

T-160338

54-00-00

47-00-00

7-00-00

7/12

HIGH CLG.

5-00-00

5-07-00

10-04-00

7-02-00

6-03-00

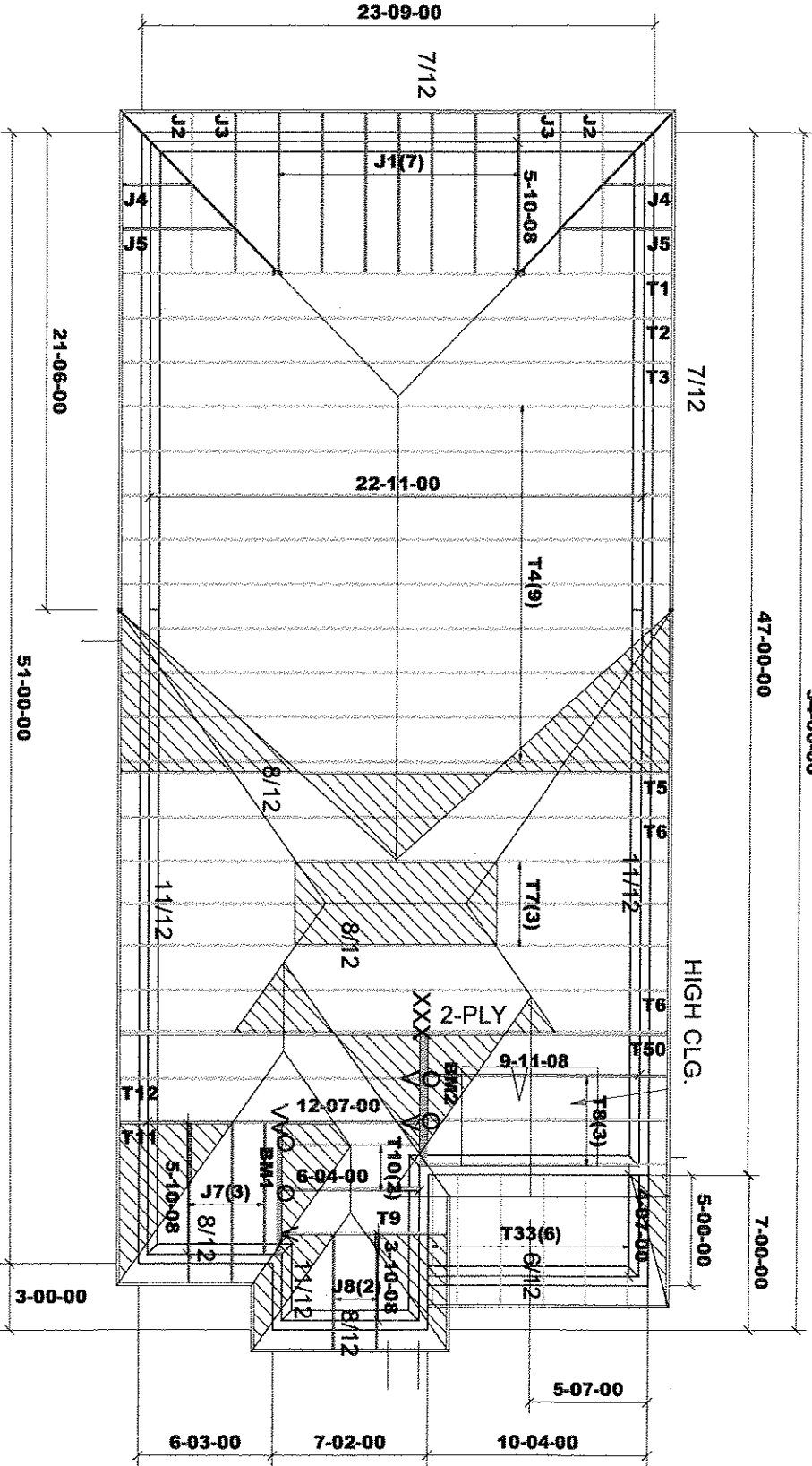
23-09-00

7/12

22-11-00

21-06-00

51-00-00



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL
ROOF FRAMING TO
CONFORM TO PART 9 OF
THE OBC LATEST EDITION
ROOF RAFTERS THAT
MEET OR CROSS OVER
TRUSSES ARE TO BE
2"x4"SPF@24"O.C WITH A
2"x4"SPF VERTICAL POST
TO THE TRUSS UNDER AT
EACH CROSS POINT.
POSTS LONGER THAN 6'
TO BE Laterally
BRACED SO THAT THE
DISTANCE BETWEEN END
POINTS AND BETWEEN
ROWS OF BRACING DOES
NOT EXCEED 6'.

DESIGN CONFORMS WITH
THE RELEVANT SECTION
OF THE LATEST EDITION
OF O.B.C. PART 9

DESIGN LOADS:
GROUND SNOW LOAD
SS= 1.5kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

Denotes
CONVENTIONAL
FRAMING



HARDWARE
LUS24(O)
LUS26DS(V)
LUS26-2(VV)
HGUS26-3(XXX)

BM1: 2-2X10
BM2: 3-2X10

M9965

Job Track: 42181
Layout ID: 267427
Plan Log: 86179

Builder / Location:
GREENPARK HOMES / VAUGHAN

Model / Elevation:
BRIDGEFORD 3 / 1 OR OPT.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

M9965

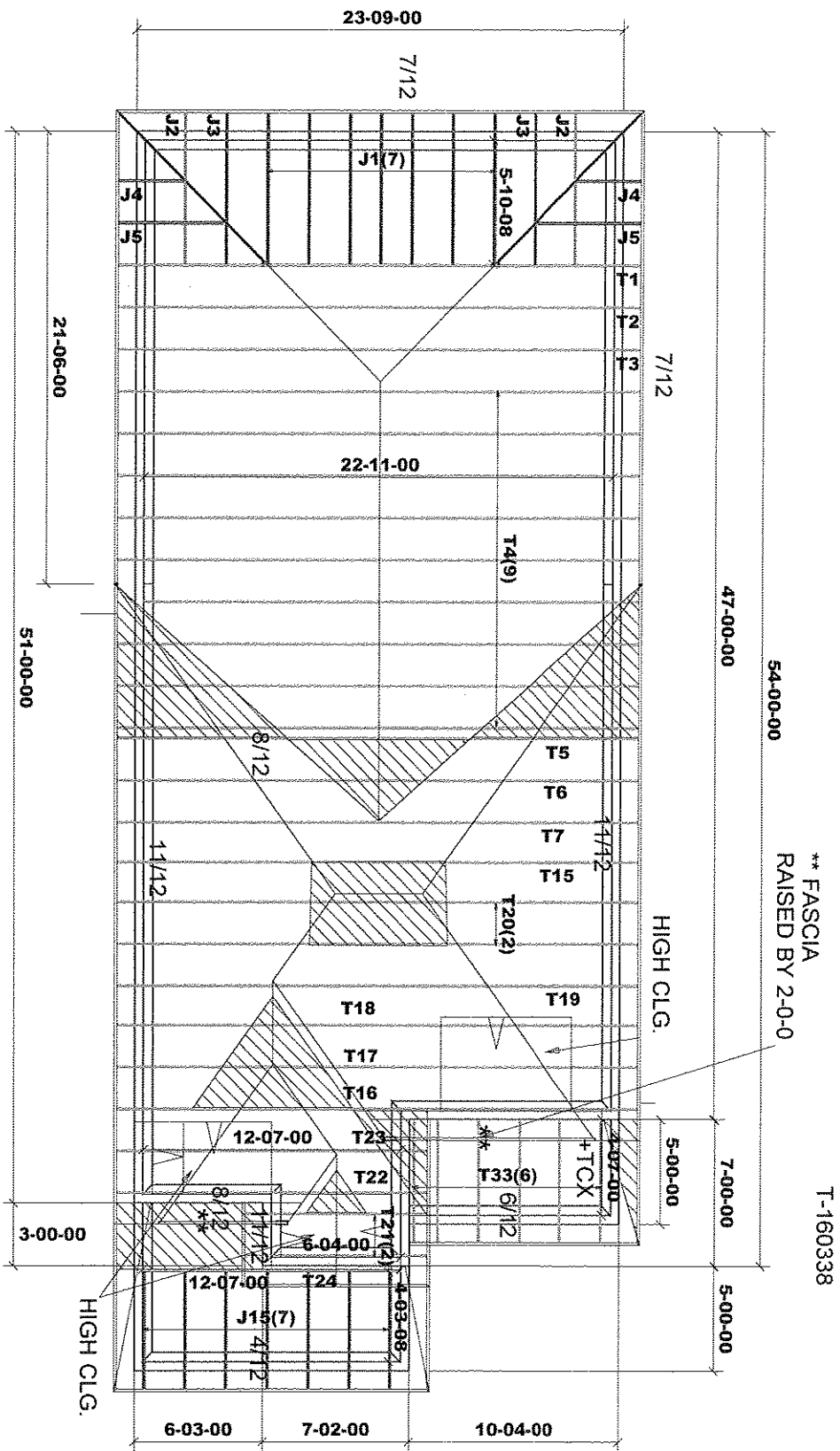


Job Track: 42181
Layout ID: 267428
Plan Log: 86179

Builder / Location: GREENPARK HOMES / VAUGHAN
Project: STARTIME HOME LTD
Date: 8/10/2016
Designer: JG

Model / Elevation: BRIDGEFORD 3 / 2 OR OPT.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
Mark ver 7.5.0



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

T-160338

** FASCIA
RAISED BY 2-0-0

HIGH CLG.

ALL CONVENTIONAL
ROOF FRAMING TO
CONFORM TO PART 9 OF
THE OBC, LATEST EDITION
ROOF RAFTERS THAT
MEET OR CROSS OVER
TRUSSES ARE TO BE
2"x4" SP@24" O.C. WITH A
2"x4" SP VERTICAL POST
TO THE TRUSS UNDER AT
EACH CROSS POINT.
POSTS LONGER THAN 6'
TO BE LATERALLY
BRACED SO THAT THE
DISTANCE BETWEEN END
POINTS AND BETWEEN
ROWS OF BRACING DOES
NOT EXCEED 6'.

DESIGN CONFORMS WITH
THE RELEVANT SECTION
OF THE LATEST EDITION
OF O.B.C. PART 9

DESIGN LOADS:
GROUND SNOW LOAD
SS= 1.5kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL
FRAMING

54-00-00

47-00-00

7112

7112

23-09-00

22-11-00

T4(b)

(S) 21

Y

HIGH CLG.

10-04-00

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL
ROOF FRAMING TO
CONFORM TO PART9 OF
THE OBC. LATEST EDITION
ROOF RAFTERS THAT
MEET OR CROSS OVER
TRUSSES ARE TO BE
2"x4"SPF@24"o.c.WITH A
2"x4"SPF VERTICAL POST
TO THE TRUSS UNDER AT
EACH CROSS POINT.
POSTS LONGER THAN 6'
TO BE LATEROALLY
BRACED SO THAT THE
DISTANCE BETWEEN END
POINTS AND BETWEEN
ROWS OF BRACING DOES
NOT EXCEED 6'.

DESIGN CONFORMS WITH
THE RELEVANT SECTION
OF THE LATEST EDITION
OF O.B.C. PART.9

DESIGN LOADS:

$$S_s = 1.5 \text{ kPa}$$

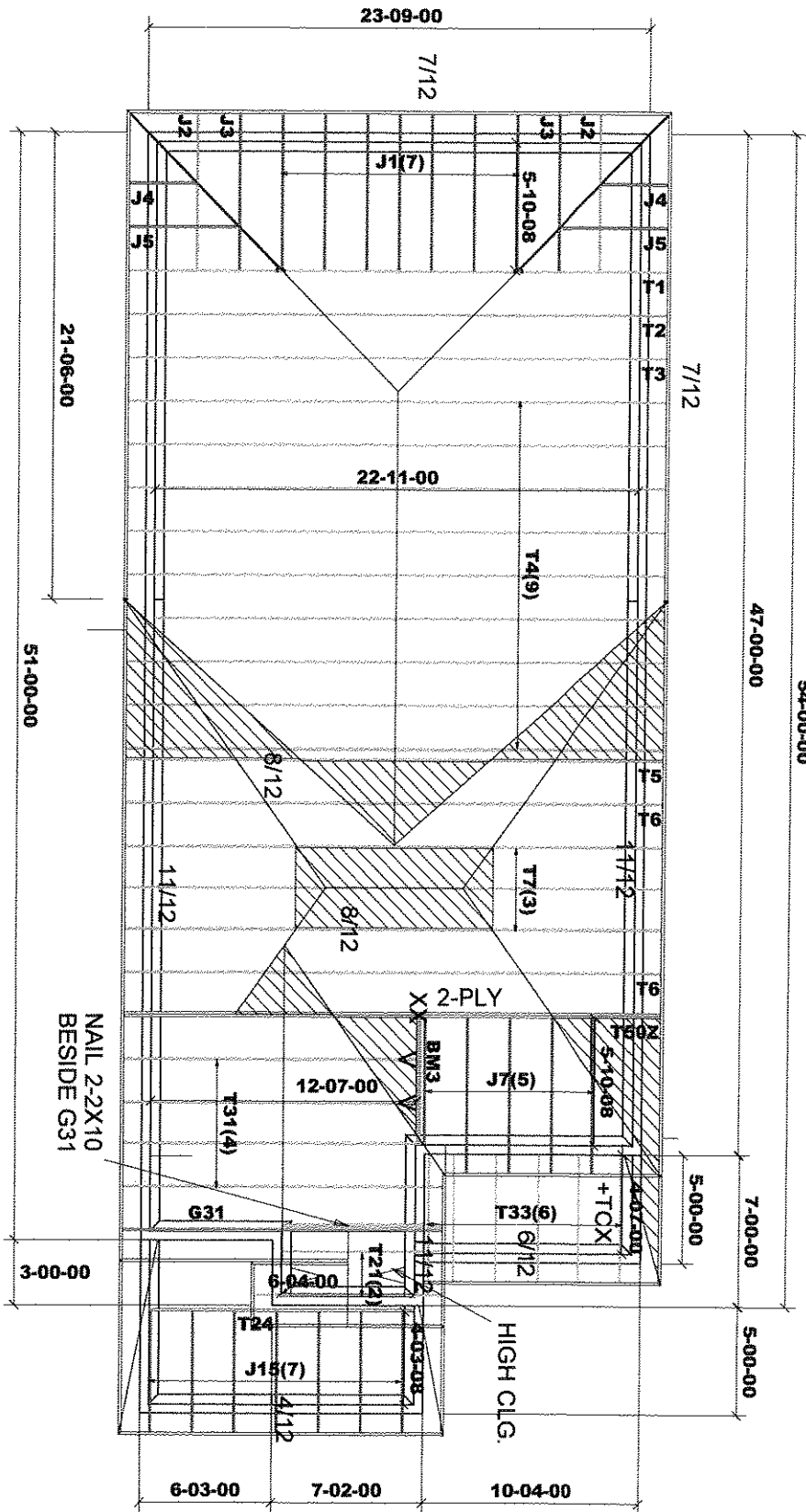
BC LIVE

Source: <http://www.fishbase.org>

DENOTES
CONVENTIONAL
FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

BM13:2-2X10



2009

Job Track: **42181**

Layout ID: 267429

Plan Log: **86179**

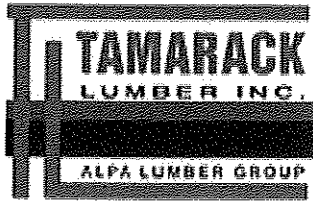
Builder / Location:

GREENPARK HOMES / VAUGHAN

Model / Elevation:

BRIDGEFORD 3 / 3 OR OPT.

Project:	STARTIME HOME LTD	
Date:	8/10/2016	Designer: JG
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.		



Delivery Shiplist

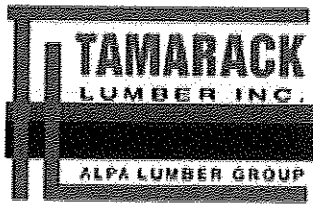
DATE	08/10/16
SALES REP	Mario

JOB TRACK:42181	LAYOUT ID: 267427	LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH	SUB-BUILDER:	
MODEL: BRIDGEFORD 3	ELEVATION: 1 OR OPT.	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	7.00 0.00	22-11-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	105.17 64.00		
	1	T2 HIP	7.00 0.00	22-11-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	95.39 59.67		
	1	T3 HIP	7.00 0.00	22-11-00	07-00-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	101.15 64.00		
	9	T4 COMMON	7.00 0.00	22-11-00	07-11-10	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	844.47 524.97		
	1	T5 HIP	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	101.16 63.83		
	1 2 Ply	T50 HIP GIRDER	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-09-01 01-09-01	237.14 148.00		
	2	T6 HIP	11.00 0.00	22-11-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	202.54 131.00		
	3	T7 HIP	11.00 0.00	22-11-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	344.58 221.01		
	3	T8 ROOF	11.00 0.00	09-11-08	06-05-14	2 X 4	2 X 4	01-03-08 00-00-00	01-09-01 02-01-03	165.15 111.51		
	1	T9 HIP GIRDER	11.00 0.00	06-04-00	03-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	36.61 25.67		
	2	T10 COMMON	11.00 0.00	06-04-00	04-07-14	2 X 4	2 X 4	00-00-00 00-00-00	01-09-01 01-09-01	57.82 40.00		
	1	T11 HIP GIRDER	11.00 0.00	12-07-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	61.96 40.33		
	1	T12 HIP	11.00 0.00	12-07-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	64.17 42.50		
	6	T33 MONOPITCH	6.00 0.00	04-07-00	03-00-15	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 02-07-11	113.64 73.98		
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		
	2	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -04-01-01	01-03-07 00-03-08	23.80 14.66		
	2	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-03-07 00-03-08	29.08 18.66		
	2	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-03-07 00-03-08	14.68 9.34		



Delivery Shiplist

DATE	08/10/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267427 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORD 3 ELEVATION: 1 OR OPT.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	19.96 13.34		
	3	J7 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	67.32 41.01		
	2	J8 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	31.66 20.00		

TOTAL # TRUSS= 54.00

TOTAL BFT OF ALL TRUSSES=

1802.17 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2838.55 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-3	
3	Hangers	LJS26DS	
4	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 9.00



Delivery Shiplist

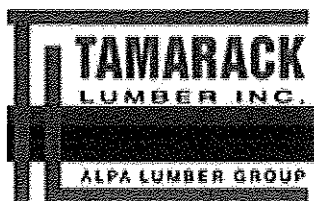
DATE	08/10/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267428 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORD 3 ELEVATION: 2 OR OPT.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	7.00 0.00	22-11-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	105.17 64.00		
	1	T2 HIP	7.00 0.00	22-11-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	95.39 59.67		
	1	T3 HIP	7.00 0.00	22-11-00	07-00-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	101.15 64.00		
	9	T4 COMMON	7.00 0.00	22-11-00	07-11-10	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	844.47 524.97		
	1	T5 HIP	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	101.16 63.83		
	1	T6 HIP	11.00 0.00	22-11-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	101.27 65.50		
	1	T7 HIP	11.00 0.00	22-11-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	114.86 73.67		
	1	T15 HIP	11.00 0.00	22-11-00	09-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	117.83 75.67		
	1	T16 HIP	11.00 0.00	22-11-00	03-04-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	99.94 64.67		
	1	T17 HIP	11.00 0.00	22-11-00	04-08-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	108.83 71.50		
	1	T18 HIP	11.00 0.00	22-11-00	06-00-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	116.31 75.17		
	1	T19 HIP	11.00 0.00	22-11-00	07-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	110.81 69.50		
	2	T20 HIP	11.00 0.00	22-11-00	08-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	225.14 142.66		
	2	T21 SCISSOR	11.00 8.00	06-04-00	04-07-14	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	65.24 43.66		
	1	T22 HIP	11.00 8.00	12-07-00	03-04-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	60.28 40.67		
	1	T23 HIP	11.00 8.00	12-07-00	04-08-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	67.69 47.50		
	1	T24 FLAT GIRDER	0.00 0.00	12-07-00	01-09-02	2 X 4	2 X 4	00-00-00 00-00-00	01-09-02 01-09-02	42.13 28.00		
	6	T33 MONOPITCH	6.00 0.00	04-07-00	03-00-15	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 02-07-11	113.64 73.98		



Delivery Shiplist

DATE	08/10/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267428 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORD 3 ELEVATION: 2 OR OPT.

ROOF TRUSSES

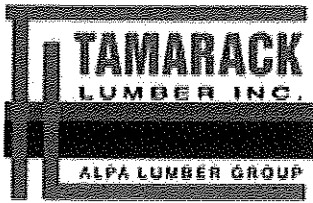
ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		
	2	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -04-01-01	01-03-07 00-03-08	23.80 14.66		
	2	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-03-07 00-03-08	29.08 18.66		
	2	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-03-07 00-03-08	14.68 9.34		
	2	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	19.96 13.34		
	7	J15 JACK-OPEN	4.00 0.00	04-03-08	01-09-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-09-02	82.04 51.31		

TOTAL # TRUSS= 55.00

TOTAL BFT OF ALL TRUSSES=

1830.62 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2881.97 LBS.



Delivery Shiplist

DATE	08/10/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267429 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORD 3 ELEVATION: 3 OR OPT.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T1 HIP GIRDER	7.00 0.00	22-11-00	04-08-09	2 X 4 2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	105.17 64.00		
	1	T2 HIP	7.00 0.00	22-11-00	05-10-09	2 X 4 2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	95.39 59.67		
	1	T3 HIP	7.00 0.00	22-11-00	07-00-09	2 X 4 2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	101.15 64.00		
	9	T4 COMMON	7.00 0.00	22-11-00	07-11-10	2 X 4 2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	844.47 519.03		
	1	T5 HIP	11.00 0.00	22-11-00	05-03-13	2 X 4 2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	101.16 63.83		
	1 2 Ply	T50Z HIP GIRDER	11.00 0.00	22-11-00	05-03-13	2 X 4 2 X 6	01-03-08 01-03-08	01-09-01 01-09-01	237.14 148.00		
	2	T6 HIP	11.00 0.00	22-11-00	06-07-13	2 X 4 2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	202.54 131.00		
	3	T7 HIP	11.00 0.00	22-11-00	07-11-13	2 X 4 2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	344.58 221.01		
	2	T21 SCISSORS	11.00 8.00	06-04-00	04-07-14	2 X 4 2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	65.24 43.66		
	1	T24 FLAT GIRDER	0.00 0.00	12-07-00	01-09-02	2 X 4 2 X 4	00-00-00 00-00-00	01-09-02 01-09-02	42.13 28.00		
	4	T31 HIP	11.00 0.00	12-07-00	05-03-13	2 X 4 2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	234.88 151.32		
	1	G31 GABLE	11.00 0.00	12-07-00	07-06-04	2 X 4 2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	64.42 42.50		
	6	T33 MONOPITCH	6.00 0.00	04-07-00	03-00-15	2 X 4 2 X 4	01-03-08 00-00-00	00-04-03 02-07-11	113.64 73.98		
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4 2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		
	2	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4 2 X 4	01-03-08 -04-01-01	01-03-07 00-03-08	23.80 14.66		
	2	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4 2 X 4	01-03-08 -02-01-01	01-03-07 00-03-08	29.08 18.66		
	2	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4 2 X 4	01-03-08 -00-01-01	01-03-07 00-03-08	14.68 9.34		
	2	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4 2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	19.96 13.34		



Delivery Shiplist

DATE	08/10/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267429 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORD 3 ELEVATION: 3 OR OPT.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	5	J7 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	112.20 68.35		
	7	J15 JACK-OPEN	4.00 0.00	04-03-08	01-09-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-09-02	82.04 51.31		

TOTAL # TRUSS= 61.00

TOTAL BFT OF ALL TRUSSES=

1860.35 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2954.77 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
2	Hangers	LJS26DS	

TOTAL # ITEMS= 3.00



Professional Engineer Seal for S. Katsoulakos, License No. 82126, Province of Ontario.

JOB NAME 267427	TRUSS NAME T1	QUANTITY 1	PLY 1	JOB DESC. 42181 TRUSS DESC.	DRWG NO.
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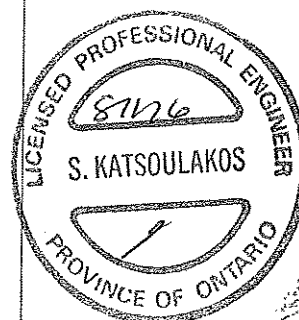
Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2018 MiTek Industries, Inc. Thu Aug 11 09:02:01 2016 Page 2

ID:OFOKkuTqKX10a1LfsiRDsuyPK5U-TRY729d3cXQFJ1c8HhFcP4yeOgDo8IPWbRPz8hyp_Eq

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	3-11-4	-41	-71	---	FRONT	VERT	TOTAL
T	7-11-4	-41	-71	---	FRONT	VERT	TOTAL
U	9-11-4	-41	-71	---	FRONT	VERT	TOTAL
V	12-11-12	-41	-71	---	FRONT	VERT	TOTAL
W	14-11-12	-41	-71	---	FRONT	VERT	TOTAL
X	18-11-12	-41	-71	---	FRONT	VERT	TOTAL
Y	20-11-12	-40	-75	---	FRONT	VERT	TOTAL



DWG NO. TAM36730-16
STRUCTURAL
COMPONENT ONLY

JOB NAME 267427	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. 42181 TRUSS DESC.	DRWG NO.
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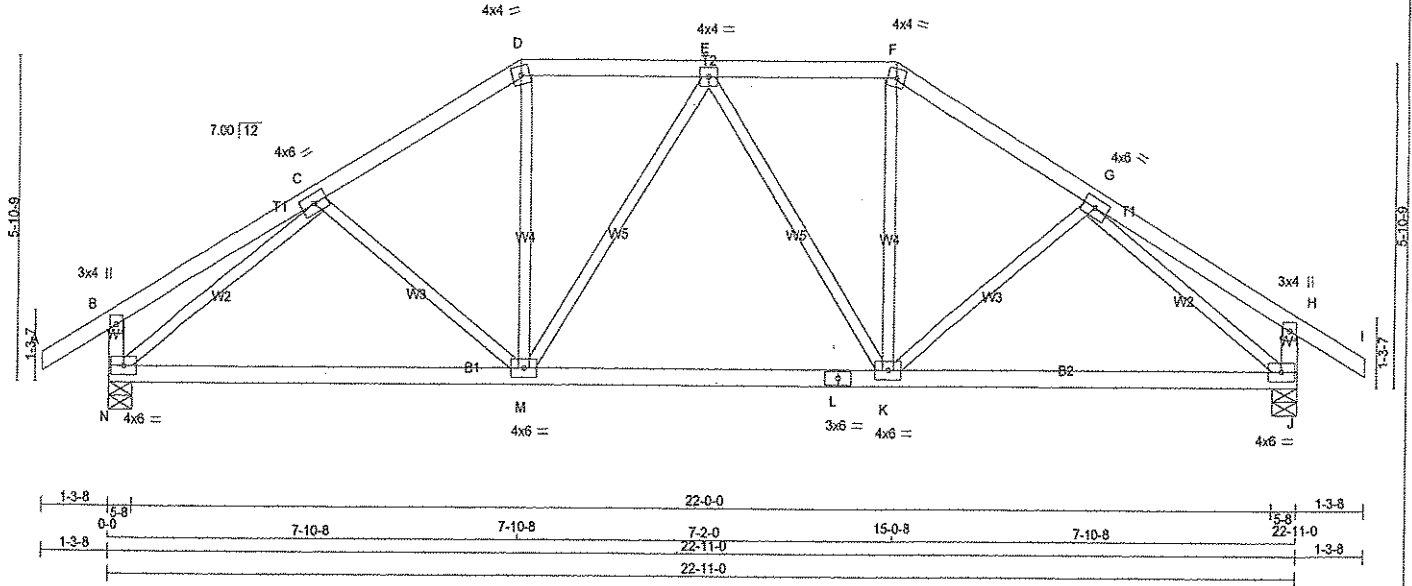
Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MitTek Industries, Inc. Thu Aug 11 09:02:02 2016 Page 1

ID:OFOKkuTqKX10a1LfsRDsuyPK5U-xd6NGVdhNqY6xBANrPmrylVvc4XTlJRgq58Wg7yp_Ep

22-11-0 3-0 24-2-8

Scale = 1:41.1



LUMBER

N. L. G. A. RULES

CHORDS

SIZE

DRY

No.2

SPF

LUMBER

DESCR.

SPF

ALL WEBS

2x3

DRY

No.2

SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B

TMV+p

MT20

3.0

4.0

C

TMWW-t

MT20

4.0

6.0

D

TTW-m

MT20

4.0

4.0

E

TMWW-t

MT20

4.0

4.0

F

TTW-m

MT20

4.0

4.0

G

TMWW-t

MT20

4.0

6.0

H

TMV+p

MT20

3.0

4.0

J

BMVW1-t

MT20

4.0

6.0

K

BMWW-t

MT20

4.0

6.0

L

BS-t

MT20

3.0

6.0

M

BMWW-t

MT20

4.0

6.0

N

BMVW1-t

MT20

4.0

6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

INPUT

REQ'D

GROSS REACTION

GROSS REACTION

BRG

BRG

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

N

1402

0

1402

0

0

5-8

1-8

J

1402

0

1402

0

0

5-8

1-8

UNFACTORED REACTIONS

1ST LCASE

MAX/MIN COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

N

1134

656 / 0

241 / 0

0 / 0

0 / 0

237 / 0

0 / 0

J

1134

656 / 0

241 / 0

0 / 0

0 / 0

237 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD (PLF)

LC1 MAX

MAX.

UNBRACED LENGTH

MEMB.

MAX. FACTORED FORCE (LBS)

MAX.

CS1 (LC)

FR-TO

FROM

TO

FR-TO

FROM

TO

A-B

0 / 29

-84.3

-84.3

0.11 (1)

10.00

C-M

-84 / 99

0.04 (1)

B-C

0 / 18

-84.3

-84.3

0.19 (1)

10.00

M-D

0 / 462

0.10 (1)

C-D

-1428 / 0

-84.3

-84.3

0.17 (1)

5.30

M-E

-213 / 0

0.17 (1)

D-E

-1225 / 0

-84.3

-84.3

0.14 (1)

5.65

E-K

-213 / 0

0.17 (1)

E-F

-1225 / 0

-84.3

-84.3

0.14 (1)

5.65

K-F

0 / 462

0.10 (1)

F-G

-1428 / 0

-84.3

-84.3

0.17 (1)

5.30

K-G

-84 / 99

0.04 (1)

G-H

0 / 18

-84.3

-84.3

0.19 (1)

10.00

N-C

-1655 / 0

0.71 (1)

H-I

0 / 29

-84.3

-84.3

0.11 (1)

10.00

G-J

-1655 / 0

0.71 (1)

N-B

-245 / 0

0.0

0.0

0.03 (1)

7.81

J-H

-245 / 0

0.0

0.0

0.03 (1)

7.81

N-M

0 / 1281

-28.0

-28.0

0.51 (2)

10.00

M-L

0 / 1337

-28.0

-28.0

0.53 (2)

10.00

L-K

0 / 1337

-28.0

-28.0

0.53 (2)

10.00

K-J

0 / 1281

-28.0

-28.0

0.51 (2)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.

LL

=

25.6

PSF

DL

=

3.0

PSF

BOT CH.

LL

=

10.5

PSF

DL

=

7.0

PSF

TOTAL LOAD

=

46.1

PSF

SPACING

=

24.0

IN

C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")

CALCULATED VERT. DEFL.(LL)= L/999 (0.17")

ALLOWABLE DEFL.(TL)= L/360 (0.76")

CALCULATED VERT. DEFL.(TL)= L/972 (0.28")

CS1: TC=0.19 (B-C:1), BC=0.53 (K-M:2), WB=0.71 (C-N:1), SSI=0.18 (M-N:3)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI)

(PLI)

(PLI)

MAX MIN

MAX MIN

MAX MIN

MT20

618

354

1667

822

2284

1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)

JSI METAL= 0.55 (L) (INPUT = 1.00)



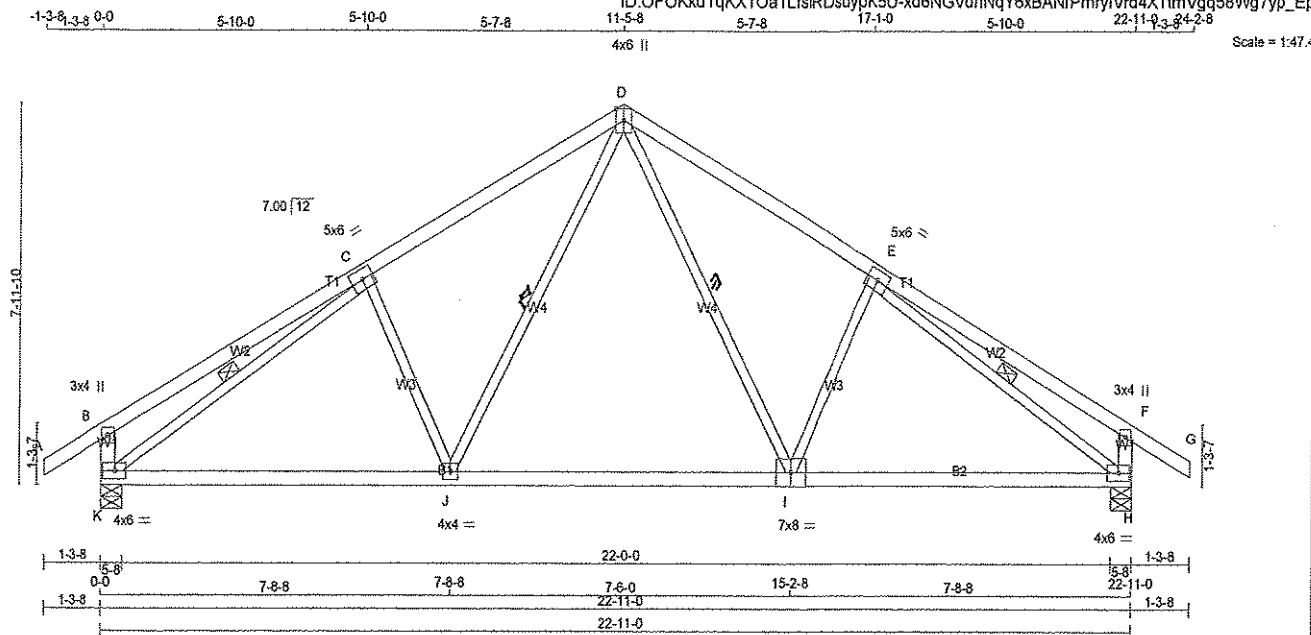
DWG NO. TAM 36739-16

STRUCTURAL

COMPONENT ONLY

JOB NAME 267427	TRUSS NAME T4	QUANTITY 9	PLY 1	JOB DESC. 42181	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.000 S Jan 15 2016 M&T Industries, Inc. Thu Aug 11 09:02:02 2016 Page 1
ID:OFOKkuTqKX1Oa1LfsiRDSuypK5U-xd6NGVdhNqY6xBANrPmryIvrd4XtImVgg58Wg7yp_Ep
22-11-03-84-2-8
Scale = 1:47.4



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
K - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BSWW-t	MT20	7.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORIZ	UPLIFT	IN-SX
K	1402	0	1402	0
H	1402	0	1402	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1134	658 / 0	241 / 0	0 / 0	0 / 0	237 / 0	0 / 0
H	1134	658 / 0	241 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-H. *0-5, 0-1*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 29	-84.3 -84.3	0.11 (1)	10.00	D-I	0 / 582	0.13 (1)
B-C	0 / 29	-84.3 -84.3	0.45 (1)	10.00	I-E	-319 / 50	0.12 (1)
C-D	-1433 / 0	-84.3 -84.3	0.39 (1)	5.02	J-D	0 / 582	0.13 (1)
D-E	-1433 / 0	-84.3 -84.3	0.39 (1)	5.02	C-J	-319 / 50	0.12 (1)
E-F	0 / 29	-84.3 -84.3	0.45 (1)	10.00	K-C	-1688 / 0	0.57 (1)
F-G	0 / 29	-84.3 -84.3	0.11 (1)	10.00	E-H	-1688 / 0	0.57 (1)
K-B	-299 / 0	0.0 0.0	0.03 (1)	7.81			
H-F	-299 / 0	0.0 0.0	0.03 (1)	7.81			
K-J	0 / 1347	-28.0 -28.0	0.53 (2)	10.00			
J-I	0 / 957	-28.0 -28.0	0.49 (2)	10.00			
I-H	0 / 1347	-28.0 -28.0	0.53 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.76")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.15")
ALLOWABLE DEFL.(TL)= $L/360$ (0.76")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.24")

CSI: TC=0.45 (B-C:1), BC=0.53 (J-K:2), WB=0.57 (C-K:1), SSI=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.43 (E) (INPUT = 1.00)



DWG NO. TAM36741-16
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM 36742-1G
STRUCTURAL
COMPONENT ONLY

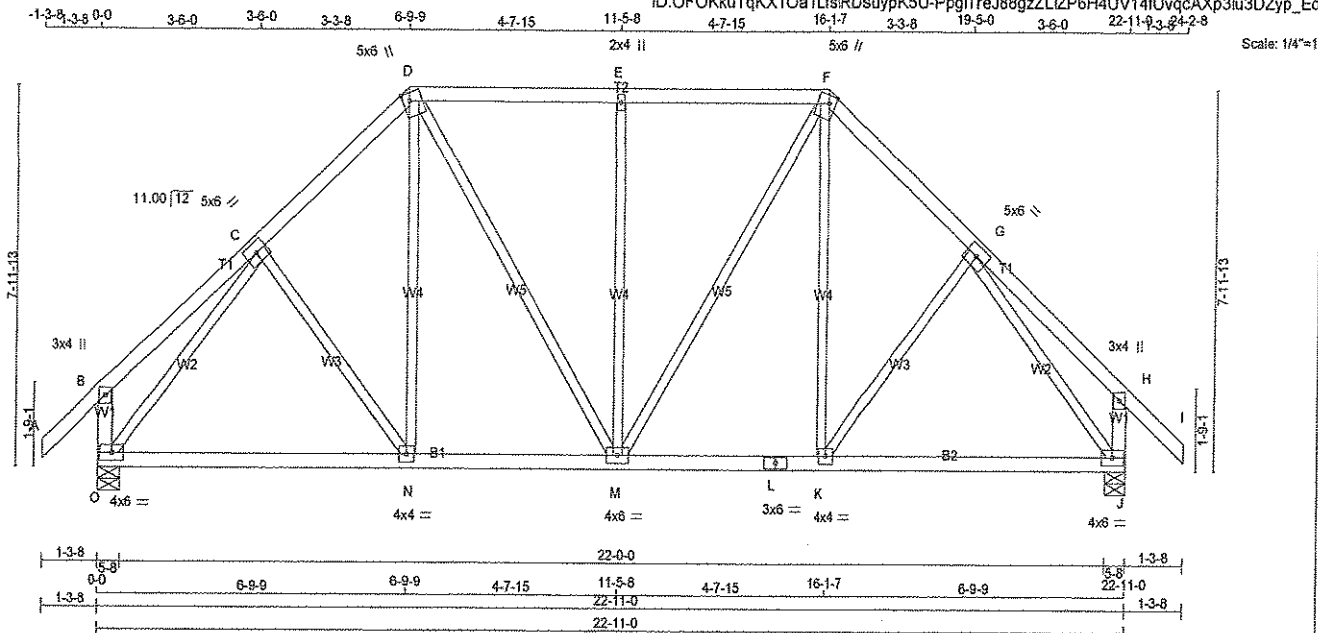


DWG NO. FAN367431G
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267427	T7	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MTEK Industries, Inc. Thu Aug 11 09:02:03 2016 Page 1
ID:OFOKkuTqKX1Oa1LfsiRDsuyPK5U-PpgITreJ88gzLIZP6H4UV14fUvqcAXp3lu3DZyp_Eo



TOTAL WEIGHT = 3 X 115 = 345 lb (M/F)

LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF			
D - F	2x4	DRY	No.2	SPF			
F - I	2x4	DRY	No.2	SPF			
O - B	2x4	DRY	No.2	SPF			
O - H	2x4	DRY	No.2	SPF			
O - L	2x4	DRY	No.2	SPF			
L - J	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
EXCEPT							

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWV-t	MT20	5.0	6.0		
D	TTWV+m	MT20	5.0	6.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWV+m	MT20	5.0	6.0	2.00	1.75
G	TMWV-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMWV-t	MT20	4.0	6.0		
K	BMWV-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWV-t	MT20	4.0	6.0		
N	BMWV-t	MT20	4.0	4.0		
O	BMWV-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	REQD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1404	0	1404	0	0	5-8	1-8
J	1404	0	1404	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD LC1 (LC)	MAX. VERT. LOAD LC1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 40	-84.3	-84.3 0.12 (1)	10.00	C-N	-7 / 93	0.02 (3)
B-C	0 / 21	-84.3	-84.3 0.15 (1)	10.00	N-D	0 / 298	0.07 (2)
C-D	-1133 / 0	-84.3	-84.3 0.13 (1)	5.84	D-M	0 / 323	0.07 (1)
D-E	-987 / 0	-84.3	-84.3 0.24 (1)	5.99	M-E	-475 / 0	0.53 (1)
E-F	-987 / 0	-84.3	-84.3 0.24 (1)	5.99	M-F	0 / 323	0.07 (1)
F-G	-1133 / 0	-84.3	-84.3 0.13 (1)	5.84	K-F	0 / 298	0.07 (2)
G-H	0 / 21	-84.3	-84.3 0.15 (1)	10.00	K-G	-7 / 93	0.02 (3)
H-I	0 / 40	-84.3	-84.3 0.12 (1)	10.00	O-C	-1370 / 0	0.72 (1)
O-B	-227 / 0	0.0	0.0 0.02 (1)	7.81	G-J	-1370 / 0	0.72 (1)
J-H	-227 / 0	0.0	0.0 0.02 (1)	7.81			
O-N	0 / 821	-28.0	-28.0 0.39 (2)	10.00			
N-M	0 / 819	-28.0	-28.0 0.39 (2)	10.00			
M-L	0 / 819	-28.0	-28.0 0.39 (2)	10.00			
L-K	0 / 819	-28.0	-28.0 0.39 (2)	10.00			
K-J	0 / 821	-28.0	-28.0 0.39 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.24 (D-E:1), BC=0.39 (K-M:2), WB=0.72 (G-J:1), SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (O) (INPUT = 0.90)
JSI METAL= 0.34 (L) (INPUT = 1.00)



DRWG NO. TAM 36744-16
STRUCTURAL
COMPONENT ONLY

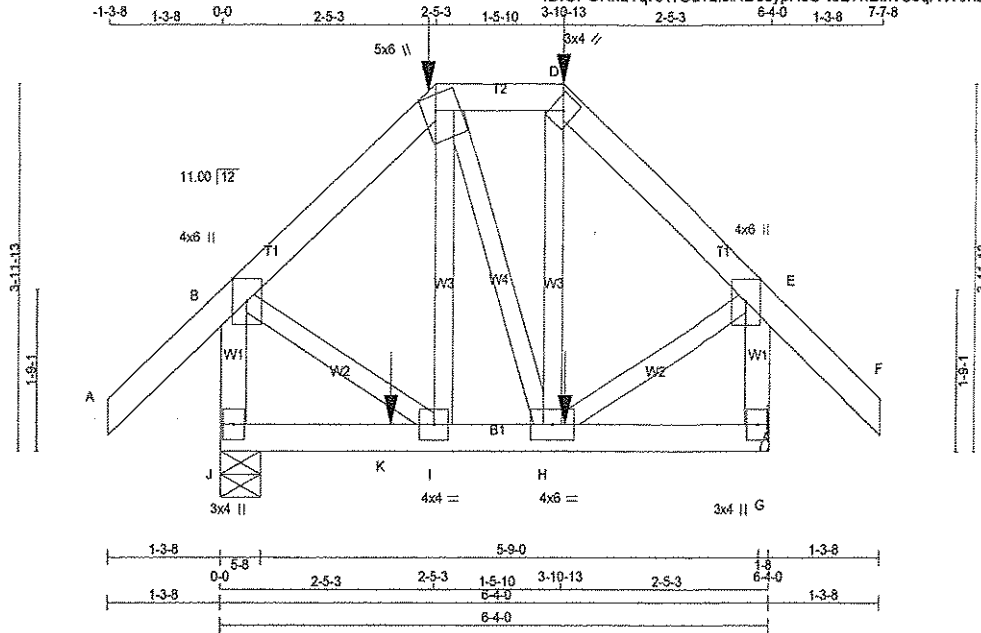
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267427	T9	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:OFOKkuTqKX10a1LfsiRDsuyPK5U40E7hBbvSoqAVKzmzqoJ1jaF7IKzLntzIPddIDyp_Er

Scale = 1:24.6



TOTAL WEIGHT = 37 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.75
D	TTW+h	MT20	3.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW+t	MT20	4.0	6.0		
I	BMWW+t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 67.0 lbs FACTORED DOWN AT 2-5-3, AND 67.0 lbs FACTORED DOWN AT 3-10-13, AND 82.6 lbs FACTORED DOWN AT 3-10-13 ON TOP CHORD, AND 41.0 lbs FACTORED DOWN AT 1-11-4, AND 41.0 lbs FACTORED DOWN AT 3-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
J 597	0	5-8	1-8
G 612	0	0	0

HANGER BY OTHERS MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
J	480	282 / 0 98 / 0 0 / 0 0 / 0 100 / 0 0 / 0
G	488	293 / 0 95 / 0 0 / 0 0 / 0 99 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-84.3 -84.3	0.13 (1)	10.00	I-C	-42 / 72	0.02 (3)
B-C	-316 / 0	-84.3 -84.3	0.10 (1)	6.25	C-H	0 / 27	0.01 (1)
C-D	-240 / 0	-84.3 -84.3	0.03 (1)	6.25	H-D	-90 / 62	0.02 (1)
D-E	-330 / 0	-84.3 -84.3	0.10 (1)	6.25	E-I	0 / 265	0.07 (1)
E-F	0 / 40	-84.3 -84.3	0.13 (1)	10.00	H-E	0 / 277	0.07 (1)
J-B	-563 / 0	0.0 0.0	0.07 (1)	7.81			
G-E	-579 / 0	0.0 0.0	0.07 (1)	7.81			
J-K	0 / 0	-28.0 -28.0	0.05 (2)	10.00			
K-I	0 / 0	-28.0 -28.0	0.05 (2)	10.00			
I-H	0 / 230	-28.0 -28.0	0.08 (2)	10.00			
H-G	0 / 0	-28.0 -28.0	0.04 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	2-5-3	-67	-67	--	FRONT	VERT	TOTAL
D	3-10-13	-67	-67	--	FRONT	VERT	TOTAL
D	3-10-13	-83	-83	--	BACK	VERT	TOTAL
H	3-11-4	-23	-41	--	BACK	VERT	TOTAL
K	1-11-4	-23	-41	--	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 46.1	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13 (E-F:1), BC=0.08 (H-I:2), WB=0.07 (E-H:1), SSI=0.09 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20 618 354	1667 622	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.62 (D) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

DWG NO. TAM 36746-16
STRUCTURAL

COMPONENT ONLY

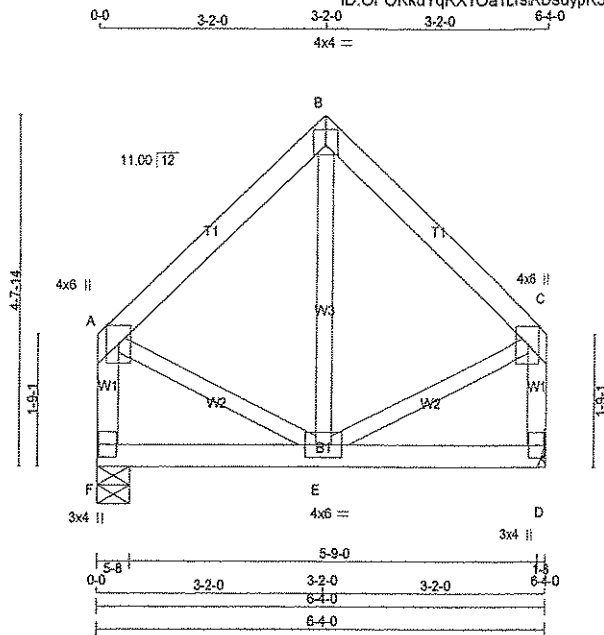


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267427	T10	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:30.1

TOTAL WEIGHT = 2 X 29 = 58 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW-p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	4.0	6.0	Edge	
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-l	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
F	358	0	358	0	0	5-8	1-8		
D	358	0	358	0	0				

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS						
F	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
D	292	162 / 0	67 / 0	0 / 0	0 / 0	63 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC	LENGTH	MEMB.	FORCE (LBS)	MAX. UNBRAC	LENGTH
A-B	-167 / 0	-84.3	-84.3	0.11 (1)	6.25	E-B	-14 / 121	0.03 (3)	
B-C	-167 / 0	-84.3	-84.3	0.11 (1)	6.25	A-E	0 / 151	0.03 (1)	
F-A	-321 / 0	0.0	0.0	0.03 (1)	7.81	E-C	0 / 151	0.03 (1)	
D-C	-321 / 0	0.0	0.0	0.03 (1)	7.81				
F-E	0 / 0	-28.0	-28.0	0.08 (3)	10.00				
E-D	0 / 0	-28.0	-28.0	0.08 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.11 (A-B:1), BC=0.08 (D-E:3), WB=0.03 (C-E:1), SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

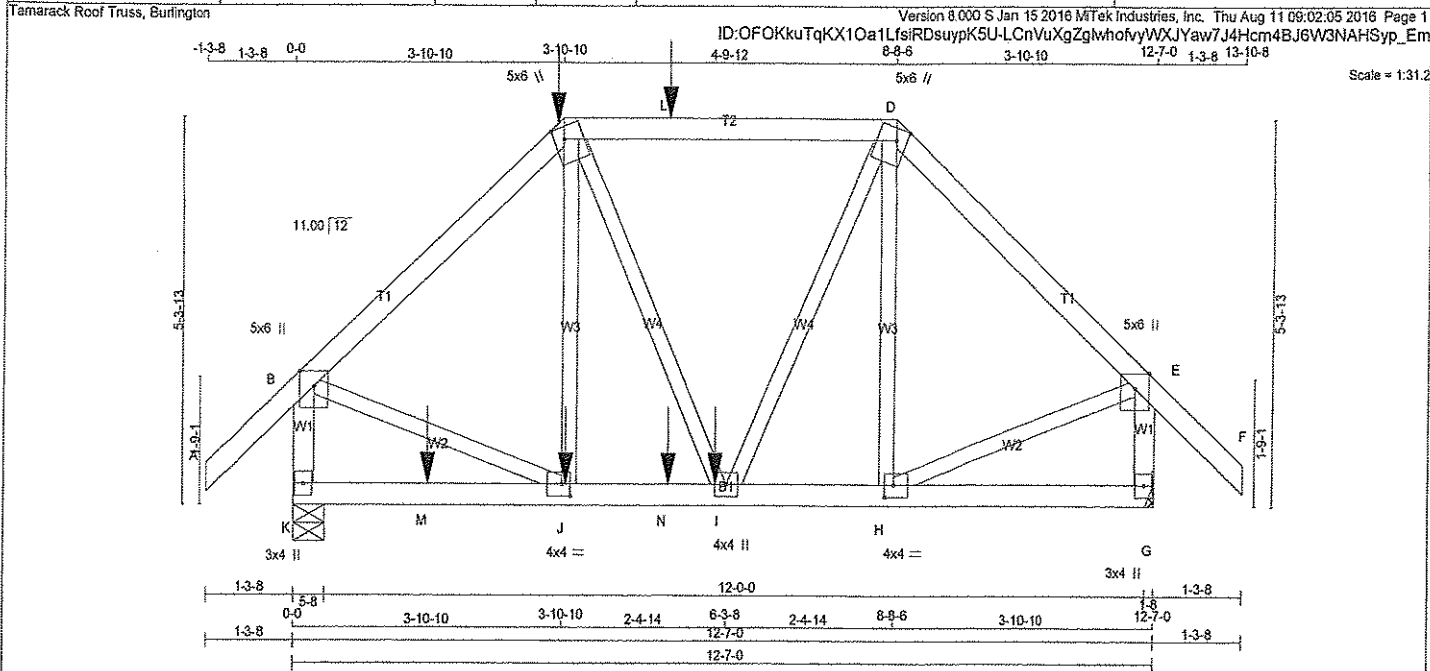
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)



DWG NO. TAM36747-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267427	T11	1	1	TRUSS DESC.		



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
K - B	2x4	DRY No.2	SPF
G - E	2x4	DRY No.2	SPF
K - G	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	8.0 Edge
C	TTWW+m	MT20	5.0	6.0 2.00 1.75
D	TTWW+m	MT20	5.0	6.0 2.00 1.75
E	TMVW+p	MT20	5.0	6.0 Edge
G	BMV1+p	MT20	3.0	4.0
H	BMWW-t	MT20	4.0	4.0 2.00 1.50
I	BMWW-t	MT20	4.0	4.0
J	BMWW-t	MT20	4.0	4.0 2.00 1.50
K	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 166.8 lbs FACTORED DOWN AT 3-10-10, AND 163.3 lbs FACTORED DOWN AT 5-5-4 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-11-4, 87.4 lbs FACTORED DOWN AT 3-11-4, AND 87.4 lbs FACTORED DOWN AT 5-5-4, AND 834.9 lbs FACTORED DOWN AT 6-1-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	1554	0	0	1-11
K	HORIZ	1554	0	0	5-8
G	HORIZ	1408	0	0	HANGER BY OTHERS

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1259	725 / 0	268 / 0	0 / 0	0 / 0	266 / 0	0 / 0
G	1126	674 / 0	223 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 40	-84.3	-84.3 0.13 (1)	J-C	-96 / 234	10.00	0.06 (3)
B-C	-1288 / 0	-84.3	-84.3 0.28 (1)	H-D	-171 / 61	5.33	0.07 (1)
C-L	-1101 / 0	-84.3	-84.3 0.63 (1)	B-J	0 / 1004	4.96	0.25 (1)
L-D	-1101 / 0	-84.3	-84.3 0.63 (1)	H-E	0 / 899	4.96	0.22 (1)
D-E	-1154 / 0	-84.3	-84.3 0.28 (1)	C-I	0 / 362	5.57	0.09 (1)
E-F	0 / 40	-84.3	-84.3 0.13 (1)	I-D	0 / 593	10.00	0.15 (1)
K-B	-1489 / 0	0.0	0.0 0.18 (1)				
G-E	-1363 / 0	0.0	0.0 0.16 (1)				
K-M	0 / 0	-28.0	-28.0 0.19 (3)				
M-J	0 / 0	-28.0	-28.0 0.19 (3)				
J-N	0 / 844	-28.0	-28.0 0.30 (2)				
N-I	0 / 844	-28.0	-28.0 0.30 (2)				
I-H	0 / 844	-28.0	-28.0 0.22 (1)				
H-G	0 / 0	-28.0	-28.0 0.11 (2)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-10	-167	-167	-	FRONT	VERT	TOTAL
I	6-1-8	-835	-835	-	FRONT	VERT	TOTAL
J	3-11-4	-50	-87	-	FRONT	VERT	TOTAL
L	5-5-4	-163	-163	-	FRONT	VERT	TOTAL
M	1-11-4	-50	-87	-	FRONT	VERT	TOTAL
N	5-5-4	-50	-87	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.42")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.63 (C-D:1), BC=0.30 (I-J:2), WB=0.25 (B-J:1), SSI=0.27 (C-D:1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1657
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)

JSI METAL= 0.35 (J) (INPUT = 1.00)



DWG NO. TAM36748-1G
 STRUCTURAL
 COMPONENT ONLY



TOTAL WEIGHT = 64 lb

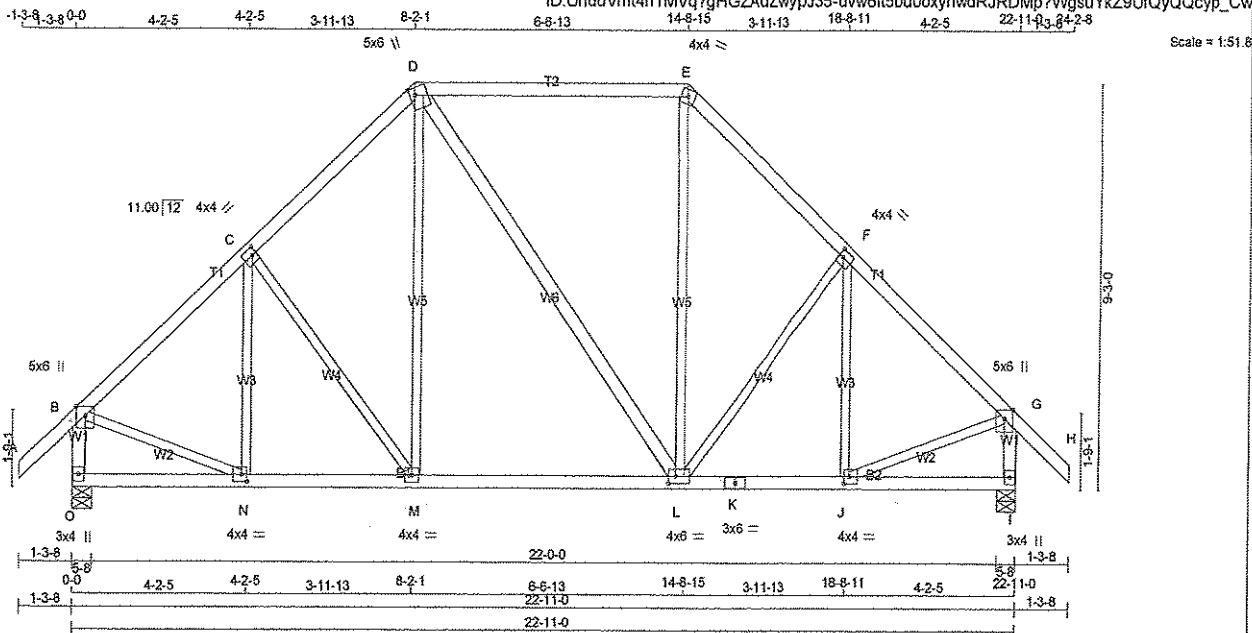
DWG NO. TAM36749-1G
STRUCTURAL
COMPONENT ONLY

JOB NAME 267428	TRUSS NAME T15	QUANTITY 1	PLY 1	JOB DESC. 42181	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 118 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REORD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT
O	1404	0	1404	0
I	1404	0	1404	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
O	1136	658 / 0
I	1136	658 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LC)
FR-TO						FR-TO			
A-B	0 / 40	-84.3	-84.3	0.12 (1)	10.00	N-C	-182 / 55	0.09 (1)	
B-C	-1162 / 0	-84.3	-84.3	0.20 (1)	5.70	C-M	-184 / 0	0.15 (1)	
C-D	-1067 / 0	-84.3	-84.3	0.19 (1)	5.89	M-D	0 / 363	0.08 (2)	
D-E	-767 / 0	-84.3	-84.3	0.48 (1)	6.15	D-L	0 / 0	0.00 (1)	
E-F	-1068 / 0	-84.3	-84.3	0.19 (1)	5.89	L-E	0 / 354	0.08 (2)	
F-G	-1162 / 0	-84.3	-84.3	0.20 (1)	5.70	L-F	-183 / 0	0.15 (1)	
G-H	0 / 40	-84.3	-84.3	0.12 (1)	10.00	J-F	-183 / 54	0.09 (1)	
O-B	-1353 / 0	0.0	0.0	0.15 (1)	6.97	B-N	0 / 925	0.21 (1)	
I-G	-1352 / 0	0.0	0.0	0.15 (1)	6.97	J-G	0 / 925	0.21 (1)	
O-N	0 / 0	-28.0	-28.0	0.11 (2)	10.00				
N-M	0 / 876	-28.0	-28.0	0.28 (2)	10.00				
M-L	0 / 767	-28.0	-28.0	0.28 (2)	10.00				
L-K	0 / 876	-28.0	-28.0	0.29 (2)	10.00				
K-J	0 / 876	-28.0	-28.0	0.29 (2)	10.00				
J-I	0 / 0	-28.0	-28.0	0.11 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.48 (D-E:1), BC=0.29 (J-L:2), WB=0.21 (B-N:1), SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(FSL)	(PLJ)	(PLI)	
MT20	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.76 (N) (INPUT = 0.50)
JSI METAL= 0.33 (N) (INPUT = 1.00)



DWG NO. TAM 36752-16
STRUCTURAL
COMPONENT ONLY



Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD

LOADING
TOTAL LOAD CASES: (4)

DWG NO. FAM36753-18
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267428	T17	1	1	TRUSS DESC.		

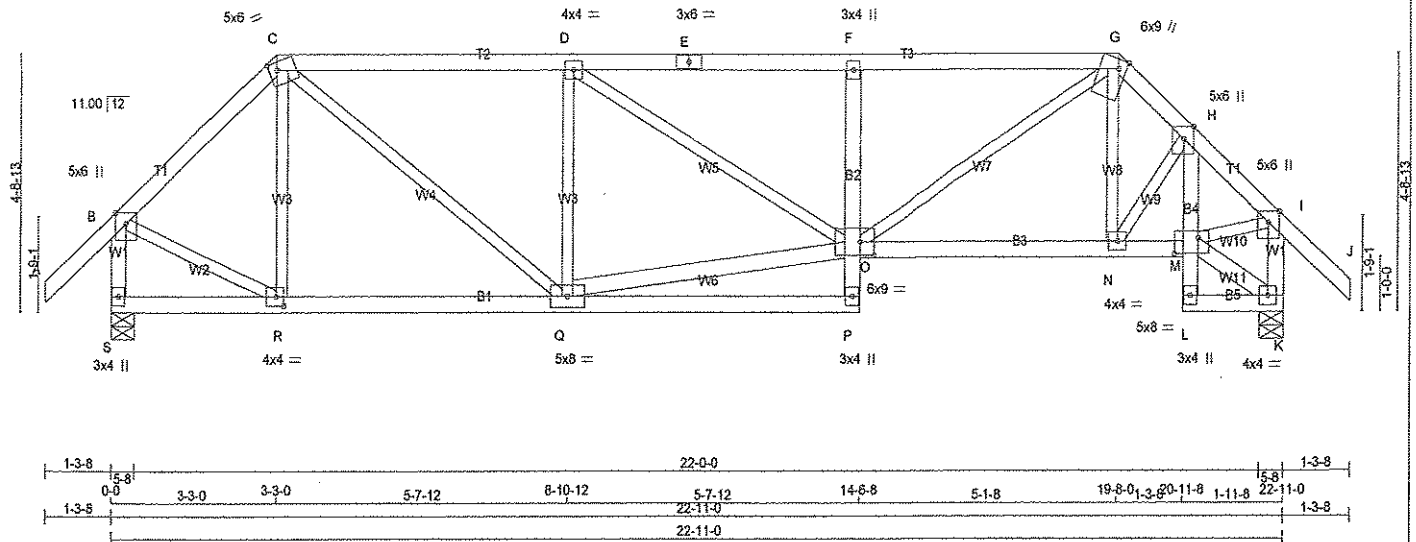
Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Thu Aug 11 09:04:04 2016 Page 1

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-1-3-8 1-3-8 0-0 3-3-0 3-3-0 5-7-12 8-10-12 5-7-12 14-6-8 5-1-8 19-8-0 1-3-8 1-1-8 22-11-0 3-8-2-8

Scale = 1:41.6



TOTAL WEIGHT = 109 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
S - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - P	2x4	DRY	No.2
O - M	2x4	DRY	No.2
L - H	2x4	DRY	No.2
L - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
Q - O	2x4	DRY	No.2
M - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMWW-l	MT20	4.0	4.0		
E	TS-l	MT20	3.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	Edge	2.25
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMVW-l	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMW+l	MT20	5.0	8.0	3.50	5.50
N	BMW+l	MT20	4.0	4.0		
O	BVMW+l	MT20	6.0	9.0	3.00	3.25
P	BMV+p	MT20	3.0	4.0		
Q	BMVW+l	MT20	5.0	8.0		
R	BMW+l	MT20	4.0	4.0	2.00	1.75
S	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	1400	0	1400	0	5-8
K	1407	0	1407	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1133	655 / 0	241 / 0	0 / 0	0 / 0	237 / 0	0 / 0
K	1138	660 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 40	-84.3	-84.3	0.12 (1)	10.00	R-C	-215 / 48
B-C	-1128 / 0	-84.3	-84.3	0.17 (1)	5.77	C-Q	0 / 1037
C-D	-1636 / 0	-84.3	-84.3	0.44 (1)	4.68	Q-D	-753 / 0
D-E	-2028 / 0	-84.3	-84.3	0.47 (1)	4.25	O-Q	0 / 1607
E-F	-2028 / 0	-84.3	-84.3	0.47 (1)	4.25	D-O	0 / 446
F-G	-2048 / 0	-84.3	-84.3	0.39 (1)	4.34	O-G	0 / 1184
G-H	-1432 / 0	-84.3	-84.3	0.06 (1)	5.41	N-G	0 / 204
H-I	-1442 / 0	-84.3	-84.3	0.13 (1)	5.30	N-H	0 / 33
I-J	0 / 40	-84.3	-84.3	0.12 (1)	10.00	B-R	0 / 899
S-B	-1369 / 0	0.0	0.0	0.15 (1)	6.94	M-K	-34 / 0
K-I	-1364 / 0	0.0	0.0	0.15 (1)	6.95	M-I	0 / 1049
S-R	0 / 0	-28.0	-28.0	0.12 (2)	10.00		
R-Q	0 / 823	-28.0	-28.0	0.31 (2)	10.00		
Q-P	0 / 69	-28.0	-28.0	0.22 (3)	10.00		
P-O	0 / 109	0.0	0.0	0.10 (1)	10.00		
O-F	-476 / 0	0.0	0.0	0.11 (1)	7.81		
O-N	0 / 1055	-28.0	-28.0	0.30 (2)	10.00		
N-M	0 / 1052	-28.0	-28.0	0.30 (2)	10.00		
L-M	0 / 44	0.0	0.0	0.05 (1)	10.00		
M-H	-104 / 0	0.0	0.0	0.04 (1)	7.81		
L-K	0 / 29	-28.0	-28.0	0.03 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.4 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.16")

CSI: TC=0.47 (D-F-1), BC=0.31 (Q-R-2), WB=0.27 (G-O-1), SSI=0.22 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

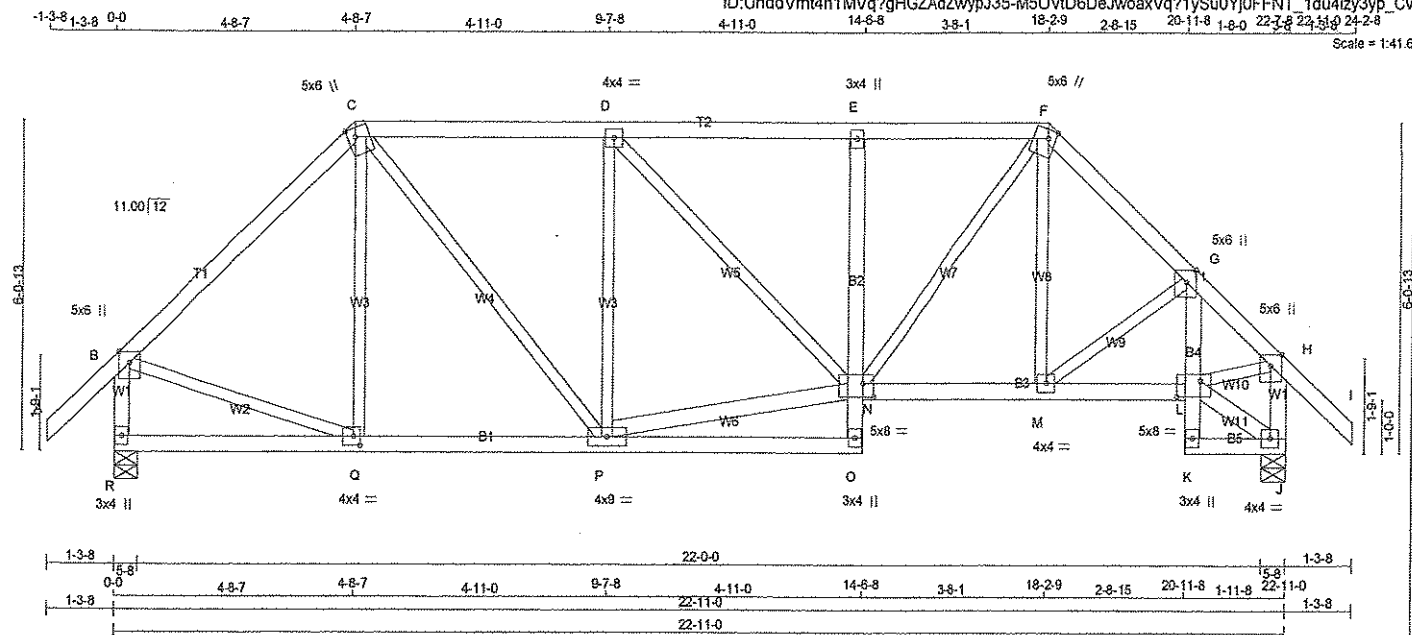
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (C) (INPUT = 0.90)
JSI METAL=0.31 (R) (INPUT = 1.00)

DWG NO. TAM 36754-10
STRUCTURAL
COMPONENT ONLY



JOB NAME 267428	TRUSS NAME T18	QUANTITY 1	PLY 1	JOB DESC. 42181	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.000 S Jan 15 2016 MTEK Industries, Inc. Thu Aug 11 09:04:04 2016 Page 1			
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		Scale = 1:41.6			



TOTAL WEIGHT = 116 lb (M)

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
R - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - E	2x4	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
K - G	2x4	DRY No.2	SPF
K - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
P - N	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	6.0 Edge
C	TTWW+m	MT20	5.0	6.0 2.00 1.75
D	TMVW-l	MT20	4.0	4.0
E	TMV+p	MT20	3.0	4.0
F	TTWW+m	MT20	5.0	6.0 2.00 1.75
G	TMVW+p	MT20	5.0	6.0 Edge 2.25
H	TMVW+p	MT20	5.0	6.0 Edge
J	BMVW-l	MT20	4.0	4.0
K	BMV+p	MT20	3.0	4.0
L	BMVW-l	MT20	5.0	8.0 3.50 5.50
M	BMVW-l	MT20	4.0	4.0
N	BMVWW-l	MT20	5.0	8.0 3.00 2.50
O	BMV+p	MT20	3.0	4.0
P	BMVWW-l	MT20	4.0	9.0
Q	BMVW-l	MT20	4.0	4.0 2.00 1.50
R	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REORD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	1400	0	1400	0	5-8
J	1407	0	1407	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1133	655 / 0	241 / 0	0 / 0	0 / 0	237 / 0	0 / 0
J	1138	660 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-84.3 -84.3	0.12 (1)	10.00	Q-C	-102 / 127	0.06 (1)
B-C	-1153 / 0	-84.3 -84.3	0.37 (1)	5.48	C-P	0 / 671	0.15 (1)
C-D	-1278 / 0	-84.3 -84.3	0.32 (1)	5.30	P-D	-625 / 0	0.36 (1)
D-E	-1451 / 0	-84.3 -84.3	0.33 (1)	5.04	P-N	0 / 1277	0.21 (1)
E-F	-1458 / 0	-84.3 -84.3	0.21 (1)	5.19	D-N	0 / 234	0.05 (1)
F-G	-1378 / 0	-84.3 -84.3	0.13 (1)	5.41	N-F	0 / 718	0.16 (1)
G-H	-1458 / 0	-84.3 -84.3	0.13 (1)	5.28	M-F	0 / 205	0.05 (2)
H-I	0 / 40	-84.3 -84.3	0.12 (1)	10.00	M-G	-59 / 0	0.02 (1)
R-B	-1346 / 0	0.0 0.0	0.15 (1)	6.99	B-Q	0 / 893	0.20 (1)
J-H	-1354 / 0	0.0 0.0	0.15 (1)	6.95	L-J	-35 / 0	0.00 (1)
					L-H	0 / 1075	0.24 (1)
R-Q	0 / 0	-28.0 -28.0	0.15 (3)	10.00			
Q-P	0 / 846	-28.0 -28.0	0.25 (2)	10.00			
P-O	0 / 35	-28.0 -28.0	0.15 (3)	10.00			
O-N	0 / 96	0.0 0.0	0.06 (1)	10.00			
N-E	-370 / 0	0.0 0.0	0.08 (1)	7.81			
N-M	0 / 1009	-28.0 -28.0	0.22 (1)	10.00			
M-L	0 / 1078	-28.0 -28.0	0.23 (1)	10.00			
K-L	0 / 44	0.0 0.0	0.05 (1)	10.00			
L-G	-91 / 21	0.0 0.0	0.05 (1)	7.81			
K-J	0 / 29	-28.0 -28.0	0.03 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.37 (B-C:1), BC=0.25 (P-Q:2), WB=0.35 (D-P:1), SS=0.19 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1637
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (J) (INPUT = 0.90)
JSI METAL= 0.32 (Q) (INPUT = 1.00)

DWG NO. TAM 36755.16
STRUCTURAL
COMPONENT ONLY

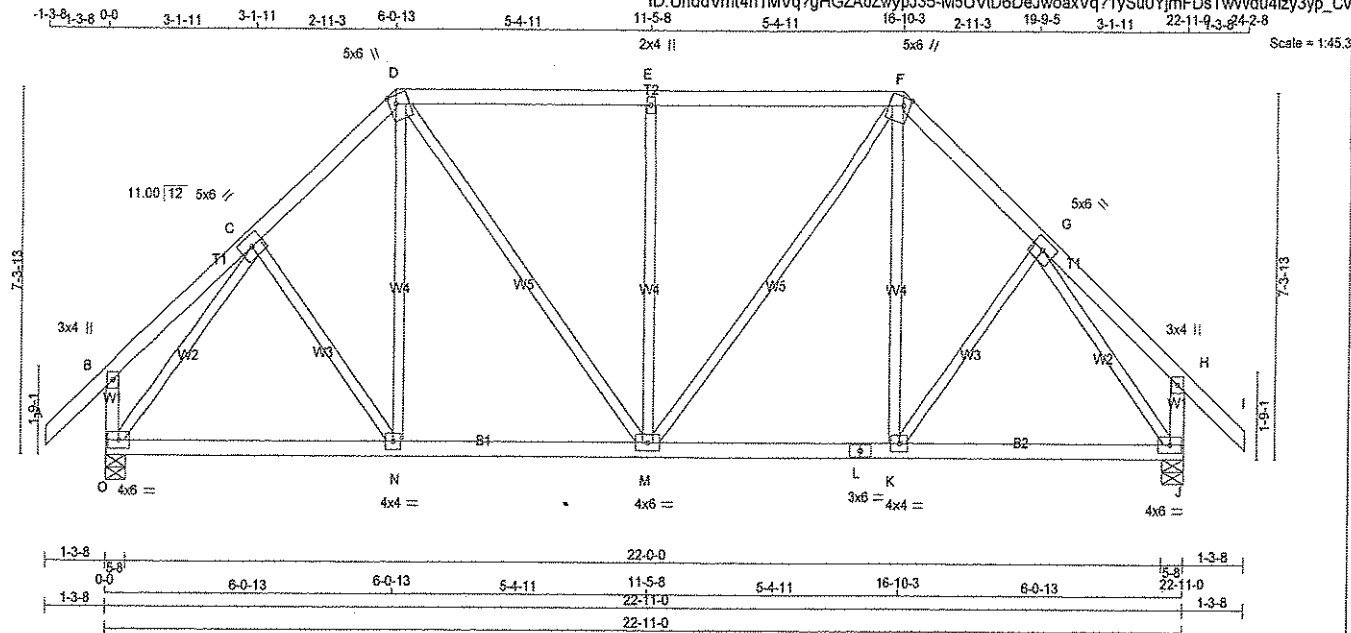


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267428	T19	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 111 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTVW+m	MT20	5.0	8.0	2.00	1.75
E	TMVW-w	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	8.0	2.00	1.75
G	TMVW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	1404	0	1404	0
J	1404	0	1404	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 40	-84.3	-84.3 0.12 (1)	10.00	C-N	0 / 110	0.02 (3)
B-C	0 / 18	-84.3	-84.3 0.11 (1)	10.00	N-D	0 / 247	0.08 (2)
C-D	-1153 / 0	-84.3	-84.3 0.10 (1)	5.83	D-M	0 / 427	0.10 (1)
D-E	-1096 / 0	-84.3	-84.3 0.33 (1)	5.64	M-E	-553 / 0	0.52 (1)
E-F	-1096 / 0	-84.3	-84.3 0.33 (1)	5.64	M-F	0 / 427	0.10 (1)
F-G	-1153 / 0	-84.3	-84.3 0.10 (1)	5.83	K-F	0 / 247	0.08 (2)
G-H	0 / 18	-84.3	-84.3 0.11 (1)	10.00	K-G	0 / 110	0.02 (3)
H-I	0 / 40	-84.3	-84.3 0.12 (1)	10.00	O-C	-1377 / 0	0.59 (1)
O-B	-217 / 0	0.0	0.0 0.02 (1)	7.81	G-J	-1377 / 0	0.59 (1)
J-H	-217 / 0	0.0	0.0 0.02 (1)	7.81			
O-N	0 / 805	-28.0	-28.0 0.34 (2)	10.00			
N-M	0 / 836	-28.0	-28.0 0.35 (2)	10.00			
M-L	0 / 836	-28.0	-28.0 0.35 (2)	10.00			
L-K	0 / 836	-28.0	-28.0 0.35 (2)	10.00			
K-J	0 / 805	-28.0	-28.0 0.34 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(56 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = U/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = U/999 (0.10")

CSI: TC=0.33 (D-E:1), BC=0.35 (K-M:2), WB=0.59 (G-J:1), SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.35 (L) (INPUT = 1.00)



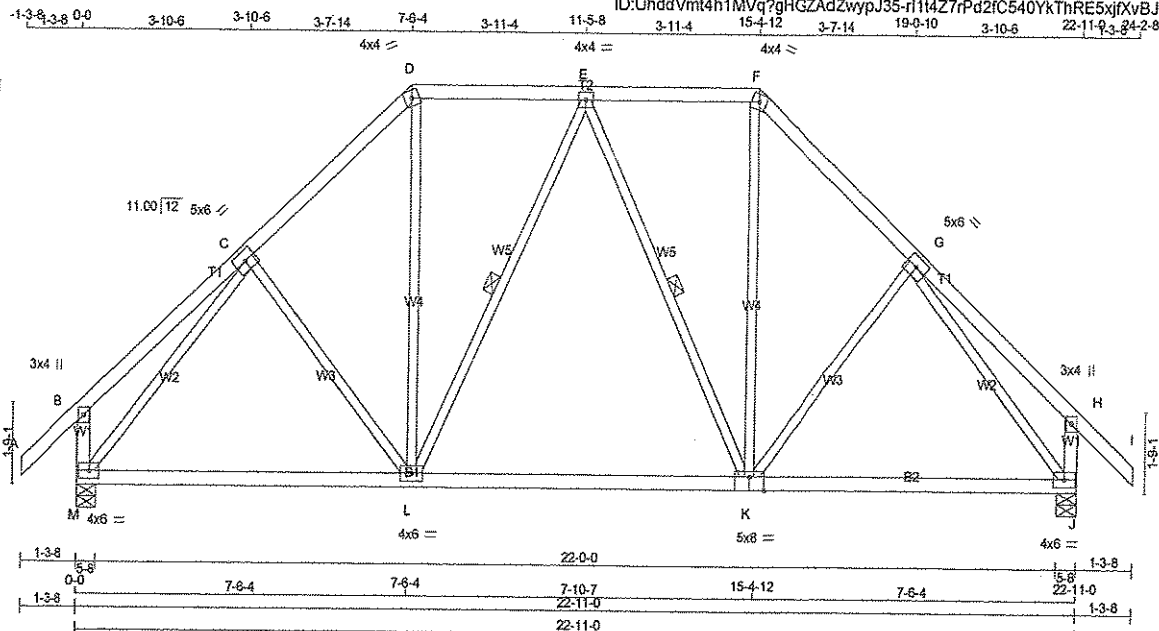
DWG NO. TAM 36756-16
STRUCTURAL
COMPONENT ONLY

JOB NAME 267428	TRUSS NAME T20	QUANTITY 2	PLY 1	JOB DESC. 42181 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:UhddVmt4h1MVq?gHGZAdZwypJ35-r114Z7rPd2fC540YkThRE5xfXvBJn6kRXUVyp_Cu



Scale = 1:48.6

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
M - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW-1-t	MT20	4.0	6.0		
K	BSWW-1-t	MT20	5.0	8.0		Edge 4.00
L	BMVW-1-t	MT20	4.0	6.0		
M	BMVW-1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
M	1404	0	1404	0	0	5-8	1-8		
J	1404	0	1404	0	0	5-8	1-8		

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
M	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0	
J	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		WEBS		FACTORED	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC1 MAX	UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	LC1 (LBS)
FR-TO		FROM TO			LENGTH	FR-TO			
A-B	0 / 40	-84.3	-84.3	0.12 (1)	10.00	C-L	-57 / 80	0.04 (1)	
B-C	0 / 24	-84.3	-84.3	0.18 (1)	10.00	L-D	0 / 478	0.11 (1)	
C-D	-1109 / 0	-84.3	-84.3	0.16 (1)	5.85	L-E	-218 / 0	0.13 (1)	
D-E	-803 / 0	-84.3	-84.3	0.16 (1)	6.25	E-K	-219 / 0	0.13 (1)	
E-F	-802 / 0	-84.3	-84.3	0.16 (1)	6.25	K-F	0 / 478	0.11 (1)	
F-G	-1109 / 0	-84.3	-84.3	0.16 (1)	5.85	K-G	-57 / 80	0.04 (1)	
G-H	0 / 24	-84.3	-84.3	0.18 (1)	10.00	M-C	-1363 / 0	0.87 (1)	
H-I	0 / 40	-84.3	-84.3	0.12 (1)	10.00	G-J	-1363 / 0	0.87 (1)	
M-B	-238 / 0	0.0	0.0	0.03 (1)	7.81				
J-H	-238 / 0	0.0	0.0	0.03 (1)	7.81				
M-L	0 / 833	-28.0	-28.0	0.48 (2)	10.00				
L-K	0 / 894	-28.0	-28.0	0.49 (2)	10.00				
K-J	0 / 833	-28.0	-28.0	0.48 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/999 (0.21")

CSI: TC=0.18 (B-C:1), BC=0.49 (K-L:2), WB=0.87 (G-J:1), SS=0.17 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1687 822 2284 1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.80)
JSI METAL= 0.39 (K) (INPUT = 1.00)



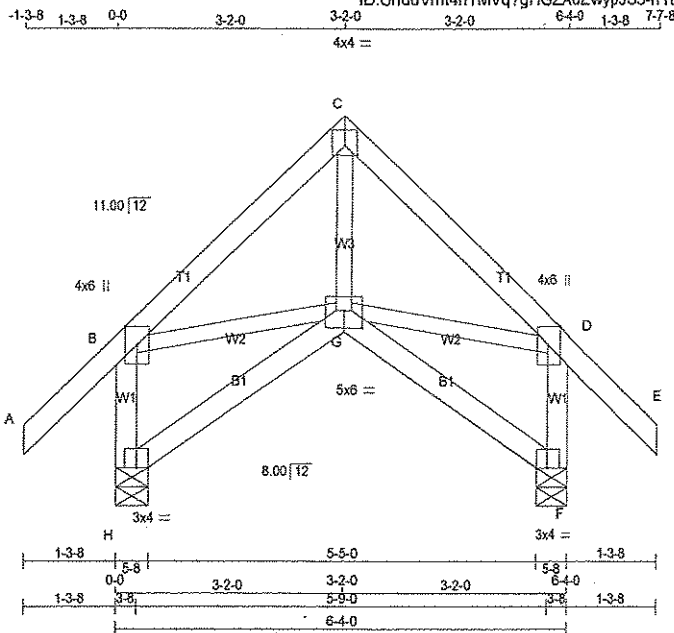
DWG NO. TAM 36757-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267428	T21	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:30.0

TOTAL WEIGHT = 2 X 33 = 65 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	6.0	Edge	
F	BVM1-p	MT20	3.0	4.0		
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BVM1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	473	0	5-8	1-8
F	473	0	5-8	1-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	371	233 / 0	66 / 0
F	371	233 / 0	66 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CS (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)	CS (LC)
FR-TO		FROM	TO					
A-B	0 / 40	-84.3	-84.3	0.12 (1)	10.00	G-C	0 / 200	0.04 (2)
B-C	-308 / 0	-84.3	-84.3	0.15 (1)	6.25	B-G	0 / 229	0.05 (1)
C-D	-308 / 0	-84.3	-84.3	0.15 (1)	6.25	G-D	0 / 229	0.05 (1)
D-E	0 / 40	-84.3	-84.3	0.12 (1)	10.00			
H-B	-428 / 0	0.0	0.0	0.05 (1)	7.81			
F-D	-428 / 0	0.0	0.0	0.05 (1)	7.81			
H-G	0 / 0	-28.0	-28.0	0.09 (3)	10.00			
G-F	0 / 0	-28.0	-28.0	0.09 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.21")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.21")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.15 (B-C:1), BC=0.09 (G-H:3), WB=0.05 (B-G:1), SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.19 (H) (INPUT = 1.00)



DWG NO. TAM36758-16
STRUCTURAL
COMPONENT ONLY

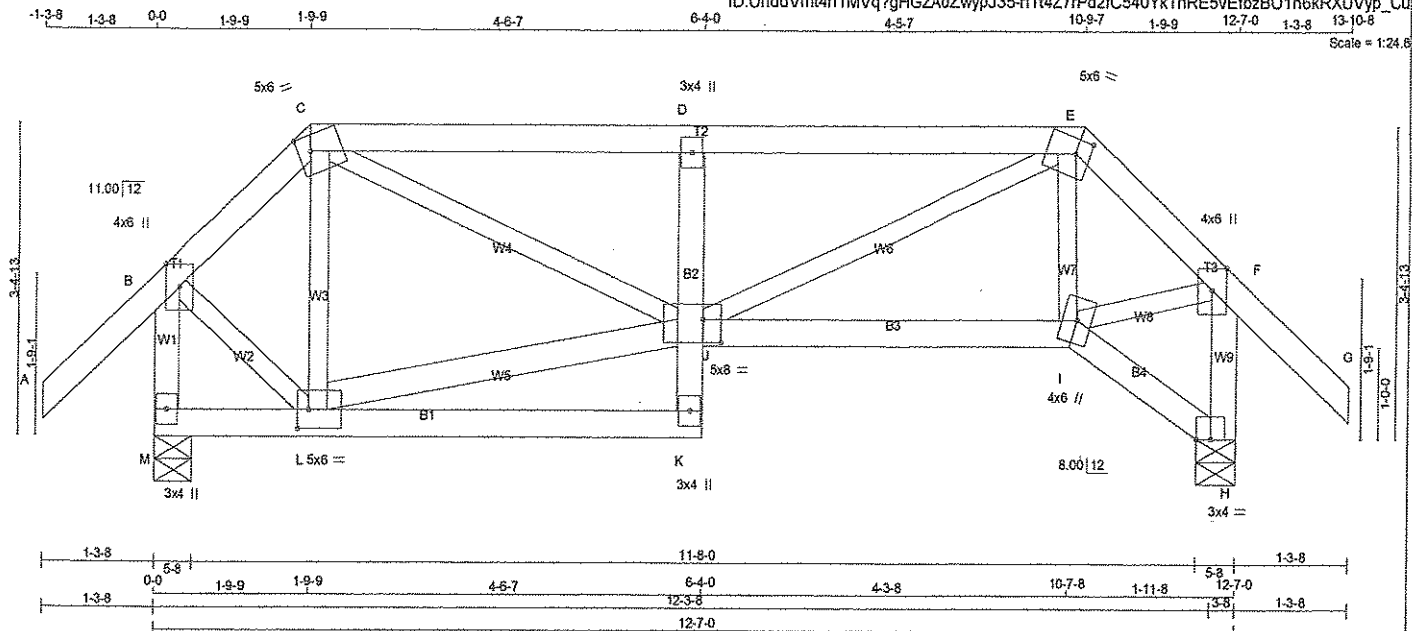
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267428	T22	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Thu Aug 11 09:04:05 2016 Page 1

ID:UhdVmt4h1MVq?gHGZAdZwpJ35-r1t4Z7rPd2fC540YkThRE5vEfbzBU1n6kRXUVyp_Cu

Scale = 1:24.6



TOTAL WEIGHT = 60 lb

LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - D	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
L - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTVW+m	MT20	5.0	6.0	2.00	1.75
D	TMV+p	MT20	3.0	4.0		
E	TTVW+m	MT20	5.0	6.0	2.00	2.00
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BBVW+m	MT20	4.0	6.0		
J	BMVWWH	MT20	5.0	8.0	3.00	2.50
K	BMV+p	MT20	3.0	4.0		
L	BMVWWH	MT20	5.0	8.0	2.50	1.50
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
M	824	0	824	0	0	5-8	1-8		
H	824	0	824	0	0	5-8	1-8		

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	659	393 / 0	132 / 0	0 / 0	0 / 0	134 / 0	0 / 0
H	659	393 / 0	132 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.61 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. (LC1)	MEMB.	FORCE (LBS)	MAX. (LC)	UNBRACED LENGTH (FR-TO)
A-B	0 / 40	-84.3	-84.3 0.12 (1)	10.00	L-C	-209 / 19	0.04 (1)
B-C	-502 / 0	-84.3	-84.3 0.11 (1)	6.25	L-J	0 / 306	0.05 (1)
C-D	-1081 / 0	-84.3	-84.3 0.28 (1)	5.71	C-J	0 / 817	0.18 (1)
D-E	-1103 / 0	-84.3	-84.3 0.28 (1)	5.67	B-L	0 / 436	0.10 (1)
E-F	-688 / 0	-84.3	-84.3 0.12 (1)	6.25	I-F	0 / 504	0.11 (1)
F-G	0 / 40	-84.3	-84.3 0.12 (1)	10.00	E-I	-16 / 100	0.02 (3)
M-B	-826 / 0	0.0	0.0 0.09 (1)	7.81	J-E	0 / 673	0.15 (1)
H-F	-797 / 0	0.0	0.0 0.09 (1)	7.81			
M-L	0 / 0	-28.0	-28.0 0.12 (2)	10.00			
L-K	0 / 45	-28.0	-28.0 0.12 (2)	10.00			
K-J	0 / 87	0.0	0.0 0.07 (1)	10.00			
J-D	-453 / 0	0.0	0.0 0.07 (1)	7.81			
J-I	0 / 492	-28.0	-28.0 0.23 (2)	10.00			
I-H	0 / 0	-28.0	-28.0 0.03 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL (LL) = $L/999$ (0.04")
ALLOWABLE DEFL (TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL (TL) = $L/999$ (0.07")

CSI: TC=0.28 (D-E:1), BC=0.23 (J-I:2), WB=0.18 (C-J:1), SSI=0.18 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.80)
JSI METAL= 0.35 (H) (INPUT = 1.00)

DWG NO. TAM 36759-16
STRUCTURAL
COMPONENT ONLY



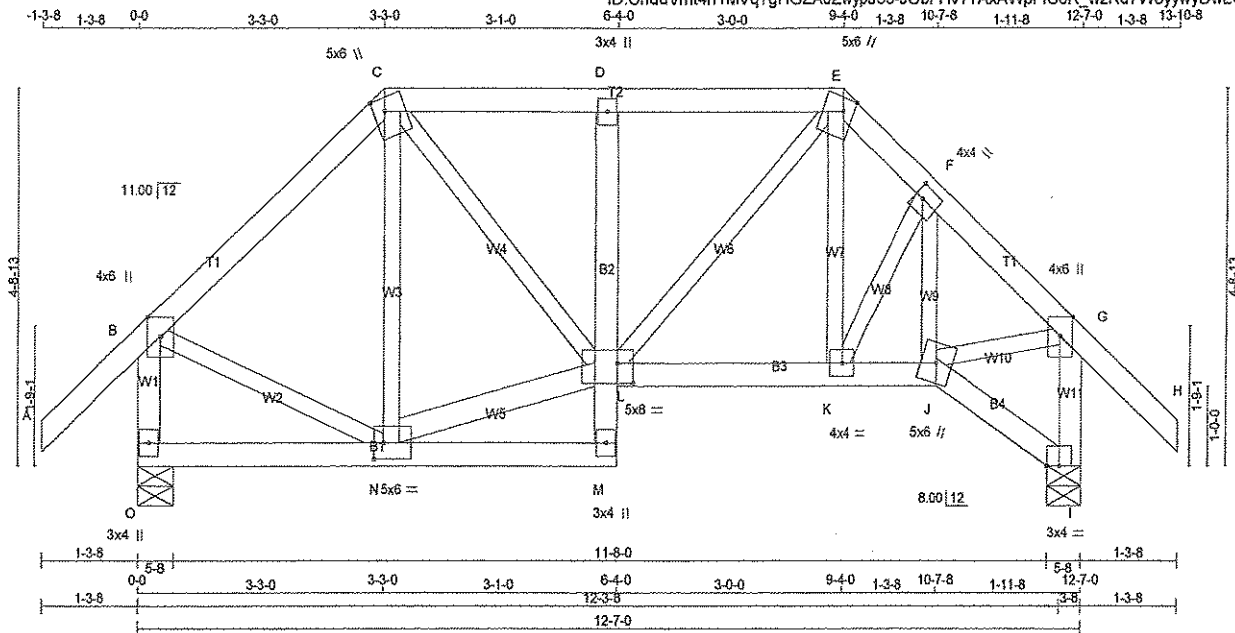
JOB NAME 267428	TRUSS NAME T23	QUANTITY 1	PLY 1	JOB DESC. 42181	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MTEK Industries, Inc. Thu Aug 11 09:04:06 2016 Page 1

ID:UhdVmt4h1MVq?gHGZAdZwypJ35-JUbFHv7TAxAWpFfC6R_wzRd7W3yywyDwLOB41xyp_Ct

Scale = 1:28.4



TOTAL WEIGHT = 69 lb

(M/F)

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
M - D	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
J - I	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
N - L	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTVW+m	MT20	5.0	6.0	2.00	1.75
D	TMV+p	MT20	3.0	4.0		
E	TTVW+m	MT20	5.0	6.0	2.00	1.75
F	TMVW+t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	4.0	6.0	Edge	
I	BVM1+p	MT20	3.0	4.0		
J	BBWV+m	MT20	5.0	6.0		
K	BMVW+t	MT20	4.0	4.0		
L	BVMVWV-I	MT20	5.0	6.0	3.00	2.50
M	BMV+p	MT20	3.0	4.0		
N	BMVWV+t	MT20	5.0	6.0	2.50	1.50
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
O	824	0	0	824	0	0	0	5-8	1-8
I	824	0	0	824	0	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	659	393/0	132/0	0/0	0/0	134/0	0/0
I	659	393/0	132/0	0/0	0/0	134/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED (PLF)	VERT. LOAD LC1 MAX (CSI) (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED (PLF)	VERT. LOAD LC1 MAX (CSI) (LC)
FR-TO				FR-TO			
A-B	0/40	-84.3	-84.3 0.12 (1)	10.00	N-C	-191/30	0.08 (1)
B-C	-534/0	-84.3	-84.3 0.12 (1)	6.25	N-L	0/398	0.06 (1)
C-D	-665/0	-84.3	-84.3 0.09 (1)	6.25	C-L	0/417	0.09 (1)
D-E	-670/0	-84.3	-84.3 0.09 (1)	6.25	L-E	0/242	0.05 (1)
E-F	-669/0	-84.3	-84.3 0.03 (1)	6.25	K-E	0/175	0.04 (2)
F-G	-723/0	-84.3	-84.3 0.05 (1)	6.25	K-F	-80/0	0.01 (1)
G-H	0/40	-84.3	-84.3 0.12 (1)	10.00	J-F	-71/3	0.01 (1)
O-B	-788/0	0.0	0.0 0.09 (1)	7.81	B-N	0/428	0.10 (1)
I-G	-766/0	0.0	0.0 0.09 (1)	7.81	J-G	0/548	0.12 (1)
O-N	0/0	-28.0	-28.0 0.08 (3)	10.00			
N-M	0/16	-28.0	-28.0 0.08 (2)	10.00			
M-L	0/53	0.0	0.0 0.03 (1)	10.00			
L-D	-301/0	0.0	0.0 0.02 (1)	7.81			
L-K	0/508	-28.0	-28.0 0.11 (2)	10.00			
K-J	0/540	-28.0	-28.0 0.12 (2)	10.00			
J-I	0/0	-28.0	-28.0 0.03 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.12 (B-C:1), BC=0.12 (J-K:2), WB=0.12 (G-J:1), SSI=0.12 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

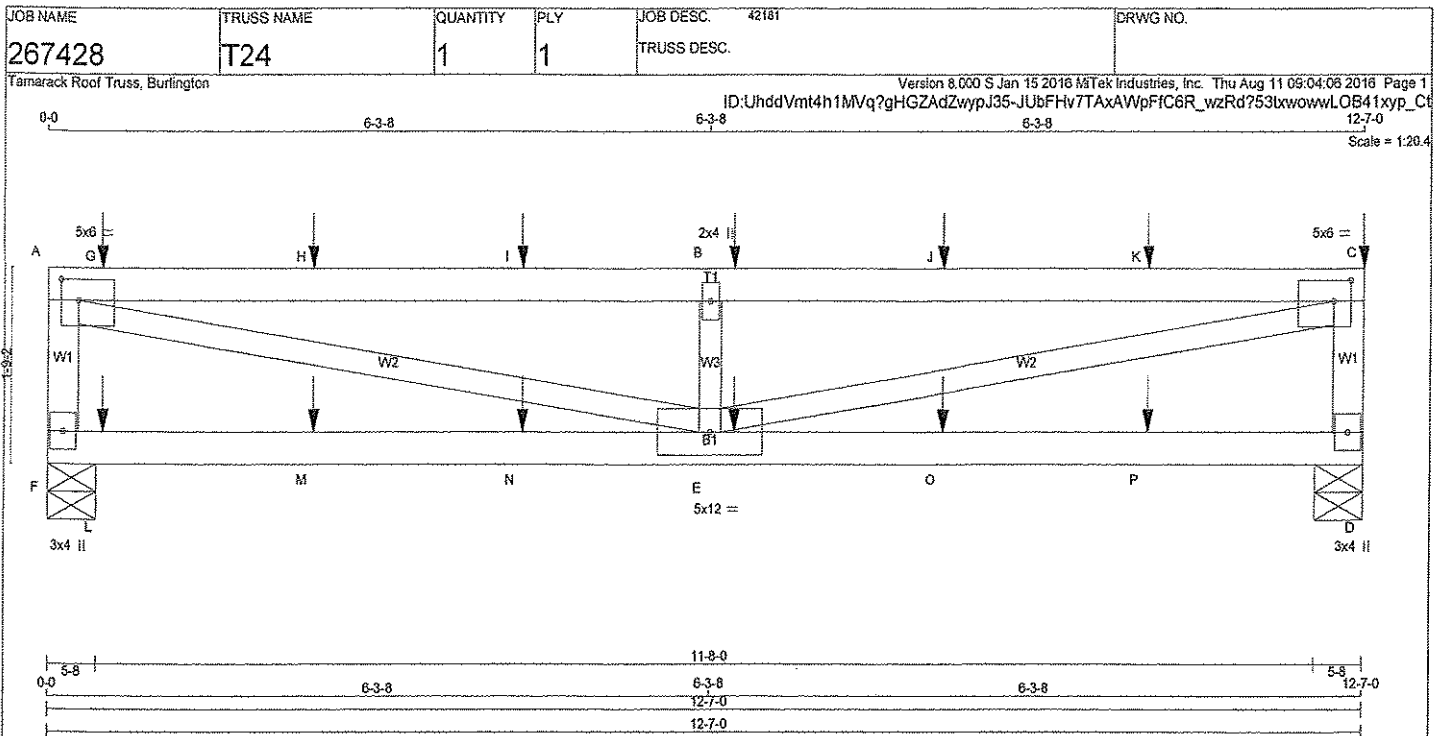
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.83 (N) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)



DWG NO. TAM36760-16
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY No.2	SPF
A - C	2x4	DRY 2100F 1.8E	SPF
D - C	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	2.00
B	TMVW-w	MT20	2.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	5.0	12.0		
F	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 98.0 lbs FACTORED DOWN AT 6-4, 75.8 lbs FACTORED DOWN AT 2-6-4, 75.8 lbs FACTORED DOWN AT 4-6-4, 75.8 lbs FACTORED DOWN AT 6-6-4, 75.8 lbs FACTORED DOWN AT 8-6-4, AND 75.8 lbs FACTORED DOWN AT 10-6-4, AND 115.3 lbs FACTORED DOWN AT 12-7-0 ON TOP CHORD, AND 63.1 lbs FACTORED DOWN AT 6-4, 52.8 lbs FACTORED DOWN AT 2-6-4, 52.8 lbs FACTORED DOWN AT 4-6-4, 52.8 lbs FACTORED DOWN AT 6-6-4, AND 52.8 lbs FACTORED DOWN AT 8-6-4, AND 52.8 lbs FACTORED DOWN AT 10-6-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP
F	1168	0	1168	0
D	1159	0	1159	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
F	COMBINED	SNOW	LIVE
F	959	532 / 0	218 / 0
D	941	540 / 0	202 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (MAX. CSI (LBS))	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LBS)
FR-TO		FROM TO			FR-TO		
F-A	-1005 / 0	0.0	0.0	0.12 (1)	7.77	E-C	0 / 2902 0.72 (1)
A-G	-2828 / 0	-84.3	-84.3	0.59 (1)	4.28	A-E	0 / 2902 0.72 (1)
G-H	-2828 / 0	-84.3	-84.3	0.59 (1)	4.28	E-B	-921 / 0 0.15 (1)
H-I	-2828 / 0	-84.3	-84.3	0.59 (1)	4.28		
I-B	-2828 / 0	-84.3	-84.3	0.59 (1)	4.28		
B-J	-2828 / 0	-84.3	-84.3	0.59 (1)	4.28		
J-K	-2828 / 0	-84.3	-84.3	0.59 (1)	4.28		
K-C	-2828 / 0	-84.3	-84.3	0.59 (1)	4.28		
D-C	-1043 / 0	0.0	0.0	0.12 (1)	7.66		
F-L	0 / 0	-28.0	-28.0	0.44 (3)	10.00		
L-M	0 / 0	-28.0	-28.0	0.44 (3)	10.00		
M-N	0 / 0	-28.0	-28.0	0.44 (3)	10.00		
N-E	0 / 0	-28.0	-28.0	0.44 (3)	10.00		
E-O	0 / 0	-28.0	-28.0	0.44 (3)	10.00		
O-P	0 / 0	-28.0	-28.0	0.44 (3)	10.00		
P-D	0 / 0	-28.0	-28.0	0.44 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	6-6-4	-76	-76		BACK	VERT	TOTAL
C	12-7-0	-115	-115		BACK	VERT	TOTAL
E	6-6-4	-53	-53		BACK	VERT	TOTAL
G	6-4	-96	-96		BACK	VERT	TOTAL
H	2-6-4	-76	-76		BACK	VERT	TOTAL
I	4-6-4	-76	-76		BACK	VERT	TOTAL
J	8-6-4	-76	-76		BACK	VERT	TOTAL
K	10-6-4	-76	-76		BACK	VERT	TOTAL
L	6-4	-60	-60		BACK	VERT	TOTAL
M	2-6-4	-53	-53		BACK	VERT	TOTAL
N	4-6-4	-53	-53		BACK	VERT	TOTAL
O	8-6-4	-53	-53		BACK	VERT	TOTAL
P	10-6-4	-53	-53		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(LL) = $L/882$ (0.17")
ALLOWABLE DEFL.(TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(TL) = $L/527$ (0.29")

CSI: TC=0.59 (A-B:1), BC=0.44 (E-F:3), WB=0.72 (C-E:1), SSI=0.37 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

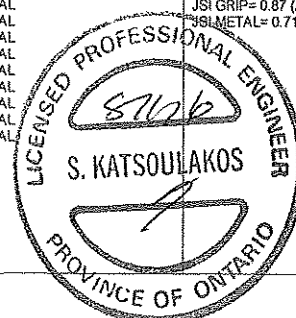
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.71 (C) (INPUT = 1.00)



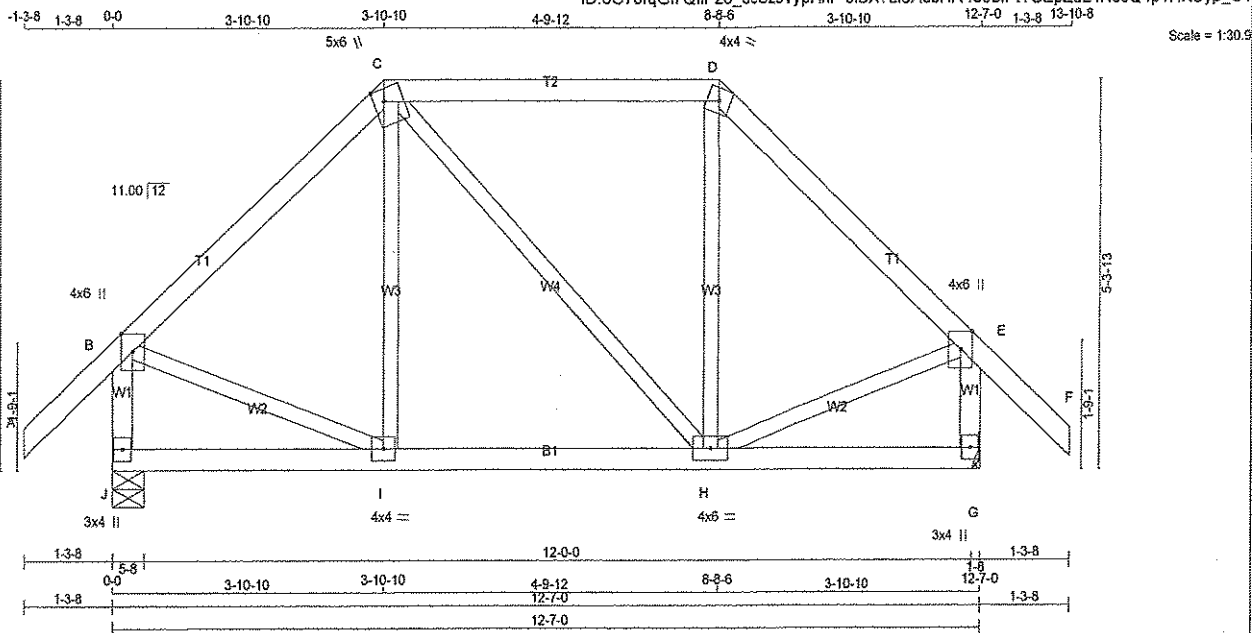
DRWG NO. TAM3676/ -16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267429	T31	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 59 = 235 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.75
D	TTWW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVWV-t	MT20	4.0	6.0		
I	BMVWV-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
J	824	0	824	0	0	5-8	1-8		
G	824	0	824	0	0				

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	659	393/0	132/0	0/0	0/0	134/0	0/0		
G	659	393/0	132/0	0/0	0/0	134/0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. (PLF)	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO			LENGTH	FR-TO		
A-B	0/40	-84.3	-84.3	0.12 (1)	10.00	I-C	-6/159	0.04 (3)	
B-C	-532/0	-84.3	-84.3	0.17 (1)	6.25	C-H	0/0	0.03 (1)	
C-D	-380/0	-84.3	-84.3	0.25 (1)	6.25	H-D	-6/159	0.04 (3)	
D-E	-532/0	-84.3	-84.3	0.17 (1)	6.25	B-I	0/414	0.09 (1)	
E-F	0/40	-84.3	-84.3	0.12 (1)	10.00	H-E	0/414	0.09 (1)	
J-B	-780/0	0.0	0.0	0.08 (1)	7.81				
G-E	-780/0	0.0	0.0	0.08 (1)	7.81				
J-I	0/0	-28.0	-28.0	0.12 (3)	10.00				
I-H	0/380	-28.0	-28.0	0.17 (2)	10.00				
H-G	0/0	-28.0	-28.0	0.12 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CS1: TC=0.25 (C-D:1), BC=0.17 (H-I:2), WB=0.09 (E-F:1), SS=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 622	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (H) (INPUT = 0.90)

JSI METAL= 0.15 (B) (INPUT = 1.00)



DWG NO. TAM36762-16
STRUCTURAL
COMPONENT ONLY

JOB NAME 267429	TRUSS NAME G31	QUANTITY 1	PLY 1	JOB DESC. 42181 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

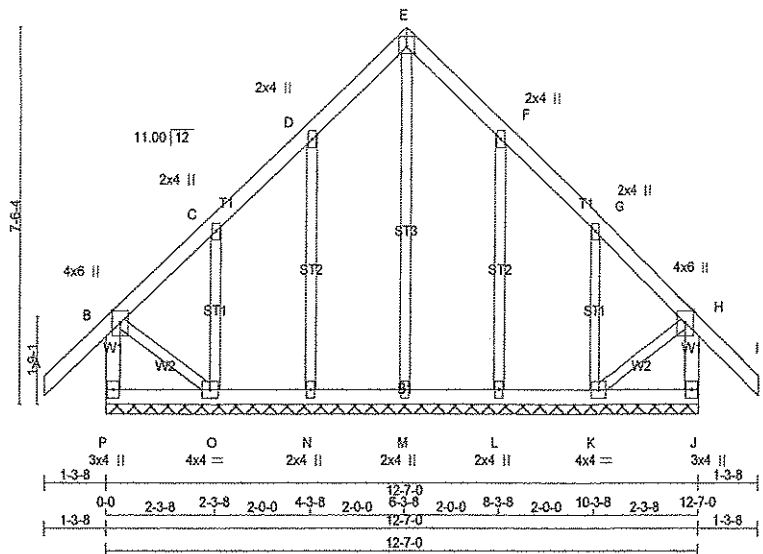
Version 8.000 S Jan 15 2016 MTEK Industries, Inc. Thu Aug 11 09:04:57 2016 Page 1

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-1-3-8 0-0 2-3-8 2-3-8 2-0-0 4-3-8 2-0-0 6-3-8 2-0-0 8-3-8 2-0-0 10-3-8 2-0-0 12-3-8 13-10-8

4x4 =

Scale = 1:45.2



TOTAL WEIGHT = 64 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMVW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		
L, M, N						
L	BMV1+w	MT20	2.0	4.0		
O	BMVW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	FR-TO
FR-TO				FR-TO			
A-B	0/40	-84.3	-84.3 0.12 (1)	M-E	-113/0	0.11 (1)	
B-C	-23/0	-84.3	-84.3 0.05 (1)	N-D	-183/0	0.09 (1)	
C-D	-29/0	-84.3	-84.3 0.05 (1)	O-C	-191/0	0.04 (1)	
D-E	-34/0	-84.3	-84.3 0.05 (1)	L-F	-183/0	0.09 (1)	
E-F	-34/0	-84.3	-84.3 0.05 (1)	K-G	-191/0	0.04 (1)	
F-G	-29/0	-84.3	-84.3 0.05 (1)	B-O	0/31	0.01 (1)	
G-H	-23/0	-84.3	-84.3 0.05 (1)	K-H	0/31	0.01 (1)	
H-I	0/40	-84.3	-84.3 0.12 (1)				
P-B	-232/0	0.0	0.0 0.03 (1)				
J-H	-232/0	0.0	0.0 0.03 (1)				
P-O	0/0	-28.0	-28.0 0.03 (2)				
O-N	0/20	-28.0	-28.0 0.04 (2)				
N-M	0/17	-28.0	-28.0 0.03 (2)				
M-L	0/17	-28.0	-28.0 0.03 (2)				
L-K	0/20	-28.0	-28.0 0.04 (2)				
K-J	0/0	-28.0	-28.0 0.03 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
CL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF
DL =	7.0	PSF	
TOTAL LOAD =	48.1	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12 (H-I), BC=0.04 (K-L), WB=0.11 (E-M), SSI=0.07 (H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)

JSI METAL= 0.05 (G) (INPUT = 1.00)

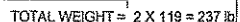


DWG NO. TAM36763-10
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM 36750-16
STRUCTURAL
COMPONENT ONLY

Version 8.000 S Jan 15 2016 MiTek Industries, Inc. Thu Aug 11 09:02:05 2016 Page 1
ID: OFOKkuTgKX10a1LfsIRDsuyPK5U-LCnVuXgZqwhofvWXJYaw7NqHcu4AT6W3NAHSyp Em

Scale = 1:41.1



A circular professional seal for a Licensed Professional Engineer in the Province of Ontario. The seal features the text "LICENSED PROFESSIONAL ENGINEER" at the top and "PROVINCE OF ONTARIO" at the bottom. In the center, the name "S. KATSOULAKOS" is printed. Above the name is a stylized signature "SK/16" enclosed in a semi-circular frame, and below the name is a stylized quill pen also enclosed in a semi-circular frame.

DWG NO. TAM36751-16
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 42181	DRWG NO.
267427	T50	1	2	TRUSS DESC.	

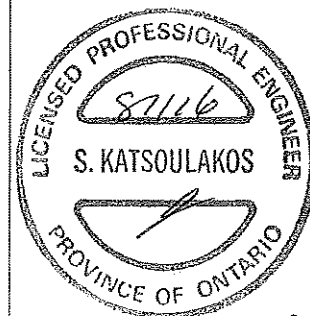
Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MTEK Industries, Inc. Thu Aug 11 09:02:05 2016 Page 2

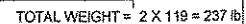
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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1865.5 lbs FACTORED DOWN AT 10-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM36751-16
STRUCTURAL
COMPONENT ONLY



Professional Engineer Seal for S. Katsoulakos, License No. 87116, Province of Ontario.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267429	T50Z	1	2	TRUSS DESC.		

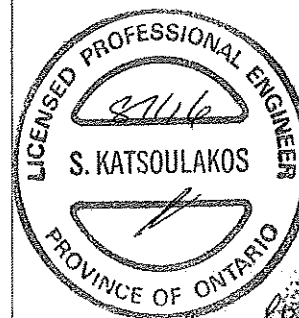
Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2018 Mitek Industries, Inc. Thu Aug 11 09:04:57 2016 Page 2

ID:OFQKkuTqKX1Oa1LfsRDsuyPK5U-JIBX?LISAc6HR430bPtyCEk2dJCNzkQ4p1HKUyp_C4

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 166.8 lbs FACTORED DOWN AT 3-10-10, 163.3 lbs FACTORED DOWN AT 5-11-4, AND 163.3 lbs FACTORED DOWN AT 7-11-4, AND 163.3 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-11-4, 87.4 lbs FACTORED DOWN AT 3-11-4, 87.4 lbs FACTORED DOWN AT 5-11-4, 87.4 lbs FACTORED DOWN AT 7-11-4, AND 87.4 lbs FACTORED DOWN AT 9-11-4, AND 1697.4 lbs FACTORED DOWN AT 10-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

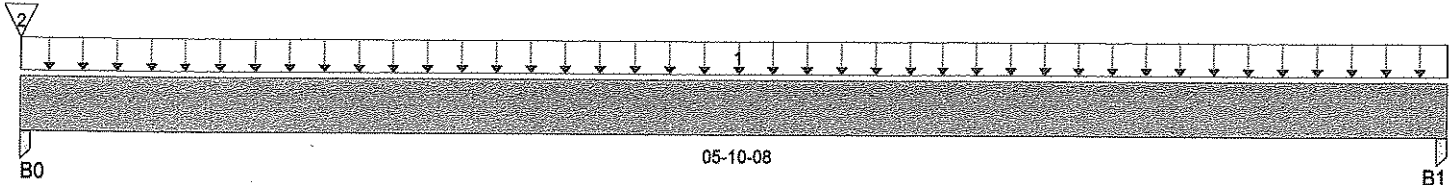


DWG NO. TAM 36764-16
STRUCTURAL
COMPONENT ONLY

BC CALC® Design Report


Build 4516
 Job Name: 42181
 Address: GREENPARK HOMES/STARTTIME
 City, Province, Postal Code: VAUGHAN, ON
 Customer:
 Code reports: NLGA

File Name: 42181-267427
 Description: Designs\FB01
 Specifier: BRIDGEFORD 3
 Designer: JG
 Company:
 Misc:



Total Horizontal Product Length = 05-10-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	235 / 0	241 / 0	611 / 0	
B1, 3-1/2"	135 / 0	141 / 0	318 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	Standard Load	Unf. Area (lb/ft^2)	L	00-00-00	05-10-08	11	10	26		04-02-00
2		Conc. Pt. (lbs)	L	00-00-00	00-00-00	100	100	293		n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	900 ft-lbs	6,040 ft-lbs	14.9%	5	02-11-04
End Shear	460 lbs	3,985 lbs	11.6%	5	01-00-12
Total Load Defl.	L/999 (0.012")	n/a	n/a	13	02-11-04
Live Load Defl.	L/999 (0.009")	n/a	n/a	17	02-11-04
Max Defl.	0.012"	n/a	n/a	13	02-11-04
Span / Depth	7	n/a	n/a		00-00-00

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Post	3-1/2" x 3"	1,336 lbs	n/a	20.7%	Unspecified
B1 Post	3-1/2" x 3"	721 lbs	n/a	11.2%	Unspecified

Notes

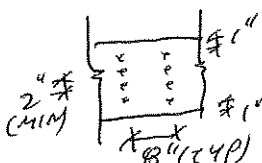
Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 The analysis of solid sawn wood members is in accordance with CSA 086 and is limited to the output shown above. All other support and design for these products, including but not limited to notching, connections, installation, and engineer/architect certification is the responsibility of the project's design professional of record.
 Importance Factor: Normal Part code: Part 4
 Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBC 2012



DWG NO. TAM36765-16
 STRUCTURAL
 COMPONENT ONLY

PROVIDE 4 ROWS OF 3 1/2" ARDOX SPIRAL NAILS @ 8" O/C FOR MULTI-PLY NAILING. MAINTAIN A MIN. 1" LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS



HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

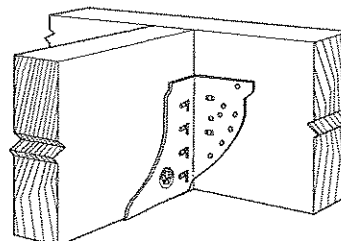
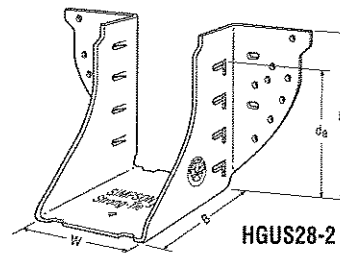
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

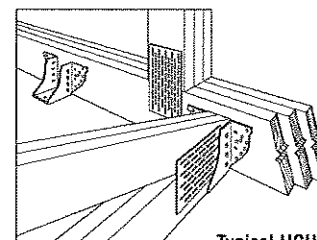
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



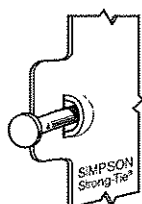
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

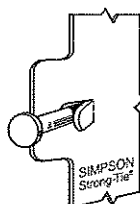
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 5/8	5 3/8	5	4 5/8	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3 5/8	5 3/8	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5 1/2	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1 3/8	7 1/8	5	6 1/8	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7 1/4	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3 1/8	9 1/8	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9 1/4	4	8 3/8	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 3/8	4	10 1/8	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 3/8	4	11 1/8	66-16d	22-16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

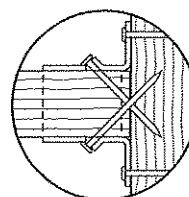


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

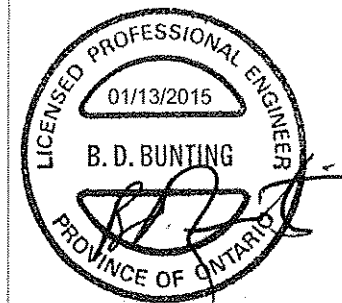
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

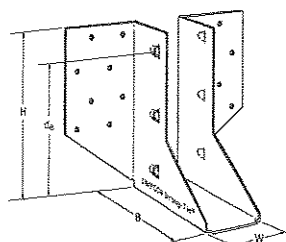
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

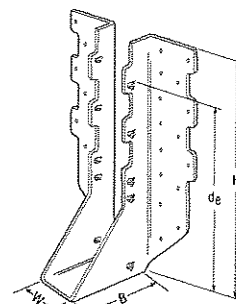
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

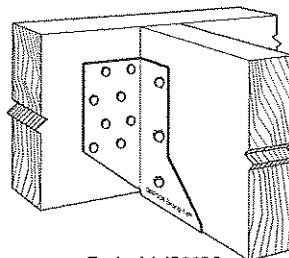
- See current catalogue for options



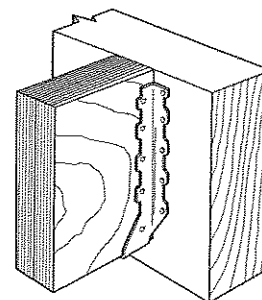
LJS26DS



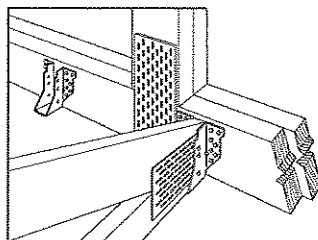
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



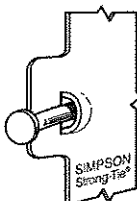
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

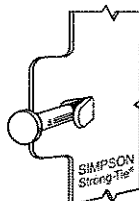
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

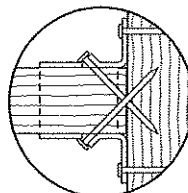


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

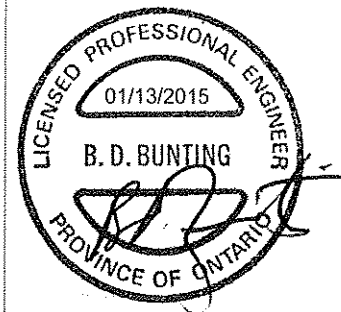
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

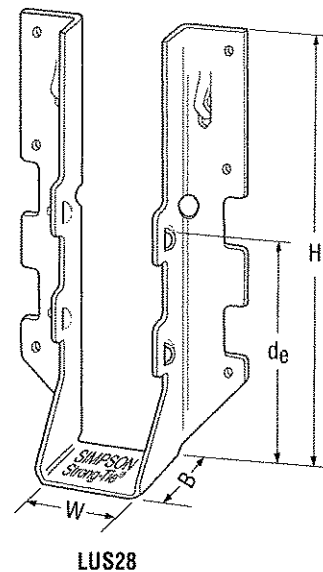
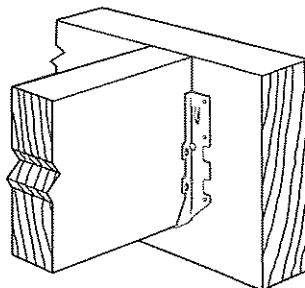
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

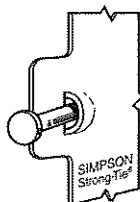
- These hangers cannot be modified.

Typical LUS Installation



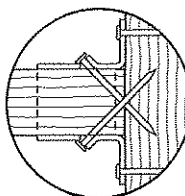
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1 1/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/16	4 3/8	1 3/4	3 3/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 3/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 1/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 13/16	1 3/4	3 3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

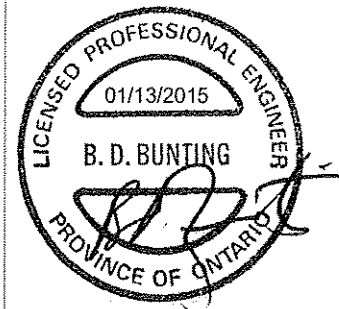


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

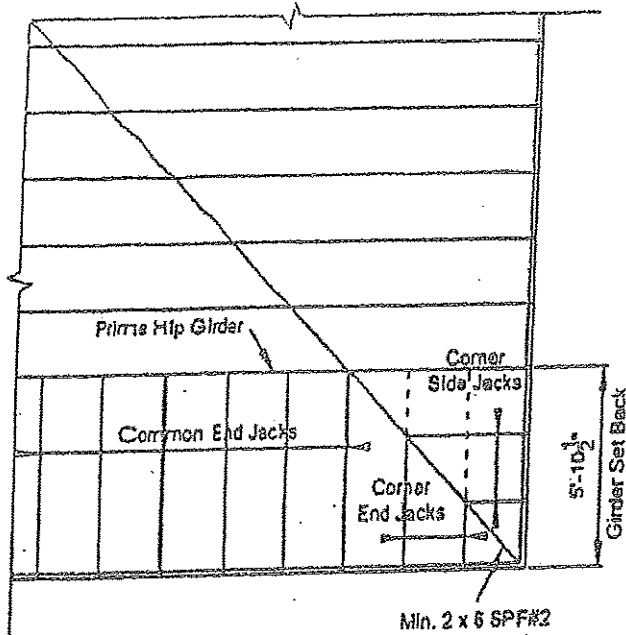
© 2015 Simpson Strong-Tie Company Inc.

T-SPECLUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

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45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

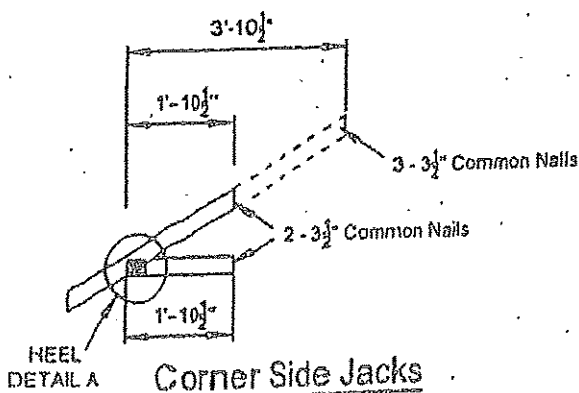
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

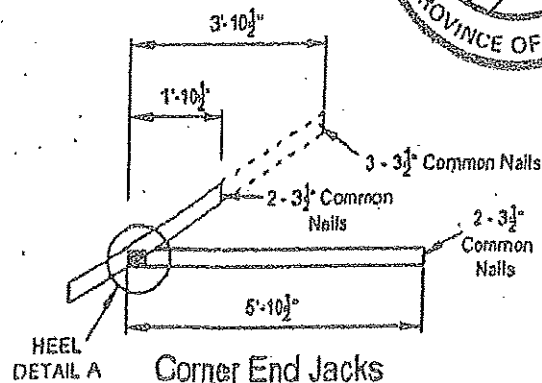
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

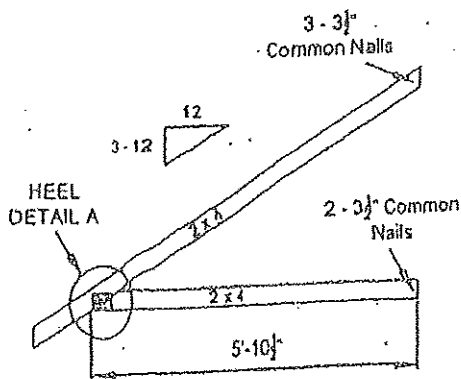
DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



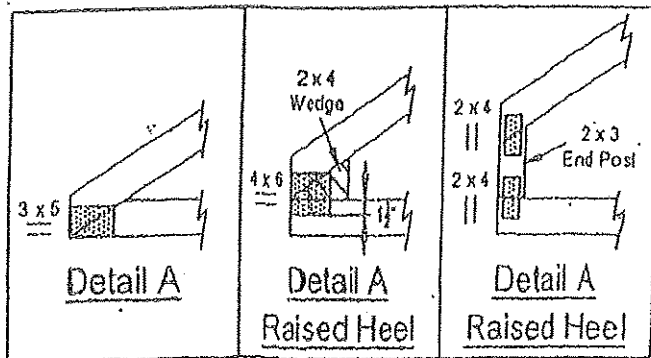
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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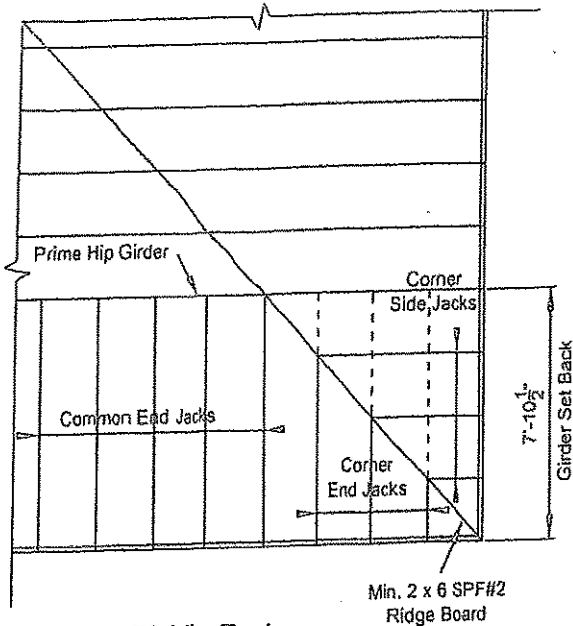
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

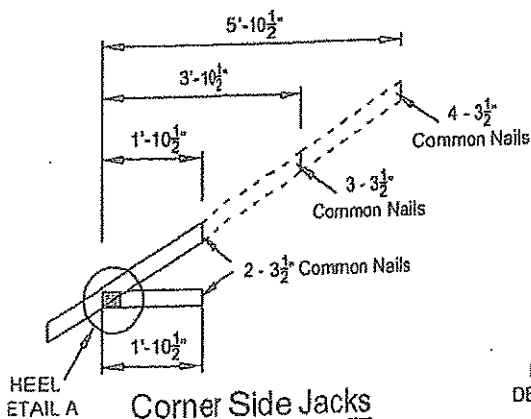
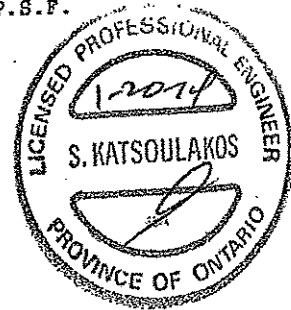
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

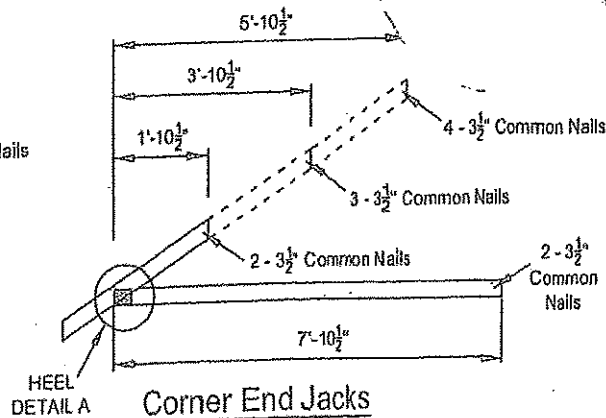


45° Hip End

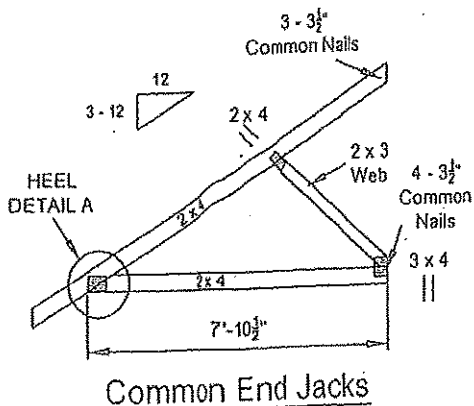
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STRUCTURAL
COMPONENT ONLY



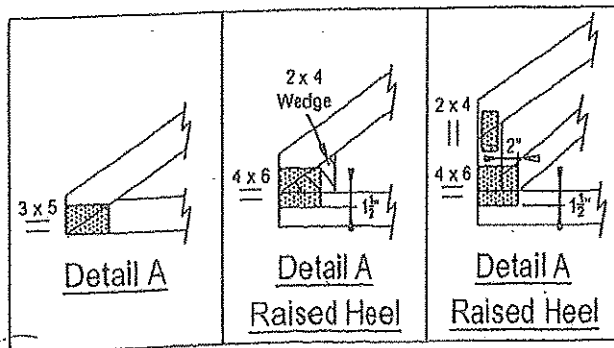
Corner Side Jacks



Corner End Jacks



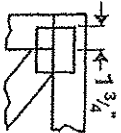
Common End Jacks



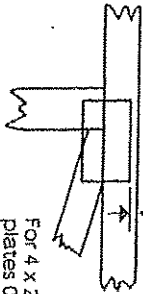
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

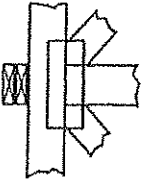
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

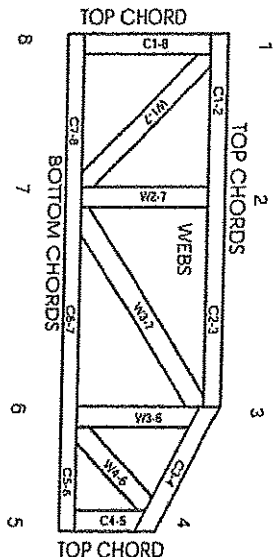


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

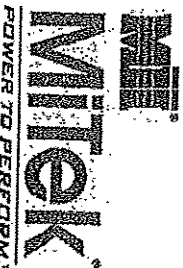
PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet: MIT-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

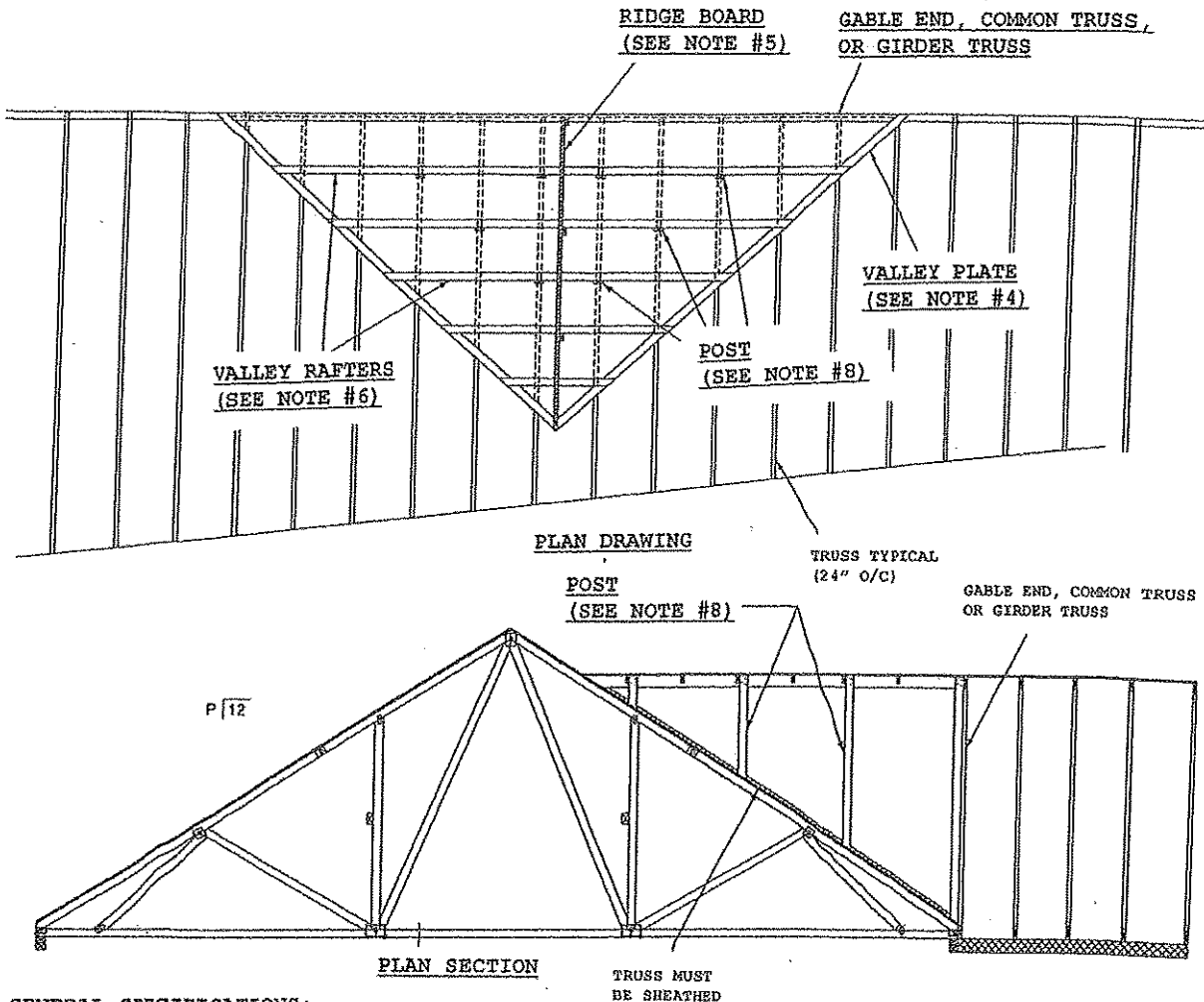
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack material on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or pultrus provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

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CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

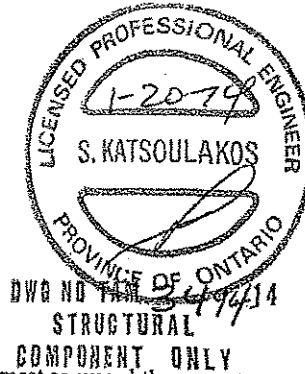


DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.