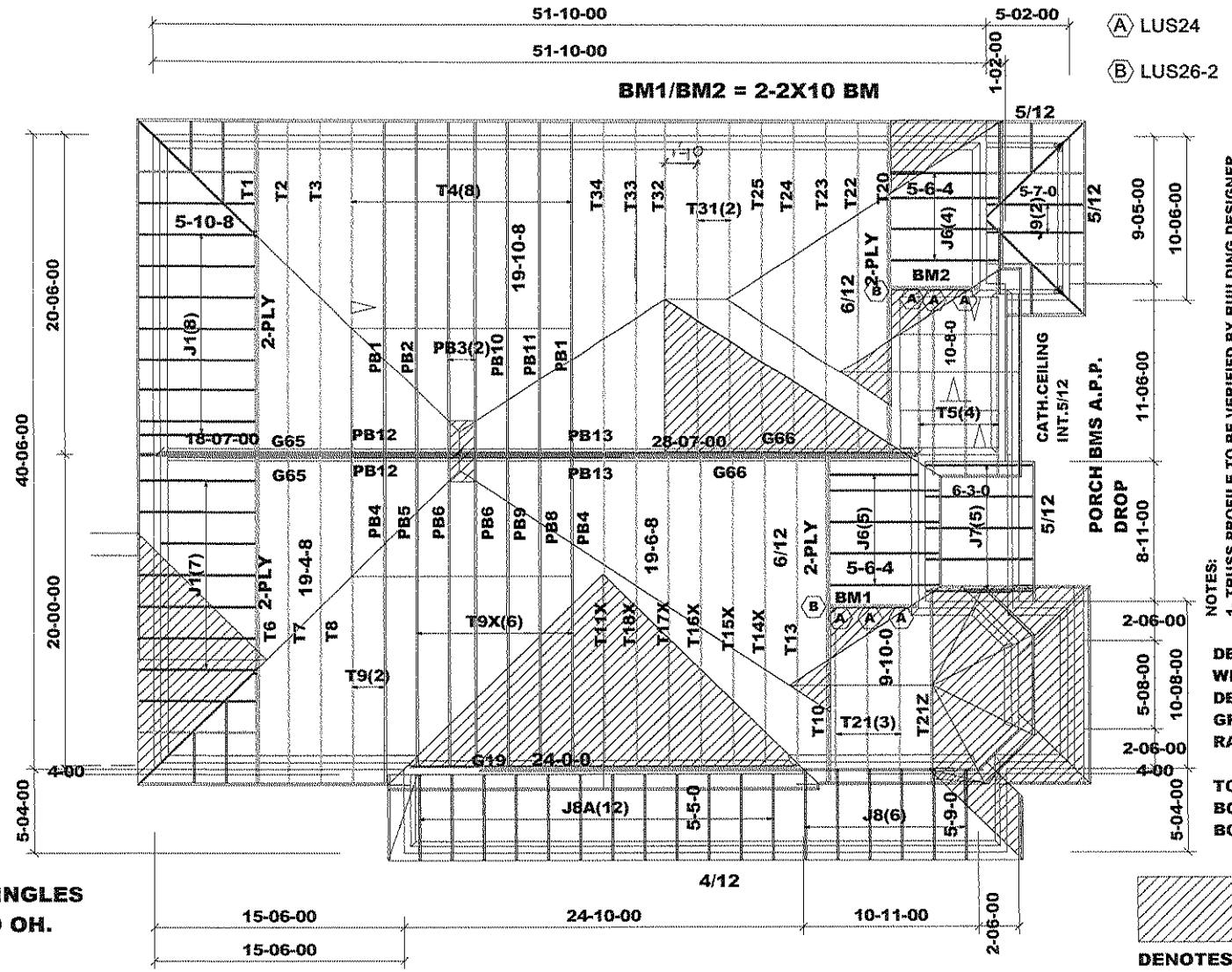
SLOPE 9/12 TYP.
UNLESS NOTED



NOTES:
1- TRUSS PROFILE TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:

GROUND SNOW	1.5 KPA
RAIN	8.4 PSF
TC DEAD	3.0 PSF
BC LIVE	10.5 PSF
BC DEAD	7. PSF

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 FASCIA BOARD
2x6 EXTERIOR WALLS

T-160416

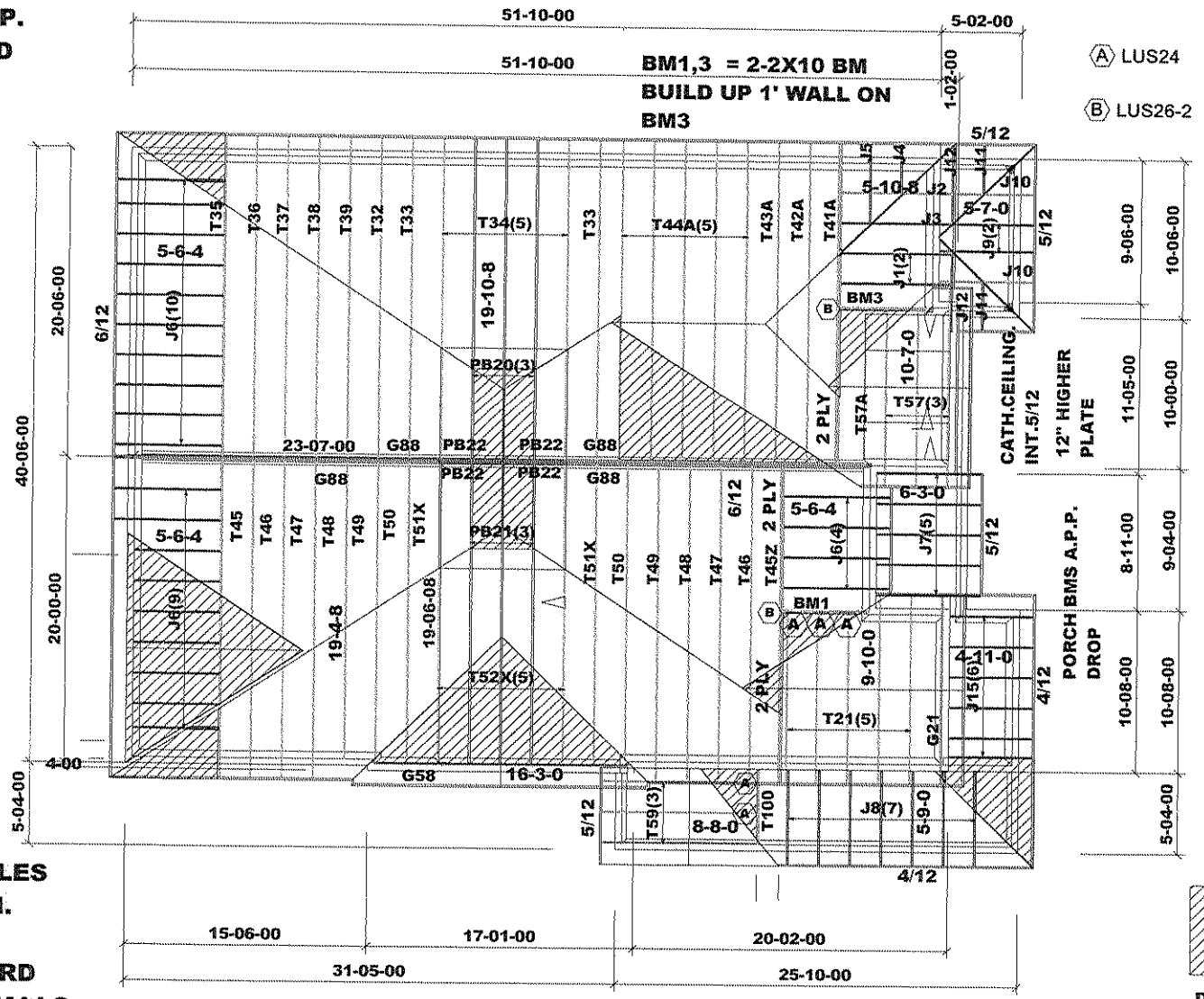
DENOTES:
CONVENTIONAL
FRAMING

M9770

TAMARACK	Job Track: 42181	Builder / Location: GREENPARK HOMES / VAUGHAN	Model / Elevation: ELMBROOK 10A/4A / EL-1
	Layout ID: 267484	Project: STAR TIME HOMES	
	Plan Log: 86179	Date: 5/25/2016 Designer: FV/JG	

Mitek ver 7.5.0

SLOPE 9/12 TYP.
UNLESS NOTED



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 FASCIA BOARD
2x6 EXTERIOR WALLS

NOTES:
1- TRUSS PROFILE TO BE VERIFIED BY BUILDING DESIGNER
ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
GROUND SNOW 1.5 KPA
RAIN 8.4 PSF
TC DEAD 3.0 PSF
BC LIVE 10.5 PSF
BC DEAD 7. PSF

DENOTES:
CONVENTIONAL
FRAMING

T-160416

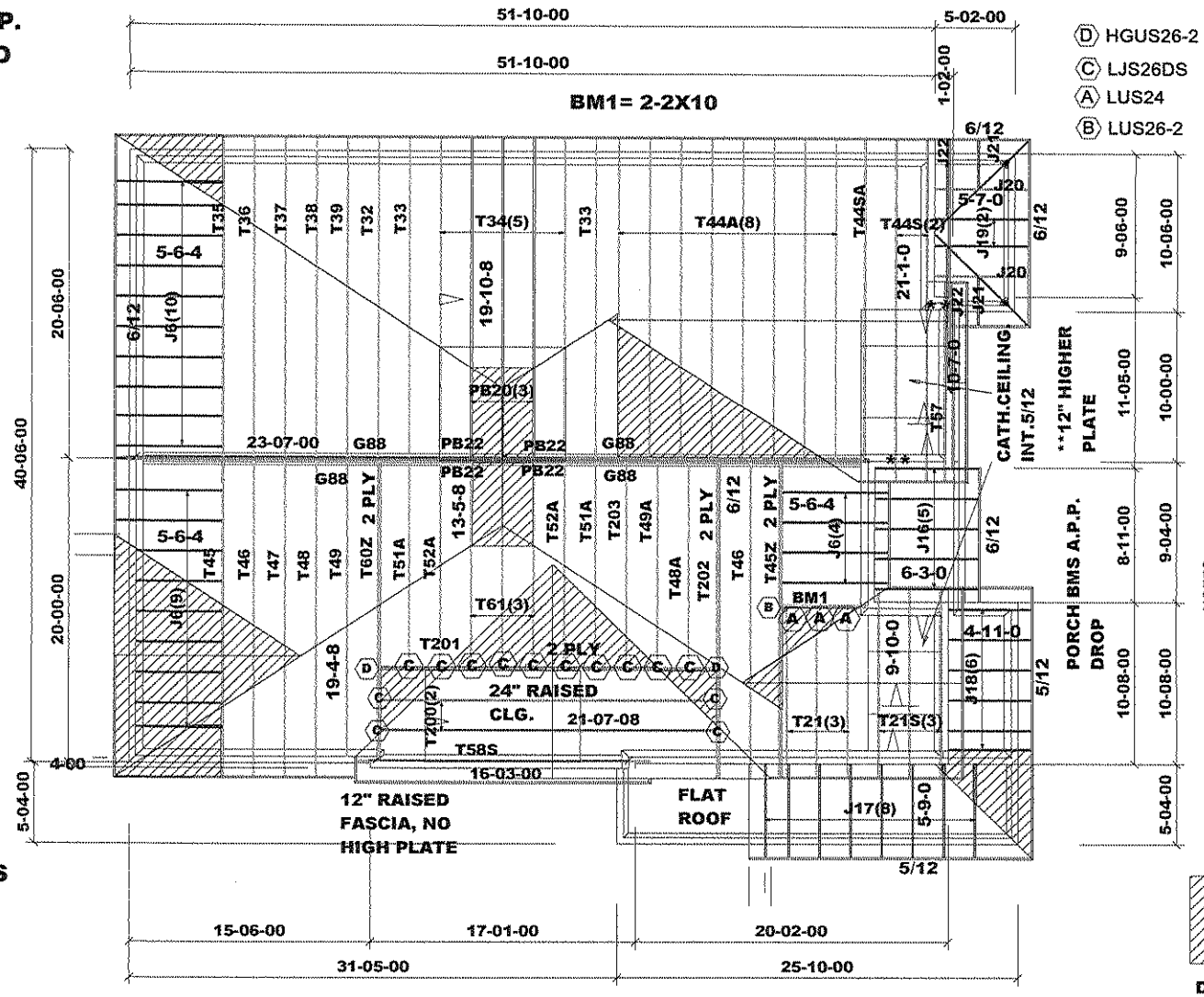
M9770

TAMARACK LUMBER INC.

Job Track: 42181	Builder / Location: GREENPARK HOMES / VAUGHAN
Layout ID: 267487	Model / Elevation: ELMBROOK 10A/4A / EL-2
Plan Log: 86179	Project: STAR TIME HOME LTD
	Date: 5/26/2016 Designer: FV/JG

Mitek ver 7.5.0

SLOPE 9/12 TYP.
UNLESS NOTED



NOTES:
1- TRUSS PROFILE TO BE VERIFIED BY BUILDING DESIGNER
ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:

GROUND SNOW	1.5 KPA
RAIN	8.4 PSF
TC DEAD	3.0 PSF
BC LIVE	10.5 PSF
BC DEAD	7. PSF

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 FASCIA BOARD
2x6 EXTERIOR WALLS

T-140822

DENOTES:
CONVENTIONAL
FRAMING

M9770

TAMARACK LUMBER INC.

Job Track:	42181
Layout ID:	267491
Plan Log:	86179

Builder / Location: GREENPARK HOMES / VAUGHAN

Model / Elevation: ELMBROOK 10A /4A / EL-3

Project: STAR TIME HOME LTD.

Date: 5/26/2016 Designer: FV/JG

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

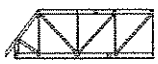
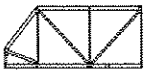
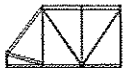
Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267485 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK 10A/4A ELEVATION: 10A-1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T6 HALF HIP	9.00 0.00	19-04-08	05-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 05-11-02	171.92 111.34		
	1	T7 HALF HIP	9.00 0.00	19-04-08	07-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 07-05-02	91.69 57.33		
	1	T8 HALF HIP	9.00 0.00	19-04-08	08-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 08-11-02	101.98 64.83		
	2	T9 PIGGYBACK	9.00 0.00	19-04-08	10-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 10-05-02	219.12 135.34		
	6	T9X PIGGYBACK	9.00 0.00	19-06-08	10-05-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-12 10-05-02	648.12 397.98		
	1 2 Ply	T10 HALF HIP	9.00 0.00	19-04-08	03-11-02	2 X 4	2 X 6	01-03-08 00-00-00	01-06-04 03-11-02	180.22 115.00		
	1	T11X HALF HIP	9.00 0.00	19-06-08	10-11-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-12 10-11-02	114.17 70.33		
	1	T13 HALF HIP	9.00 0.00	19-04-08	04-11-02	2 X 4	2 X 4	00-00-00 00-00-00	01-06-04 04-11-02	76.07 48.50		
	1	T14X HALF HIP	9.00 0.00	19-06-08	05-11-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-12 05-11-02	80.75 50.67		
	1	T15X HALF HIP	9.00 0.00	19-06-08	06-11-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-12 06-11-02	87.64 55.50		
	1	T16X HALF HIP	9.00 0.00	19-06-08	07-11-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-12 07-11-02	94.83 59.67		
	1	T17X HALF HIP	9.00 0.00	19-06-08	08-11-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-12 08-11-02	100.42 62.83		
	1	T18X HALF HIP	9.00 0.00	19-06-08	09-11-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-12 09-11-02	112.77 71.33		
	1	G19 COMMON	9.00 0.00	24-00-00	10-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	123.75 78.00		
	3	T21 COMMON	9.00 0.00	09-10-00	05-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	129.48 82.50		
	1	T21Z COMMON	9.00 0.00	09-10-00	05-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	43.16 27.50		
	1	G65 PIGGYBACK	9.00 0.00	18-07-00	10-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 10-05-02	116.05 72.17		
	1	G66 PIGGYBACK	6.00 0.00	28-07-00	10-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-05-02	156.72 96.34		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

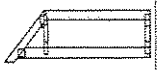
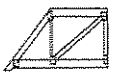
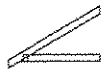
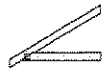
Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267485 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK 10A/4A ELEVATION: 10A-1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	PB4 PIGGYBACK	9.00 0.00	06-09-13	01-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 01-04-08	36.88 23.66		
	1	PB5 PIGGYBACK	9.00 0.00	06-09-13	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 02-10-08	25.02 16.17		
	2	PB6 PIGGYBACK	9.00 0.00	06-09-13	04-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 04-04-08	57.28 36.00		
	1	PB8 PIGGYBACK	9.00 0.00	06-09-13	02-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 02-04-08	24.10 16.17		
	1	PB9 PIGGYBACK	9.00 0.00	06-09-13	03-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 03-04-08	26.08 16.17		
	1	PB12 PIGGYBACK	9.00 0.00	06-00-05	04-10-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 04-10-14	21.28 14.00		
	1	PB13 PIGGYBACK	6.00 0.00	09-01-07	04-10-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 04-10-14	33.58 22.00		
	7	J1 JACK-OPEN	9.00 0.00	05-10-08	05-11-02	2 X 4	2 X 4	01-05-00 00-00-00	01-06-04 05-11-02	162.47 105.00		
	1	J2 JACK-OPEN	9.00 0.00	05-10-08	02-10-05	2 X 4	2 X 4	01-03-08 -04-01-01	01-06-04 00-03-08	17.21 11.67		
	1	J3 JACK-OPEN	9.00 0.00	05-10-08	04-04-05	2 X 4	2 X 4	01-03-08 -02-01-01	01-06-04 00-03-08	20.06 13.00		
	1	J4 JACK-OPEN	9.00 0.00	01-10-08	02-11-07	2 X 4	2 X 4	01-03-08 00-00-07	01-06-04 02-11-02	9.75 7.00		
	1	J5 JACK-OPEN	9.00 0.00	01-10-08	04-05-07	2 X 4	2 X 4	01-03-08 02-00-07	01-06-04 00-05-08	12.60 8.33		
	5	J6 JACK-OPEN	6.00 0.00	05-06-04	03-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-11-02	79.65 50.00		
	5	J7 JACK-OPEN	5.00 0.00	06-03-00	02-11-05	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 02-11-05	94.65 60.85		
	6	J8 JACK-OPEN	4.00 0.00	05-09-00	02-02-15	2 X 4	2 X 4	00-09-08 00-00-00	00-03-15 02-02-15	87.18 52.02		
	12	J8A JACK-OPEN	4.00 0.00	05-05-00	02-01-10	2 X 4	2 X 4	00-09-08 00-00-00	00-03-15 02-01-10	165.00 104.04		

TOTAL # TRUSS= **76.00** TOTAL BFT OF ALL TRUSSES= **2213.24** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3521.65** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267485 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK 10A/4A ELEVATION: 10A-1

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

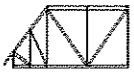
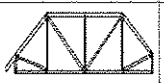
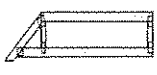
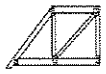
Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267486 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK10A/4A ELEVATION: 4A-1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1 HALF HIP	9.00 0.00	19-10-08	05-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 05-11-02	174.82 111.34		
	1	T2 HALF HIP	9.00 0.00	19-10-08	07-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 07-05-02	93.08 57.33		
	1	T3 HALF HIP	9.00 0.00	19-10-08	08-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 08-11-02	103.31 64.83		
	8	T4 PIGGYBACK	9.00 0.00	19-10-08	10-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 10-05-02	887.12 541.36		
	4	T5 ROOF	9.00 5.00	10-08-00	05-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	205.44 137.32		
	1 2 Ply	T20 HIP GIRDER	9.00 0.00	19-10-08	03-11-02	2 X 4	2 X 6	01-03-08 00-00-00	01-06-04 01-08-02	188.90 122.00		
	1	T22 HIP	9.00 0.00	19-10-08	04-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 01-08-02	81.50 52.17		
	1	T23 HIP	9.00 0.00	19-10-08	05-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 01-08-02	86.52 56.00		
	1	T24 HIP	9.00 0.00	19-10-08	06-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 01-08-02	87.52 55.83		
	1	T25 HIP	9.00 0.00	19-10-08	07-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 01-08-02	96.58 62.00		
	2	T31 HIP	9.00 0.00	19-10-08	08-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 01-08-02	177.90 112.34		
	1	T32 HALF HIP	9.00 0.00	19-10-08	09-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 09-00-02	103.78 64.83		
	1	T33 HALF HIP	9.00 0.00	19-10-08	09-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 09-11-02	115.67 72.67		
	1	T34 HALF HIP	9.00 0.00	19-10-08	10-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 10-11-02	117.02 72.33		
	1	G65 PIGGYBACK	9.00 0.00	18-07-00	10-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 10-05-02	116.05 72.17		
	1	G66 PIGGYBACK	6.00 0.00	28-07-00	10-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-05-02	156.72 96.34		
	2	PB1 PIGGYBACK	9.00 0.00	07-03-13	01-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 01-04-08	39.16 25.00		
	1	PB2 PIGGYBACK	9.00 0.00	07-03-13	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 02-10-08	26.49 17.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

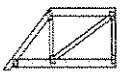
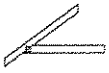
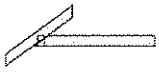
Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267486 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK10A/4A ELEVATION: 4A-1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	PB3 PIGGYBACK	9.00 0.00	07-03-13	04-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 04-04-08	59.82 38.34		
	1	PB10 PIGGYBACK	9.00 0.00	07-03-13	03-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 03-04-08	27.50 17.50		
	1	PB11 PIGGYBACK	9.00 0.00	07-03-13	02-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 02-04-08	25.61 16.83		
	1	PB12 PIGGYBACK	9.00 0.00	06-00-05	04-10-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 04-10-14	21.28 14.00		
	1	PB13 PIGGYBACK	6.00 0.00	09-01-07	04-10-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 04-10-14	33.58 22.00		
	8	J1 JACK-OPEN	9.00 0.00	05-10-08	05-11-02	2 X 4	2 X 4	01-05-00 00-00-00	01-06-04 05-11-02	185.68 120.00		
	1	J2 JACK-OPEN	9.00 0.00	05-10-08	02-10-05	2 X 4	2 X 4	01-03-08 -04-01-01	01-06-04 00-03-08	17.21 11.67		
	1	J3 JACK-OPEN	9.00 0.00	05-10-08	04-04-05	2 X 4	2 X 4	01-03-08 -02-01-01	01-06-04 00-03-08	20.06 13.00		
	1	J4 JACK-OPEN	9.00 0.00	01-10-08	02-11-07	2 X 4	2 X 4	01-03-08 00-00-07	01-06-04 02-11-02	9.75 7.00		
	1	J5 JACK-OPEN	9.00 0.00	01-10-08	04-05-07	2 X 4	2 X 4	01-03-08 02-00-07	01-06-04 00-05-08	12.60 8.33		
	4	J6 JACK-OPEN	6.00 0.00	05-06-04	03-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-11-02	63.72 40.00		
	2	J9 JACK-OPEN	5.00 0.00	04-09-00	01-11-00	2 X 4	2 X 4	01-03-08 -00-11-09	00-04-01 00-03-08	25.56 16.00		
	2	J10 JACK-OPEN	5.00 0.00	04-09-00	01-01-00	2 X 4	2 X 4	01-03-08 -02-11-09	00-04-01 00-03-08	20.62 13.34		
	2	J11 JACK-OPEN	5.00 0.00	01-10-08	01-01-00	2 X 4	2 X 4	01-03-08 -00-01-01	00-04-01 00-03-08	12.16 8.00		
	2	J12 JACK-OPEN	5.00 0.00	01-10-08	01-11-00	2 X 4	2 X 4	01-03-08 01-10-15	00-04-01 00-04-07	17.10 10.66		

TOTAL # TRUSS= **62.00** TOTAL BFT OF ALL TRUSSES= **2149.86** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3409.83** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

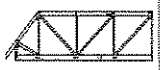
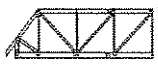
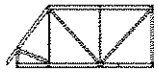
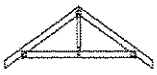
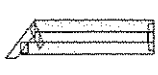
Delivery Shiplist

DATE	05/26/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267489 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK10A/4A ELEVATION: 10A-2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T21 COMMON	9.00 0.00	09-10-00	05-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	215.80 137.50		
	1	G21 COMMON	9.00 0.00	09-10-00	05-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	45.34 30.50		
	1	T45 HALF HIP	9.00 0.00	19-04-08	03-11-02	2 X 4	2 X 6	01-03-08 00-00-00	01-06-04 03-11-02	89.39 55.67		
	1 2 Ply	T45Z HALF HIP	9.00 0.00	19-04-08	03-11-02	2 X 4	2 X 6	01-03-08 00-00-00	01-06-04 03-11-02	180.22 115.00		
	2	T46 HALF HIP	9.00 0.00	19-04-08	04-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 04-11-02	155.84 99.66		
	2	T47 HALF HIP	9.00 0.00	19-04-08	05-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 05-11-02	164.38 105.34		
	2	T48 HALF HIP	9.00 0.00	19-04-08	06-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 06-11-02	178.34 113.66		
	2	T49 HALF HIP	9.00 0.00	19-04-08	07-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 07-11-02	192.78 122.00		
	2	T50 HALF HIP	9.00 0.00	19-04-08	08-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 08-11-02	203.96 129.66		
	2	T51X HALF HIP	9.00 0.00	19-06-08	09-11-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-12 09-11-02	225.54 142.66		
	5	T52X HALF HIP	9.00 0.00	19-06-08	10-11-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-12 10-11-02	570.85 351.65		
	1	G58 COMMON	9.00 0.00	16-03-00	07-07-06	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	77.06 50.50		
	3	T59 COMMON	5.00 0.00	08-08-00	02-03-00	2 X 4	2 X 4	01-03-08 01-03-08	00-05-06 00-05-06	76.17 48.99		
	2	G88 PIGGYBACK	6.00 0.00	23-07-00	10-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-11-02	267.84 170.00		
	3	PB21 PIGGYBACK	9.00 0.00	06-01-13	00-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 00-10-08	47.28 33.51		
	2	PB22 PIGGYBACK	6.00 0.00	03-01-07	01-10-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 01-10-14	20.40 14.66		
	13	J6 JACK-OPEN	6.00 0.00	05-06-04	03-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-11-02	207.09 130.00		
	5	J7 JACK-OPEN	5.00 0.00	06-03-00	02-11-05	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 02-11-05	93.45 60.85		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

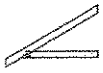
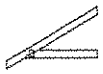
Delivery Shiplist

DATE	05/26/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267489 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK10A/4A ELEVATION: 10A-2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	7	J8 JACK-OPEN	4.00 0.00	05-09-00	02-02-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-15	105.91 65.31		
	6	J15 JACK-OPEN	4.00 0.00	04-11-00	01-11-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-11-10	79.08 48.00		

TOTAL # TRUSS= **68.00** TOTAL BFT OF ALL TRUSSES= **2025.12** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3196.72** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
5	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **6.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

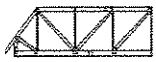
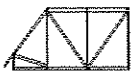
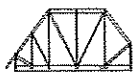
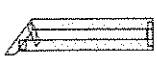
Delivery Shiplist

DATE	05/26/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267490 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK10A/4A ELEVATION: 4A-2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T32 HALF HIP	9.00 0.00	19-10-08	08-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 08-11-02	103.31 64.83		
	2	T33 HALF HIP	9.00 0.00	19-10-08	09-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 09-11-02	231.34 145.34		
	5	T34 HALF HIP	9.00 0.00	19-10-08	10-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 10-11-02	585.10 361.65		
	1	T35 HALF HIP	9.00 0.00	19-10-08	03-11-02	2 X 4	2 X 6	01-03-08 00-00-00	01-06-04 03-11-02	91.92 59.00		
	1	T36 HALF HIP	9.00 0.00	19-10-08	04-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 04-11-02	84.92 54.50		
	1	T37 HALF HIP	9.00 0.00	19-10-08	05-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 05-11-02	83.65 52.67		
	1	T38 HALF HIP	9.00 0.00	19-10-08	06-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 06-11-02	90.59 56.83		
	1	T39 HALF HIP	9.00 0.00	19-10-08	07-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 07-11-02	97.76 62.67		
	1 2 Ply	T41A HIP GIRDER	9.00 0.00	19-10-08	05-11-02	2 X 4	2 X 6	01-03-08 00-00-00	01-06-04 02-08-02	205.20 131.00		
	1	T42A HIP	9.00 0.00	19-10-08	07-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 02-08-02	90.49 57.67		
	1	T43A HIP	9.00 0.00	19-10-08	08-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 02-08-02	102.69 65.33		
	5	T44A COMMON	9.00 0.00	19-10-08	09-06-10	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 02-08-02	458.95 296.65		
	3	T57 SCISSOR	9.00 5.00	10-07-00	05-05-14	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	141.12 91.50		
	1	T57A SCISSOR	9.00 5.00	10-04-08	05-05-14	2 X 4	2 X 4	00-00-00 00-00-00	01-06-04 01-08-02	42.03 27.17		
	2	G88 PIGGYBACK	6.00 0.00	23-07-00	10-11-02	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	267.84 170.00		
	3	PB20 PIGGYBACK	9.00 0.00	06-07-13	00-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 00-10-08	50.70 33.51		
	2	PB22 PIGGYBACK	6.00 0.00	03-01-07	01-10-14	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	20.40 14.66		
	2	J1 JACK-OPEN	9.00 0.00	05-10-08	05-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 05-11-02	46.06 30.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

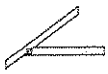
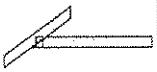
Delivery Shiplist

DATE	05/26/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267490 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK10A/4A ELEVATION: 4A-2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	J2 JACK-OPEN	9.00 0.00	05-10-08	02-10-05	2 X 4	2 X 4	01-03-08 -04-01-01	01-06-04 00-03-08	17.21 11.67		
	1	J3 JACK-OPEN	9.00 0.00	05-10-08	04-04-05	2 X 4	2 X 4	01-03-08 -02-01-01	01-06-04 00-03-08	20.06 13.00		
	1	J4 JACK-OPEN	9.00 0.00	01-10-08	02-11-07	2 X 4	2 X 4	01-03-08 00-00-07	01-06-04 02-11-02	9.75 7.00		
	1	J5 JACK-OPEN	9.00 0.00	01-10-08	04-05-07	2 X 4	2 X 4	01-03-08 02-00-07	01-06-04 00-05-08	12.60 8.33		
	10	J6 JACK-OPEN	6.00 0.00	05-06-04	03-11-02	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	159.30 100.00		
	2	J9 JACK-OPEN	5.00 0.00	04-09-00	01-11-00	2 X 4	2 X 4	01-03-08 -00-11-09	00-04-01 00-03-08	25.56 16.00		
	2	J10 JACK-OPEN	5.00 0.00	04-09-00	01-01-00	2 X 4	2 X 4	01-03-08 -02-11-09	00-04-01 00-03-08	20.62 13.34		
	2	J11 JACK-OPEN	5.00 0.00	01-10-08	01-01-00	2 X 4	2 X 4	01-03-08 -00-01-01	00-04-01 00-03-08	12.16 8.00		
	2	J12 JACK-OPEN	5.00 0.00	01-10-08	01-11-00	2 X 4	2 X 4	01-03-08 01-10-15	00-04-01 00-04-07	17.10 10.66		

TOTAL # TRUSS= **57.00** TOTAL BFT OF ALL TRUSSES= **1962.98** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3088.43** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS26-2	

TOTAL # ITEMS= **1.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

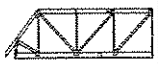
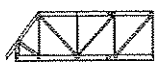
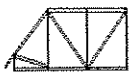
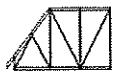
Delivery Shiplist

DATE	05/26/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267492 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK10A/4A ELEVATION: 10A-3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	3	T21 COMMON	9.00 0.00	09-10-00	05-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	129.48 82.50		
	3	T21S ROOF	9.00 5.00	09-10-00	05-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	145.35 99.00		
	1	T45 HALF HIP	9.00 0.00	19-04-08	03-11-02	2 X 4	2 X 6	01-03-08 00-00-00	01-06-04 03-11-02	89.39 55.67		
	1 2 Ply	T45Z HALF HIP	9.00 0.00	19-04-08	03-11-02	2 X 4	2 X 6	01-03-08 00-00-00	01-06-04 03-11-02	180.22 115.00		
	2	T46 HALF HIP	9.00 0.00	19-04-08	04-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 04-11-02	155.84 99.66		
	1	T47 HALF HIP	9.00 0.00	19-04-08	05-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 05-11-02	82.19 52.67		
	1	T48 HALF HIP	9.00 0.00	19-04-08	06-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 06-11-02	89.17 56.83		
	1	T48A HALF HIP	9.00 0.00	13-05-08	06-11-02	2 X 4	2 X 4	00-00-00 00-00-00	05-11-08 06-11-02	73.75 47.00		
	1	T49 HALF HIP	9.00 0.00	19-04-08	07-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 07-11-02	96.39 61.00		
	1	T49A HALF HIP	9.00 0.00	13-05-08	07-11-02	2 X 4	2 X 4	00-00-00 00-00-00	05-11-08 07-11-02	77.76 49.17		
	2	T51A HALF HIP	9.00 0.00	13-05-08	09-11-02	2 X 4	2 X 4	00-00-00 00-00-00	05-11-08 09-11-02	187.28 118.66		
	2	T52A HALF HIP	9.00 0.00	13-05-08	10-11-02	2 X 4	2 X 4	00-00-00 00-00-00	05-11-08 10-11-02	202.22 124.66		
	1	T58S ROOF	9.00 0.00	16-03-00	08-07-06	2 X 4	2 X 4	01-03-08 01-05-00	02-06-04 02-06-04	85.61 56.67		
	1 2 Ply	T60Z HALF HIP	9.00 0.00	19-04-08	08-11-02	2 X 4	2 X 6	00-00-00 00-00-00	01-06-04 08-11-02	226.60 145.66		
	3	T61 HALF HIP	9.00 0.00	13-05-08	11-11-02	2 X 4	2 X 4	00-00-00 00-00-00	05-11-08 11-11-02	283.20 177.99		
	2	G88 PIGGYBACK	6.00 0.00	23-07-00	10-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-11-02	267.84 170.00		
	2	T200 ROOF	9.00 0.00	21-07-08	11-00-04	2 X 4	2 X 4	00-00-00 00-00-00	02-08-11 03-01-03	253.14 163.00		
	1 2 Ply	T201 COMMON	9.00 0.00	21-07-08	11-00-04	2 X 4	2 X 6	00-00-00 00-00-00	02-08-11 03-01-03	245.64 153.34		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

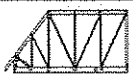
Delivery Shiplist

DATE	05/26/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267492 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK10A/4A ELEVATION: 10A-3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T202 HALF HIP	9.00 0.00	19-04-08	05-11-02	2 X 4	2 X 6	01-05-00 00-00-00	01-06-04 05-11-02	210.94 135.00		
	1	T203 HALF HIP	9.00 0.00	13-05-08	08-11-02	2 X 4	2 X 4	00-00-00 00-00-00	05-11-08 08-11-02	82.24 52.50		
	2	PB22 PIGGYBACK	6.00 0.00	03-01-07	01-10-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 01-10-14	20.40 14.66		
	13	J6 JACK-OPEN	6.00 0.00	05-06-04	03-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-11-02	207.09 130.00		
	5	J16 JACK-OPEN	6.00 0.00	06-03-00	03-05-11	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 03-05-11	96.70 60.85		
	8	J17 JACK-OPEN	5.00 0.00	05-09-00	02-08-13	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 02-08-13	123.12 74.64		
	6	J18 JACK-OPEN	5.00 0.00	04-11-00	02-04-10	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 02-04-10	80.52 48.00		

TOTAL # TRUSS= **69** TOTAL BFT OF ALL TRUSSES= **2344.13** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3692.08** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
14	Hangers	LJS26DS	
3	Hangers	LUS24	

TOTAL # ITEMS= **19**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

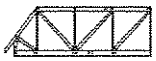
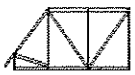
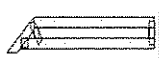
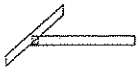
Delivery Shiplist

DATE	05/26/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267493 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK10A/4A ELEVATION: 4A-3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T32 HALF HIP	9.00 0.00	19-10-08	08-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 08-11-02	103.31 64.83		
	2	T33 HALF HIP	9.00 0.00	19-10-08	09-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 09-11-02	231.34 145.34		
	5	T34 HALF HIP	9.00 0.00	19-10-08	10-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 10-11-02	585.10 361.65		
	1	T35 HALF HIP	9.00 0.00	19-10-08	03-11-02	2 X 4	2 X 6	01-03-08 00-00-00	01-06-04 03-11-02	91.92 59.00		
	1	T36 HALF HIP	9.00 0.00	19-10-08	04-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 04-11-02	84.92 54.50		
	1	T37 HALF HIP	9.00 0.00	19-10-08	05-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 05-11-02	83.65 52.67		
	1	T38 HALF HIP	9.00 0.00	19-10-08	06-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 06-11-02	90.59 56.83		
	1	T39 HALF HIP	9.00 0.00	19-10-08	07-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 07-11-02	97.76 62.67		
	8	T44A COMMON	9.00 0.00	19-10-08	09-06-10	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 02-08-02	734.32 474.64		
	2	T44S ROOF	9.00 5.00	20-01-00	09-06-10	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	226.78 145.34		
	1	T44SA ROOF	9.00 5.00	19-10-08	09-06-10	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 01-08-02	110.23 70.67		
	1	T57 SCISSOR	9.00 5.00	10-07-00	05-05-14	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	47.04 30.50		
	2	G88 PIGGYBACK	6.00 0.00	23-07-00	10-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 10-11-02	267.84 170.00		
	3	PB20 PIGGYBACK	9.00 0.00	06-07-13	00-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-10 00-10-08	50.70 33.51		
	2	PB22 PIGGYBACK	6.00 0.00	03-01-07	01-10-14	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 01-10-14	20.40 14.66		
	10	J6 JACK-OPEN	6.00 0.00	05-06-04	03-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-11-02	159.30 100.00		
	2	J19 JACK-OPEN	6.00 0.00	04-09-00	02-02-14	2 X 4	2 X 4	01-03-08 -00-11-09	00-04-03 00-03-08	26.02 16.00		
	2	J20 JACK-OPEN	6.00 0.00	04-09-00	01-02-14	2 X 4	2 X 4	01-03-08 -02-11-09	00-04-03 00-03-08	20.92 13.34		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	05/26/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267493 LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
MODEL: ELMBROOK10A/4A ELEVATION: 4A-3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	J21 JACK-OPEN	6.00 0.00	01-10-08	01-02-14	2 X 4	2 X 4	01-03-08 -00-01-01	00-04-03 00-03-08	12.46 8.00		
	2	J22 JACK-OPEN	6.00 0.00	01-10-08	02-02-14	2 X 4	2 X 4	01-03-08 01-10-15	00-04-03 00-04-11	17.56 10.66		

TOTAL # TRUSS= **50** TOTAL BFT OF ALL TRUSSES= **1944.81** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3062.16** LBS.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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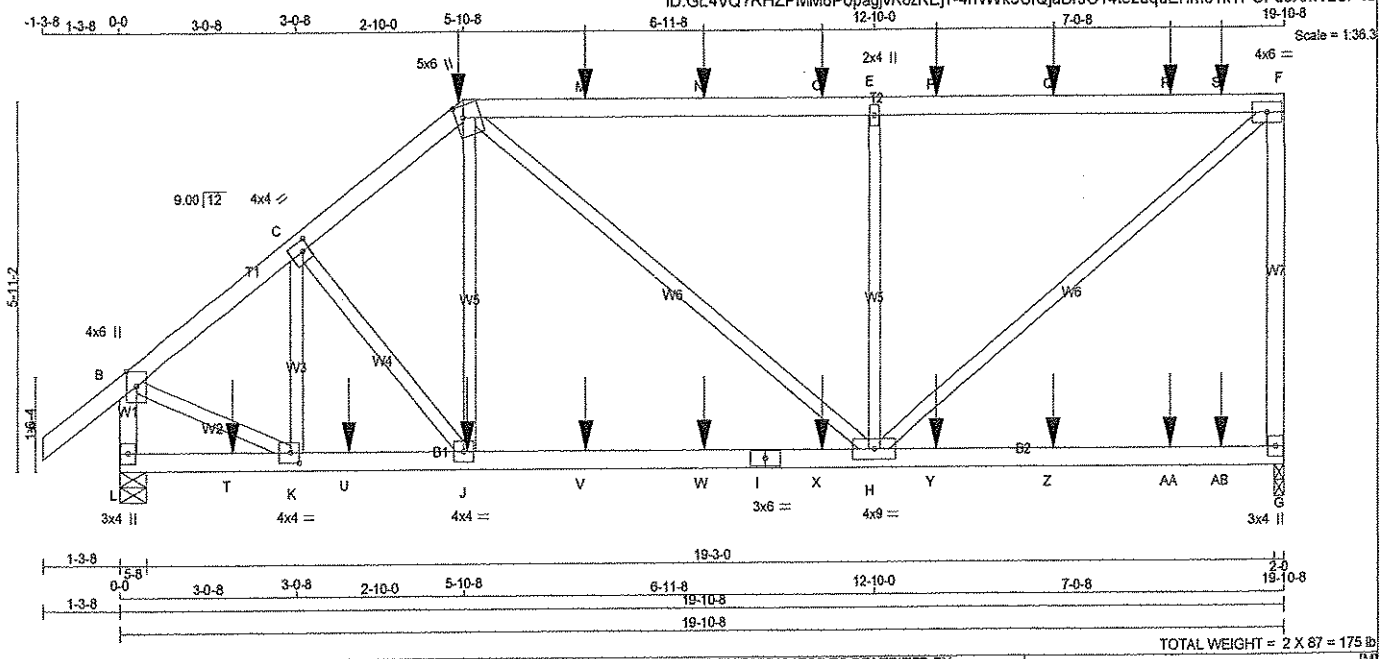

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- F	1	12	SIDE(61.0)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	1	12	SIDE(61.0)
I- G	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
J- D	1	6	SIDE(19.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2343	0	2343	0	0	2-0	1-8
L	2193	0	2193	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1906	1086 / 0	415 / 0	0 / 0	0 / 0	405 / 0	0 / 0
L	1778	1022 / 0	381 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 35	-84.3	-84.3	0.06 (1)	10.00		K- C	-541 / 0	0.07 (1)
B- C	-2081 / 0	-84.3	-84.3	0.07 (1)	6.07		C- J	0 / 187	0.02 (1)
C- D	-2288 / 0	-84.3	-84.3	0.07 (1)	5.87		J- D	0 / 433	0.05 (3)
D- M	-2088 / 0	-84.3	-84.3	0.88 (1)	4.36		B- K	0 / 1800	0.22 (1)
M- N	-2086 / 0	-84.3	-84.3	0.88 (1)	4.36		H- F	0 / 2672	0.33 (1)
N- O	-2086 / 0	-84.3	-84.3	0.88 (1)	4.36		D- H	0 / 369	0.05 (1)
O- E	-2086 / 0	-84.3	-84.3	0.88 (1)	4.36		H- E	-1464 / 0	0.41 (1)
E- P	-2086 / 0	-84.3	-84.3	0.88 (1)	4.36				
P- Q	-2086 / 0	-84.3	-84.3	0.88 (1)	4.36				
Q- R	-2086 / 0	-84.3	-84.3	0.88 (1)	4.36				
R- S	-2086 / 0	-84.3	-84.3	0.88 (1)	4.36				
S- F	-2086 / 0	-84.3	-84.3	0.88 (1)	4.36				
G- F	-2174 / 0	0.0	0.0	0.73 (1)	7.55				
L- B	-2135 / 0	0.0	0.0	0.12 (1)	7.60				
L- T	0 / 0	-28.0	-28.0	0.07 (2)	10.00				
T- K	0 / 0	-28.0	-28.0	0.07 (2)	10.00				
K- U	0 / 1676	-28.0	-28.0	0.26 (2)	10.00				
U- J	0 / 1676	-28.0	-28.0	0.26 (2)	10.00				
J- V	0 / 1799	-28.0	-28.0	0.50 (2)	10.00				
V- W	0 / 1799	-28.0	-28.0	0.50 (2)	10.00				
W- I	0 / 1799	-28.0	-28.0	0.50 (2)	10.00				
I- X	0 / 1799	-28.0	-28.0	0.50 (2)	10.00				
X- H	0 / 1799	-28.0	-28.0	0.50 (2)	10.00				
H- Y	0 / 0	-28.0	-28.0	0.39 (3)	10.00				
Y- Z	0 / 0	-28.0	-28.0	0.39 (3)	10.00				
Z-AA	0 / 0	-28.0	-28.0	0.39 (3)	10.00				
AA-AB	0 / 0	-28.0	-28.0	0.39 (3)	10.00				
AB- G	0 / 0	-28.0	-28.0	0.39 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-540	-540	---	FRONT	VERT	TOTAL
J	5-11-4	-50	-88	---	FRONT	VERT	TOTAL
M	7-11-4	-163	-163	---	FRONT	VERT	TOTAL
N	9-11-4	-163	-163	---	FRONT	VERT	TOTAL
O	11-11-4	-163	-163	---	FRONT	VERT	TOTAL
P	13-11-4	-163	-163	---	FRONT	VERT	TOTAL
Q	15-11-4	-163	-163	---	FRONT	VERT	TOTAL
R	17-11-4	-163	-163	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.88 (E-F:1) , BC=0.50 (H-J:2) , WB=0.41 (E-H:1) , SSI=0.32 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.31 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 5216 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 24406-16
STRUCTURAL COMPONENT ONLY
pvik

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T1	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 540.3 lbs FACTORED DOWN AT 5-10-8, 163.3 lbs FACTORED DOWN AT 7-11-4, 163.3 lbs FACTORED DOWN AT 9-11-4, 163.3 lbs FACTORED DOWN AT 11-11-4, 163.3 lbs FACTORED DOWN AT 13-11-4, 163.3 lbs FACTORED DOWN AT 15-11-4, AND 163.3 lbs FACTORED DOWN AT 17-11-4, AND 163.3 lbs FACTORED DOWN AT 18-9-12 ON TOP CHORD, AND 88.2 lbs FACTORED DOWN AT 1-11-4, 88.2 lbs FACTORED DOWN AT 3-11-4, 88.2 lbs FACTORED DOWN AT 5-11-4, 88.2 lbs FACTORED DOWN AT 7-11-4, 88.2 lbs FACTORED DOWN AT 9-11-4, 88.2 lbs FACTORED DOWN AT 11-11-4, 88.2 lbs FACTORED DOWN AT 13-11-4, 88.2 lbs FACTORED DOWN AT 15-11-4, AND 88.2 lbs FACTORED DOWN AT 17-11-4, AND 88.2 lbs FACTORED DOWN AT 18-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	18-9-12	-163	-163	---	FRONT	VERT	TOTAL
T	1-11-4	-50	-88	---	FRONT	VERT	TOTAL
U	3-11-4	-50	-88	---	FRONT	VERT	TOTAL
V	7-11-4	-50	-88	---	FRONT	VERT	TOTAL
W	9-11-4	-50	-88	---	FRONT	VERT	TOTAL
X	11-11-4	-50	-88	---	FRONT	VERT	TOTAL
Y	13-11-4	-50	-88	---	FRONT	VERT	TOTAL
Z	15-11-4	-50	-88	---	FRONT	VERT	TOTAL
AA	17-11-4	-50	-88	---	FRONT	VERT	TOTAL
AB	18-9-12	-50	-88	---	FRONT	VERT	TOTAL

DWG NO. TAM 24408 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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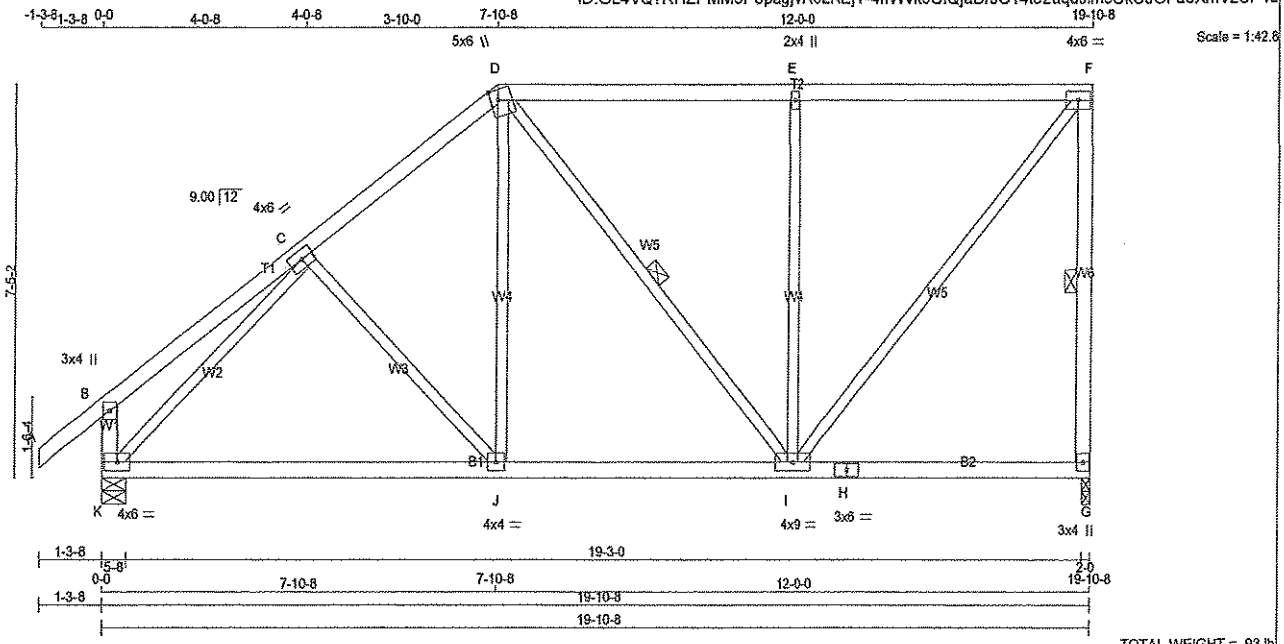


TOTAL WEIGHT = 93 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1116	0	1116	0	0	2-0	1-8
K	1232	0	1232	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
K	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	C- J	-123 / 62	0.07 (1)	
B- C	0 / 23	-84.3	-84.3	0.21 (1)	10.00	J- D	0 / 388	0.09 (2)	
C- D	-980 / 0	-84.3	-84.3	0.22 (1)	6.01	D- I	-102 / 0	0.06 (2)	
D- E	-720 / 0	-84.3	-84.3	0.53 (1)	6.12	I- E	-626 / 0	0.62 (1)	
E- F	-720 / 0	-84.3	-84.3	0.54 (1)	6.12	I- F	0 / 1114	0.25 (1)	
G- F	-1044 / 0	0.0	0.0	0.26 (1)	6.15	K- C	-1235 / 0	0.66 (1)	
K- B	-243 / 0	0.0	0.0	0.03 (1)	7.81				
K- J	0 / 850	-28.0	-28.0	0.47 (2)	10.00				
J- I	0 / 769	-28.0	-28.0	0.47 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.21 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.21 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 882 (0.27")

CSI: TC=0.54 (E-F:1) , BC=0.47 (I-J:2) , WB=0.66 (C-K:1) , SSI=0.25 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

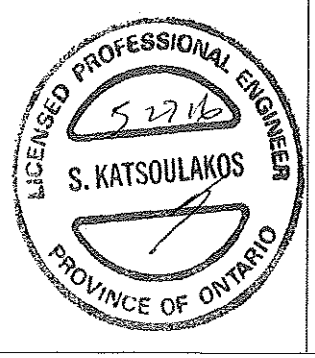
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)



DWG NO. TAM 24/05/16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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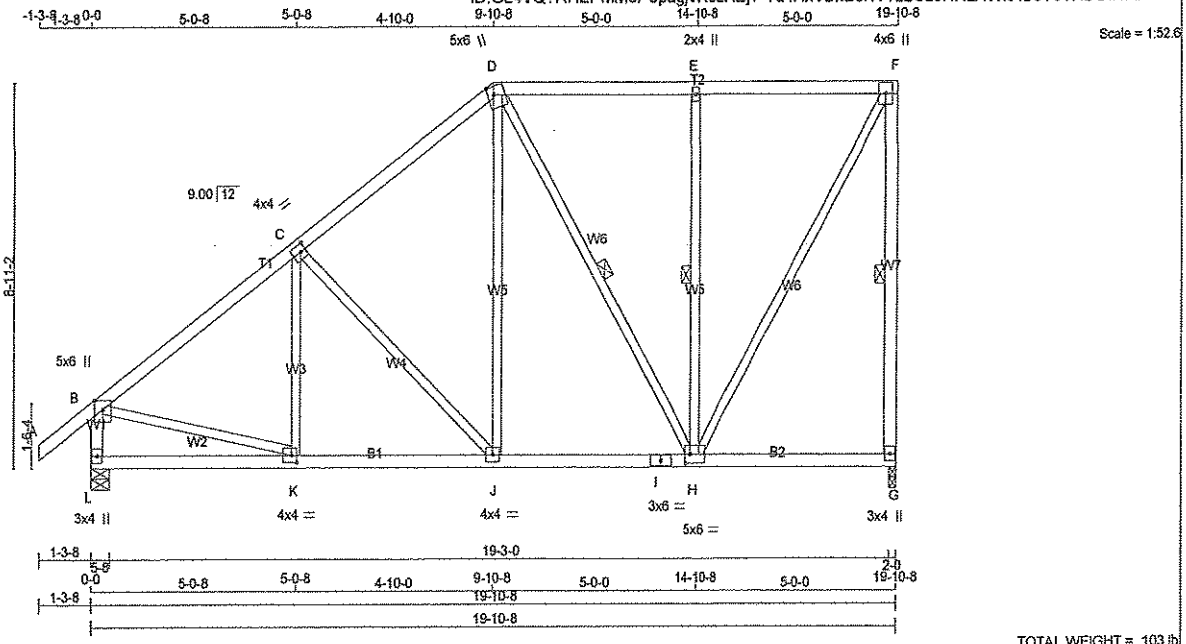

TOTAL WEIGHT = 103 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1116	0	1116	0	0	2-0	1-8
L	1232	0	1232	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	
B- C	-1104 / 0	-84.3	-84.3	0.39 (1)	5.52	
C- D	-838 / 0	-84.3	-84.3	0.37 (1)	6.13	
D- E	-523 / 0	-84.3	-84.3	0.35 (1)	6.25	
E- F	-523 / 0	-84.3	-84.3	0.35 (1)	6.25	
G- F	-1058 / 0	0.0	0.0	0.39 (1)	6.12	
L- B	-1174 / 0	0.0	0.0	0.12 (1)	7.35	
L- K	0 / 0	-28.0	-28.0	0.17 (3)	10.00	
K- J	0 / 907	-28.0	-28.0	0.27 (2)	10.00	
J- I	0 / 649	-28.0	-28.0	0.25 (2)	10.00	
I- H	0 / 649	-28.0	-28.0	0.25 (2)	10.00	
H- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-58 / 172	0.04 (3)
C- J	-372 / 0	0.35 (1)
J- D	0 / 408	0.09 (2)
D- H	-253 / 0	0.18 (1)
H- E	-520 / 0	0.27 (1)
H- F	0 / 1037	0.23 (1)
B- K	0 / 932	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.39 (F-G:1) , BC=0.27 (J-K:2) , WB=0.35 (C-J:1) , SSI=0.20 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
52016
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 24410-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T4	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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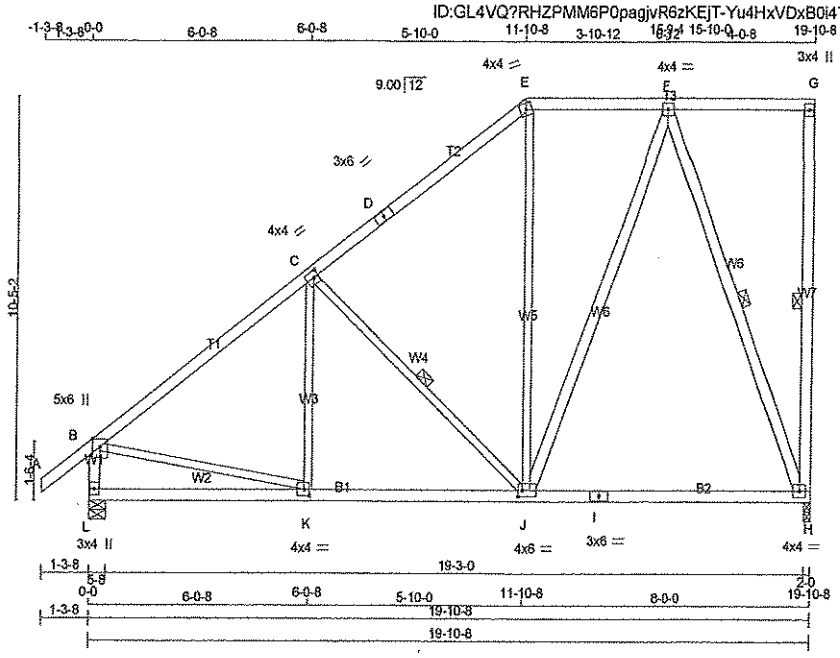

TOTAL WEIGHT = 8 X 111 = 887 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0	2.00	1.50
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1116	0	1116	0	0	2-0	1-8
L	1232	0	1232	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-J, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-1085 / 0	-84.3	-84.3	0.42 (1)	5.53
C- D	-711 / 0	-84.3	-84.3	0.40 (1)	6.25
D- E	-711 / 0	-84.3	-84.3	0.40 (1)	6.25
E- F	-539 / 0	-84.3	-84.3	0.18 (1)	6.25
F- G	0 / 0	-84.3	-84.3	0.22 (1)	10.00
H- G	-130 / 0	0.0	0.0	0.07 (1)	6.25
L- B	-1160 / 0	0.0	0.0	0.12 (1)	7.39
L- K	0 / 0	-28.0	-28.0	0.21 (2)	10.00
K- J	0 / 897	-28.0	-28.0	0.48 (2)	10.00
J- I	0 / 357	-28.0	-28.0	0.42 (2)	10.00
I- H	0 / 357	-28.0	-28.0	0.42 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-10 / 208	0.05 (3)
C- J	-498 / 0	0.23 (1)
J- E	0 / 188	0.04 (3)
J- F	0 / 512	0.08 (1)
F- H	-964 / 0	0.62 (1)
B- K	0 / 914	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 952 (0.25")

CSI: TC=0.42 (B-C:1) , BC=0.48 (J-K:2) , WB=0.62 (F-H:1) , SSI=0.20 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

DWG NO . TAM 24/4/0-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T5	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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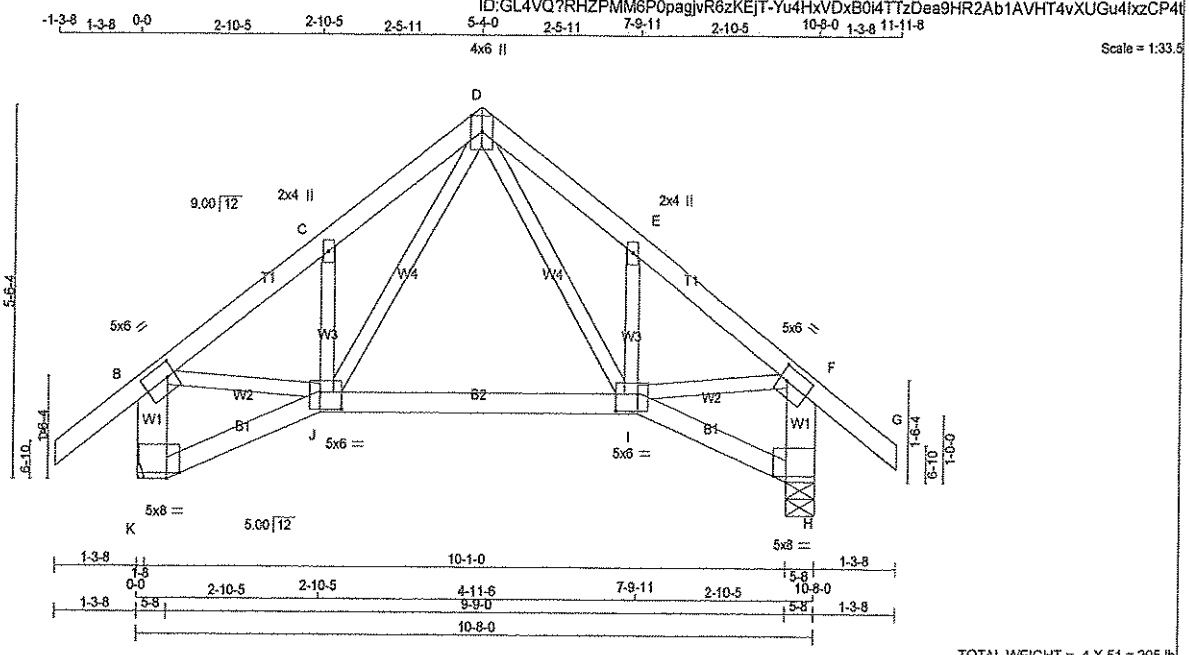


TOTAL WEIGHT = 4 X 51 = 205 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0	2.50	1.50
H	BVM1-p	MT20	5.0	8.0	Edge	2.50
I	BBWWW-p	MT20	5.0	6.0	3.00	4.00
J	BBWWW-p	MT20	5.0	6.0	3.00	4.00
K	BVM1-p	MT20	5.0	8.0	Edge	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	715	0	715	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	715	0	715	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	571	344 / 0	112 / 0	0 / 0	0 / 0	115 / 0	0 / 0
H	571	344 / 0	112 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	1	0.11 (1)	10.00
B- C	-689 / 0	-84.3	-84.3	1	0.08 (1)	6.25
C- D	-717 / 0	-84.3	-84.3	1	0.06 (1)	6.25
D- E	-717 / 0	-84.3	-84.3	1	0.06 (1)	6.25
E- F	-689 / 0	-84.3	-84.3	1	0.08 (1)	6.25
F- G	0 / 35	-84.3	-84.3	1	0.11 (1)	10.00
K- B	-675 / 0	0.0	0.0	1	0.05 (1)	7.81
H- F	-675 / 0	0.0	0.0	1	0.05 (1)	7.81
K- J	0 / 0	-28.0	-28.0	3	0.07 (3)	10.00
J- I	0 / 361	-28.0	-28.0	2	0.24 (2)	10.00
I- H	0 / 0	-28.0	-28.0	3	0.07 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 396	0.09 (1)
I- E	-268 / 0	0.04 (1)
J- D	0 / 396	0.09 (1)
J- C	-268 / 0	0.04 (1)
B- J	0 / 562	0.13 (1)
I- F	0 / 562	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.11 (F-G:1) , BC=0.24 (I-J:2) , WB=0.13 (B-J:1) , SSI=0.10 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

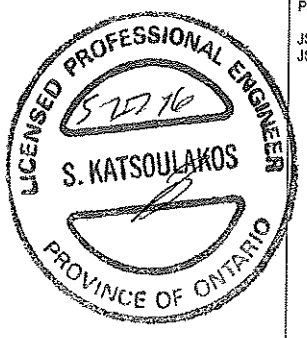
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (J) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)



DWG NO. TAM24/4/12-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T6	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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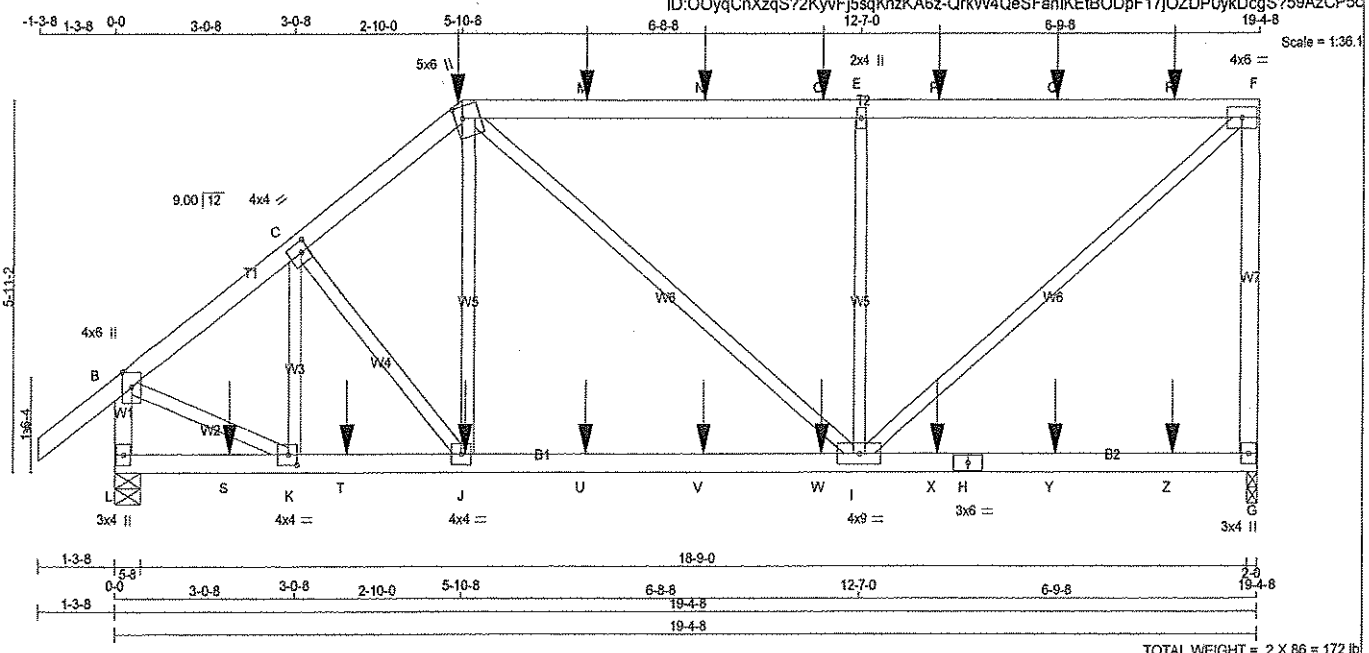


TOTAL WEIGHT = 2 X 86 = 172 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- F	1	12	SIDE(61.0)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- H	1	12	SIDE(61.0)
H- G	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
J- D	1	6	SIDE(19.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. *REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.*

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	2139	0	2139	0	0	2-0	1-8
L	2127	0	2127	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1739	992 / 0	378 / 0	0 / 0	0 / 0	369 / 0	0 / 0
L	1725	991 / 0	369 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.06 (1)	10.00		K- C	-515 / 0	0.06 (1)
B- C	-2009 / 0	-84.3	-84.3	0.07 (1)	6.15		C- J	0 / 166	0.02 (1)
C- D	-2180 / 0	-84.3	-84.3	0.07 (1)	5.96		J- D	0 / 432	0.05 (3)
D- M	-1944 / 0	-84.3	-84.3	0.77 (1)	4.86		B- K	0 / 1739	0.22 (1)
M- N	-1944 / 0	-84.3	-84.3	0.77 (1)	4.86		I- F	0 / 2526	0.31 (1)
N- O	-1944 / 0	-84.3	-84.3	0.77 (1)	4.86		D- I	0 / 281	0.03 (1)
O- E	-1944 / 0	-84.3	-84.3	0.77 (1)	4.86		I- E	-1381 / 0	0.39 (1)
E- P	-1944 / 0	-84.3	-84.3	0.78 (1)	4.86				
P- Q	-1944 / 0	-84.3	-84.3	0.78 (1)	4.86				
Q- R	-1944 / 0	-84.3	-84.3	0.78 (1)	4.86				
R- F	-1944 / 0	-84.3	-84.3	0.78 (1)	4.86				
G- F	-2005 / 0	0.0	0.0	0.68 (1)	7.78				
L- B	-2070 / 0	0.0	0.0	0.12 (1)	7.69				
L- S	0 / 0	-28.0	-28.0	0.07 (2)	10.00				
S- K	0 / 0	-28.0	-28.0	0.07 (2)	10.00				
K- T	0 / 1619	-28.0	-28.0	0.25 (2)	10.00				
T- J	0 / 1619	-28.0	-28.0	0.25 (2)	10.00				
J- U	0 / 1729	-28.0	-28.0	0.45 (2)	10.00				
U- V	0 / 1729	-28.0	-28.0	0.45 (2)	10.00				
V- W	0 / 1729	-28.0	-28.0	0.45 (2)	10.00				
W- I	0 / 1729	-28.0	-28.0	0.45 (2)	10.00				
I- X	0 / 0	-28.0	-28.0	0.35 (3)	10.00				
X- H	0 / 0	-28.0	-28.0	0.35 (3)	10.00				
H- Y	0 / 0	-28.0	-28.0	0.35 (3)	10.00				
Y- Z	0 / 0	-28.0	-28.0	0.35 (3)	10.00				
Z- G	0 / 0	-28.0	-28.0	0.35 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-540	-540	---	BACK	VERT	TOTAL
J	5-11-4	-50	-88	---	BACK	VERT	TOTAL
M	7-11-4	-163	-163	---	BACK	VERT	TOTAL
N	9-11-4	-163	-163	---	BACK	VERT	TOTAL
O	11-11-4	-163	-163	---	BACK	VERT	TOTAL
P	13-11-4	-163	-163	---	BACK	VERT	TOTAL
Q	15-11-4	-163	-163	---	BACK	VERT	TOTAL
R	17-11-4	-163	-163	---	BACK	VERT	TOTAL
S	1-11-4	-50	-88	---	BACK	VERT	TOTAL
T	3-11-4	-50	-88	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.78 (E-F:1) , BC=0.45 (I-J:2) , WB=0.39 (E-I:1) , SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT *RESPONSIBLE FOR QUALITY CONTROL IN* THE TRUSS MANUFACTURING PLANT .

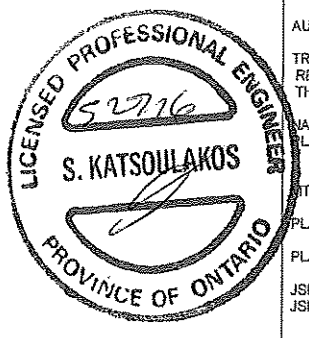
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.30 (K) (INPUT = 1.00)



DWG NO. TAM24383-16
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T6	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 540.3 lbs FACTORED DOWN AT 5-10-8, 163.3 lbs FACTORED DOWN AT 7-11-4, 163.3 lbs FACTORED DOWN AT 9-11-4, 163.3 lbs FACTORED DOWN AT 11-11-4, 163.3 lbs FACTORED DOWN AT 13-11-4, AND 163.3 lbs FACTORED DOWN AT 15-11-4, AND 163.3 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 88.2 lbs FACTORED DOWN AT 1-11-4, 88.2 lbs FACTORED DOWN AT 3-11-4, 88.2 lbs FACTORED DOWN AT 5-11-4, 88.2 lbs FACTORED DOWN AT 7-11-4, 88.2 lbs FACTORED DOWN AT 9-11-4, 88.2 lbs FACTORED DOWN AT 11-11-4, 88.2 lbs FACTORED DOWN AT 13-11-4, AND 88.2 lbs FACTORED DOWN AT 15-11-4, AND 88.2 lbs FACTORED DOWN AT 17-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	7-11-4	-50	-88	---	BACK	VERT	TOTAL
V	9-11-4	-50	-88	---	BACK	VERT	TOTAL
W	11-11-4	-50	-88	---	BACK	VERT	TOTAL
X	13-11-4	-50	-88	---	BACK	VERT	TOTAL
Y	15-11-4	-50	-88	---	BACK	VERT	TOTAL
Z	17-11-4	-50	-88	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM243B3-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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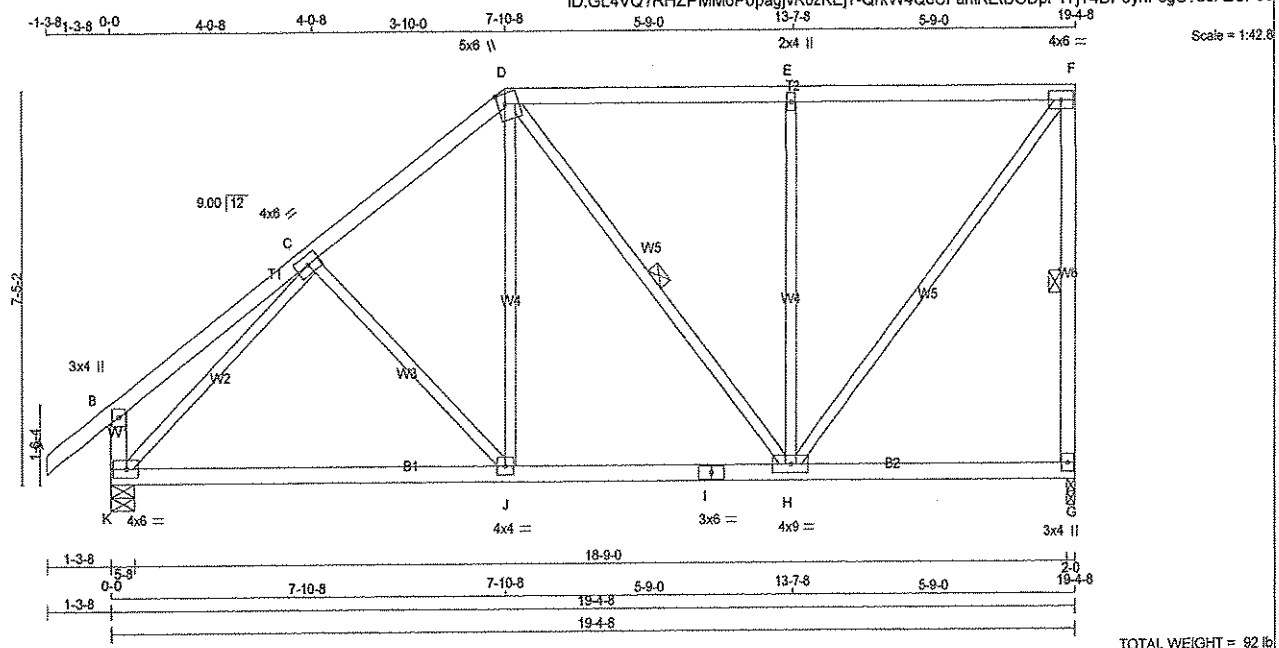

TOTAL WEIGHT = 92 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMV+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1088	0	1088	0	0	2-0	1-8
K	1204	0	1204	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	893	496 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0
K	972	566 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	C- J	-129 / 59	0.07 (1)
B- C	0 / 23	-84.3	-84.3	0.21 (1)	10.00	J- D	0 / 389	0.09 (2)
C- D	-941 / 0	-84.3	-84.3	0.22 (1)	6.10	D- H	-120 / 0	0.07 (2)
D- E	-675 / 0	-84.3	-84.3	0.49 (1)	6.25	H- E	-600 / 0	0.59 (1)
E- F	-676 / 0	-84.3	-84.3	0.49 (1)	6.25	H- F	0 / 1072	0.24 (1)
G- F	-1018 / 0	0.0	0.0	0.25 (1)	6.21	K- C	-1197 / 0	0.64 (1)
K- B	-243 / 0	0.0	0.0	0.03 (1)	7.81			
K- J	0 / 823	-28.0	-28.0	0.46 (2)	10.00			
J- I	0 / 737	-28.0	-28.0	0.47 (2)	10.00			
I- H	0 / 737	-28.0	-28.0	0.47 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.20 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 860 (0.27")

CSI: TC=0.49 (E-F:1) , BC=0.47 (H-J:2) , WB=0.64 (C-K:1) , SSI=0.24 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

DWG NO. TAM24384-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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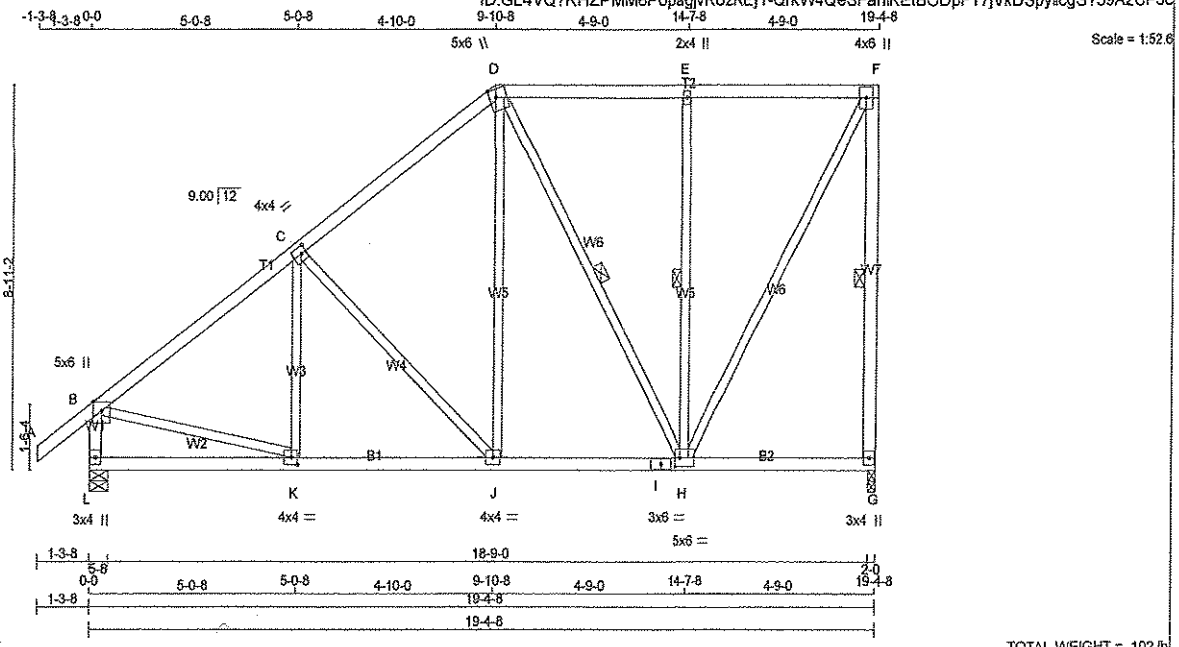

TOTAL WEIGHT = 102 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1088	0	1088	0	0	2-0	1-8
L	1204	0	1204	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	893	496 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0
L	972	586 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	K- C	-50 / 176	0.04 (3)
B- C	-1069 / 0	-84.3	-84.3	0.28 (1)	5.77	C- J	-378 / 0	0.35 (1)
C- D	-797 / 0	-84.3	-84.3	0.27 (1)	6.25	J- D	0 / 405	0.09 (2)
D- E	-487 / 0	-84.3	-84.3	0.24 (1)	6.25	D- H	-270 / 0	0.18 (1)
E- F	-487 / 0	-84.3	-84.3	0.24 (1)	6.25	H- E	-493 / 0	0.25 (1)
G- F	-1033 / 0	0.0	0.0	0.38 (1)	6.18	H- F	0 / 1004	0.23 (1)
L- B	-1146 / 0	0.0	0.0	0.12 (1)	7.43	B- K	0 / 903	0.20 (1)
L- K	0 / 0	-28.0	-28.0	0.17 (3)	10.00			
K- J	0 / 879	-28.0	-28.0	0.27 (2)	10.00			
J- I	0 / 617	-28.0	-28.0	0.23 (2)	10.00			
I- H	0 / 617	-28.0	-28.0	0.23 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.38 (F-G:1) , BC=0.27 (J-K:2) , WB=0.35 (C-J:1) , SSI=0.19 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.33 (K) (INPUT = 1.00)

DWG NO. TAM24385-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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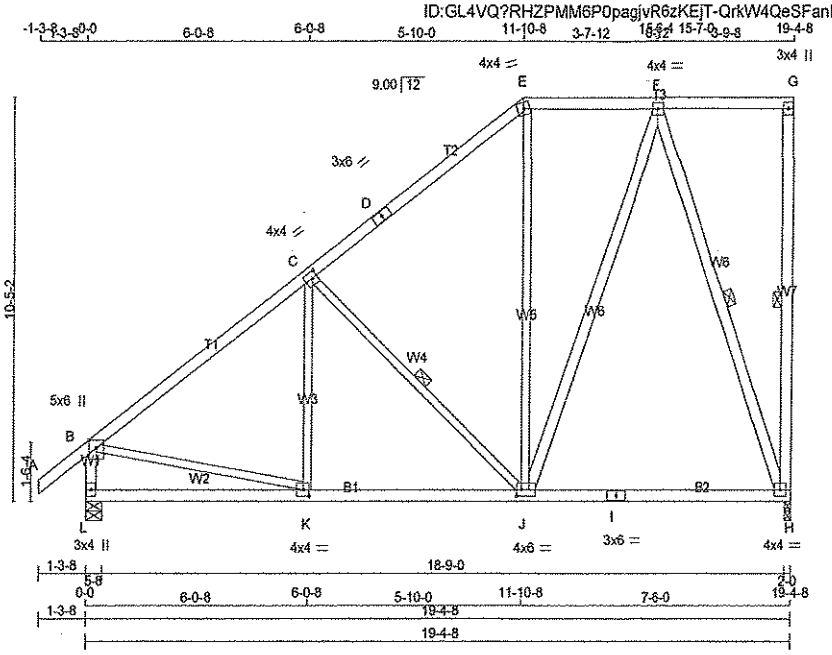

TOTAL WEIGHT = 2 X 110 = 219 lb

[M](F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0	2.00	1.50
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1088	0	1088	0	0	2-0	1-8
L	1204	0	1204	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	893	496 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0
L	972	566 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-J, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-1049 / 0	-84.3	-84.3	0.42 (1)	5.60
C- D	-668 / 0	-84.3	-84.3	0.39 (1)	6.25
D- E	-668 / 0	-84.3	-84.3	0.39 (1)	6.25
E- F	-504 / 0	-84.3	-84.3	0.15 (1)	6.25
F- G	0 / 0	-84.3	-84.3	0.20 (1)	10.00
H- G	-122 / 0	0.0	0.0	0.07 (1)	6.25
L- B	-1133 / 0	0.0	0.0	0.12 (1)	7.46
L- K	0 / 0	-28.0	-28.0	0.21 (2)	10.00
K- J	0 / 868	-28.0	-28.0	0.44 (2)	10.00
J- I	0 / 330	-28.0	-28.0	0.37 (2)	10.00
I- H	0 / 330	-28.0	-28.0	0.37 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 216	0.05 (3)
C- J	-507 / 0	0.23 (1)
J- E	0 / 174	0.04 (3)
J- F	0 / 521	0.08 (1)
F- H	-941 / 0	0.59 (1)
B- K	0 / 885	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.42 (B-C:1) , BC=0.44 (J-K:2) , WB=0.59 (F-H:1) , SSI=0.20 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.37 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
62726
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 24386-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T9X	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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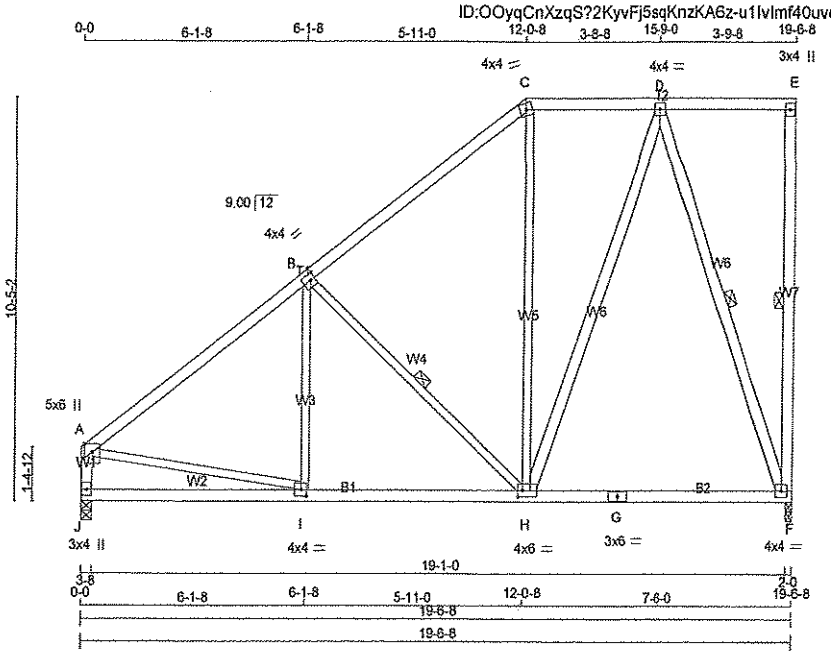


TOTAL WEIGHT = 6 X 108 = 648 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BS-t	MT20	3.0	6.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	1.50
I	BMVW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1097	0	1097	0	0	2-0	1-8
J	1097	0	1097	0	0	3-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0
J	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, B-H, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1083 / 0	-84.3	-84.3	0.43 (1)	5.51	I- B	0 / 232	0.05 (3)	
B- C	-677 / 0	-84.3	-84.3	0.40 (1)	6.25	B- H	-529 / 0	0.25 (1)	
C- D	-511 / 0	-84.3	-84.3	0.15 (1)	6.25	H- C	0 / 176	0.04 (3)	
D- E	0 / 0	-84.3	-84.3	0.20 (1)	10.00	A- I	0 / 909	0.20 (1)	
F- E	-122 / 0	0.0	0.0	0.07 (1)	6.25	H- D	0 / 531	0.09 (1)	
J- A	-1025 / 0	0.0	0.0	0.11 (1)	7.74	D- F	-951 / 0	0.60 (1)	
J- I	0 / 0	-28.0	-28.0	0.22 (3)	10.00				
I- H	0 / 896	-28.0	-28.0	0.44 (2)	10.00				
H- G	0 / 333	-28.0	-28.0	0.37 (2)	10.00				
G- F	0 / 333	-28.0	-28.0	0.37 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.43 (A-B:1) , BC=0.44 (H-I:2) , WB=0.60 (D-F:1) , SSI=0.20 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

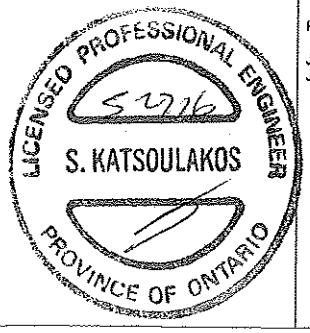
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.37 (G) (INPUT = 1.00)



DWG NO. TAM24387-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T10	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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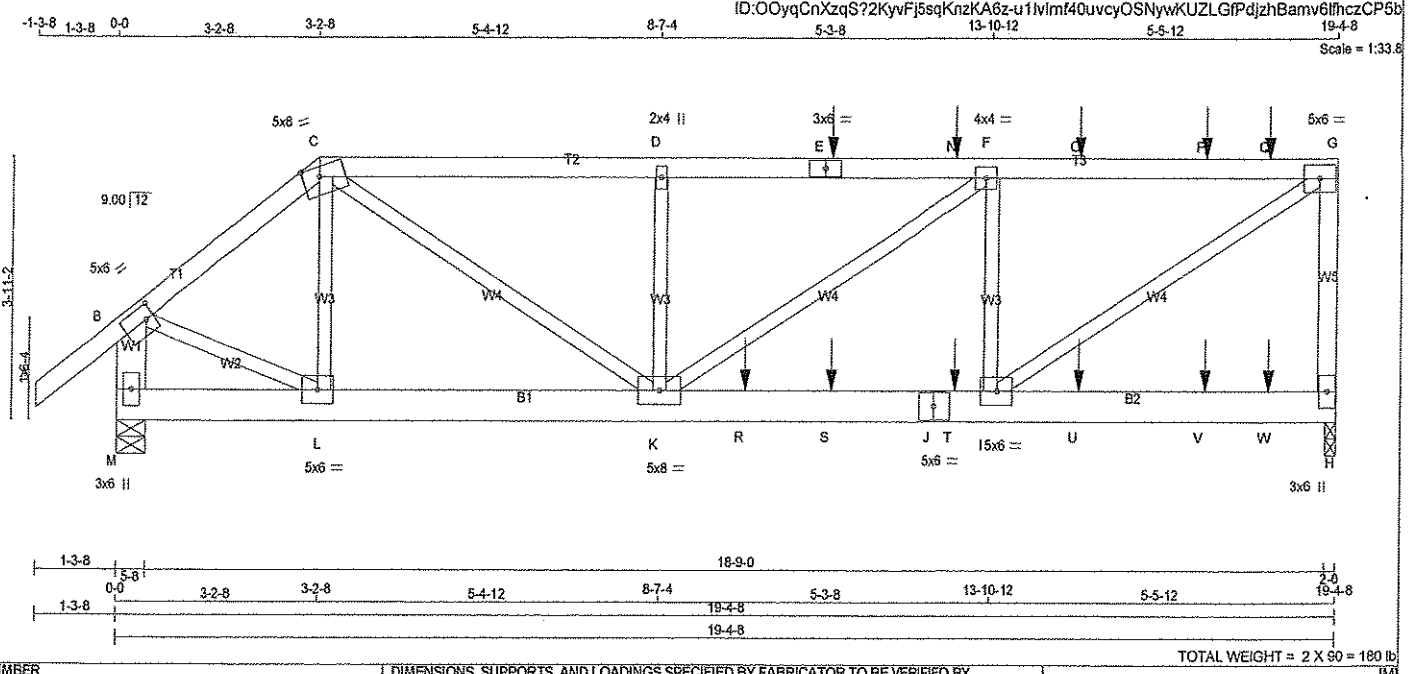


TOTAL WEIGHT = 2 X 90 = 180 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	SIDE(0.0)
E- G	1	12	SIDE(0.0)
G- H	1	12	TOP
M- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING *PATTERN SHALL BE CAPABLE OF TRANSFERING.* REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	6.0	2.50	1.50
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2325	0	2325	0	0	2-0	1-8
M	2048	0	2048	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1889	1081 / 0	408 / 0	0 / 0	0 / 0	399 / 0	0 / 0
M	1639	979 / 0	327 / 0	0 / 0	0 / 0	333 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.06 (1)	10.00	L- C	-535 / 0	0.06 (1)	
B- C	-1991 / 0	-84.3	-84.3	0.10 (1)	6.13	B- L	0 / 1675	0.21 (1)	
C- D	-3710 / 0	-84.3	-84.3	0.24 (1)	4.68	I- G	0 / 3573	0.44 (1)	
D- E	-3710 / 0	-84.3	-84.3	0.39 (1)	4.49	C- K	0 / 2555	0.32 (1)	
E- N	-3710 / 0	-84.3	-84.3	0.39 (1)	4.49	I- F	-1279 / 0	0.15 (1)	
N- F	-3710 / 0	-84.3	-84.3	0.39 (1)	4.49	K- D	-501 / 0	0.06 (1)	
F- O	-2998 / 0	-84.3	-84.3	0.36 (1)	4.92	K- F	0 / 857	0.11 (1)	
O- P	-2998 / 0	-84.3	-84.3	0.36 (1)	4.92				
P- Q	-2998 / 0	-84.3	-84.3	0.36 (1)	4.92				
Q- G	-2998 / 0	-84.3	-84.3	0.36 (1)	4.92				
H- G	-2256 / 0	0.0	0.0	0.26 (1)	7.45				
M- B	-1984 / 0	0.0	0.0	0.07 (1)	7.81				
M- L	0 / 0	-28.0	-28.0	0.03 (1)	10.00				
L- K	0 / 1572	-28.0	-28.0	0.28 (1)	10.00				
K- R	0 / 2998	-28.0	-28.0	0.60 (1)	10.00				
R- S	0 / 2998	-28.0	-28.0	0.60 (1)	10.00				
S- J	0 / 2998	-28.0	-28.0	0.60 (1)	10.00				
J- T	0 / 2998	-28.0	-28.0	0.60 (1)	10.00				
T- I	0 / 2998	-28.0	-28.0	0.60 (1)	10.00				
I- U	0 / 0	-28.0	-28.0	0.15 (2)	10.00				
U- V	0 / 0	-28.0	-28.0	0.15 (2)	10.00				
V- W	0 / 0	-28.0	-28.0	0.15 (2)	10.00				
W- H	0 / 0	-28.0	-28.0	0.15 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
E	11-3-12	-90	-90	---	FRONT	VERT	TOTAL
N	13-3-12	-90	-90	---	FRONT	VERT	TOTAL
O	15-3-12	-90	-90	---	FRONT	VERT	TOTAL
P	17-3-12	-90	-90	---	FRONT	VERT	TOTAL
Q	18-3-12	-90	-90	---	FRONT	VERT	TOTAL
R	9-11-8	-1449	-1449	---	FRONT	VERT	TOTAL
S	11-3-12	-36	-63	---	FRONT	VERT	TOTAL
T	13-3-12	-36	-63	---	FRONT	VERT	TOTAL
U	15-3-12	-36	-63	---	FRONT	VERT	TOTAL
V	17-3-12	-36	-63	---	FRONT	VERT	TOTAL
W	18-3-12	-36	-63	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.39 (D-F:1) , BC=0.60 (I-K:1) , WB=0.44 (G-I:1) , SSI=0.40 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT *RESPONSIBLE FOR QUALITY CONTROL IN* THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.37 (I) (INPUT = 1.00)

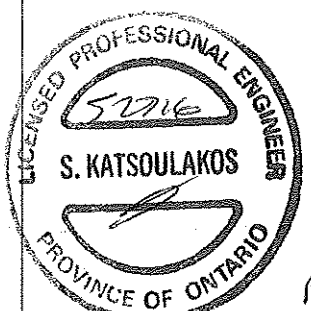
DWG NO. TAM24388-16
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267485	T10	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 90.3 lbs FACTORED DOWN AT 11-3-12, 90.3 lbs FACTORED DOWN AT 13-3-12, 90.3 lbs FACTORED DOWN AT 15-3-12, AND 90.3 lbs FACTORED DOWN AT 17-3-12, AND 90.3 lbs FACTORED DOWN AT 18-3-12 ON TOP CHORD, AND 1449.2 lbs FACTORED DOWN AT 9-11-8, 63.2 lbs FACTORED DOWN AT 11-3-12, 63.2 lbs FACTORED DOWN AT 13-3-12, 63.2 lbs FACTORED DOWN AT 15-3-12, AND 63.2 lbs FACTORED DOWN AT 17-3-12, AND 63.2 lbs FACTORED DOWN AT 18-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM243BB 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T11X	1	1	TRUSS DESC.	

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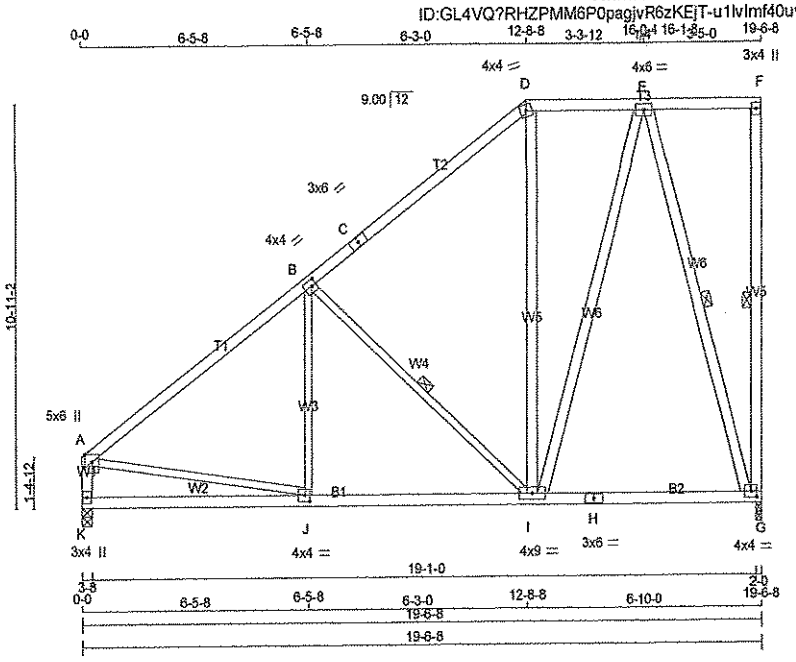

Scale = 1:61.3

TOTAL WEIGHT = 114 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
J - B	2x3	DRY	No.2	SPF
B - I	2x3	DRY	No.2	SPF
A - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TS-t	MT20	3.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0	2.00	1.75
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1097	0	1097	0	0	2-0	1-8
K	1097	0	1097	0	0	3-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0
K	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, B-I, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1075 / 0	-84.3	-84.3	0.49 (1)	5.43	J- B	0 / 269	0.06 (3)
B- C	-621 / 0	-84.3	-84.3	0.45 (1)	6.25	B- I	-582 / 0	0.30 (1)
C- D	-621 / 0	-84.3	-84.3	0.45 (1)	6.25	I- D	0 / 156	0.03 (3)
D- E	-464 / 0	-84.3	-84.3	0.13 (1)	6.25	I- E	0 / 583	0.09 (1)
E- F	0 / 0	-84.3	-84.3	0.16 (1)	10.00	E- G	-958 / 0	0.65 (1)
G- F	-109 / 0	0.0	0.0	0.07 (1)	6.25	A- J	0 / 903	0.20 (1)
K- A	-1023 / 0	0.0	0.0	0.11 (1)	7.75			
K- J	0 / 0	-28.0	-28.0	0.26 (3)	10.00			
J- I	0 / 891	-28.0	-28.0	0.39 (2)	10.00			
I- H	0 / 293	-28.0	-28.0	0.32 (2)	10.00			
H- G	0 / 293	-28.0	-28.0	0.32 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.49 (A-B:1) , BC=0.39 (I-J:2) , WB=0.65 (E-G:1) , SSI=0.21 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (J) (INPUT = 0.90)
JSI METAL= 0.33 (J) (INPUT = 1.00)

DWG NO. TAM 24389-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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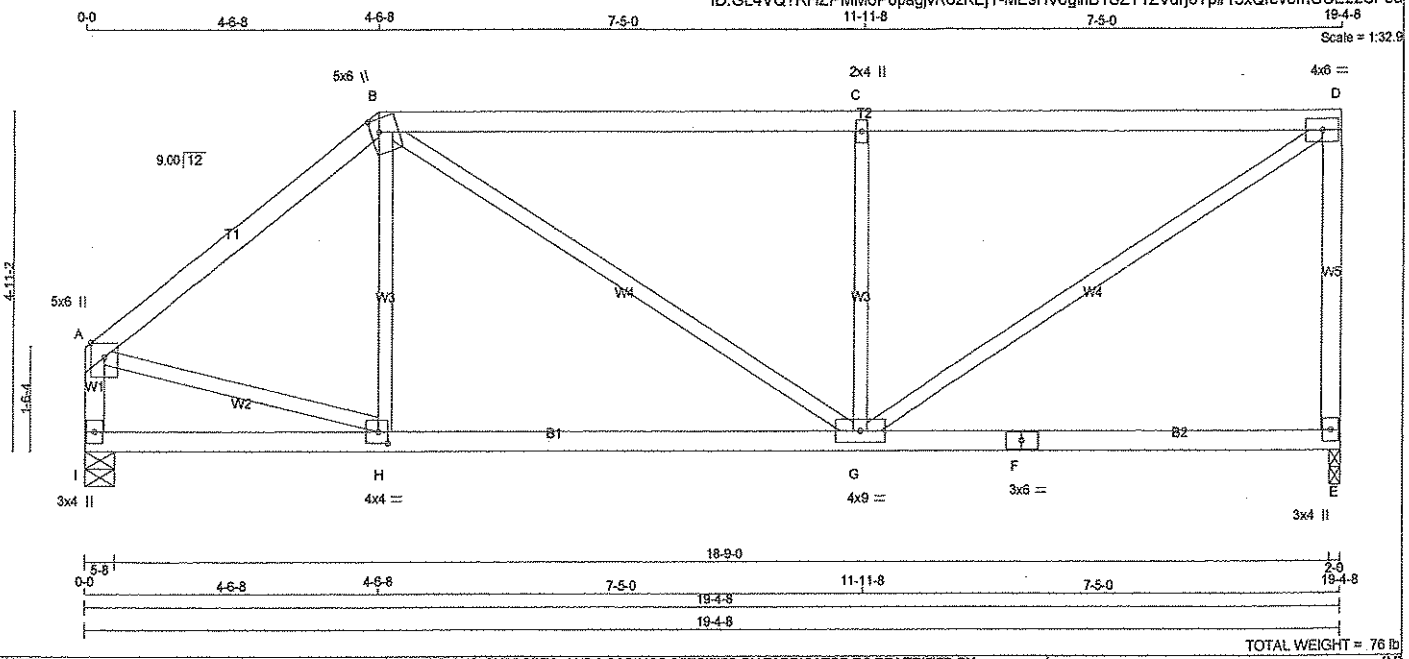


TOTAL WEIGHT = 76 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BS-t	MT20	3.0	6.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.75
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1088	0	1088	0	0	2-0	1-8
I	1088	0	1088	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	893	496 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0
I	893	496 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1054 / 0	-84.3	-84.3	0.33 (1)	5.69	H- B	-38 / 194	0.04 (3)
B- C	-1231 / 0	-84.3	-84.3	0.91 (1)	4.10	B- G	0 / 460	0.10 (1)
C- D	-1231 / 0	-84.3	-84.3	0.92 (1)	4.10	G- C	-774 / 0	0.28 (1)
E- D	-1004 / 0	0.0	0.0	0.40 (1)	7.80	G- D	0 / 1449	0.33 (1)
I- A	-1042 / 0	0.0	0.0	0.11 (1)	7.70	A- H	0 / 869	0.20 (1)
I- H	0 / 0	-28.0	-28.0	0.22 (2)	10.00			
H- G	0 / 841	-28.0	-28.0	0.49 (2)	10.00			
G- F	0 / 0	-28.0	-28.0	0.39 (3)	10.00			
F- E	0 / 0	-28.0	-28.0	0.39 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.92 (C-D:1) , BC=0.49 (G-H:2) , WB=0.33 (D-G:1) , SSI=0.30 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

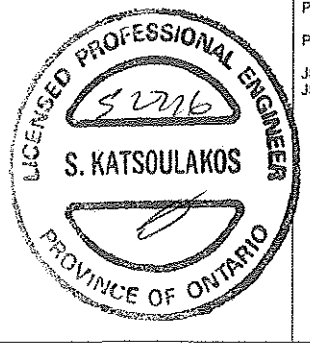
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.32 (H) (INPUT = 1.00)



DWG NO. TAM 24390-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267485	T14X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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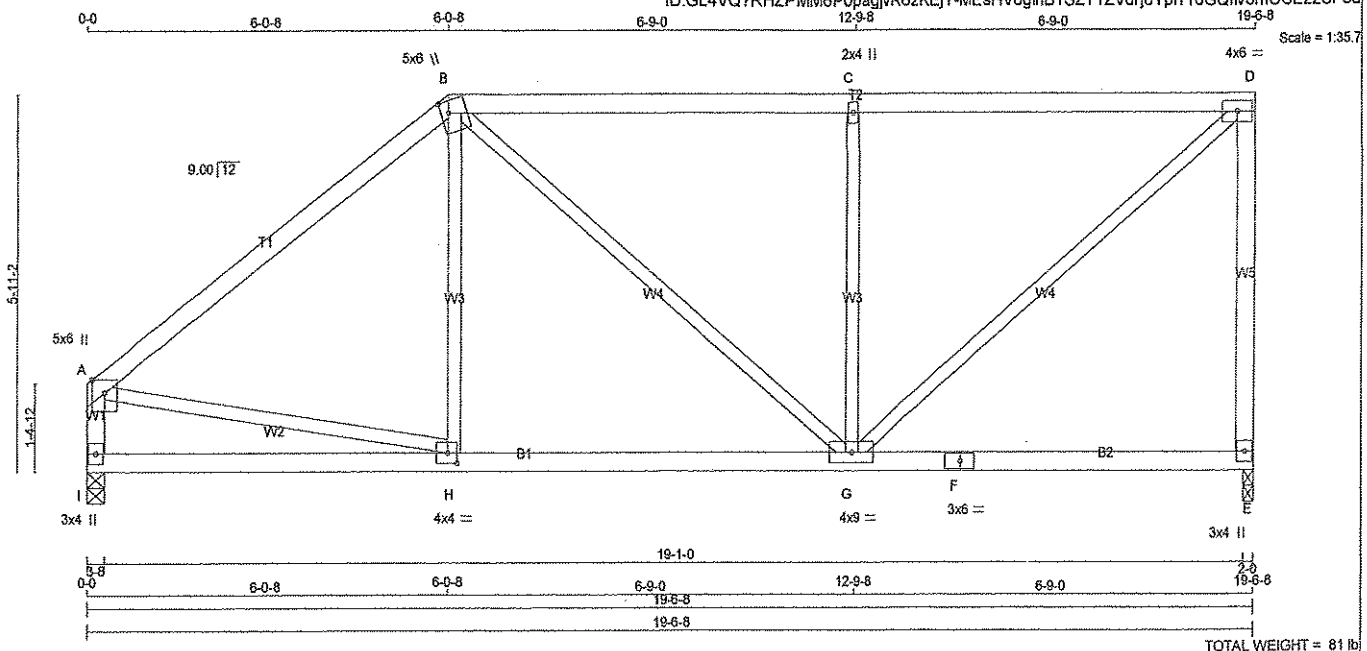


TOTAL WEIGHT = 81 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BS-t	MT20	3.0	6.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.75
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1097	0	1097	0	0	2-0	1-8
I	1097	0	1097	0	0	3-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0
I	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1045 / 0	-84.3	-84.3	0.62 (1)	5.24		H- B	0 / 261	0.06 (3)
B- C	-967 / 0	-84.3	-84.3	0.71 (1)	5.09		B- G	0 / 171	0.04 (1)
C- D	-967 / 0	-84.3	-84.3	0.71 (1)	5.09		G- C	-705 / 0	0.38 (1)
E- D	-1020 / 0	0.0	0.0	0.68 (1)	7.76		G- D	0 / 1256	0.28 (1)
I- A	-1028 / 0	0.0	0.0	0.11 (1)	7.74		A- H	0 / 847	0.19 (1)
I- H	0 / 0	-28.0	-28.0	0.26 (3)	10.00				
H- G	0 / 836	-28.0	-28.0	0.40 (2)	10.00				
G- F	0 / 0	-28.0	-28.0	0.30 (3)	10.00				
F- E	0 / 0	-28.0	-28.0	0.30 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.71 (C-D:1) , BC=0.40 (G-H:2) , WB=0.38 (C-G:1) , SSI=0.28 (C-D:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

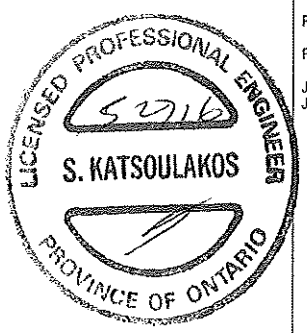
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.31 (H) (INPUT = 1.00)



DWG NO . TAM 2439-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T15X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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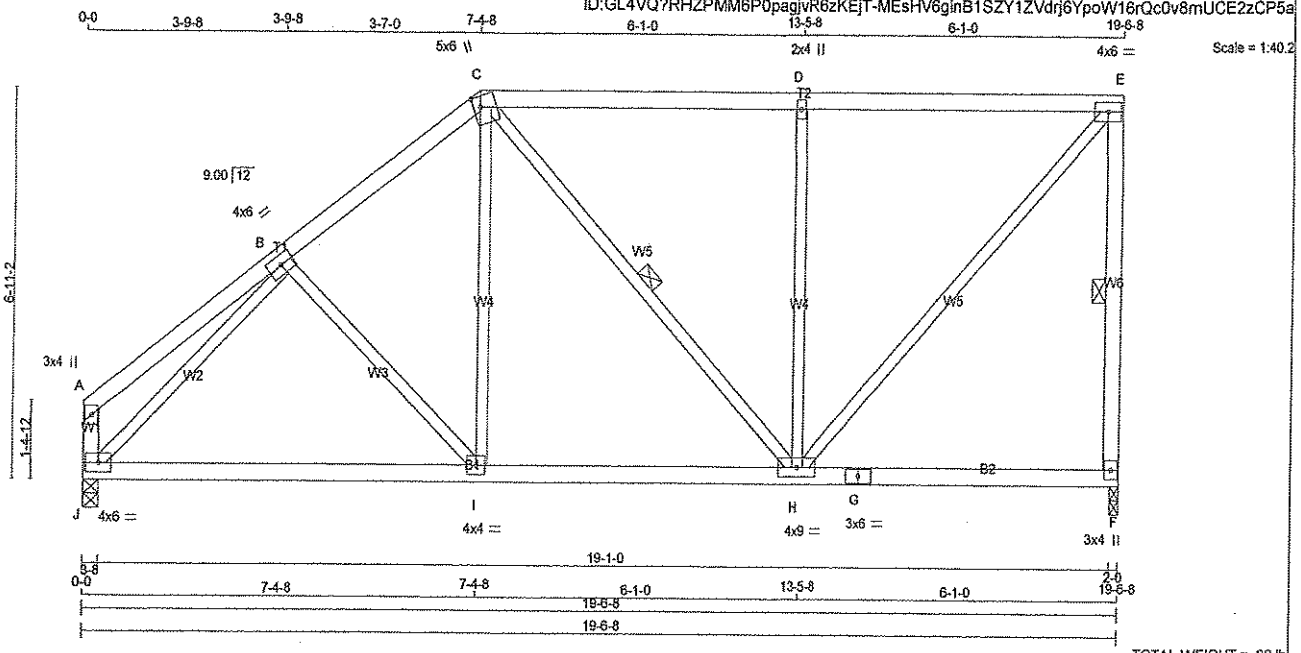

TOTAL WEIGHT = 88 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BS-t	MT20	3.0	6.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1097	0	1097	0	0	2-0	1-8
J	1097	0	1097	0	0	3-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0
J	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-84.3	-84.3	0.18 (1)		10.00
B- C	-997 / 0	-84.3	-84.3	0.19 (1)		5.02
C- D	-765 / 0	-84.3	-84.3	0.55 (1)		5.94
D- E	-765 / 0	-84.3	-84.3	0.55 (1)		5.94
F- E	-1025 / 0	0.0	0.0	0.22 (1)		6.20
J- A	-120 / 0	0.0	0.0	0.01 (1)		7.81
J- I	0 / 856	-28.0	-28.0	0.43 (2)		10.00
I- H	0 / 783	-28.0	-28.0	0.43 (2)		10.00
H- G	0 / 0	-28.0	-28.0	0.22 (3)		10.00
G- F	0 / 0	-28.0	-28.0	0.22 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- I	-109 / 62	0.05 (1)
I- C	0 / 365	0.08 (2)
C- H	-66 / 0	0.04 (3)
H- D	-635 / 0	0.52 (1)
H- E	0 / 1128	0.25 (1)
J- B	-1235 / 0	0.58 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.55 (D-E:1) , BC=0.43 (H-I:2) , WB=0.58 (B-J:1) , SSI=0.25 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.30 (B) (INPUT = 1.00)

DWG NO. TAM 24392-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T16X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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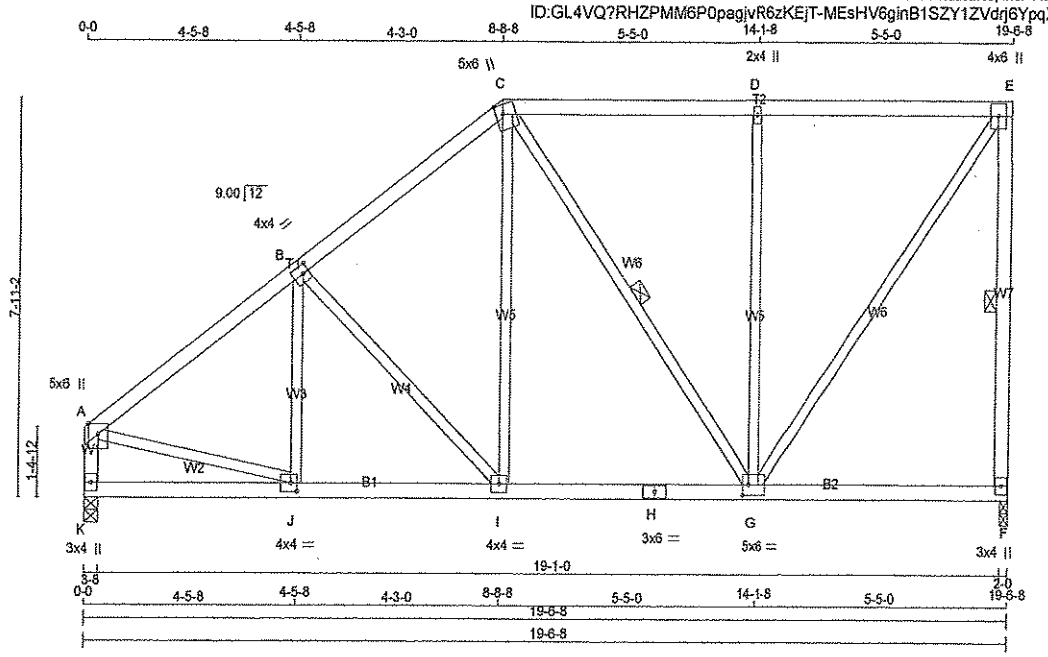

TOTAL WEIGHT = 95 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1097	0	1097	0	0	2-0	1-8
K	1097	0	1097	0	0	3-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0
K	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1111 / 0	-84.3	-84.3	0.29 (1)		5.65
B- C	-901 / 0	-84.3	-84.3	0.28 (1)		6.11
C- D	-615 / 0	-84.3	-84.3	0.42 (1)		6.25
D- E	-615 / 0	-84.3	-84.3	0.43 (1)		6.25
F- E	-1035 / 0	0.0	0.0	0.29 (1)		6.17
K- A	-1045 / 0	0.0	0.0	0.11 (1)		7.69
K- J	0 / 0	-28.0	-28.0	0.13 (3)		10.00
J- I	0 / 909	-28.0	-28.0	0.23 (2)		10.00
I- H	0 / 702	-28.0	-28.0	0.29 (2)		10.00
H- G	0 / 702	-28.0	-28.0	0.29 (2)		10.00
G- F	0 / 0	-28.0	-28.0	0.21 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-84 / 131	0.03 (3)
B- I	-301 / 0	0.20 (1)
I- C	0 / 388	0.08 (2)
C- G	-152 / 0	0.09 (1)
G- D	-564 / 0	0.67 (1)
G- E	0 / 1057	0.24 (1)
A- J	0 / 934	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.43 (D-E:1) , BC=0.29 (G-I:2) , WB=0.67 (D-G:1) , SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (G) (INPUT = 0.90)
JSI METAL= 0.34 (J) (INPUT = 1.00)

DWG NO. TAM 24393-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T17X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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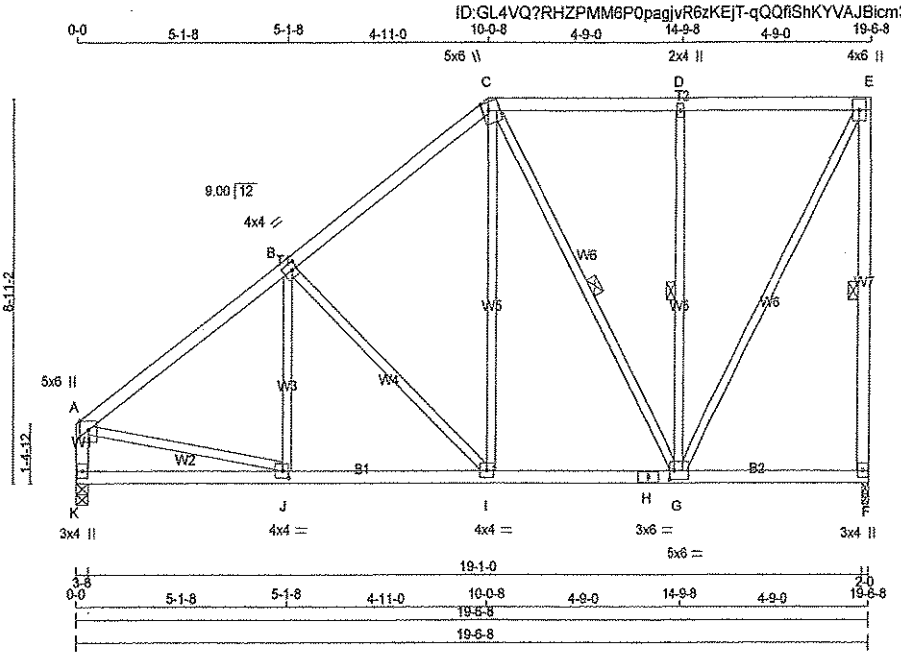

TOTAL WEIGHT = 100 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1097	0	1097	0	0	2-0	1-8
K	1097	0	1097	0	0	3-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0
K	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1108 / 0	-84.3	-84.3	0.29 (1)	5.68	
B- C	-811 / 0	-84.3	-84.3	0.28 (1)	6.25	
C- D	-492 / 0	-84.3	-84.3	0.24 (1)	6.25	
D- E	-492 / 0	-84.3	-84.3	0.24 (1)	6.25	
F- E	-1042 / 0	0.0	0.0	0.38 (1)	5.16	
K- A	-1039 / 0	0.0	0.0	0.11 (1)	7.71	
K- J	0 / 0	-28.0	-28.0	0.18 (3)	10.00	
J- I	0 / 910	-28.0	-28.0	0.28 (2)	10.00	
I- H	0 / 627	-28.0	-28.0	0.23 (2)	10.00	
H- G	0 / 627	-28.0	-28.0	0.23 (2)	10.00	
G- F	0 / 0	-28.0	-28.0	0.15 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-29 / 190	0.04 (3)
B- I	-403 / 0	0.39 (1)
I- C	0 / 417	0.09 (2)
C- G	-281 / 0	0.19 (1)
G- D	-493 / 0	0.25 (1)
G- E	0 / 1015	0.23 (1)
A- J	0 / 930	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.38 (E-F:1) , BC=0.28 (I-J:2) , WB=0.38 (B-I:1) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.34 (J) (INPUT = 1.00)

DWG NO. TAM 24394-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T18X	1	1	TRUSS DESC.	

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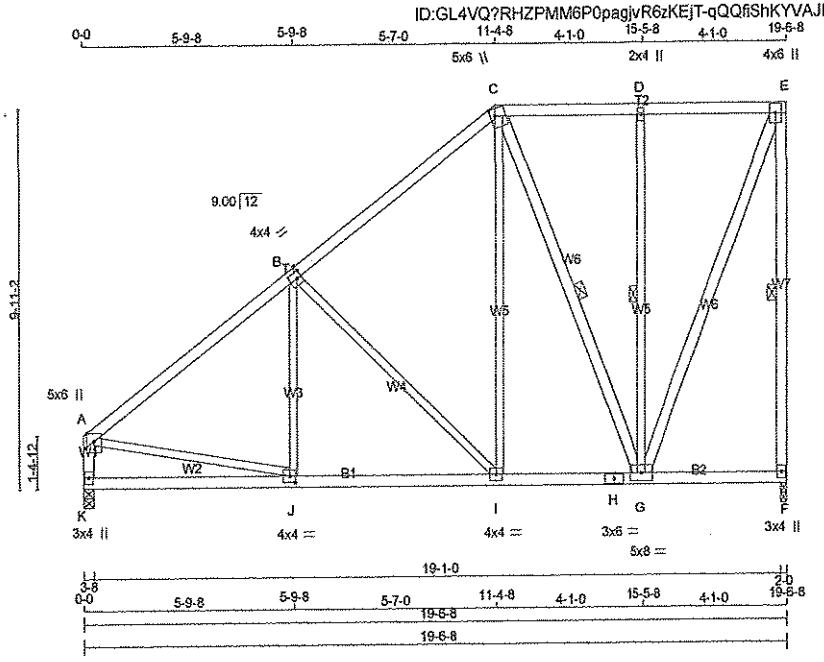

TOTAL WEIGHT = 113 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1097	0	1097	0	0	2-0	1-8
K	1097	0	1097	0	0	3-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0
K	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1096 / 0	-84.3	-84.3	0.38 (1)	5.57
B- C	-715 / 0	-84.3	-84.3	0.36 (1)	6.25
C- D	-391 / 0	-84.3	-84.3	0.17 (1)	6.25
D- E	-391 / 0	-84.3	-84.3	0.17 (1)	6.25
F- E	-1049 / 0	0.0	0.0	0.50 (1)	6.14
K- A	-1032 / 0	0.0	0.0	0.11 (1)	7.72
K- J	0 / 0	-28.0	-28.0	0.24 (3)	10.00
J- I	0 / 905	-28.0	-28.0	0.34 (2)	10.00
I- H	0 / 548	-28.0	-28.0	0.18 (2)	10.00
H- G	0 / 548	-28.0	-28.0	0.18 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.11 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 244	0.05 (3)
B- I	-500 / 0	0.63 (1)
I- C	0 / 473	0.11 (1)
C- G	-403 / 0	0.23 (1)
G- D	-422 / 0	0.28 (1)
G- E	0 / 995	0.16 (1)
A- J	0 / 920	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.50 (E-F:1) , BC=0.34 (I-J:2) , WB=0.63 (B-I:1) , SSI=0.19 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.34 (J) (INPUT = 1.00)

DWG NO. TAM 24395-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	G19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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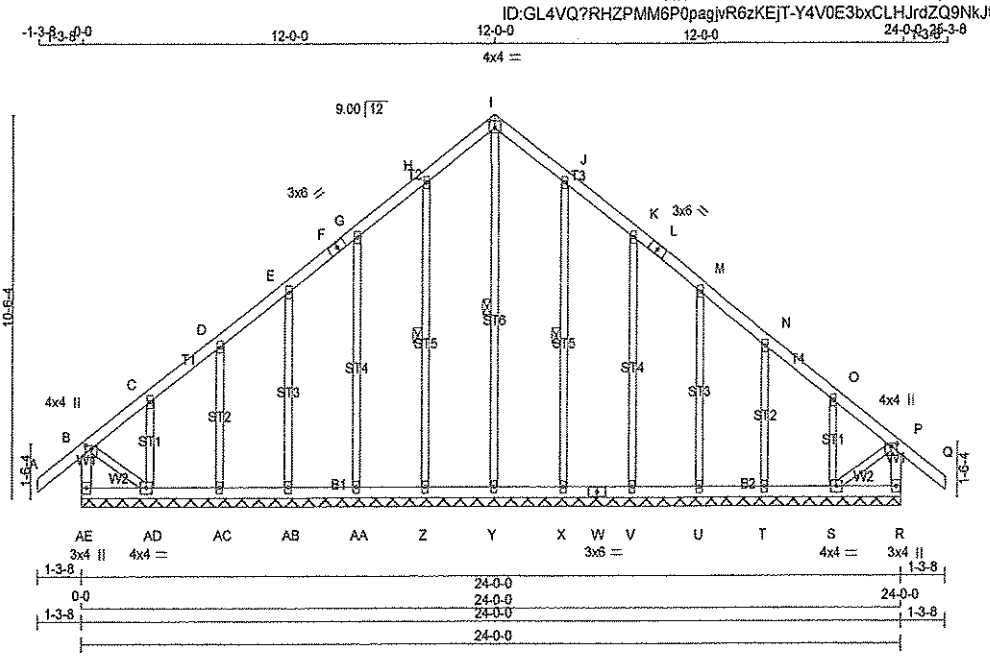

TOTAL WEIGHT = 124 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AE- B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - Q	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
AE- W	2x4	DRY	No.2	SPF
W - R	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, J, K, M, N, O						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
I	TTW-p	MT20	4.0	4.0	2.25	2.00
L	TS-t	MT20	3.0	6.0		
P	TMVW+p	MT20	4.0	4.0	1.00	2.00
R	BMV1+p	MT20	3.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T, U, V, X, Y, Z, AA, AB, AC						
T	BMW1+w	MT20	2.0	4.0		
W	BS-t	MT20	3.0	6.0		
AD	BMWW1-t	MT20	4.0	4.0		
AE	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-Y, H-Z, J-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
AE- B	-270 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-56 / 0	-84.3	-84.3	0.11 (1)	6.25
C- D	-25 / 0	-84.3	-84.3	0.04 (1)	6.25
D- E	-27 / 0	-84.3	-84.3	0.04 (1)	6.25
E- F	-23 / 0	-84.3	-84.3	0.04 (1)	6.25
F- G	-23 / 0	-84.3	-84.3	0.04 (1)	6.25
G- H	-18 / 0	-84.3	-84.3	0.05 (1)	6.25
H- I	-28 / 0	-84.3	-84.3	0.05 (1)	6.25
I- J	-28 / 0	-84.3	-84.3	0.05 (1)	6.25
J- K	-18 / 0	-84.3	-84.3	0.05 (1)	6.25
K- L	-23 / 0	-84.3	-84.3	0.04 (1)	6.25
L- M	-23 / 0	-84.3	-84.3	0.04 (1)	6.25
M- N	-27 / 0	-84.3	-84.3	0.04 (1)	6.25
N- O	-25 / 0	-84.3	-84.3	0.04 (1)	6.25
O- P	-56 / 0	-84.3	-84.3	0.11 (1)	6.25
P- Q	0 / 35	-84.3	-84.3	0.11 (1)	10.00
R- P	-270 / 0	0.0	0.0	0.03 (1)	7.81
AE-AD	0 / 0	-28.0	-28.0	0.03 (2)	10.00
AD-AC	0 / 24	-28.0	-28.0	0.03 (2)	10.00
AC-AB	0 / 21	-28.0	-28.0	0.02 (2)	10.00
AB-AA	0 / 18	-28.0	-28.0	0.02 (2)	10.00
AA- Z	0 / 16	-28.0	-28.0	0.02 (2)	10.00
Z- Y	0 / 14	-28.0	-28.0	0.02 (2)	10.00
Y- X	0 / 14	-28.0	-28.0	0.02 (2)	10.00
X- W	0 / 16	-28.0	-28.0	0.02 (2)	10.00
W- V	0 / 16	-28.0	-28.0	0.02 (2)	10.00
V- U	0 / 18	-28.0	-28.0	0.02 (2)	10.00
U- T	0 / 21	-28.0	-28.0	0.02 (2)	10.00
T- S	0 / 24	-28.0	-28.0	0.03 (2)	10.00
S- R	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- I	-121 / 0	0.09 (1)
Z- H	-191 / 0	0.10 (1)
AA- G	-163 / 0	0.17 (1)
AB- E	-165 / 0	0.09 (1)
AC- D	-176 / 0	0.05 (1)
AD- C	-119 / 0	0.02 (1)
X- J	-191 / 0	0.10 (1)
V- K	-163 / 0	0.17 (1)
U- M	-165 / 0	0.09 (1)
T- N	-176 / 0	0.05 (1)
S- O	-119 / 0	0.02 (1)
S- P	0 / 33	0.01 (1)
B-AD	0 / 33	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.03 (S-T:2) , WB=0.17 (K-V:1) , SSI=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (I) (INPUT = 0.90)
JSI METAL= 0.05 (J) (INPUT = 1.00)

DWG NO . TAM 24396 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267486	T20	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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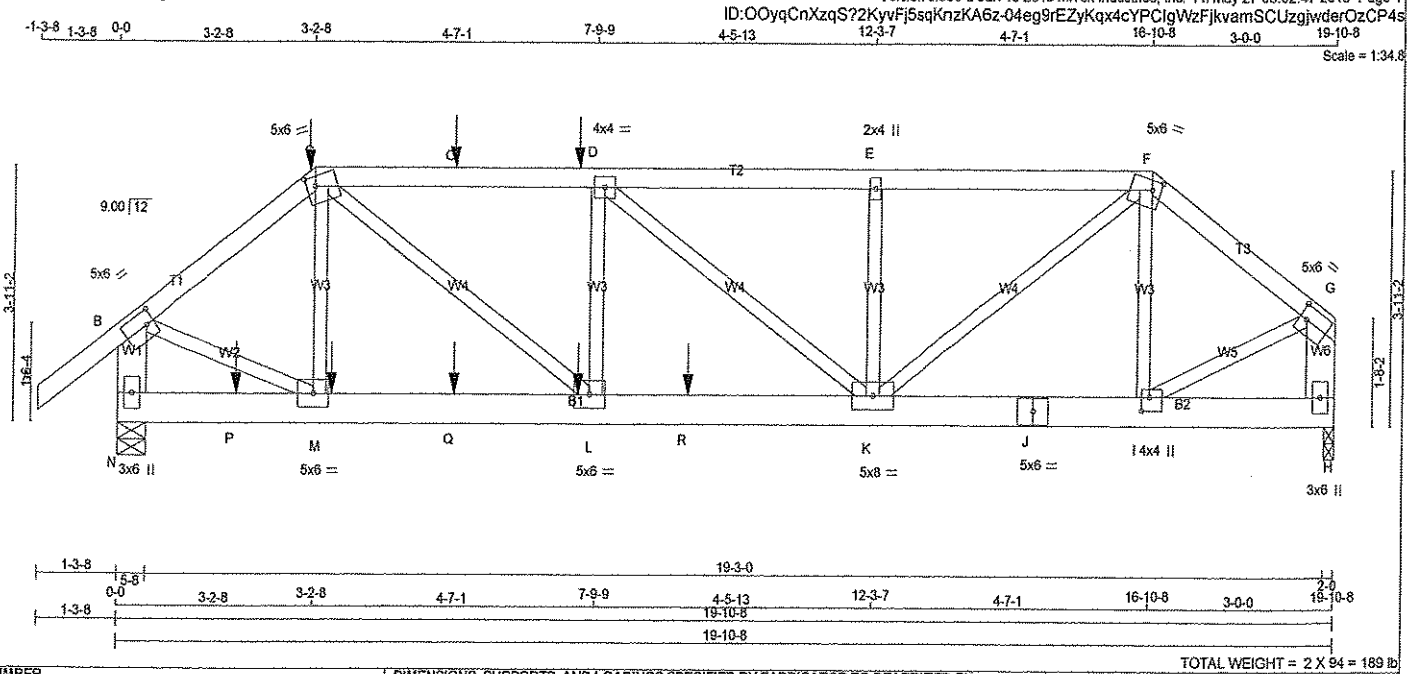

TOTAL WEIGHT = 2 X 94 = 189 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	TOP
N- B	2	12	TOP
H- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- J	2	12	SIDE(183.1)
J- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.50
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	1.75	1.75
G	TMVW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	4.0	4.0	2.50	1.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2309	0	2309	0	0	5-8	1-8
H	1813	0	1813	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1872	1077 / 0	400 / 0	0 / 0	0 / 0	395 / 0	0 / 0
H	1469	847 / 0	313 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.06 (1)		10.00	M- C	-528 / 9	0.06 (1)
B- C	-2283 / 0	-84.3	-84.3	0.10 (1)		5.82	C- L	0 / 2155	0.27 (1)
C- O	-3513 / 0	-84.3	-84.3	0.23 (1)		4.78	L- D	-207 / 119	0.02 (1)
O- D	-3513 / 0	-84.3	-84.3	0.23 (1)		4.78	D- K	-560 / 0	0.16 (1)
D- E	-3075 / 0	-84.3	-84.3	0.18 (1)		5.10	K- E	-390 / 0	0.05 (1)
E- F	-3074 / 0	-84.3	-84.3	0.16 (1)		5.13	K- F	0 / 2119	0.26 (1)
F- G	-1768 / 0	-84.3	-84.3	0.08 (1)		6.25	I- F	-528 / 0	0.06 (1)
N- B	-2238 / 0	0.0	0.0	0.08 (1)		7.81	B- M	0 / 1921	0.24 (1)
H- G	-1768 / 0	0.0	0.0	0.07 (1)		7.81	I- G	0 / 1523	0.19 (1)
N- P	0 / 0	-28.0	-28.0	0.03 (2)		10.00			
P- M	0 / 0	-28.0	-28.0	0.03 (2)		10.00			
M- Q	0 / 1806	-28.0	-28.0	0.26 (1)		10.00			
Q- L	0 / 1806	-28.0	-28.0	0.26 (1)		10.00			
L- R	0 / 3514	-28.0	-28.0	0.57 (1)		10.00			
R- K	0 / 3514	-28.0	-28.0	0.57 (1)		10.00			
K- J	0 / 1395	-28.0	-28.0	0.18 (1)		10.00			
J- I	0 / 1395	-28.0	-28.0	0.18 (1)		10.00			
I- H	0 / 0	-28.0	-28.0	0.02 (3)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	3-2-8	-150	-150	---	FRONT	VERT	TOTAL
C	3-2-8	-93	-93	---	BACK	VERT	TOTAL
D	7-6-1	-90	-90	---	BACK	VERT	TOTAL
L	7-6-1	-36	-63	---	BACK	VERT	TOTAL
M	3-6-1	-36	-63	---	BACK	VERT	TOTAL
O	5-6-1	-90	-90	---	BACK	VERT	TOTAL
P	1-11-4	-36	-63	---	BACK	VERT	TOTAL
Q	5-6-1	-36	-63	---	BACK	VERT	TOTAL
R	9-3-8	-1206	-1206	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.23 (C-D:1) , BC=0.57 (K-L:1) , WB=0.27 (C-L:1) , SSI=0.30 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.90 (I) (INPUT = 0.90)
MSI METAL= 0.30 (B) (INPUT = 1.00)

PB 1h

CONTINUED ON PAGE 2

DWG NO. TAM 24413-16
STRUCTURAL
COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO
5-27-16

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T20	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 149.8 lbs FACTORED DOWN AT 3-2-8, 92.9 lbs FACTORED DOWN AT 3-2-8, AND 90.3 lbs FACTORED DOWN AT 5-6-1, AND 90.3 lbs FACTORED DOWN AT 7-6-1 ON TOP CHORD, AND 63.2 lbs FACTORED DOWN AT 1-11-4, 63.2 lbs FACTORED DOWN AT 3-6-1, 63.2 lbs FACTORED DOWN AT 5-6-1, AND 63.2 lbs FACTORED DOWN AT 7-6-1, AND 1206.4 lbs FACTORED DOWN AT 9-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM24413-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T21	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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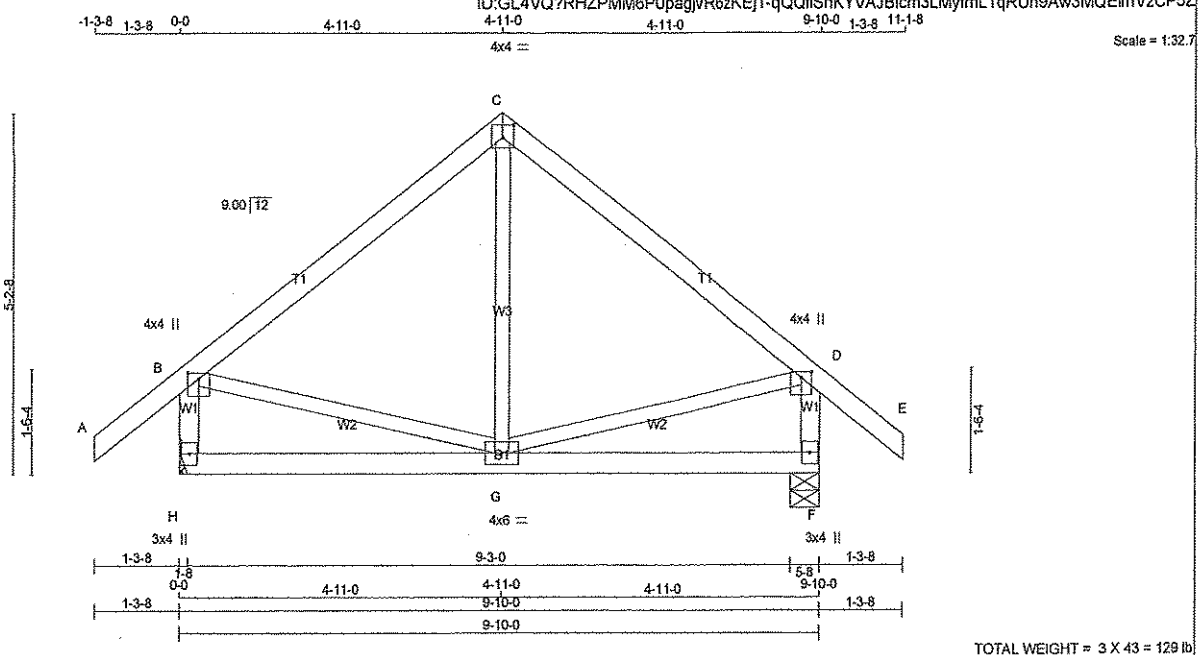

TOTAL WEIGHT = 3 X 43 = 129 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	668	0	668	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	668	0	668	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	532	322 / 0	103 / 0	0 / 0	0 / 0	107 / 0	0 / 0
F	532	322 / 0	103 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-368 / 0	-84.3	-84.3	0.26 (1)	6.25
C- D	-368 / 0	-84.3	-84.3	0.26 (1)	6.25
D- E	0 / 35	-84.3	-84.3	0.11 (1)	10.00
H- B	-616 / 0	0.0	0.0	0.06 (1)	7.81
F- D	-616 / 0	0.0	0.0	0.06 (1)	7.81
H- G	0 / 0	-28.0	-28.0	0.20 (3)	10.00
G- F	0 / 0	-28.0	-28.0	0.20 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 217	0.05 (3)
B- G	0 / 303	0.07 (1)
G- D	0 / 303	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.26 (B-C:1) , BC=0.20 (G-H:3) , WB=0.07 (B-G:1) , SSI=0.13 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (D) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM24392-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	G21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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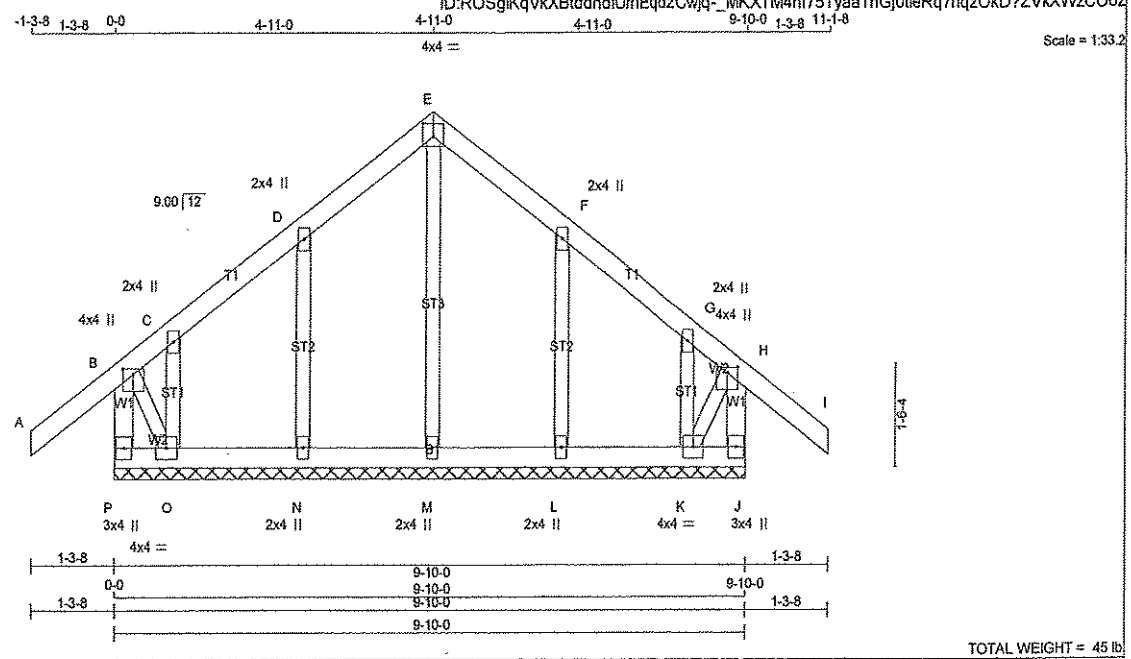


TOTAL WEIGHT = 45 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
P- B	-245 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-56 / 0	-84.3	-84.3	0.11 (1)	6.25
C- D	0 / 4	-84.3	-84.3	0.05 (1)	10.00
D- E	-9 / 0	-84.3	-84.3	0.05 (1)	10.00
E- F	-9 / 0	-84.3	-84.3	0.05 (1)	10.00
F- G	0 / 4	-84.3	-84.3	0.05 (1)	10.00
G- H	-56 / 0	-84.3	-84.3	0.11 (1)	6.25
H- I	0 / 35	-84.3	-84.3	0.11 (1)	10.00
J- H	-245 / 0	0.0	0.0	0.03 (1)	7.81
P- O	0 / 0	-28.0	-28.0	0.02 (3)	10.00
O- N	0 / 4	-28.0	-28.0	0.02 (3)	10.00
N- M	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
M- L	-2 / 0	-28.0	-28.0	0.02 (3)	10.00
L- K	0 / 4	-28.0	-28.0	0.02 (3)	10.00
K- J	0 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
M- E	-140 / 0	0.05 (1)
N- D	-202 / 0	0.04 (1)
O- C	-20 / 0	0.00 (1)
L- F	-202 / 0	0.04 (1)
K- G	-20 / 0	0.00 (1)
B- O	0 / 8	0.00 (1)
K- H	0 / 8	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (H-I:1) , BC=0.02 (N-O:3) , WB=0.05 (E-M:1) , SSI=0.07 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

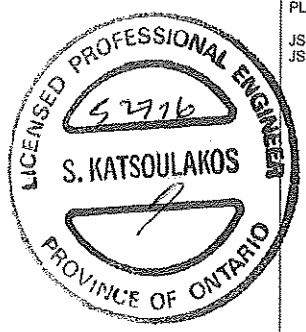
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (H) (INPUT = 0.90)
JSI METAL= 0.05 (F) (INPUT = 1.00)



DWG NO. TAM 24427-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T21Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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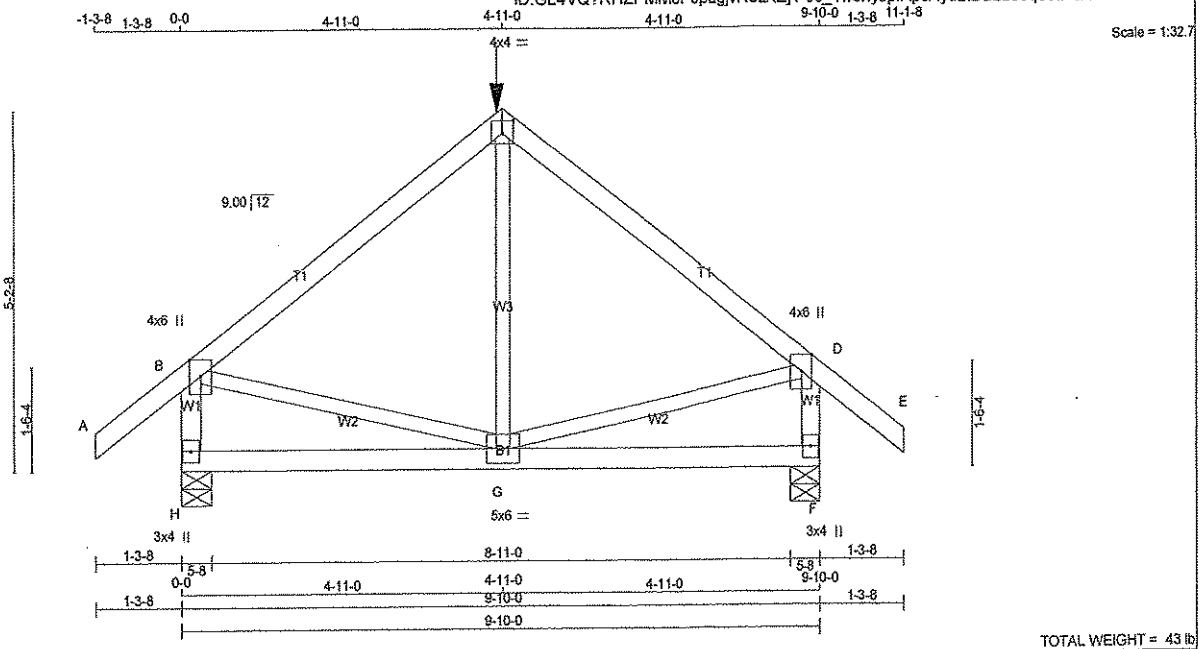

TOTAL WEIGHT = 43 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 471.9 lbs FACTORED DOWN AT 4-11-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1328	0	1328	0	0	5-8	1-8
F	1328	0	1328	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1077	620 / 0	230 / 0	0 / 0	0 / 0	226 / 0	0 / 0
F	1077	620 / 0	230 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.13 (1)	10.00	G- C	-93 / 304	0.08 (3)
B- C	-943 / 0	-153.3	-153.3	0.76 (1)	4.91	B- G	0 / 777	0.19 (1)
C- D	-943 / 0	-153.3	-153.3	0.76 (1)	4.91	G- D	0 / 777	0.19 (1)
D- E	0 / 35	-84.3	-84.3	0.13 (1)	10.00			
H- B	-1242 / 0	0.0	0.0	0.14 (1)	7.17			
F- D	-1242 / 0	0.0	0.0	0.14 (1)	7.17			
H- G	0 / 0	-45.2	-45.2	0.35 (3)	10.00			
G- F	0 / 0	-45.2	-45.2	0.35 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-11-0	-472	-472	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 4-11-0
END DISTANCE = 9-10-0
END SPAN CARRIED = 4-11-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-11-0
END SETBACK = 4-11-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.76 (B-C:1) , BC=0.35 (G-H:3) , WB=0.19 (B-G:1) , SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LICENSED PROFESSIONAL ENGINEER 5916 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 24398 16

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	T21Z	1	1	TRUSS DESC.	

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5716
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 24/398 - 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267492	T21S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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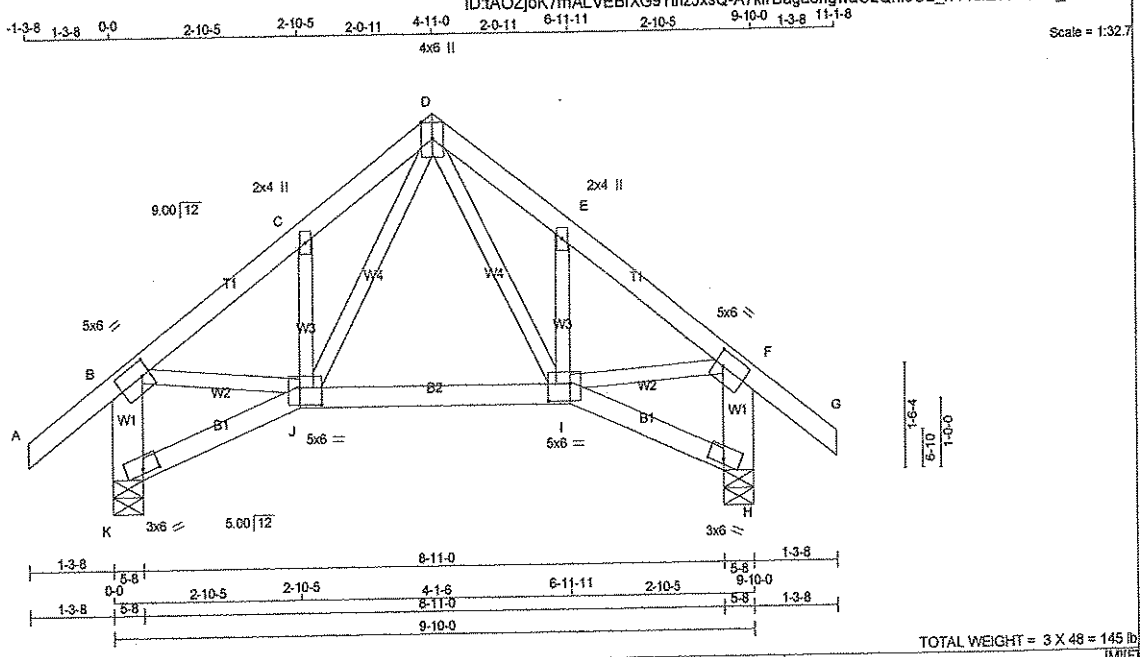

TOTAL WEIGHT = 3 X 48 = 145 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	6.0	2.50	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMW-t	MT20	5.0	6.0	2.50	1.50
H	BVM1-I	MT20	3.0	6.0	0.75	3.00
I	BBWWW-p	MT20	5.0	6.0	3.00	4.00
J	BBWWW-p	MT20	5.0	6.0	3.00	4.00
K	BVM1-I	MT20	3.0	6.0	0.75	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	668	0	668	0	0	5-8	1-8
H	668	0	668	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	532	322 / 0	103 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	532	322 / 0	103 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-615 / 0	-84.3	-84.3	0.08 (1)	6.25
C- D	-643 / 0	-84.3	-84.3	0.06 (1)	6.25
D- E	-643 / 0	-84.3	-84.3	0.06 (1)	6.25
E- F	-615 / 0	-84.3	-84.3	0.08 (1)	6.25
F- G	0 / 35	-84.3	-84.3	0.11 (1)	10.00
K- B	-628 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-628 / 0	0.0	0.0	0.04 (1)	7.81
K- J	0 / 0	-28.0	-28.0	0.07 (3)	10.00
J- I	0 / 335	-28.0	-28.0	0.18 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.07 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 357	0.08 (1)
I- E	-250 / 0	0.04 (1)
J- D	0 / 357	0.08 (1)
J- C	-250 / 0	0.04 (1)
B- J	0 / 502	0.11 (1)
I- F	0 / 502	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.11 (A-B:1) , BC=0.18 (I-J:2) , WB=0.11 (B-J:1) , SSI=0.09 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (I) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

DWG NO. TAM24455-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T22	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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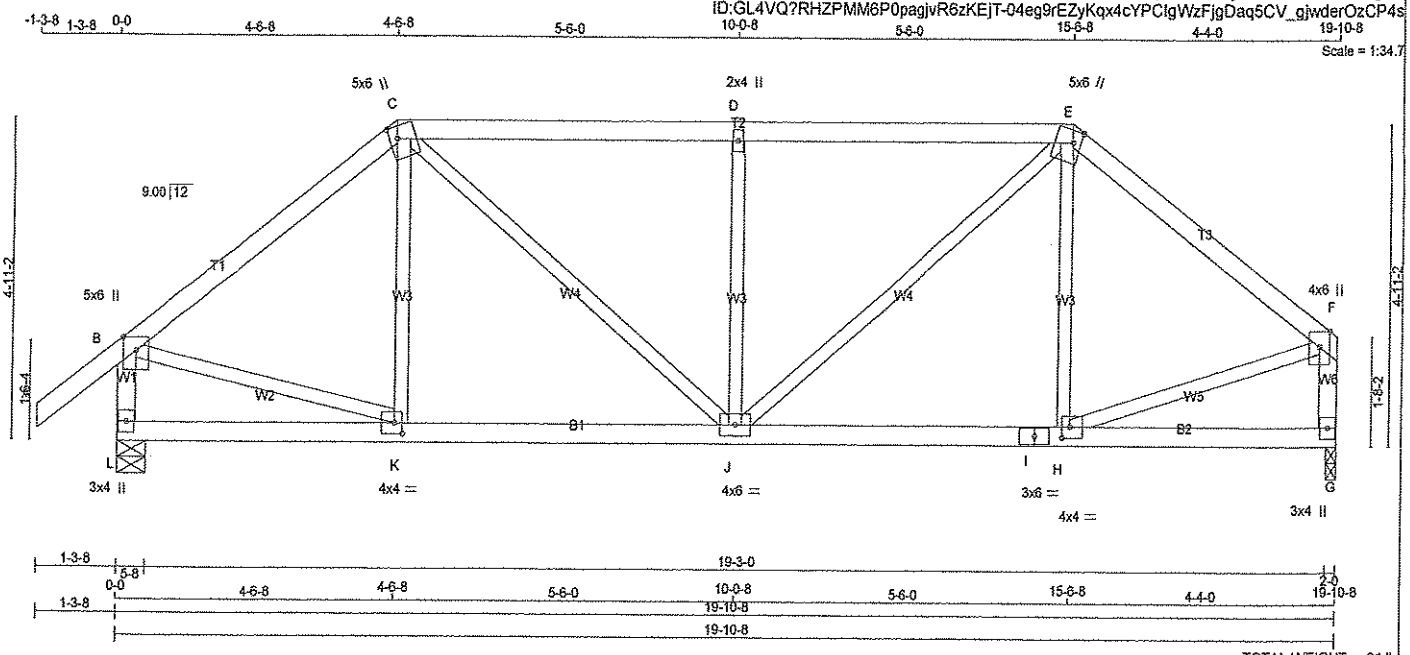

TOTAL WEIGHT = 81 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1232	0	1232	0	0	5-8	1-8
G	1116	0	1116	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00		K- C	-64 / 158	0.04 (3)
B- C	-1083 / 0	-84.3	-84.3	0.34 (1)	5.63		C- J	0 / 535	0.12 (1)
C- D	-1273 / 0	-84.3	-84.3	0.47 (1)	5.07		J- D	-566 / 0	0.20 (1)
D- E	-1273 / 0	-84.3	-84.3	0.47 (1)	5.07		J- E	0 / 572	0.13 (1)
E- F	-1049 / 0	-84.3	-84.3	0.30 (1)	5.75		H- E	-100 / 135	0.04 (1)
L- B	-1182 / 0	0.0	0.0	0.12 (1)	7.34		B- K	0 / 693	0.20 (1)
G- F	-1069 / 0	0.0	0.0	0.11 (1)	7.63		H- F	0 / 875	0.20 (1)
L- K	0 / 0	-28.0	-28.0	0.17 (3)	10.00				
K- J	0 / 864	-28.0	-28.0	0.27 (2)	10.00				
J- I	0 / 835	-28.0	-28.0	0.26 (2)	10.00				
I- H	0 / 835	-28.0	-28.0	0.26 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.16 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.47 (C-D:1) , BC=0.27 (J-K:2) , WB=0.20 (D-J:1) , SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5716
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 24/14/16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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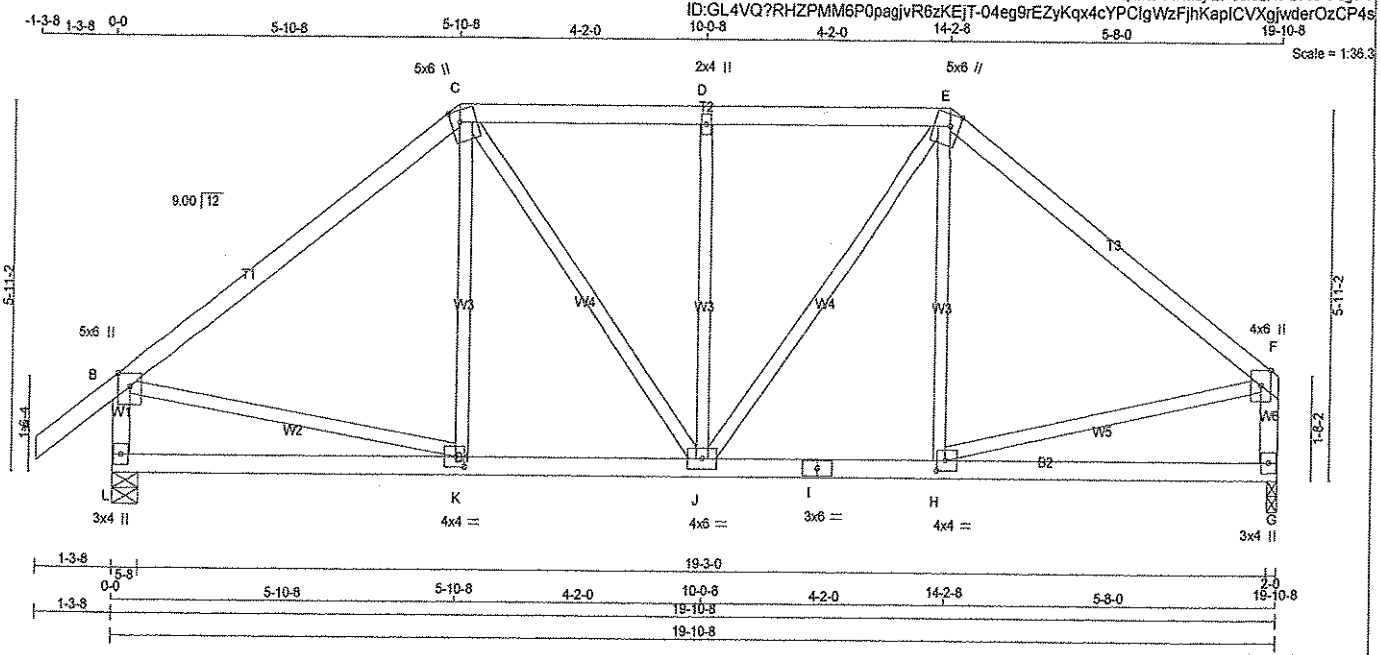

TOTAL WEIGHT = 87 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.75
F	TMW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.75
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1232	0	1232	0	0	5-8	1-8
G	1116	0	1116	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-1051 / 0	-84.3	-84.3	0.40 (1)	5.63
C- D	-1015 / 0	-84.3	-84.3	0.19 (1)	6.00
D- E	-1015 / 0	-84.3	-84.3	0.19 (1)	6.00
E- F	-1029 / 0	-84.3	-84.3	0.37 (1)	5.72
L- B	-1163 / 0	0.0	0.0	0.12 (1)	7.39
G- F	-1050 / 0	0.0	0.0	0.11 (1)	7.68
L- K	0 / 0	-28.0	-28.0	0.22 (3)	10.00
K- J	0 / 840	-28.0	-28.0	0.32 (2)	10.00
J- I	0 / 821	-28.0	-28.0	0.31 (2)	10.00
I- H	0 / 821	-28.0	-28.0	0.31 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 220	0.05 (3)
C- J	0 / 294	0.07 (1)
J- D	-424 / 0	0.23 (1)
J- E	0 / 325	0.07 (1)
H- E	-20 / 199	0.04 (3)
B- K	0 / 856	0.19 (1)
H- F	0 / 844	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.40 (B-C:1) , BC=0.32 (J-K:2) , WB=0.23 (D-J:1) , SSI=0.17 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.31 (K) (INPUT = 1.00)

DWG NO . TAM 24/15-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T24	1	1	TRUSS DESC.	

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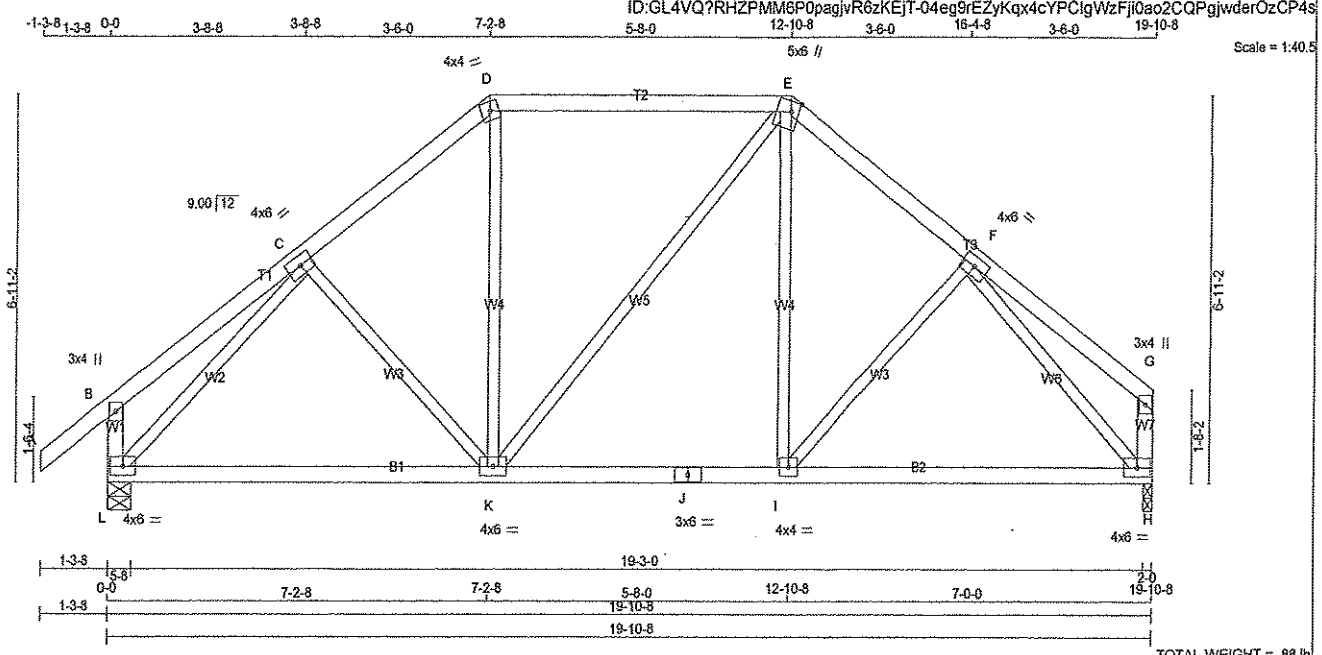


TOTAL WEIGHT = 88 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.75
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1232	0	1232	0	0	5-8	1-8
H	1116	0	1116	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0
H	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 20	-84.3	-84.3	0.17 (1)	10.00
C- D	-1010 / 0	-84.3	-84.3	0.14 (1)	6.07
D- E	-794 / 0	-84.3	-84.3	0.36 (1)	6.25
E- F	-995 / 0	-84.3	-84.3	0.13 (1)	6.12
F- G	0 / 20	-84.3	-84.3	0.16 (1)	10.00
L- B	-233 / 0	0.0	0.0	0.02 (1)	7.81
H- G	-108 / 0	0.0	0.0	0.01 (1)	7.81
L- K	0 / 843	-28.0	-28.0	0.40 (2)	10.00
K- J	0 / 783	-28.0	-28.0	0.39 (2)	10.00
J- I	0 / 783	-28.0	-28.0	0.39 (2)	10.00
I- H	0 / 803	-28.0	-28.0	0.38 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-76 / 73	0.04 (1)
K- D	0 / 308	0.07 (2)
K- E	0 / 18	0.00 (1)
I- E	0 / 289	0.06 (2)
I- F	-35 / 88	0.02 (3)
L- C	-1243 / 0	0.56 (1)
F- H	-1224 / 0	0.52 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.36 (D-E:1) , BC=0.40 (K-L:2) , WB=0.56 (C-L:1) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

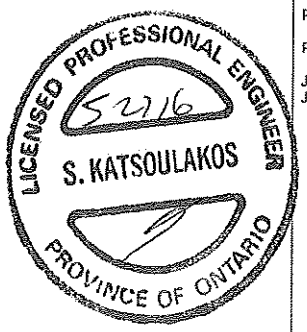
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (F) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)



DWG NO. TAM24416-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T25	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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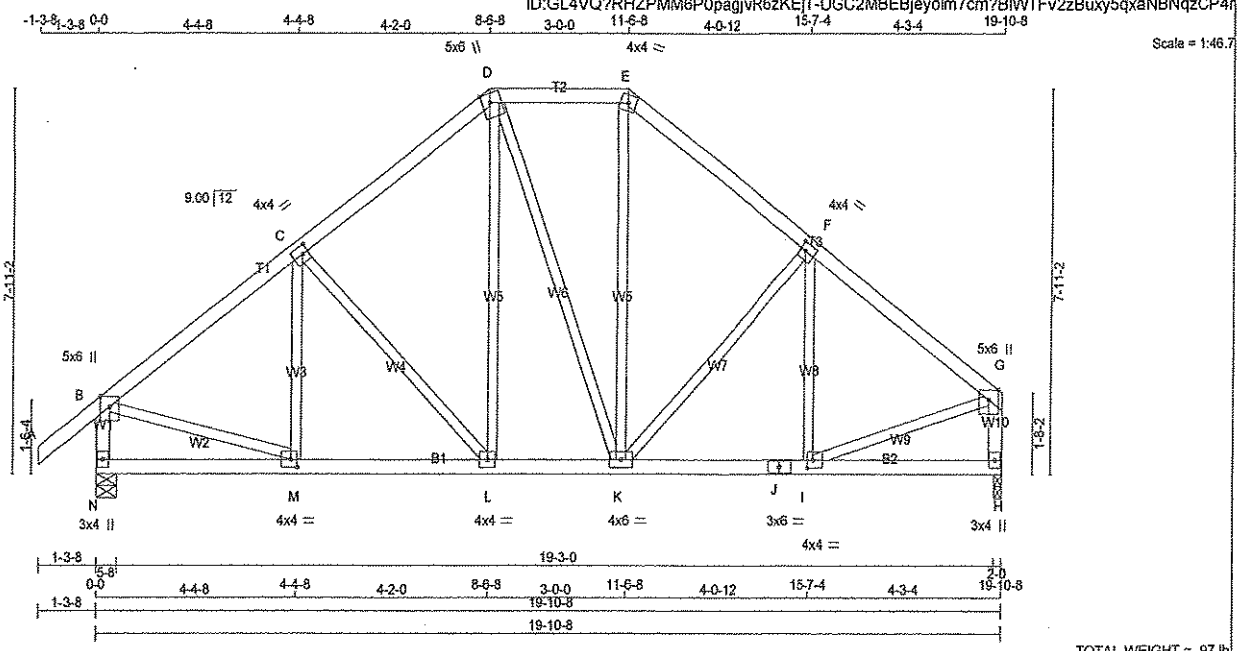

TOTAL WEIGHT = 97 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	2.00
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1232	0	1232	0	0	5-8	1-8
H	1116	0	1116	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0
H	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-1103 / 0	-84.3	-84.3	0.21 (1)	5.80
C- D	-919 / 0	-84.3	-84.3	0.20 (1)	6.21
D- E	-713 / 0	-84.3	-84.3	0.10 (1)	6.25
E- F	-915 / 0	-84.3	-84.3	0.19 (1)	6.23
F- G	-1063 / 0	-84.3	-84.3	0.20 (1)	5.89
N- B	-1182 / 0	0.0	0.0	0.12 (1)	7.34
H- G	-1066 / 0	0.0	0.0	0.11 (1)	7.63
N- M	0 / 0	-28.0	-28.0	0.13 (3)	10.00
M- L	0 / 902	-28.0	-28.0	0.23 (2)	10.00
L- K	0 / 717	-28.0	-28.0	0.16 (1)	10.00
K- J	0 / 870	-28.0	-28.0	0.22 (2)	10.00
J- I	0 / 870	-28.0	-28.0	0.22 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.12 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-108 / 123	0.04 (1)
C- L	-274 / 0	0.18 (1)
L- D	0 / 299	0.07 (2)
D- K	-10 / 0	0.01 (1)
K- E	0 / 293	0.07 (2)
K- F	-237 / 0	0.16 (1)
I- F	-143 / 100	0.05 (1)
B- M	0 / 935	0.21 (1)
I- G	0 / 912	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.21 (B-C:1) , BC=0.23 (L-M:2) , WB=0.21 (B-M:1) , SSI=0.14 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.34 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
E2716
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM24417-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T31	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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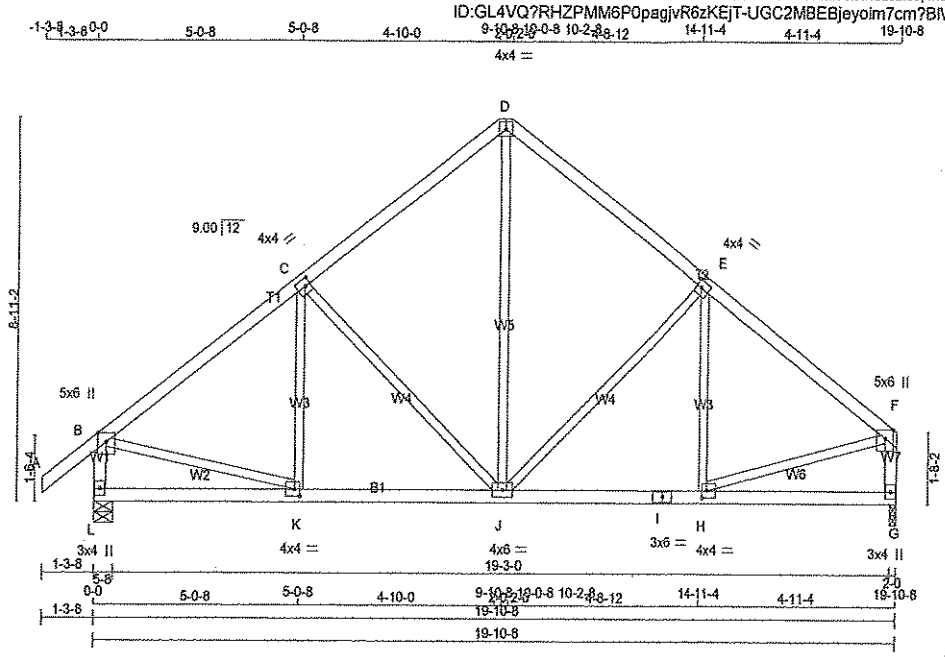

TOTAL WEIGHT = 2 X 89 = 178 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMWW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1232	0	1232	0	0	5-8	1-8
G	1116	0	1116	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-1105 / 0	-84.3	-84.3	0.29 (1)	5.68
C- D	-832 / 0	-84.3	-84.3	0.28 (1)	6.25
D- E	-831 / 0	-84.3	-84.3	0.27 (1)	6.25
E- F	-1072 / 0	-84.3	-84.3	0.28 (1)	5.77
L- B	-1174 / 0	0.0	0.0	0.12 (1)	7.36
G- F	-1061 / 0	0.0	0.0	0.11 (1)	7.64
L- K	0 / 0	-28.0	-28.0	0.17 (3)	10.00
K- J	0 / 908	-28.0	-28.0	0.27 (2)	10.00
J- I	0 / 882	-28.0	-28.0	0.26 (2)	10.00
I- H	0 / 882	-28.0	-28.0	0.26 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-59 / 171	0.04 (3)
C- J	-376 / 0	0.36 (1)
J- E	-339 / 0	0.32 (1)
H- E	-94 / 148	0.04 (1)
B- K	0 / 933	0.21 (1)
H- F	0 / 916	0.21 (1)
J- D	0 / 640	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.29 (B-C:1) , BC=0.27 (J-K:2) , WB=0.36 (C-J:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

DWG NO. TAM24418-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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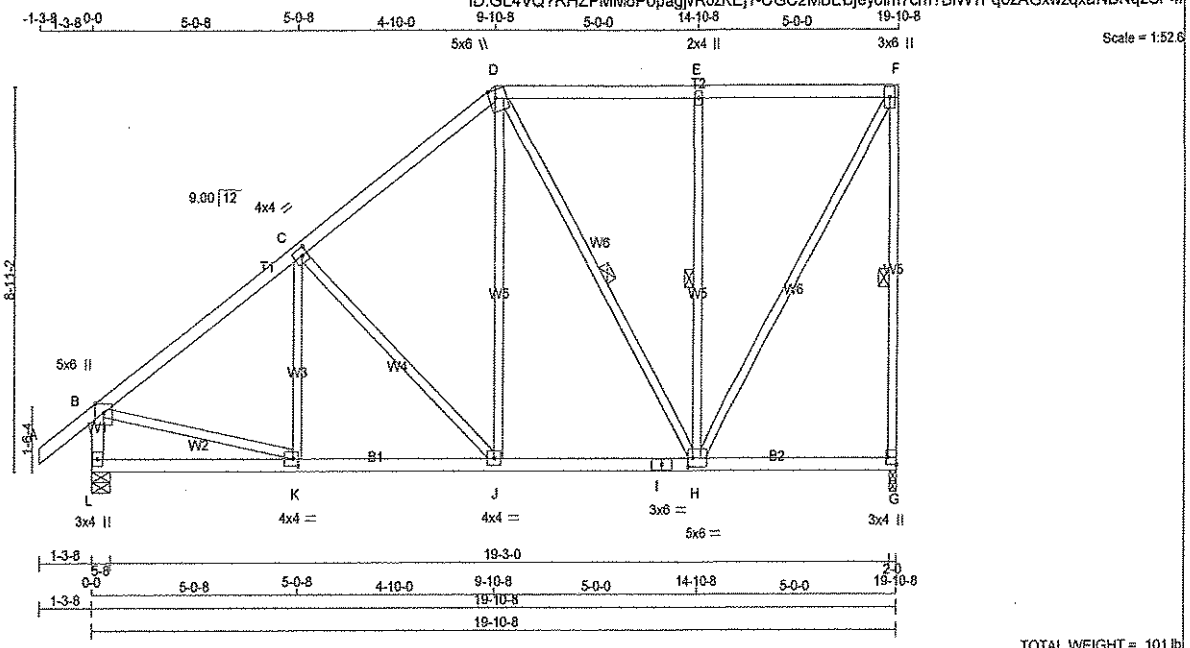

TOTAL WEIGHT = 101 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x3	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	3.0	6.0		
G	BMV1+p	MT20	3.0	4.0	2.00	Edge
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1116	0	1116	0	0	2-0	1-8
L	1232	0	1232	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-1104 / 0	-84.3	-84.3	0.39 (1)	5.52
C- D	-838 / 0	-84.3	-84.3	0.37 (1)	6.13
D- E	-520 / 0	-84.3	-84.3	0.35 (1)	6.25
E- F	-520 / 0	-84.3	-84.3	0.35 (1)	6.25
G- F	-1059 / 0	0.0	0.0	0.55 (1)	5.38
L- B	-1174 / 0	0.0	0.0	0.12 (1)	7.36
L- K	0 / 0	-28.0	-28.0	0.17 (3)	10.00
K- J	0 / 907	-28.0	-28.0	0.27 (2)	10.00
J- I	0 / 650	-28.0	-28.0	0.25 (2)	10.00
I- H	0 / 650	-28.0	-28.0	0.25 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-58 / 171	0.04 (3)
C- J	-372 / 0	0.35 (1)
J- D	0 / 410	0.09 (2)
D- H	-258 / 0	0.18 (1)
H- E	-520 / 0	0.27 (1)
H- F	0 / 1038	0.23 (1)
B- K	0 / 932	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.55 (F-G:1) , BC=0.27 (J-K:2) , WB=0.35 (C-J:1) , SSI=0.20 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO . TAM 24419-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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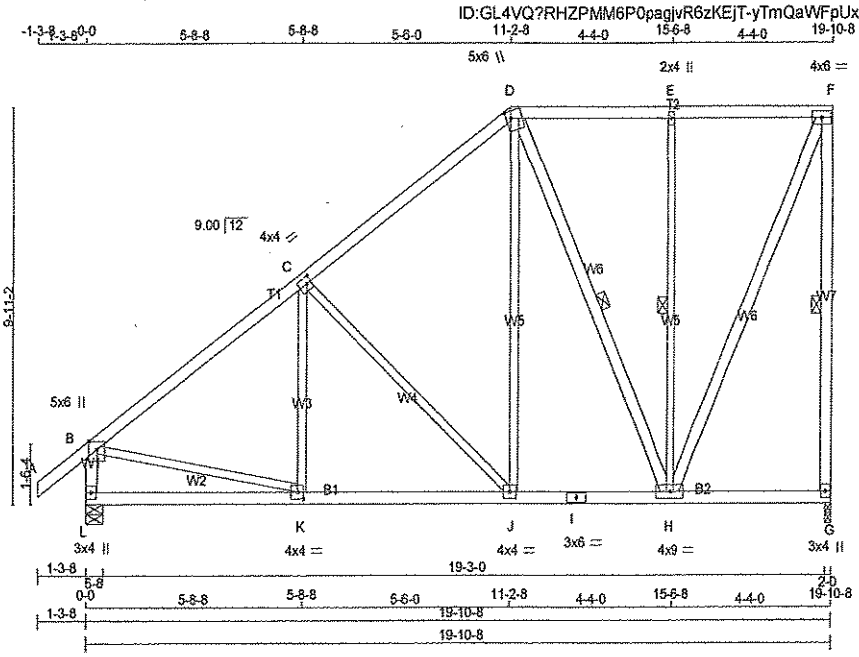

TOTAL WEIGHT = 116 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1116	0	1116	0	0	2-0	1-8
L	1232	0	1232	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-1097 / 0	-84.3	-84.3	0.37 (1)	5.58
C- D	-746 / 0	-84.3	-84.3	0.35 (1)	6.25
D- E	-419 / 0	-84.3	-84.3	0.20 (1)	6.25
E- F	-419 / 0	-84.3	-84.3	0.20 (1)	6.25
G- F	-1065 / 0	0.0	0.0	0.51 (1)	6.11
L- B	-1168 / 0	0.0	0.0	0.12 (1)	7.38
L- K	0 / 0	-28.0	-28.0	0.23 (3)	10.00
K- J	0 / 905	-28.0	-28.0	0.33 (2)	10.00
J- I	0 / 573	-28.0	-28.0	0.19 (2)	10.00
I- H	0 / 573	-28.0	-28.0	0.19 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.12 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-7 / 227	0.05 (3)
C- J	-471 / 0	0.59 (1)
J- D	0 / 459	0.10 (1)
D- H	-376 / 0	0.22 (1)
H- E	-449 / 0	0.30 (1)
H- F	0 / 1014	0.16 (1)
B- K	0 / 924	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.51 (F-G:1) , BC=0.33 (J-K:2) , WB=0.59 (C-J:1) , SSI=0.19 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
52216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 24/4/2016
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	T34	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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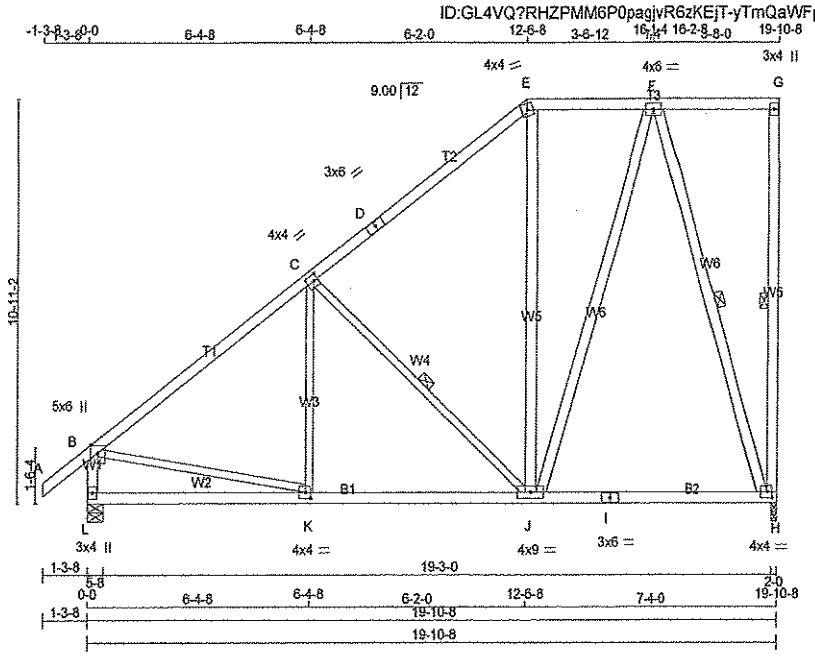

TOTAL WEIGHT = 117 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - C	2x3	DRY	No.2	SPF
C - J	2x3	DRY	No.2	SPF
B - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0	2.00	1.75
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1116	0	1116	0	0	2-0	1-8
L	1232	0	1232	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-J, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	K- C	0 / 247	0.06 (3)	
B- C	-1078 / 0	-84.3	-84.3	0.48 (1)	5.45	C- J	-552 / 0	0.28 (1)	
C- D	-656 / 0	-84.3	-84.3	0.44 (1)	6.25	J- E	0 / 168	0.03 (3)	
D- E	-656 / 0	-84.3	-84.3	0.44 (1)	6.25	J- F	0 / 564	0.09 (1)	
E- F	-493 / 0	-84.3	-84.3	0.15 (1)	6.25	F- H	-970 / 0	0.67 (1)	
F- G	0 / 0	-84.3	-84.3	0.19 (1)	10.00	B- K	0 / 909	0.20 (1)	
H- G	-117 / 0	0.0	0.0	0.07 (1)	6.25				
L- B	-1158 / 0	0.0	0.0	0.12 (1)	7.40				
L- K	0 / 0	-28.0	-28.0	0.25 (3)	10.00				
K- J	0 / 893	-28.0	-28.0	0.43 (2)	10.00				
J- I	0 / 316	-28.0	-28.0	0.36 (2)	10.00				
I- H	0 / 316	-28.0	-28.0	0.36 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.48 (B-C:1) , BC=0.43 (J-K:2) , WB=0.67 (F-H:1) , SSI=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.33 (K) (INPUT = 1.00)

DWG NO. TAM 24/2/16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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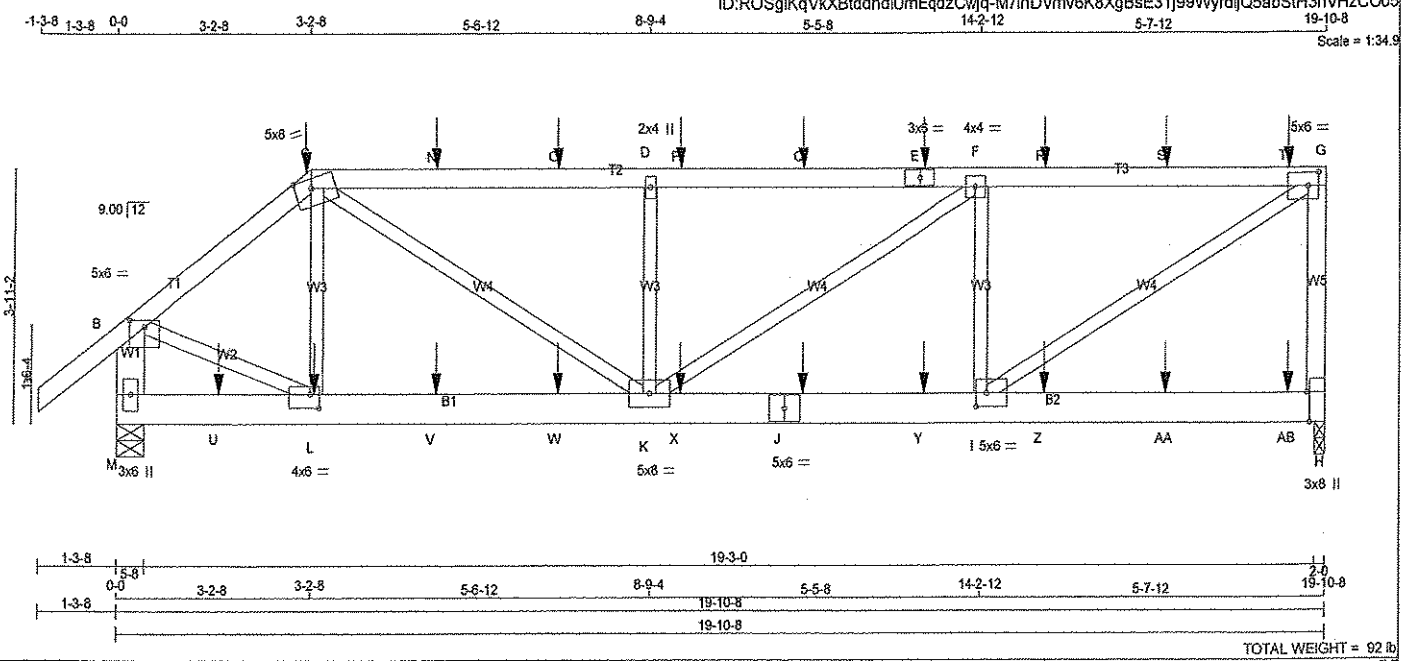

TOTAL WEIGHT = 92 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0	2.50	2.00
H	BMV1+t	MT20	3.0	8.0	Edge	0.50
I	BMWW-t	MT20	5.0	6.0	2.50	2.00
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	4.0	6.0	2.50	1.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 257.0 lbs FACTORED DOWN AT 3-2-8, 90.3 lbs FACTORED DOWN AT 5-3-4, 90.3 lbs FACTORED DOWN AT 7-3-4, 90.3 lbs FACTORED DOWN AT 9-3-4, 90.3 lbs FACTORED DOWN AT 11-3-4, 90.3 lbs FACTORED DOWN AT 13-3-4, 90.3 lbs FACTORED DOWN AT 15-3-4, AND 90.3 lbs FACTORED DOWN AT 17-3-4, AND 107.0 lbs FACTORED DOWN AT 19-3-4 ON TOP CHORD, AND 63.2 lbs FACTORED DOWN AT 1-8-7, 63.2 lbs FACTORED DOWN AT 3-3-4, 63.2 lbs FACTORED DOWN AT 5-3-4, 63.2 lbs FACTORED DOWN AT 7-3-4, 63.2 lbs FACTORED DOWN AT 9-3-4, 63.2 lbs FACTORED DOWN AT 11-3-4, 63.2 lbs FACTORED DOWN AT 13-3-4, 63.2 lbs FACTORED DOWN AT 15-3-4, AND 63.2 lbs FACTORED DOWN AT 17-3-4, AND 72.9 lbs FACTORED DOWN AT 19-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1811	0	1811	0	0	2-0	2-0
M	1899	0	1899	0	0	5-8	2-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1502	810 / 0	358 / 0	0 / 0	0 / 0	334 / 0	0 / 0
M	1567	856 / 0	365 / 0	0 / 0	0 / 0	346 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.13 (1)	10.00	L- C	-237 / 143	0.06 (1)	
B- C	-1856 / 0	-84.3	-84.3	0.22 (1)	4.68	B- L	0 / 1561	0.39 (1)	
C- N	-2584 / 0	-84.3	-84.3	0.77 (1)	3.34	I- G	0 / 2548	0.63 (1)	
N- O	-2584 / 0	-84.3	-84.3	0.77 (1)	3.34	C- K	0 / 1316	0.33 (1)	
O- D	-2584 / 0	-84.3	-84.3	0.77 (1)	3.34	I- F	-1073 / 0	0.26 (1)	
D- P	-2584 / 0	-84.3	-84.3	0.82 (1)	3.21	K- D	-762 / 0	0.18 (1)	
P- Q	-2584 / 0	-84.3	-84.3	0.82 (1)	3.21	K- F	0 / 510	0.13 (1)	
Q- E	-2584 / 0	-84.3	-84.3	0.82 (1)	3.21				
E- F	-2584 / 0	-84.3	-84.3	0.82 (1)	3.21				
F- R	-2156 / 0	-84.3	-84.3	0.78 (1)	3.56				
R- S	-2156 / 0	-84.3	-84.3	0.78 (1)	3.56				
S- T	-2156 / 0	-84.3	-84.3	0.78 (1)	3.56				
T- G	-2156 / 0	-84.3	-84.3	0.78 (1)	3.56				
H- G	-1685 / 0	0.0	0.0	0.38 (1)	6.36				
M- B	-1866 / 0	0.0	0.0	0.14 (1)	7.29				
M- U	0 / 0	-28.0	-28.0	0.12 (2)	10.00				
U- L	0 / 0	-28.0	-28.0	0.12 (2)	10.00				
L- V	0 / 1474	-28.0	-28.0	0.27 (1)	10.00				
V- W	0 / 1474	-28.0	-28.0	0.27 (1)	10.00				
W- K	0 / 1474	-28.0	-28.0	0.27 (1)	10.00				
K- X	0 / 2156	-28.0	-28.0	0.38 (1)	10.00				
X- J	0 / 2156	-28.0	-28.0	0.38 (1)	10.00				
J- Y	0 / 2156	-28.0	-28.0	0.38 (1)	10.00				
Y- I	0 / 2156	-28.0	-28.0	0.38 (1)	10.00				
I- Z	0 / 0	-28.0	-28.0	0.17 (2)	10.00				
Z-AA	0 / 0	-28.0	-28.0	0.17 (2)	10.00				
AA-AB	0 / 0	-28.0	-28.0	0.17 (2)	10.00				
AB- H	0 / 0	-28.0	-28.0	0.17 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-2-8	-257	-257	---	FRONT	VERT	TOTAL
E	13-3-4	-90	-90	---	FRONT	VERT	TOTAL
J	11-3-4	-36	-63	---	FRONT	VERT	TOTAL
L	3-3-4	-36	-63	---	FRONT	VERT	TOTAL
N	5-3-4	-90	-90	---	FRONT	VERT	TOTAL
O	7-3-4	-90	-90	---	FRONT	VERT	TOTAL
P	9-3-4	-90	-90	---	FRONT	VERT	TOTAL
Q	11-3-4	-90	-90	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.82 (D-F:1) , BC=0.38 (I-K:1) , WB=0.63 (G-I:1) , SSI=0.37 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.54 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5/27/16
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM-267490-16

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T35	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	15-3-4	-90	-90	---	FRONT	VERT	TOTAL
S	17-3-4	-90	-90	---	FRONT	VERT	TOTAL
T	19-3-4	-107	-107	---	FRONT	VERT	TOTAL
U	1-8-7	-36	-63	---	FRONT	VERT	TOTAL
V	5-3-4	-36	-63	---	FRONT	VERT	TOTAL
W	7-3-4	-36	-63	---	FRONT	VERT	TOTAL
X	9-3-4	-36	-63	---	FRONT	VERT	TOTAL
Y	13-3-4	-36	-63	---	FRONT	VERT	TOTAL
Z	15-3-4	-36	-63	---	FRONT	VERT	TOTAL
AA	17-3-4	-36	-63	---	FRONT	VERT	TOTAL
AB	19-3-4	-42	-73	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 2444216
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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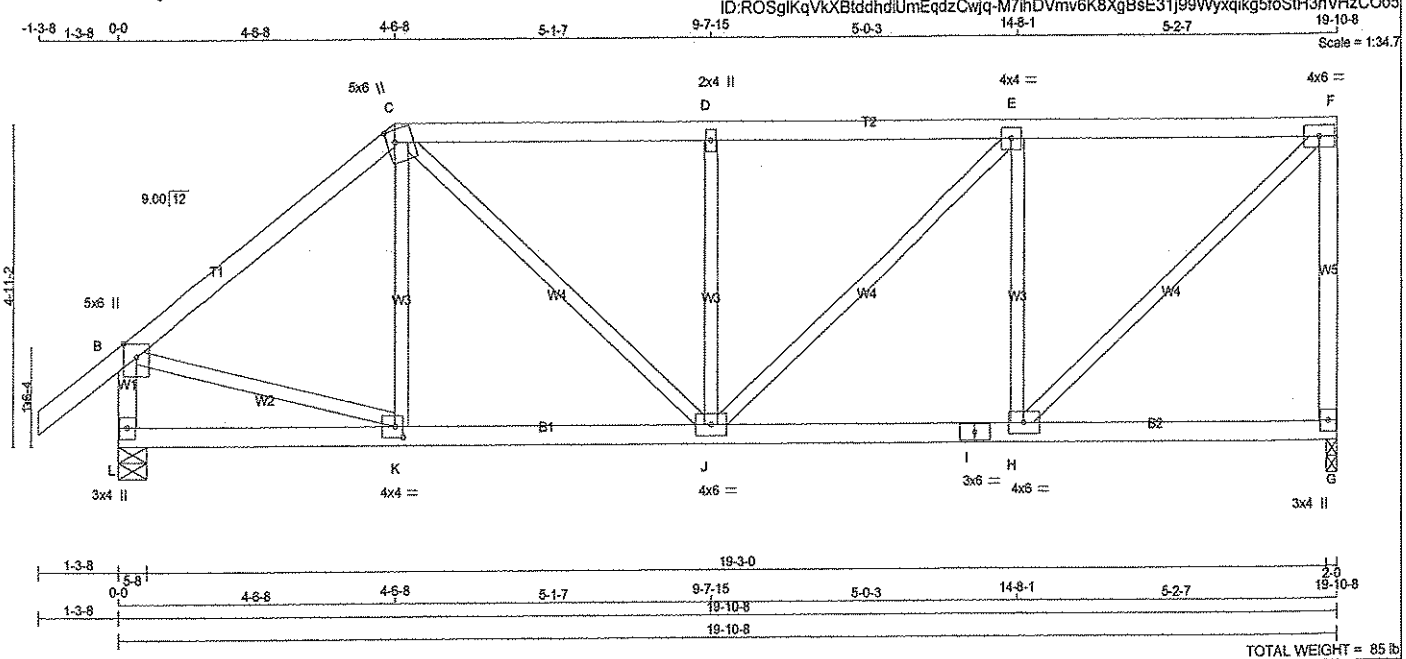

TOTAL WEIGHT = 85 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1116	0	1116	0	0	2-0	1-8
L	1232	0	1232	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00		K- C	-67 / 152	0.03 (3)
B- C	-1082 / 0	-84.3	-84.3	0.34 (1)	5.63		B- K	0 / 892	0.20 (1)
C- D	-1248 / 0	-84.3	-84.3	0.32 (1)	5.36		H- F	0 / 1321	0.30 (1)
D- E	-1248 / 0	-84.3	-84.3	0.33 (1)	5.34		C- J	0 / 519	0.12 (1)
E- F	-986 / 0	-84.3	-84.3	0.32 (1)	5.85		H- E	-716 / 0	0.26 (1)
G- F	-1057 / 0	0.0	0.0	0.43 (1)	7.66		J- D	-460 / 0	0.16 (1)
L- B	-1181 / 0	0.0	0.0	0.12 (1)	7.34		J- E	0 / 357	0.08 (1)
L- K	0 / 0	-28.0	-28.0	0.16 (3)	10.00				
K- J	0 / 863	-28.0	-28.0	0.25 (2)	10.00				
J- I	0 / 986	-28.0	-28.0	0.30 (2)	10.00				
I- H	0 / 986	-28.0	-28.0	0.30 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.18 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.43 (F-G:1) , BC=0.30 (H-J:2) , WB=0.30 (F-H:1) , SSI=0.20 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)

DWG NO. TAM 24443-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T37	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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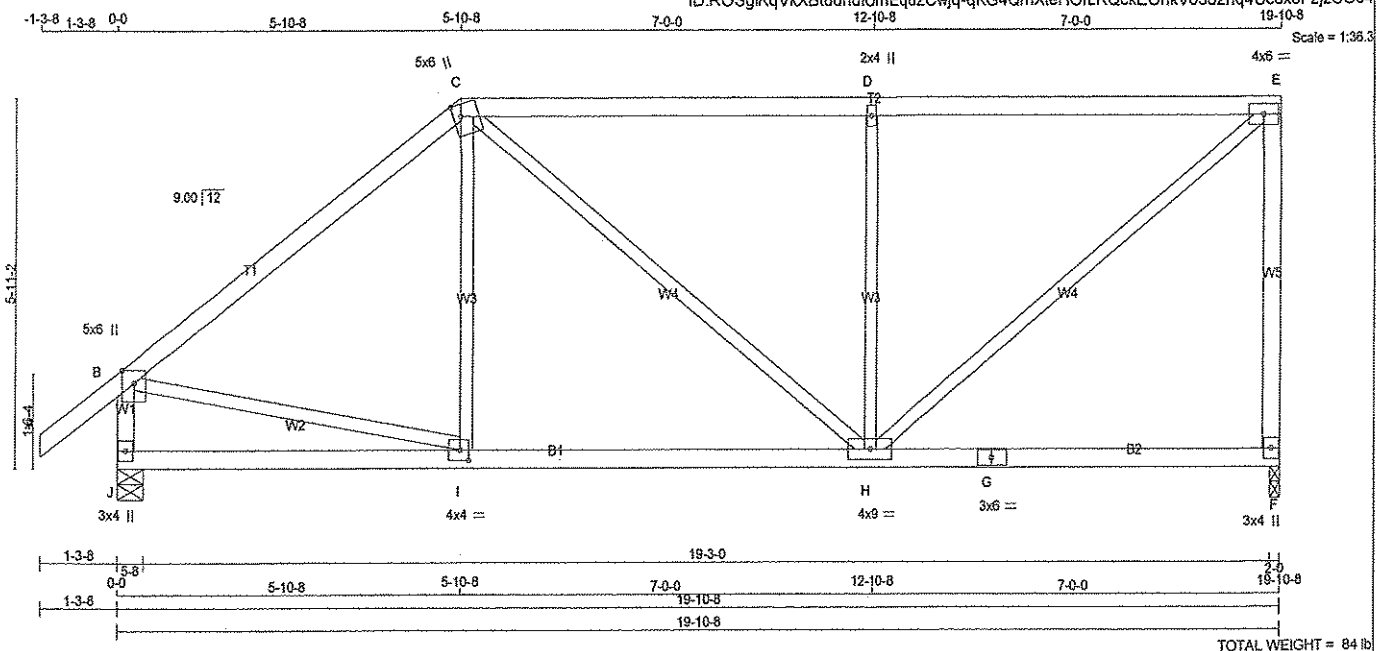


TOTAL WEIGHT = 84 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BS-t	MT20	3.0	6.0		
H	SMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1116	0	1116	0	0	2-0	1-8
J	1232	0	1232	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
J	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	
B- C	-1054 / 0	-84.3	-84.3	0.58 (1)	5.29	
C- D	-1013 / 0	-84.3	-84.3	0.78 (1)	4.82	
D- E	-1013 / 0	-84.3	-84.3	0.78 (1)	4.82	
F- E	-1036 / 0	0.0	0.0	0.69 (1)	7.71	
J- B	-1166 / 0	0.0	0.0	0.12 (1)	7.39	
J- I	0 / 0	-28.0	-28.0	0.25 (3)	10.00	
I- H	0 / 842	-28.0	-28.0	0.43 (2)	10.00	
H- G	0 / 0	-28.0	-28.0	0.33 (3)	10.00	
G- F	0 / 0	-28.0	-28.0	0.33 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 248	0.06 (3)
C- H	0 / 219	0.05 (1)
H- D	-731 / 0	0.40 (1)
H- E	0 / 1297	0.29 (1)
B- I	0 / 859	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.78 (D-E:1) , BC=0.43 (H-I:2) , WB=0.40 (D-H:1) , SSI=0.29 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

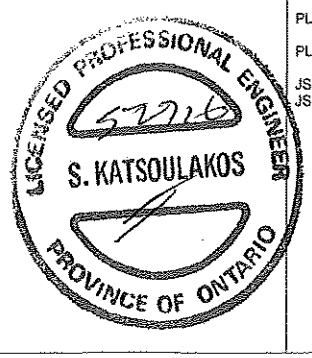
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (I) (INPUT = 0.90)
JSI METAL= 0.32 (I) (INPUT = 1.00)



DWG NO. TAM 24/4/416
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T38	1	1	TRUSS DESC.	

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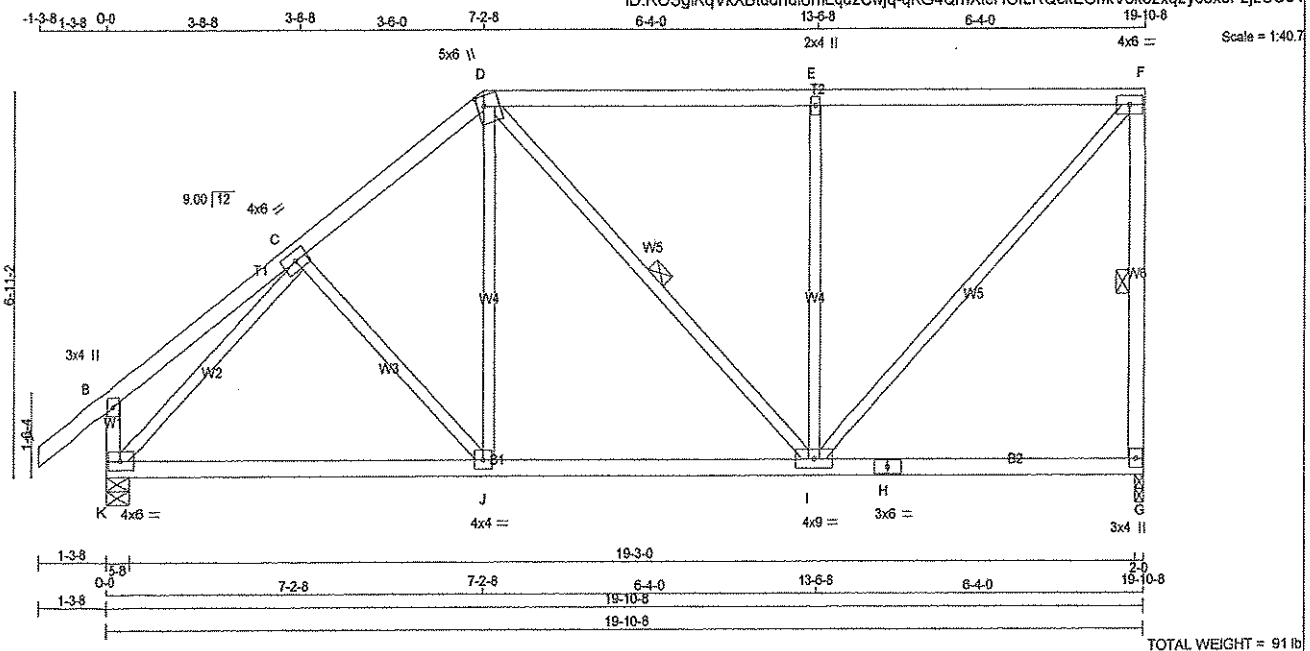

TOTAL WEIGHT = 91 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (teble is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1116	0	1116	0	0	2-0	1-8
K	1232	0	1232	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
K	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 20	-84.3	-84.3	0.17 (1)	10.00
C- D	-1013 / 0	-84.3	-84.3	0.18 (1)	5.99
D- E	-804 / 0	-84.3	-84.3	0.61 (1)	5.71
E- F	-805 / 0	-84.3	-84.3	0.61 (1)	5.71
G- F	-1042 / 0	0.0	0.0	0.23 (1)	6.16
K- B	-233 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 845	-28.0	-28.0	0.42 (2)	10.00
J- I	0 / 796	-28.0	-28.0	0.42 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.24 (3)	10.00
H- G	0 / 0	-28.0	-28.0	0.24 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-76 / 74	0.04 (1)
J- D	0 / 347	0.08 (2)
D- I	-44 / 12	0.02 (3)
I- E	-661 / 0	0.54 (1)
I- F	0 / 1161	0.26 (1)
K- C	-1245 / 0	0.56 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.61 (E-F:1) , BC=0.42 (I-J:2) , WB=0.56 (C-K:1) , SSI=0.26 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

DWG NO. TAM24445-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T39	1	1	TRUSS DESC.	

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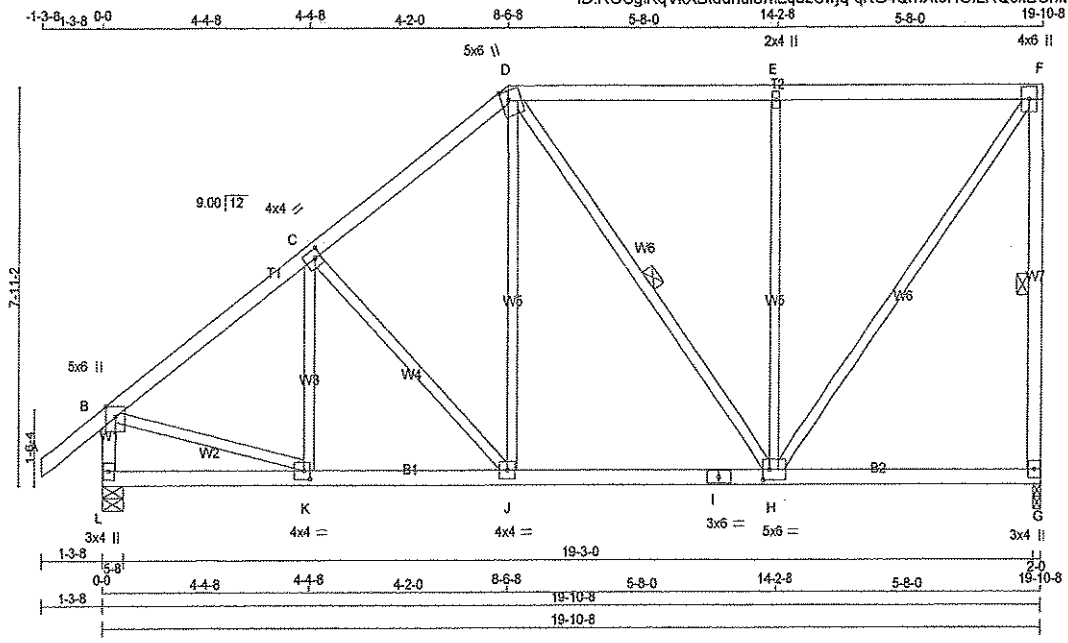

TOTAL WEIGHT = 98 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1116	0	1116	0	0	2-0	1-8
L	1232	0	1232	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	K- C	-116 / 110	0.04 (1)
B- C	-1101 / 0	-84.3	-84.3	0.28 (1)	5.68	C- J	-267 / 0	0.18 (1)
C- D	-924 / 0	-84.3	-84.3	0.27 (1)	6.07	J- D	0 / 357	0.08 (2)
D- E	-649 / 0	-84.3	-84.3	0.47 (1)	6.25	D- H	-121 / 0	0.07 (1)
E- F	-649 / 0	-84.3	-84.3	0.47 (1)	6.25	H- E	-591 / 0	0.70 (1)
G- F	-1052 / 0	0.0	0.0	0.30 (1)	6.14	H- F	0 / 1084	0.24 (1)
L- B	-1180 / 0	0.0	0.0	0.12 (1)	7.35	B- K	0 / 934	0.21 (1)
L- K	0 / 0	-28.0	-28.0	0.12 (3)	10.00			
K- J	0 / 901	-28.0	-28.0	0.22 (2)	10.00			
J- I	0 / 721	-28.0	-28.0	0.31 (2)	10.00			
I- H	0 / 721	-28.0	-28.0	0.31 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.47 (E-F:1) , BC=0.31 (H-J:2) , WB=0.70 (E-H:1) , SSI=0.23 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

DWG NO. TAM 2444616
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T41A	1	2	TRUSS DESC.	

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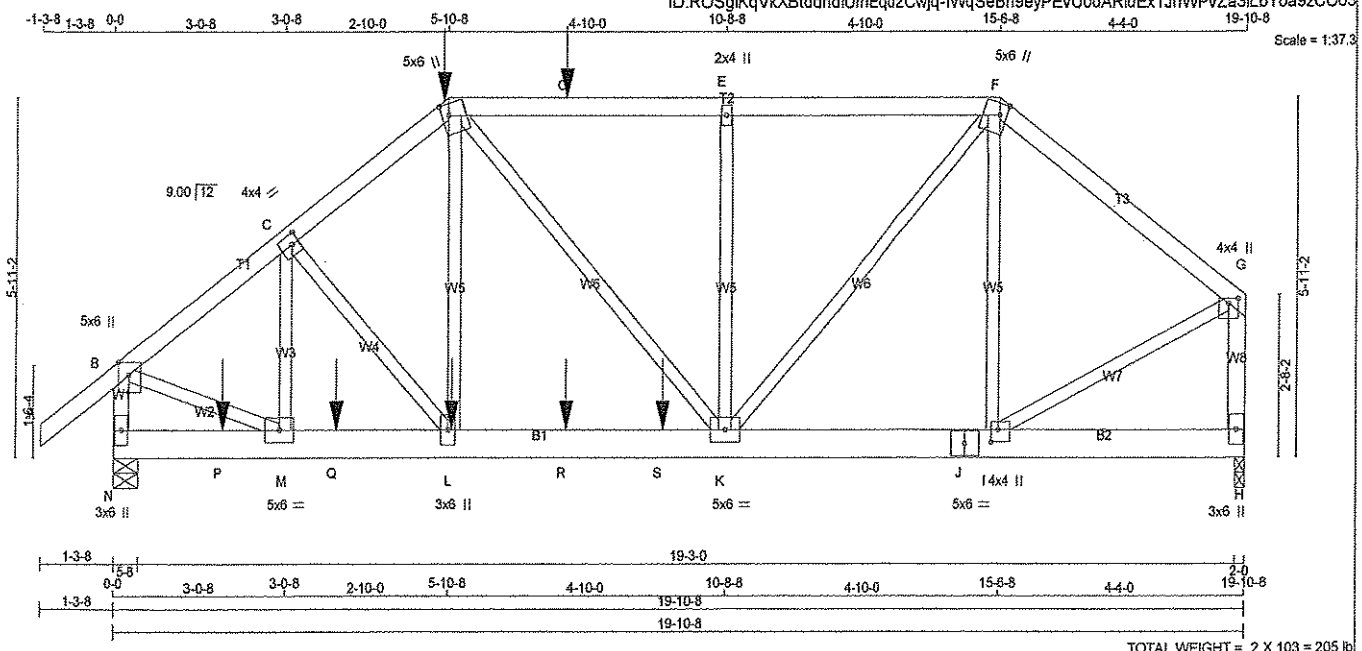

TOTAL WEIGHT = 2 X 103 = 205 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- F	1	12	SIDE(61.0)
F- G	1	12	TOP
N- B	1	12	TOP
H- G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- J	2	12	SIDE(183.1)
J- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
L- D	1	6	SIDE(19.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	4.0	4.0	2.50	1.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2411	0	2411	0	0	5-8	1-8
H	1907	0	1907	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1957	1122 / 0	421 / 0	0 / 0	0 / 0	414 / 0	0 / 0
H	1552	883 / 0	339 / 0	0 / 0	0 / 0	330 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 35	-84.3	-84.3	0.06 (1)	10.00	M- C	-698 / 0	0.03 (1)
B- C	-2341 / 0	-84.3	-84.3	0.07 (1)	5.80	C- L	0 / 313	0.04 (1)
C- D	-2633 / 0	-84.3	-84.3	0.08 (1)	5.54	L- D	0 / 493	0.06 (2)
D- O	-2408 / 0	-84.3	-84.3	0.27 (1)	5.48	D- K	0 / 477	0.06 (1)
O- E	-2408 / 0	-84.3	-84.3	0.27 (1)	5.48	K- E	-578 / 0	0.15 (1)
E- F	-2408 / 0	-84.3	-84.3	0.25 (1)	5.51	K- F	0 / 1682	0.21 (1)
F- G	-1645 / 0	-84.3	-84.3	0.17 (1)	6.25	I- F	-612 / 0	0.16 (1)
N- B	-2322 / 0	0.0	0.0	0.13 (1)	7.35	B- M	0 / 2005	0.25 (1)
H- G	-1844 / 0	0.0	0.0	0.13 (1)	7.81	I- G	0 / 1475	0.18 (1)
N- P	0 / 0	-28.0	-28.0	0.05 (2)	10.00			
P- M	0 / 0	-28.0	-28.0	0.05 (2)	10.00			
M- Q	0 / 1884	-28.0	-28.0	0.23 (1)	10.00			
Q- L	0 / 1884	-28.0	-28.0	0.23 (1)	10.00			
L- R	0 / 2095	-28.0	-28.0	0.37 (1)	10.00			
R- S	0 / 2095	-28.0	-28.0	0.37 (1)	10.00			
S- K	0 / 2095	-28.0	-28.0	0.37 (1)	10.00			
K- J	0 / 1302	-28.0	-28.0	0.20 (1)	10.00			
J- I	0 / 1302	-28.0	-28.0	0.20 (1)	10.00			
I- H	0 / 0	-28.0	-28.0	0.04 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-540	-540	---	BACK	VERT	TOTAL
L	5-11-4	-50	-88	---	BACK	VERT	TOTAL
O	7-11-4	-163	-163	---	BACK	VERT	TOTAL
P	1-11-4	-50	-88	---	BACK	VERT	TOTAL
Q	3-11-4	-50	-88	---	BACK	VERT	TOTAL
R	7-11-4	-50	-88	---	BACK	VERT	TOTAL
S	9-7-8	-1065	-1065	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.27 (D-E:1) , BC=0.37 (K-L:1) , WB=0.25 (B-M:1) , SSI=0.30 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LUMBER = 0.90 (I) (INPUT = 0.90)
PLATE = 0.24 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5776
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM-2447216
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T41A	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 540.3 lbs FACTORED DOWN AT 5-10-8, AND 183.3 lbs FACTORED DOWN AT 7-11-4 ON TOP CHORD, AND 88.2 lbs FACTORED DOWN AT 1-11-4, 88.2 lbs FACTORED DOWN AT 3-11-4, 88.2 lbs FACTORED DOWN AT 5-11-4, AND 88.2 lbs FACTORED DOWN AT 7-11-4, AND 1065.2 lbs FACTORED DOWN AT 9-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

R2/2

DWG NO. TAM2447216
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T42A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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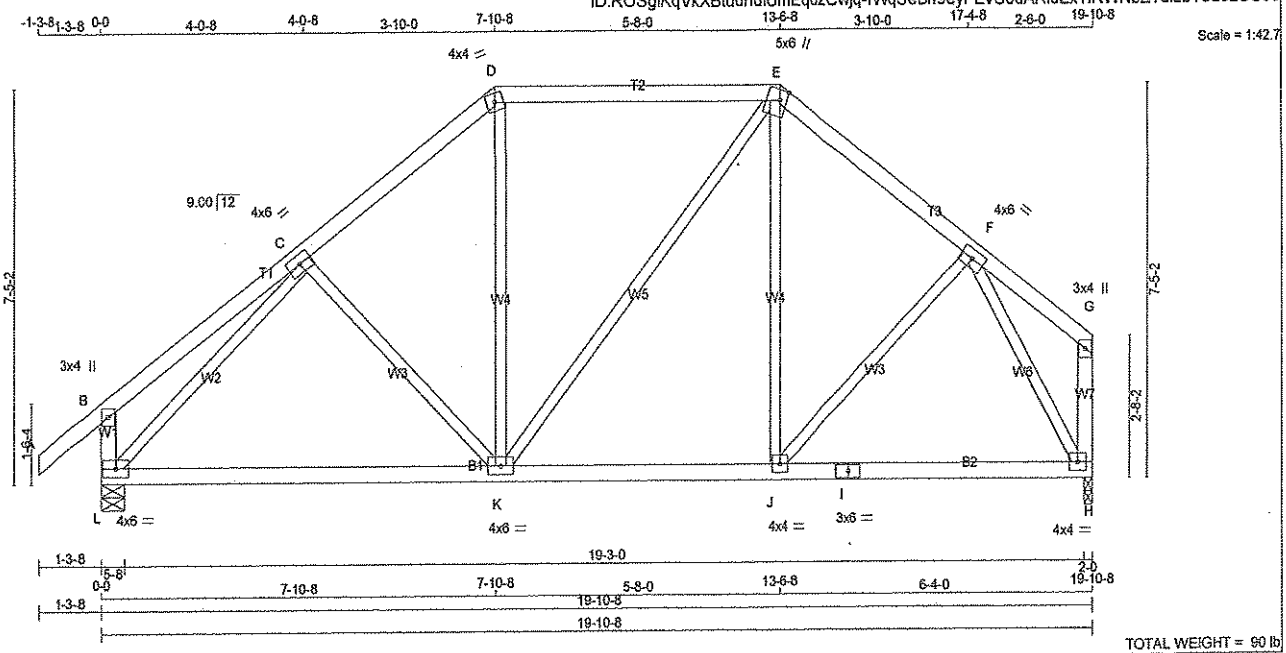

TOTAL WEIGHT = 90 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.75
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1232	0	1232	0	0	5-8	1-8
H	1116	0	1116	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0
H	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 23	-84.3	-84.3	0.21 (1)	10.00	
C- D	-979 / 0	-84.3	-84.3	0.17 (1)	6.11	
D- E	-766 / 0	-84.3	-84.3	0.35 (1)	6.25	
E- F	-880 / 0	-84.3	-84.3	0.16 (1)	6.25	
F- G	0 / 28	-84.3	-84.3	0.16 (1)	10.00	
L- B	-243 / 0	0.0	0.0	0.03 (1)	7.81	
H- G	-54 / 0	0.0	0.0	0.01 (1)	7.81	
L- K	0 / 850	-28.0	-28.0	0.46 (2)	10.00	
K- J	0 / 690	-28.0	-28.0	0.45 (2)	10.00	
J- I	0 / 569	-28.0	-28.0	0.31 (2)	10.00	
I- H	0 / 569	-28.0	-28.0	0.31 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-123 / 62	0.07 (1)
K- D	0 / 292	0.07 (2)
K- E	0 / 125	0.03 (1)
J- E	0 / 173	0.04 (3)
J- F	0 / 193	0.04 (2)
L- C	-1235 / 0	0.66 (1)
F- H	-1137 / 0	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 879 (0.27")

CSI: TC=0.35 (D-E:1) , BC=0.46 (K-L:2) , WB=0.66 (C-L:1) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO . TAM2/44B16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T43A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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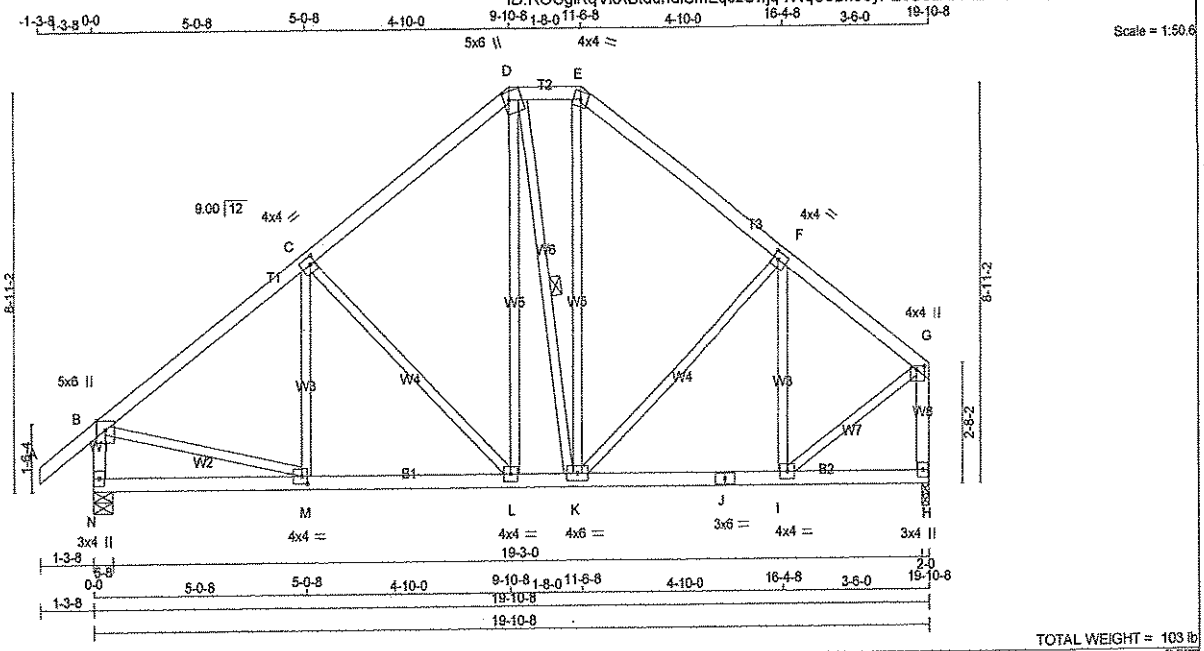

TOTAL WEIGHT = 103 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMWW+p	MT20	4.0	4.0	1.00	2.25
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1232	0	1232	0	0	5-8	1-8
H	1116	0	1116	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0
H	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)		10.00
B- C	-1106 / 0	-84.3	-84.3	0.28 (1)		5.69
C- D	-835 / 0	-84.3	-84.3	0.27 (1)		6.25
D- E	-634 / 0	-84.3	-84.3	0.03 (1)		6.25
E- F	-814 / 0	-84.3	-84.3	0.23 (1)		6.25
F- G	-835 / 0	-84.3	-84.3	0.21 (1)		6.25
N- B	-1175 / 0	0.0	0.0	0.12 (1)		7.35
H- G	-1079 / 0	0.0	0.0	0.15 (1)		7.60
N- M	0 / 0	-28.0	-28.0	0.18 (3)		10.00
M- L	0 / 908	-28.0	-28.0	0.29 (2)		10.00
L- K	0 / 646	-28.0	-28.0	0.19 (2)		10.00
K- J	0 / 694	-28.0	-28.0	0.20 (2)		10.00
J- I	0 / 694	-28.0	-28.0	0.20 (2)		10.00
I- H	0 / 0	-28.0	-28.0	0.12 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-51 / 160	0.04 (3)
C- L	-378 / 0	0.35 (1)
L- D	0 / 325	0.07 (1)
D- K	-65 / 0	0.03 (1)
K- E	0 / 259	0.06 (2)
K- F	-89 / 0	0.08 (1)
I- F	-337 / 1	0.14 (1)
B- M	0 / 933	0.21 (1)
I- G	0 / 836	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.28 (B-C:1) , BC=0.29 (L-M:2) , WB=0.35 (C-L:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.34 (M) (INPUT = 1.00)

DWG NO . TAM24449-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T44A	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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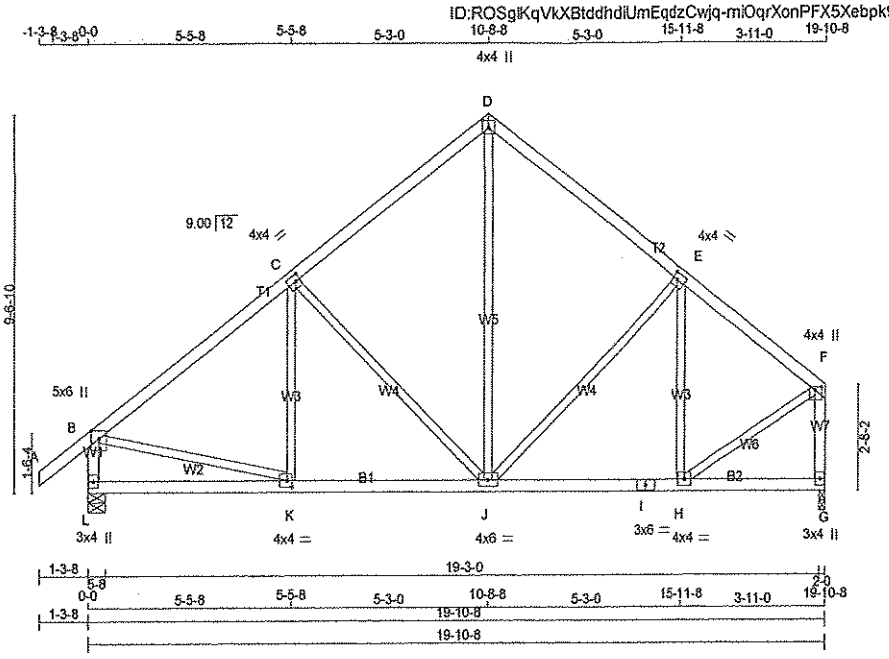

TOTAL WEIGHT = 5 X 92 = 459 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1232	0	1232	0	0	5-8	1-8
G	1116	0	1116	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	K- C	-31 / 197	0.04 (3)	
B- C	-1099 / 0	-84.3	-84.3	0.34 (1)	5.63	C- J	-427 / 0	0.47 (1)	
C- D	-788 / 0	-84.3	-84.3	0.32 (1)	6.25	J- D	0 / 562	0.13 (1)	
D- E	-781 / 0	-84.3	-84.3	0.28 (1)	6.25	J- E	-160 / 0	0.18 (1)	
E- F	-881 / 0	-84.3	-84.3	0.25 (1)	6.25	H- E	-289 / 36	0.14 (1)	
L- B	-1169 / 0	0.0	0.0	0.12 (1)	7.37	B- K	0 / 926	0.21 (1)	
G- F	-1075 / 0	0.0	0.0	0.15 (1)	7.61	H- F	0 / 836	0.19 (1)	
L- K	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
K- J	0 / 905	-28.0	-28.0	0.30 (2)	10.00				
J- I	0 / 717	-28.0	-28.0	0.23 (2)	10.00				
I- H	0 / 717	-28.0	-28.0	0.23 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.14 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.34 (B-C:1) , BC=0.30 (J-K:2) , WB=0.47 (C-J:1) , SSI=0.18 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (H) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

DWG NO. TAM2445016
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267493	T44S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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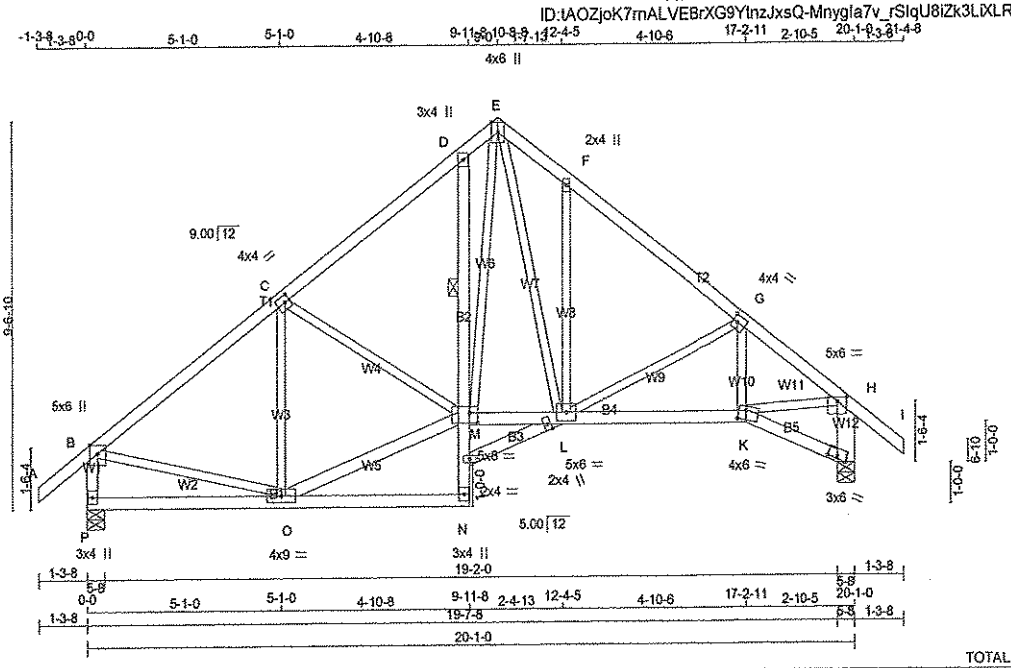


TOTAL WEIGHT = 2 X 113 = 227 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - D	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.25	3.00
J	BVM1-I	MT20	3.0	6.0	0.75	3.00
K	BBWW-m	MT20	4.0	6.0	2.75	2.00
L	BMWWW-t	MT20	5.0	6.0		
L	NP+h	MT20	2.0	4.0	Edge	
M	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
N	BMV+p	MT20	3.0	4.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMV1+p	MT20	3.0	4.0		
Q	NP-p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1244	0	1244	0	0	5-8	1-8
J	1244	0	1244	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1005	585 / 0	211 / 0	0 / 0	0 / 0	209 / 0	0 / 0
J	1005	585 / 0	211 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-M.

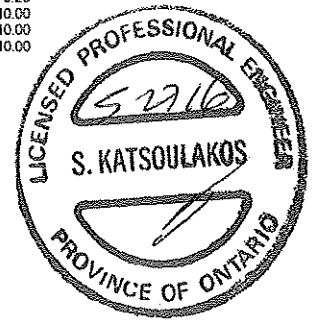
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	
B- C	-1124 / 0	-84.3	-84.3	0.28 (1)	5.67	
C- D	-1113 / 0	-84.3	-84.3	0.26 (1)	5.71	
D- E	-1124 / 0	-84.3	-84.3	0.07 (1)	5.91	
E- F	-1192 / 0	-84.3	-84.3	0.12 (1)	5.73	
F- G	-1167 / 0	-84.3	-84.3	0.17 (1)	5.73	
G- H	-1514 / 0	-84.3	-84.3	0.15 (1)	5.20	
H- I	0 / 35	-84.3	-84.3	0.11 (1)	10.00	
P- B	-1185 / 0	0.0	0.0	0.12 (1)	7.34	
J- H	-1204 / 0	0.0	0.0	0.09 (1)	7.81	
P- O	0 / 0	-28.0	-28.0	0.20 (3)	10.00	
O- N	0 / 5	-28.0	-28.0	0.20 (3)	10.00	
N- M	0 / 80	0.0	0.0	0.02 (2)	10.00	
M- D	-258 / 0	0.0	0.0	0.02 (1)	6.25	
M- L	0 / 780	-28.0	-28.0	0.23 (2)	10.00	
L- K	0 / 1230	-28.0	-28.0	0.28 (2)	10.00	
K- J	0 / 0	-28.0	-28.0	0.07 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-435 / 0	0.18 (1)
O- M	0 / 983	0.16 (1)
C- M	-39 / 0	0.02 (2)
L- F	-318 / 0	0.20 (1)
L- G	-336 / 0	0.15 (1)
K- G	0 / 127	0.03 (3)
B- O	0 / 944	0.21 (1)
K- H	0 / 1234	0.28 (1)
M- E	0 / 743	0.17 (1)
E- L	0 / 624	0.14 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.28 (B-C:1) , BC=0.28 (K-L:2) , WB=0.28 (H-K:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.33 (H) (INPUT = 1.00)

DRWG NO. TAM 24462 16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267493	T44SA	1	1	TRUSS DESC.	

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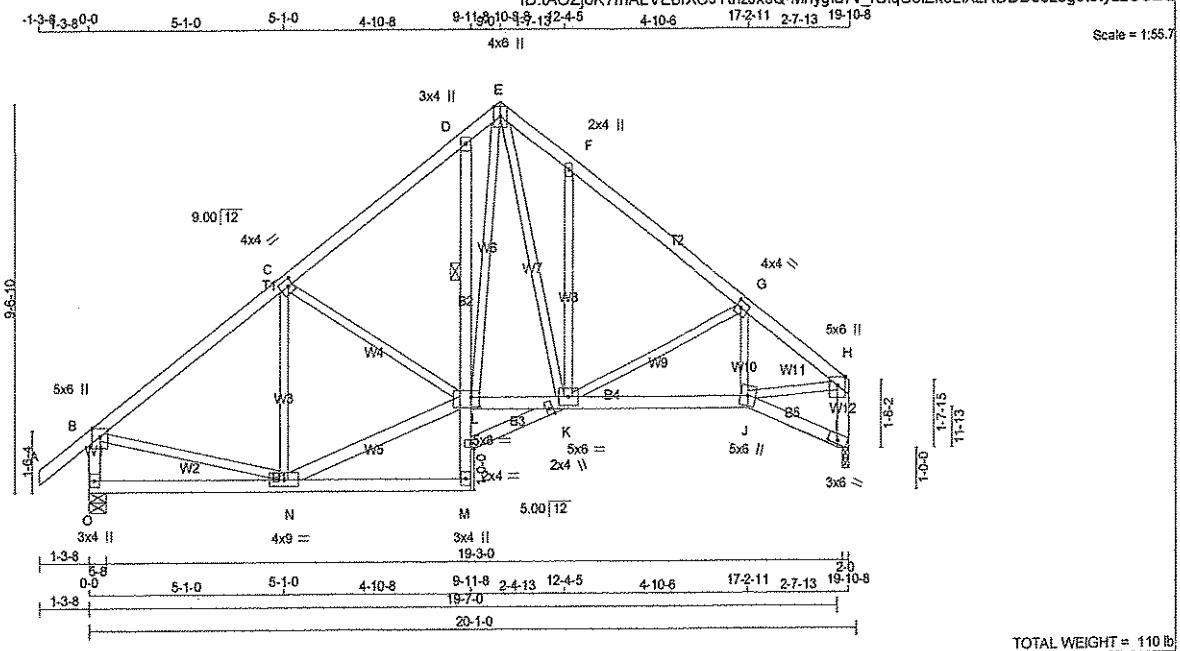


TOTAL WEIGHT = 110 lb

[M|F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - D	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	
I	BVM1-I	MT20	3.0	6.0	1.00	3.00
J	BBWW+m	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
K	NP+h	MT20	2.0	4.0	Edge	
L	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
M	BMV+p	MT20	3.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMV1+p	MT20	3.0	4.0		
P	NP-p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED DOWN	MAXIMUM FACTORED HORZ	MAXIMUM FACTORED UPLIFT	INPUT BRG	REQRD BRG
O	1232	0	1232	0	0	5-8	1-8
I	1116	0	1116	0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	995	579 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0
I	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

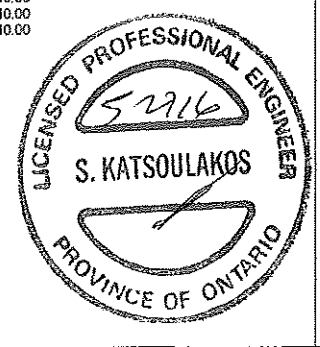
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	
B- C	-1109 / 0	-84.3	-84.3	0.27 (1)	5.70	
C- D	-1091 / 0	-84.3	-84.3	0.26 (1)	5.75	
D- E	-1103 / 0	-84.3	-84.3	0.07 (1)	5.96	
E- F	-1162 / 0	-84.3	-84.3	0.12 (1)	5.79	
F- G	-1136 / 0	-84.3	-84.3	0.17 (1)	5.79	
G- H	-1404 / 0	-84.3	-84.3	0.15 (1)	5.36	
O- B	-1173 / 0	0.0	0.0	0.12 (1)	7.36	
I- H	-1079 / 0	0.0	0.0	0.12 (1)	7.60	
O- N	0 / 0	-28.0	-28.0	0.20 (3)	10.00	
N- M	0 / 5	-28.0	-28.0	0.20 (3)	10.00	
M- L	0 / 80	0.0	0.0	0.02 (2)	10.00	
L- D	-258 / 0	0.0	0.0	0.02 (1)	6.25	
L- K	0 / 762	-28.0	-28.0	0.23 (2)	10.00	
K- J	0 / 1146	-28.0	-28.0	0.27 (2)	10.00	
J- I	0 / 0	-28.0	-28.0	0.06 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
N- C	-428 / 0	0.18 (1)
N- L	0 / 970	0.16 (1)
C- L	-44 / 0	0.02 (1)
K- F	-320 / 0	0.20 (1)
K- G	-269 / 0	0.12 (1)
J- G	-54 / 89	0.02 (3)
B- N	0 / 932	0.21 (1)
J- H	0 / 1153	0.26 (1)
L- E	0 / 742	0.17 (1)
E- K	0 / 594	0.13 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.27 (B-C:1) , BC=0.27 (J-K:2) , WB=0.26 (H-J:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.29 (J) (INPUT = 1.00)

DWG NO . TAM 24468 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	T45	1	1	TRUSS DESC.	

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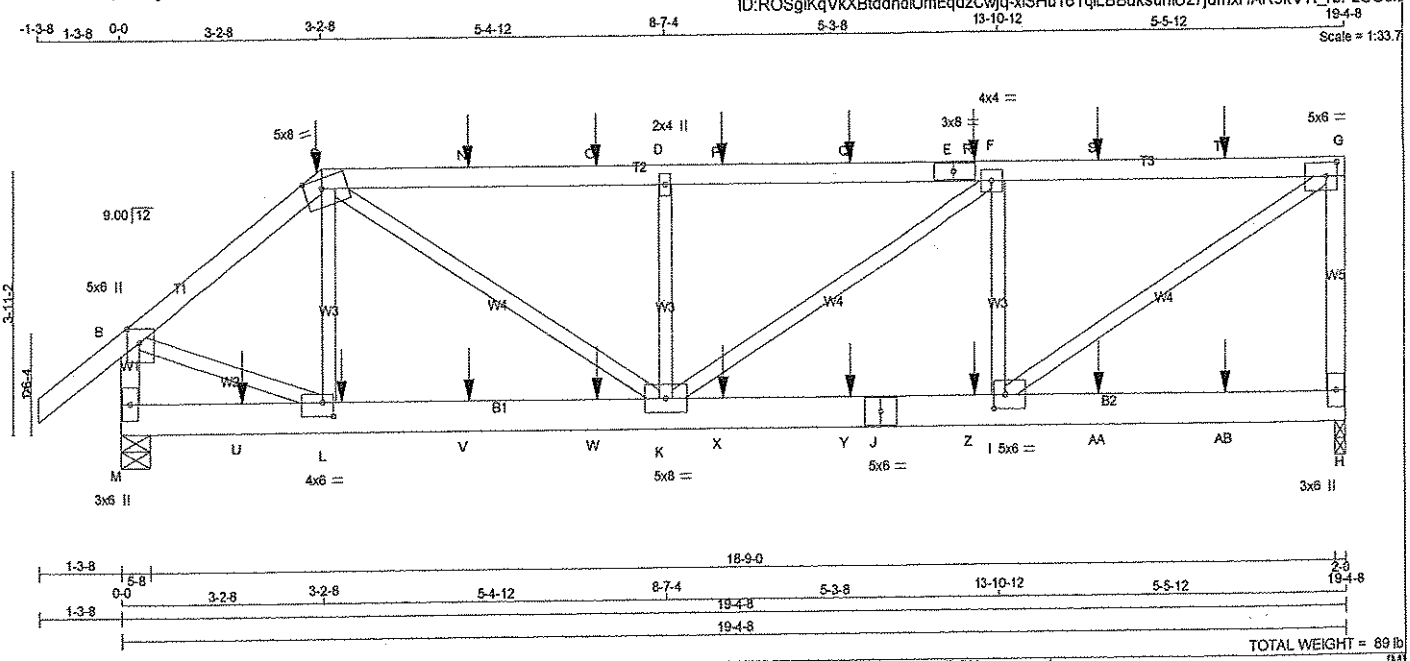

TOTAL WEIGHT = 89 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.50	2.25
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW-t	MT20	5.0	6.0	2.50	2.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.00
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	4.0	6.0	2.50	2.00
M	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 149.8 lbs FACTORED DOWN AT 3-2-8, 92.9 lbs FACTORED DOWN AT 3-2-8, 90.3 lbs FACTORED DOWN AT 5-6-1, 90.3 lbs FACTORED DOWN AT 7-6-1, 90.3 lbs FACTORED DOWN AT 9-6-1, 90.3 lbs FACTORED DOWN AT 11-6-1, 90.3 lbs FACTORED DOWN AT 13-6-1, AND 90.3 lbs FACTORED DOWN AT 15-6-1, AND 90.3 lbs FACTORED DOWN AT 17-6-1 ON TOP CHORD, AND 63.2 lbs FACTORED DOWN AT 1-11-4, 63.2 lbs FACTORED DOWN AT 3-6-1, 63.2 lbs FACTORED DOWN AT 5-6-1, 63.2 lbs FACTORED DOWN AT 7-6-1, 63.2 lbs FACTORED DOWN AT 9-6-1, 63.2 lbs FACTORED DOWN AT 11-6-1, 63.2 lbs FACTORED DOWN AT 13-6-1, AND 63.2 lbs FACTORED DOWN AT 15-6-1, AND 63.2 lbs FACTORED DOWN AT 17-6-1 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1663	0	1663	0	0	2-0	1-15
M	1828	0	1828	0	0	5-8	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1378	744 / 0	328 / 0	0 / 0	0 / 0	306 / 0	0 / 0
M	1508	825 / 0	351 / 0	0 / 0	0 / 0	332 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.13 (1)	10.00	L- C	-224 / 142	0.05 (1)	
B- C	-1774 / 0	-84.3	-84.3	0.22 (1)	4.77	B- L	0 / 1492	0.37 (1)	
C- N	-2437 / 0	-84.3	-84.3	0.70 (1)	3.53	I- G	0 / 2418	0.60 (1)	
N- O	-2437 / 0	-84.3	-84.3	0.70 (1)	3.53	C- K	0 / 1230	0.30 (1)	
O- D	-2437 / 0	-84.3	-84.3	0.70 (1)	3.53	I- F	-1040 / 0	0.25 (1)	
D- P	-2437 / 0	-84.3	-84.3	0.73 (1)	3.47	K- D	-735 / 0	0.18 (1)	
P- Q	-2437 / 0	-84.3	-84.3	0.73 (1)	3.47	K- F	0 / 491	0.12 (1)	
Q- E	-2437 / 0	-84.3	-84.3	0.73 (1)	3.47				
E- R	-2437 / 0	-84.3	-84.3	0.73 (1)	3.47				
R- F	-2437 / 0	-84.3	-84.3	0.73 (1)	3.47				
F- S	-2029 / 0	-84.3	-84.3	0.69 (1)	3.83				
S- T	-2029 / 0	-84.3	-84.3	0.69 (1)	3.83				
T- G	-2029 / 0	-84.3	-84.3	0.69 (1)	3.83				
H- G	-1566 / 0	0.0	0.0	0.36 (1)	6.55				
M- B	-1795 / 0	0.0	0.0	0.20 (1)	6.20				
M- U	0 / 0	-28.0	-28.0	0.11 (2)	10.00				
U- L	0 / 0	-28.0	-28.0	0.11 (2)	10.00				
L- V	0 / 1408	-28.0	-28.0	0.26 (1)	10.00				
V- W	0 / 1408	-28.0	-28.0	0.26 (1)	10.00				
W- K	0 / 1408	-28.0	-28.0	0.26 (1)	10.00				
K- X	0 / 2029	-28.0	-28.0	0.35 (1)	10.00				
X- Y	0 / 2029	-28.0	-28.0	0.35 (1)	10.00				
Y- J	0 / 2029	-28.0	-28.0	0.35 (1)	10.00				
J- Z	0 / 2029	-28.0	-28.0	0.35 (1)	10.00				
Z- I	0 / 2029	-28.0	-28.0	0.35 (1)	10.00				
I-AA	0 / 0	-28.0	-28.0	0.16 (2)	10.00				
AA-AB	0 / 0	-28.0	-28.0	0.16 (2)	10.00				
AB- H	0 / 0	-28.0	-28.0	0.16 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-2-8	-150	-150	---	FRONT	VERT	TOTAL
C	3-2-8	-93	-93	---	BACK	VERT	TOTAL
L	3-6-1	-36	-63	---	BACK	VERT	TOTAL
N	5-6-1	-90	-90	---	BACK	VERT	TOTAL
O	7-6-1	-90	-90	---	BACK	VERT	TOTAL
P	9-6-1	-90	-90	---	BACK	VERT	TOTAL
Q	11-6-1	-90	-90	---	BACK	VERT	TOTAL
R	13-6-1	-90	-90	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.73 (D-F:1) , BC=0.35 (I-K:1) , WB=0.60 (G-I:1) , SSI=0.34 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.51 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 5276 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 24428016

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	T45	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	15-6-1	-90	-90	---	BACK	VERT	TOTAL
T	17-6-1	-90	-90	---	BACK	VERT	TOTAL
U	1-11-4	-36	-63	---	BACK	VERT	TOTAL
V	5-6-1	-36	-63	---	BACK	VERT	TOTAL
W	7-6-1	-36	-63	---	BACK	VERT	TOTAL
X	9-6-1	-36	-63	---	BACK	VERT	TOTAL
Y	11-6-1	-36	-63	---	BACK	VERT	TOTAL
Z	13-6-1	-36	-63	---	BACK	VERT	TOTAL
AA	15-6-1	-36	-63	---	BACK	VERT	TOTAL
AB	17-6-1	-36	-63	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 2442B16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	T45Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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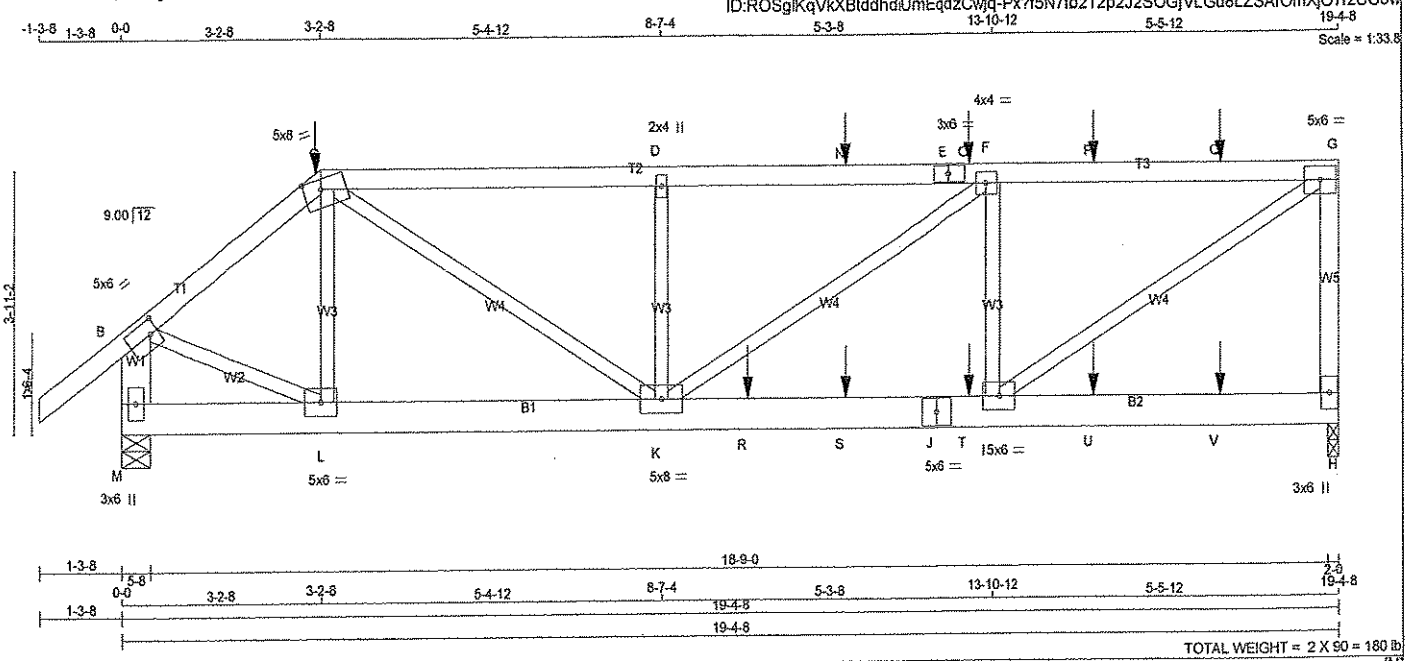

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- G	1	12	SIDE(0.0)
G- H	1	12	TOP
M- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.0)
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.50
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2248	0	2248	0	0	2-0	1-8
M	2181	0	2181	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1823	1049 / 0	390 / 0	0 / 0	0 / 0	383 / 0	0 / 0
M	1747	1040 / 0	351 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 35	-84.3	-84.3	0.06 (1)	10.00		L- C	-580 / 0	0.07 (1)
B- C	-2141 / 0	-84.3	-84.3	0.10 (1)	5.96		B- L	0 / 1801	0.22 (1)
C- D	-3766 / 0	-84.3	-84.3	0.25 (1)	4.64		I- G	0 / 3591	0.44 (1)
D- N	-3766 / 0	-84.3	-84.3	0.37 (1)	4.48		C- K	0 / 2504	0.31 (1)
N- E	-3766 / 0	-84.3	-84.3	0.37 (1)	4.48		I- F	-1294 / 0	0.15 (1)
E- O	-3766 / 0	-84.3	-84.3	0.37 (1)	4.48		K- D	-498 / 0	0.06 (1)
O- F	-3766 / 0	-84.3	-84.3	0.37 (1)	4.48		K- F	0 / 930	0.12 (1)
F- P	-3013 / 0	-84.3	-84.3	0.34 (1)	4.94				
P- Q	-3013 / 0	-84.3	-84.3	0.34 (1)	4.94				
Q- G	-3013 / 0	-84.3	-84.3	0.34 (1)	4.94				
H- G	-2204 / 0	0.0	0.0	0.25 (1)	7.51				
M- B	-2114 / 0	0.0	0.0	0.08 (1)	7.81				
M- L	0 / 0	-28.0	-28.0	0.03 (1)	10.00				
L- K	0 / 1691	-28.0	-28.0	0.30 (1)	10.00				
K- R	0 / 3013	-28.0	-28.0	0.60 (1)	10.00				
R- S	0 / 3013	-28.0	-28.0	0.60 (1)	10.00				
S- J	0 / 3013	-28.0	-28.0	0.60 (1)	10.00				
J- T	0 / 3013	-28.0	-28.0	0.60 (1)	10.00				
T- I	0 / 3013	-28.0	-28.0	0.60 (1)	10.00				
I- U	0 / 0	-28.0	-28.0	0.14 (2)	10.00				
U- V	0 / 0	-28.0	-28.0	0.14 (2)	10.00				
V- H	0 / 0	-28.0	-28.0	0.14 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-2-8	-163	-163	---	FRONT	VERT	TOTAL
N	11-6-1	-90	-90	---	FRONT	VERT	TOTAL
O	13-6-1	-90	-90	---	FRONT	VERT	TOTAL
P	15-6-1	-90	-90	---	FRONT	VERT	TOTAL
Q	17-6-1	-90	-90	---	FRONT	VERT	TOTAL
R	9-11-8	-1468	-1468	---	FRONT	VERT	TOTAL
S	11-6-1	-36	-63	---	FRONT	VERT	TOTAL
T	13-6-1	-36	-63	---	FRONT	VERT	TOTAL
U	15-6-1	-36	-63	---	FRONT	VERT	TOTAL
V	17-6-1	-36	-63	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.37 (D-F:1) , BC=0.60 (I-K:1) , WB=0.44 (G-I:1) , SSI=0.40 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.38 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 52216 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO . TAM 24429-16

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	T45Z	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 163.2 lbs FACTORED DOWN AT 3-2-8, 90.3 lbs FACTORED DOWN AT 11-6-1, 90.3 lbs FACTORED DOWN AT 13-6-1, AND 90.3 lbs FACTORED DOWN AT 15-6-1, AND 90.3 lbs FACTORED DOWN AT 17-6-1 ON TOP CHORD, AND 1468.5 lbs FACTORED DOWN AT 9-11-8, 63.2 lbs FACTORED DOWN AT 11-6-1, 63.2 lbs FACTORED DOWN AT 13-6-1, AND 63.2 lbs FACTORED DOWN AT 15-6-1, AND 63.2 lbs FACTORED DOWN AT 17-6-1 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
52276
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 21429-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	T46	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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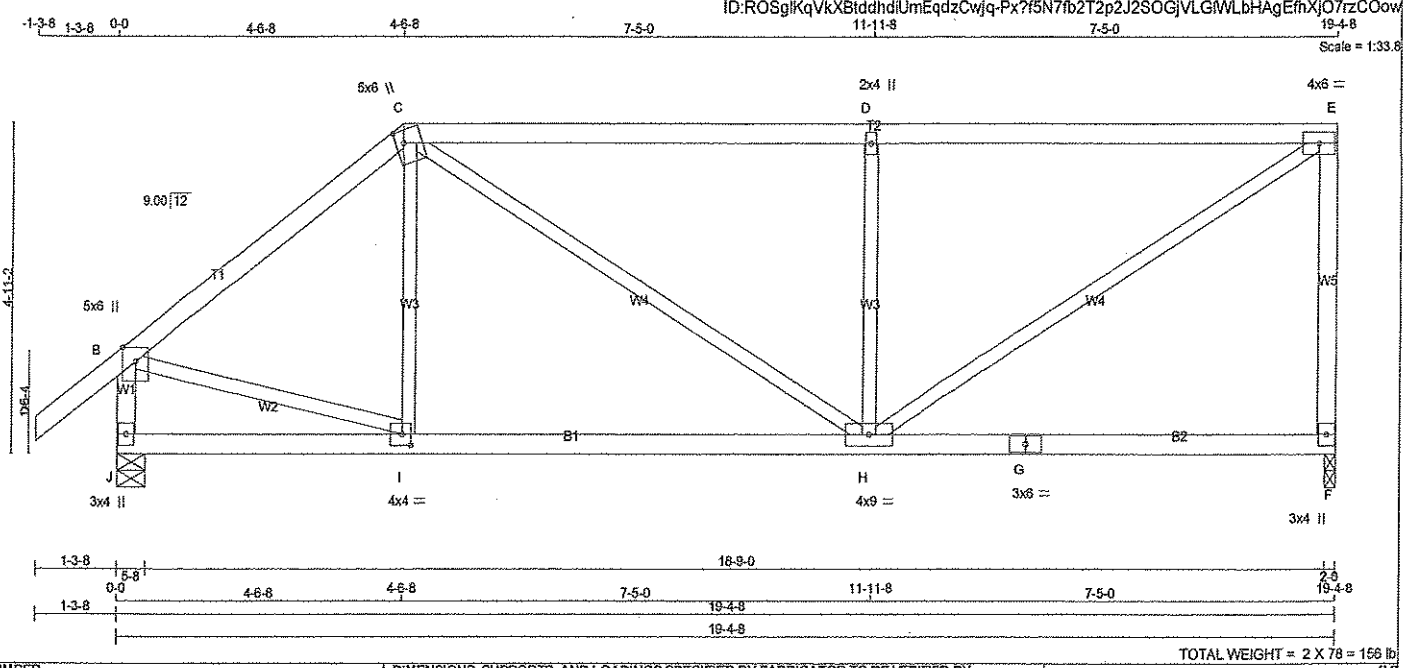

TOTAL WEIGHT = 2 X 78 = 156 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BS-t	MT20	3.0	6.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1088	0	1088	0	0	2-0	1-8
J	1204	0	1204	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	893	496 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0
J	972	566 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	I- C	-38 / 194	0.04 (3)
B- C	-1054 / 0	-84.3	-84.3	0.33 (1)	5.69	C- H	0 / 460	0.10 (1)
C- D	-1231 / 0	-84.3	-84.3	0.91 (1)	4.10	H- D	-774 / 0	0.28 (1)
D- E	-1231 / 0	-84.3	-84.3	0.92 (1)	4.10	H- E	0 / 1449	0.33 (1)
F- E	-1004 / 0	0.0	0.0	0.40 (1)	7.80	B- I	0 / 869	0.20 (1)
J- B	-1158 / 0	0.0	0.0	0.12 (1)	7.40			
J- I	0 / 0	-28.0	-28.0	0.22 (2)	10.00			
I- H	0 / 841	-28.0	-28.0	0.49 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.39 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.39 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.92 (D-E:1) , BC=0.49 (H-I:2) , WB=0.33 (E-H:1) , SSI=0.30 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.32 (I) (INPUT = 1.00)

DWG NO. TAM 24430-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267489	T47	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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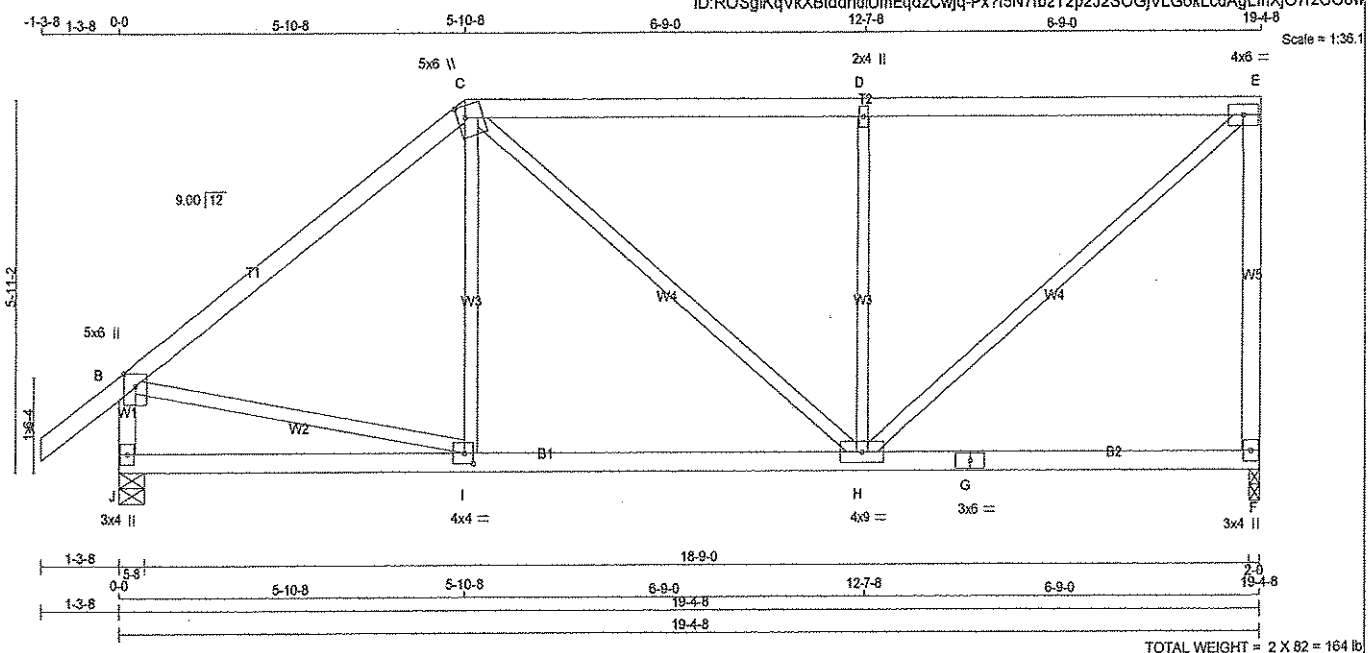

TOTAL WEIGHT = 2 X 82 = 164 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BS-t	MT20	3.0	6.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1088	0	1088	0	0	2-0	1-8
J	1204	0	1204	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	893	496 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0
J	972	586 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.11 (1)
B- C	-1016 / 0	-84.3	-84.3	0.58 (1)
C- D	-956 / 0	-84.3	-84.3	0.71 (1)
D- E	-956 / 0	-84.3	-84.3	0.71 (1)
F- E	-1010 / 0	0.0	0.0	0.67 (1)
J- B	-1137 / 0	0.0	0.0	0.12 (1)
J- I	0 / 0	-28.0	-28.0	0.25 (3)
I- H	0 / 812	-28.0	-28.0	0.40 (2)
H- G	0 / 0	-28.0	-28.0	0.31 (3)
G- F	0 / 0	-28.0	-28.0	0.31 (3)

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- C	0 / 246	0.06 (3)	10.00
C- H	0 / 187	0.04 (1)	5.37
H- D	-705 / 0	0.38 (1)	5.11
H- E	0 / 1242	0.28 (1)	5.11
B- I	0 / 828	0.19 (1)	7.78

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.71 (D-E:1) , BC=0.40 (H-I:2) , WB=0.38 (D-H:1) , SSI=0.28 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (I) (INPUT = 0.90)
JSI METAL= 0.30 (I) (INPUT = 1.00)

DWG NO. TAM2443/-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	T48	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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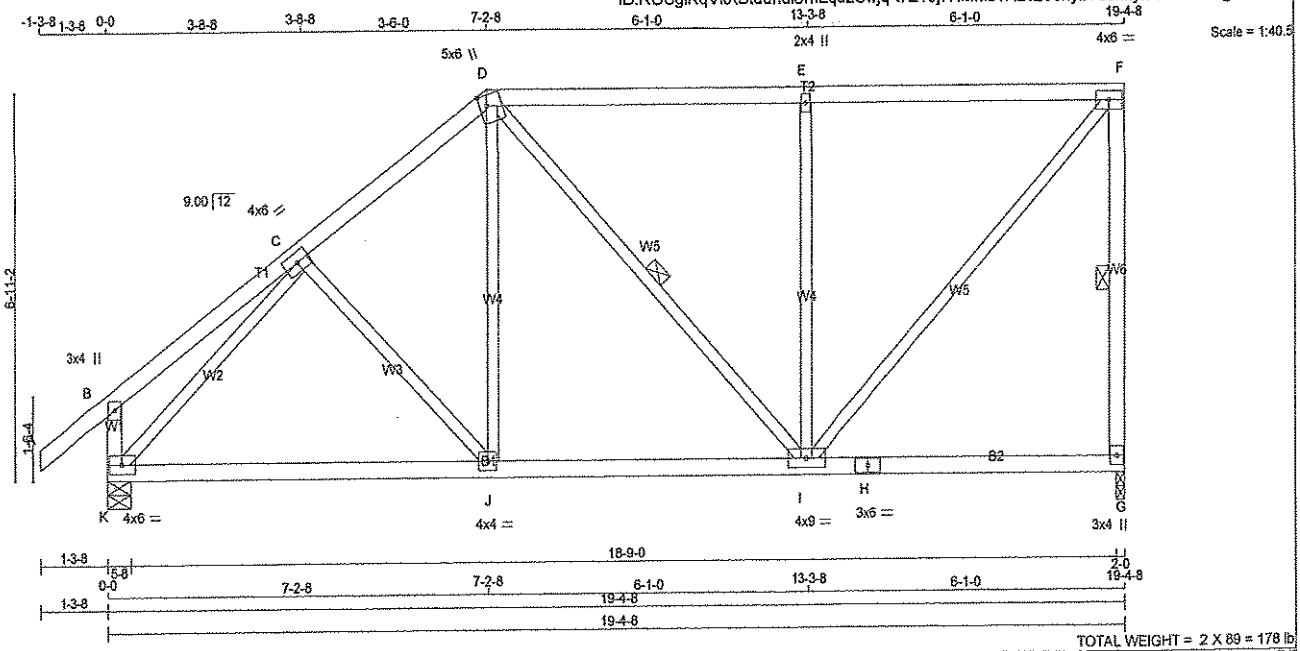


TOTAL WEIGHT = 2 X 89 = 178 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1088	0	1088	0	0	2-0	1-8
K	1204	0	1204	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	893	496 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0
K	972	566 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 20	-84.3	-84.3	0.17 (1)	10.00	
C- D	-974 / 0	-84.3	-84.3	0.18 (1)	6.08	
D- E	-756 / 0	-84.3	-84.3	0.55 (1)	5.97	
E- F	-757 / 0	-84.3	-84.3	0.55 (1)	5.97	
G- F	-1016 / 0	0.0	0.0	0.22 (1)	6.21	
K- B	-233 / 0	0.0	0.0	0.02 (1)	7.81	
K- J	0 / 818	-28.0	-28.0	0.41 (2)	10.00	
J- I	0 / 765	-28.0	-28.0	0.41 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.22 (3)	10.00	
H- G	0 / 0	-28.0	-28.0	0.22 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-83 / 71	0.04 (1)
J- D	0 / 347	0.08 (2)
D- I	-57 / 0	0.03 (3)
I- E	-635 / 0	0.52 (1)
I- F	0 / 1116	0.25 (1)
K- C	-1206 / 0	0.54 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.55 (E-F:1) , BC=0.41 (I-J:2) , WB=0.54 (C-K:1) , SSI=0.25 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

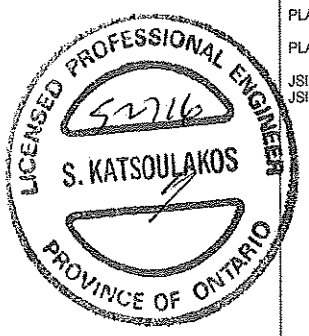
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)



DWG NO. TAM24432-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T48A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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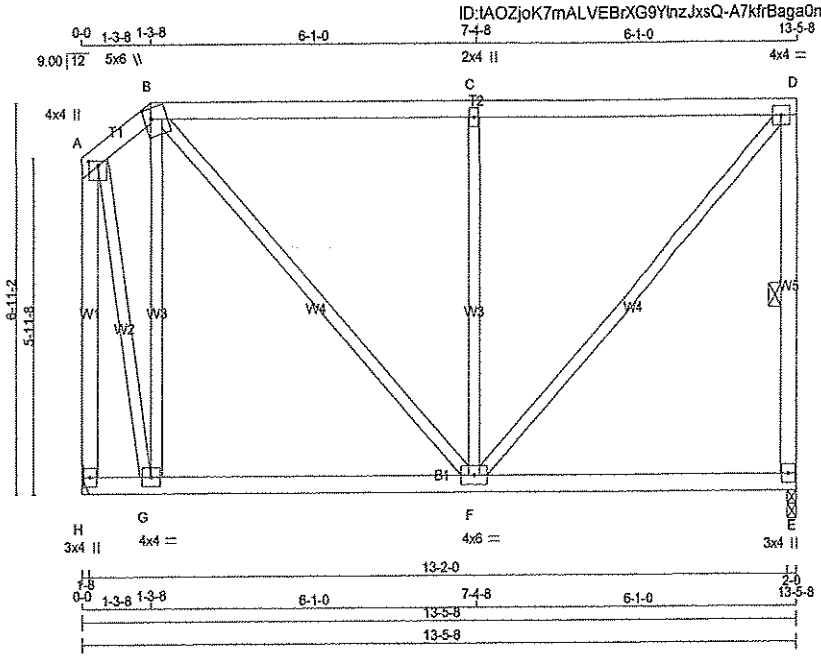

TOTAL WEIGHT = 74 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	756	0	756	0	0	2-0	1-8
H	756	0	756	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-168 / 0	-84.3	-84.3	0.02 (1)	6.25	G- B	-489 / 0	0.40 (1)
B- C	-453 / 0	-84.3	-84.3	0.54 (1)	6.25	B- F	0 / 465	0.10 (1)
C- D	-454 / 0	-84.3	-84.3	0.54 (1)	6.25	F- C	-636 / 0	0.52 (1)
E- D	-688 / 0	0.0	0.0	0.15 (1)	6.25	F- D	0 / 669	0.15 (1)
H- A	-780 / 0	0.0	0.0	0.56 (1)	7.81	A- G	0 / 638	0.14 (1)
H- G	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
G- F	0 / 140	-28.0	-28.0	0.29 (2)	10.00			
F- E	0 / 0	-28.0	-28.0	0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.56 (A-H:1) , BC=0.29 (F-G:2) , WB=0.52 (C-F:1) , SSI=0.25 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.18 (F) (INPUT = 1.00)

DWG NO. TAM 24456-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	T49	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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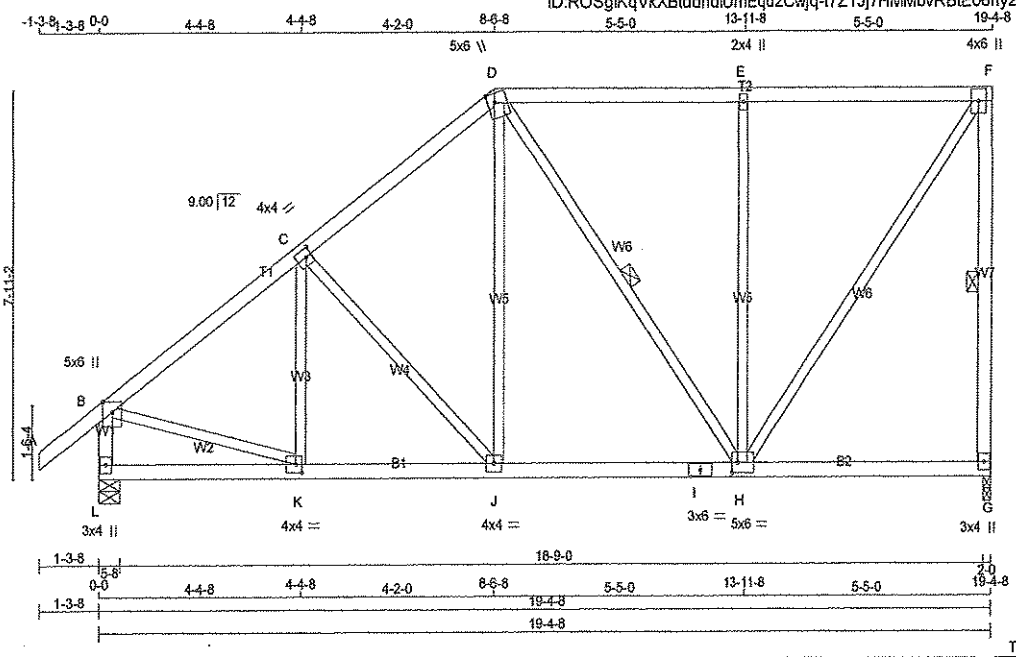

TOTAL WEIGHT = 2 X 96 = 193 lb

(M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1088	0	1088	0	0	2-0	1-8
L	1204	0	1204	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	893	496 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0
L	972	566 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-1067 / 0	-84.3	-84.3	0.28 (1)	5.75
C- D	-884 / 0	-84.3	-84.3	0.27 (1)	6.17
D- E	-608 / 0	-84.3	-84.3	0.42 (1)	6.25
E- F	-608 / 0	-84.3	-84.3	0.42 (1)	6.25
G- F	-1026 / 0	0.0	0.0	0.29 (1)	6.19
L- B	-1152 / 0	0.0	0.0	0.12 (1)	7.42
L- K	0 / 0	-28.0	-28.0	0.12 (3)	10.00
K- J	0 / 874	-28.0	-28.0	0.22 (2)	10.00
J- I	0 / 689	-28.0	-28.0	0.29 (2)	10.00
I- H	0 / 689	-28.0	-28.0	0.29 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-108 / 115	0.04 (1)
C- J	-274 / 0	0.18 (1)
J- D	0 / 355	0.08 (2)
D- H	-140 / 0	0.08 (1)
H- E	-564 / 0	0.67 (1)
H- F	0 / 1046	0.24 (1)
B- K	0 / 906	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.42 (E-F:1) , BC=0.29 (H-J:2) , WB=0.67 (E-H:1) , SSI=0.22 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.33 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 2-4433-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T49A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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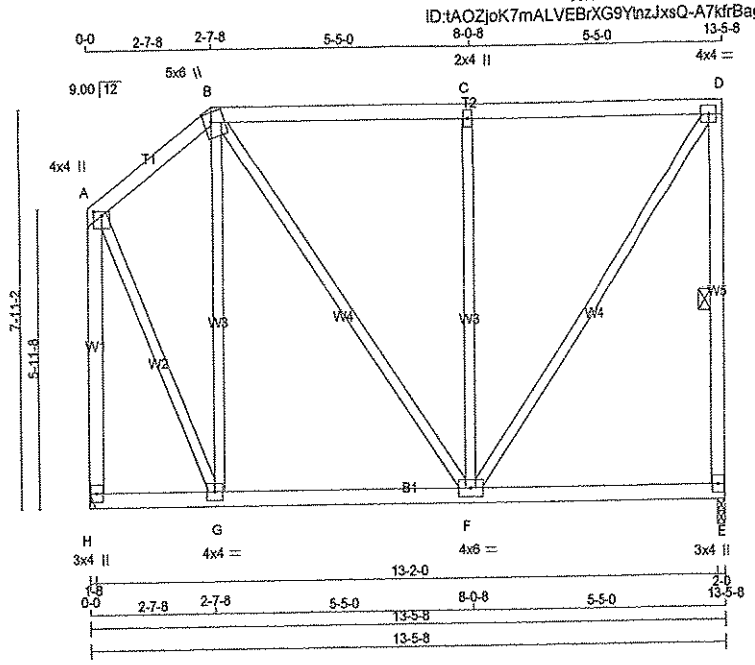


Scale = 1:45.0

TOTAL WEIGHT = 78 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.75
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	756	0	756	0	0	2-0	1-8
H	756	0	756	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-277 / 0	-84.3	-84.3	0.10 (1)	6.25	G- B	-335 / 0	0.40 (1)
B- C	-371 / 0	-84.3	-84.3	0.42 (1)	6.25	B- F	0 / 268	0.06 (1)
C- D	-371 / 0	-84.3	-84.3	0.42 (1)	6.25	F- C	-565 / 0	0.67 (1)
E- D	-694 / 0	0.0	0.0	0.20 (1)	6.25	F- D	0 / 637	0.14 (1)
H- A	-733 / 0	0.0	0.0	0.53 (1)	7.81	A- G	0 / 506	0.11 (1)
H- G	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
G- F	0 / 216	-28.0	-28.0	0.23 (2)	10.00			
F- E	0 / 0	-28.0	-28.0	0.21 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.53 (A-H:1) , BC=0.23 (F-G:2) , WB=0.67 (C-F:1) , SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

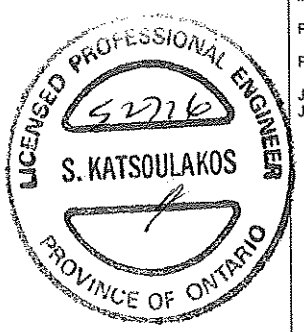
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (F) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)



DWG NO. TAM 24952-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267489	T50	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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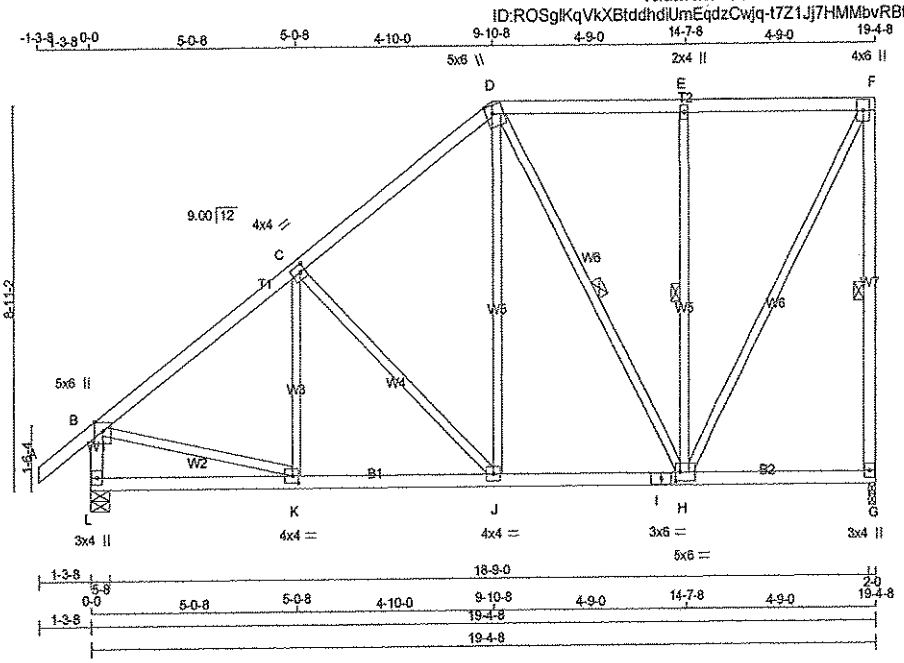

TOTAL WEIGHT = 2 X 102 = 204 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1088	0	1088	0	0	2-0	1-8
L	1204	0	1204	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	893	496 / 0	203 / 0	0 / 0	0 / 0	194 / 0	0 / 0
L	972	566 / 0	203 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.11	(1)	10.00	K- C	-50 / 176	0.04 (3)
B- C	-1069 / 0	-84.3	-84.3	0.28	(1)	5.77	C- J	-378 / 0	0.35 (1)
C- D	-797 / 0	-84.3	-84.3	0.27	(1)	6.25	J- D	0 / 405	0.09 (2)
D- E	-487 / 0	-84.3	-84.3	0.24	(1)	6.25	D- H	-270 / 0	0.18 (1)
E- F	-487 / 0	-84.3	-84.3	0.24	(1)	6.25	H- E	-493 / 0	0.25 (1)
G- F	-1033 / 0	0.0	0.0	0.38	(1)	6.18	H- F	0 / 1004	0.23 (1)
L- B	-1146 / 0	0.0	0.0	0.12	(1)	7.43	B- K	0 / 903	0.20 (1)
L- K	0 / 0	-28.0	-28.0	0.17	(3)	10.00			
K- J	0 / 879	-28.0	-28.0	0.27	(2)	10.00			
J- I	0 / 617	-28.0	-28.0	0.23	(2)	10.00			
I- H	0 / 617	-28.0	-28.0	0.23	(2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.15	(3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.38 (F-G:1) , BC=0.27 (J-K:2) , WB=0.35 (C-J:1) , SSI=0.19 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.33 (K) (INPUT = 1.00)

DWG NO. TAM 24434-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267489	T51X	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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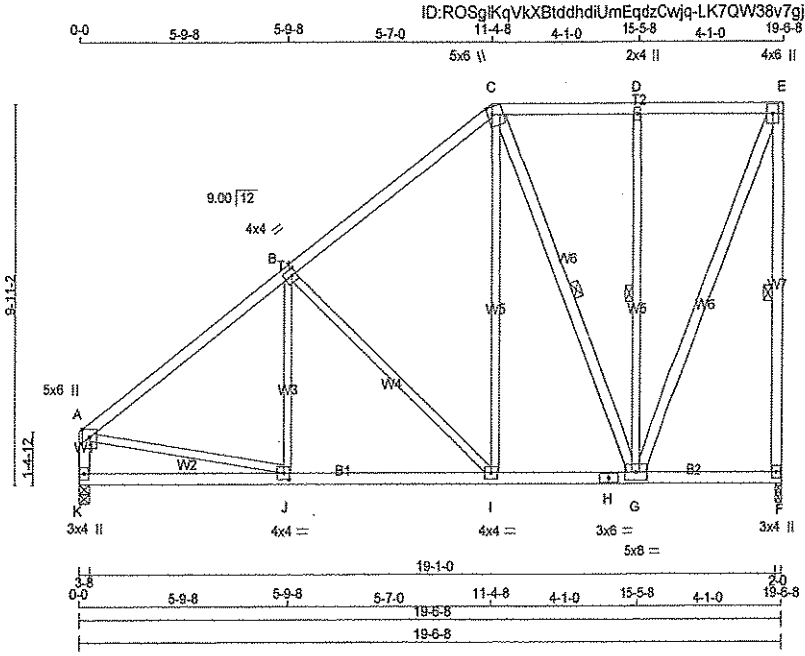

TOTAL WEIGHT = 2 X 113 = 226 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1097	0	1097	0	0	2-0	1-8
K	1097	0	1097	0	0	3-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0
K	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI	LC	MAX UNBRAC LENGTH
A- B	-1096 / 0	-84.3	-84.3	0.38	(1)	5.57
B- C	-715 / 0	-84.3	-84.3	0.36	(1)	6.25
C- D	-391 / 0	-84.3	-84.3	0.17	(1)	6.25
D- E	-391 / 0	-84.3	-84.3	0.17	(1)	6.25
F- E	-1049 / 0	0.0	0.0	0.50	(1)	6.14
K- A	-1032 / 0	0.0	0.0	0.11	(1)	7.72
K- J	0 / 0	-28.0	-28.0	0.24	(3)	10.00
J- I	0 / 905	-28.0	-28.0	0.34	(2)	10.00
I- H	0 / 548	-28.0	-28.0	0.18	(2)	10.00
H- G	0 / 548	-28.0	-28.0	0.18	(2)	10.00
G- F	0 / 0	-28.0	-28.0	0.11	(3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 244	0.05 (3)
B- I	-500 / 0	0.63 (1)
I- C	0 / 473	0.11 (1)
C- G	-403 / 0	0.23 (1)
G- D	-422 / 0	0.28 (1)
G- E	0 / 995	0.16 (1)
A- J	0 / 920	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.50 (E-F:1) , BC=0.34 (I-J:2) , WB=0.63 (B-I:1) , SSI=0.19 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.34 (J) (INPUT = 1.00)

DWG NO. TAM 24435-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T51A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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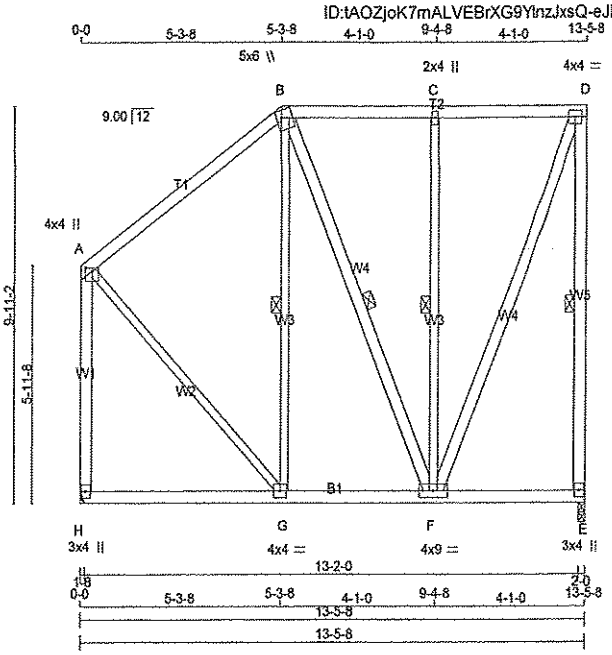

Scale = 1:56.8

TOTAL WEIGHT = 2 X 94 = 187 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	756	0	756	0	0	2-0	1-8
H	756	0	756	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E, B-G, B-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-328 / 0	-84.3	-84.3	0.41 (1)	6.25		G- B	-118 / 121	0.08 (1)
B- C	-244 / 0	-84.3	-84.3	0.23 (1)	6.25		B- F	-56 / 0	0.03 (2)
C- D	-244 / 0	-84.3	-84.3	0.23 (1)	6.25		F- C	-423 / 0	0.28 (1)
E- D	-705 / 0	0.0	0.0	0.34 (1)	6.25		F- D	0 / 621	0.10 (1)
H- A	-693 / 0	0.0	0.0	0.50 (1)	7.81		A- G	0 / 378	0.09 (1)
H- G	0 / 0	-28.0	-28.0	0.17 (2)	10.00				
G- F	0 / 261	-28.0	-28.0	0.20 (2)	10.00				
F- E	0 / 0	-28.0	-28.0	0.10 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.50 (A-H:1) , BC=0.20 (F-G:2) , WB=0.28 (C-F:1) , SSI=0.17 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (D) (INPUT = 0.50)
JSI METAL= 0.16 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
52716
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 2495816
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	T52X	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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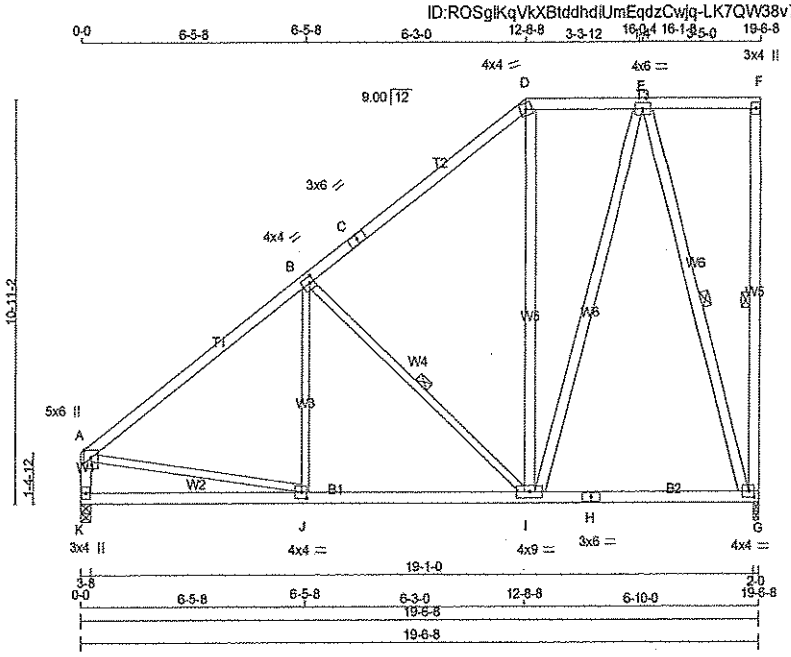

Scale = 1:61.3

TOTAL WEIGHT = 5 X 114 = 571 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
J - B	2x3	DRY	No.2	SPF
B - I	2x3	DRY	No.2	SPF
A - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TS-t	MT20	3.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0	2.00	1.75
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1097	0	1097	0	0	2-0	1-8
K	1097	0	1097	0	0	3-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0
K	901	500 / 0	205 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, B-I, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1075 / 0	-84.3	-84.3	0.49 (1)	5.43
B- C	-621 / 0	-84.3	-84.3	0.45 (1)	6.25
C- D	-621 / 0	-84.3	-84.3	0.45 (1)	6.25
D- E	-464 / 0	-84.3	-84.3	0.13 (1)	6.25
E- F	0 / 0	-84.3	-84.3	0.16 (1)	10.00
G- F	-109 / 0	0.0	0.0	0.07 (1)	6.25
K- A	-1023 / 0	0.0	0.0	0.11 (1)	7.75
K- J	0 / 0	-28.0	-28.0	0.26 (3)	10.00
J- I	0 / 891	-28.0	-28.0	0.39 (2)	10.00
I- H	0 / 293	-28.0	-28.0	0.32 (2)	10.00
H- G	0 / 293	-28.0	-28.0	0.32 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 269	0.06 (3)
B- I	-582 / 0	0.30 (1)
I- D	0 / 156	0.03 (3)
I- E	0 / 583	0.09 (1)
E- G	-958 / 0	0.65 (1)
A- J	0 / 903	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.49 (A-B:1) , BC=0.39 (I-J:2) , WB=0.65 (E-G:1) , SSI=0.21 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (J) (INPUT = 0.90)
JSI METAL= 0.33 (J) (INPUT = 1.00)

DWG NO. TAM 24436-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T52A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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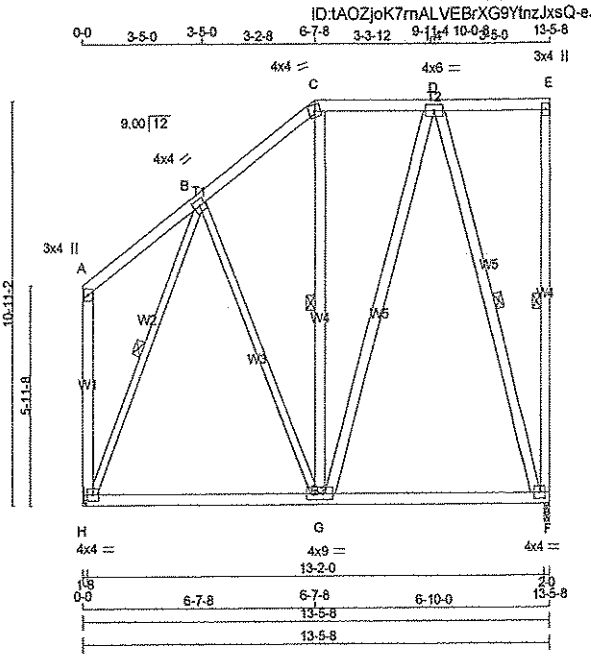

Scale = 1:61.3

TOTAL WEIGHT = 2 X 101 = 202 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF
H - B	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0	2.00	1.75
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	756	0	756	0	0	2-0	1-8
H	756	0	756	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-F, B-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-84.3	-84.3	0.15 (1)	10.00	B- G	0 / 114	0.03 (3)	
B- C	-337 / 0	-84.3	-84.3	0.15 (1)	6.25	G- C	-23 / 89	0.01 (1)	
C- D	-252 / 0	-84.3	-84.3	0.16 (1)	6.25	G- D	0 / 259	0.04 (2)	
D- E	0 / 0	-84.3	-84.3	0.16 (1)	10.00	D- F	-605 / 0	0.41 (1)	
F- E	-109 / 0	0.0	0.0	0.07 (1)	6.25	H- B	-621 / 0	0.32 (1)	
H- A	-109 / 0	0.0	0.0	0.08 (1)	7.81				
H- G	0 / 237	-28.0	-28.0	0.40 (2)	10.00				
G- F	0 / 185	-28.0	-28.0	0.39 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.16 (C-D:1) , BC=0.40 (G-H:2) , WB=0.41 (D-F:1) , SSI=0.16 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

DWG NO. TAM 24459-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T57	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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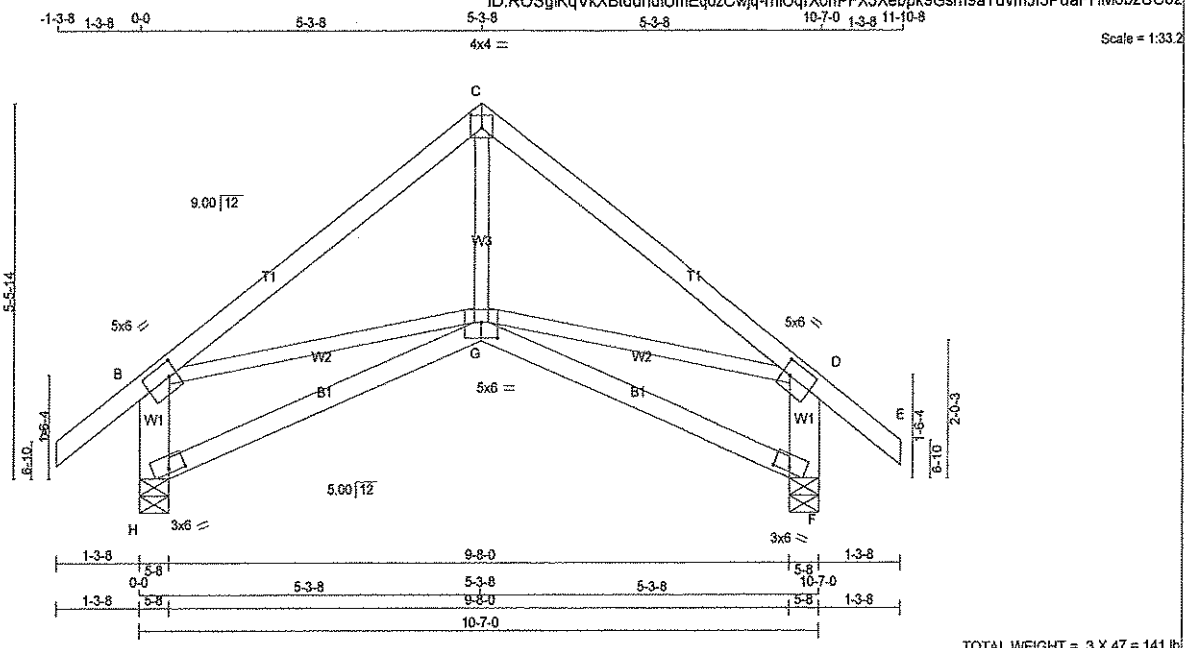

TOTAL WEIGHT = 3 X 47 = 141 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.50
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW-t	MT20	5.0	6.0	2.50	1.50
F	BVM1-I	MT20	3.0	6.0	0.75	3.00
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BVM1-I	MT20	3.0	6.0	0.75	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	710	0	710	0	0	5-8	1-8
F	710	0	710	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	567	341 / 0	111 / 0	0 / 0	0 / 0	114 / 0	0 / 0
F	567	341 / 0	111 / 0	0 / 0	0 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	G- C	0 / 375	0.03 (2)
B- C	-627 / 0	-84.3	-84.3	0.31 (1)	6.25	B- G	0 / 508	0.11 (1)
C- D	-627 / 0	-84.3	-84.3	0.31 (1)	6.25	G- D	0 / 508	0.11 (1)
D- E	0 / 35	-84.3	-84.3	0.11 (1)	10.00			
H- B	-636 / 0	0.0	0.0	0.05 (1)	7.81			
F- D	-636 / 0	0.0	0.0	0.05 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.31 (C-D:1) , BC=0.24 (F-G:3) , WB=0.11 (B-G:1) , SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (D) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)

DWG NO. TAM 24451-16

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	T57A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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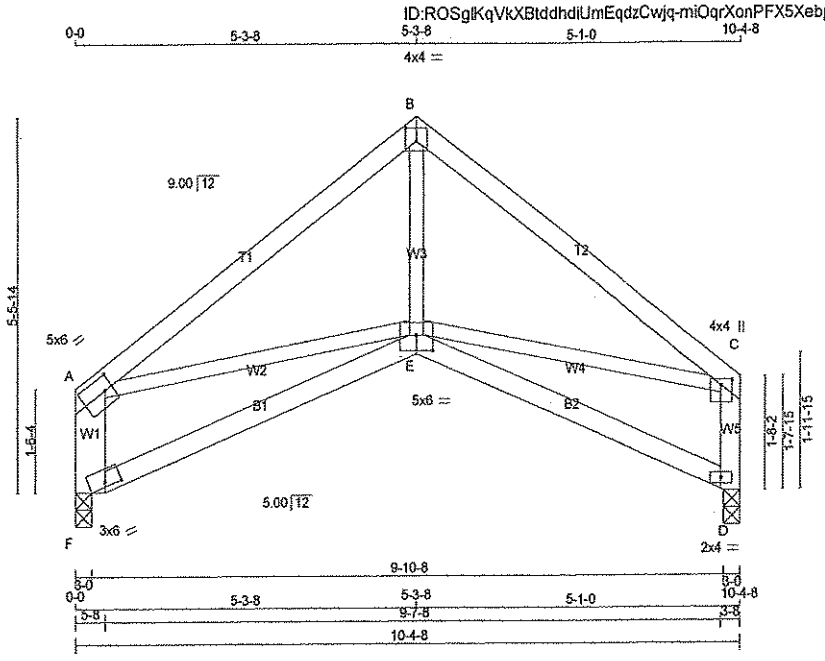

TOTAL WEIGHT = 42 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.50
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BVM1-p	MT20	2.0	4.0		
E	BBWWW-p	MT20	5.0	6.0	2.75	3.00
F	BVM1-l	MT20	3.0	6.0	0.75	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	582	0	582	0	0	3-0	1-8
D	582	0	582	0	0	3-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	478	266 / 0	109 / 0	0 / 0	0 / 0	104 / 0	0 / 0
D	478	266 / 0	109 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-602 / 0	-84.3	-84.3	0.31 (1)	6.25	E- B	0 / 357	0.08 (2)
B- C	-602 / 0	-84.3	-84.3	0.28 (1)	6.25	A- E	0 / 488	0.11 (1)
F- A	-508 / 0	0.0	0.0	0.04 (1)	7.81	E- C	0 / 486	0.11 (1)
D- C	-511 / 0	0.0	0.0	0.06 (1)	7.81			
F- E	0 / 0	-28.0	-28.0	0.23 (2)	10.00			
E- D	0 / 0	-28.0	-28.0	0.22 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.31 (A-B:1) , BC=0.23 (E-F:2) , WB=0.11 (A-E:1) , SSI=0.14 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.19 (C) (INPUT = 1.00)

DWG NO. TAM 24452-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T58S	1	1	TRUSS DESC.	

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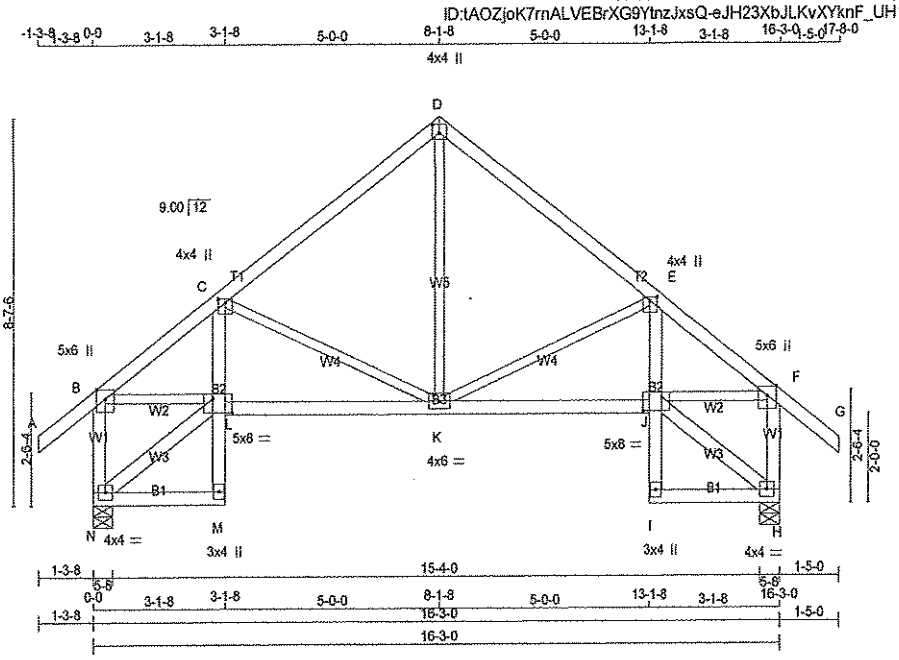

TOTAL WEIGHT = 86 lb

[M|F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	5.0	8.0	3.75	5.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-I	MT20	5.0	8.0	3.75	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1028	0	1028	0	0	5-8	1-8
H	1039	0	1039	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	828	486 / 0	171 / 0	0 / 0	0 / 0	171 / 0	0 / 0
H	835	493 / 0	171 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	
B- C	-1268 / 0	-84.3	-84.3	0.22 (1)	5.48	
C- D	-777 / 0	-84.3	-84.3	0.27 (1)	6.25	
D- E	-777 / 0	-84.3	-84.3	0.27 (1)	6.25	
E- F	-1268 / 0	-84.3	-84.3	0.22 (1)	5.48	
F- G	0 / 38	-84.3	-84.3	0.14 (1)	10.00	
N- B	-978 / 0	0.0	0.0	0.13 (1)	7.81	
H- F	-989 / 0	0.0	0.0	0.13 (1)	7.81	
N- M	0 / 12	-28.0	-28.0	0.08 (2)	10.00	
M- L	0 / 73	0.0	0.0	0.04 (1)	10.00	
L- C	0 / 171	0.0	0.0	0.05 (2)	10.00	
L- K	0 / 1056	-28.0	-28.0	0.34 (2)	10.00	
K- J	0 / 1056	-28.0	-28.0	0.34 (2)	10.00	
I- J	0 / 73	0.0	0.0	0.04 (1)	10.00	
J- E	0 / 171	0.0	0.0	0.05 (2)	10.00	
I- H	0 / 12	-28.0	-28.0	0.08 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 546	0.12 (1)
K- E	-500 / 0	0.28 (1)
C- K	-500 / 0	0.28 (1)
N- L	-14 / 0	0.00 (1)
B- L	0 / 1045	0.24 (1)
J- H	-14 / 0	0.00 (1)
J- F	0 / 1045	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.27 (C-D:1) , BC=0.34 (K-L:2) , WB=0.28 (C-K:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

DWG NO. TAM24460216

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267489	G58	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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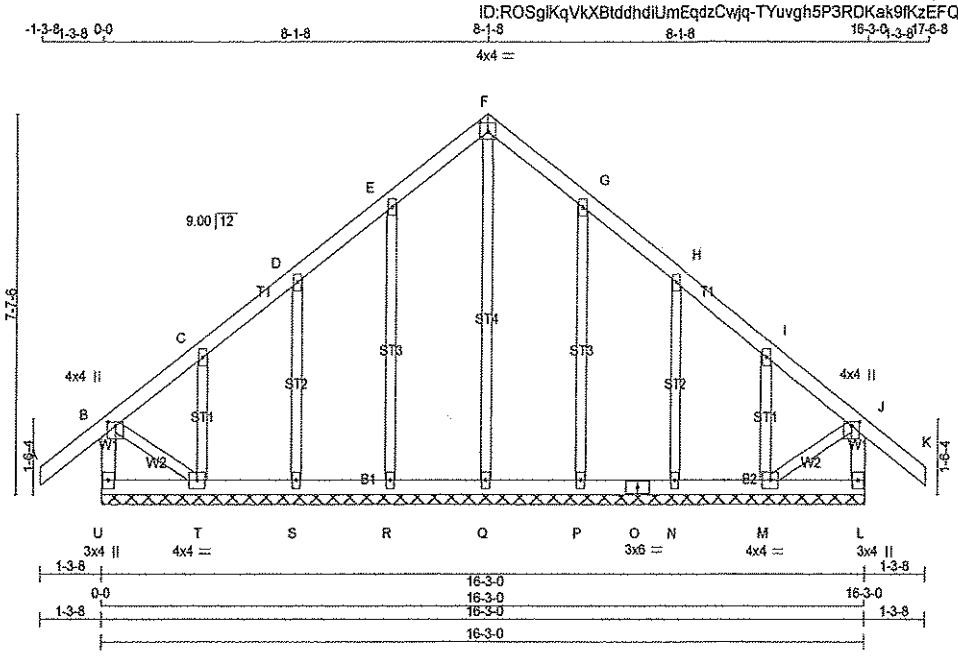


TOTAL WEIGHT = 77 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
U - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, P, Q, R, S						
N	BMW1+w	MT20	2.0	4.0		
O	BS-t	MT20	3.0	6.0		
T	BMWW1-t	MT20	4.0	4.0		
U	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
U- B	-220 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00
B- C	-24 / 0	-84.3	-84.3	0.05 (1)	6.25
C- D	-30 / 0	-84.3	-84.3	0.05 (1)	6.25
D- E	-20 / 0	-84.3	-84.3	0.05 (1)	6.25
E- F	-30 / 0	-84.3	-84.3	0.05 (1)	6.25
F- G	-30 / 0	-84.3	-84.3	0.05 (1)	6.25
G- H	-20 / 0	-84.3	-84.3	0.05 (1)	6.25
H- I	-30 / 0	-84.3	-84.3	0.05 (1)	6.25
I- J	-24 / 0	-84.3	-84.3	0.05 (1)	6.25
J- K	0 / 35	-84.3	-84.3	0.11 (1)	10.00
L- J	-220 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 / 0	-28.0	-28.0	0.03 (2)	10.00
T- S	0 / 21	-28.0	-28.0	0.03 (2)	10.00
S- R	0 / 18	-28.0	-28.0	0.02 (2)	10.00
R- Q	0 / 15	-28.0	-28.0	0.02 (2)	10.00
Q- P	0 / 15	-28.0	-28.0	0.02 (2)	10.00
P- O	0 / 18	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 18	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 21	-28.0	-28.0	0.03 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- F	-118 / 0	0.12 (1)
R- E	-193 / 0	0.11 (1)
S- D	-155 / 0	0.05 (1)
T- C	-189 / 0	0.03 (1)
P- G	-193 / 0	0.11 (1)
N- H	-155 / 0	0.05 (1)
M- I	-189 / 0	0.03 (1)
B- T	0 / 32	0.01 (1)
M- J	0 / 32	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.03 (S-T:2) , WB=0.12 (F-Q:1) , SSI=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

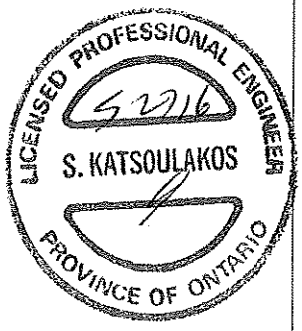
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (F) (INPUT = 0.90)
JSI METAL= 0.05 (E) (INPUT = 1.00)



DWG NO. TAM 2447216
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	T59	3	1	TRUSS DESC.	

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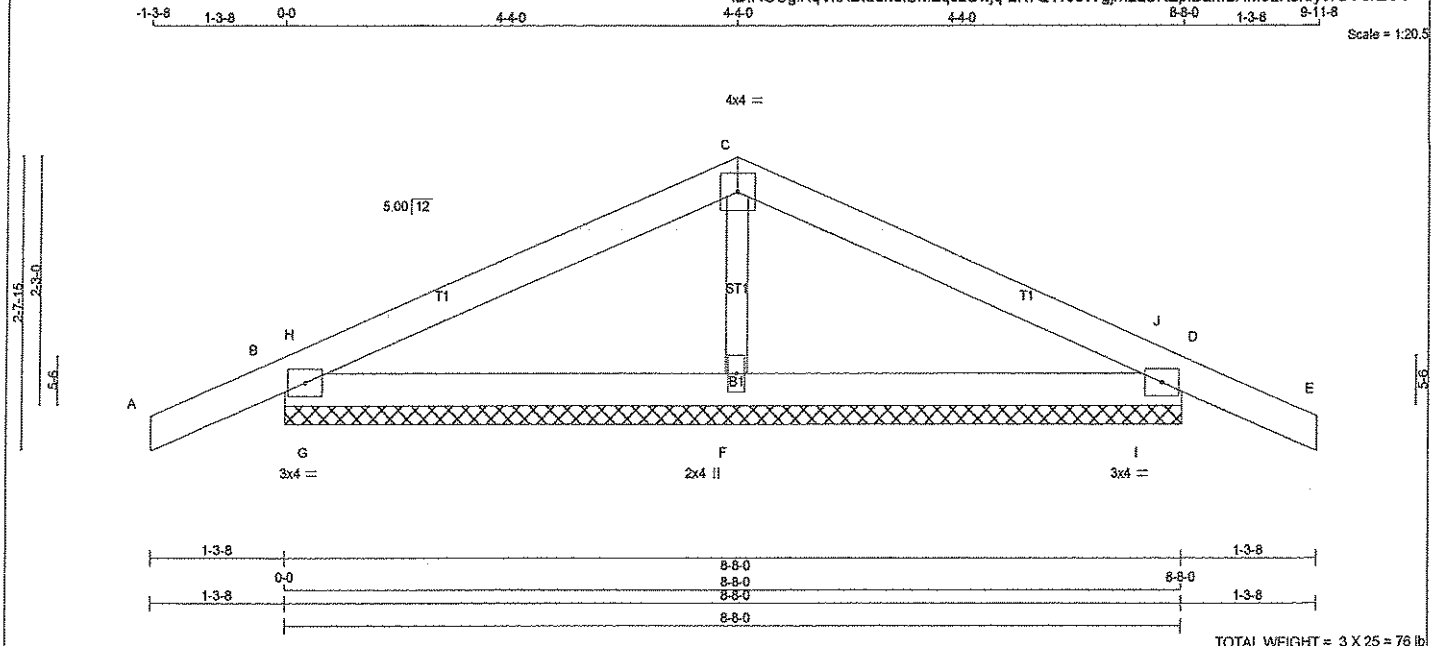

TOTAL WEIGHT = 3 X 25 = 76 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0		
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F- C	-80 / 41	0.01 (1)
B- H	-212 / 0	-84.3	-84.3	0.10 (1)	6.25	G- H	-311 / 109	0.00 (1)
H- C	-300 / 0	-84.3	-84.3	0.19 (1)	6.25	I- J	-311 / 109	0.00 (1)
C- J	-300 / 0	-84.3	-84.3	0.19 (1)	6.25			
J- D	-212 / 0	-84.3	-84.3	0.10 (1)	6.25			
D- E	0 / 17	-84.3	-84.3	0.11 (1)	10.00			
B- G	0 / 271	-28.0	-28.0	0.17 (1)	10.00			
G- F	0 / 271	-28.0	-28.0	0.19 (1)	10.00			
F- I	0 / 271	-28.0	-28.0	0.19 (1)	10.00			
I- D	0 / 271	-28.0	-28.0	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19 (C-J;1) , BC=0.19 (F-G;1) , WB=0.01 (C-F;1) , SSI=0.25 (B-G;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

DWG NO. TAM2493B16

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T60Z	1	2	TRUSS DESC.	

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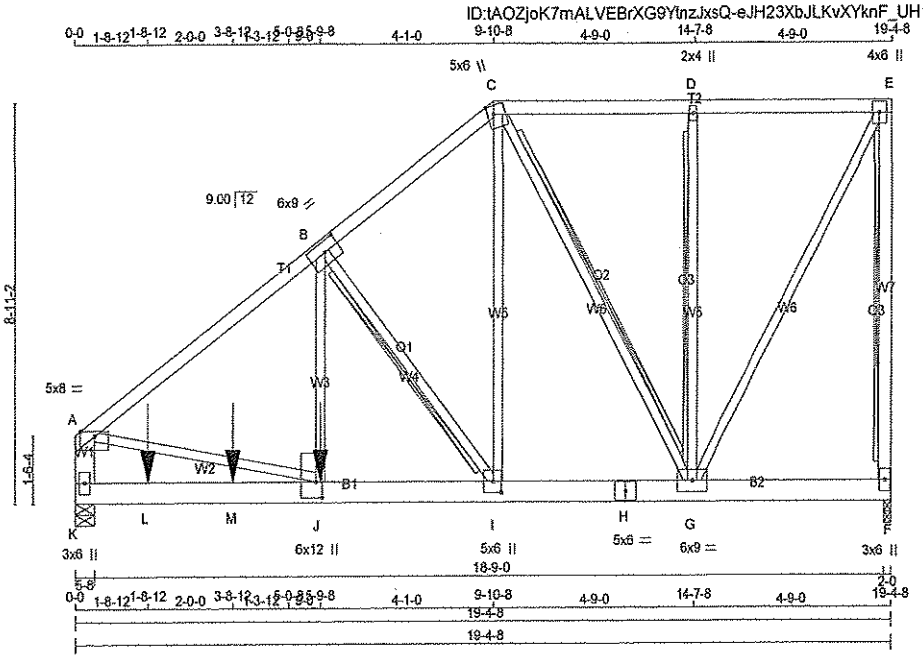

TOTAL WEIGHT = 2 X 113 = 227 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- F	1	12	TOP
K- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(183.1)
H- F	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
B- J	1	2	SIDE(1075.4)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	6.0	9.0	2.75	4.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	6.0		
G	BMWWW-t	MT20	6.0	9.0		
H	BS-t	MT20	5.0	6.0		
I	BMWW+t	MT20	5.0	6.0	3.00	2.25
J	BMWW+t	MT20	6.0	12.0	4.25	1.75
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	2864	0	2864	0	0	2-0	1-9
K	6508	0	6508	0	0	5-8	5-1

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2326	1332 / 0	503 / 0	0 / 0	0 / 0	491 / 0	0 / 0
K	5272	3042 / 0	1124 / 0	0 / 0	0 / 0	1106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-F, D-G
2x6 DRY SPF No.2 T-BRACE AT C-G, B-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-6798 / 0	-84.3	-84.3	0.61 (1)	3.45	I- C	0 / 3560	0.44 (1)
B- C	-3188 / 0	-84.3	-84.3	0.21 (1)	5.00	C- G	-2212 / 0	0.68 (1)
C- D	-1496 / 0	-84.3	-84.3	0.18 (1)	6.25	G- D	-488 / 0	0.18 (1)
D- E	-1496 / 0	-84.3	-84.3	0.18 (1)	6.25	G- E	0 / 3061	0.38 (1)
F- E	-2826 / 0	0.0	0.0	0.89 (1)	7.81	J- B	0 / 5509	0.68 (1)
K- A	-5345 / 0	0.0	0.0	0.19 (1)	6.33	B- I	-4818 / 0	0.75 (1)
						A- J	0 / 5555	0.69 (1)
K- L	0 / 0	-28.0	-28.0	0.72 (1)	10.00			
L- M	0 / 0	-28.0	-28.0	0.72 (1)	10.00			
M- J	0 / 0	-28.0	-28.0	0.72 (1)	10.00			
J- I	0 / 5456	-28.0	-28.0	0.72 (1)	10.00			
I- H	0 / 2565	-28.0	-28.0	0.26 (1)	10.00			
H- G	0 / 2565	-28.0	-28.0	0.26 (1)	10.00			
G- F	0 / 0	-28.0	-28.0	0.06 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	5-9-8	-4824	-4824	---	FRONT	VERT	TOTAL
L	1-8-12	-1186	-1186	---	FRONT	VERT	TOTAL
M	3-8-12	-1186	-1186	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 906 (0.26")

CSI: TC=0.89 (E-F:1) , BC=0.72 (J-K:1) , WB=0.75 (B-I:1) , SSI=0.45 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

- JSI:GRIP = 0.89 (B) (INPUT = 0.90)
- JSI:WEB = 0.79 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5226
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM-24461-16
STRUCTURAL
COMPONENT ONLY

pg 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T60Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1186.0 lbs FACTORED DOWN AT 1-8-12, AND 1186.0 lbs FACTORED DOWN AT 3-8-12, AND 4824.0 lbs FACTORED DOWN AT 5-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM24461-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T61	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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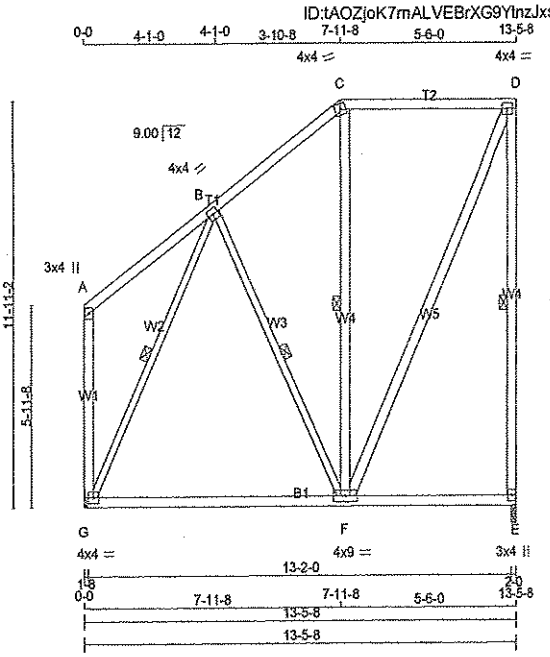

TOTAL WEIGHT = 3 X 94 = 283 lb

[MiF]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - F	2x3	DRY	No.2	SPF
G - B	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	756	0	756	0	0	2-0	1-8
G	756	0	756	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0
G	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E, B-F, C-F, B-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 24	-84.3	-84.3	0.22 (1)	10.00	B- F	-57 / 80	0.04 (1)
B- C	-305 / 0	-84.3	-84.3	0.17 (1)	6.25	F- C	-188 / 66	0.15 (1)
C- D	-222 / 0	-84.3	-84.3	0.33 (1)	6.25	F- D	0 / 530	0.09 (1)
E- D	-713 / 0	0.0	0.0	0.55 (1)	6.25	G- B	-569 / 0	0.37 (1)
G- A	-129 / 0	0.0	0.0	0.09 (1)	7.81			
G- F	0 / 248	-28.0	-28.0	0.44 (2)	10.00			
F- E	0 / 0	-28.0	-28.0	0.42 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 620 (0.26")

CSI: TC=0.55 (D-E:1) , BC=0.44 (F-G:2) , WB=0.37 (B-G:1) , SSI=0.18 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

DWG NO . TAM 24/6216
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	G65	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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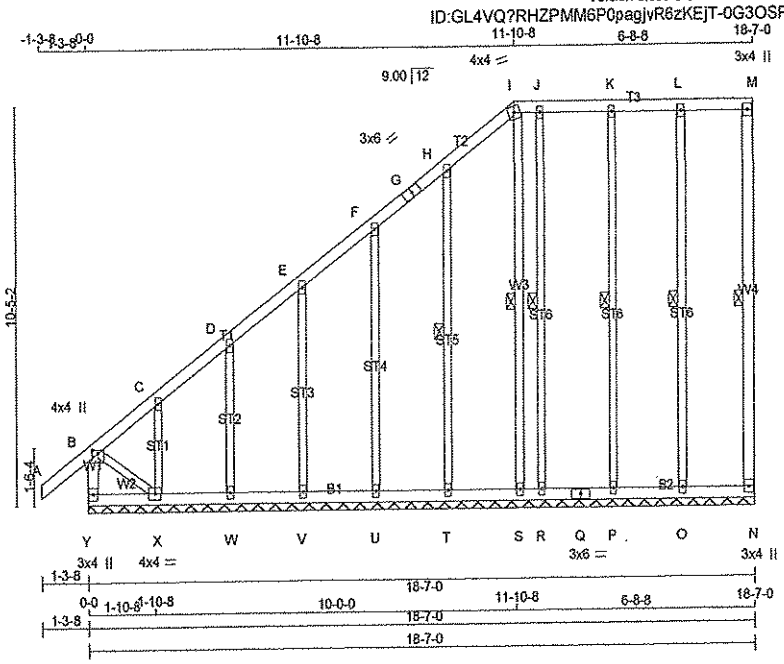

TOTAL WEIGHT = 116 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
Y - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, J, K, L						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
I	TTW-m	MT20	4.0	4.0		
M	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, R, S, T, U, V, W						
O	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF M-N, L-O, K-P, J-R, H-T, I-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- B	-251 / 0	0.0	0.0	0.03 (1)	7.81	O- L	-183 / 0	0.14 (1)
A- B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	P- K	-175 / 0	0.13 (1)
B- C	-40 / 0	-84.3	-84.3	0.11 (1)	6.25	R- J	-129 / 0	0.10 (1)
C- D	-8 / 0	-84.3	-84.3	0.04 (1)	10.00	T- H	-184 / 0	0.10 (1)
D- E	-10 / 0	-84.3	-84.3	0.04 (1)	6.25	U- F	-166 / 0	0.17 (1)
E- F	-6 / 0	-84.3	-84.3	0.04 (1)	10.00	V- E	-165 / 0	0.09 (1)
F- G	-2 / 0	-84.3	-84.3	0.04 (1)	10.00	W- D	-176 / 0	0.05 (1)
G- H	-2 / 0	-84.3	-84.3	0.04 (1)	10.00	X- C	-117 / 0	0.02 (1)
H- I	-10 / 0	-84.3	-84.3	0.04 (1)	6.25	S- I	-75 / 0	0.06 (1)
I- J	0 / 0	-84.3	-84.3	0.02 (1)	10.00	B- X	0 / 17	0.00 (1)
J- K	0 / 0	-84.3	-84.3	0.04 (1)	10.00			
K- L	0 / 0	-84.3	-84.3	0.04 (1)	10.00			
L- M	0 / 0	-84.3	-84.3	0.04 (1)	10.00			
N- M	-70 / 0	0.0	0.0	0.04 (1)	6.25			
Y- X	0 / 0	-28.0	-28.0	0.03 (2)	10.00			
X- W	0 / 11	-28.0	-28.0	0.03 (2)	10.00			
W- V	0 / 7	-28.0	-28.0	0.02 (2)	10.00			
V- U	0 / 5	-28.0	-28.0	0.02 (2)	10.00			
U- T	0 / 3	-28.0	-28.0	0.02 (2)	10.00			
T- S	0 / 0	-28.0	-28.0	0.02 (2)	10.00			
S- R	0 / 0	-28.0	-28.0	0.02 (2)	10.00			
R- Q	0 / 0	-28.0	-28.0	0.02 (2)	10.00			
Q- P	0 / 0	-28.0	-28.0	0.02 (2)	10.00			
P- O	0 / 0	-28.0	-28.0	0.03 (2)	10.00			
O- N	0 / 0	-28.0	-28.0	0.03 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.03 (W-X:2) , WB=0.17 (F-U:1) , SSI=0.08 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (I) (INPUT = 0.60)
JSI METAL= 0.05 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM24399-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	G66	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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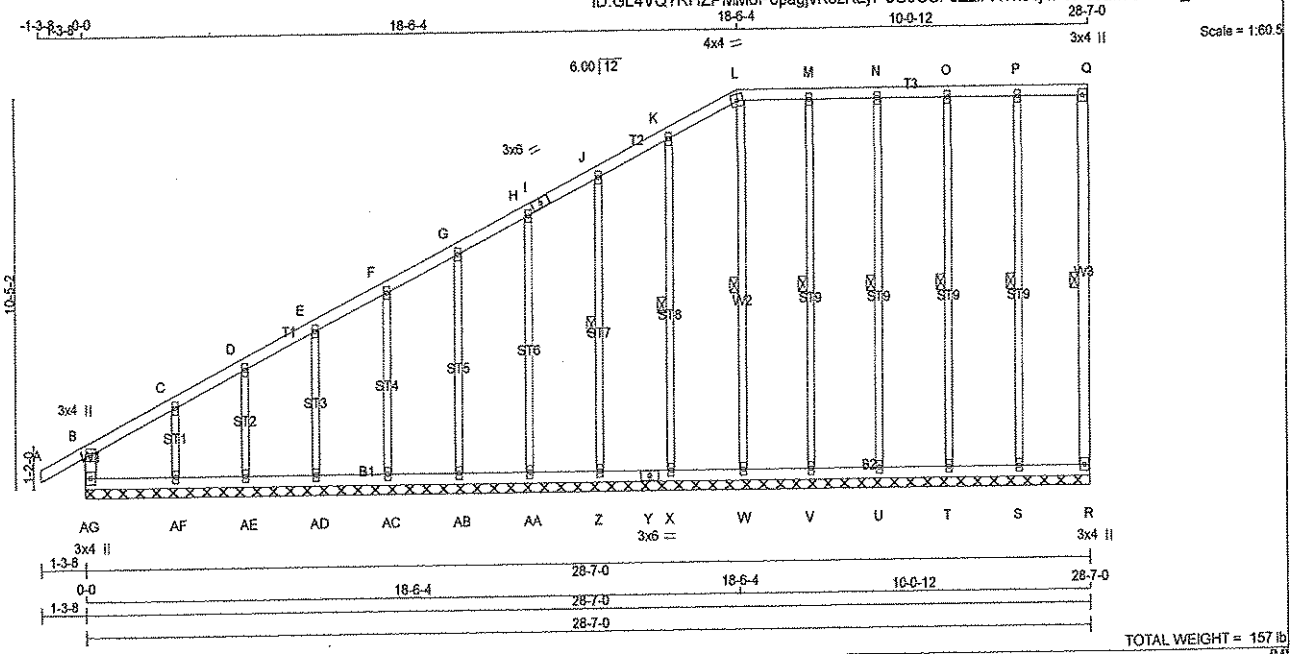

TOTAL WEIGHT = 157 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AG- B	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - Q	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
AG- Y	2x4	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, G, H, J, K, M, N, O, P						
C	TMW+w	MT20	2.0	4.0		
I	TS-t	MT20	3.0	6.0		
L	TTW-m	MT20	4.0	4.0		
Q	TMV+p	MT20	3.0	4.0		
R	BMV1+p	MT20	3.0	4.0		
S, T, U, V, W, X, Z, AA, AB, AC, AD, AE, AF						
S	BMW1+w	MT20	2.0	4.0		
Y	BS-t	MT20	3.0	6.0		
AG	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF Q-R, P-S, O-T, N-U, M-V, K-X, J-Z, L-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AG- B	-237 / 0	0.0	0.0	0.02 (2)	7.81
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-28 / 0	-84.3	-84.3	0.07 (1)	6.25
C- D	-21 / 0	-84.3	-84.3	0.05 (1)	6.25
D- E	-15 / 0	-84.3	-84.3	0.04 (1)	6.25
E- F	-12 / 0	-84.3	-84.3	0.04 (1)	6.25
F- G	-10 / 0	-84.3	-84.3	0.04 (1)	10.00
G- H	-8 / 0	-84.3	-84.3	0.04 (1)	10.00
H- I	-6 / 0	-84.3	-84.3	0.04 (1)	10.00
I- J	-6 / 0	-84.3	-84.3	0.04 (1)	10.00
J- K	-5 / 0	-84.3	-84.3	0.04 (1)	10.00
K- L	-3 / 0	-84.3	-84.3	0.04 (1)	10.00
L- M	-1 / 0	-84.3	-84.3	0.04 (1)	10.00
M- N	-1 / 0	-84.3	-84.3	0.04 (1)	10.00
N- O	-1 / 0	-84.3	-84.3	0.04 (1)	10.00
O- P	-1 / 0	-84.3	-84.3	0.04 (1)	10.00
P- Q	-1 / 0	-84.3	-84.3	0.04 (1)	10.00
R- Q	-75 / 0	0.0	0.0	0.01 (1)	6.25
AG-AF	0 / 23	-28.0	-28.0	0.04 (2)	10.00
AF-AE	0 / 17	-28.0	-28.0	0.04 (2)	10.00
AE-AD	0 / 14	-28.0	-28.0	0.02 (2)	10.00
AD-AC	0 / 11	-28.0	-28.0	0.02 (2)	10.00
AC-AB	0 / 9	-28.0	-28.0	0.02 (2)	10.00
AB-AA	0 / 7	-28.0	-28.0	0.02 (2)	10.00
AA- Z	0 / 6	-28.0	-28.0	0.02 (2)	10.00
Z- Y	0 / 4	-28.0	-28.0	0.02 (2)	10.00
Y- X	0 / 4	-28.0	-28.0	0.02 (2)	10.00
X- W	0 / 3	-28.0	-28.0	0.02 (2)	10.00
W- V	0 / 1	-28.0	-28.0	0.02 (3)	10.00
V- U	0 / 1	-28.0	-28.0	0.02 (3)	10.00
U- T	0 / 1	-28.0	-28.0	0.02 (3)	10.00
T- S	0 / 1	-28.0	-28.0	0.03 (3)	10.00
S- R	0 / 1	-28.0	-28.0	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- P	-178 / 0	0.13 (1)
T- O	-169 / 0	0.13 (1)
U- N	-168 / 0	0.13 (1)
V- M	-172 / 0	0.13 (1)
X- K	-168 / 0	0.10 (1)
Z- J	-168 / 0	0.08 (1)
AA- H	-168 / 0	0.17 (1)
AB- G	-168 / 0	0.11 (1)
AC- F	-167 / 0	0.08 (1)
AD- E	-169 / 0	0.05 (1)
AE- D	-161 / 0	0.03 (1)
AF- C	-187 / 0	0.03 (1)
W- L	-170 / 0	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.04 (AF-AG:2) , WB=0.17 (H-AA:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT *RESPONSIBLE FOR QUALITY CONTROL IN* THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI GRIP= 0.55 (L) (INPUT = 0.90)
LSI METAL= 0.05 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO . TAM 24/402 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267489	G88	2	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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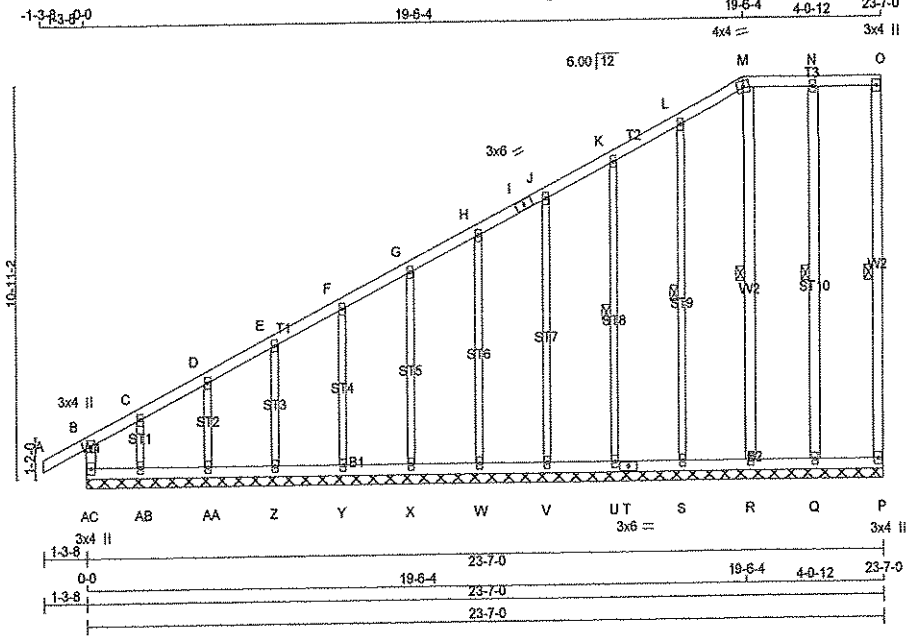
Scale = 1:63.0



TOTAL WEIGHT = 2 X 134 = 268 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AC- B	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
M - O	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
AC- T	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
ALL GABLE WEBS EXCEPT	2x3	DRY	No.2	SPF
ST1	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C, D, E, F, G, H, J, K, L, N					
C	TMW+w	MT20	2.0	4.0	
I	TS-t	MT20	3.0	6.0	
M	TTW-m	MT20	4.0	4.0	
O	TMV+p	MT20	3.0	4.0	
P	BMV1+p	MT20	3.0	4.0	
Q, R, S, U, V, W, X, Y, Z, AA, AB					
Q	BMW1+w	MT20	2.0	4.0	
T	BS-t	MT20	3.0	6.0	
AC	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF O-P, N-Q, L-S, K-U, M-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AC- B	-229 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-44 / 0	-84.3	-84.3	0.09 (1)	6.25
C- D	-18 / 0	-84.3	-84.3	0.04 (1)	6.25
D- E	-19 / 0	-84.3	-84.3	0.04 (1)	6.25
E- F	-14 / 0	-84.3	-84.3	0.04 (1)	6.25
F- G	-12 / 0	-84.3	-84.3	0.04 (1)	6.25
G- H	-10 / 0	-84.3	-84.3	0.04 (1)	10.00
H- I	-8 / 0	-84.3	-84.3	0.04 (1)	10.00
I- J	-8 / 0	-84.3	-84.3	0.04 (1)	10.00
J- K	-8 / 0	-84.3	-84.3	0.04 (1)	10.00
K- L	-5 / 0	-84.3	-84.3	0.04 (1)	10.00
L- M	-3 / 0	-84.3	-84.3	0.04 (1)	10.00
M- N	0 / 0	-84.3	-84.3	0.04 (1)	10.00
N- O	0 / 0	-84.3	-84.3	0.04 (1)	10.00
P- O	-72 / 0	0.0	0.0	0.01 (1)	6.25
AC-AB	0 / 24	-28.0	-28.0	0.04 (1)	10.00
AB-AA	0 / 21	-28.0	-28.0	0.02 (2)	10.00
AA- Z	0 / 16	-28.0	-28.0	0.02 (2)	10.00
Z- Y	0 / 13	-28.0	-28.0	0.02 (2)	10.00
Y- X	0 / 11	-28.0	-28.0	0.02 (2)	10.00
X- W	0 / 9	-28.0	-28.0	0.02 (2)	10.00
W- V	0 / 7	-28.0	-28.0	0.02 (2)	10.00
V- U	0 / 6	-28.0	-28.0	0.02 (2)	10.00
U- T	0 / 4	-28.0	-28.0	0.02 (2)	10.00
T- S	0 / 4	-28.0	-28.0	0.02 (2)	10.00
S- R	0 / 3	-28.0	-28.0	0.02 (2)	10.00
R- Q	0 / 0	-28.0	-28.0	0.03 (3)	10.00
Q- P	0 / 0	-28.0	-28.0	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- N	-184 / 0	0.11 (1)
S- L	-165 / 0	0.11 (1)
U- K	-169 / 0	0.09 (1)
V- J	-168 / 0	0.20 (1)
W- H	-168 / 0	0.14 (1)
X- G	-167 / 0	0.09 (1)
Y- F	-168 / 0	0.06 (1)
Z- E	-164 / 0	0.04 (1)
AA- D	-179 / 0	0.03 (1)
AB- C	-98 / 0	0.01 (1)
R- M	-172 / 0	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.04 (AB-AC:1) , WB=0.20 (J-V:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

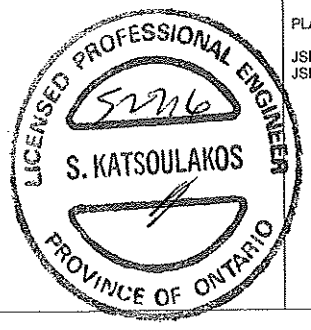
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (M) (INPUT = 0.90)
JSI METAL= 0.05 (M) (INPUT = 1.00)



DWG NO. TAM2#493#16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T200	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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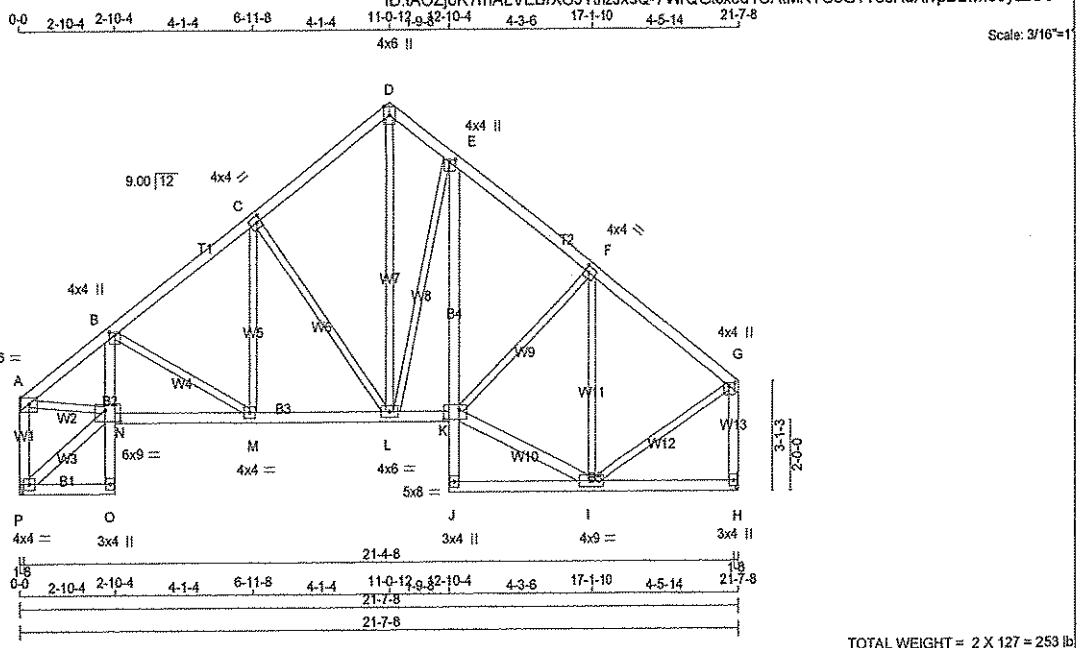

TOTAL WEIGHT = 2 X 127 = 253 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
P - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW+p	MT20	4.0	4.0	1.00	2.25
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMWW-t	MT20	5.0	8.0	3.25	5.50
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BVMWW-t	MT20	6.0	9.0	4.00	5.50
O	BMV+p	MT20	3.0	4.0		
P	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1214	0	1214	0	0	HANGER BY OTHERS IN-SX	IN-SX
						MIN. SEAT SIZE: 1-8	
P	1214	0	1214	0	0	HANGER BY OTHERS IN-SX	IN-SX
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	997	553 / 0	227 / 0	0 / 0	0 / 0	216 / 0	0 / 0
P	997	553 / 0	227 / 0	0 / 0	0 / 0	216 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1600 / 0	-84.3	-84.3	0.11 (1)	5.14
B- C	-1308 / 0	-84.3	-84.3	0.19 (1)	5.47
C- D	-971 / 0	-84.3	-84.3	0.17 (1)	6.12
D- E	-963 / 0	-84.3	-84.3	0.06 (1)	6.25
E- F	-1125 / 0	-84.3	-84.3	0.20 (1)	5.76
F- G	-904 / 0	-84.3	-84.3	0.20 (1)	6.24
P- A	-1165 / 0	0.0	0.0	0.17 (1)	7.39
H- G	-1165 / 0	0.0	0.0	0.20 (1)	7.39
P- O	0 / 14	-28.0	-28.0	0.06 (2)	10.00
O- N	0 / 66	0.0	0.0	0.05 (1)	10.00
N- B	0 / 116	0.0	0.0	0.05 (2)	10.00
N- M	0 / 1307	-28.0	-28.0	0.27 (1)	10.00
M- L	0 / 1055	-28.0	-28.0	0.24 (2)	10.00
L- K	0 / 887	-28.0	-28.0	0.19 (1)	10.00
J- K	0 / 75	0.0	0.0	0.02 (1)	10.00
K- E	0 / 323	0.0	0.0	0.07 (1)	10.00
J- I	0 / 6	-28.0	-28.0	0.15 (3)	10.00
I- H	0 / 0	-28.0	-28.0	0.15 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- M	-290 / 0	0.11 (1)
M- C	0 / 292	0.07 (2)
C- L	-502 / 0	0.46 (1)
L- D	0 / 945	0.21 (1)
L- E	-494 / 0	0.58 (1)
K- I	0 / 825	0.13 (1)
K- F	0 / 200	0.04 (1)
I- F	-681 / 0	0.45 (1)
I- G	0 / 872	0.20 (1)
P- N	-18 / 0	0.00 (1)
A- N	0 / 1298	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.20 (F-G:1) , BC=0.27 (M-N:1) , WB=0.58 (E-L:1) , SSI=0.14 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.28 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 2446316
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T201	1	2	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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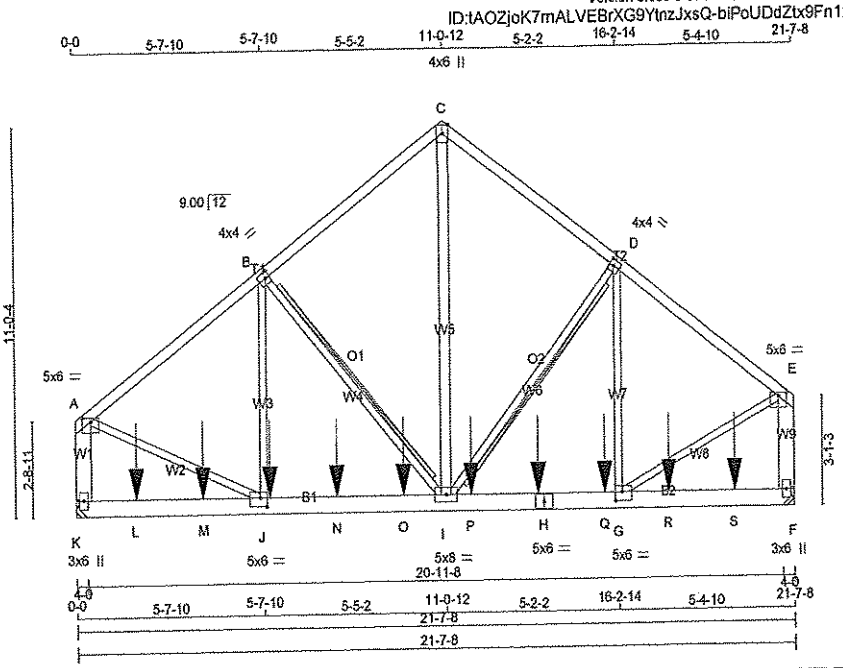

TOTAL WEIGHT = 2 X 123 = 246 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(183.1)
H- F	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
B- J	1	6	SIDE(6.7)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	6.0	1.25	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TMWW-p	MT20	5.0	6.0	1.25	3.00
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	5.0	6.0	3.00	2.50
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	6.0	3.00	2.25
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	4852	0	4852	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 2-10	
F	4851	0	4851	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 2-10	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	3924	2276 / 0	829 / 0	0 / 0	0 / 0	819 / 0	0 / 0
F	3923	2275 / 0	829 / 0	0 / 0	0 / 0	819 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT B-I, D-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4320 / 0	-84.3	-84.3	0.39 (1)	4.30	
B- C	-3379 / 0	-84.3	-84.3	0.32 (1)	4.80	
C- D	-3377 / 0	-84.3	-84.3	0.29 (1)	4.83	
D- E	-4057 / 0	-84.3	-84.3	0.33 (1)	4.46	
K- A	-4225 / 0	0.0	0.0	0.20 (1)	6.96	
F- E	-4271 / 0	0.0	0.0	0.24 (1)	6.93	
K- L	0 / 0	-28.0	-28.0	0.40 (1)	10.00	
L- M	0 / 0	-28.0	-28.0	0.40 (1)	10.00	
M- J	0 / 0	-28.0	-28.0	0.40 (1)	10.00	
J- N	0 / 3481	-28.0	-28.0	0.60 (1)	10.00	
N- O	0 / 3481	-28.0	-28.0	0.60 (1)	10.00	
O- I	0 / 3481	-28.0	-28.0	0.60 (1)	10.00	
I- P	0 / 3277	-28.0	-28.0	0.59 (1)	10.00	
P- H	0 / 3277	-28.0	-28.0	0.59 (1)	10.00	
H- Q	0 / 3277	-28.0	-28.0	0.59 (1)	10.00	
Q- G	0 / 3277	-28.0	-28.0	0.59 (1)	10.00	
G- R	0 / 0	-28.0	-28.0	0.38 (1)	10.00	
R- S	0 / 0	-28.0	-28.0	0.38 (1)	10.00	
S- F	0 / 0	-28.0	-28.0	0.38 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 1000	0.12 (1)
B- I	-1258 / 0	0.46 (1)
I- C	0 / 3672	0.32 (1)
I- D	-982 / 0	0.36 (1)
G- D	0 / 684	0.08 (1)
A- J	0 / 3762	0.47 (1)
G- E	0 / 3662	0.45 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	13-9-12	-728	-728	---	BACK	VERT	TOTAL
J	5-9-12	-728	-728	---	BACK	VERT	TOTAL
L	1-9-12	-728	-728	---	BACK	VERT	TOTAL
M	3-9-12	-728	-728	---	BACK	VERT	TOTAL
N	7-9-12	-728	-728	---	BACK	VERT	TOTAL
O	9-9-12	-728	-728	---	BACK	VERT	TOTAL
P	11-9-12	-728	-728	---	BACK	VERT	TOTAL
Q	15-9-12	-728	-728	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.39 (A-B:1) , BC=0.60 (I-J:1) , WB=0.47 (A-J:1) , SSI=0.43 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.45 (H) (INPUT = 1.00)

DWG NO. TAM 24464-16

STRUCTURAL COMPONENT ONLY

PO #

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T201	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 727.5 lbs FACTORED DOWN AT 1-9-12, 727.5 lbs FACTORED DOWN AT 3-9-12, 727.5 lbs FACTORED DOWN AT 5-9-12, 727.5 lbs FACTORED DOWN AT 7-9-12, 727.5 lbs FACTORED DOWN AT 9-9-12, 727.5 lbs FACTORED DOWN AT 11-9-12, 727.5 lbs FACTORED DOWN AT 13-9-12, 727.5 lbs FACTORED DOWN AT 15-9-12, AND 727.5 lbs FACTORED DOWN AT 17-9-12, AND 727.5 lbs FACTORED DOWN AT 19-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	17-9-12	-728	-728	---	BACK	VERT	TOTAL
S	19-9-12	-728	-728	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

P62

DWG NO. TAM24464-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T202	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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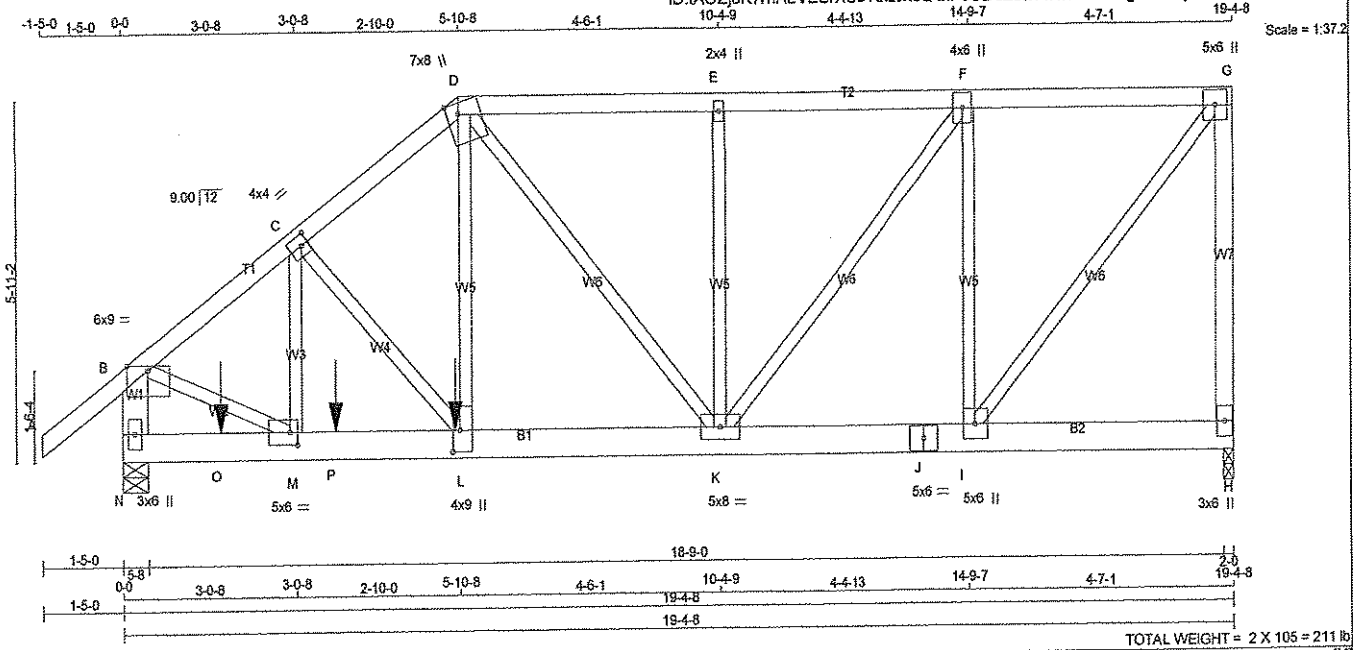


TOTAL WEIGHT = 2 X 105 = 211 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
G- H	1	12	TOP
N- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- J	2	12	SIDE(183.1)
J- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
L- D	1	3	SIDE(1075.2)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	7.0	8.0	Edge	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMVW+p	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW+t	MT20	4.0	9.0	4.25	1.50
M	BMWW-t	MT20	5.0	6.0	2.50	1.50
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2910	0	2910	0	0	2-0	1-9
N	6587	0	6587	0	0	5-8	5-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2364	1354 / 0	511 / 0	0 / 0	0 / 0	499 / 0	0 / 0
N	5320	3097 / 0	1116 / 0	0 / 0	0 / 0	1107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 38	-84.3	-84.3	0.07 (1)	10.00
B- C	-6623 / 0	-84.3	-84.3	0.24 (1)	3.65
C- D	-6414 / 0	-84.3	-84.3	0.21 (1)	3.72
D- E	-3937 / 0	-84.3	-84.3	0.18 (1)	4.63
E- F	-3937 / 0	-84.3	-84.3	0.18 (1)	4.62
F- G	-2224 / 0	-84.3	-84.3	0.14 (1)	5.83
H- G	-2849 / 0	0.0	0.0	0.89 (1)	6.80
N- B	-6162 / 0	0.0	0.0	0.22 (1)	5.97
N- O	0 / 0	-28.0	-28.0	0.25 (1)	10.00
O- M	0 / 0	-28.0	-28.0	0.25 (1)	10.00
M- P	0 / 5297	-28.0	-28.0	0.49 (1)	10.00
P- L	0 / 5297	-28.0	-28.0	0.49 (1)	10.00
L- K	0 / 5214	-28.0	-28.0	0.41 (1)	10.00
K- J	0 / 2224	-28.0	-28.0	0.17 (1)	10.00
J- I	0 / 2224	-28.0	-28.0	0.17 (1)	10.00
I- H	0 / 0	-28.0	-28.0	0.04 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	0 / 149	0.02 (3)
C- L	-275 / 0	0.05 (1)
L- D	0 / 5143	0.64 (1)
B- M	0 / 5838	0.70 (1)
I- G	0 / 3491	0.43 (1)
D- K	-2022 / 0	0.95 (1)
I- F	-2575 / 0	0.67 (1)
K- E	-390 / 0	0.10 (1)
K- F	0 / 2757	0.34 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	5-9-8	-4823	-4823	---	BACK	VERT	TOTAL
O	1-8-12	-1186	-1186	---	BACK	VERT	TOTAL
P	3-8-12	-1186	-1186	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.89 (G-H:1) , BC=0.49 (L-M:1) , WB=0.95 (D-K:1) , SSI=0.38 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.71 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5216
S. KATSOULAKOS
PROVINCE OF ONTARIO

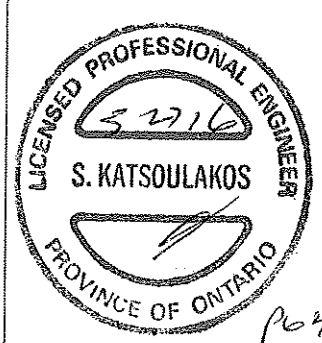
DWG NO. TAM2446516
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T202	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1186.0 lbs FACTORED DOWN AT 1-8-12, AND 1186.0 lbs FACTORED DOWN AT 3-8-12, AND 4823.3 lbs FACTORED DOWN AT 5-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 24465-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267492	T203	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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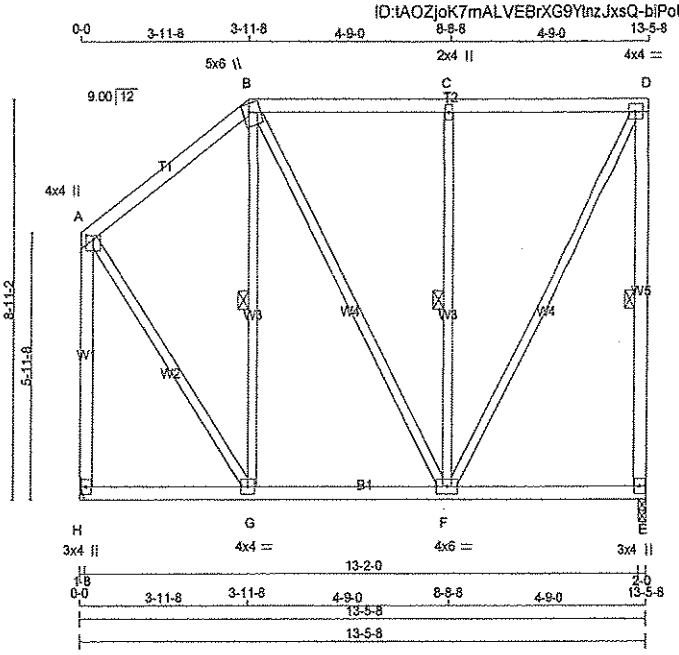

Scale = 1:50.5

TOTAL WEIGHT = 82 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.75
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	756	0	756	0	0	2-0	1-8
H	756	0	756	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	620	344 / 0	141 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E, B-G, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-318 / 0	-84.3	-84.3	0.23 (1)	6.25
B- C	-302 / 0	-84.3	-84.3	0.31 (1)	6.25
C- D	-302 / 0	-84.3	-84.3	0.31 (1)	6.25
E- D	-700 / 0	0.0	0.0	0.26 (1)	6.25
H- A	-711 / 0	0.0	0.0	0.51 (1)	7.81
H- G	0 / 0	-28.0	-28.0	0.12 (2)	10.00
G- F	0 / 252	-28.0	-28.0	0.18 (2)	10.00
F- E	0 / 0	-28.0	-28.0	0.15 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-220 / 42	0.11 (1)
B- F	0 / 106	0.02 (1)
F- C	-494 / 0	0.25 (1)
F- D	0 / 623	0.14 (1)
A- G	0 / 434	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.51 (A-H:1) , BC=0.18 (F-G:2) , WB=0.25 (C-F:1) , SSI=0.19 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.17 (F) (INPUT = 1.00)

DWG NO. TAM-2/46616
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	PB1	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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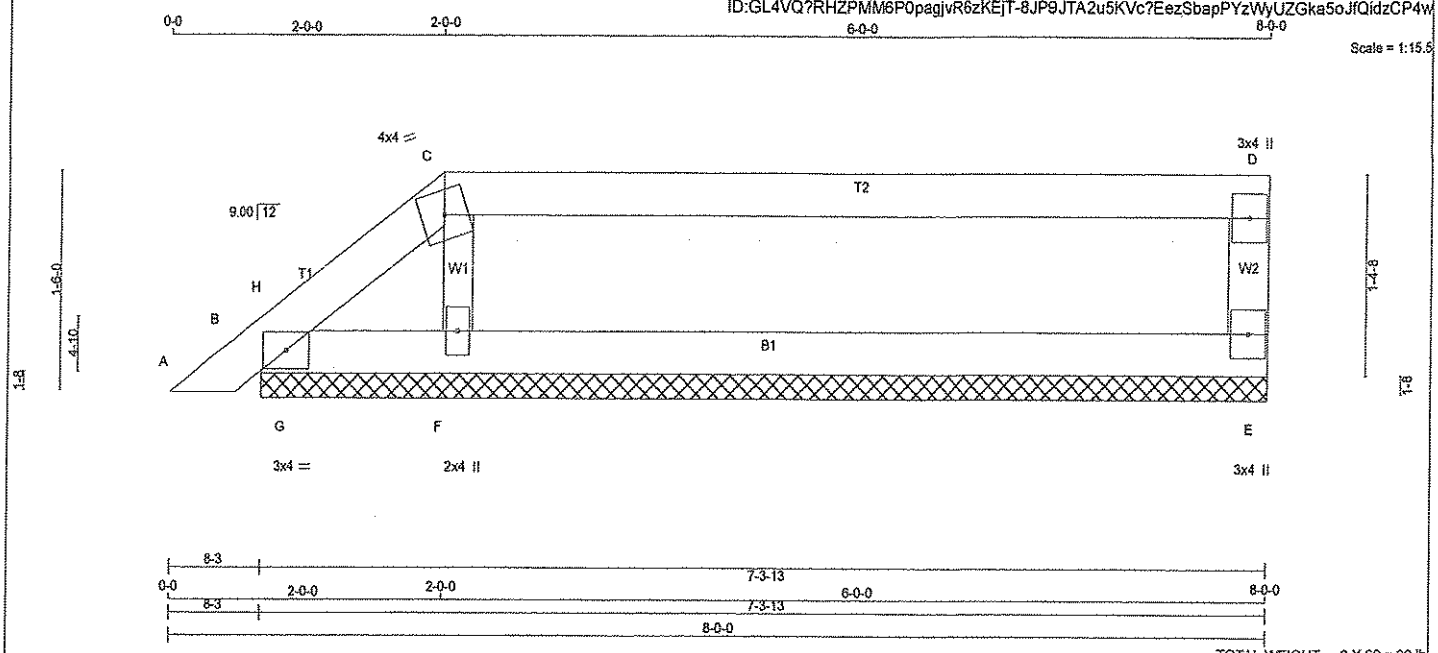

TOTAL WEIGHT = 2 X 20 = 39 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	318	0	318	0	0	7-3-13	1-8
B	75	0	75	0	-79	7-3-13	1-8
F	479	0	479	0	0	7-3-13	1-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	253	153 / 0	49 / 0	0 / 0	0 / 0	51 / 0	0 / 0
B	22	77 / 0	0 / -39	0 / 0	0 / 0	0 / -17	0 / 0
F	434	174 / 0	144 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 14	-84.3	-84.3	0.02 (1)	10.00	F-C	-255 / 11	0.04 (1)	
B-H	0 / 103	-84.3	-84.3	0.07 (3)	10.00	G-H	-208 / 0	0.00 (1)	
H-C	-46 / 0	-84.3	-84.3	0.05 (2)	6.25				
C-D	0 / 0	-84.3	-84.3	0.52 (1)	10.00				
E-D	-253 / 0	0.0	0.0	0.03 (1)	7.81				
B-G	-1 / 25	-28.0	-28.0	0.03 (1)	10.00				
G-F	-1 / 25	-28.0	-28.0	0.24 (2)	10.00				
F-E	0 / 0	-28.0	-28.0	0.24 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.52 (C-D:1) , BC=0.24 (F-G:2) , WB=0.04 (C-F:1) , SSI=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (C) (INPUT = 0.90)
JSI METAL= 0.05 (F) (INPUT = 1.00)

DWG NO. TAM 24422-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	PB2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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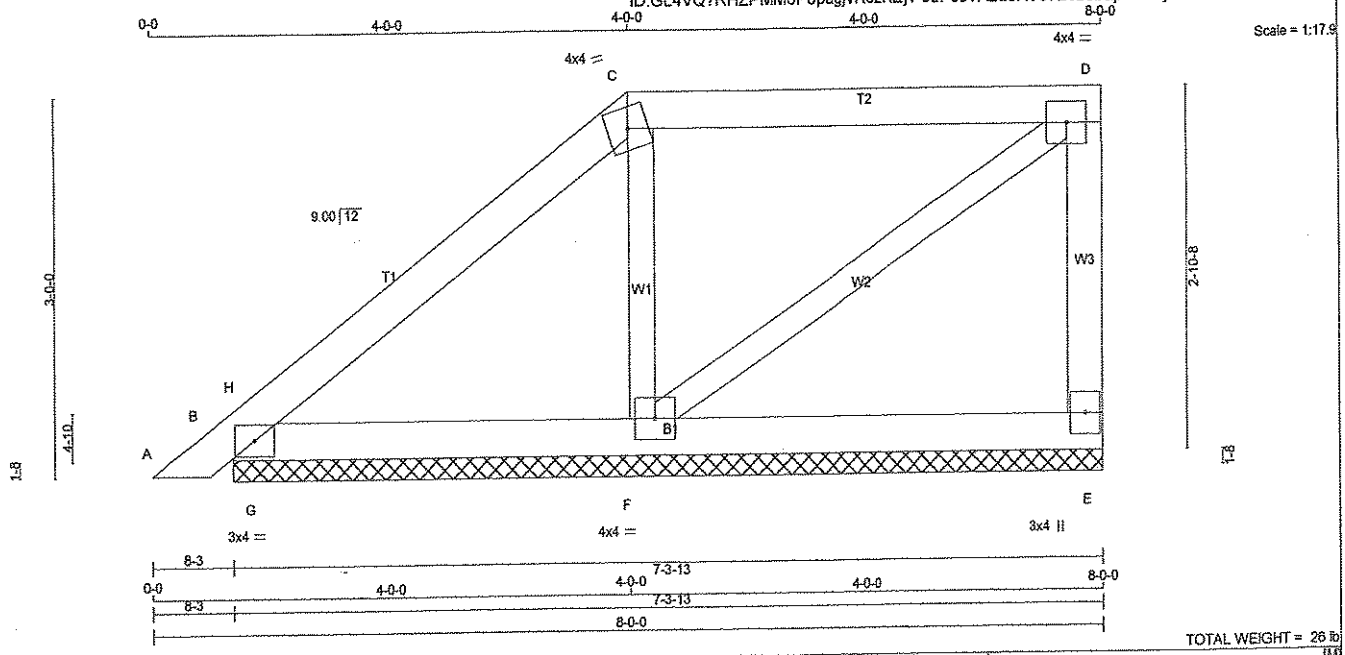

TOTAL WEIGHT = 26 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	231	0	231	0	0	7-3-13	1-8
B	256	0	256	0	0	7-3-13	1-8
F	386	0	386	0	0	7-3-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	182	113 / 0	34 / 0	0 / 0	0 / 0	36 / 0	0 / 0
B	195	133 / 0	28 / 0	0 / 0	0 / 0	34 / 0	0 / 0
F	332	160 / 0	92 / 0	0 / 0	0 / 0	80 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00
B- H	-43 / 12	-84.3	-84.3	0.04 (1)	6.25
H- C	-64 / 0	-84.3	-84.3	0.11 (1)	6.25
C- D	-36 / 0	-84.3	-84.3	0.23 (1)	6.25
E- D	-192 / 0	0.0	0.0	0.03 (1)	7.81
B- G	0 / 46	-28.0	-28.0	0.10 (1)	10.00
G- F	0 / 46	-28.0	-28.0	0.12 (2)	10.00
F- E	0 / 0	-28.0	-28.0	0.12 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-253 / 0	0.04 (1)
F- D	0 / 43	0.01 (1)
G- H	-250 / 38	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23 (C-D:1) , BC=0.12 (F-G:2) , WB=0.04 (C-F:1) , SSI=0.20 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.04 (C) (INPUT = 1.00)

DWG NO. TAM24423-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	PB3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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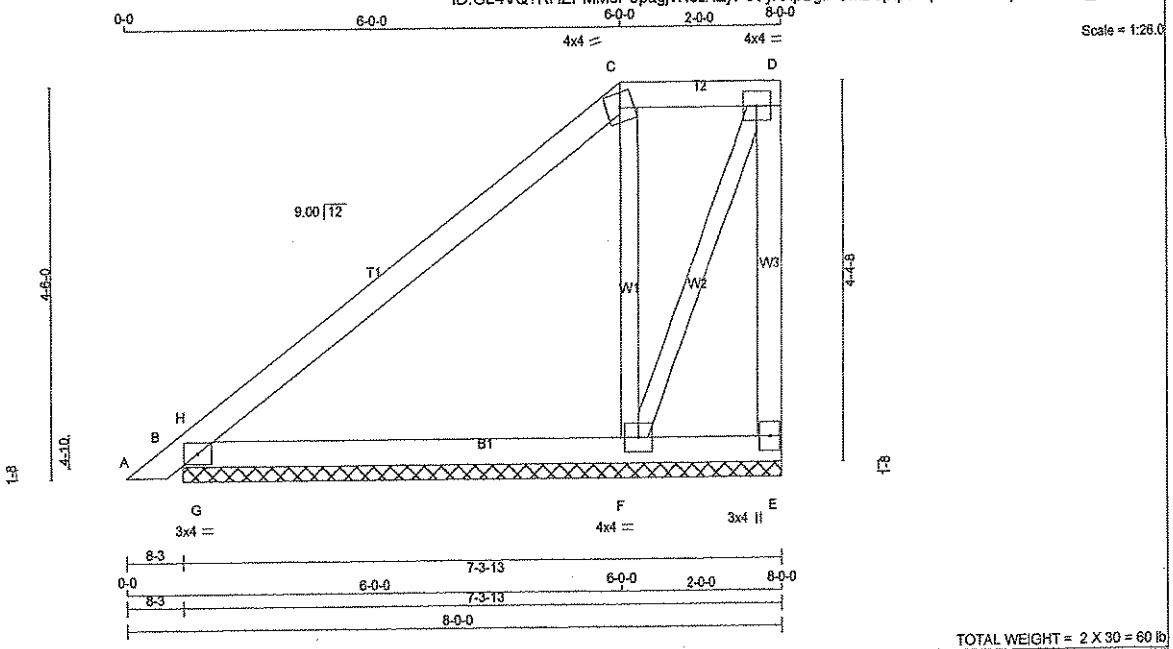

Scale = 1:26.0

TOTAL WEIGHT = 2 X 30 = 60 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	120	0	120	0	0	7-3-13	1-8
B	359	0	359	0	0	7-3-13	1-8
F	393	0	393	0	0	7-3-13	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	85	69 / 0	5 / 0	0 / 0	0 / 0	11 / 0	0 / 0
B	283	176 / 0	52 / 0	0 / 0	0 / 0	55 / 0	0 / 0
F	340	160 / 0	97 / 0	0 / 0	0 / 0	83 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00	F- C	-244 / 0	0.07 (1)	
B- H	-136 / 190	-84.3	-84.3	0.16 (1)	6.25	F- D	0 / 91	0.02 (1)	
H- C	-68 / 0	-84.3	-84.3	0.30 (1)	6.25	G- H	-599 / 156	0.00 (1)	
C- D	-38 / 0	-84.3	-84.3	0.06 (1)	6.25				
E- D	-167 / 0	0.0	0.0	0.05 (1)	7.81				
B- G	0 / 45	-28.0	-28.0	0.23 (1)	10.00				
G- F	0 / 45	-28.0	-28.0	0.24 (1)	10.00				
F- E	0 / 0	-28.0	-28.0	0.20 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30 (C-H:1) , BC=0.24 (F-G:1) , WB=0.07 (C-F:1) , SSI=0.48 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5/27/16
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 24424-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	PB4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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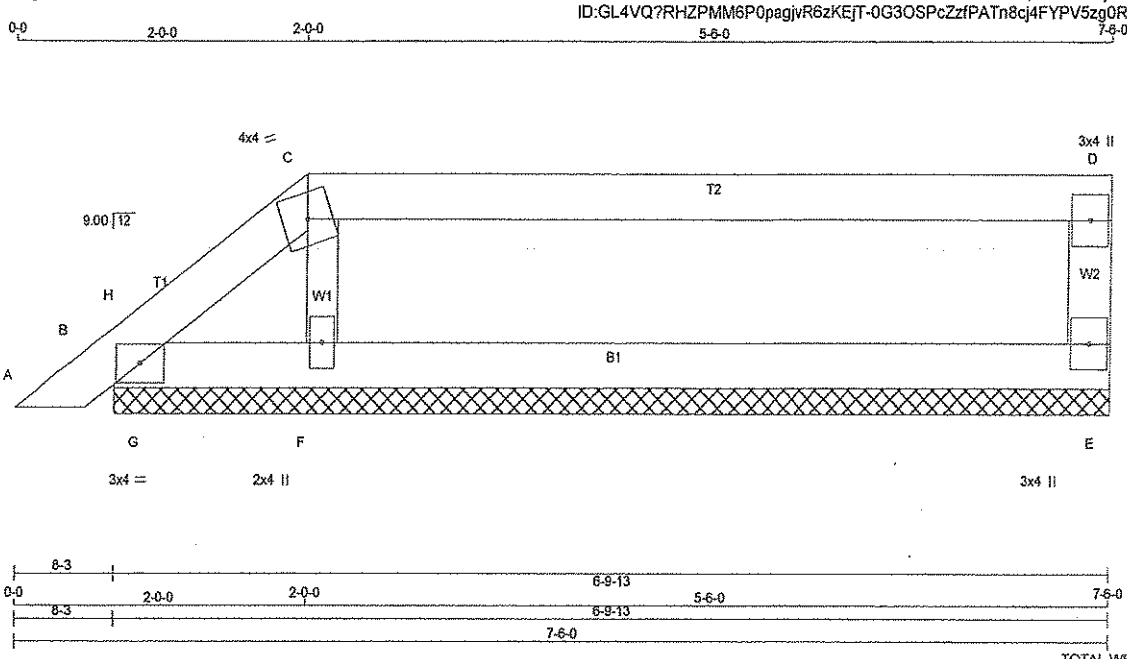


TOTAL WEIGHT = 2 X 18 = 37 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	291	0	291	0	0	6-9-13	1-8
B	86	0	86	0	-58	6-9-13	1-8
F	438	0	438	0	0	6-9-13	1-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	232	141 / 0	45 / 0	0 / 0	0 / 0	46 / 0	0 / 0
B	36	76 / 0	0 / -30	0 / 0	0 / 0	0 / -11	0 / 0
F	395	163 / 0	128 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00	F- C	-240 / 5	0.03 (1)
B- H	0 / 82	-84.3	-84.3	0.06 (3)	10.00	G- H	-171 / 0	0.00 (1)
H- C	-41 / 0	-84.3	-84.3	0.04 (2)	6.25			
C- D	0 / 0	-84.3	-84.3	0.43 (1)	10.00			
E- D	-232 / 0	0.0	0.0	0.02 (1)	7.81			
B- G	0 / 23	-28.0	-28.0	0.03 (1)	10.00			
G- F	0 / 23	-28.0	-28.0	0.20 (2)	10.00			
F- E	0 / 0	-28.0	-28.0	0.20 (2)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.43 (C-D:1) , BC=0.20 (F-G:2) , WB=0.03 (C-F:1) , SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

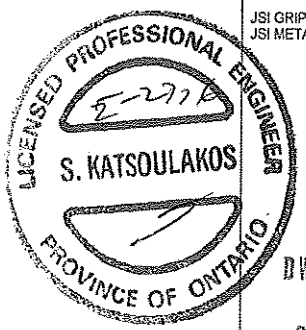
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (C) (INPUT = 0.90)
JSI METAL= 0.04 (F) (INPUT = 1.00)



DWG NO. TAM 24401-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	PB5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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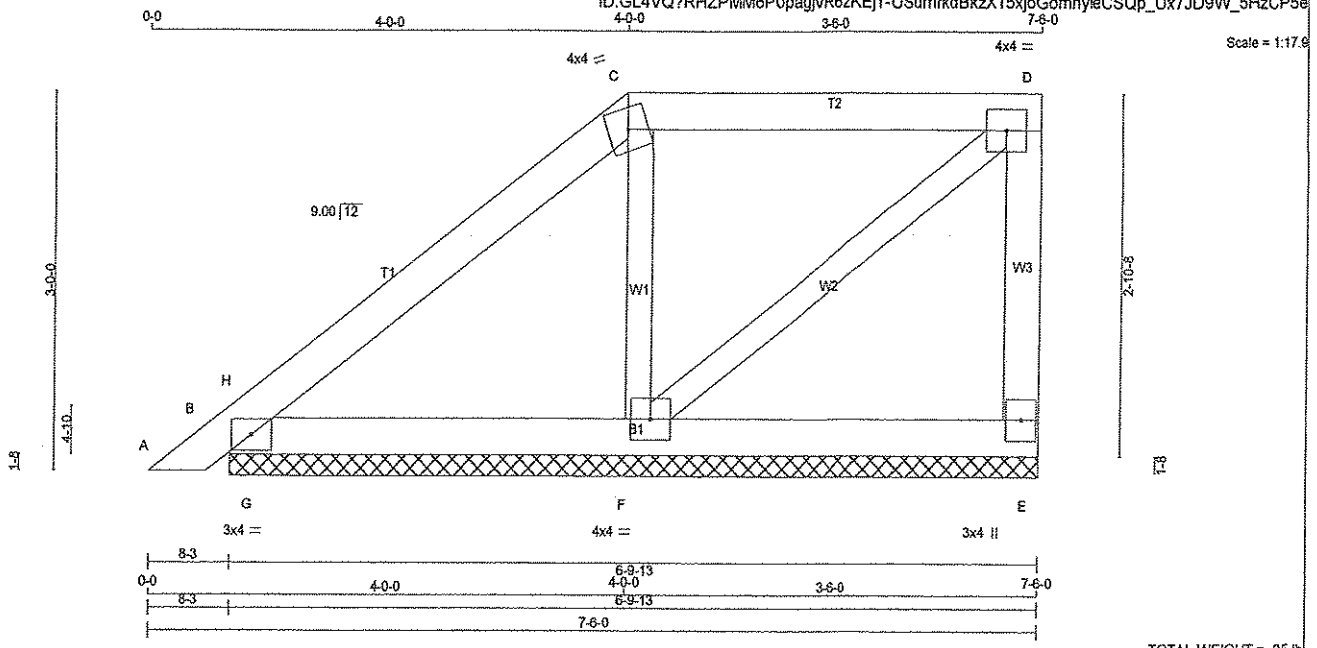


TOTAL WEIGHT = 25 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-t	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	205	0	205	0	0	6-9-13	1-8
B	256	0	256	0	0	6-9-13	1-8
F	355	0	355	0	0	6-9-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	161	100 / 0	30 / 0	0 / 0	0 / 0	31 / 0	0 / 0
B	197	131 / 0	30 / 0	0 / 0	0 / 0	35 / 0	0 / 0
F	305	148 / 0	84 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00	F- C	-233 / 0	0.04 (1)
B- H	-52 / 11	-84.3	-84.3	0.04 (1)	6.25	F- D	0 / 44	0.01 (1)
H- C	-61 / 0	-84.3	-84.3	0.11 (1)	6.25	G- H	-245 / 48	0.00 (1)
C- D	-35 / 0	-84.3	-84.3	0.18 (1)	6.25			
E- D	-174 / 0	0.0	0.0	0.03 (1)	7.81			
B- G	0 / 44	-28.0	-28.0	0.10 (1)	10.00			
G- F	0 / 44	-28.0	-28.0	0.10 (2)	10.00			
F- E	0 / 0	-28.0	-28.0	0.10 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18 (C-D:1) , BC=0.10 (F-G:2) , WB=0.04 (C-F:1) , SSI=0.20 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

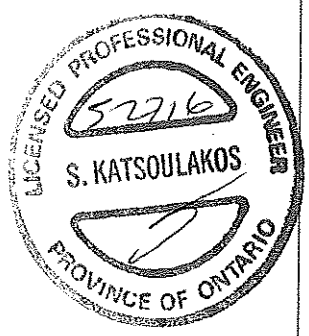
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)



DWG NO. TAM 24902-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	PB6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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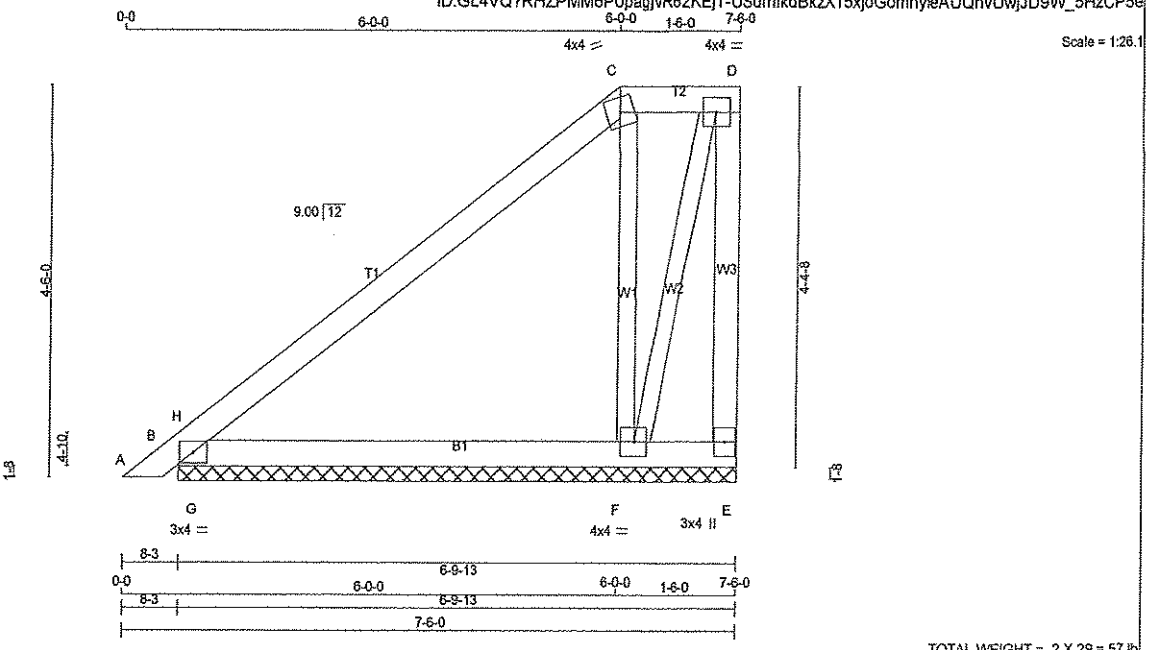


TOTAL WEIGHT = 2 X 29 = 57 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	48	0	48	0	-24	6-9-13	1-8
B	347	0	347	0	0	6-9-13	1-8
F	421	0	421	0	0	6-9-13	1-8

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	23	39 / 0	0 / -13	0 / 0	0 / 0	0 / -4	0 / 0
B	273	170 / 0	50 / 0	0 / 0	0 / 0	53 / 0	0 / 0
F	367	170 / 0	106 / 0	0 / 0	0 / 0	91 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00
B- H	-125 / 215	-84.3	-84.3	0.17 (1)	6.25
H- C	-50 / 0	-84.3	-84.3	0.30 (1)	6.25
C- D	-25 / 0	-84.3	-84.3	0.03 (1)	6.25
E- D	-135 / 0	0.0	0.0	0.04 (1)	7.81
B- G	0 / 30	-28.0	-28.0	0.23 (1)	10.00
G- F	0 / 30	-28.0	-28.0	0.24 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.21 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-233 / 0	0.07 (1)
F- D	0 / 76	0.02 (1)
G- H	-609 / 150	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30 (C-H:1) , BC=0.24 (F-G:1) , WB=0.07 (C-F:1) , SSI=0.49 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

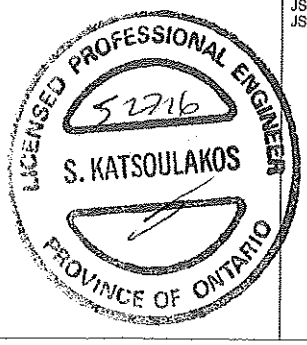
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)



DWG NO. TAM 24403-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	PB8	1	1	TRUSS DESC.	

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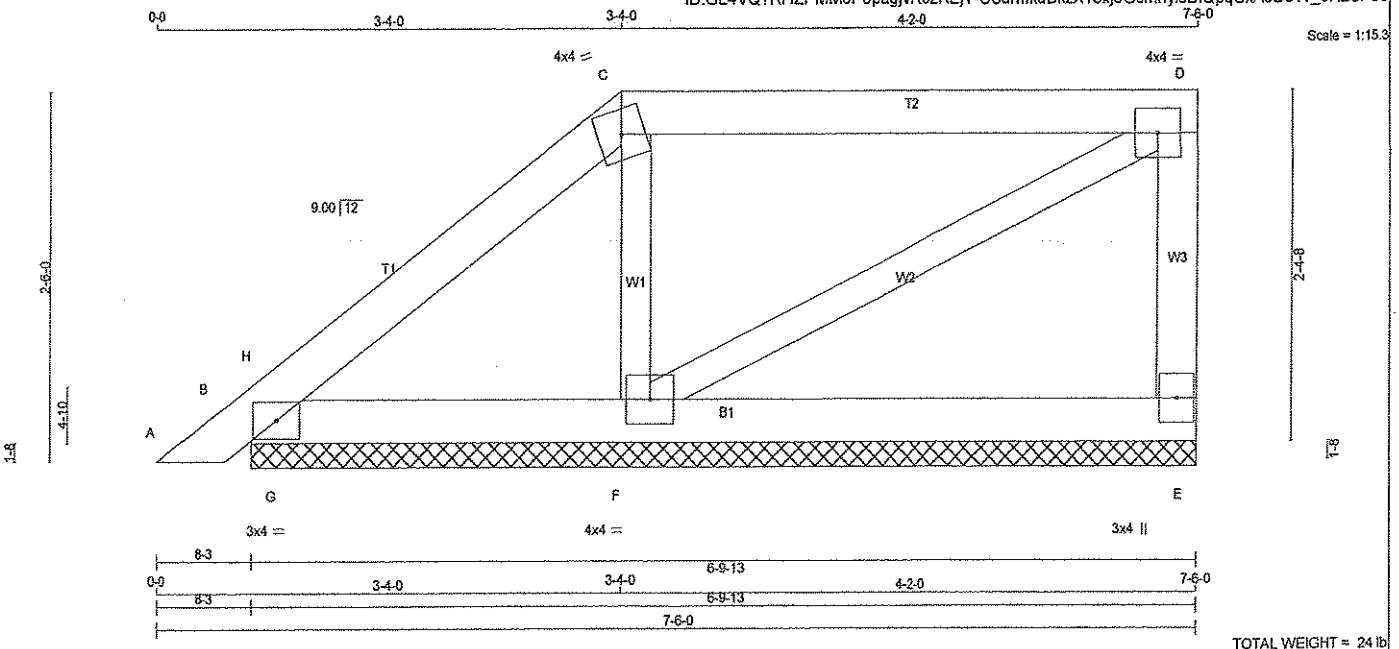

TOTAL WEIGHT = 24 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	231	0	231	0	0	6-9-13	1-8
B	210	0	210	0	0	6-9-13	1-8
F	375	0	375	0	0	6-9-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	183	112 / 0	35 / 0	0 / 0	0 / 0	36 / 0	0 / 0
B	155	113 / 0	17 / 0	0 / 0	0 / 0	25 / 0	0 / 0
F	324	154 / 0	91 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00
B- H	-10 / 0	-84.3	-84.3	0.02 (1)	6.25
H- C	-51 / 0	-84.3	-84.3	0.07 (1)	6.25
C- D	-25 / 0	-84.3	-84.3	0.25 (1)	6.25
E- D	-188 / 0	0.0	0.0	0.02 (1)	7.81
B- G	0 / 37	-28.0	-28.0	0.07 (1)	10.00
G- F	0 / 37	-28.0	-28.0	0.11 (2)	10.00
F- E	0 / 0	-28.0	-28.0	0.11 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-241 / 0	0.04 (1)
F- D	0 / 28	0.01 (1)
G- H	-180 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25 (C-D:1) , BC=0.11 (F-G:2) , WB=0.04 (C-F:1) , SSI=0.14 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM 24/404-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	PB9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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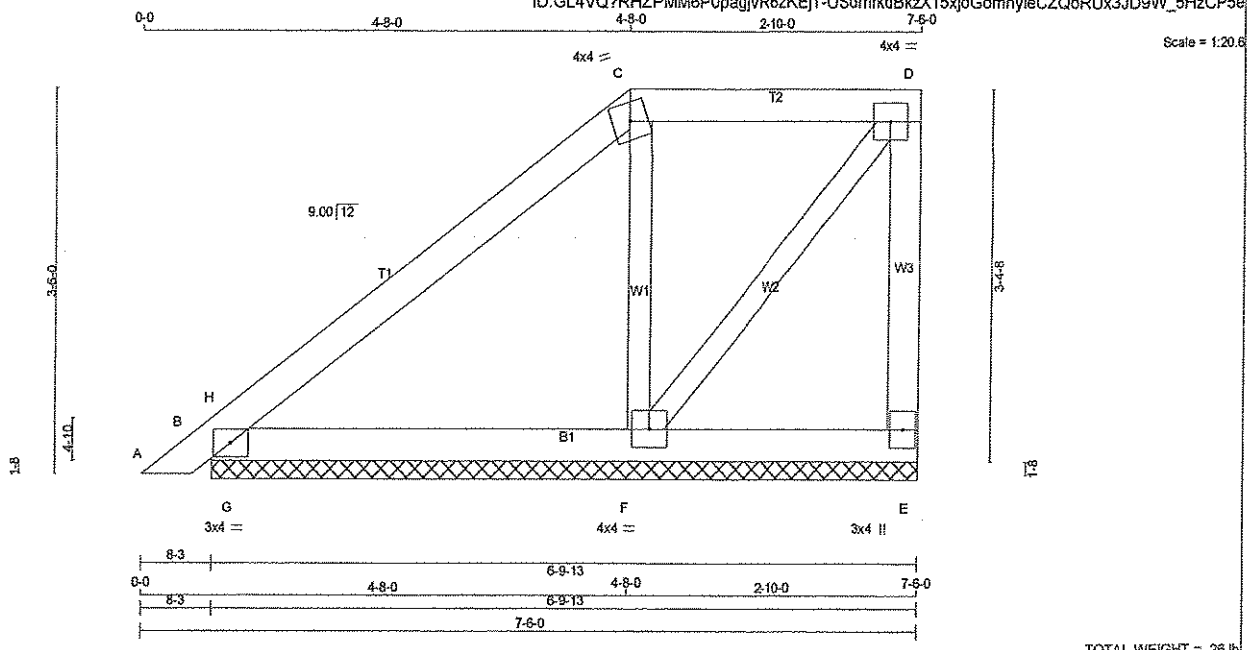

Scale = 1:20.6

TOTAL WEIGHT = 26 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	177	0	177	0	0	6-9-13	1-8
B	296	0	296	0	0	6-9-13	1-8
F	343	0	343	0	0	6-9-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	138	69 / 0	23 / 0	0 / 0	0 / 0	26 / 0	0 / 0
B	231	148 / 0	39 / 0	0 / 0	0 / 0	44 / 0	0 / 0
F	294	143 / 0	80 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00	
B- H	-87 / 48	-84.3	-84.3	0.07 (1)	6.25	
H- C	-69 / 0	-84.3	-84.3	0.17 (1)	6.25	
C- D	-41 / 0	-84.3	-84.3	0.12 (1)	6.25	
E- D	-166 / 0	0.0	0.0	0.03 (1)	7.81	
B- G	0 / 49	-28.0	-28.0	0.14 (1)	10.00	
G- F	0 / 49	-28.0	-28.0	0.14 (1)	10.00	
F- E	0 / 0	-28.0	-28.0	0.11 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
F- C	-227 / 0	0.04 (1)
F- D	0 / 62	0.01 (1)
G- H	-337 / 90	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (C-H:1) , BC=0.14 (F-G:1) , WB=0.04 (C-F:1) , SSI=0.28 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

DWG NO. TAM 24/05/16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	PB10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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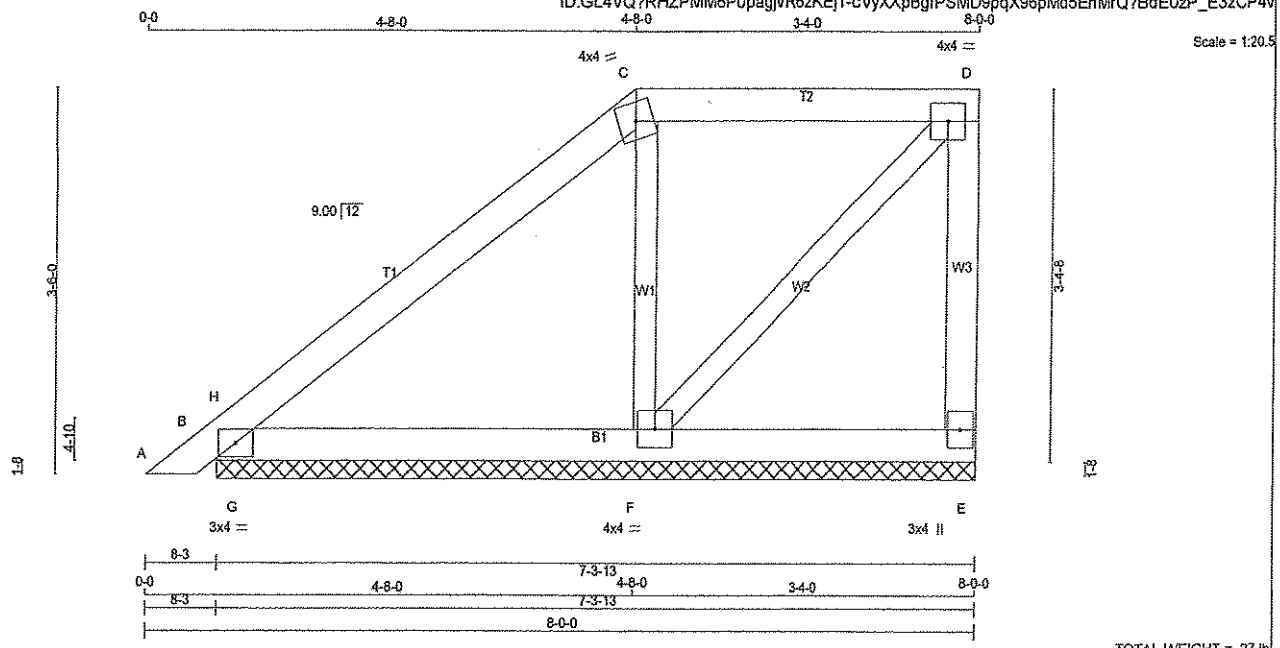

TOTAL WEIGHT = 27 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	206	0	206	0	0	7-3-13	1-8
B	299	0	299	0	0	7-3-13	1-8
F	367	0	367	0	0	7-3-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	161	102 / 0	29 / 0	0 / 0	0 / 0	31 / 0	0 / 0
B	232	150 / 0	39 / 0	0 / 0	0 / 0	43 / 0	0 / 0
F	315	153 / 0	86 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00		F- C	-245 / 0	0.05 (1)
B- H	-84 / 43	-84.3	-84.3	0.07 (1)	6.25		F- D	0 / 63	0.01 (1)
H- C	-75 / 0	-84.3	-84.3	0.17 (1)	6.25		G- H	-338 / 84	0.00 (1)
C- D	-45 / 0	-84.3	-84.3	0.16 (1)	6.25				
E- D	-183 / 0	0.0	0.0	0.03 (1)	7.81				
B- G	0 / 54	-28.0	-28.0	0.14 (1)	10.00				
G- F	0 / 54	-28.0	-28.0	0.14 (1)	10.00				
F- E	0 / 0	-28.0	-28.0	0.12 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (C-H:1) , BC=0.14 (F-G:1) , WB=0.05 (C-F:1) , SSI=0.28 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)

DWG NO. TAM 24428-16

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267486	PB11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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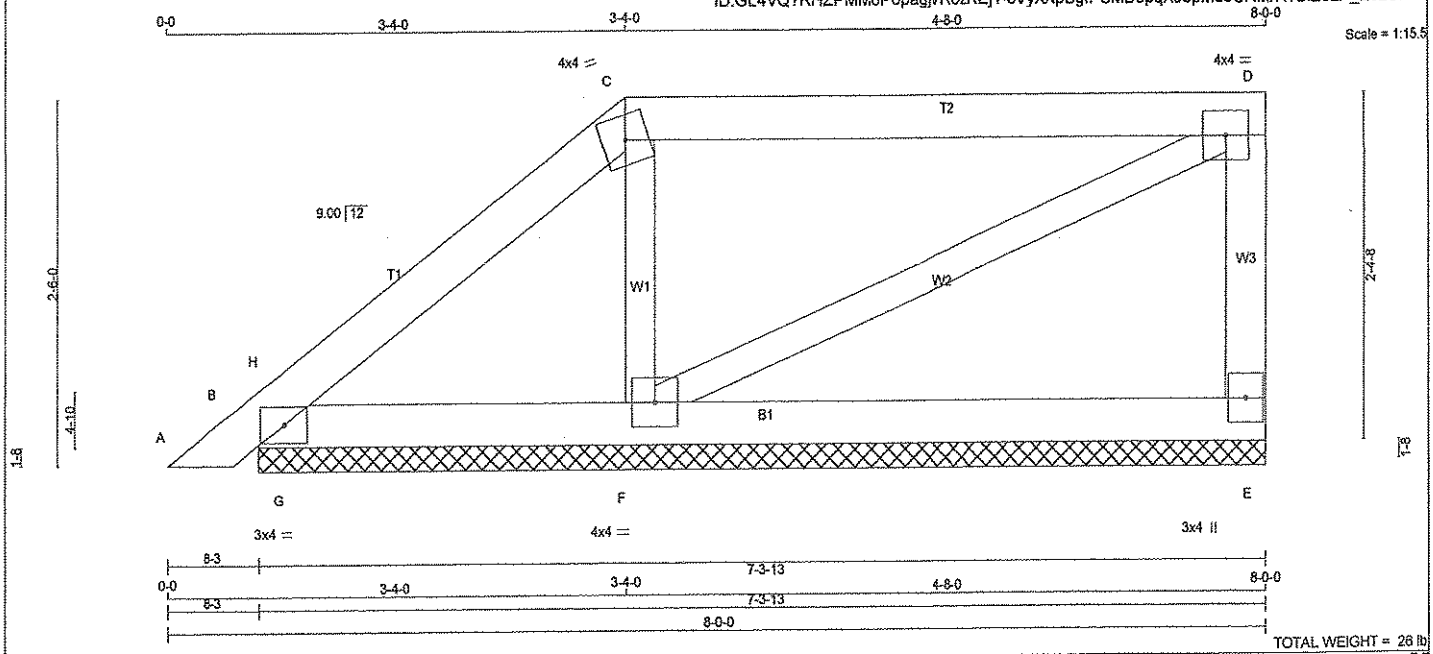

TOTAL WEIGHT = 26 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	257	0	257	0	0	7-3-13	1-8
B	207	0	207	0	0	7-3-13	1-8
F	408	0	408	0	0	7-3-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	204	125 / 0	39 / 0	0 / 0	0 / 0	40 / 0	0 / 0
B	151	114 / 0	14 / 0	0 / 0	0 / 0	23 / 0	0 / 0
F	354	166 / 0	101 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00
B- H	0 / 4	-84.3	-84.3	0.03 (1)	10.00
H- C	-52 / 0	-84.3	-84.3	0.07 (1)	6.25
C- D	-24 / 0	-84.3	-84.3	0.31 (1)	6.25
E- D	-208 / 0	0.0	0.0	0.03 (1)	7.81
B- G	0 / 37	-28.0	-28.0	0.07 (1)	10.00
G- F	0 / 37	-28.0	-28.0	0.14 (2)	10.00
F- E	0 / 0	-28.0	-28.0	0.14 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-260 / 0	0.04 (1)
F- D	0 / 27	0.01 (1)
G- H	-190 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31 (C-D:1) , BC=0.14 (F-G:2) , WB=0.04 (C-F:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.04 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM24426-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267485	PB12	1	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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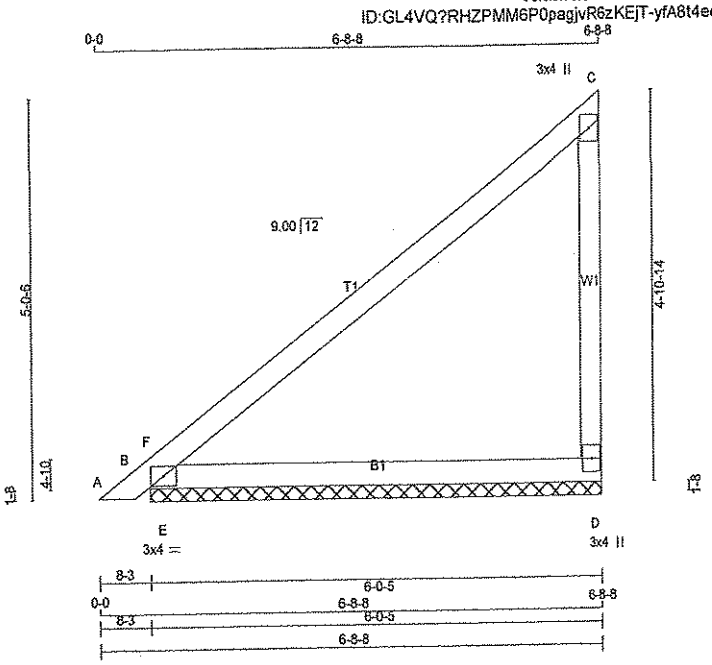

TOTAL WEIGHT = 21 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	338	0	338	0	0	6-0-5	1-8
B	388	0	388	0	0	6-0-5	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	278	154 / 0	63 / 0	0 / 0	0 / 0	60 / 0	0 / 0
B	312	185 / 0	63 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00
B- F	-226 / 245	-84.3	-84.3	0.17 (1)	6.25
F- C	-10 / 9	-84.3	-84.3	0.41 (1)	10.00
D- C	-226 / 0	0.0	0.0	0.09 (1)	7.81
B- E	0 / 0	-28.0	-28.0	0.25 (1)	10.00
E- D	0 / 0	-28.0	-28.0	0.31 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-624 / 318	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.41 (C-F:1) , BC=0.31 (D-E:1) , WB=0.00 (E-F:1) , SSI=0.53 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO . TAM24406-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267485	PB13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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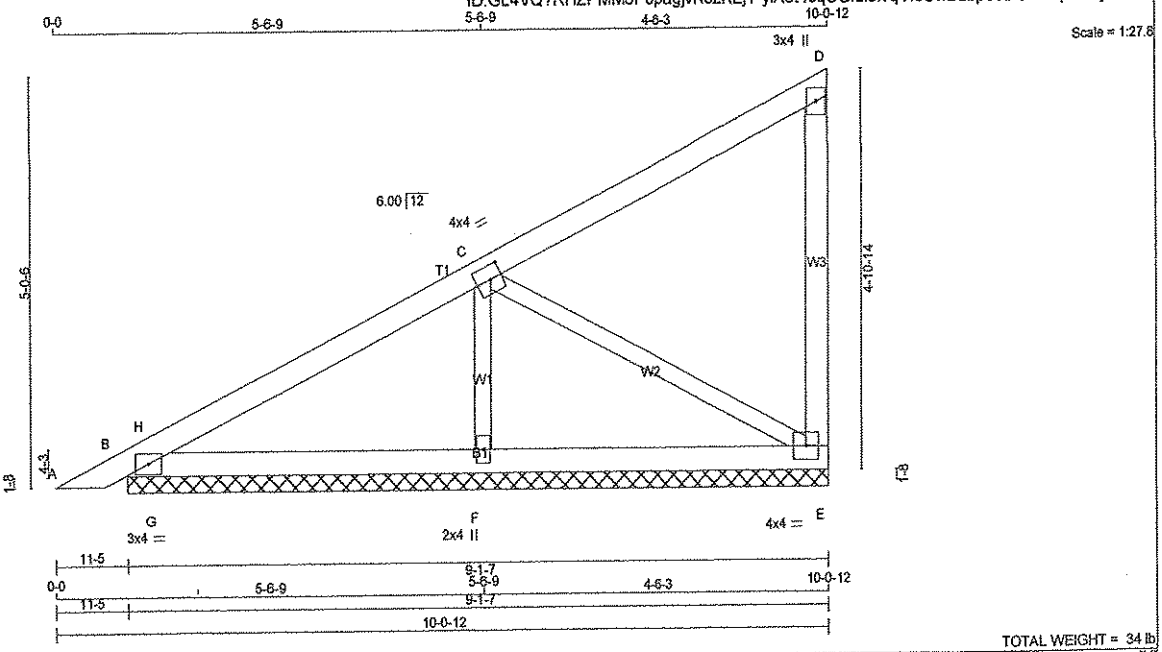

TOTAL WEIGHT = 34 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	232	0	232	0	0	9-1-7	1-8
B	311	0	311	0	0	9-1-7	1-8
F	556	0	556	0	0	9-1-7	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	186	110 / 0	38 / 0	0 / 0	0 / 0	38 / 0	0 / 0
B	241	157 / 0	39 / 0	0 / 0	0 / 0	44 / 0	0 / 0
F	464	245 / 0	115 / 0	0 / 0	0 / 0	105 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.3	-84.3	0.05 (1)	10.00
B- H	-93 / 0	-84.3	-84.3	0.05 (3)	6.25
H- C	-70 / 0	-84.3	-84.3	0.28 (1)	6.25
C- D	-20 / 0	-84.3	-84.3	0.28 (1)	6.25
E- D	-145 / 0	0.0	0.0	0.06 (1)	7.81
B- G	0 / 82	-28.0	-28.0	0.11 (1)	10.00
G- F	0 / 82	-28.0	-28.0	0.18 (2)	10.00
F- E	0 / 82	-28.0	-28.0	0.18 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-377 / 0	0.06 (1)
C- E	-92 / 0	0.04 (1)
G- H	-142 / 88	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28 (C-H:1) , BC=0.18 (F-G:2) , WB=0.06 (C-F:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (C) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
5216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 24/07/16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	PB20	3	1	TRUSS DESC.	

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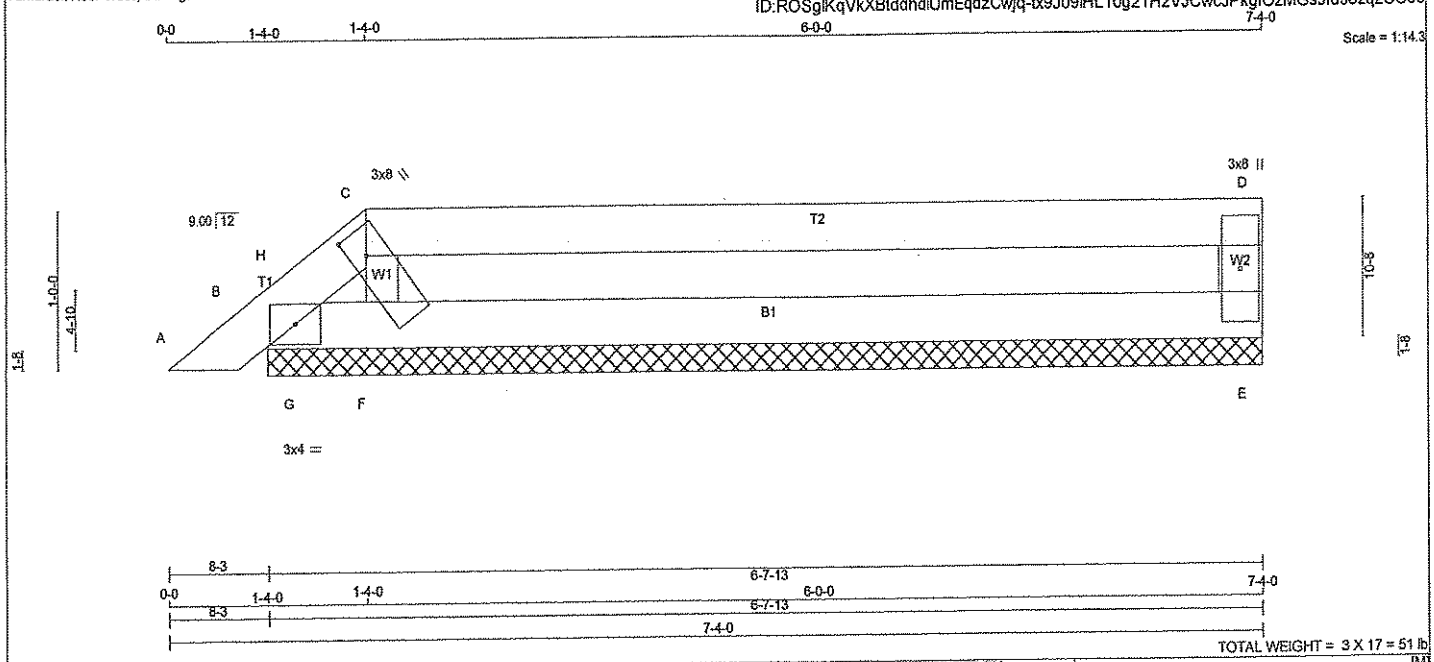

TOTAL WEIGHT = 3 X 17 = 51 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTBW*+h	MT20	3.0	8.0	2.00	1.25
D	TMBMV+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	317	0	317	0	0	6-7-13	1-8
B	0	0	0	0	-225	6-7-13	1-8
F	480	0	538	0	0	6-7-13	1-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 225 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	251	154 / 0	48 / 0	0 / 0	0 / 0	50 / 0	0 / 0
B	0	82 / 0	0 / -101	0 / 0	0 / 0	0 / -58	0 / 0
F	473	135 / 0	193 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00
B- H	0 / 145	-84.3	-84.3	0.09 (3)	10.00
H- C	-93 / 0	-84.3	-84.3	0.07 (2)	6.25
C- D	0 / 0	-84.3	-84.3	0.52 (1)	10.00
E- D	-253 / 0	0.0	0.0	0.03 (1)	7.81
B- G	-17 / 29	-28.0	-28.0	0.02 (3)	6.25
G- F	-17 / 29	-28.0	-28.0	0.26 (2)	6.25
F- E	0 / 0	-28.0	-28.0	0.26 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-157 / 92	0.02 (1)
G- H	-313 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.52 (C-D:1) , BC=0.26 (F-G:2) , WB=0.02 (C-F:1) , SSI=0.30 (F-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (C) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

DWG NO. TAM 24453-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267489	PB21	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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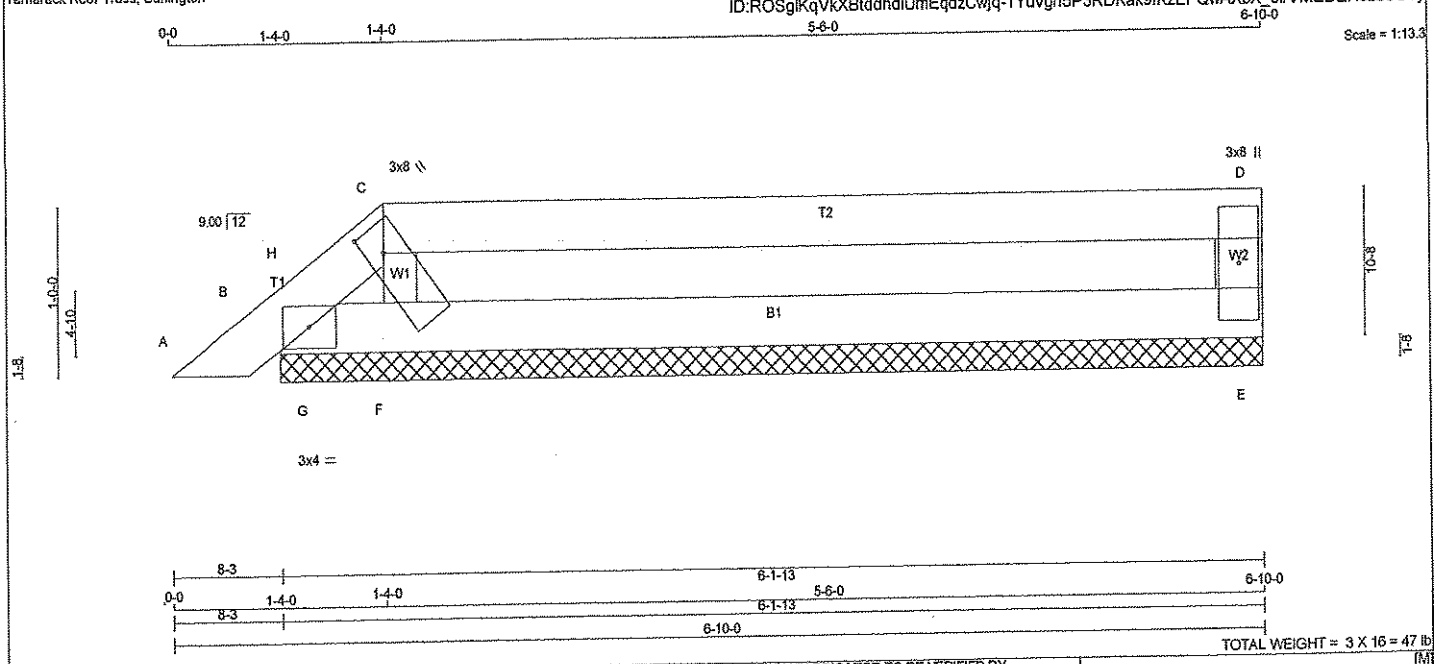

TOTAL WEIGHT = 3 X 16 = 47 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTBW*+h	MT20	3.0	8.0	2.00	1.25
D	TMBMV+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	290	0	290	0	0	6-1-13	1-8
B	22	0	22	0	-182	6-1-13	1-8
F	429	0	473	0	0	6-1-13	1-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 182 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	230	141 / 0	44 / 0	0 / 0	0 / 0	46 / 0	0 / 0
B	-48	81 / 0	0 / -83	0 / 0	0 / 0	0 / -46	0 / 0
F	419	124 / 0	168 / 0	0 / 0	0 / 0	127 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	0.02 (1)	10.00	F- C	-151 / 74	0.02 (1)
B- H	0 / 119	-84.3	-84.3	0.07 (3)	10.00	G- H	-258 / 0	0.00 (1)
H- C	-83 / 0	-84.3	-84.3	0.06 (2)	6.25			
C- D	0 / 0	-84.3	-84.3	0.43 (1)	10.00			
E- D	-232 / 0	0.0	0.0	0.02 (1)	7.81			
B- G	-13 / 27	-28.0	-28.0	0.01 (3)	6.25			
G- F	-13 / 27	-28.0	-28.0	0.22 (2)	6.25			
F- E	0 / 0	-28.0	-28.0	0.22 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.43 (C-D:1) , BC=0.22 (F-G:2) , WB=0.02 (C-F:1) , SSI=0.25 (F-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (C) (INPUT = 0.50)
JSI METAL= 0.06 (C) (INPUT = 1.00)

DWG NO. TAM2444016
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42161	DRWG NO.
267489	PB22	2	1	TRUSS DESC.	

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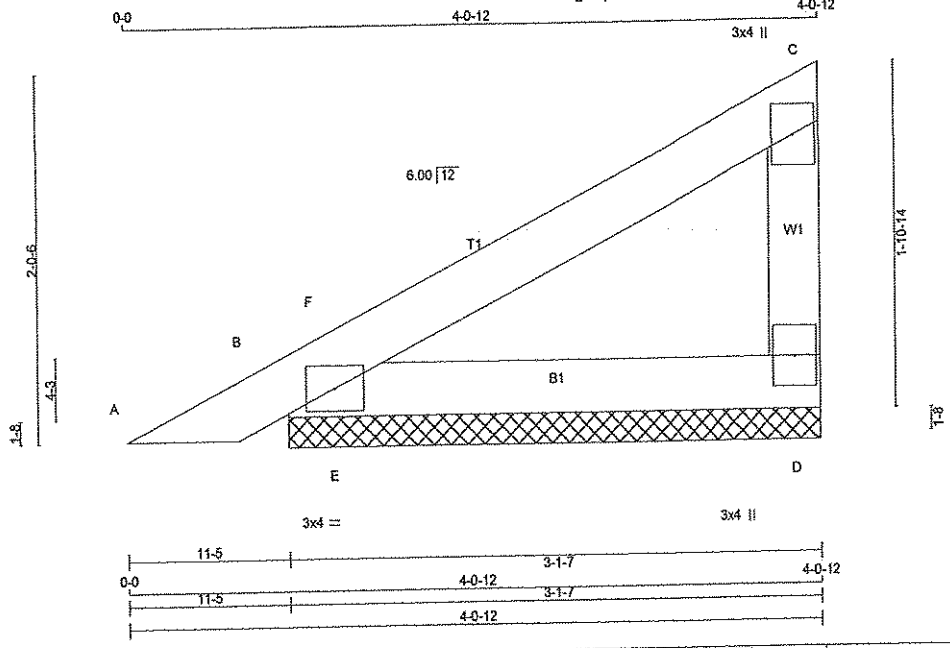
Tamarack Roof Truss, Burlington

ID:ROSglKqVkXBtddhdiUmEqdzCwjq-xlSHu161qlLBBuksuhlUz7jnPxLKRI4VTt_rbPzCOox



Scale = 1:12.5

TOTAL WEIGHT = 2 X 10 = 20 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	175	0	175	0	0	3-1-7	1-8
B	249	0	249	0	0	3-1-7	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	144	80 / 0	33 / 0	0 / 0	0 / 0	31 / 0	0 / 0
B	194	125 / 0	33 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-84.3	-84.3	0.05 (1)	10.00
B- F	-37 / 0	-84.3	-84.3	0.04 (3)	6.25
F- C	0 / 5	-84.3	-84.3	0.11 (1)	10.00
D- C	-116 / 0	0.0	0.0	0.01 (1)	7.81
B- E	0 / 0	-28.0	-28.0	0.09 (1)	10.00
E- D	0 / 0	-28.0	-28.0	0.09 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-120 / 72	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (C-F:1) , BC=0.09 (D-E:1) , WB=0.00 (E-F:1) , SSI=0.12 (B-E:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM 2441-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267490	PB22	2	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

ID:ROSgIKqVkXBtddhdiUmEqdzCwjq-tx9J09lHL10g21H2VJCwcJPr1IRkMGCJfdJ8zqzCOo6

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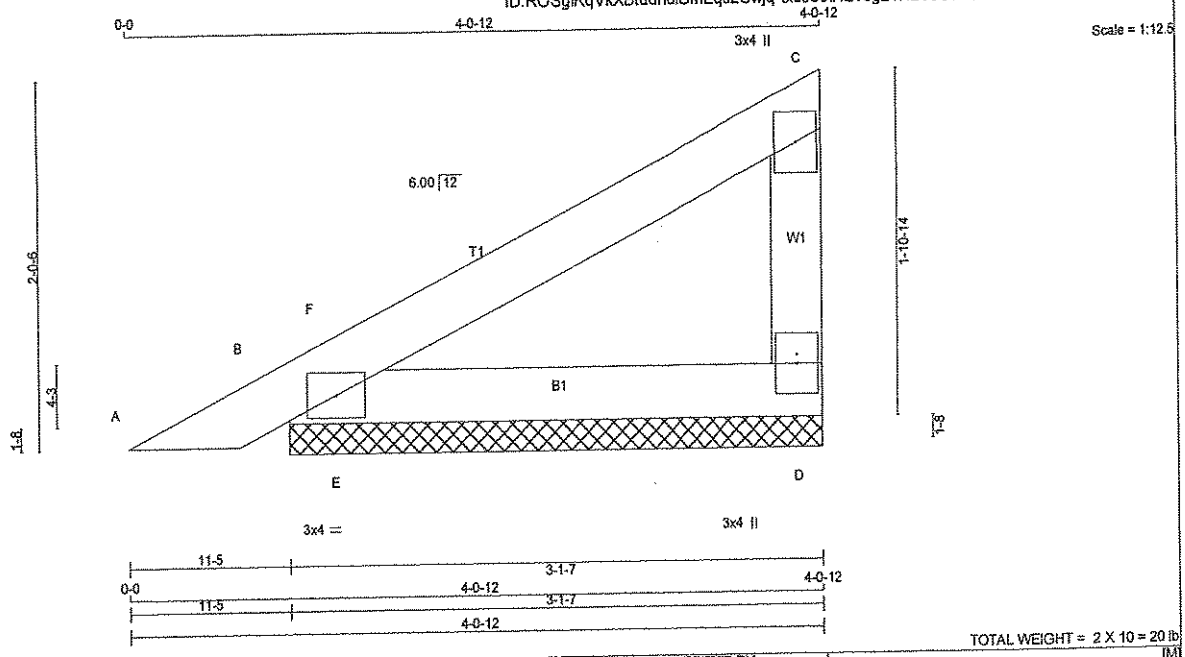

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[M]

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N. L. G. A. RULES

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A - C	2x4	DRY	No.2	SPF
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

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LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.3	-84.3	0.05 (1)	10.00	E- F	-120 / 72	0.00 (1)
B- F	-37 / 0	-84.3	-84.3	0.04 (3)	6.25			
F- C	0 / 5	-84.3	-84.3	0.11 (1)	10.00			
D- C	-116 / 0	0.0	0.0	0.01 (1)	7.81			
B- E	0 / 0	-28.0	-28.0	0.08 (1)	10.00			
E- D	0 / 0	-28.0	-28.0	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

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	DL	=	7.0	PSF
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JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM 24454-16
STRUCTURAL COMPONENT ONLY

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

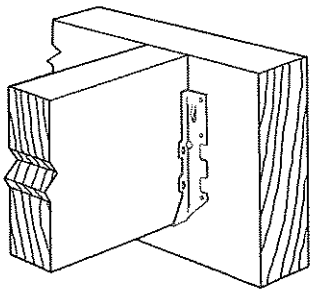
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

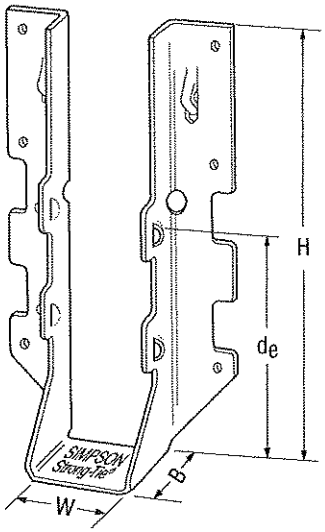
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



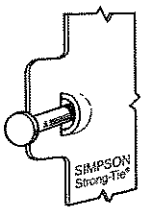
Typical LUS Installation



LUS28

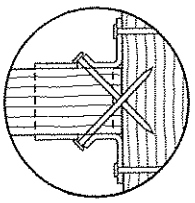
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

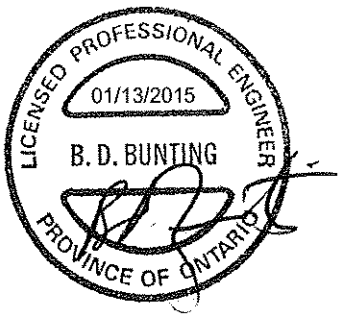


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

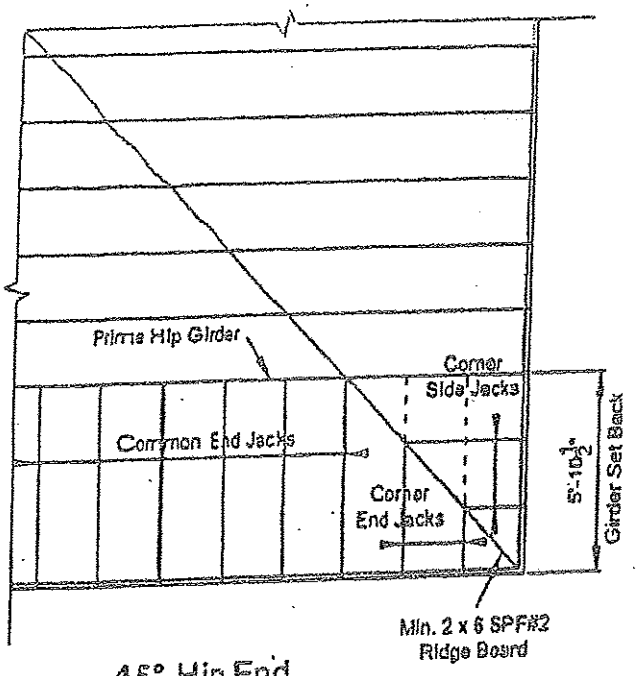


LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

800-999-5099
www.strongtie.com

MICRO CITY
ENGINEERING SERVICES INC. TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

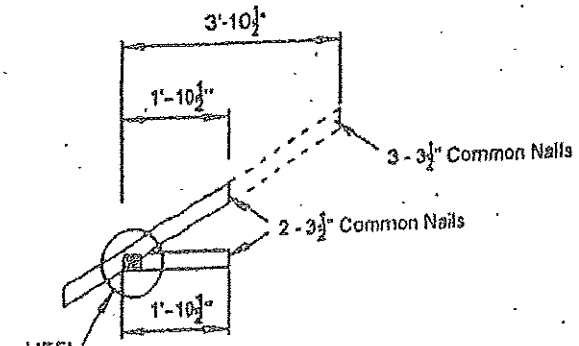
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

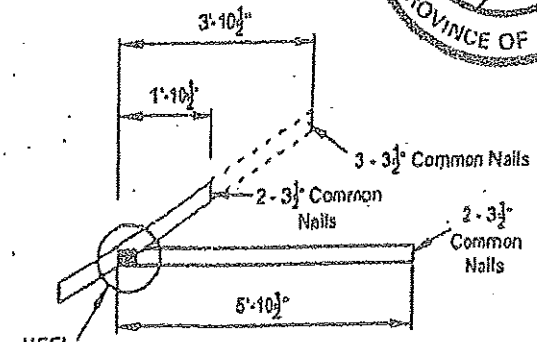
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495.14
STRUCTURAL COMPONENT ONLY

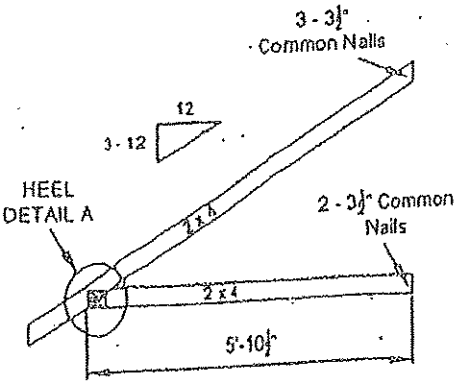
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



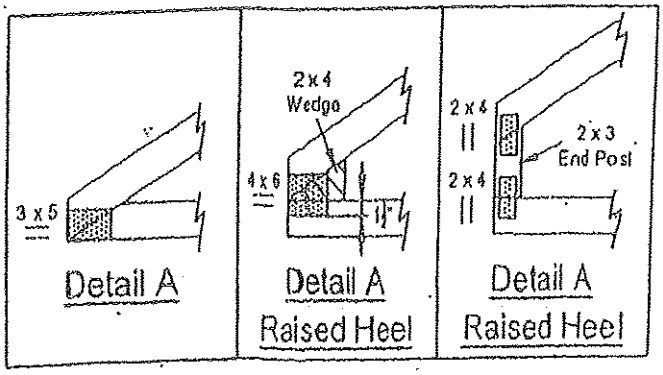
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

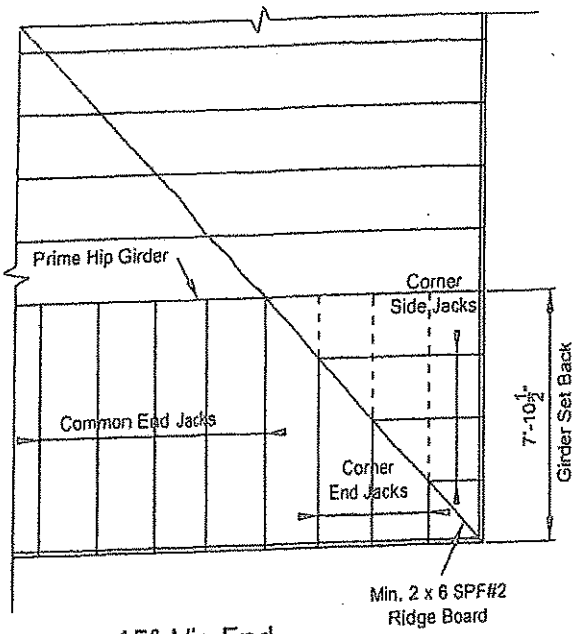
MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



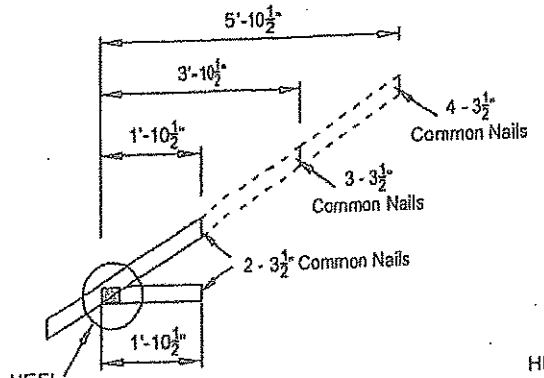
45° Hip End

DESIGN LOAD:

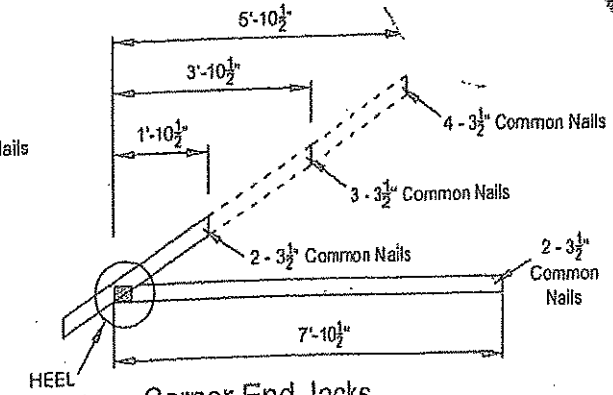
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3503 14
STRUCTURAL
COMPONENT ONLY

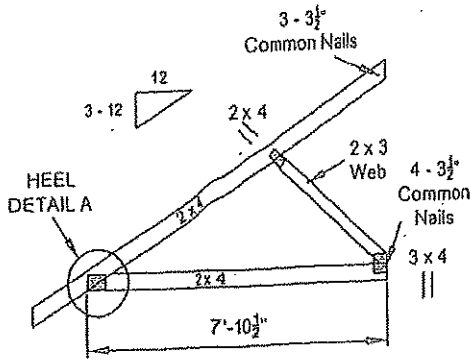
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



Corner Side Jacks



Corner End Jacks



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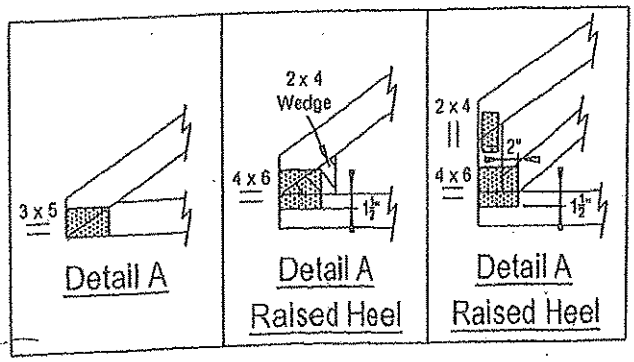
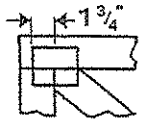


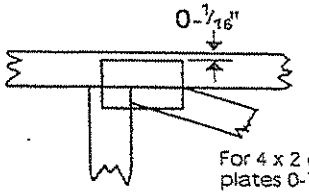
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

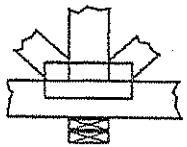
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

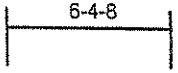
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

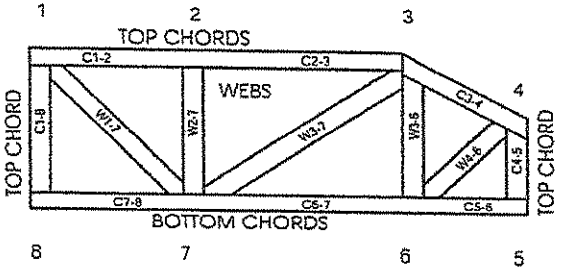
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

· CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

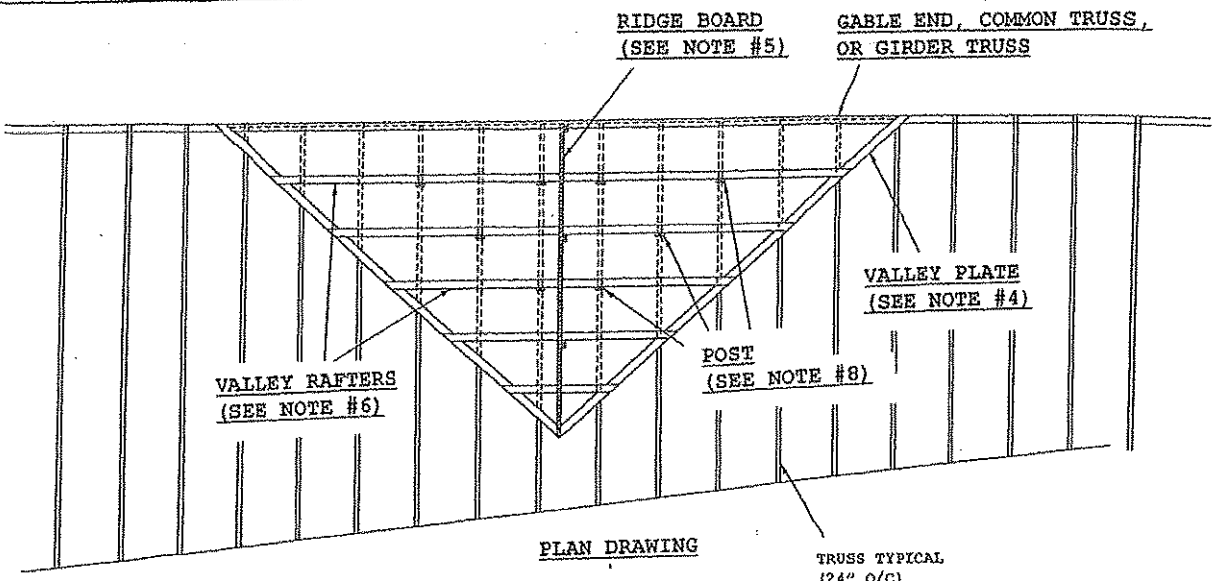
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

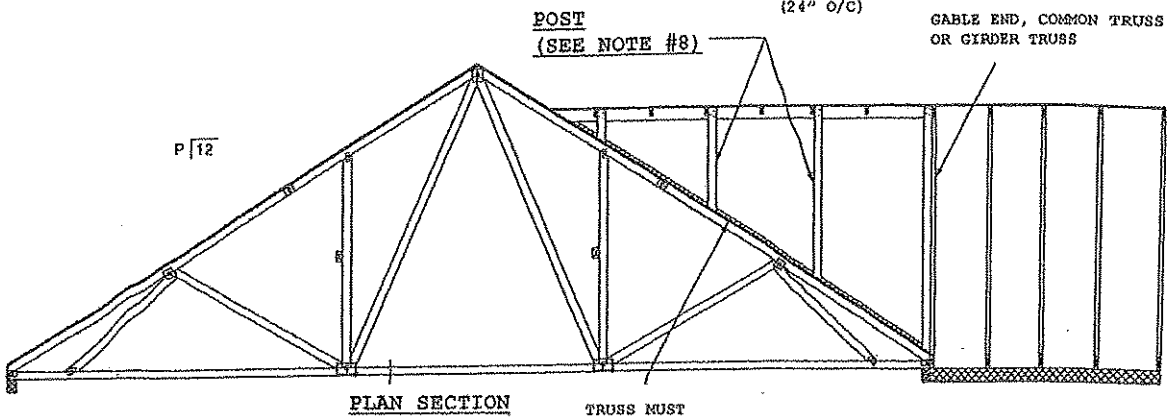
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING

TRUSS TYPICAL
(24" O/C)



PLAN SECTION

TRUSS MUST
BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

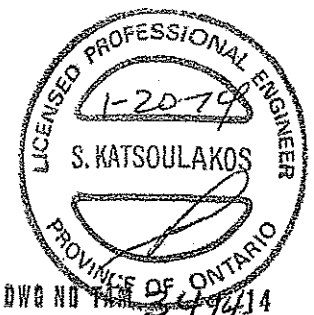
NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.6 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



DWG NO TMM-[illegible]-1-14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.