Uplift) | |
|--------------------|--------|----------------|----------------------|-----------|-----------|-------------------|----------------|------------|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind |
| Plies: | 2 | Design Method: | LSD | 1 | 723 | 360 | 0 | 0 |
| Moisture Condition | n: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 302 | 150 | 0 | 0 |
| Deflection LL: | 360 | Load Sharing: | No | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | |
| General Load | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings | and Fact | ored Reactions | | |
| Dead: | 15 PSF | | | Bearing | Length | Cap. React D/L lb | Total Ld. Case | Ld. Comb. |
| | | | | 1 - SPF 2 | 2.375" | 46% 450 / 1084 | 1534 L | 1.25D+1.5L |
| | | | | 2 - SPF 4 | 4.375" | 18% 188 / 453 | 640 L | 1.25D+1.5L |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-----------------|------------|---------|
| Moment | 2865 ft-lb | 7'5 7/16" | 10780 ft-lb | 0.266 (27%) | 1.25D+1.5L | L |
| Unbraced | 2865 ft-lb | 7'5 7/16" | 2868 ft-lb | 0.999 (100%) | 1.25D+1.5L | L |
| Shear | 1511 lb | 1 5/8" | 3620 lb | 0.417 (42%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.061 (L/3252) | 8'1 1/4" | 0.551 (L/360) | 0.110 (11%) | D | Uniform |
| LL Defl inch | 0.122 (L/1627) | 8'1 1/4" | 0.551 (L/360) | 0.220 (22%) | L | L |
| TL Defl inch | 0.183 (L/1084) | 8'1 1/4" | 0.827 (L/240) | 0.220 (22%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'10" o.c.
- 5 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-------------------|--------------|-----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 16-11-12 | (Span)0-10-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 1-4-2 | (Span)3-1-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Part. Uniform | 0-0-4 to 1-4-2 | | Тор | 8 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 4 | Part. Uniform | 0-0-7 to 16-8-9 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 5 | Point | 1-2-14 | | Near Face | 209 lb | 421 lb | 0 lb | 0 lb | F13 |
| 6 | Tie-In | 1-4-2 to 16-11-12 | (Span)0-8-9 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 7 | Part. Uniform | 1-4-2 to 16-8-10 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |
| | | | | | | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- Handling & Installation
- IARIGHING & INSEGUATION

 Lodist flanges must not be cut or drilled

 Refer to latest copy of the IJoist product information details for framing details, suffiener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged IJoists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott

KOTT





PAGE 10 OF 31 NE0618-037



Client: Project: Address:

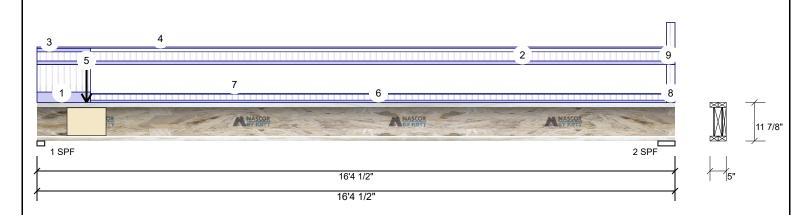
6/1/2018 Designer: SB

Job Name: CELESTIAL 1 EL-1

Project #:

2-Ply - PASSED F14-C NJH 11.875"

Level: Ground Floor



| Member Inform | nation | | | Unfactor | ed Reacti | ons UNPATTERN | ED lb (Uplift) | |
|--------------------|--------|----------------|----------------------|----------|-----------|-------------------|----------------|------------|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind |
| Plies: | 2 | Design Method: | LSD | 1 | 728 | 367 | 0 | 0 |
| Moisture Condition | : Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 344 | 173 | 0 | 0 |
| Deflection LL: | 360 | Load Sharing: | No | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | |
| General Load | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings | and Fact | ored Reactions | | |
| Dead: | 15 PSF | | | Bearing | Length | Cap. React D/L lb | Total Ld. Case | Ld. Comb. |
| | | | | 1 - SPF | 2.375" | 47% 459 / 1092 | 1551 L | 1.25D+1.5L |
| A 1 - 2 - D 10 | | | | | 5.250" | 20% 216 / 516 | 732 L | 1.25D+1.5L |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-------------|------------|---------|
| Moment | 2927 ft-lb | 7'2 1/16" | 10780 ft-lb | 0.272 (27%) | 1.25D+1.5L | L |
| Unbraced | 2927 ft-lb | 7'2 1/16" | 2945 ft-lb | 0.994 (99%) | 1.25D+1.5L | L |
| Shear | 1526 lb | 1 5/8" | 3620 lb | 0.422 (42%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.059 (L/3234) | 7'9 1/2" | 0.529 (L/360) | 0.110 (11%) | D | Uniform |
| LL Defl inch | 0.115 (L/1660) | 7'9 3/8" | 0.529 (L/360) | 0.220 (22%) | L | L |
| TL Defl inch | 0.174 (L/1097) | 7'9 7/16" | 0.793 (L/240) | 0.220 (22%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'9" o.c.

5 Bottom flange braced at bearings

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|------------------|--------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 1-4-8 | (Span)3-2-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 16-1-14 | (Span)1-0-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Part. Uniform | 0-0-0 to 1-4-8 | | Тор | 8 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 4 | Part. Uniform | 0-0-0 to 16-1-3 | | Тор | 3 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 5 | Point | 1-3-4 | | Far Face | 203 lb | 409 lb | 0 lb | 0 lb | F13 |
| 6 | Tie-In | 1-4-8 to 16-1-14 | (Span)0-8-5 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 7 | Part. Uniform | 1-4-8 to 16-1-3 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- IARIGHING & INSEGUATION

 Lodist flanges must not be cut or drilled

 Refer to latest copy of the IJoist product information details for framing details, suffiener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged IJoists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- alteral displacement and rotation
 6. Web stiffeners for point load as shown Minimum
 point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent
 ponding.

Manufacturer Info

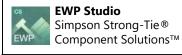
Nascor by Kott

KOTT





NE0618-037 PAGE 11 OF 31



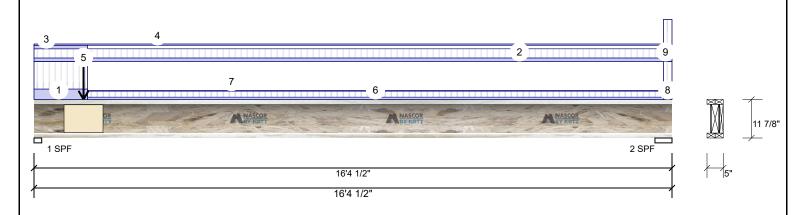
Client: Project: Address: Date: 6/1/2018 Designer: $\mathsf{S}\,\mathsf{B}$

Job Name: CELESTIAL 1 EL-1

Project #:

11.875" 2-Ply - PASSED F14-C NJH

Level: Ground Floor



.Continued from page 1

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-----------|-------------------|-------------|------|--------|--------|-------|-------|----------|
| 8 | Tie-In | 16-1-14 to 16-4-8 | (Span)3-2-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 9 | Tie-In | 16-1-14 to 16-4-8 | (Span)3-6-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 Idoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, stifferer tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott

KOTT

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Page 2 of 2

NE0618-037 PAGE 12 OF 31



Client: Project: Address:

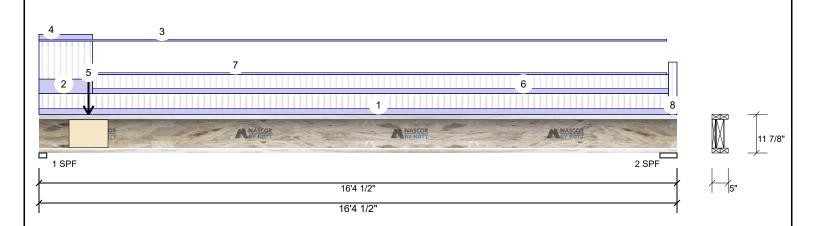
6/1/2018 Designer: SB

Job Name: CELESTIAL 1 EL-1

Project #

2-Ply - PASSED 11.875" F14-D NJH

Level: Ground Floor



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Wind Type: Application: Floor (Residential) Brg Live Dead Plies: 2 Design Method: 747 372 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 430 213 0 0 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** 40 PSF Floor Live: 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 465 / 1121 1 - SPF 2.375" 48% 1586 I 1.25D+1.5L 2 - SPF 5.250" 25% 266 / 645 911 L 1.25D+1.5L

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-------------|---------------|-------------|------------|---------|
| Moment | 3670 ft-lb | 7'6 1/2" | 10780 ft-lb | 0.340 (34%) | 1.25D+1.5L | L |
| Unbraced | 3670 ft-lb | 7'6 1/2" | 3703 ft-lb | 0.991 (99%) | 1.25D+1.5L | L |
| Shear | 1560 lb | 1 5/8" | 3620 lb | 0.431 (43%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.072 (L/2647) | 7'10 11/16" | 0.529 (L/360) | 0.140 (14%) | D | Uniform |
| LL Defl inch | 0.144 (L/1320) | 7'10 5/8" | 0.529 (L/360) | 0.270 (27%) | L | L |
| TL Defl inch | 0.216 (L/881) | 7'10 11/16" | 0.793 (L/240) | 0.270 (27%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7' o.c.

5 Bottom flange braced at bearings

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-------------------|--------------|-----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 16-1-14 | (Span)1-3-5 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 1-4-8 | (Span)3-2-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Part. Uniform | 0-0-0 to 16-1-5 | | Тор | 3 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 4 | Part. Uniform | 0-0-0 to 1-4-8 | | Тор | 8 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 5 | Point | 1-3-4 | | Near Face | 162 lb | 327 lb | 0 lb | 0 lb | F13 |
| 6 | Tie-In | 1-4-8 to 16-1-14 | (Span)1-1-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 7 | Part. Uniform | 1-4-8 to 16-1-5 | | Тор | 3 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 8 | Tie-In | 16-1-14 to 16-4-8 | (Span)3-2-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- Handling & Installation
- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid

- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott

KOTT





NE0618-037 PAGE 13 OF 31



Client: Project: Address:

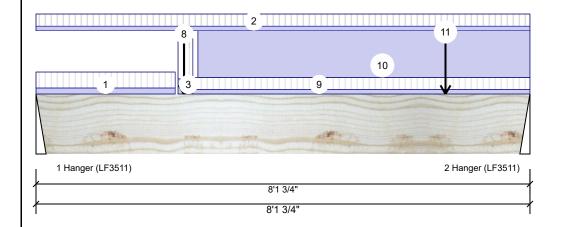
6/1/2018 Designer: SB

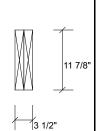
Job Name: CELESTIAL 1 EL-1

Project #:

1.750" X 11.875" 2-Ply - PASSED F4-A Forex 2.0E-3000Fb LVL

Level: Ground Floor





Wind

O

0

0

0

| wember | intormation |
|--------|-------------|
| Type: | Girder |

2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load 40 PSF Floor Live:

Application: Floor (Residential) Design Method:

Building Code: NBCC 2010 / OBC 2012 Load Sharing: No

Not Checked Deck: Vibration: Not Checked

15 PSF

Bearings and Factored Reactions

Live

718

682

Brg

1

2

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

564

642

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 705 / 1076 1 -2.000" 34% 1781 I 1.25D+1.5L Hanger

2 -2.000" 35% 802 / 1023 1825 L 1.25D+1.5L Hanger

Analysis Results

Dead:

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|------------|---------------|-------------|------------|---------|
| Moment | 3845 ft-lb | 2'6 9/16" | 34261 ft-lb | 0.112 (11%) | 1.25D+1.5L | L |
| Unbraced | 3845 ft-lb | 2'6 9/16" | 31329 ft-lb | 0.123 (12%) | 1.25D+1.5L | L |
| Shear | 1664 lb | 1'1 1/8" | 11596 lb | 0.144 (14%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.019 (L/5107) | 3'11 7/16" | 0.265 (L/360) | 0.070 (7%) | D | Uniform |
| LL Defl inch | 0.021 (L/4600) | 3'9 7/16" | 0.265 (L/360) | 0.080 (8%) | L | L |
| TL Defl inch | 0.039 (L/2421) | 3'10 3/8" | 0.397 (L/240) | 0.100 (10%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

| / Lateral eleman | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | an 000aon maan | | | | | | | |
|------------------|---|-----------------|-------------|------|--------|--------|-------|-------|------------------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 2-3-9 | (Span)1-4-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 8-1-12 | (Span)1-0-9 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Tie-In | 2-4-2 to 2-7-1 | (Span)3-5-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 4 | Point | 2-5-5 | | Тор | 85 lb | 226 lb | 0 lb | 0 lb | J5 |
| 5 | Point | 2-5-5 | | Тор | 19 lb | 52 lb | 0 lb | 0 lb | J1 |
| 6 | Point | 2-5-5 | | Тор | 77 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





NE0618-037 PAGE 14 OF 31



Client: Project: Address:

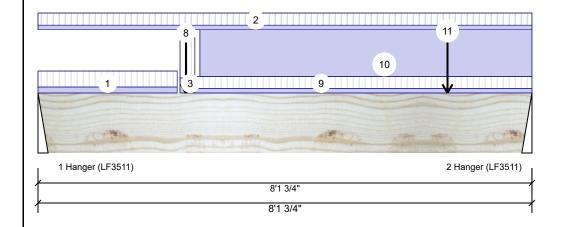
6/1/2018 Designer: $\mathsf{S}\,\mathsf{B}$

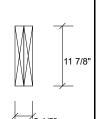
Job Name: CELESTIAL 1 EL-1

Project #:

1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED F4-A

Level: Ground Floor





Page 2 of 2

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-----------------|-------------|----------|--------|--------|-------|-------|------------------|
| 7 | Point | 2-5-5 | | Тор | 30 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |
| 8 | Point | 2-5-5 | | Far Face | 191 lb | 383 lb | 0 lb | 0 lb | F8 |
| 9 | Tie-In | 2-7-1 to 8-1-12 | (Span)1-0-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 10 | Part. Uniform | 2-8-1 to 8-1-12 | | Тор | 80 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight |
| 11 | Point | 6-9-1 | | Тор | 151 lb | 373 lb | 0 lb | 0 lb | F5 F5 |
| | Self Weight | | | | 10 PLF | | | | |

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





NE0618-037 **PAGE 15 OF 31**



Client: Project: Address:

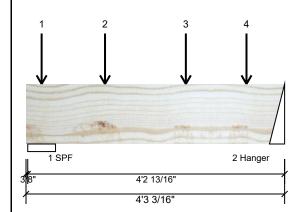
6/1/2018 Designer: SB

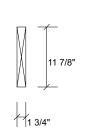
Job Name: CELESTIAL 1 EL-1

Project #:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL F5-B

Level: Ground Floor





Member Information Application: Floor (Residential) Type: Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load 40 PSF Floor Live: 15 PSF Dead:

| Unfactored | Reactions | UNPATTERNED | ib (Uplift) |
|------------|-----------|-------------|-------------|
| Dra | Livo | Dead | Snow |

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 537 | 386 | 0 | 0 |
| 2 | 294 | 120 | 0 | 0 |
| | | | | |

Bearings and Factored Reactions

| Bearing | Length | Сар. | React D/L lb | Total | Ld. Case | Ld. Comb. |
|---------------|--------|------|--------------|-------|----------|------------|
| 1 - SPF | 5.500" | 22% | 483 / 805 | 1288 | _L | 1.25D+1.5L |
| 2 - Hanger | 3.000" | 15% | 150 / 442 | 592 | _L | 1.25D+1.5L |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------------|-----------|-------------------|-------------|------------|---------|
| Moment | 572 ft-lb | 2'7 5/8" | 17130 ft-lb | 0.033 (3%) | 1.25D+1.5L | _L |
| Unbraced | 572 ft-lb | 2'7 5/8" | 11703 ft-lb | 0.049 (5%) | 1.25D+1.5L | _L |
| Shear | 585 lb | 3'1 1/16" | 5798 lb | 0.101 (10%) | 1.25D+1.5L | _L |
| Perm Defl in. | 0.001 (L/37483) | 2'4 1/4" | 0.127 (L/360) | 0.010 (1%) | D | Uniform |
| LL Defl inch | 0.003 (L/15209) | 2'4 5/8" | 0.127 (L/360) | 0.020 (2%) | L | LL |
| TL Defl inch | 0.004 (L/10819) | 2'4 9/16" | 0.191 (L/240) | 0.020 (2%) | D+L | LL |
| LL Cant | -0.000 (2L/19381) | Lt Cant | 0.200 (2L/480) | 0.000 (0%) | L | LL |
| TL Cant | -0.000 (2L/13782) | Lt Cant | 0.300 (2L/360) | 0.000 (0%) | D+L | LL |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-----------|----------|------------|-----------|--------|--------|------|------|----------|
| 1 | Point | 0-3-2 | | Near Face | 290 lb | 308 lb | 0 lb | 0 lb | F4 |
| 2 | Point | 1-3-10 | | Near Face | 73 lb | 194 lb | 0 lb | 0 lb | J3 |
| 3 | Point | 2-7-10 | | Near Face | 71 lb | 190 lb | 0 lb | 0 lb | J3 |

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-037 PAGE 16 OF 31



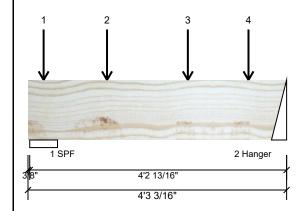
Client: Project: Address: Date: 6/1/2018 Designer: $\mathsf{S}\,\mathsf{B}$

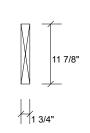
Job Name: CELESTIAL 1 EL-1

Project #:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL F5-B

Level: Ground Floor





Page 2 of 2

.Continued from page 1

ID Load Type Location Trib Width Side Live Wind Comments Dead Snow 4 Point 3-7-10 Near Face 52 lb 139 lb 0 lb 0 lb Self Weight 5 PLF

> REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Informing & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





NE0618-037 PAGE 17 OF 31



Simpson Strong-Tie®

Client: Project: Address:

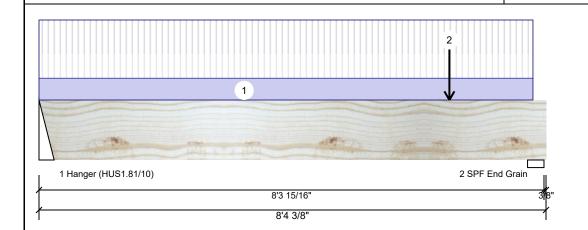
6/1/2018 Designer: SB

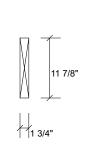
Job Name: CELESTIAL 1 EL-1

Project #

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL F6-A

Level: Ground Floor





| | Mem | ber | Info | rmation |
|--|-----|-----|------|---------|
|--|-----|-----|------|---------|

15 PSF

Actual

2025 ft-lb

2025 ft-lb

| Type: | Girder | Application: | Floor (Residential) |
|--------------------|--------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition | : Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| | | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 198 | 103 | 0 | 0 |
| 2 | 697 | 320 | 0 | 0 |

Bearings and Factored Reactions

| Bearing | Length | Cap. F | React D/L lb | Total | Ld. Case | Ld. Comb. | |
|----------------|--------|--------|--------------|-------|----------|------------|--|
| 1 - Hanger | 3.000" | 11% | 128 / 296 | 425 | L_ | 1.25D+1.5L | |
| 2 - SPF End | 3.250" | 34% | 400 / 1045 | 1445 | L_ | 1.25D+1.5L | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Analysis Results

Dead:

Analysis

Moment

Unbraced

Shear 1412 lb 7'2 7/16" 5798 lb 0.244 (24%) 1.25D+1.5L L_ Perm Defl in. 0.011 (L/9116) 4'10 3/16" 0.267 (L/360) 0.040 (4%) D Uniform LL Defl inch 0.022 (L/4336) 4'11 1/8" 0.267 (L/360) 0.080 (8%) L LL TL Defl inch 0.033 (L/2939) 4'10 13/16" 0.400 (L/240) 0.080 (8%) D+L LL LL Cant -0.000 Rt Cant 0.200 0.001 (0%) L LL (2L/2691) (2L/480)TL Cant -0.000 Rt Cant 0.300 0.001 (0%) D+L (2L/1832) (2L/360)

Location Allowed

6'9 1/4" 17130 ft-lb

6'9 1/4" 5617 ft-lb

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-------------|-----------------|-------------|------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 8-1-12 | (Span)0-8-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Point | 6-9-4 | | Тор | 340 lb | 780 lb | 0 lb | 0 lb | C5 |
| | Self Weight | | | | 5 PLF | | | | |

Comb.

0.118 (12%) 1.25D+1.5L L_

0.361 (36%) 1.25D+1.5L L_

Capacity

Case

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code Damaged Beams must not be used

Handling & Installation

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-037 PAGE 18 OF 31



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

6/1/2018 Designer: SB

Brg

1

2

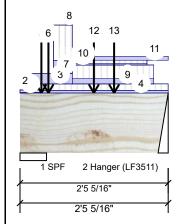
Job Name: CELESTIAL 1 EL-1

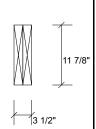
Project #:

F8-D Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED

Level: Ground Floor





Wind

O

0

0

0

| Member | Information |
|--------|-------------|
|--------|-------------|

| Type: | Girder |
|---------------------|--------|
| Plies: | 2 |
| Moisture Condition: | Dry |
| Deflection LL: | 360 |
| Deflection TL: | 240 |
| Importance: | Normal |
| General Load | |
| Floor Live: | 40 PSF |
| Dead: | 15 PSF |

Application: Floor (Residential) Design Method:

Load Sharing: No Deck:

Not Checked Vibration: Not Checked

Building Code: NBCC 2010 / OBC 2012

Bearings and Factored Reactions

Live

2902

383

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

1305

191

| Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. |
|---------------|--------|------|--------------|-------|----------|------------|
| 1 - SPF | 5.250" | 54% | 1631 / 4353 | 5984 | L | 1.25D+1.5L |
| 2 - Hanger | 2.000" | 16% | 238 / 575 | 813 | L | 1.25D+1.5L |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|--------------|----------------------|------------|---------------|------------|------------|---------|
| Moment | 528 ft-lb | 1'6 5/8" | 34261 ft-lb | 0.015 (2%) | 1.25D+1.5L | L |
| Unbraced | 528 ft-lb | 1'6 5/8" | 34261 ft-lb | 0.015 (2%) | 1.25D+1.5L | L |
| Shear | 853 lb | 1'4 3/8" | 11596 lb | 0.074 (7%) | 1.25D+1.5L | L |
| Perm Defl in | . 0.000 (L/58768) | 1'5 13/16" | 0.066 (L/360) | 0.010 (1%) | D | Uniform |
| LL Defl inch | 0.001 (L/27132) | 1'6 5/8" | 0.066 (L/360) | 0.010 (1%) | L | L |
| TL Defl inch | 0.001 (L/18576) | 1'6 3/8" | 0.098 (L/240) | 0.010 (1%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on full section width.

| ſ | ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|---|----|---------------|-----------------|--------------|------|--------|--------|-------|-------|------------------|
| l | 1 | Tie-In | 0-0-0 to 0-4-2 | (Span)0-10-4 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| l | 2 | Part. Uniform | 0-0-0 to 0-2-10 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |
| l | 3 | Part. Uniform | 0-0-0 to 0-10-6 | | Тор | 80 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight |

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-037 PAGE 19 OF 31

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

6/1/2018 Designer: $\mathsf{S}\,\mathsf{B}$

Job Name: CELESTIAL 1 EL-1

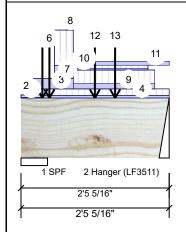
Level: Ground Floor

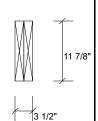
Project #:

Forex 2.0E-3000Fb LVL F8-D

1.750" X 11.875"

2-Ply - PASSED





Page 2 of 2

| Continued from | .Continued from page 1 | | | | | | | | |
|----------------|------------------------|------------------|-------------|-----------|---------|---------|-------|-------|------------------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 4 | Tie-In | 0-4-2 to 2-5-5 | (Span)2-7-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 5 | Point | 0-4-4 | | Тор | 965 lb | 2248 lb | 0 lb | 0 lb | F12 F12 |
| 6 | Point | 0-5-10 | | Far Face | 119 lb | 318 lb | 0 lb | 0 lb | J5 |
| 7 | Part. Uniform | 0-6-10 to 0-10-6 | | Тор | 103 PLF | 275 PLF | 0 PLF | 0 PLF | J5 |
| 8 | Part. Uniform | 0-9-2 to 0-10-6 | | Тор | 26 PLF | 68 PLF | 0 PLF | 0 PLF | J1 |
| 9 | Part. Uniform | 0-10-6 to 2-2-7 | | Тор | 40 PLF | 107 PLF | 0 PLF | 0 PLF | J5 |
| 10 | Part. Uniform | 0-10-6 to 2-1-2 | | Тор | 10 PLF | 27 PLF | 0 PLF | 0 PLF | J1 |
| 11 | Part. Uniform | 0-10-6 to 2-5-5 | | Тор | 31 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight |
| 12 | Point | 1-2-10 | | Near Face | 19 lb | 50 lb | 0 lb | 0 lb | J1 |
| 13 | Point | 1-6-10 | | Far Face | 107 lb | 285 lb | 0 lb | 0 lb | J5 |
| | Self Weight | | | | 10 PLF | | | | |

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS**

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. IVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

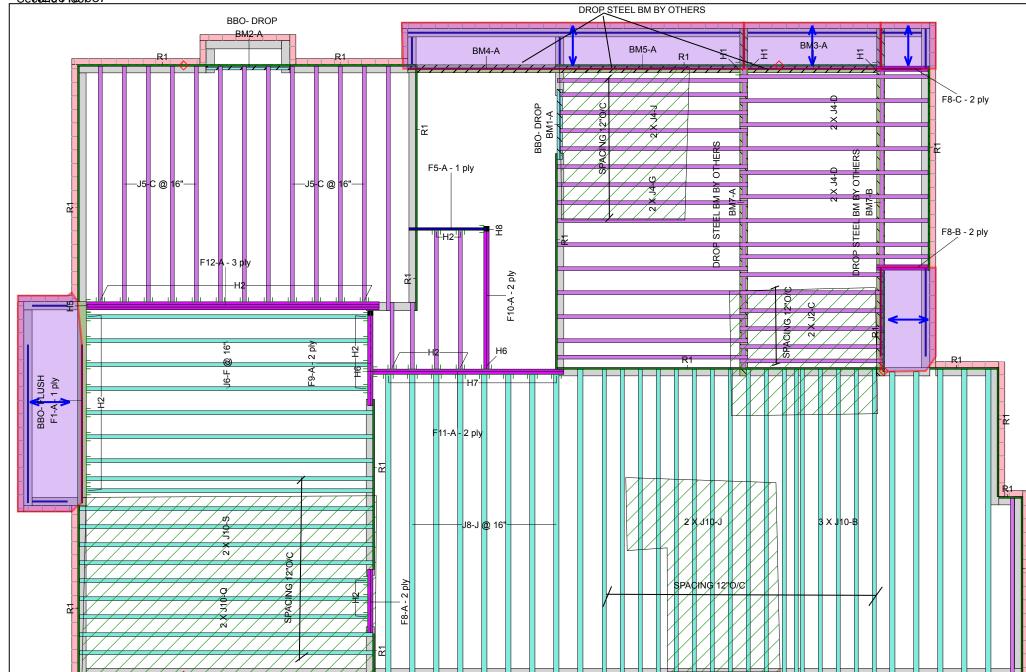
Manufacturer Info

APA: PR-L318









Architectural Drawing Info

JARDIN DESIGN GROUP 64 JARDIN DR, SUITE 3A VAUGHAN,ON L4K 3P3 Project # 17-55 Model: CELESTIAL Date: MAY 22,2018

1. OBC 2012 O.Reg 332/12 as amended

Legend

//////

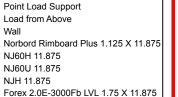
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

JOISTS SPACING 16"O/C UNLESS NOTED OTHERWISE



EWP Studio

Simpson Strong-Tie® Component Solutions™



1.75 X 9.5 (Dropped)

5.75 X 10.25

5.25 X 10.25 (Dropped)

THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

Sales Rep

Designer

Shipping

Canada

L4A 7X4

Floor

oads

Dead

Varies 20-0-0

905-642-4400

Second Floor

Desian Method

Deflection Joist

LL Span L/

TL Span L/ LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Ceilina:

Roof

Loads

Live

Dead

Snow

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Deck

Deflection Girder

Deck

Deflection Girder

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

Kott Lumber Company

Building Code NBCC 2010 / OBC

2012

15

480

360

480

360

360

240

480

360

OSB

5/8"

36

360

240

360

360

360

240

360

360

SPF Plywood

Nailed Only

Nailed & Glued

Gypsum 1/2"

Project

SB

LVL/LSL Pcs Length Label Description Width Depth Qty Plies F12 1.75 11.875 3 2.0E-3000Fb LVL F11 12-0-0 Forex 1.75 11.875 2.0E-3000Fb LVL CELESTIAL 1 EL-1 & 2 4BEDRM F10 1.75 11.875 8-0-0 Forex 2 2 Design Method 2.0E-3000Fb LVL LSD F9 1.75 11.875 6-0-0 Forex 2 2 Description 2.0E-3000Fb LVL GREEN YORK HOMES F5 1.75 11.875 6-0-0 Forex GRANELLI HOMES PROJECT 2.0E-3000Fb LVL BRAMPTON,ON 4-0-0 F8 Forex 1.75 11.875 2.0E-3000Fb LVL Created May 31, 2018 Width Depth Qty Plies Pcs Length

Joist Label Description J10 NJ60H 2.5 11.875 26 18-0-0 J6 NJ60H 2.5 11.875 9 16-0-0 J8 NJ60U 3.5 11.875 14 18-0-0 J5 NJH 2.5 11.875 13 14-0-0 2.5 11.875 J4 NJH 24 12-0-0 J3 NJH 2.5 11.875 10-0-0 1 J2 NJH 2.5 11.875 8-0-0 2.5 11.875 J1 NJH 2 4-0-0 Rim Board

Label Description Width | Depth | Qty Plies Pcs Length Norbord Rimboard R1 1.125 11.875 18 Plus 1.125 X 11.875 Blocking Label Description Width | Depth | Qty Plies Pcs Length

BLK1 NJH 2.5 11.875 LinFt Hanger

Beam/Girder Supported Member Label Pcs Description Skew Slope fasteners fasteners H1 3 Unknowr Hanger H2 34 LF2511 12 10d 1 #8x1 1/4WS H5 HUC610 (Max) 18 16d 8 16d Н6 2 HGUS410 46 16d 16 16d 8 LF3511 12 10d 2 #8x1 1/4WS H7 Н8 HUCQ1.81/10-

NOTES:

- Framer to verify dimensions on the architectural drawings
- Double joist only require filler/backer ply when supporting another member using a face-mounted hanger
- Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls
- Install single-ply flush window header along inside face of rimboard/rimjoist
- Refer to Nascor specifier guide for installation details. Squash blocks recommended to be installed at end bearing on all first leve
- joists which support loading from above exceeding two levels floor or roof. Load transfer blocks to be installed under all point loads. It shall be the framer's responsibility that floor joists and beams
- are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth) @ 16" o/c. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the esponsibility of others.

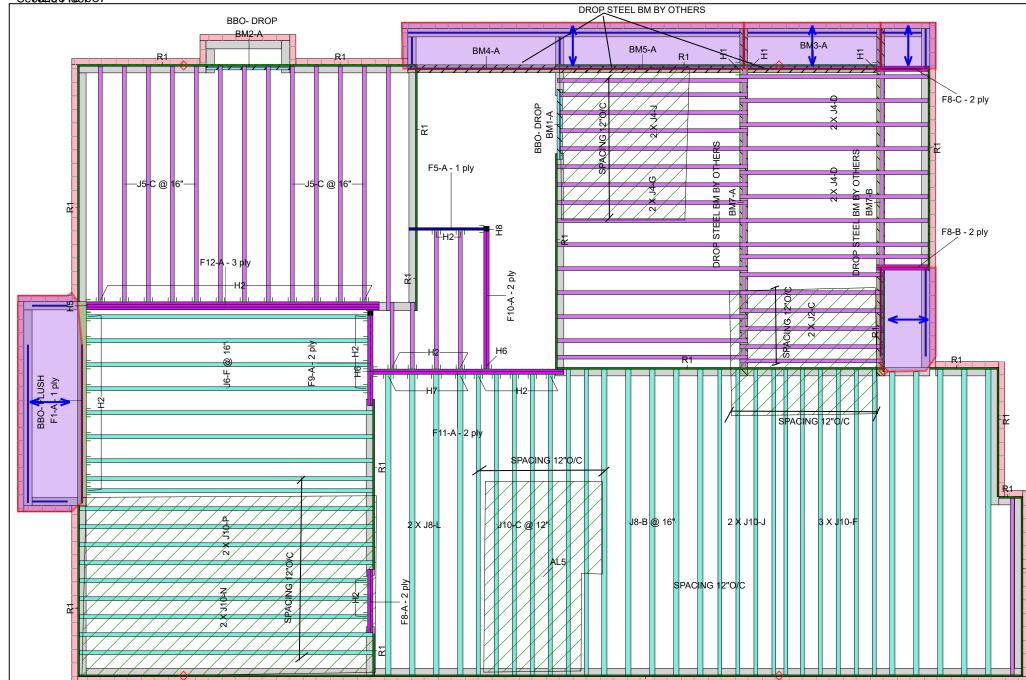
Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may be deviate from the architectural drawings. Project Engineer to review and approve the deviation prior to construction

REFER TO MULTIPLE MEMBER TO MEMBER

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Se**l\time0616**0037 PAGE 21 OF 31



Architectural Drawing Info

Legend

 \Diamond

//////

Point Load Support

Norbord Rimboard Plus 1.125 X 11.875

Forex 2.0E-3000Fb LVL 1.75 X 11.875

Load from Above

NJ60H 11 875

NJ60U 11.875

5.75 X 10.25

1.75 X 9.5 (Dropped)

5.25 X 10.25 (Dropped)

N.IH 11 875

JARDIN DESIGN GROUP 64 JARDIN DR, SUITE 3A VAUGHAN,ON L4K 3P3 Project # 17-55 Model: CELESTIAL Date: MAY 22,2018

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

JOISTS SPACING 16"O/C UNLESS NOTED OTHERWISE



EWP Studio

EWP Studio Version 18.32.085 Powered by iStruct™

Simpson Strong-Tie® Component Solutions™ THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

ENGINEERING NOTE PAGE ENP-2. THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. foundation walls and footings including anchorage of



NOTE PAGE IS AN INTEGRAL PART OF THIS Rim parallel to joists: 1-1/8" rimboard with

READ ALL NOTES ON THIS PAGE AND ON

J3 NJH 2.5 11.875 10-0-0 1 Project J2 NJH 2.5 11.875 8-0-0 8 J1 NJH 2.5 11.875 2 4-0-0 Builder's Project Rim Board Label Description Width | Depth | Qty Plies Pcs Length R1 Norbord Rimboard 1.125 11.875 18 Plus 1.125 X Canada 11.875 Blocking L4A 7X4 Label Description Width | Depth | Qty Plies Pcs Length BLK1 NJH 2.5 11.875 LinFt Varies 20-0-0 Hanger Beam/Girder Supported Member Label Pcs Description Skew Slope fasteners fasteners H1 3 Unknowr Floor Hanger oads H2 39 LF2511 12 10d 1 #8x1 1/4WS H5 HUC610 (Max) 18 16d 8 16d Dead Н6 2 HGUS410 46 16d 16 16d 4 LF3511 12 10d 2 #8x1 1/4WS H7 Н8 HUCQ1.81/10-NOTES: Framer to verify dimensions on the architectural drawings Double joist only require filler/backer ply when supporting another LL Span L/ member using a face-mounted hanger Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls. Install single-ply flush window header along inside face of rimboard/rimjoist Refer to Nascor specifier guide for installation details. Squash blocks recommended to be installed at end bearing on all first leve Decking joists which support loading from above exceeding two levels floor or roof. Deck Load transfer blocks to be installed under all point loads. It shall be the framer's responsibility that floor joists and beams **Thickness** are fastened as per the hanger manufacturer's standards. Fastener Vibration Refer to Multiple Member Connection Detail to ply to ply nailing or bolting Ceilina: Roof 2"x4" block (1/16" longer than rim depth) @ 16" o/c. All other components and structural elements supporting Loads the floor system such as beams, walls, columns and

CELESTIAL 1 EL-1 & 2 5BEDRM Design Method LSD

Description GREEN YORK HOMES GRANELLI HOMES PROJECT BRAMPTON,ON

Created May 31, 2018

Pcs Length

12-0-0

8-0-0

6-0-0

6-0-0

4-0-0

3

2

13

24

Pcs Length 26 18-0-0 9 16-0-0 Sales Rep 14 18-0-0 Designer 14-0-0 SB 12-0-0 Shipping

> **Kott Lumber Company** 14 Anderson Blvd

Stouffville, Ontario 905-642-4400

Second Floor Desian Method

Building Code NBCC 2010 / OBC 2012

15

480

360

480

360

360

240

480

360

OSB

36

360

240

360

360

360

240

360

360

Deflection Joist LL Span L/ TL Span L/ LL Cant 2L/ TL Cant 2L/ Deflection Girder

TL Span L/ LL Cant 2L/ TL Cant 2L/

5/8" Nailed & Glued Gypsum 1/2"

Live Dead Snow **Deflection Joist** LL Span L/ TL Span L/

LL Cant 2L/ TL Cant 2L/ Deflection Girder LL Span L/

TL Span L/ LL Cant 2L/ TL Cant 2L/ Decking

SPF Plywood Deck **Thickness** Nailed Only

KOT1

components and bracing for lateral stability are the

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may be deviate from the architectural

PASS THRU FRAMING SQUASH

POINT LOADS OVER BEARINGS

BLOCK IS REQUIRED AT ALL

REFER TO MULTIPLE MEMBER TO MEMBER

CONNECTION DETAIL FOR PLY TO PLY

NAILING OR BOLTING REQUIREMENTS.

drawings. Project Engineer to review and approve the deviation prior

esponsibility of others.

Second Floor

Label Description

Forex

Forex

Forex

Forex

Forex

Label Description

J10 NJ60H

J6 NJ60H

J8 NJ60U

J5 NJH

J4 NJH

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

Width Depth

1.75 11.875

1.75 11.875

1.75 11.875

Width Depth

2.5 11.875

2.5 11.875

3.5 11.875

2.5 11.875

2.5 11.875

11.875

11.875

11.875

1.75

1.75

1.75

Qty

Qty

Plies

2

Plies

LVL/LSL

F12

F11

F10

F9

F5

F8

Joist

NE0618-037 PAGE 22 OF 31



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

6/1/2018 Designer: SB

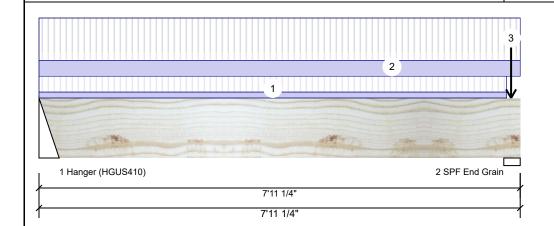
Job Name: CELESTIAL 1 EL-1

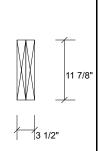
Project #

F10-A Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Member Information

| Type: | Girder | Application: | Floor (Residential) |
|---------------------|--------|----------------|----------------------|
| Plies: | 2 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 427 | 198 | 0 | 0 |
| 2 | 780 | 340 | 0 | 0 |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|----------|---------------|------------|------------|---------|
| Moment | 1544 ft-lb | 4' | 34261 ft-lb | 0.045 (5%) | 1.25D+1.5L | L |
| Unbraced | 1544 ft-lb | 4' | 31673 ft-lb | 0.049 (5%) | 1.25D+1.5L | L |
| Shear | 608 lb | 6'8 7/8" | 11596 lb | 0.052 (5%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.004 (L/19948) | 4' | 0.249 (L/360) | 0.020 (2%) | D | Uniform |
| LL Defl inch | 0.010 (L/9260) | 4' | 0.249 (L/360) | 0.040 (4%) | L | L |
| TL Defl inch | 0.014 (L/6324) | 4' | 0.373 (L/240) | 0.040 (4%) | D+L | L |

Bearings and Factored Reactions

| L | | | | | | | |
|---|-------------------------|--------|--------|--------------|-------|----------|------------|
| ſ | Bearing | Length | Cap. F | React D/L lb | Total | Ld. Case | Ld. Comb. |
| ı | 1 - Hanger | 4.000" | 9% | 248 / 641 | 888 | L | 1.25D+1.5L |
| | 2 - SPF End Grain | 3.250" | 19% | 425 / 1170 | 1594 | L | 1.25D+1.5L |

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



ID Load Type Trib Width Side Live Wind Comments Location Dead Snow 40 PSF Tie-In 0-0-0 to 7-8-9 (Span)1-5-7 Top 15 PSF 0 PSF 0 PSF 1 2 Tie-In 0-0-0 to 7-11-4 (Span) 15 PSF 40 PSF 0 PSF 0 PSF Top 3-10-10

> Point 7-9-7 Far Face 147 lb 10 PLF Self Weight

3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

366 lb

Manufacturer Info

APA: PR-L318

0 lb



0 lb F5





PAGE 23 OF 31 NE0618-037



Client: Project: Address:

6/1/2018 Designer: SB

Job Name: CELESTIAL 1 EL-1

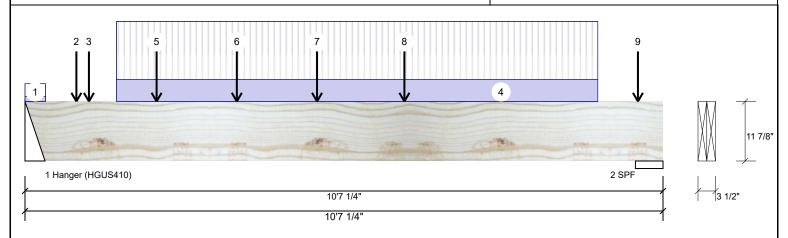
Project #:

Forex 2.0E-3000Fb LVL F11-A

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor



| Member Info | rmation | | | Unfactored Reactions UNPATTERNED lb (Uplift) | | | | | | |
|------------------|---------|----------------|----------------------|--|------------|-------------------|----------------|------------|--|--|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind | | |
| Plies: | 2 | Design Method: | LSD | 1 | 2178 | 883 | 0 | 0 | | |
| Moisture Conditi | on: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 2156 | 884 | 0 | 0 | | |
| Deflection LL: | 360 | Load Sharing: | No | | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | | |
| General Load | | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings | s and Fact | tored Reactions | | | | |
| Dead: | 15 PSF | | | Bearing | Length | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | | |
| | | | | 1 - | 4.000" | 42% 1103 / 3267 | 4370 L | 1.25D+1.5L | | |
| | | | | Hanger | | | | | | |
| Analysis Resu | ılts | | | 2-SPF | 5.500" | 37% 1105 / 3234 | 4339 L | 1.25D+1.5L | | |

| . , | | | | | | |
|---------------|----------------|------------|---------------|-------------|------------|---------|
| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
| Moment | 11617 ft-lb | 5'2 3/8" | 34261 ft-lb | 0.339 (34%) | 1.25D+1.5L | L |
| Unbraced | 11617 ft-lb | 5'2 3/8" | 29666 ft-lb | 0.392 (39%) | 1.25D+1.5L | L |
| Shear | 4502 lb | 1'3 1/8" | 11596 lb | 0.388 (39%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.049 (L/2443) | 5'2 15/16" | 0.331 (L/360) | 0.150 (15%) | D | Uniform |
| LL Defl inch | 0.119 (L/998) | 5'2 11/16" | 0.331 (L/360) | 0.360 (36%) | L | L |
| TL Defl inch | 0.168 (L/709) | 5'2 13/16" | 0.497 (L/240) | 0.340 (34%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|----------------|-------------|-----------|---------|---------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 0-4-2 | (Span)3-8-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Point | 0-10-4 | | Near Face | 139 lb | 370 lb | 0 lb | 0 lb | J8 |
| 3 | Point | 1-0-12 | | Far Face | 30 lb | 80 lb | 0 lb | 0 lb | J1 |
| 4 | Part. Uniform | 1-6-4 to 9-6-4 | | Near Face | 124 PLF | 330 PLF | 0 PLF | 0 PLF | |
| 5 | Point | 2-2-4 | | Far Face | 31 lb | 84 lb | 0 lb | 0 lb | J1 |
| 6 | Point | 3-6-4 | | Far Face | 75 lb | 200 lb | 0 lb | 0 lb | J2 |

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318

KOTT





NE0618-037 PAGE 24 OF 31



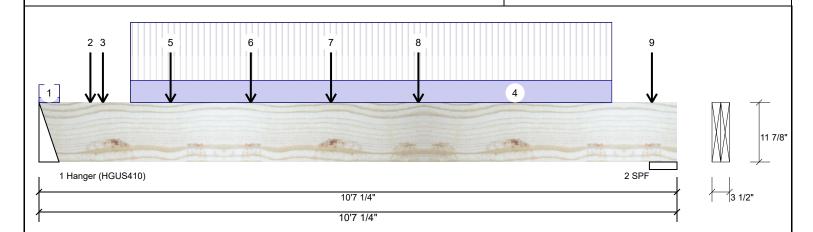
Client: Project: Address:

6/1/2018 Designer: SB

Job Name: CELESTIAL 1 EL-1

Project #:

2-Ply - PASSED Level: Second Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL F11-A



| Continued | Continued from page 1 | | | | | | | | | | |
|-----------|-----------------------|---------------------|-----------|--------|--------|------|------|----------|--|--|--|
| ID | Load Type | Location Trib Width | Side | Dead | Live | Snow | Wind | Comments | | | |
| 7 | Point | 4-10-4 | Far Face | 81 lb | 215 lb | 0 lb | 0 lb | J2 | | | |
| 8 | Point | 6-3-11 | Far Face | 198 lb | 427 lb | 0 lb | 0 lb | F10 | | | |
| 9 | Point | 10-2-4 | Near Face | 110 lb | 293 lb | 0 lb | 0 lb | J8 | | | |
| | Self Weight | | | 10 PLF | | | | | | | |

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS**

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400





Page 2 of 2

PAGE 25 OF 31 NE0618-037

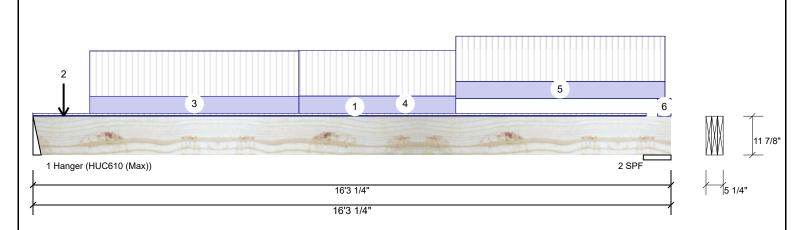
EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

6/1/2018 SB Designer:

Job Name: CELESTIAL 1 EL-1

Project #

3-Ply - PASSED Level: Second Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL F12-A



| Member Info | rmation | | | Unfactored Reactions UNPATTERNED lb (Uplift) | | | | | | | |
|--------------------|---------|----------------|----------------------|--|------------|-------------------|----------------|------------|--|--|--|
| Туре: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind | | | |
| Plies: | 3 | Design Method: | LSD | 1 | 2091 | 898 | 0 | 0 | | | |
| Moisture Condition | on: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 2248 | 965 | 0 | 0 | | | |
| Deflection LL: | 360 | Load Sharing: | Yes | | | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | | | |
| General Load | | | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearing | s and Fact | tored Reactions | | | | | |
| Dead: | 15 PSF | | | Bearing | Length | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | | | |
| | | | | 1 - | 2.500" | 44% 1123 / 3136 | 4259 L | 1.25D+1.5L | | | |
| | | | | Hanger | | | | | | | |
| Analysis Resu | lts | | | 2 - SPF | 8.469" | 17% 1206 / 3373 | 4578 L | 1.25D+1.5L | | | |
| | | | | | | | | | | | |

L

Case Analysis Actual Location Allowed Capacity Comb. Moment 0.309 (31%) 1.25D+1.5L L 16539 ft-lb 7'10 3/4" 53447 ft-lb Unbraced 16539 ft-lb 7'10 3/4" 49731 ft-lb 0.333 (33%) 1.25D+1.5L L 4714 lb 1'1 5/8" 17394 lb 0.271 (27%) 1.25D+1.5L L Shear Perm Defl in. 0.109 (L/1707) 7'10 11/16" 0.516 (L/360) 0.210 (21%) D Uniform LL Defl inch 0.254 (L/731) 7'10 11/16" 0.516 (L/360) 0.490 (49%) L L

7'10 11/16" 0.774 (L/240) 0.470 (47%) D+L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



TL Defl inch **Design Notes**

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.

0.363 (L/512)

- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

| / Edicial Sicildeniess ratio based on fall section with. | | | | | | | | | | |
|--|----|---------------|-------------------|-------------|----------|--------|---------|-------|-------|----------|
| | ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| | 1 | Tie-In | 0-0-0 to 15-7-10 | (Span)0-7-5 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| | 2 | Point | 0-9-6 | | Far Face | 123 lb | 328 lb | 0 lb | 0 lb | J5 |
| | 3 | Part. Uniform | 1-5-6 to 6-9-6 | | Far Face | 97 PLF | 258 PLF | 0 PLF | 0 PLF | |
| | 4 | Part. Uniform | 6-9-6 to 10-9-6 | | Far Face | 98 PLF | 261 PLF | 0 PLF | 0 PLF | |
| | 5 | Part. Uniform | 10-9-6 to 16-1-6 | | Far Face | 97 PLF | 258 PLF | 0 PLF | 0 PLF | |
| | 6 | Tie-In | 15-11-2 to 16-3-4 | (Span)3-8-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| | | Self Weight | | | | 14 PLF | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318

KOTT





PAGE 26 OF 31 NE0618-037



Client: Project: Address:

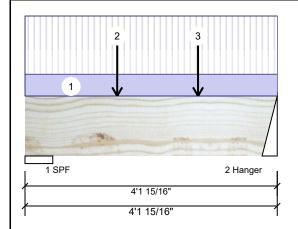
6/1/2018 Designer: SB

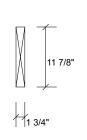
Job Name: CELESTIAL 1 EL-1

Project #

1.750" X 11.875" - PASSED F5-A Forex 2.0E-3000Fb LVL

Level: Second Floor





Wind

0

0

1.25D+1.5L

1.25D+1.5L

0

748 L

733 L

Member Information Application: Type: Floor (Residential) Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: Deflection LL: 360 No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load 40 PSF Floor Live: 15 PSF Dead:

| Unfactored | Reactions | UNPATTER | NED lb (Uplift) | |
|------------|-----------|----------|-----------------|--|
| Bra | Live | Dead | Snow | |

151

147

373

366

1

2

2 -

Hanger

1 - SPF 5.500"

3.000"

| Bearings and Fac | tored Reactions | | |
|------------------|-------------------|----------------|-----------|
| Bearing Length | Cap. React D/L lb | Total Ld. Case | Ld. Comb. |

188 / 559

184 / 549

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|-------------|---------------|------------|------------|---------|
| Moment | 711 ft-lb | 2'3 1/4" | 17130 ft-lb | 0.041 (4%) | 1.25D+1.5L | L |
| Unbraced | 711 ft-lb | 2'3 1/4" | 12369 ft-lb | 0.057 (6%) | 1.25D+1.5L | L |
| Shear | 540 lb | 2'11 13/16" | 5798 lb | 0.093 (9%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.001 (L/29239) | 2'2 1/2" | 0.119 (L/360) | 0.010 (1%) | D | Uniform |
| LL Defl inch | 0.004 (L/11617) | 2'2 9/16" | 0.119 (L/360) | 0.030 (3%) | L | L |
| TL Defl inch | 0.005 (L/8314) | 2'2 9/16" | 0.179 (L/240) | 0.030 (3%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

13%

19%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-----------------|------------|-----------|--------|--------|-------|-------|----------|
| 1 | Part. Uniform | 0-0-0 to 4-1-15 | | Тор | 30 PLF | 80 PLF | 0 PLF | 0 PLF | |
| 2 | Point | 1-6-4 | | Near Face | 72 lb | 191 lb | 0 lb | 0 lb | J2 |
| 3 | Point | 2-10-4 | | Near Face | 81 lb | 215 lb | 0 lb | 0 lb | J2 |
| | Self Weight | | | | 5 PLF | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-037 PAGE 27 OF 31



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: Project: Address: 6/1/2018

Designer: SB

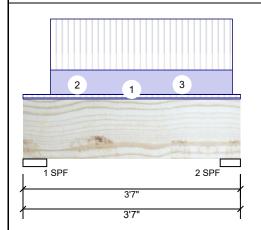
Job Name: CELESTIAL 1 EL-1

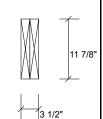
Project #:

F8-A Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Member Information

| Type: | Girder |
|---------------------|--------|
| Plies: | 2 |
| Moisture Condition: | Dry |
| Deflection LL: | 360 |
| Deflection TL: | 240 |
| Importance: | Normal |
| General Load | |
| Floor Live: | 40 PSF |
| Dead: | 15 PSF |

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 470 | 239 | 0 | 0 |
| 2 | 550 | 277 | 0 | 0 |

Bearings and Factored Reactions

| Bearing | Length | Cap. Re | act D/L lb | Total | Ld. Case | Ld. Comb. |
|---------|--------|---------|------------|-------|----------|-------------|
| 1 - SPF | 4.700" | 10% | 299 / 704 | 1003 | L | 1.25D+1.5L |
| 0 CDE | 4.000" | 1.40/ | 247 / 025 | 1170 | | 1 25D 11 5I |

Analysis Results

| | Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---|---------------|--------------------|------------|---------------|------------|------------|---------|
| | Moment | 794 ft-lb | 1'9 7/8" | 34261 ft-lb | 0.023 (2%) | 1.25D+1.5L | L |
| | Unbraced | 794 ft-lb | 1'9 7/8" | 34261 ft-lb | 0.023 (2%) | 1.25D+1.5L | L |
| | Shear | 1016 lb | 1'3 13/16" | 11596 lb | 0.088 (9%) | 1.25D+1.5L | L |
| | Perm Defl in. | 0.001 (L/43361) | 1'9 7/8" | 0.100 (L/360) | 0.010 (1%) | D | Uniform |
| | LL Defl inch | 0.002 (L/21753) | 1'9 7/8" | 0.100 (L/360) | 0.020 (2%) | L | L |
| | TL Defl inch | 0.002 (L/14486) | 1'9 7/8" | 0.149 (L/240) | 0.020 (2%) | D+L | L |
| _ | | | | | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARING**



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings

| | Bottom braced at bearings. | | | | |
|----|---|--------------|------------|------|------|
| 6 | Lateral slenderness ratio based on full sed | ction width. | | | |
| ID | Load Type | Location | Trib Width | Side | Dead |

| o zaterar eremae. | | TOOGUSTI TITUUTI | | | | | | | | |
|-------------------|---------------|------------------|-------------|----------|---------|---------|-------|-------|----------|--|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments | |
| 1 | Tie-In | 0-0-0 to 3-7-0 | (Span)1-0-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | |
| 2 | Tapered Start | 0-0-0 | | Тор | 1 PLF | 0 PLF | 0 PLF | 0 PLF | | |
| | End | 3-7-0 | | | 2 PLF | 0 PLF | 0 PLF | 0 PLF | | |
| 3 | Part. Uniform | 0-5-7 to 3-5-7 | | Far Face | 150 PLF | 316 PLF | 0 PLF | 0 PLF | | |
| | Self Weight | | | | 10 PLF | | | | | |
| | | | | | | | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







PAGE 28 OF 31 NE0618-037

EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

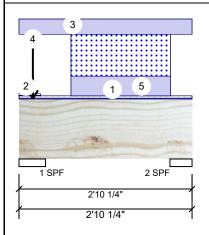
6/1/2018 Designer: SB

Job Name: CELESTIAL 1 EL-1

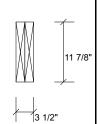
Project #:

1.750" X 11.875" 2-Ply - PASSED Forex 2.0E-3000Fb LVL F8-B

Level: Second Floor



15 PSF



| Member Information | | | | | | |
|---------------------|--------|----------------|----------------------|--|--|--|
| Туре: | Girder | Application: | Floor (Residential) | | | |
| Plies: | 2 | Design Method: | LSD | | | |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 | | | |
| Deflection LL: | 360 | Load Sharing: | No | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | |
| Importance: | Normal | Vibration: | Not Checked | | | |
| General Load | | | | | | |
| Floor Live: | 40 PSF | | | | | |
| | | | | | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | vvina |
|-----|------|------|------|-------|
| 1 | 20 | 514 | 745 | 0 |
| 2 | 17 | 232 | 215 | 0 |
| | | | | |

Bearings and Factored Reactions

| Bearing | Length | Сар. | React D/L lb | Total | Ld. Case | Ld. Comb. |
|---------|--------|------|--------------|-------|----------|---------------------|
| 1 - SPF | 5.250" | 16% | 642 / 1128 | 1770 | L | 1.25D+1.5S +0.5L |
| 2 - SPF | 4.375" | 7% | 290 / 331 | 622 | L | 1.25D+1.5S +0.5L |

Analysis Results

Dead:

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|------------|---------------|------------|---------------------|---------|
| Moment | 318 ft-lb | 1'6 1/16" | 34261 ft-lb | 0.009 (1%) | 1.25D+1.5S +0.5L | L |
| Unbraced | 318 ft-lb | 1'6 1/16" | 34261 ft-lb | 0.009 (1%) | 1.25D+1.5S +0.5L | L |
| Shear | 82 lb | 1'4 3/8" | 11596 lb | 0.007 (1%) | 1.25D+1.5S +0.5L | L |
| Perm Defl in. | 0.000 (L/64863) | 1'5 13/16" | 0.073 (L/360) | 0.010 (1%) | D | Uniform |
| LL Defl inch | 0.000 (L/59654) | 1'6 1/16" | 0.073 (L/360) | 0.010 (1%) | S+0.5L | L |
| TL Defl inch | 0.001 (L/31078) | 1'6" | 0.109 (L/240) | 0.010 (1%) | D+S+0.5L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







PAGE 29 OF 31 NE0618-037



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

6/1/2018 Designer:

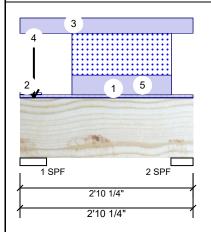
 $\mathsf{S}\,\mathsf{B}$

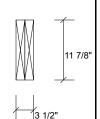
Job Name: CELESTIAL 1 EL-1

Project #:

1.750" X 11.875" 2-Ply - PASSED Forex 2.0E-3000Fb LVL F8-B

Level: Second Floor





Page 2 of 2

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-----------------|--------------|------|---------|--------|---------|-------|------------------|
| 1 | Tie-In | 0-0-0 to 2-10-4 | (Span)0-7-3 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 0-4-4 | (Span)0-4-13 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Part. Uniform | 0-0-0 to 2-10-4 | | Тор | 80 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight |
| 4 | Point | 0-2-12 | | Тор | 307 lb | 0 lb | 601 lb | 0 lb | F2 F2 |
| 5 | Part. Uniform | 0-10-5 to 2-6-0 | | Тор | 103 PLF | 0 PLF | 219 PLF | 0 PLF | |
| | Self Weight | | | | 10 PLF | | | | |

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS**

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





PAGE 30 OF 31 NE0618-037



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

6/1/2018 Designer: SB

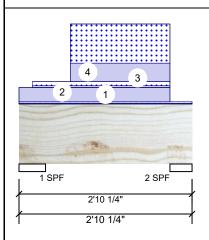
Job Name: CELESTIAL 1 EL-1

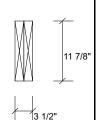
Project #:

F8-C Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





| Mem | ber l | Infor | mation |
|-----|-------|-------|--------|
| | | | |

| Type: | Girder |
|---------------------|--------|
| Plies: | 2 |
| Moisture Condition: | Dry |
| Deflection LL: | 360 |
| Deflection TL: | 240 |
| Importance: | Normal |
| General Load | |
| Floor Live: | 40 PSF |

40 PSF 15 PSF Application: Floor (Residential) Design Method:

NBCC 2010 / OBC 2012

Load Sharing: No Deck:

Building Code:

Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 13 | 217 | 175 | 0 |
| 2 | 12 | 211 | 237 | 0 |

Bearings and Factored Reactions

| Bearing Length | Cap. R | eact D/L lb | Total | Ld. Case | Ld. Comb. | |
|----------------|--------|-------------|-------|----------|---------------------|--|
| 1 - SPF 5.250" | 5% | 271 / 268 | 540 | L | 1.25D+1.5S +0.5L | |
| 2 - SPF 4.375" | 7% | 263 / 356 | 619 | L | 1.25D+1.5S | |

Analysis Results

Dead:

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------------|------------|---------------|------------|------------|---------|
| Moment | 342 ft-lb | 1'6" | 33918 ft-lb | 0.010 (1%) | 1.25D+1.5S | L |
| Unbraced | 342 ft-lb | 1'6" | 33918 ft-lb | 0.010 (1%) | 1.25D+1.5S | L |
| Shear | 84 lb | 1'4 3/8" | 11480 lb | 0.007 (1%) | 1.25D+1.5S | L |
| Perm Defl in. | . 0.000 (L/61871) | 1'5 13/16" | 0.073 (L/360) | 0.010 (1%) | D | Uniform |
| LL Defl inch | 0.000 (L/53786) | 1'6 1/16" | 0.073 (L/360) | 0.010 (1%) | S+0.5L | L |
| TL Defl inch | 0.001 (L/28775) | 1'5 15/16" | 0.109 (L/240) | 0.010 (1%) | D+S+0.5L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

| 6 Lateral sienderness ratio based on full section width. | | | | | | | | | | |
|--|----|---------------|------------------|-------------|------|---------|--------|---------|-------|------------------|
| | ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| | 1 | Tie-In | 0-0-0 to 2-10-4 | (Span)0-5-5 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| | 2 | Part. Uniform | 0-0-0 to 2-5-14 | | Тор | 80 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight |
| | 3 | Part. Uniform | 0-2-10 to 2-5-14 | | Тор | 10 PLF | 0 PLF | 23 PLF | 0 PLF | |
| | 4 | Part. Uniform | 0-10-3 to 2-5-14 | | Тор | 103 PLF | 0 PLF | 219 PLF | 0 PLF | |
| | | Self Weight | | | | 10 PLF | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





NE0618-037 PAGE 31 OF 31



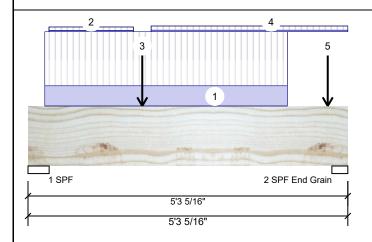
Client: Project: Address: Date: 6/1/2018 Designer: S B

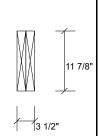
Job Name: CELESTIAL 1 EL-1

Project #:

F9-A Forex 2.0E-3000Fb LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Second Floor





| Туре: | Girder | Application: | Floor (Residential) |
|---------------------|--------|----------------|----------------------|
| Plies: | 2 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| | | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 2238 | 908 | 0 | 0 |
| 2 | 1602 | 647 | 0 | 0 |

Bearings and Factored Reactions

| Bearing | Length | Cap. F | React D/L lb | Total | Ld. Case | Ld. Comb. |
|----------------|--------|--------|--------------|-------|----------|------------|
| 1 - SPF | 4.188" | 50% | 1135 / 3357 | 4492 | L | 1.25D+1.5L |
| 2 - SPF End | 3.153" | 39% | 809 / 2402 | 3211 | L | 1.25D+1.5L |

Analysis Results

Dead:

Member Information

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-------------|------------|---------|
| Moment | 6315 ft-lb | 1'10 5/8" | 34261 ft-lb | 0.184 (18%) | 1.25D+1.5L | L |
| Unbraced | 6315 ft-lb | 1'10 5/8" | 33194 ft-lb | 0.190 (19%) | 1.25D+1.5L | L |
| Shear | 3943 lb | 1'3 5/16" | 11596 lb | 0.340 (34%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.008 (L/7299) | 2'2 5/8" | 0.160 (L/360) | 0.050 (5%) | D | Uniform |
| LL Defl inch | 0.019 (L/2954) | 2'2 5/8" | 0.160 (L/360) | 0.120 (12%) | L | L |
| TL Defl inch | 0.027 (L/2103) | 2'2 5/8" | 0.239 (L/240) | 0.110 (11%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.

15 PSF

- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

| • | | | | | | | | | | |
|----|---------------|-----------------|-------------|-----------|---------|---------|-------|-------|----------|--|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments | |
| 1 | Part. Uniform | 0-3-5 to 4-3-5 | | Far Face | 117 PLF | 313 PLF | 0 PLF | 0 PLF | | |
| 2 | Tie-In | 0-4-2 to 1-8-14 | (Span)1-0-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | |
| 3 | Point | 1-10-10 | | Near Face | 883 lb | 2178 lb | 0 lb | 0 lb | F11 | |
| 4 | Tie-In | 2-0-6 to 5-3-5 | (Span)1-2-8 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | |
| 5 | Point | 4-11-5 | | Far Face | 114 lb | 303 lb | 0 lb | 0 lb | J6 | |
| | Self Weight | | | | 10 PLF | | | | | |

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumbe

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info
Forex
APA: PR-L318



