

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0518-228 GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 2

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

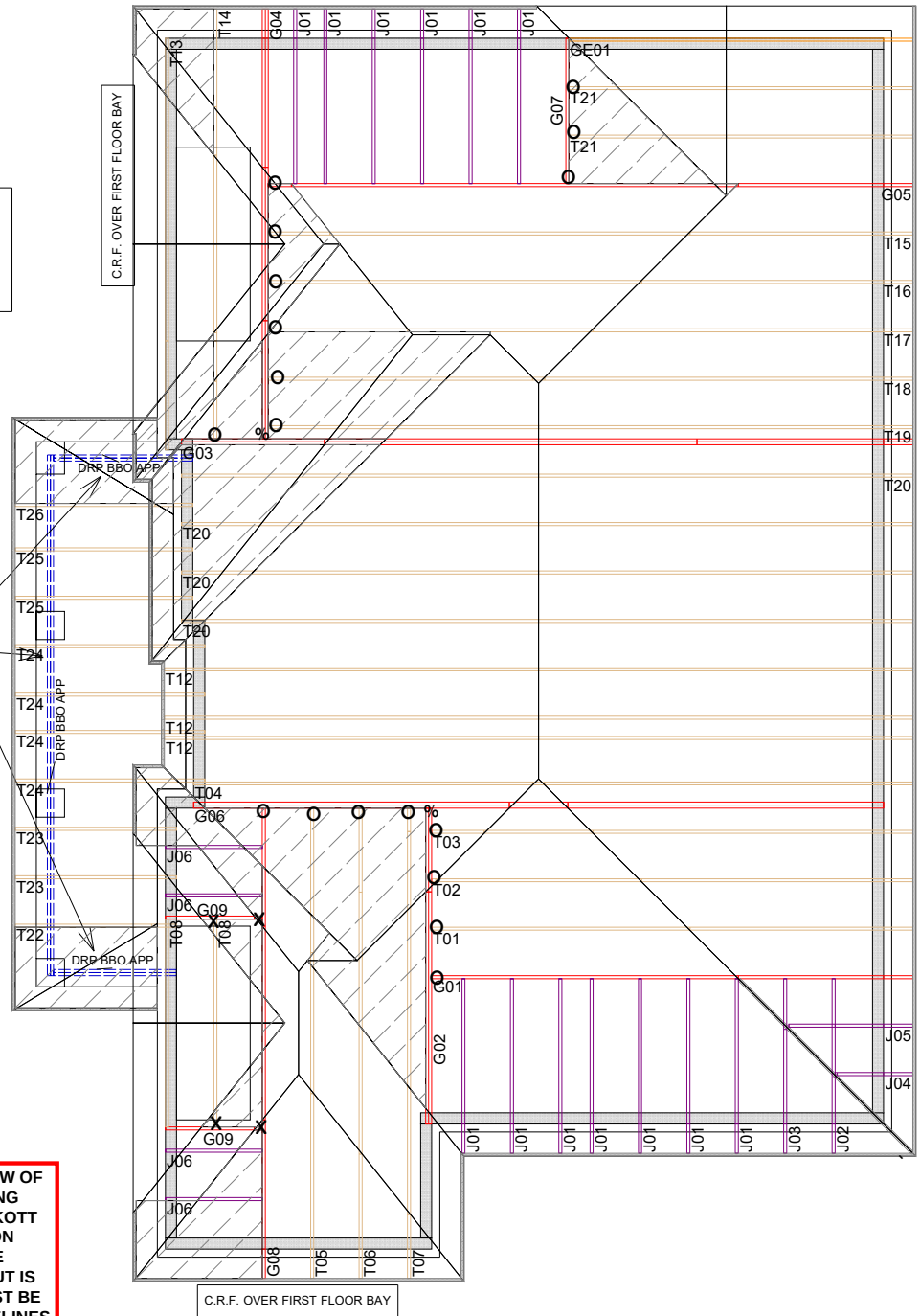
- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

Layout based on plans provided by
Jardin Design Group Inc.
Revision #2 dated May 22, 2018

PORCH BEAMS ASSUMED TO BE
2x2x10, NO BEAM SCHEDULE
PROVIDED ON PLANS
UNDERSIDE OF BEAMS TO BE 8"
ABOVE FIN. FIRST FLOOR



THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF
STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING
AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT
LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON
THE CALCULATION PAGE OF EACH COMPONENT. THE
PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS
SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE
INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES
AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL
FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED
AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY
AND PERMANENT BRACINGS OF THE ROOF AND BUILDING
ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER
AND TO CONFORM TO BCSI GUIDELINES.



ROOF TRUSS LAYOUT



CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24
- HGUS26

O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS,
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

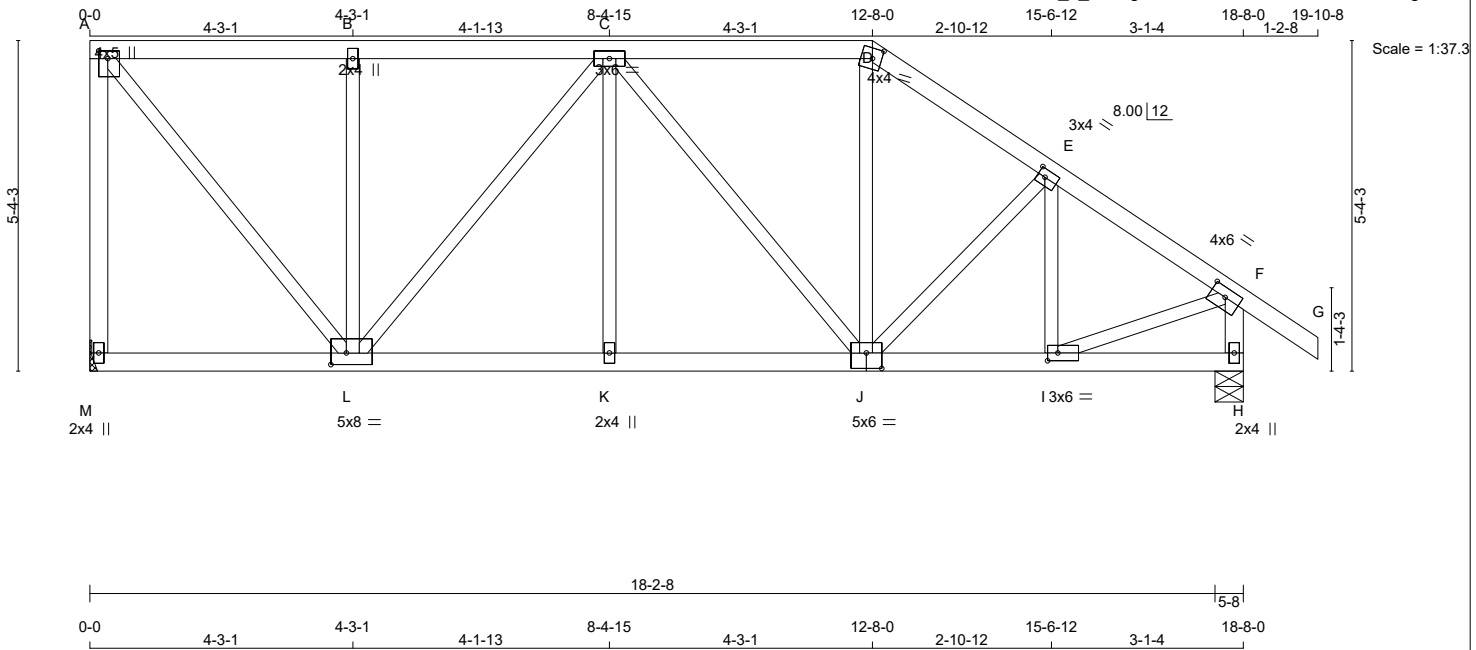
THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011

ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	AMILIA 12 ELE 2
Customer:	GREEN YORK HOMES
Project:	GRANELLI HOME
Location:	BRAMPTON, ON
Drawn By:	DM
Date:	5/30/2018

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS
MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C RAFTER THAT
CROSS MEET OVER TRUSSES TO BE 2X4 SPF @ 24" O/C WITH 2X4 VERT POST TO THE
TRUSS BELOW EA CROSS PT. VERT POS. THAN 6' TO HAVE A LATERAL BRACING SO THAT
THE DISTANCE BETEN PT & VERT ROWS OF BRACING DOES NOT EXCEED 6'



TOTAL WEIGHT = 87 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
M - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	5.0	1.50	1.75
B	TMW+w	MT20	2.0	4.0		
C	TMWWW-t	MT20	3.0	6.0		
D	TTW-m	MT20	4.0	4.0	2.00	1.75
E	TMWW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW-t	MT20	4.0	6.0	1.75	3.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	3.0	6.0	1.50	2.00
J	BSWWW-l	MT20	5.0	6.0	3.00	3.00
K	BMW+w	MT20	2.0	4.0		
L	BMWWW-t	MT20	5.0	8.0	2.25	3.00
M	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 12-8-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
M	1728	0	1728	0	0	MECHANICAL
H	1674	0	1674	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1213	847 / 0	0 / 0	0 / 0	0 / 0	366 / 0	0 / 0
H	1176	814 / 0	0 / 0	0 / 0	0 / 0	362 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
M-A	-1667 / 0	0.0	0.0 0.85 (1)	A-L	0 / 1848	0.46 (1)	
A-B	-1190 / 0	-145.9	-145.9 0.40 (1)	L-B	-668 / 0	0.29 (1)	
B-C	-1190 / 0	-145.9	-145.9 0.41 (1)	L-C	-757 / 0	0.59 (1)	
C-D	-1487 / 0	-145.9	-145.9 0.43 (1)	K-C	0 / 157	0.06 (4)	
D-E	-1798 / 0	-77.4	-77.4 0.15 (1)	C-J	-288 / 0	0.22 (1)	
E-F	-1706 / 0	-77.4	-77.4 0.15 (1)	J-D	0 / 301	0.08 (4)	
F-G	0 / 28	-77.4	-77.4 0.10 (1)	J-E	-5 / 75	0.02 (1)	
H-F	-1624 / 0	0.0	0.0 0.18 (1)	I-E	-373 / 0	0.08 (1)	
				I-F	0 / 1505	0.37 (1)	
M-L	0 / 0	-34.4	-34.4 0.16 (4)				
L-K	0 / 1671	-34.4	-34.4 0.37 (1)				
K-J	0 / 1671	-34.4	-34.4 0.35 (1)				
J-I	0 / 1429	-34.4	-34.4 0.31 (1)				
I-H	0 / 0	-34.4	-34.4 0.08 (4)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	12-8-0	-348	-348	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CprimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 6-0-0
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL)= L/999 (0.09")

CSI: TC=0.85/1.00 (A-M:1), BC=0.37/1.00 (K-L:1), WB=0.59/1.00 (C-L:1), SSI=0.32/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



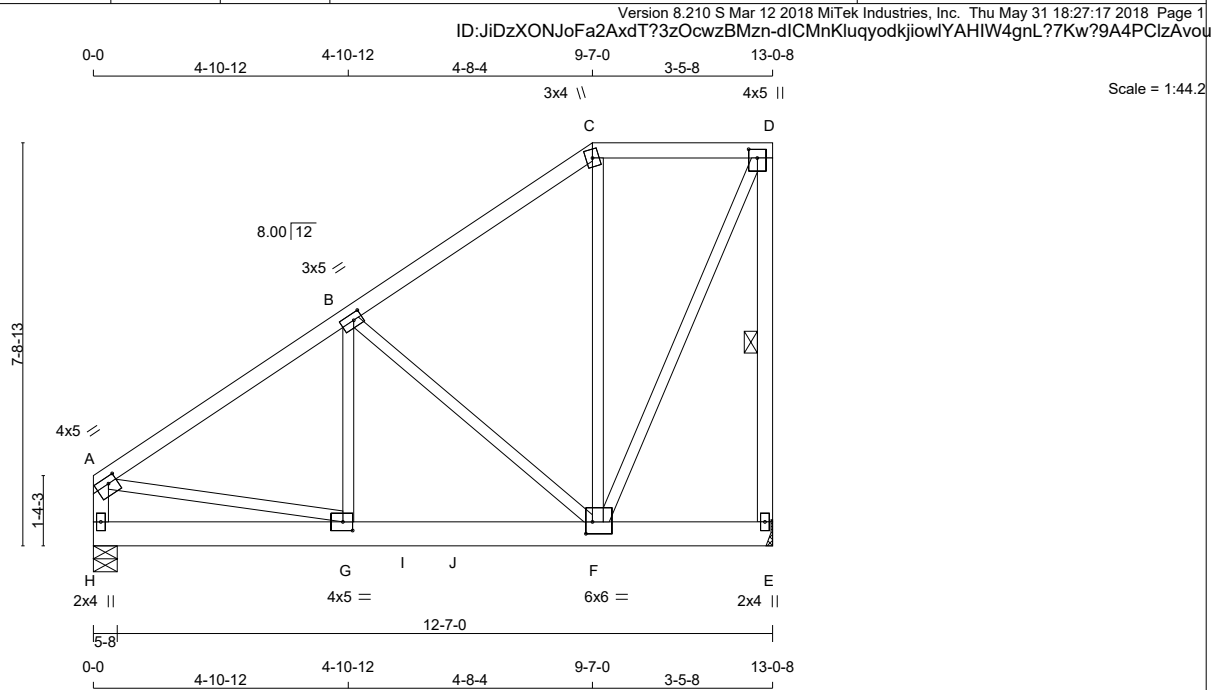
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 2	DRWG NO.
NE0518-228	G01	1	1	TRUSS DESC.		PAGE 4 OF 53

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:27:16 2018 Page 2
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PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 1.00)

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THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 73 = 146 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
D - E	12	TOP
H - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H - E	12	SIDE(193.7)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
E	3225	0	0	MECHANICAL
H	2089	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	2266	1571 / 0	0 / 0	0 / 0	0 / 0	695 / 0	0 / 0
H	1468	1018 / 0	0 / 0	0 / 0	0 / 0	450 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E. DBS = 10-0-0 . CBF = 176 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A - B	-2809 / 0	-77.4	-77.4 0.20 (1)	5.28	G - B	0 / 1304	0.16 (1)
B - C	-1487 / 0	-77.4	-77.4 0.16 (1)	6.25	B - F	-1516 / 0	0.55 (1)
C - D	-1224 / 0	-77.4	-77.4 0.09 (1)	6.25	F - C	0 / 536	0.07 (1)
E - D	-2815 / 0	0.0	0.0 0.38 (1)	5.47	F - D	0 / 2952	0.37 (1)
H - A	-2173 / 0	0.0	0.0 0.12 (1)	7.55	A - G	0 / 2399	0.30 (1)
H - G	0 / 0	-18.2	-18.2 0.23 (1)	10.00			
G - I	0 / 2356	-18.2	-18.2 0.60 (1)	10.00			
I - J	0 / 2356	-18.2	-18.2 0.60 (1)	10.00			
J - F	0 / 2356	-405.7	-405.7 0.60 (1)	10.00			
F - E	0 / 0	-405.7	-405.7 0.33 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	6-0-12	-1727	-1727	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 7-0-0
START SPAN CARRIED = 18-8-0
END DISTANCE = 13-0-8
END SPAN CARRIED = 18-8-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.60/1.00 (F-G:1) , WB=0.55/1.00 (B-F:1) , SSI=0.53/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	5.0	1.50	2.00
B	TMWW-t	MT20	3.0	5.0	1.50	2.00
C	TTW+m	MT20	3.0	4.0		
D	TMVW+p	MT20	4.0	5.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	6.0	6.0	2.75	1.50
G	BMWW-t	MT20	4.0	5.0	2.00	2.25
H	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 1726.5 lbs FACTORED DOWN AT 6-0-12
ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354 1667	822 2284 1656

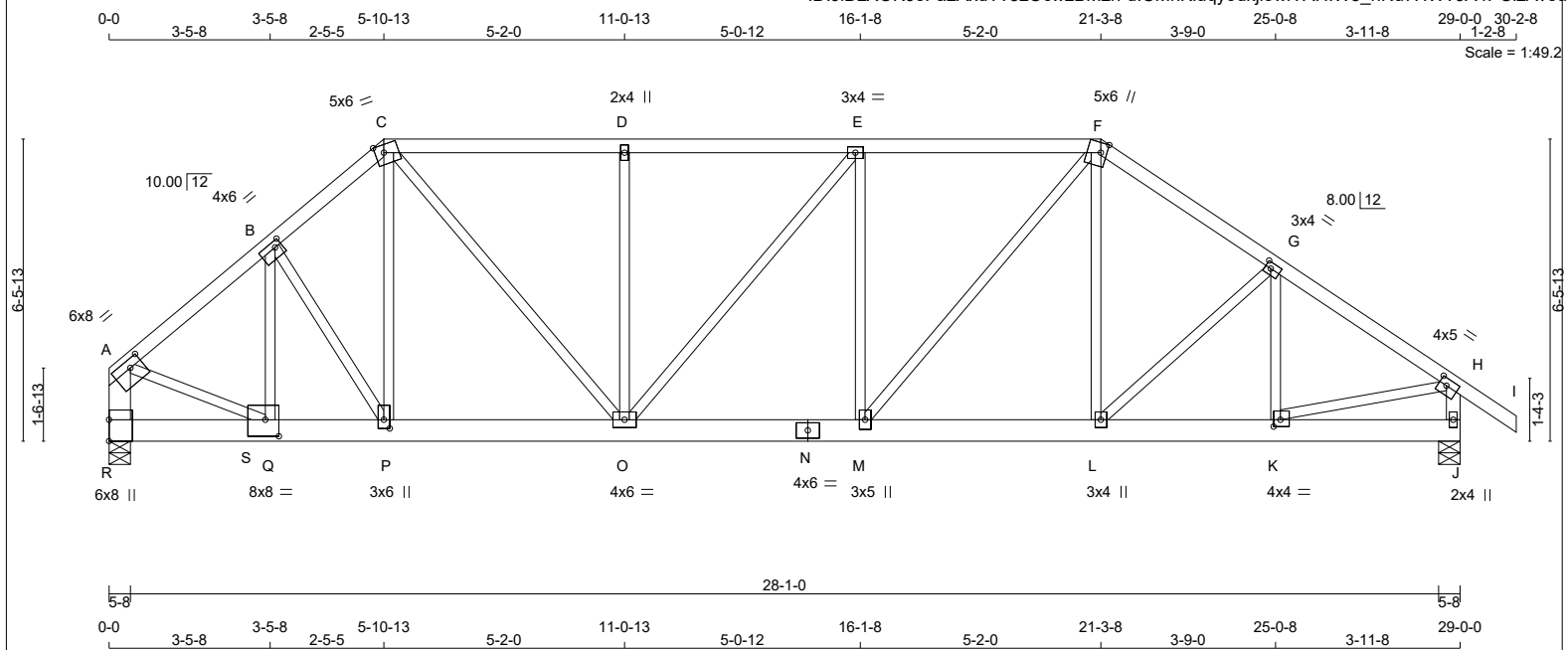
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.44 (A) (INPUT = 1.00)

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THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 150 = 299 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - A	2x6	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-F	12	TOP
F-I	12	TOP
J-H	12	TOP
R-A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-N	12	SIDE(168.3)
N-J	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
R	7134	0	7134	0	0	5-8	5-8	5-8	5-8
J	2187	0	2187	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS		1ST LCASE		COMBINED		SNOW		LIVE		PERM.LIVE		WIND		DEAD		SOIL	
JT	5012	3475	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
R	1534	1077	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS									
MAX. FACTORED		FACTORED		MAX.		MAX. FACTORED		MEMB.		MAX.		MAX.	
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	MEMB.	FORCE	MAX	MEMB.	FORCE
	(LBS)	(PLF)		CSI (LC)	UNBRAC								
FR-TO		FROM	TO		LENGTH	FR-TO							
A-B	-7031 / 0	-77.4	-77.4	0.29 (1)	3.52	Q-B	0 / 3860	0.48 (1)					
B-C	-4832 / 0	-77.4	-77.4	0.13 (1)	4.30	B-P	-3189 / 0	0.60 (1)					
C-D	-3638 / 0	-77.4	-77.4	0.21 (1)	4.75	P-C	0 / 2967	0.37 (1)					
D-E	-3638 / 0	-77.4	-77.4	0.21 (1)	4.75	C-O	-170 / 0	0.11 (1)					
E-F	-3133 / 0	-77.4	-77.4	0.20 (1)	5.05	O-D	-415 / 0	0.14 (1)					
F-G	-2646 / 0	-77.4	-77.4	0.12 (1)	5.48	O-E	0 / 791	0.10 (1)					
G-H	-2523 / 0	-77.4	-77.4	0.12 (1)	5.58	M-E	-1034 / 0	0.34 (1)					
H-I	0 / 28	-77.4	-77.4	0.05 (1)	10.00	M-F	0 / 1470	0.18 (1)					
R-A	-6433 / 0	0.0	0.0	0.24 (1)	5.86	L-F	0 / 78	0.01 (4)					
J-H	-2134 / 0	0.0	0.0	0.12 (1)	7.60	L-G	0 / 94	0.01 (1)					
						K-G	-481 / 0	0.06 (1)					
R-S	0 / 0	-354.9	-354.9	0.25 (1)	10.00	A-Q	0 / 5691	0.70 (1)					
S-Q	0 / 0	-18.2	-18.2	0.25 (1)	10.00	K-H	0 / 2173	0.27 (1)					
Q-P	0 / 5400	-18.2	-18.2	0.48 (1)	10.00								
P-O	0 / 3748	-18.2	-18.2	0.28 (1)	10.00								
O-N	0 / 3133	-18.2	-18.2	0.22 (1)	10.00								
N-M	0 / 3133	-18.2	-18.2	0.22 (1)	10.00								
M-L	0 / 2183	-18.2	-18.2	0.16 (1)	10.00								
L-K	0 / 2113	-18.2	-18.2	0.16 (1)	10.00								
K-J	0 / 0	-18.2	-18.2	0.03 (1)	10.00								

FACTORED CONCENTRATED LOADS (LBS)		LOC.		LC1		MAX-		MAX+		FACE		DIR.		TYPE	
JT	Q	3-5-8	-5439	-5439	---	BACK	VERT	TOTAL							

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 16-6-8
END DISTANCE = 3-0-0
END SPAN CARRIED = 16-6-8
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF G.S.L. (DEFINED BY USER)

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.29/1.00 (A-B:1), BC=0.48/1.00 (P-Q:1), WB=0.70/1.00 (A-Q:1), SSI=0.23/1.00 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	8.0	2.00	3.25
B	TMWW-t	MT20	4.0	6.0	1.50	1.75
C	TTWW-m	MT20	5.0	6.0	2.00	2.25
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	3.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	5.0	1.75	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.75	1.75
L	BMWW+t	MT20	3.0	4.0		
M	BMWW+t	MT20	3.0	5.0		
N	BS-t	MT20	4.0	6.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BMWW+t	MT20	3.0	6.0	2.25	1.50
Q	BMWW-t	MT20	8.0	8.0	4.25	3.50
R	BMV1+t	MT20	6.0	8.0	5.50	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 5438.6 lbs FACTORED DOWN AT 3-5-8
ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

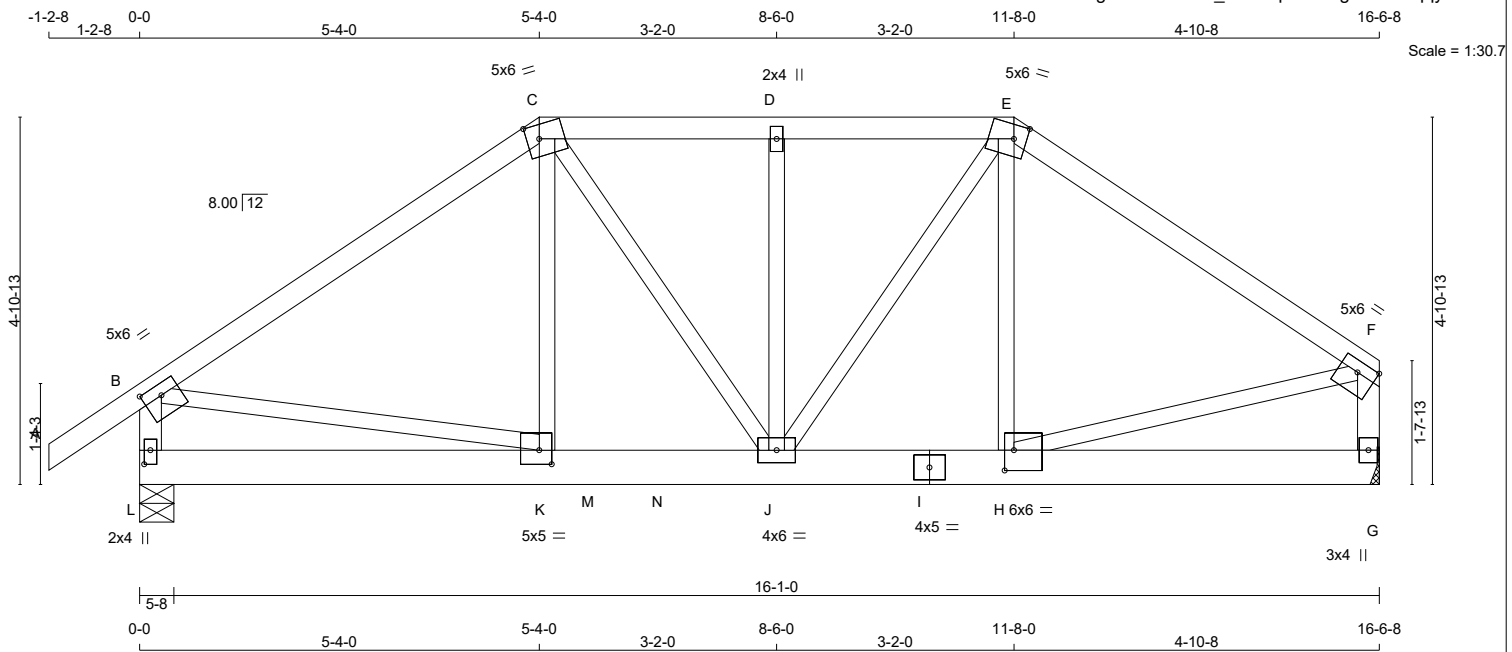
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)

JSI METAL= 0.67 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 81 = 163 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A- C	12	TOP
C- E	12	TOP
E- F	12	TOP
L- B	12	TOP
G- F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L- I	9	SIDE(274.4)
I- G	9	SIDE(274.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
L	3995	0	3995	0	5-8
G	5439	0	5439	0	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2804	1957 / 0	0 / 0	0 / 0	0 / 0	847 / 0	0 / 0
G	3821	2650 / 0	0 / 0	0 / 0	0 / 0	1172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-77.4	-77.4 0.05 (1)	10.00	K-C	0 / 1522	0.19 (1)
B-C	-5270 / 0	-77.4	-77.4 0.41 (1)	3.93	C-J	0 / 1309	0.16 (1)
C-D	-5167 / 0	-77.4	-77.4 0.15 (1)	4.16	J-D	-225 / 0	0.04 (1)
D-E	-5167 / 0	-77.4	-77.4 0.15 (1)	4.16	J-E	0 / 1268	0.16 (1)
E-F	-5298 / 0	-77.4	-77.4 0.34 (1)	3.96	H-E	0 / 1589	0.20 (1)
L-B	-3997 / 0	0.0	0.0 0.22 (1)	5.92	B-K	0 / 4445	0.55 (1)
G-F	-4240 / 0	0.0	0.0 0.24 (1)	5.77	H-F	0 / 4538	0.56 (1)
L-K	0 / 0	-18.2	-18.2 0.10 (1)	10.00			
K-M	0 / 4415	-18.2	-18.2 0.68 (1)	10.00			
M-N	0 / 4415	-18.2	-18.2 0.68 (1)	10.00			
N-J	0 / 4415	-567.0	-567.0 0.68 (1)	10.00			
J-I	0 / 4438	-567.0	-567.0 0.68 (1)	10.00			
I-H	0 / 4438	-567.0	-567.0 0.68 (1)	10.00			
H-G	0 / 0	-567.0	-567.0 0.47 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	6-0-12	-2516	-2516	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 7-0-0
START SPAN CARRIED = 25-5-0
END DISTANCE = 16-6-8
END SPAN CARRIED = 25-5-0
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF G.S.L. (DEFINED BY USER)

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.41/1.00 (B-C:1), BC=0.68/1.00 (J-K:1), WB=0.56/1.00 (F-H:1), SSI=0.72/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-t	MT20	5.0	6.0	1.75	Edge
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	6.0	6.0	3.25	1.50
I	BS-t	MT20	4.0	5.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	5.0	2.25	2.00
L	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2516.5 lbs FACTORED DOWN AT 6-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.68 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 2	DRWG NO.
NE0518-228	G05	1	1	TRUSS DESC.		PAGE 12 OF 53

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:27:19 2018 Page 2
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HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 55.0 lbs FACTORED DOWN AT 24-5-10
ON TOP CHORD, AND 1043.3 lbs FACTORED
DOWN AT 13-0-12 ON BOTTOM CHORD.
DESIGN FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

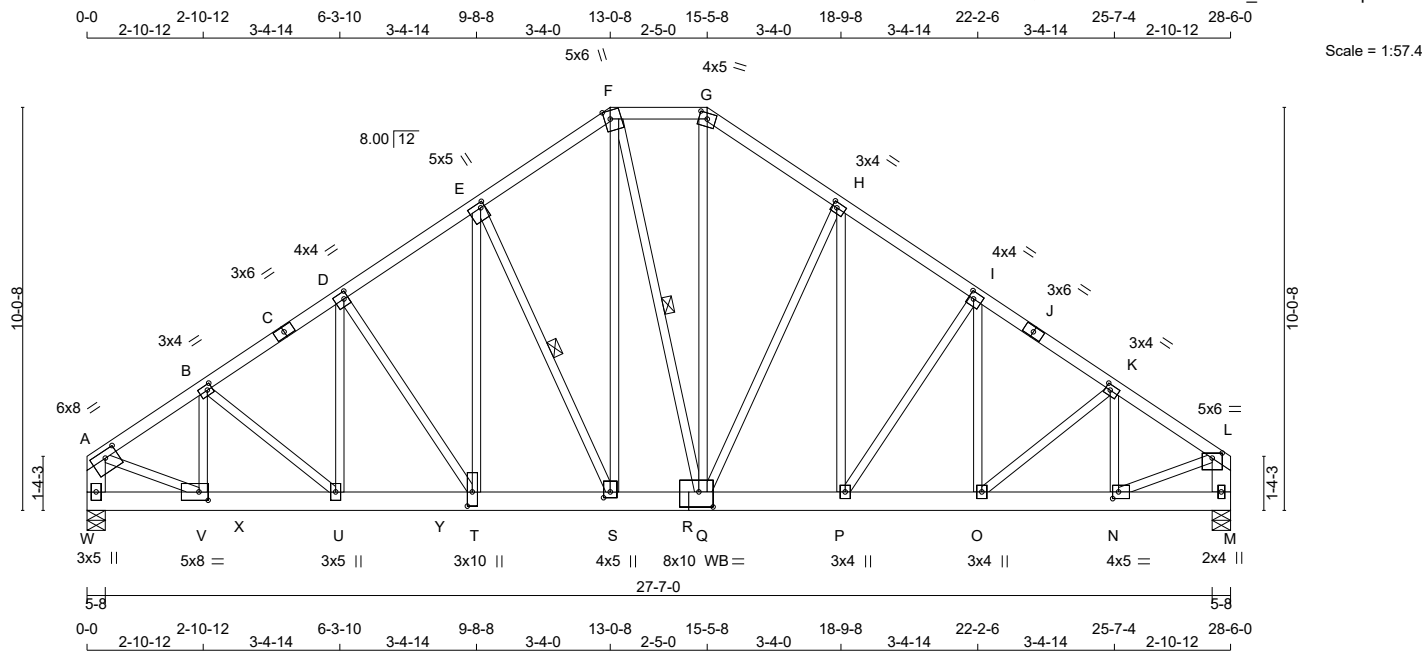
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - G	2x4	DRY No.2	SPF
G - J	2x4	DRY No.2	SPF
J - L	2x4	DRY No.2	SPF
W - A	2x6	DRY No.2	SPF
M - L	2x6	DRY No.2	SPF
W - R	2x6	DRY No.2	SPF
R - M	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - F	12	TOP
F - G	12	TOP
G - J	12	TOP
J - L	12	TOP
W - A	12	TOP
M - L	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W - R	12	SIDE(188.2)
R - M	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
W	6489	0	6489	0	5-8
M	3053	0	3053	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4559	3161 / 0	0 / 0	0 / 0	0 / 0	1398 / 0	0 / 0
M	2145	1487 / 0	0 / 0	0 / 0	0 / 0	658 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

- 1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S. DBS = 6-0-0 . CBF = 160 LBS.
- 1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q. DBS = 20-0-0 . CBF = 179 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A - B	-7609 / 0	-77.4 -77.4	0.27 (1)	V - B	-192 / 0	0.02 (1)	
B - C	-7569 / 0	-77.4 -77.4	0.29 (1)	B - U	-40 / 0	0.01 (1)	
C - D	-7569 / 0	-77.4 -77.4	0.29 (1)	U - D	0 / 1613	0.20 (1)	
D - E	-6533 / 0	-77.4 -77.4	0.22 (1)	D - T	-1564 / 0	0.52 (1)	
E - F	-4398 / 0	-77.4 -77.4	0.14 (1)	T - E	0 / 4767	0.59 (1)	
F - G	-3351 / 0	-77.4 -77.4	0.08 (1)	E - S	-4262 / 0	0.98 (1)	
G - H	-4020 / 0	-77.4 -77.4	0.12 (1)	S - F	0 / 3627	0.45 (1)	
H - I	-4061 / 0	-77.4 -77.4	0.12 (1)	F - Q	-1429 / 0	0.51 (1)	
I - J	-4066 / 0	-77.4 -77.4	0.12 (1)	Q - G	0 / 2030	0.25 (1)	
J - K	-4066 / 0	-77.4 -77.4	0.12 (1)	Q - H	-114 / 0	0.08 (1)	
K - L	-3501 / 0	-77.4 -77.4	0.09 (1)	P - H	-155 / 5	0.08 (1)	
W - A	-6393 / 0	0.0 0.0	0.23 (1)	P - I	-14 / 0	0.06 (1)	
M - L	-2993 / 0	0.0 0.0	0.11 (1)	O - I	-255 / 0	0.06 (1)	
				O - K	0 / 608	0.08 (1)	
W - V	0 / 0	-18.2 -18.2	0.07 (1)	N - K	-957 / 0	0.09 (1)	
V - X	0 / 6332	-18.2 -18.2	0.57 (1)	A - V	0 / 6659	0.82 (1)	
X - U	0 / 6332	-394.7 -394.7	0.57 (1)	N - L	0 / 3072	0.38 (1)	
U - Y	0 / 6301	-394.7 -394.7	0.57 (1)				
Y - T	0 / 6301	-18.2 -18.2	0.57 (1)				
T - S	0 / 5438	-18.2 -18.2	0.43 (1)				
S - R	0 / 3684	-18.2 -18.2	0.42 (1)				
R - Q	0 / 3684	-18.2 -18.2	0.42 (1)				
Q - P	0 / 3378	-18.2 -18.2	0.40 (1)				
P - O	0 / 3385	-18.2 -18.2	0.27 (1)				
O - N	0 / 2921	-18.2 -18.2	0.22 (1)				
N - M	0 / 0	-18.2 -18.2	0.04 (1)				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 23.3	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.3	PSF
TOTAL LOAD = 33.6 PSF			

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 3-10-0
START SPAN CARRIED = 18-2-8
END DISTANCE = 8-10-0
END SPAN CARRIED = 18-2-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.29/1.00 (B-D:1), BC=0.57/1.00 (T-U:1), WB=0.98/1.00 (E-S:1), SSI=0.26/1.00 (T-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	6.0	8.0	2.00 3.75
B, H, K					
B	TMWW-t	MT20	3.0	4.0	1.50 1.50
C	TS-t	MT20	3.0	6.0	
D	TMWW-t	MT20	4.0	4.0	2.00 1.25
E	TMWW-t	MT20	5.0	5.0	1.50 1.25
F	TTWW+m	MT20	5.0	6.0	Edge 1.75
G	TTW-m	MT20	4.0	5.0	1.75 2.50
I	TMWW-t	MT20	4.0	4.0	2.00 1.50
J	TS-t	MT20	3.0	6.0	
L	TMVW-p	MT20	5.0	6.0	1.50 3.00
M	BMV1+p	MT20	2.0	4.0	
N	BMWW-t	MT20	4.0	5.0	2.00 1.75
O	BMWW+t	MT20	3.0	4.0	
P	BMWW+t	MT20	3.0	4.0	
Q					
R					
R	BSWWW-I	MT20	8.0	10.0	4.50 4.25
S	BMWW+t	MT20	4.0	5.0	1.75 2.00
T	BMWW+t	MT20	3.0	10.0	4.25 1.50
U	BMWW+t	MT20	3.0	5.0	
V	BMWW-t	MT20	5.0	8.0	2.50 2.75
W	BMV1+p	MT20	3.0	5.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1710.8 lbs FACTORED DOWN AT 2-10-12, AND 3223.9 lbs FACTORED DOWN AT 9-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)								
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	
T	9-8-8	-3224	-3224	---	FRONT	VERT	TOTAL	
V	2-10-12	-1711	-1711	---	FRONT	VERT	TOTAL	

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354 1667	822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)

JSI METAL= 0.72 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



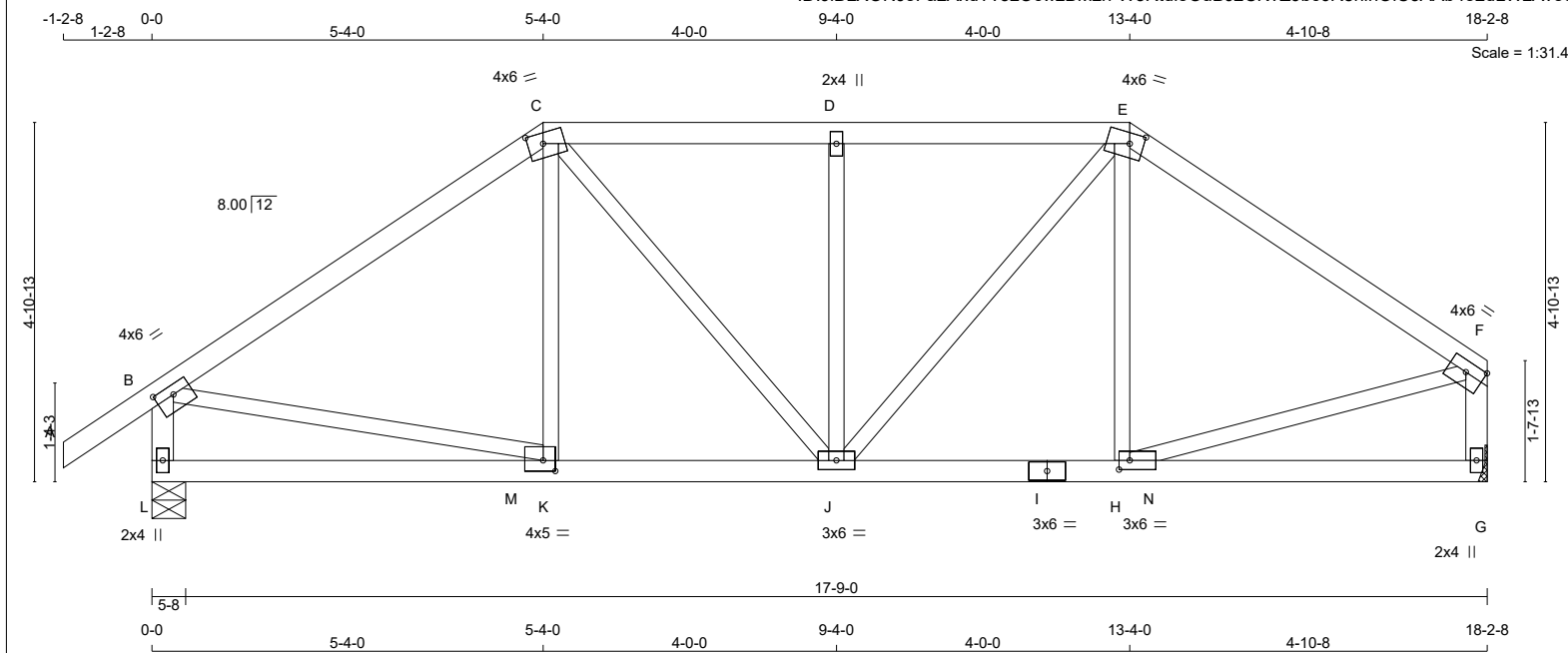
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0518-228	G07	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 2	PAGE 16 OF 53
TRUSS DESC.					

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ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-W3RtdioOuBJ2CK?Z9bc6R8hpfOoa3Dbb4o2dLWzAvog

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.24 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 76 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	3.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TMVW-t	MT20	4.0	6.0	1.75	Edge
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-t	MT20	3.0	6.0	1.50	1.75
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	5.0	1.75	2.00
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 263.6 lbs FACTORED DOWN AT 5-4-0, AND 240.9 lbs FACTORED DOWN AT 13-4-0 ON TOP CHORD, AND 505.3 lbs FACTORED DOWN AT 4-11-12, AND 505.3 lbs FACTORED DOWN AT 13-8-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	1783	0	1783	0	5-8	5-8
G	1709	0	1709	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1252	875 / 0	0 / 0	0 / 0	0 / 0	377 / 0	0 / 0
G	1202	829 / 0	0 / 0	0 / 0	0 / 0	372 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-77.4	-77.4 0.10 (1)	10.00	K-C	0 / 382	0.09 (1)
B-C	-1943 / 0	-77.4	-77.4 0.57 (1)	4.19	C-J	0 / 136	0.03 (1)
C-D	-1712 / 0	-77.4	-77.4 0.26 (1)	4.77	J-D	-367 / 0	0.13 (1)
D-E	-1712 / 0	-77.4	-77.4 0.26 (1)	4.77	J-E	0 / 271	0.07 (1)
E-F	-1839 / 0	-77.4	-77.4 0.46 (1)	4.42	H-E	0 / 264	0.08 (4)
L-B	-1691 / 0	0.0	0.0 0.19 (1)	6.35	B-K	0 / 1643	0.41 (1)
G-F	-1621 / 0	0.0	0.0 0.19 (1)	6.45	H-F	0 / 1582	0.39 (1)
L-M	0 / 0	-31.4	-31.4 0.29 (1)	10.00			
M-K	0 / 0	-31.4	-31.4 0.29 (1)	10.00			
K-J	0 / 1623	-18.2	-18.2 0.58 (1)	10.00			
J-I	0 / 1534	-18.2	-18.2 0.54 (1)	10.00			
I-H	0 / 1534	-18.2	-18.2 0.54 (1)	10.00			
H-N	0 / 0	-31.4	-31.4 0.27 (1)	10.00			
N-G	0 / 0	-31.4	-31.4 0.27 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-4-0	-264	-264	---	BACK	VERT	TOTAL
E	13-4-0	-241	-241	---	BACK	VERT	TOTAL
M	4-11-12	-505	-505	---	BACK	VERT	TOTAL
N	13-8-4	-505	-505	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	0.0 PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 5-4-0
RIGHT SETBACK = 4-10-8
END SETBACK = 5-4-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO BACK SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 5-4-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 5-4-0
RIGHT SETBACK = 4-10-8
END SETBACK = 5-4-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO BACK SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 4-10-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0518-228	G08	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 2	PAGE 18 OF 53

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ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-W3RtdioOuBJ2CK?Z9bcc6R8hihOIC3AAb4o2dLWzAvog

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.58/1.00 (J-K:1) ,
WB=0.41/1.00 (B-K:1) , SSI=0.50/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

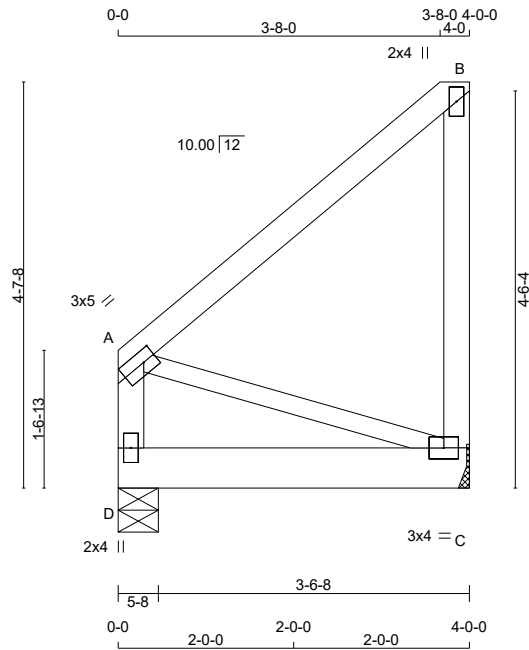
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.58 (I) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 22 = 44 lb [M]

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - A	2x4	DRY	No.2	SPF	
D - C	2x6	DRY	No.2	SPF	
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	1.75
B	TMV+p	MT20	2.0	4.0		
C	BMVW1-t	MT20	3.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
C	506	0	506	0	0	MECHANICAL
D	506	0	506	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
C	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	355	246 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
D	355	246 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		VERT. LOAD	LC1	MAX	MAX. UNBRACED LENGTH	MEMB. FR-TO	WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED (PLF)						MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO							
A-B	0 / 0	-77.4 -77.4	0.23 (1)	10.00	A-C	0 / 0	0.00 (1)		
C-B	-155 / 0	0.0 0.0	0.06 (1)	7.81					
D-A	-155 / 0	0.0 0.0	0.02 (1)	7.81					
D-C	0 / 0	-175.6 -175.6	0.26 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	33.6	PSF	

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
START DISTANCE = 0-0
START SPAN CARRIED = 8-7-0
END DISTANCE = 4-0-0
END SPAN CARRIED = 8-7-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.23/1.00 (A-B:1), BC=0.26/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

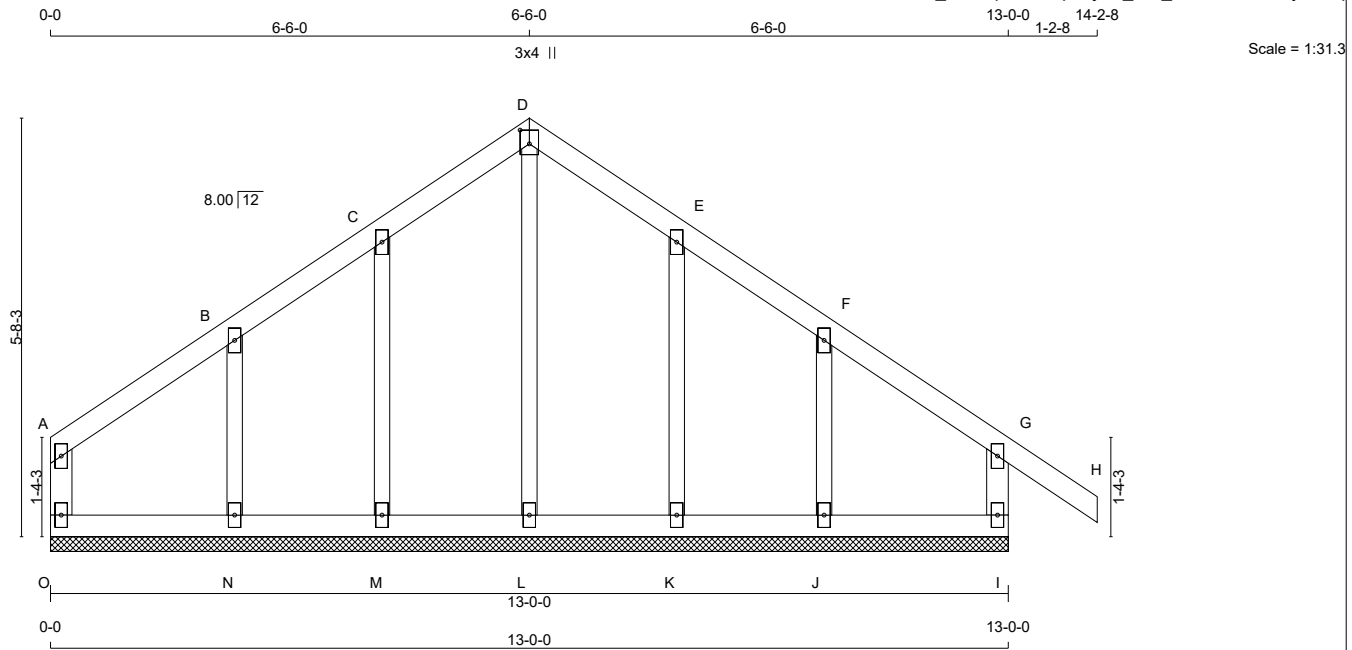
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0518-228	G09	2	1	GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 2	PAGE 20 OF 53
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:27:22 2018 Page 2
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn- F?Fr2p0fURvqUaljl8L MDygo7SoimlJSoAtyzAvop

JSI GRIP= 0.15 (C) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 51 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0-0 OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	2.0	4.0	
B, C, E, F	TMW+w	MT20	2.0	4.0	
D	TTW+p	MT20	3.0	4.0	2.25 1.50
G	TMV+p	MT20	2.0	4.0	
I	BMV1+p	MT20	2.0	4.0	
J, K, L, M, N	BMV1+w	MT20	2.0	4.0	
O	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM TO			FR-TO		
O-A	-113 / 0	0.0	0.0 0.04 (1)	7.81	L-D	-131 / 0	0.06 (1)
A-B	-39 / 0	-77.4	-77.4 0.05 (1)	6.25	M-C	-148 / 0	0.04 (1)
B-C	-37 / 0	-77.4	-77.4 0.05 (1)	6.25	N-B	-176 / 0	0.03 (1)
C-D	-30 / 0	-77.4	-77.4 0.04 (1)	6.25	K-E	-151 / 0	0.04 (1)
D-E	-30 / 0	-77.4	-77.4 0.04 (1)	6.25	J-F	-166 / 0	0.03 (1)
E-F	-36 / 0	-77.4	-77.4 0.04 (1)	6.25			
F-G	-43 / 0	-77.4	-77.4 0.06 (1)	6.25			
G-H	0 / 28	-77.4	-77.4 0.09 (1)	10.00			
I-G	-220 / 0	0.0	0.0 0.03 (1)	7.81			
O-N	0 / 34	-18.2	-18.2 0.02 (4)	10.00			
N-M	0 / 29	-18.2	-18.2 0.02 (4)	10.00			
M-L	0 / 26	-18.2	-18.2 0.02 (4)	10.00			
L-K	0 / 26	-18.2	-18.2 0.02 (4)	10.00			
K-J	0 / 29	-18.2	-18.2 0.02 (4)	10.00			
J-I	0 / 34	-18.2	-18.2 0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (G-H:1), BC=0.03/1.00 (I-J:1), WB=0.06/1.00 (D-L:1), SSI=0.06/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

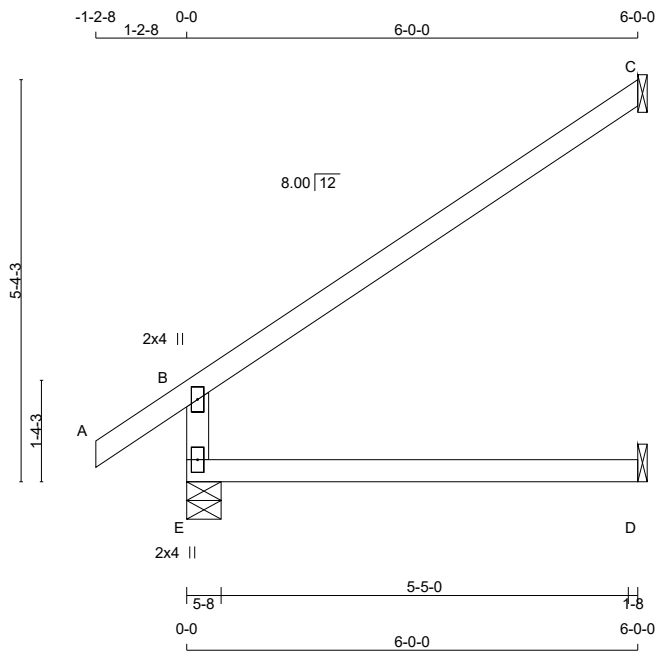
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (A) (INPUT = 0.90)
JSI METAL= 0.07 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:30.6

TOTAL WEIGHT = 13 X 18 = 234 lb [M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
E	454	0	454	0	0	5-8	5-8		
C	174	0	174	0	0	1-8	1-8		
D	46	0	51	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	316	235 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 (LC)
FR-TO		FROM	TO	FR-TO			
E-B	-390 / 0	0.0	0.0 0.11 (4)	7.81			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00			
B-C	-32 / 0	-77.4	-77.4 0.36 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES		GRIP(DRY)		SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284	1656	

PLATE PLACEMENT TOL. = 0.250 inches

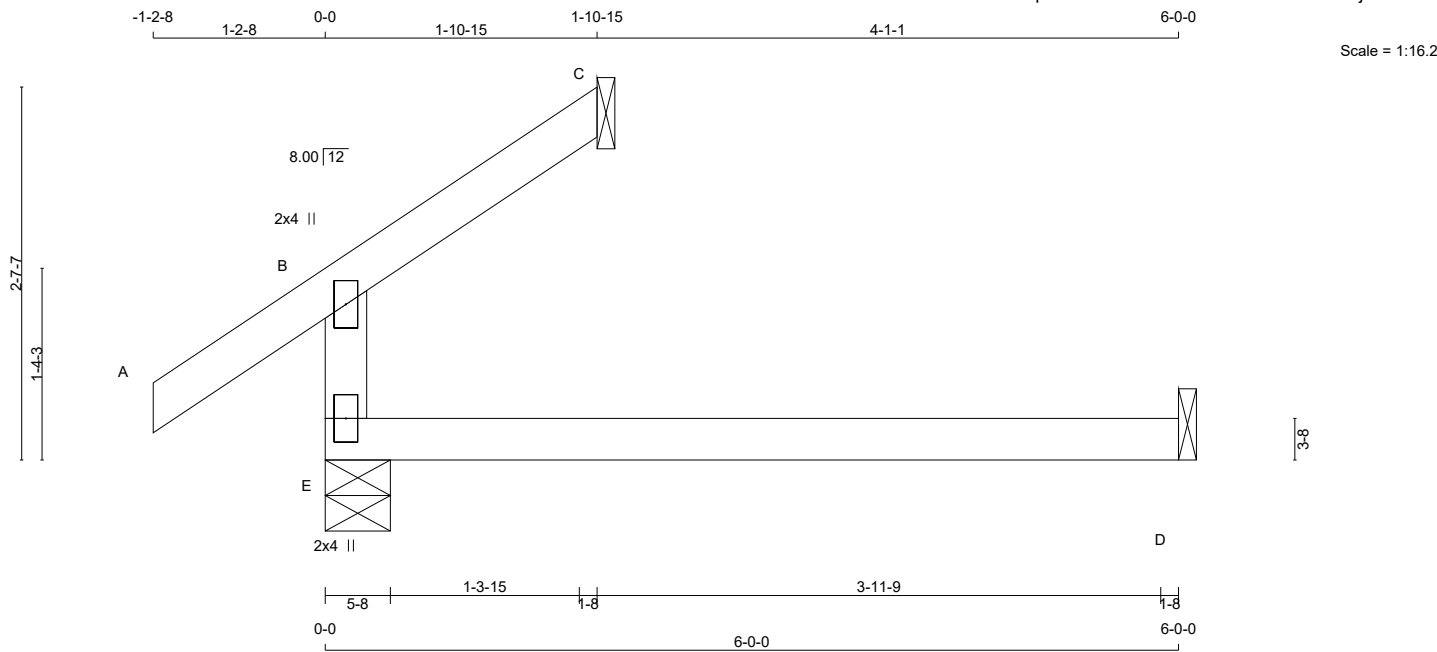
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 12 lb
[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	255	0	255	0	0	5-8	5-8	
C	56	0	56	0	0	1-8	1-8	
D	46	0	51	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	181	115 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO		LENGTH FR-TO
E-B	-192 / 0	0.0 0.0 0.11 (4)	7.81	
A-B	0 / 28	-77.4 -77.4 0.09 (1)	10.00	
B-C	-10 / 0	-77.4 -77.4 0.04 (1)	10.00	
E-D	0 / 0	-18.2 -18.2 0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3 PSF
DL =	3.0 PSF	
BOT CH.	LL =	0.0 PSF
DL =	7.3 PSF	
TOTAL LOAD =	33.6 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.11/1.00 (B-E:4), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (D-E:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

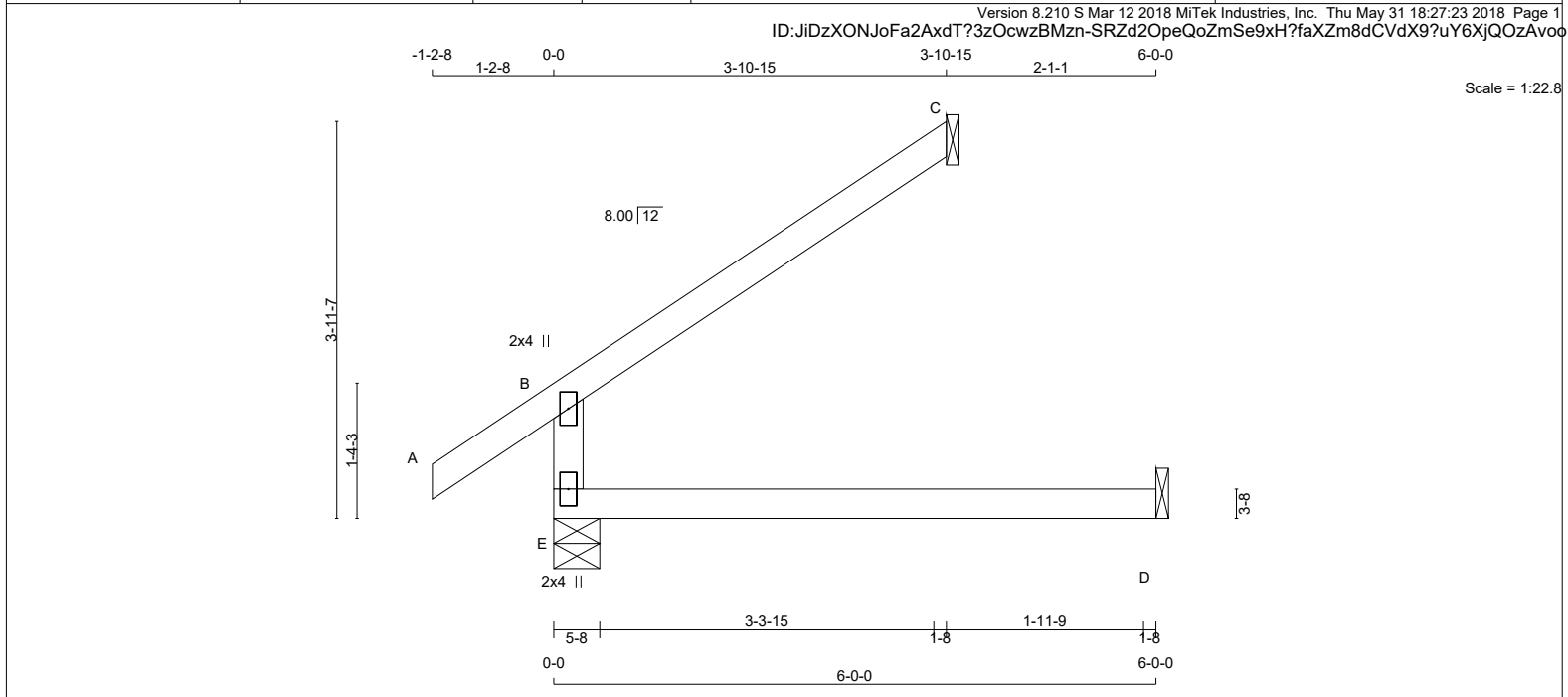
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 15 lb [M][F]

LUMBER				DESCR.	
N. L. G. A. RULES				SPF	
CHORDS	SIZE	LUMBER		SPF	
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						
BEARINGS						
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG		
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
E	353	0	353	0	5-8	5-8
C	114	0	114	0	1-8	1-8
D	46	0	51	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS						
JT	1ST LCASE	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. PERM. LIVE	MAX./MIN. WIND	MAX./MIN. DEAD
E	247	174 / 0	0 / 0	0 / 0	0 / 0	73 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING						
TOTAL LOAD CASES: (4)						
MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MAX. FACTORED UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)
FR-TO		FROM	TO		FR-TO	
E-B	-289 / 0	0.0	0.0 0.11 (4)	7.81		
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00		
B-C	-21 / 0	-77.4	-77.4 0.15 (1)	6.25		
E-D	0 / 0	-18.2	-18.2 0.13 (4)	10.00		

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.04")

CSI: TC=0.15/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

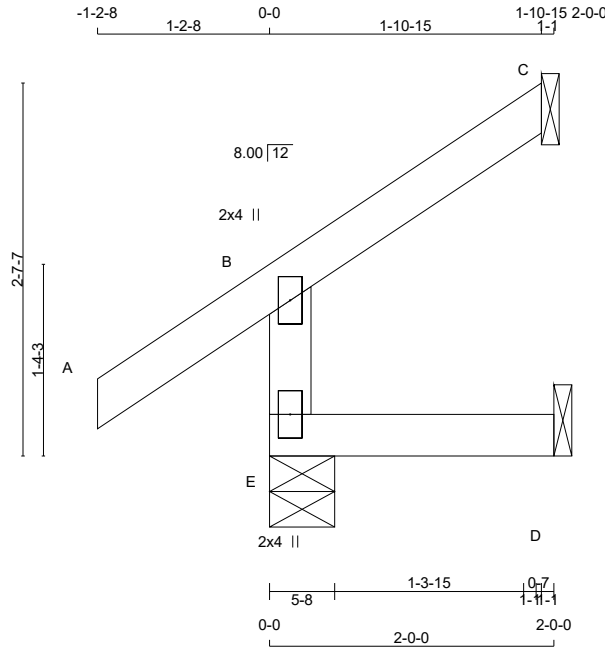
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 8 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	212	0	212	0	5-8	5-8
C	56	0	56	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS								
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND			
E	146	115 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0	
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0	
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. MEMB. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1
FR-TO		FROM	TO					
E-B	-192 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00			
B-C	-10 / 0	-77.4	-77.4	0.05 (1)	10.00			
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.09/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

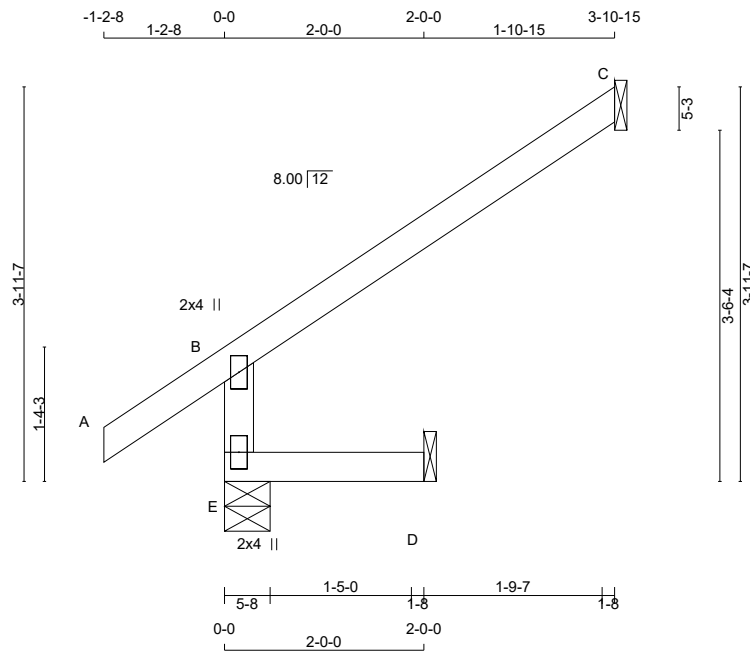
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 11 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	309	0	309	0	5-8	5-8
C	114	0	114	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	
E	212	174 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO		FROM	TO					
E-B	-289 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00			
B-C	-21 / 0	-77.4	-77.4	0.20 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.20/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

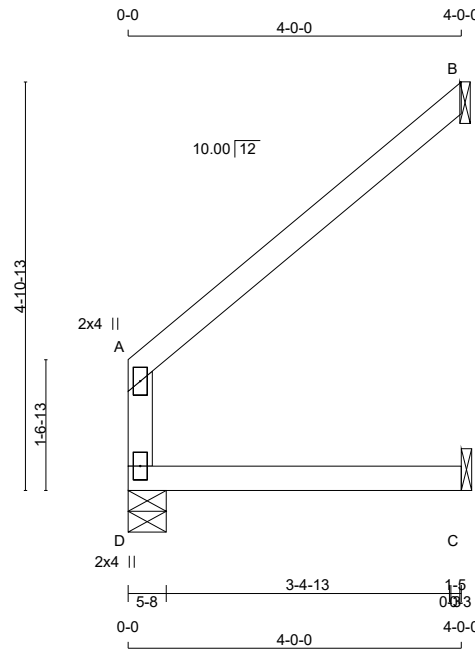
JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:27:25 2018 Page 1
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-OqhOT3rvyPpUhyJKOQh2c_rUs?Cn?3VB?Q0qUHzAvon



Scale = 1:27.7

TOTAL WEIGHT = 4 X 12 = 48 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN
A	TMV+p	MT20	2.0	4.0
D	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	191	0	191	0	5-8	5-8
B	142	0	142	0	1-8	1-8
C	49	0	49	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	WIND		
D	134	93 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0	0 / 0
B	97	84 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	0 / 0
C	38	9 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC1)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	LENGTH	FR-TO	
D- A	-167 / 0	0.0	0.0	0.06 (1)	7.81	
A- B	-8 / 0	-77.4	-77.4	0.17 (1)	10.00	
D- C	0 / 0	-18.2	-18.2	0.09 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	33.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.02")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.09/1.00 (C-D:1) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

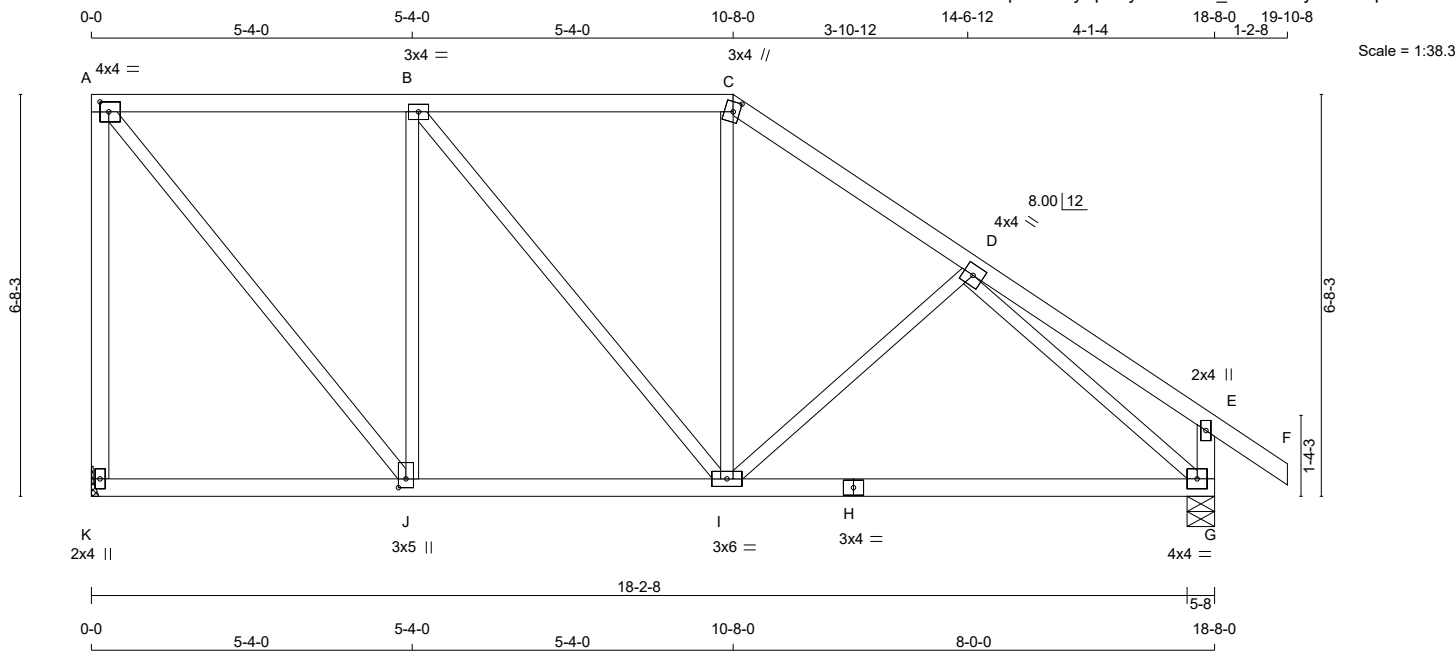
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (A) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
K - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.75
B	TMWW-t	MT20	3.0	4.0		
C	TTW+m	MT20	3.0	4.0	2.00	1.25
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	4.0		
I	BMWWW-t	MT20	3.0	6.0		
J	BMWW+t	MT20	3.0	5.0	1.75	1.50
K	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
K	892	0	892	0	0
G	992	0	992	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	627	435 / 0	0 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
G	695	495 / 0	0 / 0	0 / 0	0 / 0	0 / 0	200 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	UNBRAC LENGTH	FR-TO		
K-A	-850 / 0	0.0	0.0 0.80 (1)	7.81	A-J	0 / 903	0.20 (1)
A-B	-579 / 0	-77.4	-77.4 0.38 (1)	6.25	J-B	-604 / 0	0.45 (1)
B-C	-655 / 0	-77.4	-77.4 0.38 (1)	6.25	B-I	0 / 121	0.03 (1)
C-D	-805 / 0	-77.4	-77.4 0.21 (1)	6.25	I-C	0 / 160	0.04 (4)
D-E	0 / 20	-77.4	-77.4 0.20 (1)	10.00	I-D	-159 / 10	0.08 (1)
E-F	0 / 28	-77.4	-77.4 0.09 (1)	10.00	D-G	-1049 / 0	0.51 (1)
G-E	-219 / 0	0.0	0.0 0.02 (1)	7.81			
K-J	0 / 0	-18.2	-18.2 0.11 (4)	10.00			
J-I	0 / 579	-18.2	-18.2 0.29 (4)	10.00			
I-H	0 / 771	-18.2	-18.2 0.30 (4)	10.00			
H-G	0 / 771	-18.2	-18.2 0.30 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL)= L/999 (0.14")

CSI: TC=0.80/1.00 (A-K:1), BC=0.30/1.00 (G-I:4), WB=0.51/1.00 (D-G:1), SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES		PLATE GRIP(DRY)		SHEAR (PLI)		SECTION (PLI)	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX	MIN	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

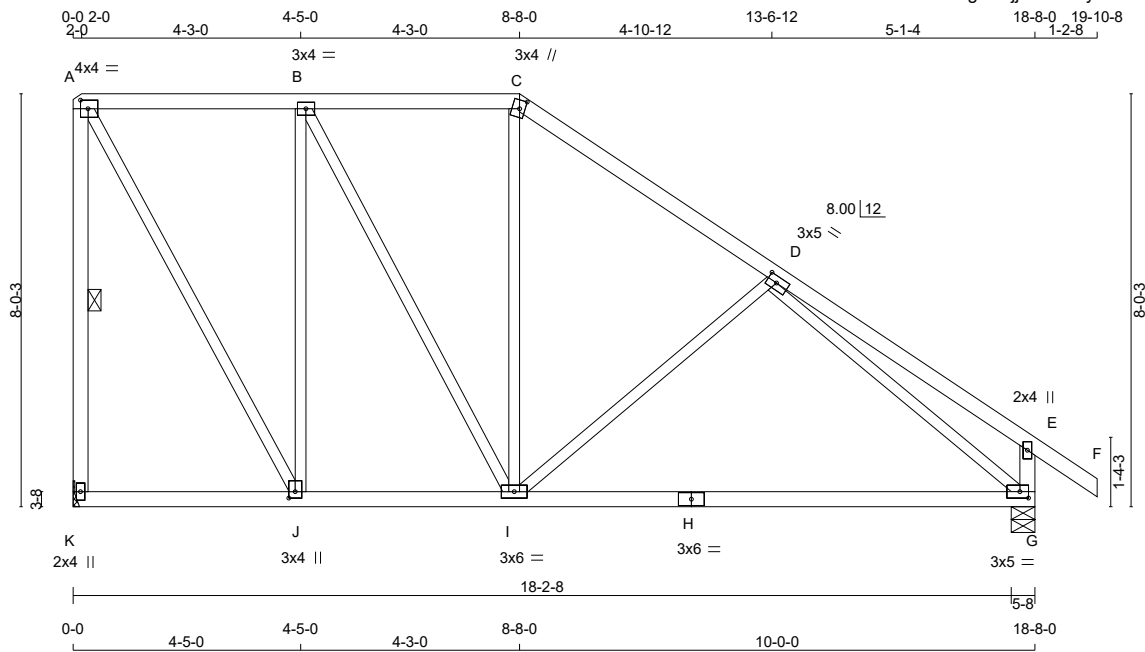
JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.37 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



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ID:JiDzXONJoFa2AxdT73zOcwzBMzn-s0EmgPsXjjxLJ6uWy8Ci8CodJPSikJgKE4mN0jzAvol



TOTAL WEIGHT = 92 lb [M][F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
K - A	2x4	DRY No.2	SPF
G - E	2x4	DRY No.2	SPF
K - H	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.75
B	TMWW-t	MT20	3.0	4.0		
C	TTW+m	MT20	3.0	4.0	2.00	1.25
D	TMWW-t	MT20	3.0	5.0	1.50	2.25
E	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	3.0	5.0	1.50	2.00
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	3.0	6.0		
J	BMWW-t	MT20	3.0	4.0	1.50	1.50
K	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG MECHANICAL	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
K	892	0	892	0	0	5-8
G	992	0	992	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD
K	627	435 / 0	0 / 0	0 / 0	0 / 0	0 / 0	192 / 0
G	695	495 / 0	0 / 0	0 / 0	0 / 0	0 / 0	200 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF A-K. DBS = 20-0-0 . CBF = 106 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)
FR-TO		FROM TO			FR-TO		
A-B	-411 / 0	-77.4 -77.4	0.18 (1)	6.25	A-J	0 / 829	0.19 (1)
B-C	-557 / 0	-77.4 -77.4	0.18 (1)	6.25	J-B	-683 / 0	0.84 (1)
C-D	-694 / 0	-77.4 -77.4	0.25 (1)	6.25	B-I	0 / 307	0.07 (1)
D-E	0 / 26	-77.4 -77.4	0.32 (1)	10.00	I-C	0 / 98	0.03 (4)
E-F	0 / 28	-77.4 -77.4	0.09 (1)	10.00	I-D	-279 / 0	0.23 (1)
K-A	-851 / 0	0.0 0.0	0.25 (1)	6.25	D-G	-1019 / 0	0.80 (1)
G-E	-247 / 0	0.0 0.0	0.03 (1)	7.81			
K-J	0 / 0	-18.2 -18.2	0.11 (4)	10.00			
J-I	0 / 411	-18.2 -18.2	0.44 (4)	10.00			
I-H	0 / 768	-18.2 -18.2	0.46 (4)	10.00			
H-G	0 / 768	-18.2 -18.2	0.46 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/776 (0.29")

CSI: TC=0.32/1.00 (D-E:1), BC=0.46/1.00 (G-I:4), WB=0.84/1.00 (B-J:1), SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

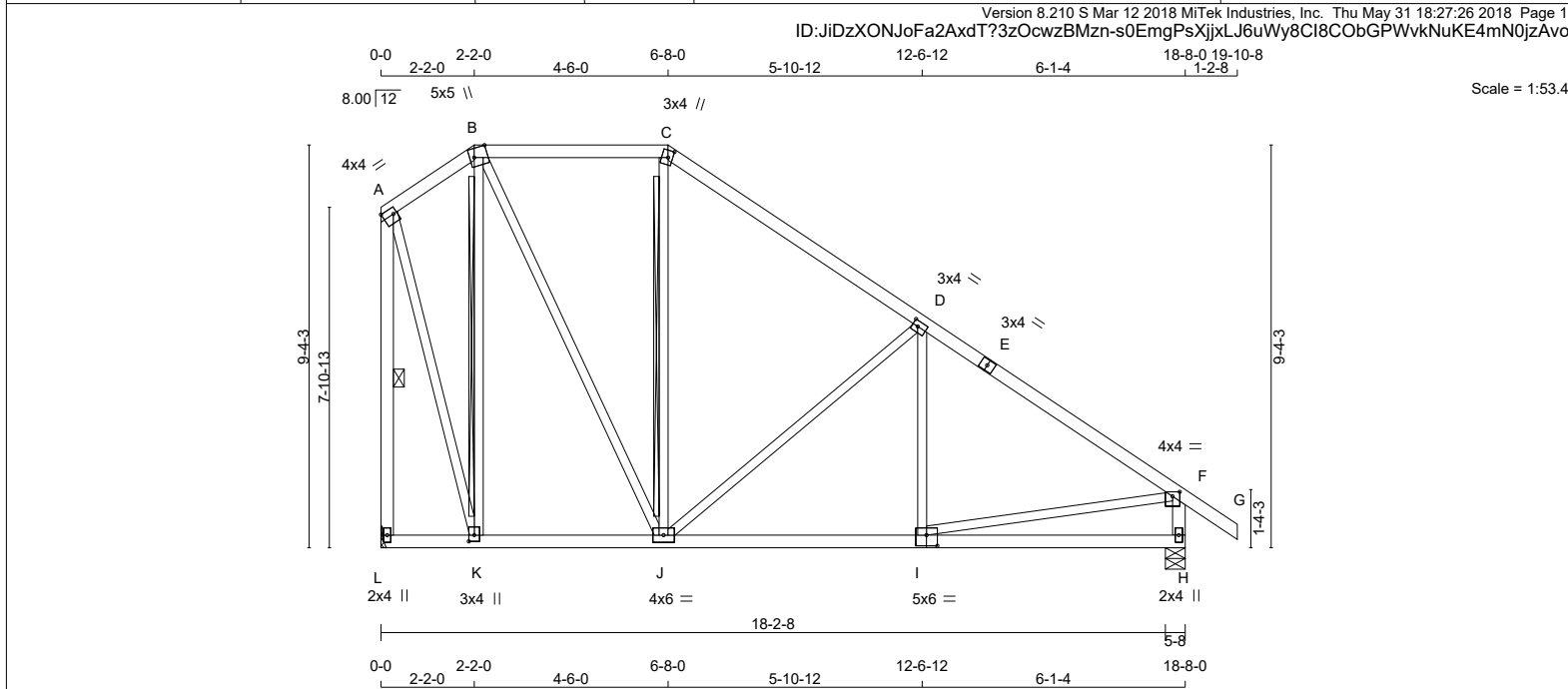
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.45 (H) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 101 [M][F]																																																																																																																																																																																																																																																																																																																																																											
LUMBER N. L. G. A. RULES <table><tr><th>CHORDS</th><th>SIZE</th><th></th><th>LUMBER</th><th>DESCR.</th></tr><tr><td>A - B</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>B - C</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>C - E</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>E - G</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>L - A</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>H - F</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>L - I</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>I - H</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr></table> ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. PLATES (table is in inches) <table><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr><tr><td>A</td><td>TMVW-t</td><td>MT20</td><td>4.0</td><td>4.0</td><td>1.75</td><td>Edge</td></tr><tr><td>B</td><td>TTWW+m</td><td>MT20</td><td>5.0</td><td>5.0</td><td>Edge</td><td>3.75</td></tr><tr><td>C</td><td>TTW+m</td><td>MT20</td><td>3.0</td><td>4.0</td><td>2.00</td><td>1.25</td></tr><tr><td>D</td><td>TMWW-t</td><td>MT20</td><td>3.0</td><td>4.0</td><td>1.50</td><td>1.50</td></tr><tr><td>E</td><td>TS-t</td><td>MT20</td><td>3.0</td><td>4.0</td><td></td><td></td></tr><tr><td>F</td><td>TMVW-p</td><td>MT20</td><td>4.0</td><td>4.0</td><td>1.25</td><td>2.00</td></tr><tr><td>H</td><td>BMV1+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr><tr><td>I</td><td>BSWW-l</td><td>MT20</td><td>5.0</td><td>6.0</td><td>3.00</td><td>3.00</td></tr><tr><td>J</td><td>BMWWW-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>K</td><td>BMWW+t</td><td>MT20</td><td>3.0</td><td>4.0</td><td>1.75</td><td>1.50</td></tr><tr><td>L</td><td>BMV1+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr></table> Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.					CHORDS	SIZE		LUMBER	DESCR.	A - B	2x4	DRY	No.2	SPF	B - C	2x4	DRY	No.2	SPF	C - E	2x4	DRY	No.2	SPF	E - G	2x4	DRY	No.2	SPF	L - A	2x4	DRY	No.2	SPF	H - F	2x4	DRY	No.2	SPF	L - I	2x4	DRY	No.2	SPF	I - H	2x4	DRY	No.2	SPF	JT	TYPE	PLATES	W	LEN	Y	X	A	TMVW-t	MT20	4.0	4.0	1.75	Edge	B	TTWW+m	MT20	5.0	5.0	Edge	3.75	C	TTW+m	MT20	3.0	4.0	2.00	1.25	D	TMWW-t	MT20	3.0	4.0	1.50	1.50	E	TS-t	MT20	3.0	4.0			F	TMVW-p	MT20	4.0	4.0	1.25	2.00	H	BMV1+p	MT20	2.0	4.0			I	BSWW-l	MT20	5.0	6.0	3.00	3.00	J	BMWWW-t	MT20	4.0	6.0			K	BMWW+t	MT20	3.0	4.0	1.75	1.50	L	BMV1+p	MT20	2.0	4.0			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th></th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQRD BRG</th></tr><tr><td>JT</td><td>VERT</td><td>HORZ</td><td>DOWN</td><td>HORZ</td></tr><tr><td>L</td><td>892</td><td>0</td><td>892</td><td>0</td></tr><tr><td>H</td><td>992</td><td>0</td><td>992</td><td>0</td></tr></table> A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT L TO RESIST THE MAX FACTORED REACTIONS. UNFACTORED REACTIONS <table><tr><th>1ST LCASE</th><th colspan="6">MAX./MIN. COMPONENT REACTIONS</th></tr><tr><td>JT</td><td>COMBINED</td><td>SNOW</td><td>LIVE</td><td>PERM.LIVE</td><td>WIND</td><td>DEAD</td></tr><tr><td>L</td><td>627</td><td>435 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>192 / 0</td></tr><tr><td>H</td><td>695</td><td>495 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>200 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF A-L. DBS = 20-0-0 . CBF = 110 LBS. 2x4 DRY SPF No.2 T-BRACE AT B-K, C-J DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12. FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. UNBRACED LENGTH</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. UNBRACED LENGTH</th><th>MAX. FACTORED FORCE (LBS)</th></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>A-B</td><td>-239 / 0</td><td>-77.4</td><td>-77.4 0.05 (1)</td><td>6.25</td><td>K-B</td><td>-600 / 0</td><td>0.39 (1)</td></tr><tr><td>B-C</td><td>-422 / 0</td><td>-77.4</td><td>-77.4 0.20 (1)</td><td>6.25</td><td>B-J</td><td>0 / 528</td><td>0.12 (1)</td></tr><tr><td>C-D</td><td>-541 / 0</td><td>-77.4</td><td>-77.4 0.36 (1)</td><td>6.25</td><td>J-C</td><td>-61 / 50</td><td>0.04 (1)</td></tr><tr><td>D-E</td><td>-929 / 0</td><td>-77.4</td><td>-77.4 0.39 (1)</td><td>5.91</td><td>J-D</td><td>-494 / 0</td><td>0.63 (1)</td></tr><tr><td>E-F</td><td>-929 / 0</td><td>-77.4</td><td>-77.4 0.39 (1)</td><td>5.91</td><td>I-D</td><td>-8 / 102</td><td>0.04 (4)</td></tr><tr><td>F-G</td><td>0 / 28</td><td>-77.4</td><td>-77.4 0.09 (1)</td><td>10.00</td><td>A-K</td><td>0 / 690</td><td>0.16 (1)</td></tr><tr><td>L-A</td><td>-878 / 0</td><td>0.0</td><td>0.0 0.26 (1)</td><td>6.25</td><td>I-F</td><td>0 / 810</td><td>0.18 (1)</td></tr><tr><td>H-F</td><td>-947 / 0</td><td>0.0</td><td>0.0 0.10 (1)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>L-K</td><td>0 / 0</td><td>-18.2</td><td>-18.2 0.04 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>K-J</td><td>0 / 191</td><td>-18.2</td><td>-18.2 0.11 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>J-I</td><td>0 / 799</td><td>-18.2</td><td>-18.2 0.23 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>I-H</td><td>0 / 0</td><td>-18.2</td><td>-18.2 0.17 (4)</td><td>10.00</td><td></td><td></td><td></td></tr></table>						FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	JT	VERT	HORZ	DOWN	HORZ	L	892	0	892	0	H	992	0	992	0	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	L	627	435 / 0	0 / 0	0 / 0	0 / 0	192 / 0	H	695	495 / 0	0 / 0	0 / 0	0 / 0	200 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)	FR-TO		FROM TO		FR-TO				A-B	-239 / 0	-77.4	-77.4 0.05 (1)	6.25	K-B	-600 / 0	0.39 (1)	B-C	-422 / 0	-77.4	-77.4 0.20 (1)	6.25	B-J	0 / 528	0.12 (1)	C-D	-541 / 0	-77.4	-77.4 0.36 (1)	6.25	J-C	-61 / 50	0.04 (1)	D-E	-929 / 0	-77.4	-77.4 0.39 (1)	5.91	J-D	-494 / 0	0.63 (1)	E-F	-929 / 0	-77.4	-77.4 0.39 (1)	5.91	I-D	-8 / 102	0.04 (4)	F-G	0 / 28	-77.4	-77.4 0.09 (1)	10.00	A-K	0 / 690	0.16 (1)	L-A	-878 / 0	0.0	0.0 0.26 (1)	6.25	I-F	0 / 810	0.18 (1)	H-F	-947 / 0	0.0	0.0 0.10 (1)	7.81				L-K	0 / 0	-18.2	-18.2 0.04 (4)	10.00				K-J	0 / 191	-18.2	-18.2 0.11 (4)	10.00				J-I	0 / 799	-18.2	-18.2 0.23 (4)	10.00				I-H	0 / 0	-18.2	-18.2 0.17 (4)	10.00				DESIGN CRITERIA SPECIFIED LOADS: <table><tr><th>TOP CH.</th><th>LL</th><th>DL</th><th>PSF</th></tr><tr><td></td><td>23.3</td><td>3.0</td><td>PSF</td></tr><tr><th>BOT CH.</th><th>LL</th><th>DL</th><th>PSF</th></tr><tr><td></td><td>0.0</td><td>7.3</td><td>PSF</td></tr><tr><td>TOTAL LOAD</td><td>33.6</td><td></td><td>PSF</td></tr></table> SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.62") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03") ALLOWABLE DEFL.(TL)= L/360 (0.62") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.06") CSI: TC=0.39/1.00 (D-F:1), BC=0.23/1.00 (I-J:4), WB=0.63/1.00 (D-J:1), SSI=0.19/1.00 (D-F:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES <table><tr><th>PLATE</th><th>GRIP(DRY)</th><th>SHEAR</th><th>SECTION</th></tr><tr><td></td><td>(PSI)</td><td>(PLI)</td><td>(PLI)</td></tr><tr><td></td><td>MAX MIN</td><td>MAX MIN</td><td>MAX MIN</td></tr><tr><td>MT20</td><td>618 354</td><td>1667 822</td><td>2284 1656</td></tr></table> PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (B) (INPUT = 0.90) JSI METAL= 0.30 (F) (INPUT = 1.00)					TOP CH.	LL	DL	PSF		23.3	3.0	PSF	BOT CH.	LL	DL	PSF		0.0	7.3	PSF	TOTAL LOAD	33.6		PSF	PLATE	GRIP(DRY)	SHEAR	SECTION		(PSI)	(PLI)	(PLI)		MAX MIN	MAX MIN	MAX MIN	MT20	618 354	1667 822	2284 1656
CHORDS	SIZE		LUMBER	DESCR.																																																																																																																																																																																																																																																																																																																																																							
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L-K	0 / 0	-18.2	-18.2 0.04 (4)	10.00																																																																																																																																																																																																																																																																																																																																																							
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I-H	0 / 0	-18.2	-18.2 0.17 (4)	10.00																																																																																																																																																																																																																																																																																																																																																							
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BOT CH.	LL	DL	PSF																																																																																																																																																																																																																																																																																																																																																								
	0.0	7.3	PSF																																																																																																																																																																																																																																																																																																																																																								
TOTAL LOAD	33.6		PSF																																																																																																																																																																																																																																																																																																																																																								
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LICENSED PROFESSIONAL ENGINEER

N.A. EL-MASRI

PROVINCE OF ONTARIO

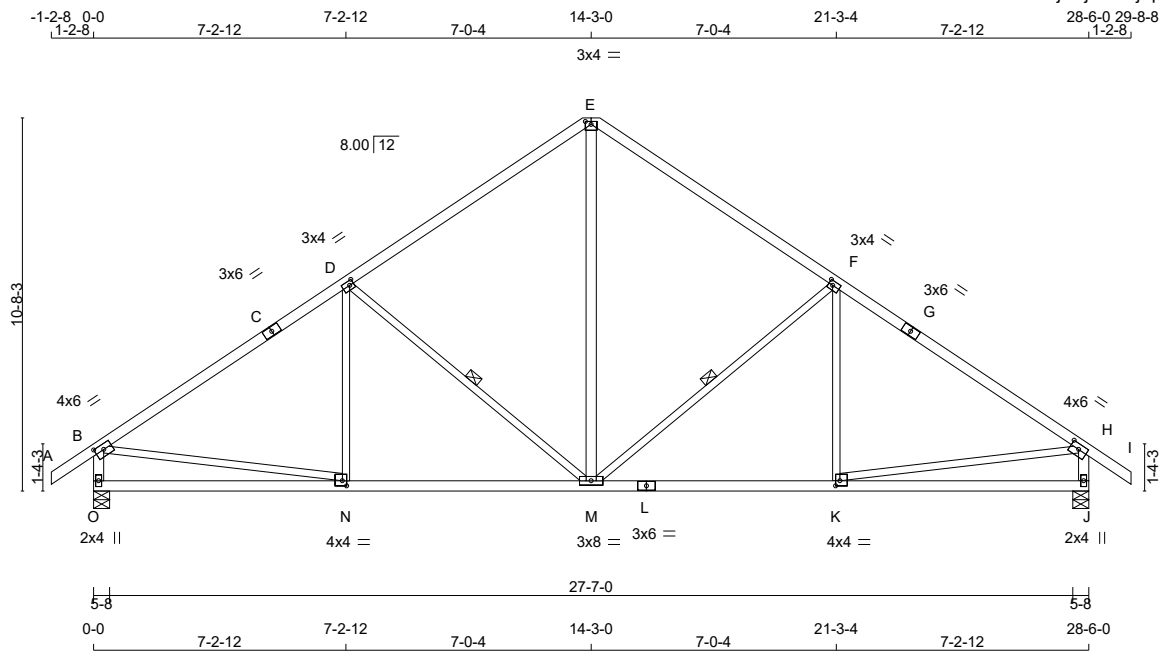
May 31, 2018

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Scale = 1:66.0

TOTAL WEIGHT = 123 lb
[M][F]

LUMBER				DESCR.	
CHORDS	SIZE	DRY	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
M - E	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.75 3.00
C	TS-t	MT20	3.0	6.0	
D	TMWW-t	MT20	3.0	4.0	1.50 1.50
E	TTW-p	MT20	3.0	4.0	1.00 2.00
F	TMWW-t	MT20	3.0	4.0	1.50 1.50
G	TS-t	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	6.0	1.75 3.00
J	BMV1+p	MT20	2.0	4.0	
K	BMWW-t	MT20	4.0	4.0	1.75 1.50
L	BS-t	MT20	3.0	6.0	
M	BMWWW-t	MT20	3.0	8.0	
N	BMWW-t	MT20	4.0	4.0	1.75 1.50
O	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	JT	GROSS REACTION	JT	BRG	JT	BRG
O	1462 0	O	1462 0	O	5-8	O	5-8
J	1462 0	J	1462 0	J	5-8	J	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	JT	COMBINED	JT	COMBINED	JT	COMBINED
O	1025	O	1025	O	1025	O	1025
J	1025	J	1025	J	1025	J	1025

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M, D-M. DBS = 20-0-0. CBF = 69 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	M-E	0 / 832	0.13 (1)	
B-C	-1591 / 0	-77.4 -77.4	0.63 (1)	M-F	-554 / 0	0.32 (1)	
C-D	-1591 / 0	-77.4 -77.4	0.63 (1)	K-F	-47 / 108	0.04 (4)	
D-E	-1154 / 0	-77.4 -77.4	0.56 (1)	D-M	-554 / 0	0.32 (1)	
E-F	-1154 / 0	-77.4 -77.4	0.56 (1)	N-D	-47 / 108	0.04 (4)	
F-G	-1591 / 0	-77.4 -77.4	0.63 (1)	B-N	0 / 1368	0.31 (1)	
G-H	-1591 / 0	-77.4 -77.4	0.63 (1)	K-H	0 / 1368	0.31 (1)	
H-I	0 / 28	-77.4 -77.4	0.09 (1)				
O-B	-1408 / 0	0.0 0.0	0.14 (1)				
J-H	-1408 / 0	0.0 0.0	0.14 (1)				
O-N	0 / 0	-18.2 -18.2	0.23 (4)				
N-M	0 / 1354	-18.2 -18.2	0.34 (1)				
M-L	0 / 1354	-18.2 -18.2	0.34 (1)				
L-K	0 / 1354	-18.2 -18.2	0.34 (1)				
K-J	0 / 0	-18.2 -18.2	0.23 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.3	PSF	
TOTAL LOAD = 33.6	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.63/1.00 (F-H:1), BC=0.34/1.00 (K-M:1), WB=0.32/1.00 (F-M:1), SSI=0.22/1.00 (F-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

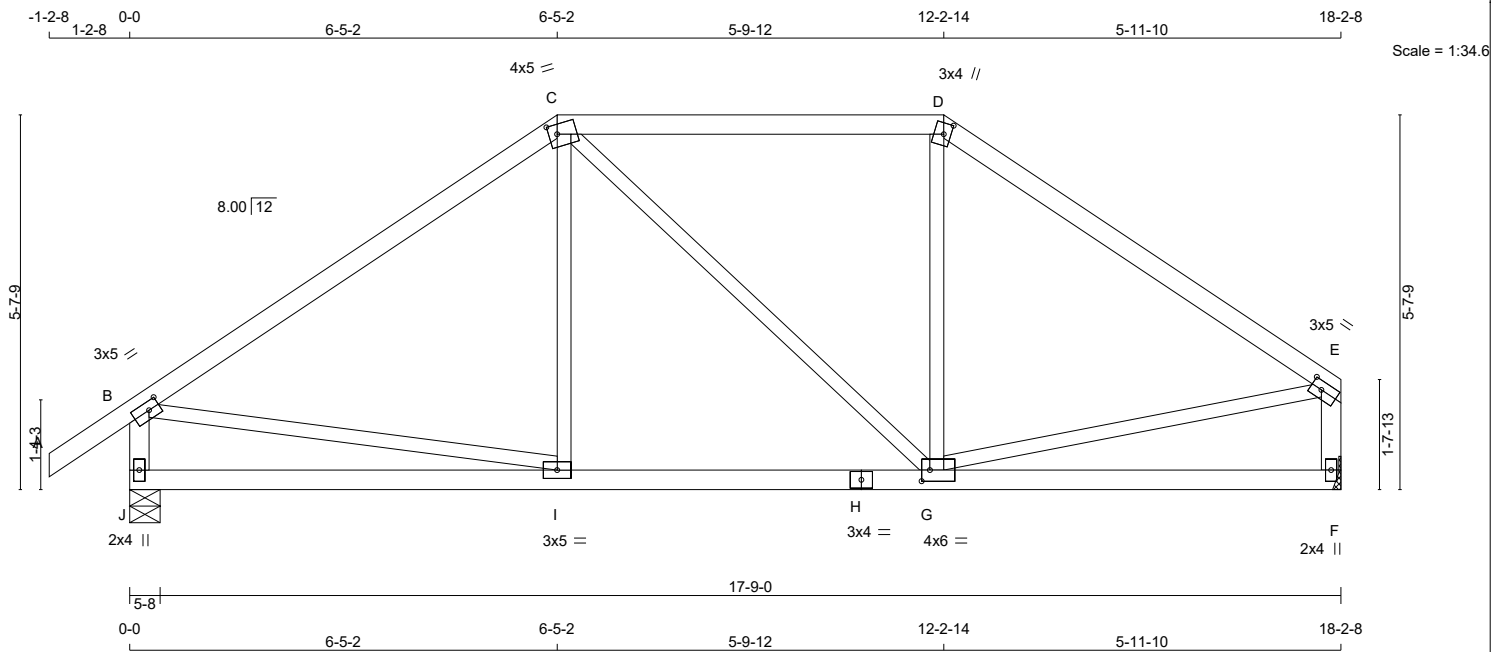
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.51 (K) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 73 lb [M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.00
C	TTWW-m	MT20	4.0	5.0	1.75	1.50
D	TTWW+m	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	3.0	5.0	1.50	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	6.0	2.00	1.50
H	BS-t	MT20	3.0	4.0		
I	BMWW-t	MT20	3.0	5.0		
J	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
J	970	0	970	0	5-8
F	870	0	870	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	679	484 / 0	0 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0
F	612	424 / 0	0 / 0	0 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	UNBRAC LENGTH	FR-TO		
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	I-C	0 / 102	0.04 (4)
B-C	-833 / 0	-77.4	-77.4 0.43 (1)	6.08	C-G	-33 / 0	0.04 (1)
C-D	-669 / 0	-77.4	-77.4 0.34 (1)	6.25	G-D	-8 / 91	0.03 (4)
D-E	-805 / 0	-77.4	-77.4 0.37 (1)	6.25	B-I	0 / 701	0.16 (1)
J-B	-921 / 0	0.0	0.0 0.09 (1)	7.81	G-E	0 / 685	0.15 (1)
F-E	-824 / 0	0.0	0.0 0.09 (1)	7.81			
J-I	0 / 0	-18.2	-18.2 0.16 (4)	10.00			
I-H	0 / 693	-18.2	-18.2 0.22 (4)	10.00			
H-G	0 / 693	-18.2	-18.2 0.22 (4)	10.00			
G-F	0 / 0	-18.2	-18.2 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.43/1.00 (B-C:1), BC=0.22/1.00 (G-I:4), WB=0.16/1.00 (B-I:1), SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

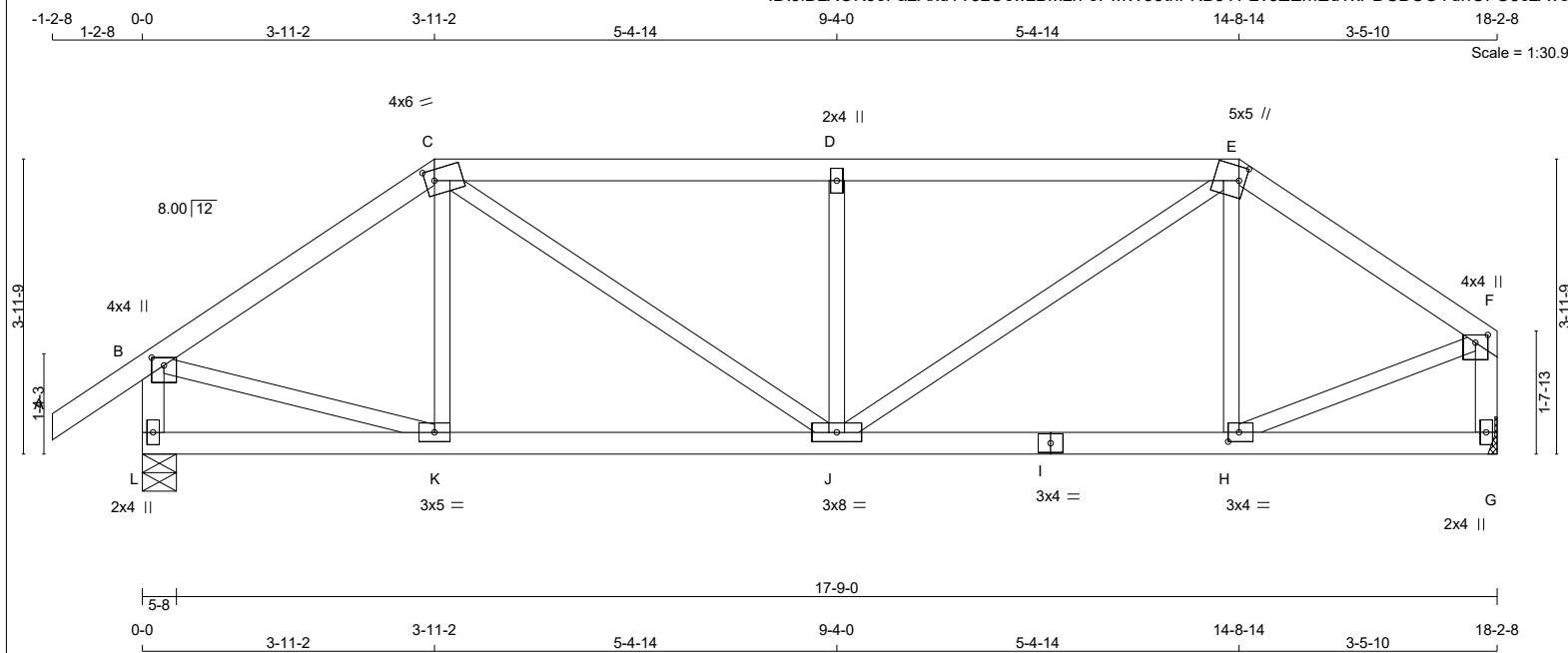
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 72 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	5.0	2.25	1.00
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-t	MT20	3.0	4.0	1.50	1.75
I	BS-t	MT20	3.0	4.0		
J	BMWWW-t	MT20	3.0	8.0		
K	BMWW-t	MT20	3.0	5.0		
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	970	0	970	0	5-8	5-8
G	870	0	870	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	679	484 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0
G	612	424 / 0	0 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS			MAX. UNBRACED LENGTH	MEMB.	WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)			MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		FROM	TO
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	K-C	-94 / 46	0.02 (1)
B-C	-900 / 0	-77.4	-77.4	0.22 (1)	6.21	C-J	0 / 501	0.11 (1)
C-D	-1159 / 0	-77.4	-77.4	0.41 (1)	5.36	J-D	-511 / 0	0.12 (1)
D-E	-1159 / 0	-77.4	-77.4	0.41 (1)	5.36	J-E	0 / 579	0.13 (1)
E-F	-824 / 0	-77.4	-77.4	0.17 (1)	6.25	H-E	-163 / 21	0.04 (1)
L-B	-943 / 0	0.0	0.0	0.10 (1)	7.81	B-K	0 / 771	0.17 (1)
G-F	-848 / 0	0.0	0.0	0.09 (1)	7.81	H-F	0 / 730	0.16 (1)
L-K	0 / 0	-18.2	-18.2	0.09 (4)	10.00			
K-J	0 / 745	-18.2	-18.2	0.17 (1)	10.00			
J-I	0 / 680	-18.2	-18.2	0.16 (1)	10.00			
I-H	0 / 680	-18.2	-18.2	0.16 (1)	10.00			
H-G	0 / 0	-18.2	-18.2	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.41/1.00 (C-D:1), BC=0.17/1.00 (J-K:1), WB=0.17/1.00 (B-K:1), SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

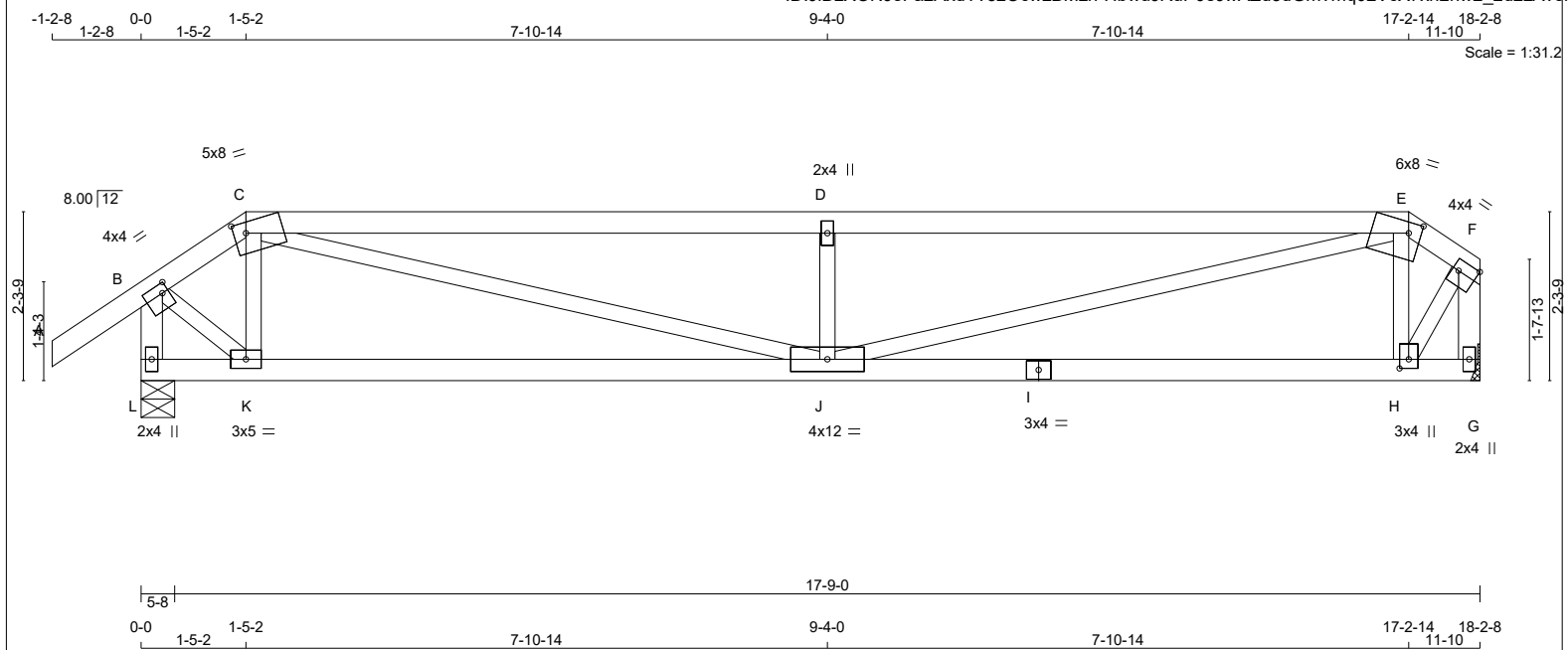
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.25 (H) (INPUT = 1.00)



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TOTAL WEIGHT = 66 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY 2100F 1.8E	SPF
E - F	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	4.0	1.50 1.00
C	TTWW-m	MT20	5.0	8.0	1.75 2.00
D	TMW+w	MT20	2.0	4.0	
E	TTWW-m	MT20	6.0	8.0	1.75 2.00
F	TMVW-t	MT20	4.0	4.0	1.75 Edge
G	BMV1+p	MT20	2.0	4.0	
H	BMWW-t	MT20	3.0	4.0	1.50 1.50
I	BS-t	MT20	3.0	4.0	
J	BMWWW-t	MT20	4.0	12.0	
K	BMWW-t	MT20	3.0	5.0	
L	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS	REACT	GROSS	REACT	BRG	IN-SX	BRG	IN-SX
L	974	0	974	0	0	5-8	5-8		
G	867	0	867	0	0	MECHANICAL			

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	682	486 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0
G	609	422 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	K-C	-244 / 9	0.04 (1)
B-C	-774 / 0	-77.4	-77.4 0.09 (1)	6.25	C-J	0 / 1688	0.38 (1)
C-D	-2244 / 0	-77.4	-77.4 0.64 (1)	4.71	J-D	-741 / 0	0.11 (1)
D-E	-2244 / 0	-77.4	-77.4 0.64 (1)	4.71	J-E	0 / 1834	0.41 (1)
E-F	-589 / 0	-77.4	-77.4 0.02 (1)	6.25	H-E	-396 / 0	0.06 (1)
L-B	-1036 / 0	0.0	0.0 0.11 (1)	7.71	B-K	0 / 747	0.17 (1)
G-F	-973 / 0	0.0	0.0 0.10 (1)	7.81	H-F	0 / 775	0.17 (1)
L-K	0 / 0	-18.2	-18.2 0.20 (4)	10.00			
K-J	0 / 608	-18.2	-18.2 0.26 (1)	10.00			
J-I	0 / 466	-18.2	-18.2 0.25 (4)	10.00			
I-H	0 / 466	-18.2	-18.2 0.25 (4)	10.00			
H-G	0 / 0	-18.2	-18.2 0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.3 PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.22")

CSI: TC=0.64/1.00 (C-D:1), BC=0.26/1.00 (J-K:1), WB=0.41/1.00 (E-J:1), SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 2	DRWG NO.
NE0518-228	T07	1	1	TRUSS DESC.		PAGE 35 OF 53

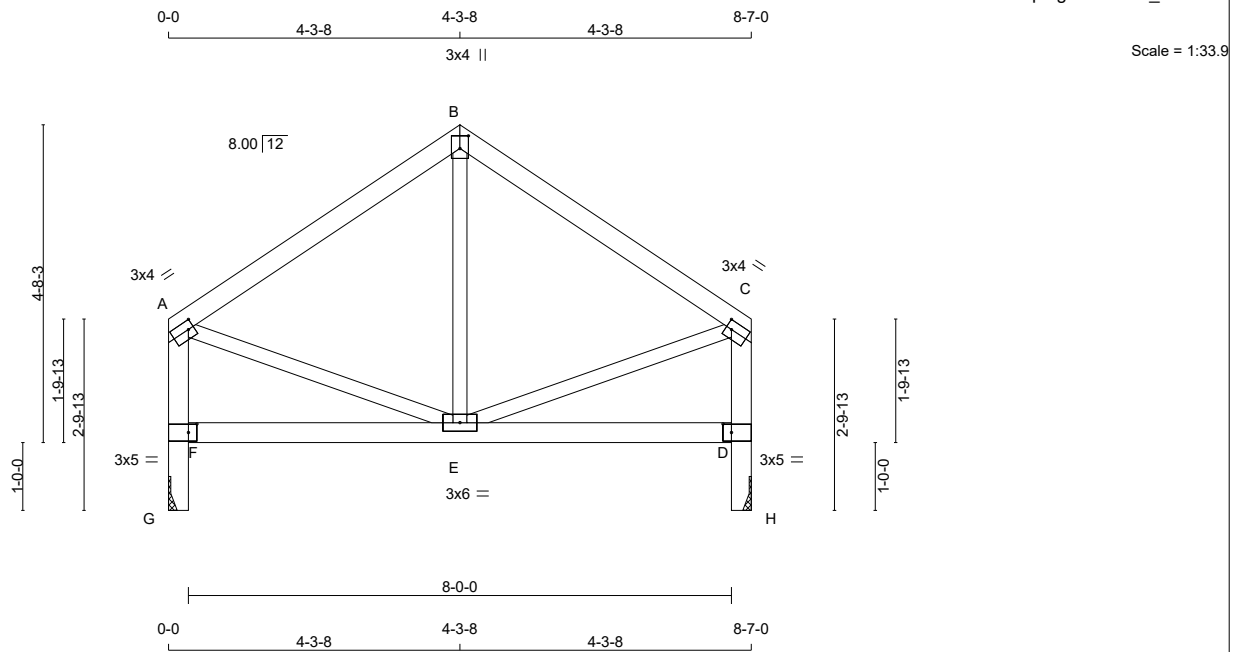
Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:27:29 2018 Page 2
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-HbwuJRuP0eJwAZd5dGm?mq02VcX7xn2nw2_2d2zAvoi

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.37 (E) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.



Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:27:29 2018 Page 1
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-HbwuJRU0eJwAZd5dGm?mq09gcZdxtnw2_2d2zAvoi



TOTAL WEIGHT = 2 X 37 = 75 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TMVW-t	MT20	3.0	4.0	1.50	1.00
D	BVM-I	MT20	3.0	5.0	1.50	1.50
E	BMWWW-t	MT20	3.0	6.0		
F	BVM-I	MT20	3.0	5.0	1.50	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP
G	410	0	410	0
H	410	0	410	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G, H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
G	288	200 / 0	0 / 0	0 / 0	88 / 0	0 / 0
H	288	200 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	UNBRAC LENGTH	FR-TO
A-B	-253 / 0	-77.4	-77.4	0.25 (1)	6.25	E-B	-51 / 59	0.02 (4)	
B-C	-253 / 0	-77.4	-77.4	0.25 (1)	6.25	A-E	0 / 223	0.05 (1)	
G-F	-410 / 0	0.0	0.0	0.04 (1)	7.81	E-C	0 / 223	0.05 (1)	
F-A	-380 / 0	0.0	0.0	0.04 (1)	7.81				
H-D	-410 / 0	0.0	0.0	0.04 (1)	7.81				
D-C	-380 / 0	0.0	0.0	0.04 (1)	7.81				
F-E	0 / 0	-18.2	-18.2	0.10 (4)	10.00				
E-D	0 / 0	-18.2	-18.2	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.25/1.00 (B-C:1), BC=0.10/1.00 (D-E:4), WB=0.05/1.00 (C-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

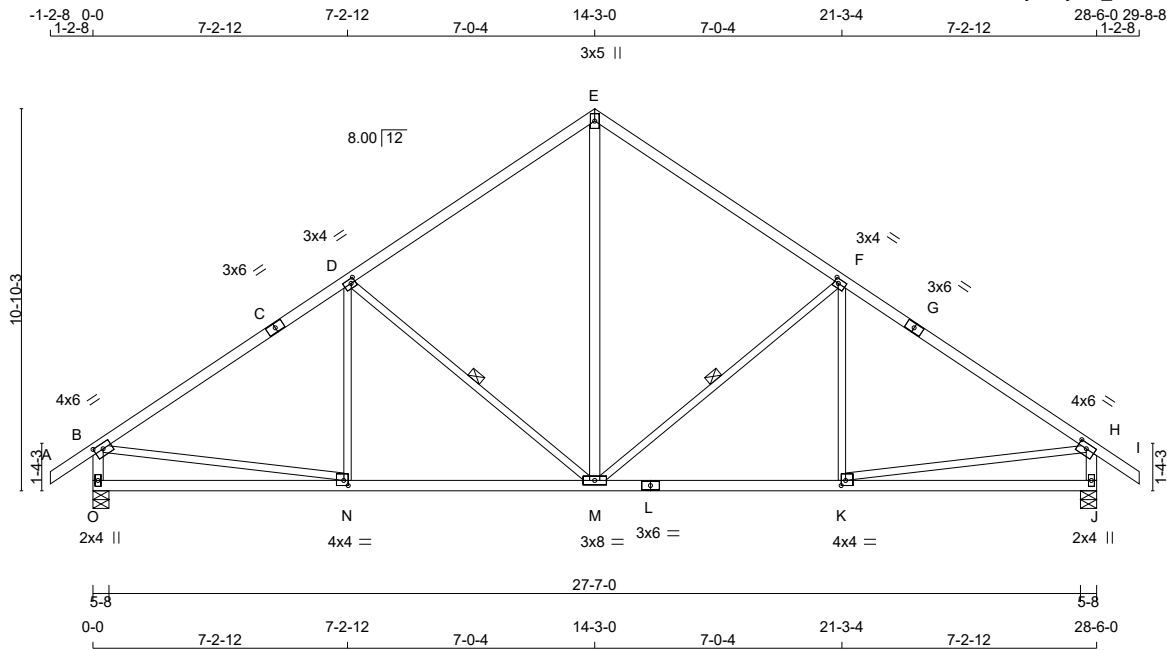
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (A) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 124 = 371 lb

LUMBER				DESCR.	
CHORDS	SIZE	DRY	LUMBER	SPF	No.2
A - C	2x4	DRY	No.2	SPF	No.2
C - E	2x4	DRY	No.2	SPF	No.2
E - G	2x4	DRY	No.2	SPF	No.2
G - I	2x4	DRY	No.2	SPF	No.2
O - B	2x4	DRY	No.2	SPF	No.2
J - H	2x4	DRY	No.2	SPF	No.2
O - L	2x4	DRY	No.2	SPF	No.2
L - J	2x4	DRY	No.2	SPF	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	No.2
M - E	2x4	DRY	No.2	SPF	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.75 3.00
C	TS-t	MT20	3.0	6.0	
D	TMWW-t	MT20	3.0	4.0	1.50 1.50
E	TTW+p	MT20	3.0	5.0	
F	TMWW-t	MT20	3.0	4.0	1.50 1.50
G	TS-t	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	6.0	1.75 3.00
J	BMV1+p	MT20	2.0	4.0	
K	BMWW-t	MT20	4.0	4.0	1.75 1.50
L	BS-t	MT20	3.0	6.0	
M	BMWWW-t	MT20	3.0	8.0	
N	BMWW-t	MT20	4.0	4.0	1.75 1.50
O	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	JT	GROSS REACTION	JT	GROSS REACTION	JT	GROSS REACTION
O	1462 0	O	1462 0	O	5-8	O	5-8
J	1462 0	J	1462 0	J	5-8	J	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	JT	COMBINED
O	1025 724 / 0	O	1025 724 / 0
J	1025 724 / 0	J	1025 724 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M, D-M. DBS = 20-0-0. CBF = 69 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 28	-77.4 -77.4	0.09 (1)	M-E	0 / 832	0.13 (1)	
B-C	-1591 / 0	-77.4 -77.4	0.63 (1)	M-F	-554 / 0	0.32 (1)	
C-D	-1591 / 0	-77.4 -77.4	0.63 (1)	K-F	-47 / 108	0.04 (4)	
D-E	-1154 / 0	-77.4 -77.4	0.56 (1)	D-M	-554 / 0	0.32 (1)	
E-F	-1154 / 0	-77.4 -77.4	0.56 (1)	N-D	-47 / 108	0.04 (4)	
F-G	-1591 / 0	-77.4 -77.4	0.63 (1)	B-N	0 / 1368	0.31 (1)	
G-H	-1591 / 0	-77.4 -77.4	0.63 (1)	K-H	0 / 1368	0.31 (1)	
H-I	0 / 28	-77.4 -77.4	0.09 (1)				
O-B	-1408 / 0	0.0 0.0	0.14 (1)				
J-H	-1408 / 0	0.0 0.0	0.14 (1)				
O-N	0 / 0	-18.2 -18.2	0.23 (4)				
N-M	0 / 1354	-18.2 -18.2	0.34 (1)				
M-L	0 / 1354	-18.2 -18.2	0.34 (1)				
L-K	0 / 1354	-18.2 -18.2	0.34 (1)				
K-J	0 / 0	-18.2 -18.2	0.23 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.63/1.00 (F-H:1), BC=0.34/1.00 (K-M:1), WB=0.32/1.00 (F-M:1), SSI=0.22/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

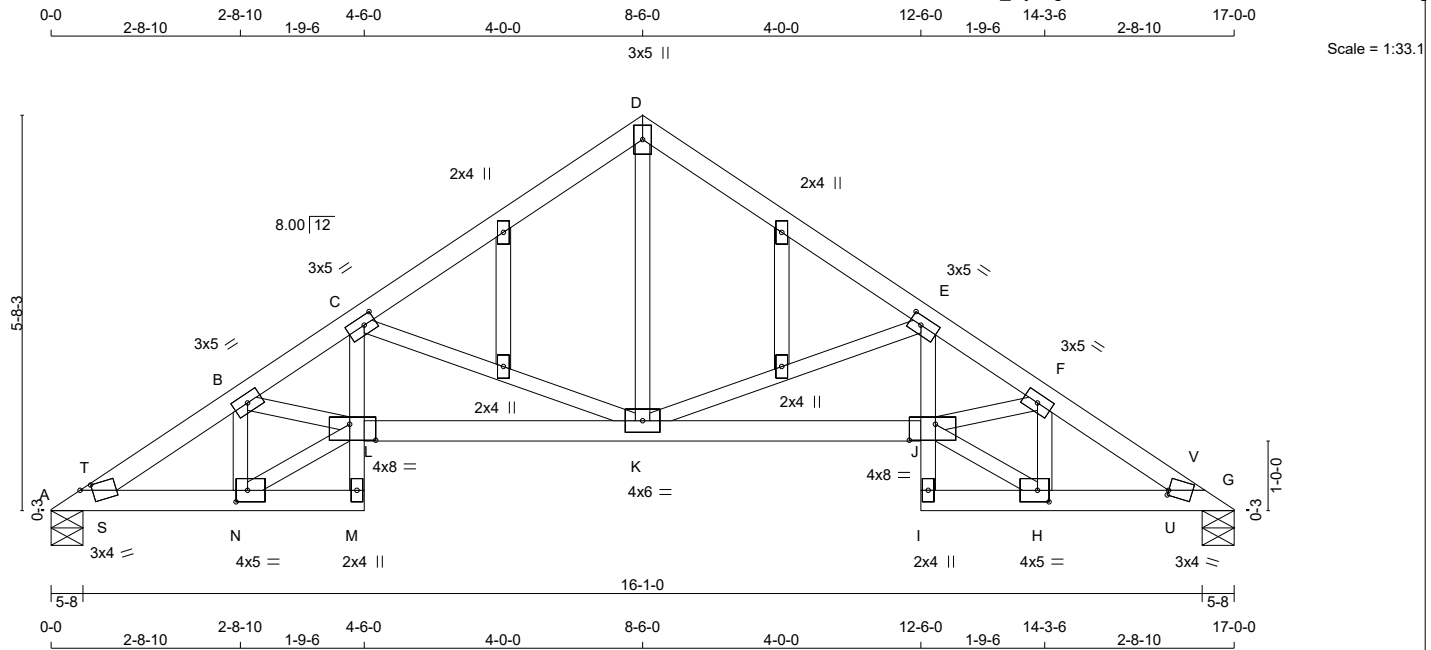
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.51 (N) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 66 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	813	0	813	0	5-8	5-8
G	813	0	813	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	571	396 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
G	571	396 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO				
A- T	-1471 / 0	-77.4 -77.4	0.16 (1)	5.24	K- D	0 / 791	0.18 (1)
T- B	-1479 / 0	-77.4 -77.4	0.18 (1)	5.21	K- E	-985 / 0	0.32 (1)
B- C	-2020 / 0	-77.4 -77.4	0.13 (1)	4.66	H- F	-589 / 0	0.09 (1)
C- D	-974 / 0	-77.4 -77.4	0.18 (1)	6.12	C- K	-985 / 0	0.32 (1)
D- E	-974 / 0	-77.4 -77.4	0.18 (1)	6.12	N- B	-589 / 0	0.09 (1)
E- F	-2020 / 0	-77.4 -77.4	0.13 (1)	4.66	N- L	0 / 1457	0.33 (1)
F- V	-1479 / 0	-77.4 -77.4	0.18 (1)	5.21	B- L	0 / 448	0.10 (1)
V- G	-1471 / 0	-77.4 -77.4	0.16 (1)	5.24	J- H	0 / 1457	0.33 (1)
					J- F	0 / 448	0.10 (1)
A- S	0 / 1246	-18.3 -18.3	0.47 (1)	10.00	S- T	-109 / 0	0.00 (1)
S- N	0 / 1268	-18.2 -18.2	0.47 (1)	10.00	U- V	-109 / 0	0.00 (1)
N- M	0 / 28	-18.2 -18.2	0.05 (1)	10.00			
M- L	-7 / 8	0.0 0.0	0.06 (1)	10.00			
L- C	0 / 678	0.0 0.0	0.23 (1)	10.00			
L- K	0 / 1717	-18.3 -18.3	0.33 (1)	10.00			
K- J	0 / 1717	-18.3 -18.3	0.33 (1)	10.00			
I- J	-7 / 8	0.0 0.0	0.06 (1)	10.00			
J- E	0 / 678	0.0 0.0	0.23 (1)	10.00			
I- H	0 / 28	-18.2 -18.2	0.05 (1)	10.00			
H- U	0 / 1268	-18.2 -18.2	0.47 (1)	10.00			
U- G	0 / 1246	-18.2 -18.2	0.47 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.18/1.00 (B-T:1), BC=0.47/1.00 (N-S:1), WB=0.33/1.00 (L-N:1), SSI=0.19/1.00 (A-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

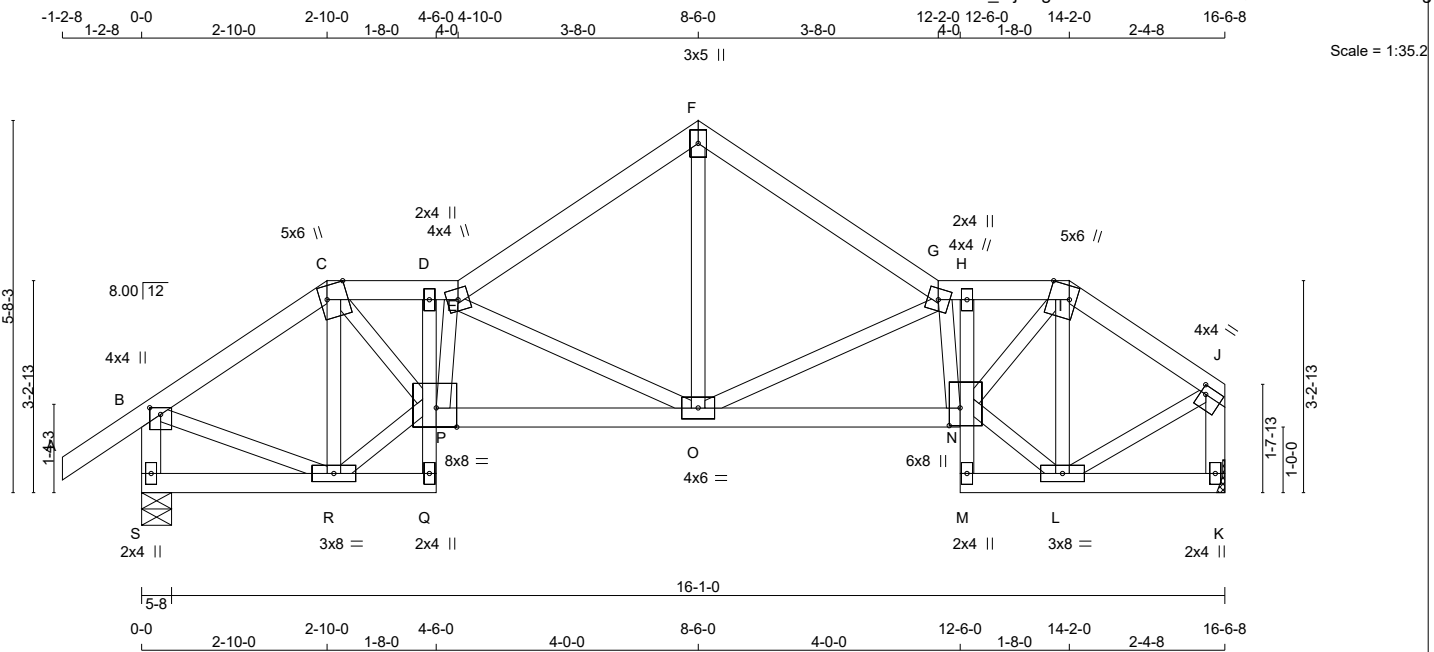
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.54 (G) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - D	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
M - H	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	Edge 3.75	
D	TMV+p	MT20	2.0	4.0		
E	TTWW+m	MT20	4.0	4.0		
F	TTW+p	MT20	3.0	5.0		
G	TTWW+m	MT20	4.0	4.0		
H	TMV+p	MT20	2.0	4.0		
I	TTWW+m	MT20	5.0	6.0	Edge 3.75	
J	TMVW+t	MT20	4.0	4.0	1.50	1.00
K	BMV1+p	MT20	2.0	4.0		
L	BMVWW-t	MT20	3.0	8.0		
M	BMV+p	MT20	2.0	4.0		
N	BVMWWW+i	MT20	6.0	8.0	3.25	2.00
O	BMVWW-t	MT20	4.0	6.0		
P	BVMWWW-i	MT20	8.0	8.0	Edge 3.75	
Q	BMV+p	MT20	2.0	4.0		
R	BMVWW-t	MT20	3.0	8.0		
S	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
S	891	0	891	0
K	791	0	791	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	623	445 / 0	0 / 0	0 / 0	0 / 0	178 / 0	0 / 0
K	556	385 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FACTORED CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	R-C	-583 / 0	0.11 (1)
B-C	-779 / 0	-77.4	-77.4 0.09 (1)	6.25	R-P	0 / 733	0.17 (1)
C-D	-1301 / 0	-77.4	-77.4 0.04 (1)	5.63	C-P	0 / 1073	0.24 (1)
D-E	-1312 / 0	-77.4	-77.4 0.02 (1)	5.64	P-E	-382 / 0	0.06 (1)
E-F	-908 / 0	-77.4	-77.4 0.14 (1)	6.25	E-O	-723 / 0	0.21 (1)
F-G	-908 / 0	-77.4	-77.4 0.14 (1)	6.25	O-F	0 / 719	0.16 (1)
G-H	-1221 / 0	-77.4	-77.4 0.02 (1)	5.80	O-G	-629 / 0	0.19 (1)
H-I	-1210 / 0	-77.4	-77.4 0.04 (1)	5.79	G-N	-418 / 0	0.06 (1)
I-J	-680 / 0	-77.4	-77.4 0.06 (1)	6.25	N-L	0 / 633	0.14 (1)
S-B	-870 / 0	0.0	0.0 0.09 (1)	7.81	N-I	0 / 1061	0.24 (1)
K-J	-774 / 0	0.0	0.0 0.08 (1)	7.81	L-I	-609 / 0	0.12 (1)
					B-R	0 / 685	0.15 (1)
					L-J	0 / 640	0.14 (1)

S-R	0 / 0	-18.2	-18.2 0.03 (4)	10.00
R-Q	0 / 21	-18.2	-18.2 0.04 (4)	10.00
Q-P	0 / 3	0.0	0.0 0.06 (1)	10.00
P-D	-14 / 18	0.0	0.0 0.04 (1)	7.81
P-O	0 / 1396	-18.2	-18.2 0.27 (1)	10.00
O-N	0 / 1313	-18.2	-18.2 0.25 (1)	10.00
M-N	0 / 6	0.0	0.0 0.05 (1)	10.00
N-H	-26 / 14	0.0	0.0 0.04 (1)	7.81
M-L	0 / 20	-18.2	-18.2 0.03 (4)	10.00
L-K	0 / 0	-18.2	-18.2 0.03 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.14/1.00 (E-F:1), BC=0.27/1.00 (O-P:1), WB=0.24/1.00 (C-P:1), SSI=0.09/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

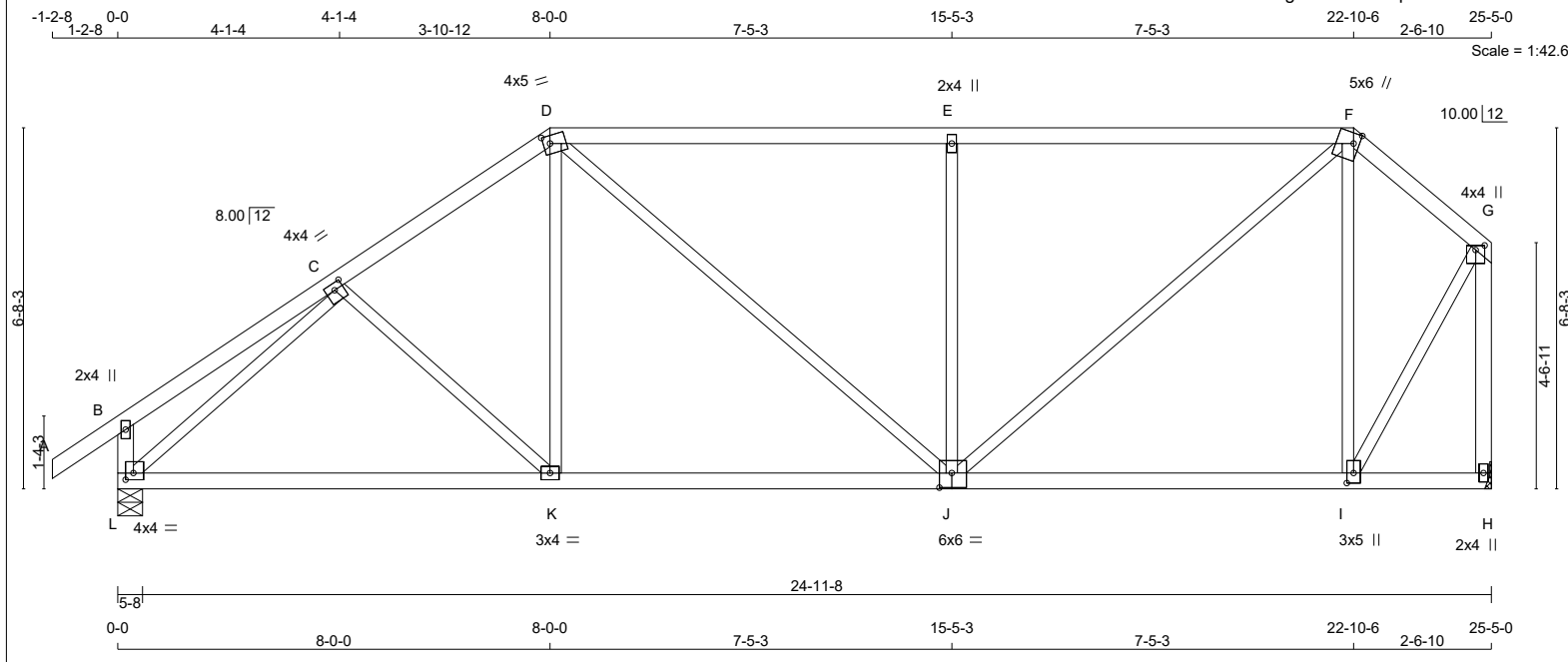
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.28 (N) (INPUT = 1.00)

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 111 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - G	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	2.00
D	TTWW-m	MT20	4.0	5.0	1.75	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.25
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW+t	MT20	3.0	5.0	2.25	1.50
J	BSWWW-l	MT20	6.0	6.0	3.25	2.75
K	BMWW-t	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	4.0	1.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
L	1315	0	1315	0
H	1215	0	1215	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	921	652 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
H	854	592 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			MEMB.	WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	C-K	-80 / 33	0.04 (1)
B-C	0 / 19	-77.4	-77.4 0.19 (1)	10.00	K-D	0 / 215	0.05 (4)
C-D	-1296 / 0	-77.4	-77.4 0.22 (1)	5.43	D-J	0 / 239	0.05 (1)
D-E	-1245 / 0	-77.4	-77.4 0.86 (1)	4.32	J-E	-709 / 0	0.53 (1)
E-F	-1245 / 0	-77.4	-77.4 0.86 (1)	4.32	J-F	0 / 1047	0.24 (1)
F-G	-601 / 0	-77.4	-77.4 0.09 (1)	6.25	I-F	-614 / 0	0.45 (1)
L-B	-220 / 0	0.0	0.0 0.02 (1)	7.81	L-C	-1525 / 0	0.74 (1)
H-G	-1217 / 0	0.0	0.0 0.44 (1)	7.26	I-G	0 / 864	0.19 (1)
L-K	0 / 1120	-18.2	-18.2 0.35 (4)	10.00			
K-J	0 / 1064	-18.2	-18.2 0.36 (4)	10.00			
J-I	0 / 450	-18.2	-18.2 0.21 (4)	10.00			
I-H	0 / 0	-18.2	-18.2 0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.86/1.00 (D-E:1), BC=0.36/1.00 (J-K:4), WB=0.74/1.00 (C-L:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

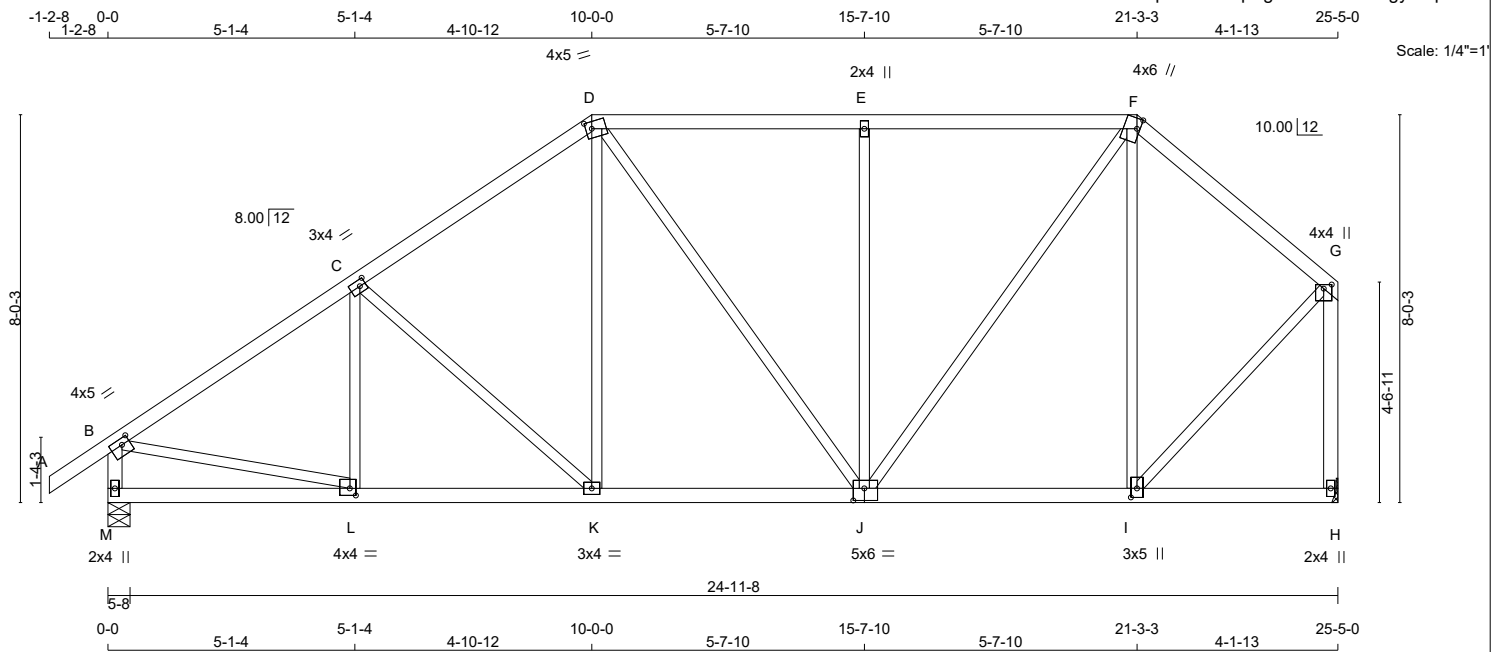
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 120 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - G	2x4	DRY No.2	SPF
M - B	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW-m	MT20	4.0	5.0	1.75	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	4.0	6.0	2.50	0.75
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW+t	MT20	3.0	5.0	2.25	1.50
J	BSWWW-l	MT20	5.0	6.0	3.00	2.75
K	BMWW-t	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.75	1.50
M	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
M	1315	0	1315	0	5-8	5-8
H	1215	0	1215	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	921	652 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
H	854	592 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	L-C	-139 / 43	0.05 (1)	
B-C	-1392 / 0	-77.4	-77.4	0.28 (1)	5.23	C-K	-295 / 0	0.24 (1)	
C-D	-1179 / 0	-77.4	-77.4	0.26 (1)	5.58	K-D	0 / 293	0.07 (1)	
D-E	-990 / 0	-77.4	-77.4	0.33 (1)	5.86	D-J	0 / 48	0.01 (1)	
E-F	-990 / 0	-77.4	-77.4	0.33 (1)	5.86	J-E	-534 / 0	0.66 (1)	
F-G	-732 / 0	-77.4	-77.4	0.18 (1)	6.25	J-F	0 / 738	0.17 (1)	
M-B	-1275 / 0	0.0	0.0	0.13 (1)	7.13	I-F	-455 / 0	0.56 (1)	
H-G	-1187 / 0	0.0	0.0	0.43 (1)	7.33	B-L	0 / 1202	0.27 (1)	
						I-G	0 / 790	0.18 (1)	
M-L	0 / 0	-18.2	-18.2	0.10 (4)	10.00				
L-K	0 / 1179	-18.2	-18.2	0.24 (1)	10.00				
K-J	0 / 962	-18.2	-18.2	0.21 (1)	10.00				
J-I	0 / 555	-18.2	-18.2	0.15 (4)	10.00				
I-H	0 / 0	-18.2	-18.2	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
	23.3	PSF
	3.0	PSF
BOT CH.	LL	0.0 PSF
	DL	7.3 PSF
TOTAL LOAD		33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.08")

CSI: TC=0.43/1.00 (G-H:1), BC=0.24/1.00 (K-L:1), WB=0.66/1.00 (E-J:1), SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

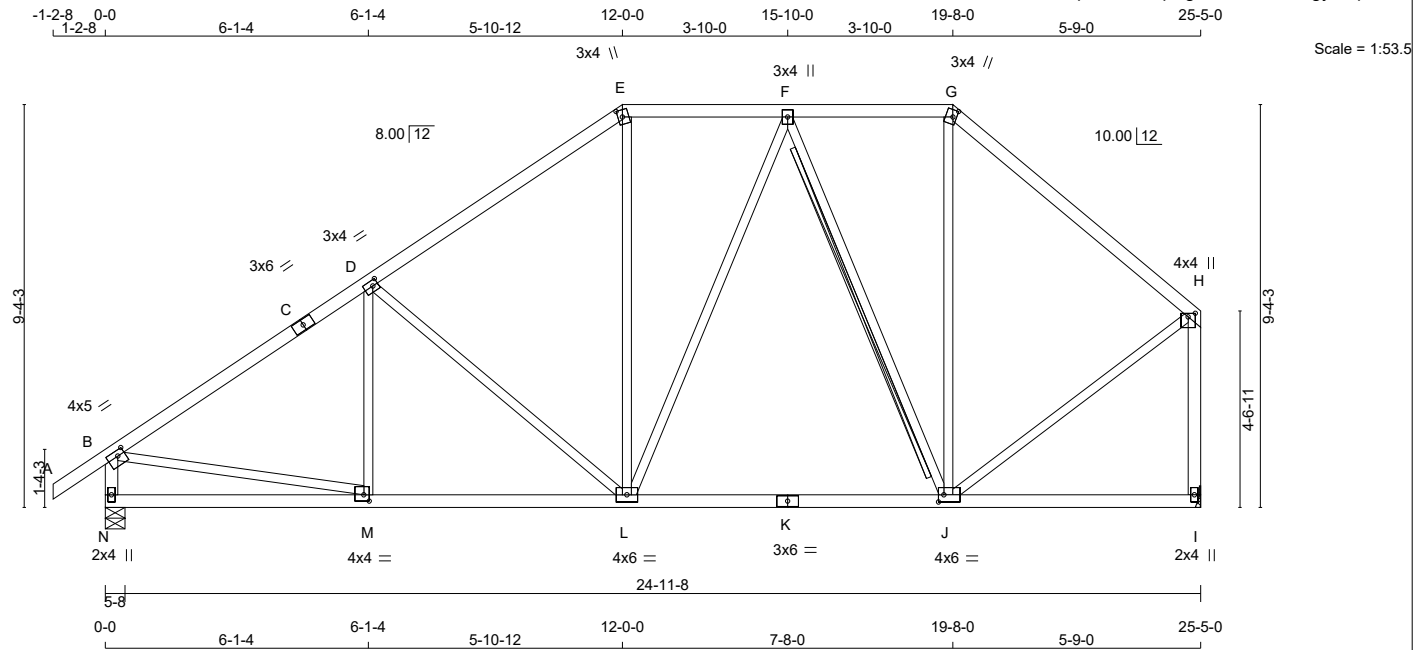
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 121 lb
[M][F]

LUMBER			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+m	MT20	3.0	4.0	2.00	1.25
F	TMWW-t	MT20	3.0	4.0		
G	TTW+m	MT20	3.0	4.0	2.00	1.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWWW-t	MT20	4.0	6.0	2.00	1.50
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0	1.75	1.50
N	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
N	1315	0	1315	0	5-8
I	1215	0	1215	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
N	921	652 / 0	0 / 0	0 / 0	270 / 0	0 / 0
I	854	592 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	M-D	-82 / 75	0.04 (1)
B-C	-1391 / 0	-77.4 -77.4	0.41 (1)	5.06	D-L	-430 / 0	0.55 (1)
C-D	-1391 / 0	-77.4 -77.4	0.41 (1)	5.06	L-E	0 / 284	0.06 (1)
D-E	-1061 / 0	-77.4 -77.4	0.38 (1)	5.64	J-G	0 / 172	0.04 (4)
E-F	-858 / 0	-77.4 -77.4	0.15 (1)	6.25	B-M	0 / 1199	0.27 (1)
F-G	-614 / 0	-77.4 -77.4	0.14 (1)	6.25	J-H	0 / 754	0.17 (1)
G-H	-798 / 0	-77.4 -77.4	0.34 (1)	6.25	F-J	-515 / 0	0.41 (1)
N-B	-1268 / 0	0.0 0.0	0.13 (1)	7.15	L-F	0 / 127	0.03 (1)
I-H	-1176 / 0	0.0 0.0	0.42 (1)	7.35			
N-M	0 / 0	-18.2 -18.2	0.15 (4)	10.00			
M-L	0 / 1183	-18.2 -18.2	0.28 (1)	10.00			
L-K	0 / 810	-18.2 -18.2	0.26 (4)	10.00			
K-J	0 / 810	-18.2 -18.2	0.26 (4)	10.00			
J-I	0 / 0	-18.2 -18.2	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.42/1.00 (H-I:1), BC=0.28/1.00 (L-M:1), WB=0.55/1.00 (D-L:1), SSI=0.19/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

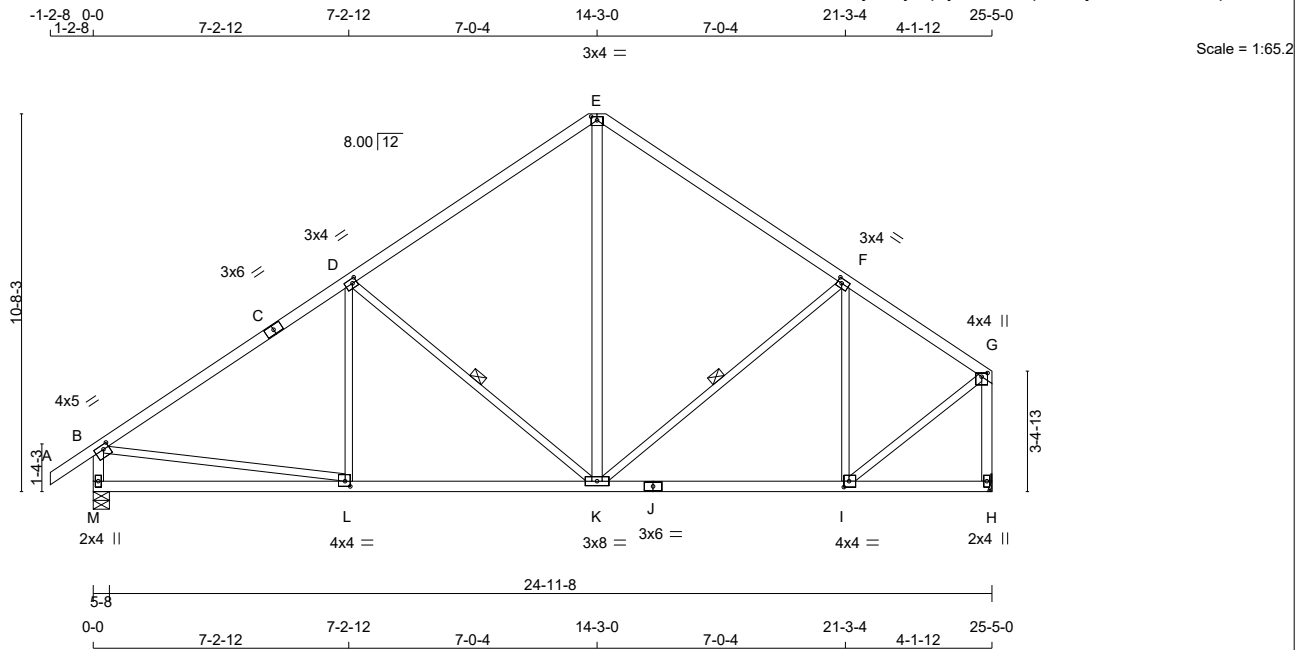
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 115 lb

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCRIPTION
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

J	T	TYPE	PLATES	W	LEN	Y	X
B	T	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	T	TS-t	MT20	3.0	6.0		
D	B	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	T	TTW-p	MT20	3.0	4.0	1.25	2.00
F	T	TMVW-t	MT20	3.0	4.0	1.50	1.50
G	T	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	B	BMV1+p	MT20	2.0	4.0		
I	B	BMVW-t	MT20	4.0	4.0	2.00	1.75
J	B	BS-t	MT20	3.0	6.0		
K	B	BMVWVW-t	MT20	3.0	8.0		
L	B	BMVW-t	MT20	4.0	4.0	1.75	1.75
M	B	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1315	0	1315	0	0	5-8	5-8
H	1215	0	1215	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX. FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	921	652 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
H	854	592 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-K, F-K. DBS = 20-0-0 . CBF = 72 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. UNBRAC	MAX. FACTORED		MAX CSI (LC)	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CSI (LC)		MEMB.	FORCE (LBS)		
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	L-D	-23 / 114	0.04 (4)	
B-C	-1372 / 0	-77.4	-77.4	0.60 (1)	4.79	D-K	-576 / 0	0.33 (1)	
C-D	-1372 / 0	-77.4	-77.4	0.60 (1)	4.79	K-E	0 / 549	0.09 (1)	
D-E	-915 / 0	-77.4	-77.4	0.55 (1)	5.65	K-F	-80 / 0	0.05 (1)	
E-F	-905 / 0	-77.4	-77.4	0.48 (1)	5.81	I-F	-475 / 0	0.29 (1)	
F-G	-904 / 0	-77.4	-77.4	0.38 (1)	5.96	B-L	0 / 1184	0.27 (1)	
M-B	-1261 / 0	0.0	0.0	0.13 (1)	7.16	I-G	0 / 986	0.22 (1)	
H-G	-1191 / 0	0.0	0.0	0.23 (1)	7.32				
M-L	0 / 0	-18.2	-18.2	0.22 (4)	10.00				
L-K	0 / 1172	-18.2	-18.2	0.31 (4)	10.00				
K-J	0 / 791	-18.2	-18.2	0.23 (4)	10.00				
J-I	0 / 791	-18.2	-18.2	0.23 (4)	10.00				
I-H	0 / 0	-18.2	-18.2	0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL LOAD			=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.60/1.00 (B-D:1) , BC=0.31/1.00 (K-L:4) ,
WB=0.33/1.00 (D-K:1) , SSI=0.22/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

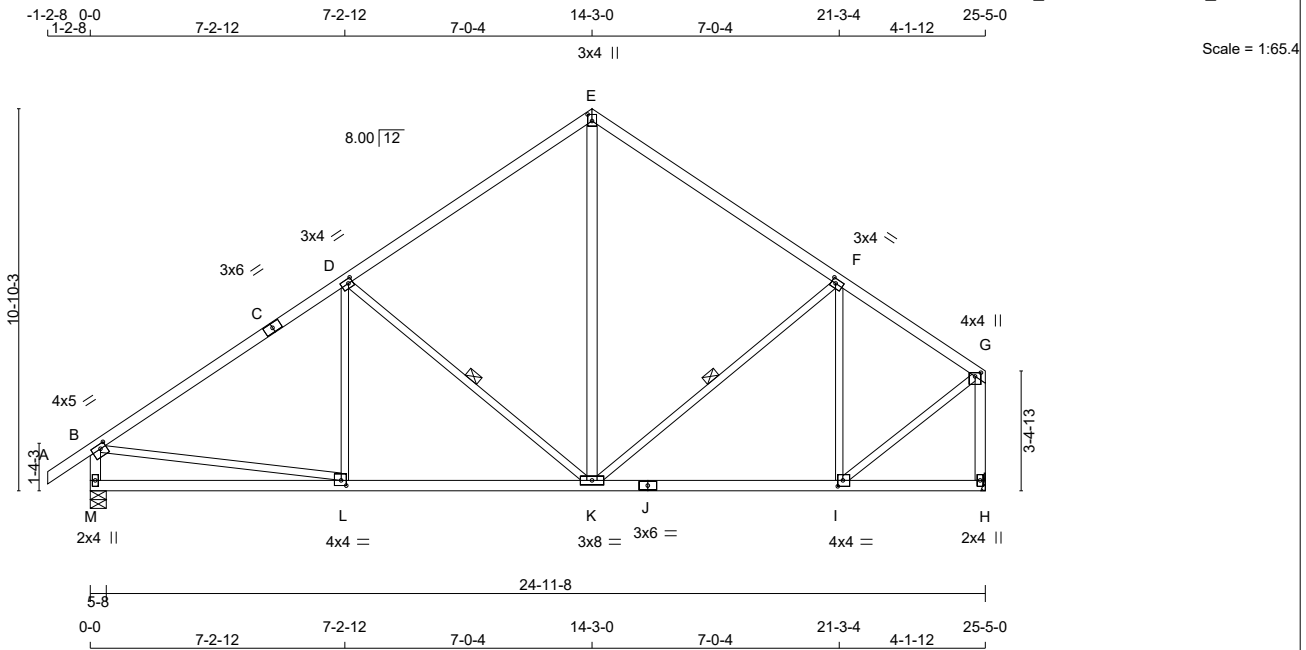
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 115 lb
[M][F]

LUMBER					DESCR.
CHORDS	SIZE	DRY	LUMBER	No.2	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
K - E	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+p	MT20	3.0	4.0	2.25	1.50
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	3.0	8.0		
L	BMWW-t	MT20	4.0	4.0	1.75	1.75
M	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	JT	GROSS REACTION	JT	BRG	JT	BRG
M	1315 0	M	1315 0	M	5-8	M	5-8
H	1215 0	H	1215 0	H	MECHANICAL	H	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
M	921	652 / 0	M	921	652 / 0
H	854	592 / 0	H	854	592 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-K, F-K. DBS = 20-0-0. CBF = 72 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	L-D	-23 / 114	0.04 (4)	
B-C	-1372 / 0	-77.4 -77.4	0.60 (1)	D-K	-576 / 0	0.33 (1)	
C-D	-1372 / 0	-77.4 -77.4	0.60 (1)	K-E	0 / 549	0.09 (1)	
D-E	-915 / 0	-77.4 -77.4	0.55 (1)	K-F	-80 / 0	0.05 (1)	
E-F	-905 / 0	-77.4 -77.4	0.48 (1)	I-F	-475 / 0	0.29 (1)	
F-G	-904 / 0	-77.4 -77.4	0.38 (1)	B-L	0 / 1184	0.27 (1)	
M-B	-1261 / 0	0.0 0.0	0.13 (1)	I-G	0 / 986	0.22 (1)	
H-G	-1191 / 0	0.0 0.0	0.23 (1)				
M-L	0 / 0	-18.2 -18.2	0.22 (4)				
L-K	0 / 1172	-18.2 -18.2	0.31 (4)				
K-J	0 / 791	-18.2 -18.2	0.23 (4)				
J-I	0 / 791	-18.2 -18.2	0.23 (4)				
I-H	0 / 0	-18.2 -18.2	0.14 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.60/1.00 (B-D:1), BC=0.31/1.00 (K-L:4), WB=0.33/1.00 (D-K:1), SSI=0.22/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

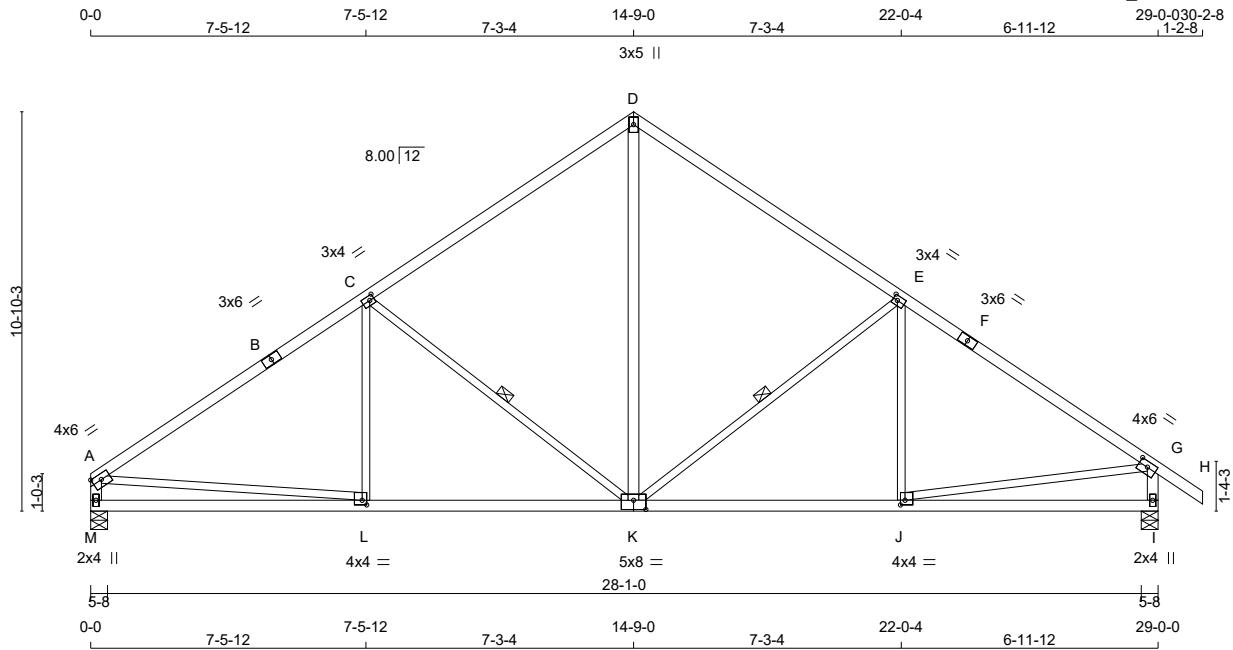
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:62.6

TOTAL WEIGHT = 4 X 123 = 491 lb

LUMBER				DESCR.	
CHORDS	SIZE	DRY	LUMBER	SPF	
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
M - A	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
K - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	4.0	6.0	1.75 Edge
B	TS-t	MT20	3.0	6.0	
C	TMWW-t	MT20	3.0	4.0	1.50 1.50
D	TTW+p	MT20	3.0	5.0	
E	TMWW-t	MT20	3.0	4.0	1.50 1.50
F	TS-t	MT20	3.0	6.0	
G	TMVW-t	MT20	4.0	6.0	1.75 3.00
I	BMV1+p	MT20	2.0	4.0	
J	BMWW-t	MT20	4.0	4.0	1.50 1.50
K	BSWWW-t	MT20	5.0	8.0	3.00 4.00
L	BMWW-t	MT20	4.0	4.0	1.50 1.50
M	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1386	0	1386	0	0	5-8	5-8
I	1486	0	1486	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	974	675 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
I	1042	735 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-K. DBS = 20-0-0. CBF = 79 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)
A-B	-1718 / 0	-77.4 -77.4	0.70 (1)	L-C	0 / 130	0.05 (4)	
B-C	-1718 / 0	-77.4 -77.4	0.70 (1)	C-K	-634 / 0	0.38 (1)	
C-D	-1195 / 0	-77.4 -77.4	0.61 (1)	K-D	0 / 856	0.14 (1)	
D-E	-1192 / 0	-77.4 -77.4	0.59 (1)	K-E	-547 / 0	0.32 (1)	
E-F	-1636 / 0	-77.4 -77.4	0.61 (1)	J-E	-61 / 103	0.04 (4)	
F-G	-1636 / 0	-77.4 -77.4	0.61 (1)	A-L	0 / 1468	0.33 (1)	
G-H	0 / 28	-77.4 -77.4	0.09 (1)	J-G	0 / 1408	0.32 (1)	
M-A	-1330 / 0	0.0 0.0	0.13 (1)				
I-G	-1434 / 0	0.0 0.0	0.15 (1)				
M-L	0 / 0	-18.2 -18.2	0.24 (4)				
L-K	0 / 1462	-18.2 -18.2	0.36 (1)				
K-J	0 / 1393	-18.2 -18.2	0.34 (1)				
J-I	0 / 0	-18.2 -18.2	0.22 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/999 (0.14")

CSI: TC=0.70/1.00 (A-C:1), BC=0.36/1.00 (K-L:1), WB=0.38/1.00 (C-K:1), SSI=0.23/1.00 (A-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

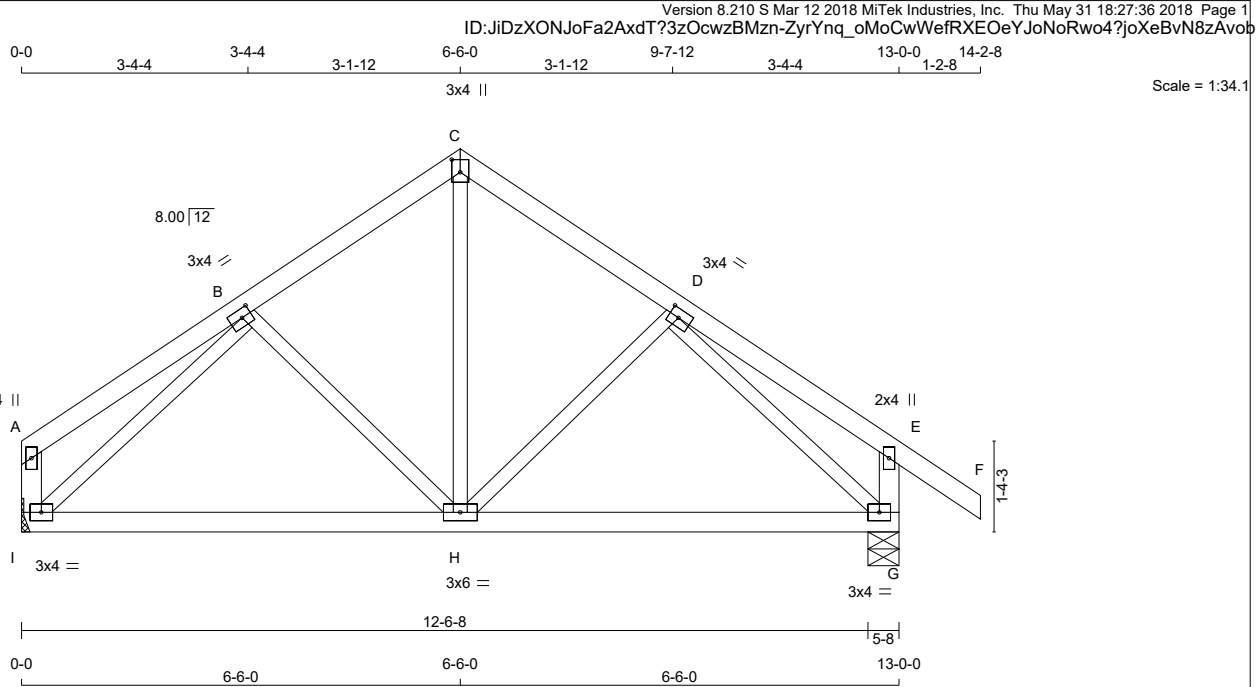
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.55 (L) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 55 = 109 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	3.0	4.0	1.50	1.75
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMWW-t	MT20	3.0	4.0	1.50	1.75
E	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	3.0	4.0		
H	BMVWW-t	MT20	3.0	6.0		
I	BMVW1-t	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UP
JT	437	0	0	0
I	621	0	0	0
G	721	0	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	437	303 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
G	504	363 / 0	0 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 15	-77.4	-77.4 0.13 (1)	H-C	0 / 328	0.07 (1)	
B-C	-488 / 0	-77.4	-77.4 0.10 (1)	H-D	-130 / 7	0.04 (1)	
C-D	-488 / 0	-77.4	-77.4 0.10 (1)	B-H	-130 / 7	0.04 (1)	
D-E	0 / 15	-77.4	-77.4 0.13 (1)	I-B	-682 / 0	0.22 (1)	
E-F	0 / 28	-77.4	-77.4 0.09 (1)	D-G	-682 / 0	0.22 (1)	
I-A	-97 / 0	0.0	0.0 0.01 (1)				
G-E	-197 / 0	0.0	0.0 0.02 (1)				
I-H	0 / 487	-18.2	-18.2 0.25 (4)				
H-G	0 / 487	-18.2	-18.2 0.25 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.13/1.00 (D-E:1) , BC=0.25/1.00 (G-H:4) ,
WB=0.22/1.00 (D-G:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

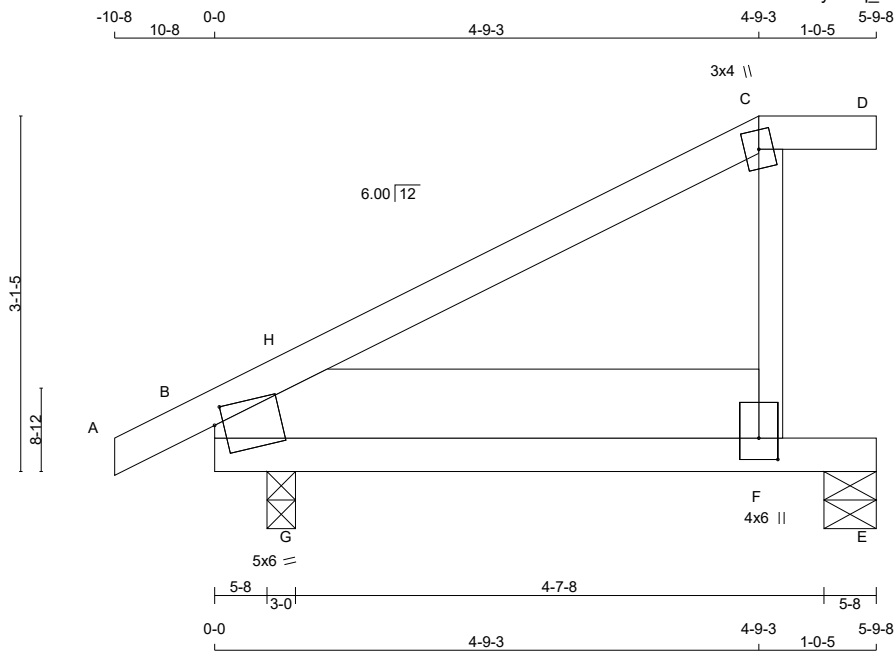
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:20.2

TOTAL WEIGHT = 28 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x8	DRY	No.2	SPF
ALL WEBS 2x3 DRY: SEASONED LUMBER.				
			No.2	SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBMW1-m	MT20	5.0	6.0	1.75 1.00
C	TTW+m	MT20	3.0	4.0	
F	BMWW+t	MT20	4.0	6.0	2.25 2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
E	270	0	270	0	0	5-8	5-8	5-8	5-8
B	357	0	357	0	0	3-0	3-0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	190	131 / 0	0 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
B	249	183 / 0	0 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 2	-77.4	-77.4 0.05 (1)	10.00	F-C	-205 / 0	0.04 (1)
B-H	-636 / 0	-77.4	-77.4 0.15 (1)	6.25	F-H	-510 / 0	0.06 (1)
H-C	-21 / 0	-77.4	-77.4 0.15 (1)	6.25	G-H	0 / 277	0.00 (1)
C-D	0 / 0	-77.4	-77.4 0.06 (1)	10.00			
B-G	0 / 508	-18.2	-18.2 0.23 (1)	10.00			
G-F	0 / 508	-18.2	-18.2 0.41 (1)	10.00			
F-E	0 / 0	-18.2	-18.2 0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 765 (0.09")

CSI: TC=0.15/1.00 (B-H:1), BC=0.41/1.00 (F-G:1),
WB=0.06/1.00 (F-H:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

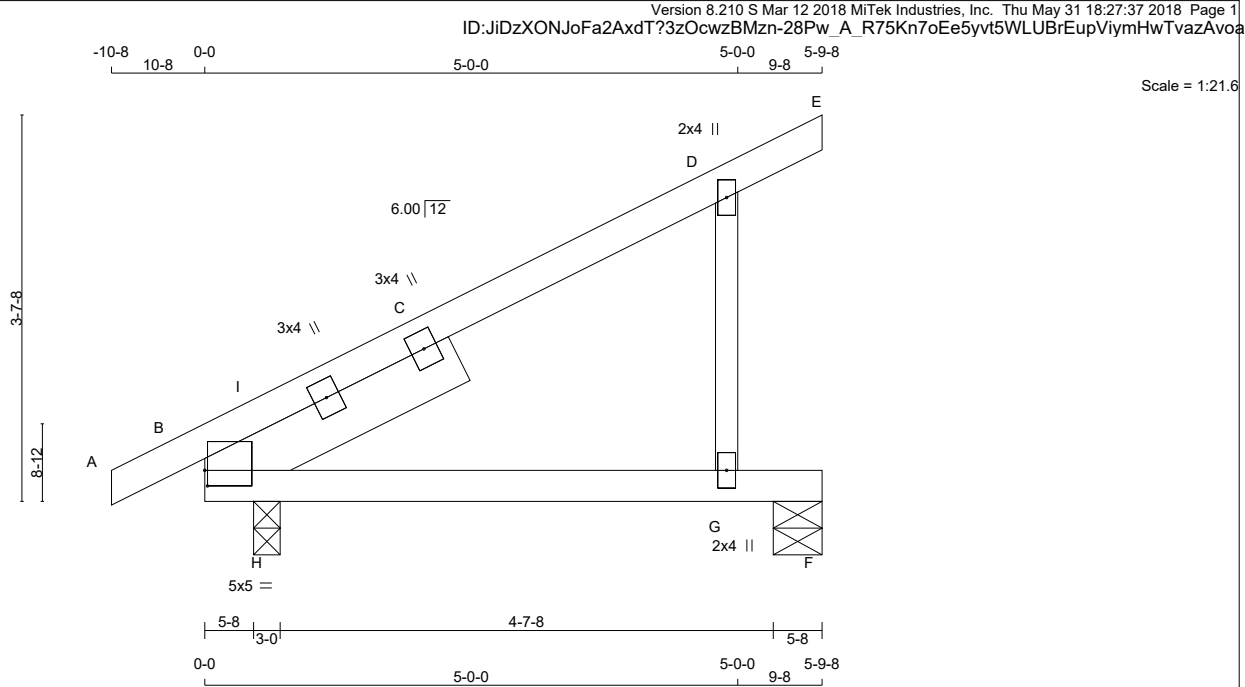
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.17 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 22 = 44 lb [M][F]

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	
A - E	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
REINFORCING MEMBERS					
HW1	2x6	DRY	No.2	SPF	
ALL WEBS 2x3 DRY: SEASONED LUMBER.					SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	MT20	5.0	5.0	1.75	0.25
B	RT+I	MT20	3.0	4.0		
B	RT+I	MT20	3.0	4.0		
D	TMW+w	MT20	2.0	4.0		
G	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
B	357	0	357	0	3-0	3-0	
F	275	0	275	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	249	183 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
F	193	134 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 2	-77.4 -77.4	0.05 (1)	G-D	-266 / 0	0.05 (1)	
B-I	-304 / 0	-77.4 -77.4	0.07 (1)	H-I	-215 / 0	0.00 (1)	
I-C	-157 / 0	-77.4 -77.4	0.34 (1)	H-C	0 / 340	0.04 (1)	
C-D	-157 / 0	-77.4 -77.4	0.34 (1)				
D-E	-18 / 0	-77.4 -77.4	0.04 (1)				
B-H	0 / 249	-18.2 -18.2	0.09 (1)				
H-G	0 / 6	-18.2 -18.2	0.33 (1)				
G-F	0 / 0	-18.2 -18.2	0.33 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 869 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 473 (0.15")

CSI: TC=0.34/1.00 (D-I:1), BC=0.33/1.00 (G-H:1), WB=0.05/1.00 (D-G:1), SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

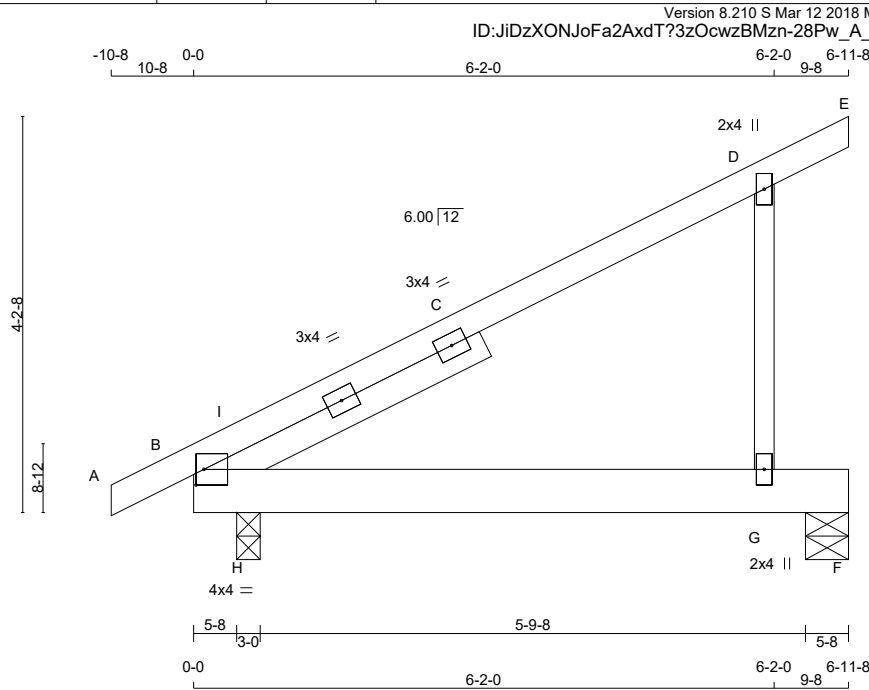
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (D) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 4 X 29 = 115 lb
[M][F]

LUMBER				DESCR.
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	
A - E	2x4	DRY	No.2	SPF
B - F	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS				
	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBR1-I	MT20	4.0	4.0	2.00	1.00
B	RT-I	MT20	3.0	4.0		
B	RT-I	MT20	3.0	4.0		
D	TMW+w	MT20	2.0	4.0		
G	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
B		411	0	411	0	0	3-0	3-0	
F		332	0	332	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	287	209 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
F	233	162 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FACTORED CSI (LC)	
FR-TO		FROM	TO		FR-TO				
A-B	0 / 5	-77.4	-77.4	0.05 (1)	10.00	G-D	-258 / 0	0.06 (1)	
B-I	-82 / 243	-77.4	-77.4	0.19 (1)	6.25	H-I	-475 / 0	0.00 (1)	
I-C	-41 / 27	-77.4	-77.4	0.29 (1)	6.25	H-C	-92 / 86	0.02 (4)	
C-D	-41 / 27	-77.4	-77.4	0.29 (1)	6.25				
D-E	-18 / 0	-77.4	-77.4	0.04 (1)	6.25				
B-H	-68 / 69	-18.2	-18.2	0.17 (1)	6.25				
H-G	0 / 5	-18.2	-18.2	0.26 (1)	10.00				
G-F	0 / 0	-18.2	-18.2	0.19 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
LL	3.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
LL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/744 (0.11")

CSI: TC=0.29/1.00 (D-I:1), BC=0.26/1.00 (G-H:1), WB=0.06/1.00 (D-G:1), SSI=0.35/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

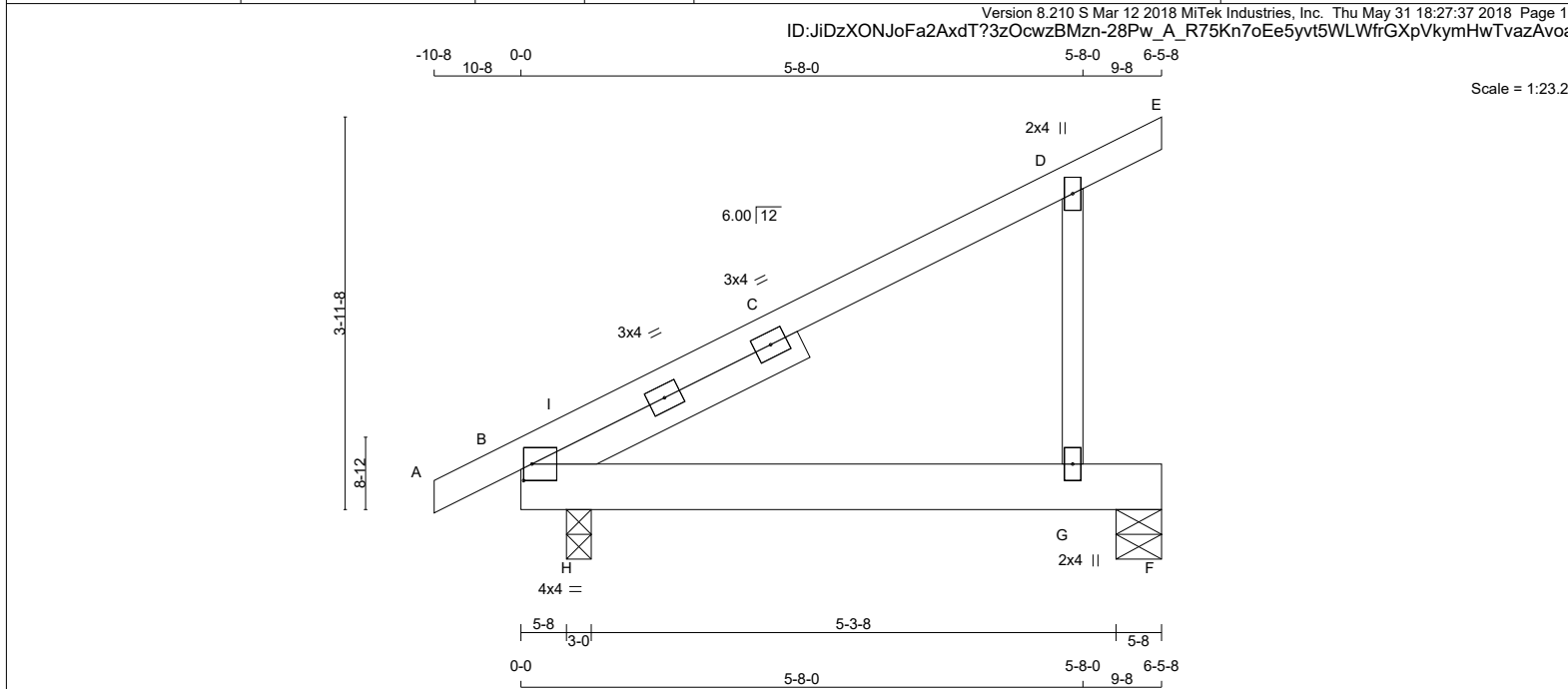
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (B) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 27 = 54 lb
[M][F]

LUMBER					DESCR.
CHORDS	SIZE	DRY	LUMBER	No.2	
A - E	2x4	DRY	No.2		SPF
B - F	2x6	DRY	No.2		SPF
REINFORCING MEMBERS					DESCR.
HW1	2x4	DRY	No.2		
ALL WEBS	2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBR1-I	MT20	4.0	4.0	2.00	1.00
B	RT-I	MT20	3.0	4.0		
B	RT-I	MT20	3.0	4.0		
D	TMW+w	MT20	2.0	4.0		
G	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
B	388	0	388	0	0	0	3-0	3-0	
F	308	0	308	0	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
B	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	271	198 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	216	150 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 5	-77.4	-77.4 0.05 (1)	10.00	G-D	-243 / 0	0.05 (1)
B-I	-74 / 178	-77.4	-77.4 0.15 (1)	6.25	H-I	-401 / 0	0.00 (1)
I-C	-36 / 14	-77.4	-77.4 0.25 (1)	6.25	H-C	-63 / 76	0.02 (4)
C-D	-36 / 14	-77.4	-77.4 0.25 (1)	6.25			
D-E	-18 / 0	-77.4	-77.4 0.04 (1)	6.25			
B-H	-44 / 60	-18.2	-18.2 0.14 (1)	6.25			
H-G	0 / 5	-18.2	-18.2 0.22 (1)	10.00			
G-F	0 / 0	-18.2	-18.2 0.18 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/920 (0.08")

CSI: TC=0.25/1.00 (D-I:1), BC=0.22/1.00 (G-H:1), WB=0.05/1.00 (D-G:1), SSI=0.30/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

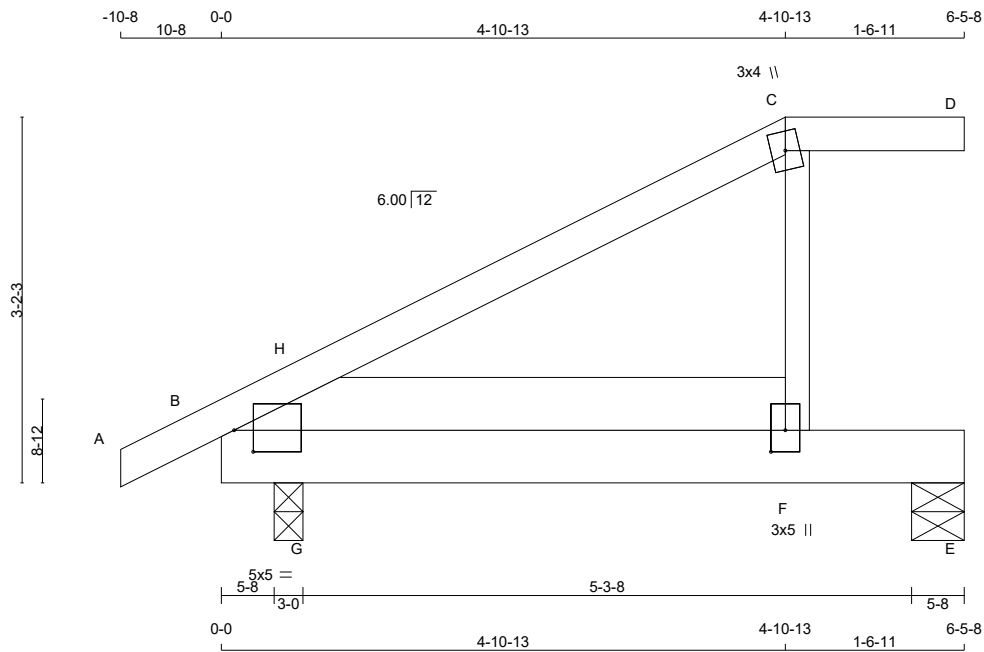
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:20.0

TOTAL WEIGHT = 31 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
B - E	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY: SEASONED LUMBER.				
			No.2	SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBW1-I	MT20	5.0	5.0	2.25 2.00
C	TTW+m	MT20	3.0	4.0	
F	BMWW+t	MT20	3.0	5.0	2.25 1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	294	0	294	0	5-8	5-8
B	396	0	396	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
E	207	142 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
B	276	203 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 5	-77.4	-77.4 0.05 (1)	10.00	F-C	-248 / 0	0.04 (1)
B-H	-719 / 0	-77.4	-77.4 0.16 (1)	6.25	F-H	-598 / 0	0.10 (1)
H-C	-23 / 0	-77.4	-77.4 0.16 (1)	6.25	G-H	0 / 291	0.00 (1)
C-D	0 / 0	-77.4	-77.4 0.13 (1)	10.00			
B-G	0 / 599	-18.2	-18.2 0.14 (1)	10.00			
G-F	0 / 599	-18.2	-18.2 0.34 (1)	10.00			
F-E	0 / 0	-18.2	-18.2 0.27 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3 PSF
	DL =	3.0 PSF
BOT CH.	LL =	0.0 PSF
	DL =	7.3 PSF
TOTAL LOAD	=	33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.16/1.00 (B-H:1), BC=0.34/1.00 (F-G:1), WB=0.10/1.00 (F-H:1), SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (F) (INPUT = 0.90)
JSI METAL= 0.26 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



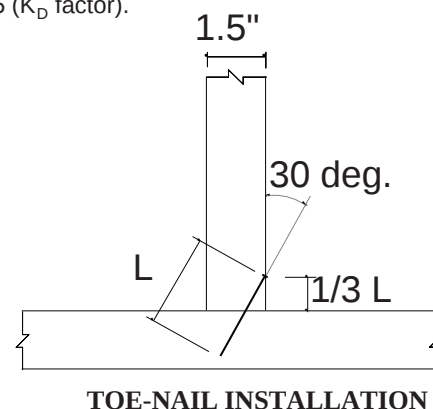
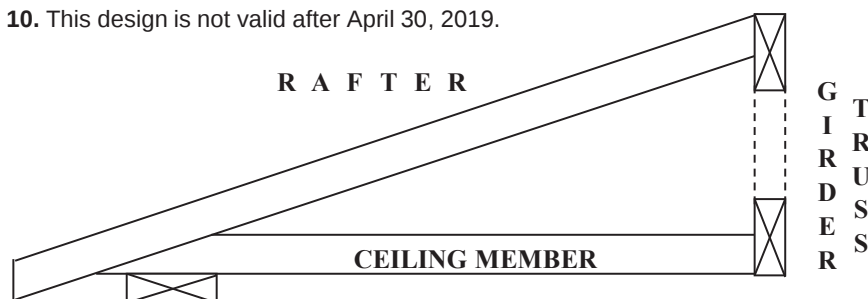
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

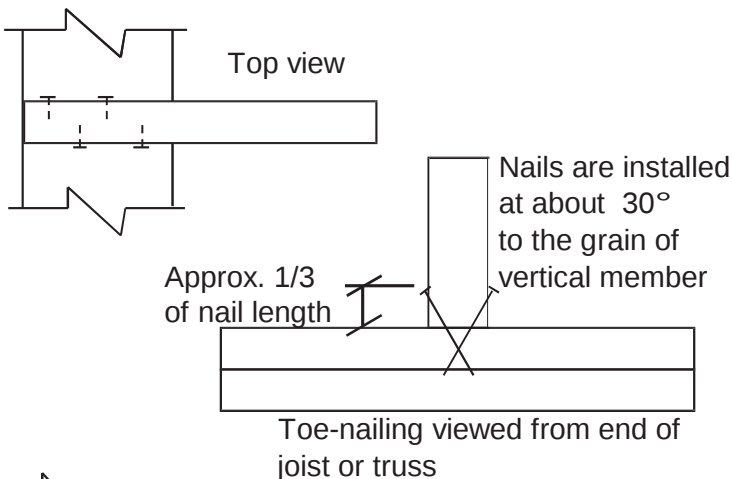
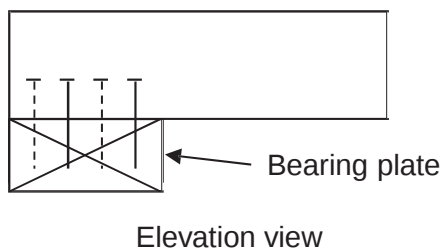
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

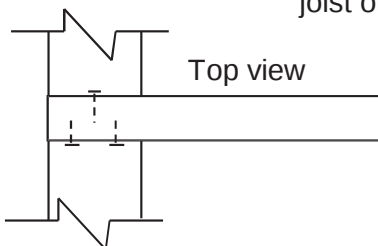
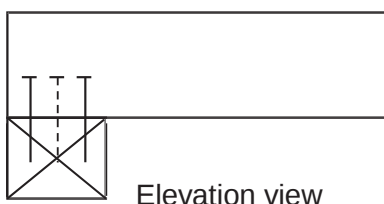
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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100 Industrial Rd.
Bradford, Ontario L3Z 3G7

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April 26, 2017