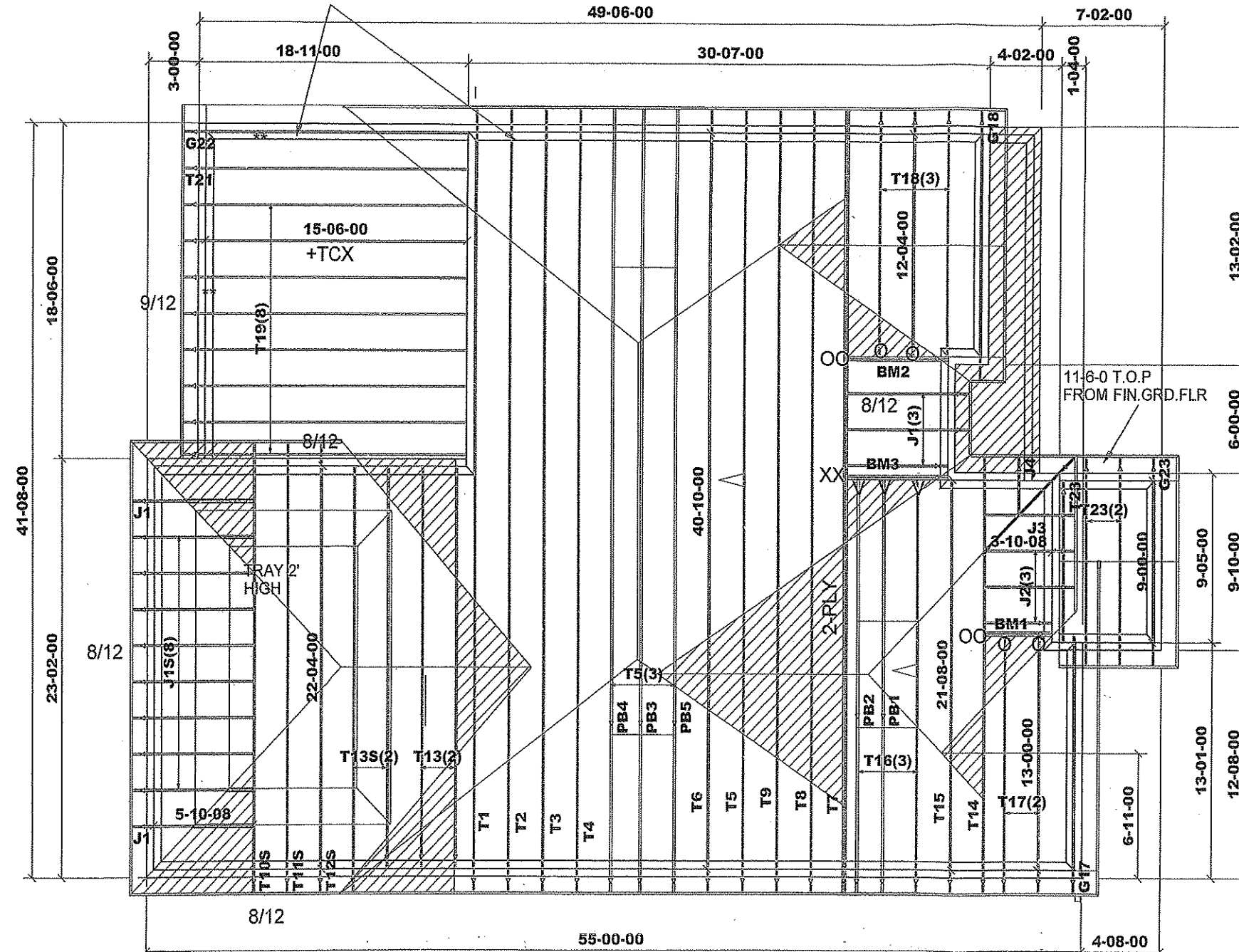


**12-1-0 T.O.P
11'-0 RAISED
FASCIA
8-1-0 T.O.P SPLIT

12/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-170284



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL
ROOF FRAMING TO
CONFORM TO PART9 OF
THE OBC.LATEST
EDITION ROOF RAFTERS
THAT MEET OR CROSS
OVER TRUSSES ARE TO
BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF
VERTICAL POST TO THE
TRUSS UNDER AT EACH
CROSS POINT.POSTS
LONGER THAN 6' TO BE
LATERALLY BRACED SO
THAT THE DISTANCE
BETWEEN END POINTS
AND BETWEEN ROWS
OF BRACING DOES NOT
EXCEED 6'.

DESIGN CONFORMS
WITH THE RELEVANT
SECTION OF THE
LATEST EDITION OF
O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 1.3 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL
FRAMING

HARDWARE
LUS24(O)
LUS24-2(OO)
LJS28DS(V)
LUS26-2(VV)
HGUS26-2(XX)

DATE: 5/24/17
BCIN:26064; FIRM:BCIN #29991
ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED
ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING
DESIGNER
ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING
DESIGNER PRIOR TO TRUSS INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO
NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS WORK IS DELEGATED TO A
QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT.
ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE
BEAMS SPECIFIED BY THE BUILDING DESIGNER AND/OR
PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS
PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE
ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION.

LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH
INDIVIDUAL LOT.

DWG #TAM THROUGH DWG #TAM, INCLUSIVE, DATED ;

STRUCTURAL COMPONENTS ONLY); THIS LAYOUT MUST BE READ TOGETHER WITH
REFERENCED TRUSS COMPONENT DRAWINGS ANDSEALED HARDWARE DRAWINGS.
HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY.
REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING
REQUIREMENTS.
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2 X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS
SPECIFIED ON REFERENCED SEALED TRUSS COMPONENT DRAWINGS USING 2 - 3-1/4"
COMMON WIRE NAILS PER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF "X-BRACING"
ALONG THESE SAME WEB MEMBERS AT LOCATIONS INDICATED (") USING THE SAME
NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
PROVIDE 2 X 4 #2 SPF T-BRACE ALONG FULL LENGTH OF LATERALLY BRACED WEB
MEMBERS OF 1 PLY TRUSSES COMPONENTS REQUIRING "T-BRACE" USING 1 ROW OF 3-
1/4" COMMON WIRE NAILS AT 6" O/C. SIMILARLY PROVIDE 2 X 6 BRACE WITH 2 ROWS OF
NAILING FOR 2 PLY TRUSS COMPONENTS AND 2 X 8 BRACE WITH 3 ROWS OF NAILING
FOR 3 PLY TRUSS COMPONENTS.

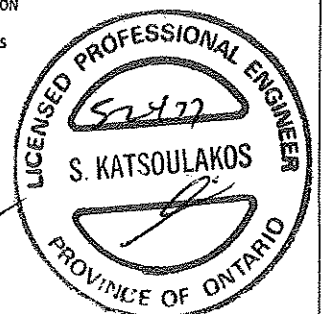
(1) 2 X 6 #2 SPF ON FLAT OVER PLYWOOD SHEATHING.
(2) 2 X 6 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS
COMPONENTS AND AT LOCATIONS MARKED BY "X".
(3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O/C WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X
4 VERTICAL SUPPORTS TO TRUSS BASE BELOW WHERE NECESSARY.

ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE
TRUSS COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH
STUDS AT24" O/C.

ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE
(CURRENT EDITION).

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN
WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION
3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED
AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES
AND/OR CATEGORIES.
REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

DWG #TAM 2574077
BCIN: 26064
FIRM: 29991
SEALED STRUCTURAL
COMPONENTS ONLY



MAY 24, 2017
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS
COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:44080/281216)
DWG #TAM25734-17 THROUGH DWG #TAM25736-17, INCLUSIVE, AND ALL DATED 5-24-17;
DWG #TAM19455-17 THROUGH DWG #TAM19475-17, INCLUSIVE, AND ALL DATED 4-26-17;
DWG #TAM19480-17 THROUGH DWG #TAM19487-17, INCLUSIVE, AND ALL DATED 4-26-17;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

110,600

Job Track: 44080	Builder: GREENYORK HOMES /	Location: BRAMPTON	Model: YORK 1 / A	Elevation: C:\MITEK\CA742AJ\JOBS\B1300036006
Layout ID: 281216	Project: DEGREY DRIVE	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.		
Plan Log: 91851	Date: 4/26/2017	Designer: JG	MiTek ver 7.5.0	

MAY 24, 2017
 THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
 (LAYOUT: 44080/281218)
 DWG #TAM25737-17 THROUGH DWG #TAM25739-17, INCLUSIVE, AND ALL DATED 5-24-17;
 DWG #TAM19488-17 THROUGH DWG #TAM19508-17, INCLUSIVE, AND ALL DATED 4-26-17;
 DWG #TAM19512-17 THROUGH DWG #TAM19517-17, INCLUSIVE, AND ALL DATED 4-26-17;
 DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
 (STRUCTURAL COMPONENTS ONLY)

**12-1-0 T.O.P
 11'-0" RAISED FASCIA
 8-1-0 T.O.P SPLIT

14/12 ROOF PITCH
 BEAMS(BM): 2-2X10
 UNLESS NOTED

T-170284

12" FINISH O.H
 R.T.M.C
 3" STUCCO
 2X6 EXTERIOR WALLS
 ASPHALT SHINGLES
 2X6 FASCIA BOARD

ALL CONVENTIONAL
 ROOF FRAMING TO
 CONFORM TO PART 9 OF
 THE O.B.C. LATEST
 EDITION ROOF RAFTERS
 THAT MEET OR CROSS
 OVER TRUSSES ARE TO
 BE 2"x4"SPF@24"o.c.
 WITH A 2"x4"SPF
 VERTICAL POST TO THE
 TRUSS UNDER AT EACH
 CROSS POINT. POSTS
 LONGER THAN 6' TO BE
 Laterally BRACED SO
 THAT THE DISTANCE
 BETWEEN END POINTS
 AND BETWEEN ROWS
 OF BRACING DOES NOT
 EXCEED 6'.

DESIGN CONFORMS
 WITH THE RELEVANT
 SECTION OF THE
 LATEST EDITION OF
 O.B.C. PART 9

DESIGN LOADS:
 GROUND SNOW LOAD
 $S_s = 1.3 \text{ kPa}$
 TC DEAD 3 PSF
 BC LIVE 10.5 PSF
 BC DEAD 7 PSF

DENOTES
 CONVENTIONAL
 FRAMING

HARDWARE
 LUS24(O)
 LJS28DS(V)
 LUS26-2(VV)
 HGUS26-2(XX)

MOVE WINDOW TO CENTRE
 LINE @ 6'-7"

DATE: 5/24/17
 BCIN: 26064; FIRM: BCIN #29991
 ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED
 ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING
 DESIGNER
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LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH
 INDIVIDUAL LOT.

DWG #TAM THROUGH DWG #TAM, INCLUSIVE, DATED ;

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 HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY.
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 (IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

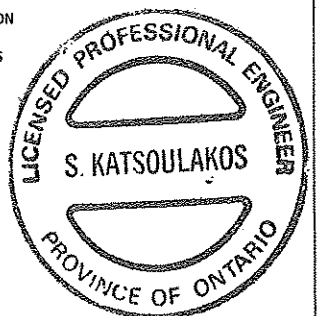
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 ALONG THESE SAME WEB MEMBERS AT LOCATIONS INDICATED (*) USING THE SAME
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 (3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O.C. WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X
 4 VERTICAL SUPPORTS TO TRUSS BASE BELOW WHERE NECESSARY).

ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE
 TRUSS COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH
 STUDS AT 24" O.C.

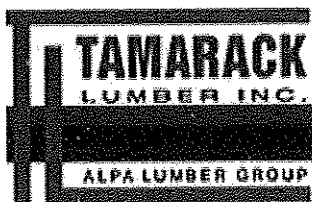
ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE
 (CURRENT EDITION).

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN
 WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION
 3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED
 AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES
 AND/OR CATEGORIES.
 REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.



DWG #TAM 2574177
 BCIN: 26064
 FIRM: 29991
 SEALED STRUCTURAL
 COMPONENTS ONLY

Job Track: 44080		Builder: GREENYORK HOMES /		Location: BRAMPTON		Model: YORK 1 / B		Elevation: 610000000000	
Layout ID: 281218		Project: DEGREY DRIVE		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.					
Plan Log: 91851		Date: 4/26/2017		Designer: JG		MITek ver 7.5.0			



Delivery Shiplist

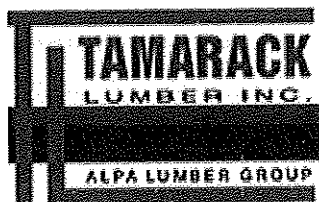
DATE	05/19/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281216 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 1 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP	12.00 0.00	40-10-00	06-03-14	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	194.66 118.33		
	1	T2 HIP	12.00 0.00	40-10-00	07-09-14	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	205.85 125.00		
	1	T3 HIP	12.00 0.00	40-10-00	09-03-14	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	239.36 144.50		
	1	T4 HIP	12.00 0.00	40-10-00	10-09-14	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	254.51 155.67		
	4	T5 HIP	12.00 0.00	40-10-00	09-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	957.28 580.68		
	1	T6 HIP	12.00 0.00	40-10-00	10-07-13	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	240.18 147.00		
	1 2 Ply	T7 HIP GIRDER	12.00 0.00	40-10-00	05-03-13	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	453.44 274.00		
	1	T8 HIP	12.00 0.00	40-10-00	06-07-13	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	197.02 118.33		
	1	T9 HIP	12.00 0.00	40-10-00	07-11-13	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	207.12 125.00		
	1	T10S HIP GIRDER	8.00 12.00	22-04-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	98.15 65.83		
	1	T11S HIP	8.00 12.00	22-04-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	95.75 64.67		
	1	T12S HIP	8.00 12.00	22-04-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	100.52 66.67		
	2	T13 COMMON	8.00 0.00	22-04-00	08-10-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-04-13	185.34 112.66		
	2	T13S ROOF	8.00 12.00	22-04-00	08-10-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	188.94 125.68		
	1	T14 HIP GIRDER	12.00 0.00	21-08-00	05-09-00	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	106.96 65.17		
	1	T15 HIP	12.00 0.00	21-08-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	103.96 65.67		
	3	T16 PIGGYBACK	12.00 0.00	21-08-00	09-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	338.97 213.99		
	2	T17 COMMON	12.00 0.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	135.72 86.66		



Delivery Shiplist

DATE	05/19/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281216 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 1 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	G17 COMMON	12.00 0.00	13-00-00	08-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	69.94 44.33		
	3	T18 COMMON	12.00 0.00	12-04-00	08-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	194.97 126.00		
	1	G18 COMMON	12.00 0.00	12-04-00	08-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	66.31 43.00		
	8	T19 MONOPITCH	9.00 0.00	15-06-00	14-04-14	2 X 4	2 X 4	01-03-08 00-00-00	02-05-04 14-04-14	803.44 485.36		
	1	T21 HALF HIP	9.00 0.00	15-06-00	11-11-08	2 X 4	2 X 4	01-03-08 00-00-00	02-05-04 11-11-08	107.49 66.67		
	1	G22 HALF HIP	9.00 0.00	15-06-00	09-11-08	2 X 4	2 X 4	01-03-08 00-00-00	02-05-04 09-11-08	102.42 64.67		
	3	T23 COMMON	12.00 0.00	09-00-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	134.01 86.01		
	1	G23 COMMON	12.00 0.00	09-00-00	06-04-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	47.90 30.67		
	1	PB1 PIGGYBACK	12.00 0.00	04-09-06	01-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	16.17 10.67		
	1	PB2 PIGGYBACK	12.00 0.00	04-09-06	02-09-14	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	14.99 10.00		
	1	PB3 PIGGYBACK	12.00 0.00	24-09-12	03-10-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	88.44 55.83		
	1	PB4 PIGGYBACK	12.00 0.00	24-09-12	02-10-07	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	84.70 53.83		
	1	PB5 PIGGYBACK	12.00 0.00	24-09-12	02-06-06	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	83.55 50.50		
	5	J1 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	112.20 68.35		
	8	J1S JACK-OPEN	8.00 12.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-03-13	208.00 145.36		
	3	J2 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	53.70 33.99		
	1	J3 JACK-OPEN	12.00 0.00	03-10-08	03-07-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-10-08 00-03-08	14.54 9.33		
	1	J4 JACK-OPEN	12.00 0.00	01-10-08	03-07-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-10-08 00-03-08	10.86 7.50		

TOTAL # TRUSS= 69.00

TOTAL BFT OF ALL TRUSSES=

4047.58 BFT. TOTAL WEIGHT OF ALL TRUSSES= 6517.36 LBS.



Delivery Shiplist

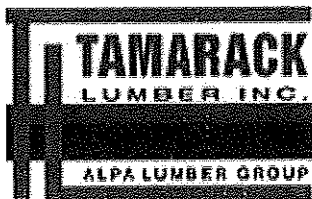
DATE	05/19/17
SALES REP	Rick

JOB TRACK: 44080	LAYOUT ID: 281216	LOCATION: BRAMPTON
BUILDER: GREENYORK HOMES/DEGREY DRIVE.	SUB-BUILDER:	
MODEL: YORK 1	ELEVATION: A	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
4	Hangers	LUS24	
2	Hangers	LUS24-2	

TOTAL # ITEMS= 10.00



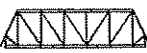
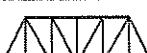
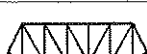
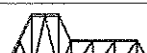
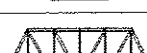
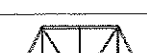
Delivery Shiplist

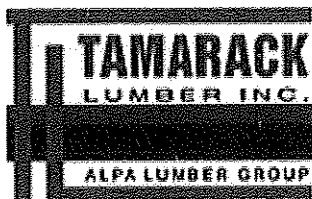
DATE	05/19/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281218 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 1 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T30 HIP	14.00 0.00	40-10-00	06-03-08	2 X 4	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	202.12 123.83		
	1	T31 HIP	14.00 0.00	40-10-00	07-09-08	2 X 4	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	219.38 134.33		
	1	T32 HIP	14.00 0.00	40-10-00	09-03-08	2 X 4	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	240.54 144.17		
	1	T33 HIP	14.00 0.00	40-10-00	10-09-08	2 X 4	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	271.44 163.67		
	3	T34 PIGGYBACK	14.00 0.00	40-10-00	09-03-06	2 X 4	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	721.35 432.51		
	1	T35 HIP	14.00 0.00	40-10-00	10-05-06	2 X 4	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	251.88 152.67		
	1	T37 HIP	14.00 0.00	40-10-00	08-01-06	2 X 4	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	222.06 135.67		
	1	T38 HIP	14.00 0.00	40-10-00	06-11-06	2 X 4	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	208.00 127.50		
	1	T39 PIGGYBACK	14.00 0.00	40-10-00	10-00-00	2 X 4	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	223.37 138.67		
	1 2 Ply	T40 ROOF	14.00 0.00	40-10-00	11-02-04	2 X 6	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	503.16 304.34		
	1	T41 ROOF	14.00 0.00	34-10-00	09-09-09	2 X 6	2 X 6	01-01-08 00-00-00	01-11-00 01-11-00	218.82 135.00		
	1	T42 HIP GIRDER	14.00 0.00	12-08-00	05-03-04	2 X 4	2 X 4	01-01-08 01-01-08	01-11-00 01-11-00	66.11 41.83		
	1	T43 HIP	14.00 0.00	12-08-00	07-07-04	2 X 4	2 X 4	01-01-08 01-01-08	01-11-00 01-11-00	68.40 43.67		
	1	T44 HIP GIRDER	14.00 0.00	21-08-00	04-08-12	2 X 4	2 X 6	01-01-08 01-01-08	01-11-00 01-11-00	112.84 69.00		
	1	T45 HIP	14.00 0.00	21-08-00	06-04-12	2 X 4	2 X 4	01-01-08 01-01-08	01-11-00 01-11-00	97.20 59.33		
	1	T46 HIP	14.00 0.00	21-08-00	08-00-12	2 X 4	2 X 4	01-01-08 01-01-08	01-11-00 01-11-00	105.67 68.00		
	1	T47S HIP GIRDER	8.00 12.00	22-04-00	05-02-08	2 X 4	2 X 6	01-01-08 01-01-08	01-03-08 01-03-08	96.50 65.83		
	1	T48S HIP	8.00 12.00	22-04-00	06-06-08	2 X 4	2 X 4	01-01-08 01-01-08	01-03-08 01-03-08	94.27 63.67		



Delivery Shiplist

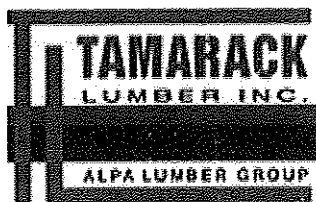
DATE	05/19/17
SALES REP	Rick

JOB TRACK:44080 LAYOUT ID: 281218 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 1 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T49S HIP	8.00 12.00	22-04-00	07-10-08	2 X 4	2 X 4	01-01-08 01-01-08	01-03-08 01-03-08	99.06 66.67		
	2	T50 COMMON	8.00 0.00	22-04-00	08-08-13	2 X 4	2 X 4	00-00-00 00-00-00	01-03-08 01-03-08	183.94 112.66		
	2	T50S ROOF	8.00 12.00	22-04-00	08-08-13	2 X 4	2 X 4	00-00-00 00-00-00	01-03-08 01-03-08	180.20 120.34		
	8	T51 MONOPITCH	9.00 0.00	15-06-00	14-03-06	2 X 4	2 X 4	01-01-08 00-00-00	02-03-12 14-03-06	795.44 480.00		
	1	T52 HALF HIP	9.00 0.00	15-06-00	12-04-00	2 X 4	2 X 4	01-01-08 00-00-00	02-03-12 12-04-00	109.72 66.67		
	1	G52 HALF HIP	9.00 0.00	15-06-00	10-00-00	2 X 4	2 X 4	01-01-08 00-00-00	02-03-12 10-00-00	101.65 64.67		
	1	T53 HIP GIRDER	20.00 0.00	09-00-00	05-11-05	2 X 4	2 X 4	00-05-00 00-05-00	00-03-08 00-03-08	51.67 33.84		
	1	T54 COMMON	20.00 0.00	08-09-00	08-07-13	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 00-03-08	49.38 31.67		
	1	PB30 PIGGYBACK	14.00 0.00	27-02-04	02-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	91.81 59.67		
	1	PB31 PIGGYBACK	14.00 0.00	27-02-04	02-10-10	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	92.77 58.00		
	1	PB32 PIGGYBACK	14.00 0.00	06-09-08	02-08-12	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	25.83 17.00		
	8	J30 JACK-OPEN	8.00 0.00	05-10-08	05-02-08	2 X 4	2 X 4	01-01-08 00-00-00	01-03-08 05-02-08	139.84 85.36		
	8	J30S JACK-OPEN	8.00 12.00	05-10-08	05-02-08	2 X 4	2 X 4	01-01-08 00-00-00	01-03-08 03-02-08	203.36 145.36		
	2	J31 JACK-OPEN	7.00 0.00	05-10-08	04-07-06	2 X 4	2 X 4	01-01-08 00-00-00	01-02-04 04-07-06	33.94 21.34		
	4	J32 JACK-OPEN	10.00 0.00	03-10-08	04-08-12	2 X 4	2 X 4	01-01-08 00-00-00	01-06-00 04-08-12	65.56 42.68		
	5	J33 JACK-OPEN	14.00 0.00	02-10-08	05-03-04	2 X 4	2 X 4	01-01-08 00-00-00	01-11-00 05-03-04	74.20 47.50		
	2	J34 JACK-OPEN	14.00 0.00	02-10-08	04-00-00	2 X 4	2 X 4	01-01-08 -01-01-01	01-11-00 00-03-08	25.86 16.34		
	2	J35 JACK-OPEN	14.00 0.00	01-10-08	04-00-00	2 X 4	2 X 4	00-00-00 -00-01-01	01-11-00 00-03-08	18.00 12.66		



Delivery Shiplist

DATE	05/19/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281218 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 1 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	J36 JACK-OPEN	20.00 0.00	02-10-08	05-11-05	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 05-11-05	39.30 26.01		
	2	J37 JACK-OPEN	20.00 0.00	02-10-08	04-01-09	2 X 4	2 X 4	00-05-00 -01-01-01	00-03-08 00-03-08	21.36 14.66		
	2	J38 JACK-OPEN	20.00 0.00	01-10-08	04-01-09	2 X 4	2 X 4	00-05-00 -00-01-01	00-03-08 00-03-08	19.08 13.34		

TOTAL # TRUSS= 79.00

TOTAL BFT OF ALL TRUSSES=

3940.13 BFT. TOTAL WEIGHT OF ALL TRUSSES= 6345.08 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
2	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 9.00

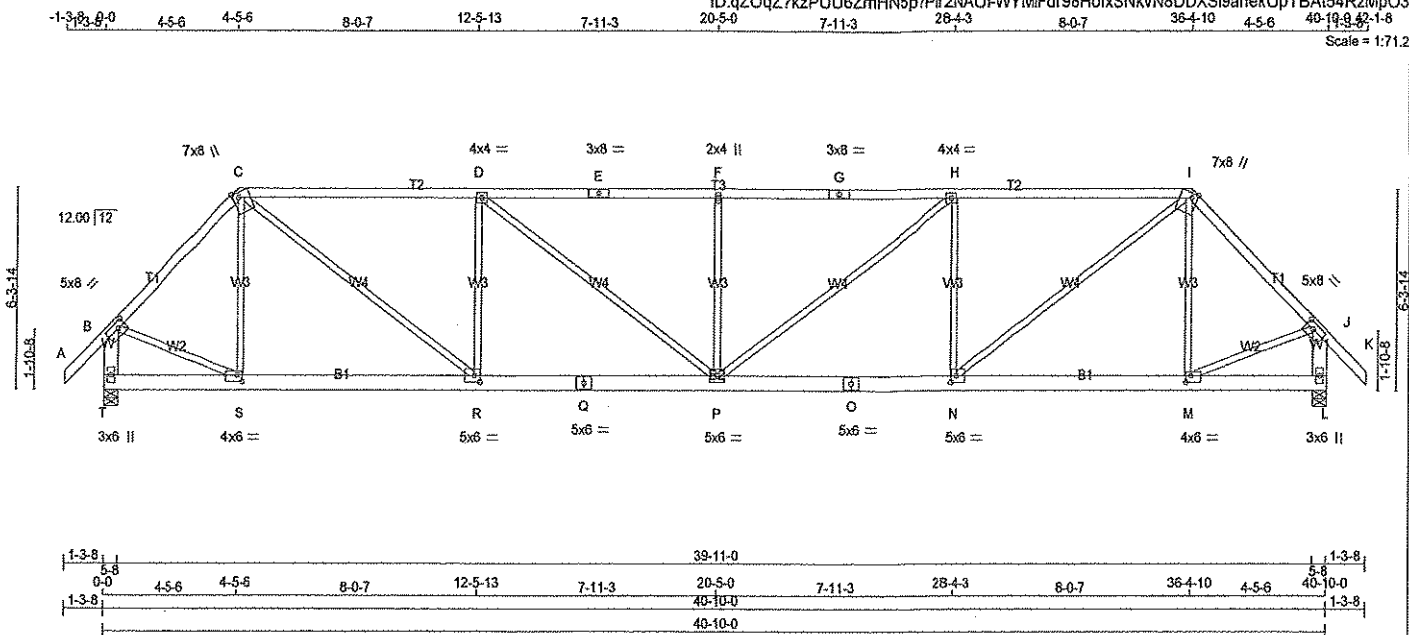
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:71.2



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E
C - E	2x4	DRY	2100F 1.8E
E - G	2x4	DRY	2100F 1.8E
G - I	2x4	DRY	2100F 1.8E
I - K	2x4	DRY	2100F 1.8E
T - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
T - O	2x6	DRY	No.2
Q - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.00
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMVW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW+m	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW-t	MT20	5.0	8.0	2.25	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	6.0	2.50	2.00
N	BMVW-t	MT20	5.0	6.0	2.50	2.25
O	BS-t	MT20	5.0	6.0		
P	BMVWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMVW-t	MT20	5.0	6.0	2.50	2.25
S	BMVW-t	MT20	4.0	6.0	2.50	2.00
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
T	2259	0	0	5-8
L	2259	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1861	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0
L	1861	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 38	-77.4	-77.4 0.07 (1)	10.00	S-C	-256 / 91	0.15 (1)
B-C	-2087 / 0	-77.4	-77.4 0.22 (1)	5.41	C-R	0 / 2203	0.50 (1)
C-D	-3240 / 0	-77.4	-77.4 0.75 (1)	4.07	R-D	-1085 / 0	0.65 (1)
D-E	-3763 / 0	-77.4	-77.4 0.82 (1)	3.79	D-P	0 / 653	0.15 (1)
E-F	-3763 / 0	-77.4	-77.4 0.82 (1)	3.79	P-F	-563 / 0	0.34 (1)
F-G	-3763 / 0	-77.4	-77.4 0.82 (1)	3.79	P-H	0 / 653	0.15 (1)
G-H	-3763 / 0	-77.4	-77.4 0.82 (1)	3.79	H-H	-1085 / 0	0.65 (1)
H-I	-3240 / 0	-77.4	-77.4 0.75 (1)	4.07	I-I	0 / 2203	0.50 (1)
I-J	-2087 / 0	-77.4	-77.4 0.22 (1)	5.41	M-I	-256 / 91	0.15 (1)
J-K	0 / 38	-77.4	-77.4 0.07 (1)	10.00	B-S	0 / 1543	0.35 (1)
T-B	-2226 / 0	0.0	0.0 0.16 (1)	6.85	M-J	0 / 1543	0.35 (1)
L-J	-2226 / 0	0.0	0.0 0.16 (1)	6.85			
T-S	0 / 0	-28.0	-28.0 0.15 (2)	10.00			
S-R	0 / 1466	-28.0	-28.0 0.29 (2)	10.00			
R-Q	0 / 3240	-28.0	-28.0 0.46 (1)	10.00			
Q-P	0 / 3240	-28.0	-28.0 0.46 (1)	10.00			
P-O	0 / 3240	-28.0	-28.0 0.46 (1)	10.00			
O-N	0 / 3240	-28.0	-28.0 0.46 (1)	10.00			
N-M	0 / 1466	-28.0	-28.0 0.29 (2)	10.00			
M-L	0 / 0	-28.0	-28.0 0.15 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.38")

CSI: TC=0.82 (D-F:1), BC=0.46 (N-P:1), WB=0.65 (H-N:1), SSI=0.29 (H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.57 (Q) (INPUT = 1.00)



DWG NO. YAM19455-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T2	1	1	TRUSS DESC.		

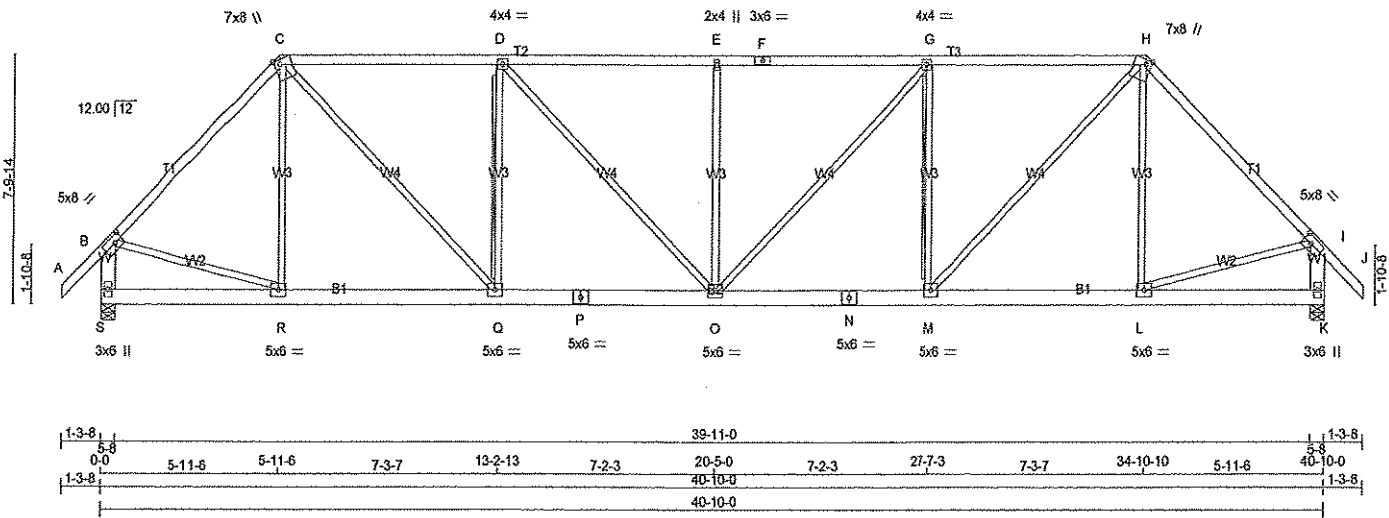
Tamarac Roof Truss, Burlington

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-1-3-8 5-11-6 5-11-6 7-3-7 13-2-13 7-2-3 20-5-0 7-2-3 27-7-3 7-3-7 34-10-10 5-11-6 40-10-0 1-3-8

Scale = 1/71.2



TOTAL WEIGHT = 205 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	1650F 1.5E
B - F	2x4	DRY	1650F 1.5E
F - H	2x4	DRY	1650F 1.5E
H - J	2x4	DRY	1650F 1.5E
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	5.0	8.0	2.50 3.00
C	TTWW+m	MT20	7.0	8.0	Edge 2.25
D	TMVW-t	MT20	4.0	4.0	
E	TMVW-w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMVW-t	MT20	4.0	4.0	
H	TTWW+m	MT20	7.0	8.0	Edge 2.25
I	TMVW-t	MT20	5.0	8.0	2.50 3.00
K	BMV1+p	MT20	3.0	6.0	
L, M, Q, R					
L	BMVW-t	MT20	5.0	6.0	
N	BS-t	MT20	5.0	6.0	
O	BMVWW-t	MT20	5.0	6.0	
P	BS-t	MT20	5.0	6.0	
S	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
S	2259 0	2259 0	0 0	5-8 5-8	
K	2259 0	2259 0	0 0	5-8 5-8	

UNFACTORED REACTIONS							
JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS				
S	1881	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0
K	1881	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-Q, G-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED L/C1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH	MAX. FACTORED UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 38	-77.4	-77.4 0.09 (1)	10.00	R-C	-146 / 166	0.16 (1)
B-C	-2118 / 0	-77.4	-77.4 0.58 (1)	4.69	C-Q	0 / 1658	0.37 (1)
C-D	-2854 / 0	-77.4	-77.4 0.70 (1)	4.09	Q-D	-982 / 0	0.83 (1)
D-E	-2994 / 0	-77.4	-77.4 0.74 (1)	3.87	D-O	0 / 490	0.11 (1)
E-F	-2994 / 0	-77.4	-77.4 0.74 (1)	3.87	O-E	-512 / 0	0.55 (1)
F-G	-2994 / 0	-77.4	-77.4 0.74 (1)	3.87	O-G	0 / 490	0.11 (1)
G-H	-2854 / 0	-77.4	-77.4 0.70 (1)	4.09	M-G	-982 / 0	0.83 (1)
H-I	-2118 / 0	-77.4	-77.4 0.58 (1)	4.69	M-H	0 / 1658	0.37 (1)
I-J	0 / 38	-77.4	-77.4 0.09 (1)	10.00	L-H	-146 / 166	0.16 (1)
S-B	-2195 / 0	0.0	0.0 0.16 (1)	6.89	B-R	0 / 1535	0.35 (1)
K-I	-2195 / 0	0.0	0.0 0.16 (1)	6.89	L-I	0 / 1535	0.35 (1)
S-R	0 / 0	-28.0	-28.0 0.14 (2)	10.00			
R-Q	0 / 1492	-28.0	-28.0 0.28 (2)	10.00			
Q-P	0 / 2654	-28.0	-28.0 0.38 (1)	10.00			
P-O	0 / 2654	-28.0	-28.0 0.38 (1)	10.00			
O-N	0 / 2654	-28.0	-28.0 0.38 (1)	10.00			
N-M	0 / 2654	-28.0	-28.0 0.38 (1)	10.00			
M-L	0 / 1492	-28.0	-28.0 0.28 (2)	10.00			
L-K	0 / 0	-28.0	-28.0 0.14 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.36")
CALCULATED VERT. DEFL. (LL) = L/999 (0.18")
ALLOWABLE DEFL. (TL) = L/360 (1.36")
CALCULATED VERT. DEFL. (TL) = L/999 (0.29")

CSI: TC=0.74 (E-G-1), BC=0.38 (M-O-1), WB=0.83 (D-Q-1), SSI=0.27 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

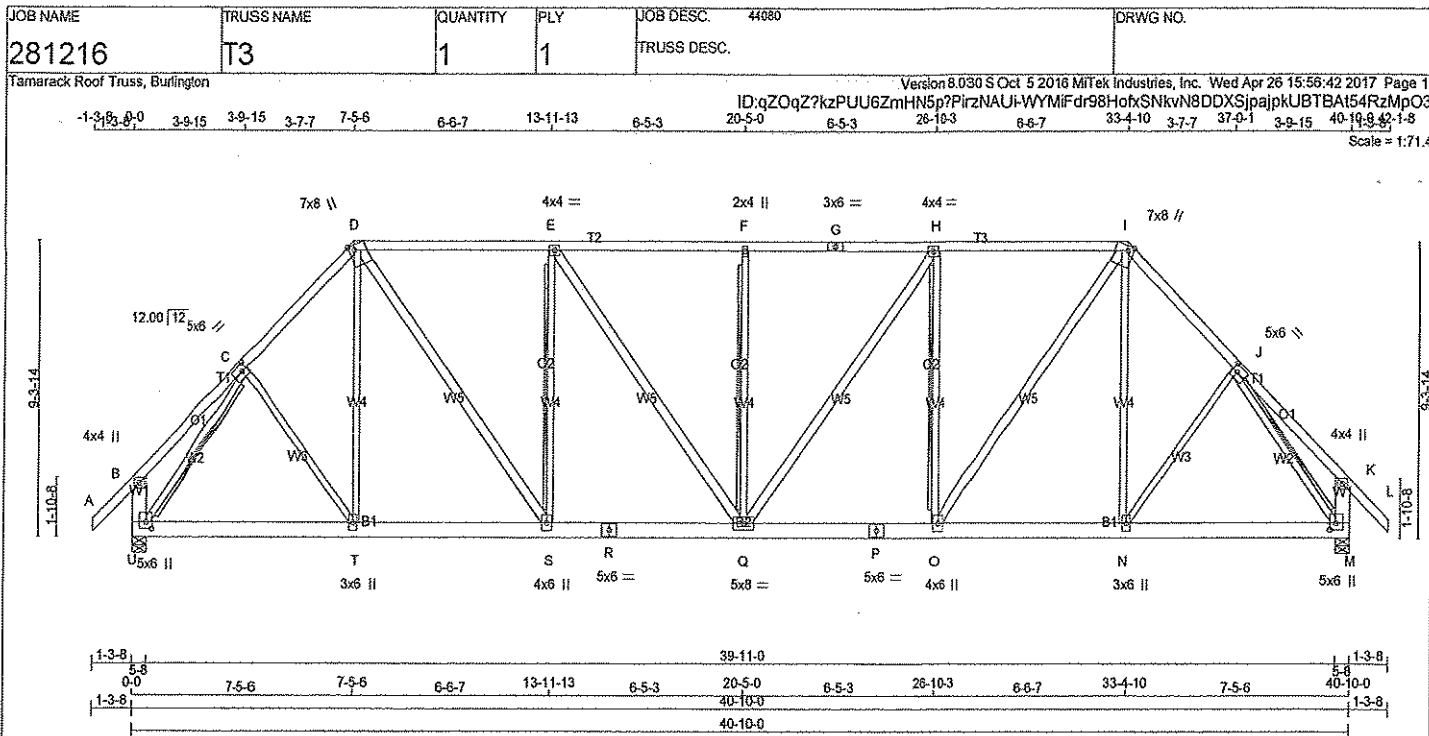
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (M) (INPUT = 0.90)
JSI METAL= 0.48 (P) (INPUT = 1.00)



DWG NO. TAM 19456-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS 2x3 DRY No.2 SPF			
EXCEPT			
D - S	2x4	DRY	No.2
E - Q	2x4	DRY	No.2
Q - H	2x4	DRY	No.2
O - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMVW+1	MT20	5.0	6.0	2.50	2.00
D	TTVW+m	MT20	7.0	8.0	Edge	2.25
E	TMVW+1	MT20	4.0	4.0		
F	TMVW+p	MT20	2.0	4.0		
G	TS-1	MT20	3.0	6.0		
H	TMVW+1	MT20	4.0	4.0		
I	TTVW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW+1	MT20	5.0	6.0	2.50	2.00
K	TMV+p	MT20	4.0	4.0		
M	BMVW+1+p	MT20	5.0	6.0	2.50	2.25
N	BMVW+1	MT20	3.0	6.0		
O	BMVW+1	MT20	4.0	6.0		
P	BS-1	MT20	5.0	6.0		
Q	BMVW+1	MT20	5.0	6.0		
R	BS-1	MT20	5.0	6.0		
S	BMVW+1	MT20	4.0	6.0		
T	BMVW+1	MT20	3.0	6.0		
U	BMVW+1+p	MT20	5.0	6.0	2.50	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
U	2259	0	2259	0	0	5-8	5-8	
M	2259	0	2259	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
U	1861	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0
M	1861	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-S, H-O, C-U, J-M
2x3 DRY SPF No.2 T-BRACE AT F-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 38	-77.4 -77.4	0.11 (1)	10.00	C-T	0 / 184	0.04 (2)
B-C	0 / 22	-77.4 -77.4	0.16 (1)	10.00	T-D	0 / 256	0.06 (3)
C-D	-2128 / 0	-77.4 -77.4	0.24 (1)	4.46	D-S	0 / 1309	0.21 (1)
D-E	-2258 / 0	-77.4 -77.4	0.68 (1)	3.85	S-E	-877 / 0	0.54 (1)
E-F	-2484 / 0	-77.4 -77.4	0.71 (1)	3.66	E-Q	0 / 386	0.06 (1)
F-G	-2484 / 0	-77.4 -77.4	0.71 (1)	3.66	Q-F	-458 / 0	0.64 (1)
G-H	-2484 / 0	-77.4 -77.4	0.71 (1)	3.66	Q-H	0 / 386	0.06 (1)
H-I	-2258 / 0	-77.4 -77.4	0.68 (1)	3.85	O-H	-877 / 0	0.54 (1)
I-J	-2128 / 0	-77.4 -77.4	0.24 (1)	4.46	O-I	0 / 1309	0.21 (1)
J-K	0 / 22	-77.4 -77.4	0.16 (1)	10.00	N-I	0 / 256	0.06 (3)
K-L	0 / 38	-77.4 -77.4	0.11 (1)	10.00	N-J	0 / 184	0.04 (2)
U-B	-220 / 0	0.0 0.0	0.02 (1)	7.81	U-C	-2403 / 0	0.69 (1)
M-K	-220 / 0	0.0 0.0	0.02 (1)	7.81	J-M	-2403 / 0	0.69 (1)
U-T	0 / 1403	-28.0 -28.0	0.29 (2)	10.00			
T-S	0 / 1485	-28.0 -28.0	0.30 (2)	10.00			
S-R	0 / 2258	-28.0 -28.0	0.32 (1)	10.00			
R-Q	0 / 2258	-28.0 -28.0	0.32 (1)	10.00			
Q-P	0 / 2258	-28.0 -28.0	0.32 (1)	10.00			
P-O	0 / 2258	-28.0 -28.0	0.32 (1)	10.00			
O-N	0 / 1485	-28.0 -28.0	0.30 (2)	10.00			
N-M	0 / 1403	-28.0 -28.0	0.29 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.14")
ALLOWABLE DEFL. (TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.23")

CSI: TC=0.71 (E-F:1), BC=0.32 (Q-S:1), WB=0.69 (J-M:1), SSI=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

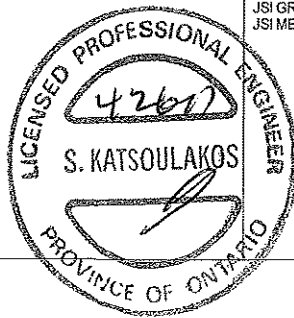
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1658

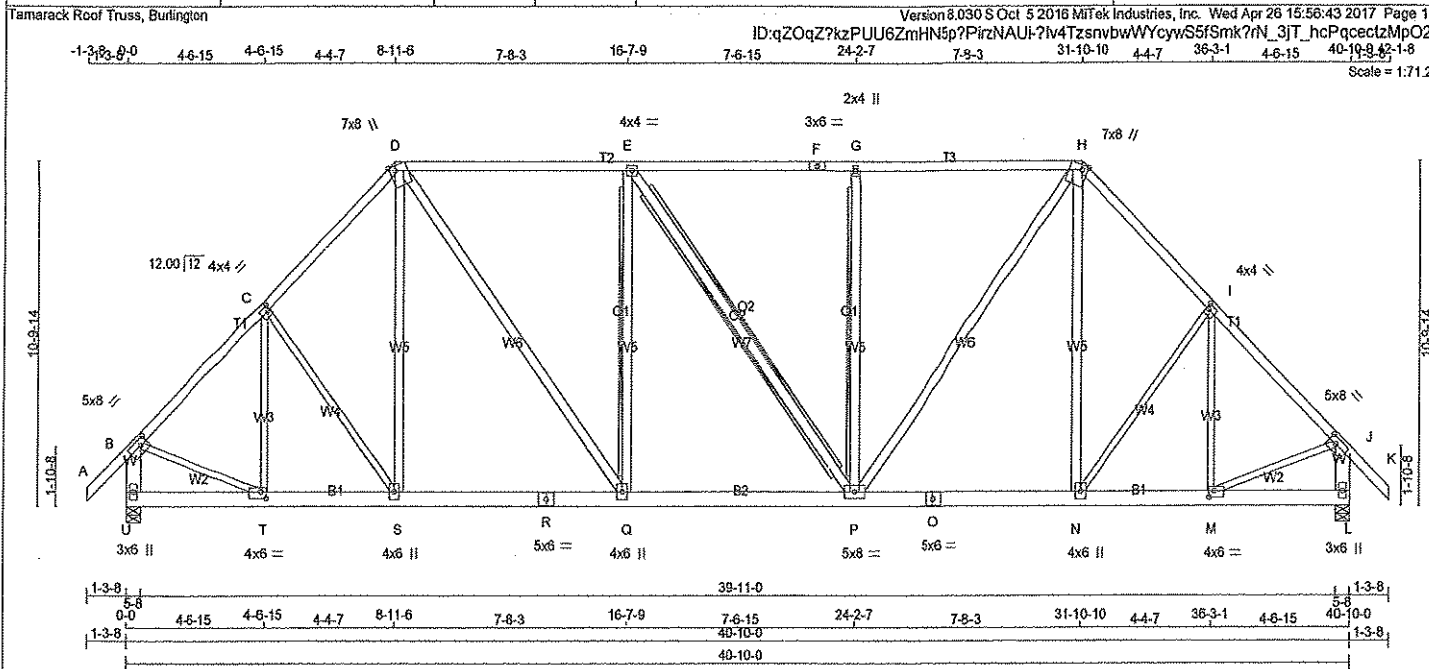
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.59 (J) (INPUT = 1.00)



DRWG NO. TAM 19457-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS 2x4 DRY No.2			
EXCEPT			
T - C	2x3	DRY	No.2
C - S	2x3	DRY	No.2
N - I	2x3	DRY	No.2
M - I	2x3	DRY	No.2
B - T	2x3	DRY	No.2
M - J	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-I	MT20	5.0	8.0	2.25 3.00
C	TMVW-I	MT20	4.0	4.0	2.00 1.00
D	TTWW+m	MT20	7.0	8.0	Edge 2.25
E	TMVW-I	MT20	4.0	4.0	
F	TS-I	MT20	3.0	6.0	
G	TMVW+m	MT20	2.0	4.0	
H	TTWW+m	MT20	7.0	8.0	Edge 2.25
I	TMVW-I	MT20	4.0	4.0	2.00 1.00
J	TMVW-I	MT20	5.0	8.0	2.25 3.00
L	BMV1+p	MT20	3.0	6.0	
M	BMVW-I	MT20	4.0	6.0	2.50 2.00
N, Q, S					
N	BMVW-I	MT20	4.0	6.0	
O	BS-I	MT20	5.0	6.0	
P	BMVW-I	MT20	5.0	6.0	
R	BS-I	MT20	5.0	6.0	
T	BMVW-I	MT20	4.0	6.0	2.50 2.00
U	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
U	VERT	2259	0	2259	0
L	HORZ	2259	0	2259	0

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1861	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0
L	1861	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-Q, G-P
2x4 DRY SPF No.2 I-BRACE AT E-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD	MAX. (PLF)	MEMB.	MAX. FORCE (LBS)	MAX. (PLF)	MAX. (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 38	-77.4	-77.4 0.11 (1)	10.00	T-C	-361 / 0	0.23 (1)
B-C	-2070 / 0	-77.4	-77.4 0.36 (1)	4.39	C-S	-55 / 0	0.06 (1)
C-D	-2089 / 0	-77.4	-77.4 0.37 (1)	4.37	S-D	0 / 332	0.05 (2)
D-E	-2084 / 0	-77.4	-77.4 0.92 (1)	3.67	D-Q	0 / 1062	0.17 (1)
E-F	-2082 / 0	-77.4	-77.4 0.91 (1)	3.67	Q-E	-638 / 0	0.48 (1)
F-G	-2082 / 0	-77.4	-77.4 0.91 (1)	3.67	E-P	-4 / 0	0.00 (1)
G-H	-2082 / 0	-77.4	-77.4 0.91 (1)	3.69	P-G	-639 / 0	0.48 (1)
H-I	-2089 / 0	-77.4	-77.4 0.37 (1)	4.37	P-H	0 / 1058	0.17 (1)
I-J	-2070 / 0	-77.4	-77.4 0.38 (1)	4.39	N-H	0 / 334	0.05 (2)
J-K	0 / 38	-77.4	-77.4 0.11 (1)	10.00	N-I	-53 / 0	0.06 (1)
U-B	-2195 / 0	0.0	0.0 0.16 (1)	6.89	M-I	-363 / 0	0.23 (1)
L-J	-2195 / 0	0.0	0.0 0.16 (1)	6.89	B-T	0 / 1555	0.35 (1)
					M-J	0 / 1555	0.35 (1)
U-T	0 / 0	-28.0	-28.0 0.07 (2)	10.00			
T-S	0 / 1484	-28.0	-28.0 0.25 (2)	10.00			
S-R	0 / 1454	-28.0	-28.0 0.29 (2)	10.00			
R-Q	0 / 1454	-28.0	-28.0 0.29 (2)	10.00			
Q-P	0 / 2084	-28.0	-28.0 0.34 (2)	10.00			
P-O	0 / 1455	-28.0	-28.0 0.28 (2)	10.00			
O-N	0 / 1455	-28.0	-28.0 0.28 (2)	10.00			
N-M	0 / 1483	-28.0	-28.0 0.25 (2)	10.00			
M-L	0 / 0	-28.0	-28.0 0.07 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.92 (D-E-1), BC=0.34 (P-Q-2), WB=0.48 (G-P-1), SSI=0.28 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)



DRWG NO. TAM 19458-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T5	4	1	TRUSS DESC.		

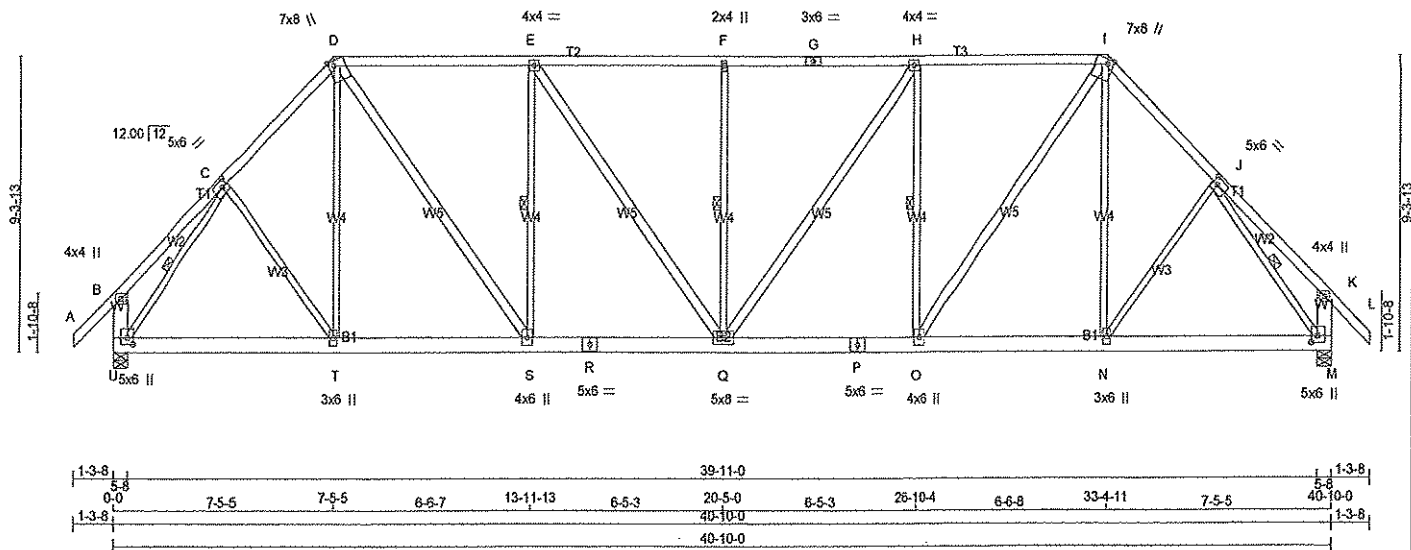
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Wed Apr 26 15:58:43 2017 Page 1

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-1-3-8 3-8-15 3-9-15 3-7-7 7-5-5 6-6-7 13-11-13 6-5-3 20-5-0 6-5-3 26-10-4 6-6-8 33-4-11 3-7-6 37-0-1 3-9-14 40-10-0 40-10-0 1-3-8

Scale = 1:71.4



TOTAL WEIGHT = 4 X 239 = 957 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - S	2x4	DRY	No.2
E - Q	2x4	DRY	No.2
Q - H	2x4	DRY	No.2
O - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED	MAXIMUM FACTORED	INPUT
	GROSS REACTION	GROSS REACTION	BRG
JT	VERT	DOWN	UPLIFT
U	2259	0	5-8
M	2259	0	5-8

UNFACTORED REACTIONS

1ST LCASE			
JT	COMBINED	SNOW	LIVE
U	1861	1016 / 0	429 / 0
M	1861	1016 / 0	429 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, F-Q, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (PLF)
A-B	0 / 38	-77.4	-77.4 0.11 (1)
B-C	0 / 22	-77.4	-77.4 0.16 (1)
C-D	-2128 / 0	-77.4	-77.4 0.24 (1)
D-E	-2259 / 0	-77.4	-77.4 0.69 (1)
E-F	-2485 / 0	-77.4	-77.4 0.71 (1)
F-G	-2485 / 0	-77.4	-77.4 0.71 (1)
G-H	-2485 / 0	-77.4	-77.4 0.71 (1)
H-I	-2259 / 0	-77.4	-77.4 0.69 (1)
I-J	-2128 / 0	-77.4	-77.4 0.24 (1)
J-K	0 / 22	-77.4	-77.4 0.16 (1)
K-L	0 / 38	-77.4	-77.4 0.11 (1)
U-B	-220 / 0	0.0	0.0 0.02 (1)
M-K	-220 / 0	0.0	0.0 0.02 (1)
U-T	0 / 1402	-28.0	-28.0 0.29 (2)
T-S	0 / 1485	-28.0	-28.0 0.30 (2)
S-R	0 / 2259	-28.0	-28.0 0.32 (1)
R-Q	0 / 2259	-28.0	-28.0 0.32 (1)
Q-P	0 / 2259	-28.0	-28.0 0.32 (1)
P-O	0 / 2259	-28.0	-28.0 0.32 (1)
O-N	0 / 1485	-28.0	-28.0 0.30 (2)
N-M	0 / 1402	-28.0	-28.0 0.29 (2)

WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
C-T	0 / 185	0.04 (2)	0.04 (2)
T-D	0 / 256	0.06 (3)	0.06 (3)
D-S	0 / 1310	0.21 (1)	0.21 (1)
S-E	-878 / 0	0.48 (1)	0.48 (1)
E-Q	0 / 387	0.08 (1)	0.08 (1)
Q-F	-458 / 0	0.25 (1)	0.25 (1)
F-H	0 / 387	0.08 (1)	0.08 (1)
H-O	-878 / 0	0.48 (1)	0.48 (1)
O-I	0 / 1310	0.21 (1)	0.21 (1)
I-N	0 / 256	0.06 (3)	0.06 (3)
N-J	0 / 185	0.04 (2)	0.04 (2)
U-C	-2403 / 0	0.64 (1)	0.64 (1)
J-M	-2403 / 0	0.64 (1)	0.64 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.14")
ALLOWABLE DEFL.(TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.23")

CSI: TC=0.71 (E-F:1), BC=0.32 (Q-S:1), WB=0.64 (C-U:1), SSI=0.24 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

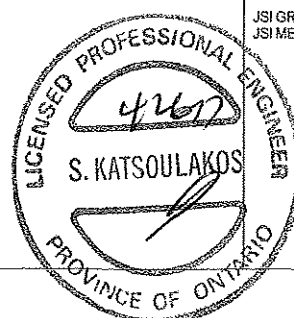
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

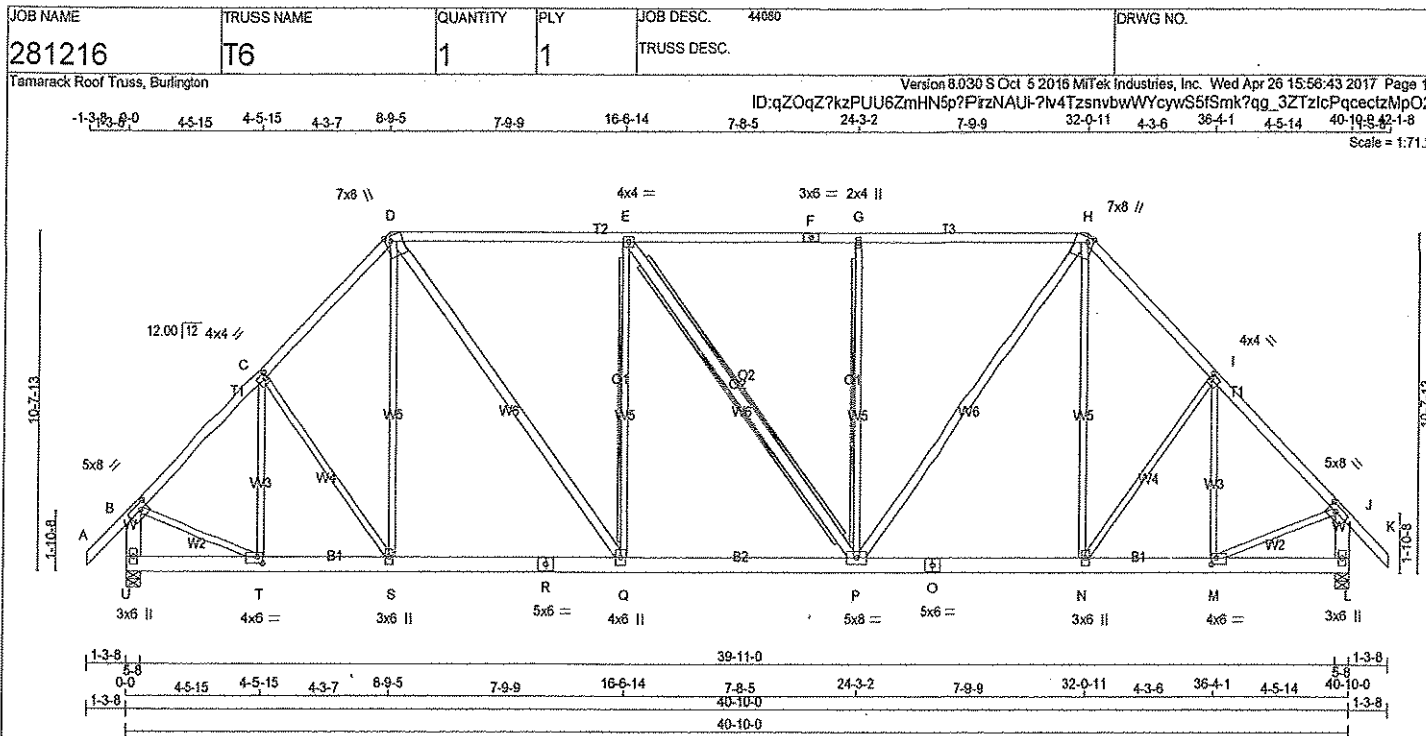
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.59 (C) (INPUT = 1.00)



DWG NO. TAML9459-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - Q	2x4	DRY	No.2
E - P	2x4	DRY	No.2
P - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	8.0	2.25	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW+m	MT20	7.0	8.0	Edge	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW+m	MT20	2.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMWW-t	MT20	4.0	4.0	2.00	1.00
J	TMWW-t	MT20	5.0	8.0	2.25	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.50	2.00
N	BMWW-t	MT20	3.0	6.0		
P	BMWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	6.0	2.50	2.00
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
U	2259	0	2259	0	5-8
L	2259	0	2259	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
U	1881	1016/0	429/0
L	1881	1016/0	429/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-Q, G-P
2x4 DRY SPF No.2 I-BRACE AT E-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FORCE (LBS)	LC1 MAX (LC)	MEMB.
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/38	-77.4	-77.4 0.11 (1)	10.00	T-C	-374/0	0.23 (1)
B-C	-2084/0	-77.4	-77.4 0.36 (1)	4.41	C-S	-39/7	0.04 (1)
C-D	-2095/0	-77.4	-77.4 0.36 (1)	4.38	S-D	0/324	0.07 (2)
D-E	-2119/0	-77.4	-77.4 0.96 (1)	3.59	D-Q	0/1092	0.18 (1)
E-F	-2116/0	-77.4	-77.4 0.95 (1)	3.59	Q-E	-647/0	0.57 (1)
F-G	-2116/0	-77.4	-77.4 0.95 (1)	3.59	E-P	-5/0	0.00 (1)
G-H	-2116/0	-77.4	-77.4 0.95 (1)	3.61	P-G	-648/0	0.57 (1)
H-I	-2096/0	-77.4	-77.4 0.36 (1)	4.38	P-H	0/1086	0.17 (1)
I-J	-2084/0	-77.4	-77.4 0.36 (1)	4.41	N-H	0/327	0.07 (2)
J-K	0/38	-77.4	-77.4 0.11 (1)	10.00	N-I	-38/7	0.04 (1)
U-B	-2196/0	0.0	0.0 0.16 (1)	6.89	M-I	-377/0	0.23 (1)
L-J	-2195/0	0.0	0.0 0.16 (1)	6.89	B-T	0/1553	0.35 (1)
U-T	0/0	-28.0	-28.0 0.07 (2)	10.00	M-J	0/1553	0.35 (1)
T-S	0/1479	-28.0	-28.0 0.25 (2)	10.00			
S-R	0/1458	-28.0	-28.0 0.30 (2)	10.00			
R-Q	0/1458	-28.0	-28.0 0.30 (2)	10.00			
Q-P	0/2119	-28.0	-28.0 0.35 (2)	10.00			
P-O	0/1458	-28.0	-28.0 0.29 (2)	10.00			
O-N	0/1458	-28.0	-28.0 0.29 (2)	10.00			
N-M	0/1479	-28.0	-28.0 0.25 (2)	10.00			
M-L	0/0	-28.0	-28.0 0.07 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.36")
CALCULATED VERT. DEFL. (LL) = L/999 (0.11")
ALLOWABLE DEFL. (TL) = L/360 (1.36")
CALCULATED VERT. DEFL. (TL) = L/999 (0.19")

CSI: TC=0.96 (D-E-1), BC=0.35 (P-Q-2), WB=0.57 (G-P-1), SSI=0.28 (D-E-1)

DOI: LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

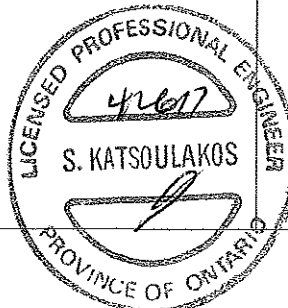
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)



DRWG NO. TAM 1946a-17
STRUCTURAL
COMPONENT ONLY

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FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
AC	28-5-4	-50	-87	---	FRONT	VERT	TOTAL
AD	28-4-8	-1042	-1042	---	FRONT	VERT	TOTAL

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 149.9 lbs FACTORED DOWN AT 22-5-4,
AND 149.9 lbs FACTORED DOWN AT 24-5-4,
AND 149.9 lbs FACTORED DOWN AT 28-5-4 ON
TOP CHORD, AND 2834.2 lbs FACTORED DOWN
AT 21-9-8, 87.4 lbs FACTORED DOWN AT
22-5-4, 87.4 lbs FACTORED DOWN AT 24-5-4,
AND 87.4 lbs FACTORED DOWN AT 26-5-4, AND
1042.2 lbs FACTORED DOWN AT 28-4-8 ON
BOTTOM CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER



DWG NO. TAM 1946-17
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM 19462-17
STRUCTURAL
COMPONENT ONLY

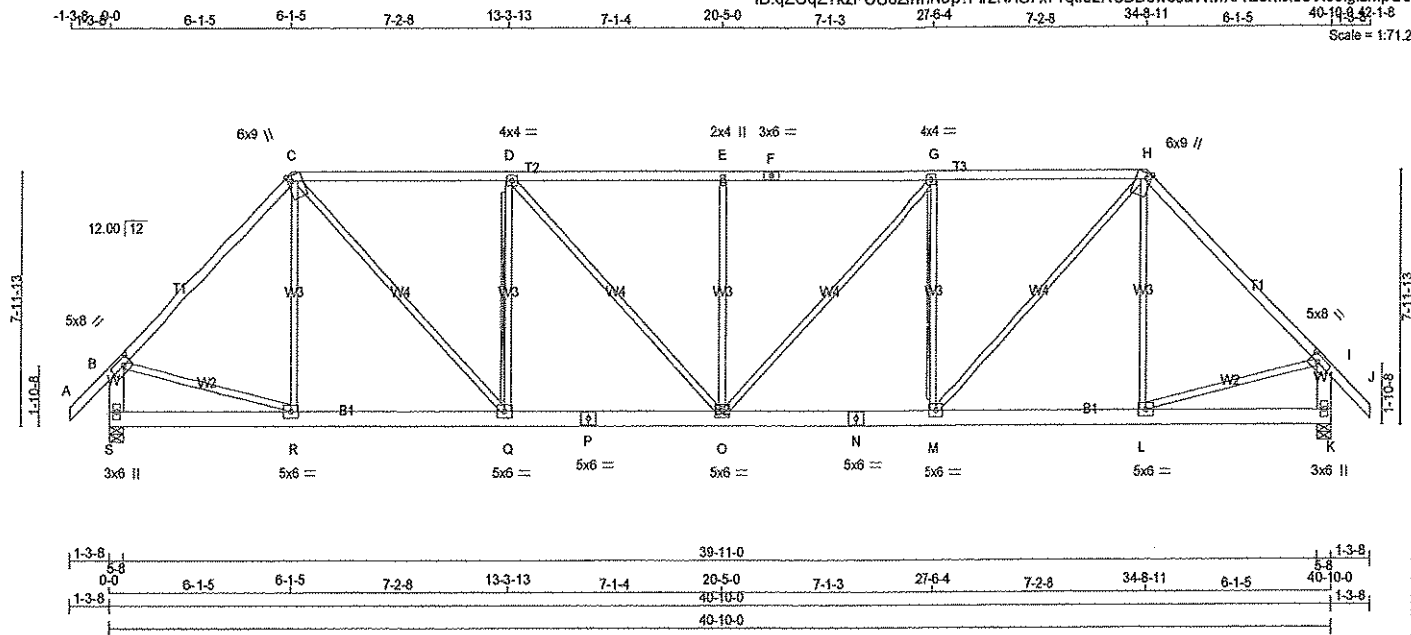
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T9	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:qZQz7kzPUU6ZmHN5p?PirzNAUj-x71qtFu2RCBDow6JaWw94E8nlx08vI85igIzMp00

Scale = 1:71.2



TOTAL WEIGHT = 207 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E
C - F	2x4	DRY	1650F 1.5E
F - H	2x4	DRY	1650F 1.5E
H - J	2x4	DRY	1650F 1.5E
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-t	MT20	5.0	8.0	2.50	3.00
K	BMV1+p	MT20	3.0	6.0		
L, M, Q, R						
L	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
S	2259	0	2259	0	5-8
K	2259	0	2259	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1881	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0
K	1881	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-Q, G-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO			
A-B	0 / 38	-77.4 -77.4 0.09 (1)	10.00
B-C	-2118 / 0	-77.4 -77.4 0.62 (1)	4.65
C-D	-2604 / 0	-77.4 -77.4 0.38 (1)	4.14
D-E	-2929 / 0	-77.4 -77.4 0.71 (1)	3.93
E-F	-2929 / 0	-77.4 -77.4 0.71 (1)	3.93
F-G	-2929 / 0	-77.4 -77.4 0.71 (1)	3.93
G-H	-2604 / 0	-77.4 -77.4 0.68 (1)	4.14
H-I	-2118 / 0	-77.4 -77.4 0.62 (1)	4.65
I-J	0 / 38	-77.4 -77.4 0.09 (1)	10.00
S-B	-2192 / 0	0.0 0.0 0.16 (1)	6.89
K-I	-2192 / 0	0.0 0.0 0.16 (1)	6.89
S-R	0 / 0	-28.0 -28.0 0.14 (2)	10.00
R-Q	0 / 1492	-28.0 -28.0 0.28 (2)	10.00
Q-P	0 / 2604	-28.0 -28.0 0.37 (1)	10.00
P-O	0 / 2604	-28.0 -28.0 0.37 (1)	10.00
O-N	0 / 2604	-28.0 -28.0 0.37 (1)	10.00
N-M	0 / 2604	-28.0 -28.0 0.37 (1)	10.00
M-L	0 / 1492	-28.0 -28.0 0.28 (2)	10.00
L-K	0 / 0	-28.0 -28.0 0.14 (2)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
D.L. = 3.0 PSF
BOT CH. LL = 10.5 PSF
D.L. = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.71 (D-E:1), BC=0.37 (O-Q:1), WB=0.87 (D-Q:1), GSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

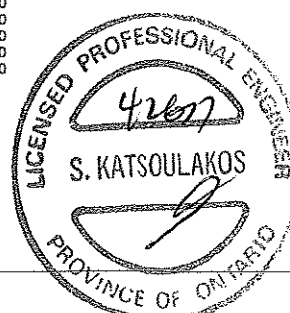
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

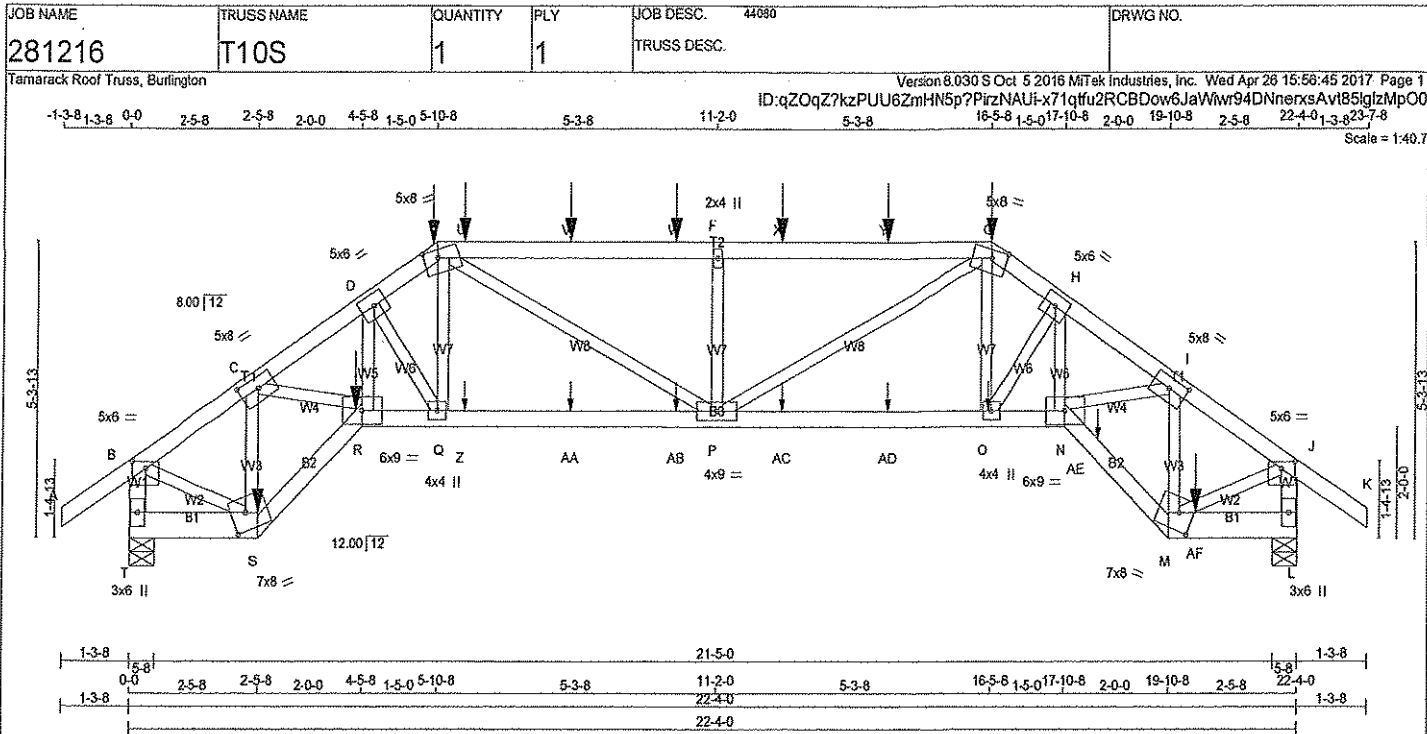
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (C) (INPUT = 0.90)
JSI METAL = 0.45 (P) (INPUT = 1.00)



DWG NO. TAM 1946317
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	2100F 1.8E SPF
E - G	2x4	DRY	2100F 1.8E SPF
G - K	2x4	DRY	2100F 1.8E SPF
T - B	2x4	DRY	No.2 SPF
L - J	2x4	DRY	No.2 SPF
T - S	2x6	DRY	No.2 SPF
S - R	2x4	DRY	No.2 SPF
R - N	2x4	DRY	No.2 SPF
N - M	2x4	DRY	No.2 SPF
M - L	2x6	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-w	MT20	5.0	8.0	2.25	4.00
D	TMVW-t	MT20	5.0	6.0		
E	TTVW-m	MT20	5.0	8.0	1.75	3.25
F	TMVW-w	MT20	2.0	4.0		
G	TTVW-m	MT20	5.0	8.0	1.75	3.25
H	TMVW-t	MT20	5.0	6.0		
I	TMVW-t	MT20	5.0	8.0	2.25	4.00
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BBWW-m	MT20	7.0	8.0	3.75	3.25
N	BBWW-p	MT20	6.0	9.0		
O	BBWW-t	MT20	4.0	4.0		
P	BBWW-w	MT20	4.0	9.0		
Q	BBWW-t	MT20	4.0	4.0		
R	BBWW-p	MT20	6.0	9.0		
S	BBWW-m	MT20	7.0	8.0	3.75	3.25
T	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 270.7 lbs FACTORED DOWN AT 5-10-8, 270.7 lbs FACTORED DOWN AT 16-5-8, 230.8 lbs FACTORED DOWN AT 6-4-12, 212.3 lbs FACTORED DOWN AT 8-4-12, 212.3 lbs FACTORED DOWN AT 10-4-12, 212.3 lbs FACTORED DOWN AT 12-4-12, AND 212.3 lbs FACTORED DOWN AT 14-4-12, AND 215.8 lbs FACTORED DOWN AT 18-5-8 ON TOP CHORD, AND 110.3 lbs FACTORED DOWN AT 2-5-8, 8.7 lbs FACTORED DOWN AT 4-5-8, AT 6-4-12, AT 8-4-12, AT 10-4-12, AT 12-4-12, AT 14-4-12, AT 16-4-12, AND AT 18-4-12, AND 99.1 lbs FACTORED DOWN AT 20-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
T	2256	0	2256	0
L	2272	0	2272	0

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
T	1894
L	1907

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX	MEMB.	MAX. FACTORED	FACTORED	MAX
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX	LC1
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/29	-77.4	-77.4 0.07 (1)	10.00	S-C	-2392/0	0.44 (1)
B-C	-2259/0	-77.4	-77.4 0.08 (1)	5.38	C-R	0/2436	0.60 (1)
C-D	-4969/0	-77.4	-77.4 0.20 (3)	3.78	R-D	0/1154	0.29 (1)
D-E	-4205/0	-77.4	-77.4 0.15 (1)	4.11	D-Q	-996/0	0.19 (1)
E-U	-4688/0	-77.4	-77.4 0.82 (1)	3.05	Q-E	0/889	0.22 (1)
U-V	-4688/0	-77.4	-77.4 0.82 (1)	3.05	E-P	0/1320	0.33 (1)
V-W	-4688/0	-77.4	-77.4 0.82 (1)	3.05	P-F	-1157/0	0.24 (1)
W-F	-4688/0	-77.4	-77.4 0.82 (1)	3.05	F-G	0/1285	0.32 (1)
F-X	-4688/0	-77.4	-77.4 0.82 (1)	3.05	G-O	0/882	0.22 (1)
X-Y	-4688/0	-77.4	-77.4 0.82 (1)	3.05	O-H	-989/0	0.18 (1)
Y-G	-4688/0	-77.4	-77.4 0.82 (1)	3.05	H-N	0/1145	0.29 (1)
G-H	-4242/0	-77.4	-77.4 0.15 (1)	4.10	N-I	0/2461	0.61 (1)
H-I	-5001/0	-77.4	-77.4 0.20 (1)	3.77	I-M	-2414/0	0.44 (1)
I-J	-2263/0	-77.4	-77.4 0.08 (1)	5.37	B-S	0/2030	0.50 (1)
J-K	0/29	-77.4	-77.4 0.07 (1)	10.00	M-J	0/2034	0.50 (1)
T-B	-2221/0	0.0	0.0 0.25 (1)	5.65			
L-J	-2225/0	0.0	0.0 0.25 (1)	5.65			

T-S	0/0	-28.0	-28.0 0.03 (2)	10.00
S-R	0/2494	-28.0	-28.0 0.46 (1)	10.00
R-Q	0/4081	-28.0	-28.0 0.81 (1)	10.00
Q-Z	0/3539	-28.0	-28.0 0.72 (1)	10.00
Z-AA	0/3539	-28.0	-28.0 0.72 (1)	10.00
AA-AB	0/3539	-28.0	-28.0 0.72 (1)	10.00
AB-P	0/3539	-28.0	-28.0 0.72 (1)	10.00
P-AC	0/3569	-28.0	-28.0 0.72 (1)	10.00
AC-AD	0/3569	-28.0	-28.0 0.72 (1)	10.00
AD-O	0/3569	-28.0	-28.0 0.72 (1)	10.00
O-N	0/4088	-28.0	-28.0 0.81 (1)	10.00
N-AE	0/2498	-28.0	-28.0 0.46 (1)	10.00
AE-M	0/2498	-28.0	-28.0 0.46 (1)	10.00
M-AF	0/0	-28.0	-28.0 0.05 (2)	10.00
AF-L	0/0	-28.0	-28.0 0.05 (2)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
E	5-10-8	-271	-271	FRONT	VERT	TOTAL
G	16-5-8	-271	-271	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	43.8	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.74")
CALCULATED VERT. DEFL. (LL) = L/999 (0.21")
ALLOWABLE DEFL. (TL) = L/360 (0.74")
CALCULATED VERT. DEFL. (TL) = L/798 (0.34")

CSI: TC=0.82 (E-F:1), BC=0.81 (N-O:1), WB=0.61 (I-N:1), SSI=0.53 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

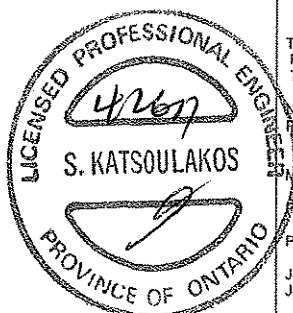
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.87 (G) (INPUT = 1.00)



DRWG NO. TAM 19464-17

STRUCTURAL
COMPONENT ONLY

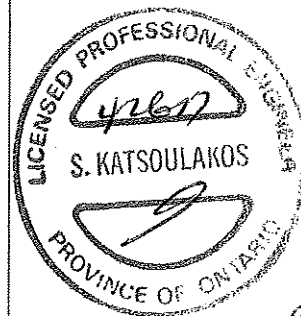
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JOB NAME 281216	TRUSS NAME T10S	QUANTITY 1	PLY 1	JOB DESC. 44080 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
G	16-5-8	-216	-216	---	BACK	VERT
O	16-4-12	---	---	---	BACK	VERT
R	4-5-8	-5	-9	---	BACK	VERT
S	2-5-8	-63	-110	---	BACK	VERT
U	6-4-12	-231	-231	---	BACK	VERT
V	8-4-12	-212	-212	---	BACK	VERT
W	10-4-12	-212	-212	---	BACK	VERT
X	12-4-12	-212	-212	---	BACK	VERT
Y	14-4-12	-212	-212	---	BACK	VERT
Z	6-4-12	---	---	---	BACK	VERT
AA	8-4-12	---	---	---	BACK	VERT
AB	10-4-12	---	---	---	BACK	VERT
AC	12-4-12	---	---	---	BACK	VERT
AD	14-4-12	---	---	---	BACK	VERT
AE	18-4-12	---	---	---	BACK	VERT
AF	20-4-12	-57	-99	---	BACK	VERT
						TOTAL



1622
DWG NO. TAM19464-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T11S	1	1	TRUSS DESC.		

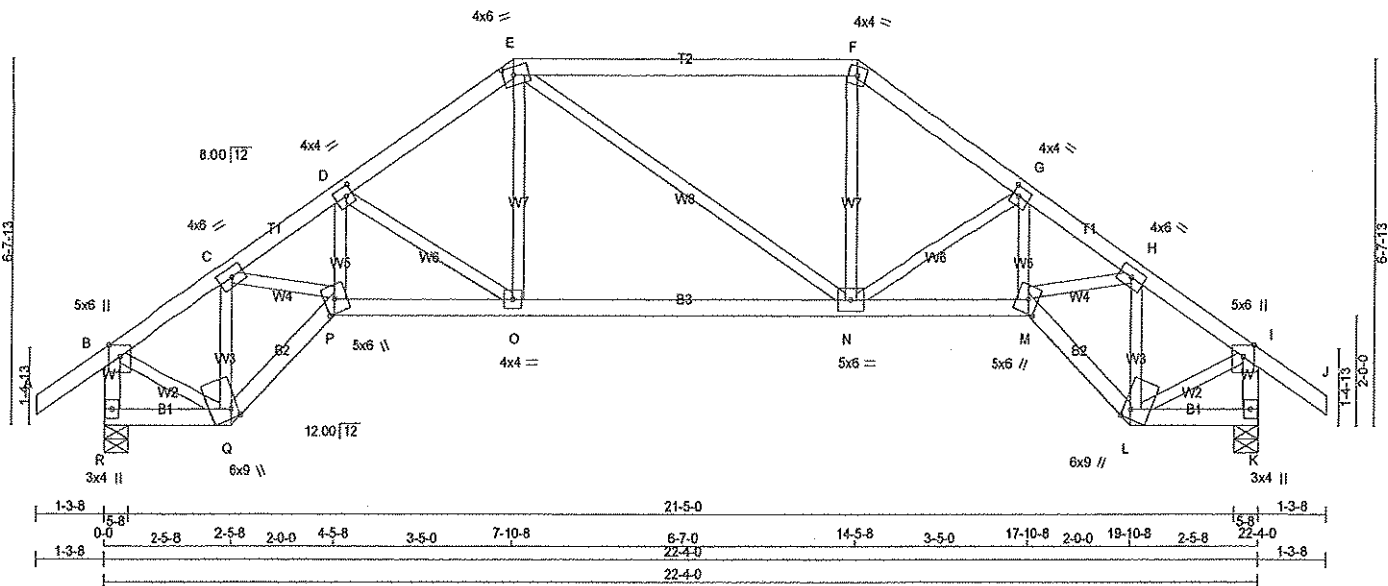
Tamarack Roof Truss, Burlington

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-1-3-8 1-3-8 0-0 2-5-8 2-5-8 2-0-0 4-5-8 3-5-0 7-10-8 6-7-0 14-5-8 3-5-0 17-10-8 2-0-0 19-10-8 2-5-8 22-4-0 1-3-8 23-7-8

Scale = 1:41.2



TOTAL WEIGHT = 96 lb

(M/F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW+I	MT20	4.0	6.0		
D	TMWW+I	MT20	4.0	4.0	2.00	1.50
E	TTWW+m	MT20	4.0	6.0	1.75	2.50
F	TTWW+m	MT20	4.0	4.0		
G	TMWW+I	MT20	4.0	4.0	2.00	1.50
H	TMWW+I	MT20	4.0	6.0		
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BBWW+m	MT20	6.0	9.0	Edge	1.75
M	BBWW+m	MT20	5.0	6.0	3.00	2.25
N	BMWW+I	MT20	5.0	6.0		
O	BMWW+I	MT20	4.0	4.0		
P	BBWW+m	MT20	5.0	6.0	3.00	2.25
Q	BBWW+m	MT20	6.0	9.0	Edge	1.75
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
R	1283	0	1283	0	0	5-8
K	1283	0	1283	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1050	584/0	235/0	0/0	0/0	232/0	0/0
K	1050	584/0	235/0	0/0	0/0	232/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/129	-77.4 -77.4	0.10 (1)	10.00	Q-C	-1264/0	0.22 (1)
B-C	-1157/0	-77.4 -77.4	0.07 (1)	5.85	C-P	0/1229	0.28 (1)
C-D	-2506/0	-77.4 -77.4	0.11 (1)	4.29	P-D	0/642	0.14 (1)
D-E	-1677/0	-77.4 -77.4	0.14 (1)	5.01	D-O	-794/0	0.22 (1)
E-F	-1401/0	-77.4 -77.4	0.47 (1)	4.93	Q-E	0/585	0.13 (2)
F-G	-1678/0	-77.4 -77.4	0.14 (1)	5.01	E-N	0/0	0.00 (1)
G-H	-2506/0	-77.4 -77.4	0.11 (1)	4.29	N-F	0/586	0.13 (2)
H-I	-1157/0	-77.4 -77.4	0.07 (1)	5.85	N-G	-793/0	0.22 (1)
I-J	0/129	-77.4 -77.4	0.10 (1)	10.00	M-G	0/641	0.14 (1)
R-B	-1248/0	0.0 0.0	0.13 (1)	7.19	M-H	0/1229	0.28 (1)
K-I	-1248/0	0.0 0.0	0.13 (1)	7.19	L-H	-1264/0	0.22 (1)
					B-Q	0/1048	0.24 (1)
					L-I	0/1049	0.24 (1)
R-Q	0/0	-28.0 -28.0	0.05 (2)	10.00			
Q-P	0/1293	-28.0 -28.0	0.23 (1)	10.00			
P-O	0/2061	-28.0 -28.0	0.43 (2)	10.00			
O-N	0/1400	-28.0 -28.0	0.36 (2)	10.00			
N-M	0/2061	-28.0 -28.0	0.44 (2)	10.00			
M-L	0/1293	-28.0 -28.0	0.23 (1)	10.00			
L-K	0/0	-28.0 -28.0	0.05 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.74")
CALCULATED VERT. DEFL. (LL) = $L/989$ (0.11")
ALLOWABLE DEFL. (TL) = $L/360$ (0.74")
CALCULATED VERT. DEFL. (TL) = $L/989$ (0.18")

CSI: TC=0.47 (E-F:1), BC=0.44 (M-N:2), WB=0.28 (C-P:1), SSI=0.20 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.36 (H) (INPUT = 1.00)



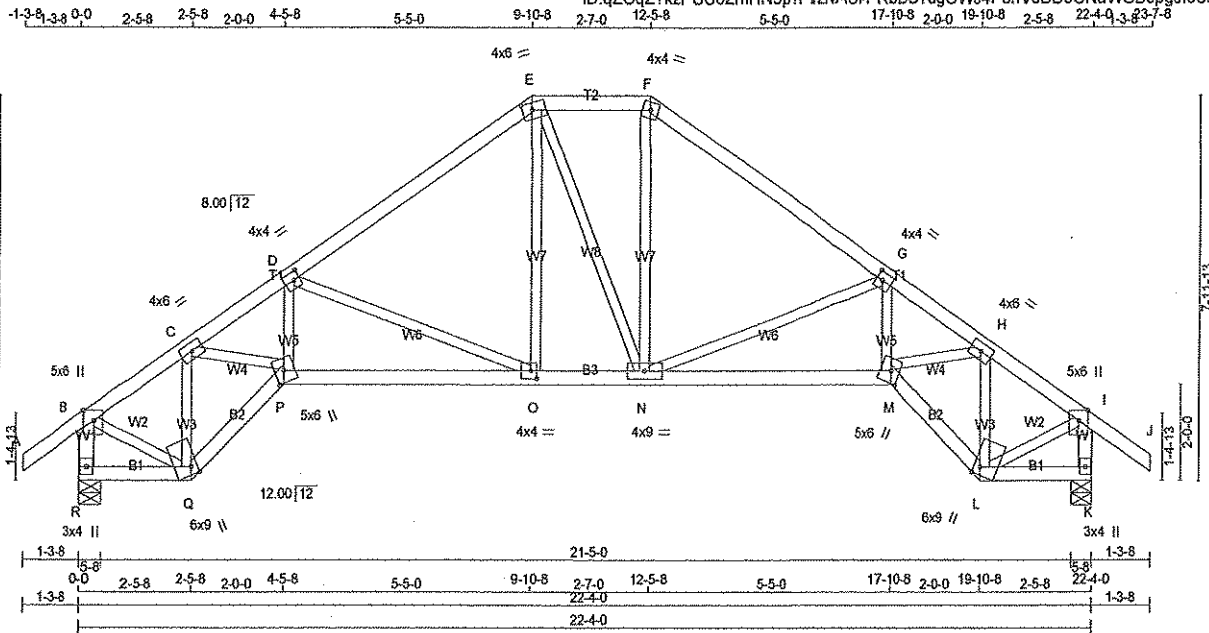
DRWG NO. TAM 1946517
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T12S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:47.0

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
R - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
R - Q	2x4	DRY	No.2
Q - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.50
E	TTVW-m	MT20	4.0	6.0	1.75	2.50
F	TTVW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-t	MT20	4.0	6.0		
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV+p	MT20	3.0	4.0		
L	BBVW+m	MT20	6.0	9.0	Edge	1.75
M	BBVW+m	MT20	5.0	6.0	3.00	2.25
N	BBVW+m	MT20	4.0	9.0		
O	BBVW-t	MT20	4.0	4.0	2.00	1.50
P	BBVW+m	MT20	5.0	6.0	3.00	2.25
Q	BBVW+m	MT20	6.0	9.0	Edge	1.75
R	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	1283	0	1283	0	5-8	5-8
K	1283	0	1283	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
R	1050	584 / 0	235 / 0	0 / 0	232 / 0	0 / 0	0 / 0
K	1050	584 / 0	235 / 0	0 / 0	232 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0 / 29	-77.4	-77.4 0.10 (1)	10.00	Q-C	-1248 / 0	0.22 (1)
B-C	-1150 / 0	-77.4	-77.4 0.09 (1)	5.85	C-P	0 / 1304	0.29 (1)
C-D	-2539 / 0	-77.4	-77.4 0.25 (1)	4.12	P-D	0 / 641	0.14 (1)
D-E	-1395 / 0	-77.4	-77.4 0.30 (1)	5.19	D-O	-1050 / 0	0.62 (1)
E-F	-1155 / 0	-77.4	-77.4 0.08 (1)	5.85	O-E	0 / 489	0.11 (1)
F-G	-1397 / 0	-77.4	-77.4 0.30 (1)	5.19	E-N	0 / 5	0.60 (1)
G-H	-2539 / 0	-77.4	-77.4 0.25 (1)	4.12	N-F	0 / 484	0.11 (1)
H-I	-1150 / 0	-77.4	-77.4 0.09 (1)	5.85	F-G	-1048 / 0	0.62 (1)
I-J	0 / 29	-77.4	-77.4 0.10 (1)	10.00	M-G	0 / 639	0.14 (1)
R-B	-1248 / 0	0.0	0.0 0.13 (1)	7.19	M-H	0 / 1304	0.29 (1)
K-I	-1248 / 0	0.0	0.0 0.13 (1)	7.19	L-H	-1248 / 0	0.22 (1)
					B-Q	0 / 1035	0.23 (1)
					L-I	0 / 1035	0.23 (1)
R-Q	0 / 0	-28.0	-28.0 0.05 (2)	10.00			
Q-P	0 / 1277	-28.0	-28.0 0.22 (1)	10.00			
P-O	0 / 2122	-28.0	-28.0 0.45 (1)	10.00			
O-N	0 / 1153	-28.0	-28.0 0.31 (2)	10.00			
N-M	0 / 2122	-28.0	-28.0 0.44 (1)	10.00			
M-L	0 / 1277	-28.0	-28.0 0.22 (1)	10.00			
L-K	0 / 0	-28.0	-28.0 0.05 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.74")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.11")
ALLOWABLE DEFL. (TL) = $L/360$ (0.74")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.19")

CSI: TC=0.30 (F-G:1), BC=0.45 (O-P:1), WB=0.62 (D-O:1), SSI=0.15 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.37 (H) (INPUT = 1.00)



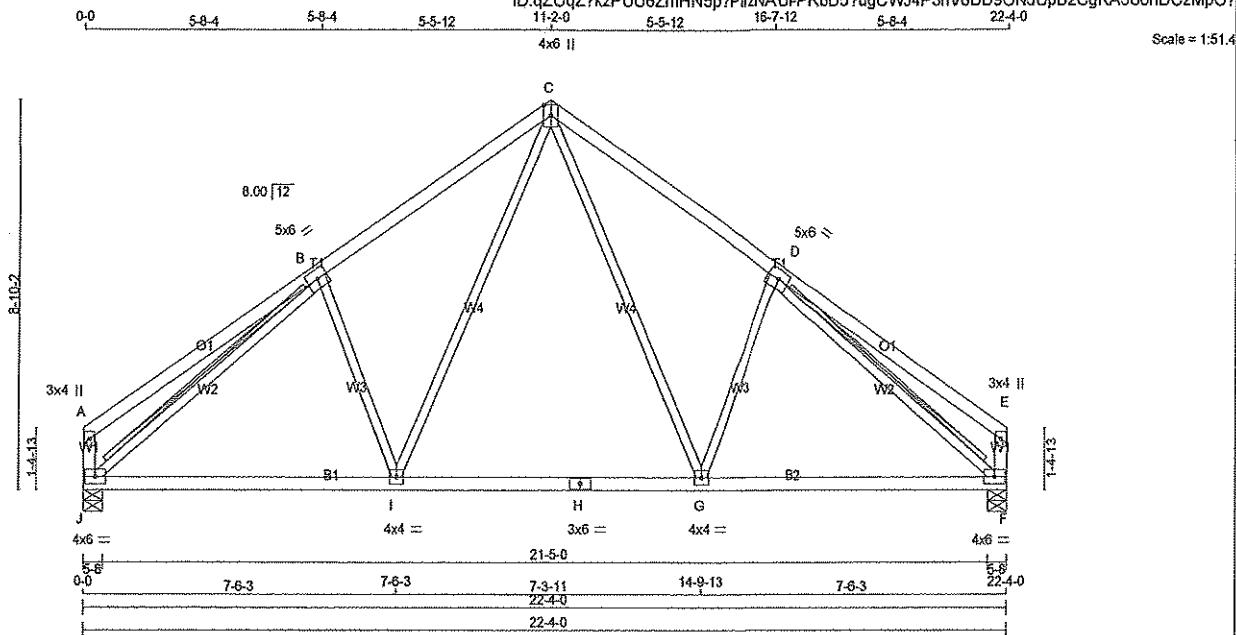
DWG NO. TAM 1946617
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T13	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 93 = 185 lb

LUMBER			
N L G A RULES	CHORDS	SIZE	DRY
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
J - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMWW-t	MT20	5.0	6.0
C	TTWW+p	MT20	4.0	6.0 Edge
D	TMWW-t	MT20	5.0	6.0
E	TMV+p	MT20	3.0	4.0
F	BMVW1-t	MT20	4.0	6.0
G	BMWW-t	MT20	4.0	4.0
H	BS-t	MT20	3.0	6.0
I	BMWW-t	MT20	4.0	4.0
J	BMVW1-t	MT20	4.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	UP	BRG	BRG	IN-SX
J	1177	0	1177	0	0	5-8	5-8	5-8	5-8
F	1177	0	1177	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
J	978	520 / 0	235 / 0	0 / 0	223 / 0
F	978	520 / 0	235 / 0	0 / 0	223 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF NO.2 T-BRACE AT B-J, D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 29	C-G	0 / 525
B-C	-1197 / 0	G-D	-278 / 47
C-D	-1197 / 0	D-E	-278 / 47
D-E	0 / 29	E-F	-278 / 47
F-G	-164 / 0	G-H	-1421 / 0
H-I	-164 / 0	I-J	-1421 / 0
J-K	0 / 1077	K-L	0 / 1077
L-M	0 / 766	M-N	0 / 766
N-O	0 / 766	O-P	0 / 766
P-Q	0 / 1077	Q-R	0 / 1077

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.39 (D-E-1), BC=0.49 (F-G-2), WB=0.56 (D-F-1), SSI=0.17 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

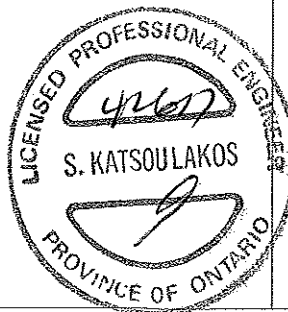
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.76 (F) (INPUT = 0.90)
JSI METAL= 0.37 (D) (INPUT = 1.00)



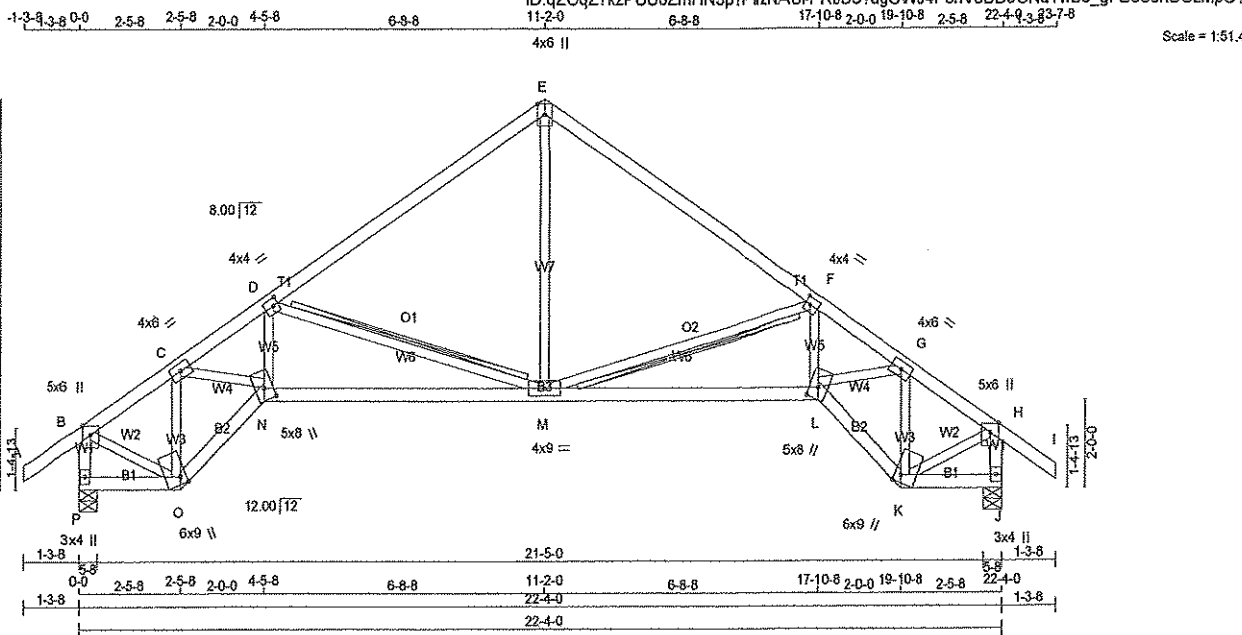
DWG NO. TAM 19467-19
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T13S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 94 = 189 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMWW-t	MT20	4.0	6.0		
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BBWW+m	MT20	6.0	9.0	Edge	1.75
L	BBWW+m	MT20	5.0	8.0	3.25	2.50
M	BBWW+m	MT20	4.0	9.0		
N	BBWW+m	MT20	5.0	8.0	3.25	2.50
O	BBWW+m	MT20	6.0	9.0	Edge	1.75
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
FACTORED			MAXIMUM FACTORED			INPUT	REQD	
	GROSS REACTION		GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
P	1283	0	1283	0	0	5-8	5-8	
J	1283	0	1283	0	0	5-8	5-8	

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
P	1050	584 / 0	235 / 0	0 / 0	0 / 0
J	1050	584 / 0	235 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF NO.2 T-BRACE AT F-M, D-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 29	M-E	0 / 953
B-C	-1143 / 0	M-F	-1207 / 0
C-D	-2569 / 0	L-F	0 / 613
D-E	-1284 / 0	L-G	0 / 1374
E-F	-1284 / 0	K-G	-1232 / 0
F-G	-2569 / 0	D-M	-1207 / 0
G-H	-1143 / 0	N-D	0 / 613
H-I	0 / 29	C-N	0 / 1374
P-B	-1248 / 0	O-C	-1232 / 0
J-H	-1248 / 0	B-O	0 / 1023
		K-H	0 / 1023
P-O	0 / 0		
O-N	0 / 1261		
N-M	0 / 2180		
M-L	0 / 2180		
L-K	0 / 1261		
K-J	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 7.0 PSF
DL = 10.5 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, EBC 2012, ABC 2014
- CSA 066-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.45 (E-F:1), BC=0.63 (L-M:2), WB=0.88 (F-M:1), SI=0.19 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

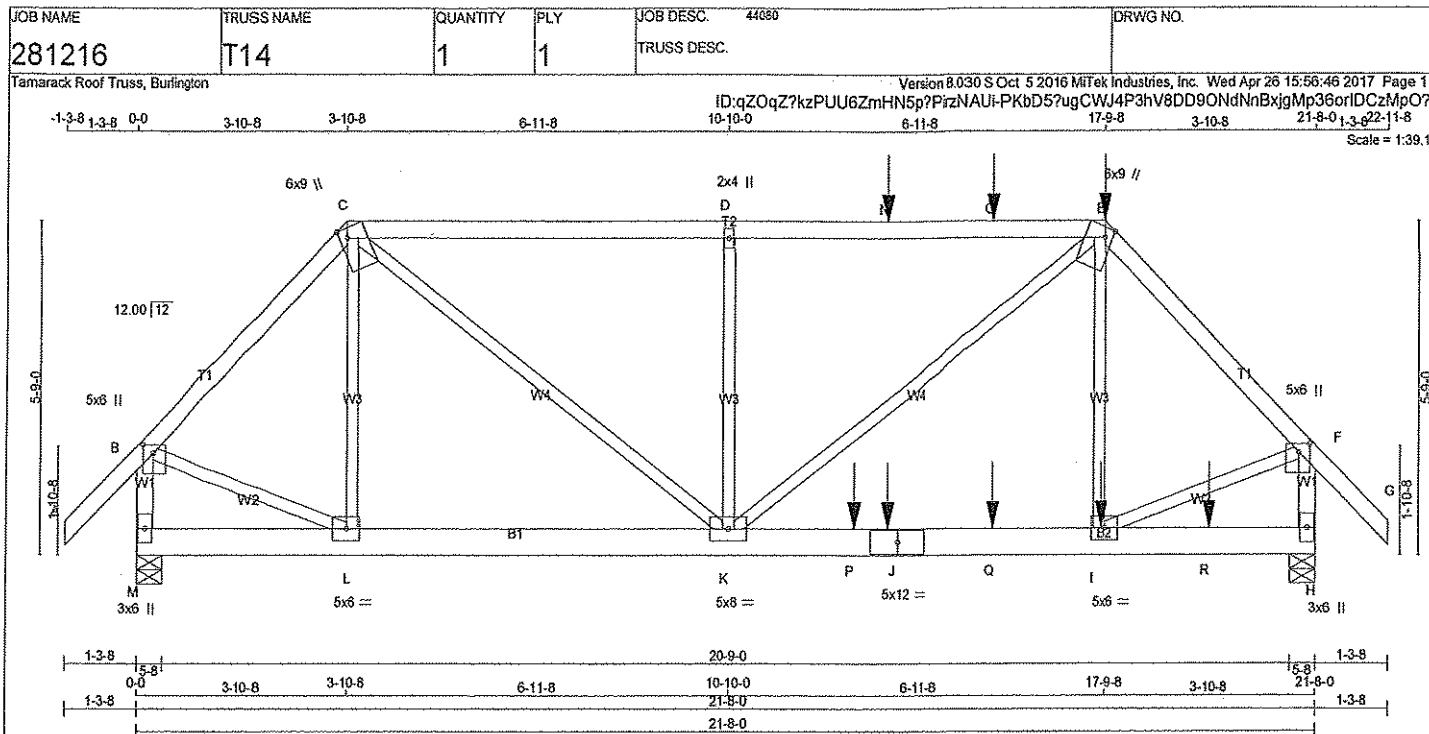
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.42 (F) (INPUT = 1.00)



DWG NO. TAM 1946517
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.25
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMVW+p	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	9.0	Edge	1.75
F	TMVW+p	MT20	5.0	6.0	1.75	2.25
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-I	MT20	5.0	6.0		
J	BS-I	MT20	5.0	12.0		
K	BMWWW-I	MT20	5.0	8.0		
L	BMWW-I	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 72.5 lbs FACTORED DOWN AT 13-8-12, AND 72.5 lbs FACTORED DOWN AT 15-8-12, AND 244.4 lbs FACTORED DOWN AT 17-9-8 ON TOP CHORD, AND 858.4 lbs FACTORED DOWN AT 13-1-8, 42.6 lbs FACTORED DOWN AT 13-8-12, 42.6 lbs FACTORED DOWN AT 15-8-12, AND 42.6 lbs FACTORED DOWN AT 17-8-12, AND 42.6 lbs FACTORED DOWN AT 19-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
M	1700	0	1700	0	5-8	5-8
H	2144	0	2144	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1378	788 / 0	293 / 0	0 / 0	0 / 0	297 / 0	0 / 0
H	1730	1001 / 0	359 / 0	0 / 0	0 / 0	370 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 38	-77.4 -77.4 0.10 (1)	10.00	L-C -306 / 52 0.15 (1)
B-C	-1428 / 0	-77.4 -77.4 0.21 (1)	5.81	C-K 0 / 1585 0.39 (1)
C-D	-2257 / 0	-77.4 -77.4 0.84 (1)	3.91	K-D -734 / 0 0.36 (1)
D-H	-2257 / 0	-77.4 -77.4 0.84 (1)	3.91	K-E 0 / 1047 0.26 (1)
H-O	-2257 / 0	-77.4 -77.4 0.84 (1)	3.91	I-E 0 / 334 0.08 (3)
O-E	-2257 / 0	-77.4 -77.4 0.84 (1)	3.91	B-L 0 / 1069 0.26 (1)
E-F	-2022 / 0	-77.4 -77.4 0.23 (1)	5.08	I-F 0 / 1514 0.37 (1)
F-G	0 / 38	-77.4 -77.4 0.10 (1)	10.00	
M-B	-1635 / 0	0.0 0.0 0.19 (1)	6.43	
H-F	-2208 / 0	0.0 0.0 0.26 (1)	5.87	
M-L	0 / 0	-28.0 -28.0 0.06 (3)	10.00	
L-K	0 / 1000	-28.0 -28.0 0.52 (1)	10.00	
K-P	0 / 1426	-28.0 -28.0 0.90 (1)	10.00	
P-J	0 / 1426	-28.0 -28.0 0.90 (1)	10.00	
J-Q	0 / 1426	-28.0 -28.0 0.90 (1)	10.00	
Q-I	0 / 1426	-28.0 -28.0 0.90 (1)	10.00	
I-R	0 / 0	-28.0 -28.0 0.39 (1)	10.00	
R-H	0 / 0	-28.0 -28.0 0.39 (1)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
E	17-9-8	-244	-244	---	FRONT	VERT	TOTAL
I	17-8-12	-24	-43	---	FRONT	VERT	TOTAL
J	13-8-12	-24	-43	---	FRONT	VERT	TOTAL
N	13-8-12	-73	-73	---	FRONT	VERT	TOTAL
O	15-8-12	-73	-73	---	FRONT	VERT	TOTAL
P	13-1-8	-859	-859	---	FRONT	VERT	TOTAL
Q	15-8-12	-24	-43	---	FRONT	VERT	TOTAL
R	19-8-12	-24	-43	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.72")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.15")
ALLOWABLE DEFL. (TL) = $L/360$ (0.72")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.24")

CSI: TC=0.84 (D-E-1), BC=0.90 (I-K-1), WB=0.39 (C-K-1), SSI=0.46 (I-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

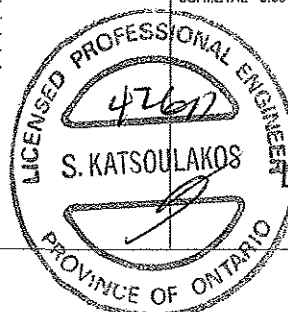
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1687 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.88 (J) (INPUT = 1.00)



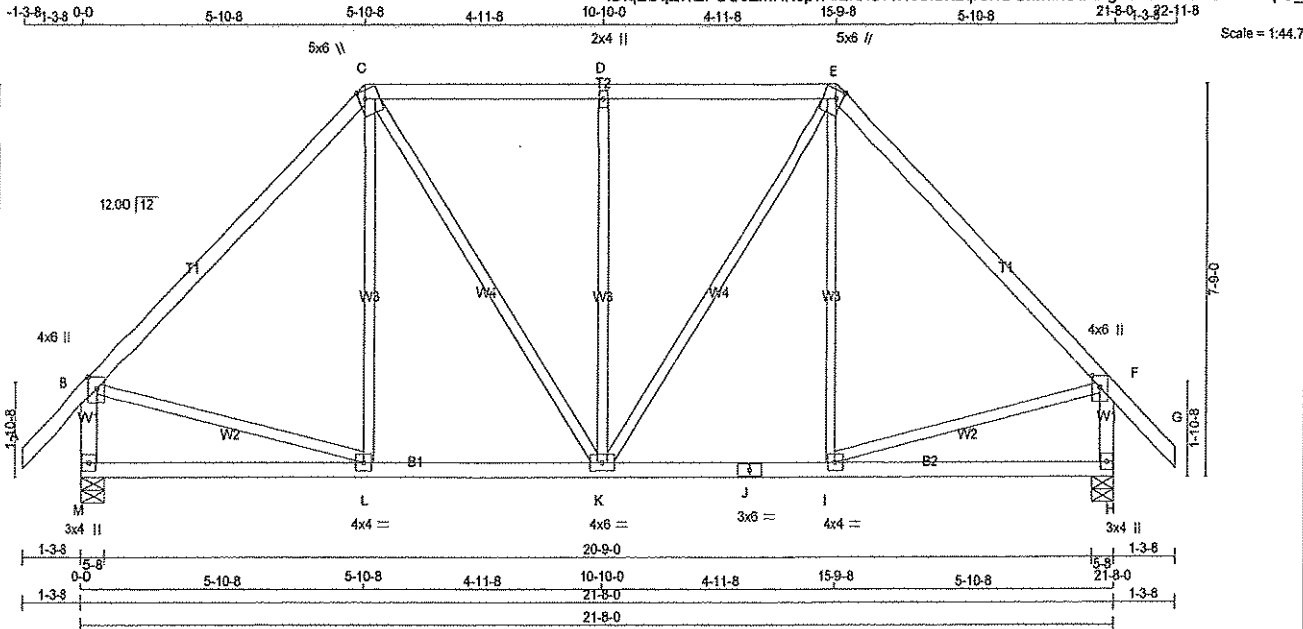
DRWG NO. TAM 19469-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
281216	T15	1	1	TRUSS DESC.	

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ID:qZOqZ?kzPUU6ZmHN5p?PrzNAUHW9bILVlqRx1DGhhwCwaAgubR8Pn6CKSaslezmPo



TOTAL WEIGHT = 104 lb
(M/F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
M	1249	0	1249	0	5-8	5-8
H	1249	0	1249	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
M	1022	570 / 0	228 / 0	0 / 0	0 / 0	225 / 0	0 / 0
H	1022	570 / 0	228 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 38	-77.4 -77.4	0.11 (1)	10.00	L-C	0 / 221	0.05 (3)
B-C	-953 / 0	-77.4 -77.4	0.37 (1)	5.80	C-K	0 / 344	0.08 (1)
C-D	-893 / 0	-77.4 -77.4	0.25 (1)	6.25	K-D	-485 / 0	0.52 (1)
D-E	-893 / 0	-77.4 -77.4	0.25 (1)	6.25	K-E	0 / 344	0.08 (1)
E-F	-953 / 0	-77.4 -77.4	0.37 (1)	5.80	I-E	0 / 221	0.05 (3)
F-G	0 / 38	-77.4 -77.4	0.11 (1)	10.00	B-L	0 / 693	0.16 (1)
M-B	-1182 / 0	0.0 0.0	0.13 (1)	7.34	I-F	0 / 693	0.16 (1)
H-F	-1182 / 0	0.0 0.0	0.13 (1)	7.34			
M-L	0 / 0	-28.0 -28.0	0.23 (3)	10.00			
L-K	0 / 672	-28.0 -28.0	0.31 (2)	10.00			
K-J	0 / 672	-28.0 -28.0	0.31 (2)	10.00			
J-I	0 / 672	-28.0 -28.0	0.31 (2)	10.00			
I-H	0 / 0	-28.0 -28.0	0.23 (3)	10.00			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW+l	MT20	4.0	4.0		
J	BS-l	MT20	3.0	6.0		
K	BMVW+l	MT20	4.0	6.0		
L	BMVW+l	MT20	4.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF
DL =	7.0	PSF	
TOTAL LOAD =	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, ECBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.72")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL)= $L/360$ (0.72")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.37 (E-F:1), BC=0.31 (I-K:2), WB=0.52 (D-K:1), SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.25 (L) (INPUT = 1.00)



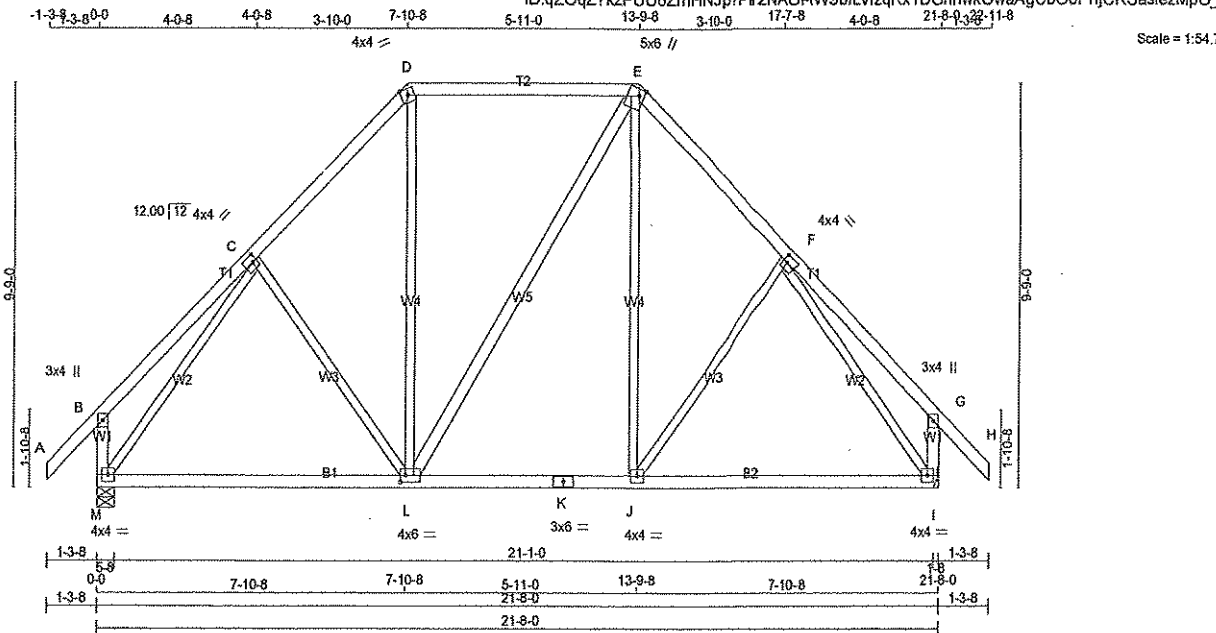
DWG NO. TAM 19470-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T16	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:54.7

TOTAL WEIGHT = 3 X 113 = 339 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
M - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS			
EXCEPT	SIZE	LUMBER	DESCR.
L - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0	Edge	
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMVWW-t	MT20	4.0	6.0	2.00	1.50
M	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
M	1249	0	1249	0	0	5-8	5-8		
I	1249	0	1249	0	0				

UNFACTORED REACTIONS		1ST LOSE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
M	1022	570 / 0	228 / 0	0 / 0	0 / 0
I	1022	570 / 0	228 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
FR-TO	FORCE	FR-TO	FORCE
(LBS)	(LBS)	(LBS)	(LBS)
A-B	0 / 38	C-L	-84 / 62
B-C	0 / 25	L-D	0 / 348
C-D	-905 / 0	L-E	0 / 0
D-E	-523 / 0	J-E	0 / 347
E-F	-804 / 0	J-F	-84 / 62
F-G	0 / 25	M-C	-1148 / 0
G-H	0 / 38	F-I	-1148 / 0
M-B	-223 / 0		
I-G	-223 / 0		
M-L	0 / 670		
L-K	0 / 623		
K-J	0 / 623		
J-I	0 / 670		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.72")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.17")
ALLOWABLE DEFL. (TL) = $L/360$ (0.72")
CALCULATED VERT. DEFL. (TL) = $L/914$ (0.28")

CSI: TC=0.35 (D-E:1), BC=0.45 (I-J:2), WB=0.93 (C-M:1), SSI=0.18 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)



DRWG NO. TAM 19471-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T17	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

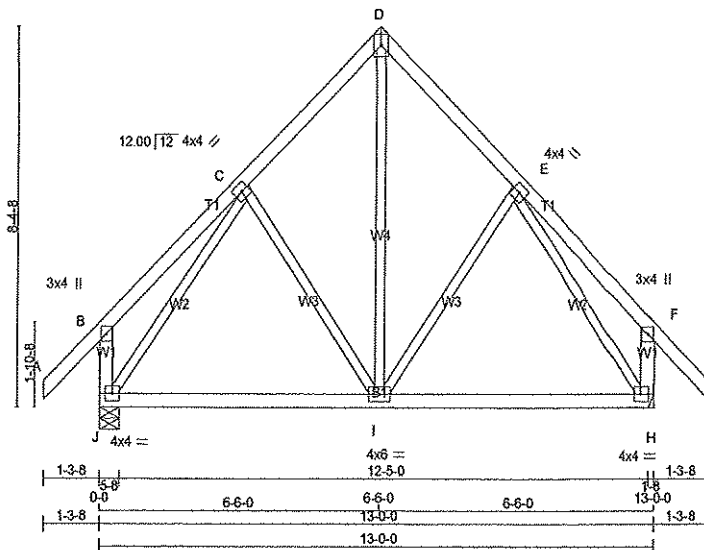
Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Wed Apr 26 15:58:47 2017 Page 1

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1-3-8 3-8 0-0 3-4-4 3-4-4 3-1-12 6-6-0 3-1-12 9-7-12 3-4-4 13-0-0 3-4-4 3-8

4x6 //

Scale = 1:49.8



TOTAL WEIGHT = 2 X 68 = 136 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	793	0	793	0
H	793	0	793	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	643	368 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0
H	643	368 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 38	-77.4	-77.4 0.11 (1)	10.00	I-D	0 / 402	0.09 (2)
B-C	0 / 20	-77.4	-77.4 0.13 (1)	10.00	I-E	-101 / 35	0.06 (1)
C-D	-437 / 0	-77.4	-77.4 0.10 (1)	6.25	C-I	-101 / 35	0.06 (1)
D-E	-437 / 0	-77.4	-77.4 0.10 (1)	6.25	J-C	-629 / 0	0.34 (1)
E-F	0 / 20	-77.4	-77.4 0.13 (1)	10.00	E-H	-629 / 0	0.34 (1)
F-G	0 / 38	-77.4	-77.4 0.11 (1)	10.00			
J-B	-203 / 0	0.0	0.0 0.02 (1)	7.81			
H-F	-203 / 0	0.0	0.0 0.02 (1)	7.81			
J-I	0 / 353	-28.0	-28.0 0.38 (2)	10.00			
I-H	0 / 353	-28.0	-28.0 0.38 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.13 (B-C:1), BC=0.38 (I-J:2), WB=0.34 (E-H:1), SSI=0.15 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)



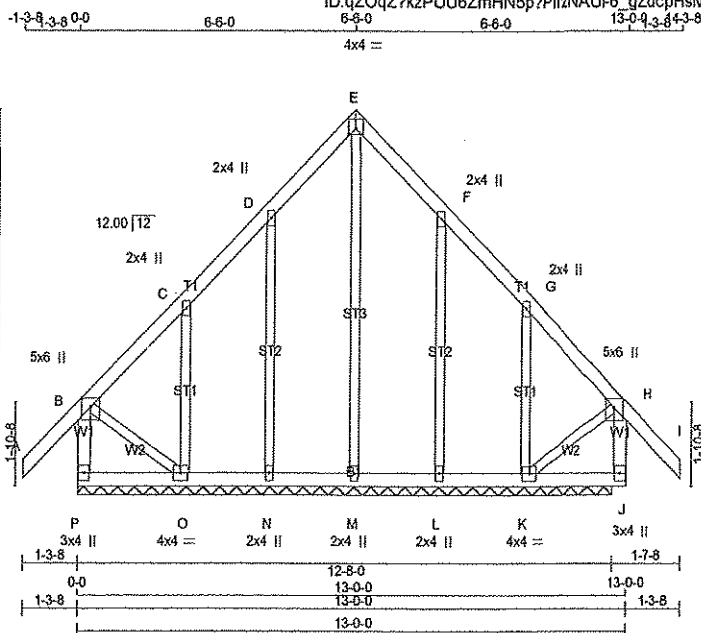
DWG NO. TAM 1947217
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	G17	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:50.5

TOTAL WEIGHT = 70 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV+p	MT20	3.0	4.0		
K	BMWW1-i	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-i	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CSI (LC))	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	LC1 MAX (CSI (LC))
FR-TO				FR-TO			
P-B	0/46	0.0	0.01 (3)	M-E	-393/0	10.00	0.53 (1)
A-B	0/38	-77.4	-77.4 0.11 (1)	N-D	-171/0	10.00	0.11 (1)
B-C	0/187	-77.4	-77.4 0.08 (1)	O-C	-183/0	10.00	0.05 (1)
C-D	0/181	-77.4	-77.4 0.08 (1)	L-F	-161/0	10.00	0.10 (1)
D-E	0/174	-77.4	-77.4 0.08 (1)	K-G	-197/0	10.00	0.08 (1)
E-F	0/175	-77.4	-77.4 0.07 (1)	B-O	-145/0	10.00	0.03 (1)
F-G	0/175	-77.4	-77.4 0.10 (1)	K-H	-145/0	10.00	0.03 (1)
G-H	0/190	-77.4	-77.4 0.10 (1)				
H-I	0/38	-77.4	-77.4 0.11 (1)				
J-H	0/47	0.0	0.0 0.01 (3)				
P-O	0/0	-28.0	-28.0 0.04 (2)				
O-N	-129/0	-28.0	-28.0 0.04 (2)				
N-M	-132/0	-28.0	-28.0 0.03 (2)				
M-L	-132/0	-28.0	-28.0 0.03 (2)				
L-K	-129/0	-28.0	-28.0 0.06 (2)				
K-J	0/0	-28.0	-28.0 0.05 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-08
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (H-I:1), BC=0.06 (K-L:2), WB=0.53 (E-M:1), SS=0.06 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

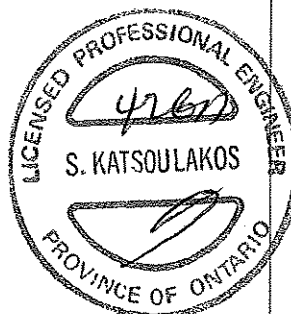
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (E) (INPUT = 0.90)

JSI METAL= 0.08 (E) (INPUT = 1.00)



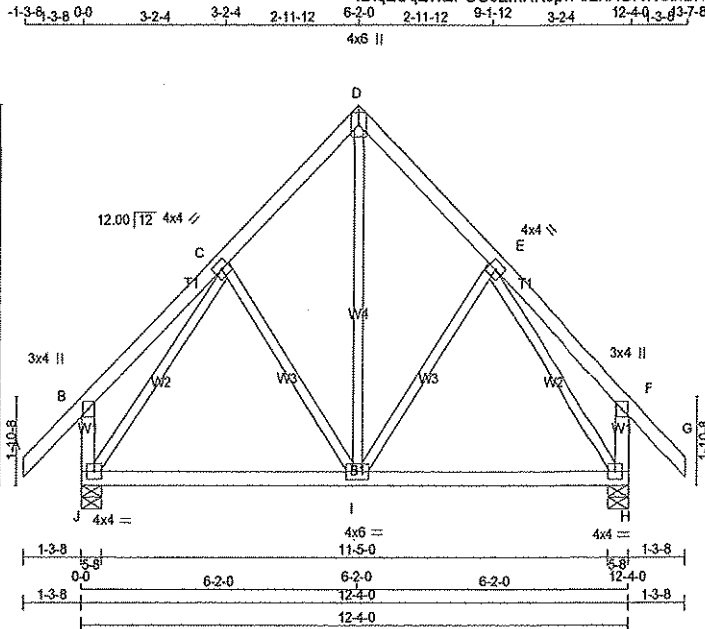
DWG NO. TAMI9473-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44680	DRWG NO.
281216	T18	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale: 1/4"=1'

TOTAL WEIGHT = 3 X 65 = 195 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
J	758	0	758	0	0	5-8	5-8	5-8	5-8
H	758	0	758	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
J	613	352 / 0 130 / 0 0 / 0 0 / 0 132 / 0 0 / 0
H	613	352 / 0 130 / 0 0 / 0 0 / 0 132 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD LC1 MAX (PLF) CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	FR-TO
FR-TO				FR-TO			
A-B	0 / 38	-77.4	-77.4 0.11 (1)	10.00	I-D	0 / 376	0.08 (2)
B-C	0 / 19	-77.4	-77.4 0.11 (1)	10.00	I-E	-91 / 36	0.05 (1)
C-D	-410 / 0	-77.4	-77.4 0.09 (1)	6.25	C-I	-91 / 36	0.05 (1)
D-E	-410 / 0	-77.4	-77.4 0.09 (1)	6.25	J-C	-593 / 0	0.29 (1)
E-F	0 / 19	-77.4	-77.4 0.11 (1)	10.00	E-H	-593 / 0	0.29 (1)
F-G	0 / 38	-77.4	-77.4 0.11 (1)	10.00			
J-B	-198 / 0	0.0	0.0 0.02 (1)	7.81			
H-F	-198 / 0	0.0	0.0 0.02 (1)	7.81			
J-I	0 / 329	-28.0	-28.0 0.35 (2)	10.00			
I-H	0 / 329	-28.0	-28.0 0.35 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.11 (E-F:1), BC=0.35 (I-J:2), WB=0.29 (E-H:1), SSI=0.15 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.22 (C) (INPUT = 1.00)



DWG NO. TAM 19474-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	G18	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

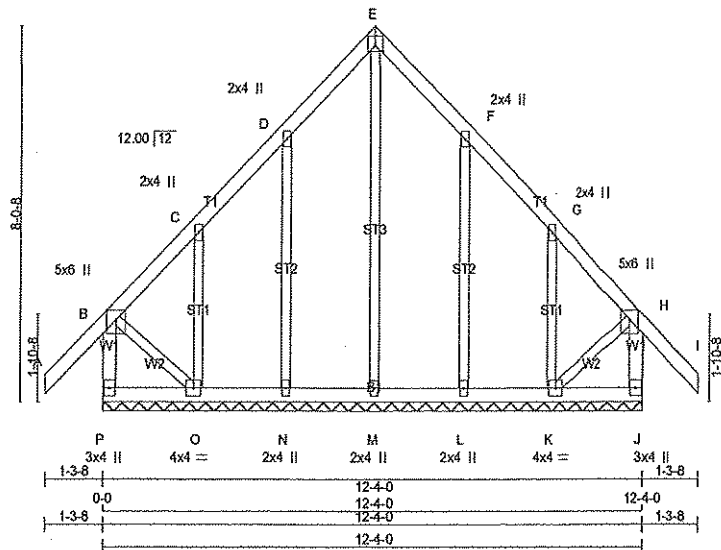
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-1-3-8 0-0 6-2-0 6-2-0 12-4-0 12-4-0 12-4-0 12-4-0

4x4 =

Scale = 1:48.5



TOTAL WEIGHT = 66 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2-0-0	OC.	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C, D, F, G					
C TMW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	4.0	1.50	2.00
H TMVW+p	MT20	5.0	6.0	Edge	
J BMV1+p	MT20	3.0	4.0		
K BMWW1-t	MT20	4.0	4.0		
L, M, N					
L BMW1+w	MT20	2.0	4.0		
O BMWW1-t	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
P-B	-212/0	0.0	0.0	0.02 (1)	7.81	M-E	-101/0	0.12 (1)	
A-B	0/38	-77.4	-77.4	0.11 (1)	10.00	N-D	-170/0	0.10 (1)	
B-C	-22/0	-77.4	-77.4	0.04 (1)	6.25	O-C	-168/0	0.04 (1)	
C-D	-27/0	-77.4	-77.4	0.04 (1)	6.25	L-F	-170/0	0.10 (1)	
D-E	-33/0	-77.4	-77.4	0.04 (1)	6.25	K-G	-168/0	0.04 (1)	
E-F	-33/0	-77.4	-77.4	0.04 (1)	6.25	B-O	0/29	0.01 (1)	
F-G	-27/0	-77.4	-77.4	0.04 (1)	6.25	K-H	0/29	0.01 (1)	
G-H	-22/0	-77.4	-77.4	0.04 (1)	6.25				
H-I	0/38	-77.4	-77.4	0.11 (1)	10.00				
J-H	-212/0	0.0	0.0	0.02 (1)	7.81				
P-O	0/0	-28.0	-28.0	0.03 (2)	10.00				
O-N	0/19	-28.0	-28.0	0.03 (2)	10.00				
N-M	0/16	-28.0	-28.0	0.02 (2)	10.00				
M-L	0/16	-28.0	-28.0	0.02 (2)	10.00				
L-K	0/19	-28.0	-28.0	0.03 (2)	10.00				
K-J	0/0	-28.0	-28.0	0.03 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (K-L:2), WB=0.12 (E-M:1), SSI=0.08 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

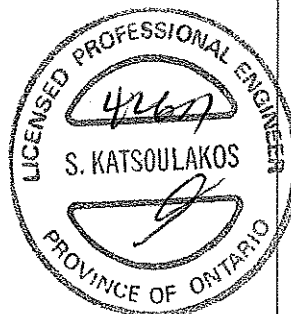
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(FSL)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687
	822	2284	1658

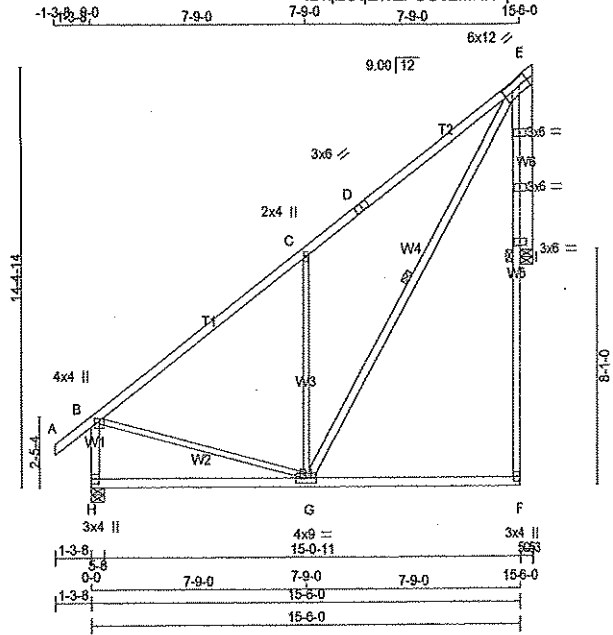
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.20 (E) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)



DWG NO. TAM 1942517
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 8 X 100 = 803 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
H - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF

ALL WEBS EXCEPT

CHORDS	SIZE	LUMBER	DESCR.
G - E	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMVWV1-t	MT20	6.0	12.0	3.00	3.50
F	BMV+p	MT20	3.0	4.0		
G	BMVWV1-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
J(E)	804	0	804	0	5-8 (5-5)	5-8
H	936	0	936	0	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): E

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	668	355 / 0	160 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	762	431 / 0	165 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (L.C.)	MAX. UMBRAC LENGTH	MEMB.	MAX. FACTORED	FORCE (LBS)	MAX CSI (L.C.)
	FR-TO						FR-TO		
A-B	0 / 32	-77.4	-77.4	0.11 (1)	10.00	G-C	-735 / 0	0.98 (1)	
B-C	-597 / 0	-77.4	-77.4	0.88 (1)	5.52	G-E	0 / 991	0.16 (1)	
C-D	-666 / 0	-77.4	-77.4	0.91 (1)	5.30	B-G	0 / 532	0.12 (1)	
D-E	-666 / 0	-77.4	-77.4	0.91 (1)	5.30	E-J	-805 / 0	0.42 (1)	
F-I	0 / 141	0.0	0.0	0.17 (1)	10.00	I-J	0 / 34	0.00 (1)	
I-E	0 / 141	0.0	0.0	0.17 (1)	10.00				
H-B	-852 / 0	0.0	0.0	0.11 (1)	7.81				
H-G	0 / 0	-28.0	-28.0	0.49 (3)	10.00				
G-F	0 / 14	-28.0	-28.0	0.50 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL)= L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL)= L/961 (0.19")

CSI: TC=0.91 (C-E:1), BC=0.50 (F-G:3), WB=0.98 (C-G:1), SSI=0.23 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

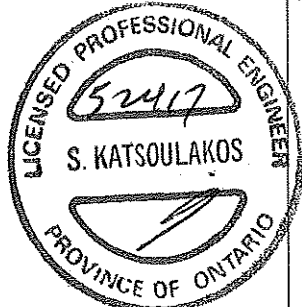
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)



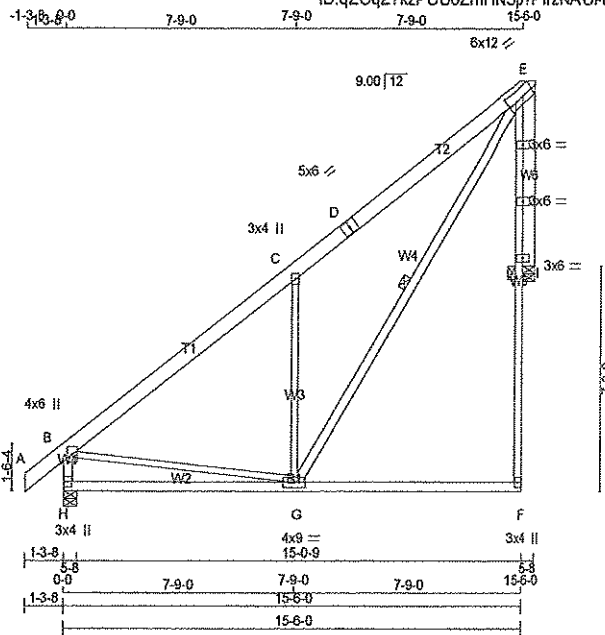
DWG NO. TAM25734-17
STRUCTURAL
COMPONENT ONLY

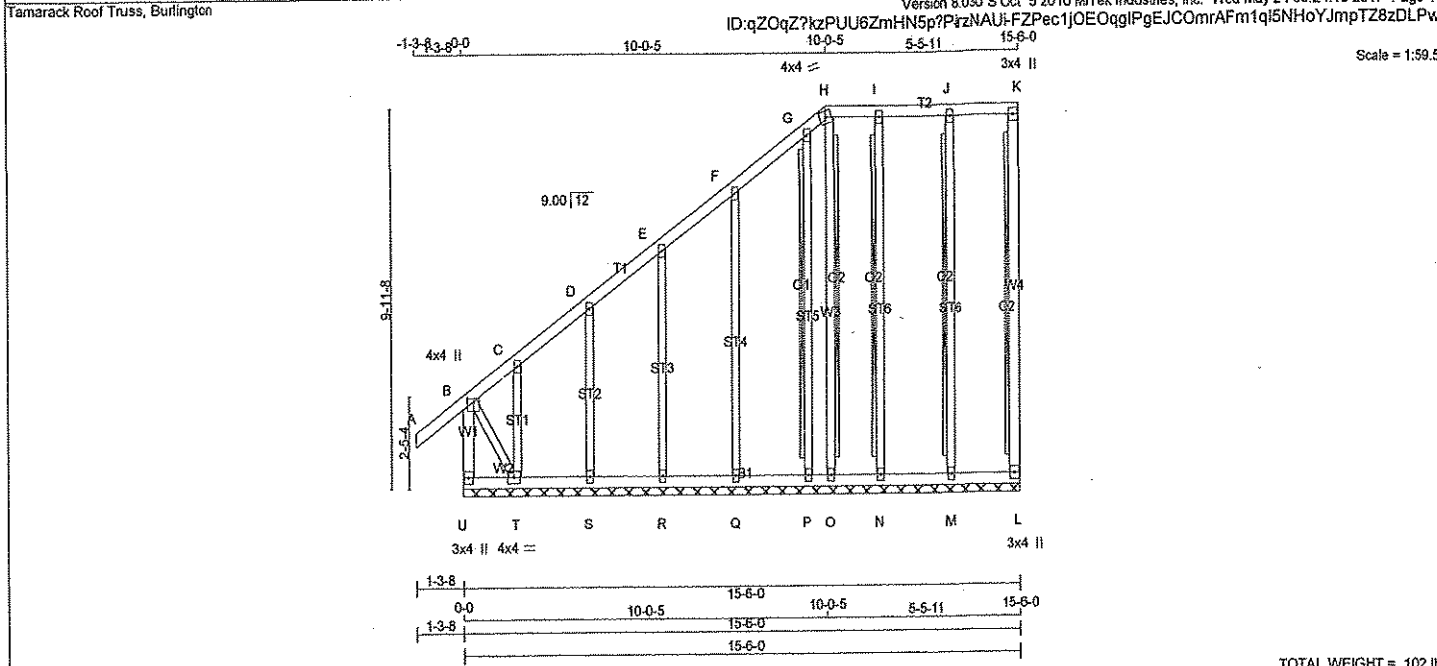
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T20	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS SIZE

LUMBER

DESCR.

SPF

U - B 2x4 DRY No.2

A - H 2x4 DRY No.2

H - K 2x4 DRY No.2

L - K 2x4 DRY No.2

U - L 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

ALL GABLE WEBS 2x3 DRY No.2

DRY; SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE PLATES

B TMVW+p MT20 4.0 4.0 1.00 2.00

C, D, E, F, G, I, J

C TMW+w MT20 2.0 4.0

H TTW+m MT20 4.0 4.0

K TMV+p MT20 3.0 4.0

L BMV1+p MT20 3.0 4.0

M, N, O, P, Q, R, S

M BMW1+w MT20 2.0 4.0

T BMWW1-i MT20 4.0 4.0

U BMV1+p MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT K-L, J-M, I-N, G-P, H-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD (PLF)

LC1 MAX. (LBS)

LC2 MAX. (LBS)

UNBRACED LENGTH (FT)

WEBS

MEMB.

MAX. FACTORED FORCE (LBS)

MAX. FACTORED CSI (LBS)

FR-TO

U-B

A-B

B-C

C-D

D-E

E-F

F-G

G-H

H-I

I-J

J-K

L-K

U-T

T-S

S-R

R-Q

Q-P

P-O

O-N

N-M

M-L

-230 / 0

0 / 32

-41 / 0

-5 / 0

-8 / 0

-3 / 0

-7 / 0

-13 / 0

0 / 0

0 / 0

0 / 0

-63 / 0

0 / 0

0 / 9

0 / 6

0 / 3

0 / 2

0 / 0

0 / 0

0 / 0

0.0

-77.4

-77.4

-77.4

-77.4

-77.4

-77.4

-77.4

-77.4

-77.4

-77.4

0.0

-28.0

-28.0

-28.0

-28.0

-28.0

-28.0

-28.0

-28.0

0.03 (1)

0.11 (1)

0.10 (1)

0.04 (1)

0.04 (1)

0.04 (1)

0.02 (1)

0.03 (1)

0.04 (1)

0.04 (1)

0.04 (1)

0.04 (1)

0.02 (2)

0.02 (2)

0.02 (2)

0.02 (2)

0.02 (2)

0.02 (2)

0.03 (2)

7.81

10.00

6.25

10.00

10.00

10.00

6.25

10.00

10.00

10.00

7.81

10.00

10.00

10.00

10.00

10.00

10.00

10.00

10.00

M-J

N-I

P-G

Q-F

R-E

S-D

T-C

B-T

O-H

-173 / 0

-143 / 0

-113 / 0

-162 / 0

-150 / 0

-164 / 0

-75 / 0

0 / 18

-47 / 0

0.13 (1)

0.11 (1)

0.08 (1)

0.20 (1)

0.11 (1)

0.06 (1)

0.02 (1)

0.00 (1)

0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

CL = 3.0 PSF

BOT CH. LL = 10.5 PSF

CL = 7.0 PSF

TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (M-N:2), WB=0.20 (F-Q:1), SSI=0.07 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

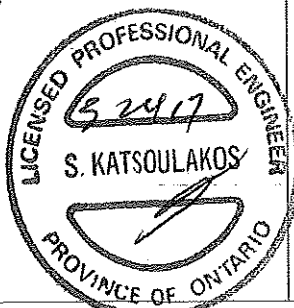
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.60)

JSI METAL= 0.04 (D) (INPUT = 1.00)



DRWG NO. TAM25736-17

STRUCTURAL

COMPONENT ONLY

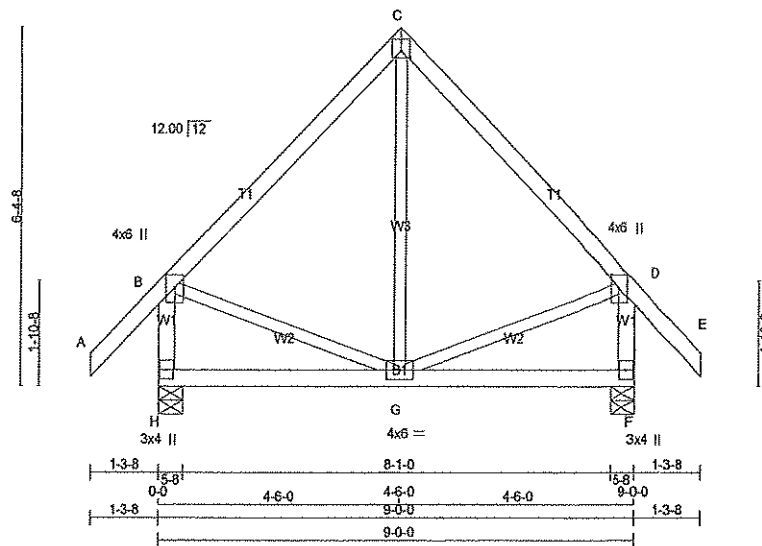
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	T23	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:40.3



TOTAL WEIGHT = 3 X 45 = 134 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00	
C	TTW+p	MT20	4.0	4.0	1.50	2.00	
D	TMVW+p	MT20	4.0	6.0	2.75	2.00	
F	BMV1+p	MT20	3.0	4.0			
G	BMVWWV-1	MT20	4.0	6.0			
H	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REORD
GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
JT	VERT	HORZ	DOWN	HORZ
H	582	0	582	0
F	582	0	582	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	467	275 / 0	95 / 0
F	467	275 / 0	95 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE	VERT.	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE
FR-TO								
A-B	0 / 38	-77.4	-77.4	0.11 (1)	10.00	G-C	0 / 199	0.04 (3)
B-C	-266 / 0	-77.4	-77.4	0.20 (1)	6.25	B-G	0 / 199	0.04 (1)
C-D	-266 / 0	-77.4	-77.4	0.20 (1)	6.25	G-D	0 / 199	0.04 (1)
D-E	0 / 38	-77.4	-77.4	0.11 (1)	10.00			
H-B	-534 / 0	0.0	0.0	0.06 (1)	7.81			
F-D	-534 / 0	0.0	0.0	0.06 (1)	7.81			
H-G	0 / 0	-28.0	-28.0	0.17 (3)	10.00			
G-F	0 / 0	-28.0	-28.0	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.20 (B-C:1), BC=0.17 (G-H:3), WB=0.04 (D-G:1), SSI=0.11 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)



DWG NO. TAM 1948017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	G23	1	1	TRUSS DESC.		

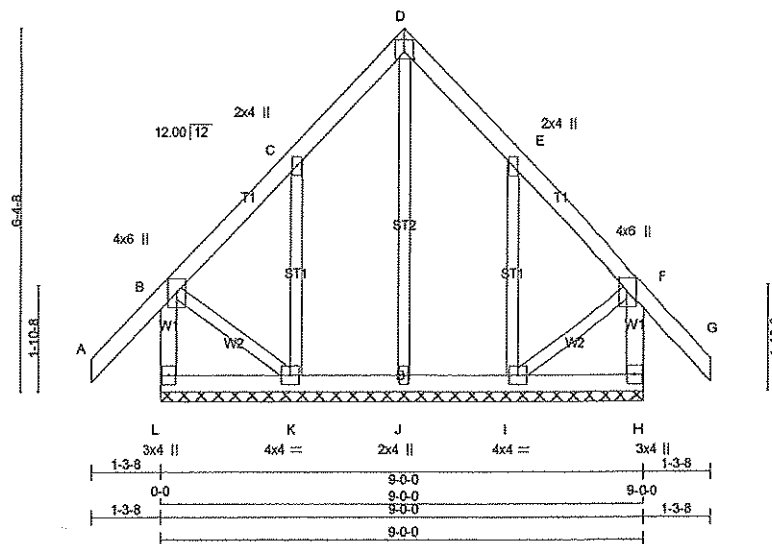
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Apr 26 15:56:40 2017 Page 1

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-1-3-8 1-3-8 0-0 4-6-0 4-6-0 9-0-0 1-3-8 10-3-8
4x4 =

Scale = 1:39.8



TOTAL WEIGHT = 48 lb

LUMBER				DESCR.	
CHORDS	SIZE	DRY	LUMBER	SPF	
L - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 2-0-0 OC.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	4.0	1.50	2.00
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW1-I	MT20	4.0	4.0		
J	BMVW1+w	MT20	2.0	4.0		
K	BMVW1-I	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
L-B	-207 / 0	0.0	0.0 0.02 (1)	J-D	-95 / 0	0.06 (1)	0.06 (1)
A-B	0 / 38	-77.4	-77.4 0.11 (1)	K-C	-211 / 0	0.06 (1)	0.06 (1)
B-C	-9 / 0	-77.4	-77.4 0.07 (1)	I-E	-211 / 0	0.06 (1)	0.06 (1)
C-D	-32 / 0	-77.4	-77.4 0.07 (1)	B-K	0 / 19	0.00 (1)	0.00 (1)
D-E	-32 / 0	-77.4	-77.4 0.07 (1)	I-F	0 / 19	0.00 (1)	0.00 (1)
E-F	-9 / 0	-77.4	-77.4 0.07 (1)				
F-G	0 / 38	-77.4	-77.4 0.11 (1)				
H-F	-207 / 0	0.0	0.0 0.02 (1)				
L-K	0 / 0	-28.0	-28.0 0.04 (3)				
K-J	0 / 11	-28.0	-28.0 0.04 (2)				
J-I	0 / 11	-28.0	-28.0 0.04 (2)				
I-H	0 / 0	-28.0	-28.0 0.04 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA C86-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.04 (J-K:2), WB=0.06 (C-K:1), SSI=0.06 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

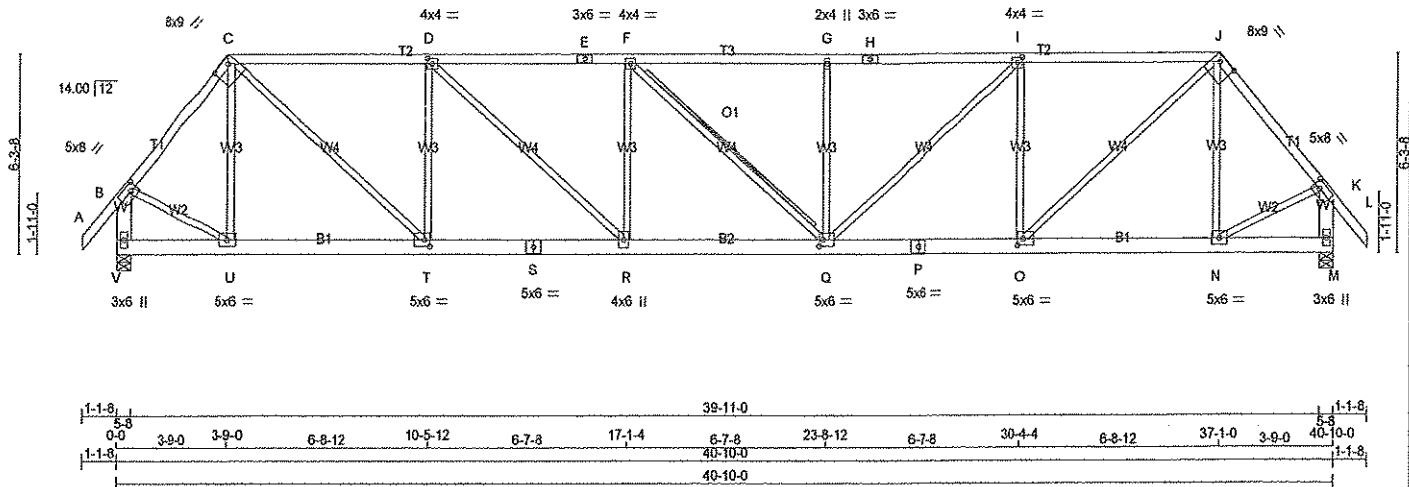
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (D) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)



DWG NO. TAM19481-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 202 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E
C - E	2x4	DRY	1650F 1.5E
E - H	2x4	DRY	1650F 1.5E
H - J	2x4	DRY	1650F 1.5E
J - L	2x4	DRY	1650F 1.5E
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	8.0	2.50	2.50
C	TTWW-h	MT20	8.0	9.0	2.00	6.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TTWW-h	MT20	8.0	9.0	2.00	6.25
K	TMWW-t	MT20	5.0	8.0	2.50	2.50
M	BMV1+p	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.00
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0	2.50	1.50
R	BMWW-t	MT20	4.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.00
U	BMWW-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
V	2247	0	2247	0	5-8
M	2247	0	2247	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0
M	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 36	U-C	-302 / 29
B-C	-1984 / 0	C-T	0 / 2173
C-D	-2902 / 0	T-D	-1249 / 0
D-E	-3668 / 0	D-R	0 / 1026
E-F	-3668 / 0	R-F	-495 / 0
F-G	-3663 / 0	F-Q	-6 / 0
G-H	-3663 / 0	Q-G	-496 / 0
H-I	-3663 / 0	Q-I	0 / 1018
I-J	-2904 / 0	O-I	-1248 / 0
J-K	-1984 / 0	O-J	0 / 2174
K-L	0 / 36	N-J	-304 / 29
V-B	-2221 / 0	B-U	0 / 1360
M-K	-2220 / 0	N-K	0 / 1360
V-U	0 / 0		
U-T	0 / 1266		
T-S	0 / 2903		
S-R	0 / 2903		
R-Q	0 / 3668		
Q-P	0 / 2904		
P-O	0 / 2904		
O-N	0 / 1266		
N-M	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, AIBC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")

CSI: TC=0.67 (G-I), BC=0.50 (Q-R), WB=0.74 (D-T), SSI=0.24 (I-J)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

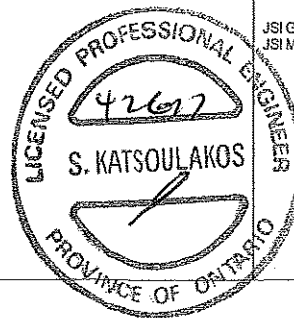
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

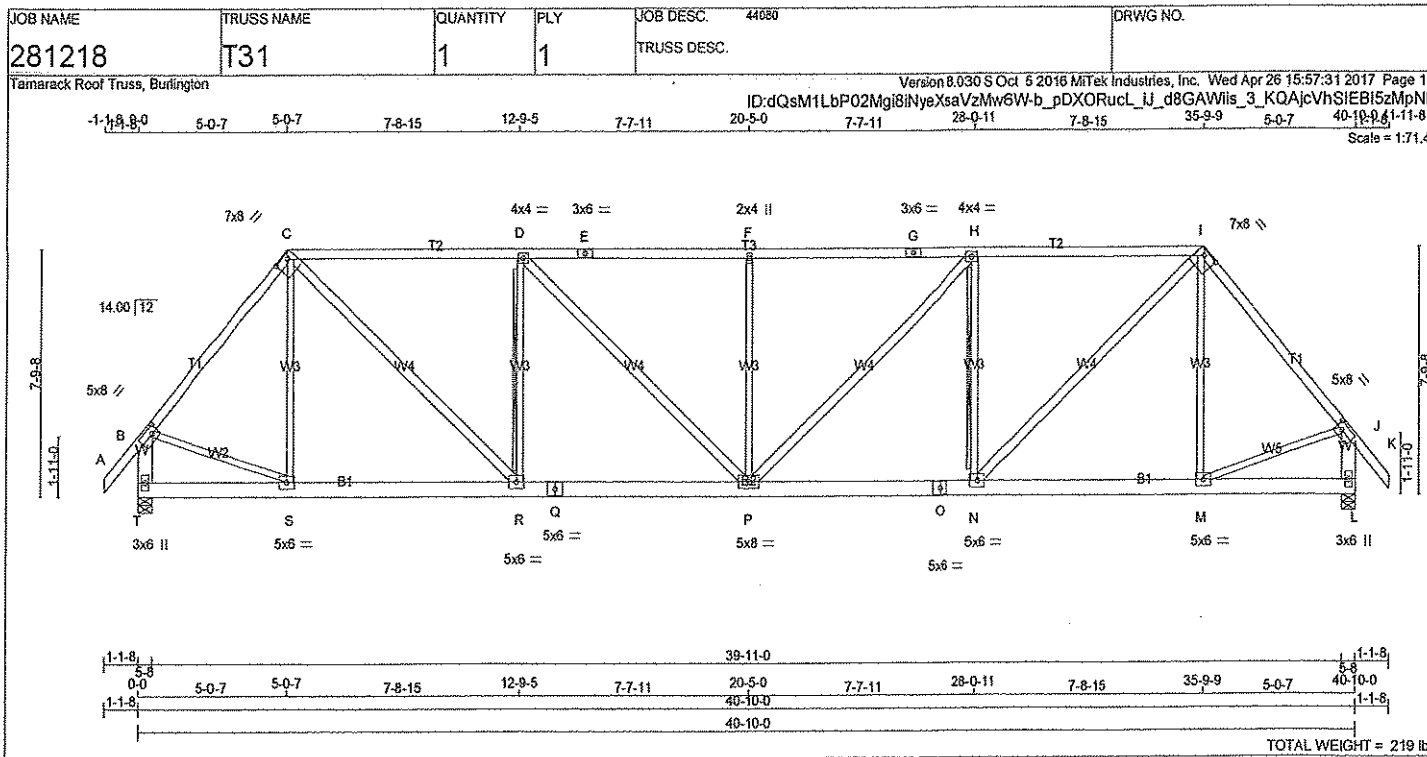
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.51 (P) (INPUT = 1.00)



DWG NO. TAM19488-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E
C - E	2x4	DRY	1650F 1.5E
E - G	2x4	DRY	1650F 1.5E
G - I	2x4	DRY	1650F 1.5E
I - K	2x4	DRY	1650F 1.5E
T - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
T - Q	2x6	DRY	No.2
Q - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
C - R	2x4	DRY	No.2
D - P	2x4	DRY	No.2
P - H	2x4	DRY	No.2
N - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-I	MT20	5.0	8.0 2.50 2.50
C	TTWW-h	MT20	7.0	8.0 1.75 5.00
D	TMVW-I	MT20	4.0	4.0
E	TS-I	MT20	3.0	6.0
F	TMVW-w	MT20	2.0	4.0
G	TS-I	MT20	3.0	6.0
H	TMVW-I	MT20	4.0	4.0
I	TTWW-h	MT20	7.0	8.0 1.75 5.00
J	TMVW-I	MT20	5.0	8.0 2.50 2.50
L	BMV1+p	MT20	3.0	6.0
M, N, R, S				
O	BMVW-I	MT20	5.0	6.0
P	BS-I	MT20	5.0	6.0
Q	BS-I	MT20	5.0	6.0
T	BMV1+p	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
T	2247	0	0	2247	0	0	5-8	5-8	5-8
L	2247	0	0	2247	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0
L	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-R, H-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 36	-77.4 -77.4	0.07 (1)	10.00	S-C	-173 / 142	0.18 (1)
B-C	-2015 / 0	-77.4 -77.4	0.40 (1)	4.97	C-R	0 / 1829	0.29 (1)
C-D	-2628 / 0	-77.4 -77.4	0.82 (1)	3.96	R-D	-1041 / 0	0.87 (1)
D-E	-3014 / 0	-77.4 -77.4	0.87 (1)	3.71	D-P	0 / 537	0.09 (1)
E-F	-3014 / 0	-77.4 -77.4	0.87 (1)	3.71	P-F	-545 / 0	0.58 (1)
F-G	-3014 / 0	-77.4 -77.4	0.87 (1)	3.71	P-H	0 / 537	0.09 (1)
G-H	-3014 / 0	-77.4 -77.4	0.87 (1)	3.71	H-N	-1041 / 0	0.87 (1)
H-I	-2628 / 0	-77.4 -77.4	0.82 (1)	3.96	N-I	0 / 1829	0.29 (1)
I-J	-2015 / 0	-77.4 -77.4	0.40 (1)	4.97	M-I	-173 / 142	0.18 (1)
J-K	0 / 36	-77.4 -77.4	0.07 (1)	10.00	B-S	0 / 1358	0.31 (1)
T-B	-2197 / 0	0.0 0.0	0.16 (1)	6.88	M-J	0 / 1358	0.31 (1)
L-J	-2197 / 0	0.0 0.0	0.16 (1)	6.88			
T-S	0 / 0	-28.0 -28.0	0.13 (2)	10.00			
S-R	0 / 1304	-28.0 -28.0	0.28 (2)	10.00			
R-Q	0 / 2628	-28.0 -28.0	0.39 (1)	10.00			
Q-P	0 / 2628	-28.0 -28.0	0.39 (1)	10.00			
P-O	0 / 2628	-28.0 -28.0	0.39 (1)	10.00			
O-N	0 / 2628	-28.0 -28.0	0.39 (1)	10.00			
N-M	0 / 1304	-28.0 -28.0	0.28 (2)	10.00			
M-L	0 / 0	-28.0 -28.0	0.13 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.87 (D-F:1), BC=0.39 (N-P:1), WB=0.87 (D-R:1), SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

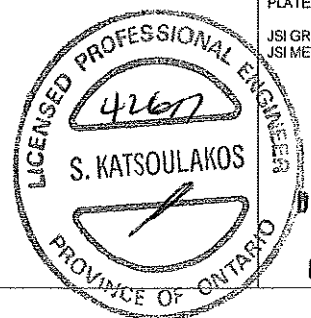
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.83 (M) (INPUT = 0.90)
JSI METAL= 0.42 (J) (INPUT = 1.00)



DRWG NO. 7AM (9489)-17
STRUCTURAL
COMPONENT ONLY

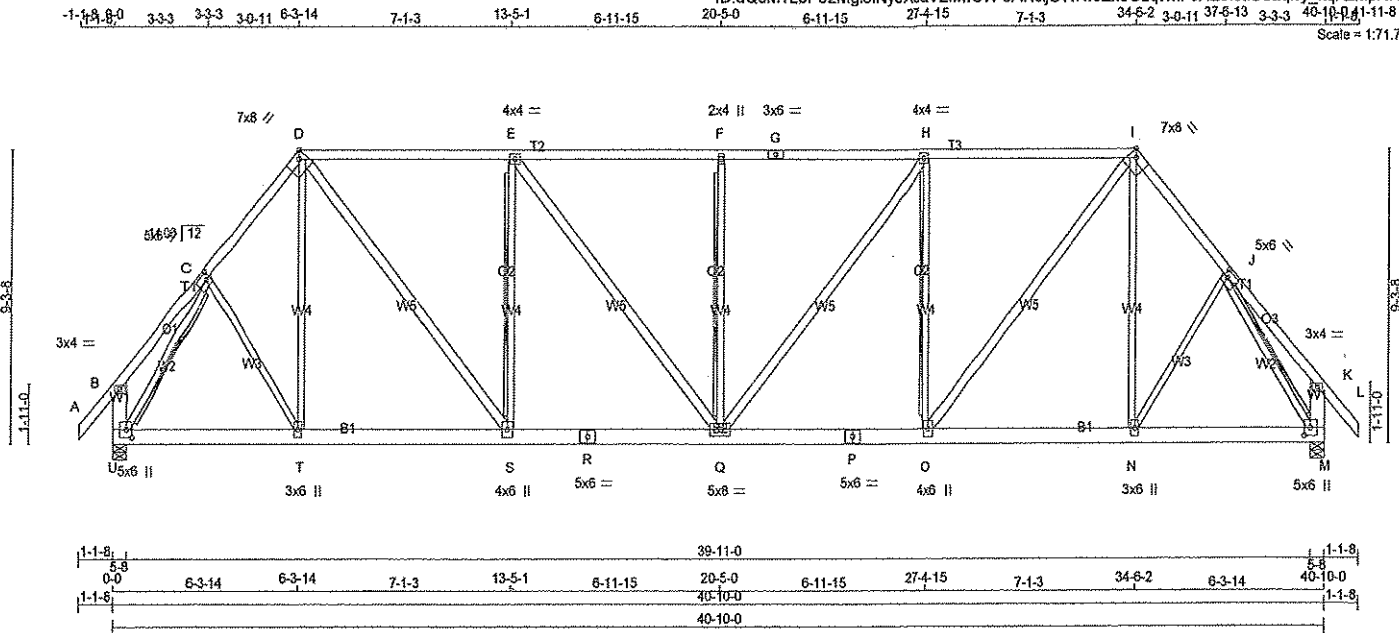
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44088	DRWG NO.
281218	T32	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: dQsM1LbP02Mgi8iNyeXsaVzWw6W-3ANcljSWNfGZx8CLqt1x3F3EekniS6aqhy_kqXzMpNH

Scale = 1:71.7



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - S	2x4	DRY	No.2
E - Q	2x4	DRY	No.2
Q - H	2x4	DRY	No.2
O - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW-h	MT20	7.0	8.0	Edge	2.75
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTWW-h	MT20	7.0	8.0	Edge	2.75
J	TMVW-t	MT20	5.0	6.0	2.50	1.75
K	TMV-p	MT20	3.0	4.0		
M	BMVW1+p	MT20	5.0	6.0	3.00	2.25
N	BMVW-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMVW-t	MT20	4.0	6.0		
T	BMVW-t	MT20	3.0	6.0		
U	BMVW1+p	MT20	5.0	6.0	3.00	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	DOWN	UPLIFT	BRG
U	2247	0	2247	0	5-8
M	2247	0	2247	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
U	1853	1008 / 0	429 / 0
M	1853	1008 / 0	429 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-S, H-O, C-U, J-M
2x3 DRY SPF No.2 T-BRACE AT F-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX	MEMB.	MAX. FACTORED	MAX	MAX
	FORCE	VERT. LOAD	CS1 (LC)		FORCE	CS1 (LC)	CS1 (LC)
FR-TO	(LBS)	FROM TO	LENGTH	FR-TO	(LBS)	LENGTH	LENGTH
A-B	0 / 36	-77.4	-77.4 0.09 (1)	10.00	C-T	0 / 202	0.05 (2)
B-C	0 / 18	-77.4	-77.4 0.10 (1)	10.00	T-D	0 / 216	0.05 (3)
C-D	-2039 / 0	-77.4	-77.4 0.17 (1)	4.61	D-S	0 / 1479	0.24 (1)
D-E	-2231 / 0	-77.4	-77.4 0.83 (1)	3.67	S-E	-952 / 0	0.58 (1)
E-F	-2499 / 0	-77.4	-77.4 0.68 (1)	3.45	E-Q	0 / 433	0.07 (1)
F-G	-2499 / 0	-77.4	-77.4 0.88 (1)	3.45	Q-F	-498 / 0	0.69 (1)
G-H	-2499 / 0	-77.4	-77.4 0.88 (1)	3.45	Q-H	0 / 433	0.07 (1)
H-I	-2231 / 0	-77.4	-77.4 0.83 (1)	3.67	O-H	-952 / 0	0.58 (1)
I-J	-2039 / 0	-77.4	-77.4 0.17 (1)	4.61	O-I	0 / 1479	0.24 (1)
J-K	0 / 18	-77.4	-77.4 0.10 (1)	10.00	N-I	0 / 216	0.05 (3)
K-L	0 / 36	-77.4	-77.4 0.09 (1)	10.00	N-J	0 / 202	0.05 (2)
U-B	-192 / 0	0.0	0.0 0.01 (1)	7.81	U-C	-2322 / 0	0.61 (1)
M-K	-192 / 0	0.0	0.0 0.01 (1)	7.81	J-M	-2322 / 0	0.61 (1)
U-T	0 / 1210	-28.0	-28.0 0.24 (2)	10.00			
T-S	0 / 1308	-28.0	-28.0 0.26 (2)	10.00			
S-R	0 / 2232	-28.0	-28.0 0.34 (1)	10.00			
R-Q	0 / 2232	-28.0	-28.0 0.34 (1)	10.00			
Q-P	0 / 2232	-28.0	-28.0 0.34 (1)	10.00			
P-O	0 / 2232	-28.0	-28.0 0.34 (1)	10.00			
O-N	0 / 1308	-28.0	-28.0 0.26 (2)	10.00			
N-M	0 / 1210	-28.0	-28.0 0.24 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.88 (F-H:1), EC=0.34 (O-Q:1), WB=0.69 (F-Q:1), SSI=0.26 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

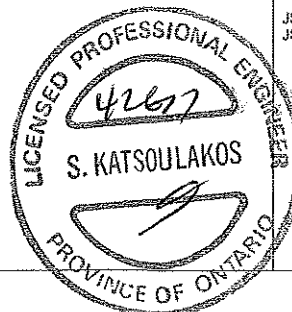
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

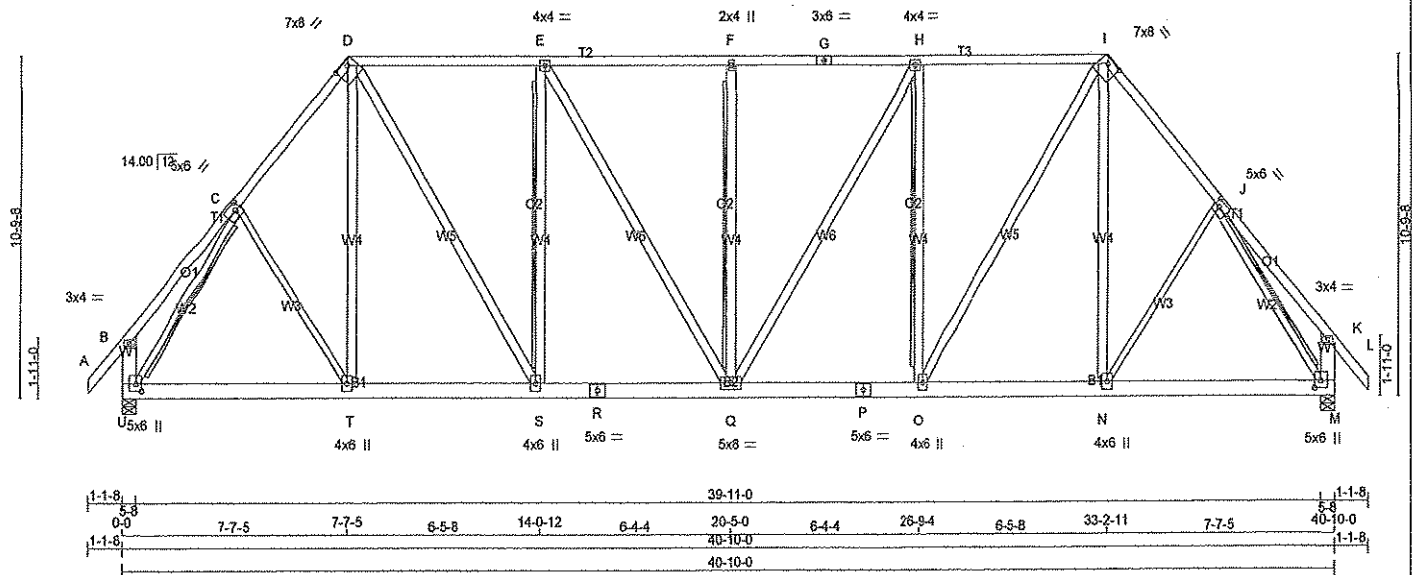
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)



DWG NO. TAM 19490-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281218	TRUSS NAME T33	QUANTITY 1	PLY 1	JOB DESC. 44080	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.030 S Oct. 5 2016 MiTek Industries, Inc. Wed Apr 26 15:57:32 2017 Page 1	
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Scale = 1:71.7					



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
C - T	2x3	DRY	No.2
N - J	2x3	DRY	No.2
U - C	2x3	DRY	No.2
J - M	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV-p	MT20	3.0	4.0	
C TMWW-t	MT20	5.0	6.0	2.50 2.00
D TTWW-h	MT20	7.0	8.0	1.75 5.00
E TMWW-t	MT20	4.0	4.0	
F TMWW-w	MT20	2.0	4.0	
G TS-t	MT20	3.0	6.0	
H TMWW-t	MT20	4.0	4.0	
I TTWW-h	MT20	7.0	8.0	1.75 5.00
J TMWW-t	MT20	5.0	6.0	2.50 2.00
K TMV-p	MT20	3.0	4.0	
M BMVVW1+p	MT20	5.0	6.0	2.75 2.25
N, O, S, T				
N BMWW-t	MT20	4.0	6.0	
P BS-t	MT20	5.0	6.0	
Q BMWW-t	MT20	5.0	6.0	
R BS-t	MT20	5.0	6.0	
U BMVVW1+p	MT20	5.0	6.0	2.75 2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED	MAXIMUM FACTORED	INPUT
	GROSS REACTION	GROSS REACTION	BRG
JT	VERT	HORZ	DOWN
U	2247	0	2247
M	2247	0	2247

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1853	1008/0	429/0	0/0	0/0	416/0	0/0
M	1853	1008/0	429/0	0/0	0/0	416/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-S, F-Q, H-O, C-U, J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0/36	-77.4	-77.4 0.09 (1)	10.00	C-T	0/141	0.03 (3)
B-C	0/24	-77.4	-77.4 0.17 (1)	10.00	T-D	0/284	0.05 (3)
C-D	-2025/0	-77.4	-77.4 0.26 (1)	4.55	D-S	0/1223	0.20 (1)
D-E	-1943/0	-77.4	-77.4 0.62 (1)	4.15	S-E	-867/0	0.65 (1)
E-F	-2134/0	-77.4	-77.4 0.64 (1)	3.97	E-Q	0/365	0.06 (1)
F-G	-2134/0	-77.4	-77.4 0.64 (1)	3.97	Q-F	-452/0	0.34 (1)
G-H	-2134/0	-77.4	-77.4 0.64 (1)	3.97	Q-H	0/365	0.06 (1)
H-I	-1943/0	-77.4	-77.4 0.62 (1)	4.15	O-H	-867/0	0.65 (1)
I-J	-2025/0	-77.4	-77.4 0.26 (1)	4.55	O-I	0/1223	0.20 (1)
J-K	0/24	-77.4	-77.4 0.17 (1)	10.00	N-I	0/284	0.05 (3)
K-L	0/36	-77.4	-77.4 0.09 (1)	10.00	N-J	0/141	0.03 (3)
U-B	-208/0	0.0	0.0 0.02 (1)	7.81	U-C	-2315/0	0.79 (1)
M-K	-208/0	0.0	0.0 0.02 (1)	7.81	J-M	-2315/0	0.79 (1)
U-T	0/1251	-28.0	-28.0 0.28 (2)	10.00			
T-S	0/1298	-28.0	-28.0 0.29 (2)	10.00			
S-R	0/1944	-28.0	-28.0 0.28 (1)	10.00			
R-Q	0/1944	-28.0	-28.0 0.28 (1)	10.00			
Q-P	0/1944	-28.0	-28.0 0.28 (1)	10.00			
P-O	0/1944	-28.0	-28.0 0.28 (1)	10.00			
O-N	0/1298	-28.0	-28.0 0.29 (2)	10.00			
N-M	0/1251	-28.0	-28.0 0.28 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.12")
ALLOWABLE DEFL. (TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.20")

CSI: TC=0.64 (E-F:1), BC=0.29 (N-O:2), WB=0.79 (J-M:1), SSI=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

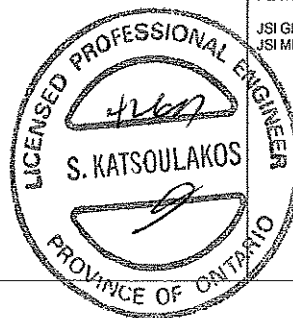
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1867
	822	2284	1656

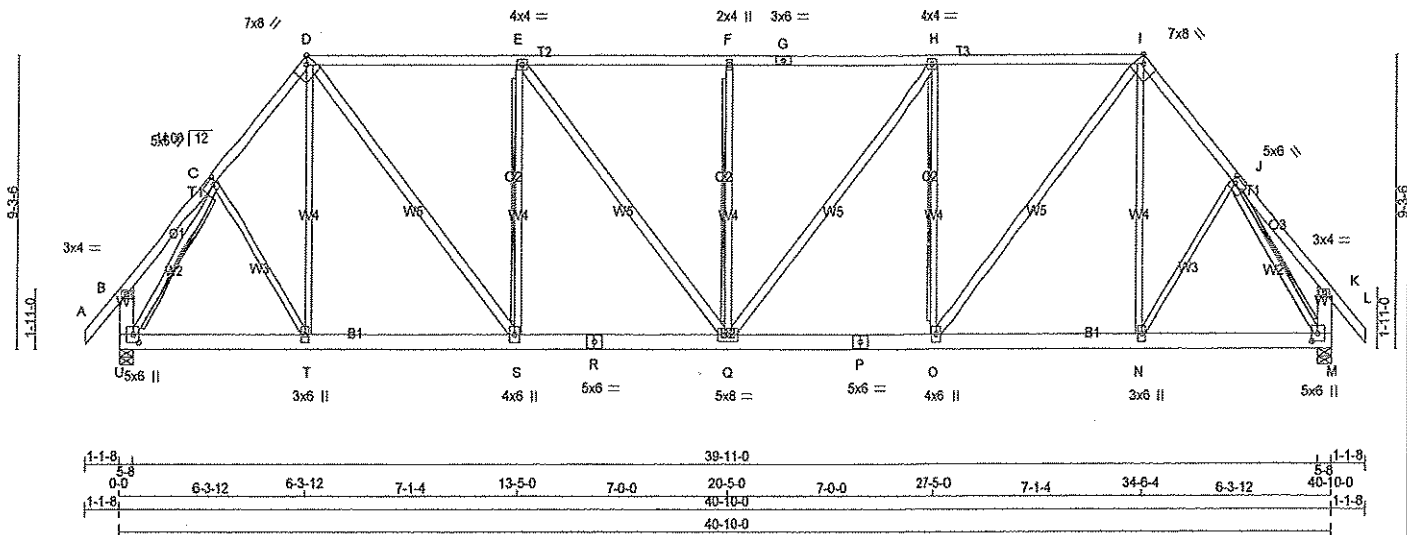
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.57 (C) (INPUT = 1.00)



DWG NO. TAM/9491-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - S	2x4	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMWW-h	MT20	5.0	6.0	2.50	1.75
D	TTWW-h	MT20	7.0	8.0	Edge	2.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW-w	MT20	2.0	4.0		
G	TS-l	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-h	MT20	7.0	8.0	Edge	2.75
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TMV-p	MT20	3.0	4.0		
M	BMVW1+p	MT20	5.0	6.0	3.00	2.25
N	BMWWH	MT20	3.0	6.0		
O	BMWWH	MT20	4.0	6.0		
P	BS-l	MT20	5.0	6.0		
Q	BMWWH-t	MT20	5.0	8.0		
R	BS-l	MT20	5.0	6.0		
S	BMWWH	MT20	4.0	6.0		
T	BMWWH	MT20	3.0	6.0		
U	BMVW1+p	MT20	5.0	6.0	3.00	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
U	2247	0	0	5-8
M	2247	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERM LIVE WIND DEAD SOIL
U	1853	1008 / 0	429 / 0 0 / 0 0 / 0 416 / 0 0 / 0
M	1853	1008 / 0	429 / 0 0 / 0 0 / 0 416 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-S, H-Q, C-U, J-M
2x3 DRY SPF No.2 T-BRACE AT F-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 36	-77.4	-77.4	0.09 (1)	C-T	0 / 203	0.05 (2)
B-C	0 / 18	-77.4	-77.4	0.10 (1)	T-D	0 / 215	0.05 (3)
C-D	-2039 / 0	-77.4	-77.4	0.17 (1)	D-S	0 / 1481	0.24 (1)
D-E	-2234 / 0	-77.4	-77.4	0.83 (1)	S-E	-953 / 0	0.58 (1)
E-F	-2502 / 0	-77.4	-77.4	0.88 (1)	E-Q	0 / 433	0.07 (1)
F-G	-2502 / 0	-77.4	-77.4	0.88 (1)	Q-F	-499 / 0	0.69 (1)
G-H	-2502 / 0	-77.4	-77.4	0.88 (1)	Q-H	0 / 433	0.07 (1)
H-I	-2234 / 0	-77.4	-77.4	0.83 (1)	O-H	-953 / 0	0.58 (1)
I-J	-2039 / 0	-77.4	-77.4	0.17 (1)	O-I	0 / 1481	0.24 (1)
J-K	0 / 18	-77.4	-77.4	0.10 (1)	N-I	0 / 215	0.05 (3)
K-L	0 / 36	-77.4	-77.4	0.09 (1)	N-J	0 / 203	0.05 (2)
U-B	-192 / 0	0.0	0.0	0.01 (1)	U-C	-2322 / 0	0.61 (1)
M-K	-192 / 0	0.0	0.0	0.01 (1)	J-M	-2322 / 0	0.61 (1)
U-T	0 / 1210	-28.0	-28.0	0.24 (2)			
T-S	0 / 1308	-28.0	-28.0	0.26 (2)			
S-R	0 / 2234	-28.0	-28.0	0.34 (1)			
R-Q	0 / 2234	-28.0	-28.0	0.34 (1)			
Q-P	0 / 2234	-28.0	-28.0	0.34 (1)			
P-O	0 / 2234	-28.0	-28.0	0.34 (1)			
O-N	0 / 1308	-28.0	-28.0	0.26 (2)			
N-M	0 / 1210	-28.0	-28.0	0.24 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.14")
ALLOWABLE DEFL. (TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.23")

CSI: TC=0.88 (F-H-1), BC=0.34 (O-Q-1), WB=0.89 (F-Q-1), SSI=0.26 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

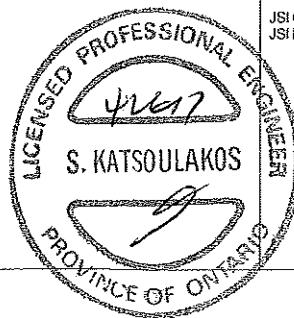
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.55 (C) (INPUT = 1.00)



DRWG NO. TAM 19492-17
STRUCTURAL COMPONENT ONLY

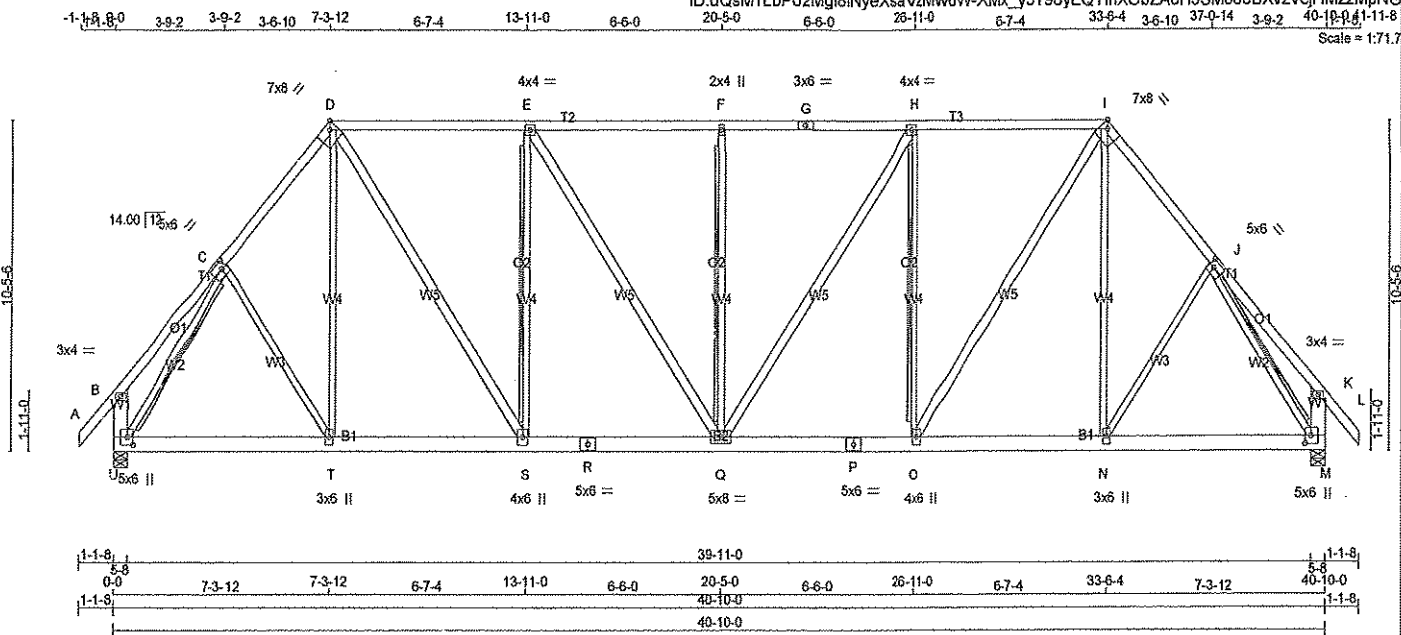
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T35	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: dQsM1LbP02Mg8iNyeXsaVzWw6W-XMx_y3T98yEQYInXObZAoH3SM888BxvzvcjHMzzMpNG

Scale = 1:71.7



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - S	2x4	DRY	No.2
E - Q	2x4	DRY	No.2
Q - H	2x4	DRY	No.2
O - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	8.0	2.50	2.00
D	TTVW-h	MT20	7.0	8.0	Edge	2.75
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTVW-h	MT20	7.0	8.0	Edge	2.75
J	TMVW-t	MT20	5.0	6.0	2.50	2.00
K	TMV-p	MT20	3.0	4.0		
M	BMVW1+p	MT20	5.0	6.0	3.00	2.25
N	BMVW-h	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMVW-h	MT20	4.0	6.0		
T	BMVW-t	MT20	3.0	6.0		
U	BMVW1+p	MT20	5.0	6.0	3.00	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORZ	UPLIFT
U 2247	0	2247	0
M 2247	0	2247	0

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM LIVE
U 1853	1008 / 0	429 / 0	0 / 0
M 1853	1008 / 0	429 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-S, F-Q, H-O, C-U, J-M

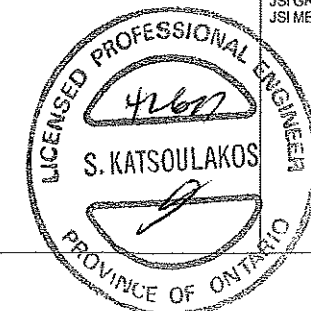
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)
FR-TO			
A-B	0 / 36	-77.4	-77.4 0.09 (1)
B-C	0 / 23	-77.4	-77.4 0.15 (1)
C-D	-2029 / 0	-77.4	-77.4 0.24 (1)
D-E	-2003 / 0	-77.4	-77.4 0.66 (1)
E-F	-2207 / 0	-77.4	-77.4 0.69 (1)
F-G	-2207 / 0	-77.4	-77.4 0.69 (1)
G-H	-2207 / 0	-77.4	-77.4 0.69 (1)
H-I	-2003 / 0	-77.4	-77.4 0.66 (1)
I-J	-2029 / 0	-77.4	-77.4 0.24 (1)
J-K	0 / 23	-77.4	-77.4 0.15 (1)
K-L	0 / 36	-77.4	-77.4 0.09 (1)
U-B	-205 / 0	0.0	0.0 0.01 (1)
M-K	-205 / 0	0.0	0.0 0.01 (1)
U-T	0 / 1243	-28.0	-28.0 0.27 (2)
T-S	0 / 1301	-28.0	-28.0 0.28 (2)
S-R	0 / 2003	-28.0	-28.0 0.30 (1)
R-Q	0 / 2003	-28.0	-28.0 0.30 (1)
Q-P	0 / 2003	-28.0	-28.0 0.30 (1)
P-O	0 / 2003	-28.0	-28.0 0.30 (1)
O-N	0 / 1301	-28.0	-28.0 0.28 (2)
N-M	0 / 1243	-28.0	-28.0 0.27 (2)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	=	23.3
BOT CH.	LL	PSF
DL	=	10.5
DL	=	7.0
TOTAL LOAD	=	43.8

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (1.36")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.13")
ALLOWABLE DEFL. (TL) = 1/360 (1.36")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.21")

CSI: TC=0.69 (E-F:1), BC=0.30 (Q-S:1), WB=0.75 (J-M:1), SSI=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

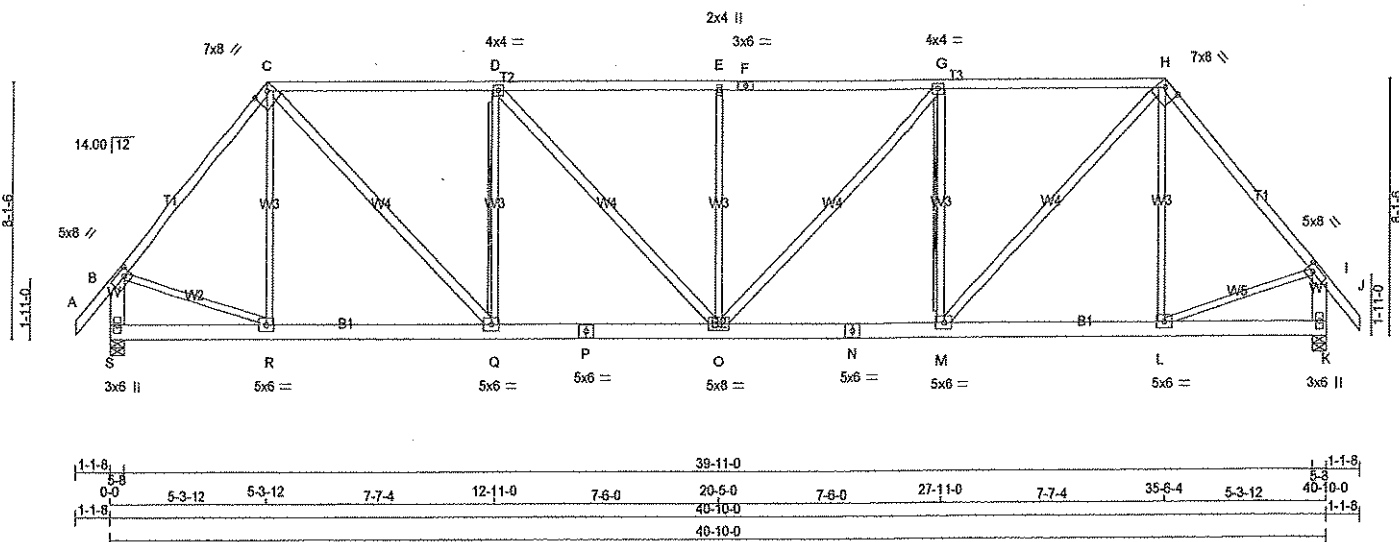
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.57 (C) (INPUT = 1.00)

DWG NO. TAM 19493-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281218	TRUSS NAME T37	QUANTITY 1	PLY 1	JOB DESC. 44688	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Apr 26 15:57:33 2017 Page 1	
ID: dQsM1LbP02MgI8iNyeXsaVzWw6W-XMx_y3T98yEQYnXObZAoH3QR87sBUezvcjHMzzMpNG				40-18.0 1-11.8	
Scale = 1:71.4					



TOTAL WEIGHT = 222 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	1650F 1.5E
C - F	2x4	DRY	1650F 1.5E
F - H	2x4	DRY	1650F 1.5E
H - J	2x4	DRY	1650F 1.5E
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
C - Q	2x4	DRY	No.2
D - O	2x4	DRY	No.2
O - G	2x4	DRY	No.2
M - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMVW-I	MT20	5.0	8.0 2.50 2.50
C TTWW-h	MT20	7.0	8.0 2.00 5.25
D TMVW-I	MT20	4.0	4.0
E TMVW-w	MT20	2.0	4.0
F TS-I	MT20	3.0	6.0
G TMVW-I	MT20	4.0	4.0
H TTWW-h	MT20	7.0	8.0 2.00 5.25
I TMVW-t	MT20	5.0	8.0 2.50 2.50
K BMV1+p	MT20	3.0	6.0
L, M, Q, R			
N BMVW-I	MT20	5.0	6.0
O BS-I	MT20	5.0	6.0
P BS-III	MT20	5.0	6.0
S BMV1+p	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORZ	UPLIFT
S 2247	0	2247	0
K 2247	0	2247	0

UNFACTORED REACTIONS			
1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM.LIVE
S 1853	1008 / 0	429 / 0	0 / 0
K 1853	1008 / 0	429 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-Q, G-M

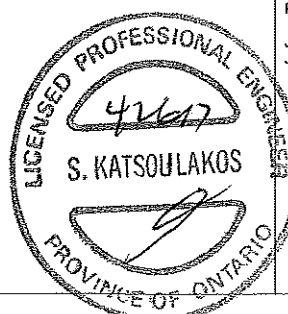
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)
FR-TO			
A-B	0 / 36	-77.4	-77.4 0.07 (1)
B-C	-2019 / 0	-77.4	-77.4 0.45 (1)
C-D	-2530 / 0	-77.4	-77.4 0.77 (1)
D-E	-2886 / 0	-77.4	-77.4 0.81 (1)
E-F	-2886 / 0	-77.4	-77.4 0.81 (1)
F-G	-2886 / 0	-77.4	-77.4 0.81 (1)
G-H	-2530 / 0	-77.4	-77.4 0.77 (1)
H-I	-2019 / 0	-77.4	-77.4 0.45 (1)
I-J	0 / 36	-77.4	-77.4 0.07 (1)
S-B	-2192 / 0	0.0	0.0 0.16 (1)
K-I	-2192 / 0	0.0	0.0 0.16 (1)
S-R	0 / 0	-28.0	-28.0 0.13 (3)
R-Q	0 / 1307	-28.0	-28.0 0.27 (2)
Q-P	0 / 2530	-28.0	-28.0 0.38 (1)
P-O	0 / 2530	-28.0	-28.0 0.38 (1)
O-N	0 / 2530	-28.0	-28.0 0.38 (1)
N-M	0 / 2530	-28.0	-28.0 0.38 (1)
M-L	0 / 1307	-28.0	-28.0 0.27 (2)
L-K	0 / 0	-28.0	-28.0 0.13 (3)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.81 (E-G:1), BC=0.38 (M-O:1), WB=0.96 (D-Q:1), SS=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.45 (N) (INPUT = 1.00)

DWG NO. FAM19494-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T38	1	1	TRUSS DESC.		

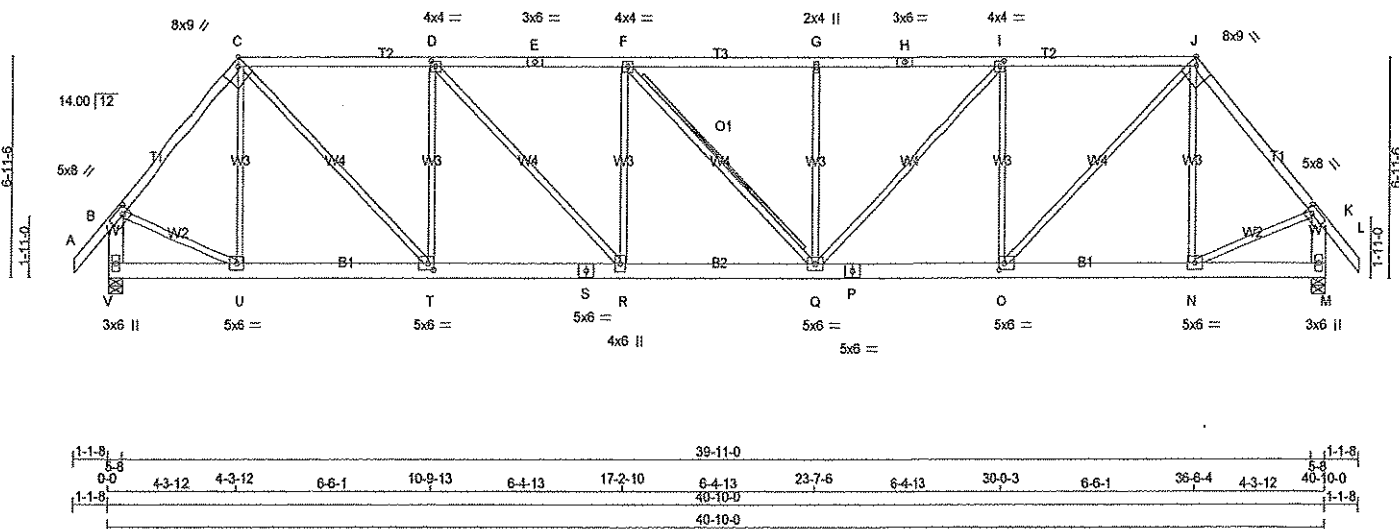
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Apr 26 15:57:34 2017 Page 1

ID: dQsM1LbP02Mgi8NyeXsaVzWw6W-7ZVMAPtnvGMGASmXl4PKUcacYRxxwJ78GTruQzMpNF

-1-1-8 4-3-12 4-3-12 6-6-1 10-9-13 6-4-13 17-2-10 6-4-13 23-7-6 6-4-13 30-0-3 6-6-1 36-6-4 4-3-12 40-10-0 1-1-8

Scale = 1:71.4



TOTAL WEIGHT = 208 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	8.0	2.50	2.50
C	TTWW-h	MT20	8.0	9.0	Edge	2.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TTWW-h	MT20	8.0	9.0	Edge	2.75
K	TMWW-t	MT20	5.0	8.0	2.50	2.50
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.25
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-h	MT20	4.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.25
U	BMWW-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
V	2247	0	2247	0
M	2247	0	2247	0

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0
M	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 36	-77.4	-77.4 0.09 (1)	10.00	U-C	-249 / 63	0.19 (1)
B-C	-1991 / 0	-77.4	-77.4 0.37 (1)	4.47	C-T	0 / 1951	0.44 (1)
C-D	-2662 / 0	-77.4	-77.4 0.73 (1)	3.57	T-D	-1207 / 0	0.93 (1)
D-E	-3305 / 0	-77.4	-77.4 0.84 (1)	3.16	D-R	0 / 921	0.21 (1)
E-F	-3305 / 0	-77.4	-77.4 0.84 (1)	3.16	R-F	-478 / 0	0.37 (1)
F-G	-3300 / 0	-77.4	-77.4 0.69 (1)	3.38	F-Q	-6 / 0	0.00 (1)
G-H	-3300 / 0	-77.4	-77.4 0.85 (1)	3.16	Q-G	-479 / 0	0.37 (1)
H-I	-3300 / 0	-77.4	-77.4 0.85 (1)	3.16	I-Q	0 / 913	0.21 (1)
I-J	-2663 / 0	-77.4	-77.4 0.73 (1)	3.56	Q-I	-1203 / 0	0.93 (1)
J-K	-1991 / 0	-77.4	-77.4 0.37 (1)	4.47	O-J	0 / 1953	0.44 (1)
K-L	0 / 36	-77.4	-77.4 0.09 (1)	10.00	N-J	-251 / 63	0.19 (1)
V-B	-2207 / 0	0.0	0.0 0.16 (1)	6.87	B-U	0 / 1359	0.31 (1)
M-K	-2207 / 0	0.0	0.0 0.16 (1)	6.87	N-K	0 / 1359	0.31 (1)
V-U	0 / 0	-28.0	-28.0 0.10 (2)	10.00			
U-T	0 / 1288	-28.0	-28.0 0.22 (2)	10.00			
T-S	0 / 2662	-28.0	-28.0 0.37 (1)	10.00			
S-R	0 / 2662	-28.0	-28.0 0.37 (1)	10.00			
R-Q	0 / 3305	-28.0	-28.0 0.45 (1)	10.00			
Q-P	0 / 2664	-28.0	-28.0 0.37 (1)	10.00			
P-O	0 / 2664	-28.0	-28.0 0.37 (1)	10.00			
O-N	0 / 1288	-28.0	-28.0 0.22 (2)	10.00			
N-M	0 / 0	-28.0	-28.0 0.10 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.22")
ALLOWABLE DEFL.(TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.36")

CSI: TC=0.85 (G-I:1), BC=0.45 (Q-R:1), WB=0.93 (D-T:1), SSI=0.24 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

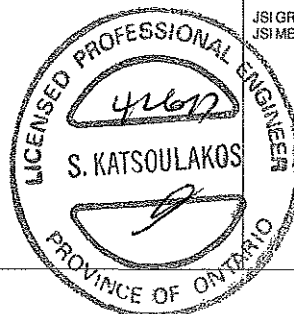
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.43 (P) (INPUT = 1.00)



DWG NO. TAM 19495-17
STRUCTURAL
COMPONENT ONLY

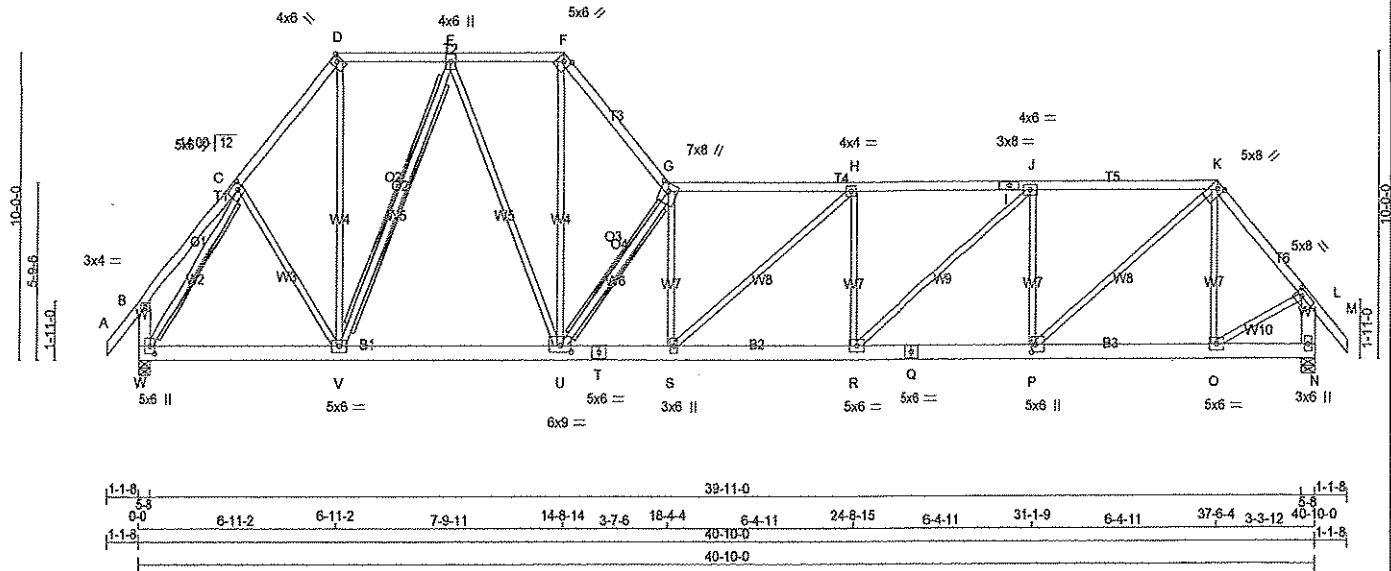
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T39	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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1-1-8 3-6-13 3-6-13 3-4-5 6-11-2 3-10-14 10-10-0 3-10-14 14-8-14 3-7-6 18-4-4 6-4-11 24-8-15 6-4-11 31-1-9 6-4-11 37-6-4 3-3-12 40-10-0 1-1-8
Scale = 1:73.9



TOTAL WEIGHT = 223 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
K - M	2x4	DRY	No.2
W - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
W - T	2x6	DRY	No.2
T - Q	2x6	DRY	No.2
Q - N	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV-p	MT20	3.0	4.0		
C TMWW-I	MT20	5.0	6.0	2.50	2.00
D TTW+h	MT20	4.0	6.0	Edge	
E TMWW+I	MT20	4.0	6.0		
F TTW+h	MT20	5.0	6.0	Edge	
G TTWW+m	MT20	7.0	8.0	3.00	3.25
H TMWW-I	MT20	4.0	4.0		
I TS-I	MT20	3.0	8.0		
J TMWW-I	MT20	4.0	6.0		
K TTWW+h	MT20	5.0	8.0	2.00	2.25
L TMWW-I	MT20	5.0	8.0	2.25	2.50
N BMV+I+p	MT20	3.0	6.0		
O BMWW-I	MT20	5.0	6.0		
P BMWW+I	MT20	5.0	6.0	3.00	1.50
Q BS-I	MT20	5.0	6.0		
R BMWW-I	MT20	5.0	6.0		
S BMWW+I	MT20	3.0	6.0		
T BS-I	MT20	5.0	6.0		
U BMWW+I	MT20	6.0	9.0	2.50	4.50
V BMWW+I	MT20	5.0	6.0		
W BMWW+I+p	MT20	5.0	6.0	3.00	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	UPLIFT
W	2247	0	5-8
N	2247	0	5-8

UNFACTORED REACTIONS						
1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
JT COMBINED	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0
N	1853	1008 / 0	429 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 I-BRACE AT E-V, G-U
2x4 DRY SPF No.2 T-BRACE AT C-W

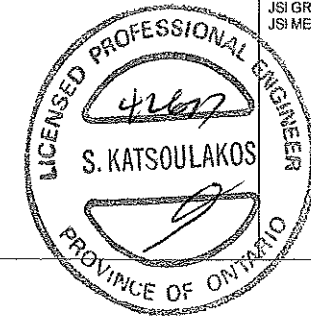
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO			
A-B	0 / 36	-77.4	-77.4 0.09 (1)
B-C	-1 / 19	-77.4	-77.4 0.12 (1)
C-D	-2037 / 0	-77.4	-77.4 0.20 (1)
D-E	-1320 / 0	-77.4	-77.4 0.17 (1)
E-F	-2112 / 0	-77.4	-77.4 0.21 (1)
F-G	-3210 / 0	-77.4	-77.4 0.37 (1)
G-H	-4036 / 0	-77.4	-77.4 0.92 (1)
H-I	-3948 / 0	-77.4	-77.4 0.91 (1)
I-J	-3948 / 0	-77.4	-77.4 0.91 (1)
J-K	-3007 / 0	-77.4	-77.4 0.72 (1)
K-L	-1930 / 0	-77.4	-77.4 0.21 (1)
L-M	0 / 36	-77.4	-77.4 0.09 (1)
W-B	-202 / 0	0.0	0.0 0.01 (1)
N-L	-2230 / 0	0.0	0.0 0.16 (1)
W-V	0 / 1232	-28.0	-28.0 0.29 (2)
U-U	0 / 1789	-28.0	-28.0 0.34 (2)
V-U	0 / 4036	-28.0	-28.0 0.53 (1)
T-S	0 / 4036	-28.0	-28.0 0.53 (1)
S-R	0 / 3948	-28.0	-28.0 0.57 (1)
R-Q	0 / 3008	-28.0	-28.0 0.41 (1)
Q-P	0 / 3008	-28.0	-28.0 0.41 (1)
P-O	0 / 1242	-28.0	-28.0 0.22 (1)
O-N	0 / 0	-28.0	-28.0 0.10 (2)



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN./C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.33")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/901 (0.54")

CSI: TC=0.92 (G-H:1), BC=0.57 (R-S:1), WB=0.93 (G-U:1), SSI=0.23 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.99 (C) (INPUT = 0.99)
JSI METAL= 0.78 (G) (INPUT = 1.00)

DRWG NO. TAM 19496-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T40	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:dQsM1LbP02Mgi8iNyeXsaVzMw8W-TI3kNIUPgaU7obwwV0beti9uBxmZfO7GNwCORszMpNE

PLATES (table is in inches)

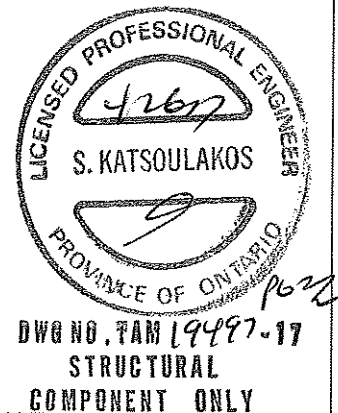
JT	TYPE	PLATES	W	LEN	Y	X
J	TMVW-t	MT20	5.0	8.0	2.00	2.50
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	1.75
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	4.0	4.0	2.50	2.00
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	12.0	2.50	3.75
T	BMWW-t	MT20	4.0	6.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 93.1 lbs FACTORED DOWN AT 36-10-12 ON TOP CHORD, AND 2660.1 lbs FACTORED DOWN AT 34-11-8, AND 70.0 lbs FACTORED DOWN AT 36-10-12, AND 70.0 lbs FACTORED DOWN AT 38-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	36-10-12	-40	-70	--	FRONT	VERT	TOTAL
X	38-10-12	-40	-70	--	FRONT	VERT	TOTAL



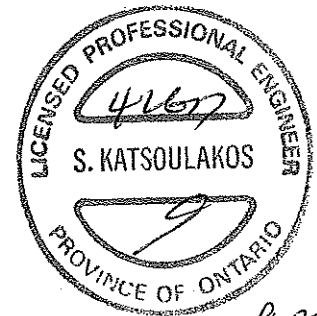
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T41	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

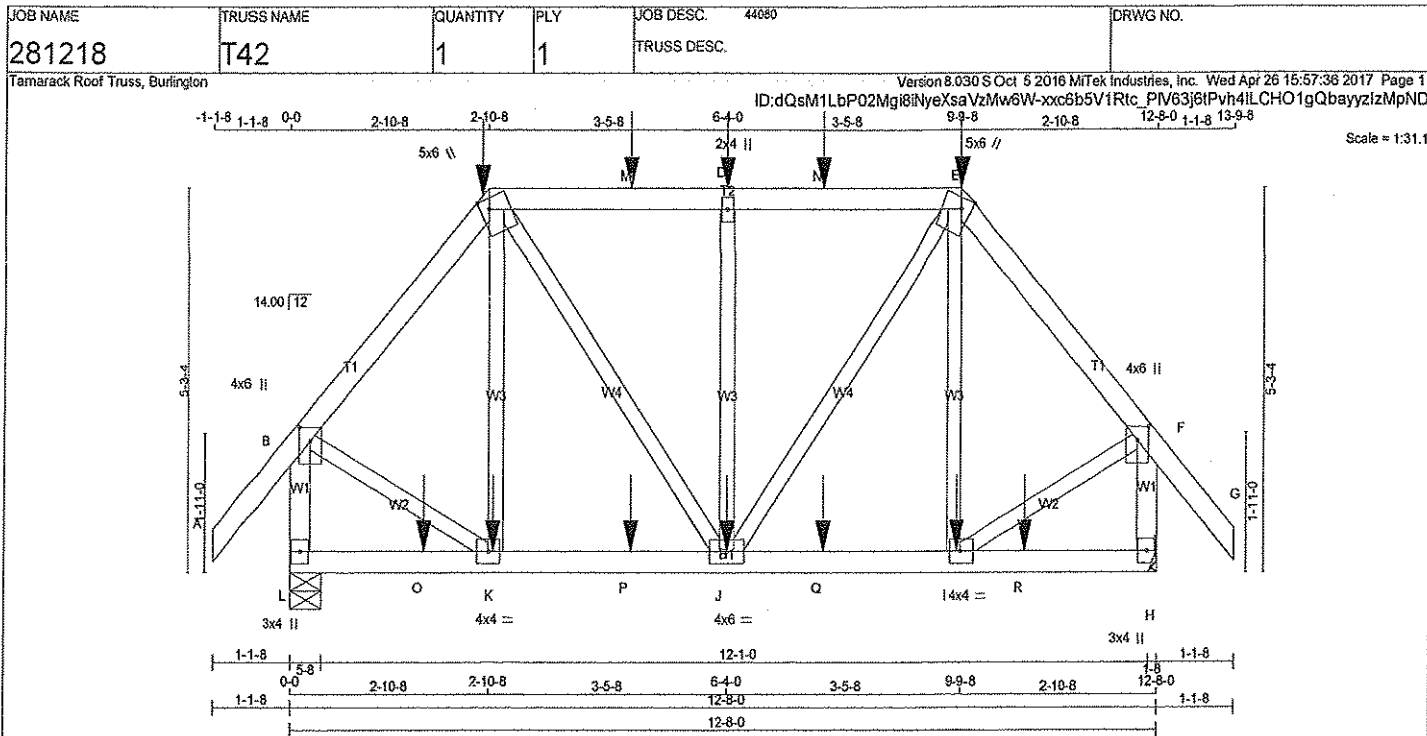
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FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
G	26-10-12	-93	-93	---	FRONT	VERT
L	26-10-12	-41	-71	---	FRONT	VERT
M	22-10-12	-41	-71	---	FRONT	VERT
N	21-9-8	-1456	-1456	---	FRONT	VERT
T	22-10-12	-93	-93	---	FRONT	VERT
U	24-10-12	-93	-93	---	FRONT	VERT
V	28-10-12	-93	-93	---	FRONT	VERT
W	30-10-12	-93	-93	---	FRONT	VERT
X	24-10-12	-41	-71	---	FRONT	VERT
Y	28-10-12	-41	-71	---	FRONT	VERT
Z	30-10-12	-41	-71	---	FRONT	VERT
AA	32-10-12	-41	-71	---	FRONT	VERT
						TYPE
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL



DWG NO. TAM 1949817
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMVW+p	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.50
F	TMVW+p	MT20	4.0	6.0	2.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 141.9 lbs FACTORED DOWN AT 2-10-8, 33.8 lbs FACTORED DOWN AT 4-11-4, 33.8 lbs FACTORED DOWN AT 6-4-0, AND 33.8 lbs FACTORED DOWN AT 7-8-12, AND 141.9 lbs FACTORED DOWN AT 9-9-8 ON TOP CHORD, AND 19.6 lbs FACTORED DOWN AT 1-11-4, 18.9 lbs FACTORED DOWN AT 2-11-4, 18.9 lbs FACTORED DOWN AT 4-11-4, 18.9 lbs FACTORED DOWN AT 6-4-0, 18.9 lbs FACTORED DOWN AT 7-8-12, AND 18.9 lbs FACTORED DOWN AT 9-8-12, AND 19.6 lbs FACTORED DOWN AT 10-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQ'D
L	994	994	0	5-8
H	994	994	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
L	799	466 / 0	162 / 0	0 / 0	171 / 0
H	799	466 / 0	162 / 0	0 / 0	171 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. LC1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 36	-77.4	-77.4	0.09 (1)	10.00	K-C	-103 / 101
B-C	-685 / 0	-77.4	-77.4	0.13 (1)	6.25	C-J	0 / 311
C-D	-618 / 0	-77.4	-77.4	0.19 (1)	6.25	J-D	-460 / 0
D-E	-618 / 0	-77.4	-77.4	0.19 (1)	6.25	E-J	0 / 311
E-F	-618 / 0	-77.4	-77.4	0.19 (1)	6.25	F-E	-103 / 101
F-G	-685 / 0	-77.4	-77.4	0.13 (1)	6.25	G-F	0 / 499
G-H	0 / 36	-77.4	-77.4	0.09 (1)	10.00	H-G	0 / 499
H-I	-859 / 0	0.0	0.0	0.12 (1)	7.81		
I-J	-859 / 0	0.0	0.0	0.12 (1)	7.81		
L-O	0 / 0	-28.0	-28.0	0.08 (2)	10.00		
O-K	0 / 0	-28.0	-28.0	0.08 (2)	10.00		
K-P	0 / 441	-28.0	-28.0	0.14 (2)	10.00		
P-J	0 / 441	-28.0	-28.0	0.14 (2)	10.00		
J-Q	0 / 441	-28.0	-28.0	0.14 (2)	10.00		
Q-I	0 / 441	-28.0	-28.0	0.14 (2)	10.00		
I-R	0 / 0	-28.0	-28.0	0.08 (2)	10.00		
R-H	0 / 0	-28.0	-28.0	0.08 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	2-10-8	-142	-142	---	FRONT	VERT	TOTAL
D	6-4-0	-34	-34	---	FRONT	VERT	TOTAL
E	9-9-8	-142	-142	---	FRONT	VERT	TOTAL
I	9-8-12	-11	-19	---	FRONT	VERT	TOTAL
J	6-4-0	-11	-19	---	FRONT	VERT	TOTAL
K	2-11-4	-11	-19	---	FRONT	VERT	TOTAL
M	4-11-4	-34	-34	---	FRONT	VERT	TOTAL
N	7-8-12	-34	-34	---	FRONT	VERT	TOTAL
O	1-11-4	-11	-20	---	FRONT	VERT	TOTAL
P	4-11-4	-11	-19	---	FRONT	VERT	TOTAL
Q	7-8-12	-11	-19	---	FRONT	VERT	TOTAL
R	10-8-12	-11	-20	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, EBCB 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.19 (C-D-1), BC=0.14 (J-K-2), WB=0.17 (D-J-1), SS=0.16 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

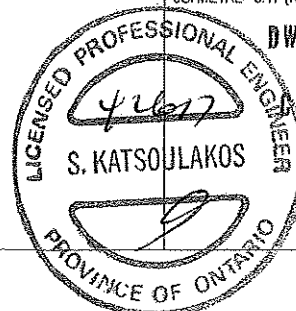
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (B) (INPUT = 0.90)
JSI METAL= 0.17 (K) (INPUT = 1.00)



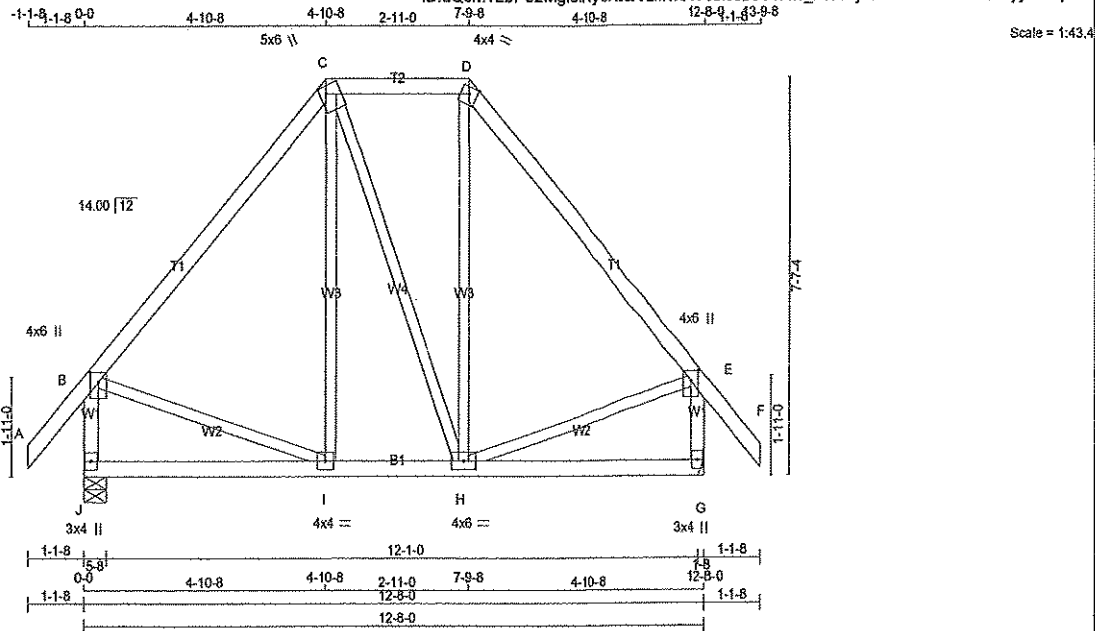
DWG NO. TAM 19499-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T43	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 68 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMVW+p	MT20	4.0	6.0	2.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
	FACTORED			MAXIMUM FACTORED			INPUT	REQ'D
	GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	763	0		763	0	0		5-8
G	763	0		763	0	0	HANGER BY OTHERS	
							MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	620	353 / 0	133 / 0	0 / 0	0 / 0	134 / 0	0 / 0	134 / 0	0 / 0
G	620	353 / 0	133 / 0	0 / 0	0 / 0	134 / 0	0 / 0	134 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 38	-77.4	-77.4	0.09 (1)	I-C	0 / 156	0.04 (3)
B-C	-438 / 0	-77.4	-77.4	0.24 (1)	C-H	0 / 2	0.00 (3)
C-D	-263 / 0	-77.4	-77.4	0.08 (1)	H-D	0 / 158	0.04 (3)
D-E	-438 / 0	-77.4	-77.4	0.24 (1)	B-I	0 / 295	0.07 (1)
E-F	0 / 38	-77.4	-77.4	0.09 (1)	H-E	0 / 295	0.07 (1)
J-B	-704 / 0	0.0	0.0	0.08 (1)			
G-E	-704 / 0	0.0	0.0	0.08 (1)			
J-I	0 / 0	-28.0	-28.0	0.15 (2)			
I-H	0 / 283	-28.0	-28.0	0.17 (2)			
H-G	0 / 0	-28.0	-28.0	0.15 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSI: TC=0.24 (D-E:1), BC=0.17 (H-I:2), WB=0.07 (E-H:1), SS=0.11 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

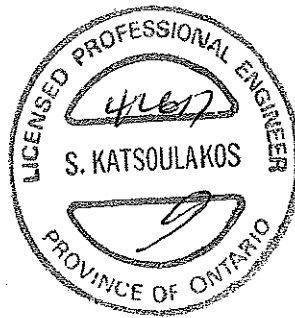
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

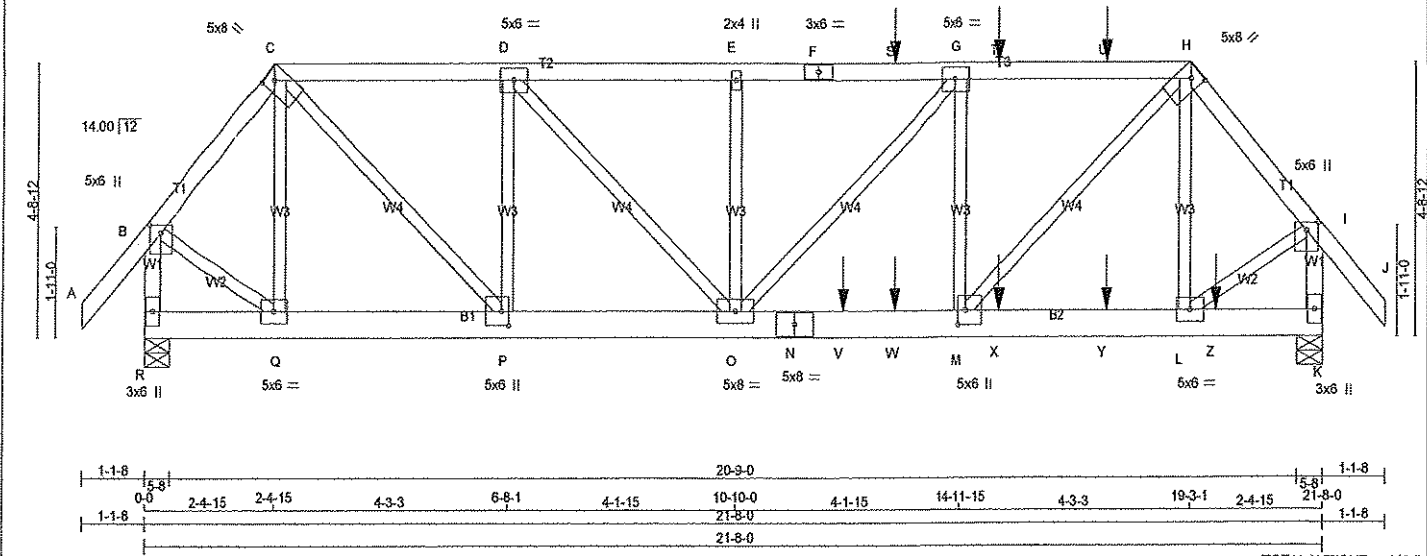
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.48 (H) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)



DRWG NO. TAM 1950017
STRUCTURAL
COMPONENT ONLY



LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x6	DRY	1650F 1.5E	SPF
N - K	2x6	DRY	1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+h	MT20	5.0	6.0	1.75	2.50
C	TTWW+h	MT20	5.0	8.0	2.00	2.25
D	TMVW+t	MT20	5.0	6.0		
E	TMVW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW+l	MT20	5.0	6.0		
H	TTWW+h	MT20	5.0	8.0	2.00	2.25
I	TMVW+p	MT20	5.0	6.0	1.75	2.50
K	BMV1+p	MT20	3.0	6.0		
L	BMVW+l	MT20	5.0	6.0		
M	BMVW+t	MT20	5.0	6.0	3.00	1.50
N	BS-t	MT20	5.0	8.0		
O	BMVWVW+t	MT20	5.0	8.0		
P	BMVW+l	MT20	5.0	6.0	3.00	1.50
Q	BMVW+t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 72.5 lbs FACTORED DOWN AT 13-8-12, AND 72.5 lbs FACTORED DOWN AT 15-8-12, AND 72.5 lbs FACTORED DOWN AT 17-8-12 ON TOP CHORD, AND 1332.4 lbs FACTORED DOWN AT 12-9-8, 41.5 lbs FACTORED DOWN AT 13-8-12, 41.5 lbs FACTORED DOWN AT 15-8-12, AND 41.5 lbs FACTORED DOWN AT 17-8-12, AND 41.5 lbs FACTORED DOWN AT 19-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	1864	0	1864	0	5-8	5-8
K	2255	0	2255	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
R	1514	861/0	325/0	0/0	0/0	328/0	0/0
K	1836	1036/0	399/0	0/0	0/0	400/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC1) (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC1) (LBS)
FR-TO					FR-TO		
A-B	0/36	-77.4	-77.4 0.09 (1)	10.00	Q-C	-377/0	0.12 (1)
B-C	-1472/0	-77.4	-77.4 0.12 (1)	5.28	C-P	0/1765	0.44 (3)
C-D	-2182/0	-77.4	-77.4 0.30 (1)	4.30	P-D	-1293/0	0.42 (1)
D-E	-3082/0	-77.4	-77.4 0.37 (1)	3.62	D-O	0/1303	0.32 (1)
E-F	-3082/0	-77.4	-77.4 0.47 (1)	3.49	O-E	-299/0	0.10 (1)
F-S	-3082/0	-77.4	-77.4 0.47 (1)	3.49	Q-G	0/401	0.10 (1)
S-G	-3082/0	-77.4	-77.4 0.47 (1)	3.49	M-G	-803/0	0.26 (1)
G-T	-2805/0	-77.4	-77.4 0.45 (1)	3.67	M-H	0/2385	0.59 (1)
T-U	-2805/0	-77.4	-77.4 0.45 (1)	3.67	L-H	-640/0	0.21 (1)
U-H	-2805/0	-77.4	-77.4 0.45 (1)	3.67	B-Q	0/1100	0.27 (1)
H-I	-1772/0	-77.4	-77.4 0.13 (1)	4.88	L-I	0/1324	0.33 (1)
I-J	0/36	-77.4	-77.4 0.09 (1)	10.00			
R-B	-1867/0	0.0	0.0 0.22 (1)	6.09			
K-I	-2208/0	0.0	0.0 0.28 (1)	5.67			
R-Q	0/0	-28.0	-28.0 0.05 (2)	10.00			
Q-P	0/942	-28.0	-28.0 0.13 (1)	10.00			
P-O	0/2182	-28.0	-28.0 0.37 (1)	10.00			
O-N	0/2805	-28.0	-28.0 0.77 (1)	10.00			
N-V	0/2805	-28.0	-28.0 0.77 (1)	10.00			
V-W	0/2805	-28.0	-28.0 0.77 (1)	10.00			
W-M	0/2805	-28.0	-28.0 0.77 (1)	10.00			
M-X	0/1129	-28.0	-28.0 0.29 (1)	10.00			
X-Y	0/1129	-28.0	-28.0 0.29 (1)	10.00			
Y-L	0/1129	-28.0	-28.0 0.29 (1)	10.00			
L-Z	0/0	-28.0	-28.0 0.03 (3)	10.00			
Z-K	0/0	-28.0	-28.0 0.03 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
S	13-8-12	-73	-73		FRONT	VERT	TOTAL
T	15-8-12	-73	-73		FRONT	VERT	TOTAL
U	17-8-12	-73	-73		FRONT	VERT	TOTAL
V	12-9-8	-1332	-1332		FRONT	VERT	TOTAL
W	13-8-12	-24	-41		FRONT	VERT	TOTAL
X	15-8-12	-24	-41		FRONT	VERT	TOTAL
Y	17-8-12	-24	-41		FRONT	VERT	TOTAL
Z	19-8-12	-24	-41		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
 ALLOWABLE DEFL.(TL) = L/360 (0.72")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.47 (E-G-1), BC=0.77 (M-O-1),
 WB=0.59 (H-M-1), SSI=0.51 (M-O-1)

DOX LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
 JSI METAL= 0.79 (N) (INPUT = 1.00)



DWG NO. TAM 19501-17
 STRUCTURAL
 COMPONENT ONLY

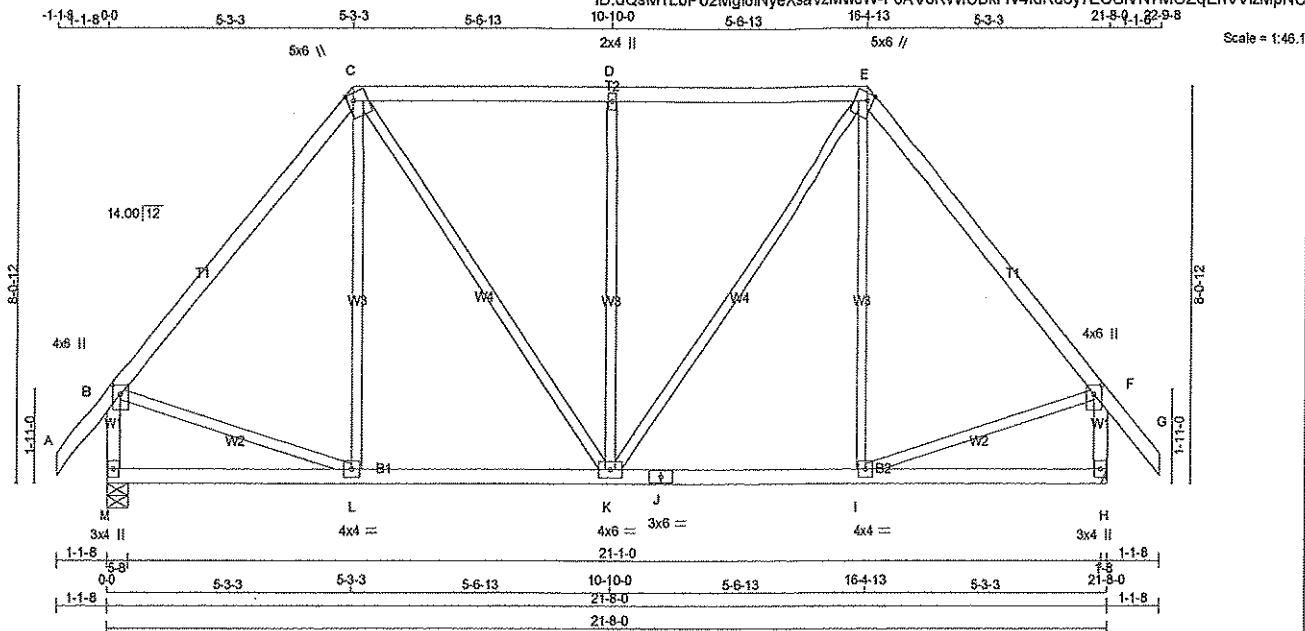
DWG NO. TAM 19502-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44680	DRWG NO.
281218	T46	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: dQsM1LbP02Mgi8INyeXsaVzMW6W-P8AVoRWiCBkr1v4IdRd8y7ECCNVN7MOZqEhVVizMpNC



TOTAL WEIGHT = 106 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.50
F	TMVW+p	MT20	4.0	6.0	2.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BS-1	MT20	-3.0	6.0		
K	BMVW-w	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
FACTORED		MAXIMUM FACTORED		INPUT	REQRD
GROSS REACTION		GROSS REACTION		BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
M	1237	0	1237	0	0
H	1237	0	1237	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1014	582 / 0	228 / 0	0 / 0	0 / 0	224 / 0	0 / 0
H	1014	582 / 0	228 / 0	0 / 0	0 / 0	224 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 38	-77.4	-77.4 0.09 (1)	10.00	L-C	0 / 205	0.05 (3)
B-C	-927 / 0	-77.4	-77.4 0.44 (1)	5.85	C-K	0 / 409	0.09 (1)
C-D	-839 / 0	-77.4	-77.4 0.42 (1)	6.02	K-D	-523 / 0	0.65 (1)
D-E	-839 / 0	-77.4	-77.4 0.42 (1)	6.02	K-E	0 / 409	0.09 (1)
E-F	-927 / 0	-77.4	-77.4 0.44 (1)	5.85	E-I	0 / 205	0.05 (3)
F-G	0 / 36	-77.4	-77.4 0.09 (1)	10.00	B-L	0 / 625	0.14 (1)
M-B	-1178 / 0	0.0	0.0 0.13 (1)	7.35	I-F	0 / 625	0.14 (1)
H-F	-1178 / 0	0.0	0.0 0.13 (1)	7.35			
M-L	0 / 0	-28.0	-28.0 0.20 (2)	10.00			
L-K	0 / 601	-28.0	-28.0 0.28 (2)	10.00			
K-J	0 / 601	-28.0	-28.0 0.28 (2)	10.00			
J-I	0 / 601	-28.0	-28.0 0.28 (2)	10.00			
I-H	0 / 0	-28.0	-28.0 0.20 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.72")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.72")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.06")

CSI: TC=0.44 (B-C:1), BC=0.28 (K-L:2), WB=0.65 (D-K:1), SSI=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

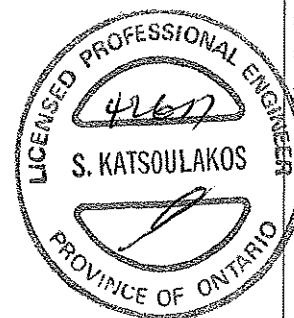
NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

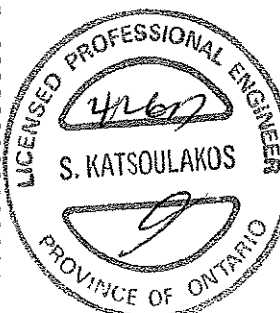
JSI GRIP= 0.81 (I) (INPUT = 0.90)

JSI METAL= 0.23 (L) (INPUT = 1.00)



DWG NO. TAM 1950311
STRUCTURAL
COMPONENT ONLY

COMPONENT CONTINUED ON PAGE 2



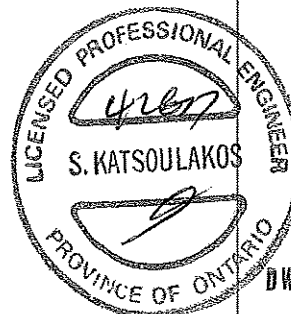
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T47S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

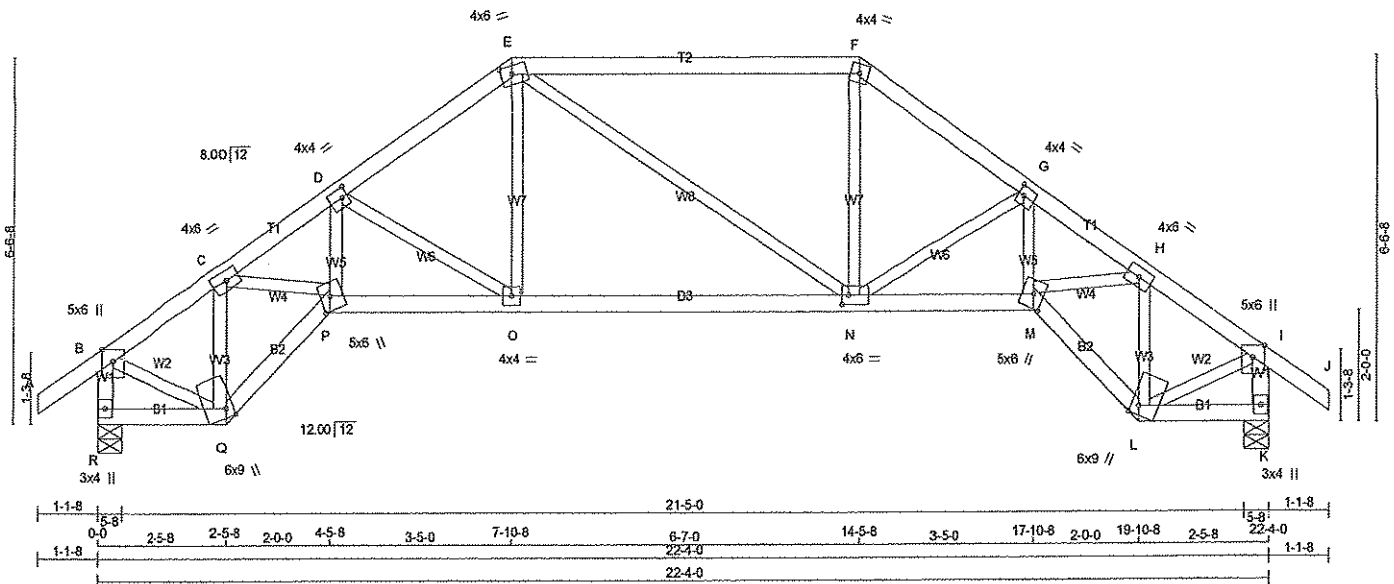
Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Apr 26 15:57:37 2017 Page 2

ID:dQsM1LbP02Mg8iNyeXsaVzMw6W-P8AVoRWfCBkr1v4IdRd6y7E54MR7MZZqEhVVzMPNC

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
G	16-5-8	-271	-271	---	FRONT	VERT
M	19-10-8	-54	-94	---	BACK	VERT
N	17-10-8	-5	-9	---	BACK	VERT
Q	5-11-4	---	---	---	BACK	VERT
U	7-11-4	-212	-212	---	BACK	VERT
V	9-11-4	-212	-212	---	BACK	VERT
W	11-11-4	-212	-212	---	BACK	VERT
X	13-11-4	-212	-212	---	BACK	VERT
Y	15-11-4	-231	-231	---	BACK	VERT
Z	1-11-4	-47	-83	---	BACK	VERT
AA	3-11-4	---	---	---	BACK	VERT
AB	7-11-4	---	---	---	BACK	VERT
AC	9-11-4	---	---	---	BACK	VERT
AD	11-11-4	---	---	---	BACK	VERT
AE	13-11-4	---	---	---	BACK	VERT
AF	15-11-4	---	---	---	BACK	VERT



pg 2
DWG NO. TAM 19504-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 94 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE DRY LUMBER DESCR.
A - E 2x4 DRY No.2 SPF
E - F 2x4 DRY No.2 SPF
F - J 2x4 DRY No.2 SPF
R - B 2x4 DRY No.2 SPF
K - I 2x4 DRY No.2 SPF
R - Q 2x4 DRY No.2 SPF
Q - P 2x4 DRY No.2 SPF
P - M 2x4 DRY No.2 SPF
M - L 2x4 DRY No.2 SPF
L - K 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C TMVW-t	MT20	4.0	6.0		
D TMVW-t	MT20	4.0	4.0	2.00	1.25
E TTWW-m	MT20	4.0	6.0	1.75	2.50
F TTWW-m	MT20	4.0	4.0		
G TMVW-t	MT20	4.0	4.0	2.00	1.25
H TMVW-t	MT20	4.0	6.0		
I TMVW+p	MT20	5.0	6.0	Edge	
K BMV1+p	MT20	3.0	4.0		
L BBWW+m	MT20	6.0	9.0	Edge	1.75
M BBWW+m	MT20	5.0	6.0	3.00	2.25
N BMWW-t	MT20	4.0	6.0	2.00	1.50
O BMWW-t	MT20	4.0	4.0		
P BBWW+m	MT20	5.0	6.0	3.00	2.25
Q BBWW+m	MT20	6.0	9.0	Edge	1.75
R BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	1270	0	1270	0	5-8	5-8
K	1270	0	1270	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
R	1041	576 / 0	235 / 0	0 / 0	0 / 0	231 / 0	0 / 0
K	1041	576 / 0	235 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (CSI (LC))	MEMB.	MAX FACTORED FORCE (LBS)	MAX. FACTORED (CSI (LC))	MEMB.
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-77.4	-77.4 0.08 (1)	10.00	Q-C	-1275 / 0	0.22 (1)
B-C	-1208 / 0	-77.4	-77.4 0.07 (1)	5.76	C-P	0 / 1295	0.29 (1)
C-D	-2649 / 0	-77.4	-77.4 0.11 (1)	4.19	P-D	0 / 723	0.16 (1)
D-E	-1719 / 0	-77.4	-77.4 0.15 (1)	4.86	D-O	-871 / 0	0.23 (1)
E-F	-1437 / 0	-77.4	-77.4 0.47 (1)	4.88	O-E	0 / 583	0.13 (1)
F-G	-1720 / 0	-77.4	-77.4 0.15 (1)	4.86	E-N	0 / 0	0.00 (1)
G-H	-2649 / 0	-77.4	-77.4 0.11 (1)	4.19	N-F	0 / 584	0.13 (1)
H-I	-1208 / 0	-77.4	-77.4 0.07 (1)	5.76	F-G	-870 / 0	0.23 (1)
I-J	0 / 26	-77.4	-77.4 0.08 (1)	10.00	M-G	0 / 722	0.16 (1)
R-B	-1235 / 0	0.0	0.0 0.13 (1)	7.22	M-H	0 / 1295	0.29 (1)
K-I	-1235 / 0	0.0	0.0 0.13 (1)	7.22	L-H	-1275 / 0	0.22 (1)
R-Q	0 / 0	-28.0	-28.0 0.05 (2)	10.00	B-Q	0 / 1076	0.24 (1)
Q-P	0 / 1347	-28.0	-28.0 0.24 (1)	10.00	L-I	0 / 1076	0.24 (1)
P-O	0 / 2172	-28.0	-28.0 0.45 (1)	10.00			
O-N	0 / 1436	-28.0	-28.0 0.37 (2)	10.00			
N-M	0 / 2172	-28.0	-28.0 0.46 (1)	10.00			
M-L	0 / 1347	-28.0	-28.0 0.24 (1)	10.00			
L-K	0 / 0	-28.0	-28.0 0.05 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/989 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/989 (0.20")

CSI: TC=0.47 (E-F-1), BC=0.46 (M-N-1), WB=0.29 (C-P-1), SSI=0.20 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

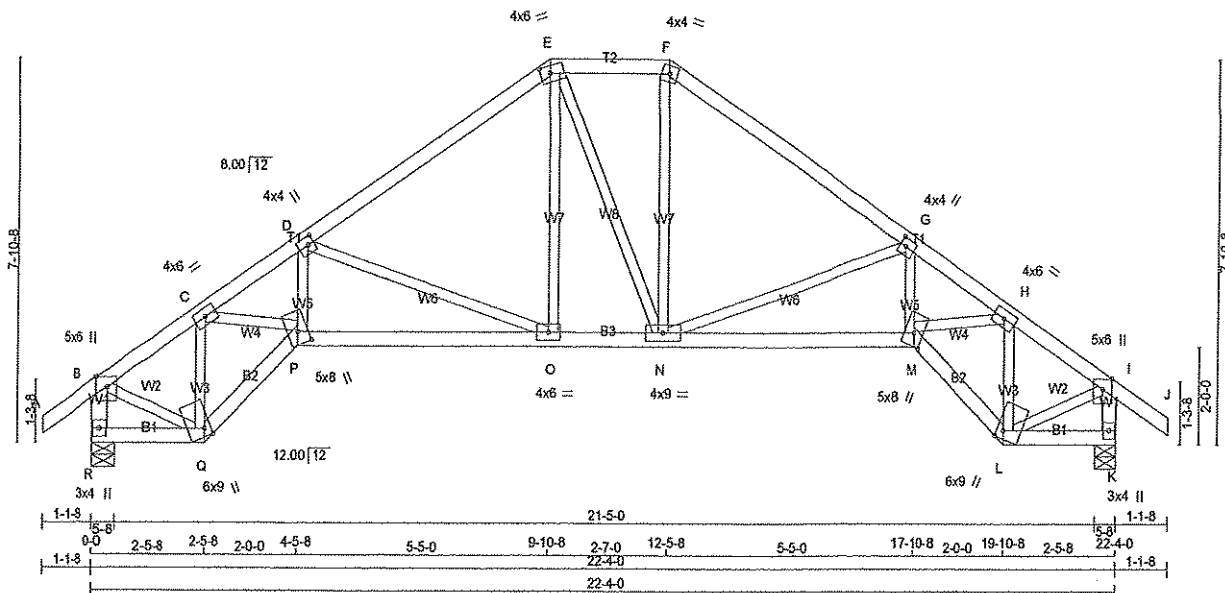
JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.38 (H) (INPUT = 1.00)



DRWG NO. TAM 19505-19
STRUCTURAL COMPONENT ONLY

-1-1-8 1-8 0-0 2-5-8 2-5-8 2-0-0 4-5-8 5-5-0 9-10-8 2-7-0 12-5-8 5-5-0 17-10-8 2-0-0 19-10-8 2-5-8 22-4-9 1-8 3-5-8

Scale = 1:46.4



TOTAL WEIGHT = 99 lb (M/F)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
R - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
R - Q	2x4	DRY	No.2
Q - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - K	2x4	DRY	No.2
DESCR. SPF			
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVV+p	MT20	5.0	6.0	Edge
C	TMWW-t	MT20	4.0	6.0	
D	TMWW+t	MT20	4.0	4.0	2.00 1.50
E	TTWW-m	MT20	4.0	6.0	1.75 2.50
F	TTW-m	MT20	4.0	4.0	
G	TMWW+t	MT20	4.0	4.0	2.00 1.50
H	TMWW-t	MT20	4.0	6.0	
I	TMVV+p	MT20	5.0	6.0	Edge
K	BMV1+p	MT20	3.0	4.0	
L	BBWW+m	MT20	6.0	9.0	Edge 1.75
M	BBWW+m	MT20	5.0	8.0	3.25 2.50
N	BMWW-t	MT20	4.0	9.0	
O	BMWW-t	MT20	4.0	6.0	
P	BBWW+m	MT20	5.0	8.0	3.25 2.50
Q	BBWW+m	MT20	6.0	9.0	Edge 1.75
R	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
R	1270	0	1270	0	0	5-8	5-8	5-8	5-8
K	1270	0	1270	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1041	576 / 0	235 / 0	0 / 0	0 / 0	231 / 0	0 / 0
K	1041	576 / 0	235 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-77.4	-77.4 0.08 (1)	10.00	Q-C	-1259 / 0	0.22 (1)
B-C	-1198 / 0	-77.4	-77.4 0.09 (1)	5.76	C-P	0 / 1373	0.31 (1)
C-D	-2685 / 0	-77.4	-77.4 0.25 (1)	4.02	P-D	0 / 724	0.16 (1)
D-E	-1421 / 0	-77.4	-77.4 0.30 (1)	5.16	D-O	-1142 / 0	0.66 (1)
E-F	-1178 / 0	-77.4	-77.4 0.09 (1)	5.81	O-E	0 / 503	0.11 (1)
F-G	-1423 / 0	-77.4	-77.4 0.30 (1)	5.15	E-N	0 / 5	0.00 (1)
G-H	-2685 / 0	-77.4	-77.4 0.25 (1)	4.02	N-F	0 / 509	0.11 (1)
H-I	-1198 / 0	-77.4	-77.4 0.09 (1)	5.76	F-G	-1140 / 0	0.66 (1)
I-J	0 / 26	-77.4	-77.4 0.08 (1)	10.00	M-G	0 / 722	0.16 (1)
R-B	-1235 / 0	0.0	0.0 0.13 (1)	7.22	M-H	0 / 1373	0.31 (1)
K-I	-1235 / 0	0.0	0.0 0.13 (1)	7.22	L-H	-1259 / 0	0.22 (1)
R-Q	0 / 0	-28.0	-28.0 0.05 (2)	10.00	B-Q	0 / 1063	0.24 (1)
Q-P	0 / 1330	-28.0	-28.0 0.23 (1)	10.00	L-I	0 / 1063	0.24 (1)
P-O	0 / 2236	-28.0	-28.0 0.47 (1)	10.00			
O-N	0 / 1176	-28.0	-28.0 0.32 (2)	10.00			
N-M	0 / 2237	-28.0	-28.0 0.46 (1)	10.00			
M-L	0 / 1330	-28.0	-28.0 0.23 (1)	10.00			
L-K	0 / 0	-28.0	-28.0 0.05 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD = 43.8 PSF				

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.30 (F-G:1), BC=0.47 (O-P:1), WB=0.66 (D-O:1), SSI=0.15 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

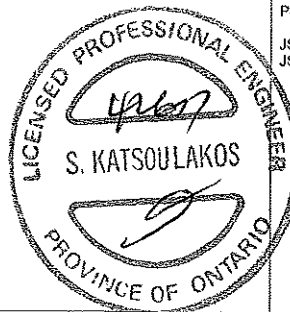
NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MAX MIN	MAX MIN
MT20	618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.40 (H) (INPUT = 1.00)



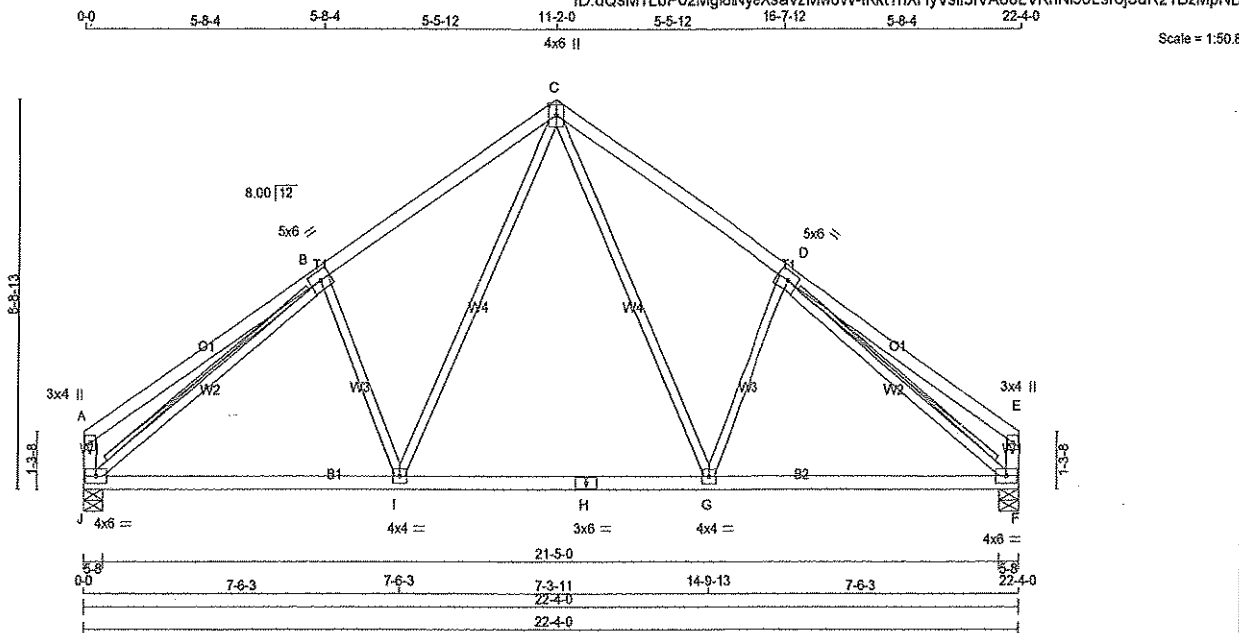
DWG NO. TAM 1950617
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T50	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:50

TOTAL WEIGHT = 2 X 92 = 184 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
J	1177	0	1177	0
F	1177	0	1177	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE
J	978	520 / 0	235 / 0	0 / 0
F	978	520 / 0	235 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT B-J, D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 80% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 28	-77.4 -77.4	0.39 (1)	10.00	C-G	0 / 539	0.12 (1)
B-C	-1218 / 0	-77.4 -77.4	0.33 (1)	5.43	G-D	-290 / 40	0.13 (1)
C-D	-1218 / 0	-77.4 -77.4	0.33 (1)	5.43	I-C	0 / 539	0.12 (1)
D-E	0 / 28	-77.4 -77.4	0.39 (1)	10.00	B-I	-290 / 40	0.13 (1)
J-A	-164 / 0	0.0 0.0	0.02 (1)	7.81	J-B	-1440 / 0	0.56 (1)
F-E	-164 / 0	0.0 0.0	0.02 (1)	7.81	D-F	-1440 / 0	0.56 (1)
J-I	0 / 1102	-28.0 -28.0	0.49 (2)	10.00			
I-H	0 / 776	-28.0 -28.0	0.45 (2)	10.00			
H-G	0 / 776	-28.0 -28.0	0.45 (2)	10.00			
G-F	0 / 1102	-28.0 -28.0	0.49 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.74")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.13")
ALLOWABLE DEFL.(TL) = $L/360$ (0.74")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.22")

CSI: TC=0.39 (D-E:1), BC=0.49 (F-G:2), WB=0.56 (D-F:1), SSF=0.17 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

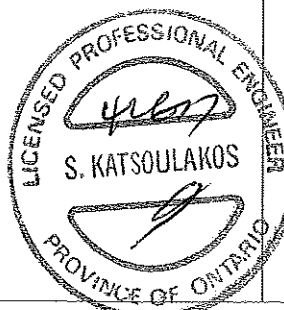
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (J) (INPUT = 0.90)
JSI METAL= 0.37 (D) (INPUT = 1.00)



DWG NO. TAM 19507-19
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T50S	2	1	TRUSS DESC.		

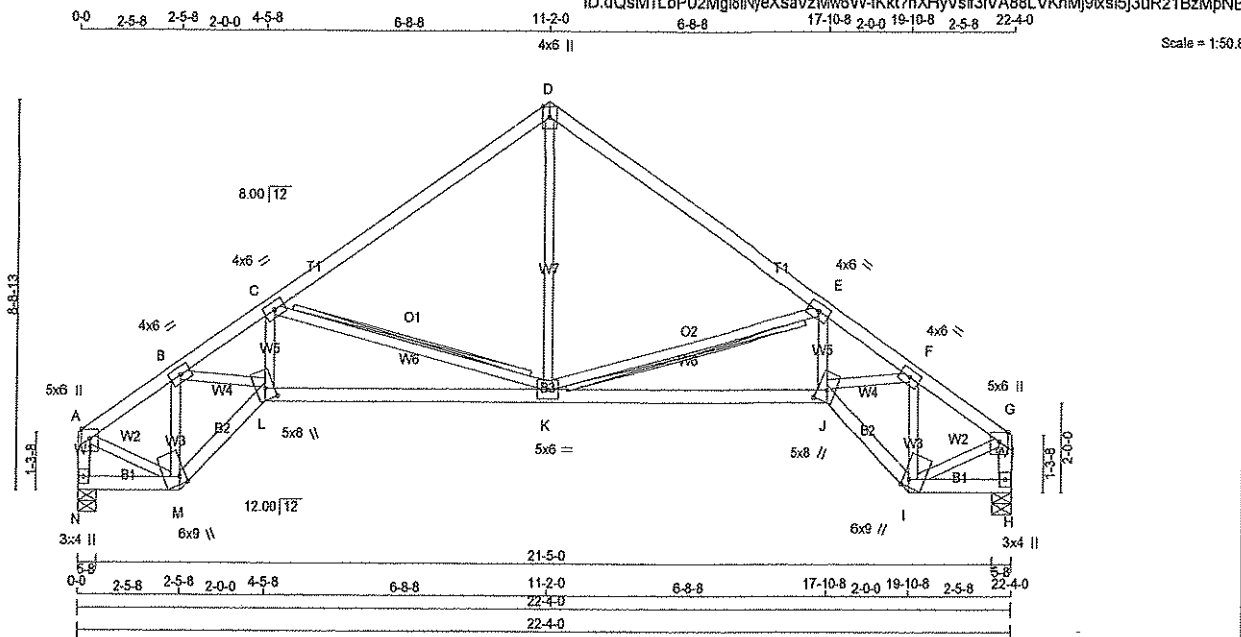
Tamarack Roof Truss, Burlington

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17-10-8 2-0-0 19-10-8 2-5-8 22-4-0

Scale = 1:50.8



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
N - A	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
N - M	2x4	DRY No.2	SPF
M - L	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
J - I	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF

ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV+p	MT20	5.0	6.0	Edge	
B, C, E, F						
B	TMWW-I	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
G	TMVV+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BBWW+m	MT20	6.0	9.0	Edge 1.75	
J	BBWW+m	MT20	5.0	8.0	3.25 2.50	
K	BBWW+m	MT20	5.0	6.0		
L	BBWW+m	MT20	5.0	8.0	3.25 2.50	
M	BBWW+m	MT20	6.0	9.0	Edge 1.75	
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
N	1177	0	1177	0	0
H	1177	0	1177	0	0

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	978	520 / 0	235 / 0	0 / 0	0 / 0	223 / 0	0 / 0
H	978	520 / 0	235 / 0	0 / 0	0 / 0	223 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-K, C-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	
A-B	-1191 / 0	-77.4	-77.4 0.12 (1)	K-D	0 / 975	0.22 (1)	
B-C	-2720 / 0	-77.4	-77.4 0.38 (1)	K-E	-1307 / 0	0.94 (1)	
C-D	-1284 / 0	-77.4	-77.4 0.46 (1)	J-E	0 / 699	0.16 (1)	
D-E	-1284 / 0	-77.4	-77.4 0.46 (1)	J-F	0 / 1445	0.33 (1)	
E-F	-2720 / 0	-77.4	-77.4 0.38 (1)	I-F	-1243 / 0	0.21 (1)	
F-G	-1191 / 0	-77.4	-77.4 0.12 (1)	C-K	-1307 / 0	0.94 (1)	
N-A	-1142 / 0	0.0	0.0 0.12 (1)	L-C	0 / 699	0.16 (1)	
H-G	-1142 / 0	0.0	0.0 0.12 (1)	B-L	0 / 1445	0.33 (1)	
				M-B	-1243 / 0	0.21 (1)	
N-M	0 / 0	-28.0	-28.0 0.05 (2)	A-M	0 / 1050	0.24 (1)	
M-L	0 / 1314	-28.0	-28.0 0.23 (1)	I-G	0 / 1050	0.24 (1)	
L-K	0 / 2298	-28.0	-28.0 0.64 (2)				
K-J	0 / 2298	-28.0	-28.0 0.64 (2)				
J-I	0 / 1314	-28.0	-28.0 0.23 (1)				
I-H	0 / 0	-28.0	-28.0 0.05 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.46 (D-E:1), BC=0.64 (K-L:2), WB=0.94 (C-K:1), SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

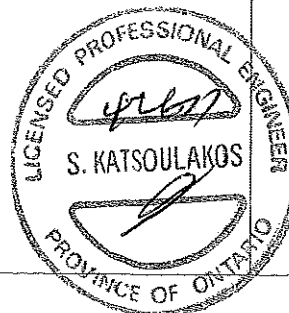
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1657
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.42 (K) (INPUT = 1.00)



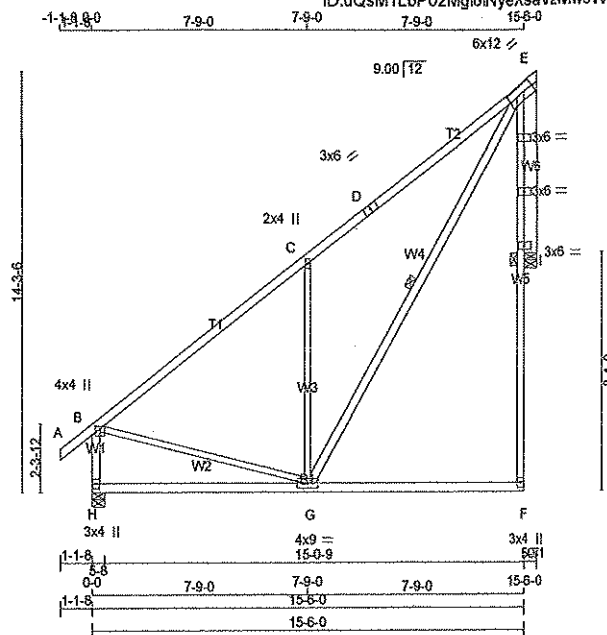
BWB NO. TAM 19508-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T51	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 8 X 89 = 795 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+w	MT20	2.0	4.0		
D	TS-1	MT20	3.0	6.0		
E	TMVWVW-1	MT20	6.0	12.0	3.00	3.50
F	BMV+p	MT20	3.0	4.0		
G	BMVWVW-1	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
J(E)	804	0	804	0
H	923	0	923	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J(E)	669	356 / 0	160 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	753	423 / 0	165 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO								
A-B	0 / 28	-77.4	-77.4	0.08 (1)	10.00	G-C	-735 / 0	0.94 (1)
B-C	-607 / 0	-77.4	-77.4	0.89 (1)	5.49	G-E	0 / 1000	0.16 (1)
C-D	-675 / 0	-77.4	-77.4	0.91 (1)	5.28	B-G	0 / 538	0.12 (1)
D-E	-675 / 0	-77.4	-77.4	0.91 (1)	5.28	E-J	-805 / 0	0.40 (1)
F-I	0 / 141	0.0	0.0	0.16 (1)	10.00	I-J	0 / 33	0.00 (1)
I-E	0 / 141	0.0	0.0	0.16 (1)	10.00			
H-B	-638 / 0	0.0	0.0	0.10 (1)	7.81			
H-G	0 / 0	-28.0	-28.0	0.49 (3)	10.00			
G-F	0 / 14	-28.0	-28.0	0.50 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL)= L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL)= L/961 (0.19")

CSI: TC=0.91 (C-E:1), BC=0.50 (F-G:3), WB=0.94 (C-G:1), SSI=0.23 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

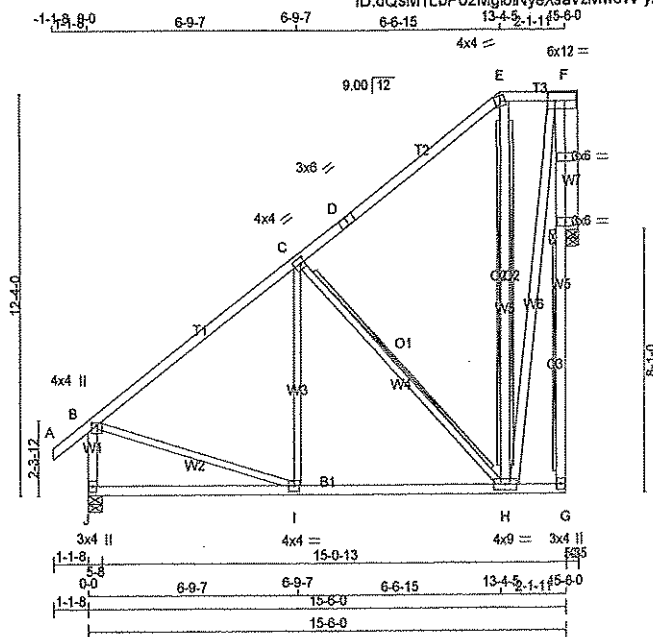


DRWG NO. TAM 25737-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	T52	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 110 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVWW-t	MT20	8.0	12.0	3.00	3.50
G	BMV+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	KP-p	MT20	3.0	6.0		
K	KP-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
L(F)	803	0	803	0
J	924	0	924	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L(F)	688	355 / 0	160 / 0	0 / 0	0 / 0	152 / 0	0 / 0
J	754	423 / 0	165 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L(F), J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT G-K
2x4 DRY SPF No.2 T-BRACE AT C-H
2x4 DRY SPF No.2 I-BRACE AT E-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
A-B	0 / 28	-77.4	-77.4	0.08 (1)	10.00	I-C	0 / 303	0.07 (3)	
B-C	-629 / 0	-77.4	-77.4	0.05 (1)	6.20	C-H	-572 / 0	0.44 (1)	
C-D	-213 / 0	-77.4	-77.4	0.60 (1)	6.25	H-E	-168 / 33	0.15 (1)	
D-E	-213 / 0	-77.4	-77.4	0.60 (1)	6.25	H-F	0 / 735	0.12 (1)	
E-F	-137 / 0	-77.4	-77.4	0.06 (1)	6.25	B-I	0 / 555	0.12 (1)	
G-K	-11 / 0	0.0	0.0	0.19 (1)	7.81	F-L	-805 / 0	0.18 (1)	
K-F	-11 / 0	0.0	0.0	0.19 (1)	7.81	K-L	0 / 50	0.00 (1)	
J-B	-849 / 0	0.0	0.0	0.11 (1)	7.81				
J-I	0 / 0	-28.0	-28.0	0.33 (3)	10.00				
I-H	0 / 533	-28.0	-28.0	0.40 (2)	10.00				
H-G	0 / 17	-28.0	-28.0	0.16 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 089-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.52")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.52")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.13")

CSI: TC=0.65 (B-C:1), BC=0.40 (H-I:2), WB=0.44 (C-H:1), SSI=0.20 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

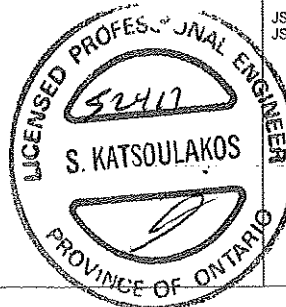
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

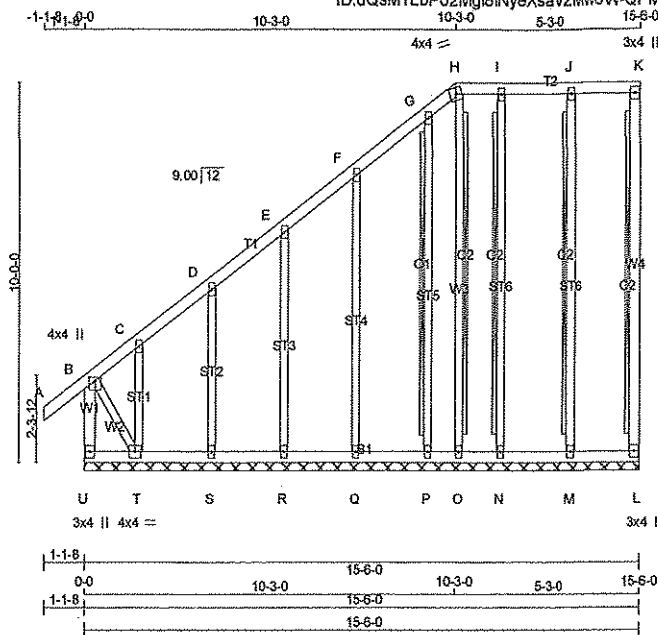


DRWG NO. TAM 25738-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	G52	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:59.7

TOTAL WEIGHT = 102 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
U - B	2x4	DRY	No.2
A - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
L - K	2x4	DRY	No.2
U - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" O.C.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, G, I, J					
C TMVW+w	MT20	2.0	4.0		
H TTW+m	MT20	4.0	4.0		
K TMV+p	MT20	3.0	4.0		
L BMV1+p	MT20	3.0	4.0		
M, N, O, P, Q, R, S					
M BMW1+w	MT20	2.0	4.0		
T BMWW1-t	MT20	4.0	4.0		
U BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT K-L, J-M, I-N, G-P, H-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. LC1 (LC)	FACTORED MAX. LC2 (LC)
FR-TO		FROM TO		FR-TO			
U-B	-204 / 0	0.0	0.0	0.03 (1)	7.81	M-J	-175 / 0
A-B	0 / 28	-77.4	-77.4	0.08 (1)	10.00	N-I	-133 / 0
B-C	-33 / 0	-77.4	-77.4	0.08 (1)	6.25	P-G	-119 / 0
C-D	-6 / 0	-77.4	-77.4	0.04 (1)	10.00	Q-F	-161 / 0
D-E	-8 / 0	-77.4	-77.4	0.04 (1)	10.00	R-E	-150 / 0
E-F	-4 / 0	-77.4	-77.4	0.04 (1)	10.00	S-D	-162 / 0
F-G	-6 / 0	-77.4	-77.4	0.04 (1)	10.00	T-C	-90 / 0
G-H	-11 / 0	-77.4	-77.4	0.02 (1)	6.25	O-H	-50 / 0
H-I	0 / 0	-77.4	-77.4	0.02 (1)	10.00	B-T	0 / 19
I-J	0 / 0	-77.4	-77.4	0.04 (1)	10.00		
J-K	0 / 0	-77.4	-77.4	0.04 (1)	10.00		
L-K	-63 / 0	0.0	0.0	0.04 (1)	7.81		
U-T	0 / 0	-28.0	-28.0	0.02 (2)	10.00		
T-S	0 / 9	-28.0	-28.0	0.02 (2)	10.00		
S-R	0 / 6	-28.0	-28.0	0.02 (2)	10.00		
R-Q	0 / 4	-28.0	-28.0	0.02 (2)	10.00		
Q-P	0 / 2	-28.0	-28.0	0.02 (2)	10.00		
P-O	0 / 0	-28.0	-28.0	0.02 (2)	10.00		
O-N	0 / 0	-28.0	-28.0	0.02 (2)	10.00		
N-M	0 / 0	-28.0	-28.0	0.03 (2)	10.00		
M-L	0 / 0	-28.0	-28.0	0.03 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08 (A-B:1), BC=0.03 (M-N:2), WB=0.19 (F-Q:1), SSI=0.07 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

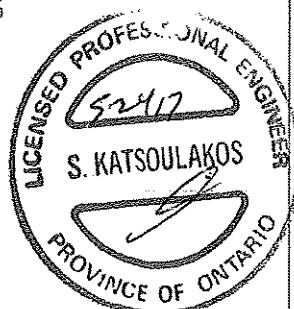
MT20 618 354 1667 822 2284 1650

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)

JSI METAL= 0.04 (D) (INPUT = 1.00)



DWG NO. TAM 25739-17

STRUCTURAL COMPONENT ONLY

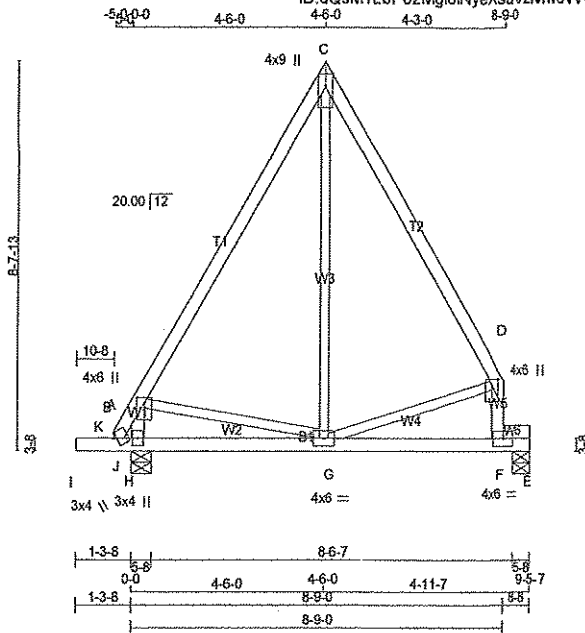
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44088	DRWG NO.
281218	T54	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: dQsM1LbP02Mgi8iNyeXsaVzMw6VW-MWIFD7Xvjo_ZGDEhkrat1YJaRZ9pbP3stYAcadzMpNA

Scale = 1:50.3



TOTAL WEIGHT = 49 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
I - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM+m	MT20	3.0	4.0	1.25	Edge
B	TMVW+p	MT20	4.0	6.0	1.50	2.00
C	TTW+p	MT20	4.0	9.0	3.50	Edge
D	TMVW+p	MT20	4.0	6.0	1.50	2.00
F	BMVW-t	MT20	4.0	6.0		
G	BMVW+t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
E	436	0	436	0
H	642	0	642	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	371	184 / 0	97 / 0	0 / 0	0 / 0	89 / 0	0 / 0
H	534	283 / 0	128 / 0	0 / 0	0 / 0	122 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
A-K	-25 / 0	-77.4	-77.4	0.06 (1)	6.25	G-C	0 / 289	0.06 (2)	6.25
K-B	-75 / 159	-77.4	-77.4	0.27 (1)	6.25	B-G	0 / 48	0.01 (3)	6.25
B-C	-325 / 0	-77.4	-77.4	0.26 (1)	6.25	G-D	0 / 160	0.04 (1)	6.25
C-D	-306 / 0	-77.4	-77.4	0.25 (1)	6.25	J-K	-84 / 205	0.00 (1)	6.25
H-B	-742 / 0	0.0	0.0	0.07 (1)	7.81				
F-D	-465 / 0	0.0	0.0	0.05 (1)	7.81				
I-A	0 / 0	-105.4	-105.4	0.05 (1)	10.00				
A-J	0 / 137	-28.0	-28.0	0.09 (1)	10.00				
J-H	0 / 137	-28.0	-28.0	0.09 (3)	10.00				
H-G	0 / 137	-28.0	-28.0	0.22 (2)	10.00				
G-F	0 / 0	-28.0	-28.0	0.41 (1)	10.00				
F-E	0 / 0	-28.0	-28.0	0.17 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/177 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/177 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.27 (B-K-1), BC=0.41 (F-G-1), WB=0.06 (C-G-2), SSI=0.20 (B-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.72 (F) (INPUT = 0.90)
JSI METAL= 0.32 (F) (INPUT = 1.00)

DWG NO. TAM 29513-17
STRUCTURAL
COMPONENT ONLY

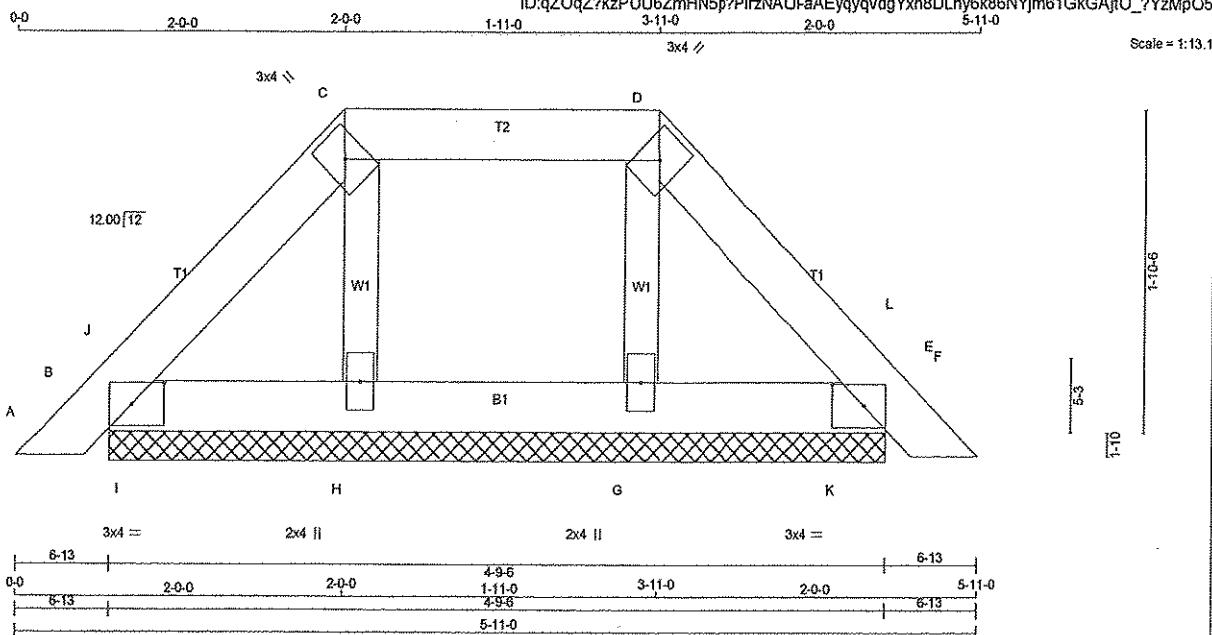


JOB NAME 281216	TRUSS NAME PB1	QUANTITY 1	PLY 1	JOB DESC. 44080 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Wed Apr 26 15:56:40 2017 Page 1

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TOTAL WEIGHT = 16 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
C - D 2x4 DRY
D - F 2x4 DRY
B - E 2x4 DRY
ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

LUMBER DESCR.
No.2 SPF
No.2 SPF
No.2 SPF
No.2 SPF
No.2 SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
B	144	0	144	0
E	144	0	144	0
H	144	0	144	0
G	144	0	144	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	108	75/0	14/0	0/0	0/0	0/0	19/0	0/0
E	108	75/0	14/0	0/0	0/0	0/0	19/0	0/0
H	125	58/0	36/0	0/0	0/0	0/0	32/0	0/0
G	125	58/0	36/0	0/0	0/0	0/0	32/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/11	-77.4	-77.4 0.01 (1)	10.00	H-C	-85/0	0.01 (1)
B-J	-48/0	-77.4	-77.4 0.00 (3)	6.25	G-D	-85/0	0.01 (1)
J-C	-50/0	-77.4	-77.4 0.02 (1)	6.25	I-J	-58/14	0.00 (1)
C-D	-28/0	-77.4	-77.4 0.05 (1)	6.25	K-L	-58/14	0.00 (1)
D-L	-50/0	-77.4	-77.4 0.02 (1)	6.25			
L-E	-48/0	-77.4	-77.4 0.00 (3)	6.25			
E-F	0/11	-77.4	-77.4 0.01 (1)	10.00			
B-I	0/34	-28.0	-28.0 0.02 (1)	10.00			
I-H	0/34	-28.0	-28.0 0.02 (1)	10.00			
H-G	0/28	-28.0	-28.0 0.02 (2)	10.00			
G-K	0/34	-28.0	-28.0 0.02 (1)	10.00			
K-E	0/34	-28.0	-28.0 0.02 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(65% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05 (C-D:1), BC=0.02 (G-K:1), WB=0.01 (D-G:1), SSI=0.06 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

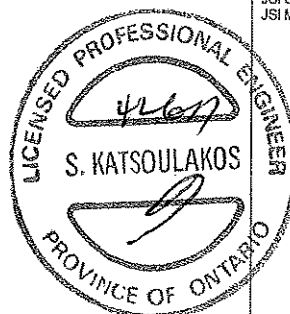
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.20 (C) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)



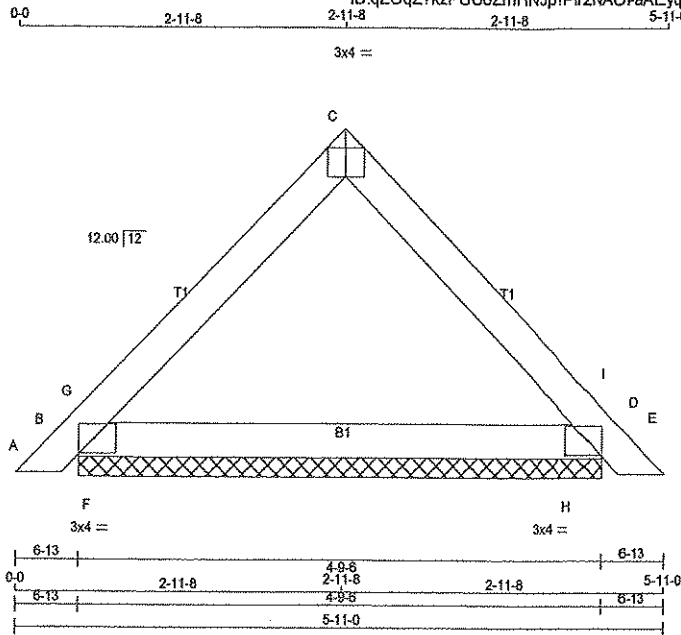
DWG NO. TAM19482-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281216	TRUSS NAME PB2	QUANTITY 1	PLY 1	JOB DESC. 44080 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:19.4

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.50
C	T1-p	MT20	3.0	4.0	Edge 2.00
D	TMB1-I	MT20	3.0	4.0	1.50 2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
B	288	0	288	0	4-9-6
D	288	0	288	0	4-9-6

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	234	133 / 0	50 / 0	0 / 0	0 / 0	51 / 0	0 / 0
D	234	133 / 0	50 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
A-B	0 / 11	-77.4 -77.4	0.01 (1)	10.00	F-G	0 / 328	0.00 (1)
B-G	-339 / 0	-77.4 -77.4	0.08 (3)	6.25	H-I	0 / 328	0.00 (1)
G-C	-129 / 0	-77.4 -77.4	0.08 (1)	6.25			
C-I	-129 / 0	-77.4 -77.4	0.08 (1)	6.25			
I-D	-339 / 0	-77.4 -77.4	0.08 (3)	6.25			
D-E	0 / 11	-77.4 -77.4	0.01 (1)	10.00			
B-F	0 / 98	-28.0 -28.0	0.07 (3)	10.00			
F-H	0 / 98	-28.0 -28.0	0.12 (2)	10.00			
H-D	0 / 98	-28.0 -28.0	0.07 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, EBCB 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09 (C-G:1), BC=0.12 (F-H:2), WB=0.00 (F-G:1), SS=0.17 (D-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DRWG NO. TAM(9483-17)
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

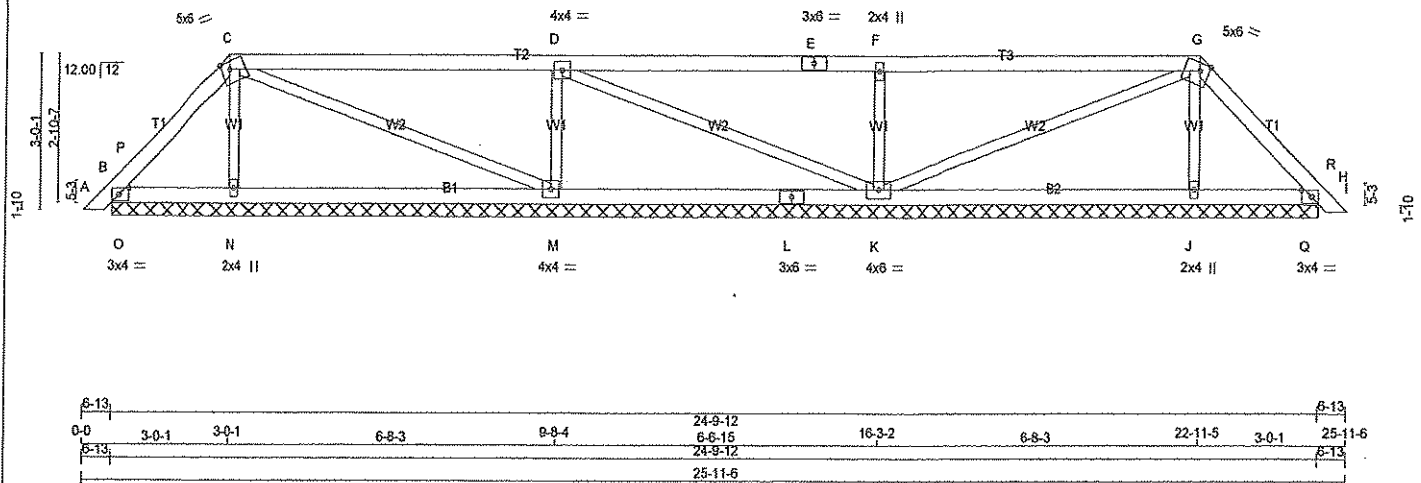


DWG NO. FAM 1948/17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281216	TRUSS NAME PB4	QUANTITY 1	PLY 1	JOB DESC. 44080	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Apr 26 15:56:41 2017 Page 1
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0-0 3-0-1 3-0-1 6-8-3 9-8-4 6-6-15 16-3-2 6-8-3 22-11-5 3-0-1 25-11-6
Scale = 1:43.7



TOTAL WEIGHT = 85 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
G - I	2x4	DRY	No.2 SPF
B - L	2x4	DRY	No.2 SPF
L - H	2x4	DRY	No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	UP/LIFT	IN-SX	BRG	IN-SX
B	198	0	198	0	0	0	24-9-12 (10-18-18)-12		
N	385	0	385	0	0	0	24-9-12 (10-18-18)-12		
M	746	0	746	0	0	0	24-9-12 (10-18-18)-12		
K	783	0	783	0	0	0	24-9-12 (10-18-18)-12		
J	391	0	391	0	0	0	24-9-12 (10-18-18)-12		
H	183	0	183	0	0	0	24-9-12 (10-18-18)-12		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	135	118/0	1/0	0/0	0/0	16/0	0/0
N	352	136/0	119/0	0/0	0/0	97/0	0/0
M	614	336/0	141/0	0/0	0/0	137/0	0/0
K	639	358/0	141/0	0/0	0/0	140/0	0/0
J	356	140/0	119/0	0/0	0/0	97/0	0/0
H	125	110/0	1/0	0/0	0/0	15/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	MAX. LC1 MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0/11	-77.4 -77.4	0.01 (1)	10.00	N-C	-210/0	0.04 (1)
B-P	0/57	-77.4 -77.4	0.04 (1)	10.00	C-M	-27/0	0.03 (1)
P-C	-97/0	-77.4 -77.4	0.05 (1)	8.25	M-D	-549/0	0.09 (1)
C-D	-28/0	-77.4 -77.4	0.46 (1)	6.25	D-K	-39/0	0.04 (1)
D-E	0/8	-77.4 -77.4	0.46 (1)	10.00	K-F	-584/0	0.10 (1)
E-F	0/8	-77.4 -77.4	0.46 (1)	10.00	F-G	-50/0	0.05 (1)
F-G	0/8	-77.4 -77.4	0.46 (1)	10.00	G-H	-215/0	0.04 (1)
G-H	-77/0	-77.4 -77.4	0.05 (1)	6.25	H-I	-222/0	0.00 (1)
H-I	0/60	-77.4 -77.4	0.04 (1)	10.00	I-J	-225/0	0.00 (1)
I-J	0/11	-77.4 -77.4	0.01 (1)	10.00			
B-O	0/62	-28.0 -28.0	0.07 (1)	10.00			
O-N	0/62	-28.0 -28.0	0.19 (2)	10.00			
N-M	0/53	-28.0 -28.0	0.25 (2)	10.00			
M-L	0/28	-28.0 -28.0	0.25 (2)	10.00			
L-K	0/28	-28.0 -28.0	0.25 (2)	10.00			
K-J	-2/39	-28.0 -28.0	0.25 (2)	10.00			
J-Q	0/48	-28.0 -28.0	0.19 (2)	10.00			
Q-H	0/48	-28.0 -28.0	0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.46 (D-F:1), BC=0.25 (M-N:2), WB=0.10 (F-K:1), SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

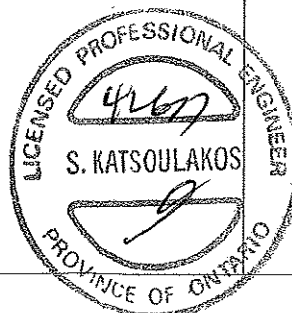
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (K) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)



DWG NO. TAM 1948517
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281216	PB5	1	1	TRUSS DESC.		

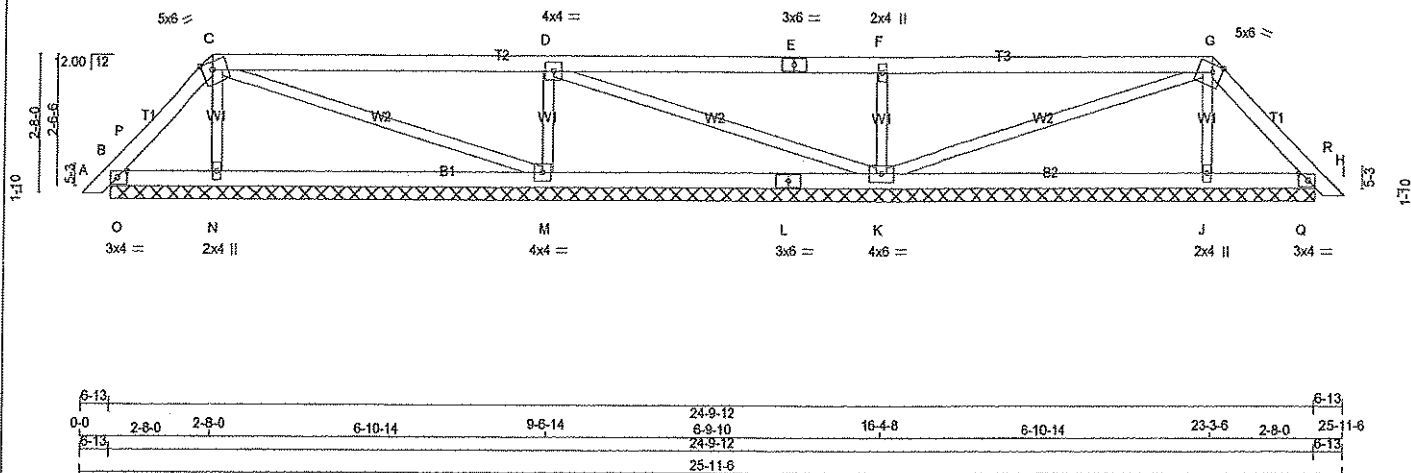
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Apr 26 15:58:41 2017 Page 1

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0-0 2-8-0 2-8-0 6-10-14 9-6-14 6-9-10 16-4-8 6-10-14 23-3-6 2-8-0 25-11-6

Scale = 1:43.7



TOTAL WEIGHT = 84 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
B	163	0	163	0
N	397	0	397	0
M	774	0	774	0
K	798	0	798	0
J	402	0	402	0
H	152	0	152	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
B	104	106 / 0 0 / -10 0 / 0 0 / 0 7 / 0 0 / 0
N	365	139 / 0 125 / 0 0 / 0 0 / 0 101 / 0 0 / 0
M	636	349 / 0 145 / 0 0 / 0 0 / 0 142 / 0 0 / 0
K	652	364 / 0 145 / 0 0 / 0 0 / 0 144 / 0 0 / 0
J	368	142 / 0 125 / 0 0 / 0 0 / 0 102 / 0 0 / 0
H	96	99 / 0 0 / -10 0 / 0 0 / 0 6 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)		FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 11	-77.4 -77.4	0.01 (1)	N-C	-215 / 0	0.03 (1)	
B-P	0 / 88	-77.4 -77.4	0.05 (2)	C-M	-21 / 2	0.02 (1)	
P-C	-84 / 0	-77.4 -77.4	0.03 (1)	M-D	-574 / 0	0.09 (1)	
C-D	-22 / 1	-77.4 -77.4	0.50 (1)	D-K	-29 / 0	0.03 (1)	
D-E	0 / 6	-77.4 -77.4	0.50 (1)	K-F	-583 / 0	0.09 (1)	
E-F	0 / 6	-77.4 -77.4	0.50 (1)	K-G	-39 / 0	0.04 (1)	
F-G	0 / 6	-77.4 -77.4	0.50 (1)	J-G	-221 / 0	0.04 (1)	
G-R	-69 / 0	-77.4 -77.4	0.03 (1)	O-P	-200 / 0	0.00 (1)	
R-H	0 / 90	-77.4 -77.4	0.05 (2)	Q-R	-203 / 0	0.00 (1)	
H-I	0 / 11	-77.4 -77.4	0.01 (1)				
B-O	-3 / 52	-28.0 -28.0	0.06 (1)				
O-N	-3 / 52	-28.0 -28.0	0.21 (2)				
N-M	-4 / 42	-28.0 -28.0	0.27 (2)				
M-L	-1 / 22	-28.0 -28.0	0.27 (2)				
L-K	-1 / 22	-28.0 -28.0	0.27 (2)				
K-J	-5 / 31	-28.0 -28.0	0.27 (2)				
J-Q	-4 / 42	-28.0 -28.0	0.21 (2)				
Q-H	-4 / 42	-28.0 -28.0	0.05 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 7.0 PSF
DL = 10.5 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.50 (C-D:1), BC=0.27 (M-N:2), WB=0.09 (F-K:1), SS=0.25 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

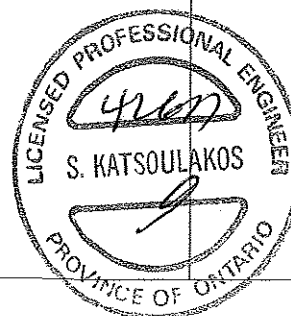
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.11 (G) (INPUT = 1.00)

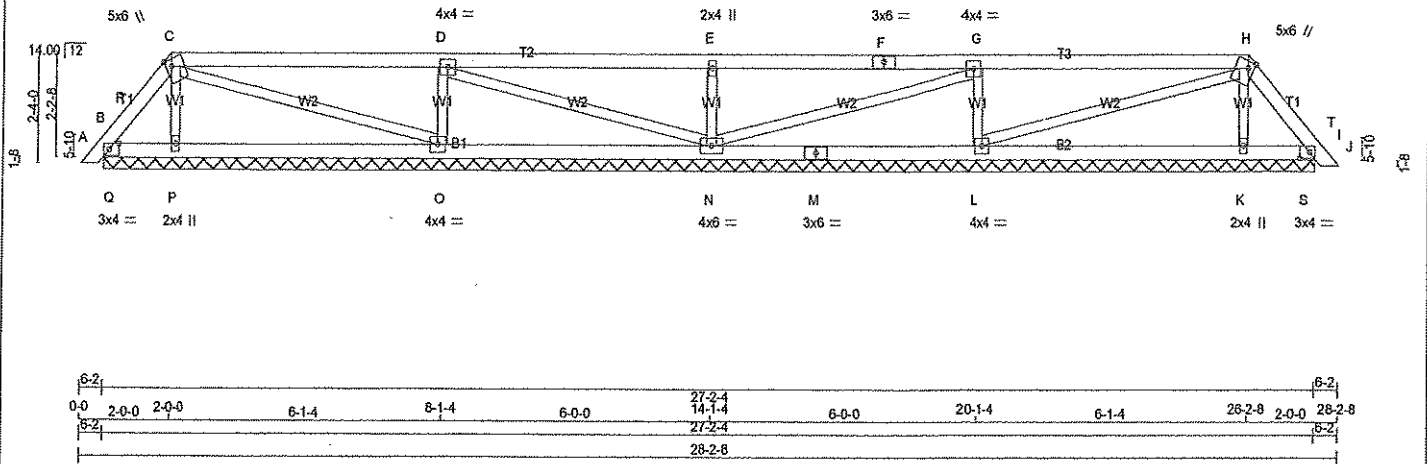


DWG NO. TAM/9466-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	PB30	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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28-2-8 2-0-0 28-2-8
Scale: 1/4"=1'



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
B - M	2x4	DRY	No.2
M - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN
B	TMB1-I	MT20	3.0	4.0
C	TTWW+m	MT20	5.0	6.0
D	TMWW-I	MT20	4.0	4.0
E	TMWW-w	MT20	2.0	4.0
F	TS-I	MT20	3.0	6.0
G	TMWW-I	MT20	4.0	4.0
H	TTWW+m	MT20	5.0	6.0
I	TMB1-I	MT20	3.0	4.0
K	BMW1-w	MT20	2.0	4.0
L	BMWW1-I	MT20	4.0	4.0
M	BS-I	MT20	3.0	6.0
N	BMWW1-I	MT20	4.0	6.0
O	BMWW1-I	MT20	4.0	4.0
P	BMW1-w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD	BRG	BRG	
B	119	0	119	0	-30	27-2-4 (11-3-27-2-4			
P	333	0	337	0	0	27-2-4 (11-3-27-2-4			
O	703	0	703	0	0	27-2-4 (11-3-27-2-4			
N	615	0	615	0	0	27-2-4 (11-3-27-2-4			
L	703	0	703	0	0	27-2-4 (11-3-27-2-4			
K	333	0	337	0	0	27-2-4 (11-3-27-2-4			
I	119	0	119	0	-30	27-2-4 (11-3-27-2-4			

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT I FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	67	87/0	0/1-19	0/0	0/0	0/1-1	0/0
P	312	110/0	113/0	0/0	0/0	90/0	0/0
O	576	320/0	129/0	0/0	0/0	127/0	0/0
N	512	271/0	124/0	0/0	0/0	118/0	0/0
L	576	320/0	129/0	0/0	0/0	127/0	0/0
K	312	110/0	113/0	0/0	0/0	90/0	0/0
I	67	87/0	0/1-19	0/0	0/0	0/1-1	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, P, O, N, L, K, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM	TO		FR-TO		FROM	TO	
A-B	0/10	-77.4	-77.4	0.01 (1)	10.00	P-C	-171/0	0.03 (1)	
B-R	0/95	-77.4	-77.4	0.04 (2)	10.00	C-O	-18/3	0.01 (1)	
R-C	-73/0	-77.4	-77.4	0.03 (2)	6.25	O-D	-526/0	0.08 (1)	
C-D	-13/1	-77.4	-77.4	0.42 (1)	6.25	D-N	-38/0	0.03 (1)	
D-E	0/20	-77.4	-77.4	0.42 (1)	10.00	N-E	-428/0	0.06 (1)	
E-F	0/20	-77.4	-77.4	0.42 (1)	10.00	N-G	-38/0	0.03 (1)	
F-G	0/20	-77.4	-77.4	0.42 (1)	10.00	L-G	-526/0	0.08 (1)	
G-H	-13/1	-77.4	-77.4	0.42 (1)	6.25	L-H	-18/3	0.01 (1)	
H-T	-73/0	-77.4	-77.4	0.03 (2)	6.25	K-H	-171/0	0.03 (1)	
T-I	0/95	-77.4	-77.4	0.04 (2)	10.00	Q-R	-150/0	0.00 (1)	
I-J	0/10	-77.4	-77.4	0.01 (1)	10.00	S-T	-150/0	0.00 (1)	
B-Q	-3/40	-28.0	-28.0	0.03 (1)	10.00				
Q-P	-3/40	-28.0	-28.0	0.16 (2)	10.00				
P-O	-4/31	-28.0	-28.0	0.21 (2)	10.00				
O-N	-1/14	-28.0	-28.0	0.21 (2)	10.00				
N-M	-1/14	-28.0	-28.0	0.21 (2)	10.00				
M-L	-1/14	-28.0	-28.0	0.21 (2)	10.00				
L-K	-4/31	-28.0	-28.0	0.21 (2)	10.00				
K-S	-3/40	-28.0	-28.0	0.16 (2)	10.00				
S-I	-3/40	-28.0	-28.0	0.03 (1)	10.00				

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 23.3 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 43.8 PSF
SPACING	= 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.42 (E-G:1), BC=0.21 (K-L:2), WB=0.08 (G-L:1), SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (N) (INPUT = 0.90)
JSI METAL= 0.14 (M) (INPUT = 1.00)



DRWG NO. 19514-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281218	PB31	1	1	TRUSS DESC.		

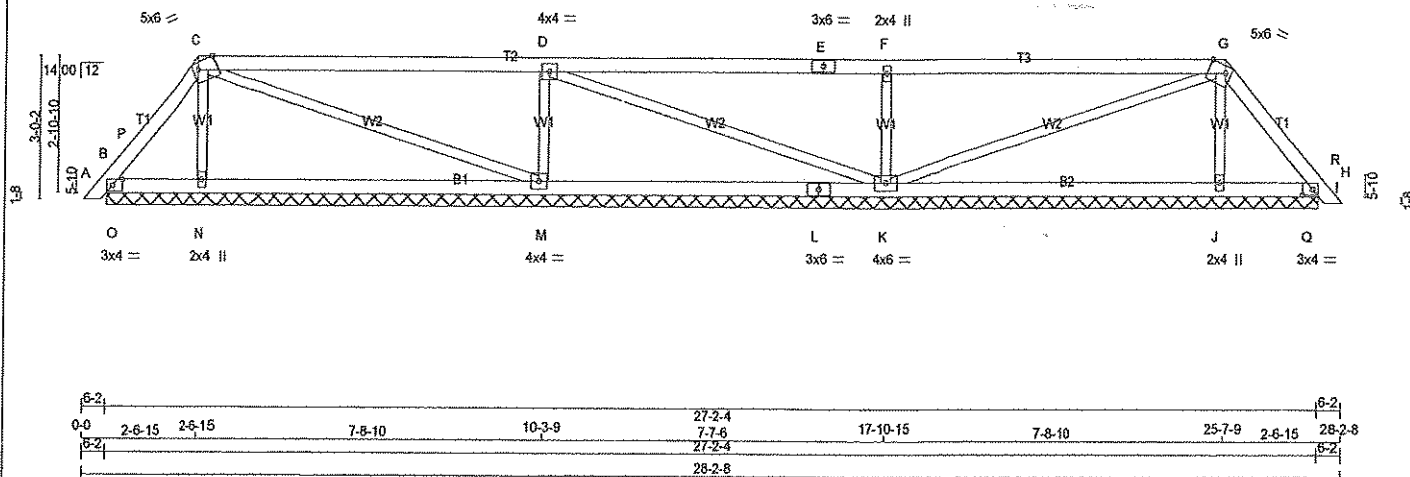
Tamarack Roof Truss, Burlington

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0-0 2-6-15 2-6-15 7-8-10 10-3-9 7-7-6 17-10-15 7-8-10 25-7-9 2-6-15 28-2-8

Scale: 1/4"=1'



TOTAL WEIGHT = 93 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
G - I	2x4	DRY	No.2 SPF
B - L	2x4	DRY	No.2 SPF
L - H	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
DRY, SEASONED LUMBER			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW-m	MT20	5.0	6.0	Edge	4.50
D	TMWW-I	MT20	4.0	4.0		
E	TS-I	MT20	3.0	6.0		
F	TMWW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	Edge	4.50
H	TMB1-I	MT20	3.0	4.0	1.50	2.50
J	BMWW1-w	MT20	2.0	4.0		
K	BMWW1-I	MT20	4.0	6.0		
L	BS-I	MT20	3.0	6.0		
M	BMWW1-I	MT20	4.0	4.0		
N	BMWW1-w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
		DOWN	HORIZ	DOWN	HORIZ		
B	161	0	161	0	-30	27-2-4 (11-3-27-2-4	
N	426	0	429	0	0	27-2-4 (11-3-27-2-4	
M	885	0	865	0	0	27-2-4 (11-3-27-2-4	
K	894	0	894	0	0	27-2-4 (11-3-27-2-4	
J	434	0	434	0	0	27-2-4 (11-3-27-2-4	
H	146	0	146	0	-31	27-2-4 (11-3-27-2-4	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN COMPONENT REACTIONS					
		1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD
B	94	114/0	0/-21	0/0	0/0	0/0	1/0
N	399	141/0	144/0	0/0	0/0	114/0	0/0
M	711	390/0	162/0	0/0	0/0	158/0	0/0
K	731	408/0	162/0	0/0	0/0	161/0	0/0
J	405	146/0	144/0	0/0	0/0	115/0	0/0
H	84	105/0	0/-21	0/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0/10	-77.4	-77.4 0.01 (1)	10.00	N-C	-222/0	0.04 (1)
B-P	0/149	-77.4	-77.4 0.07 (2)	10.00	C-M	-23/4	0.03 (1)
P-C	-101/0	-77.4	-77.4 0.04 (2)	6.25	M-D	-641/0	0.11 (1)
C-D	-26/3	-77.4	-77.4 0.62 (1)	6.25	D-K	-35/0	0.05 (1)
D-E	0/7	-77.4	-77.4 0.52 (1)	10.00	K-F	-652/0	0.11 (1)
E-F	0/7	-77.4	-77.4 0.62 (1)	10.00	K-G	-44/2	0.06 (1)
F-G	0/7	-77.4	-77.4 0.62 (1)	10.00	J-G	-229/0	0.04 (1)
G-R	-82/0	-77.4	-77.4 0.04 (2)	6.25	O-P	-247/0	0.00 (1)
R-H	0/151	-77.4	-77.4 0.07 (2)	10.00	Q-R	-251/0	0.00 (1)
H-I	0/10	-77.4	-77.4 0.01 (1)	10.00			
B-O	-6/57	-28.0	-28.0 0.06 (1)	10.00			
O-N	-6/57	-28.0	-28.0 0.26 (2)	10.00			
N-M	-7/48	-28.0	-28.0 0.34 (2)	10.00			
M-L	-3/26	-28.0	-28.0 0.34 (2)	10.00			
L-K	-3/26	-28.0	-28.0 0.34 (2)	10.00			
K-J	-6/36	-28.0	-28.0 0.34 (2)	10.00			
J-Q	-7/45	-28.0	-28.0 0.26 (2)	10.00			
Q-H	-7/45	-28.0	-28.0 0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.62 (C-D:1), BC=0.34 (M-N:2), WB=0.11 (F-K:1), SS=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

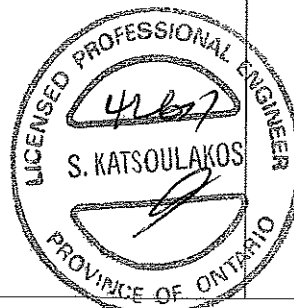
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

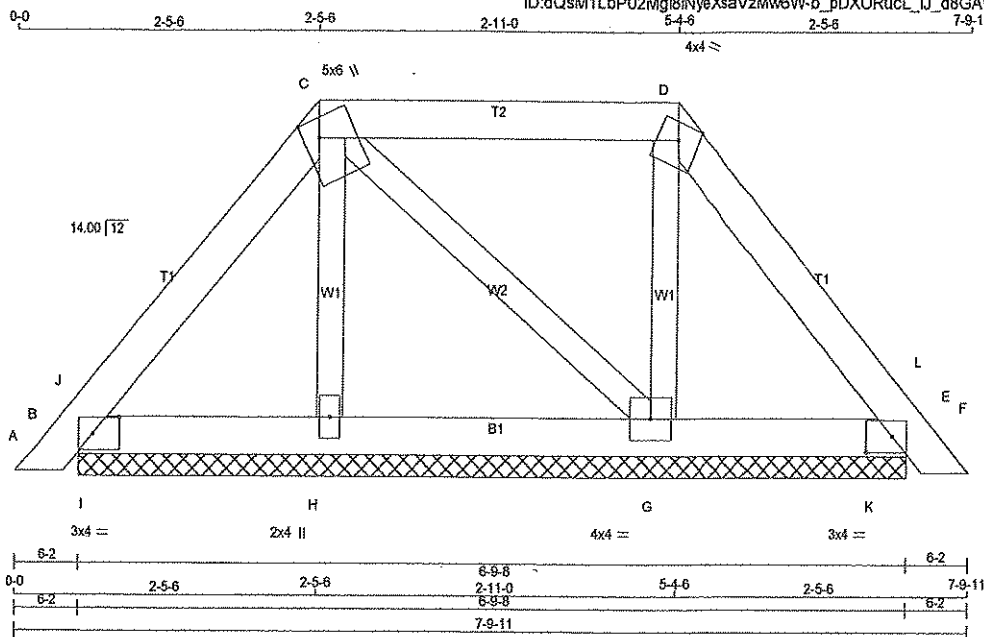
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)



DWG NO. TAM19515-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281218	TRUSS NAME PB32	QUANTITY 1	PLY 1	JOB DESC. 44080	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Wed Apr 26 15:57:31 2017 Page 1			ID: dQsM1LbP02Mgi8NyeXsaVzWw6W-b_pDXORucL_IJ_d8GAWis_FrKWejpmhSIEB15zMpNI



Scale = 1:17.5

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0 1.50 2.50
C	TTWW+m	MT20	5.0	6.0 1.75 1.50
D	TTWW-m	MT20	4.0	4.0 Edge
E	TMB1-I	MT20	3.0	4.0 1.50 2.50
G	BMW1-I	MT20	4.0	4.0
H	BMW1+w	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQ'D BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	198	0	198	0	0	6-9-8	6-9-8	6-9-8	6-9-8
E	181	0	181	0	0	6-9-8	6-9-8	6-9-8	6-9-8
H	178	0	178	0	0	6-9-8	6-9-8	6-9-8	6-9-8
G	220	0	220	0	0	6-9-8	6-9-8	6-9-8	6-9-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
B	148	105/0	17/0
E	136	95/0	18/0
H	162	64/0	54/0
G	190	89/0	54/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0/10	-77.4	-77.4	0.01 (1)	10.00	H-C	-93/0
B-J	-62/0	-77.4	-77.4	0.02 (1)	6.25	C-G	-19/0
J-C	-98/0	-77.4	-77.4	0.03 (1)	6.25	G-D	-121/0
C-D	-40/0	-77.4	-77.4	0.11 (1)	6.25	I-J	-113/17
D-L	-75/0	-77.4	-77.4	0.03 (1)	6.25	K-L	-118/17
L-E	-38/0	-77.4	-77.4	0.01 (1)	6.25		
E-F	0/10	-77.4	-77.4	0.01 (1)	10.00		
B-I	0/59	-28.0	-28.0	0.04 (1)	10.00		
I-H	0/59	-28.0	-28.0	0.04 (2)	10.00		
H-G	0/55	-28.0	-28.0	0.04 (2)	10.00		
G-K	0/45	-28.0	-28.0	0.04 (2)	10.00		
K-E	0/45	-28.0	-28.0	0.04 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (C-D:1), BC=0.04 (H-I:2), WB=0.02 (D-G:1), SSI=0.10 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
CGMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

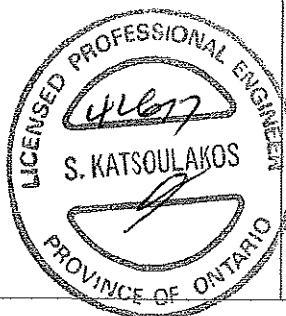
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

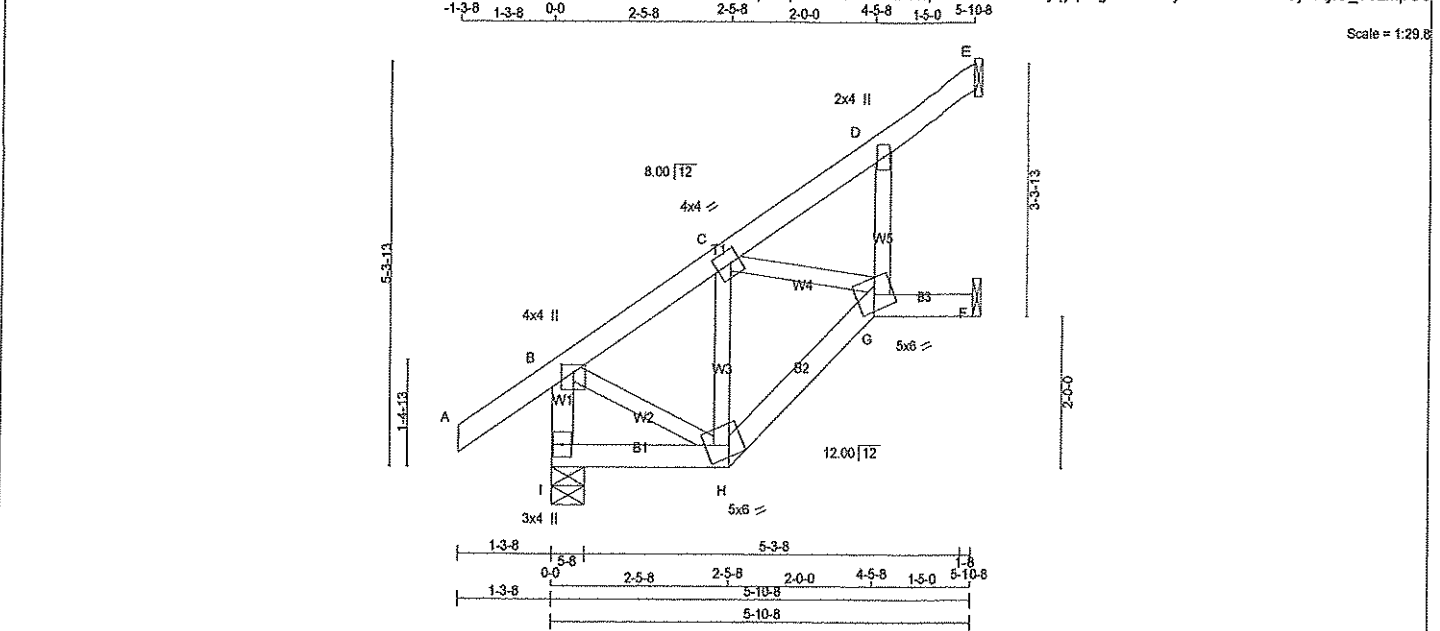
JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



DRWG NO. TAM 19516-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281216	TRUSS NAME J1S	QUANTITY 8	PLY 1	JOB DESC. 44080	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Apr 26 15:56:40 2017 Page 1
ID:qZQz7kzPUU6ZmHN5p7PirzNAUi-aAEyqqvqdgYxh8DLny6k8NRAm5KGjMAjto_?YzMpOS



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
I - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW+p	MT20	4.0	4.0	1.25 2.00
C	TMWW-I	MT20	4.0	4.0	2.00 1.50
D	TMWW-w	MT20	2.0	4.0	
G	BBWW-m	MT20	5.0	6.0	
H	BBWW-m	MT20	5.0	6.0	Edge 2.25
I	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	SNOW	LIVE	DOWN	UP	BRG	IN-SX	IN-SX	
I	418	0	416	0	0	5-8	5-8	5-8	
E	290	0	290	0	0	1-8	1-8	1-8	
F	20	0	35	0	0	1-8	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) E, F

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL
I	329	201 / 0	62 / 0	0 / 0	0 / 0	67 / 0	0 / 0	0 / 0	0 / 0
E	232	137 / 0	47 / 0	0 / 0	0 / 0	49 / 0	0 / 0	0 / 0	0 / 0
F	25	0 / 0	15 / 0	0 / 0	0 / 0	10 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	FORCE (LBS)	MAX. (LC)	MAX. (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
I-B	-381 / 0	0.0	0.0 0.04 (1)	H-C	-234 / 0	0.64 (1)	0.64 (1)
A-B	0 / 29	-77.4	-77.4 0.10 (1)	G-D	0 / 312	0.07 (1)	0.07 (1)
B-C	-230 / 0	-77.4	-77.4 0.16 (1)	B-H	0 / 233	0.05 (1)	0.05 (1)
C-D	-121 / 0	-77.4	-77.4 0.45 (1)	C-G	-210 / 0	0.03 (1)	0.03 (1)
D-E	0 / 131	-77.4	-77.4 0.47 (1)				
I-H	0 / 0	-28.0	-28.0 0.05 (2)				
H-G	0 / 290	-28.0	-28.0 0.07 (2)				
G-F	0 / 0	-28.0	-28.0 0.02 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/680$ (0.10")

CSI: TC=0.47 (D-E-1), BC=0.07 (G-H-2), WB=0.07 (D-G-1), SS=0.19 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

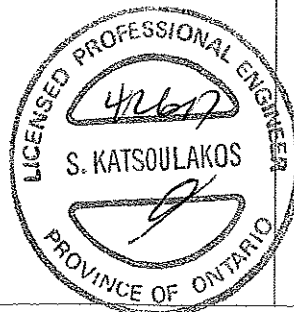
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
		822	2284
			1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)



DWG NO. TAM 19487-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281218	TRUSS NAME J30S	QUANTITY 8	PLY 1	JOB DESC. 44080 TRUSS DESC.	DRWG NO.
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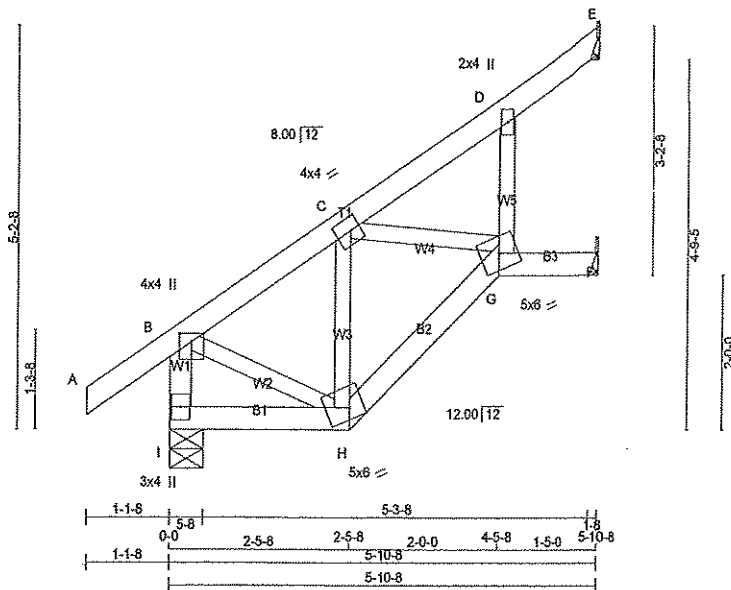
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Apr 26 15:57:30 2017 Page 1

ID: dQsM1LbP02Mgi8iNyeXsaVzW6W-6nFrK2QGr1mhq2yIS?TAeS_Yv90_LkXDeVdiezMpNj

-1-1-8 0-0 2-5-8 2-5-8 2-0-0 4-5-8 1-5-0 5-10-8

Scale = 1:29.2



TOTAL WEIGHT = 8 X 25 = 203 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-l	MT20	4.0	4.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
G	BBWW-m	MT20	5.0	6.0		
H	BBWW-m	MT20	5.0	6.0	Edge	2.25
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
I	403	0	403	0	0	5-8	5-8		
E	290	0	290	0	0				
F	20	0	35	0	0				

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): E

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	321	193 / 0	62 / 0	0 / 0	0 / 0	66 / 0	0 / 0
E	232	137 / 0	47 / 0	0 / 0	0 / 0	49 / 0	0 / 0
F	25	0 / 0	15 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO
I-B	-368 / 0	0.0	0.0
A-B	0 / 26	-77.4	-77.4
B-C	-240 / 0	-77.4	-77.4
C-D	-121 / 0	-77.4	-77.4
D-E	0 / 131	-77.4	-77.4
I-H	0 / 0	-28.0	-28.0
H-G	0 / 302	-28.0	-28.0
G-F	0 / 0	-28.0	-28.0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/680$ (0.10")

CSI: TC=0.47 (D-E:1), BC=0.07 (G-H:2), WB=0.07 (D-G:1), SSI=0.19 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

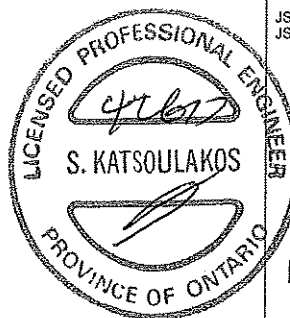
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)



DWG NO. TAM 19517-17
STRUCTURAL
COMPONENT ONLY

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information			Application number:	
Building number, street name			Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description		
B. Individual who reviews and takes responsibility for design activities				
Name SAM KATSOULAKOS, P. ENG.		Firm MICRO CITY ENGINEERING SERVICES INC.		
Street address R.R #1, PO BOX 61			Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail	
Telephone number (519) 287-2242 Business		Fax number (519) 287-5750	Cell number	
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]				
☐ House ☐ Small Buildings ☐ Large Buildings ☐ Complex Buildings ☐ HVAC – House ☐ Building Services ☐ Detection, Lighting and Power ☐ Fire Protection ☒ Building Structural ☐ Plumbing – House ☐ Plumbing – All Buildings ☐ On-site Sewage Systems				
Description of designer's work GREENYORK HOMES – DEGREY DRIVE – MODEL: YORK 1 – ELEV. A (SCHEDULE IS NOT ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM25740-17 DATED 5-24-17). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.				
D. Declaration of Designer				
I, <u>SAM KATSOULAKOS, P. ENG</u> declare that (choose one as appropriate): (print name) ☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories. Individual BCIN: <u>26064</u> Firm BCIN: <u>29991</u> ☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code. Individual BCIN: _____ Basis for exemption from registration: _____ ☐ The design work is exempt from the registration and qualification requirements of the Building Code. Basis for exemption from registration and qualification: _____				
I certify that: 1. The information contained in this schedule is true to the best of my knowledge. 2. I have submitted this application with the knowledge and consent of the firm.				
Date		Signature of Designer		

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d) of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG#TAM25740-17-S
DWG#TAM 25742-17-S

5-24-17

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information		Application number:	
Building number, street name		Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description	
B. Individual who reviews and takes responsibility for design activities			
Name SAM KATSOULAKOS, P. ENG.		Firm MICRO CITY ENGINEERING SERVICES INC.	
Street address R.R #1, PO BOX 61		Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750	Cell number	
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]			
☐ House ☐ Small Buildings ☐ Large Buildings ☐ Complex Buildings ☐ HVAC – House ☐ Building Services ☐ Detection, Lighting and Power ☐ Fire Protection ☒ Building Structural ☐ Plumbing – House ☐ Plumbing – All Buildings ☐ On-site Sewage Systems			
Description of designer's work GREENYORK HOMES – DEGREY DRIVE – MODEL: YORK 1 – ELEV. B (SCHEDULE IS NOT ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM25741-17 DATED 5-24-17). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.			
D. Declaration of Designer			
I, <u>SAM KATSOULAKOS, P. ENG</u> declare that (choose one as appropriate): (print name)			
☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.			
Individual BCIN: <u>26064</u>			
Firm BCIN: <u>29991</u>			
☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code.			
Individual BCIN: _____			
Basis for exemption from registration: _____			
☐ The design work is exempt from the registration and qualification requirements of the Building Code.			
Basis for exemption from registration and qualification: _____			
I certify that:			
1. The information contained in this schedule is true to the best of my knowledge.			
2. I have submitted this application with the knowledge and consent of the firm.			
Date		Signature of Designer	
<u>5-24-17</u>			

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d. of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG#TAM 25741-17-S
DWG#TAM 25743-17-S

52417


HGUS – Double Shear Joist Hangers

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

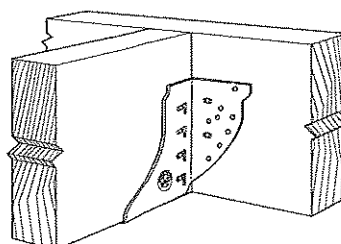
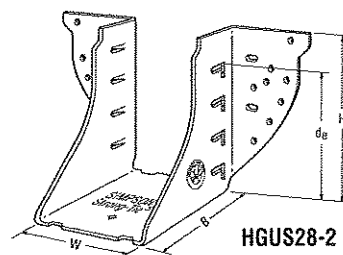
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

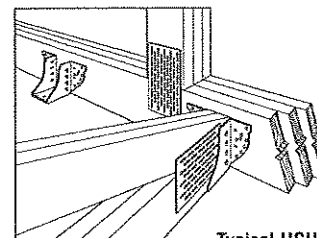
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



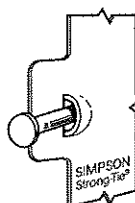
Typical HGUS
Installation



Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

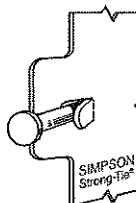
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
HGUS26	12	1½	5½	5	4½ ₃₂	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3½ ₁₆	5½ ₁₆	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4½ ₁₆	5½	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6½ ₁₆	5½ ₁₆	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3½ ₁₆	7½ ₁₆	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4½ ₁₆	7½	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6½ ₁₆	7½ ₁₆	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3½ ₁₆	9½ ₁₆	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGU210-3	12	4½ ₁₆	9½	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGU210-4	12	6½ ₁₆	9½ ₁₆	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6½ ₁₆	10½	4	10½	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6½ ₁₆	12½	4	11½	66-16d	22-16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

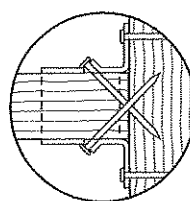


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

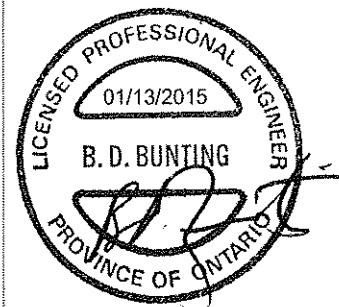
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

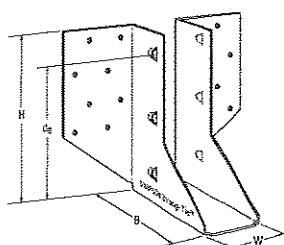
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%
No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

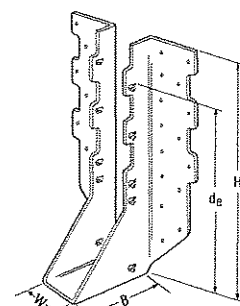
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

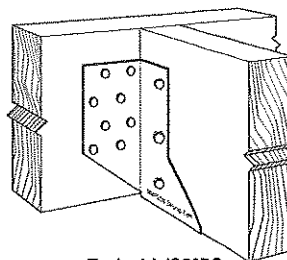
- See current catalogue for options



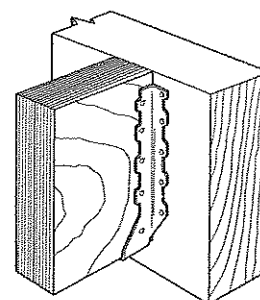
LJS26DS



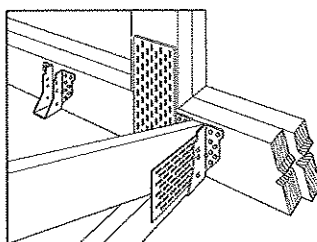
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



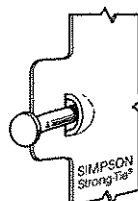
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

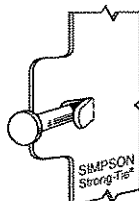
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 3/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

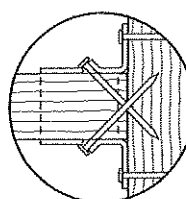


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

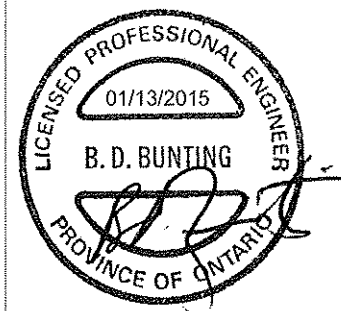
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

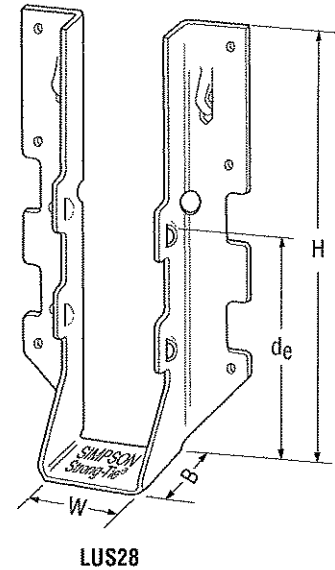
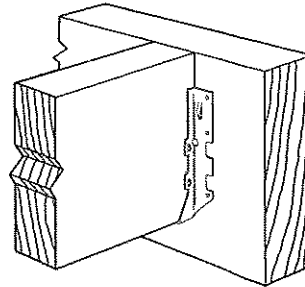
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

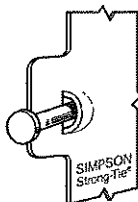
- These hangers cannot be modified.

Typical LUS Installation



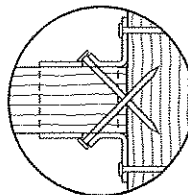
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d ₂ ¹	Face	Joist	D. Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1½	3½	1¾	1½	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	4-16d	2-16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3¾	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¾	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¾	2	3¾	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1½	7½	1¾	3¾	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¾	8-16d	6-16d	2580	3345	2320	2375

1. d₂ is the distance from the seat of the hanger to the highest joist nail.

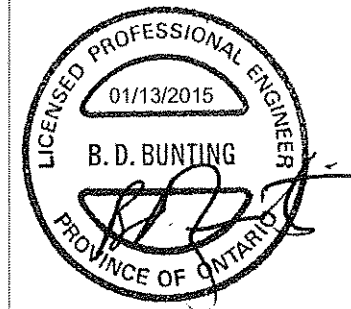


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECLUS15 1/15 exp. 12/16

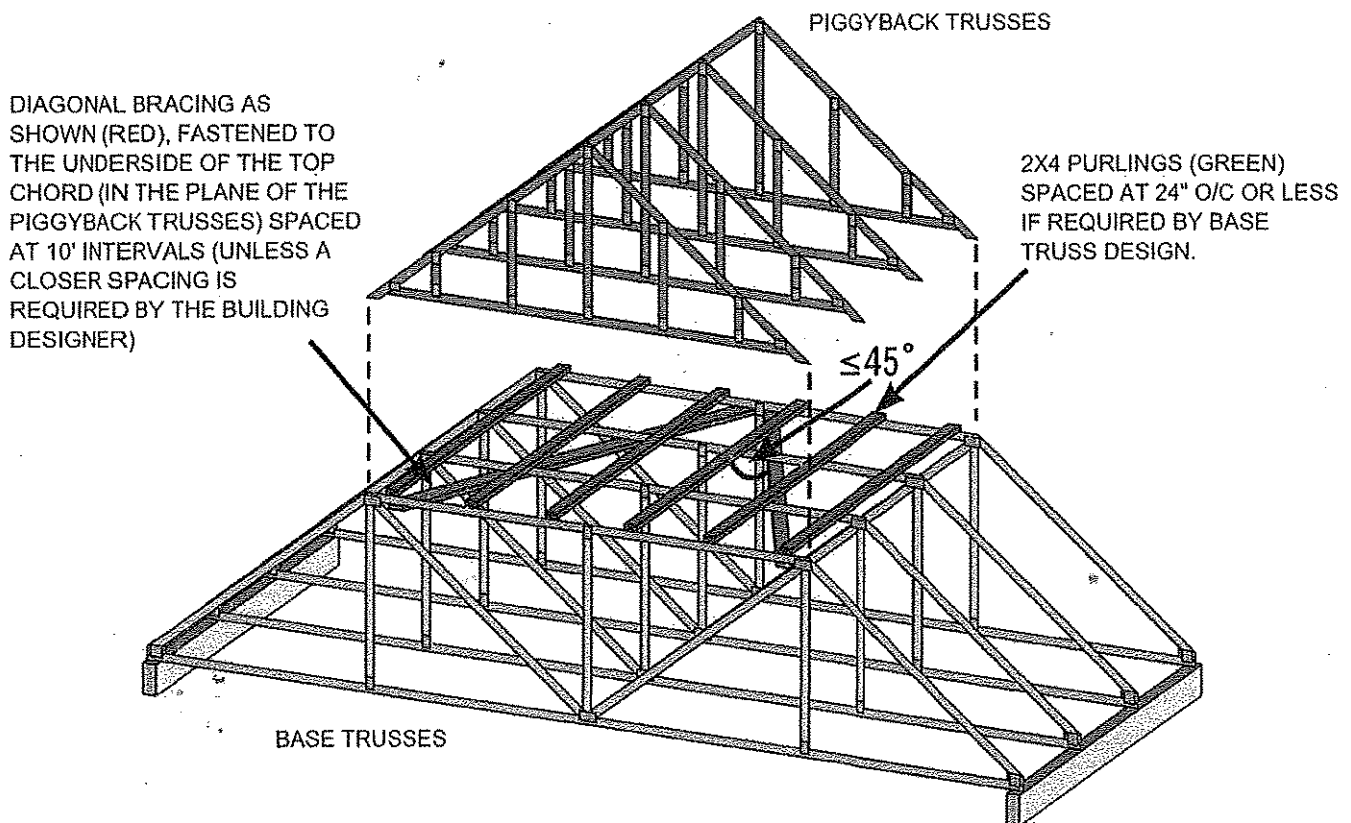
800-999-5099
www.strongtie.com

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

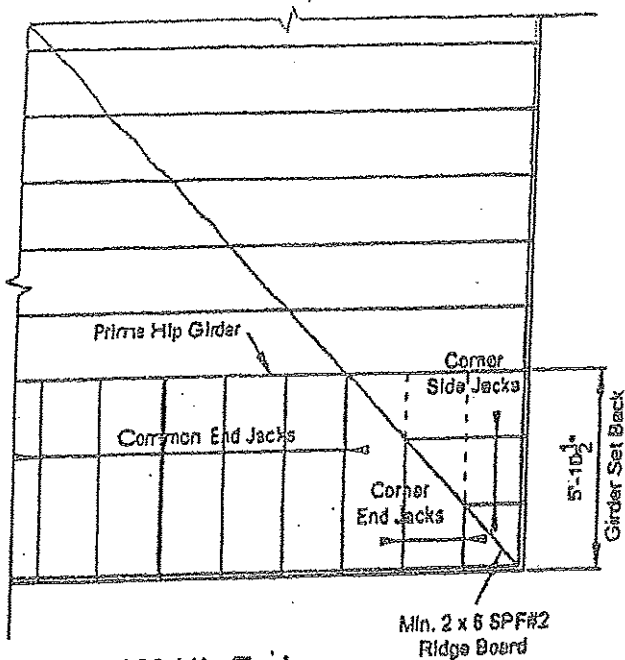
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

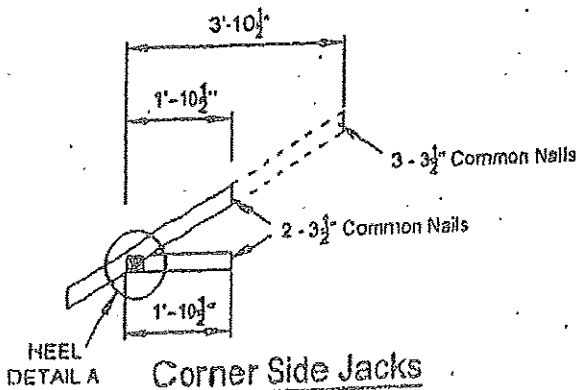
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

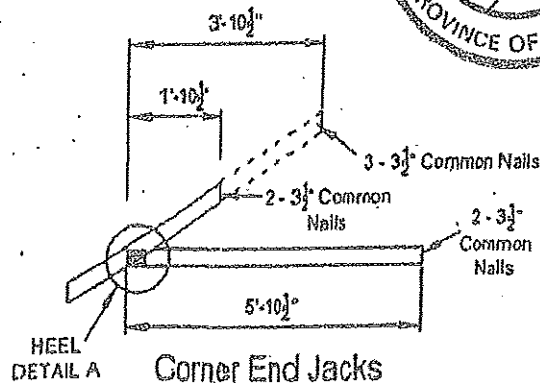
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

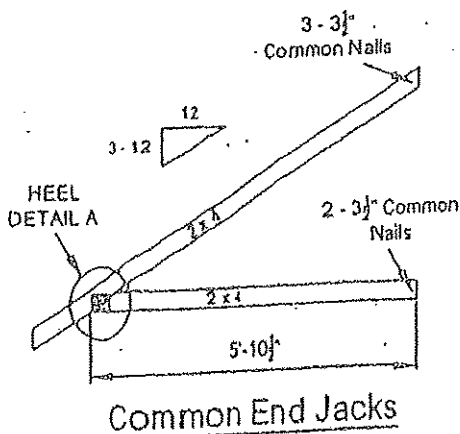
DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



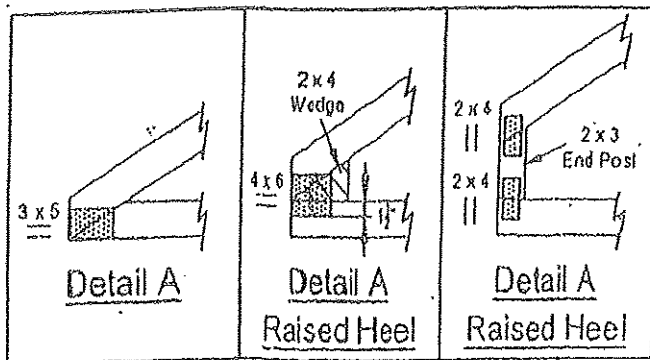
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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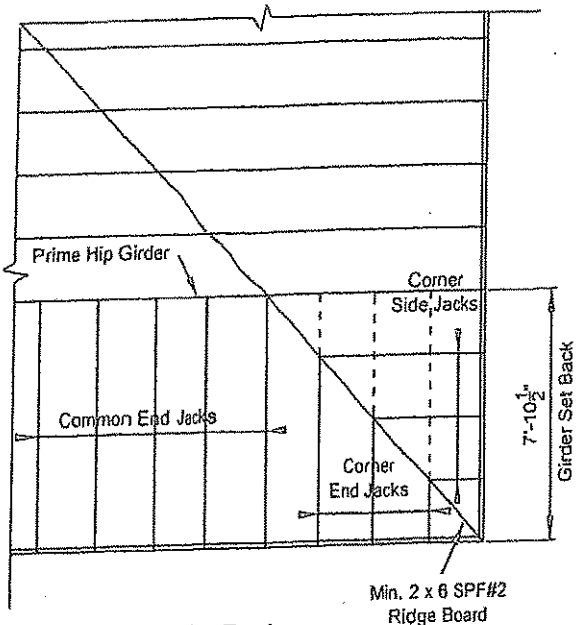
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

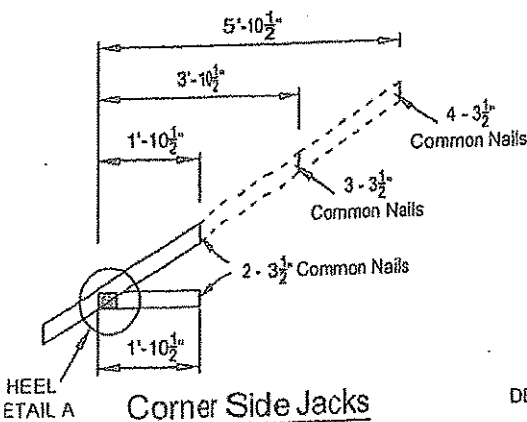
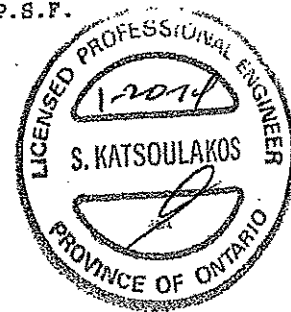
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

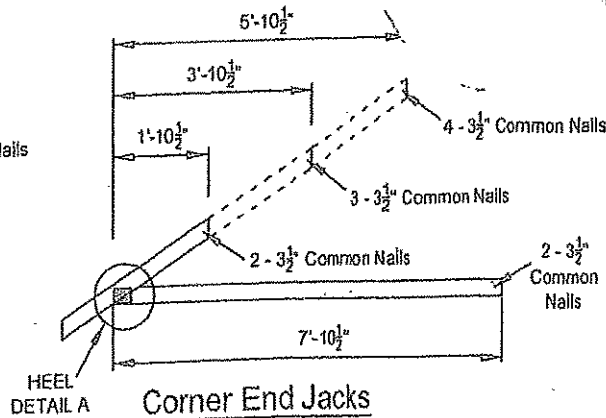


45° Hip End

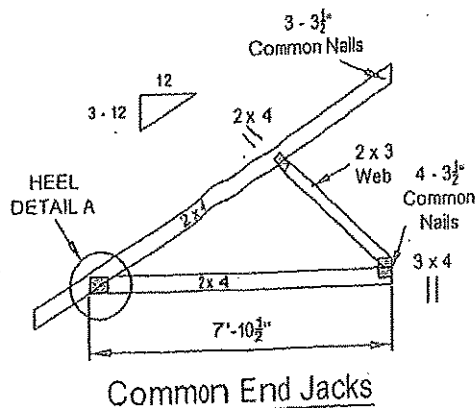
DWG NO TAM 3503.14
STRUCTURAL
COMPONENT ONLY



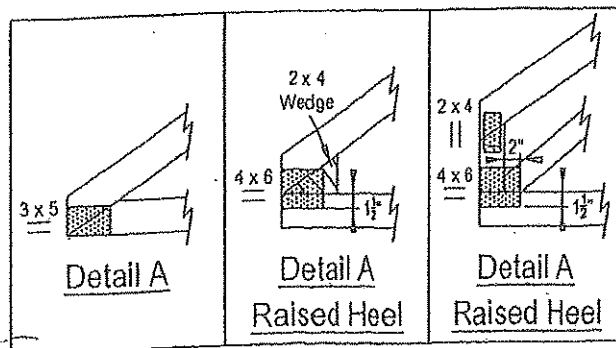
Corner Side Jacks



Corner End Jacks



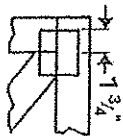
Common End Jacks



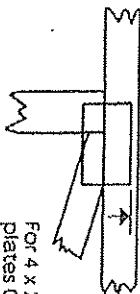
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

PLATE LOCATION AND ORIENTATION

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths or mm.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{8}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MITEK software or upon request.

PLATE SIZE

4 X 4

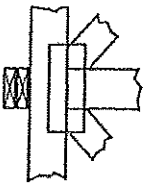
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



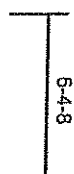
indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Trust Design Procedures and Specifications

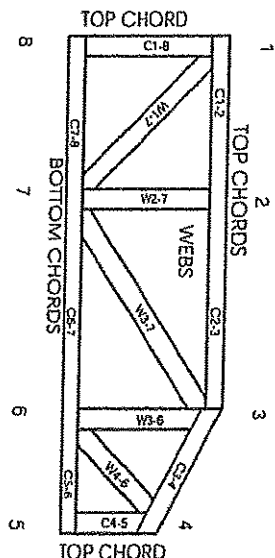
DSB-89:
BCSI:
Design Standard for Bracing.
Building Component Safety Information.

**Building Component Safety Information
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.**

Numbering System



5-4-8 dimensions shown in ft.-in.-sixteenths or mm.
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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KITZ

POWER TO PERFORM.

MITek Engineering Reference Sheet: MIT-7473C rev. 10-'06

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

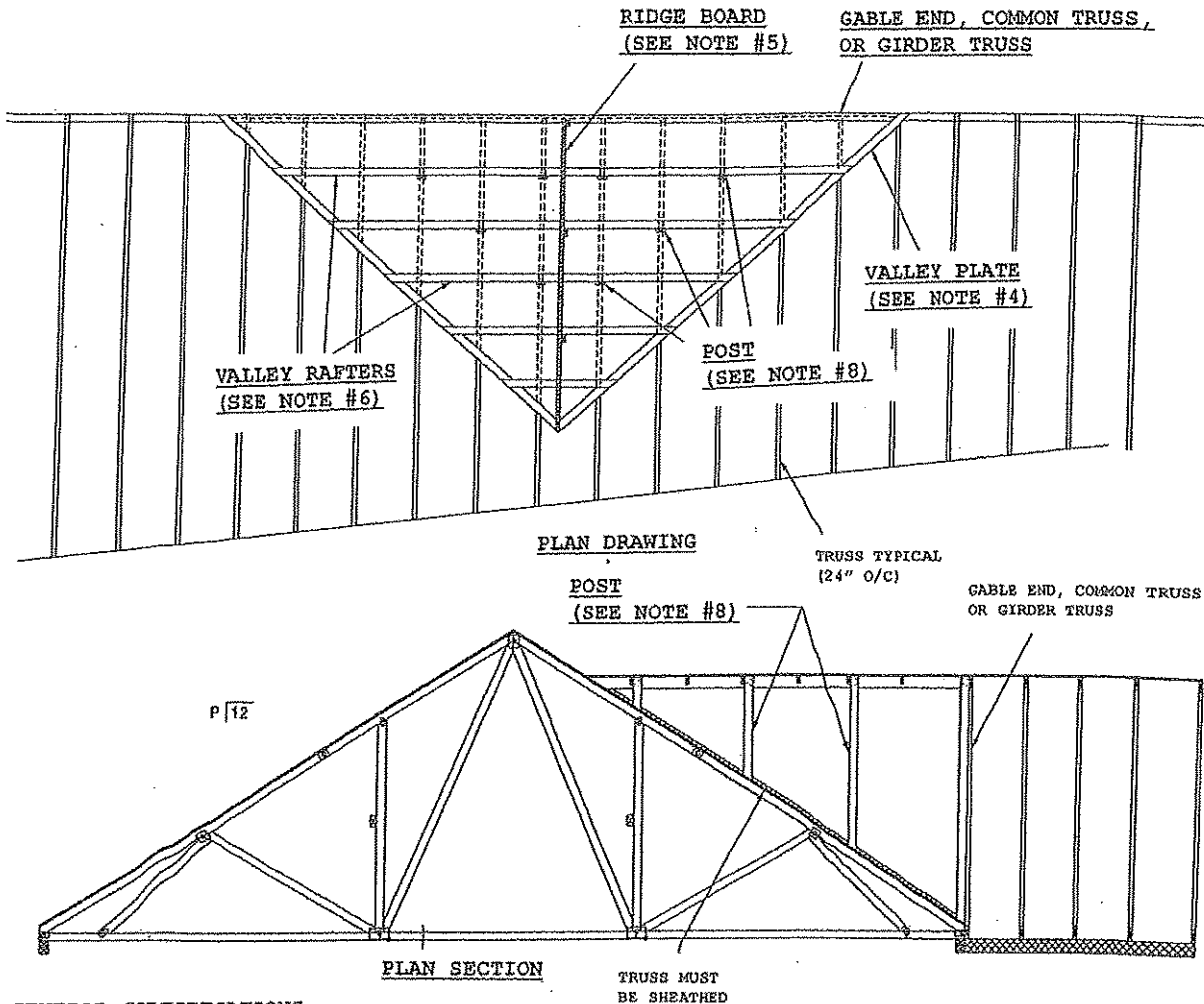
1. Diagonal or X-bracing. Is always required. See BCSS.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral bracing members may require bracing, or alternative 1, 4, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never sack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully, knots and waste at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is *not* sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.).
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.).
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.).
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

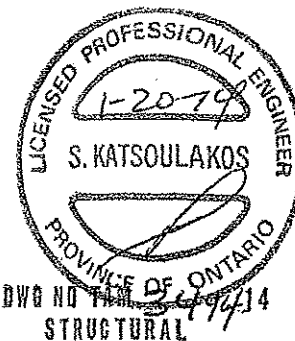


DWG NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.