

		Products		
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	17
J2	14-00-00	11 7/8" NI-40x	1	19
J3	12-00-00	11 7/8" NI-40x	1	9
J4	10-00-00	11 7/8" NI-40x	1	4
J5	8-00-00	11 7/8" NI-40x	1	5
J6	6-00-00	11 7/8" NI-40x	1	5
J7	4-00-00	11 7/8" NI-40x	1	2
J8	20-00-00	11 7/8" NI-80	1	18
B1	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B2 🗸	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	6
B8 <sup>&lt;</sup>	16-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B10 <	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B16 /	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B17 ′	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B5 /	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4 /	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B6/	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B7~	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B3 ~	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	.1	1
B9/	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

	Connecto	r Summary
Qty	Manuf	Product
7	H1	IUS2.56/11.88
3	H1	IUS2.56/11.88
11	H2	IUS3.56/11.88
2	H3	HUS1.81/10
1	H3	HUS1.81/10

REFER TO THE NORDIC **INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6.

#### LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 20.0 lb/ft<sup>2</sup> TILED AREAS: 20 lb/ft<sub>2</sub>

SUBFLOOR: 5/8" GLUED AND NAILED



FROM PLAN DATED:

MARCH 2017

**BUILDER:**GREENYORK HOMES

SITE: OSTIENSE

----

MODEL: AUBURN 4

**ELEVATION:** 1

LOT:

**CITY:** BRAMPTON

SALESMAN: R D DESIGNER: LBV REVISION:

**DATE:** 2017-06-08

1st FLOOR

DATE 67517

BCIN: 26064; FIRM: 29991

ENGINEERING ONLY - DIMENSIONS TO BE VERIFIED ON SITE SUPPORTING STRUCTURE TO BE VERIFIED BY QUALIFIED BUILDING DESIGNER. ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO JOIST(S) AND FLOOR BEAM(S) INSTALLATION. ALL NOTES DESIGNATING MORE OR LESS DAS PER PLAN WORKD DO NOT REPRESENT A PART OF THE SCOPE OF WORK WITHIN THE BOUNDARIES OF THE SEAL. THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT. ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNER(S) PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE FLOOR SYSTEM COMPONENTS REVIEWED IN THIS SUBMISSION. MUNICIPALITY HAVING JURISDICTION TO OBTAIN LOT SPECIFIC SCHEDULE 1 FORM

CAPACITY WITH RESPECT TO THE FLOOR SYSTEM COMPONENTS REVIEWED IN THIS SUBMISSION.

MUNICIPALITY HAVING JURISDICTION TO OBTAIN LOT SPECIFIC SCHEDULE 1 FORM

FROM THIS OFFICE PRIOR TO BUILDING PERMIT APPROVAL.

INSTALLERS OF THIS FLOOR SYSTEM AND THEIR COMPANIES HAVE THE RESPONSIBILITY OF ENSURING THEY

HAVE A COPY OF THE NORDIC INSTALLATION GUIDE AND ANY OTHER MANUFACTURER'S PRODUCT LITERATURE

WHICH WILL AID IN THE OVERALL PROPER INSTALLATION OF THIS FLOOR SYSTEM. INSTALLERS ARE TO READ

ALL PRODUCT LITERATURE AND INSTALLATION GUIDELINES BEFORE PROCEEDING. THE SUPPLIER AND SEALING

ENGINEER OF THIS FLOOR SYSTEM ARE NOT RESPONSIBLE FOR SURPLUS OR DEFICIT OF PRODUCTS AT

PROJECT'S END. THIS LAYOUT IS A GUIDE ONLY. CONFIRMATION OF ALL QUANTITIES, LENGHTS, AND DETAILS,

REMAINS THE RESPONSIBILITY OF THE FLOOR SYSTEM INSTALLATION CONTRACTOR.

DWG# TAM 30851 THROUGH DWG# TAM 30862 17 INCLUSIVE DATED 65517

SEALED STRUCTURAL COMPONENTS ONLY:

SEALED, THIRD PARTY LVL TYPE BEAMS, BUILT-UP CONVENTIONAL BEAMS, HEADERS, AND CONCENTRATED LOADED NORDIC WOOD-I JOIST ONLY. 2 X 6 SQUASH BLOCK REQUIRED AT ALL EXTERIOR SUPPORTS OR AS PEF PROJECT ENGINEER'S SPECIFICATIONS. WEB FILLER REINFORCEMENT REQUIRED AT ALL HANGER SUPPORTED JOIST EXCEEDING A REACTION OF 1500 LBS (FACTORED)-SEE DETAILS.

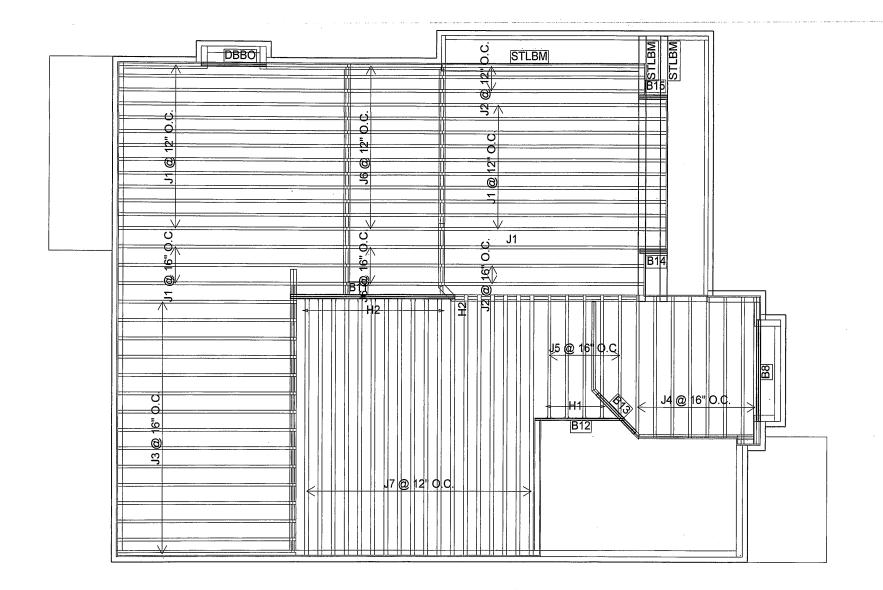
A COMPLETE FRAMING PLAN REQUIRES THE NORDIC PUBLISHED LITERATURE. WHICH INCLUDES INSTALLATION

A COMPLETE FRAMING PLAN REQUIRES THE NORDIC PUBLISHED LITERATURE, WHICH INCLUDES INSTALLATION REQUIREMENTS, HANDLING AND STORAGE GUIDELINES, AND FORMS AN INTEGRAL PART OF THIS SEALED DOCUMENT. INSTALL SQUASH BLOCKS FOR TRANSFERRING POINT LOADS FROM GIRDER TRUSSES, HEADERS, AND BEAMS DOWN TO FOUNDATION COMPONENTS. FOR PROPER INSTALLATION, SEE NORDIC LITERATURE. PROVIDE 2 X 4 OR 2 X 6 STUD GRADE OR BETTER SQUASH BLOCKS, MATCHING SUPPORTED WALL WIDTH ABOVE BLOCKS. INSTALL SQUASH BLOCKS ON EACH SIDE OF JOIST. BLOCKING TO BE 1/16D DEEPER THAN JOIS DEPTH. SEE NORDIC LITERATURE FOR NAILING REQUIREMENT.

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN WORK ON BE REGISTERED UNDER SUBSECTION 3.25 OF THE ONTARIO BUILDING CAP HE FIRM IS REGISTERED, IN APPROPRIATE CLASSES AND/OR CATEGORY

REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

DWG # TAM 50 BCIN: 26064 FIRM: 29991 SEALED STRUCTURAL COMPONENTS ONLY S. KATSOULAKOS S



Length			
_0.19	Product	Plies	Net Qty
18-00-00	11 7/8" NI-40x	1	27
16-00-00	11 7/8" NI-40x	1	5
14-00-00	11 7/8" NI-40x	1	15
12-00-00	11 7/8" NI-40x	1	8
10-00-00	11 7/8" NI-40x	1	5
8-00-00	11 7/8" NI-40x	1	16
20-00-00	11 7/8" NI-80	1	18
4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
	18-00-00 16-00-00 14-00-00 12-00-00 10-00-00 8-00-00 20-00-00 4-00-00 14-00-00 8-00-00 4-00-00	18-00-00	18-00-00

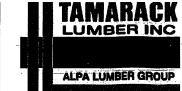
	Connecto	r Summary
Qty	Manuf	Product
4	H1	IUS2.56/11.88
12	H2	IUS3.56/11.88

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 &2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6.

#### LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 20.0 lb/ft2 TILED AREAS: 20 lb/ft2

SUBFLOOR: 5/8" GLUED AND NAILED



FROM PLAN DATED: **MARCH 2017** 

**BUILDER:** 

**GREENYORK HOMES** 

SITE:

**OSTIENSE** 

**MODEL:** AUBURN 4

**ELEVATION: 1** 

LOT:

**CITY: BRAMPTON** 

SALESMAN: RD **DESIGNER:** LBV **REVISION:** 

**DATE:** 2017-06-09

2nd FLOOR

S. KATSOULAKOS

DATE 67.5 12

BCIN: 26064; FIRM: 29991

ENGINEERING ONLY - DIMENSIONS TO BE VERIFIED ON SITE SUPPORTING STRUCTURE TO BE VERIFIED BY QUALIFIED BUILDING DESIGNER. ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO JOIST(S) AND FLOOR BEAM(S) INSTALLATION. ALL NOTES DESIGNATING MORE OR LESS DAS PER PLAN WORKD DO NOT REPRESENT A PART OF THE SCOPE OF WORK WITHIN THE BOUNDARIES OF THE SEAL. THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT. ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNER(S) PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE FLOOR SYSTEM COMPONENTS REVIEWED IN THIS SUBMISSION. MUNICIPALITY HAVING JURISDICTION TO OBTAIN LOT SPECIFIC SCHEDULE 1 FORM

FROM THIS OFFICE PRIOR TO BUILDING PERMIT APPROVAL. INSTALLERS OF THIS FLOOR SYSTEM AND THEIR COMPANIES HAVE THE RESPONSIBILITY OF ENSURING THEY HAVE A COPY OF THE NORDIC INSTALLATION GUIDE AND ANY OTHER MANUFACTURER'S PRODUCT LITERATURE WHICH WILL AID IN THE OVERALL PROPER INSTALLATION OF THIS FLOOR SYSTEM, INSTALLERS ARE TO READ ALL PRODUCT LITERATURE AND INSTALLATION GUIDELINES BEFORE PROCEEDING. THE SUPPLIER AND SEALING ENGINEER OF THIS FLOOR SYSTEM ARE NOT RESPONSIBLE FOR SURPLUS OR DEFICIT OF PRODUCTS AT PROJECT'S END. THIS LAYOUT IS A GUIDE ONLY. CONFIRMATION OF ALL QUANTITIES, LENGHTS, AND DETAILS, REMAINS THE RESPONSIBILITY OF THE FLOOR SYSTEM INSTALLATION CONTRACTOR.

DWG# TAM3084617 THROUGH DWG# TAM 3085047 INCLUSIVE DATED 61577

SEALED STRUCTURAL COMPONENTS ONLY: +30863-17
SEALED, THIRD PARTY LVL TYPE BEAMS, BUILT-UP CONVENTIONAL BEAMS, HEADERS, AND CONCENTRATED LOADED NORDIC WOOD-1 JOIST ONLY. 2 X 6 SQUASH BLOCK REQUIRED AT ALL EXTERIOR SUPPORTS OR AS PEF PROJECT ENGINEER'S SPECIFICATIONS. WEB FILLER REINFORCEMENT REQUIRED AT ALL HANGER SUPPORTED JOIST EXCEEDING A REACTION OF 1500 LBS (FACTORED)-SEE DETAILS.

A COMPLETE FRAMING PLAN REQUIRES THE NORDIC PUBLISHED LITERATURE, WHICH INCLUDES INSTALLATION REQUIREMENTS, HANDLING AND STORAGE GUIDELINES, AND FORMS AN INTEGRAL PART OF THIS SEALED DOCUMENT. INSTALL SQUASH BLOCKS FOR TRANSFERRING POINT LOADS FROM GIRDER TRUSSES, HEADERS, AND BEAMS DOWN TO FOUNDATION COMPONENTS. FOR PROPER INSTALLATION, SEE NORDIC LITERATURE. PROVIDE 2 X 4 OR 2 X 6 STUD GRADE OR BETTER SQUASH BLOCKS, MATCHING SUPPORTED WALL WIDTH ABOVE BLOCKS. INSTALL SQUASH BLOCKS ON EACH SIDE OF JOIST. BLOCKING TO BE 1/160 DEEPER THAN JOIS DEPTH. SEE NORDIC LITERATURE FOR NAILING REQUIREMENT.

REGISTERED CIONAL TARE RESPONSIBILITY FOR THE DESIGN WORK ON BEHAVE OF FRANCE REGISTERED UNDER SUBSECTION 3.2.5 OF THE ONTARIO BUILDING CODE. AM QUARTED AND HE FIRM IS REGISTERED, IN APPROPRIATE CLASSES AND/OR CATEGORIES

REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

SEALED STRUCTURAL COMPONENTS ONLY

## **Schedule 1: Designer Information**

Use one form for each individual who review	ws and takes re			the project.
A. Project Information		Application n	umber:	
Building number, street name			Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other descri	ption	
B. Individual who reviews and take	s responsibil	ity for design activities		
Name	teritorio e esta de social de la composición dela composición de la composición de la composición dela composición dela composición dela composición de la composición dela composición de la composición de la composición dela composición	Firm		
SAM KATSOULAKOS, P. ENG.		MICRO CITY ENGI	NEERING SERVI	CES INC.
Street address			Unit no.	Lot/con.
R.R #1, PO BOX 61				
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail	
Telephone number (519) 287-2242 Business	Fax number (519) 287-575	0	Cell number	
C. Design activities undertaken by			ulding Code T	able 3.5.2.1 of
Division C]				
☐ House		- House	Building      Section 1     Building      Section 2     Building      Section 3     Section 4     Building      Section 4     Section 4     Building      Section 4     Building      Section 4     Section 4     Building      Section 4     Section	
☐ Small Buildings		g Services	☐ Plumbing	
☐ Large Buildings ☐ Complex Buildings	☐ Detecti	on, Lighting and Power		- All Buildings
_ complex mananigo	☐ Fire Pro	otection	LI On-site S	ewage Systems
Description of designer's work GREENYC 1ST FLOOR (SCHEDULE IS NOT ISSUED	AS LOT SEE	<u> 0211EN2E – MODEL: AUB</u>	<u>URN 4 – ELEV. 1</u>	•
REVIEW PRE-ENGINEERED FLOOR SYS	TEM COMPON	FNT DRAWINGS AND LAV	JUT DI ACEMENI	T DI AN GLIDDI IED DV
TAMARACK ROOF TRUSSES INC. (SEE D	)WG #TAM3086	34-17 DATED 6-15-17)	JOTELAGENIEN	I FLAN SUFFLIED DI
SUPPORTING STRUCTURE TO BE REVIE	EWED AND VER	RIFED BY QUALIFIED BUIL	DING DESIGNER	<b>.</b> .
D. Declaration of Designer				
I, <u>SAM KATSOULAKOS,</u> P. ENG		de	eclare that (choos	e one as appropriate):
(print name				
☑ I review and take responsibili C, of the Building Code. I am o	ty for the desigr qualified, and th	nwork on behalf of a firm reg e firm is registered, in the ap	istered under sub propriate classes	section 3.2.4.of Division /categories.
Individual BCIN:2606	·			
muividual BCIN2000	,4			
Firm BCIN: 2999	1			*
☐ I review and take responsibility under subsection 3.2.5.of Divi			opriate category a	as an "other designer"
	•			
Basis for exemption from				
☐ The design work is exempt fro	m the registration	on and qualification requirem	ents of the Buildi	ng Code.
Basis for exemption from	registration and	qualification:		
I certify that:	-			
The information contained in this so	chedule is true t	o the best of mv knowledge.		
I have submitted this application with a submitted this application with a submitted this application.		•		
,				
	1522	0		
Date 6	11 01	Signature of Designer		

#### NOTE:

- 1. For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d).of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- 2. Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG#TAN 30 864-17-8 DWG#TAN 30866-17-8 61517

## **Schedule 1: Designer Information**

Use one form for each individual who review	ws and takes re			the project.
A. Project Information		Applicati	on number:	
Building number, street name			Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other	description	
B. Individual who reviews and take	s responsibil	ity for design activi	ties	
Name	ne d'acceptance de la	Firm	VIOLEN (NO. 1)	Company of Assessment and Assessment of the Asse
SAM KATSOULAKOS, P. ENG.		MICRO CITY	ENGINEERING SERVIC	
Street address R.R #1, PO BOX 61			Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail	
Telephone number (519) 287-2242 Business	Fax number (519) 287-575	0	Cell number	
C. Design activities undertaken by	1 '		B. TBuilding Code Tal	ole 3.5.2.1. of
Division C]				
☐ House		– House	⊠ Building Si	
☐ Small Buildings		g Services	☐ Plumbing – er ☐ Plumbing –	
☐ Large Buildings ☐ Complex Buildings		ion, Lighting and Powe otection		wage Systems
☐ Complex Buildings  Description of designer's work GREENYO				wago oyotomo
2ND FLOOR (SCHEDULE IS NOT ISSUE	D AS LOT SPE	CIFIC)		
REVIEW PRE-ENGINEERED FLOOR SYS	STEM COMPON	IENT DRAWINGS AND	LAYOUT PLACEMENT	PLAN SUPPLIED BY
TAMARACK ROOF TRUSSES INC. (SEE I	DWG #TAM308	65-17 DATED 6-15-17)	).	
SUPPORTING STRUCTURE TO BE REVI	EWED AND VE	RIFED BY QUALIFIED	BUILDING DESIGNER.	
D. Declaration of Designer				
I, <u>SAM KATSOULAKOS, P. ENG</u>			declare that (choose	one as appropriate):
(print nam	ie)			
☑ I review and take responsibil C, of the Building Code. I am	ity for the design qualified, and th	n work on behalf of a fi ne firm is registered, in	rm registered under subs the appropriate classes/	section 3.2.4.of Division categories.
Individual BCIN: <u>260</u> 0	64			
Firm BCIN: 299	91			
		t and the state of		a an "athar daalanar"
☐ I review and take responsibili under subsection 3.2.5.of Div	ty for the design	n and am qualified in th Ruilding Code	e appropriate category a	s an "otner designer"
Individual BCIN:	ision C, or the L	dilding Code.		
ilidividdai Boliv.		····		
Basis for exemption from	registration:			
☐ The design work is exempt fr	om the registrat	ion and qualification re	quirements of the Buildin	ıg Code.
Basis for exemption from			·	
I certify that:	-			
The information contained in this s	schedule is true	to the best of my know	vledge.	
I have submitted this application v				
2. 11070 0001111000 1110 0001011		-		
Date	1577	Signature of Designer		

#### NOTE:

- 1. For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d).of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

  DWG#TAM 30867-17-\$

  DWG#TAM 30867-17-\$

## NORDIC STRUCTURES

COMPANY TAMARACK LUMBER BURLINGTON June 7, 2017 12:50 PROJECT J1 GRD FLR

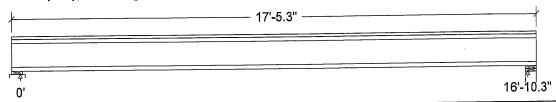
### **Design Check Calculation Sheet**

Nordic Sizer - Canada 6.4

#### Loads:

Load	Туре	Distribution			-	Magnitud Start	e End	Unit
			tern	Start	End			
Load1	Dead	Full Area				20.00		psf
Load2	Live	Full Area				40.00		psf

## Maximum Reactions (lbs), Bearing Resistances (lbs) and Bearing Lengths (in):



Unfactored:			174
Dead	174		349
Live	349		347
Factored:			741
Total	741		/ 11
Bearing:			
Resistance		·	2336
Joist	2336		7735
Support	6726		7755
Des ratio			0.32
Joist	0.32		0.10
Support	0.11		#2
Load case	#2		4-3/8
Length	4-3/8		1-3/4
Min req'd	1-3/4		No
Stiffener	No		1.00
Kd	1.00		1.00
KB support	1.00		769
fcp sup	769		1.15
Kzcp sup	1.00		

\*Minimum bearing length for joists is 2" for exterior supports

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

## Nordic Joist 11-7/8" NI-40x Floor joist @ 12" o.c.

Supports: 1 - Lumber Sill plate, No.1/No.2; 2 - Lumber Wall, No.1/No.2; Total length: 17'-5.3"; 5/8" nailed and glued OSB sheathing

This section PASSES the design code check.

### Limit States Design using CSA O86-14 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 716	Vr = 2336	lbs	Vf/Vr = 0.31
Moment(+)	Mf = 3019	Mr = 6255	lbs-ft	Mf/Mr = 0.48
Perm. Defl'n	$0.09 = \langle L/999 \rangle$	0.56 = L/360	in	0.17
Live Defl'n	$0.19 = \langle L/999$	0.42 = L/480	in	OFE 3510/Vg 0.45
Total Defl'n	0.28 = L/710	0.84 = L/240	in o	0.34
Bare Defl'n	0.22 = L/928	0.56 = L/360	in &	6/5/2 \ 6.0.39
Vibration	$L_{max} = 16' - 10$	$L_{V} = 18'-4$	ft / 😤 📞	61572
Defl'n	= 0.029	= 0.038	in O	WATSOULAKOS MOL. 18
DCII II			g 3	Will good and and and and and and and and and an

STRUCTURAL COMPONENT ONLY

#### J1 GRD FLR

#### Nordic Sizer - Canada 6.4

Page 2

	Additional											
	FACTORS:			KH	KZ	KL	KT	KS	KN	LC#		
	Vr	2336	1.00	1.00	-	-	-		-	#2		
	Mr+			1.00	-	1.000	-	-	-	#2		
	EI				-	-	-	-	-	#2		
	CRITICAL LO											
	Shear											
	Moment(+)	: LC #2	= 1.25	5D + 1.5I								
	Deflectio											
				+ 1.0L								
				+ 1.0L								
				+ 1.0L								
	Bearing	: Suppor	ct 1 - I	C #2 = 1	25D +	1.5L						
				C #2 = 1				<b>m</b>	±1 1			
-	Load Type	s: D=dead	d W=wir	nd S=sno	ow H=ea	arth, grou	ndwate	r E=ear	tnquake			
ĺ		L=live	e(use,oc	cupancy)	Ls=1:	ive(stora	ge,equ:	ipment)	r=rrre			
	Load Patt	erns: s=9	5/2 L=I	L+Ls _=r	no patte	ern Load	in this	s span				
	All Load		lons (L	s) are	istea :	in the An	arysis	output				
	CALCULATIO	DNS:	_				06 11					
	Deflectio	n: Elefi	= 4	133e06 I	)-in2	<= 0.18e	06 LDS	/73		nort 1		
	"Live" de	flection	= Defle	ection fi	com all	non-dead	Toads	(TIA6,	wilid, Si	110w)		
											n n n n n n	4.0

#### **Design Notes:**

CONFORMS TO OBG 2012

- 1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC Part 4) and the CSA O86-14 Engineering Design in Wood standard (May 2014 edition).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



DWG NO .TAM 3084/-17 STRUCTURAL COMPONENT ONLY

## NORDIC STRUCTURES

COMPANY TAMARACK LUMBER BURLINGTON June 7, 2017 15:26 PROJECT J1 GARAGE

### **Design Check Calculation Sheet**

Nordic Sizer - Canada 6.4

#### Loads:

Load	Type	Distribution	Pat-	Location	[ft]	Magnitude		Unit
			tern	Start	End	Start	End	
Load1	Dead	Full Area	No			20.00		psf
Load2	Live	Full Area	Yes			40.00		psf

### Maximum Reactions (lbs), Bearing Resistances (lbs) and Bearing Lengths (in):

			· · · · · ·	<del></del>
Unfactored: Dead Live	158 316		745 1490	-450 59
Factored: Uplift Total	672		3166	1969
Bearing: Resistance Joist Support	2135 4490		5587 <del>-</del>	
Des ratio Joist Support Load case	0.31 0.15 #4	·	0.57 - #2	
Length Min req'd Stiffener	2-5/8 1-3/4 No		6 3-1/2 No	
Kd KB support fcp sup Kzcp sup	1.00 1.00 769 1.11		1.00	

\*Minimum bearing length for joists is 2" for exterior supports

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

### Nordic Joist 11-7/8" NI-40x Floor joist @ 16" o.c.

Supports: 1 - Lumber Wall, No.1/No.2; 2,3 - Steel Beam, W; Total length: 16'-11.9"; 5/8" nailed and glued OSB sheathing This section PASSES the design code check.

### Limit States Design using CSA O86-14 and Vibration Criterion:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 2122	Vr = 2336	lbs	Vf/Vr = 0.91
Moment(+)	Mf = 1898	Mr = 6255	lbs-ft	Mf/Mr = 0.30
Moment (-)	Mf = 2913	Mr = 6255	lbs-ft	OFENS/MC = 0.47
Perm. Defl'n	0.04 = < L/999	0.50 = L/360	in #	0.07
Live Defl'n	$0.07 = \langle L/999 \rangle$	0.38 = L/480	in 🔊	.20
Total Defl'n	$0.11 = \langle L/999 \rangle$	0.75 = L/240	in /3	1027/ 20/15
Bare Defl'n	$0.09 = \langle L/999 \rangle$	0.50 = L/360	in M	KATSOULAKOS 5 17
Vibration	Lmax = 15'-0	Lv = 18'-9	ft   4 S	KATSOULABOS SI
Defl'n	= 0.022	= 0.044	in	0 50
			B 8	

DWO NO .TAM 30842 17
STRUCTURAL
COMPONENT ONLY

OLINCE OF ON

J1 GARAGE

#### Nordic Sizer - Canada 6.4

Page 2

```
Additional Data:
                                                                              LC#
FACTORS:
                      KD
                              KH
                                                KL
                                                                              #2
                     1.00
                             1.00
۷r
            2336
                                              1.000
                                                                              #4
                     1.00 1.00
             6255
Mr+
                                                                              #2
            6255
                     1.00
                             1.00
                                              1.000
Mr-
                                                                              #4
            371.1 million
EI
CRITICAL LOAD COMBINATIONS:
          : LC \#2 = 1.25D + 1.5L
Moment(+): LC \#4 = 1.25D + 1.5L (pattern: L)
Moment(-): LC \#2 = 1.25D + 1.5L
 Deflection: LC #1 = 1.0D (permanent)
              LC #4 = 1.0D + 1.0L (pattern: L_)
                                                     (live)
             LC #4 = 1.0D + 1.0L (pattern: L_)
LC #4 = 1.0D + 1.0L (pattern: L_)
                                                     (total)
                                                     (bare joist)
           : Support 1 - LC #4 = 1.25D + 1.5L (pattern: L_)
Support 2 - LC #2 = 1.25D + 1.5L
Support 3 - LC #1 = 1.4D
Bearing
Load Types: D=dead W=wind S=snow H=earth,groundwater E=earthquake
              L=live(use,occupancy) Ls=live(storage,equipment) f=fire
Load Patterns: s=S/2 L=L+Ls _=no pattern load in this span
All Load Combinations (LCs) are listed in the Analysis output
CALCULATIONS:
                           448e06 lb-in2 K= 6.18e06 lbs
 Deflection: Eleff =
 "Live" deflection = Deflection from all non-dead loads (live, wind, snow...)
```

- Design Notes:

  CONFORM\$ TO OBC 2012

  1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC Part 4) and the CSA O86-14 Engineering Design in Wood standard (May 2014 edition).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



DWG NO . TAM 30842-17 STRUCTURAL COMPONENT ONLY

## NORDIC **STRUCTURES**

COMPANY TAMARACK LUMBER BURLINGTON June 7, 2017 15:03

PROJECT J7 2ND FLR

### **Design Check Calculation Sheet**

Nordic Sizer - Canada 6.4

#### Loads:

Load	Туре	Distribution	Pat- tern	Location Start	[ft] End	Magnitud Start	e End	Unit
Load1 Load2	Dead Live	Full Area Full Area				20.00 40.00		psf psf

### Maximum Reactions (lbs), Bearing Resistances (lbs) and Bearing Lengths (in):

---- 19'-0.4" -18'-5.4" 0'

Unfactored: Dead Live	190 381	190 381
Factored:	809	809
Total Bearing:	009	
Resistance		2336
Joist	2336	10829
Support	10829	10029
Des ratio	0.35	0.35
Joist Support	0.33	0.07
Load case	#2	#2
Length	4-3/8	4-3/8
Min req'd	1-3/4	1-3/4
Stiffener	No	No 1.00
Kd	1.00	1.00
KB support	1.00 769	769
fcp sup Kzcp sup	1.15	1.15

\*Minimum bearing length for joists is 2" for exterior supports

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

Nordic Joist 11-7/8" NI-80 Floor joist @ 12" o.c. Supports: 1 - Lumber Sill plate, No.1/No.2; 2 - Lumber Wall, No.1/No.2; Total length: 19'-0.4"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling This section PASSES the design code check.

### Limit States Design using CSA O86-14 and Vibration Criterion:

Criterion	Analysis Value	Design	Value	Unit	Analysis/Design
Shear	Vf = 784	Vr =	2336	lbs	Vf/Vr = 0.34
Moment(+)	Mf = 3617	Mr =	11609	lbs-ft	Mf/Mr = 0.31
Perm. Defl'n	$0.10 = \langle L/999$	0.61 =	L/360	in	0.16
Live Defl'n	$0.20 = \langle L/999 \rangle$	0.46 =	L/480	in س	OFESSION 0.43
Total Defl'n	0.29 = L/751	0.92 =	L/240	in seeds	0.32
Bare Defl'n	$0.22 = \langle L/999 \rangle$	0.61 =	L/360	in 🎉	61512 6 .35
Vibration	Lmax = 18'-5	Lv =	20 <b>'-</b> 6	ft /3 (	61711
Defl'n	= 0.026	=	0.034	in w	78 AVOS
DOIT II					KATSUULANUO 30

DWG NO .TAN 30843 - 17 STRUCTURAL COMPONENT ONLY

POVINCE OF ONE

#### J7 2ND FLR

#### Nordic Sizer - Canada 6.4

Page 2

<b>Additiona</b>	l Data:								"	
FACTORS:	f/E	KD	KH	KZ	KL	KT	KS	KN	LC#	
Vr	2336	1.00	1.00	-	_	-	-	-	#2	
Mr+			1.00	-	1.000	-	-	-	#2	
ΕI	547.1 m			-	_	-	-	-	#2	
CRITICAL L	OAD COMB	INATIONS	S:							
Shear	: LC #2	= 1.2	5D + 1.51	Ĺ			,			
Moment(+	) : LC #2	= 1.2	5D + 1.51	Ĺ						
Deflecti	on: LC #1									
			D + 1.0L							
			D + 1.0L							
			D + 1.0L							
Bearing	: Suppo	rt 1 -	LC #2 = 3	1.25D +	1.5L					
	Suppo	rt 2 -	LC #2 = 3	1.25D +	1.5L		_			
Load Typ	es: D=dea	.d W=wi	nd S=sno	ow H=e	arth, grou	ındwate	r E=ear	tnquake		
	L=liv	re(use,o	ccupancy	) Ls=1	ive(stora	age,equ	ipment)	r=iire	•	
Load Pat	terns: s=	:S/2 L=	L+Ls _=	no patt	ern load	in thi	s span			
All Load	Combinat	ions (L	Cs) are .	listed	in the Ar	nalysıs	output			
CALCULATI	IONS:									
Deflecti	on: Elef	f =	613e06 l	o-in2	K= 6.186	e06 lbs				
"Live" d	eflection	L = Defl	ection f	rom all	. non-dead	d loads	(live,	wina, s	IIOW)	
		· · · · · · · · · · · · · · · · · · ·								

**Design Notes:** 

CONFORMS TO OBC 2012

- 1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC Part 4) and the CSA O86-14 Engineering Design in Wood standard (May 2014 edition).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



DWG NO. TAM30843-97 STRUCTURAL COMPONENT ONLY



COMPANY TAMARACK LUMBER BURLINGTON June 7, 2017 12:50

**PROJECT** J8 GRD FLR

### **Design Check Calculation Sheet**

Nordic Sizer - Canada 6.4

#### Loads:

Load	Type	Distribution	Pat- tern	Location Start	[ft] End	Magnitude Start Er	
Load1 Load2	Dead Live	Full Area Full Area				20.00 40.00	psf psf

# Maximum Reactions (lbs), Bearing Resistances (lbs) and Bearing Lengths (in):

—— 18'**-**11.2" – 18'-4.3" 0'

	U	
Unfactored: Dead Live	189 379	189 379
Factored: Total	805	805
Bearing:   Resistance   Joist   Support	2336 10829	2336 10829
Des ratio Joist Support Load case	0.34 0.07 #2	0.34 0.07 #2
Length Min req'd Stiffener	4-3/8 1-3/4 No	4-3/8 1-3/4 No 1.00
Kd KB support fcp sup	1.00 1.00 769 1.15	1.00 1.00 769 1.15

\*Minimum bearing length for joists is 2" for exterior supports

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

## Nordic Joist 11-7/8" NI-80 Floor joist @ 12" o.c.

Supports: 1 - Lumber Sill plate, No.1/No.2; 2 - Lumber Wall, No.1/No.2; Total length: 18'-11.2"; 5/8" nailed and glued OSB sheathing This section PASSES the design code check.

## Limit States Design using CSA O86-14 and Vibration Criterion:

DWG NO . TAM 30844.17 STRUCTURAL COMPONENT ONLY

POVINCE OF O

J8 GRD FLR

#### Nordic Sizer - Canada 6.4

Page 2

Additiona									W		
FACTORS:	f/E	KD ·	KH	KZ	KL	KT	KS	KN	LC#		
Vr	2336	1.00	1.00	-	-	-	_	-	#2		
Mr+	11609	1.00	1.00	-	1.000	-	-	-	#2		,
EI		illion	_	_	_	-	-	-	#2		
CRITICAL LO	OAD COMB	INATIONS	<b>S</b> :								
Shear	: LC #2	= 1.25	5D + 1.5I								
Moment (+	) : LC #2	= 1.25	5D + 1.5I	_							
Deflection	on: LC #1	= 1.01	) (perma	anent)							
			0 + 1.0L								
			0 + 1.0L								
			) + 1.0L								
Bearing	: Suppo	rt 1 - 1	LC #2 = 3	L.25D +	1.5L						
	Suppo	rt 2 - 1	LC #2 = 3	L.25D +	1.5L		_	. 1 1			
Load Type	es: D=dea	d W=win	nd S=sno	ow H=e	arth,grou	indwate:	r E=ear	tnquake			
	L=liv	e(use,o	ccupancy	Ls=l	ive(stora	ige, equ	ipment)	r=r1re			
Load Pat	terns: s=	S/2 L=1	L+Ls _=r	no patt	ern load	in thi	s span				
	Combinat	ions (L	Cs) are :	Listed	in the An	alysis	output				
CALCULATI	ONS:										
Deflecti	on: Elef	f =	613e06 l	o-in2	K= 6.18∈	06 lbs					
"Live" d	eflection	= Defl	ection f	com all	non-deac	l loads	(Ilve,	wind, s	110W)		
							<del></del>	CONI	nr eman	ORC 201	7

**Design Notes:** 

CONFORMS TO OBC 2012

- 1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC Part 4) and the CSA O86-14 Engineering Design in Wood standard (May 2014 edition).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



DWO NO. TAM30844-17 STRUCTURAL COMPONENT ONLY

## NORDIC STRUCTURES

COMPANY TAMARACK LUMBER BURLINGTON June 7, 2017 15:27 PROJECT J1 2ND FLR

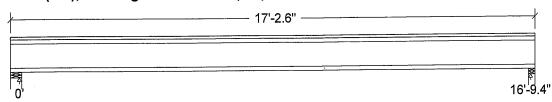
### **Design Check Calculation Sheet**

Nordic Sizer - Canada 6.4

#### Loads:

Load	Туре	Distribution		Location Start	[ft] End	Magnitude Start	e End	Unit
			tern	Start	EIIG			psf
Load1	Dead	Full Area				20.00		T.
Load2	Live	Full Area				40.00		psf

## Maximum Reactions (lbs), Bearing Resistances (lbs) and Bearing Lengths (in):



l	Unfactored:	0.00		228	
	Dead	232		455	
	Live	463		433	l
	Factored:	-		967	
	Total	984		967	
	Bearing:				
l	Resistance			0105	
ļ	Joist	2336		2135	
1	Support	7735		4490	
	Des ratio			0 45	
	Joist	0.42		0.45	
	Support	0.13		0.22	
	Load case	#2		#2	
	Length	4-3/8	•	2-5/8	
Ĺ	Min req'd	1-3/4	·	1-3/4	ı
	Stiffener	No		No	
	Kd	1.00		1.00	
	KB support	1.00		1.00	
	fcp sup	769		769	
	Kzcp sup	1.15		1.11	ĺ

\*Minimum bearing length for joists is 2" for exterior supports

Bearing for wall supports is perpendicular-to-grain bearing on top plate. No stud design included.

## Nordic Joist 11-7/8" NI-40x Floor joist @ 16" o.c.

Supports: All - Lumber Wall, No.1/No.2

Total length: 17'-2.6"; 5/8" nailed and glued OSB sheathing with 1/2" gypsum ceiling This section PASSES the design code check.

### Limit States Design using CSA O86-14 and Vibration Criterion:

1	•			
Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Vf = 951	Vr = 2336	lbs	Vf/Vr = 0.41
Moment(+)	Mf = 3990	Mr = 6255	lbs-ft	Mf/Mr = 0.64
Perm. Defl'n	0.12 = <l 999<="" td=""><td>0.56 = L/360</td><td>in وير</td><td>AFESSION 0.22</td></l>	0.56 = L/360	in وير	AFESSION 0.22
Live Defl'n	0.24 = L/832	0.42 = L/480	in 💆 🤉	0.58
Total Defl'n	0.36 = L/555	0.84 = L/240	in /O	2 0.43
Bare Defl'n	0.29 = L/704	0.56 = L/360	in /9/	61317
Vibration	Lmax = 16'-9	Lv = 17'-8	ft 🗓	KATSOULAKOS 5 85
Defl'n	= 0.032	= 0.038	in   9 S	KATSOULAKUS 50 85
L			3 650	

OWO NO. TAM30845 = 17
STRUCTURAL
COMPONENT ONLY

#### J1 2ND FLR

#### Nordic Sizer - Canada 6.4

Page 2

Additiona	l Data:									
FACTORS:		KD	KH	KZ	KL	KT	KS		LC#	
Vr	2336	1.00	1.00	_	_	-	-	-	#2	
Mr+	6255	1.00	1.00	-	1.000	-	-	-	#2	
ΕI	371.1 m	illion	-	-	-	-	-	-	#2	
CRITICAL LO	OAD COMB	INATIONS	<b>S</b> :							
	: LC #2									
	) : LC #2									
Deflection	on: LC #1									
			+ 1.0L							
			+ 1.0L							
			+ 1.0L							
Bearing	: Suppo							•		
	Suppo	rt 2 - 1	LC #2 = 1	L.25D +	1.5L			+ l l		
Load Type	es: D=dea	d W=wi	nd S=sno	ow H=e	arth,grou	nawate	r E=ear	tnquake		
	L=liv	e (use, o	ccupancy	Ls=l:	ive(stora	ge, equ	ipment)	r=rire		
Load Pat	terns: s=	S/2 L=1	L+Ls _=1	no patt	ern load	in this	s span			
	Combinat	ions (L	Cs) are	listed :	in the An	alysis	output			
CALCULATI						06.71				
Deflecti	on: Elef	f = '	148e06 l	o-in2 ]	K = 6.18e	U6 lbs	(3 :	استانا	mar: \	
"Live" d	eflection	= Defle	ection for	rom all	non-dead	Loads	(live,	wind, s	now)	
			·					e a N	EDDMS TO ORC	201

#### **Design Notes:**

- CONFORMS TO OBC 2012
- 1. WoodWorks analysis and design are in accordance with the 2010 National Building Code of Canada (NBC Part 4) and the CSA O86-14 Engineering Design in Wood standard (May 2014 edition).
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Refer to technical documentation for installation guidelines and construction details.
- 4. Nordic I-joists are listed in CCMC evaluation report 13032-R.
- 5. Joists shall be laterally supported at supports and continuously along the compression edge.
- 6. The design assumptions and specifications have been provided by the client. Any damages resulting from faulty or incorrect information, specifications, and/or designs furnished, and the correctness or accuracy of this information is their responsibility. This analysis does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of this component based on the design criteria and loadings shown.



DWO NO .TAM30845\_17
STRUCTURAL
COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B15(i2569)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:07

BC CALC® Design Report



Build 5033 Job Name: Address:

City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

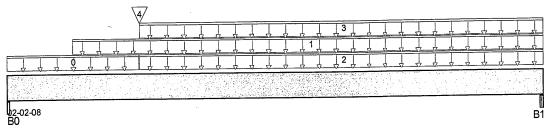
Description: Designs\Flush Beams\1st Floor\Flush Beams\B15(i2569)

Specifier:

Designer: LBV

Company:

Misc:



		Tota	l Hor	izontal Produ	ct Length = 0	2-02-08				
Reaction Summary (D	own / Uplift) ( Live	lbs)	ıd	S	now	Wind				
B0, 6-1/2"	56 / 0	123	3/0	-	1/0					
B1, 6-1/2"	82 / 0	) 172	2/0	1	59/0					
						Live	Dead	Snow	Wind	Trib.
Load Summary Tag Description	Load T	·vpe	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0 FC5 Floor Material		n. (lb/ft)	L	00-00-00	00-06-08	5				n/a
1 WALL		n. (lb/ft)	L	00-03-04	02-02-08		80			n/a
2 ROOF		n. (lb/ft)	L	00-06-08	02-02-08	66	60	144		n/a
3 FC5 Floor Material		n. (lb/ft)	L	00-06-08	02-02-08	8	4			n/a
4 FC5 Floor Material		Pt. (lbs)	L	00-06-08	00-06-08	12	6			n/a
	Factored	Factored	[	Demand/	Load	Location				
Controls Summary	Demand	Resistance	F	Resistance	Case					
Pos. Moment	88 ft-lbs	38,727 ft-lbs	S	0.2%	13	01-	01-04			

De mand/

De mand/

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	88 ft-lbs	38.727 ft-lbs	0.2%	13	01-01-04
End Shear	205 lbs	14,464 lbs	1.4%	13	
Span / Depth	1.3	n/a	n/a		00-00-00

Ro arii	ng Supports	Dim . (L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Beam	6-1/2" x 3-1/2"	303 lbs	2.5%	1.1%	Unspecified
B1	Beam	6-1/2" x 3-1/2"	495 lbs	4.1%	1.8%	Unspecified

#### Notes

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA GONFORMS TO OBC 2012

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWO NO . TAN30846-17 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B15(i2569)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:07

BC CALC® Design Report

**Build 5033** Job Name: Address:

City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B15(i256

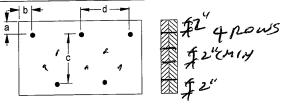
Specifier:

Designer: LBV

Company:

Misc:

#### **Connection Diagram**



a minimum = 2"

c = 7-7/8"

d= 💯 6 b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Chair r Nails

ARDOX SPIRAL

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO . TAM 30846-17 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B11(i2740)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:09

BC CALC® Design Report



Build 5033

Address: City, Province, Postal Code:BRAMPTON,

Customer:

Job Name:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

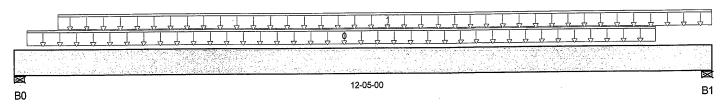
Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i2740)

Specifier:

Designer: LB\

Company:

Misc:



#### Total Horizontal Product Length = 12-05-00

Reaction Summary (	Down / Uplift) ( lbs ) Live	De ad	Snow	Wind	
B0, 5-1/2"	2,134/0	1,139/0			
B1, 6-5/8"	2,582/0	1,365/0			

10	ad Summary				Live	Dead	Snow Wind	Trib.
	Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
. 0	FC 5 Floor Material	Unf. Lin. (lb/ft)	L 00-02-12	11-05-00	19	9		n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L 00-09-04	12-05-00	386	193		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	14,209 ft-lbs	38,727 ft-lbs	36.7%	1	06-03-04
End Shear	4,433 lbs	14,464 lbs	30.6%	1.	10-10-08
Total Load Defl.	L/564 (0.245")	0.577"	42.5%	4	06-01-12
Live Load Defl.	L/863 (0.16")	0.384"	41.7%	5	06-01-12
Max Defl.	0.245"	n/a	n/a	4	06-01-12
Span / Depth	11.7	n/a	n/a	•	00-00-00

	•			Demand/ Resistance	Resistance	
Bea	ring Supports	Dim.(LxW)	Demand	Support	Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	4,625 lbs	45%	19.7%	Unspecified
B1	Wall/Plate	6-5/8" x 3-1/2"	5,579 lbs	45%	19.7%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86. CONFORMS TO UBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



p6 1 2

OWO NO . TAM 30847 = 17 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B11(i2740)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:09

BC CALC® Design Report

\*

Build 5033

Job Name: Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i274

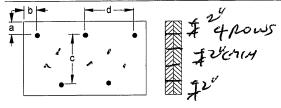
Specifier:

Designer: LBV

Company:

Misc:

#### **Connection Diagram**



#### Calculated Side Load = 769.3 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d t. \_\_non Nails

312" ARDOX SPIRAL

#### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO.TAM30847 -17 STRUCTURAL COMPONENT ONLY



## Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12(i2768)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:11

Trib

BC CALC® Design Report

\*

Build 5033 Job Name:

Address:

City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

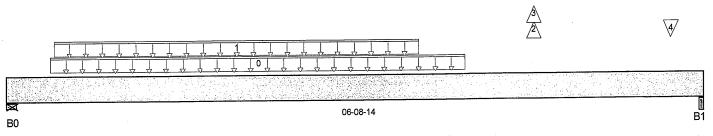
Description: Designs\Flush Beams\1st Floor\Flush Beams\B12(i2768)

Specifier:

Designer: LBV

Company:

Misc:



Total Horizontal Product Length = 06-08-14

Reaction Summary (Down /	Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5-1/2" B1, 4-1/16"	1,088 / 45 766 / 155	542/0 325/0			

	and Crosses was	•		Live	e Dead	Snow wind	IIID.
	ad Summary Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L 00-05-00	04-05-00 181	90		n/a
1	Us er Load	Unf. Lin. (lb/ft)	L 00-05-08	03-11-08 240	120		n/a
2	J5(i2722)	Conc. Pt. (lbs)	L 05-01-00	05-01-00 45	-78		n/a
3	J5(i2722)	Conc. Pt. (lbs)	L 05-01-00	05-01-00 -20	0		n/a
4	J5(j2747)	Conc. Pt. (lbs)	L 06-05-00	06-05-00 240	120		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,036 ft-lbs	19,364 ft-lbs	15′.7%	1	02-11-00
Neg. Moment	-67 ft-lbs	-19,364 ft-lbs	0.3%	4	05-01-00
End Shear	1.600 lbs	7,232 lbs	22.1%	1	01-05-06
Total Load Defl.	L/999 (0.028")	n/a	n/a	6	03-03-00
Live Load Defl.	L/999 (0.019")	n/a	n/a	8	03-04-00
Max Defl.	0.028"	n/a	n/a	6	03-03-00
Span / Depth	6.1	n/a	n/a		00-00-00

Reari	ng Supports	Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Wall/Plate	5-1/2" x 1-3/4"	2,308 lbs	44.9%	19.7%	Unspecified
B1	Beam	4-1/16" x 1-3/4"	1,556 lbs	20%	17.9%	Unspecified

Notes



Charle Mind

OWO NO. TAM 30940.17
STRUCTURAL
COMPONENT ONLY



## Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B12(i2768)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:11

BC CALC® Design Report

**Build 5033** Job Name:

Address:

City, Provinœ, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B12(i276

Designer: LBV

Company.

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Disclosure

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DWB NO . TAN 30848.17 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8(i2572)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:11

BC CALC® Design Report



Build 5033 Job Name:

Address:

City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

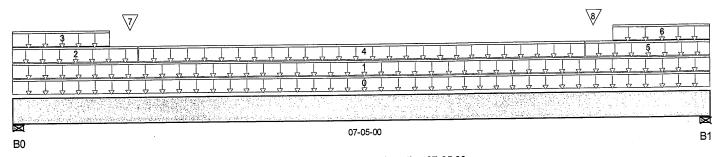
Description: Designs\Flush Beams\1st Floor\Flush Beams\88(i2572)

Specifier:

Designer: LBV

Company:

Misc:



Total Horizontal Produc	t Length = 07-05-00
-------------------------	---------------------

Reaction Summary (I	Down / Uplift) ( lbs ) Live	De ad	Snow	Wind	
B0, 5-1/2"	273/0	502/0	412/0		
B1, 5-1/2"	273/0	502/0	411/0		

	. 1.0					Live	Dead	Snow	Wind	Trib.
	ad Summary Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	LOWROOF	Unf. Lin. (lb/ft)	L	00-00-00	07-05-00	11	10	24		n/a
1	FC5 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	07-05-00	27	13			n/a
2	E22(i2779)	Unf. Lin. (lb/ft)	L	00-00-00	01-04-00		81			n/a
3	E22(i2779)	Unf. Lin. (lb/ft)	L	00-00-00	01-00-08	36	34	87		n/a
4	E21(i2778)	Unf. Lin. (lb/ft)	Ĺ	01-04-00	06-01-00		41			n/a
5	E20(i2777)	Unf. Lin. (lb/ft)	Ī	06-01-00	07-05-00		81			n/a
-	` '	Unf. Lin. (lb/ft)	ī	06-04-08	07-05-00	36	34	87		n/a
6	E20(i2777)	Conc. Pt. (lbs)	- 1	01-03-00	01-03-00	96	131	233		n/a
/ 8	E22(i2779) F20(i2777)	Conc. Pt. (lbs)	L.	06-02-00	06-02-00		130	231		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,363 ft-lbs	38,727 ft-lbs	3.5%	13	03-08-08
End Shear	994 lbs	14,464 lbs	6.9%	13	01-05-06
Total Load Defl.	L/999 (0.009")	n/a	n/a	45	03-08-08
Live Load Defl.	L/999 (0.004")	n/a	n/a	61	03-08-08
Max Defl.	0.009"	n/a	n/a	45	03-08-08
Span / Depth	6.7	n/a	n/a		00-00-00

				De mand/ Resistance		
Beari	ng Supports	Dim . (L x W)	Demand	Support	Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	1,383 lbs	13.5%	5.9%	Unspecified
B1	Wall/Plate	5-1/2" x 3-1/2"	1,380 lbs	13.4%	5.9%	Unspecified

Notes



DWB NO.TAM30849-17 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8(i2572)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:11

BC CALC® Design Report

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\88(i2572

Specifier:

Designer:

Company.

Misc:

**Build 5033** 

Job Name: Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

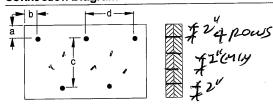
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBG 2012

Unbalanced snow loads determined from building geometry were used in selected products verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Connection Diagram



c = 7-7/8" a minimum = 2" d= 🍩 6 b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d 🦙 🕾 Nails

312" ARDOX SPIRAL

#### Disclosure

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DWG NO . TAN3-249-17 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B13(i2978)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 9, 2017 12:07:48

BC CALC® Design Report



Build 5033 Job Name: Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

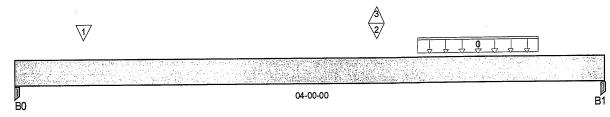
File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B13(

Specifier:

Designer: LBV Company:

Misc:



Total Horizontal	Product	Length	= 04-00-00
------------------	---------	--------	------------

Reaction Summary (Dowr	n / Uplift) (lbs)				
Be aring	Live	De ad	Snow	Wind	
B0, 3-1/2"	732/53	365/0			
B1, 3-1/2"	534/101	248/0			

Load Cummons			Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
0 R1(i2833)	Unf. Lin. (lb/ft)	L 02-08-11	03-06-11 0	0		n/a
1 J5(i3282)	Conc. Pt. (lbs)	L 00-05-11	00-05-11 477	238		n/a
2 -	Conc. Pt. (lbs)	L 02-05-04	02-05-04 770	327		n/a
3 -	Conc. Pt. (lbs)	L 02-05-04	02-05-04 -154			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,321 ft-lbs	25,408 ft-lbs	5.2%	1	02-03-04
End Shear	1,092 lbs	11,571 lbs	9.4%	1	02-11-00
Total Load Defl.	L/999 (0.004")	n/a	n/a	6	02-01-00
Live Load Defl.	L/999 (0.003")	n/a	n/a	8	02-01-00
Max Defl.	0.004"	n/a	n/a	6	02-01-00
Span / Depth	4.5	n/a	n/a		00-00-00

				De mand/ Resistance	Demand/ Resistance	
Bearir	ng Supports	Dim. (L x W)	Demand	Support	Member	Material
B0	Post	3-1/2" x 3-1/2"	1,554 lbs	15.6%	10.4%	Unspecified
B1	Post	3-1/2" x 3-1/2"	1,111 lbs	11.2%	7.4%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria. Calculations assume unbraced length of Top: 01-06-11, Bottom: 01-06-11 Resistance Factor phi has been applied to all presented results per CSA 086. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012 O86.

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9



DWO NO . FAM 30850 17 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B13(i2978)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 9, 2017 12:07:48

BC CALC® Design Report

Build 5033 Job Name: Address:

City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1 mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B1

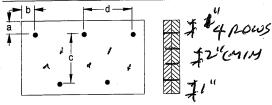
Specifier:

Designer: LBV

Company:

Misc:

#### **Connection Diagram**



a minimum = **(** b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Charles ARDOX SPIRAL

#### Disclosure

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DWG NO . TAM 30850 19 STRUCTURAL COMPONENT ONLY



### Boise Cascade Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i2578)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:12

BC CALC® Design Report

City, Province, Postal Code:BRAMPTON,



Dry i span i No cantilevers i 0/12

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i2578)

Specifier:

Designer: LBV

Company.

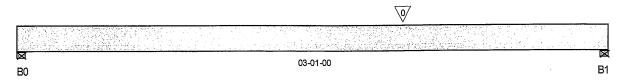
Customer: Code reports:

**Build 5033** 

Job Name: Address:

CCMC 12472-R

Misc:



Total Horizontal Product Length = 03-01-00

Reaction Summary					
Bearing	Live	De ad	Snow	Wind	
B0, 3-1/2"	16 / 0	69 / 0			
B1, 3-1/2"	32 / 0	128/0			

Load Summary			Liv	/e Dead	Snow Wind	Trib.
Tag Description	Load Type	Ref. Start	En d 1.0	0.65	1.00 1.15	
0 Bk1(i2436)	Conc. Pt. (lbs)	L 02-00-00	02-00-00 48	167		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	145 ft-lbs	16,515 ft-lbs	0.9%	0	02-00-00
End Shear	164 lbs	7,521 lbs	2.2%	0	02-00-00
Total Load Defl.	L/999 (0")	n/a	n/a	4	01-07-14
Live Load Defl.	L/999 (0")	n/a	n/a	5	01-07-14
Max Defl.	0"	n/a	n/a	4	01-07-14
Span / Depth	3.3	n/a	n/a		00-00-00

				Resistance	Resistance	
Bear	ing Supports	Dim. (L x W)	Demand	Support	Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	97 lbs	2.3%	1%	Unspecified
B1	Wall/Plate	3-1/2" x 3-1/2"	179 lbs	4.2%	1.8%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 01-07-04, Bottom: 01-07-04.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

CONFORMS TO OBG 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWG NO. TAM 30881-17 STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i2578)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:12

BC CALC® Design Report

**Build 5033** Job Name:

Address: City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i257-

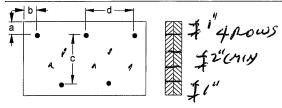
Specifier:

Designer: LBV

Company.

Misc:

#### Connection Diagram



a minimum = 🕊 " b minimum = 3"

Calculated Side Load = 91.1 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: Comment of a Nails 'ARDOX SPIRAL

#### Disclosure

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## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i2576)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:12

BC CALC® Design Report



Build 5033 Job Name:

Address: City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

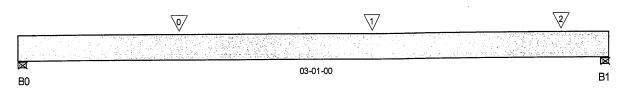
Description: Designs\Flush Beams\Basment\Flush Beams\B1(i2576)

Specifier:

Designer: LBV

Company:

Misc:



Total Horizontal Product Length = 03-01-00

Reaction Summary (Down / Uplift) (lbs)								
Be aring	Live	De ad	Snow	Wind				
B0, 3-1/2"	633/0	423/0						
B1.3-1/2"	960/0	617/0						

Load Summan				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 J2(i2509)	Conc. Pt. (lbs)	L 00-10-00	00-10-00	540	348		n/a
1 J2(i2368)	Conc. Pt. (lbs)	L 01-10-00	01-10-00	548	355		n/a
2 J2(i2426)	Conc. Pt. (lbs)	L 02-10-00	02-10-00	505	307		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,106 ft-lbs	25,408 ft-lbs	4.4%	1	01-10-00
End Shear	1,072 lbs	11,571 lbs	9.3%	1	01-01-00
Total Load Defl.	L/999 (0.002")	n/a	n/a	4	01-06-05
Live Load Defl.	L/999 (0.001")	n/a	n/a	5	01-06-05
Max Defl.	0.002"	n/a	n/a	4	01-06-05
Span / Depth	3.3	n/a	n/a		00-00-00

Beari	ng Supports	Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	1,478 lbs	22.6%	9.9%	Unspecified
B1	Wall/Plate	3-1/2" x 3-1/2"	2,211 lbs	33.8%	14.8%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-09-08, Bottom: 00-09-08.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWO NO. FAM 3005247 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i2576)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:12

BC CALC® Design Report

Build 5033 Job Name: Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1(i257)

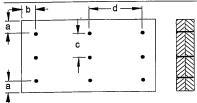
Specifier:

Designer: LBV

Company:

Misc:

#### **Connection Diagram**



a minimum = 2"

c = 2-3/4"

b minimum = 3"

Calculated Side Load = 1,184.4 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d ARDOX SPIRAL

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO . TAM 3005217 STRUCTURAL COMPONENT ONLY



## Boise Cascade Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B7(i2570)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:12

BC CALC® Design Report

**Build 5033** Job Name: Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B7(i2570)

Specifier:

LBV Designer: Company.

Misc:

	2
	04-09-14 B1
В0	ы

Total Horizontal	Product	Length	= 04-09-14
------------------	---------	--------	------------

		Total Tiorizonia			
Reaction Summary (	Down / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 1-3/4"	364/0	214/0			
B1. 5-1/4"	649/0	373/0			

				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
	Unf. Lin. (lb/ft)	1 00-00-00	02-10-04	21	10		n/a
0 FC4 Floor Material	The state of the s	I 02-10-04	04-07-04		14		n/a
1 FC4 Floor Material	Unf. Lin. (lb/ft)		02-11-02		503		n/a
2 B9(i2567)	Conc. Pt. (lbs)	L 02-11-02	02-11-02	504	503		

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2,091 ft-lbs	19,364 ft-lbs	10.8%	1	02-11-02
End Shear	1,356 lbs	7,232 lbs	18.8%	1	03-04-12
Total Load Defl.	L/999 (0.008")	n/a	n/a	4	02-05-01
Live Load Defl.	L/999 (0.005")	n/a	n/a	5	02-05-01
	0.008"	n/a	n/a	4	02-05-01
Max Defl. Span / Depth	4.4	n/a	n/a		00-00-00

			· <u>.</u>	De mand/ Resistance		Material
Bearing Sup	ports	Dim. (L x W)	De mand 813 lbs	Support 32.7%	Member 21.8%	Unspecified
B0 Post B1 Beam		1-3/4" x 1-3/4" 5-1/4" x 1-3/4"	1,440 lbs	29.3%	12.8%	Unspecified

#### Notes

O86.

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\...\B10(i2761)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:12

BC CALC® Design Report

Build 5033 Job Name: Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

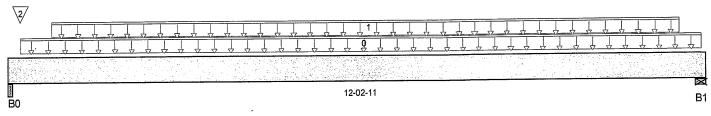
Description: Designs\Flush Beams\Basment\Flush Beams\B10(i2761

Specifier:

Designer: LBV

Company:

Misc:



#### Total Horizontal Product Length = 12-02-11

Reaction Summary (Down	/ Uplift) ( lbs ) Live	De ad	Snow	Wind	
B0, 5-1/4"	4,219/0	2,279 / 0			
B1, 7-7/8"	2,204/0	1,175 / 0			

1.	ad Cumman				Live	Dead	Snow	Wind	Trib.
	oad Summary g Description	Load Type	Ref. Sta	art End	1.00	0.65	1.00	1.15	
	FC4 Floor Material	Unf. Lin. (lb/ft)	L 00-0	2-10 12-01-12	2 7	4	- "		n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L 00-0	9-02 11-09-02	374	186			n/a
2	15(i1134)	Conc. Pt. (lbs)	L 00-0	2-10 00-02-10	2,189	1,194			n/a

CONFORMS TO DBC 2012

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	13,132 ft-lbs	38,727 ft-lbs	33.9%	1	06-03-02
End Shear	4.217 lbs	14,464 lbs	29.2%	1	01-05-02
Total Load Defl.	L/625 (0.216")	0.563"	38.4%	4	06-00-02
Live Load Defl.	L/956 (0.141")	0.375"	37.7%	5	06-00-02
Max Defl.	0.216"	n/a	n/a	4	06-00-02
Span / Depth	11.4	n/a	n/a		00-00-00

Poori	ng Supports	Dim . (L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Beam	5-1/4" x 3-1/2"	9,177 lbs	93.5%	40.9%	Unspecified
B1	Wall/Plate	7-7/8" x 3-1/2"	4,774 lbs	32.5%	14.2%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

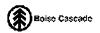
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

DWG NO . TAN 3005417 STRUCTURAL COMPONENT ONLY



BC CALC® Design Report

### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\...\B10(i2761)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:12

**Build 5033** 

Job Name:

Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B10(i27)

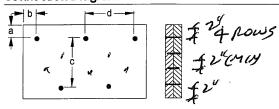
Specifier:

Designer:

Company.

Misc:

#### Connection Diagram



a minimum = 2"

c = 7-7/8"

d= 100 6 b minimum = 3"

Calculated Side Load = 715.2 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d ARDOX SPIRAL

#### Disclosure

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OWE NO . TAN 30854 17 STRUCTURAL COMPONENT ONLY



## Boise Cascade Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B6(i2749)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:12

BC CALC® Design Report



**Build 5033** Job Name: Address:

City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B6(i2749)

Specifier:

Designer: LBV

Company:

Misc:

	<b>♥</b>	<u> </u>	2	3
B0			06-00-00	B1

Total Horizontal Product Length = 06-00-00

Reaction Summary (Down / Uplift) ( lbs )								
Bearing	Live	De ad	Snow	Wind				
B0, 3-1/2"	242/0	138/0	-					
R1	233/0	134/0						

Load Summary				Live	Dead	Snow Wind	Trib.
Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 J6(i2554)	Conc. Pt. (lbs)	L 01-00-08	01-00-08	109	54		n/a
1 J6(i2542)	Conc. Pt. (lbs)	L 02-03-08	02-03-08	123	61		n/a
2 J6(i2551)	Conc. Pt. (lbs)	L 03-07-08	03-07-08	127	63		n/a
3 J6(i2358)	Conc. Pt. (lbs)	L 04-11-08	04-11-08	116	58		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	825 ft-lbs	19,364 ft-lbs	4.3%	1	03-07-08
End Shear	480 lbs	7,232 lbs	6.6%	1	04-10-02
Total Load Defl.	L/999 (0.007")	n/a	n/a	4	03-00-08
Live Load Defl.	L/999 (0.004")	n/a	n/a	5	03-00-08
Max Defl.	0.007"	n/a	n/a	4	03-00-08
Span / Depth	5.7	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Bear	ring Supports	Dim.(L x W)	De man d	Support	Member	Material
B0	Post	3-1/2" x 1-3/4"	535 lbs	10.8%	7.2%	Unspecified
B1	Hanger	2" x 1-3/4"	517 lbs	n/a	12.1%	HUS1.81/10

## Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBG 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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## Boise Cascade Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B9(i2567)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:12

BC CALC® Design Report

Build 5033 Job Name:

Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

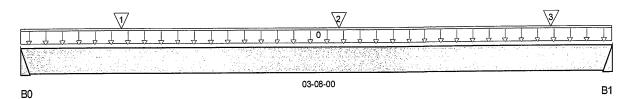
Description: Designs\Flush Beams\Basment\Flush Beams\B9(i2567)

Specifier:

Designer: LBV

Company:

Misc:



Total Horizontal Product Length = 03-08-00

	(Down / Uplift) (lbs)	Do not	Cm =	Wind	
Be aring	Live	De ad	Snow		 
B0	830/0	429/0			
B1	906/0	505/0			

Load Summary		Live		Live	Dead	Snow Wind	Trib.
Tag Description	Load Type	Load Type Ref. Start En			0.65	1.00 1.15	
0 User Load	Unf. Lin. (lb/ft)	L 00-00-00	03-08-00	240	120		n/a
1 J2(i2351)	Conc. Pt. (lbs)	L 00-07-08	00-07-08	256	128		n/a
2 J2(i2418)	Conc. Pt. (lbs)	L 01-11-08	01-11-08	325	162		n/a
3 J2(i2562)	Conc. Pt. (lbs)	L 03-03-08	03-03-08	275	182		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,587 ft-lbs	19,364 ft-lbs	8.2%	1	01-11-08
End Shear	890 lbs	7,232 lbs	12.3%	1	01-01-14
Total Load Defl.	L/999 (0.005")	n/a	n/a	4	01-10-05
Live Load Defl.	L/999 (0.003")	n/a	n/a	5	01-10-05
Max Defl.	0.005"	n/a	n/a	4	01-10-05
Span / Depth	3.5	n/a	n/a		00-00-00

				De mand/ Resistance	Demand/ Resistance	
Bearing Supports		Dim.(LxW)	Demand	Support	Member	Material
B0	Hanger	2" x 1-3/4"	1,781 lbs	n/a	41.7%	HUS1.81/10
B1	Hanger	2" x 1-3/4"	1,991 lbs	n/a	46.6%	HUS1.81/10

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBG 2012

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain installation Guide or ask questions, please call 1-800-964-6999 before installation.

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## Boise Cascade Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i2615)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:12

BC CALC® Design Report



Build 5033 Job Name:

Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

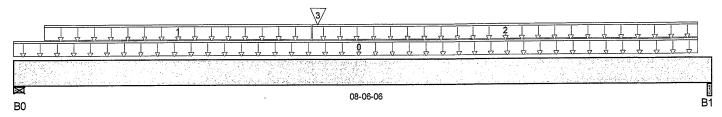
Description: Designs\Flush Beams\Basment\Flush Beams\B5(i2615)

Specifier:

Designer: LBV

Company:

Misc:



Total Horizontal Product Length = 08-06-06

Reaction Summary (Down / Uplift) ( lbs )								
Be aring	Live	De ad	Snow	Wind				
B0, 4-3/8"	802/0	436/0						
B1, 4-5/8"	361/0	214/0						

Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re 1	. Start	En d	1.00	0.65	1.00	1.15	
0	FC4 Floor Material	Unf. Lin. (lb/ft)	L.	00-00-00	08-04-06	4	2			n/a
1	Us er Load	Unf. Lin. (lb/ft)	L	00-04-06	03-07-06	240	120			n/a
2	FC4 Floor Material	Unf. Lin. (lb/ft)	L	03-07-06	08-04-06	22	11			n/a
3	B6 (i2749)	Conc. Pt. (lbs)	L	03-08-04	03-08-04	230	132			n/a

	Factore d	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	2,950 ft-lbs	19,364 ft-lbs	15.2%	1	03-08-04
End Shear	1,213 lbs	7,232 lbs	16.8%	1	01-04-04
Total Load Defl.	L/999 (0.043")	n/a	n/a	4	04-00-03
Live Load Defl.	L/999 (0.028")	n/a	n/a	5	04-00-03
Max Defl.	0.043"	n/a	n/a	4	04-00-03
Span / Depth	8	n/a	n/a		00-00-00

Bearing Supports Dim. (L x W)			De man d	De mand/ Re sistance Support	Demand/ Resistance Member	Material	
B0	Wall/Plate	4-3/8" x 1-3/4"	1,749 lbs	42.8%	18.7%	Unspecified	
B1	Beam	4-5/8" x 1-3/4"	809 lbs	18.7%	8.2%	Unspecified	

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBG 2012 O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w.ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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PROFESSIONAL

S. KATSOULAKOS

ROVINCE OF

DWG NO . TAM 3085219 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\...\B8(i2568)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:13

**Build 5033** 

Job Name:

Address:

City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

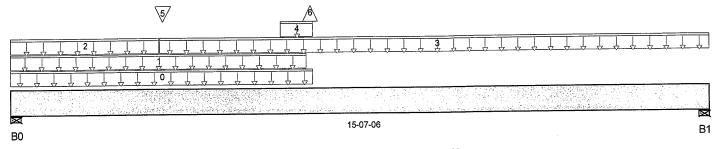
Description: Designs\Flush Beams\Basment\Flush Beams\B8(i2568)

Specifier:

Designer: LBV

Company:

Misc:



Total Horizontal Product Length = 15-07-06

Reaction Summary (D	Down / Uplift) ( lbs ) Live	Dead	Snow	Wind	
B0, 3-1/2"	1,554 / 26	1,326 / 0			
B1. 4-3/8"	840/20	633/0		* 4	

	l C					Live	Dead	Snow	wina	i rib.
	ad Summary Description	Load Type	Ref. St	tart E	in d	1.00	0.65	1.00	1.15	
0	18(i1137)	Unf. Lin. (lb/ft)	L 00-	00-00 0	6-08-08		81			n/a
1	18(i1137)	Unf. Lin. (lb/ft)	L 00-	-00-00 0	6-06-12	14	10			n/a
2	FC4 Floor Material	Unf. Lin. (lb/ft)	L 00-	-00-00 0	3-03-08	14	7			n/a
3	FC4 Floor Material	Unf. Lin. (lb/ft)	L 03-	03-08 1	5-07-06	27	13			n/a
4	18(i1137)	Unf. Lin. (lb/ft)	L 05-	-11-10 0	80-80-6	1,475	734			n/a
5	B9(i2567)	Conc. Pt. (lbs)	L 03-	-04-06 0	3-04-06	832	431			n/a
6	18(i1137)	Conc. Pt. (lbs)	L 06-	-07-10 0	6-07-10	-46				n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	15,110 ft-lbs	38,727 ft-lbs	39%	1	06-03-03
End Shear	3.758 lbs	14,464 lbs	26%	1	01-03-06
Total Load Defl.	L/452 (0.401")	0.754"	53.1%	6	07-02-03
Live Load Defl.	L/784 (0.231")	0.503"	45.9%	8	07-02-03
Max Defl.	0.401"	n/a	n/a	6	07-02-03
Span / Depth	15.2	n/a	n/a		00-00-00

				Demand/ Resistance	De mand/ Resistance		
Bearing Supports		Dim . (L x W)	Demand	Support	Member	Material	
B0	Wall/Plate	3-1/2" x 3-1/2"	3,989 lbs	61%	26.7%	Unspecified	
B1	Wall/Plate	4-3/8" x 3-1/2"	2,052 lbs	25.1%	11%	Unspecified	

Notes



DW8 NO . TAM 3085417 STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\...\B8(i2568)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:13

BC CALC® Design Report

\*

Build 5033

Job Name:

Address:

City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B8(i256

Specifier:

Designer: LBV

Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

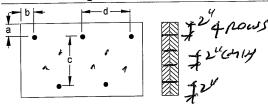
O86.

CONFORMS TU OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### **Connection Diagram**



a minimum = 2" c = 7-7/8" d = 6

Calculated Side Load = 114.4 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 2 1/44h. They make an Nails

312" ARDOX SPIRAL

#### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™,
ALLJOIST®, BC RIM BOARD™, BCI®,
BOISE GLULAM™, SIMPLE FRAMING
SYSTEM®, VERSA-LAM®, VERSA-RIM
PLUS®, VERSA-RIM®,
VERSA-STRAND®, VERSA-STUD® are
trademarks of Boise Cascade Wood
Products L.L.C.



DWG NO .TAM30850-17 STRUGTURAL COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\...\B16(i2566)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:13

BC CALC® Design Report

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Build 5033

Job Name: Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

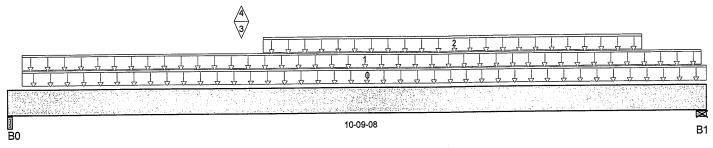
File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B16(i2566

Specifier:

Designer: LBV Company.

Misc:



#### Total Horizontal Product Length = 10-09-08

Reaction Summary (Do	own / Uplift) (lbs) Live	Dead	Snow	Wind	
B0, 5-1/4"	631/31	539/0			
B1. 4-3/8"	374/15	442/0			

	ad Cummon				Live	Dead	Snow Wind	Trib.
	oad Summary g Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0	FC4 Floor Material	Unf. Lin. (lb/ft)	L 00-02-10	10-09-08	22	11		n/a
1	FC4 Floor Material	Unf. Lin. (lb/ft)	L 00-02-10	10-08-09	6	3		n/a
2	FC4 Floor Material	Unf. Lin. (lb/ft)	L 03-10-13	09-09-08		42		n/a
3	-	Conc. Pt. (lbs)	L 03-06-14	03-06-14	706	457		n/a
4	_	Conc. Pt. (lbs)	L 03-06-14	03-06-14	<del>-4</del> 6			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Dem and	Resistance	Resistance	Case	
Pos. Moment	4,742 ft-lbs	38,727 ft-lbs	12.2%	1	03-06-13
End Shear	1.526 lbs	14,464 lbs	10.6%	1	01-05-02
Total Load Defl.	L/999 (0.057")	n/a	n/a	6	05-01-06
Live Load Defl.	L/999 (0.029")	n/a	n/a	8	05-01-06
Max Defl.	0.057"	n/a	n/a	6	05-01-06
Span / Depth	10.2	n/a	n/a		00-00-00

Bearing Supports		Dim.(L x W)	De man d	De man d/ Re sistance Support	De mand/ Resistance Member	Material
B0	Beam	5-1/4" x 3-1/2"	1,621 lbs	16.5%	7.2%	Unspecified
B1	Wall/Plate	4-3/8" x 3-1/2"	1,113 lbs	13.6%	6%	Unspecified

Notes

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DWG NO. TAM 30059-17 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\...\B16(i2566)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:13

BC CALC® Design Report

**Build 5033** 

Job Name: Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B16(i25)

Specifier:

Designer: LBV

Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

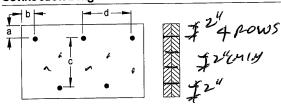
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBG 2012

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### **Connection Diagram**



a minimum = 2" c = 7-7/8"

b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d A. ... Nails

31/2" ARDOX SPIRAL

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\...\B17(i2565)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:13

BC CALC® Design Report

\*

Build 5033 Job Name:

Address: City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

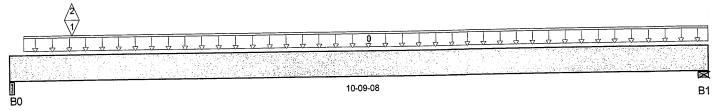
Description: Designs\Flush Beams\Basment\Flush Beams\B17(i2565

Specifier:

Designer: LBV

Company:

Misc:



Total Horizontal Product Length = 10-09-08

Reaction Summary (Down	/ Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5-1/4"	656/100	444/0			
B1, 4-3/8"	174/6	154/0			

				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
		1 00-02-10	10-09-08	27	13		n/a
<ol><li>FC4 Floor Material</li></ol>	Unf. Lin. (lb/ft)				328		n/a
1 PBO10(i1545)	Conc. Pt. (lbs)	L 00-11-06	00-11-06	548	320		
2 PBO10(i1545)	Conc. Pt. (lbs)	L 00-11-06	00-11-06	-106			n/a
Z FBO10(11343)	00110.1 11 (120)						

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location	
Pos. Moment	1,302 ft-lbs	38,727 ft-lbs	3.4%	1	04-05-03	
End Shear	851 lbs	14,464 lbs	5.9%	1	01-05-02	
Total Load Defl.	L/999 (0.018")	n/a	n/a	6	05-02-03	
Live Load Defl.	L/999 (0.01")	n/a	n/a	8	05-02-03	
Max Defl.	0.018"	n/a	n/a	6	05-02-03	
Span / Depth	10.2	n/a	n/a		00-00-00	

		- (/ NA)	Do woond	De man d/ Re sistance Support	Resistance Member	Material
Be arin	i <b>g Supports</b> Beam	Dim . (L x W) 5-1/4" x 3-1/2"	Demand 1.540 lbs	15.7%	6.9%	Unspecified
В0 В1	Wall/Plate	4-3/8" x 3-1/2"	454 lbs	5.6%	2.4%	Unspecified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBG 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWG NO .TAM 30060-17 STRUCTURAL COMPONENT ONLY



## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\...\B17(i2565)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:13

BC CALC® Design Report

**Build 5033** Job Name: Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B17(i25)

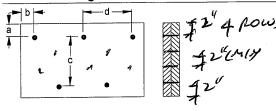
Specifier:

Designer:

Company:

Misc:

#### Connection Diagram



a minimum = 2" b minimum = 3"

c = 7-7/8" d = 🐠

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d ( Nails

312" ARDOX SPIRAL

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DW6 NO. TAM 3086217 STRUCTURAL COMPONERT ORLY



## Boiso Cascado Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B4(i2738)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:13

Build 5033

Job Name:

Address: City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B4(i2738)

Specifier:

LBV Designer:

Company:

Misc:

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	<u> </u>
04.00.10	
04-00-10	B1
0	

#### Total Horizontal Product Length = 04-00-10

Reaction Summary (D	own / Uplift) ( lbs ) Live	De ad	Snow	Wind	
B0, 3-1/2"	440/0	232/0			
B1, 4-3/8"	437/0	231/0			·

			Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
0 FC4 Floor Material	Unf. Lin. (lb/ft)	L 00-03-08	04-00-10 19	10		n/a
1 User Load	Unf. Lin. (lb/ft)	L 00-03-08	03-07-12 240	120		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
	863 ft-lbs	19,364 ft-lbs	4.5%	1	01-11-14
Pos. Moment End Shear	879 lbs	7,232 lbs	12.2%	1	02-08-06
Total Load Defl.	L/999 (0.003")	n/a	n/a	4	01-11-14
Live Load Defl.	L/999 (0.002")	n/a	n/a	5	01-11-14
Max Defl.	0.003"	n/a	n/a	4	01-11-14
Span / Depth	3.6	n/a	n/a		00-00-00

			De man d/ Re sistance			Material	
Bearing Supports		Dim. (L x W)	De man d	Support	Member		
B0	Wall/Plate	3-1/2" x 1-3/4"	950 lbs	29%	12.7%	Unspecified	
B1	Wall/Plate	4-3/8" x 1-3/4"	944 lbs	23.1%	10.1%	Unspecified	

#### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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# Boiso Coscodo Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i2608)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:13

**Build 5033** 

Job Name:

Address: City, Province, Postal Code:BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

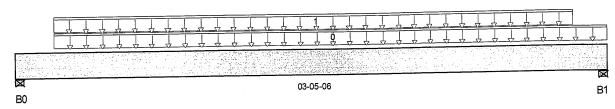
Description: Designs\Flush Beams\Basment\Flush Beams\B3(i2608)

Specifier:

LBV Designer:

Company:

Misc:



Total Horizontal Product Length = 03-05-06

Reaction Summary (	Down / Uplift) (lbs) Live	De ad	Snow	Wind	 	
B0, 2-3/4"	44 / 0	123/0				
B1, 2-3/8"	50 / 0	126/0				

				Live	Dead	Snow	wina	IIID.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00	1.15	
		L 00-02-12	03-05-06	29	15			n/a
0 FC4 Floor Material	• •				60			n/a
1 User Load	Unf. Lin. (lb/ft)	L 00-02-12	00 00 00					

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	139 ft-lbs	12,586 ft-lbs	1.1%	0	01-08-14
End Shear	59 lbs	4,701 lbs	1.2%	0	01-02-10
Total Load Defl.	L/999 (0")	n/a	n/a	4	01-08-14
Live Load Defl.	L/999 (0")	n/a	n/a	5	01-08-14
Max Defl.	0"	n/a	n/a	4	01-08-14
Span / Depth	3.2	n/a	n/a		00-00-00

Dessina	Supports	Dim . (L x W)	Demand	De man d/ Re sistance Support	Demand/ Resistance Member	Material
BO W	Supports lall/Plate lall/Plate	2-3/4" x 1-3/4" 2-3/8" x 1-3/4"	172 lbs 176 lbs	10.3% 12.2%	4.5% 5.3%	Un specified Un specified

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

#### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14(i2571)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:04

BC CALC® Design Report

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Build 5033

Job Name: Address:

City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

File Name: AUBURN 4 EL 1.mmdl

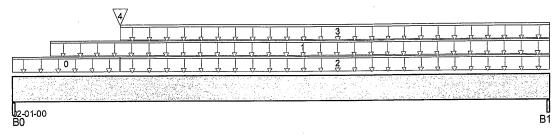
Description: Designs\Flush Beams\1st Floor\Flush Beams\B14(i2571)

Specifier:

Designer: LBV

Company:

Misc:



Total Horizontal Product Length = 02-01-00

Re	action Summary (Dow	n / Uplift) (lbs)								
	aring	Live	De ad	S	now	Wind				
B0,	5"	52 / 0	120/0	8	1/0					
В1,	6-1/2"	82 / 0	172/0	1	59/0					
Lo	ad Summary					Live	Dead	Snow	Wind	Trib.
	Description	Load Type	Ref	. Start	En d	1.00	0.65	1.00	1.15	
0	FC5 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	00-05-00	5 .				n/a
1	WALL	Unf. Lin. (lb/ft)	L	00-01-12	02-01-00		80			n/a
2	ROOF	Unf. Lin. (lb/ft)	L	00-05-00	02-01-00	66	60	144		n/a
3	FC5 Floor Material	Unf. Lin. (lb/ft)	L	00-05-00	02-01-00	8	4			n/a
4	FC5 Floor Material	Conc. Pt. (lbs)	L	00-05-00	00-05-00	9 .	4			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location	
Pos. Moment	87 ft-lbs	38,727 ft-lbs	0.2%	13	00-11-12	
End Shear	191 lbs	14,464 lbs	1.3%	13	01-04-14	
Span / Depth	1.3	n/a	n/a		00-00-00	

Beari	ing Supports	Dim . (L x W)	De mand	Resistance Support	Resistance Member	Material
B0	Beam	5" x 3-1/2"	298 lbs	3.2%	1.4%	Unspecified
B1	Beam	6-1/2" x 3-1/2"	495 lbs	4.1%	1.8%	Unspecified

#### Notes

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

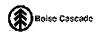
Unbalanced snow loads determined from building geometry were used in selected products verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWO NO . TAM 3096317 STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B14(i2571)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

June 7, 2017 15:44:04

BC CALC® Design Report

File Name: AUBURN 4 EL 1.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B14(i257

Specifier:

Designer:

Company:

Misc:

**Build 5033** 

Job Name:

Address:

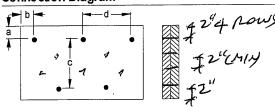
City, Province, Postal Code: BRAMPTON,

Customer:

Code reports:

CCMC 12472-R

### **Connection Diagram**



a minimum = 2"

c = 7-7/8"

b minimum = 3" d = 223

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Sirker Nails
3½ ARDOX SPIRAL

#### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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